

RT. FA-8  
(SBI-7)

SEC. G-VB



**STATE OF ILLINOIS**  
**DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**PLANS FOR PROPOSED**  
**STATE BOND ISSUE HIGHWAY**

(SBI 7) G-VB GRUNDY 35  
 F-10(12)

P-93-008-64

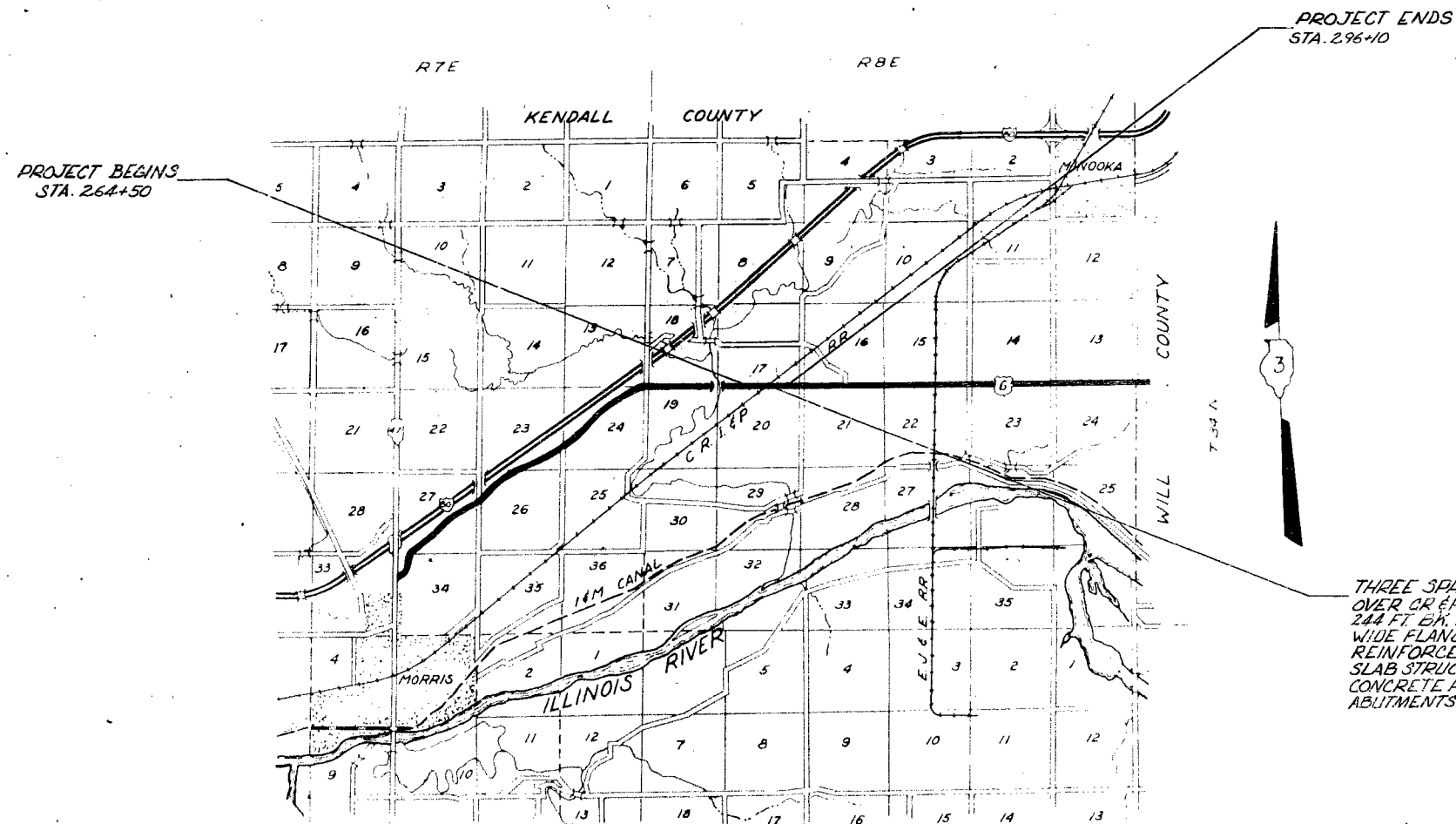
INDEX OF SHEETS

Sheet No.	Description
1	Title Sheet and Index of Sheets
2-3	Typical Sections - Details - Culvert & S.S. Schedule
3A	Shoulder Details-Alternate B
4	Summary of Quantities
5	Schedule of Quantities
6	Drainage Plan for Grade Separation
7-8	Plan and Profile S. B. I. Route 7
9-23	Bridge Plans
24	Box Culvert and Sign Information
	Cross Sections
25-35	S. B. I. Route 7 Sta 264+50 to Sta 296+10
	STANDARDS
1686-3	Symbols and Abbreviations
1744-2	R. O. W. Markers
1909-10	Detail of Bridge Approaches
1976	Reinforced Conc. Hdws
2113-1	Name Plate Detail
2135	Permanent Survey Markers
2143-3	Width Transition
2158-6	Sign
2205	Plant Hardiness Map
2230-7	SPBGR
2231-3	SPBGR Applications
2237-6	Shoulder Details
2228-1	Metal End sections for Pipe Culverts
2258-1	Paved Ditch
2298-3	Traffic Control Devices
2299-3	Traffic Control Devices
2323	Pavement Joints
2327	Sub Surface Drains

SCALES  
 PLAN 1 INCH = 100 FT.  
 PROFILE HOR. 1 INCH = 100 FT.  
 PROFILE VERT. 1 INCH = 10 FT.  
 CROSS-SECTIONS 1 INCH = 5 FT.

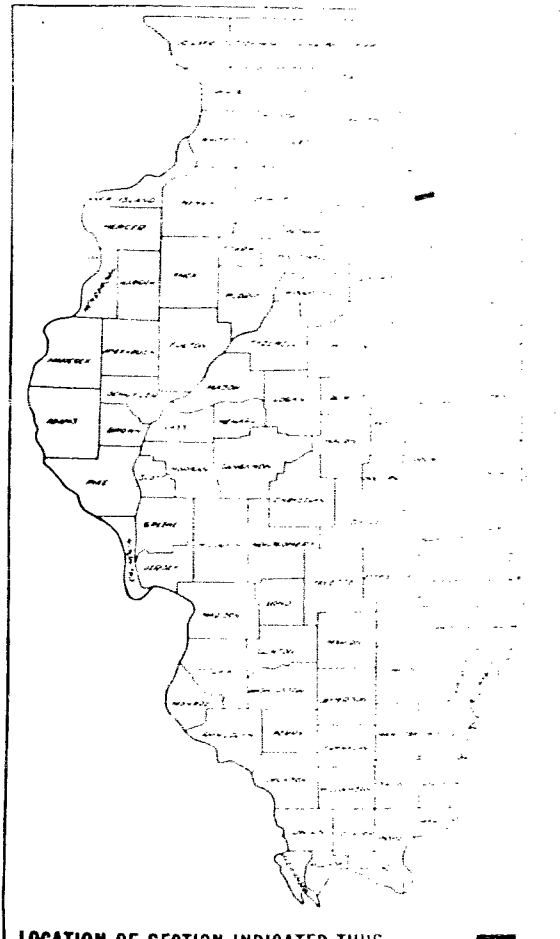
F. A. ROUTE 8 (SBI RT. 7) SECTION G-VB  
 PROJECT NO. FG-10-(12)  
 GRUNDY COUNTY

C - 93 - 051 - 65



THREE SPAN BRIDGE  
 OVER CR & P. R.R. STA. 279+22.68  
 244 FT. BK. TO BK. OF ABUTMENTS  
 WIDE FLANGE STEEL BEAM WITH  
 REINFORCED CONCRETE DECK  
 SLAB STRUCTURE ON REINFORCED  
 CONCRETE PIERS AND PILE BENT  
 ABUTMENTS.

LAYOUT: 1 INCH = 1 MILE  
 GROSS LENGTH OF PROJECT = 3160 FEET = 0.598 MILES  
 NET LENGTH OF PROJECT = 3160 FEET = 0.598 MILES



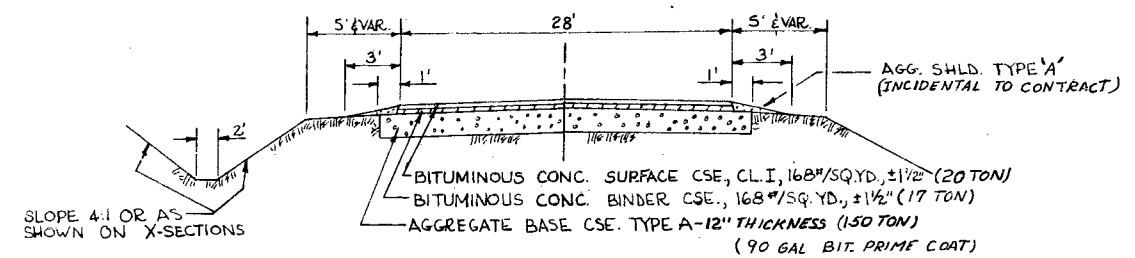
LOCATION OF SECTION INDICATED THUS: —

DESIGN DESIGNATION: COLLECTOR  
 400(91) C 3.72 (BIT 20)

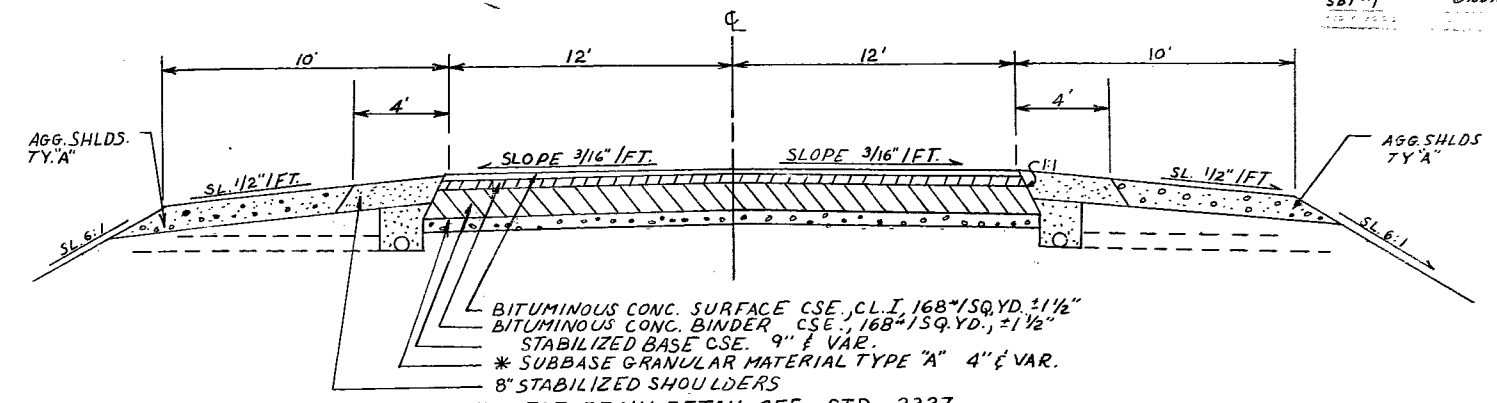
STATE OF ILLINOIS  
 DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
 APPROVED: *[Signature]*  
 EXAMINED: *[Signature]*  
 11-29-71  
 UNDER SECRETARY, CHIEF TRANSPORTATION ENGINEER  
 11-29-71  
 SECRETARY

DEPARTMENT OF TRANSPORTATION  
 FEDERAL HIGHWAY ADMINISTRATION  
 DIVISION ENGINEER

CONTACT NO. 10000



**SECTION VIEW OF COMMERCIAL ENTRANCE  
RT. STA. 293+75**



**TYPICAL SECTION**  
NOTE: THE SHOULDER DESIGN SHOWN ON THIS TYPICAL SECTION SHEET REFLECTS ALTERNATE A. FOR ALTERNATE B, SEE SHEET NO. JA

\* SEE DETAIL BELOW: TRANSITION TAPER TO GRADE CHANGES.

**GENERAL NOTES**

Where section or sub-section monuments are encountered, the Engineer shall be notified before such monuments are removed. The Contractor shall protect and carefully preserve all monuments until an authorized surveyor or agent has witnessed or otherwise referenced their location.

The thickness of bituminous mixture shown on the plans is the nominal thickness. Deviations from the nominal thickness will be permitted when such deviations occur due to irregularities in the existing surface or base on which the bituminous mixture is placed.

The nominal thickness for subbase granular material; base and surface courses are shown on typical sections, standards, schedules or special details. The constructed thickness of the above items shall not be less than 90 per cent of the nominal thickness at any location.

Trees along the edge of the right of way shall be saved, if in the opinion of the Engineer they do not interfere with construction operations. Payment shall be made on the quantity of inch diameter removed.

It shall be the Contractor's responsibility to determine the actual location of all underground utility facilities. He shall also obtain from the respective utility companies detailed information relative to the location of their facilities and the working schedules of the utility companies for their marking of the exact location.

For the purpose of this contract, spring seeding is defined as that performed between December 31st to July 1st. Fall seeding is defined as that performed between July 1st and December 31st. Seeding will not be permitted at any time when the ground is frozen, wet, or in unillable condition.

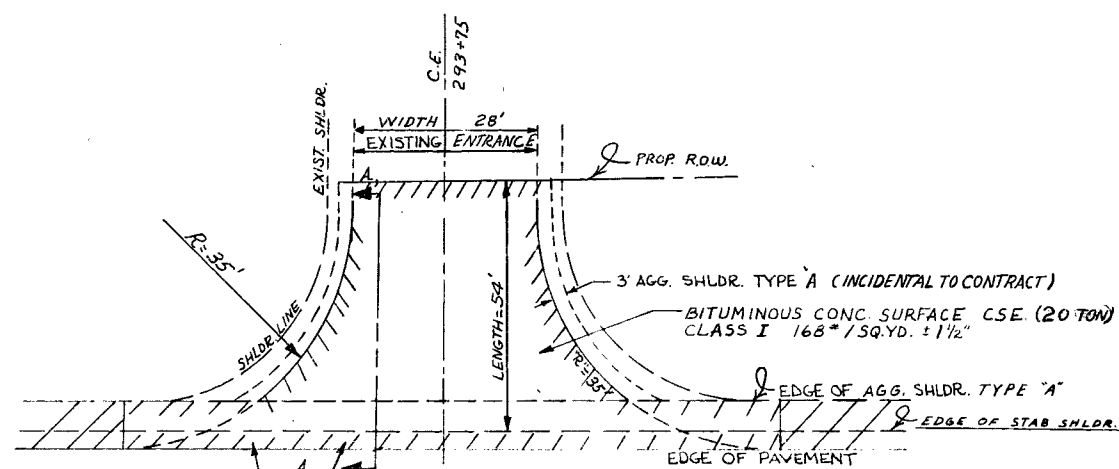
Sign Panels are identified by Illinois standard numbers and are to conform to Illinois Standard Design. Details of these Standards are available at the office of the District Engineer, Ottawa, Illinois.

**EARTH WORK**

THE END AREAS SHOWN ON THE CROSS SECTIONS AND EARTH WORK QUANTITIES SHOWN ON THE SCHEDULES AND OR PLAN AND PROFILE SHEETS ARE FOR ALTERNATE "A" ONLY.

THE EARTH WORK FOR ALTERNATE "B" IS SHOWN ON THE SUMMARY OF QUANTITIES ONLY.

**TEMPORARY BITUMINOUS TAPERS  
145' WEST OF STATION 264+50  
(SEE STD. 2143 FOR WIDTH TRANSITION TAPER)**



**PLAN OF COMMERCIAL ENTRANCE  
RT. STA. 293+75**  
STAB. SHOULDER (8") INCLUDED IN (8") STAB. SHOULDER QUANT.

State of Illinois  
Division of Highways  
District Three

Prepared By: *Ralph J. ...*  
F.M. District Engineer of Design

Date: 3-26-71

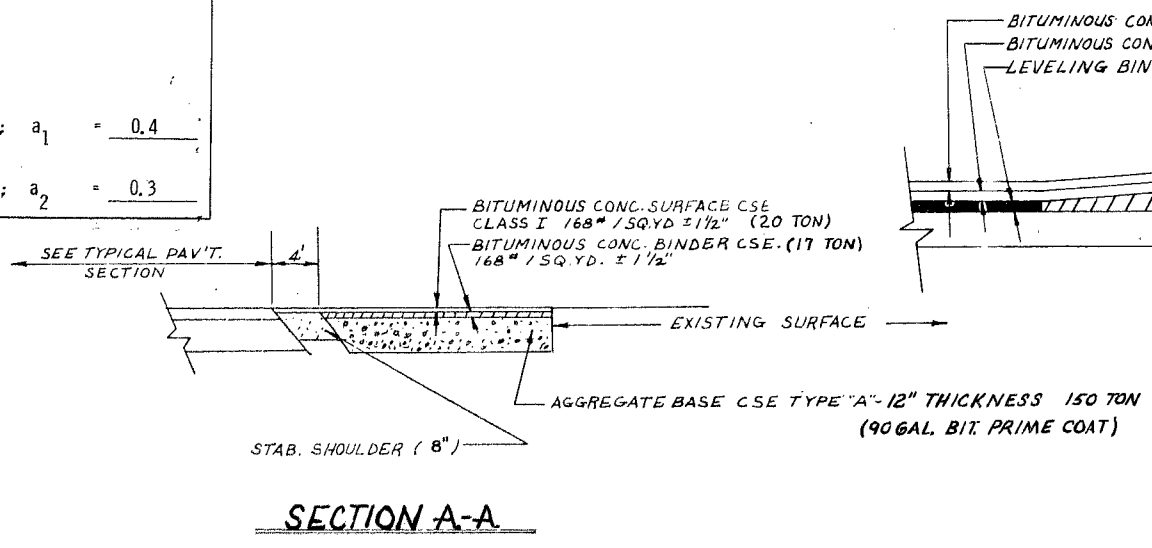
Examined By: *Claude ...*  
District Engineer of Construction

*Philip M. ...*  
District Engineer of Maintenance

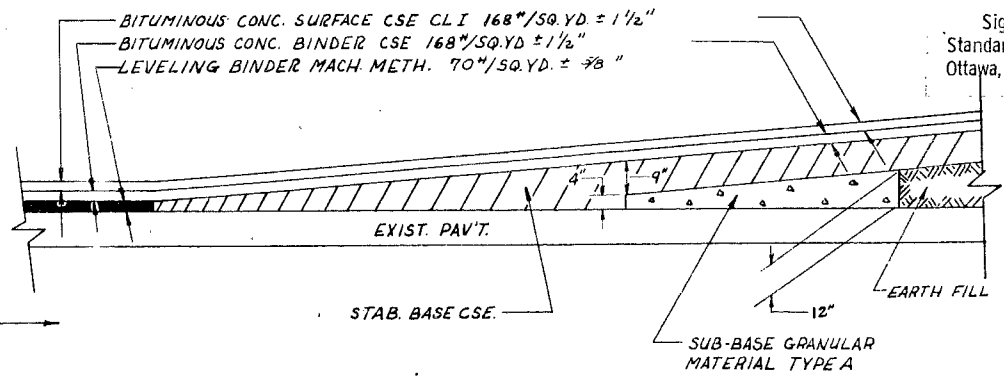
*Lolita ...*  
District Engineer of Materials

*Arwin ...*  
District Engineer of Traffic

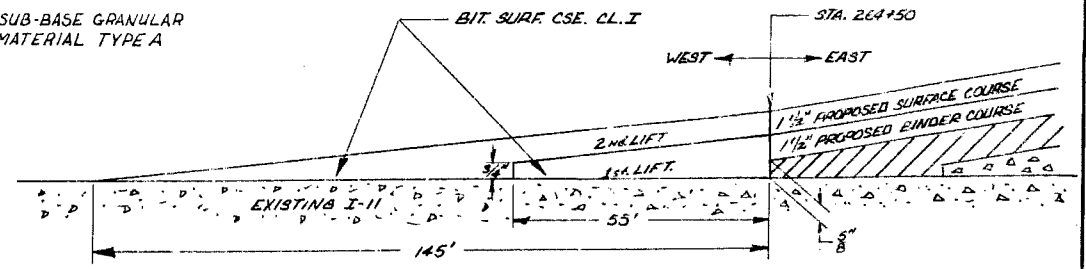
Structural Design Traffic: Year 1981 ; P.C. = 2032
U.S. = 448 ; M.U. = 90
Class II Roads and Streets
Minimum Soil Support: CBR = 2.9
Per Cent of S.D.T. in Design Lane: $U_p = 50\%$ ; $U_s = 50\%$
$U_m = 50\%$
T.F. = 0.485 ; $D_t = 3.72$
Pavement Structure Materials:
Surface Course Type: Bituminous Concrete ; $a_1 = 0.4$
Base Course Type: Stabilized Base Cse. ; $a_2 = 0.3$

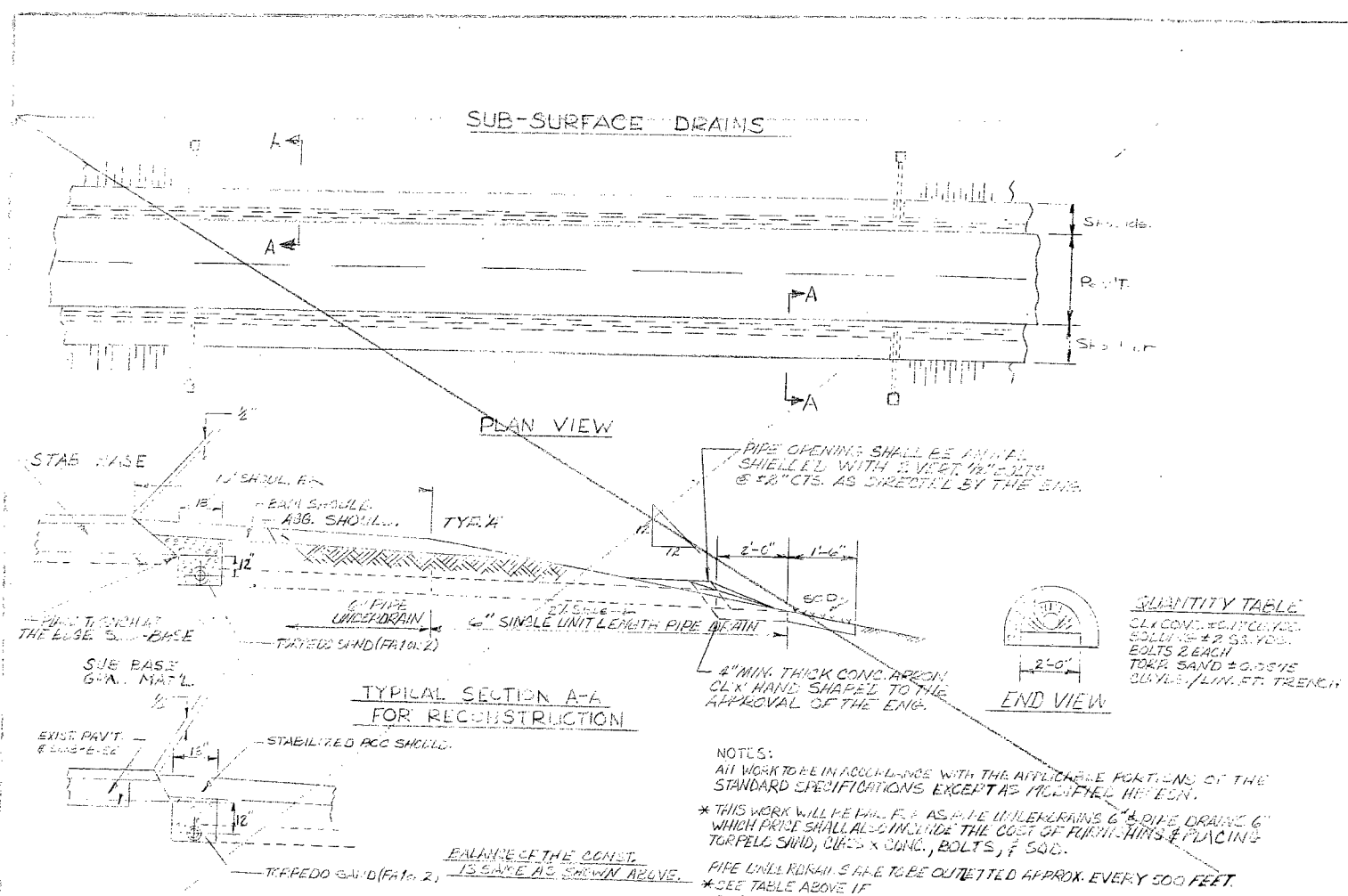


**SECTION A-A**



**TRANSITION TAPER TO GRADE CHANGES**





CULVERT AND STORM SEWER SCHEDULE													
Station	Type	Standard And Design Number	Class "X" Concrete Cu. Yds.	Reinforcement Bars Lbs.	Type P-1 15" Lin. Ft.	Pipe Culverts RCCP Ty-6 36" Lin. Ft.	End Sections 15" Each	Storm Sewer Lin. Ft.					
								6"	8"	10"	12"		
LT. 274+00.0E	15"x26" P-1	2228											
E 276+60	36"x156" Ty6	RCCP; D36-2, 1976	*3.5	100	26	156'	2						
E 281+45.5	Box Culvert	8'x6'x235' - 4 5/8"	279.8	59,833									
Rt. 293+75	15"x46" P-1	2228-1			46		2						
291+40 Across ROW													
<b>Total</b>												180'	**200
			283.3	59,933	72	156'	4	**200	380	**200	**200		

\* 3.5 Cu. Yd. Class "X" Conc. Hdwl

\*\* Added to establish a unit bid price

**QUANTITY TABLE**

CLY CONC. 281.8 CU. YD.  
 SOD 27.5 SQ. YD.  
 BOLTS 2 EACH  
 NUTS 2 EACH  
 TORQ SAND 10,000 LBS  
 SOD 1/2 LIN. FT. TRENCH

**NOTES:**

ALL WORK TO BE IN ACCORDANCE WITH THE APPLICABLE PORTIONS OF THE STANDARD SPECIFICATIONS EXCEPT AS MODIFIED HEREIN.

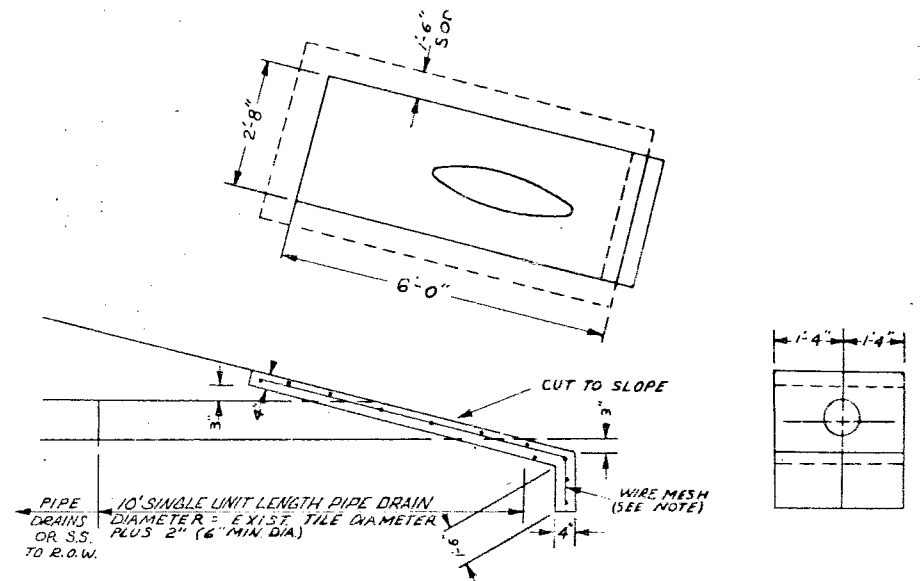
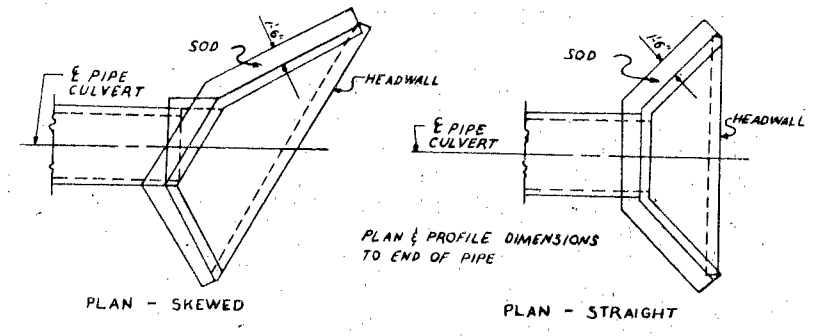
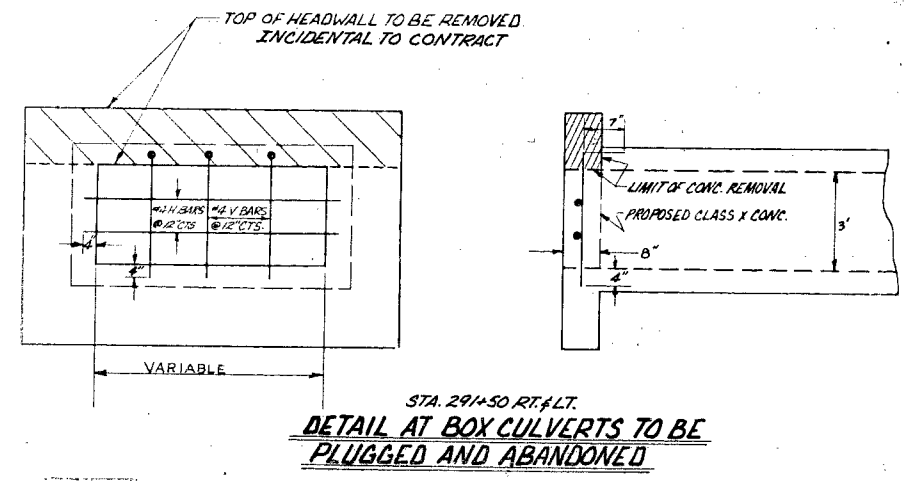
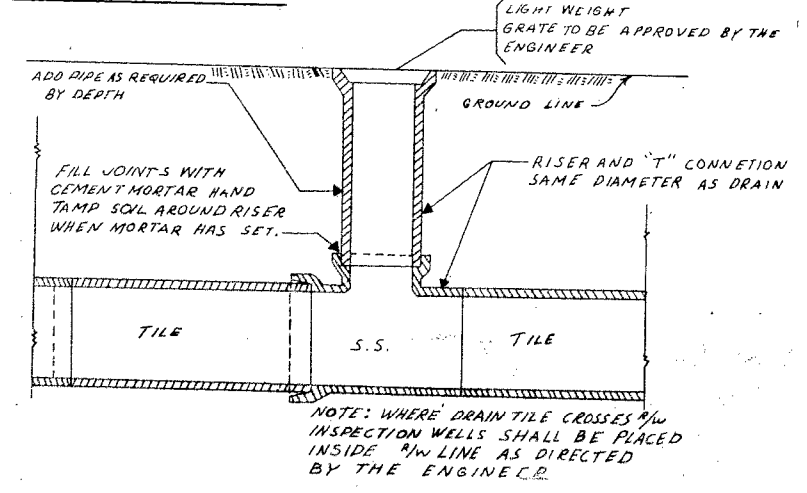
\* THIS WORK WILL BE PAID FOR AS PIPE UNDERDRAINS & PIPE DRAINS 6" WHICH PRICE SHALL ALSO INCLUDE THE COST OF FURNISHING & PLACING TORQUE SAND, CURB & CONC., BOLTS, & SOD.

PIPE UNDERDRAINS SHALL BE OUTLETED APPROX. EVERY 500 FEET.

\* SEE TABLE ABOVE IF ANY ITEMS ARE APPLICABLE.

