

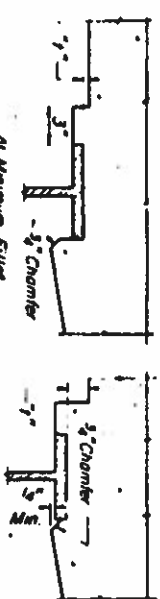
Frame 5

Location	Swiss	Theoretical Grade & Inflow	Theoretical Grade Electrons Adjusted For Dead Load Deduction
<i>£ Brg W. Abut.</i>	23621.756	518.485	518.485
	23661.743	518.254	518.201
	23667.170	518.080	518.088
	23663.717	517.883	517.888
	23664.104	517.674	517.672
	23663.151	517.462	517.459
<i>£ Abut B</i>	23660.361	517.277	517.277
	23670.348	517.061	517.075
	23670.135	516.852	516.868
	23670.222	516.622	516.637
	23670.309	516.408	516.445
	23671.238	516.183	516.234
	23670.213	515.953	516.009
	23670.270	515.744	515.717
	23670.257	515.520	515.552
	23670.344	515.300	515.300
	23670.231	515.084	515.089
<i>£ Abut C</i>	23673.056	515.008	515.009
	23673.643	514.793	514.795
	23673.630	514.562	514.567
	23673.617	514.336	514.335
	23673.591	514.106	514.113
<i>£ Brg E. Abut.</i>	236819.282	513.765	513.765

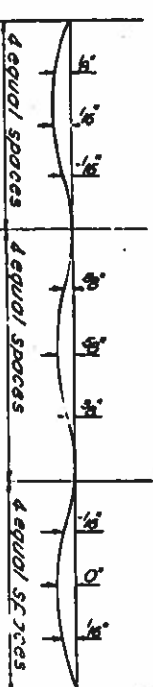
The diagram shows the elevation of a roof structure. It features a series of vertical rafters labeled 1 through 6 from top to bottom. Horizontal purlins are shown intersecting the rafters. The roof is divided into three bays by two vertical joints, labeled 'Joint B' and 'Joint C'. The ends of the roof are labeled 'Bay W. Abut.' and 'Bay E. Abut.'. The rafters are spaced at 10'-0" intervals. The purlins are spaced at 5'-0" intervals. The roof is supported by a series of vertical posts labeled 'Varies'.

DESIGNED	W. E. Brown
CHECKED	W. E. Brown
DRAWN	S. G. Borton
CHECKED	W. E. Brown

EXAMINED Sept 7 1886
 PASSED Page & T. D. Emerson Jr
 APPROVED W. J. Atwater



To determine "1" above all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown above. These elevations subtracted from the "Theoretical Grade Elevation Adjusted for Dead Load Deflection" shown above minus slab thickness, equals the fillet heights "1" above top flange of beams.



DEAD LOAD DEFLECTION DIAGRAM

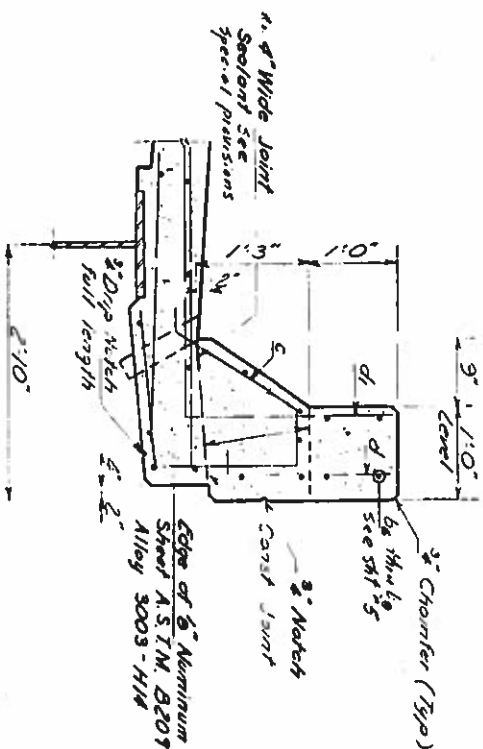
(includes weight of concrete only)
 Note: The above deflections are not to be used in the first
 if the engineer is working from the loads & reactions
 adjusted for dead load deflections as shown above

TOP OF SLAB ELEVATIONS
F.A.I. RT. 57 SEC. 41-34B
JEFFERSON COUNTY
STA. 2367 D+06.95

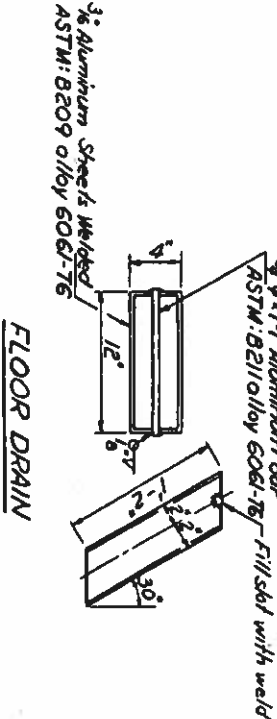
REPORT NO.	POSITION	DEPARTMENT	REPORTING SUPERVISOR	REPORT DATE
000 37	ENGINE	JEFFERSON	50	1
UNIT, SPECIAL DEPT., NO. 9		BARRETT	TOL. AND REMARKS	

13 EML:574





CURB SECTION
Cost of Aluminum Drains and Sheets
shall be incidental to Class X Conc.
* * * At South Side only

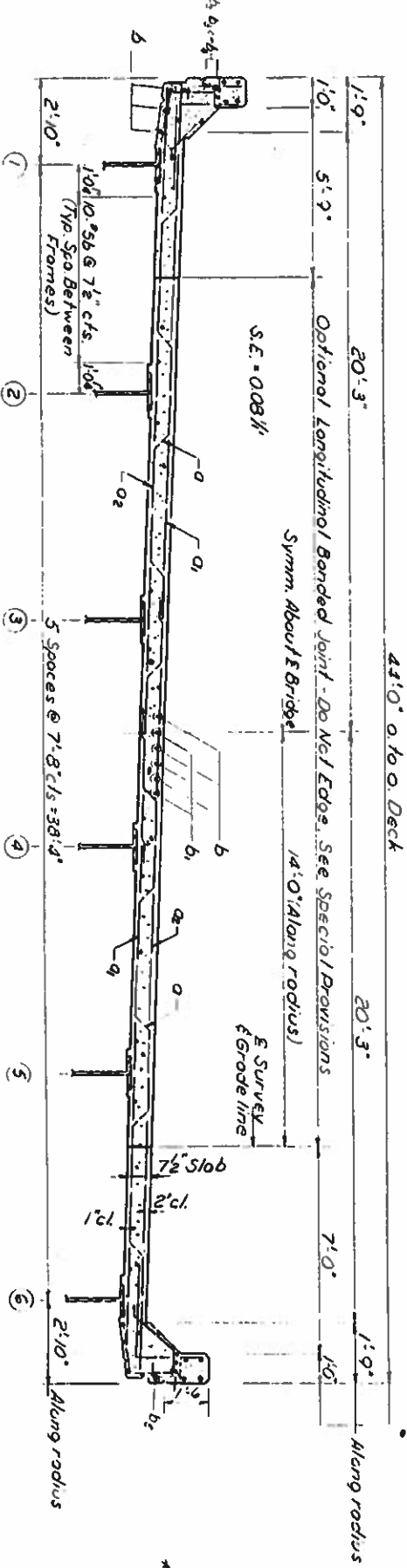
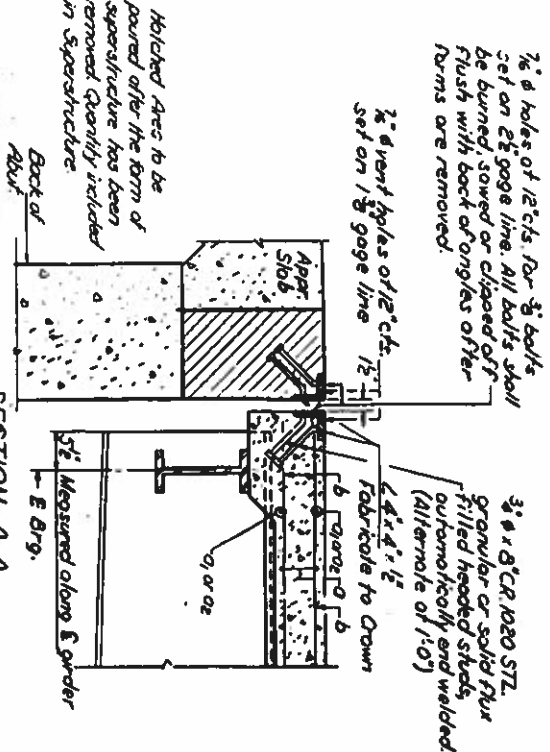


