

B.M. 151: Chiseled box on SE wingwall of SN 031-0018, Sta. 85+92, 18.2' Left, Elev. 512.96.

EXISTING STRUCTURE: S.N. 031-0018, originally constructed in 1952 as FA Route 164 Sec. 410-B at Station 85+20.00, using cast-in-place concrete deck beams with 7" concrete deck, 3 spans, 147'-0" back-back abutments, 35'-8" out-out width, open pile bent abutment on concrete piles, wall piers with footings on timber piles, open abutment with footing on timber piles. In 1988 the bridge was repaired, including a concrete deck overlay and new bridge railing.

Existing structure shall be removed and replaced using staged construction to maintain one lane of traffic.

No salvage.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL NOTES

Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts in painted areas and M164 Type 3 in unpainted areas. Bolts 7/8 in. ϕ , holes 15/16 in. ϕ , unless otherwise noted.

All structural steel shall be AASHTO M 270 Grade 50W.

Calculated weight of Structural Steel = 89040 lbs.

No field welding is permitted except as specified in the contract documents.

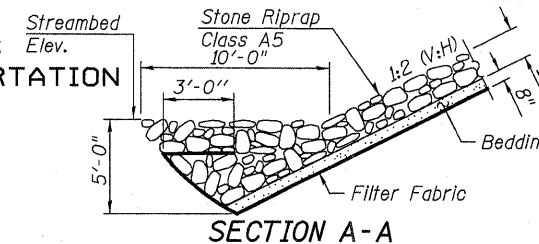
Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60. See Special Provisions. Reinforcement bars designated (E) shall be epoxy coated.

Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete cap plus 3 inches. Those areas shall be primed in the shop with a Department approved zinc rich primer. No field painting shall be required. All structural steel shall be cleaned as specified in the Special Provision for "Surface Preparation and Painting Requirements for Weathering Steel".

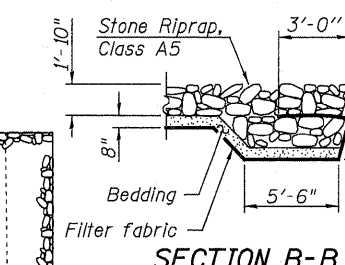
The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. Bearing seat surfaces shall be constructed or adjusted to their designated elevations within a tolerance of 1/8 inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.

Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.