**TYPICAL SECTION A-A FOR SLOPES AT WIDENING & RESURFACING**

**INSPECTION WELL**



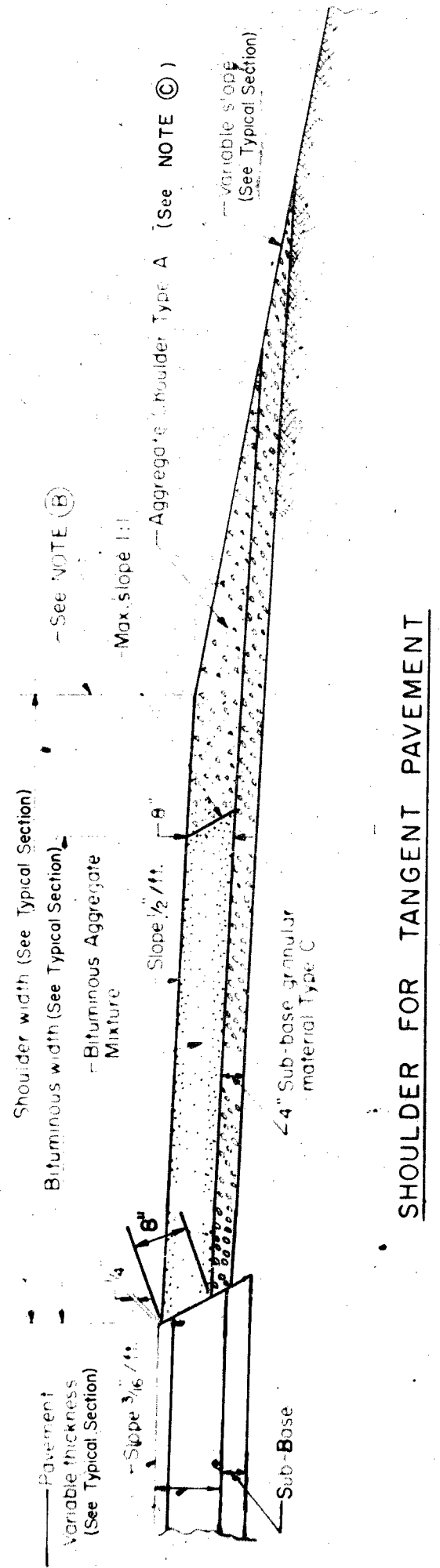
NOTE: COST OF FURNISHING & INSTALLING WIRE MESH SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE PER CUBIC YARD FOR CLASS "X" CONCRETE (HEADWALLS) WIRE MESH TO WEIGH NOT LESS THAN 58 LBS. PER 100 SQ. FT.

**DETAIL SPECIAL FIELD TILE HEADWALL**

**SOD AT END SECTION**

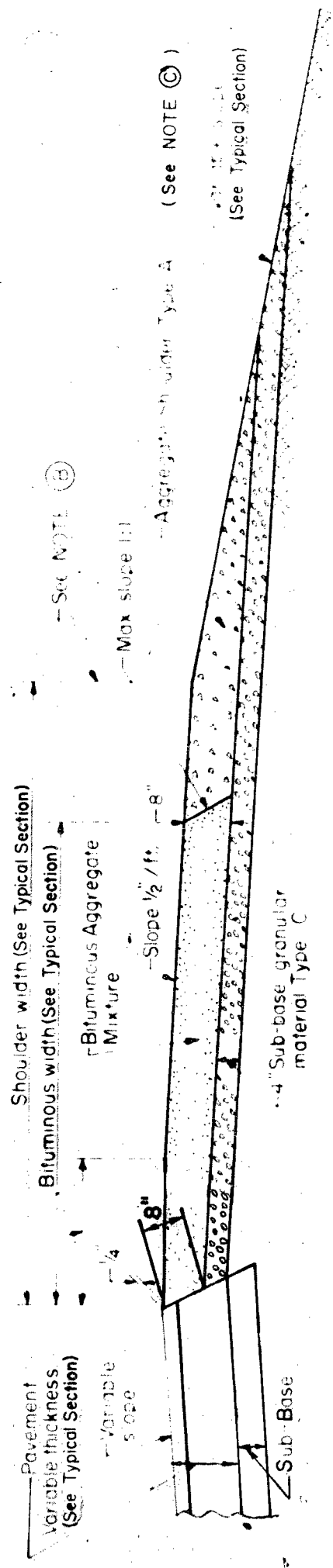
# SHOULDER DETAILS ALTERNATE "B"

PA. 8  
SBL 7 G-VB Grundy 35 3A

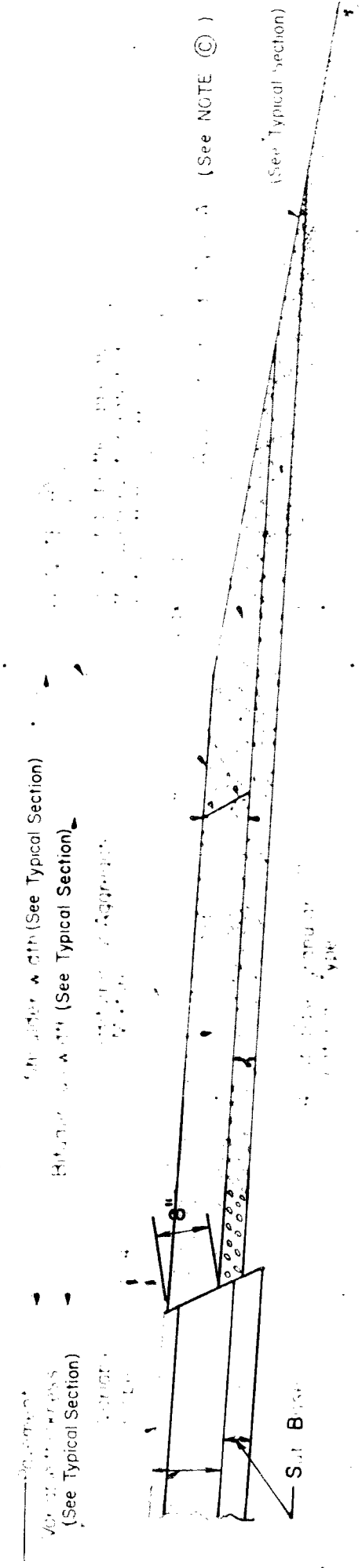


## SHOULDER FOR TANGENT PAVEMENT

When the superlevation of pavement exceeds 0.04 ft./ft., a 2 foot portion of the shoulder that is adjacent to pavement shall be sloped so that the algebraic difference between pavement and shoulder will not be greater than 0.08 ft./ft.



## SHOULDER FOR SUPERELEVATED PAVEMENT (OUTSIDE OF CURVE)



## SHOULDER FOR SUPERELEVATED PAVEMENT (INSIDE OF CURVE)

**NOTE C**  
The wedge portion of the Aggregate Shoulder Type A may be compacted in accordance with Type B shoulder requirements.

SUMMARY OF QUANTITIES

Code No.	Item	Construction Type Code	Unit	Total Quantity	Roadway Section 6203	Bridge Section X171
210001	TRENCH BACKFILL		CU YD	100	100	
213001	SUB-BASE --- GRANULAR MATERIAL, TYPE A		TON	266	266	
215001	AGGREGATE SHOULDERS, TYPE A		TON	2461	2461	
X21601	EXCAVATION		CU YD	3704	3704	
301001	AGGREGATE BASE COURSE, TYPE A		TON	150	150	
301046	STABILIZED BASE COURSE		TON	3880	3880	
402001	AGGREGATE SURFACE COURSE, TYPE A		TON	628	628	
406001	BITUMINOUS MATERIALS PRIME COAT		GALLON	184	184	
406007	BITUMINOUS CONCRETE BINDER COURSE		TON	668	668	
406008	BITUMINOUS CONCRETE SURFACE COURSE, CLASS I		TON	673	673	
408006	PORTLAND CEMENT CONCRETE PAVEMENT 16 1/2 - 10 - 16 1/2		SQ YD	290	290	
X50201	STRUCTURE EXCAVATION		CU YD	680		680
503002	CLASS X CONCRETE HEADWALL		CU YD	3.5	3.5	
503004	PROTECTIVE COAT		SQ YD	1620	290	1330
504003	CLASS X CONCRETE		CU YD	1141.4	285.1	856.3
507025	STUD SHEAR CONNECTORS		EACH	2934		2934
507030	FURNISHING AND ERECTING STRUCTURAL STEEL		L SUM	1		1
508005	ALUMINUM RAILING		LIN FT	478		478
511025	PIPE CULVERTS, TYPE I, 15"		LIN FT	72	72	
511451	PIPE CULVERTS, TYPE 6 RCCP 36"		LIN FT	156	156	
511829	CULVERT END SECTIONS 15"		EACH	4	4	
512001	REINFORCEMENT BARS		POUND	223.15	75.695	147.420
513005	FURNISHING CREOSOTED PILES 20.1 TO 38 FEET		LIN FT	594		594
513012	FURNISHING STEEL PILES 8 BP 36		LIN FT	3200		3200
513022	DRIVING TIMBER PILES		LIN FT	594		594
513026	DRIVING STEEL PILES		LIN FT	3200		3200
513032	TEST PILE STEEL 8 BP 36		EACH	2		2
514001	NAME PLATES		EACH	1		1
603002	STORM SEWERS, TYPE I 6"		LIN FT	200	200	
603003	STORM SEWERS, TYPE I 8"		LIN FT	380	380	
603004	STORM SEWERS, TYPE I 10"		LIN FT	200	200	
603005	STORM SEWERS, TYPE I 12"		LIN FT	200	200	
616119	PAVED DITCH 8 FEET		LIN FT	122	122	
617001	PAVEMENT REMOVAL		SQ YD	682	682	
618001	SLOPE WALLS 4"		SQ YD	905		905
628001	STEEL PLATE BEAM GUARD RAIL, SINGLE RAIL		LIN FT	3700	3700	
X62801	TERMINAL SECTION, SINGLE RAIL		EACH	4	4	
639001	FURNISHING AND ERECTING RIGHT OF WAY MARKERS		EACH	15	15	
642001	SEEDING, CLASS I		ACRE	6.6	6.6	
642003	SEEDING, CLASS III		ACRE	3.9	3.9	
642004	NITROGEN FERTILIZER NUTRIENTS		POUND	948	948	
642005	PHOSPHORUS FERTILIZER NUTRIENTS		POUND	787	787	
642006	POTASSIUM FERTILIZER NUTRIENTS		POUND	629	629	
642007	AGRICULTURAL GROUND LIMESTONE		TON	33	33	
643002	ASPHALT COATED MULCH		TON	21	21	
643005	EMULSIFIED ASPHALT		GALLON	2100	2100	
644001	SODDING		SQ YD	277	277	
644002	SUPPLEMENTAL WATERING		UNIT	1	1	
646001	** ENGINEER'S FIELD OFFICE, TYPE A		EACH	1**		
646003	** ENGINEER'S FIELD LABORATORY		EACH	1**		
XZ1602	TOPSOIL PLACEMENT		SQ YD	33336	33336	
Z10227	EXPLORATION TRENCH 52 IN DEPTH		LIN FT	3000	3000	
Z10281	PERMANENT SURVEY MARKERS, TYPE I		EACH	3	3	
Z10561	INSPECTION WELLS		EACH	8	8	
Z10565	NEOPRENE EXPANSION JOINT		LIN FT	143		143
Z21219	* REMOVAL OF STANDARD SIGN ASSEMBLY		EACH	2		2
XZ1100	* TRAINEES		HRS	1000*		
215010	STABILIZED SHOULDERS (6")		SQ YD	2592	2592	
202001	ALTERNATE A					
204001	EARTH EXCAVATION		CU YD	3927	3927	
607002	BORROW EXCAVATION		CU YD	210,332	210,332	
607152	PIPE DRAINS 6"		LIN FT	571	571	
	PIPE UNDERDRAINS 6"		LIN FT	5,870	5,870	
202001	ALTERNATE B					
204001	EARTH EXCAVATION		CU YD	3,968	3,968	
607002	BORROW EXCAVATION		CU YD	210,242	210,242	
213007	PIPE DRAINS 6"		LIN FT	485	485	
	SUB-BASE GRANULAR MATERIAL, TYPE C		TON	2,055	2,055	

SUMMARY OF QUANTITIES

\* SPECIALTY ITEM  
 \*\* CONSTRUCTION TYPE CODE: CE59  
 † CONSTRUCTION TYPE CODE: Y080

BITUMINOUS MATERIAL QUANTITIES						
Station To	Station	Length Feet	Area Sq. Yds	112#Sq Yd/In. Bit. Conc. Bind. Cse Tons	112#Sq Yd/In. Bit. Conc. Surf. Cse. Tons	0.10Gal/Sq Yd. Bit. Mat'l's Prime Coat Gallons
264+50	277+86.49	1336.49	3564	304	301	21
277+86.49	281+38.87	Bridge and approach omission				
281+38.87	296+10	1471.13	3923	335	331	33
* Temporary Taper				12	21	40
Commercial Ent. Rt. Sta. 293+75				17	20	90
Totals		2807.62	7487	668	673	184

\* See Detail

PAVED DITCH 8 FEET		
Station	Side	Lin. Ft.
276+60	Rt	42
283+30	Lt	80
Total		122 Lin. Ft.

Refer to drainage Plan sheet No.

SCHEDULE OF SODDING			
	Station	Side	Sq. Yds.
Standard Hdws	276+60	Rt & Lt	6
	281+45.5	Rt & Lt	13
End Sections	293+75	Rt	3
* Paved Ditches	276+60	Rt	34
	283+30	Lt	59
** Bridge Cone SlopeWall & GUTTERS			162
Total			277 Sq. Yds.

\* SOD AT PAVED DITCHES IS 3 FT. WIDTH EACH SIDE & END  
\*\* SOD AT SLOPEWALL & GUTTERS IS 3 FT. WIDTH

STABILIZED SHOULDERS 8" & AGGREGATE SHOULDERS TYPE A				
Station To	Station	Length	Stabilized Shoulders 8"	Aggregate Shldrs Type A
			Lt & Rt Sq. Yds.	Lt & Rt Tons
264+50	278+41.48	1391.48	1236	1174
278+41.48	280+83.88	Bridge Omission		
280+83.88	296+10	1526.12	1356	1287
Totals		2917.6	2592	2461

STEEL PLATE BEAM GUARD RAILS (SINGLE RAIL) & TERMINAL SECTIONS			
Bridge Quadrant	S. P. B. G. R. Single Rail Lin. Ft.	Terminal Section Each	
N. W.	912.5	1	
S. W.	862.5	1	
N. E.	937.5	1	
S. E.	987.5	1	
Totals		3700	4

SEEDING QUANTITIES AND FERTILIZER NUTRIENTS							
Type Of Seeding	Area (Acre)	Nit. (LBS)	Phos. (LBS)	Pot. (LBS)	Agricultural Limestone (Ton)	Asphalt Coated Mulch (Ton)	Emulsified Asphalt (Gal)
Class I:							
Sta 264+50 - Sta 272+00	2.5	300	180	120	8	5	500
Sta 272+00 - Sta 288+00	1.9	228	137	91	6	4	400
Sta 288+00 - Sta 296+10	2.2	264	158	106	7	4	400
Class I Totals	6.6	792	475	317	21	13	1300
Class III:							
* Sta 272+00 - Sta 288+00	3.9	156	312	312	12	8	800
Class III Totals	3.9	156	312	312	12	8	800
Grand Totals		948	787	629	33	21	2100

\* Type III Seeding shall be placed within the above station limits from the top of the side slope to the toe of the side slope.

RATES OF APPLICATION		
	Type I	Type III
Nitrogen Fertilizer Nutrients	120 Lbs/Acre	40 Lbs/Acre
Phosphorus Fertilizer Nutrients	72 Lbs/Acre	80 Lbs/Acre
Potassium Fertilizer Nutrients	48 Lbs/Acre	80 Lbs/Acre
Agricultural Limestone	3 Tons/Acre	3 Tons/Acre
Asphalt Coated Mulch	2 Tons/Acre	2 Tons/Acre
Emulsified Asphalt	100 Gal/Ton	100 Gal/Ton

(1) STABILIZED BASE COURSE & SUB-BASE GRANULAR MATERIAL TYPE A					
Station To	Station	Length	Stabilized Depth	Stabilized Base Course (9" & Var) Ton	(2) Sub-Base Granular Material Type A Ton
264+50	265+50	100'	Var.	107	-
265+50	266+15	65'	-	-	90 (Var)
265+50	277+86.49	1236.49'	9"	1748	-
277+86.49	278+41.48	*54.99' - Bridge Omission			
278+41.48	280+83.88	*54.99' - Bridge Omission			
280+83.88	281+38.87	54.99'	-	-	43 (6")
281+38.87	294+50	1311.13'	9"	1854	-
293+85	294+50	65'	-	-	90 (Var)
294+50	296+10	160'	Var.	171	-
Totals				3880	266

(1) Station to Station Limits Should be Considered Approximate  
(2) See Transition Taper to Grade Change  
Approach Slabs: SEE STD 1909 - Method II

PIPE UNDERDRAINS 6" & PIPE DRAINS 6"						
Station To	Station	Side	Lin. Ft.	Station	Side	T-Joints
264+50	278+17	Rt	1367	265+00	Rt & Lt	
264+50	278+47	Lt	1397	272+00	Rt & Lt	
280+79	296+10	Rt	1531	286+00	Rt & Lt	
281+09	296+10	Lt	1501	291+00	Rt & Lt	
Total Longitudinal Pipe Underdrains 5796 Lin. Ft.						
8 Laterals @ 9.25 Ft.			74			
Total Pipe Underdrains 6"			5870 Lin. Ft.			
Total Pipe Drains 6"						
8 Laterals @ 10.75 Ft.			86 Lin. Ft.			
STA. 291+25 TO 296+10 LT. ALONG ROW			485 LIN. FT.			
TOTAL PIPE DRAINS 6"			571 LIN. FT.			

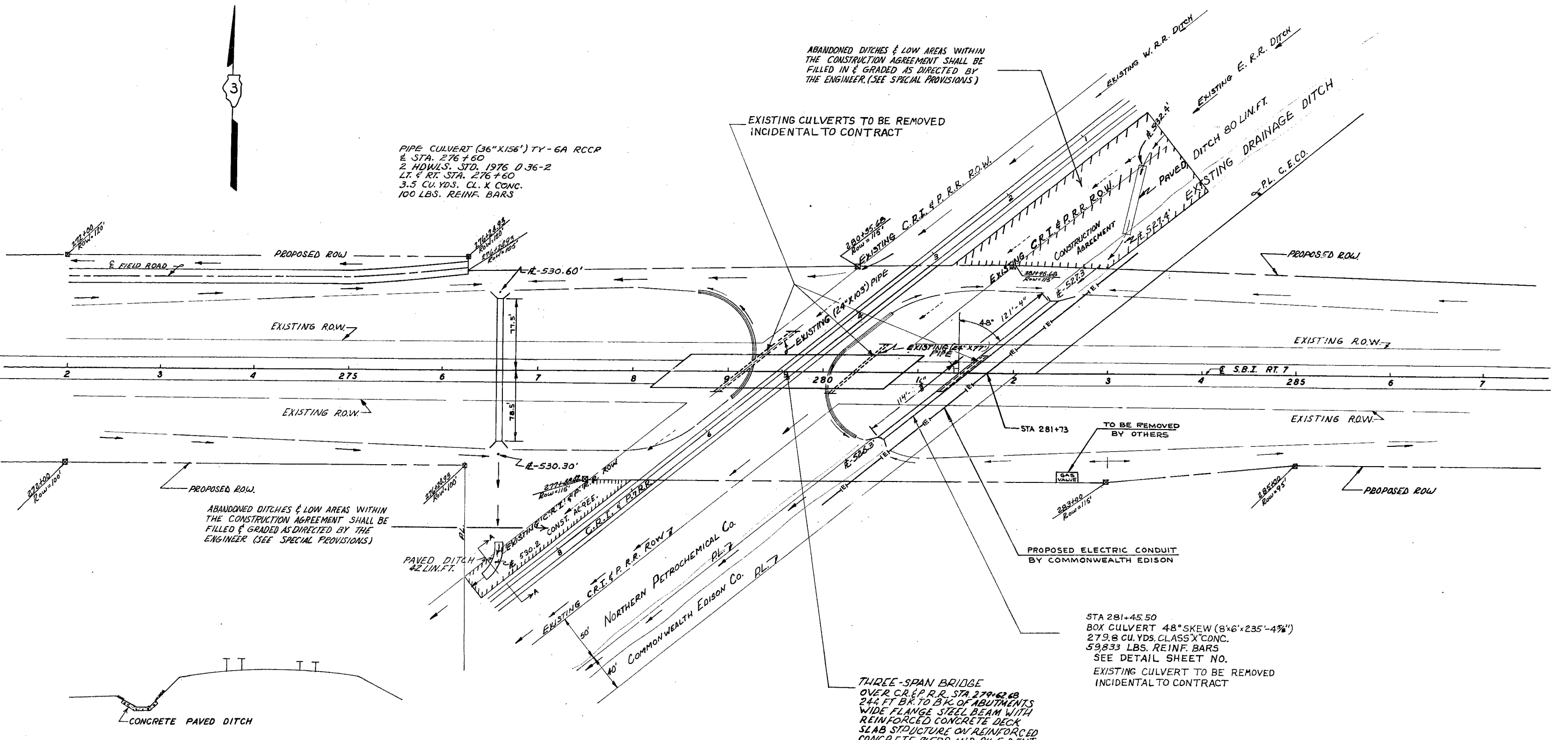
R. O. W. MARKERS		
Station	Side	Distance
268+00	Lt	105'
268+00	Rt	85'
272+00	Lt	120'
272+00	Rt	100'
276+24.93	Lt	120'
276+24.93	Rt	100'
277+48.42	Rt	115'
280+35.68	Lt	115'
281+95.68	Lt	115'
283+00	Rt	115'
285+00	Rt	95'
291+00	Rt	95'
292+00	Lt	90'
295+00	Lt & Rt	60'
Total		15

PAVEMENT REMOVAL			
Station To	Station	Lin. Ft.	Sq. Yds.
276+56	276+64	8	16
277+95	279+45± Skew	150	300
279+80± Skew	281+63	183	366
Totals			682

EARTHWORK QUANTITIES					
Roadway	Station To	Station	Cut Cu. Yds.	Fill Cu. Yds.	Borrow Cu. Yds.
S. B. I. 7	264+50	278+41	1271	87,353	103,535
BRIDGE OMISSION					
S. B. I. 7	280+85	296+10	2656	91,227	106,797
Totals			3927	178,580	210,332

PERMANENT SURVEY MARKERS, TYPE I		
Station	Location	Count
275+00	P. I. on E	1
276+24.93	1/4 Sec. Corner 1' Rt	1
291+00	P. C. T.	1
Total		3

TOPSOIL QUANTITIES		
STA. to STA.	Needed Cu Yds	Placement Sq Yds
264+50 278+41	1758	15822
280+85 296+10	1946	17484
TOTAL	3704	33306



PIPE CULVERT (36"x156") TY-6A RCCP  
 @ STA. 276+60  
 2 HDWLS. STD. 1976 D36-2  
 LT. & RT. STA. 276+60  
 3.5 CU. YDS. CL. X CONC.  
 100 LBS. REINF. BARS

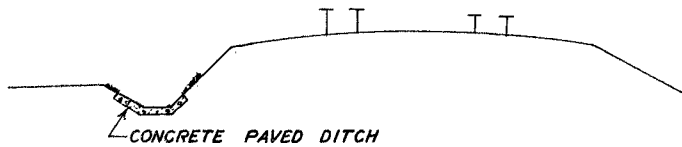
ABANDONED DITCHES & LOW AREAS WITHIN  
 THE CONSTRUCTION AGREEMENT SHALL BE  
 FILLED IN & GRADED AS DIRECTED BY  
 THE ENGINEER. (SEE SPECIAL PROVISIONS)

EXISTING CULVERTS TO BE REMOVED  
 INCIDENTAL TO CONTRACT

ABANDONED DITCHES & LOW AREAS WITHIN  
 THE CONSTRUCTION AGREEMENT SHALL BE  
 FILLED & GRADED AS DIRECTED BY THE  
 ENGINEER (SEE SPECIAL PROVISIONS)

STA 281+45.50  
 BOX CULVERT 48° SKEW (8'x6'x235'-4 3/8")  
 279.8 CU. YDS. CLASS X CONC.  
 59,833 LBS. REINF. BARS  
 SEE DETAIL SHEET NO.  
 EXISTING CULVERT TO BE REMOVED  
 INCIDENTAL TO CONTRACT

THREE-SPAN BRIDGE  
 OVER C.R.I. & P.R.R. STA 279+62.68  
 242 FT BK. TO BK. OF ABUTMENTS  
 WIDE FLANGE STEEL BEAM WITH  
 REINFORCED CONCRETE DECK  
 SLAB STRUCTURE ON REINFORCED  
 CONCRETE PIERS AND PILE BENT  
 ABUTMENTS



SECTION A-A

**DRAINAGE PLAN  
 GRADE SEPARATION  
 S.B.I. RTE. 7 & C.R.I. & P. R.R.  
 S.B.I. RTE. 7 SECTION G-VB  
 GRUNDY COUNTY**

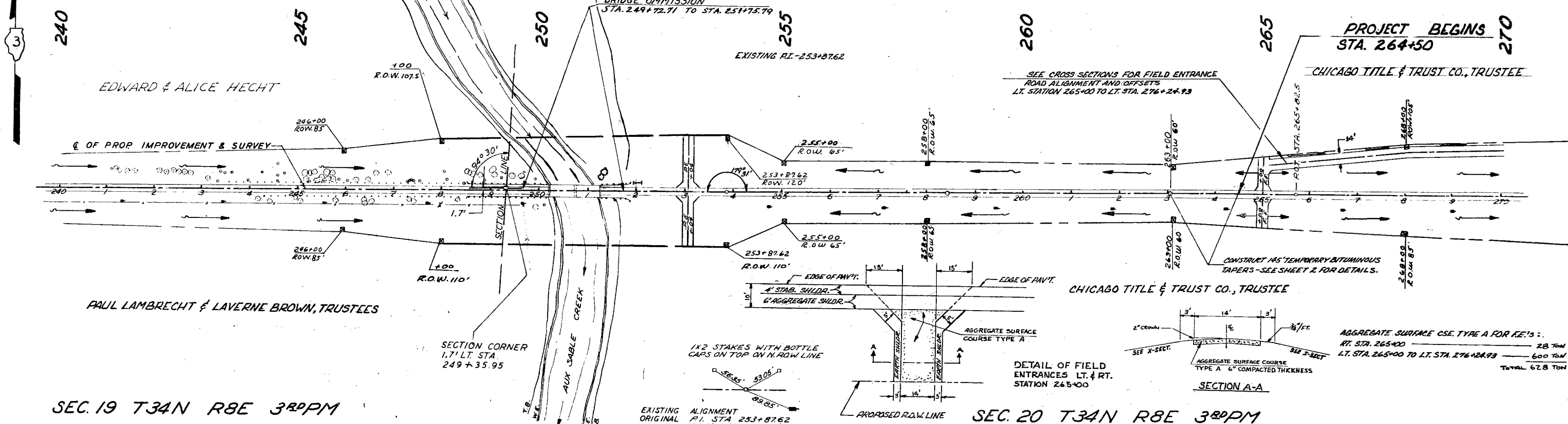
SCALE 1" = 50'



SEC. 18 T34N R8E 3RD PM

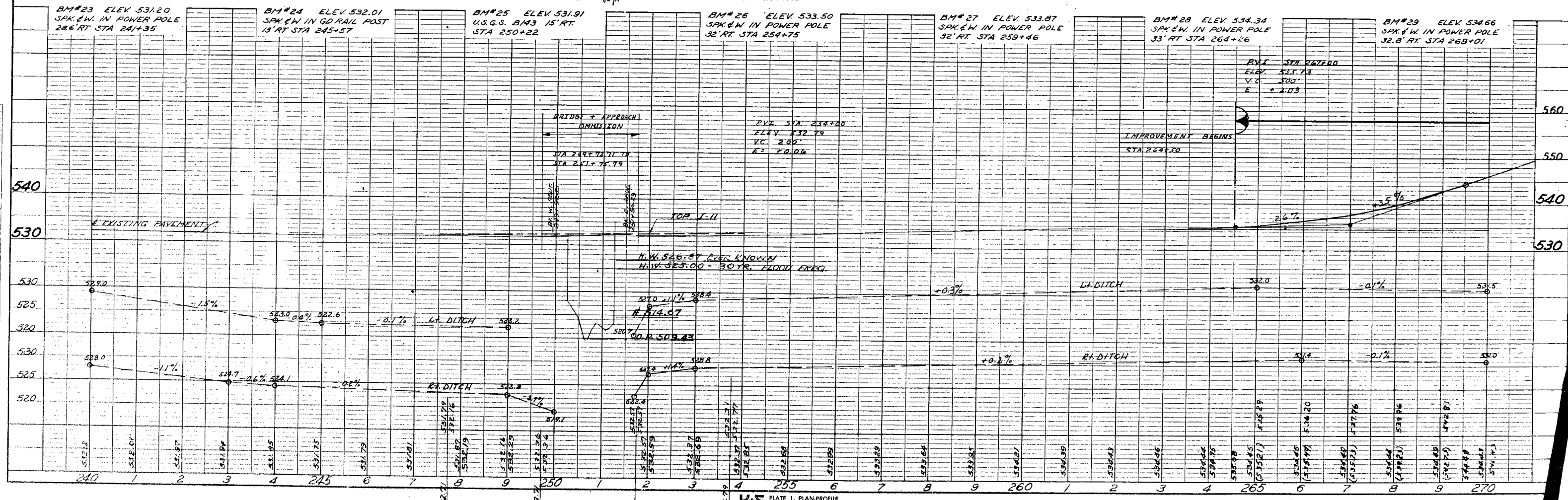
SEC. 17 T34N R8E 3RD PM

SBI-7 G-VB GRUNDY 35 7  
EA-8  
F-10 (12)



SEC. 19 T34N R8E 3RD PM

SEC. 20 T34N R8E 3RD PM



SECTION	COUNTY	SHEET
GVB	GRUNDY	35
DATE	PROJECT	NO.
5-44	SBI-7	8

SEC. 17. T. 34N., R. 8E. 320P.M.

SEC. 20. T. 34N., R. 8E. 320P.M.

