

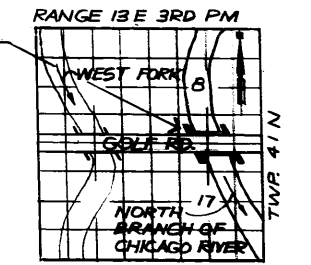
TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
REMOVAL OF EXISTING SUPERSTRUCTURES	EACH	1		1
CONCRETE REMOVAL	CU. YD.		28.8	28.8
EXPANSION BOLTS 3/4 INCH	EACH		348	348
STRUCTURE EXCAVATION	CU. YD.		55	55
FLOOR DRAINS	EACH	8		8
PROTECTIVE COAT	SG. YD.	513		513
CLASS X CONCRETE	CU. YD.	166.4	45.5	211.9
FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE I-BEAMS, 36 IN.	LIN. FT.	634		634
ALUMINUM RAILING, TYPE L	LIN. FT.	79		79
REINFORCEMENT BARS	POUND	15,170	4,250	19,420
REINFORCEMENT BARS (EPOXY COATED)	POUND	20,360		20,360
NAME PLATES	EACH			1
NEOPRENE EXPANSION JOINT 2"	LIN. FT.	58		58
REPAIR CONCRETE STRUCTURES	SG. FT.		22	22
EPOXY CRACK SEALING	LIN. FT.		112	112
ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	16		16

BRIDGE NO. 016-0356

Proposed New Super Structure Station 199+68.00. Concrete Deck Slab with Precast I-Beams on Existing Piers and Abutments, Two Simple Spans, 25° Skew, to Replace Existing Super Structure. Repair of Substructure is Included.

EXISTING BRIDGE
Two-span Conc. T-Beam Bridge built in 1928 - Section 584 B 56'-8" o.a.o.; 80'-7" bk.to bk. Abuts.



WATERWAY INFORMATION

Drainage Area 92.2^{sq mi} Low Grade Elev 624.2 @ Stat 201+00

Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft. Existing Prop.	Net H.W. EL.	Head - Ft. Existing Prop.	Head - Ft. Existing Prop.	Head - Ft. Existing Prop.
Design	50	1255	674 726	619.6	0	0	619.6 619.6
Base	100	1430	721 773	620.3	0	0	620.3 620.3
Max. Calc.	500	1840	782	621.3	0	0	621.3

DESIGN DATA

LOADING
HS 20-44 Plus 2.5psf Future Wearing Surface
Design Specifications - AASHTO 1977, Interim Specs. 1978-1979-1980
Load Factor Design - Superstructure

DESIGN STRESSES
 $f_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinf. bars)
 $f_c = 1,400$ psi
 $f_s = 24,000$ psi } substructure
 $n = 9$

Precast Prestressed Units
 $f_s = 270,000$ psi (Prestressing Steel)
 $f_{si} = 189,000$ psi
 $f_c = 5,000$ psi
 $f_{ci} = 4,000$ psi
 $p_i = 26,300$ lbs per strand

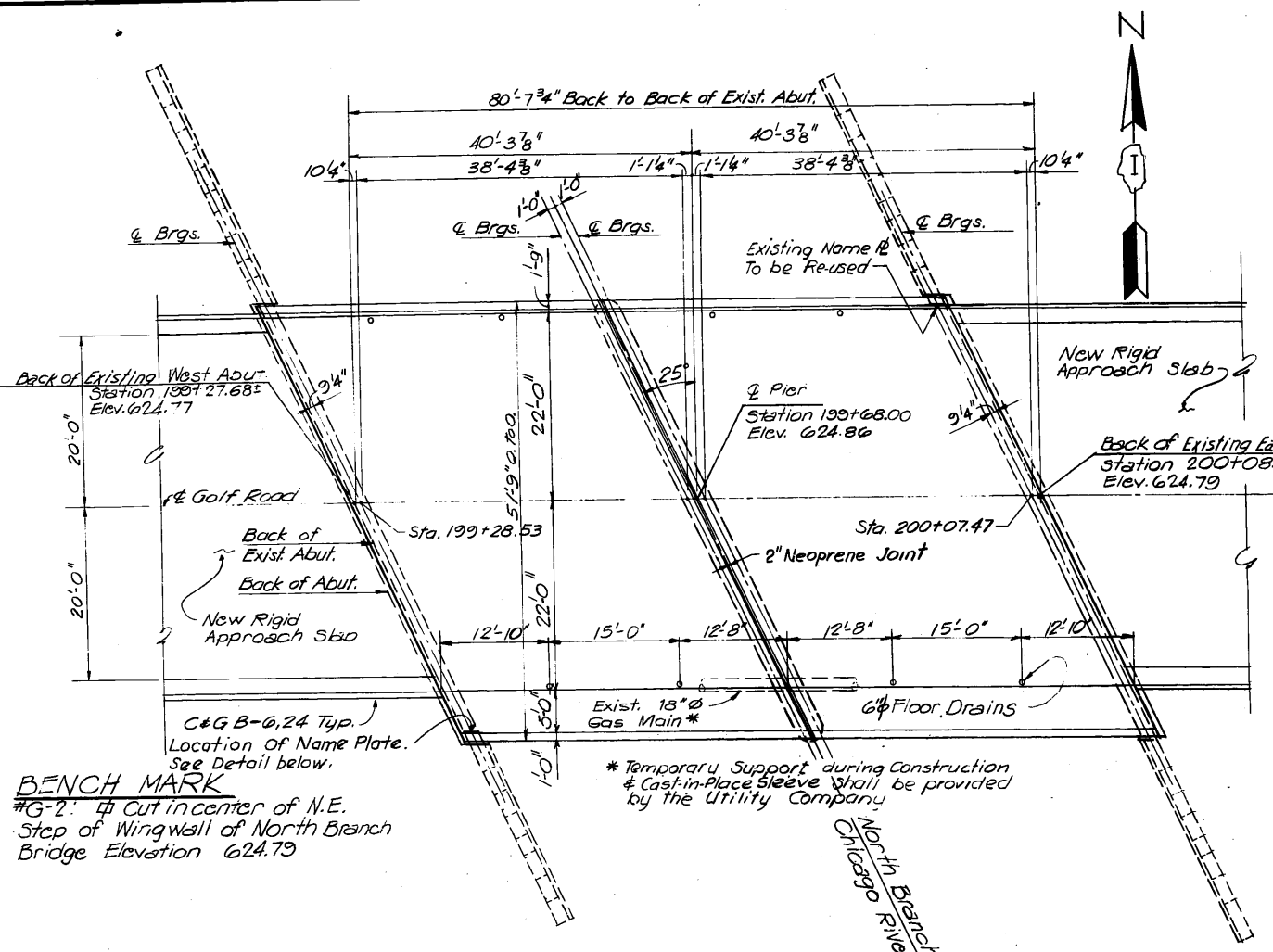
GENERAL NOTES

PLAN DIMENSIONS AND DETAILS RELATIVE TO EXISTING STRUCTURE HAVE BEEN TAKEN FROM EXISTING PLANS AND ARE SUBJECT TO NOMINAL CONSTRUCTION VARIATIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY SUCH DIMENSIONS AND DETAILS IN THE FIELD AND MAKE NECESSARY APPROVED ADJUSTMENTS PRIOR TO CONSTRUCTION OR ORDERING OF MATERIALS. SUCH VARIATIONS SHALL NOT BE CAUSE FOR ADDITIONAL COMPENSATION FOR A CHANGE IN THE SCOPE OF THE WORK, HOWEVER, THE CONTRACTOR WILL BE PAID FOR THE QUANTITY ACTUALLY FURNISHED AT THE UNIT PRICE BID FOR THE WORK.

EXPANSION BOLTS SHALL CONSIST OF APPROVED EXPANSION ANCHORS, PROVIDING MINIMUM CERTIFIED PROOF LOAD = 4,080 LBS., AND 3/4" ϕ x 12" HOOKED BOLTS.

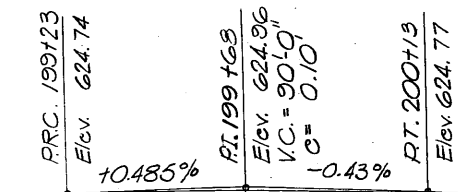
REINFORCEMENT BARS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-31 or M-53 GRADE 60.

THE BACK FACE OF NEW ABUTMENT PORTION SHALL BE WATERPROOFED ACCORDING TO ARTICLE 503.11 OF STANDARD SPECIFICATIONS.



BENCH MARK
#G-2: 4" Cut in center of N.E. Step of Wingwall of North Branch Bridge Elevation 624.79

* Temporary Support during Construction & Cast-in-Place Sleeve shall be provided by the Utility Company



STATION 199+68.00
BUILT 198 BY
STATE OF ILLINOIS
FAP ROUTE 557 SECTION 1977-119-W & RS
FA PROJECT IX-557 ()
LOADING HS 20
STRUCTURE NO. 016-0356

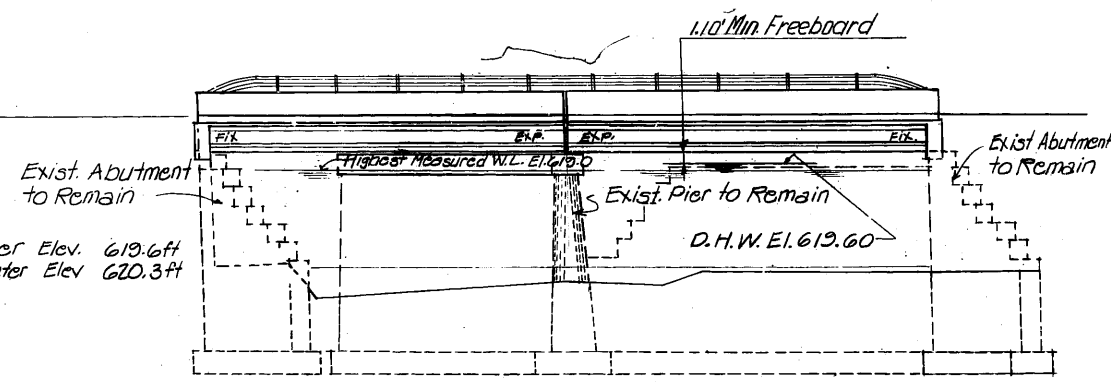
NAME PLATE
(SEE STD. 2113)

"I CERTIFY THAT TO THE BEST OF MY KNOWLEDGE, INFORMATION AND BELIEF, THIS BRIDGE DESIGN IS STRUCTURALLY ADEQUATE FOR THE DESIGN LOADING SHOWN ON THE PLANS. THE DESIGN IS AN ENGINEERING ONE FOR THE STYLE OF STRUCTURE AND COMPLIES WITH REQUIREMENTS OF THE CURRENT AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES."

