

INTERIOR BEAM MOMENT TABLE

| | | 0.4 Span 1 or 0.6 Span 5 | Pier #1 or Pier #4 | 0.5 Span 2 or 0.5 Span 4 | Pier #2 or Pier #3 | 0.5 Span 3 |
|------------------------|--------------------|-----------------------------|-----------------------|-----------------------------|-----------------------|------------|
| Is | (in ⁴) | 7800 | 13200 | 9040 | 16100 | 9040 |
| Ic (n) | (in ⁴) | 18590 | - | 20523 | - | 20523 |
| Ic (3n) | (in ⁴) | 13811 | - | 15259 | - | 15259 |
| Ss | (in ³) | 439 | 720 | 504 | 892 | 504 |
| Sc (n) | (in ³) | 610 | - | 682 | - | 682 |
| Sc (3n) | (in ³) | 554 | - | 621 | - | 621 |
| Q | (K/ft) | 0.77 | 1.18 | 0.79 | 1.22 | 0.79 |
| M @ | (ft-k) | 237.5 | 1092.4 | 399.5 | 1358.2 | 322.7 |
| S @ | (K/ft) | 0.32 | - | 0.32 | - | 0.32 |
| Ms @ | (ft-k) | 113.4 | - | 190.4 | - | 154.5 |
| M L | (ft-k) | 530.4 | 522.4 | 680.4 | 660.6 | 674.6 |
| M (Imp) | (ft-k) | 131.6 | 119.6 | 143.6 | 139.4 | 142.4 |
| 5/8[M L + M (Imp)] | (ft-k) | 1103.3 | 1070.0 | 1373.3 | 1333.3 | 1361.7 |
| Ma | (ft-k) | 1890.5 | 2811.1 | 2552.2 | 3499.0 | 2390.6 |
| Fs @ non-comp k.s.i. | | 6.49 | 18.21 | 9.50 | 18.26 | 7.68 |
| Fs @ (comp) k.s.i. | | 2.46 | - | 3.68 | - | 2.99 |
| Fs 5/8(L + Imp) k.s.i. | | 21.73 | 17.83 | 24.17 | 17.93 | 23.97 |
| Fs (Overload) k.s.i. | | 30.68 | 36.04 | 37.35 | 36.19 | 34.64 |
| Fs (Total) k.s.i. | | 39.88 | 46.85 | 48.56 | 47.05 | 45.03 |
| VR | (k) | 56 | - | 58 | - | 57 |

EXTERIOR BEAM MOMENT TABLE

| | | 0.4 Span 1 or 0.6 Span 5 | Pier #1 or Pier #4 | 0.5 Span 2 or 0.5 Span 4 | Pier #2 or Pier #3 | 0.5 Span 3 |
|------------------------|--------------------|-----------------------------|-----------------------|-----------------------------|-----------------------|------------|
| Is | (in ⁴) | 7800 | 13200 | 9040 | 16100 | 9040 |
| Ic (n) | (in ⁴) | 17885 | - | 19728 | - | 19728 |
| Ic (3n) | (in ⁴) | 13191 | - | 14595 | - | 14595 |
| Ss | (in ³) | 439 | 720 | 504 | 892 | 504 |
| Sc (n) | (in ³) | 602 | - | 674 | - | 674 |
| Sc (3n) | (in ³) | 545 | - | 611 | - | 611 |
| Q | (K/ft) | 0.86 | 1.273 | 0.88 | 1.32 | 0.88 |
| M @ | (ft-k) | 265.1 | 1185.2 | 443.1 | 1469.2 | 357.2 |
| S @ | (K/ft) | 0.32 | - | 0.32 | - | 0.32 |
| Ms @ | (ft-k) | 112.2 | - | 187.5 | - | 151.7 |
| M L | (ft-k) | 520.4 | 531.3 | 664.7 | 672.2 | 659.8 |
| M (Imp) | (ft-k) | 129.6 | 121.7 | 140.3 | 141.8 | 139.2 |
| 5/8[M L + M (Imp)] | (ft-k) | 1083.3 | 1088.3 | 1341.7 | 1356.7 | 1331.7 |
| Ma | (ft-k) | 1898.8 | 2955.6 | 2564.0 | 3673.7 | 2392.8 |
| Fs @ non-comp k.s.i. | | 7.25 | 19.72 | 10.54 | 19.76 | 8.50 |
| Fs @ (comp) k.s.i. | | 2.47 | - | 3.68 | - | 2.98 |
| Fs 5/8(L + Imp) k.s.i. | | 21.58 | 18.15 | 23.88 | 18.23 | 23.70 |
| Fs (Overload) k.s.i. | | 31.30 | 37.87 | 38.10 | 37.99 | 35.20 |
| Fs (Total) k.s.i. | | 40.69 | 49.23 | 49.50 | 49.40 | 45.76 |
| VR | (k) | 49 | - | 51 | - | 51 |