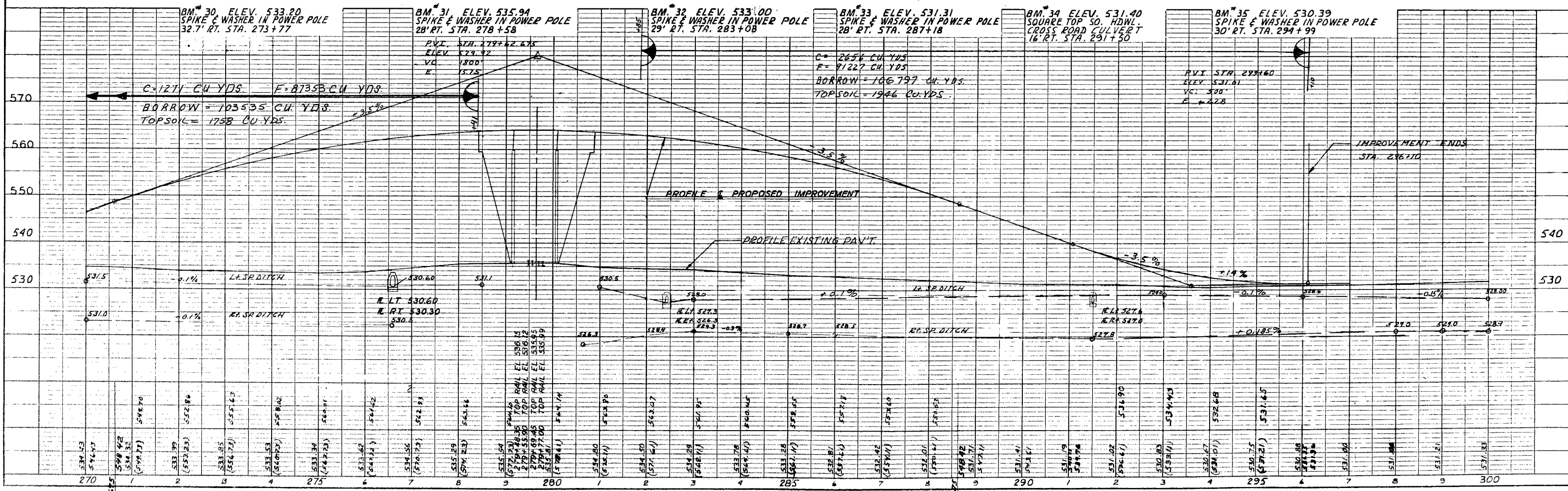
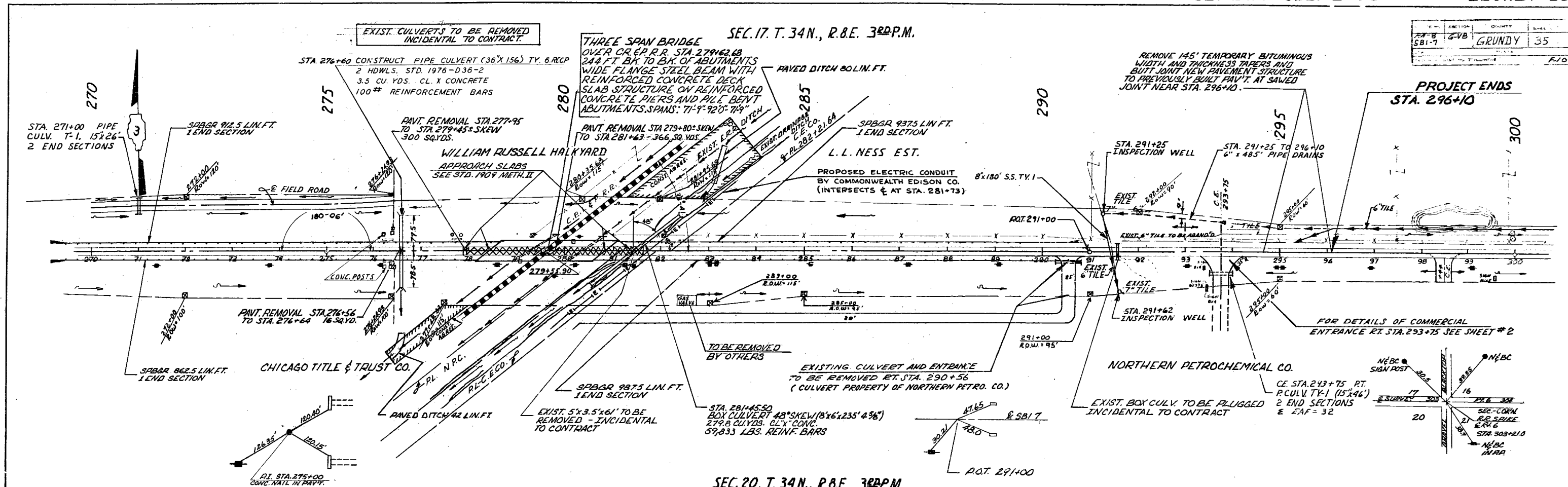


PLATE 1, PLAN-PROFILE KEUFFEL & ESSER CO.

48 7012 IMPERIAL TRACING COLOR

PLAN  
 SURVEYED  
 PLOTTED  
 CHECKED  
 DATE  
 5-44

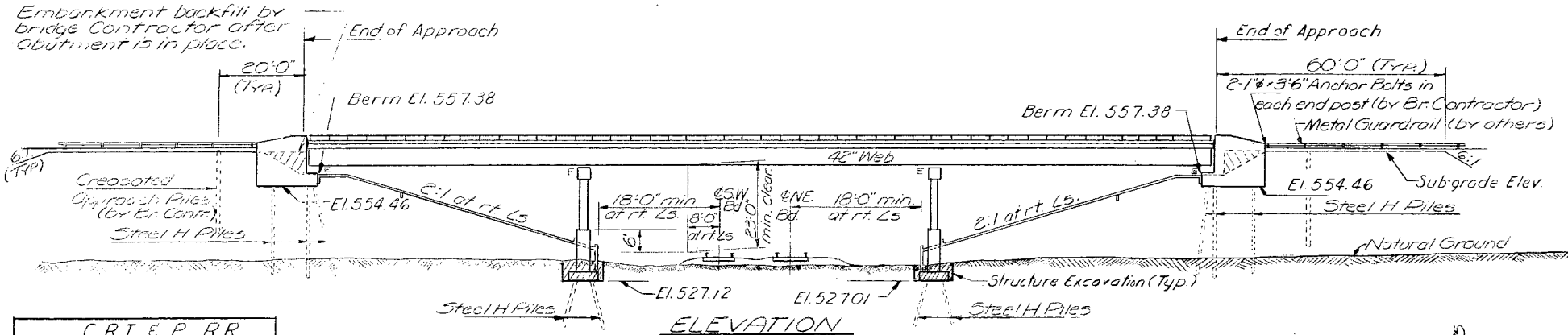
PROFILE  
 DATE  
 5-44

B.M. No. 31 Spike & Washer in Power Pole  
28' Rt. Sta. 279+58 Elev 555.94

STATE OF ILLINOIS  
DIVISION OF HIGHWAYS

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 1
8	G-VB	GRUNDY	35	9	15 SHEETS
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT			

Note:  
The Engineer shall determine in the field the location of the deck drains in the spans where R.R. signal and communication pole lines are involved. No deck drains shall be permitted in spans over tracks.



C.R.I. & P. R.R.  
BUILT 19 BY  
STATE OF ILLINOIS  
SBI RT. 7 SEC. 6-VB  
PROJ. FG-10 (12)  
STATION 279+62.68  
LOADING HS 20  
NAME PLATE  
Std. 2113-1

**GENERAL NOTES**

All reinforcement bars shall be lapped 24 diameters unless otherwise shown.

Fasteners shall be high strength bolts. Bolts  $\frac{3}{4}$ " $\phi$ ; open holes  $\frac{1}{16}$ " $\phi$ , unless otherwise noted.

Calculated weight of Structural Steel = 274,260 Lbs.

The basic lead silico chromate paint system shall be used for shop and field painting of Structural Steel.

Field welding of construction accessories will not be permitted to the bottom flange of beams or girders nor to the top flange for a distance equal to one-fourth the span length each way from the pier supports. Field welding in other areas will be permitted only when approved by the Engineer.

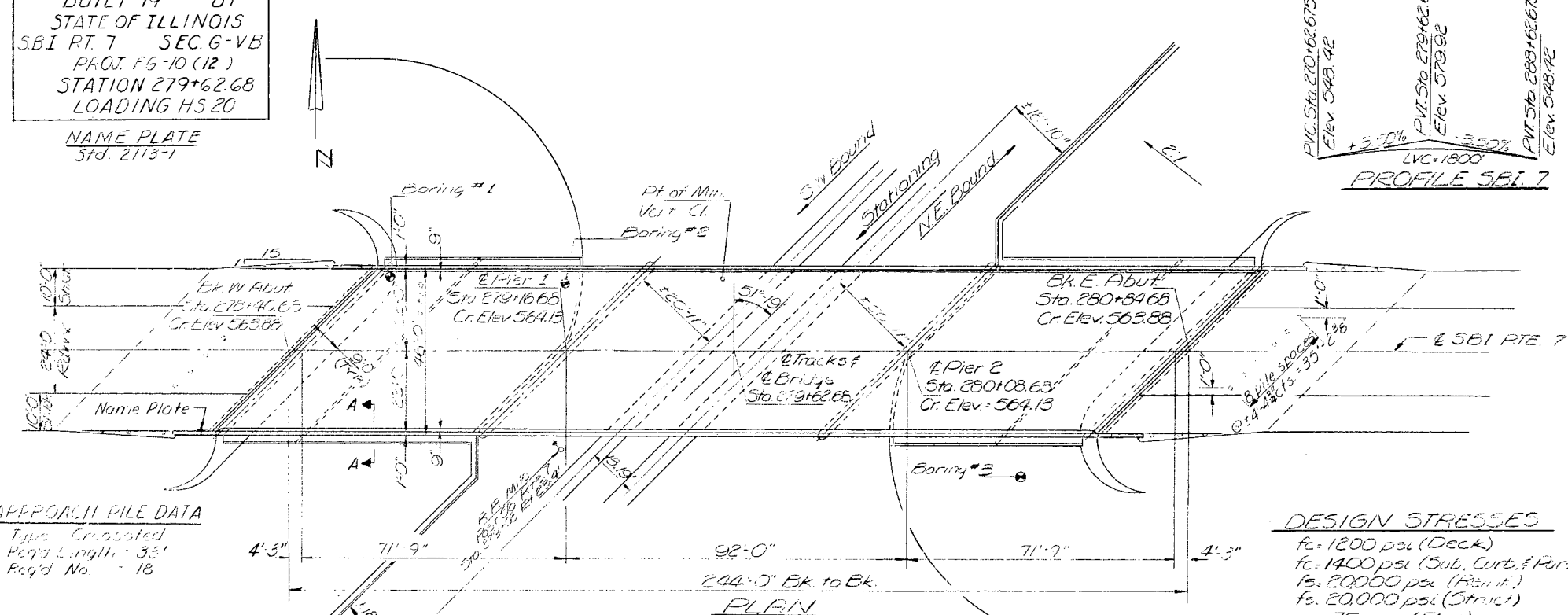
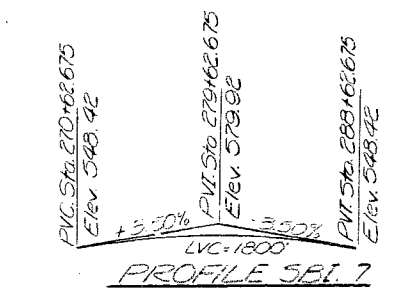
Anchor bolts shall be set before bolting diaphragms over supports.

Slope wall shall be reinforced with welded wire fabric 6" x 6" mesh, weighing 58# per 100 sq. ft.

The Contractor shall drive 2 steel (6 BP 36) test piles in permanent locations, one at the East Abutment and one at Pier 1, as directed by the Engineer before ordering the remainder of piles.

The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments.

The concrete rail section above the mandatory construction joint at the top of the slab shall be constructed of Class X Concrete, except the aggregates shall conform to the requirements of Handrail Concrete.



APPROACH PILE DATA  
Type: Crossed  
Pile Length: 35'  
Req'd. No.: 18

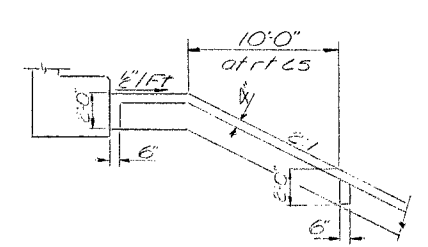
**DESIGN STRESSES**

$f_c = 1200$  psi (Deck)  
 $f_c = 1400$  psi (Sub. Curb. & Parapet)  
 $f_s = 20000$  psi (Rein.)  
 $f_s = 20000$  psi (Struct.)  
 $v_c = 75$  psi (Frgs)  
 $n = 10$  Allowable F/W 5.25" Soft  
Design Specifications 1969 AASHTO (as applicable) Allowable 4" Deflection  
LOADING HS 20-44 Composite 4:100

TOTAL BILL OF MATERIALS

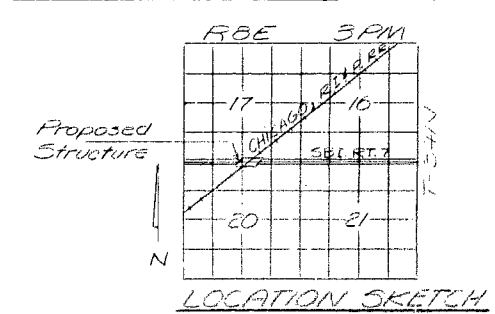
ITEM	UNIT	SUPER	SUB	TOTAL
Structure Excavation	Sq. Yds.		650	650
Protective Coat	Sq. Yds.			1330
Class X Concrete	Cu. Yds.	330.7	525.6	856.3
Structural Steel	Lbs.			1
Stud Shear Connectors	Ea.	2934		2934
Aluminum Foiling	Lin. Ft.	478		478
Reinforcement Bars	Lbs.	94540	52880	147420
Steel Piles 8 BP 36	Lin. Ft.		3200	3200
Crossed Piles 20.1 to 38"	Lin. Ft.		594	594
Test Piles Steel 8BP 36	Ea.		2	2
Name Plates	Ea.			1
Slope Walls 4"	Sq. Yds.		905	905
NEGLIGIBLE EXPANSION JOINT 2 1/2"	Lin. Ft.	145		145

Note  
For Sec. A-A & details of slope wall see Shs. # 2 & 3.



TRACKS - PROFILE

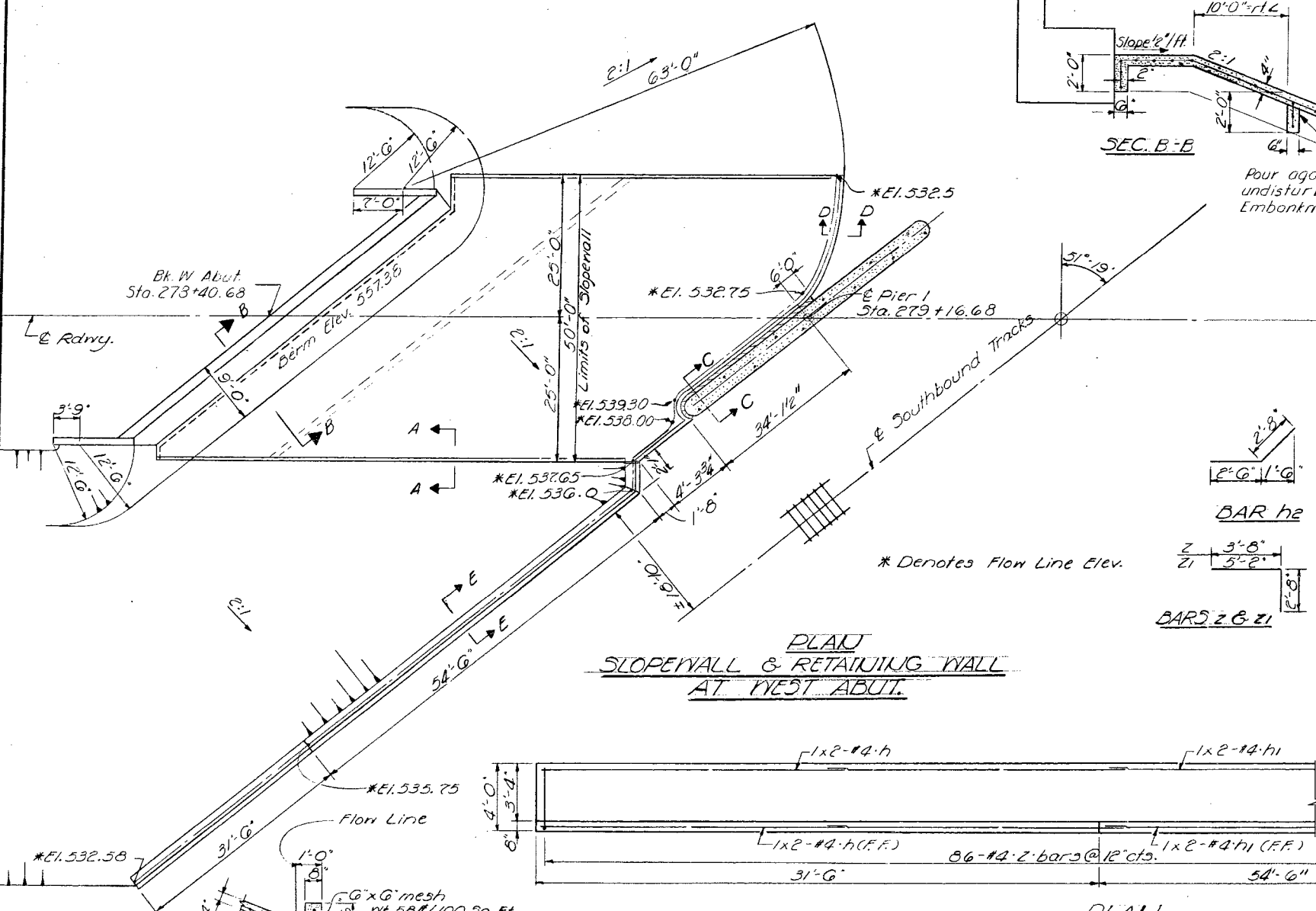
536.56	536.33	536.10	536.43	535.93	535.87	535.70	535.55	535.61
SW Ed. Track								
NE Ed. Track								



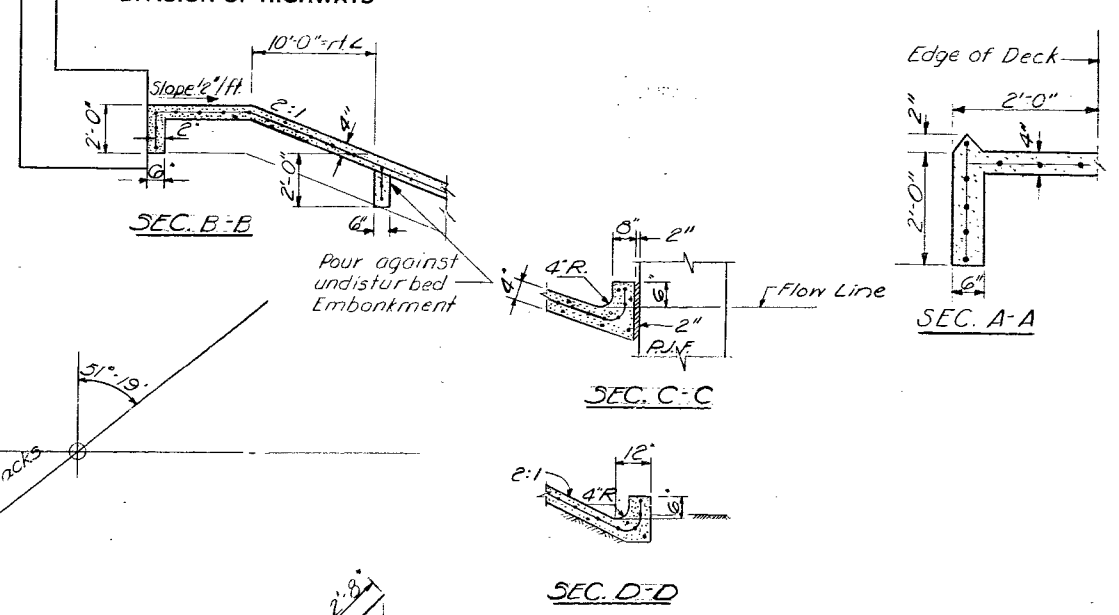
PROJ FG-10 (12)  
GENERAL PLAN AND ELEVATION  
BRIDGE OVER CHICAGO-ROCK ISLAND  
AND PACIFIC RAILROAD  
SBI RTE. 7 SEC. 6-VB  
GRUNDY COUNTY  
STA. 279+62.68

DESIGNED J.M. Patel  
CHECKED J. J. [Signature]  
DRAWN H.K.R.  
CHECKED S. J. [Signature]

EXAMINED [Signature] FEBRUARY 3 1971  
PASSED [Signature]  
APPROVED [Signature]  
ENGINEER OF DESIGN  
CHIEF HIGHWAY ENGINEER



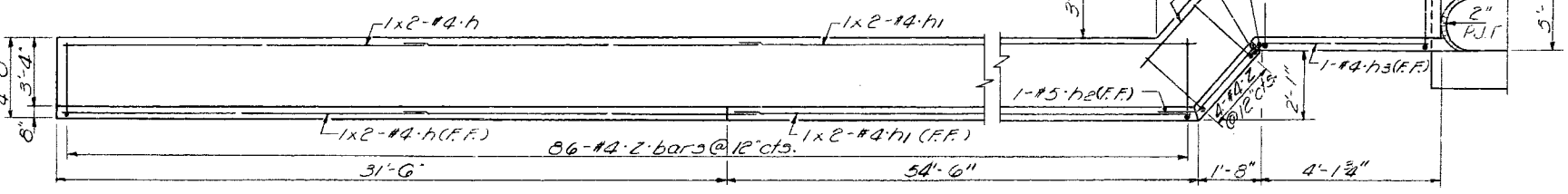
PLAN  
SLOPEWALL & RETAINING WALL  
AT WEST ABUT.



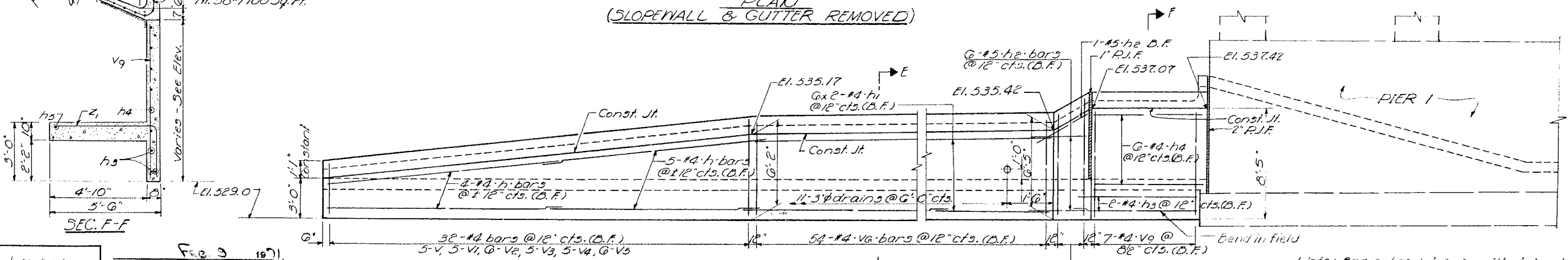
SEC. B-B  
SEC. A-A  
SEC. C-C  
SEC. D-D  
SEC. E-E

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h11	13	#4	16'-9"	—
h111	10	#4	27'-9"	—
h12	7	#5	5'-2"	—
h13	4	#4	5'-6"	—
h14	6	#4	4'-0"	—
h15	1	#4	6'-3"	—
v11	5	#4	3'-10"	—
v111	5	#4	4'-8"	—
v12	6	#4	4'-10"	—
v13	5	#4	5'-5"	—
v14	5	#4	5'-11"	—
v15	6	#4	6'-6"	—
v16	54	#4	7'-1"	—
v17	2	#4	7'-6"	—
v18	2	#4	8'-0"	—
v19	7	#4	7'-11"	—
z	93	#4	6'-4"	—
z1	6	#5	7'-10"	—
Class X Concrete		Cu. Yds.	26.7	
Reinforcement Bars		Lbs.	1330	



PLAN  
(SLOPEWALL & GUTTER REMOVED)



ELEVATION

Note: Bar subscript 'w' omitted in drawings.

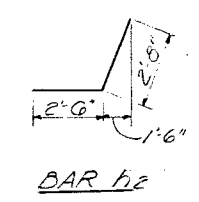
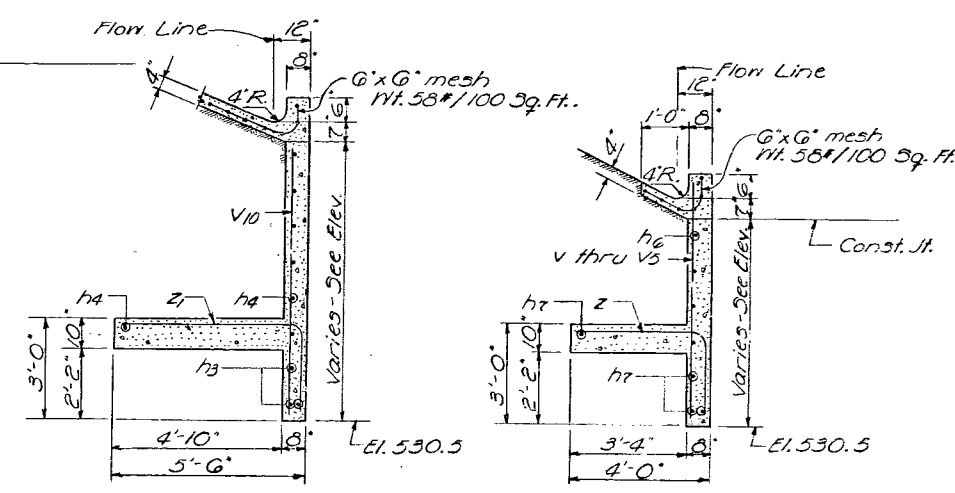
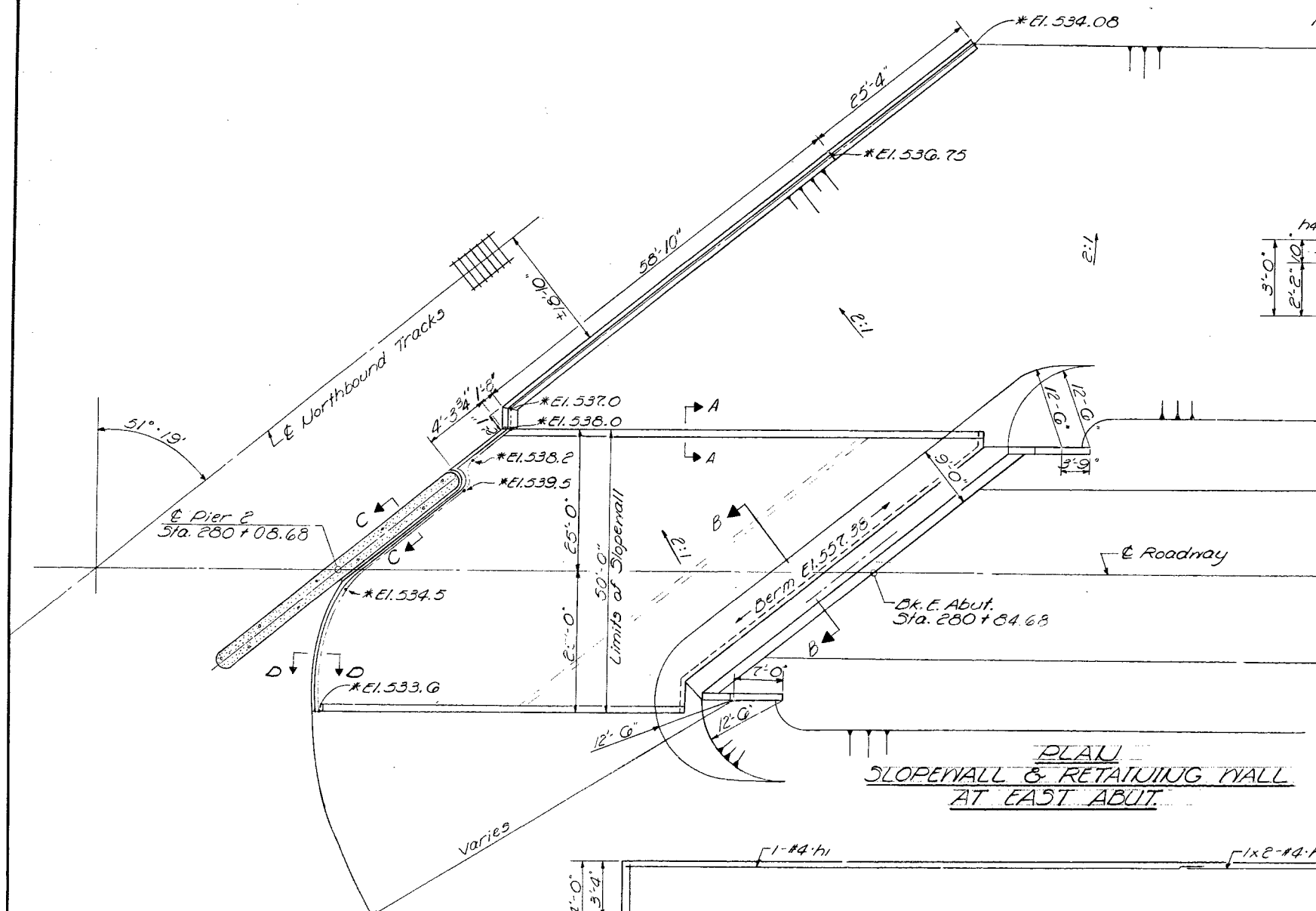
DESIGNED	J. M. Ruhl	EXAMINED	[Signature]
CHECKED	A. J. Schuyler	PASSED	[Signature]
DRAWN	Jacobs	APPROVED	[Signature]
CHECKED	G. V. K.		

WEST SLOPE WALL  
S.B.I. RT. 7 SEC. G-VB  
GRUNDY COUNTY  
STA. 279+62.68



STATE OF ILLINOIS  
DIVISION OF HIGHWAYS

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
8	G-VB	GRUNDY	35	11
SHEET NO. 3 15 SHEETS				

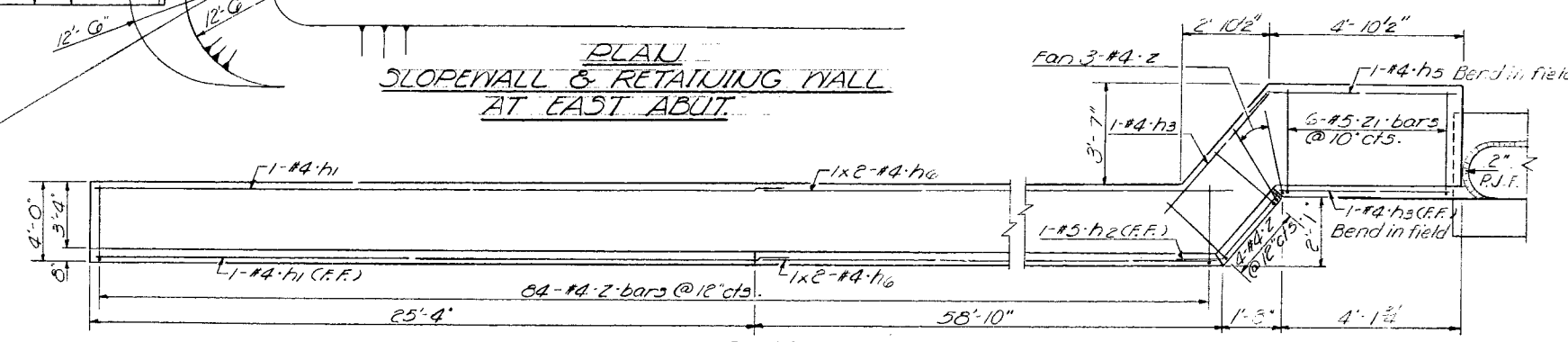


**BILL OF MATERIAL**

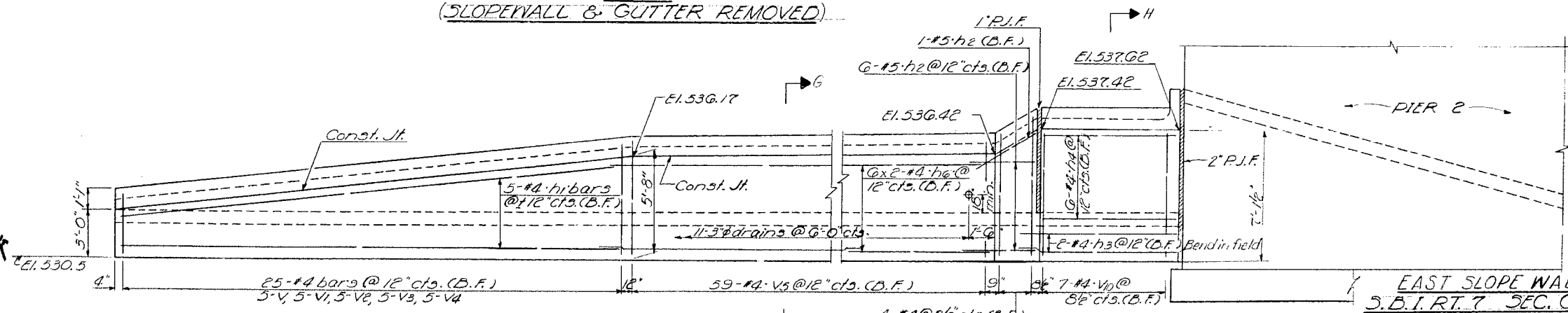
Bar	No.	Size	Length	Shape
h1a	7	#4	27'-9"	
h1b	4	#4	5'-6"	
h1c	6	#4	4'-0"	
h1d	1	#4	6'-3"	
h1e	16	#4	29'-3"	
h1f	7	#5	5'-2"	
v1	5	#4	3'-10"	
v1i	5	#4	4'-4"	
v1e	5	#4	4'-10"	
v1s	5	#4	5'-5"	
v1a	5	#4	5'-11"	
v1b	59	#4	6'-6"	
v1c	9	#4	6'-10"	
v1d	2	#4	7'-4"	
z	31	#4	6'-4"	
z1	6	#5	7'-10"	
Class X Concrete			Cu. Yds.	25.1
Reinforcement Bars			Lbs.	1347

PLAN  
SLOPEWALL & RETAINING WALL  
AT EAST ABUT.

