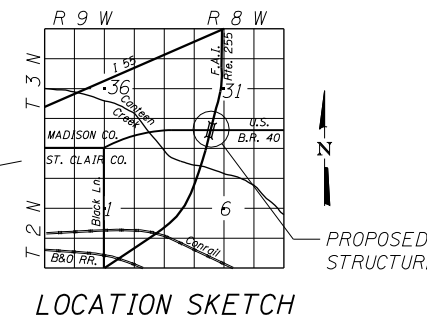
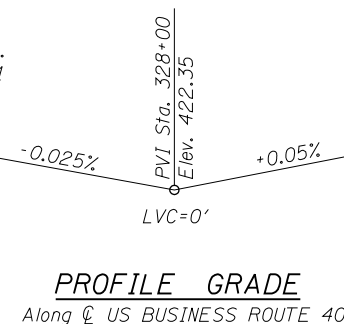
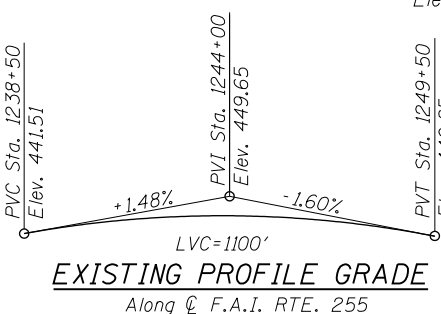
