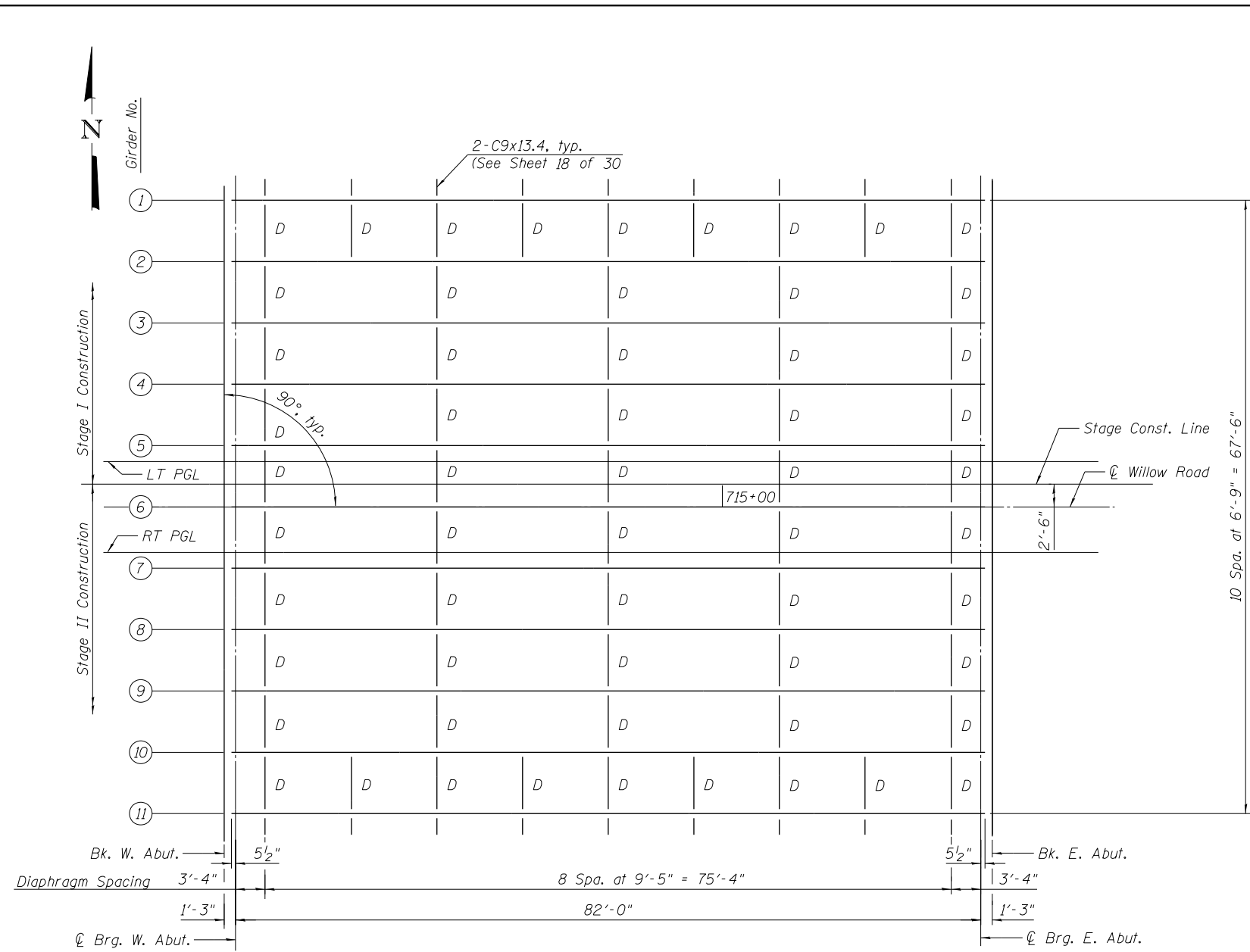
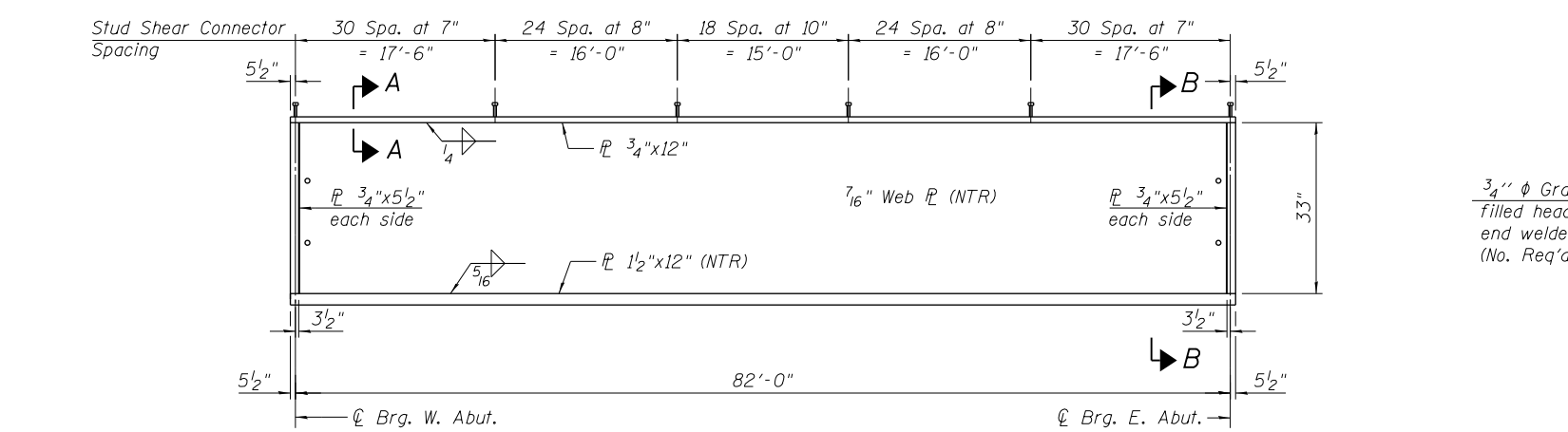