APPROVED
FOR STRUCTURAL ADEQUACY ONLY
[Signature]
Engineer of Bridges and Structures



Zevell Berman
ZEVELL BERMAN
REGISTERED STRUCTURAL ENGINEER
STATE OF ILLINOIS NO. 2092
DATE FEB. 1, 1983



SOUTH ELEVATION

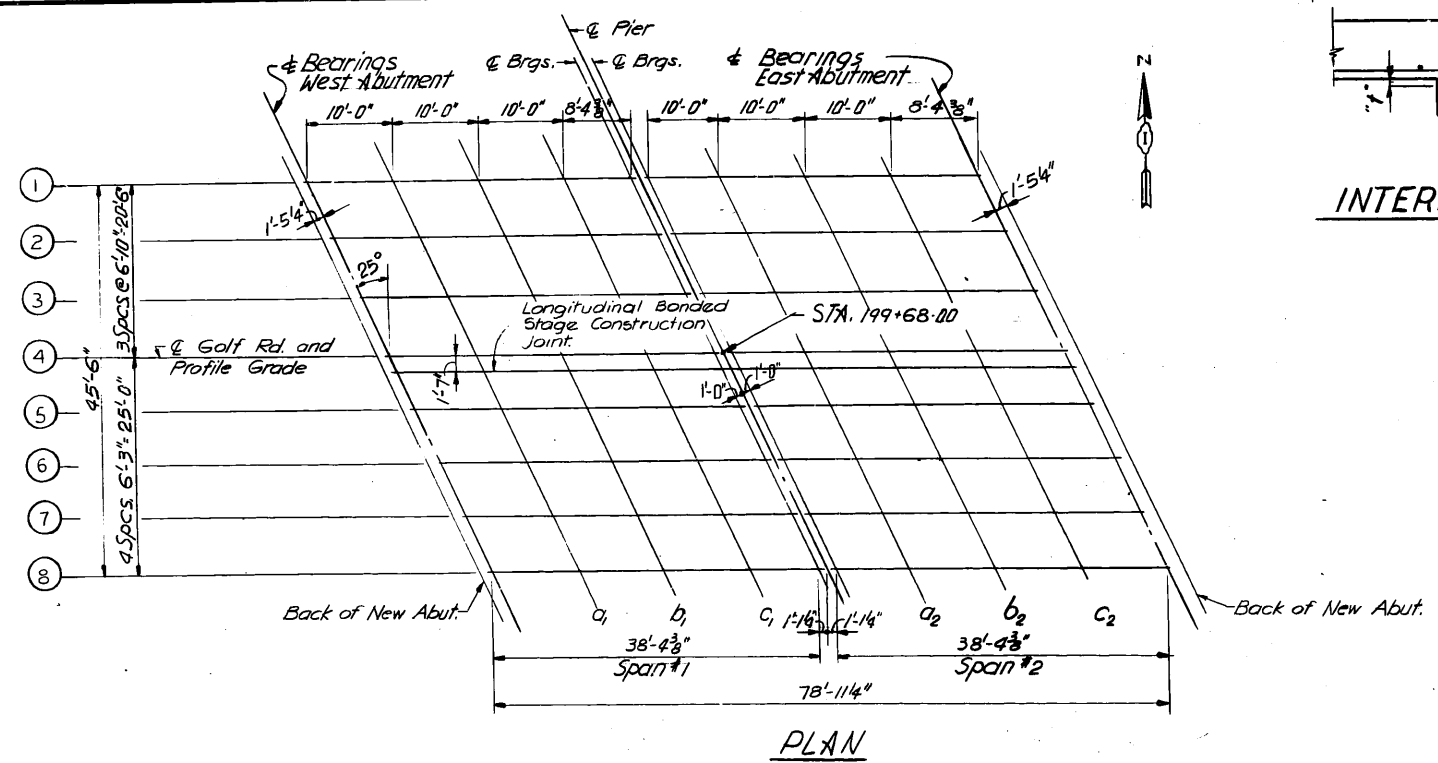
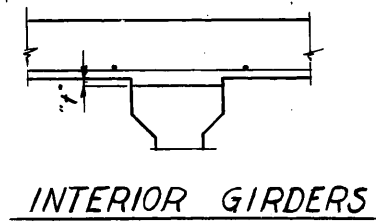
DESIGNED BY *R.K.*
DRAWN BY *V.K.*
CHECKED BY *T.K.*
APPROVED BY *Z.B.*

NELSON OSTROM BASKIN BERMAN & ASSOC., INC.
CONSULTING ENGINEERS
CHICAGO, ILLINOIS

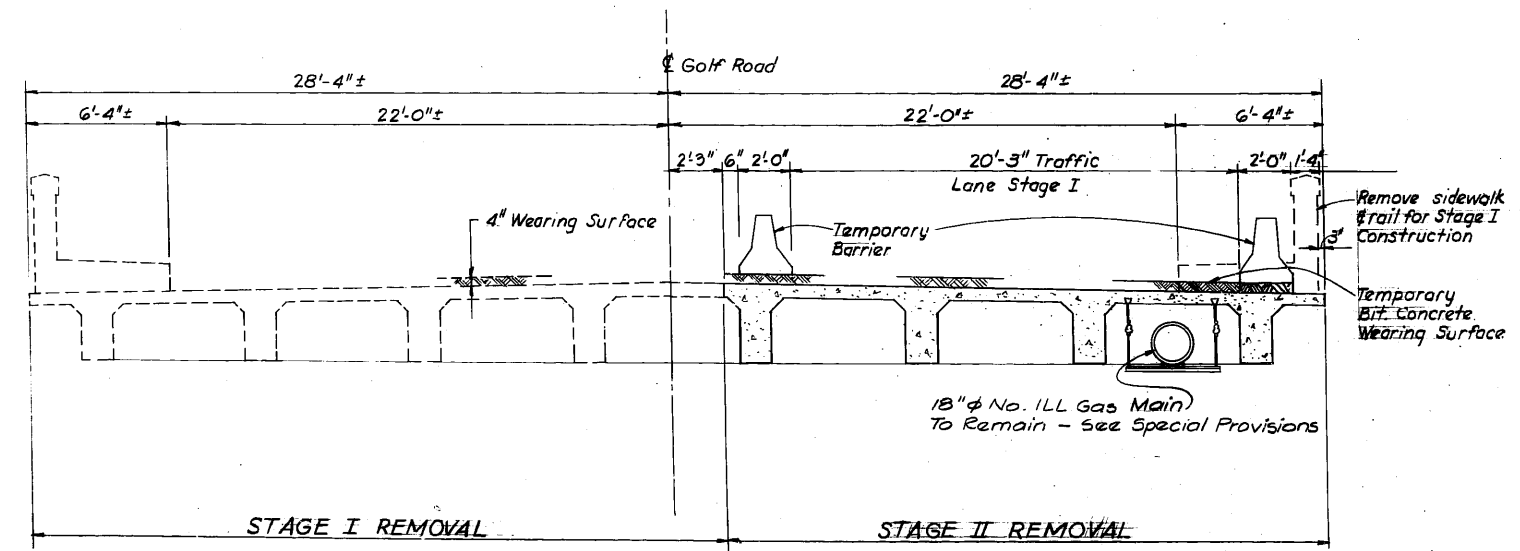
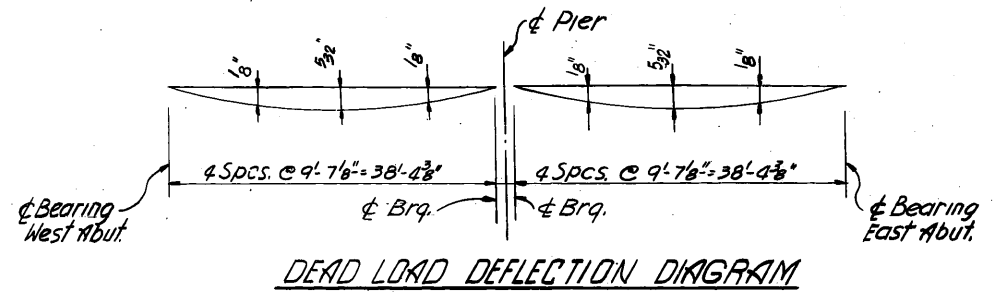
GENERAL PLAN & ELEVATION
RECONSTRUCTION
GOLF ROAD
OVER
NORTH BRANCH OF CHICAGO RIVER
FAP ROUTE 557 SECTION 1977-119-W & RS
COOK COUNTY
STATION 199+68.00
DATE: FEB. 1, 1983

METHOD OF DETERMINING FILLET HEIGHTS

After all precast prestressed beams has been erected, elevations of the top flanges of the beams shall be taken at intervals shown on this sheet. These elevations subtracted from the Theoretical Grade Elevations adjusted for Dead Load Deflections minus 7/8" slab thickness equals the fillet heights above top flanges of beams.

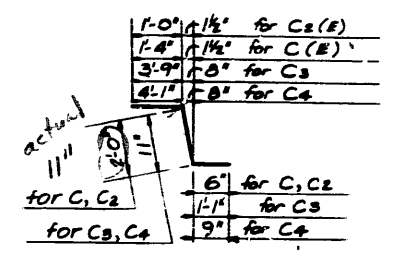
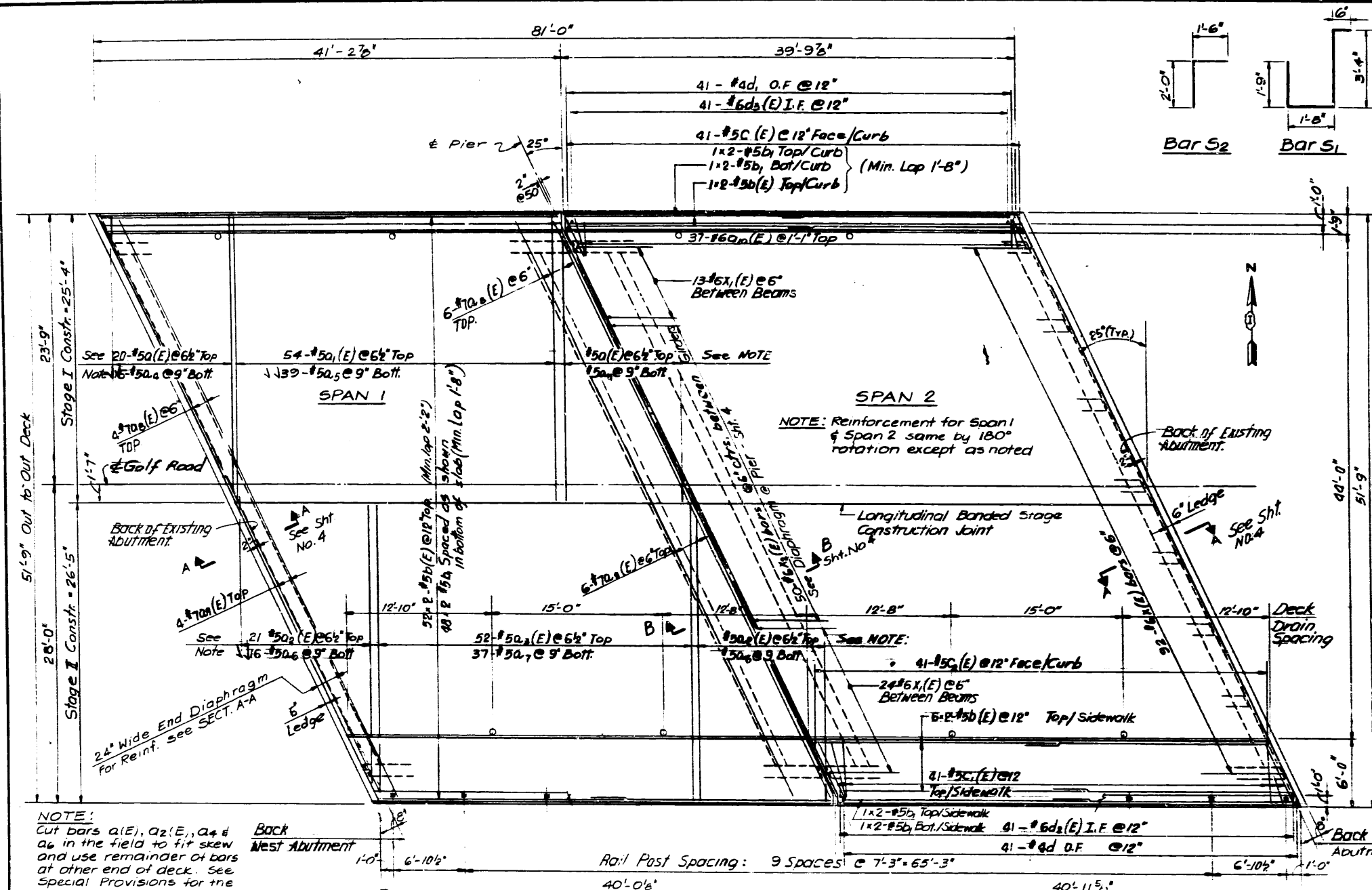