INTERIOR BEAM REACTION TABLE

| | | Abuts. | Piers 1 & 4 | Piers 2 & 3 |
|-----------|-----|--------|-------------|-------------|
| R @ | (k) | 28.7 | 119.5 | 132.4 |
| R L | (k) | 40.0 | 56.8 | 62.3 |
| Imp. | (k) | 10.0 | 13.0 | 13.1 |
| R (Total) | (k) | 78.7 | 189.3 | 207.8 |

Note: Reactions are not factored

EXTERIOR BEAM REACTION TABLE

| | | Abuts. | Piers 1 & 4 | Piers 2 & 3 |
|-----------|-----|--------|-------------|-------------|
| R @ | (k) | 30.9 | 129.1 | 142.7 |
| R L | (k) | 35.1 | 57.3 | 62.8 |
| Imp. | (k) | 8.7 | 13.1 | 13.3 |
| R (Total) | (k) | 74.7 | 199.5 | 218.7 |

Note: Reactions are not factored

Is and Ss are the moment of Inertia and section modulus of the steel section used in computing fs (Total & Overload).

Ic(n) and Sc(n) are the moment of Inertia and section modulus of the composite section used in computing stresses due to Live Load.

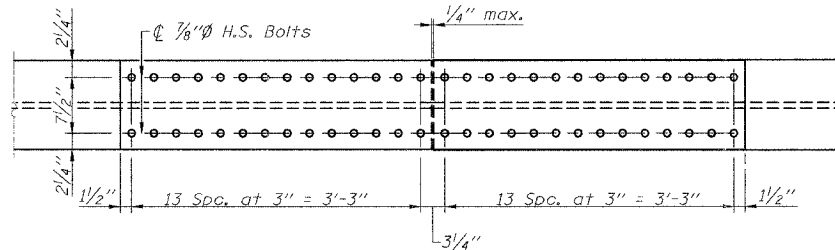
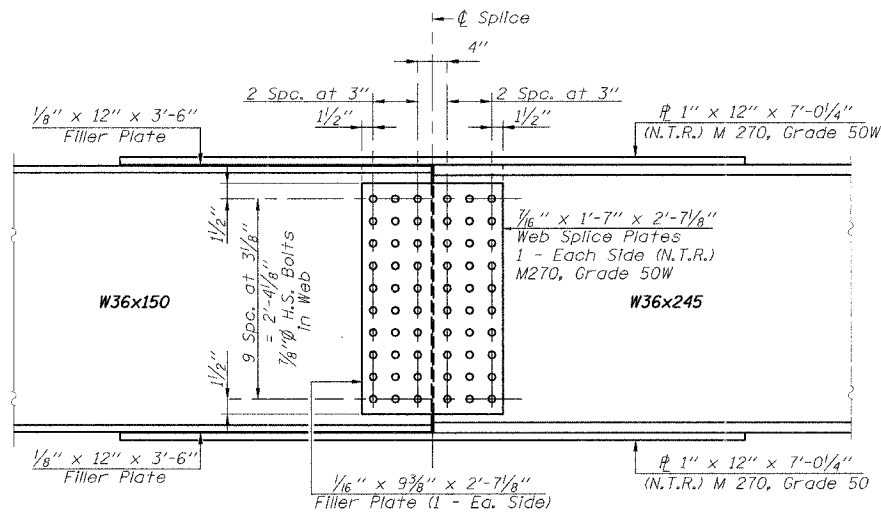
Ic(3n) and Sc(3n) are the moment of inertia and section modulus of the composite section used in computing stresses due to superimposed dead loads. (See AASHTO 10.38).

