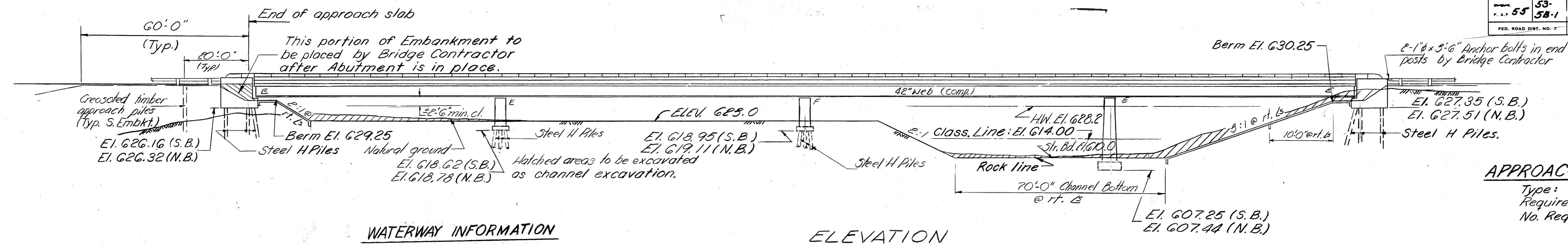


BM: #10 RR Spike in hedge corner post 300' Rt. Sta 188+00 - Elev 624.41

STATE OF ILLINOIS

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 1 18 SHEETS
55	53-5B-1	LIVINGSTON	29	7	
F.A.I. RT. 55	ILLINOIS	FED. AID PROJECT			



**WATERWAY INFORMATION**

Drainage Area 587 Sq. mi.  
Character rolling timber  
Required Opening (50 yr flood) 5100 Sq. Ft.  
Present Opening 59 Sq. Ft.  
Proposed Opening 3100 Sq. Ft.  
Ordinary Water Elev. 614.0  
Low water Elev. 613.0  
Q (50 yr) = 15,700 cfs  
Created head = 0.90'

**ELEVATION**

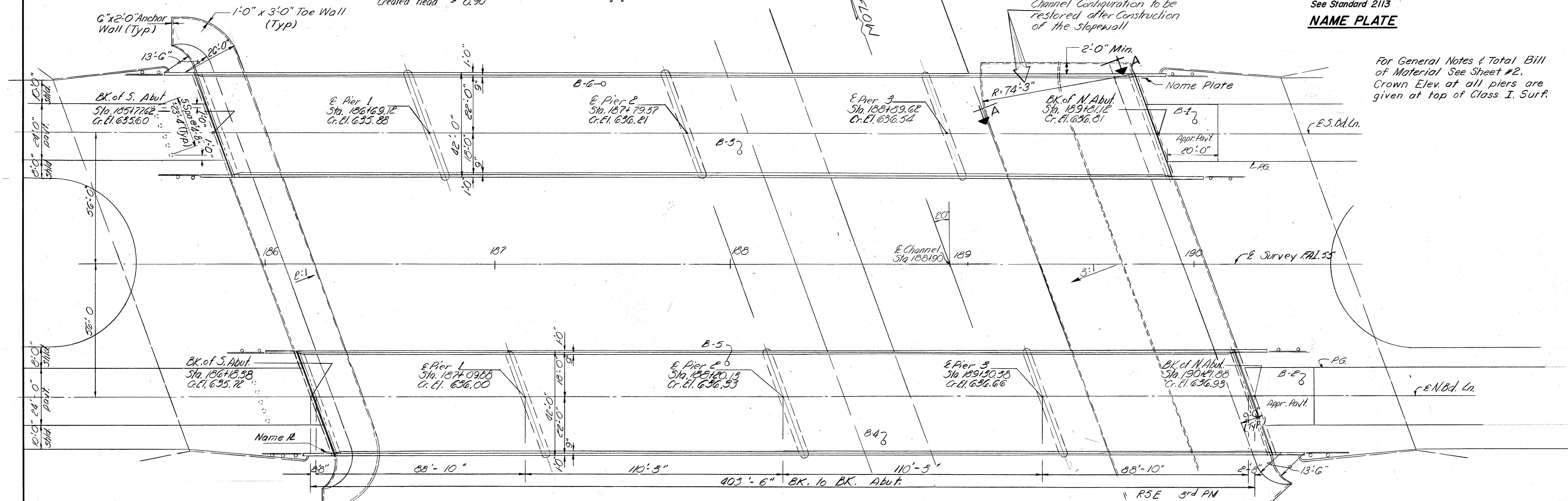
**APPROACH PILE DATA**

Type: Crested  
Required Length: 16'-0"  
No. Required: 12

STATION 188+90  
BUILT 197 BY  
STATE OF ILLINOIS  
F.A.I. RT. 55 SEC. 53-5B-1  
F.A. PROJECT I-55-5 (45)  
LOADING HS20 & ALT.

**NAME PLATE**

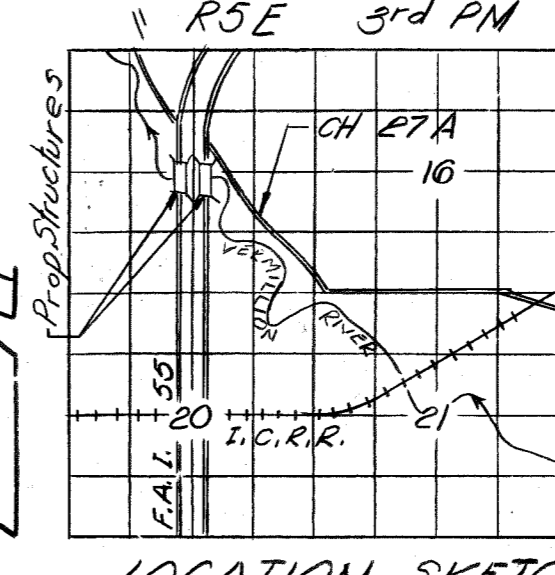
For General Notes & Total Bill of Material See Sheet #2.  
Crown Elev. at all piers are given at top of Class. I. Surf.



**PLAN**

**DESIGN STRESSES**

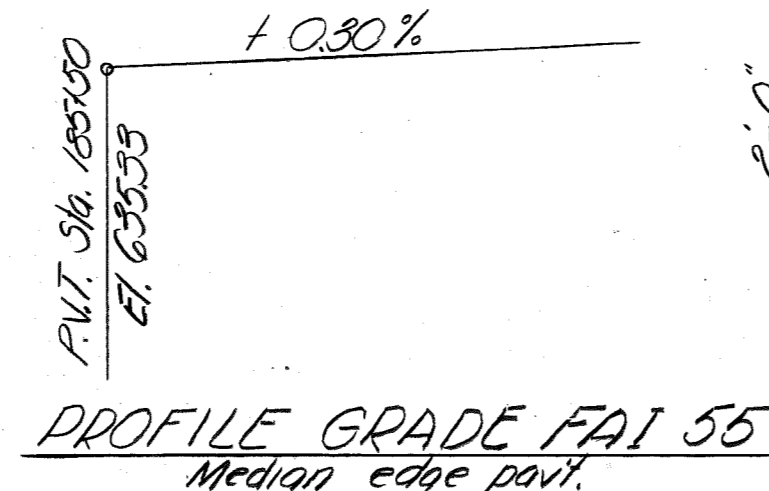
$f_c$  = 1200 psi ( Deck Slab )  
 $f_c$  = 1400 psi ( Curb, Parapet, Sub )  
 $f_s$  = 20000 psi ( Reinf. )  
 $f_s$  = 20000 psi ( Struct. )  
 $V_c$  = 75 psi ( Ftgs. )  
 $n$  = 10  
Allow 25 #/sq. ft. fibre W.S.