SPAN 1						SPAN 2						
STATION	OFFSET	ELEV	DEFL	ADJ ELEV		STATION	OFFSET	ELEV	DEFL	ADJ ELEV		
B1 BK. W. ABUT.	19917.3886	20.4999	624.394	0.000	624.394	B1	CL PIER	19958.4407	20.4999	624.529	0.000	624.529
CL BRG. W. ABUT.	19918.8727	20.4999	624.402	0.000	624.402	CL E. BRG. PIER	19959.5440	20.4999	624.531	0.000	624.531	
Q1	19928.8727	20.4999	624.449	0.009	624.458	Q2	19979.5440	20.4999	624.533	0.009	624.546	
b1	19938.8727	20.4999	624.486	0.013	624.499	b2	19979.5440	20.4999	624.533	0.013	624.546	
c1	19948.8727	20.4999	624.513	0.008	624.521	c2	19989.5440	20.4999	624.539	0.008	624.527	
CL W. BRG. PIER	19951.3878	20.4999	624.528	0.000	624.528	CL BRG. E. ABUT.	19997.9087	20.4999	624.500	0.000	624.500	
CL PIER	19958.4407	20.4999	624.529	0.000	624.529	BK. E. ABUT.	19999.4948	20.4999	624.495	0.000	624.495	
B2 BK. W. ABUT.	19920.5780	13.6666	624.516	0.000	624.516	B2	CL PIER	19961.6271	13.6666	624.640	0.000	624.640
CL BRG. W. ABUT.	19922.1591	13.6666	624.524	0.000	624.524	CL E. BRG. PIER	19962.7305	13.6666	624.641	0.000	624.641	
Q1	19932.1591	13.6666	624.568	0.009	624.577	Q2	19972.7305	13.6666	624.644	0.009	624.653	
b1	19942.1591	13.6666	624.602	0.012	624.614	b2	19982.7305	13.6666	624.637	0.012	624.649	
c1	19952.1591	13.6666	624.626	0.008	624.634	c2	19992.7305	13.6666	624.619	0.008	624.627	
CL W. BRG. PIER	19960.5237	13.6666	624.639	0.000	624.639	CL BRG. E. ABUT.	20001.0951	13.6666	624.597	0.000	624.597	
CL PIER	19961.5271	13.6666	624.640	0.000	624.640	BK. E. ABUT.	20002.6812	13.6666	624.592	0.000	624.592	
B3 BK. W. ABUT.	19922.7584	6.8333	624.639	0.000	624.639	B3	CL PIER	19964.8135	6.8333	624.749	0.000	624.749
CL BRG. W. ABUT.	19923.2435	6.8333	624.646	0.000	624.646	CL E. BRG. PIER	19965.9169	6.8333	624.749	0.000	624.749	
Q1	19933.2435	6.8333	624.687	0.009	624.696	Q2	19975.9169	6.8333	624.749	0.009	624.758	
b1	19943.2435	6.8333	624.718	0.012	624.730	b2	19985.9169	6.8333	624.739	0.012	624.751	
c1	19953.2435	6.8333	624.739	0.008	624.747	c2	19995.9169	6.8333	624.718	0.008	624.726	
CL W. BRG. PIER	19963.7101	6.8333	624.748	0.000	624.748	CL BRG. E. ABUT.	20004.2815	6.8333	624.693	0.000	624.693	
CL PIER	19964.8135	6.8333	624.749	0.000	624.749	BK. E. ABUT.	20005.8676	6.8333	624.688	0.000	624.688	
B4 BK. W. ABUT.	19926.2458	0.0000	624.760	0.000	624.760	B4	CL PIER	19967.9999	0.0000	624.857	0.000	624.857
CL BRG. W. ABUT.	19928.5313	0.0000	624.767	0.000	624.767	CL E. BRG. PIER	19969.1033	0.0000	624.857	0.000	624.857	
Q1	19938.5313	0.0000	624.805	0.008	624.813	Q2	19979.1033	0.0000	624.854	0.008	624.862	
b1	19948.5313	0.0000	624.832	0.012	624.844	b2	19989.1033	0.0000	624.840	0.012	624.852	
c1	19958.5313	0.0000	624.830	0.007	624.837	c2	19999.1033	0.0000	624.816	0.007	624.823	
CL W. BRG. PIER	19968.8868	0.0000	624.837	0.000	624.837	CL BRG. E. ABUT.	20009.4679	0.0000	624.789	0.000	624.789	
CL PIER	19969.9999	0.0000	624.857	0.000	624.857	BK. E. ABUT.	20009.0540	0.0000	624.783	0.000	624.783	
LONG. BD. CONSTR. JT.						LONG. BD. STAGE CONSTR. JT.						
BK. W. ABUT.	19927.8841	-1.5832	624.739	0.000	624.739	CL PIER	19968.7382	-1.5832	624.833	0.000	624.833	
CL BRG. W. ABUT.	19929.2702	-1.5832	624.745	0.000	624.745	CL E. BRG. PIER	19969.8416	-1.5832	624.833	0.000	624.833	
Q1	19939.2702	-1.5832	624.782	0.008	624.790	Q2	19979.8416	-1.5832	624.828	0.008	624.836	
b1	19949.2702	-1.5832	624.809	0.012	624.821	b2	19989.8416	-1.5832	624.814	0.012	624.826	
c1	19959.2702	-1.5832	624.826	0.007	624.833	c2	19999.8416	-1.5832	624.790	0.007	624.797	
CL W. BRG. PIER	19969.7849	-1.5832	624.832	0.000	624.832	CL BRG. E. ABUT.	20008.2062	-1.5832	624.761	0.000	624.761	
CL PIER	19968.7382	-1.5832	624.833	0.000	624.833	BK. E. ABUT.	20009.7923	-1.5832	624.755	0.000	624.755	
B5 BK. W. ABUT.	19928.8652	-6.2498	624.675	0.000	624.675	B5	CL PIER	19970.9143	-6.2498	624.760	0.000	624.760
CL BRG. W. ABUT.	19931.1465	-6.2498	624.681	0.000	624.681	CL E. BRG. PIER	19972.0177	-6.2498	624.760	0.000	624.760	
Q1	19941.1465	-6.2498	624.716	0.008	624.724	Q2	19982.0177	-6.2498	624.753	0.008	624.761	
b1	19951.1465	-6.2498	624.741	0.011	624.752	b2	19992.0177	-6.2498	624.737	0.011	624.748	
c1	19961.1465	-6.2498	624.755	0.007	624.762	c2	20002.0177	-6.2498	624.710	0.007	624.717	
CL W. BRG. PIER	19971.8849	-6.2498	624.760	0.000	624.760	CL BRG. E. ABUT.	20010.3823	-6.2498	624.680	0.000	624.680	
CL PIER	19971.9999	-6.2498	624.760	0.000	624.760	BK. E. ABUT.	20011.9584	-6.2498	624.673	0.000	624.673	
B6 BK. W. ABUT.	19932.7147	-12.4998	624.589	0.000	624.589	B6	CL PIER	19973.8288	-12.4998	624.662	0.000	624.662
CL BRG. W. ABUT.	19934.3808	-12.4998	624.595	0.000	624.595	CL E. BRG. PIER	19974.9321	-12.4998	624.661	0.000	624.661	
Q1	19944.3808	-12.4998	624.627	0.008	624.635	Q2	19984.9321	-12.4998	624.552	0.008	624.660	
b1	19954.3808	-12.4998	624.649	0.011	624.660	b2	19994.9321	-12.4998	624.632	0.011	624.643	
c1	19964.3808	-12.4998	624.660	0.007	624.667	c2	20004.9321	-12.4998	624.603	0.007	624.610	
CL W. BRG. PIER	19974.7854	-12.4998	624.662	0.000	624.662	CL BRG. E. ABUT.	20013.2968	-12.4998	624.570	0.000	624.570	
CL PIER	19973.8288	-12.4998	624.662	0.000	624.662	BK. E. ABUT.	20014.8829	-12.4998	624.563	0.000	624.563	
B7 BK. W. ABUT.	19935.8891	-18.7498	624.502	0.000	624.502	B7	CL PIER	19976.7432	-18.7498	624.563	0.000	624.563
CL BRG. W. ABUT.	19937.2752	-18.7498	624.508	0.000	624.508	CL E. BRG. PIER	19977.8465	-18.7498	624.562	0.000	624.562	
Q1	19947.2752	-18.7498	624.537	0.008	624.545	Q2	19987.8465	-18.7498	624.550	0.008	624.558	
b1	19957.2752	-18.7498	624.555	0.011	624.566	b2	19997.8465	-18.7498	624.527	0.011	624.538	
c1	19967.2752	-18.7498	624.564	0.007	624.571	c2	20007.8465	-18.7498	624.494	0.007	624.501	
CL W. BRG. PIER	19977.8898	-18.7498	624.563	0.000	624.563	CL BRG. E. ABUT.	20016.2112	-18.7498	624.460	0.000	624.460	
CL PIER	19976.7432	-18.7498	624.563	0.000	624.563	BK. E. ABUT.	20017.7973	-18.7498	624.453	0.000	624.453	
B8 BK. W. ABUT.	19938.8035	-24.9998	624.414	0.000	624.414	B8	CL PIER	19979.6576	-24.9998	624.463	0.000	624.463
CL BRG. W. ABUT.	19940.1896	-24.9998	624.419	0.000	624.419	CL E. BRG. PIER	19980.7610	-24.9998	624.462	0.000	624.462	
Q1	19950.1896	-24.9998	624.445	0.010	624.455	Q2	19990.7610	-24.9998	624.446	0.010	624.456	
b1	19960.1896	-24.9998	624.461	0.013	624.474	b2	20000.7610	-24.9998	624.421	0.013	624.434	
c1	19970.1896	-24.9998	624.467	0.008	624.475	c2	20010.7610	-24.9998	624.385	0.008	624.393	
CL W. BRG. PIER	19980.8842	-24.9998	624.464	0.000	624.464	CL BRG. E. ABUT.	20019.1256	-24.9998	624.350	0.000	624.350	
CL PIER	19979.8876	-24.9998	624.463	0.000	624.463	BK. E. ABUT.	20020.7117	-24.9998	624.343	0.000	624.343	



NOTE: Elevations given are to top of slab. The theoretical top of slab elevations for beam #B are based on the extension of the 1/8 in. per foot crown to the E of the beam.