FLOOR DRAIN

VALUE OF A & B

A	B
NCurb	5 1/2" 1-9 1/2"
SCurb	2 1/2" 1-6 3/4"

SECTION A-A



NEAR MIDSPAN

CROSS SECTION

NEAR JOINTS

BILL OF MATERIAL

Bar No.	Size	Length	Shape
1	3/8"	23' 7"	W
2	3/8"	19' 0"	W
3	3/8"	26' 9"	W
4	3/8"	26' 9"	W
5	3/8"	26' 9"	W
6	3/8"	26' 9"	W
7	3/8"	26' 9"	W
8	3/8"	26' 9"	W
9	3/8"	26' 9"	W
10	3/8"	26' 9"	W
11	3/8"	26' 9"	W
12	3/8"	26' 9"	W
13	3/8"	26' 9"	W
14	3/8"	26' 9"	W
15	3/8"	26' 9"	W
16	3/8"	26' 9"	W
17	3/8"	26' 9"	W
18	3/8"	26' 9"	W
19	3/8"	26' 9"	W
20	3/8"	26' 9"	W
21	3/8"	26' 9"	W
22	3/8"	26' 9"	W
23	3/8"	26' 9"	W
24	3/8"	26' 9"	W
25	3/8"	26' 9"	W
26	3/8"	26' 9"	W
27	3/8"	26' 9"	W
28	3/8"	26' 9"	W
29	3/8"	26' 9"	W
30	3/8"	26' 9"	W
31	3/8"	26' 9"	W
32	3/8"	26' 9"	W
33	3/8"	26' 9"	W
34	3/8"	26' 9"	W
35	3/8"	26' 9"	W
36	3/8"	26' 9"	W
37	3/8"	26' 9"	W
38	3/8"	26' 9"	W
39	3/8"	26' 9"	W
40	3/8"	26' 9"	W
41	3/8"	26' 9"	W
42	3/8"	26' 9"	W
43	3/8"	26' 9"	W
44	3/8"	26' 9"	W
45	3/8"	26' 9"	W
46	3/8"	26' 9"	W
47	3/8"	26' 9"	W
48	3/8"	26' 9"	W
49	3/8"	26' 9"	W
50	3/8"	26' 9"	W
51	3/8"	26' 9"	W
52	3/8"	26' 9"	W
53	3/8"	26' 9"	W
54	3/8"	26' 9"	W
55	3/8"	26' 9"	W
56	3/8"	26' 9"	W
57	3/8"	26' 9"	W
58	3/8"	26' 9"	W
59	3/8"	26' 9"	W
60	3/8"	26' 9"	W
61	3/8"	26' 9"	W
62	3/8"	26' 9"	W
63	3/8"	26' 9"	W
64	3/8"	26' 9"	W
65	3/8"	26' 9"	W
66	3/8"	26' 9"	W
67	3/8"	26' 9"	W
68	3/8"	26' 9"	W
69	3/8"	26' 9"	W
70	3/8"	26' 9"	W
71	3/8"	26' 9"	W
72	3/8"	26' 9"	W
73	3/8"	26' 9"	W
74	3/8"	26' 9"	W
75	3/8"	26' 9"	W
76	3/8"	26' 9"	W
77	3/8"	26' 9"	W
78	3/8"	26' 9"	W
79	3/8"	26' 9"	W
80	3/8"	26' 9"	W
81	3/8"	26' 9"	W
82	3/8"	26' 9"	W
83	3/8"	26' 9"	W
84	3/8"	26' 9"	W
85	3/8"	26' 9"	W
86	3/8"	26' 9"	W
87	3/8"	26' 9"	W
88	3/8"	26' 9"	W
89	3/8"	26' 9"	W
90	3/8"	26' 9"	W
91	3/8"	26' 9"	W
92	3/8"	26' 9"	W
93	3/8"	26' 9"	W
94	3/8"	26' 9"	W
95	3/8"	26' 9"	W
96	3/8"	26' 9"	W
97	3/8"	26' 9"	W
98	3/8"	26' 9"	W
99	3/8"	26' 9"	W
100	3/8"	26' 9"	W

* * * This quantity includes 6310 lbs
Reinforcement Bars listed on
Sheet # 5.

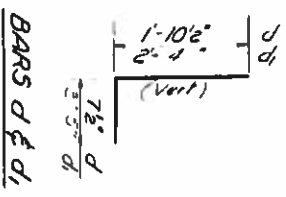
SLAB DETAILS
FAIRFAX SEC. 413HB
JEFFERSON CO.
STA 2387D+0685

DESIGNED BY: M. H. H. H.

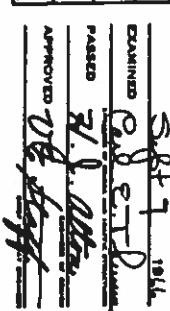
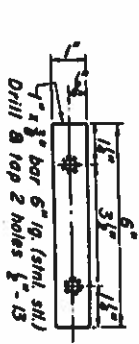
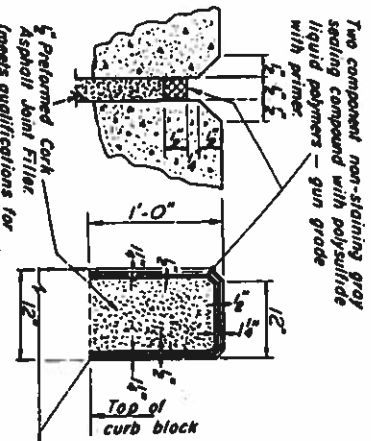
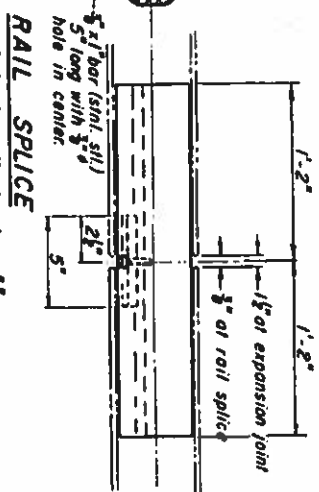
CHECKED BY: M. H. H. H.

APPROVED BY: M. H. H. H.