PLAN  
(SLOPEWALL & GUTTER REMOVED)



Note: For Sections A-A, B-B, C-C & D-D See Sheet No 2  
Bar subscript 'i' omitted in drawings.

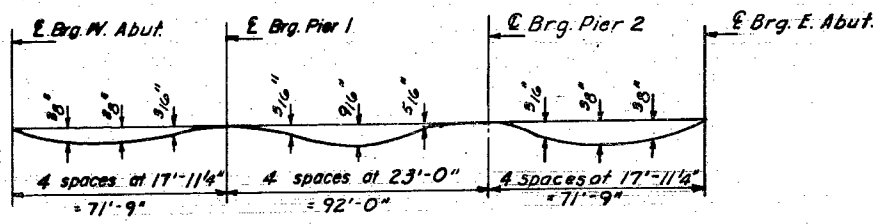


ELEVATION

EAST SLOPE WALL  
S.D.I.R.T. 7 SEC. G-VB  
GRUNDY COUNTY  
STA. 279 + 62.68

DESIGNED J. M. Patel  
CHECKED [Signature]  
DRAWN jacob  
CHECKED J11

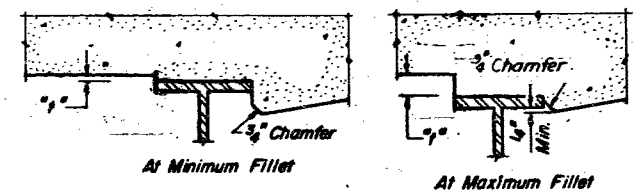
FEB 9 1971  
EXAMINED [Signature]  
PASSED [Signature]  
APPROVED [Signature]  
CHIEF HIGHWAY ENGINEER



**DEAD LOAD DEFLECTION DIAGRAM**

(Includes weight of concrete only)  
 Note: The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below.

STATE OF ILLINOIS  
 DIVISION OF HIGHWAYS



To determine "i": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below, minus slab thickness, equals the fillet heights "i" above top flange of beams.

PROJECT NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
8	G-VB	GRUNDY	35	12
ILLINOIS		FEB. AND PROJECT		

SHEET NO. 4  
15 SHEETS

**Girder 1**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	27865.659	-20.000	563.638	563.633
E Brg. W. Abut	27869.909	-20.000	563.649	563.649
A	27879.909	-20.000	563.683	563.700
B	27889.909	-20.000	563.713	563.744
C	27899.909	-20.000	563.739	563.772
D	27909.909	-20.000	563.762	563.791
E	27919.909	-20.000	563.781	563.799
F	27929.909	-20.000	563.795	563.804
G	27939.909	-20.000	563.806	563.807
E Brg. Pier 1	27941.659	-20.000	563.808	563.808
H	27951.659	-20.000	563.814	563.824
I	27961.659	-20.000	563.816	563.837
J	27971.659	-20.000	563.815	563.845
K	27981.659	-20.000	563.809	563.849
L	27991.659	-20.000	563.800	563.841
M	28001.659	-20.000	563.787	563.819
N	28011.659	-20.000	563.769	563.793
O	28021.659	-20.000	563.748	563.761
P	28031.659	-20.000	563.724	563.726
E Brg. Pier 2	28033.659	-20.000	563.718	563.718
Q	28043.659	-20.000	563.689	563.696
R	28053.659	-20.000	563.655	563.671
S	28063.659	-20.000	563.618	563.645
T	28073.659	-20.000	563.577	563.610
U	28083.659	-20.000	563.531	563.563
V	28093.659	-20.000	563.482	563.502
W	28103.659	-20.000	563.430	563.433
E Brg. E. Abut.	28105.409	-20.000	563.420	563.420
Bk. E. Abut.	28109.659	-20.000	563.396	563.396

**Girder 2**

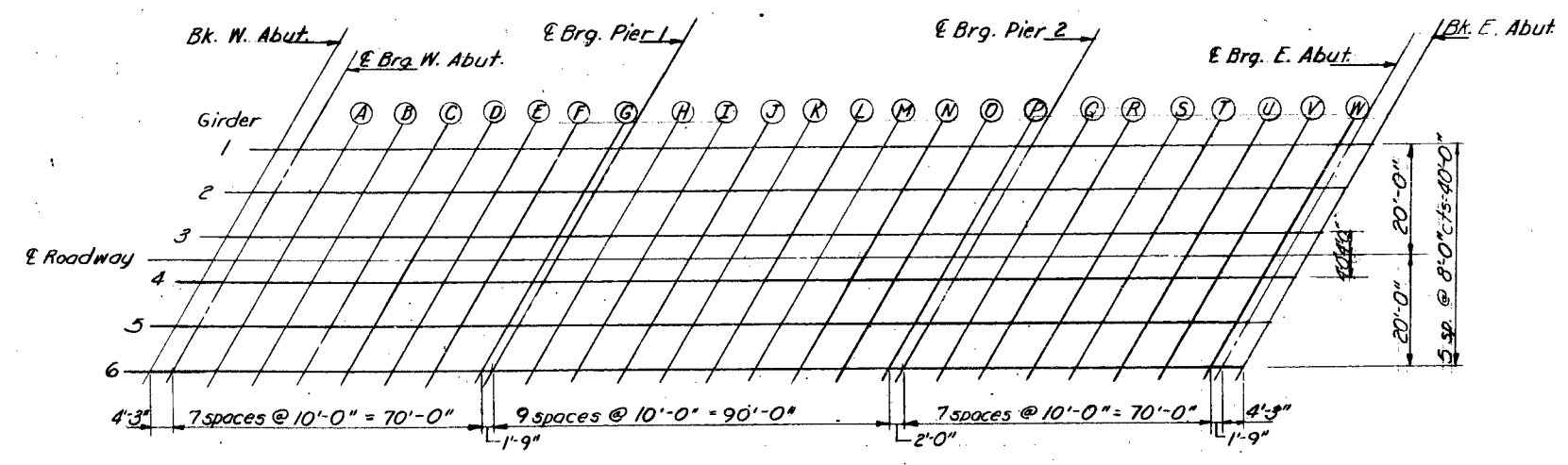
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	27855.667	-12.000	563.760	563.760
E Brg. W. Abut.	27859.917	-12.000	563.777	563.777
A	27869.917	-12.000	563.815	563.832
B	27879.917	-12.000	563.849	563.880
C	27889.917	-12.000	563.880	563.913
D	27899.917	-12.000	563.906	563.935
E	27909.917	-12.000	563.928	563.946
F	27919.917	-12.000	563.947	563.956
G	27929.917	-12.000	563.962	563.963
E Brg. Pier 1	27931.667	-12.000	563.964	563.964
H	27941.667	-12.000	563.974	563.984
I	27951.667	-12.000	563.980	564.001
J	27961.667	-12.000	563.982	564.013
K	27971.667	-12.000	563.981	564.020
L	27981.667	-12.000	563.975	564.017
M	27991.667	-12.000	563.966	563.998
N	28001.667	-12.000	563.953	563.976
O	28011.667	-12.000	563.936	563.948
P	28021.667	-12.000	563.915	563.917
E Brg. Pier 2	28023.667	-12.000	563.910	563.910
Q	28033.667	-12.000	563.885	563.892
R	28043.667	-12.000	563.855	563.871
S	28053.667	-12.000	563.822	563.849
T	28063.667	-12.000	563.784	563.817
U	28073.667	-12.000	563.743	563.774
V	28083.667	-12.000	563.698	563.718
W	28093.667	-12.000	563.649	563.652
E Brg. E. Abut.	28095.417	-12.000	563.640	563.640
Bk. E. Abut	28099.667	-12.000	563.618	563.618

**Girder 3**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	27845.676	-4.000	563.841	563.841
E Brg. W. Abut.	27849.926	-4.000	563.860	563.860
A	27859.926	-4.000	563.902	563.919
B	27869.926	-4.000	563.940	563.971
C	27879.926	-4.000	563.974	564.007
D	27889.926	-4.000	564.005	564.034
E	27899.926	-4.000	564.031	564.049
F	27909.926	-4.000	564.053	564.062
G	27919.926	-4.000	564.072	564.073
E Brg. Pier 1	27921.676	-4.000	564.075	564.075
H	27931.676	-4.000	564.089	564.099
I	27941.676	-4.000	564.099	564.120
J	27951.676	-4.000	564.105	564.136
K	27961.676	-4.000	564.107	564.147
L	27971.676	-4.000	564.106	564.147
M	27981.676	-4.000	564.100	564.133
N	27991.676	-4.000	564.091	564.114
O	28001.676	-4.000	564.078	564.091
P	28011.676	-4.000	564.061	564.063
E Brg. Pier 2	28013.676	-4.000	564.057	564.057
Q	28023.676	-4.000	564.035	564.043
R	28033.676	-4.000	564.009	564.026
S	28043.676	-4.000	563.980	564.007
T	28053.676	-4.000	563.946	563.980
U	28063.676	-4.000	563.909	563.940
V	28073.676	-4.000	563.868	563.888
W	28083.676	-4.000	563.823	563.826
E Brg. E. Abut.	28085.426	-4.000	563.815	563.815
Bk. E. Abut.	28089.676	-4.000	563.794	563.794

**E Roadway**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	27840.680	0.0	563.881	563.881
E Brg. W. Abut	27844.930	0.0	563.900	563.900
A	27854.930	0.0	563.944	563.961
B	27864.930	0.0	563.984	564.015
C	27874.930	0.0	564.020	564.053
D	27884.930	0.0	564.052	564.082
E	27894.930	0.0	564.081	564.099
F	27904.930	0.0	564.105	564.114
G	27914.930	0.0	564.126	564.127
E Brg. Pier 1	27916.680	0.0	564.129	564.129
H	27926.680	0.0	564.145	564.155
I	27936.680	0.0	564.157	564.178
J	27946.680	0.0	564.165	564.196
K	27956.680	0.0	564.169	564.209
L	27966.680	0.0	564.170	564.211
M	27976.680	0.0	564.166	564.199
N	27986.680	0.0	564.159	564.182
O	27996.680	0.0	564.148	564.160
P	28006.680	0.0	564.132	564.134
E Brg. Pier 2	28008.680	0.0	564.125	564.129
Q	28018.680	0.0	564.109	564.117
R	28028.680	0.0	564.085	564.101
S	28038.680	0.0	564.058	564.085
T	28048.680	0.0	564.026	564.059
U	28058.680	0.0	563.991	564.027
V	28068.680	0.0	563.952	563.971
W	28078.680	0.0	563.908	563.911
E Brg. E. Abut.	28080.430	0.0	563.900	563.900
Bk. E. Abut.	28084.680	0.0	563.881	563.881



DESIGNED	J. M. [Signature]	EXAMINED	[Signature]
CHECKED	A. T. [Signature]	PASSED	[Signature]
DRAWN	P. G. Barnett	APPROVED	[Signature]
CHECKED	B. F. K.	CHIEF HIGHWAY ENGINEER	[Signature]

FEB 9 1971

PLAN

DECK ELEVATIONS  
 S.B.I. RT. 7 SEC. 6-VB  
 GRUNDY COUNTY  
 STA. 279+62.68

STATE OF ILLINOIS  
DIVISION OF HIGHWAYS

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 5 15 SHEETS
P. A. 8	G-VB	GRUNDY	35	13	
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT			

Girder 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	27835.684	4.000	563.794	563.794
E Brg. W. Abut.	27839.934	4.000	563.815	563.815
A	27849.934	4.000	563.860	563.877
B	27859.934	4.000	563.902	563.933
C	27869.934	4.000	563.940	563.973
D	27879.934	4.000	563.974	564.004
E	27889.934	4.000	564.005	564.023
F	27899.934	4.000	564.031	564.040
G	27909.934	4.000	564.053	564.055
E Brg. Pier 1	27911.684	4.000	564.057	564.057
H	27921.684	4.000	564.075	564.085
I	27931.684	4.000	564.089	564.110
J	27941.684	4.000	564.099	564.129
K	27951.684	4.000	564.105	564.145
L	27961.684	4.000	564.107	564.149
M	27971.684	4.000	564.106	564.138
N	27981.684	4.000	564.100	564.124
O	27991.684	4.000	564.091	564.104
P	28001.684	4.000	564.078	564.080
E Brg. Pier 2	28003.684	4.000	564.075	564.075
Q	28013.684	4.000	564.057	564.065
R	28023.684	4.000	564.035	564.051
S	28033.684	4.000	564.009	564.037
T	28043.684	4.000	563.980	564.013
U	28053.684	4.000	563.946	563.978
V	28063.684	4.000	563.909	563.929
W	28073.684	4.000	563.868	563.871
E Brg. E. Abut.	28075.434	4.000	563.860	563.860
Bk. E. Abut.	28079.684	4.000	563.841	563.841

Girder 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	27825.693	12.000	563.618	563.618
E Brg. W. Abut.	27829.943	12.000	563.640	563.640
A	27839.943	12.000	563.690	563.707
B	27849.943	12.000	563.735	563.766
C	27859.943	12.000	563.777	563.810
D	27869.943	12.000	563.815	563.845
E	27879.943	12.000	563.849	563.867
F	27889.943	12.000	563.880	563.889
G	27899.943	12.000	563.906	563.907
E Brg. Pier 1	27901.693	12.000	563.910	563.910
H	27911.693	12.000	563.932	563.942
I	27921.693	12.000	563.950	563.971
J	27931.693	12.000	563.964	563.994
K	27941.693	12.000	563.974	564.014
L	27951.693	12.000	563.980	564.022
M	27961.693	12.000	563.982	564.015
N	27971.693	12.000	563.981	564.004
O	27981.693	12.000	563.975	563.988
P	27991.693	12.000	563.966	563.968
E Brg. Pier 2	27993.693	12.000	563.964	563.964
Q	28003.693	12.000	563.950	563.957
R	28013.693	12.000	563.932	563.948
S	28023.693	12.000	563.910	563.938
T	28033.693	12.000	563.884	563.918
U	28043.693	12.000	563.855	563.886
V	28053.693	12.000	563.821	563.861
W	28063.693	12.000	563.784	563.787
E Brg. E. Abut.	28065.443	12.000	563.777	563.777
Bk. E. Abut.	28069.693	12.000	563.760	563.760

Girder 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	27815.701	20.000	563.396	563.396
E Brg. W. Abut.	27819.951	20.000	563.420	563.420
A	27829.951	20.000	563.474	563.491
B	27839.951	20.000	563.523	563.554
C	27849.951	20.000	563.569	563.602
D	27859.951	20.000	563.611	563.640
E	27869.951	20.000	563.649	563.667
F	27879.951	20.000	563.683	563.692
G	27889.951	20.000	563.713	563.715
E Brg. Pier 1	27891.701	20.000	563.718	563.718
H	27901.701	20.000	563.744	563.754
I	27911.701	20.000	563.766	563.787
J	27921.701	20.000	563.783	563.814
K	27931.701	20.000	563.797	563.837
L	27941.701	20.000	563.808	563.849
M	27951.701	20.000	563.814	563.846
N	27961.701	20.000	563.816	563.839
O	27971.701	20.000	563.815	563.827
P	27981.701	20.000	563.809	563.811
E Brg. Pier 2	27983.701	20.000	563.808	563.808
Q	27993.701	20.000	563.797	563.805
R	28003.701	20.000	563.783	563.799
S	28013.701	20.000	563.765	563.793
T	28023.701	20.000	563.744	563.777
U	28033.701	20.000	563.718	563.749
V	28043.701	20.000	563.688	563.708
W	28053.701	20.000	563.655	563.658
E Brg. E. Abut.	28055.451	20.000	563.649	563.649
E Brg. E. Abut.	28059.701	20.000	563.633	563.633

DESIGNED	J. M. Miller
CHECKED	A. J. Robinson
DRAWN	Bev Robinson
CHECKED	J. K.

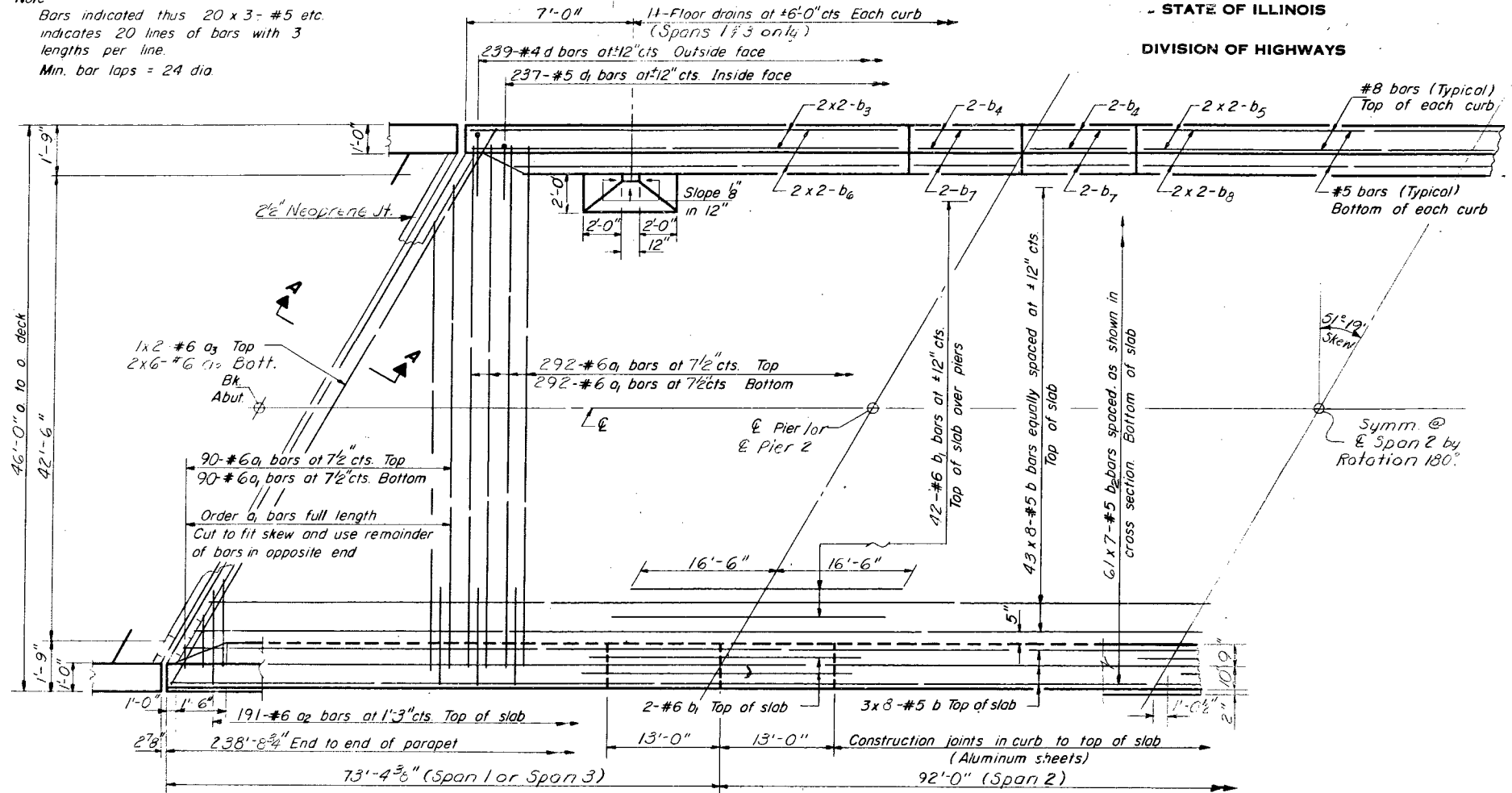
EXAMINED	[Signature]	1971
PASSED	[Signature]	
APPROVED	[Signature]	

DECK ELEVATIONS  
S.B.I. RT. 7 SEC. G-VB  
GRUNDY COUNTY  
STA. 279+62.68

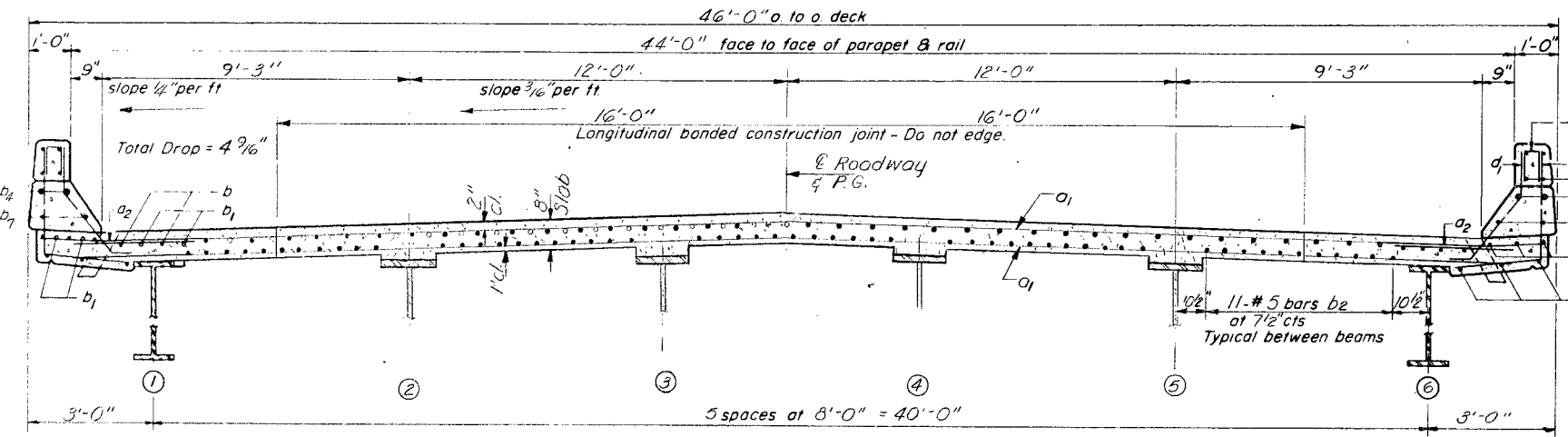
Note  
 Bars indicated thus 20 x 3 - #5 etc.  
 indicates 20 lines of bars with 3  
 lengths per line.  
 Min. bar laps = 24 dia

STATE OF ILLINOIS  
 DIVISION OF HIGHWAYS

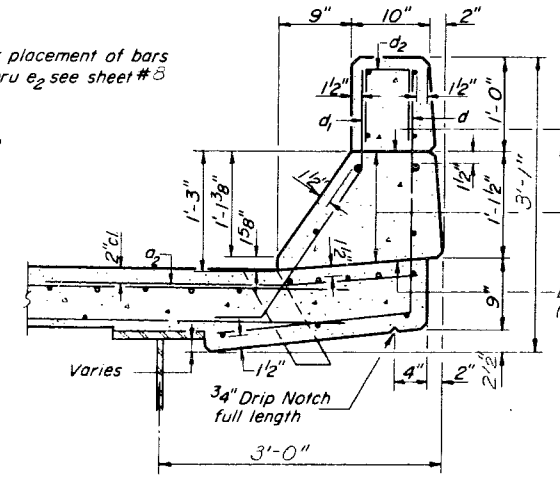
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO.
8	G-VB	GRUNDY	35	14	15 SHEETS



HALF PLAN



CROSS SECTION  
 LOOKING EAST



CURB SECTION

BILL OF MATERIAL

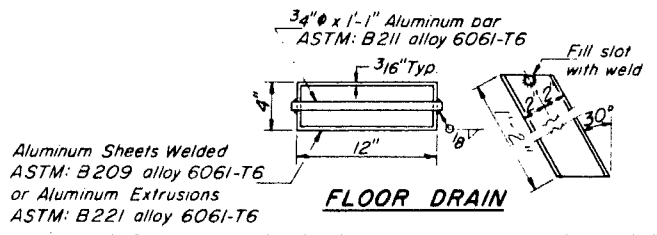
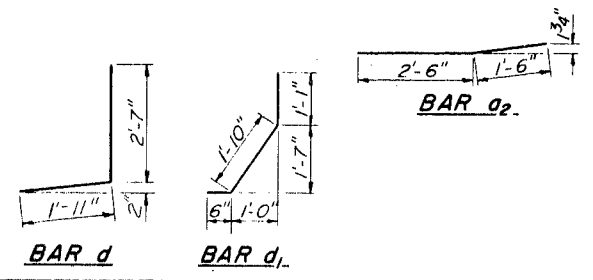
Bar	No	Size	Length	Shape
a <sub>1</sub>	764	#6	44'-0"	—
a <sub>2</sub>	382	#6	4'-0"	—
a <sub>3</sub>	12	#6	37'-0"	—
b	392	#5	31'-0"	—
b <sub>1</sub>	92	#6	33'-0"	—
b <sub>2</sub>	427	#5	35'-3"	—
b <sub>3</sub>	16	#8	31'-0"	—
b <sub>4</sub>	16	#8	12'-9"	—
b <sub>5</sub>	8	#8	34'-0"	—
b <sub>6</sub>	16	#5	30'-9"	—
b <sub>7</sub>	16	#5	12'-9"	—
b <sub>8</sub>	8	#5	33'-9"	—
d	478	#4	4'-6"	J
d <sub>1</sub>	474	#5	3'-5"	J
Reinforcement Bars				Lbs 93,200
Class X Concrete				Cu Yds 35.3

Proper reinforcement and Class X Concrete are relied on for strength.

DESIGNED J. M. Patel  
 CHECKED A. S. ...  
 DRAWN ...  
 CHECKED ...

EXAMINED ...  
 PASSED ...  
 APPROVED ...

I-4 to 6-L (>30°) 3-1-68



Aluminum Sheets Welded  
 ASTM: B209 alloy 6061-T6  
 or Aluminum Extrusions  
 ASTM: B221 alloy 6061-T6

FLOOR DRAIN

Note For placement of bars  
 d<sub>2</sub> & e thru e<sub>2</sub> see sheet #5

Bonded Construction Jt.  
 (Optional)  
 1/8\"/>

Bonded Construction Jt.  
 (Mandatory)

Cost of Aluminum Drains and Sheets  
 shall be incidental to Class X Concrete

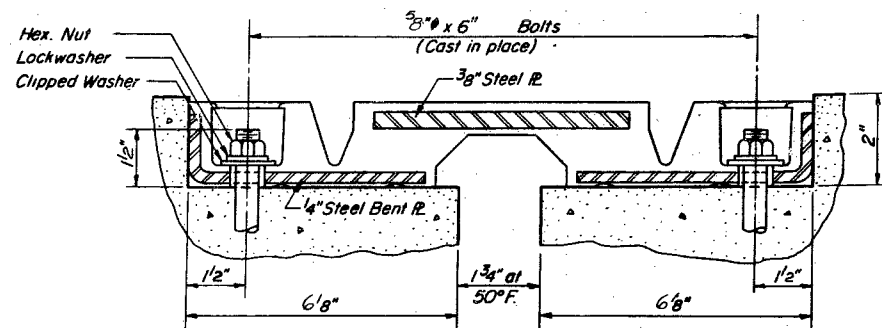
SUBSTRUCTURE  
 S.B.I. RT. 7 SEC. 6-1E  
 GRUNDY COUNTY  
 STA. 279+62.68



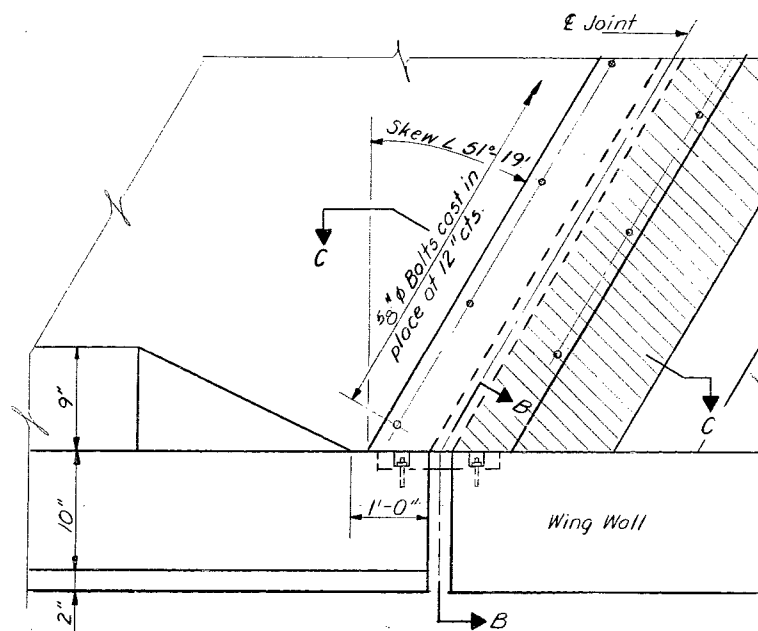
STATE OF ILLINOIS  
DIVISION OF HIGHWAYS

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
8	G-VB	GRUNDY	35	15
F.A. 8		ILLINOIS		FED. AID PROJECT

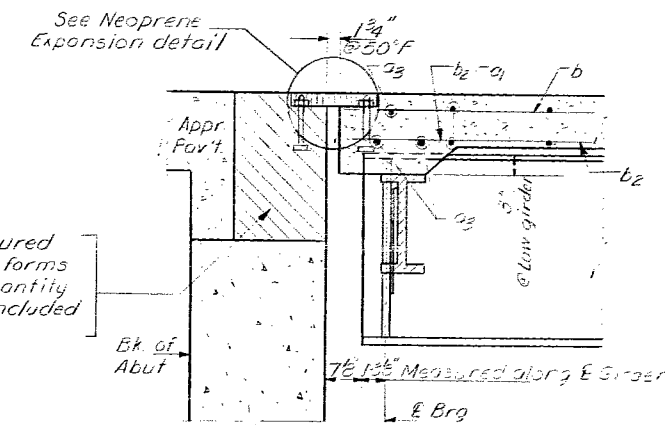
SHEET NO. 7  
15 SHEETS



**NEOPRENE EXPANSION JOINT (2 1/2")**  
See Special Provisions

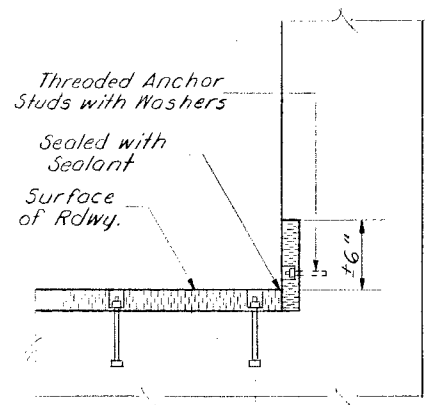


**PLAN**

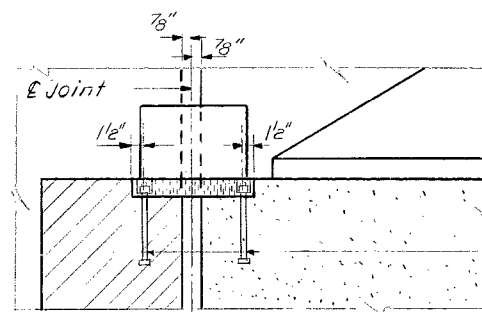


**SECTION A-A**

Hatched area to be poured after superstructure forms have been removed. Quantity of Class X Concrete included with superstructure.