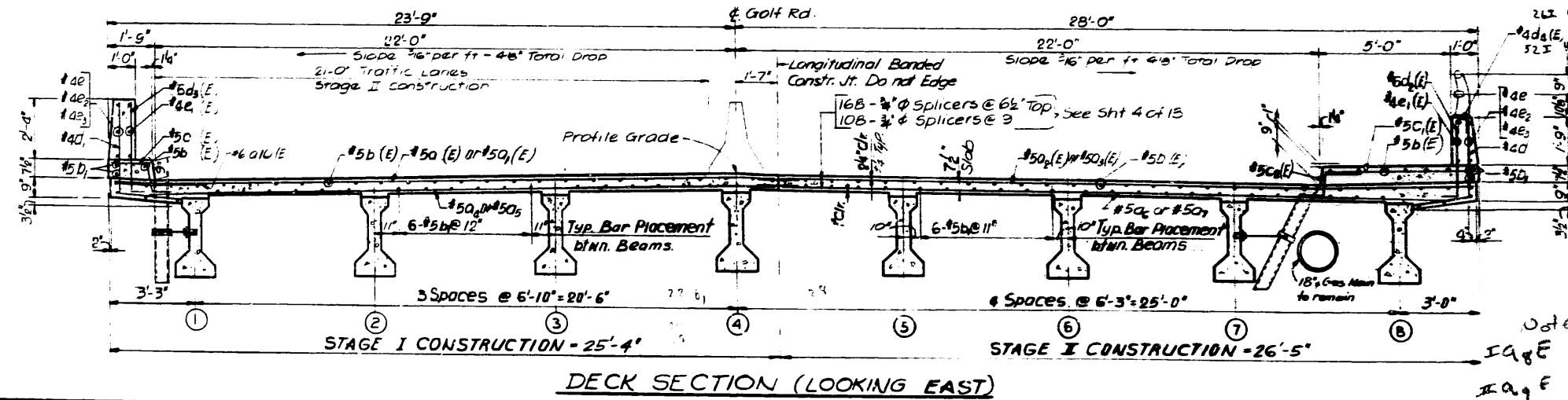
DECK ELEVATIONS
GOLF ROAD
OVER
NORTH BRANCH OF CHICAGO RIVER
FAP ROUTE 557 SECTION 1977-119-W & R
COOK COUNTY
STATION 89+68.00
DATE: FEB. 1, 1983



BILL OF MATERIALS				
BAR	NO.	SIZE	LENGTH	SHAPE
I	Q (E)	40	#5	26'-4"
I	Q ₁ (E)	108	#5	24'-10"
II	Q ₂ (E)	42	#5	27'-5"
II	Q ₃ (E)	104	#5	25'-11"
I	Q ₄	30	#5	25'-10"
I	Q ₅	78	#5	23'-10"
II	Q ₆	32	#5	26'-11"
II	Q ₇	76	#5	24'-11"
II	Q ₈	20	#7	27'-2"
II	Q ₉	20	#7	28'-7"
II	Q ₁₀ (E)	74	#6	4'-0"
II	b (E)	256	#5	21'-1"
II	b ₁	208	#5	21'-1"
I	C (E)	82	#5	2'-9"
II	C ₁ (E)	82	#5	5'-7"
II	C ₂ (E)	82	#5	2'-5"
II	C ₃	4	#6	6'-10"
II	C ₄	4	#6	6'-10"
II	d	82	#4	4'-10"
I	d ₁	82	#4	5'-9"
II	d ₂ (E)	82	#6	3'-9"
II	d ₃ (E)	82	#6	4'-5"
II	d ₄ (E)	20	#4	2'-0"
II	d ₅	2	#6	22'-11"
II	d ₆	2	#6	6'-5"
II	e	1212	#4	13'-2"
II	e ₁ (E)	1212	#4	13'-2"
II	e ₂	336	#4	12'-8"
II	e ₃ (E)	336	#4	12'-10"
II	e ₄ (E)	336	#4	13'-6"
II	e ₅	336	#4	13'-6"
II	e ₆ (E)	3	#4	13'-0"
I	m	12	#4	6'-9"
I	m ₁	4	#4	4'-8"
II	m ₂	8	#4	6'-2"
I	m ₃	4	#4	28'-9"
II	m ₄	4	#4	27'-7"
II	m ₅	2	#6	28'-9"
II	m ₆	2	#6	27'-7"
II	m ₇	42	#6	5'-8"
II	m ₈	12	#6	5'-0"
II	m ₉	30	#6	6'-3"
II	m ₁₀	10	#6	4'-4"
II	m ₁₁	4	#6	3'-9"
II	m ₁₂	6	#6	6'-2"
II	m ₁₃	2	#4	4'-9"
II	m ₁₄	4	#6	1'-3"
I	S ₁	72	#4	7'-3"
I	S ₂	72	#4	3'-6"

NOTE:
Cut bars a₁(E), a₂(E), a₃ & a₆ in the field to fit skew and use remainder of bars at other end of deck. See Special Provisions for the procedure for epoxy coating of cut reinforcement bars.

BILL OF MATERIALS				
BAR	NO.	SIZE	LENGTH	SHAPE
X (E)	184	#6	8'-0"	—
X ₁ (E)	74	#6	8'-9"	—
X ₂ (E)	100	#6	8'-11"	—
ITEM UNIT QUANT				
Class "X" Concrete	Cu. Yd.	166.4		
Reinforcement Bars	Lbs	15170		
Reinf Bars (Epoxy Coated)	Lbs	20360		
Protective Coat	Sq. Yd.	513		

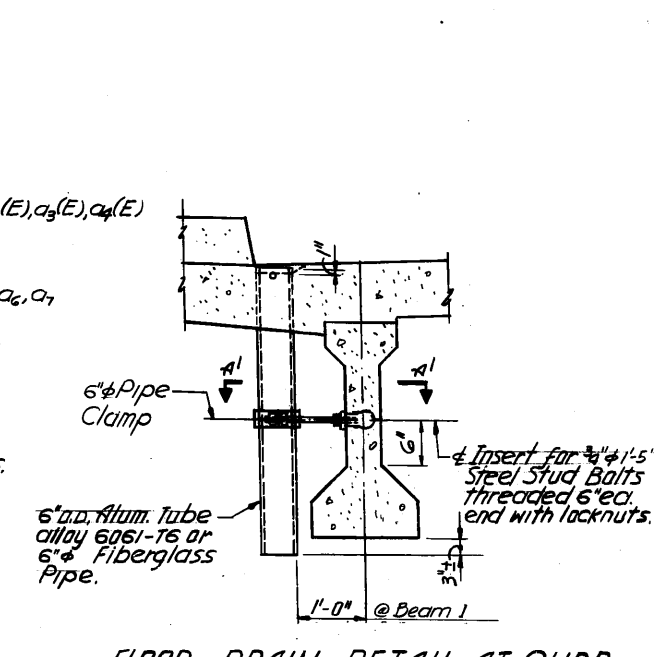
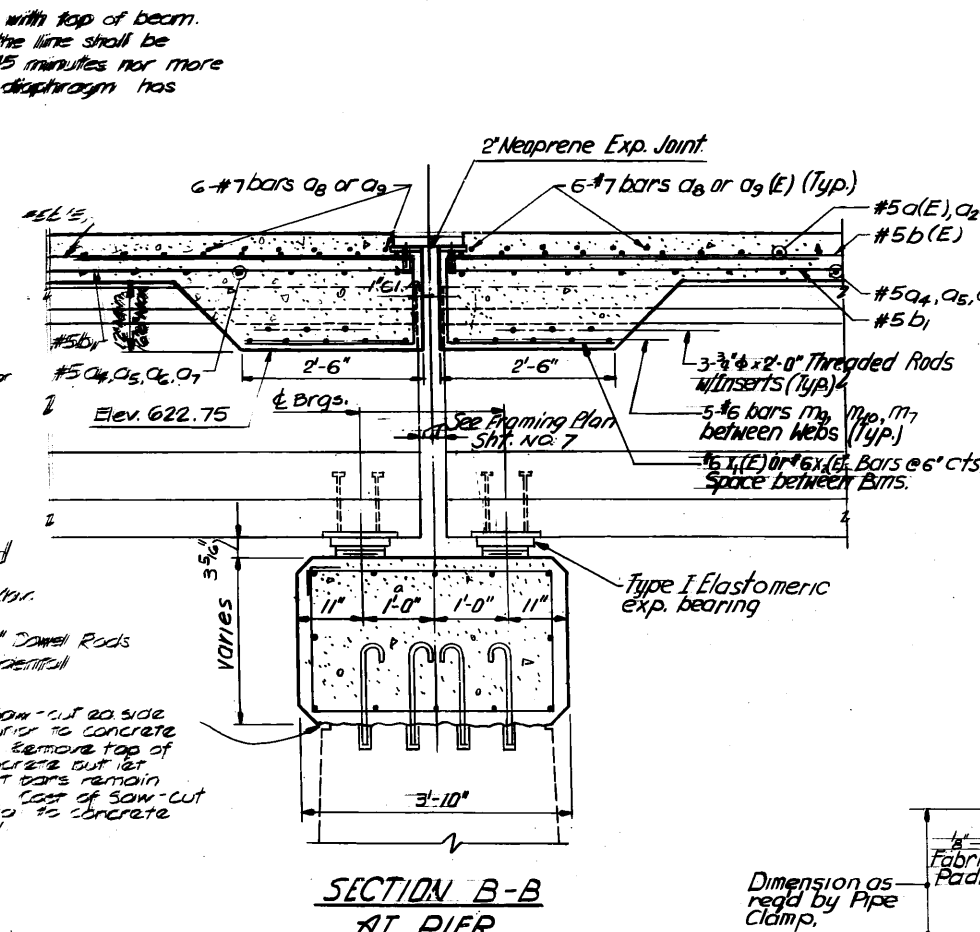
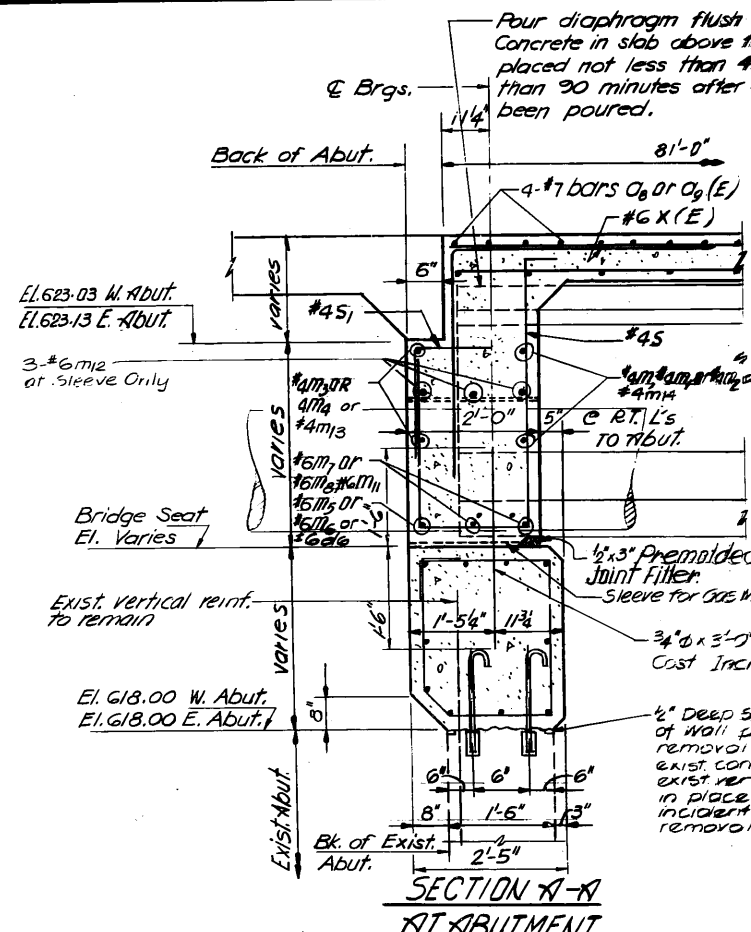