Revised 11-1-67



PLAN

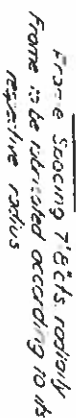


PARAPET JOINT DETAIL

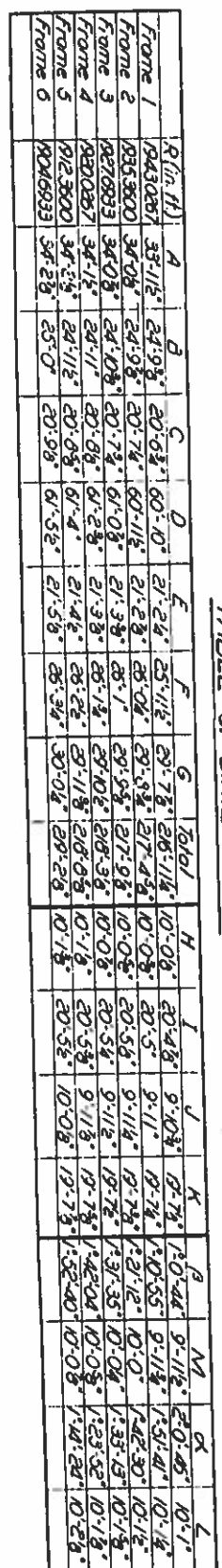
ALUMINUM HANDRAIL
F.A.I. RT. 57 SEC 41-3HB
JEFFERSON COUNTY
STA 33870+0505

Box	No.	Size	Length	Shape
b ₁	56	#5	13'-0"	—
b ₂	24	#5	14'-9"	—
b ₇	32	#5	10'-3"	—
d	439	#4	2'-6"	—
d ₁	439	#6	5'-9"	—
Reinforcement Bars			Lbs	6310
Aluminum Handrail			Ln. Ft.	439

This quantity is included in the quantity of Sheet #4.



Free Spacing Effects radii
Frame is be calculated according to its
respective radius

[illegible][illegible]

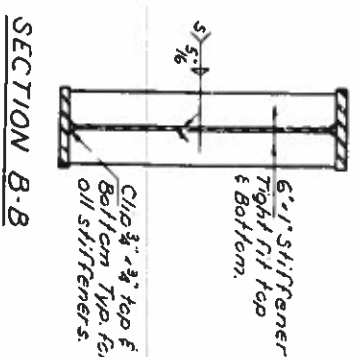
specifications.
Weight of Structural Steel billed on Sheet #4
Contact Surface of oil field splices shall be free
of all oil and paint.

STRUCTURAL STEEL
FAIRFAX SEC. 41-3HB
JEFFERSON COUNTY
STA 2387D+06.95

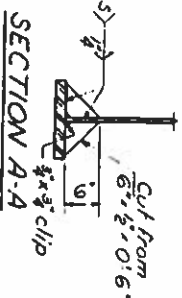
Subj 7 1966
EXAMINED Carl E. D. Sullivan
PASSED 21 Oct 66
APPROVED W. J. [Signature]
DATE OF REVIEW 21 Oct 66



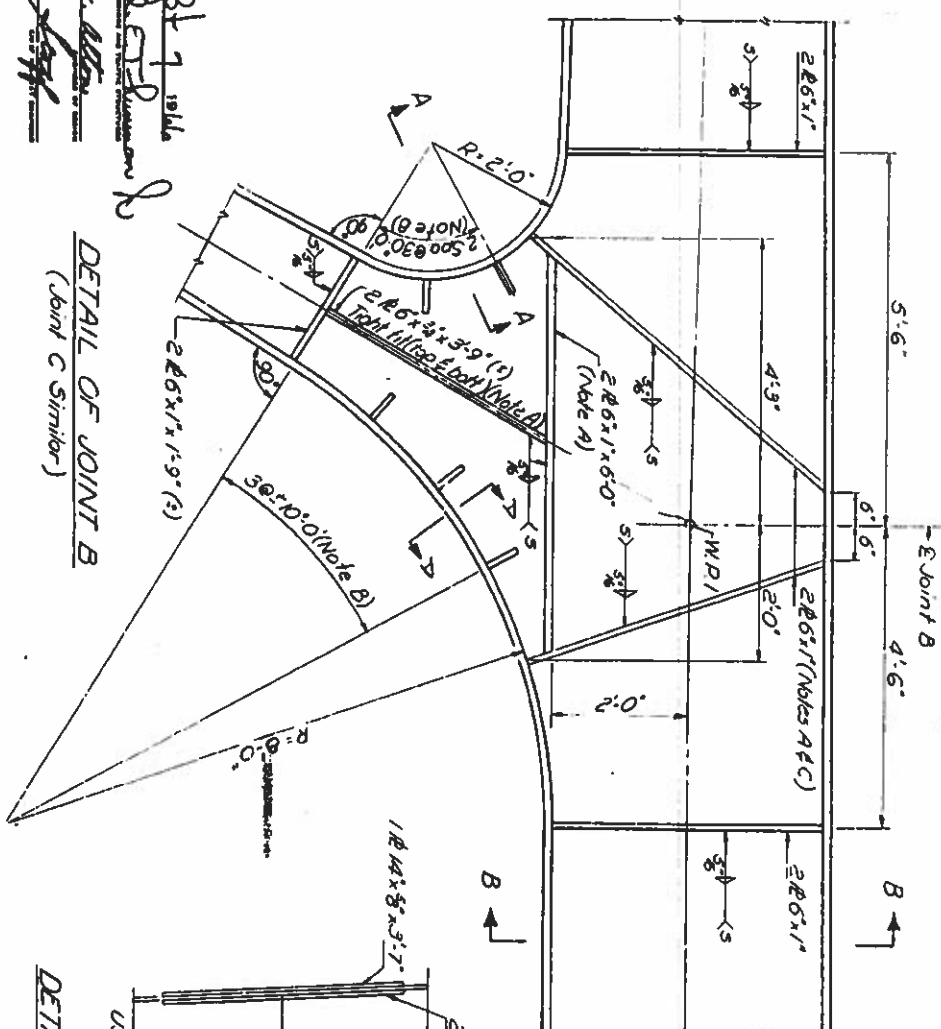
TYPICAL CROSS SECTION



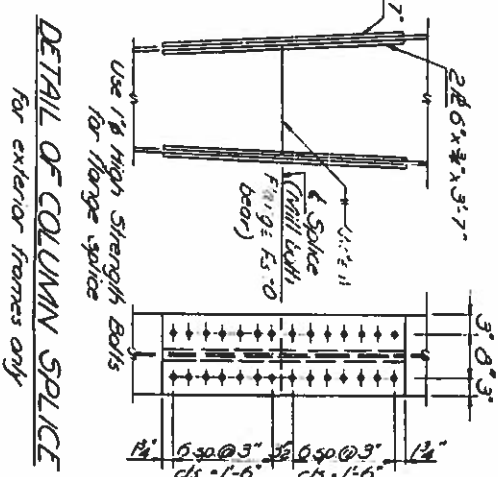
SECTION B-B



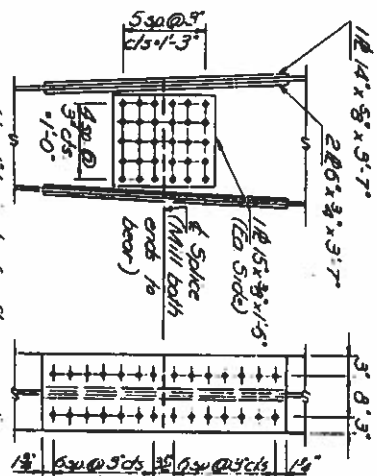
SECTION A-A



DETAIL OF JOINT B



DETAIL OF COLUMN SPLICE
for exterior frames only



DETAIL OF COLUMN SPLICE
for interior frames only

TOP OF WEB ELEVATIONS (For Fabrication Only)							
	1 Bay W4	1 Splice 1	1 Bay 8 Splice 2	1 Splice 3	1 Bay 8 Splice 4	1 Bay 8	
Frame 1	520.03	519.89	519.25	517.55	517.09	516.52	515.88
Frame 2	519.93	519.19	518.65	518.10	516.85	515.81	515.12
Frame 3	518.22	518.48	517.23	517.48	516.3	515.67	514.45
Frame 4	518.51	517.17	517.22	516.17	515.42	514.38	513.64
Frame 5	517.79	517.05	516.50	516.04	514.85	513.64	512.99
Frame 6	517.06	516.32	515.77	515.51	513.86	513.49	512.91
							512.26

For Dimensions A thru K see sheet 10.
Column splices are optional, & shall
be omitted if columns can be shipped
in one piece.