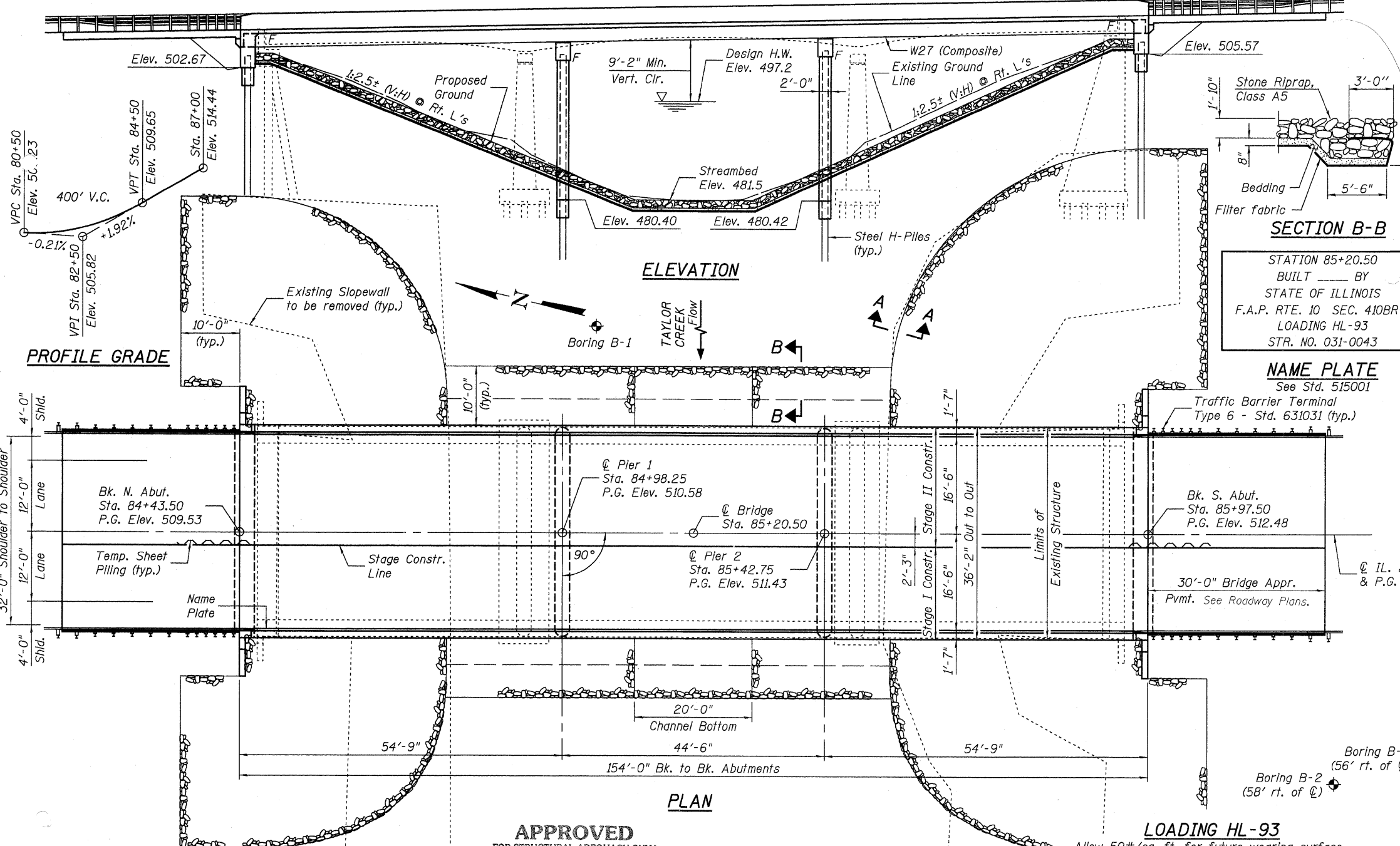
The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at substructures specified or approved by the Engineer before ordering the remainder of piles.



SECTION A-A



SECTION B-B



ELEVATION

PLAN

STATION 85+20.50
BUILT BY
STATE OF ILLINOIS
F.A.P. RTE. 10 SEC. 410BR-1
LOADING HL-93
STR. NO. 031-0043

NAME PLATE
See Std. 515001

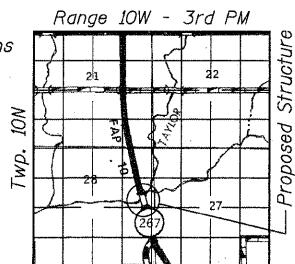
TOTAL BILL OF MATERIAL

| ITEM | UNIT | SUPER | SUB | TOTAL |
|---|-------|-------|-------|-------|
| Porous Granular Embankment, Special | Cu Yd | -- | 98 | 98 |
| Stone Riprap, Class A5 | Sq Yd | -- | 1661 | 1661 |
| Filter Fabric | Sq Yd | -- | 1661 | 1661 |
| Removal Of Existing Structures | Each | 1 | -- | 1 |
| Slope Wall Removal | Sq Yd | -- | 1080 | 1080 |
| Structure Excavation | Cu Yd | -- | 224 | 224 |
| Concrete Structures | Cu Yd | -- | 175.2 | 175.2 |
| Concrete Superstructure | Cu Yd | 191.9 | -- | 191.9 |
| Bridge Deck Grooving | Sq Yd | 530 | -- | 530 |
| Concrete Encasement | Cu Yd | -- | 8.4 | 8.4 |
| Protective Coat | Sq Yd | 694 | -- | 694 |
| Furnishing And Erecting Structural Steel | L Sum | 1 | -- | 1 |
| Stud Shear Connectors | Each | 2700 | -- | 2700 |
| Reinforcement Bars, Epoxy Coated | Pound | 45480 | 13080 | 58560 |
| Bar Splacers | Each | 552 | 124 | 676 |
| Furnishing Steel Piles HP 12x53 | Foot | -- | 1050 | 1050 |
| Furnishing Steel Piles HP 12x63 | Foot | -- | 1050 | 1050 |
| Driving Piles | Foot | -- | 2100 | 2100 |
| Test Pile Steel HP12x53 | Each | -- | 2 | 2 |
| Test Pile Steel HP12x63 | Each | -- | 2 | 2 |
| Pile Shoes | Each | -- | 24 | 24 |
| Temporary Sheet Piling | Sq Ft | -- | 299 | 299 |
| Name Plates | Each | 1 | -- | 1 |
| Anchor Bolts, 1" | Each | -- | 48 | 48 |
| Geocomposite Wall Drain | Sq Yd | -- | 62 | 62 |
| Pipe Underdrains For Structures 4" | Foot | -- | 134 | 134 |
| Underwater Struct. Excav. Protection - Loc. 1 | Each | -- | 1 | 1 |
| Underwater Struct. Excav. Protection - Loc. 2 | Each | -- | 1 | 1 |