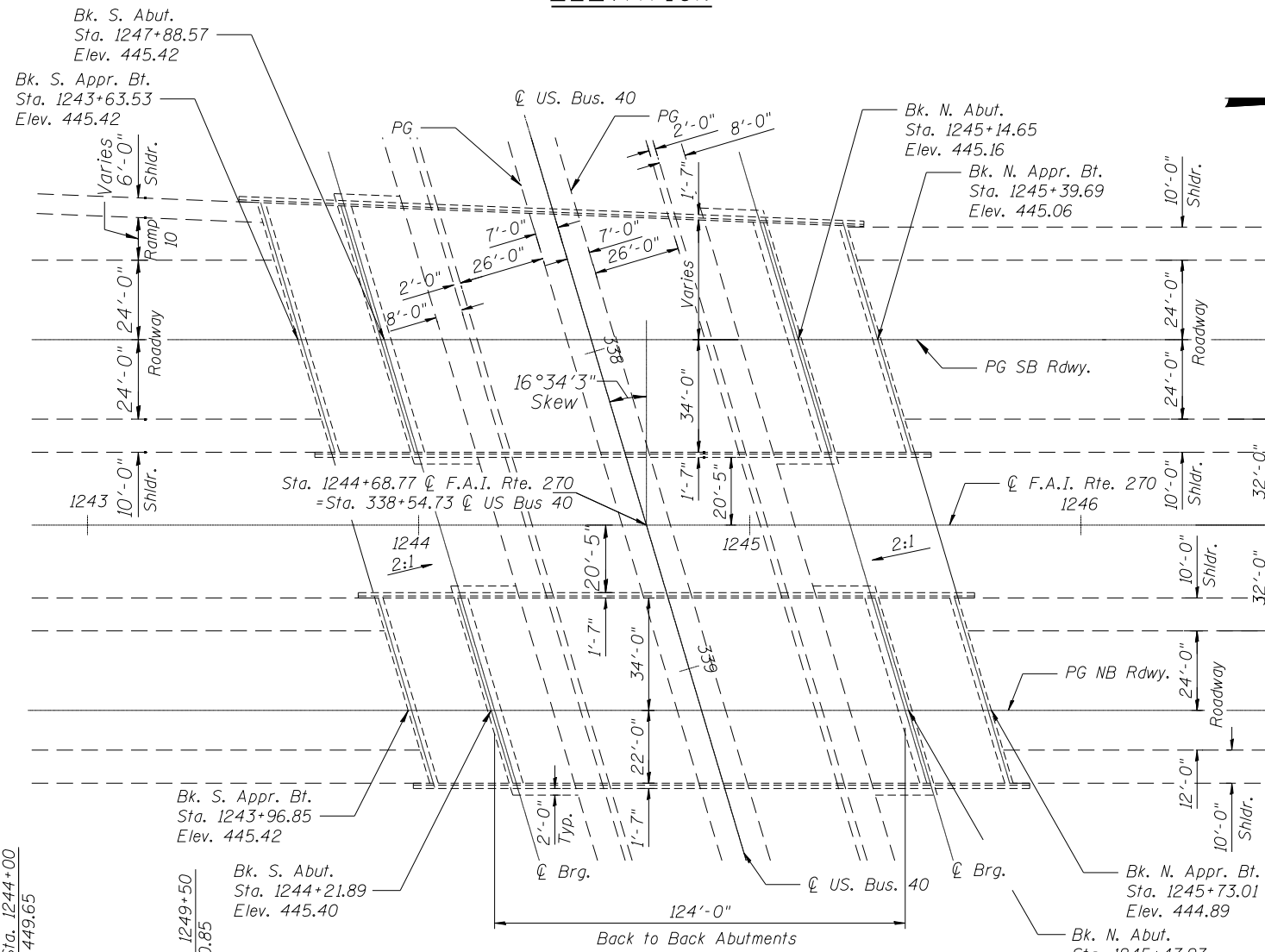
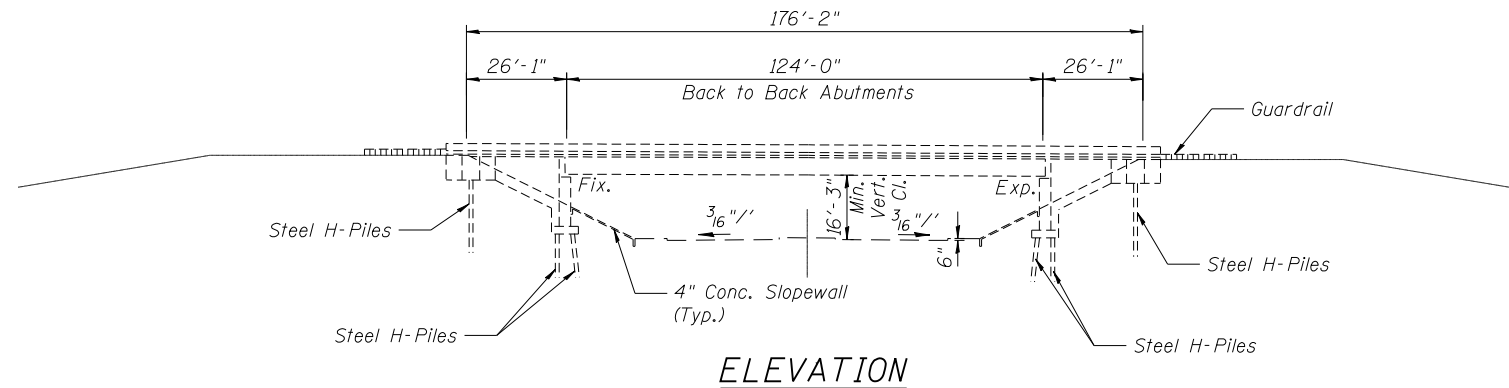


