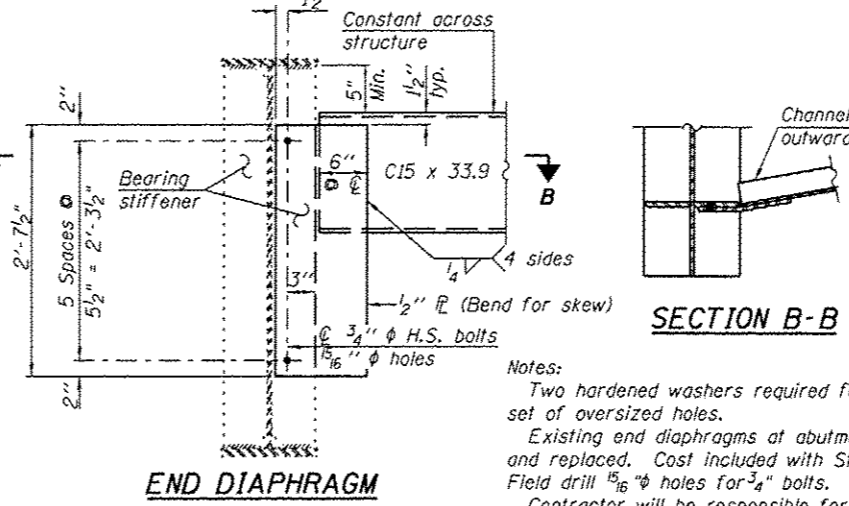
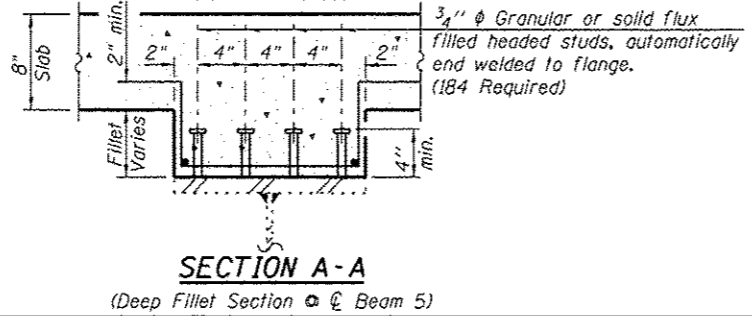
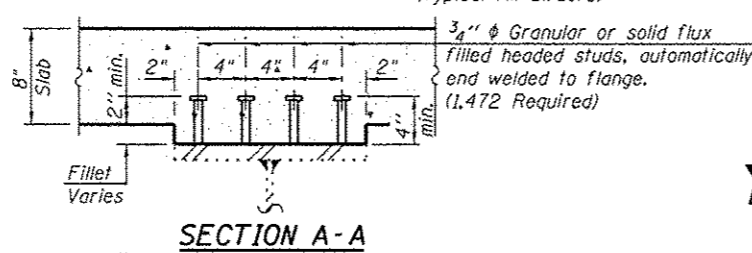
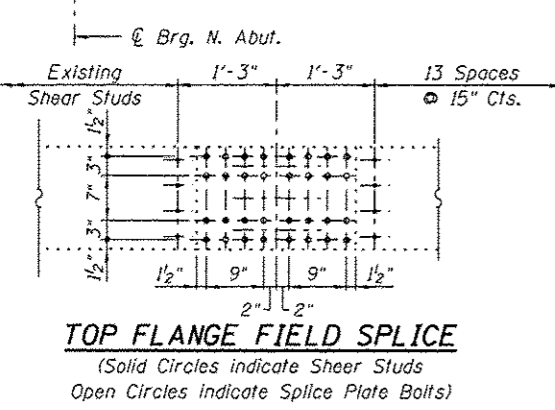
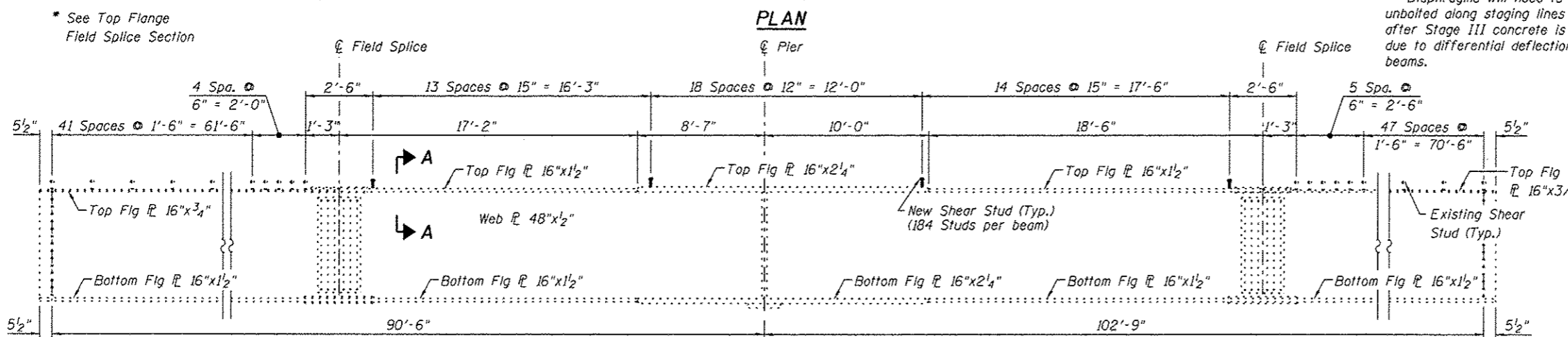