Note:
Work this sheet with sheet #6. See
sheet #8 for all other details.

Note A:
Frames 1/6 shall have stiffeners on inside face with the following deviations; Plate $6 \times 1' 10.6 \times 1/8$ f. Plate $6 \times 2' 10.6 \times 1/4$

Note B:
The exterior face of frames 1/6 use A @ $10^{\circ} 0' f$ @ $30^{\circ} 0'$ respectively.

Note C:
Cut the Stiffener to miss diaphragm if necessary

1221
Jas In Accordance with A.W.S Dec-6

ELEVATION

DESIGNED	<i>W. L. Allen</i>
CHECKED	<i>W. L. Allen</i>
DRAWN	THOMAS A. LEWIS
CHECKED	<i>W. L. Allen</i>
APPROVED	<i>W. L. Allen</i>
EXAMINED	<i>Sgt 7 1946</i>
SIGNED AND SEALED BY THE SECRETARY	
<i>Paul E. D.</i>	
SIGNED AND SEALED BY THE SECRETARY	
<i>W. L. Allen</i>	
APPROVED BY THE SECRETARY	
<i>W. L. Allen</i>	
SIGNED AND SEALED BY THE SECRETARY	
<i>W. L. Allen</i>	

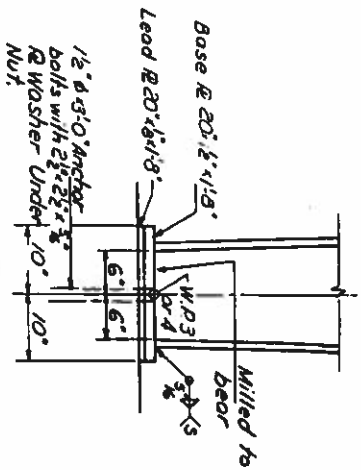
DETAIL OF JOINT B
(Joint C Similar)

DETAIL OF COLUMN SPLICE

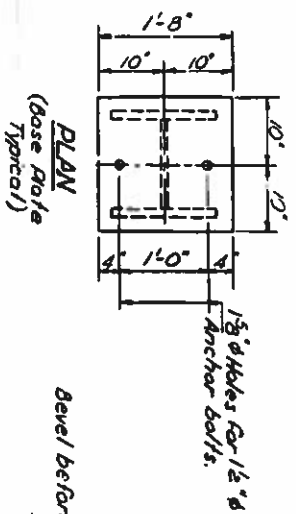
DETAIL OF COLUMN SPLICE

FRAME ELEVATION
FA.1.R.57 SEC.41-3H
JEFFERSON CO.

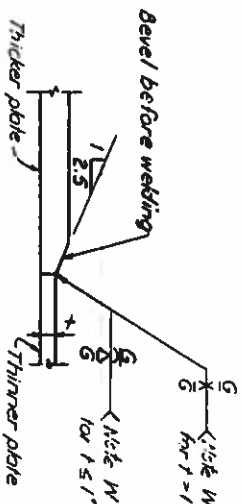
Re NH 5-12-61 Detail of Column Splice (exterior frame) &
Nth w added



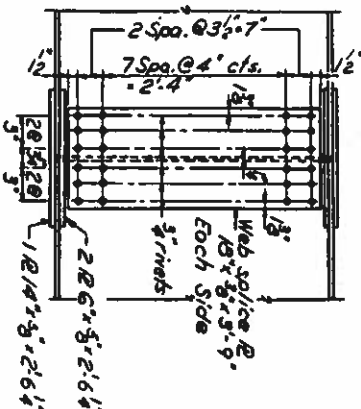
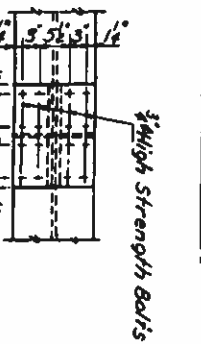
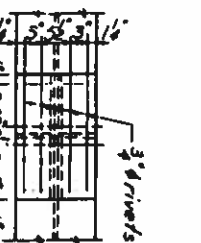
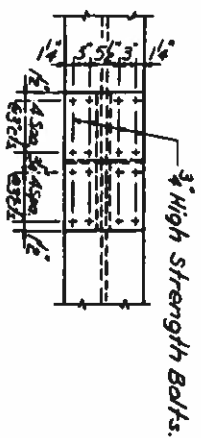
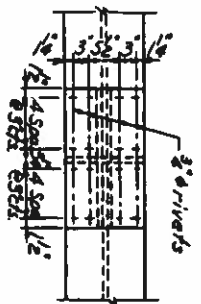
DETAIL B
(Relate 30° to get
proper orientation)



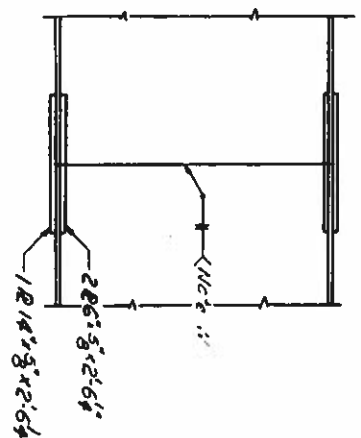
PLAN
(Base Angle
Typical)



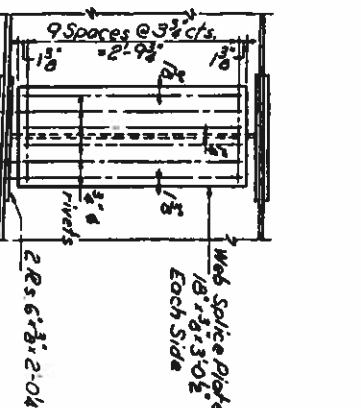
SHOP FLANGE SPlice



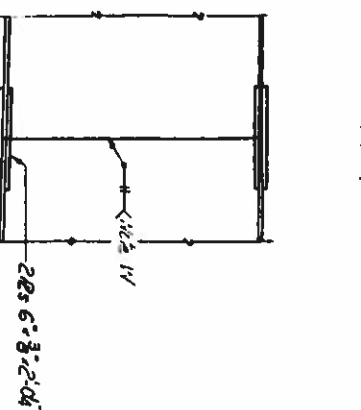
FIELD SPlice
(For Splices 2 & 3
Inferior frames only)



FIELD SPlice
(For Splices 2 & 3
Exterior frames only)

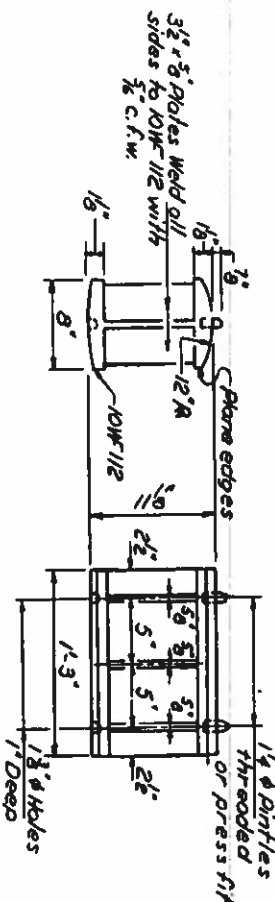


FIELD SPlice
(For Splices 1 & 4
Inferior frames only)

