

INTERIOR GIRDER MOMENT TABLE		
0.5 Span		
I_s	(in ⁴)	26,876
$I_c(n)$	(in ⁴)	71,211
$I_c(3n)$	(in ⁴)	49,496
S_s	(in ³)	1,416
$S_c(n)$	(in ³)	1,886
$S_c(3n)$	(in ³)	1,731
DC1	(k/')	0.930
M _{DC1}	(k)	1,829
DC2	(k/')	0.030
M _{DC2}	(k)	59
DW	(k/')	0.300
M _{DW}	(k)	590
M _{ℓ + IM}	(k)	2,000
M _u (Strength I)	(k)	6,745
f _s DC1	(ksi)	15.5
f _s DC2	(ksi)	0.4
f _s DW	(ksi)	4.1
f _s 1.3(ℓ+IM)	(ksi)	16.5
f _s (Service II)	(ksi)	36.5
f _s (Total)(Strength I)	(ksi)	48.4
V _r	(k)	54.0

INTERIOR GIRDER REACTION TABLE		
Abutment		
R _{DC1}	(k)	57.7
R _{DC2}	(k)	1.9
R _{DW}	(k)	18.8
R _{ℓ + IM}	(k)	88.9
R _{Total}	(k)	167.3

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in⁴ and in³).

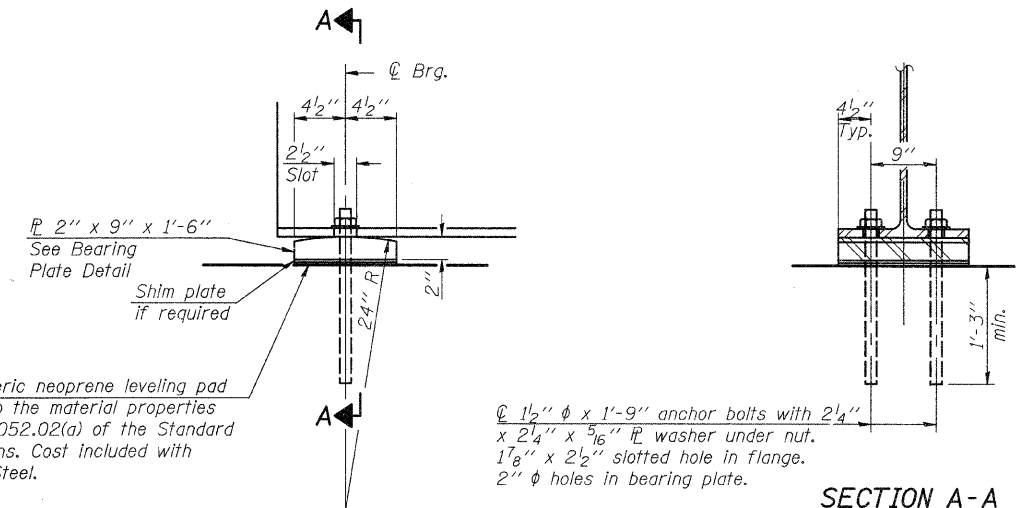
DC1: Un-factored non-composite dead load (kips/ft.).
M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).
DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
M_{ℓ + IM}: Un-factored live load moment plus dynamic load allowance (Impact) ((kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).
 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{ℓ + IM}$

f_s (Service II): Sum of stresses as computed below (ksi).
 $f_{sDC1} + f_{sDC2} + f_{sDW} + 1.3 f_s(ℓ + IM)$

f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
 $1.25 (f_{sDC1} + f_{sDC2}) + 1.5 f_{sDW} + 1.75 f_s(ℓ + IM)$

V_r: Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

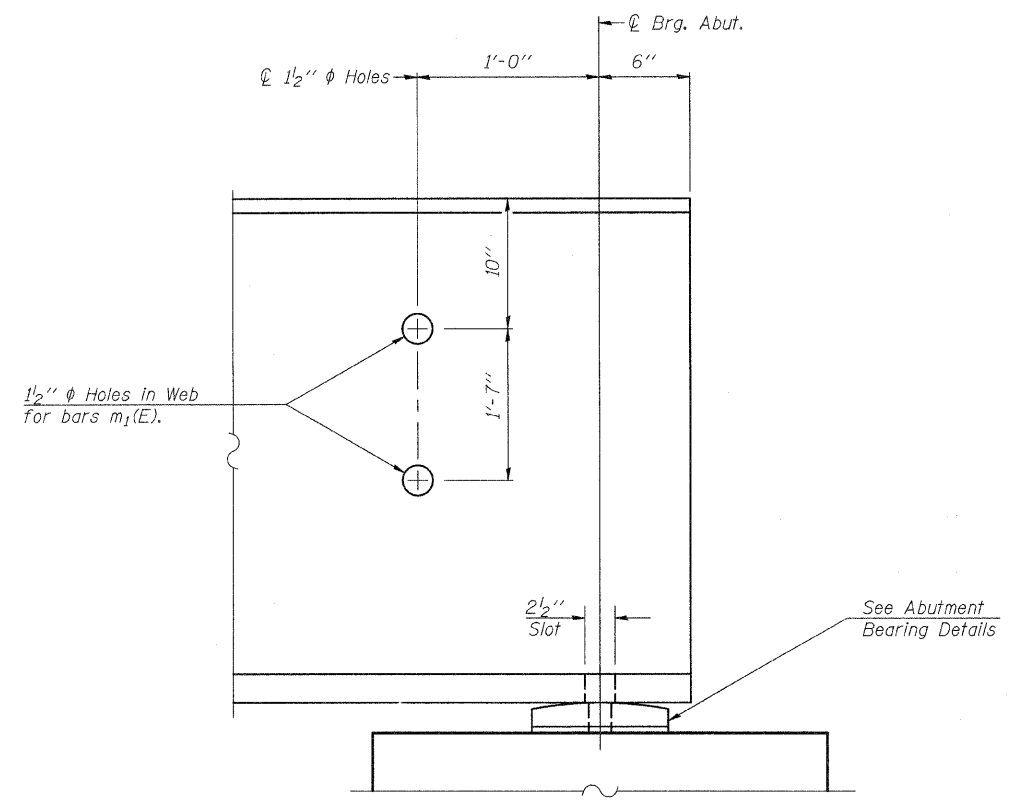


ELEVATION AT ABUTMENTS

ABUTMENT BEARING

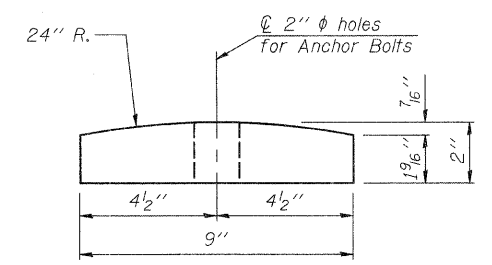
(10 Required)
Weight included with Structural Steel

Notes: Two 1/8" adjusting shims, of the dimension of the bottom bearing plate, shall be provided for each bearing in addition to all other plates or shims.
Anchor bolts shall be ASTM F1554 Grade 36, all-thread of the diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (F_y=36 ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
Structural steel plates of the Bearing Assemblies shall conform to the requirements of AASHTO M270 Grade 50W.



GIRDER END ELEVATION

(Typical)



BEARING PLATE DETAIL

BILL OF MATERIAL

Item	Unit	Total
Anchor Bolts, 1/2"	Each	20