**SEC. B-B**



**SEC. C-C**

5/8" x 6" bolts.  
To be cast into concrete

**INTERIOR GIRDER MOMENT TABLE**

	0.4 Sp. lor 3	Pier lor 2	0.5 Sp 2
$I_s$ (in <sup>4</sup> )	12584.8	21797.8	12584.8
$I_c$ (in <sup>4</sup> )	31812.7		31812.7
$S_s$ (in <sup>3</sup> )	622.3	979.7	622.3
$S_c$ (in <sup>3</sup> )	862.5		862.5
$e$ (ft)	0.988	1.310	0.988
$M_e$ (k)	304.52	957.55	297.06
$f_s @$ (ksi)	5.87	11.73	5.73
$S_e$ (k/ft)	0.306		0.306
$MS_e$ (k)	111.96		131.04
$M_e$ (k)	609.75	498.98	678.02
$M_{Imp}$ (k)	154.88	120.75	155.94
Total (k)	876.59	619.73	965.00
$f_s (+S_e)$ (ksi)	12.20	7.59	13.43
$f_s$ total (ksi)	18.07	19.32	19.16
$VR$ (k)	61.57		61.57

**INTERIOR GIRDER REACTION TABLE**

	ABUT. E & W	PIER 1 & 2
$R_e$ (k)	33.08	120.02
$R_e$ (k)	44.22	63.36
Imp (k)	11.22	15.33
$R$ Total (k)	88.53	198.71

Note: Composite in positive moment areas only.

DESIGNED	J.M. Patel	EXAMINED	[Signature]
CHECKED	A.J. Wharff	PASSED	[Signature]
DRAWN	BKR	APPROVED	[Signature]
CHECKED	[Signature]	CHIEF HIGHWAY ENGINEER	

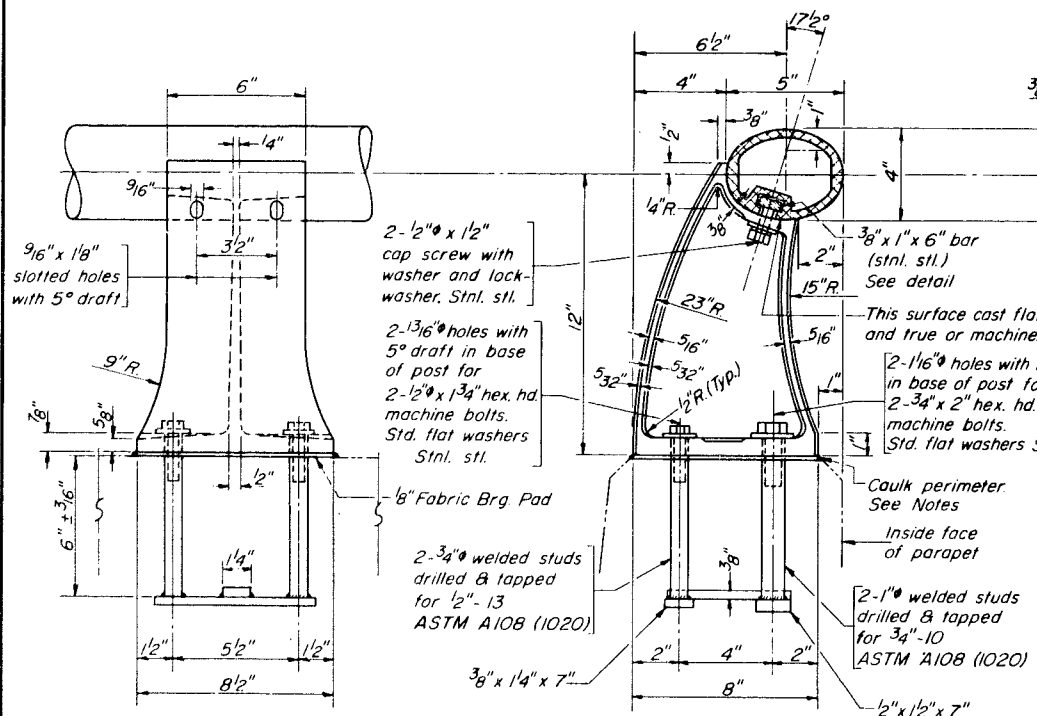
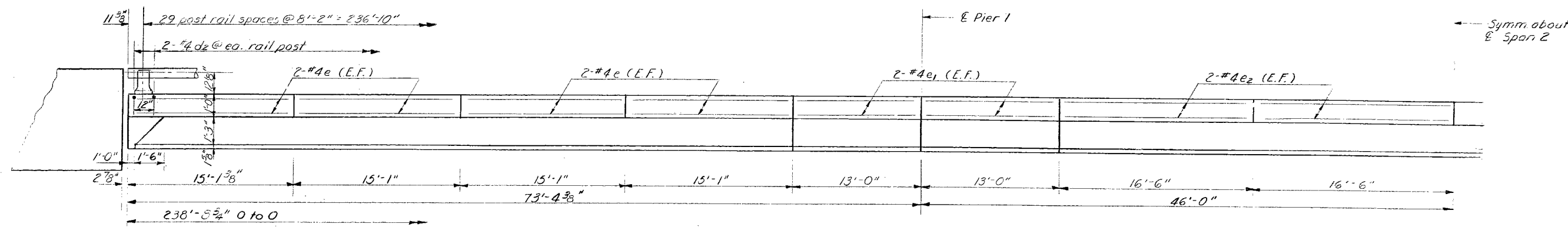
FEB 9 1971

**SUPERSTRUCTURE DETAILS**  
S.B.I. RT 7 SEC. G-VB  
GRUNDY COUNTY  
STA 279+62.68

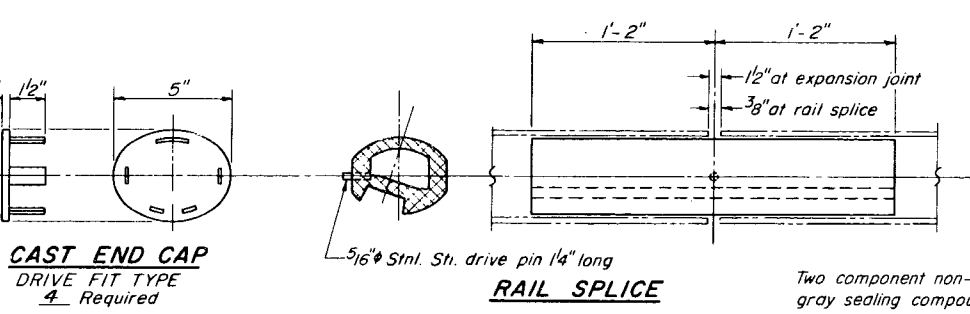
STATE OF ILLINOIS  
DIVISION OF HIGHWAYS

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
8	G-VB	GRUNDY	35	16
FED. ROAD DIST. NO. 1		ILLINOIS	FED. AID PROJECT	

SHEET NO. 8  
15 SHEETS

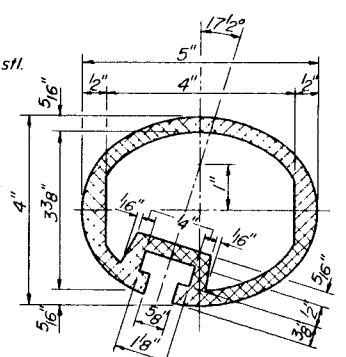


RAIL POST DETAILS

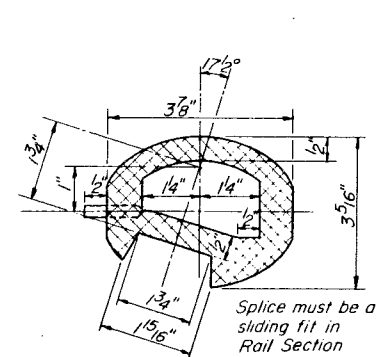


CAST END CAP  
DRIVE FIT TYPE  
4 Required

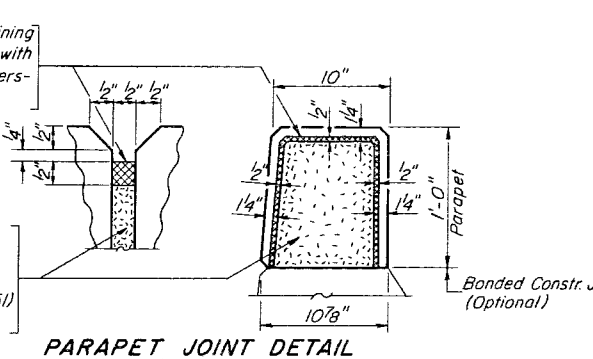
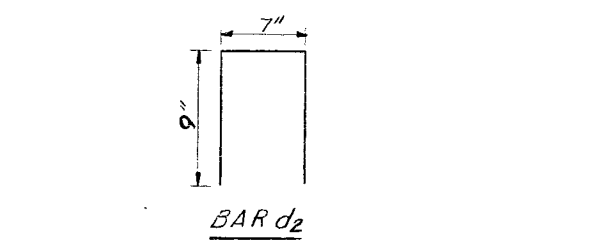
RAIL SPLICE



SEC. THRU ELLIPTICAL  
RAIL SECTION



SEC. THRU SPLICE



PARAPET JOINT DETAIL

**NOTES:**  
All Aluminum Alloy Extruded Rail shall be supplied in modular lengths of 30 feet, except at the end of bridge or over open joints in bridge deck where the rail shall be attached to a minimum of 2 posts. If the rail is on a horizontal curve of 2300 foot radius or less, the modular lengths may be reduced but shall be attached to a minimum of 2 posts.  
All joints in rail shall be spliced per detail.  
Provide 1-1/8" and 2-1/16" Aluminum Shims for 25% of the Posts. Rail element shall be parallel to Grade - high spots shall be ground and low spots shimmed.  
Seal perimeter of base of post to parapet with two component non-staining gray sealing compound with polysulfide liquid polymers, gun grade with primer. Fabric Bearing Pad shall have same dimensions as base of post.  
Aluminum alloy rail shall conform to ASTM B221 alloy 6061-T6 or 6351-T5 with min. yield 35 ksi, min tensile 38 ksi, and elongation of 10% in 2 inches.

PARAPETS & RAILS  
BILL OF MATERIAL

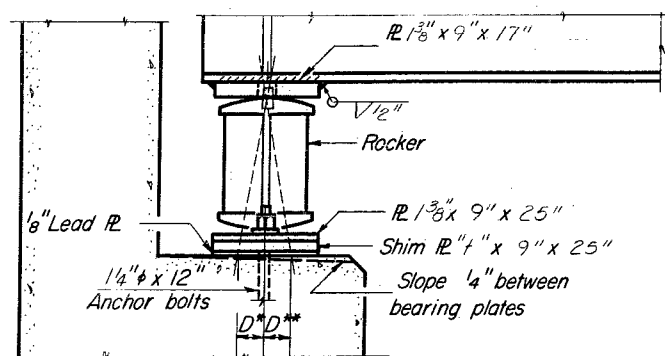
Bar	No.	Size	Length	Shape
d <sub>2</sub>	120	#4	.2'-1"	□
e	64	#4	14'-10"	—
e <sub>1</sub>	32	#4	12'-9"	—
e <sub>2</sub>	32	#4	16'-3"	—
Reinforcement Bars				Lbs. 1420
Class X Concrete				Cu. Yds. 15.4
Aluminum Railing				Lin. Ft. 473

ALUMINUM RAILING  
SBI RT 7 SEC. G-VB  
GRUNDY COUNTY  
STA. 279+62.68

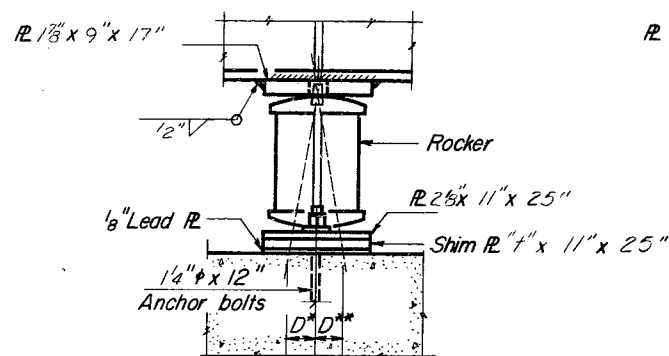
DESIGNED	J. M. Patel
CHECKED	A. V. Whang
DRAWN	Bob Robinson
CHECKED	S. V. K.
EXAMINED	<i>[Signature]</i>
PASSED	
APPROVED	

R-17 4-22-68 9-18-69

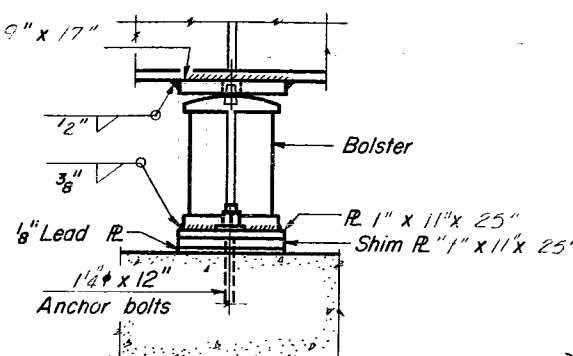




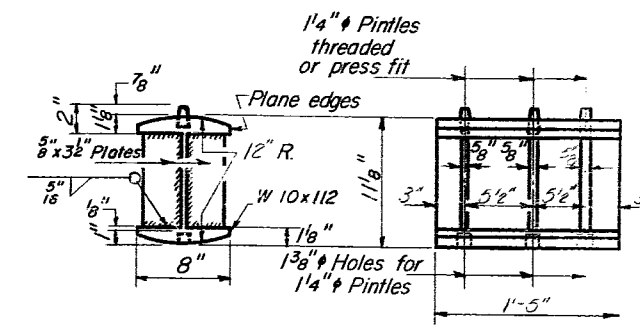
SECTION



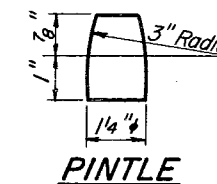
ELEVATION



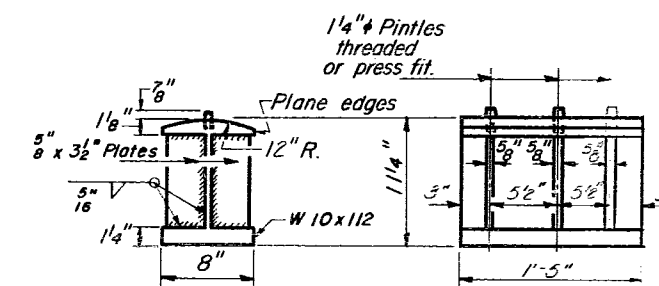
ELEVATION



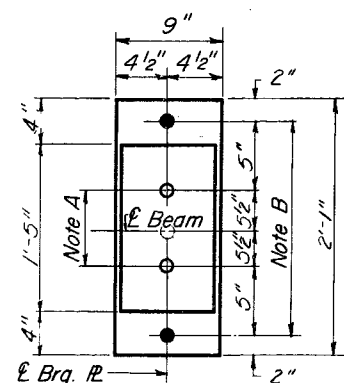
ROCKER



PINTLE

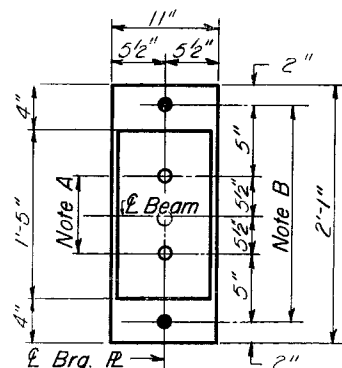


BOLSTER



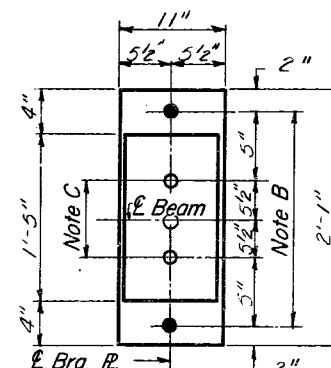
PLAN

AT ABUTMENT



PLAN

AT PIER 2



PLAN

AT PIER 1

**NOTE A**  
1 3/8" Holes - 1" deep in top R.  
for pintles. Thread or press fit  
pintles into bottom R.

**NOTE B**  
1 3/8" Holes for 1 1/4" anchor bolts.  
1/2 x 2 1/2 x 2 1/2 R. Washers  
under nut.

**NOTE C**  
1 3/8" Holes 1" deep in top R.  
only for 1 1/4" pintles.

**NOTES ON SETTING OF ANCHOR BOLTS AT EXP. BRGS.**

- a) D\* (Side of brg. away from fixed brg.)  
D\* = 1/8" per each 100' of expansion for every 15° fall below the normal temp. of 50°F.
- D\*\* (Side of brg. toward fixed brg.)  
D\*\* = 1/8" per each 100' of expansion for every 15° rise above the normal temp. of 50°F.

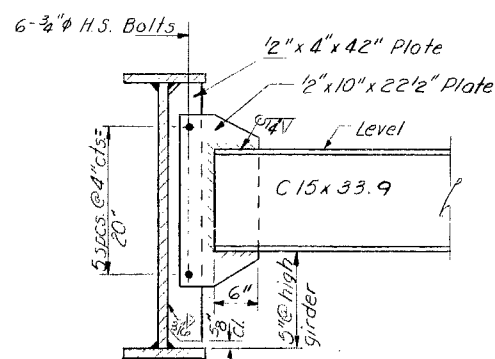
**BEARING ASSEMBLY DETAILS**

\* TOP OF WEB ELEVATIONS

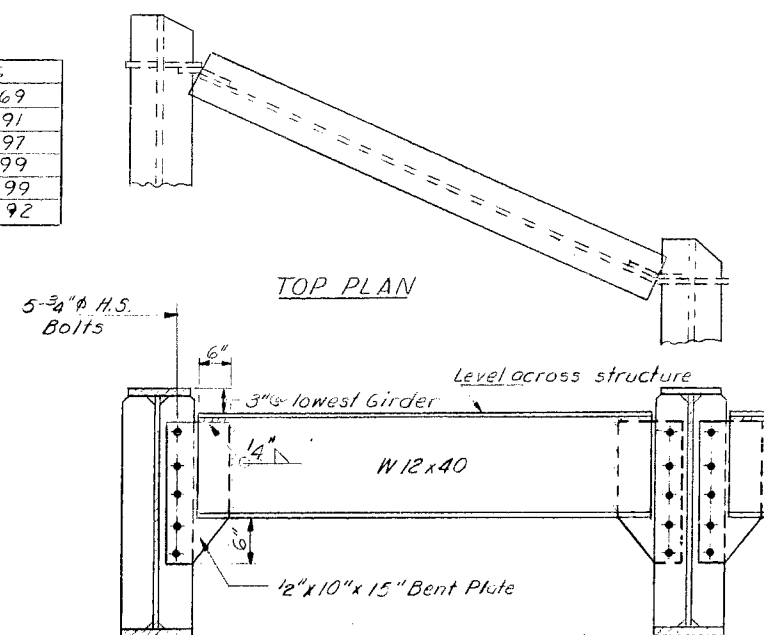
	G-1	G-2	G-3	G-4	G-5	G-6
C Brg. W. Abut.	562.92	563.05	563.13	563.09	562.91	562.69
E Brg. Pier 1	563.00	563.16	563.27	563.26	563.11	562.91
E Splice 1	563.02	563.19	563.31	563.30	563.16	562.97
E Brg. Pier 2	562.89	563.09	563.24	563.26	563.14	562.99
E Splice 2	562.87	563.07	563.22	563.25	563.14	562.99
E Brg. E. Abut.	562.69	562.91	563.09	563.13	563.05	562.92

\* For fabrication only

- b) After beams have been erected and dimensions D\* or D\*\* determined, holes shall be drilled and anchor bolts shall be grouted in place. All fixed anchor bolts may be built into the masonry.



DIAPHRAGM D1  
(45 Req'd)



DIAPHRAGM D  
(10 Req'd)

1" thickness in inches

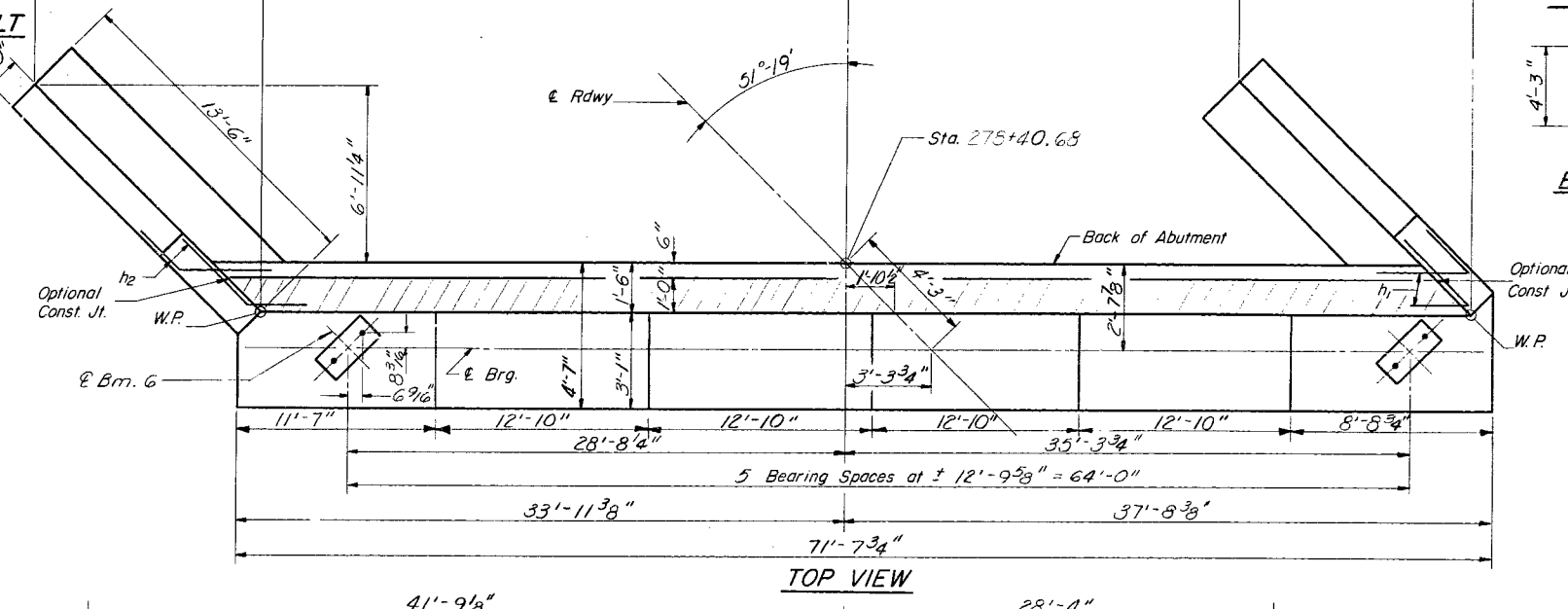
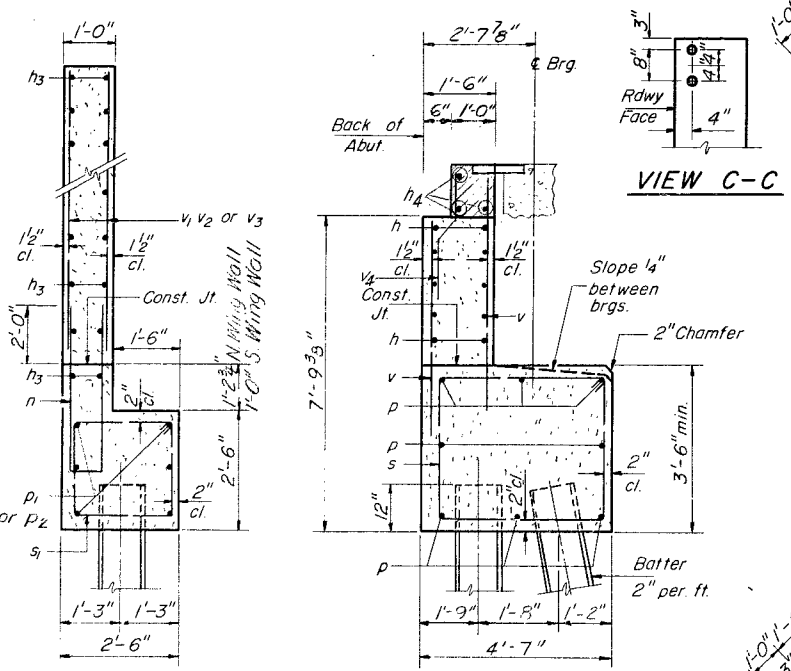
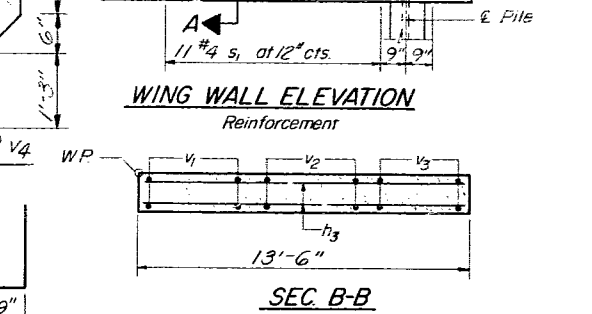
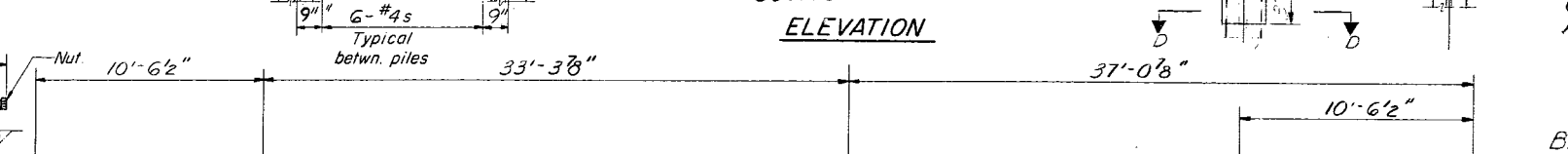
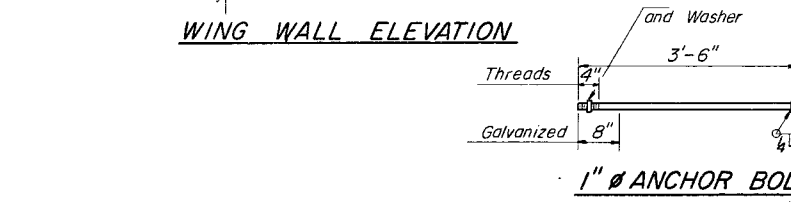
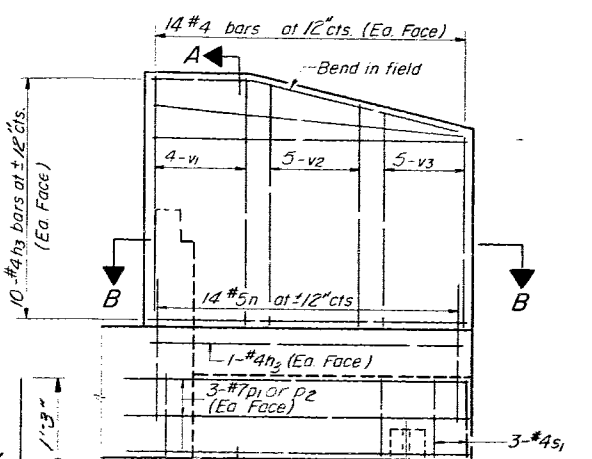
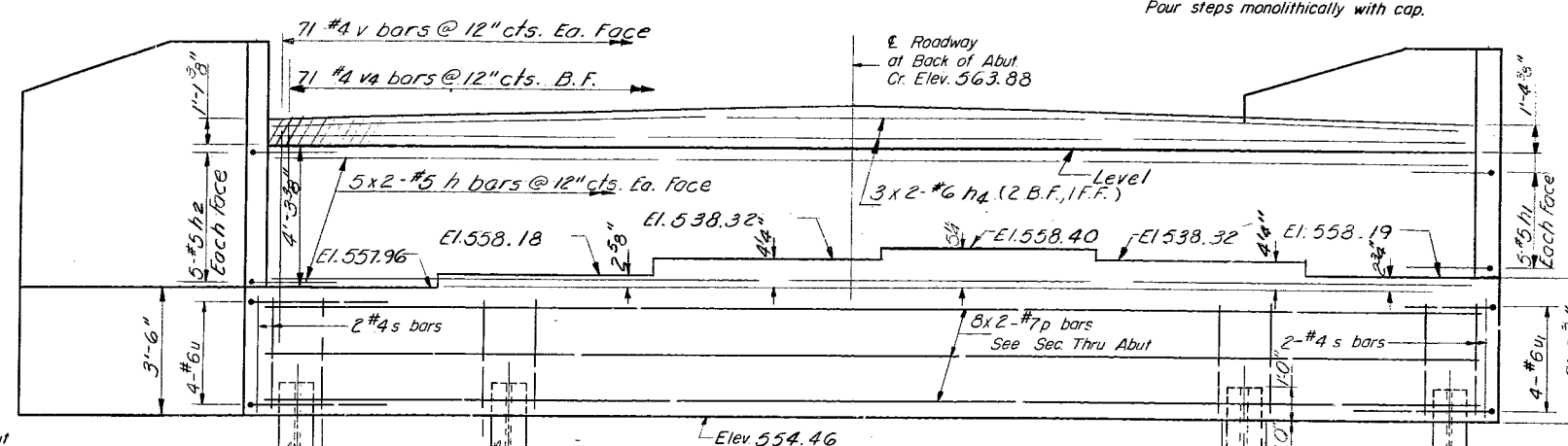
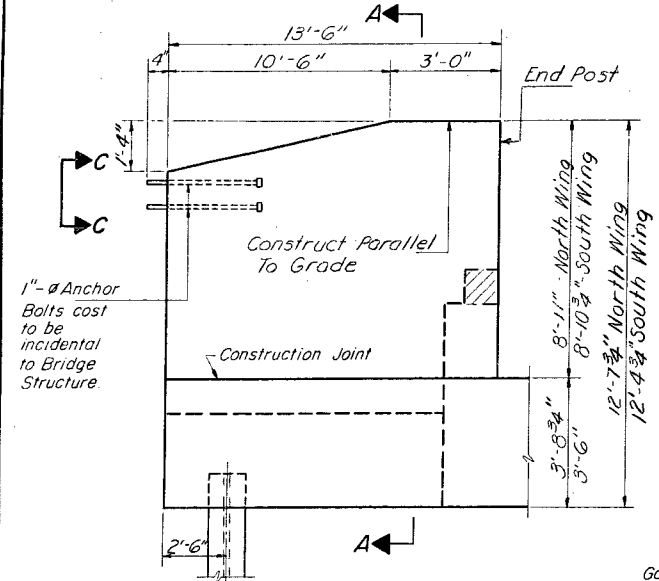
	G-1	G-2	G-3	G-4	G-5	G-6
West Abut				1/2		
Pier 1			1/8			
Pier 2				1/4		
East Abut			1/2			

DESIGNED	J. M. Patel	EXAMINED	[Signature]
CHECKED	A. J. White, Jr.	PASSED	[Signature]
DRAWN	P. G. Barnett	APPROVED	[Signature]
CHECKED	S. V. K.		

BEARING & DIAPHRAGM DETAILS  
S.B.I. RT 7 SEC. G-VB  
GRUNDY COUNTY  
STA. 279+62.68

Hatched area to be poured after forms of superstructure have been removed.