PROJECT I-55-5 (45) 195  
GENERAL PLAN & ELEVATION  
F.A.I. RT. 55 OVER VERMILION RIVER  
F.A.I. RT. 55 SEC. 53-5B-1  
LIVINGSTON COUNTY  
STATION 188+90

DESIGNED	Wei Hsiao
CHECKED	R. F. Rodkey
DRAWN	Paul Barnett
CHECKED	R. Rodkey

EXAMINED	June 17 1911
PASSED	
APPROVED	

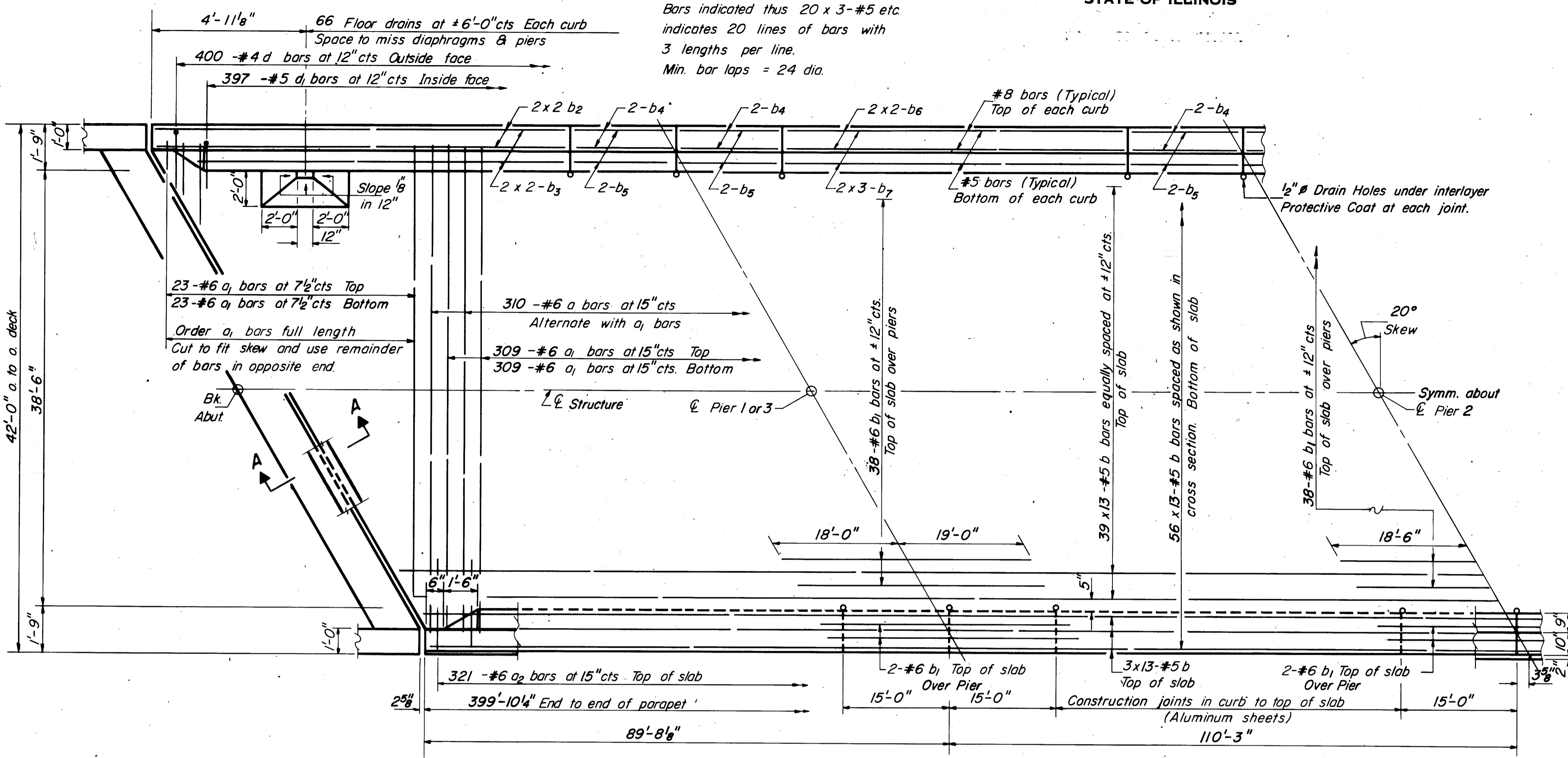


**SEC. A-A**  
(N. Embankment)

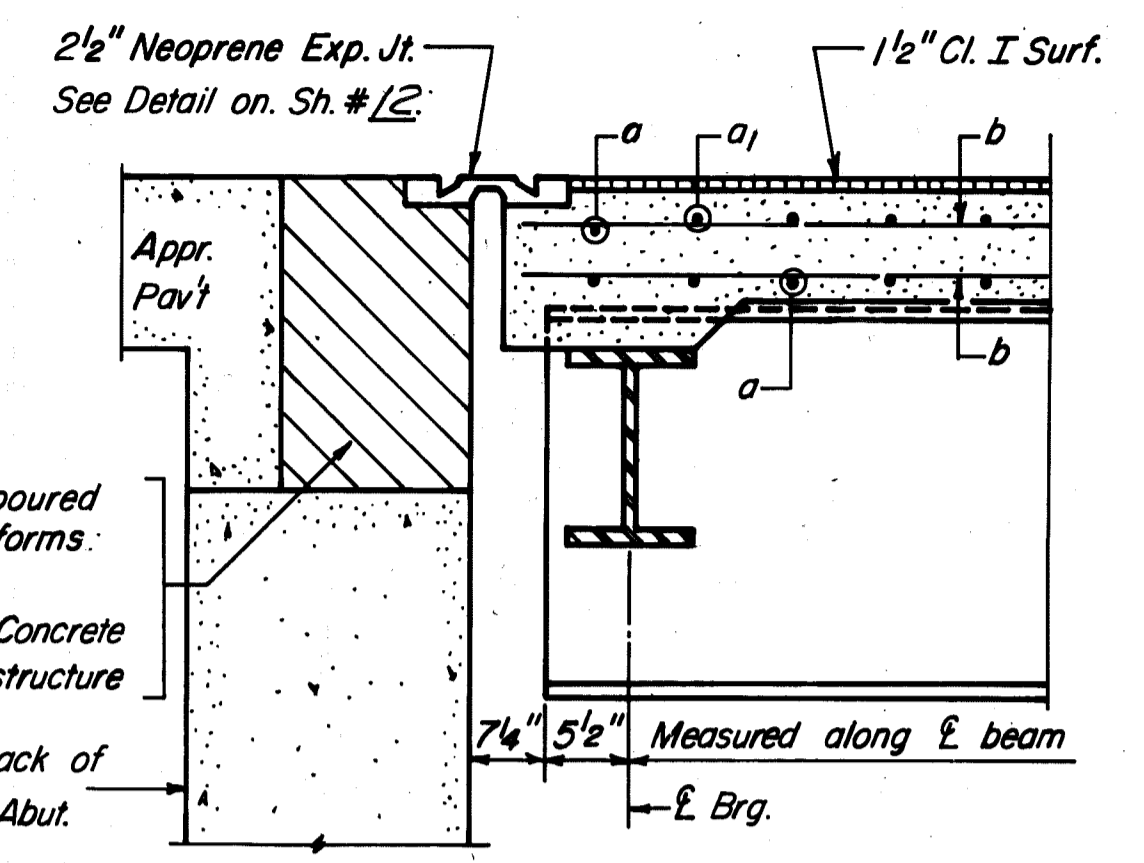
LOADING HS 20-44 & ALT.