INDEX OF SHEETS

| Sheet No. | Description |
|-----------|--|
| 1 | General Plan, General Notes & Bill of Material |
| 2 | Stage Construction Details, Temp Sheet Piling |
| 3 | Temp Concrete Barrier for Stage Construction |
| 4-6 | Top of Slab Elevations |
| 7-9 | Superstructure Details |
| 10 | Concrete Parapet Slipforming Option |
| 11 | Structural Steel & Framing Plan |
| 12 | Bearings |
| 13 | North and South Abutments |
| 14 | Piers 1 and 2 |
| 15 | Pile Details |
| 16 | Bar Splicer Assembly Details |
| 17-19 | Soil Borings |

GENERAL PLAN
ILLINOIS 267 OVER
TAYLOR CREEK
STRUCTURE NO. 031-0043



LOCATION SKETCH

DESIGN SCOUR ELEVATION TABLE

| Design Scour Elevation (ft.) | N. Abut. | Pier 1 | Pier 2 | S. Abut. |
|------------------------------|----------|--------|--------|----------|
| | 502.7 | 480.4 | 480.4 | 505.6 |

Johnson, Depp & Quisenberry
CONSULTING ENGINEERS
Springfield, Illinois

DESIGNED: JDQ DRAWN: P. Ray
CHECKED: DCD CHECKED: DCD



Signed: *David Depp*
Date: 10-7-2008
Lic. Expires: 11-30-2008

APPROVED
FOR STRUCTURAL ADEQUACY ONLY

Ralph E. Anderson (P.E.)
ENGINEER OF BRIDGES AND STRUCTURES

WATERWAY INFORMATION

Existing Low Grade Elevation: 506.23 @ Sta. 81+00
Prop. Low Grade Elevation: 506.23 @ Sta. 81+00

| Flood | Freq. Yr. | Q C.F.S. | Opening Sq. Ft. | | Nat. Head - Ft. | | Headwater El. | | |
|-------------|-----------|----------|-----------------|---------|-----------------|-------|---------------|--------|--------|
| | | | Exist. | Prop. | H.W.E. Exist. | Prop. | Exist. | Prop. | |
| Scour | 10 | 4100 | 533.80 | 672.72 | 494.71 | 1.23 | 1.12 | 495.94 | 495.83 |
| Design | 50 | 6600 | 737.35 | 893.34 | 497.18 | 2.67 | 2.51 | 499.85 | 499.69 |
| Base | 100 | 7720 | 806.50 | 969.48 | 497.96 | 3.68 | 3.46 | 501.64 | 501.42 |
| Overtopping | | | | | | | | | |
| Max. Calc. | 500 | 10500 | 972.14 | 1153.75 | 499.73 | 5.54 | 5.33 | 505.27 | 505.06 |

LOADING HL-93
Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS
2007 AASHTO LRFD Bridge Design Specifications

DESIGN STRESSES

FIELD UNITS
f'c = 3,500 psi
fy = 60,000 psi (Reinforcement)
fy = 50,000 psi (M270 Grade 50W)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
Bedrock Acceleration Coefficient (A) = 0.060g
Site Coefficient (S) = 1.0

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