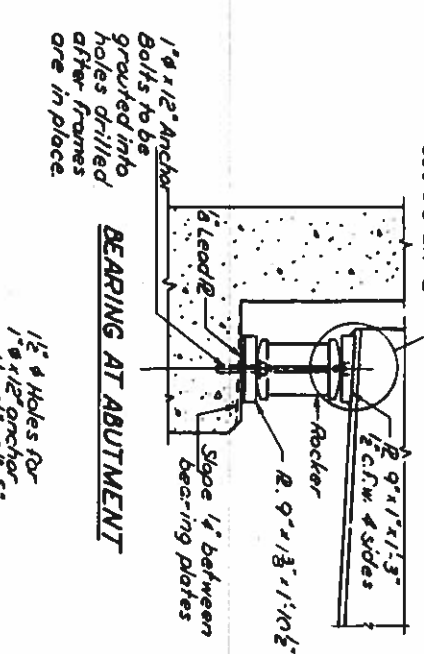


FIELD SPlice
(For Splices 1 & 4
Exterior frames only)

Note: Joints in accordance
with AWS D2.0-66.

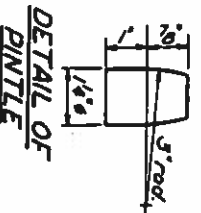


ROCKER

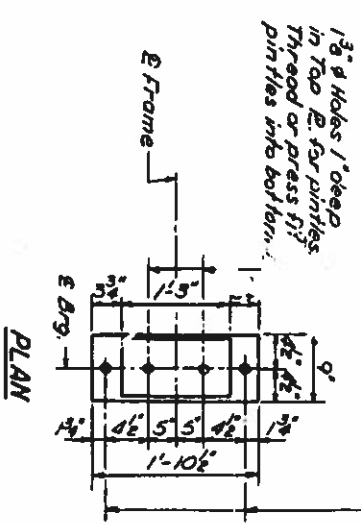


BEARING AT ABUTMENT

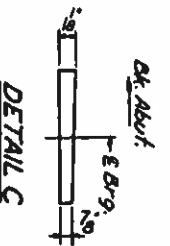
DESIGNED	AL. Henry
CHECKED	W. M. C. Lewis
DRAWN	Thomas A. Lewis
CHECKED	W. M. C. Lewis
EXAMINED	W. M. C. Lewis
PAVED	W. M. C. Lewis
APPROVED	W. M. C. Lewis



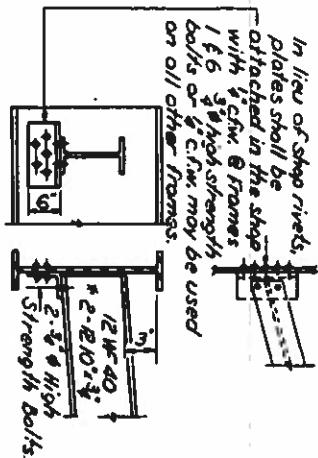
DETAIL OF
PINNLE



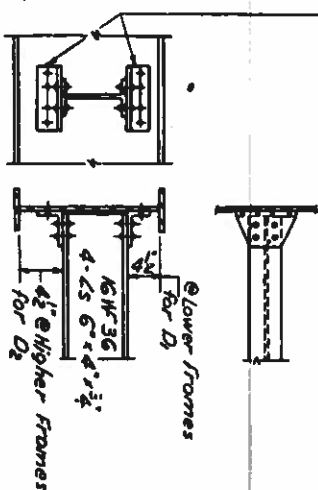
PLAN



DETAIL C



DIAPHRAGM D
10 Required
*Bend 18 to fit slope.

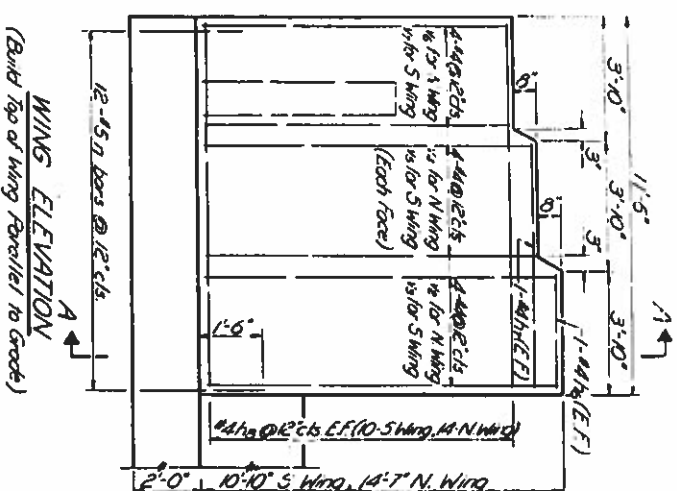


DIAPHRAGM D, 1/2 D
D, 20 Required
D, 30 Required

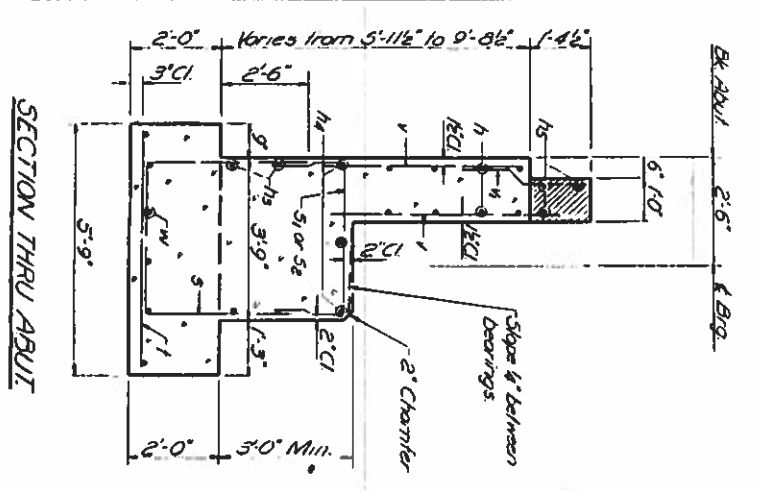
Note:
Work this sheet with Sheets #6 & #7
Contract surfaces of all field splices
shall be free of oil or fat.