**LOCATION SKETCH**

Note:  
Bars indicated thus 20 x 3-#5 etc.  
indicates 20 lines of bars with  
3 lengths per line.  
Min. bar laps = 24 dia.



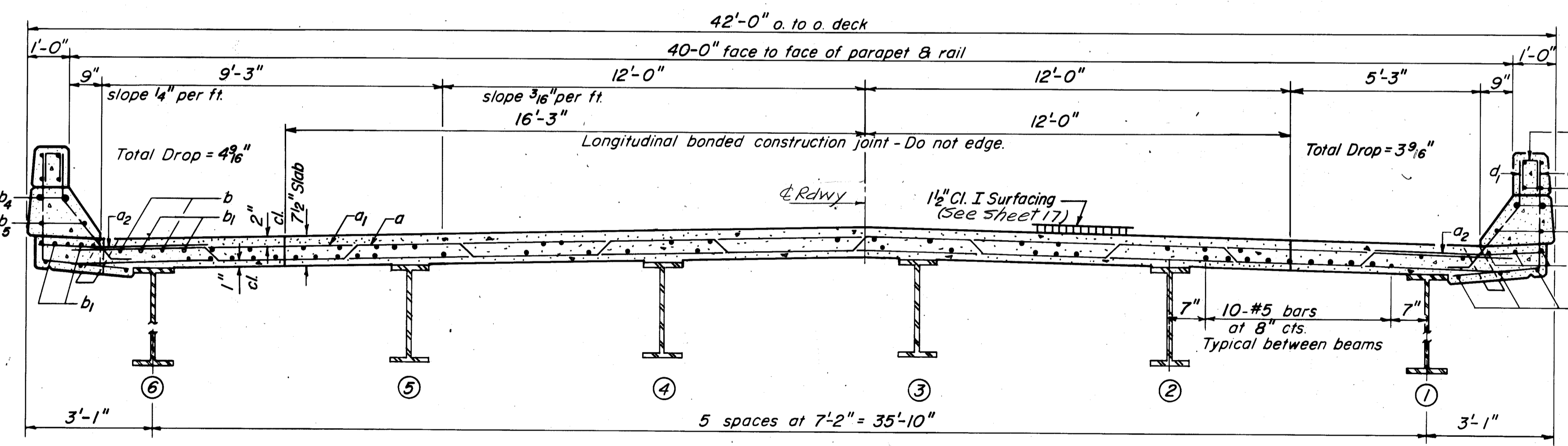
HALF PLAN



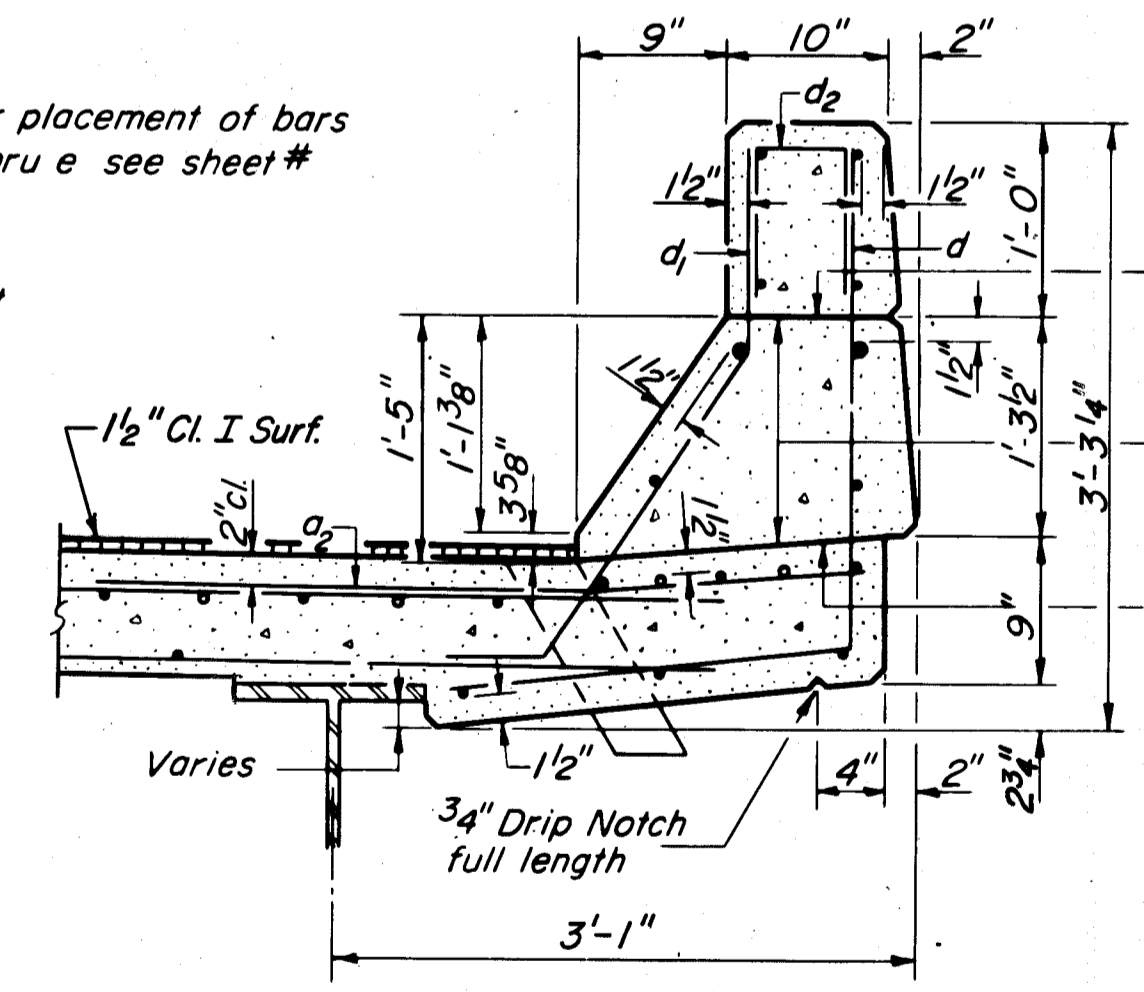
SECTION A-A

TWO BRIDGES  
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a	620	#6	42'-0"	~
a <sub>1</sub>	1328	#6	40'-6"	~
a <sub>2</sub>	1284	#6	4'-0"	~
b	2626	#5	32'-0"	~
b <sub>1</sub>	252	#6	37'-0"	~
b <sub>2</sub>	32	#8	38'-3"	~
b <sub>3</sub>	32	#5	37'-9"	~
b <sub>4</sub>	48	#8	14'-9"	~
b <sub>5</sub>	48	#5	14'-9"	~
b <sub>6</sub>	32	#8	41'-0"	~
b <sub>7</sub>	48	#5	27'-6"	~
d	1600	#4	4'-9"	J
d <sub>1</sub>	1588	#5	3'-9"	J
Reinforcement Bars			Lbs.	252,590
Class X Concrete			Cu. Yds.	927.5