NOTE:
Work this Sheet with Sheets No. 4 & 5 of 13.

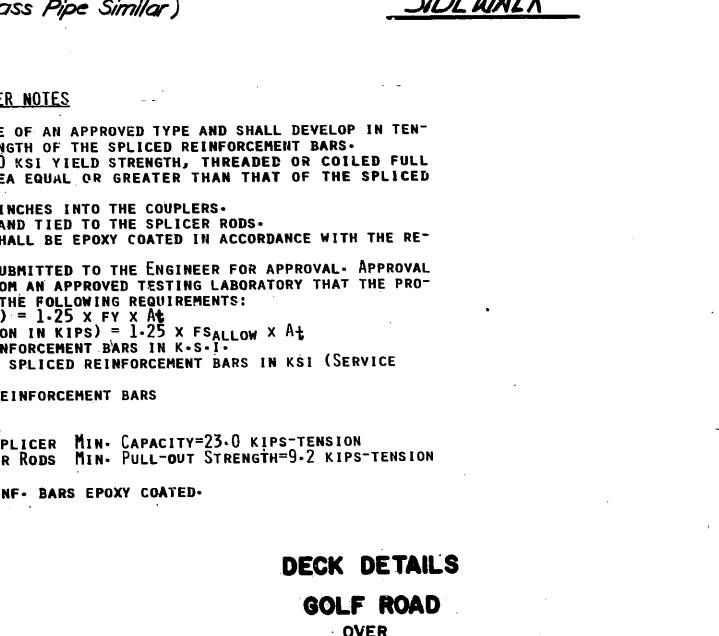
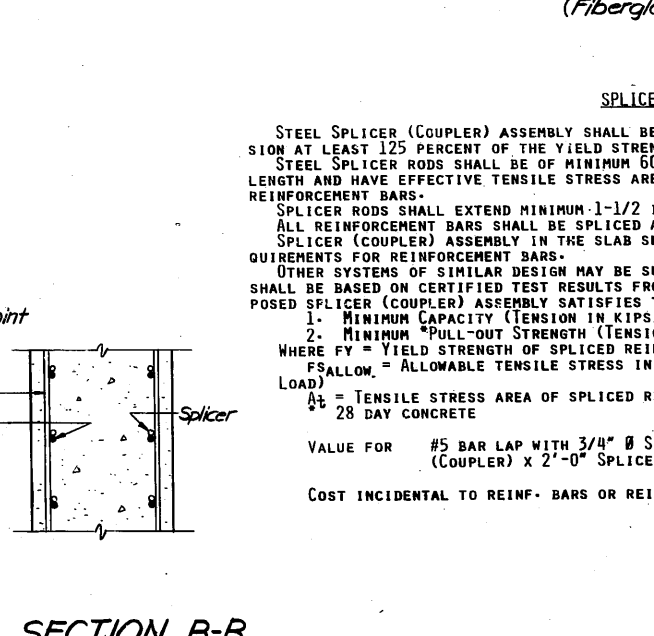
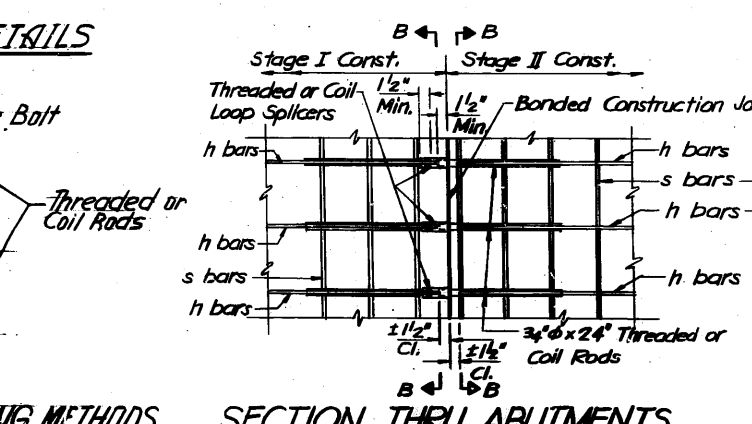
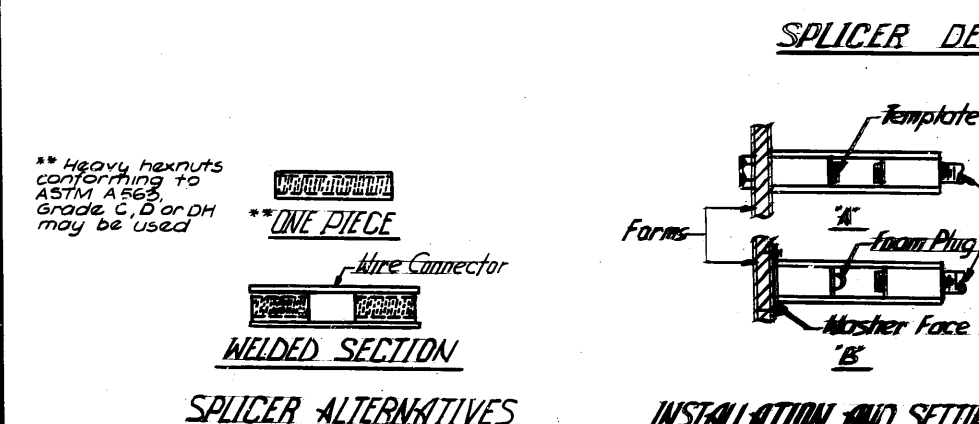
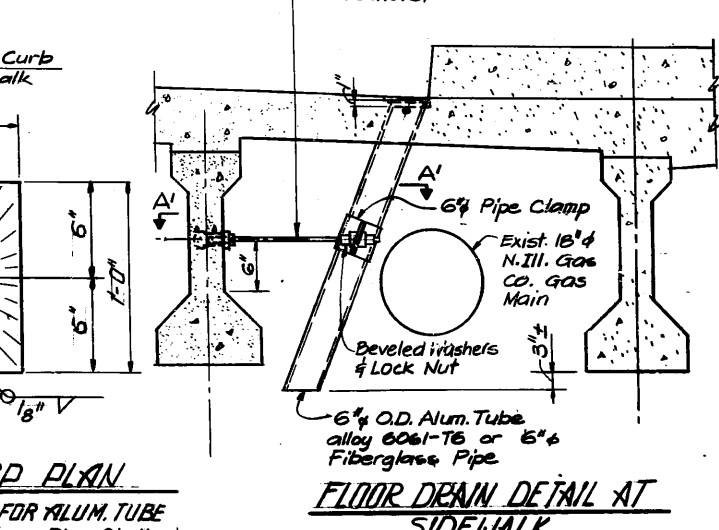
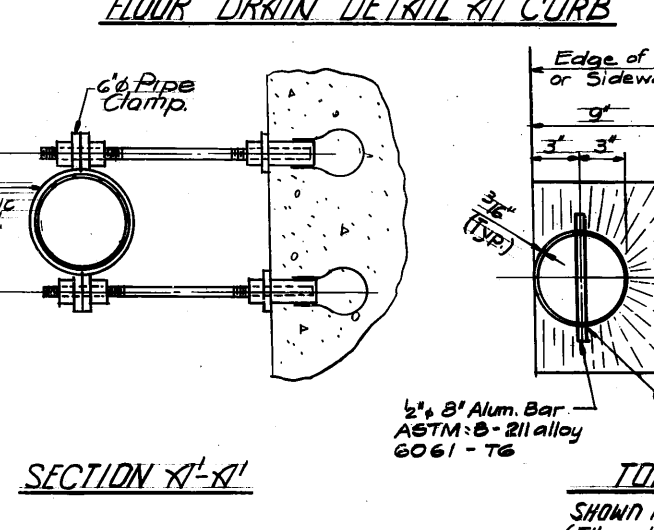
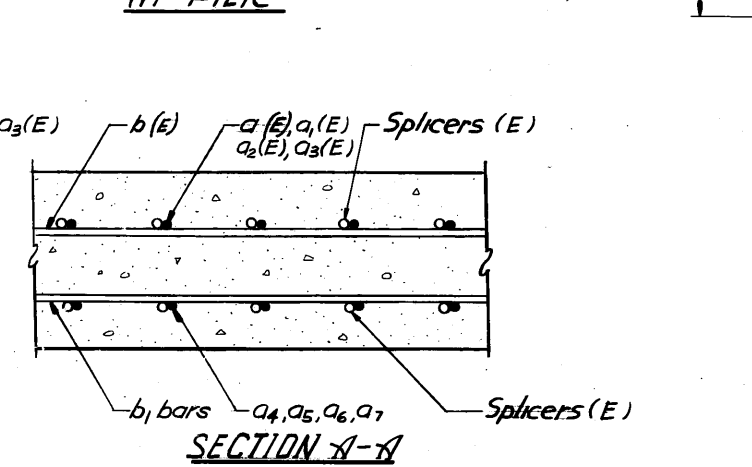
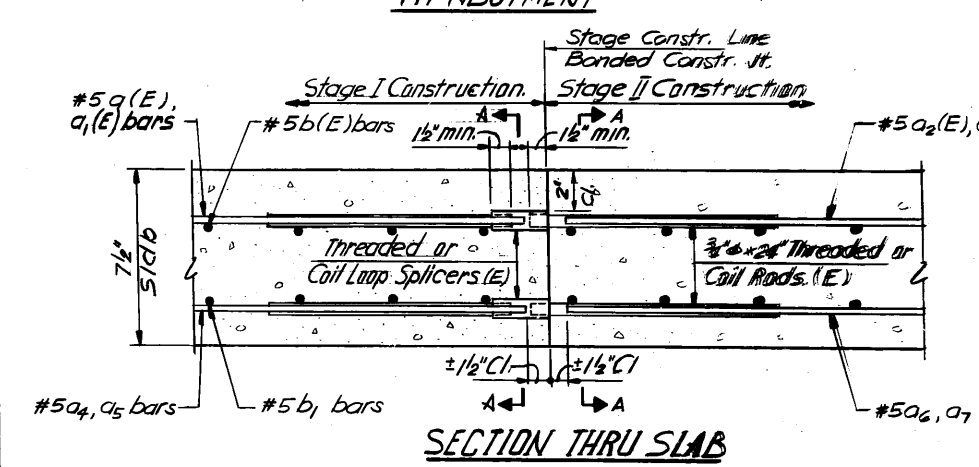
DECK SLAB PLAN

GOLF ROAD
OVER
NORTH BRANCH OF CHICAGO RIVER
FAP ROUTE 557 SECTION 1977-119-W BRS
COOK COUNTY
STATION 199+68.00
DATE FEB 1, 1983

Pour diaphragm flush with top of beam. Concrete in slab above the line shall be placed not less than 45 minutes nor more than 90 minutes after diaphragm has been poured.