STRUCTURAL STEEL DETAILS
F.A.R. 57 SEC. 41.34B
JEFFERSON COUNTY
STATION 23270+05.95

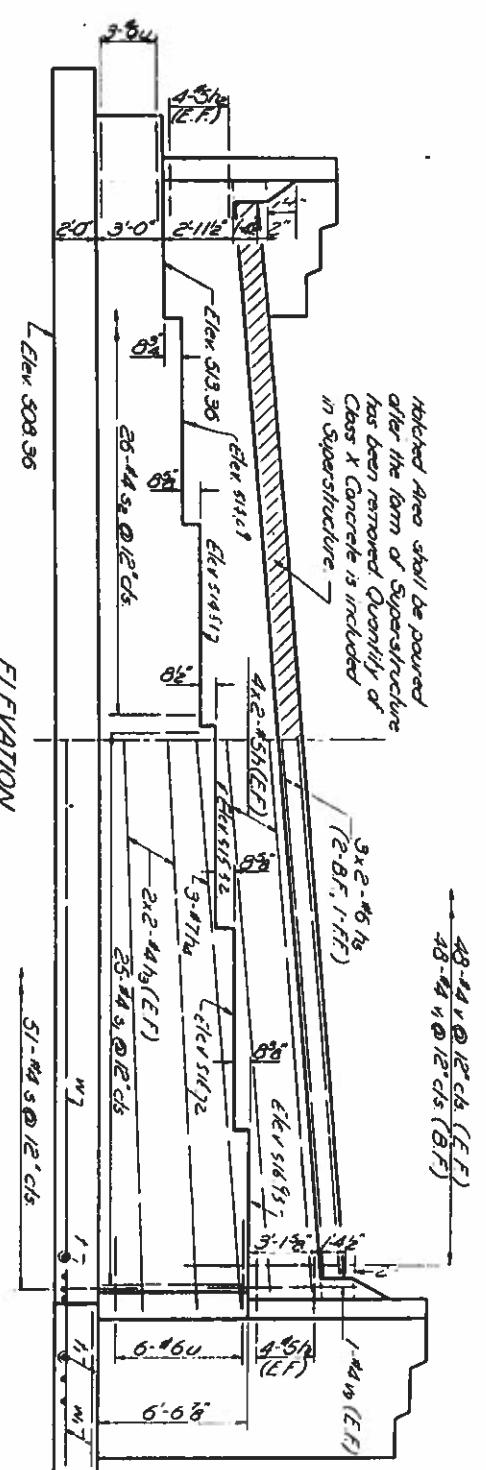
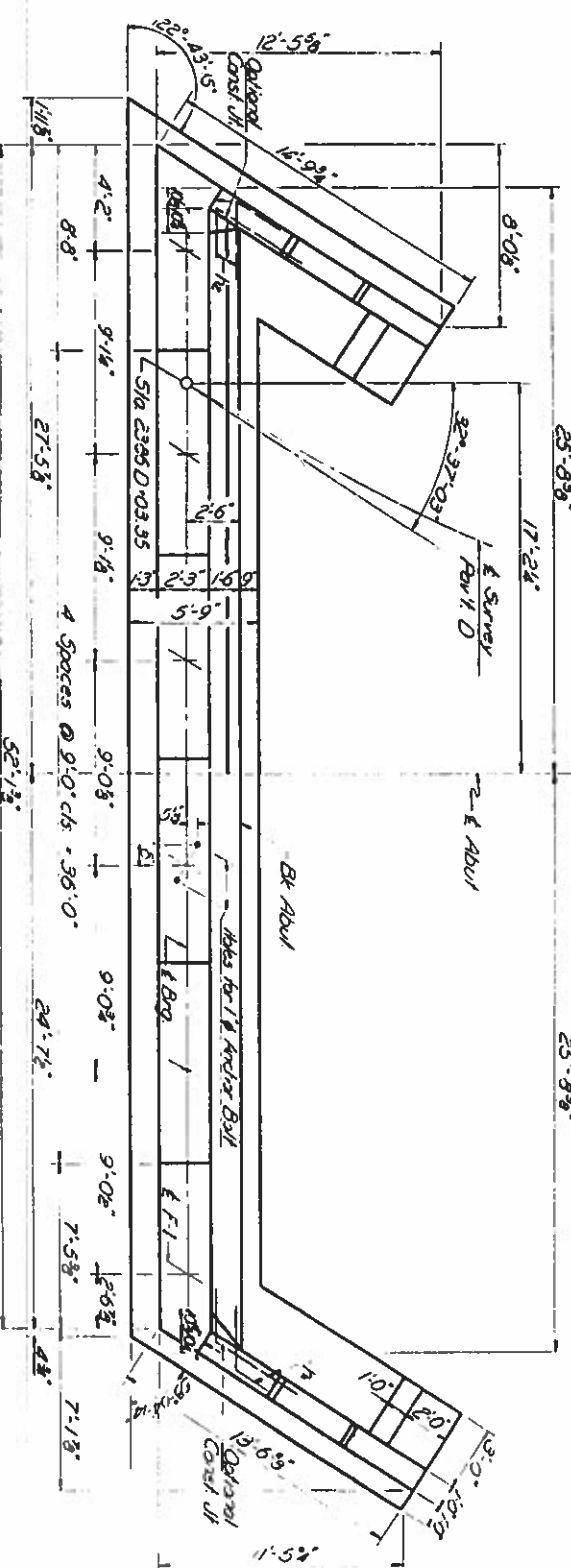
Rev W. M. C. Lewis 6-12-67 Field Splice (Exterior frames only) 1/3
Note in detail.



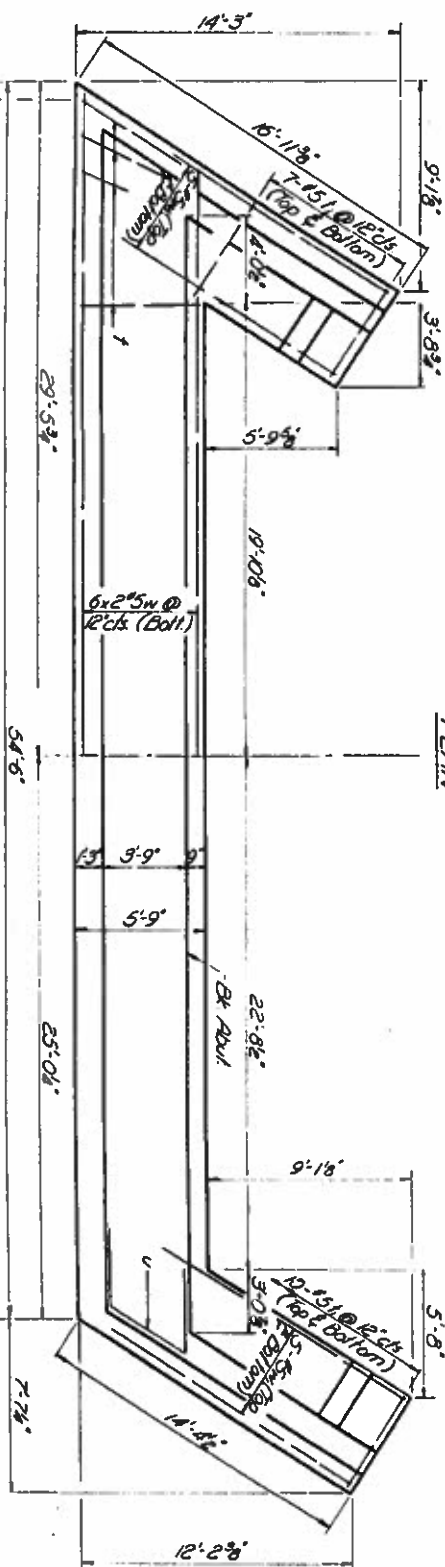
WING ELEVATION
(Top of Wing Parallel to Grade)



SECTION THRU ABUT.

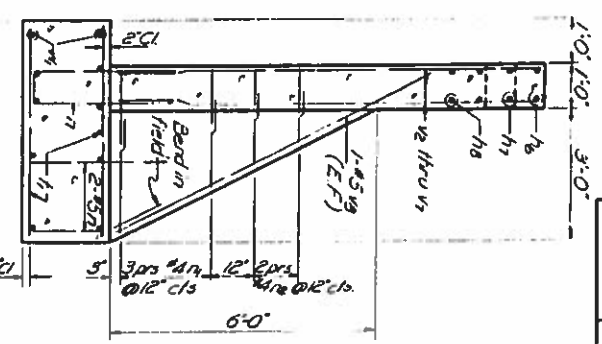
ELEVATION

PLAN

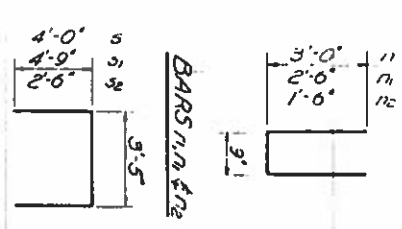


FOOTING PLAN

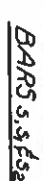
Note: Bars indicated thus 6x2-8 etc. indicate 6 lines of bars with 2 lengths each line.