CROSS SECTION  
SBL LOOKING NORTH  
NBL LOOKING SOUTH

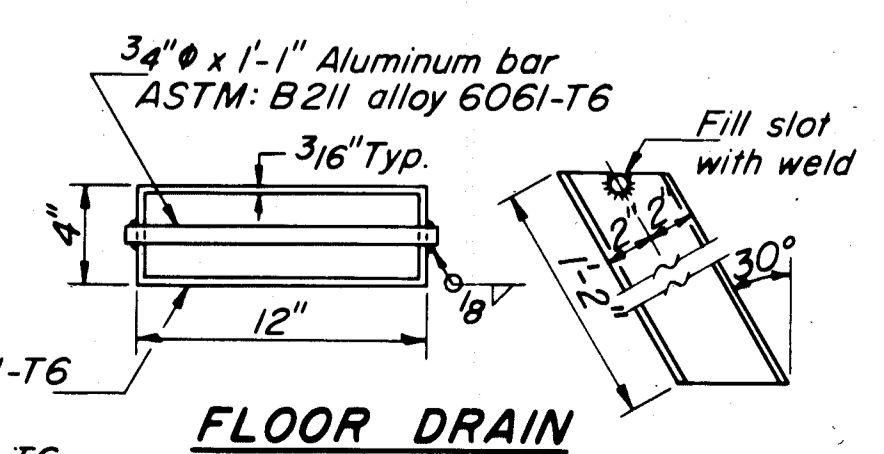
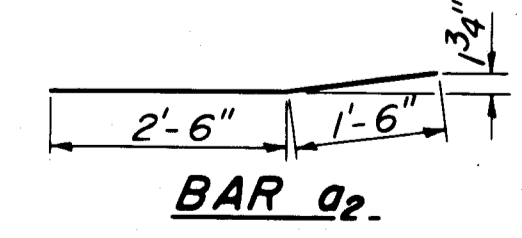
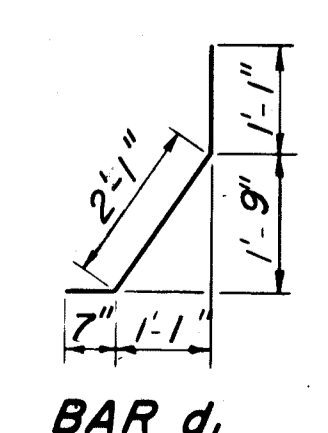
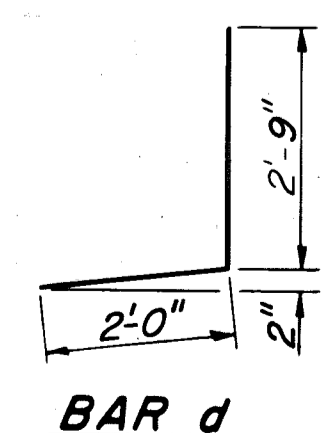
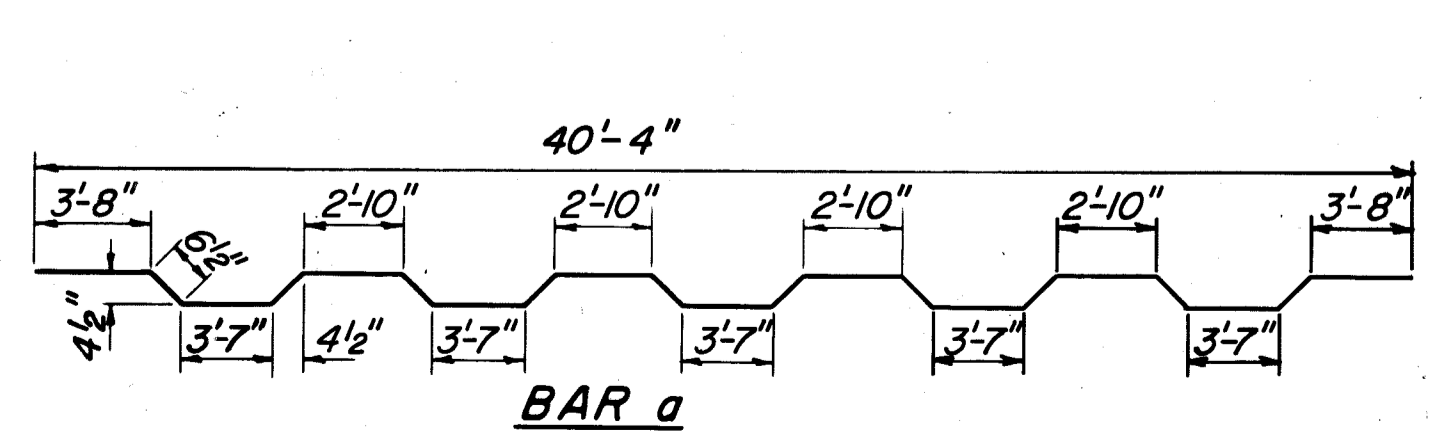


CURB SECTION

Cost of Aluminum Drains and Sheets shall be incidental to Class X Concrete

DESIGNED *Weithorn*  
CHECKED R. F. RODKEY  
DRAWN P. Barnett  
CHECKED R. Rodkey

EXAMINED *[Signature]* June 17 1971  
PASSED  
APPROVED *[Signature]* CHIEF HIGHWAY ENGINEER