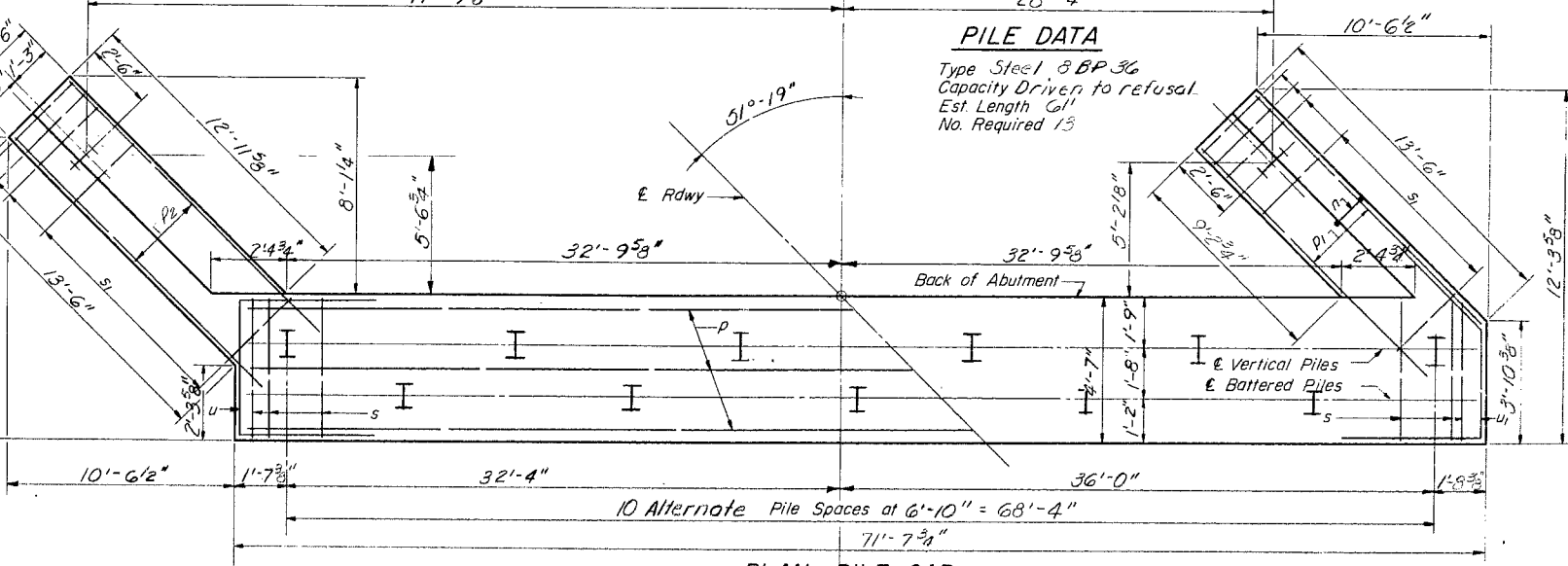
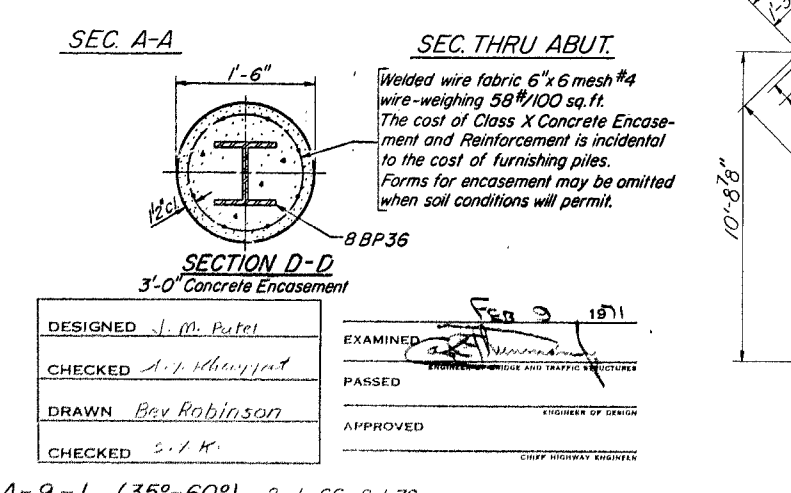
Space reinforcement in cap to miss anchor bolts.  
Pour steps monolithically with cap.



**ONE ABUTMENT  
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h	20	#5	35'-9"	U
h1	10	#5	7'-3"	U
h2	10	#5	7'-3"	U
h3	44	#4	13'-3"	U
h4	6	#6	36'-0"	U
n	28	#5	9'-3"	U
p	16	#7	36'-6"	U
p1	6	#7	13'-3"	U
p2	6	#7	14'-6"	U
s	64	#4	15'-7"	U
s1	28	#4	9'-5"	U
u	4	#6	11'-0"	U
u1	4	#6	10'-0"	U
v	142	#4	7'-0"	U
v1	16	#4	8'-9"	U
v2	20	#4	8'-0"	U
v3	20	#4	7'-3"	U
v4	71	#4	3'-3"	U

Class X Concrete Cu. Yds. 76.4  
Reinforcement Bars Lbs. 5500  
Steel Piles 8BP36 Lin. Ft. 793



**PILE DATA**  
Type Steel 8BP36  
Capacity Driven to refusal.  
Est. Length 61'  
No. Required 13

DESIGNED J. M. Patel  
CHECKED A. J. Whynjard  
DRAWN Bev Robinson  
CHECKED S. J. K.

EXAMINED *[Signature]*  
PASSED  
APPROVED *[Signature]*  
CHIEF HIGHWAY ENGINEER

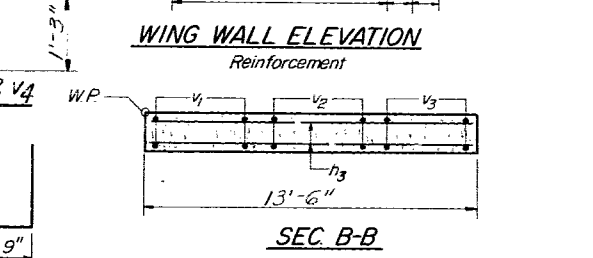
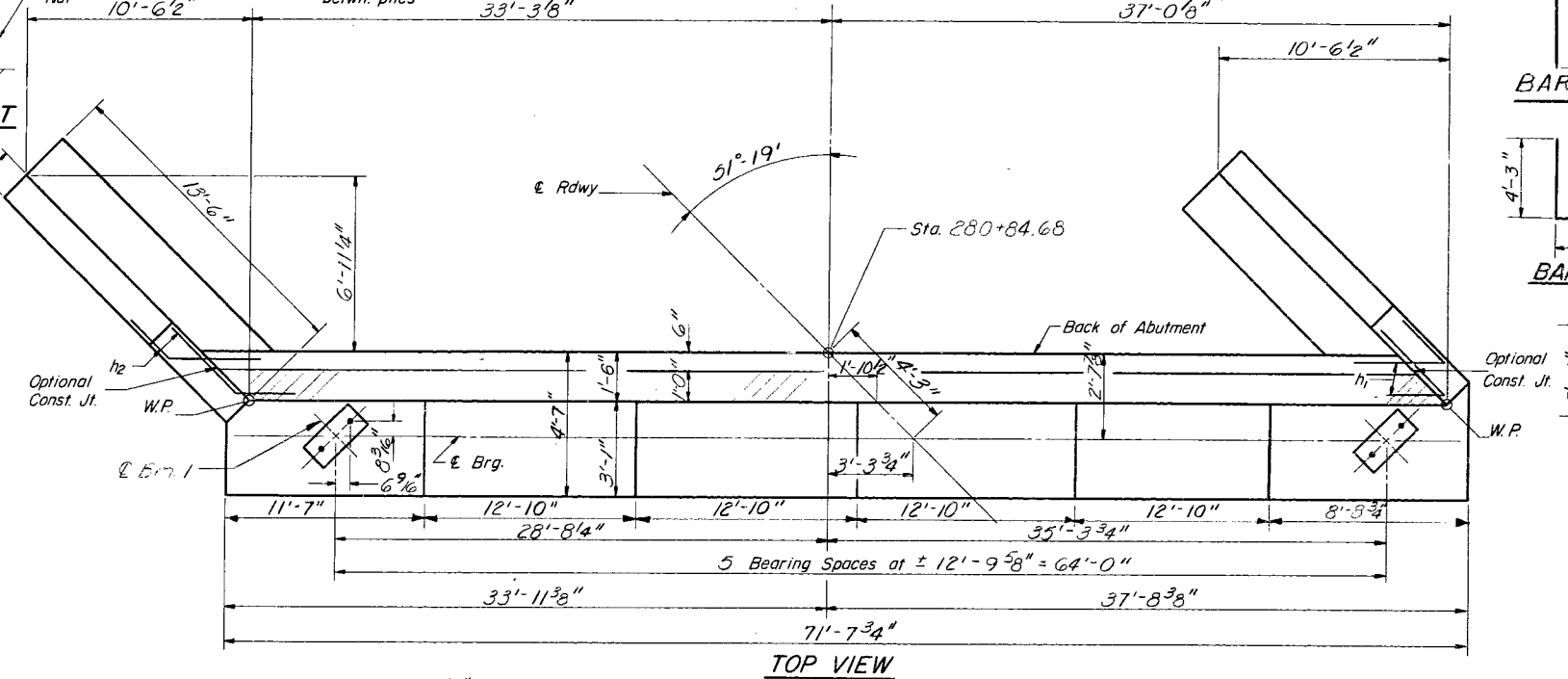
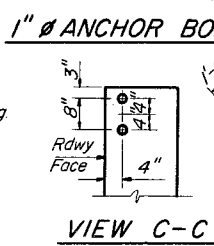
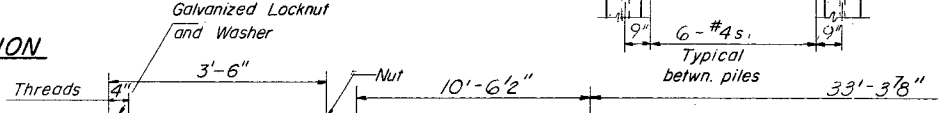
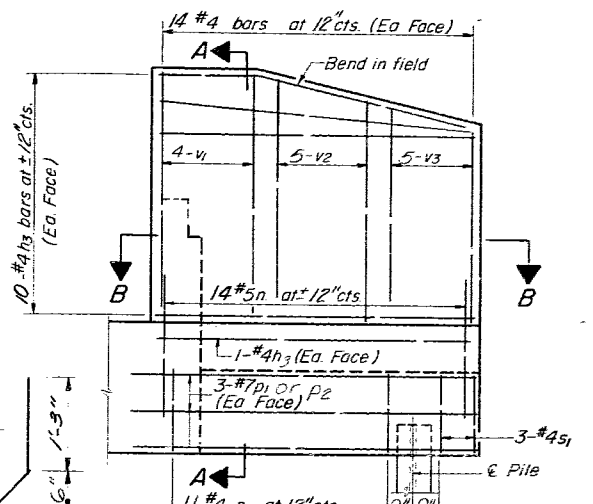
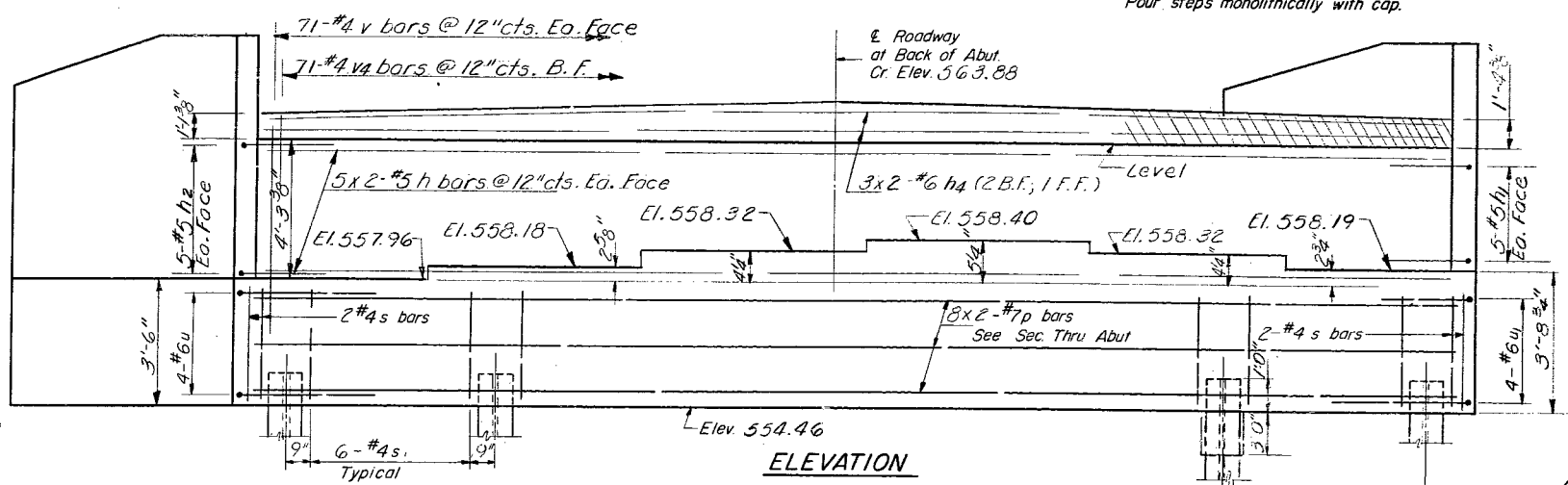
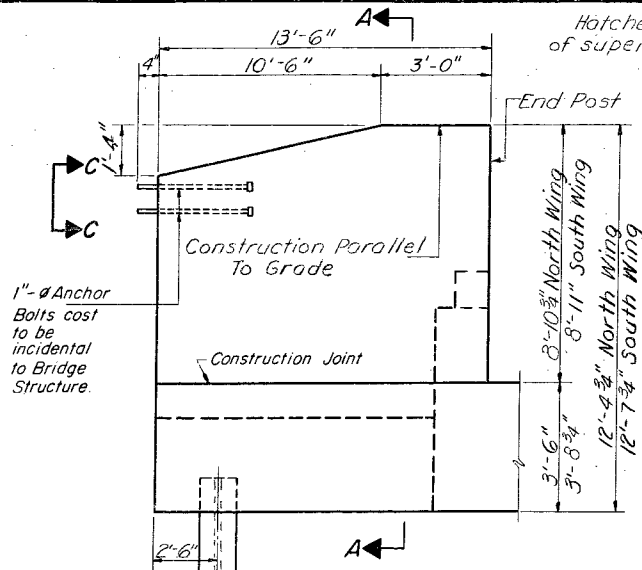
Feb 3 1971

Hatched area to be poured after forms of superstructure have been removed.

NORTH WING

SOUTH WING

Space reinforcement in cap to miss anchor bolts.  
Pour steps monolithically with cap.



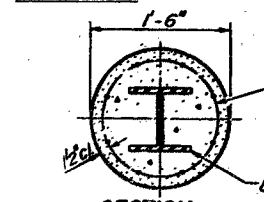
ONE ABUTMENT  
BILL OF MATERIAL

Bar	No	Size	Length	Shape
h	20	#5	35'-9"	—
h1	10	#5	7'-3"	—
h2	10	#5	7'-3"	—
h3	44	#4	13'-3"	—
h4	6	#6	36'-0"	—
n	28	#5	9'-3"	U
p	16	#7	36'-6"	—
p1	6	#7	13'-3"	—
p2	6	#7	14'-6"	—
s	64	#4	15'-7"	□
s1	28	#4	9'-5"	□
u	4	#6	11'-0"	C
u1	4	#6	10'-0"	S
v	142	#4	7'-0"	—
v1	16	#4	8'-9"	—
v2	20	#4	8'-0"	—
v3	20	#4	7'-3"	—
v4	71	#4	3'-3"	—

Class X Concrete Cu Yds 72.4  
Reinforcement Bars Lbs. 5500  
Steel Piles 8BP36 Lin Ft. 696  
Test Piles Steel 8BP36 Ea. 1

SEC. A-A

SEC. THRU ABUT.



Welded wire fabric 6"x6 mesh #4 wire weighing 58#/100 sq. ft. The cost of Class X Concrete Encasement and Reinforcement is incidental to the cost of furnishing piles. Forms for encasement may be omitted when soil conditions will permit.

DESIGNED	J. M. Patel
CHECKED	A. J. K.../...
DRAWN	H.K.K.
CHECKED	S. J. H.

EXAMINED	FEB 9 1971
PASSED	...
APPROVED	...

EAST ABUTMENT  
S.B.I. RT. 7 SEC. 6-VB  
GRUNDY COUNTY  
STA. 279+62.63



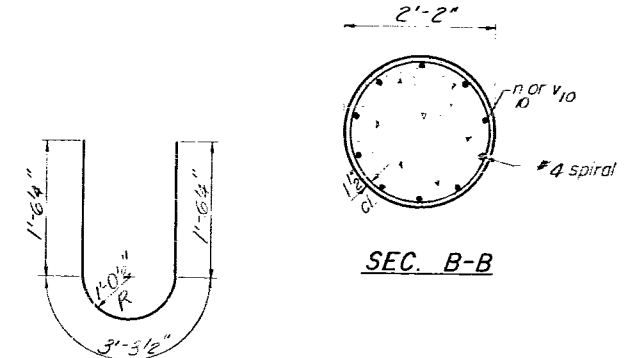
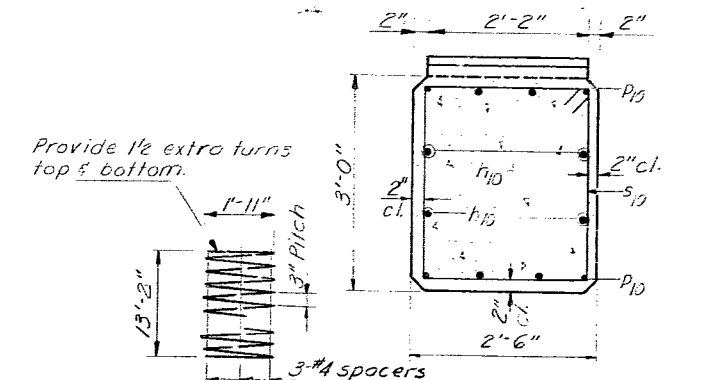
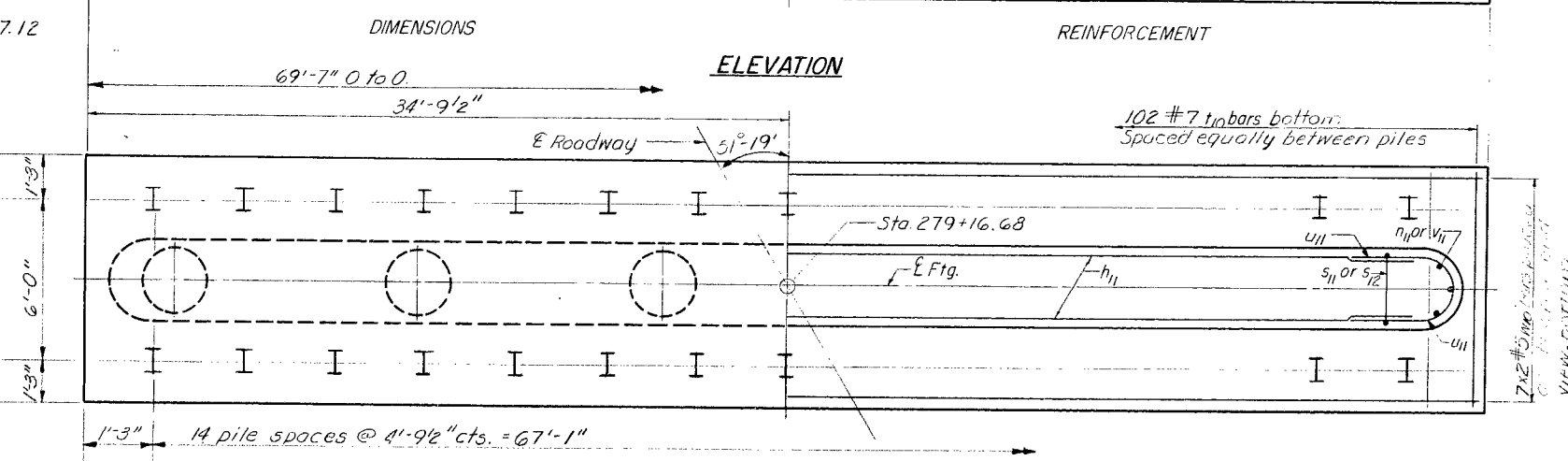
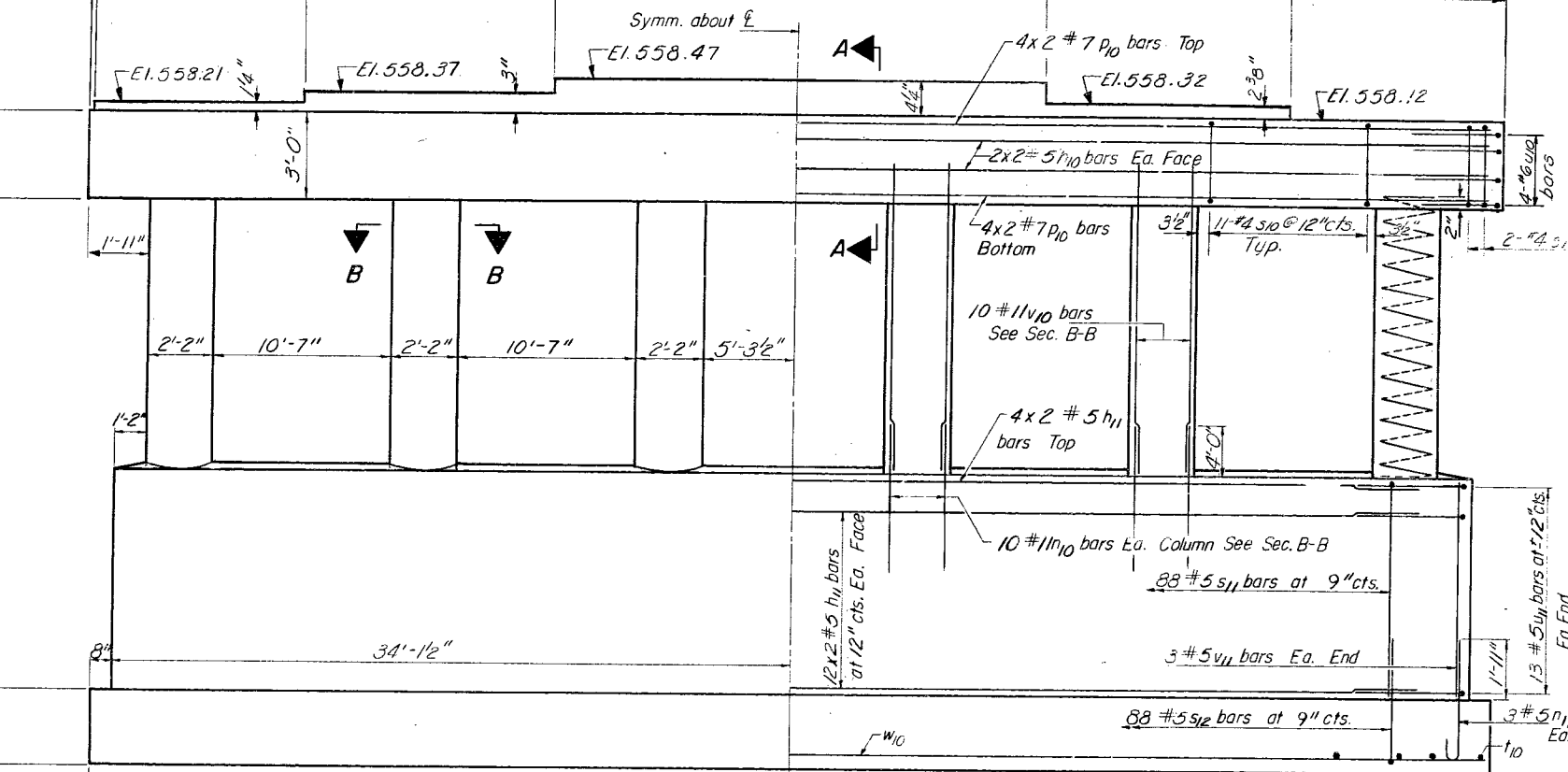
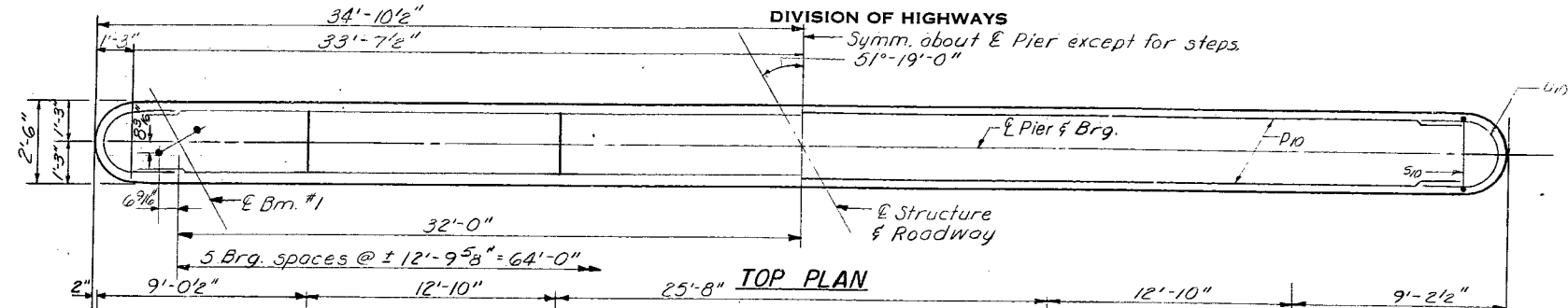
**NOTES:**

Space Reinforcement in cap to miss anchor bolts.  
 All edges shall have standard 3/4" chamfer except as noted.  
 Pour steps monolithically with cap.  
 Min. Spiral lap=1/2 turns.

Bars shown thus 20x3-#5 etc. indicates  
 20 lines of bars with 3 lengths per line.

STATE OF ILLINOIS

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 12
8	G-VB	GRUNDY	35	21	15 SHEETS
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT		



**BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
n10	60	#11	10'-6"	—
n11	6	#5	4'-3"	—
P10	16	#7	34'-9"	—
s10	59	#4	10'-5"	□
s11	88	#5	27'-2"	□
s12	88	#5	10'-0"	□
sp	6	#4		www
v10	102	#7	8'-3"	—
u10	8	#6	6'-4"	—
u11	26	#5	6'-4"	—
v11	6	#5	12'-6"	—
w10	14	#5	35'-6"	—

Class X Concrete	Cu. Yds.	160.5
Reinforcement Bars	Lbs.	19,580
Steel Piles 8BP36	Lin. Ft.	241
Test Piles Steel 8BP36	Eo.	1

**A&B DIMENSIONS**

Bar	A	B
s11	2'-2"	12'-6"
s12	2'-2"	3'-11"

**PILE DATA**

Type: Steel 8BP 36  
 Capacity: Driven to refusal  
 Est. Length: 29'  
 No. Required: 30 including  
 1 test pile

DESIGNED	J. M. Patel	EXAMINED	[Signature]
CHECKED	A. J. Whipple	PASSED	[Signature]
DRAWN	Ben Robinson	APPROVED	[Signature]
CHECKED	S. V. K.	CHIEF HIGHWAY ENGINEER	

P-26 10-5-70

PIER 1  
 S.B.I. RT. 7 SEC. G-VB  
 GRUNDY COUNTY  
 STA. 279+62.68

**NOTES:**

Space Reinforcement in cap to miss anchor bolts.  
 All edges shall have standard 3/4" chamfer except as noted.  
 Pour steps monolithically with cap.  
 Min. Spiral lap = 1/2 turns.  
 Bars shown thus 20x3 etc. indicates 20 lines of bars with 3 lengths per line.

STATE OF ILLINOIS

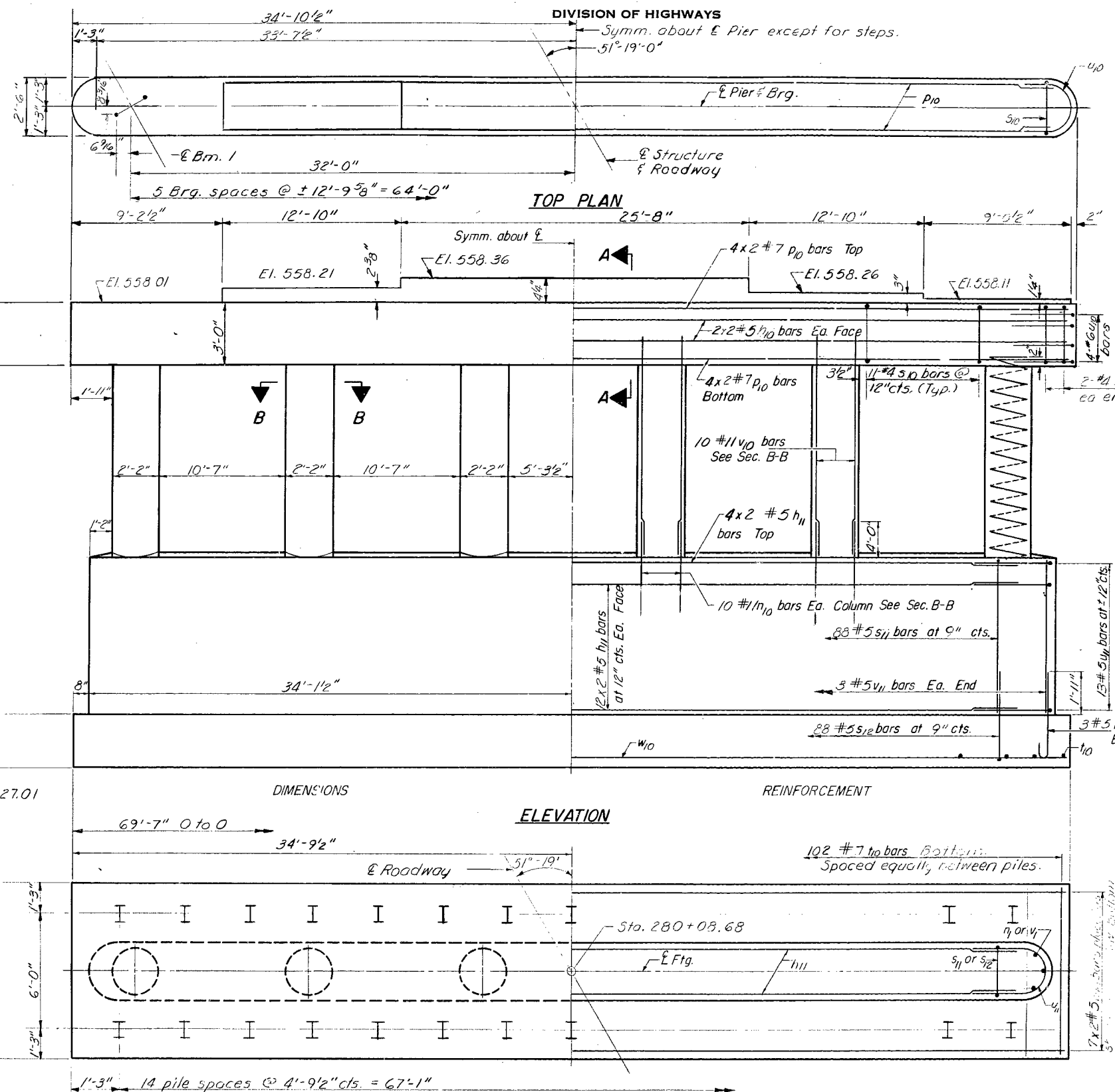
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 14
8	G-VB	GRUNDY	35	22	15 SHEETS
FED. ROAD DIST. NO. 7		CLASSIFICATION	FED. AID PROJECT		

DIVISION OF HIGHWAYS

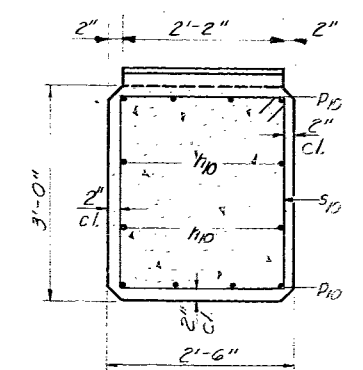
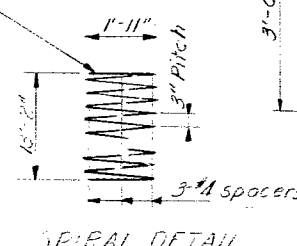
TOP PLAN

ELEVATION

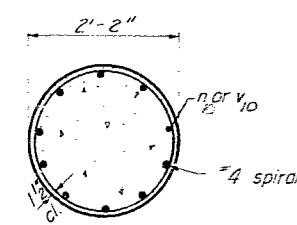
FOOTING PLAN



Provide 1/2 extra turns. (Top & bottom)