SECTION A-A



BARS n.n. #n2



BARS 5, 4 & 3

BAR U

四

BILL OF MATERIAL				
Bar	No	Size	Length	Shape
h	16	#5	25'-6"	—
h	8	#5	5'-0"	┘
h	8	#5	3'-0"	┘
h	8	#4	25'-6"	—
h	3	#7	5'-9"	—
h	6	#6	24'-6"	—
h	4	#4	3'-6"	—
h	4	#4	7'-3"	—
h	4	#4	11'-2"	—
g	28	#5	6'-9"	┘
g	12	#4	5'-9"	┘
g	8	#4	3'-9"	┘
s	51	#4	11'-5"	┘
s	25	#4	12'-11"	┘
s	26	#4	8'-5"	┘
1	55	#5	5'-3"	—
1	34	#5	4'-9"	—
u	9	#6	10'-0"	┘
v	96	#4	5'-6"	—
v	48	#4	3'-3"	—
v	8	#4	14'-5"	—
v	8	#4	10'-6"	—
v	8	#4	13'-9"	—
v	8	#4	10'-0"	—
v	8	#4	13'-0"	—
v	8	#4	9'-2"	—
v	4	#5	7'-6"	—
v	4	#4	6'-3"	—
w	12	#5	27'-9"	—
w	20	#5	11'-0"	—
Class X Concrete Cylinders 892				
Reinforcement Bars Lbs. 4670				

WEST ABUTMENT
FAI RT 57 SEC 41-34B
JEFFERSON COUNTY
STA 2387 D+06 95

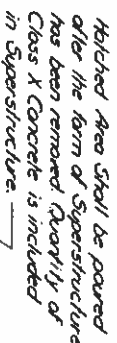
Für NH 6-1-17 den 3. Elektion



11-11-11

AS REVISED
WEST ABUTMENT
FAI RT. 37 SEC 21-34B
JEFFERSON COUNTY
STA 2367D+06 95

Rev. 9-21-67 AM incorrect and then circled, corrected then given.

BILL OF MATERIAL

59-451 @ 12' c/s (Bottom)
(For 3 bars N. End)

FOOTING PLAN

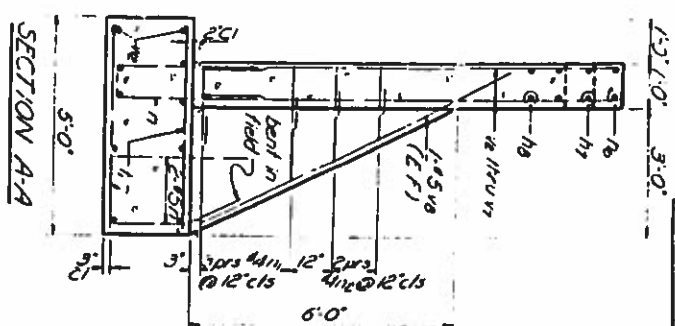
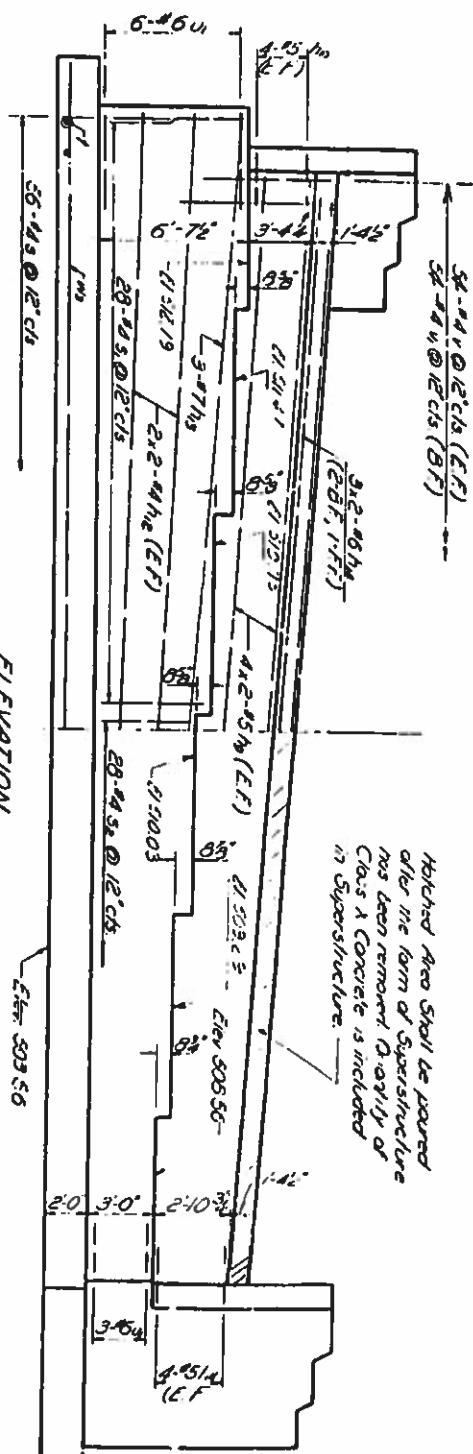
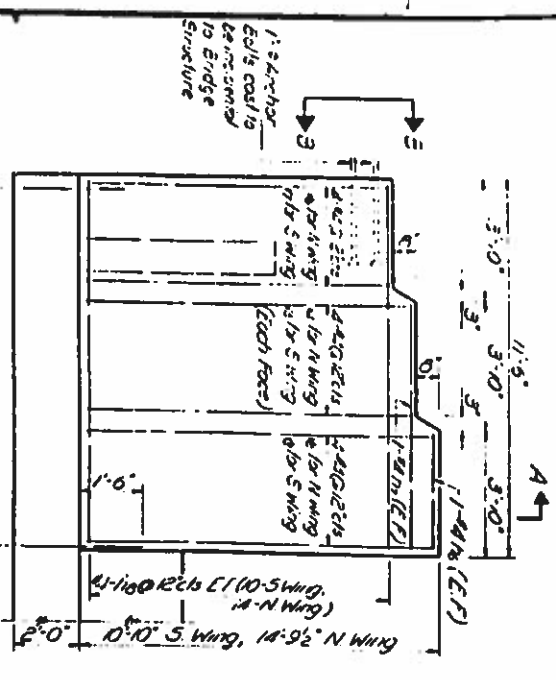
Note: Bars indicated thus 6x2 - #5 etc. indicates 6 lines of bars with 2 length: each line.

BAR V.