NOTES:
Fiberglass pipe shall conform to ASTM D2996 Designation code RTRP-11RE-5112. Pipe with class C or F filler are acceptable.
The surface of the Fiberglass pipe shall be free of band inhibiting agent.
Pipe clamp and steel stud bolts shall be painted with Red Lead for first coat and two coats of Aluminum Paint.



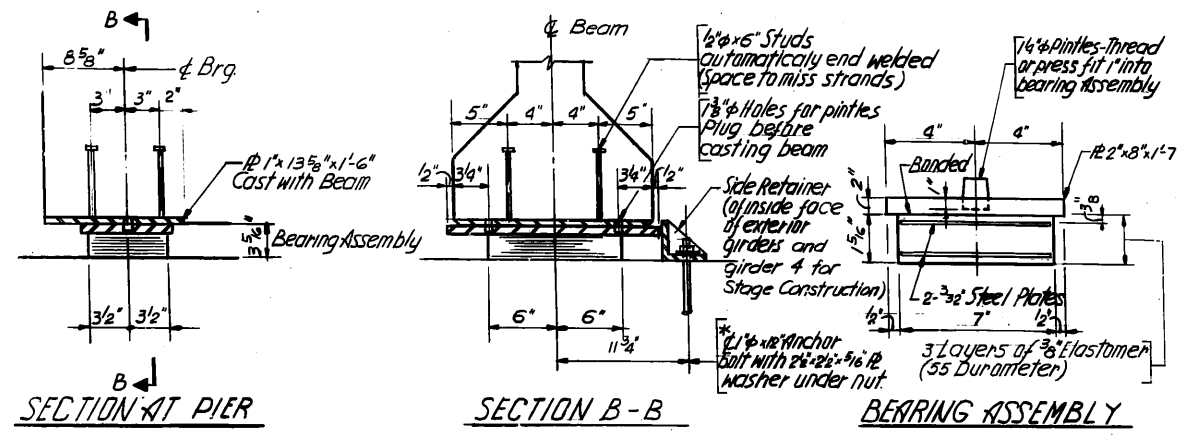
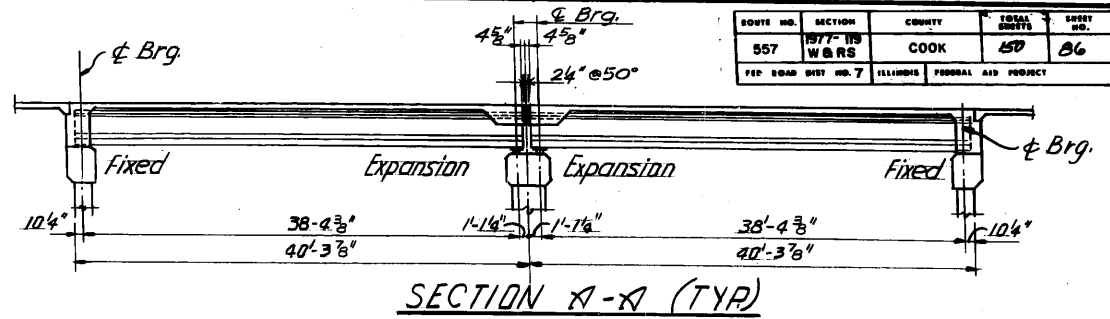
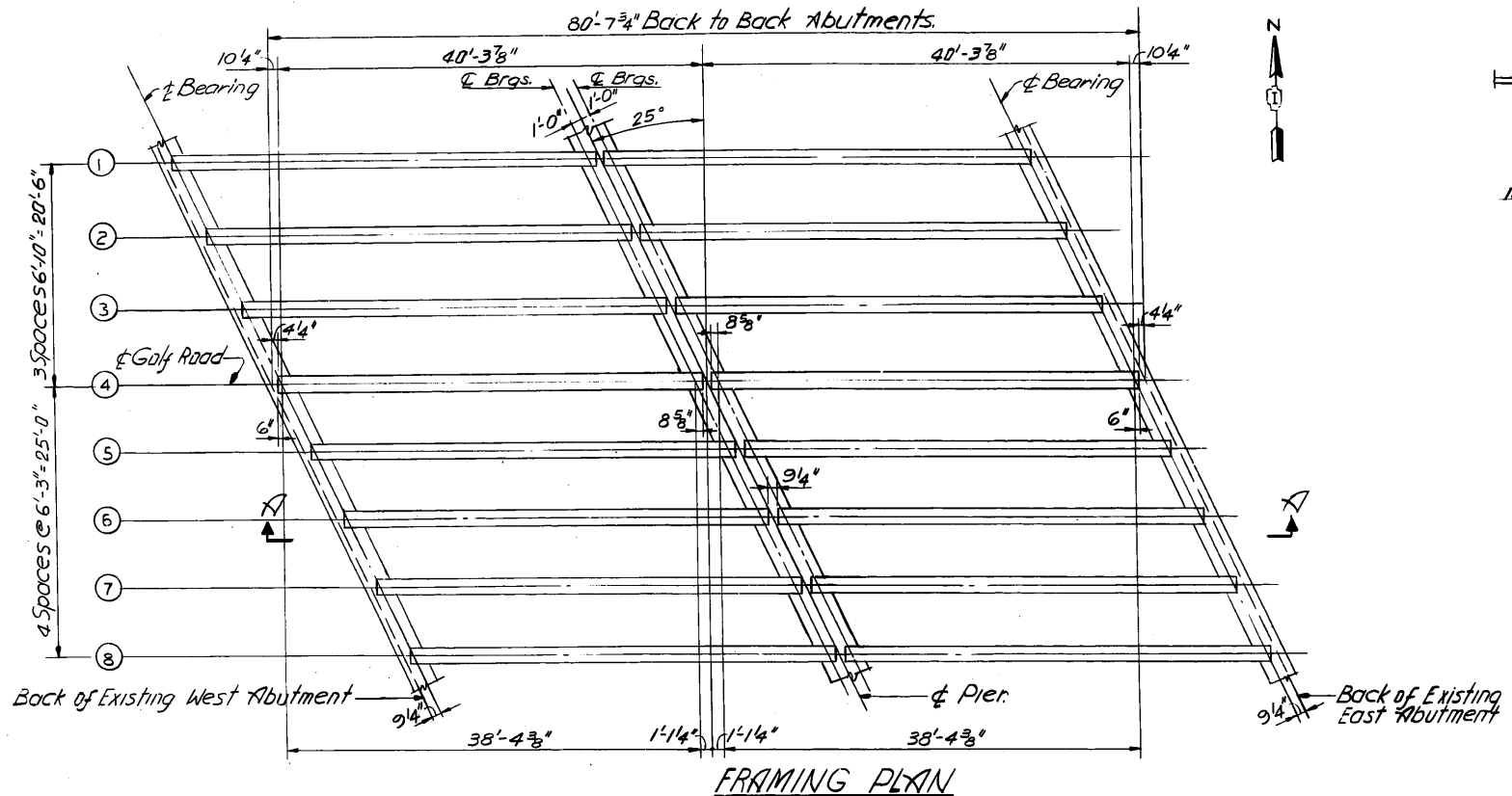
SPLICER NOTES
STEEL SPLICER (COUPLER) ASSEMBLY SHALL BE OF AN APPROVED TYPE AND SHALL DEVELOP IN TENSION AT LEAST 125 PERCENT OF THE YIELD STRENGTH OF THE SPLICED REINFORCEMENT BARS.
STEEL SPLICER RODS SHALL BE OF MINIMUM 60 KSI YIELD STRENGTH, THREADED OR COILED FULL LENGTH AND HAVE EFFECTIVE TENSILE STRESS AREA EQUAL OR GREATER THAN THAT OF THE SPLICED REINFORCEMENT BARS.
SPLICER RODS SHALL EXTEND MINIMUM 1-1/2 INCHES INTO THE COUPLERS.
ALL REINFORCEMENT BARS SHALL BE SPLICED AND TIED TO THE SPLICER RODS.
SPLICER (COUPLER) ASSEMBLY IN THE SLAB SHALL BE EPOXY COATED IN ACCORDANCE WITH THE REQUIREMENTS FOR REINFORCEMENT BARS.
OTHER SYSTEMS OF SIMILAR DESIGN MAY BE SUBMITTED TO THE ENGINEER FOR APPROVAL. APPROVAL SHALL BE BASED ON CERTIFIED TEST RESULTS FROM AN APPROVED TESTING LABORATORY THAT THE PROPOSED SPLICER (COUPLER) ASSEMBLY SATISFIES THE FOLLOWING REQUIREMENTS:
1. MINIMUM CAPACITY (TENSION IN KIPS) = 1.25 x F_y x A_s
2. MINIMUM PULL-OUT STRENGTH (TENSION IN KIPS) = 1.25 x F_sALLOW x A_s
WHERE F_y = YIELD STRENGTH OF SPLICED REINFORCEMENT BARS IN K-S-I.
F_sALLOW = ALLOWABLE TENSILE STRESS IN SPLICED REINFORCEMENT BARS IN KSI (SERVICE LOAD)
A_s = TENSILE STRESS AREA OF SPLICED REINFORCEMENT BARS
A_s = 28 DAY CONCRETE
VALUE FOR #5 BAR LAP WITH 3/4" Ø SPLICER MIN. CAPACITY=23.0 KIPS-TENSION (COUPLER) x 2'-0" SPLICER RODS MIN. PULL-OUT STRENGTH=9.2 KIPS-TENSION
COST INCIDENTAL TO REINF. BARS OR REINF. BARS EPOXY COATED.