VR is the maximum Live Load + Impact shear range in span.

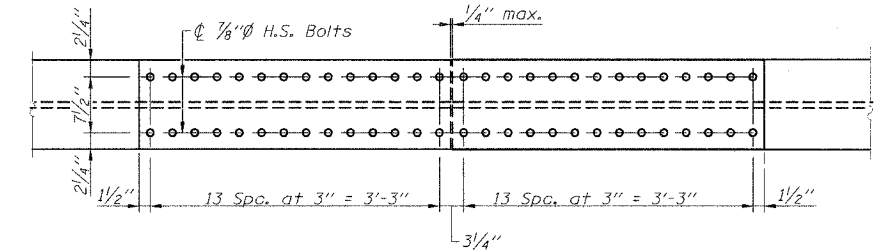
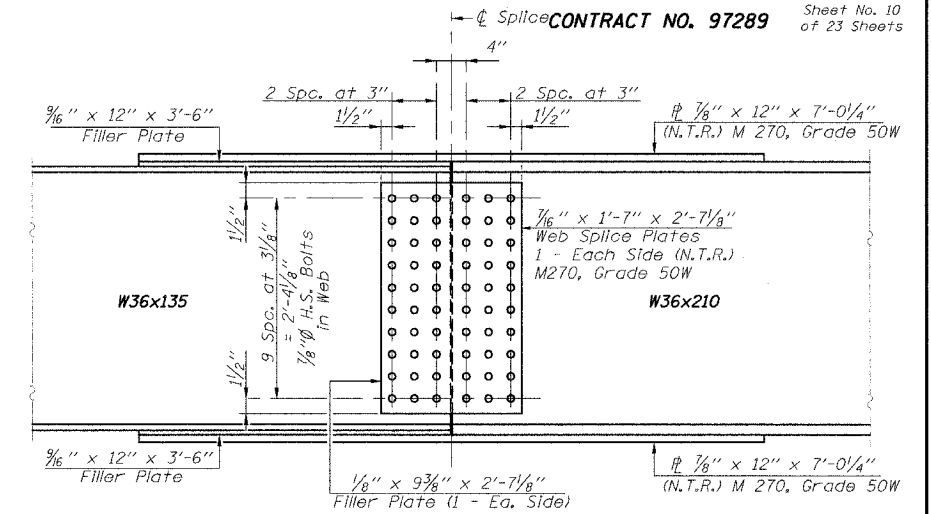
Ma (Applied Moment) = 1.3 [M @ + Ms @ + 5/8(M L + M Imp)]

Fs (Overload) is the sum of the stresses due to M @ + Ms @ + 5/8(M L + M Imp).