FLOOR DRAIN

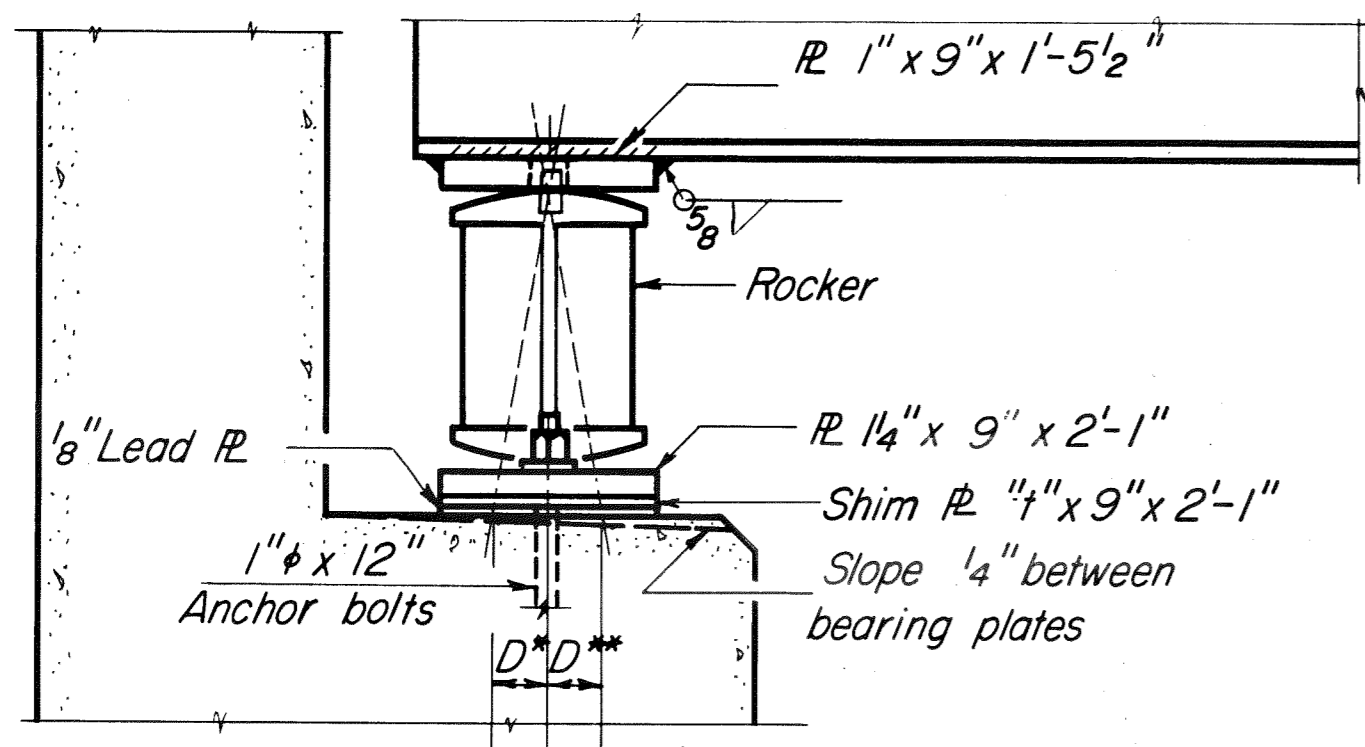
Aluminum Sheets Welded  
ASTM: B 209 alloy 6061-T6  
or Aluminum Extrusions  
ASTM: B 221 alloy 6061-T6

The lengths and quantities of longitudinal reinforcement and Class X Concrete in parapets are not included in above quantities. See sheet

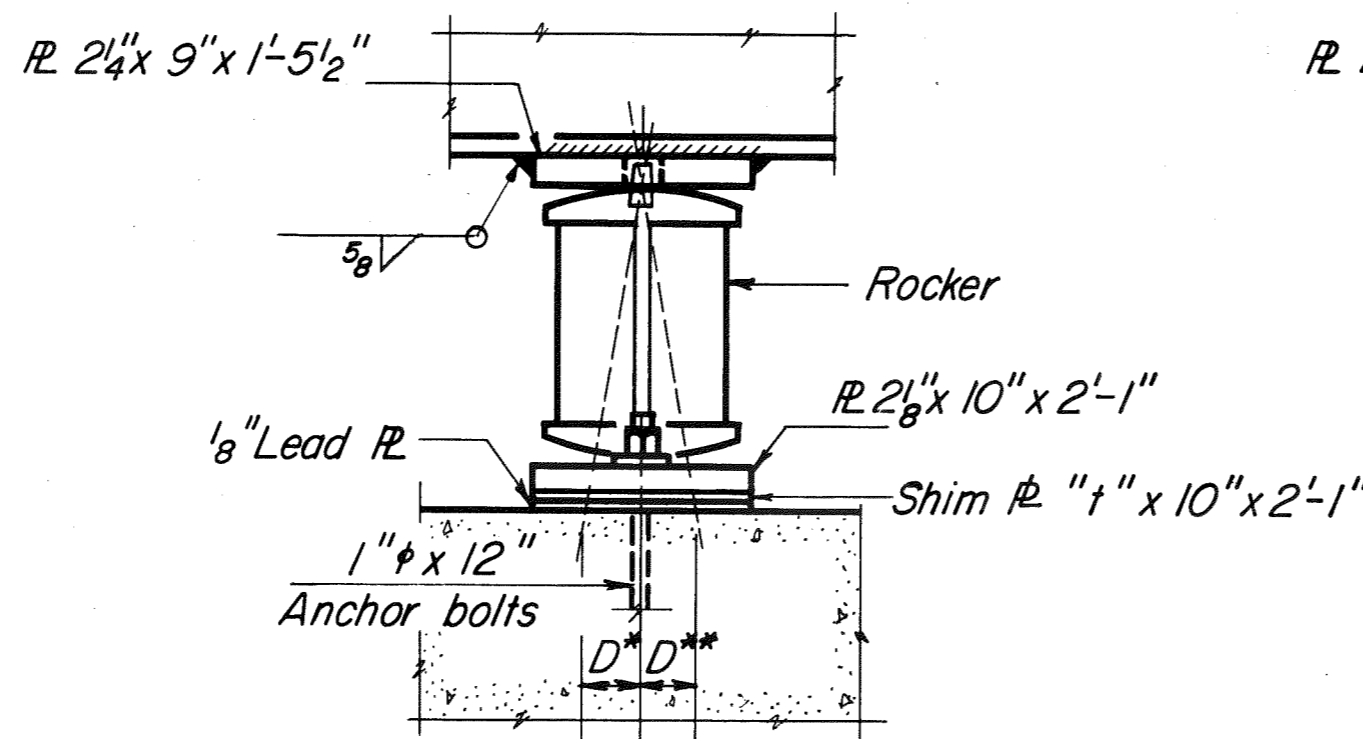
**SUPERSTRUCTURE**  
F.A.I. RT.55 SEC. 53-5B-1  
LIVINGSTON COUNTY  
STA. 188+90