INTERIOR GIRDER MOMENT TABLE			
	0.4 Sp. 1	Pier	0.6 Sp. 2
I_s	(in ⁴) 24,929	50,090	24,929
$I_c(n)$	(in ⁴) 67,413	57,050	67,413
$I_c(3n)$	(in ⁴) 47,912	57,050	47,912
S_s	(in ³) 1218	1908	1218
$S_c(n)$	(in ³) 1659	2000	1659
$S_c(3n)$	(in ³) 1525	2000	1525
R	(k/ft) 1.063	1.185	1.063
M_D	(k) 470	1448	760
s_D	(k/ft) 0.528	0.528	0.528
M_{sD}	(k) 269	676	400
M_L	(k) 789	815	934
M_{Imp}	(k) 183	183	205
$^{5/8} [M_L + M_{Imp}]$	(k) 1620	1663	1898
M_a	(k) 3067	4923	3975
M_u	(k) 5860	5848	5860
$f_s \text{ } \emptyset \text{ non-comp}$	(ksi) 4.63	9.11	7.49
$f_s \text{ } \emptyset \text{ (comp)}$	(ksi) 2.12	4.06	3.15
$f_s \text{ } ^{5/8} [M_L + M_{Imp}]$	(ksi) 11.72	9.98	13.73
$f_s \text{ (Overload)}$	(ksi) 18.47	23.15	24.37
$f_s \text{ (Total)}$	(ksi) -	-	-
VR	(k) 48.1	58.4	59.9

* Compact Section
 ** Braced non-compact and partially braced section

INTERIOR GIRDER REACTION TABLE			
	N. Abut.	Pier	S. Abut.
R_D	(k) 47.6	199.7	59.1
R_L	(k) 44.4	71.2	45.1
Imp.	(k) 10.3	16.5	9.9
R_{Total}	(k) 102.3	287.4	114.1

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads (in⁴ and in³).
 $I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total and Overload) due to short-term composite live loads (in⁴ and in³).
 $I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads (in⁴ and in³).
 \emptyset : Un-factored non-composite dead load (kips/ft.).
 M_D : Un-factored moment due to non-composite dead load (kip-ft.).
 s_D : Un-factored long-term composite (superimposed) dead load (kips/ft.).
 M_{sD} : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).
 M_L : Un-factored live load moment (kip-ft.).
 M_I : Un-factored moment due to impact (kip-ft.).
 M_a : Factored design moment (kip-ft.).
 $1.3 [M_D + M_{sD} + \frac{5}{8} (M_L + M_I)]$
 M_u : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).
 $f_s \text{ (Overload)}$: Sum of stresses as computed from the moments below (ksi).
 $M_D + M_{sD} + \frac{5}{8} (M_L + M_I)$
 $f_s \text{ (Total)}$: Sum of stresses as computed from the moments below on non-compact section (ksi).
 $1.3 [M_D + M_{sD} + \frac{5}{8} (M_L + M_I)]$
 VR: Maximum \emptyset + impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).



Notes:
 Two hardened washers required for each set of oversized holes.
 Existing end diaphragms at abutments shall be removed and replaced. Cost included with Structural Steel Removal. Field drill 15/16" \emptyset holes for 3/4" bolts.
 Contractor will be responsible for checking to see if proposed hole locations conflict with existing holes. In such a case, match existing holes.

BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting	Pound	4,940
Structural Steel	Pound	6,800