Bench Mark: B.M. 12-20D Nail S.Side Tel. Pole
NE Cor. Arnold St. & USBus 40. Elev. 422.09

Existing Structure: The original structure was constructed in 1986 as FAI Route 255, Section 60-7HB-4. The dual superstructures consist of single span composite welded plate girder bridges with 7 1/2" decks. The decks consist of a 5" slab cast upon 2 1/2" precast pre-stressed panels. The substructures consist of sand-filled vaulted abutments supported on steel piles. The back-to-back of approach slab dimension is 176'-2" and out-to-out of deck dimension measures between 72'-7 1/8" to 77'-7 3/8". The span length is 124' (CI bearing to CI bearing) with a 16°-34'-03" right forward skew. Two lanes of traffic will be maintained in each direction utilizing stage construction.



GENERAL NOTES

1. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding 1/4 in. deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
2. Plan dimensions, elevations and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
3. Joint opening shall be adjusted according to Art. 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50 deg. F.
4. Reinforcement bars designated (E) shall be epoxy coated.
5. The new concrete deck surfaces shall have its final finish tined according to Article 420.09(e)(1).
6. The Contractor shall use extreme care during concrete removal so as not to damage the existing 2 1/2" precast, prestressed stay-in-place forms.

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Stone Riprap, Class A3	Ton	6
Filter Fabric	Sq Yd	14
Polymerized Hot-Mix Asphalt Surface Course, Stone Matrix Asphalt, N80	Ton	228
Concrete Removal	Cu Yd	19.8
Protective Coat	Sq Yd	62
Concrete Superstructure	Cu Yd	21.8
Reinforcement Bars, Epoxy Coated	Pound	3290
Bar Splicers	Each	16
Preformed Joint Strip Seal	Foot	267.0
Waterproofing Membrane System	Sq Yd	2489
Concrete Sealer	Sq Ft	2390
Concrete Joint Sealer	Foot	293
Controlled Low-Strength Material	Cu Yd	2
Deck Slab Repair - (Partial Depth)	Sq Yd	108
Deck Slab Repair - (Full Depth, Type I)	Sq Yd	4

INDEX OF SHEETS

1. General Plan and Elevation
2. Stage Construction N.B.
3. Stage Construction S.B.
4. Temporary Concrete Barrier details for stage construction
5. Deck patching plan N.B.
6. North & South Expansion Joint Concrete Removal and Replacement N.B.
7. Deck Patching Plan S.B.
8. North Expansion Joint Concrete Removal and Replacement S.B.
9. North Expansion Joint Concrete Removal and Replacement S.B.
10. Deck Joint Details
11. Shallow strip seal Details
12. Splicer Assembly and Mechanical Splicer Details
13. Parapet Removal and Replacement
14. Slopewall repairs N.B.
15. Slopewall repairs S.B.

DESIGN SPECIFICATIONS

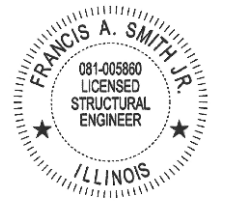
Original Construction
AASHTO 1977 (Service Load Design)
and Applicable Interims, 1978, 79 & 80

LOADING HS20-44

Original Construction
or Alternate Military Loading
Allow 25 p.s.f. for Fut.W.S.

DESIGN STRESSES

Original Construction
f'c = 3,500 psi
fy = 60,000 psi (Reinforcement)
fs = 20,000 psi (M183) & 27,000 psi (M222 & M223 G50 psi (Struct. Steel)
fc = 1,400 psi, fs = 24,000 psi (Substructure)



DATE: 03/09/2012
F. ALLEN SMITH, P.E., S.E.
NO. 081-005860
EXP. DATE 11/30/2012

4/01/23 PM - g:\CHIN\013\Bridges\2020827_south_phase_final\electronic_submittal\FAI_255_60-(7,8)RS-2_CAD_Files\060-0238&0239_0600238-76A89-01-GPE.dgn

TranSystems	USER NAME = sdgnenn	DESIGNED - WAE	REVISED -
	PLOT SCALE = 5/8" = 1' / IN.	CHECKED - FAS	REVISED -
	PLOT DATE = 10/17/2012	DRAWN - DMG	REVISED -
		CHECKED - SLZ	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**GENERAL PLAN AND ELEVATION
STRUCTURE NO. 060-0238 AND 060-0239**

SHEET NO. 1 OF 15 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
255	60-(7,8) RS-2	MADISON	261	196
				CONTRACT NO. 76A89
ILLINOIS FED. AID PROJECT				