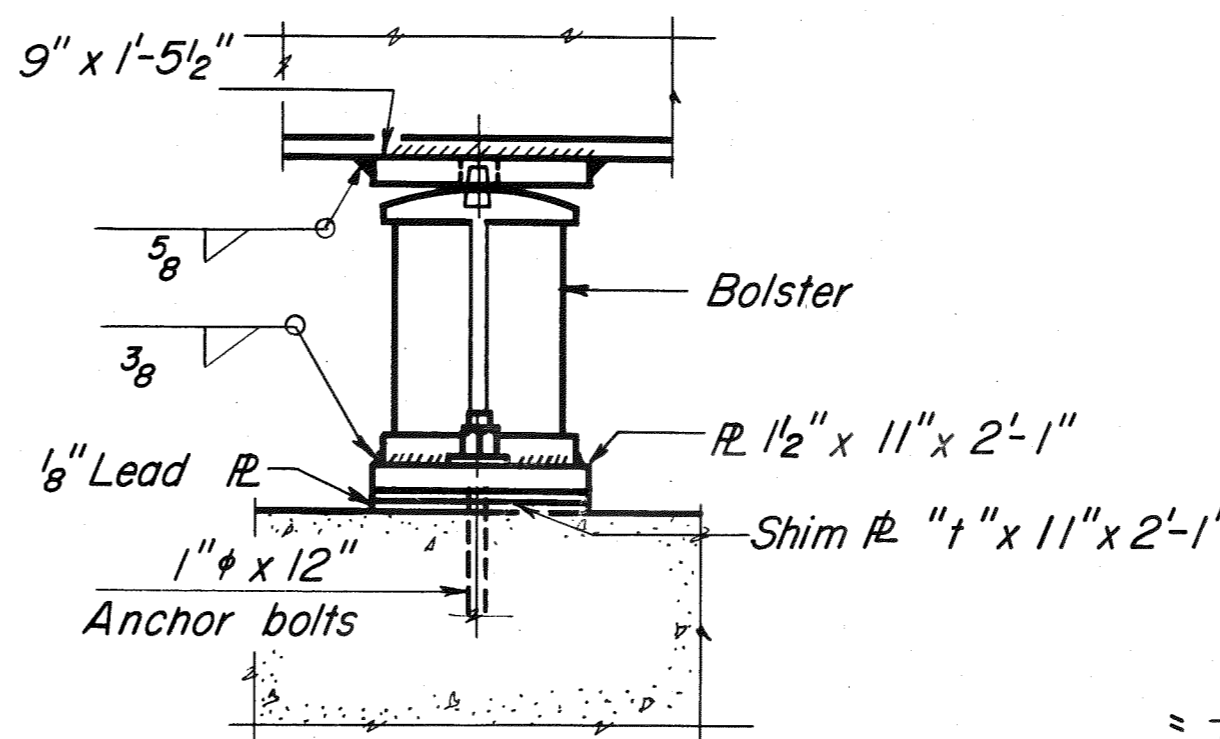




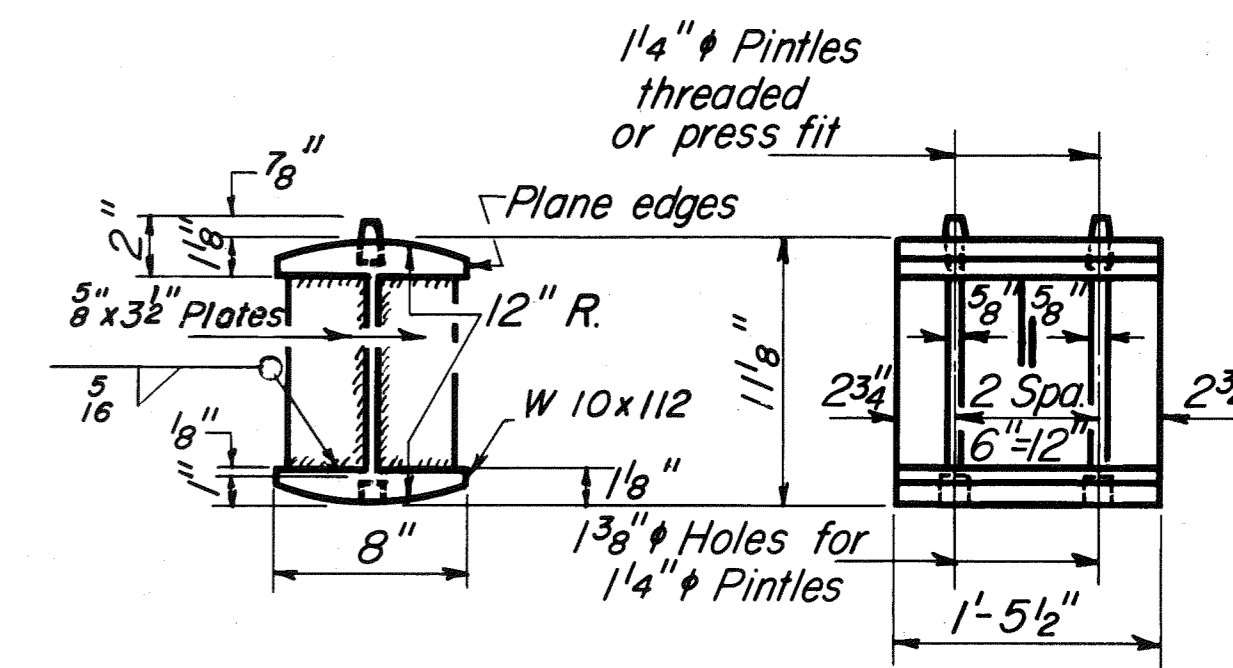
SECTION



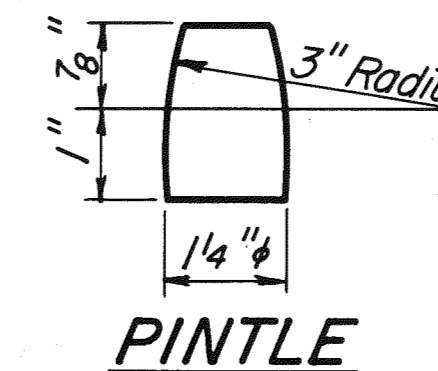
ELEVATION



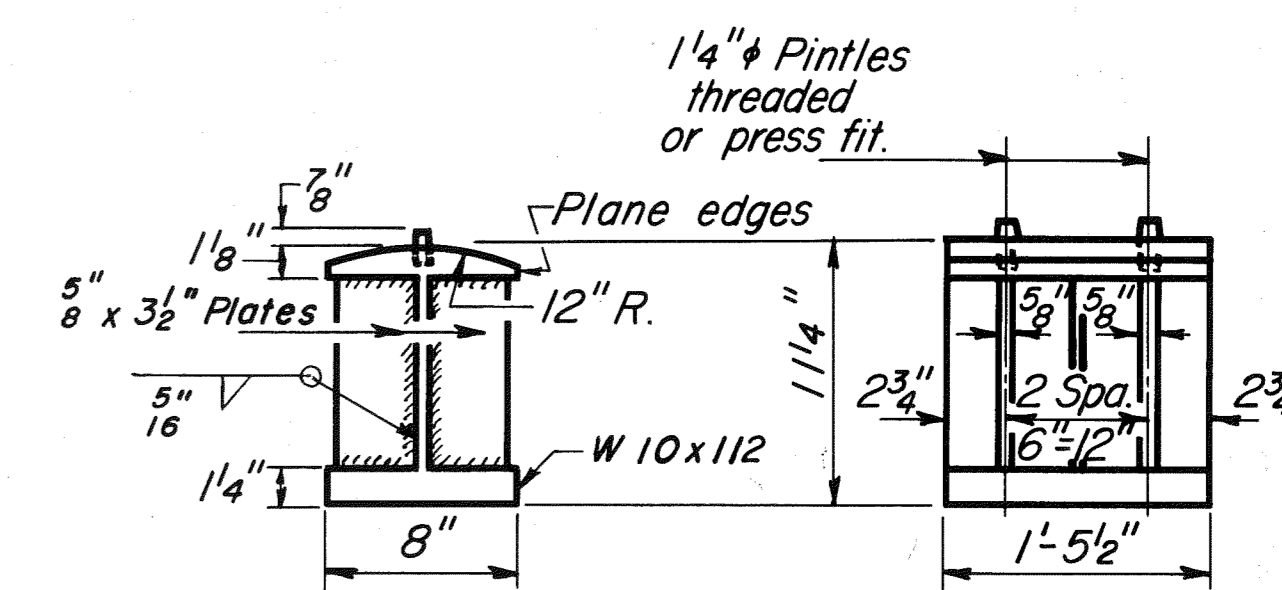
ELEVATION



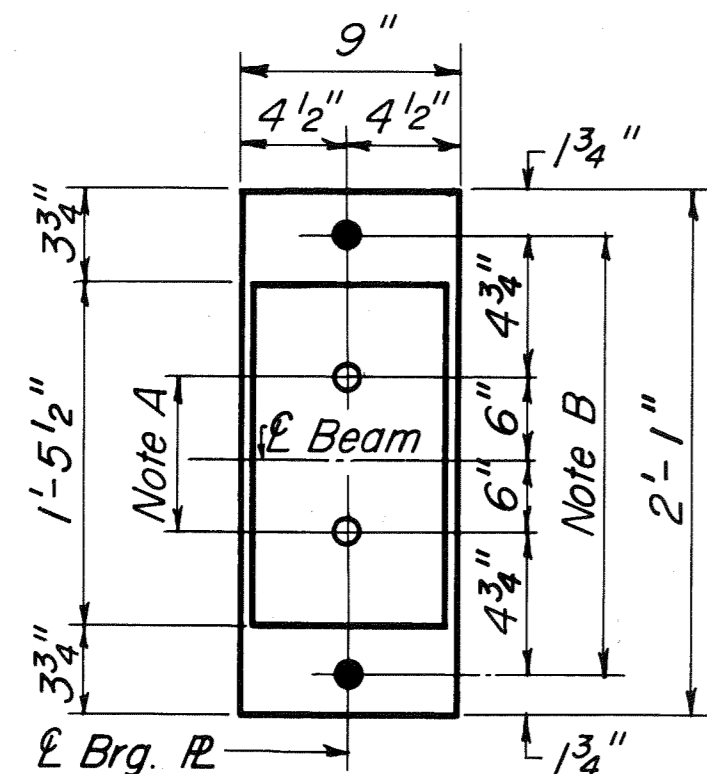
ROCKER



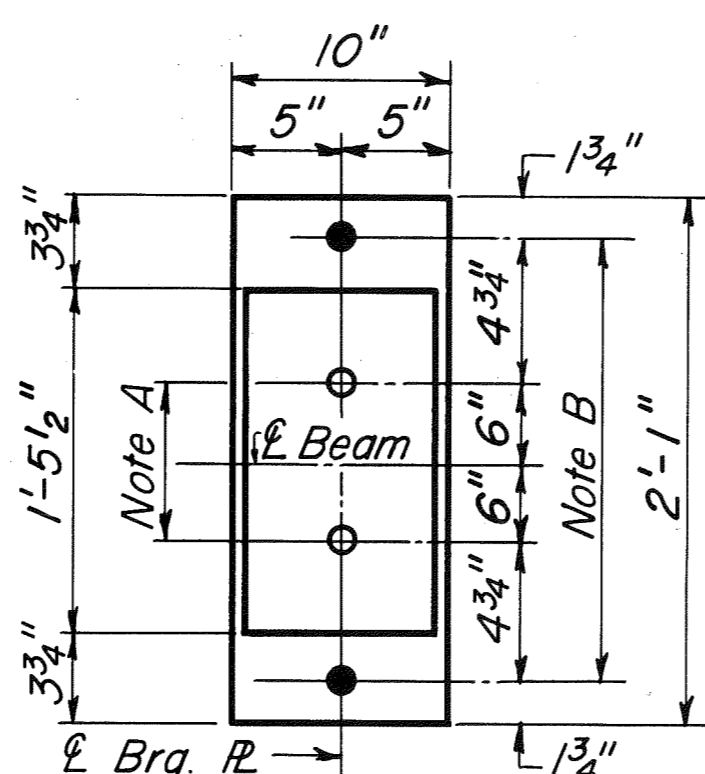
PINTLE



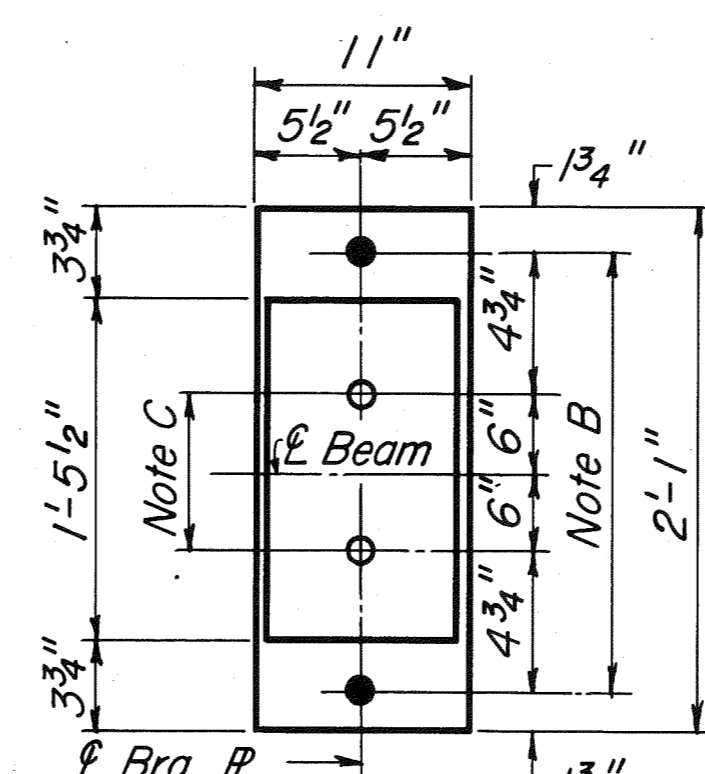
BOLSTER



PLAN  
AT ABUTMENT



PLAN  
AT PIERS 1 & 3



PLAN  
AT PIER 2

**NOTE A**  
1 3/8" Holes - 1" deep in top R.  
for pintles. Thread or press fit  
pintles into bottom R.

**NOTE B**  
1 1/2" Holes for 1" anchor bolts.  
1 1/2" x 2 1/2" x 2 1/2" R. Washers  
under nut.

**NOTE C**  
1 3/8" Holes 1" deep in top R.  
only for 1 1/4" pintles.

**NOTES ON SETTING OF ANCHOR BOLTS AT EXP. BRGS.**

- a)  $D^*$  (Side of brg. away from fixed brg.)  
 $D^* = \frac{1}{8}$ " per each 100' of expansion for every 15° fall below the normal temp. of 50°F.
- $D^{**}$  (Side of brg. toward fixed brg.)  
 $D^{**} = \frac{1}{8}$ " per each 100' of expansion for every 15° rise above the normal temp. of 50°F.
- b) After beams have been erected and dimensions  $D^*$  or  $D^{**}$  determined, holes shall be drilled and anchor bolts shall be grouted in place. All fixed anchor bolts may be built into the masonry.