DESIGNED BY V.K.
DRAWN BY V.K.
CHECKED BY T.H.L.
APPROVED BY Z.B.
NELSON OSTROM BASKIN BERMAN & ASSOC., INC.
CONSULTING ENGINEERS
CHICAGO, ILLINOIS

A: Set splicer by means of a template bolt.
B: Set splicer by nailing to wood forms or cementing to steel forms.
(E) Indicates Epoxy Coating, See Special Provisions

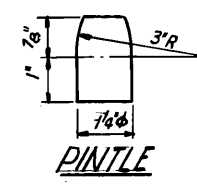
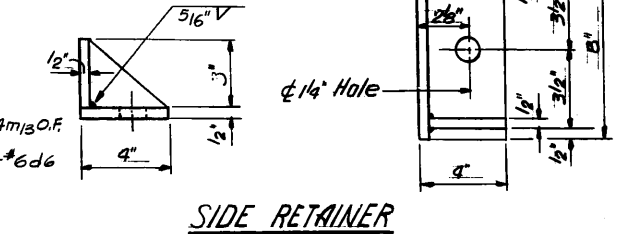
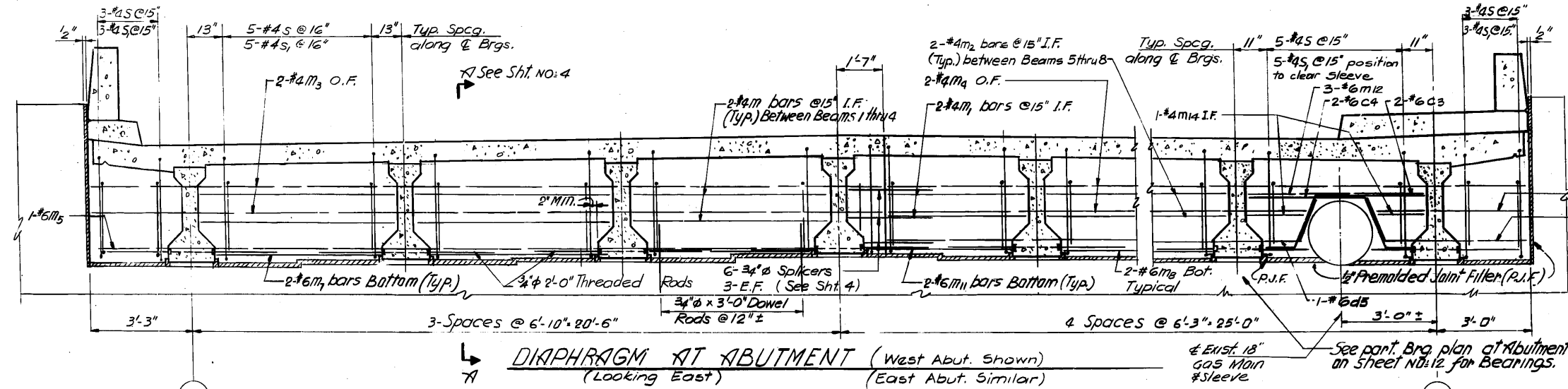
Work this Sheet with Sheet No. 3 of 13

DECK DETAILS
GOLF ROAD
OVER
NORTH BRANCH OF CHICAGO RIVER
FAP ROUTE 557 SECTION 1977-119-W & R S
COOK COUNTY
STATION 190+68.00
DATE: FEB. 1, 1983

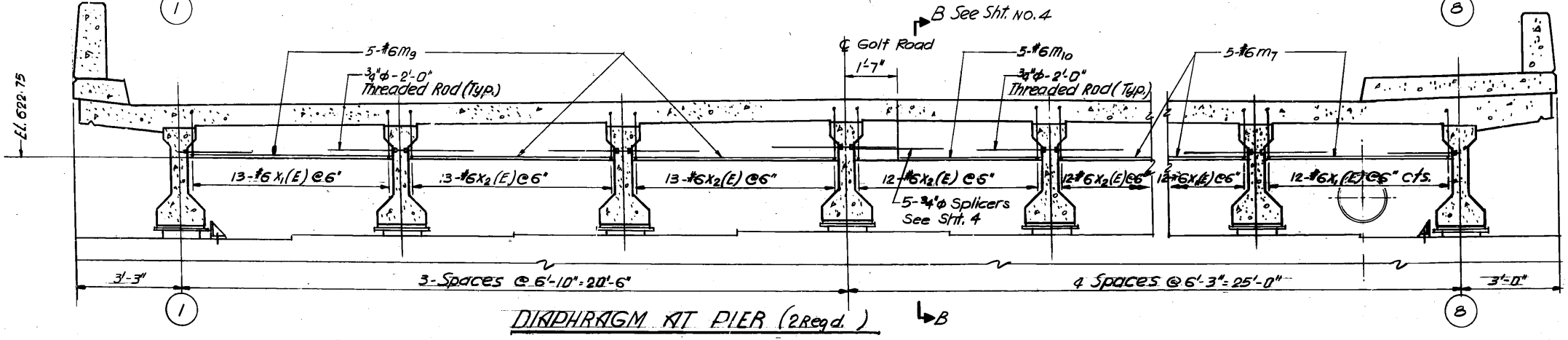


Note: Shim plates shall not be placed under Bearing Assembly.

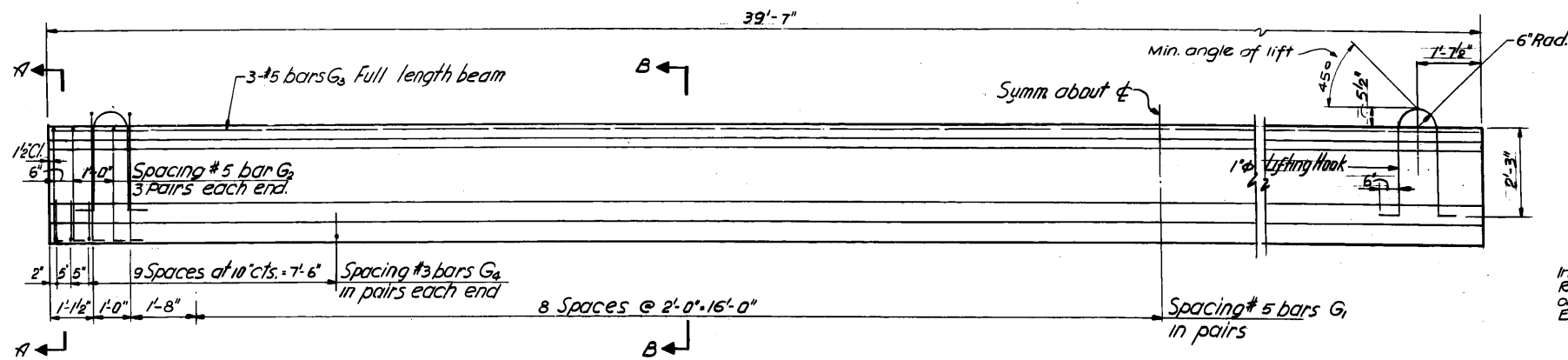
Note: After beams have been erected holes of expansion bearings shall be drilled and anchor bolts grouted in place.



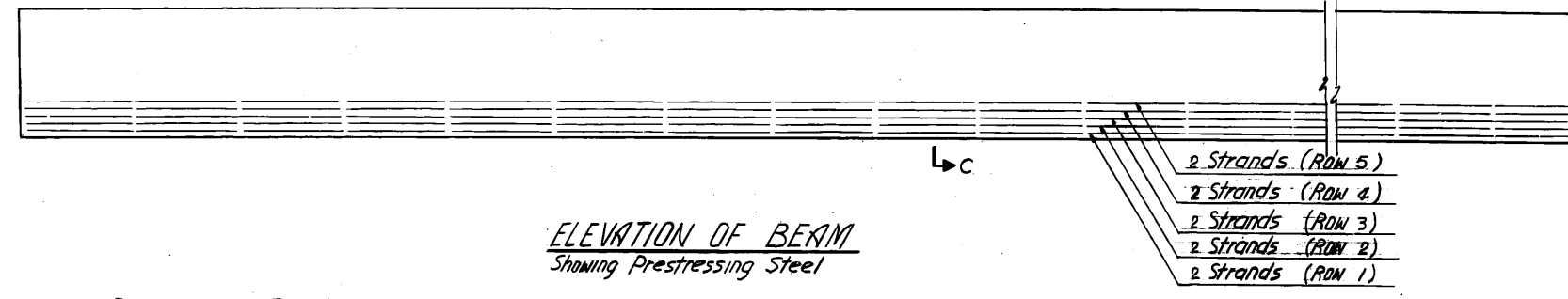
Note: For Bill of Materials See Sheet No 3 of 13.



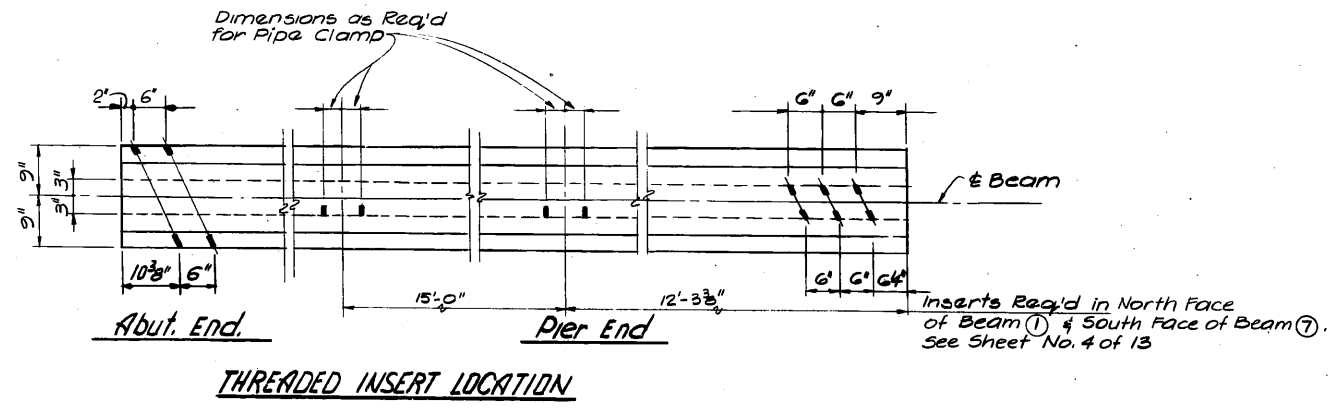
FRAMING PLAN & DIAPHRAGM DETAILS
GOLF ROAD
OVER
NORTH BRANCH OF CHICAGO RIVER
FAP ROUTE 557 SECTION 1977-119-W & RS
COOK COUNTY
STATION 199+68-00
DATE: FEB. 1, 1983



