

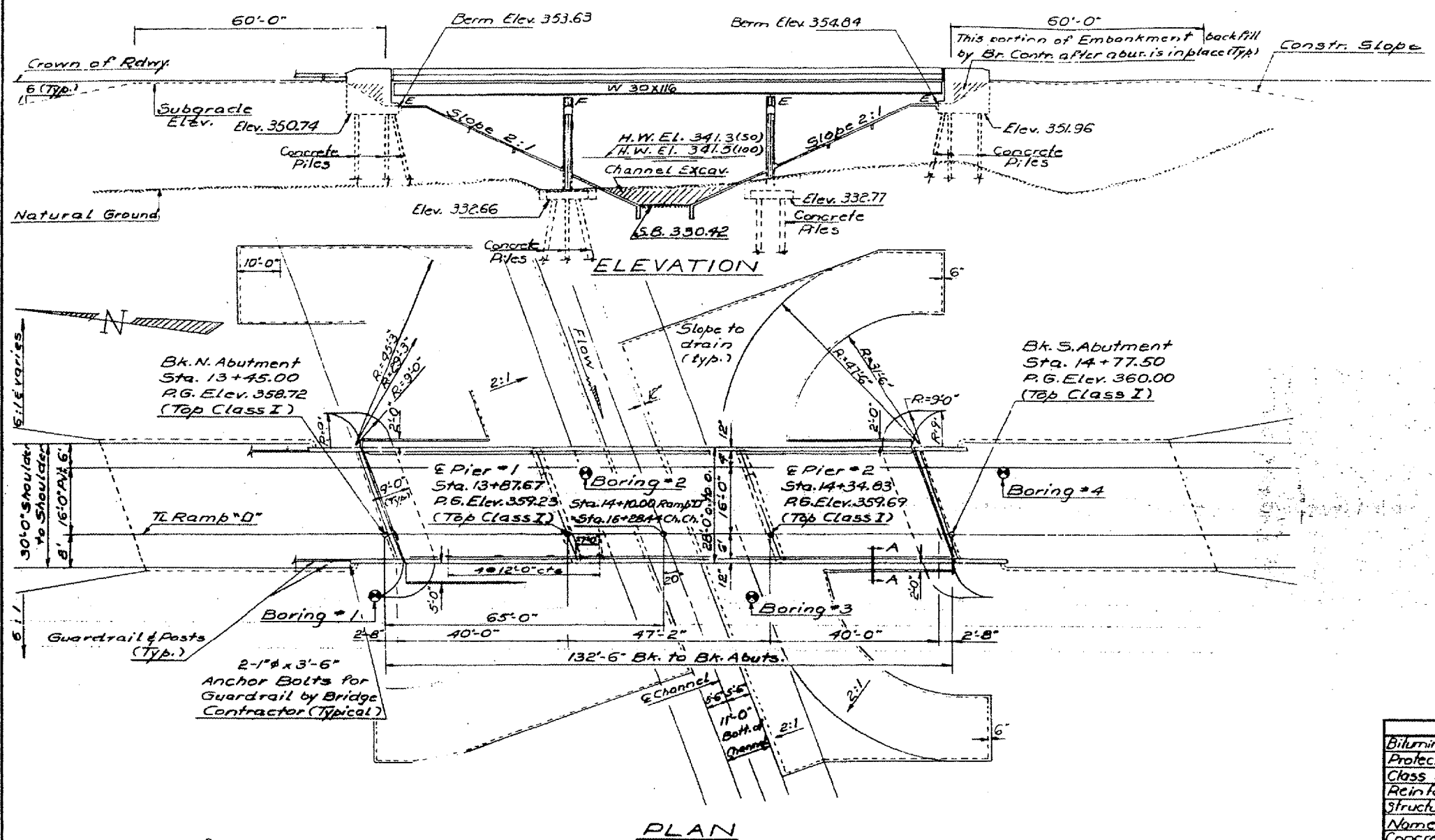
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