**BEARING ASSEMBLY DETAILS**

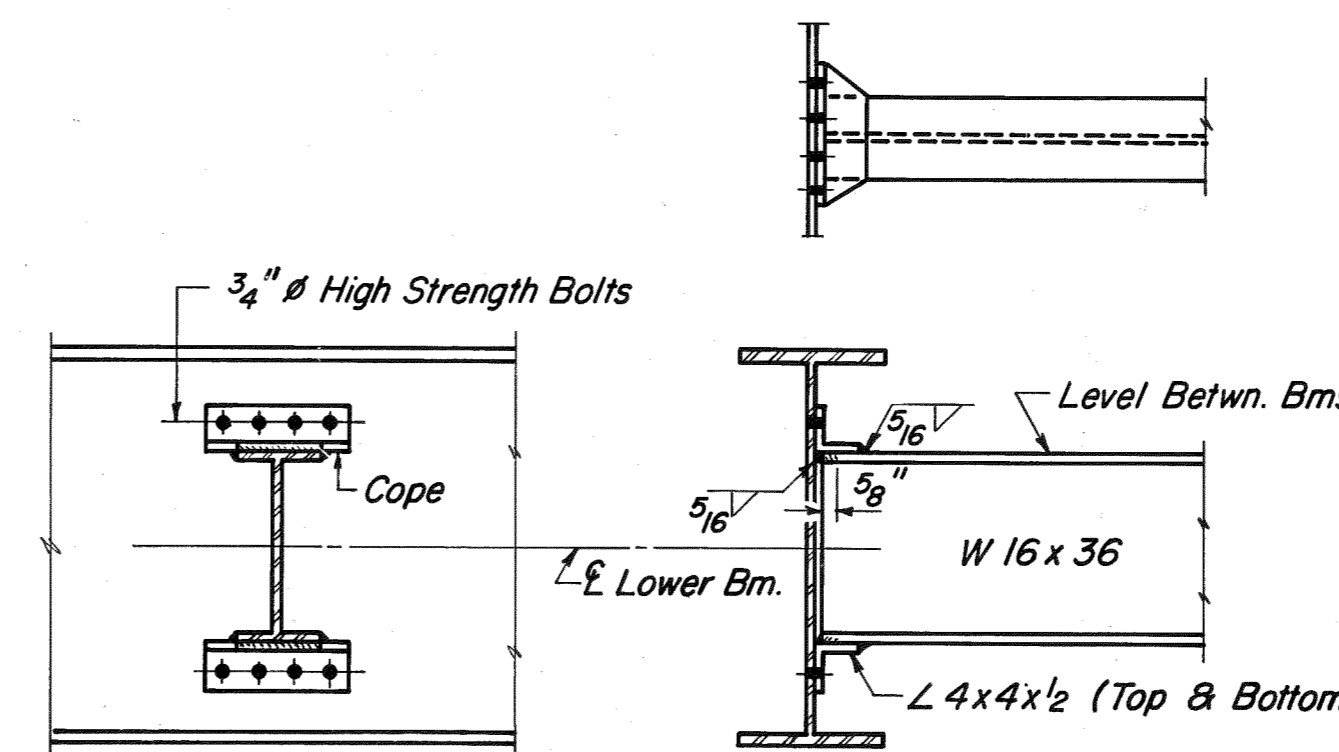
**INTERIOR GIRDER MOMENT TABLE**

	.4 Span 1	Pier 1	.5 Span 2	Pier 2
$I_s$ (in <sup>4</sup> )	15745	29884	15745	34063
$I_c$ (in <sup>4</sup> )	40309		40309	
$S_s$ (in <sup>3</sup> )	836	1314	836	1481
$S_c$ (in <sup>3</sup> )	1136		1136	
$Q$ (K/I)	.89	1.43	.89	1.43
$M_R$ (K)	458	1435	385	1496
$f_s R$ (K.S.I.)	6.6	13.1	5.5	12.1
$s_R$ (K/I)	.54		.54	
$M_s R$ (K)	323		322	
$M_L$ (K)	734	606	764	682
$M_{IMP}$ (K)	172	135	163	145
Total (K)	1229	741	1249	827
$f_s$ Total (K.S.I.)	13.0	6.8	13.2	6.7
$V_r$ (K)	19.6	19.9	18.7	18.8
	46.2		48.1	

$I_s$  and  $S_s$  are the moment of inertia and section modulus of the steel section.  
 $I_c$  and  $S_c$  are the moment of inertia and section modulus of the composite sec. used in computing  $f_s$ .  
 $V_r$  is the maximum L + impact shear range in span.

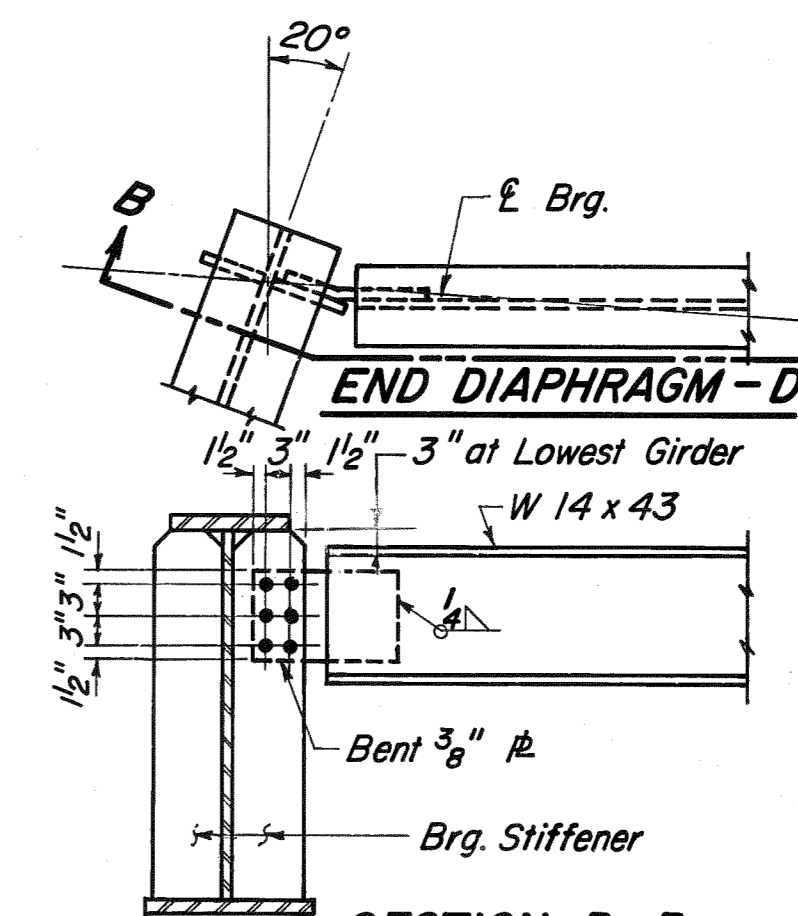
**INTERIOR GIRDER REACTION TABLE**

	Abut.	Pier 1 or 3	Pier 2
$R_R$ (K)	47.4	157.9	158.8
$R_L$ (K)	41.2	65.6	68.2
Imp. (K)	9.6	14.6	14.5
$R$ Total (K)	98.2	238.1	241.5



INTERIOR DIAPHRAGM D1

170 Required



SECTION B-B

**BEARING DETAILS**

F.A.I. RT. 55 SEC. 53-5B-1  
LIVINGSTON COUNTY  
STATION 188 + 90

**SHIM PLATE THICKNESS "t"**

(At All Supports)

Loc.	Bm.	1	2	3	4	5	6
South Bound		0	0	0	1/2"	3/8"	0
North Bound		1/2"	1/2"	5/8"	0	0	0

DESIGNED *Wei Hsueh*  
CHECKED R. F. ROEKEY  
DRAWN P.G. Barnett  
CHECKED R. Redkey

EXAMINED *R. F. Redkey* June 17 1971  
PASSED  
APPROVED  
ENGINEER OF DESIGN  
CHIEF HIGHWAY ENGINEER