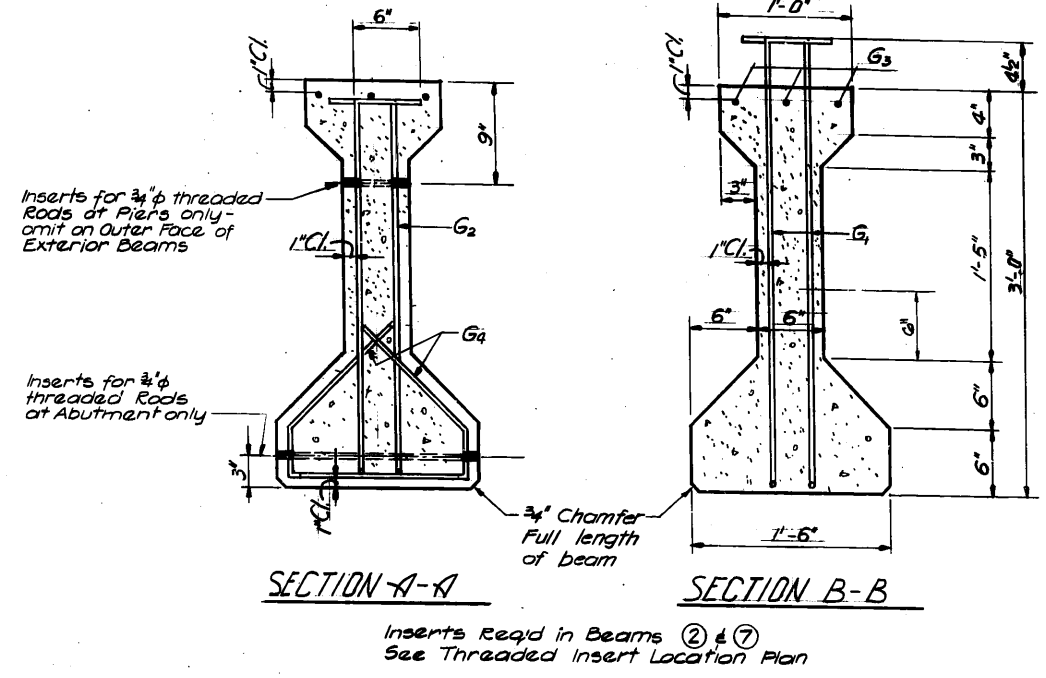
ELEVATION OF BEAM (16 Required)
Showing Reinforcement & Dimensions



ELEVATION OF BEAM
Showing Prestressing Steel



THREADED INSERT LOCATION



SECTION A-A

SECTION B-B

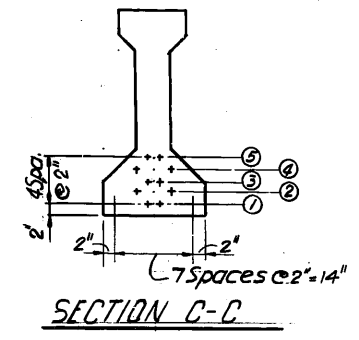
Inserts Req'd in Beams ② & ⑦
See Threaded Insert Location Plan

***BAR LIST**

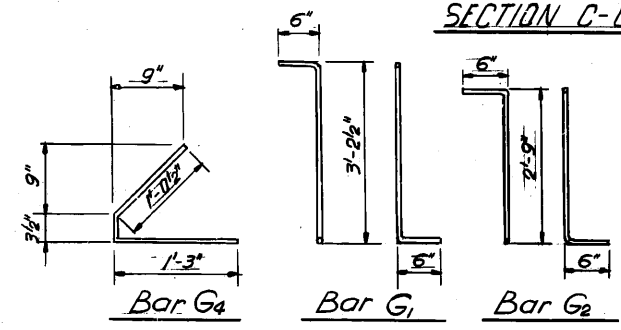
Bar No.	Size	Length	Shape
G ₁	#5	4'-2 1/2"	TL
G ₂	#5	3'-9"	TL
G ₃	#5	39'-3"	—
G ₄	#3	2'-7"	L

*For one beam only.

*Place strands symmetrically about & Beam.



SECTION C-C



NOTES:
All inserts and threaded rods for inserts, reinforcing and Prestressing Steel, and other items which are cast into the Precast Concrete I-Beams shall be included in the contract unit price per linear foot of Furnishing and Erecting Precast Prestressed Concrete I-Beams, 36 in.
Prestressing Steel shall have a nominal diameter of 7/8" and a minimum cross sectional area of 0.65 sq. in.
Inserts for 3/4" threaded rods are to be two strut, coil type for interior I-Beams and single coil, flared loop type for exterior I-Beams.
Steel for lifting hooks shall be non-deformed bars.
fy: 40,000 psi

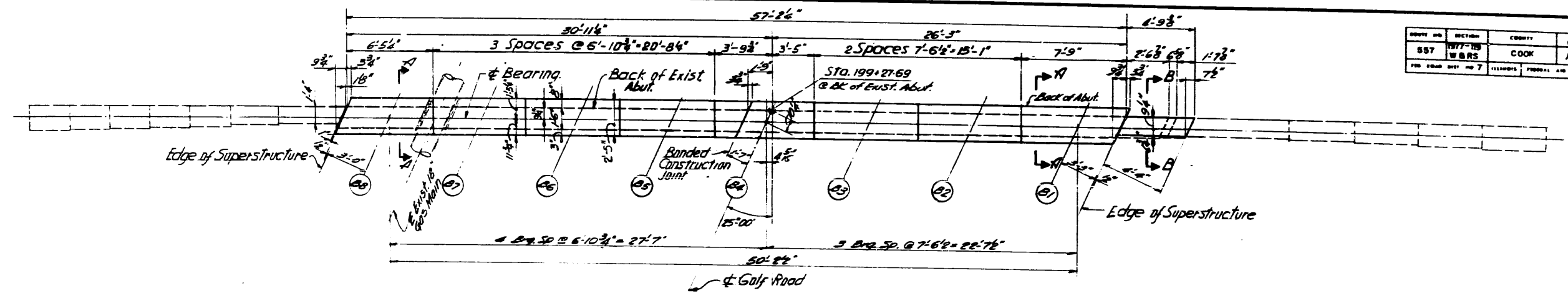
BILL OF MATERIAL

Item	Unit	Total
Furnishing & Erecting Precast Prestressed Concrete I-Beams, 36"	Lin. ft.	634

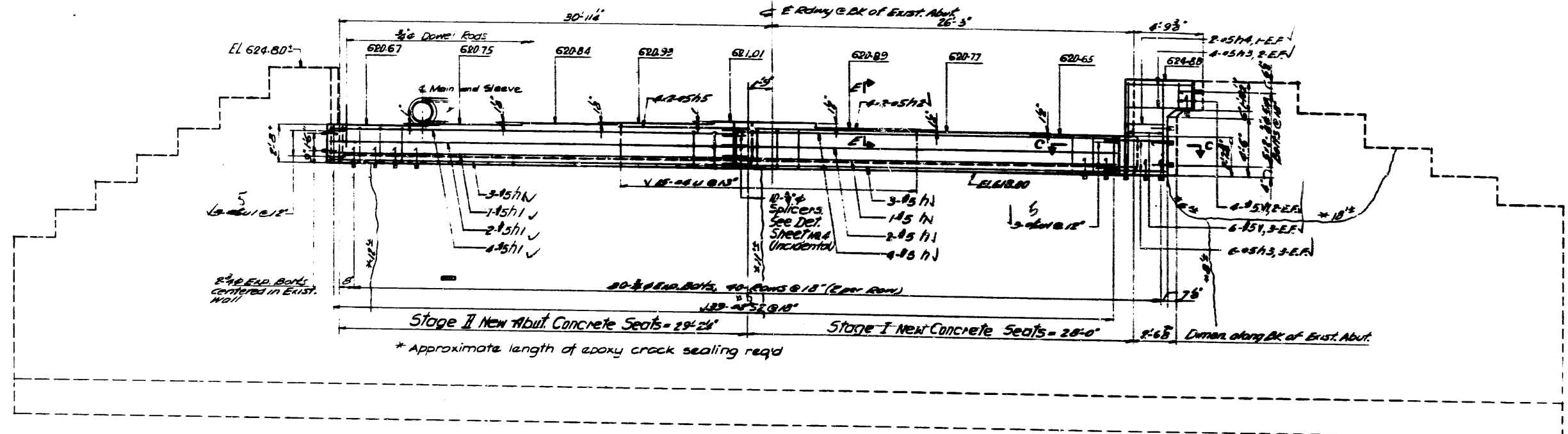
P.P.C. GIRDER DETAILS
GOLF ROAD
OVER
NORTH BRANCH OF CHICAGO RIVER
FAP ROUTE 557 SECTION 1977-119-W & RS
COOK COUNTY
STATION 199+58.00
DATE: FEB. 1, 1983

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
557	1977-119	COOK	150	88
FAP ROUTE 557 - NO 7		ILLINOIS FEDERAL AID PROJECT		

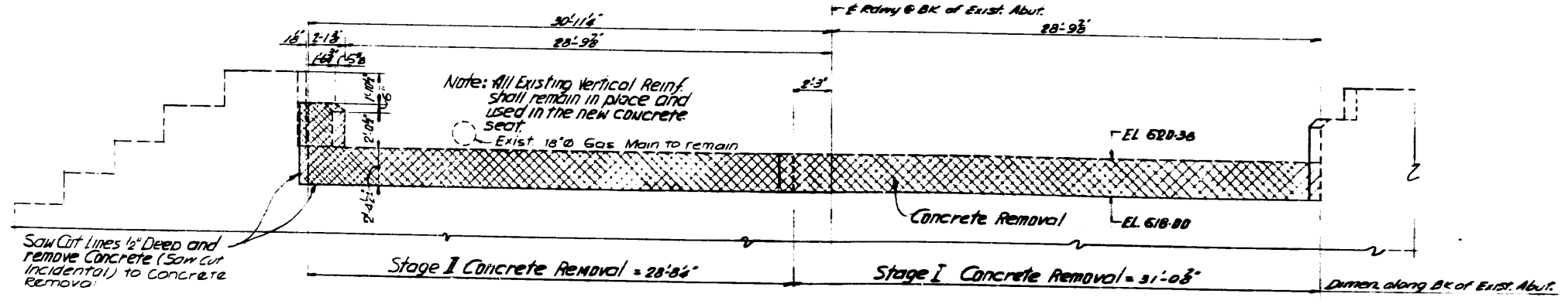
SHEET NO. 10
13 SHEETS



PLAN ABUTMENT SEATS



ELEVATION ABUTMENT SEAT
LOOKING WEST



PARTIAL ELEVATION OF ABUTMENT
Showing Concrete Removal

WORK THIS SHEET WITH SHEET NO.

WEST ABUTMENT DETAILS
GOLF ROAD
OVER
NORTH BRANCH OF CHICAGO RIVER
FAP ROUTE 557 SECTION 1977-119-W & RS
COOK COUNTY
STATION 199+68.00
DATE: FEB 1, 1983

DESIGNED BY: P.E.
DRAWN BY: V.K.
CHECKED BY: T.H.
APPROVED BY: E.C.O.

NELSON OSTROM BASKIN BERMAN & ASSOC. INC.
CONSULTING ENGINEERS
CHICAGO, ILLINOIS

Saw Cut Lines 1/2" Deep and remove concrete (Saw cut incidental) to concrete removal.