SEC. A-A



SEC. B-B

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h10	8	#5	34'-3"	U
h11	56	#5	33'-6"	U
v10	60	#11	10'-6"	U
v11	6	#5	4'-2"	U
p10	16	#7	34'-9"	U
s10	59	#4	10'-5"	U
s11	55	#5	27'-2"	U
s12	88	#5	10'-0"	U
sp	6	#4		WWW
h10	102	#7	7'-3"	U
u10	8	#6	6'-4"	U
u11	26	#5	6'-4"	U
v10	60	#11	15'-6"	U
v11	6	#5	12'-6"	U
w10	14	#6	33'-8"	U
Class X Concrete		Cu. Yds.	160.5	
Reinforcement Bars		Lbs.	19,580	
Steel Piles 8.B.P.36		Ln. Ft.	870	

A&B DIMENSIONS

Bar	A	B
s11	2'-2"	12'-6"
s12	2'-2"	3'-11"

**PILE DATA**  
 Type: Steel 8 BP 36  
 Capacity: Drive to refusal  
 Est. Length: 29'  
 No. Required: 30

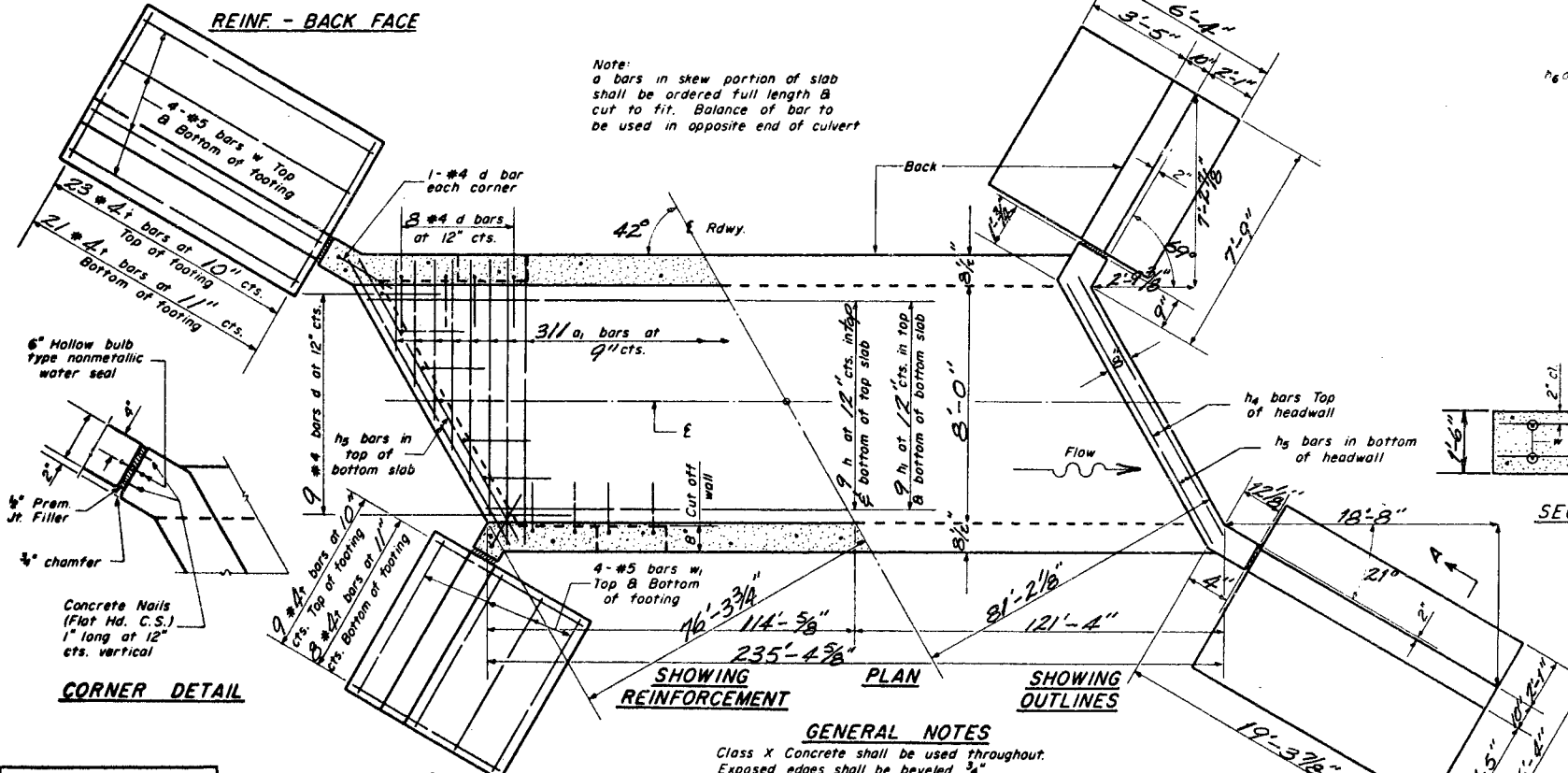
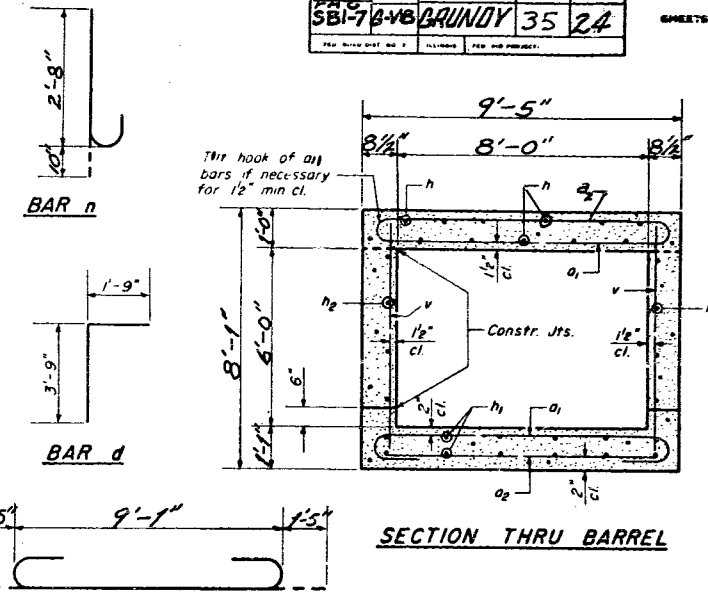
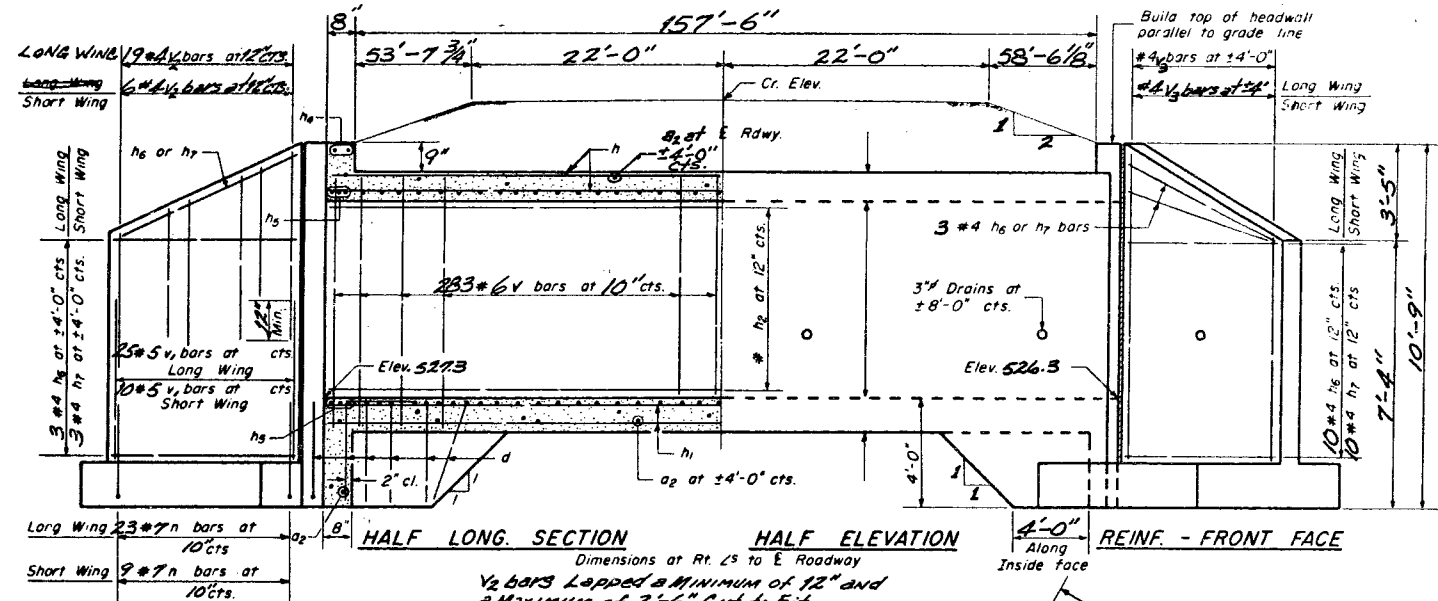
DESIGNED	J. M. Patel	EXAMINED	[Signature]
CHECKED	A. I. Mahajan	PASSED	[Signature]
DRAWN	By Robinson	APPROVED	[Signature]
CHECKED	S. V. H.	CHIEF HIGHWAY ENGINEER	

P-26 10-5-70

PIER 2  
 S.B.I. RT. 7 SEC. G-VB  
 GRUNDY COUNTY  
 STA. 279+62.68







**BILL OF MATERIAL**

Bar	No.	Size	Length
a1	622	#10	11'-11"
a2	122	#4	8'-3"
d	54	#4	5'-6"
h	126	#6	35'-0"
h1	126	#6	35'-0"
h2	84	#6	35'-0"
h3	4	#6	13'-9"
h4	12	#6	13'-9"
h5	34	#4	18'-8"
h6	34	#4	6'-9"
n	64	#7	3'-6"
v	122	#4	6'-1"
v1	566	#6	7'-9"
v2	70	#5	3'-0"
v3	50	#4	6'-10"
v4	14	#4	9'-0"
v5			
w	16	#5	18'-8"
w1	16	#5	6'-9"
Class X Concrete			Cu. Yds. 279.8
Reinforcement Bars			Lbs. 59,833

**GENERAL NOTES**  
Class X Concrete shall be used throughout.  
Exposed edges shall be beveled 3/8".  
For backfilling and embankment see S'd. Spec's.  
All bars shall be lapped 20 diameters unless otherwise specified.  
The top of the culvert, the backs of the sidewalls above the lower construction joint and backs of the wings above the tops of the footings shall be waterproofed in accordance with Art. 51.21 of the S'd. Spec's.  
Nonmetallic water seal used in the wingwall joints shall extend from the top of the footing to within 6" of the top of the headwall.

DESIGNED **A.G. KLEIN**  
CHECKED **R.J.M. 1-19-71**  
DRAWN  
CHECKED  
EXAMINED  
PASSED  
APPROVED

STA. 281+45.50  
BOX CULVERT 48° SKEW  
SBI-7  
SEC. (GMB) R. WERS.  
BRUNDY COUNTY  
LOADING H20-S16

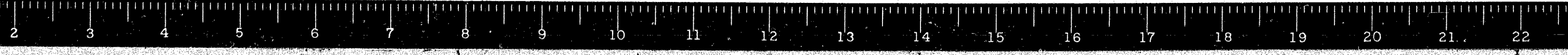
U.S. 6 From Sta. 264+50 to Sta. 296+10

LOCATION AND DIRECTION W.B.	SIGN STANDARD	SKETCH OR DESCRIPTION	NEW PANEL AREA (SQ. FT.)	REMOVE	RELOCATE	WOOD AND TELESCOPING MARKS	OFFSET (FT.)	REMARKS
WB. Station 264+50			69					Begin Const. 264+50
274+10		(X/R) *		✓				Remove & don't Replace
296+10								End Const. Sta. 296+10
WB. Station 296+10								Begin Const. Sta. 296+10
266+75		(X/R) *		✓				Remove & don't Replace
264+50								End Const. Sta. 264+50

ROUTE NO.	S.E.C.	COUNTY	TOTAL SHEETS	SHEET NO.
FA 8	G-VB	GRUNDY	35	24
SBI-7				
STATE DIST. NO.	ILLINOIS	PROJECT	F-10 ( )	

SIGN TABULATION SHEET

\* SHOWN FOR IDENTIFICATION ONLY - NOT TO BE REPLACED  
 \*\* REPLACE WITH STATE FURNISHED PANEL

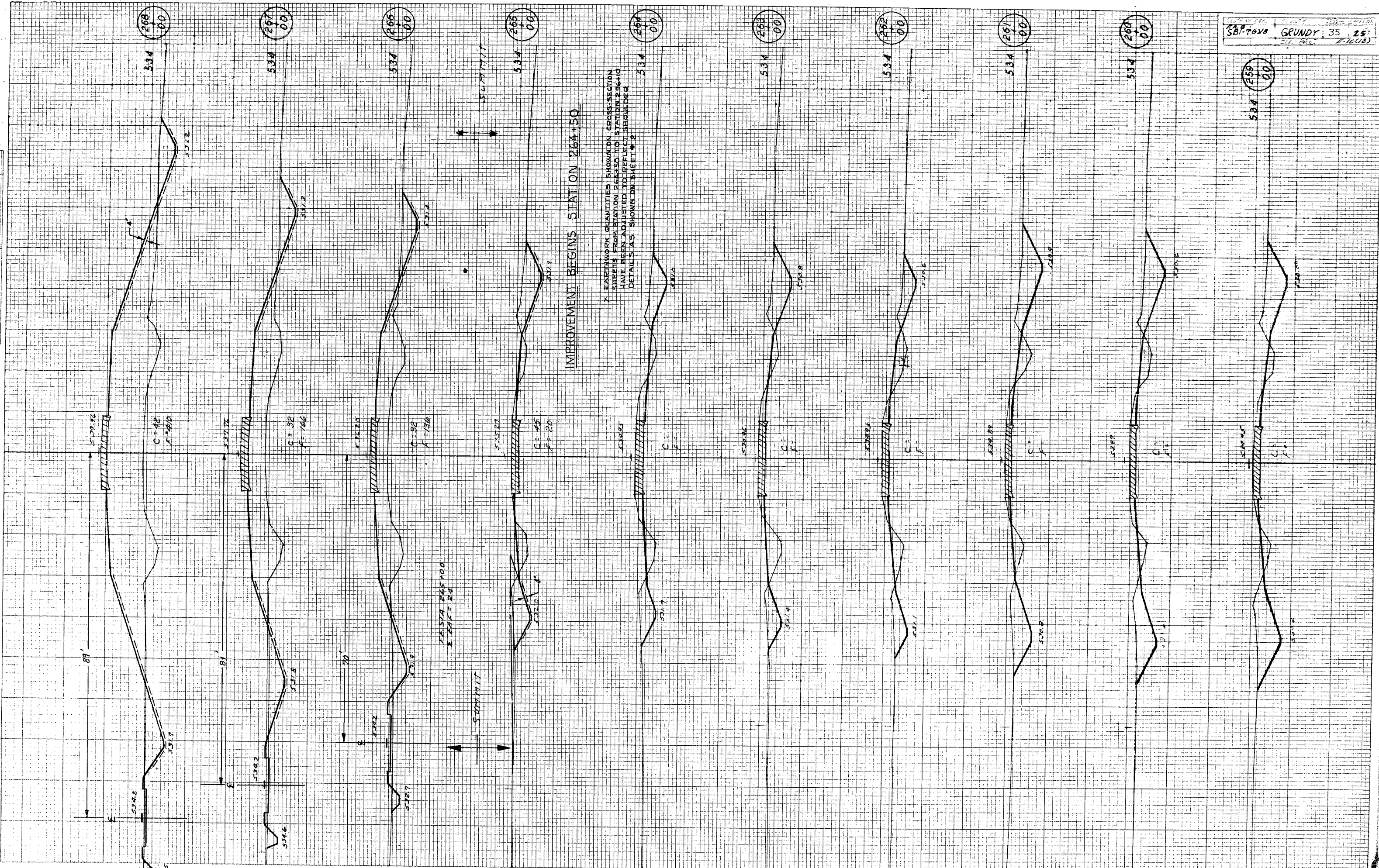


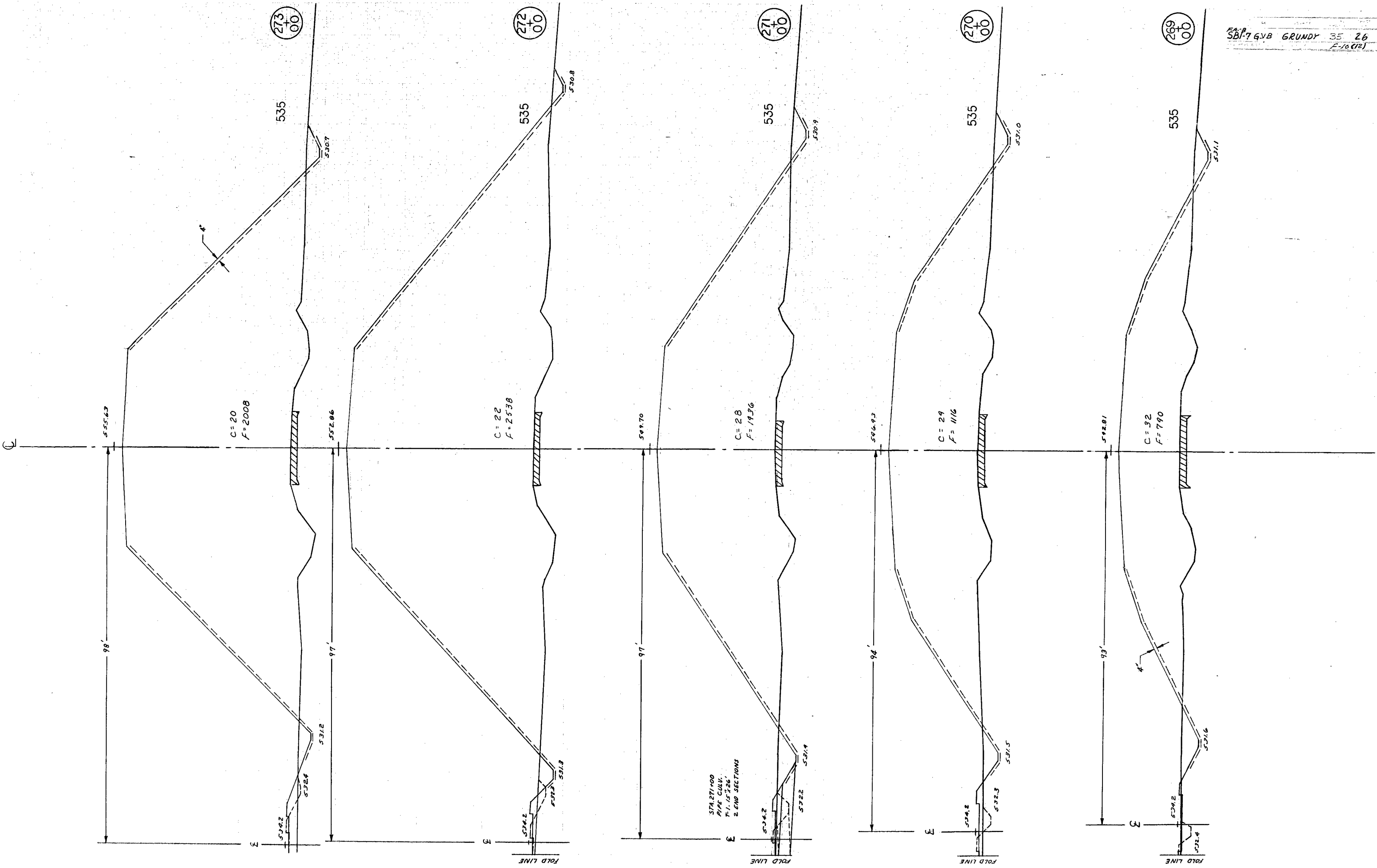


ORIGINAL SURVEY PLATES  
 NO. 1000  
 DATE  
 BY  
 SURVEYED  
 CHECKED  
 FINAL SURVEY PLATES  
 NO. 1000  
 DATE  
 BY  
 SURVEYED  
 CHECKED

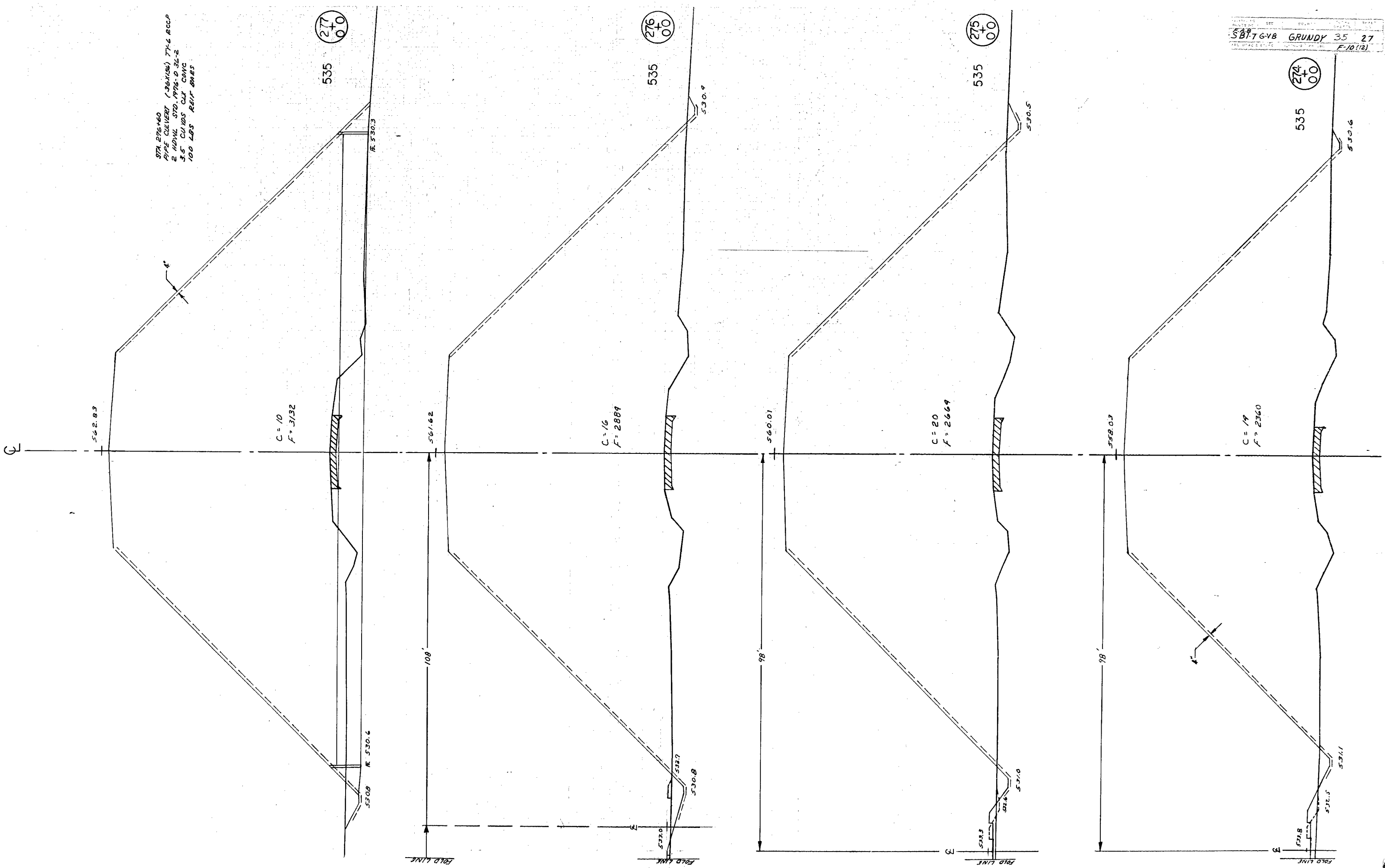
ORIGINAL SURVEY PLATES  
 NO. 1000  
 DATE  
 BY  
 SURVEYED  
 CHECKED

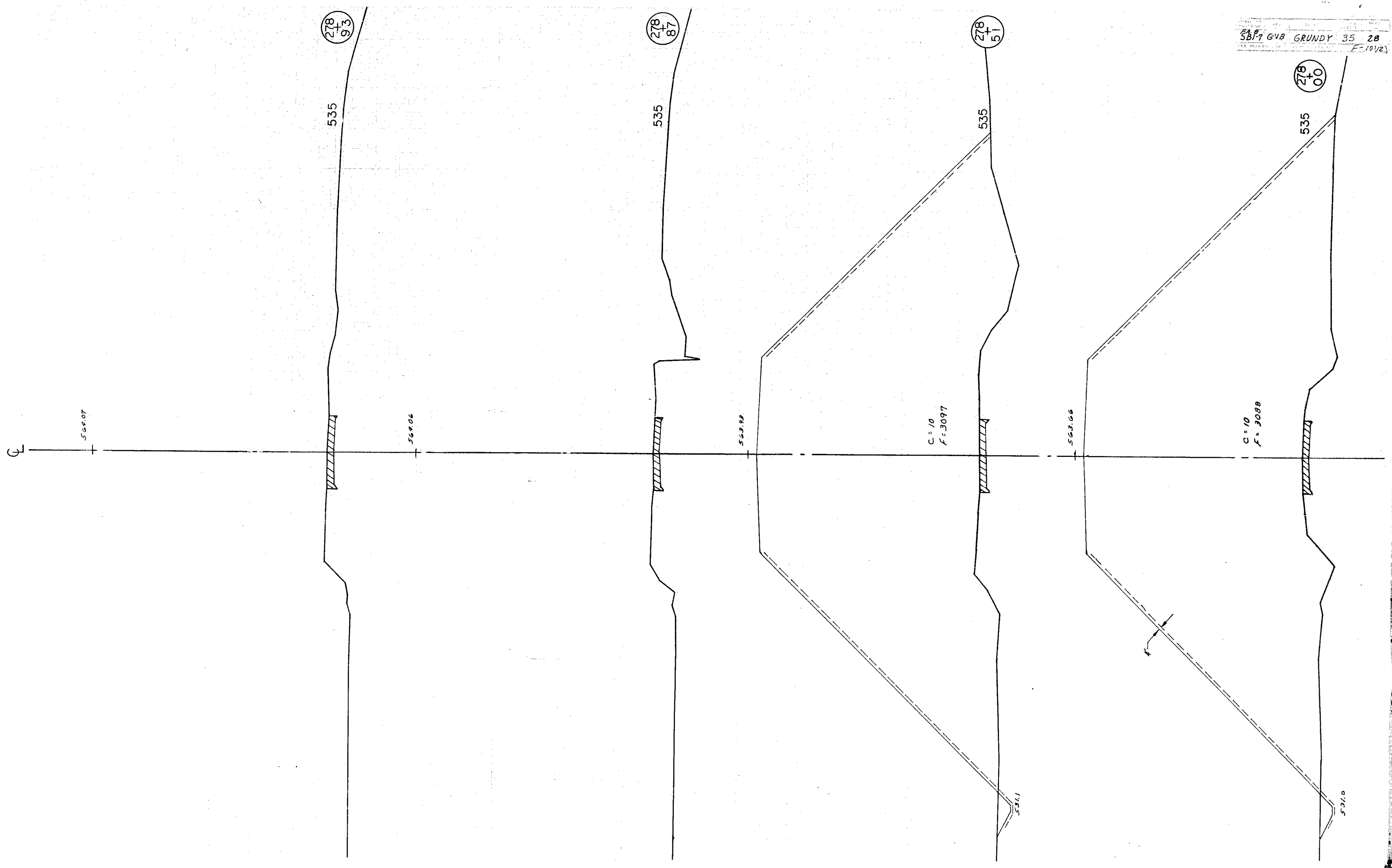
SBP-7600 GRUNDY 35 25  
 E-1002





STA 276+60  
 PIPE CULVERT (36x18x) 7'-4" eccp  
 2 HOLES STD. 1976-D 32" x 2"  
 3.5' CURBS CLIP CONC  
 100 LBS REEF BRGS