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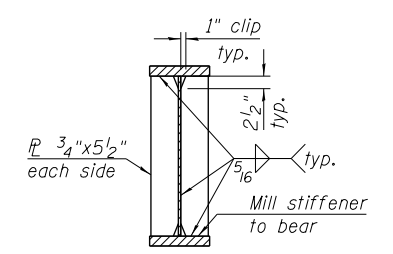
FRAMING PLAN



GIRDER ELEVATION

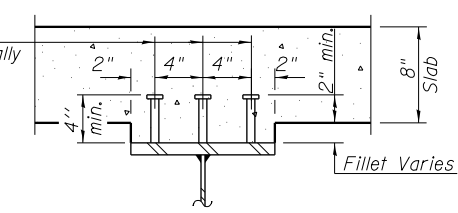
INTERIOR GIRDER MOMENT TABLE		
0.5 Span		
I_s	(in ⁴)	8,626
$I_c(n)$	(in ⁴)	25,551
$I_c(3n)$	(in ⁴)	18,312
S_s	(in ³)	609
$S_c(n)$	(in ³)	849
$S_c(3n)$	(in ³)	784
DC1	(k/')	0.89
M _{DC1}	(k)	745
DC2	(k/')	0.47
M _{DC2}	(k)	398
DW	(k/')	0.20
M _{DW}	(k)	168
M _{ℓ+IM}	(k)	1232
M _u (Strength I)	(k)	3837
$\phi_r M_n$	(k)	4018
f_s DC1	(ksi)	14.7
f_s DC2	(ksi)	6.1
f_s DW	(ksi)	2.6
f_s (ℓ+IM)	(ksi)	17.4
f_s (Service II)	(ksi)	46.0
0.95R _n F _{yr}	(ksi)	47.5
f_s (Total)(Strength I)	(ksi)	-
$\phi_r F_n$	(ksi)	-
V _r	(k)	48.5

INTERIOR GIRDER REACTION TABLE		
Abut.		
R _{DC1}	(k)	37.1
R _{DC2}	(k)	19.4
R _{DW}	(k)	8.2
R _{ℓ+IM}	(k)	80.6
R _{Total}	(k)	145.3



SECTION B-B

3/4" φ Granular or solid flux filled headed studs, automatically end welded to flange. (No. Req'd= 4.191)



SECTION A-A

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in⁴ and in³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{ℓ+IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).
 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{ℓ+IM}$

$\phi_r M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft.).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
 M_{DC1} / S_{nc}

f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
 $M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.

f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
 $M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

f_s (ℓ+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
 $M_{ℓ+IM} / S_c(n)$ or $M_{DW} / S_c(cr)$ as applicable.

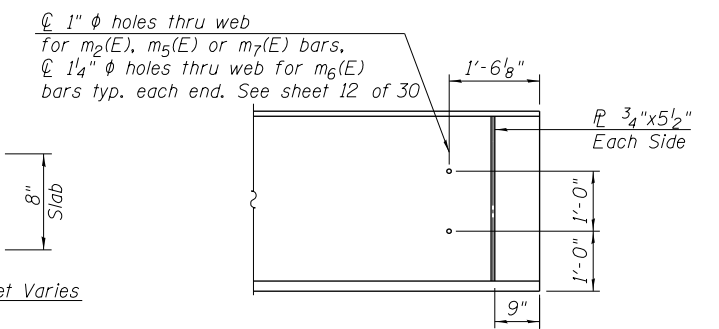
f_s (Service II): Sum of stresses as computed below (ksi).
 $f_{sDC1} + f_{sDC2} + f_{sDW} + 1.3 f_s(\ell + IM)$

0.95R_nF_{yr}: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
 $1.25 (f_{sDC1} + f_{sDC2}) + 1.5 f_{sDW} + 1.75 f_s(\ell + IM)$

$\phi_r F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

V_r: Maximum factored shear range in span computed according to Article 6.10.10.



END OF BEAM ELEVATION

Notes:
 Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
 All plates of girder shall be AASHTO M 270, Grade 50.
 All diaphragms, angles and connecting plates, may be AASHTO M270, Grade 36.