B.M. Survey marker - Eastbound Lanes
 (West Lanes) Sta. 199+00 - Elev. 351.54

1	2	3	4	5	6	7	8	9	10	11	12

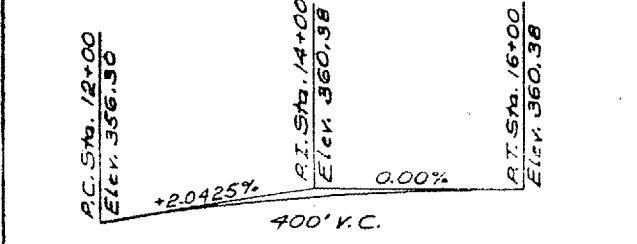
GENERAL NOTES

Fasteners shall be high strength bolts. Bolts 3/8" open holes 1/16", unless otherwise noted.
 See Special Provisions for Boring Data.
 The basic lead silico chromate paint shall be used for shop and field painting of Structural Steel.
 Field welding of construction accessories will not be permitted to the bottom flange of beams nor to the top flange for a distance equal to one-fourth the span length each way from the pier supports. Field welding in other areas will be permitted only when approved by the Engineer.
 Anchor bolts shall be set before bolting diaphragms over supports.
 Slope wall shall be reinforced with welded wire fabric 6"x6" mesh, weighing 58# per 100 sq. ft.
 Layout of slope walls may be varied in the field to suit ground conditions as directed by the Engineer.
 The Contractor shall drive one concrete test pile in a permanent location at Pier 2 as directed by the Engineer before ordering the remainder of piles.
 Calculated weight of Structural Steel = 74,290 Lbs.
 Concrete piles of abutments shall be driven in holes prepared through the embankment in accordance with Article 513.09(c) of the Standard Specifications.
 Backfill shall be placed behind the abutment after the superstructure has been poured and the falsework removed. See Article 502.11 of the Standard Specifications.
 The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments and piers.
 The concrete rail section above the mandatory construction joint at the top of the slab shall be constructed of Class X Concrete, except the aggregates shall conform to the requirements of Handrail Concrete.
 Protective Coat shall not be applied to surfaces to which Waterproofing Membrane System is applied.
 Bearing seal surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8 inch. Adjustment shall be made either by grinding the surface or by shimming the bearing. Two 6" adjusting shims, of the dimensions of the bottom bearing plate, shall be provided for each bearing in addition to all other plates or shims.
 The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Notch Toughness Zone 2. These components are the tension flanges, webs and all splice plate material of the wide flange beams.



TOTAL BILL OF MATERIAL

Item	Unit	Super	Sub	Total
Biluminous Concrete Surface Course, Class I	Tons	20		20
Protective Coat	Sq. Yd.	100	19	119
Class X Concrete	Cu. Yd.	122.3	156.8	279.1
Reinforcement Bars	Pound	29170	12980	42150
Structural Steel	Lump Sum			1
Name Plates	Each	1		1
Concrete Piles	Lin. Ft.		1680	1680
Test Piles Concrete	Each		1	1
Slope Wall (6')	Sq. Yd.		1606	1606
* Waterproofing Membrane System	Sq. Yd.	352		352
Preformed Joint Sealer (2")	Lin. Ft.	60		60
Cofferdam Excavation	Cu. Yd.		55	55
Cofferdam (Pier 1)	Each		1	1
Cofferdam (Pier 2)	Each		1	1



WATERWAY INFORMATION

Drainage Area 3915 Acres
 Design Discharge (50yr) 1785 c.f.s.
 Existing Opening (below 50yr HWE) Sq. Ft.
 Required Opening (below 50yr HWE) 323 Sq. Ft.
 Proposed Opening (below 50yr HWE) 321 Sq. Ft.
 Created Head for Design Flood 100-Year Discharge 2268 c.f.s.
 Created Head for 100-Year Flood Ft.

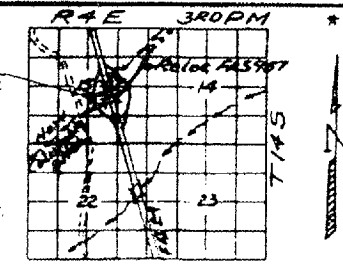
DESIGN STRESSES

Proposed Structure
 fc = 1200 psi (Deck Slab)
 fc = 1400 psi (Curb, parapet, sub)
 fs = 20000 psi (Reinf.)
 fs = 20000 psi (Struct. M183)
 vc = 75 psi (Ftg.)
 n = 10

PROPOSED GRADE PROFILE RAMP 'D'
 (along TL)

DESIGNED	Examined
CHECKED	Examined
DRAWN	Examined
CHECKED	Examined

Leading MS20-44 & Alt.
 1973 AASHTO, 1974 and 1975 Interim Specifications.
 Allow 25' for Future W.S.



LOCATION SKETCH

GENERAL PLAN & ELEVATION

RAM FOR INFORMATION ONLY:
 F.A.I. BRIDGE NO. 9 STRUCTURE
 064-0038