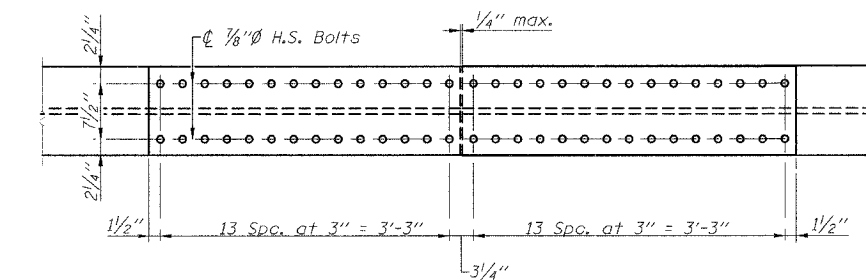
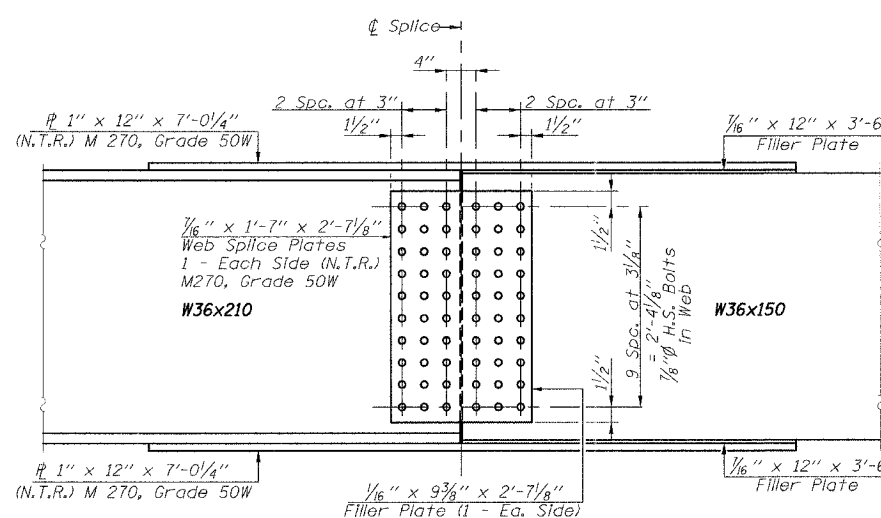
Fs(Total) (Non-compact section) is the sum of the stresses due to 1.3[M @ + Ms @ + 5/8(M L + M Imp)].



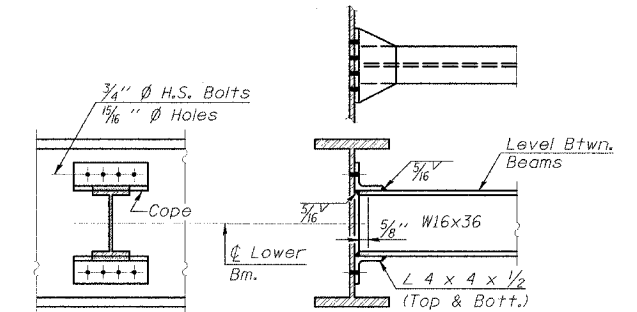
SPLICE NOS. 3, 4, 5 & 6



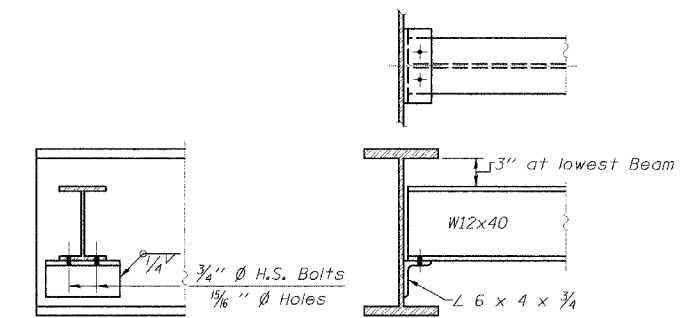
SPLICE NOS. 1 & 8



SPLICE NOS. 2 & 7



DIAPHRAGM D
(100 Required)



DIAPHRAGM D1
(8 Required)

STRUCTURAL STEEL DETAILS
 F.A.S. 731 - C.H. 2
 OVER APPLE CREEK
 SECTION 01-00071-00-BR
 GREENE COUNTY