564.07

535

278  
93

564.06

535

278  
87

564.99

C = 10  
F = 3097

535

278  
51

563.66

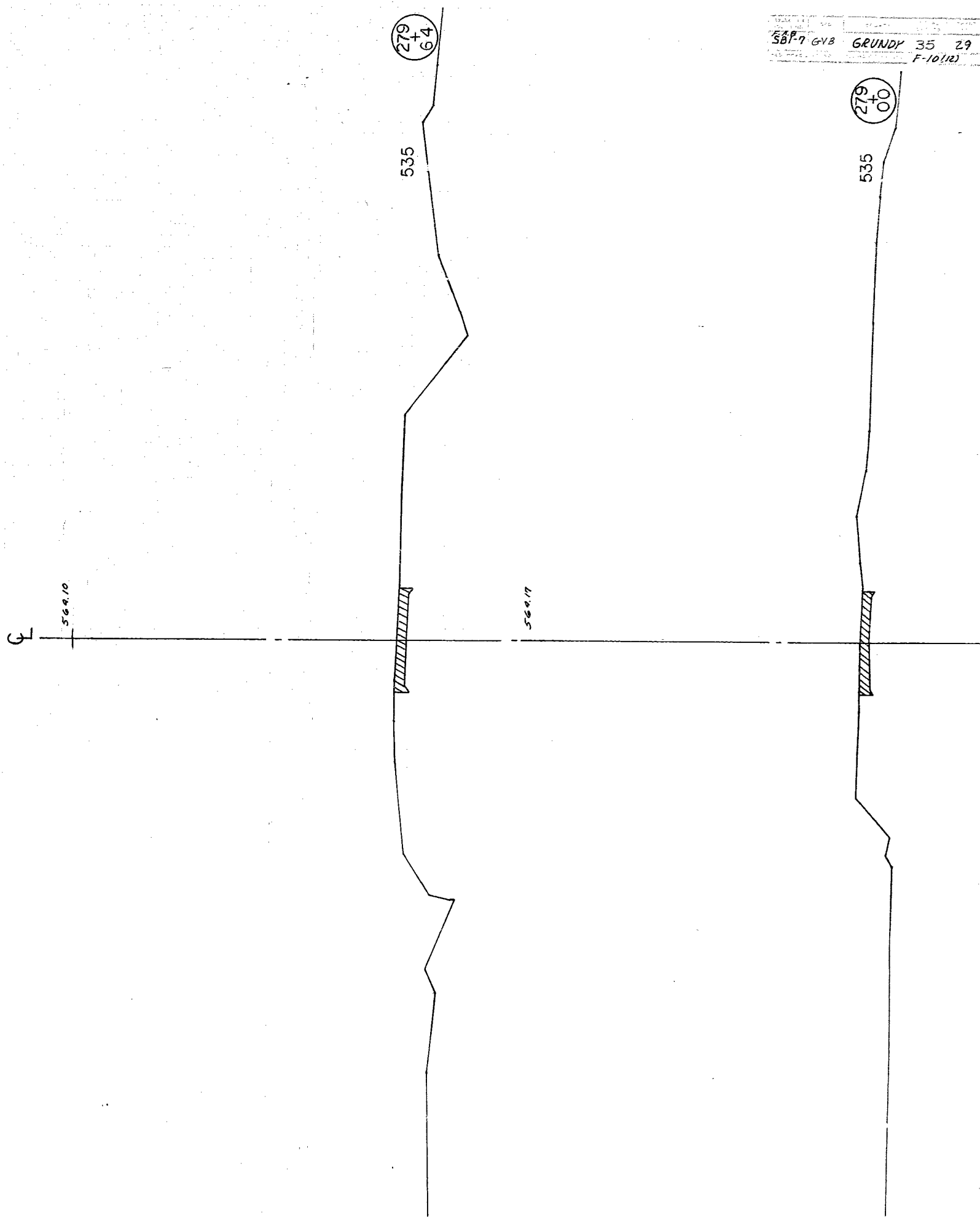
C = 10  
F = 3088

535

278  
00

561.0

5879 GYB GRUNDY 35 29  
F-10/12





SBP-7 GVB GRUNDY 35 30  
F-10U2

280  
±  
00

535

280  
±  
08

535

280  
±  
30

535

OLD LINE

280  
±  
55

535

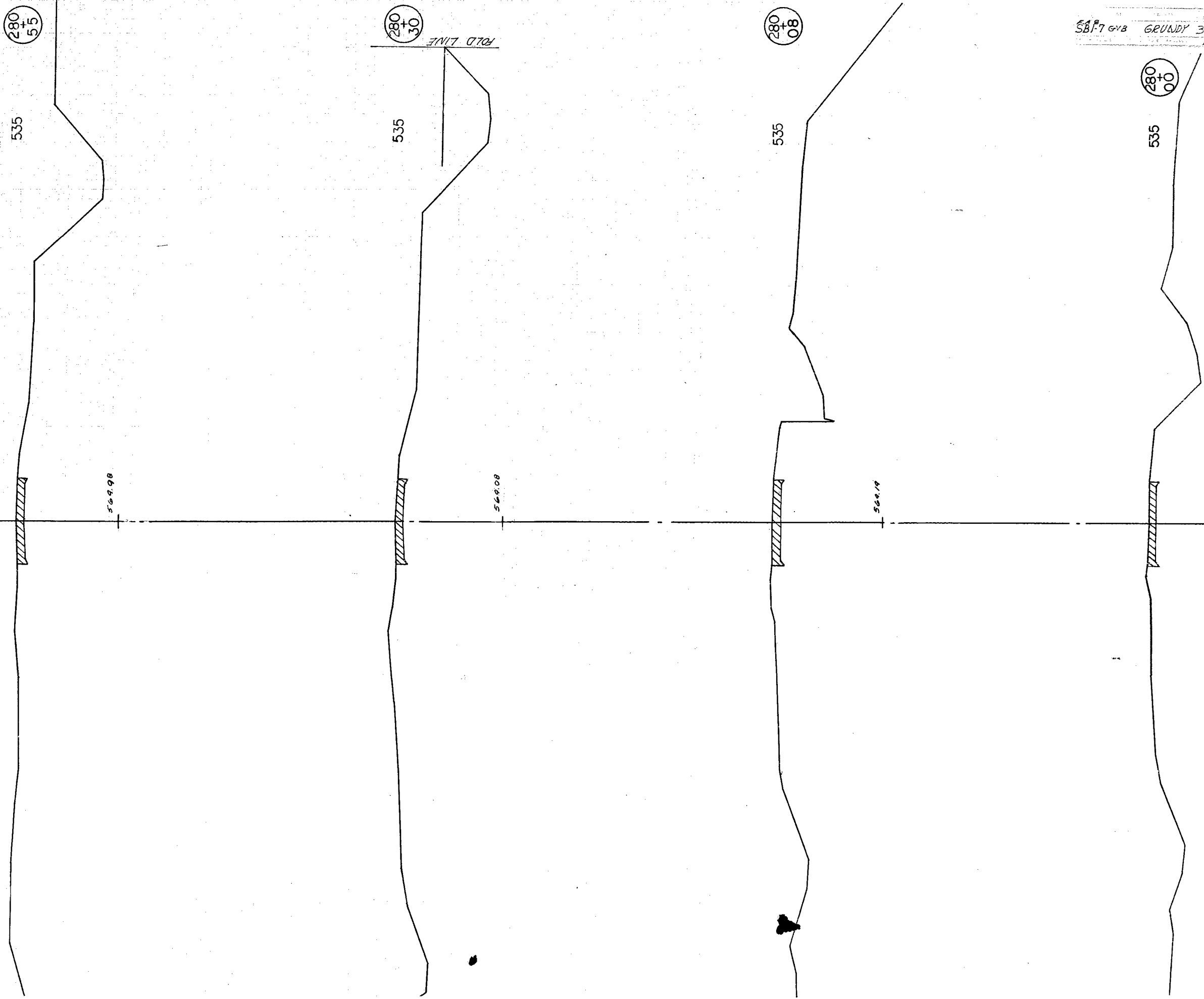
Q

569.01

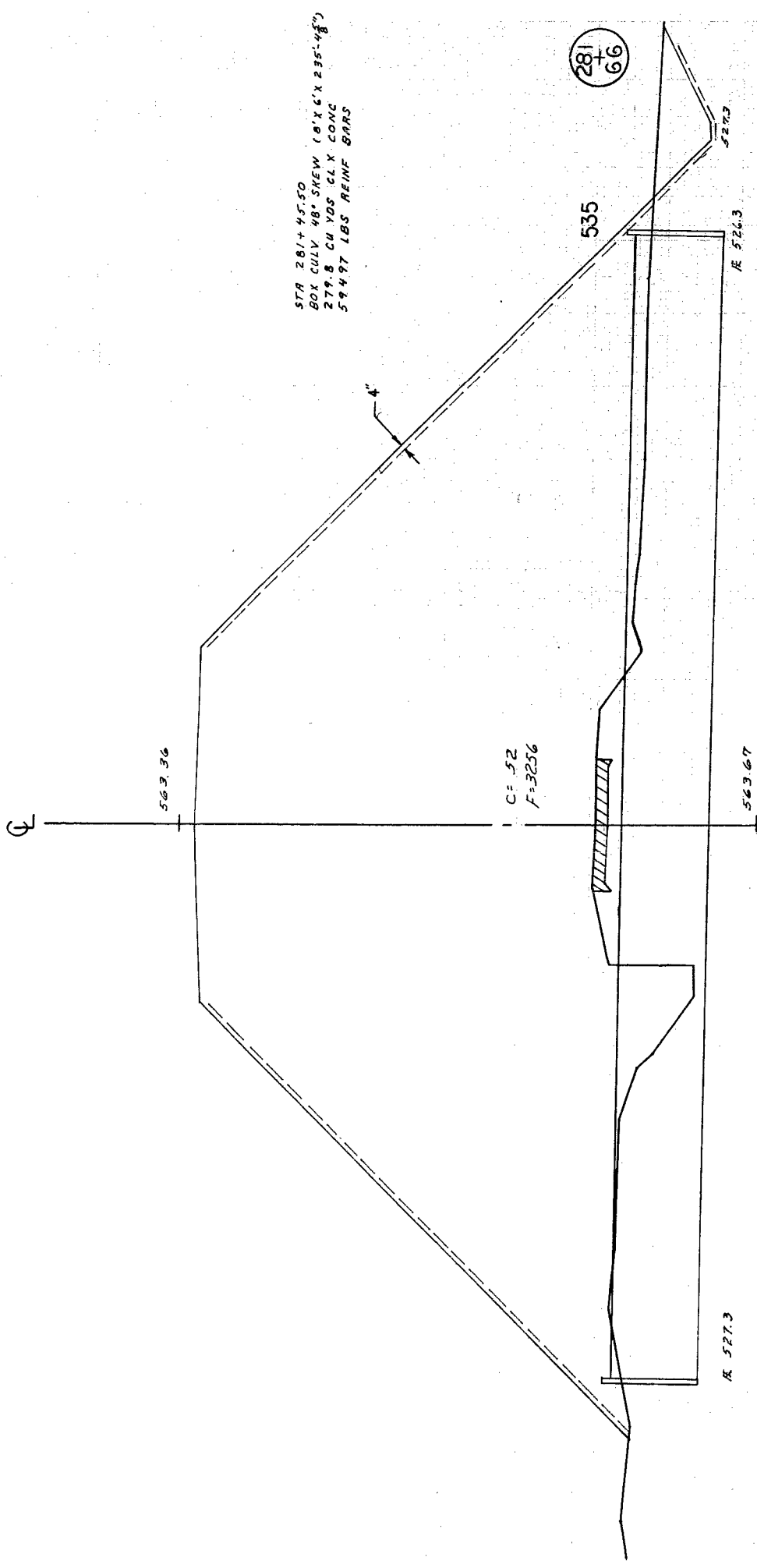
569.98

569.08

569.19



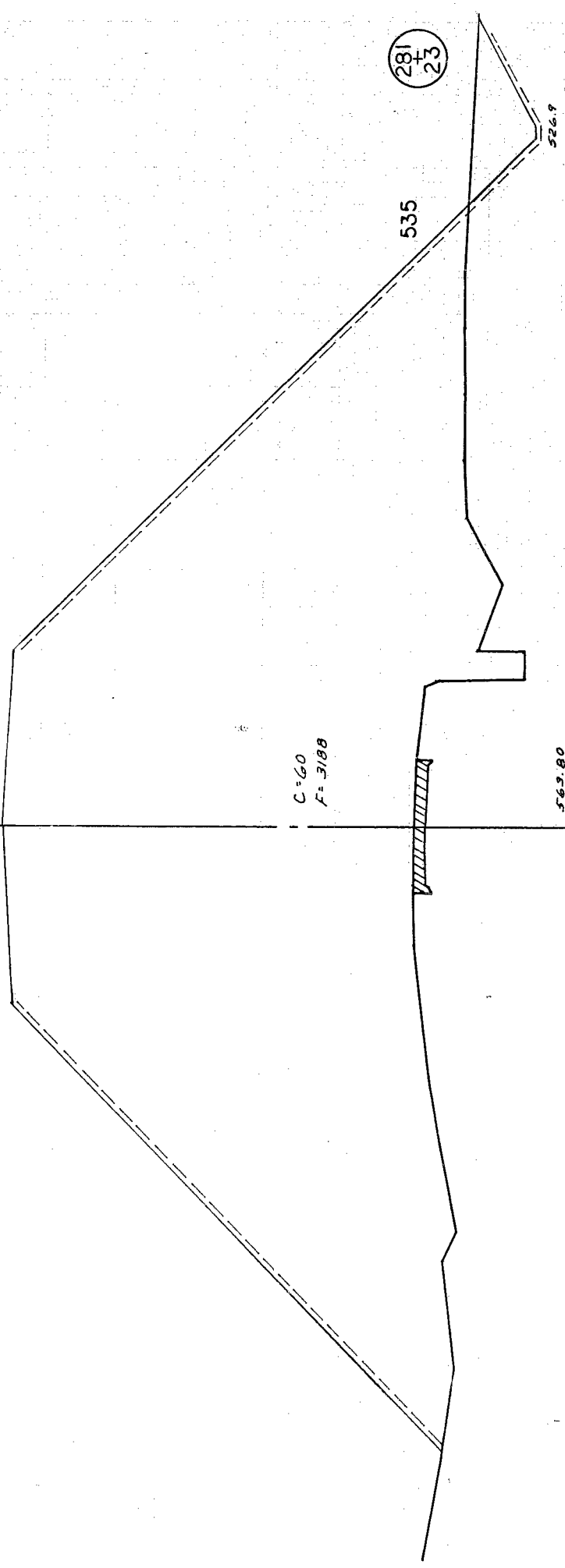
STA 281+45.50  
BOX CULV 48" SKEW (81'6" X 235'-48")  
27'8" CU YDS CLX CONC  
59497 LBS REINF BRAS



C = 52  
F = 3256

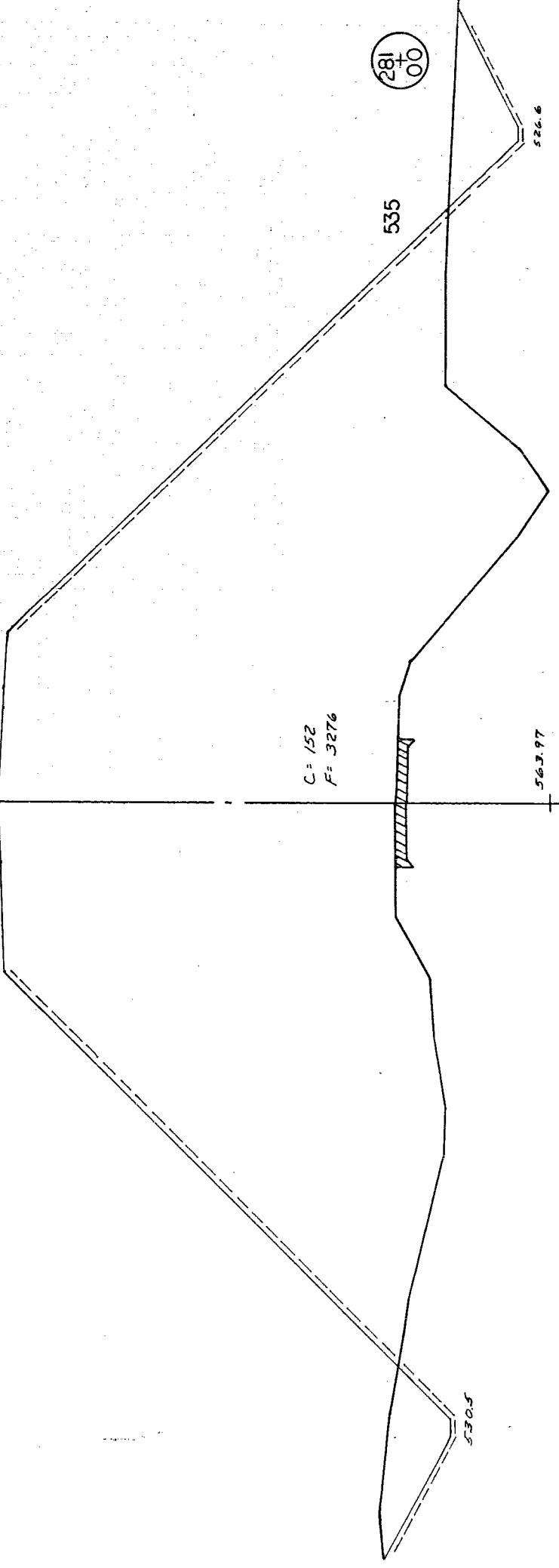
281  
66

C = 60  
F = 3188



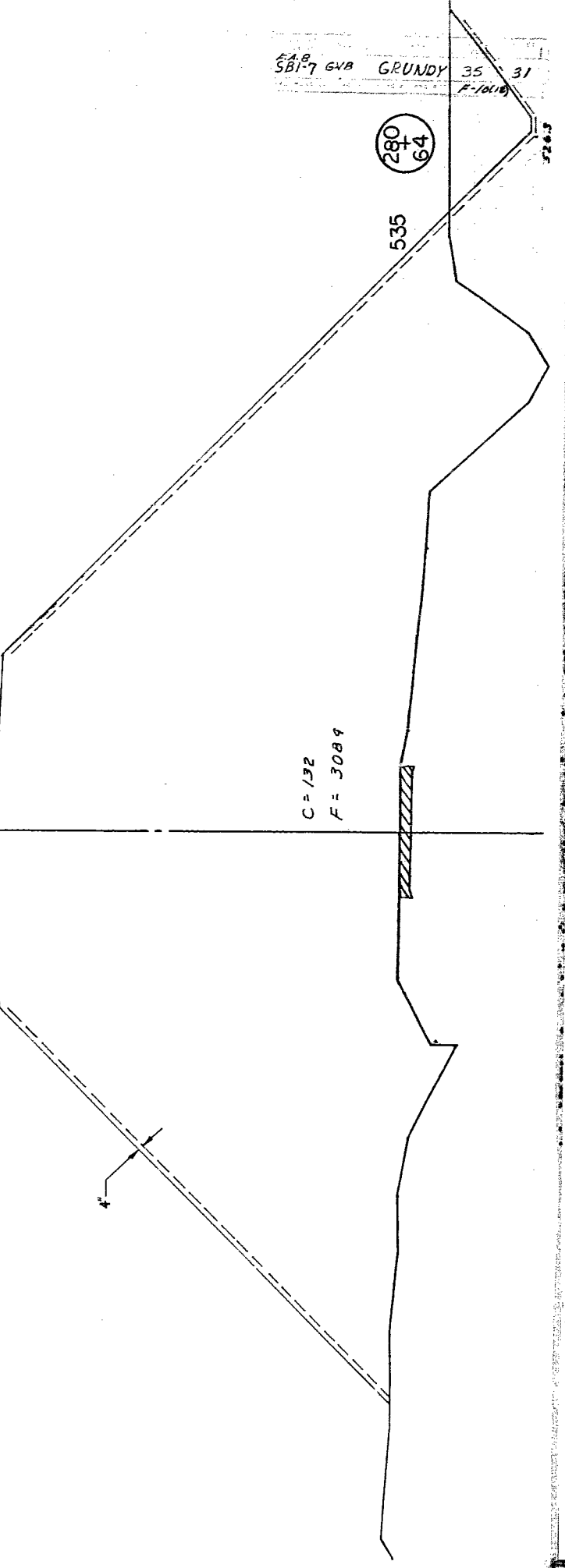
281  
23

C = 152  
F = 3276



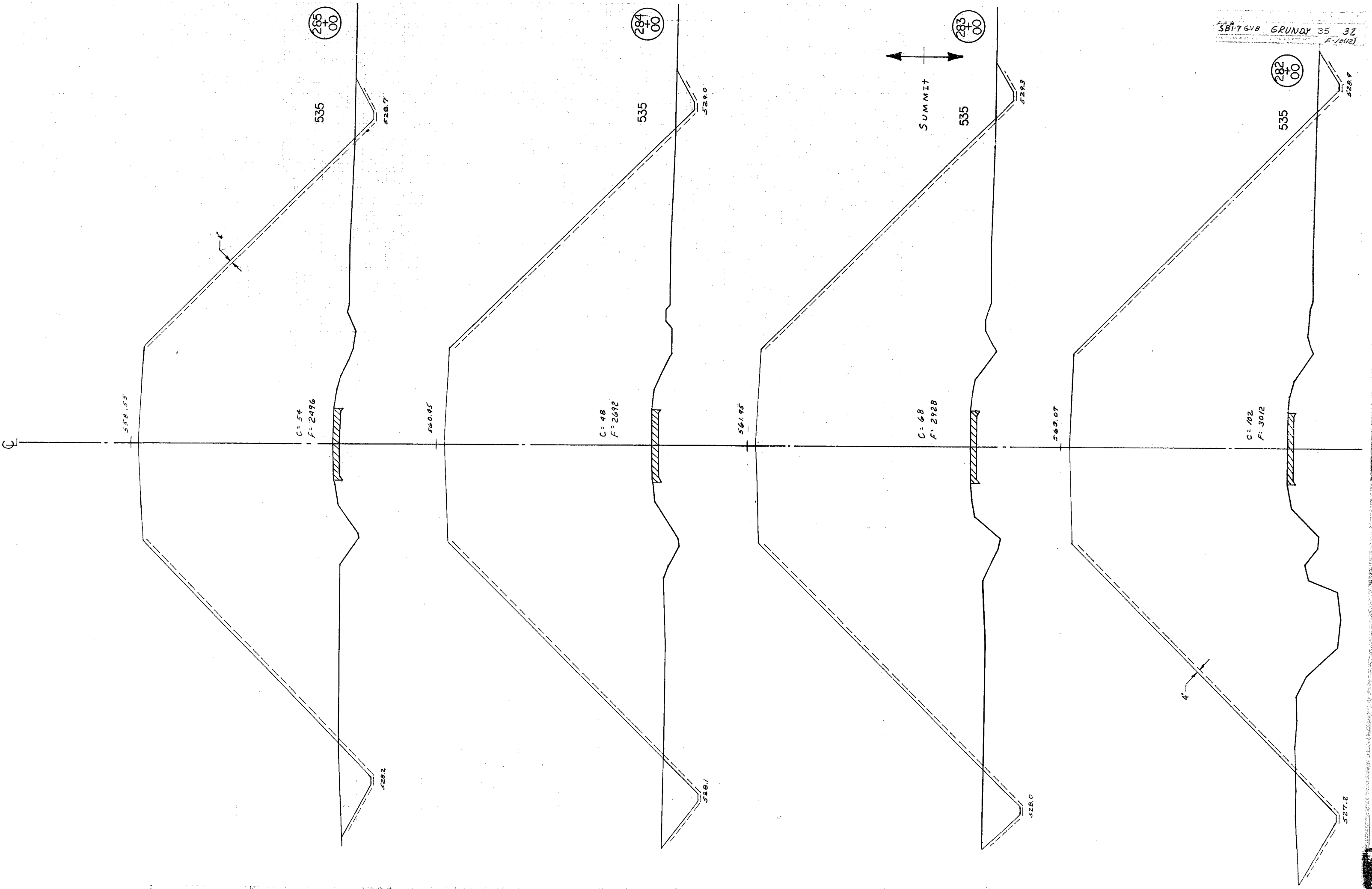
281  
00

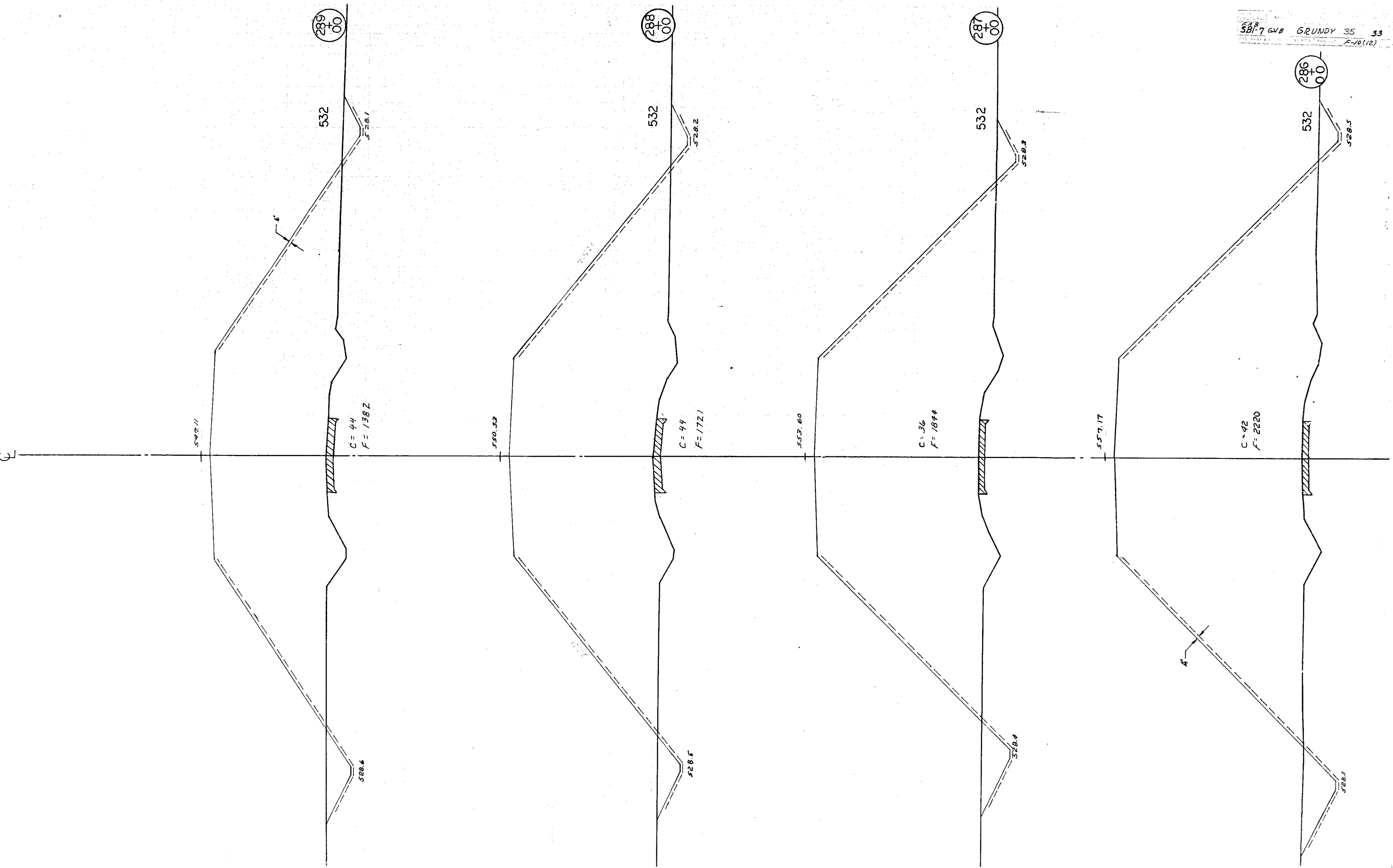
C = 132  
F = 3089

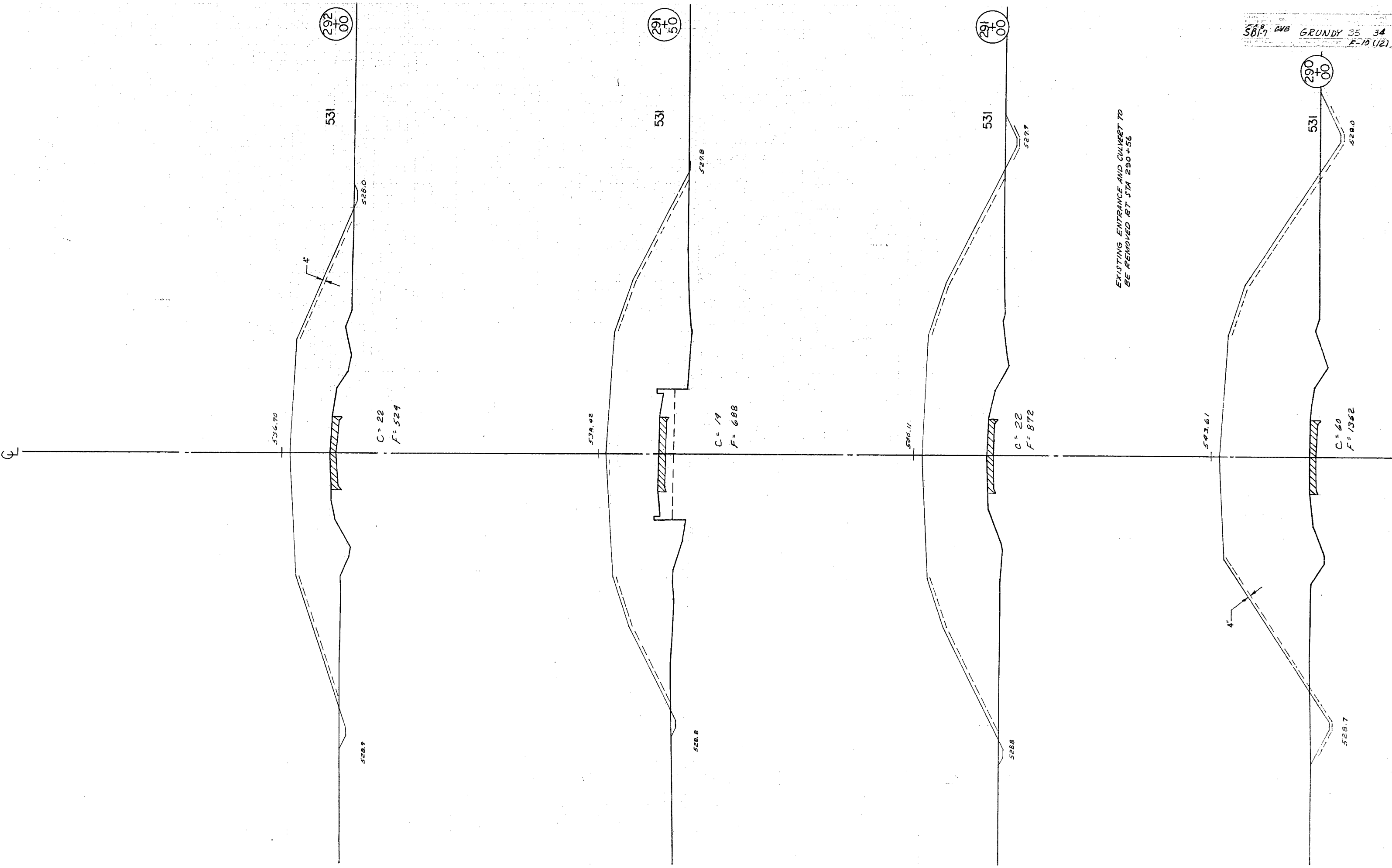


280  
64

FAB 58127 648 GRUNDY 35 31  
F-10118

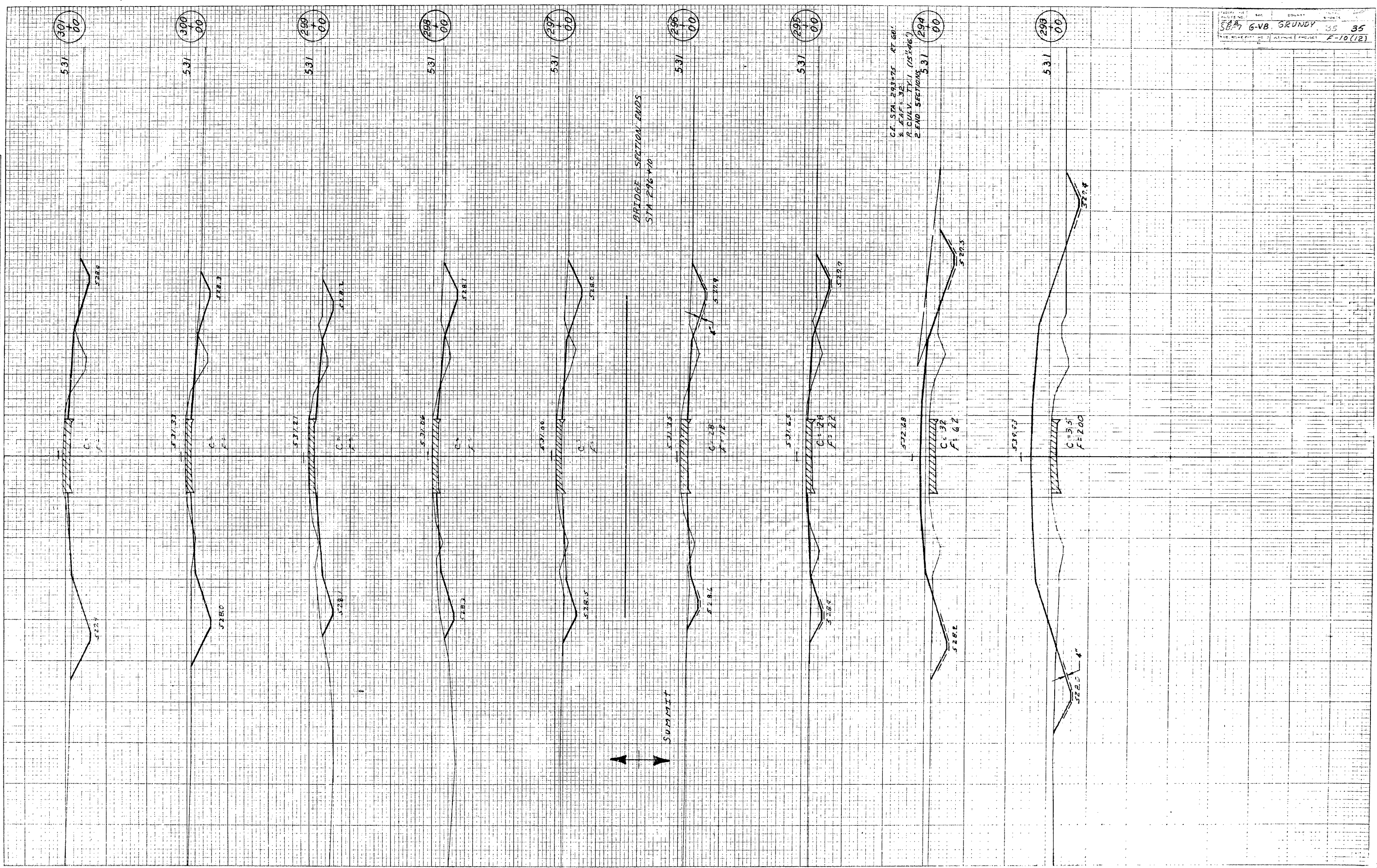






FINAL SURVEY NOTE BOOK NO. DATE

ORIGINAL SURVEY NOTE BOOK NO. DATE



BRIDGE SECTION ENDS  
STA 296+00

SUMMIT

CE STA 293+75 RT 68'  
EAF = 32'  
PCULV TYP. (15'-46")  
2 END SECTIONS 531+00

PLATE NO. 35  
COUNTY GRUNDY  
PROJECT 6-VB  
DATE 10/1/69  
SCALE F-10(12)