EAST ABUTMENT
EAL RT 57 SEC 41-38B
JEFFERSON COUNTY
STA. 2387D+06.95

STATE OF ILLINOIS
DEPARTMENT OF PUBLIC WORKS & BUILDINGS
DIVISION OF HIGHWAYS

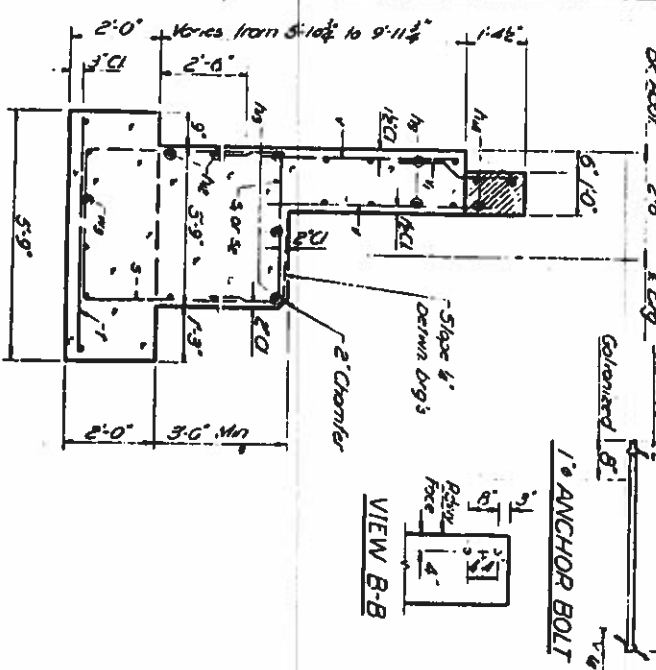
Sheet No.	10
Project No.	57A 2387D-0695
Date	1/8/68
Drawn by	SG
Checked by	SG
Approved by	SG



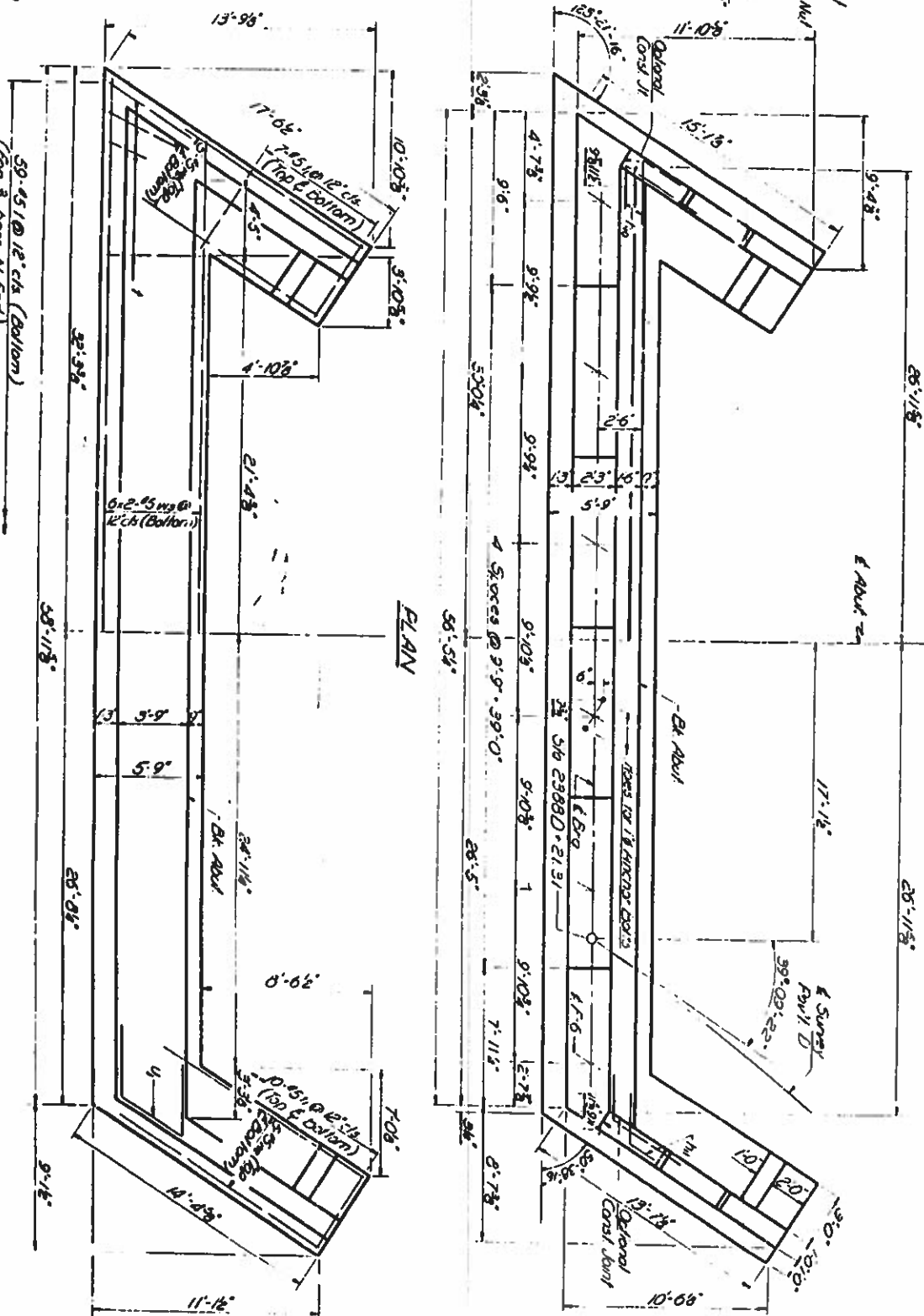
BILL OF MATERIAL

Item	Quantity	Unit	Material
1	1.00	cu yd	Concrete
2	1.00	sq ft	Formwork
3	1.00	lb	Reinforcing Steel
4	1.00	sq ft	Formwork
5	1.00	lb	Reinforcing Steel
6	1.00	sq ft	Formwork
7	1.00	lb	Reinforcing Steel
8	1.00	sq ft	Formwork
9	1.00	lb	Reinforcing Steel
10	1.00	sq ft	Formwork
11	1.00	lb	Reinforcing Steel
12	1.00	sq ft	Formwork
13	1.00	lb	Reinforcing Steel
14	1.00	sq ft	Formwork
15	1.00	lb	Reinforcing Steel
16	1.00	sq ft	Formwork
17	1.00	lb	Reinforcing Steel
18	1.00	sq ft	Formwork
19	1.00	lb	Reinforcing Steel
20	1.00	sq ft	Formwork

VIEW B-B



PLAN



DESIGNED BY	SG
CHECKED BY	SG
DRAWN BY	SG
APPROVED BY	SG

FOOTING PLAN

Note: Bars indicated with 6x2-#5 etc. indicates 6 lines of bars with 2 length, each line.

AS Revised 1-8-68

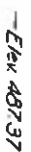
EAST ABUTMENT
EAT RT 57 SEC. 41-34B
JEFFERSON COUNTY
57A 2387D-0695

Rev. W.H. 1-8-68 Remove Brush Blot

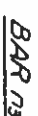
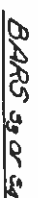
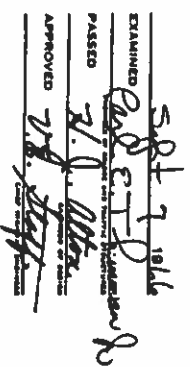
[illegible]

* 3 pairs each under
F-4 thru F-6
4 pairs each under
F-1 thru F-3

Min Bar Lcp - 20 diameter
unless otherwise noted.
All exposed edges shall have
Standard 3/4" Chamfers.
Bend 13, 14, or the bar in field
to fit if necessary.



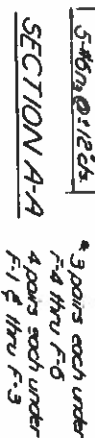
(At Rt. 1 To Local Tanager
Station 2387010695)



FOOTING AT HINGE 1
FAT RT 57 SEC. 41-3HB
JEFFERSON COUNTY
STA 2387D+06.95

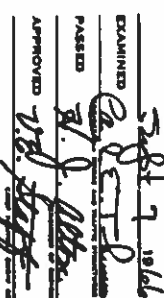
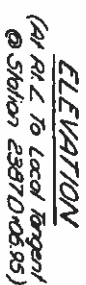


F-1	F-2	F-3	F-4	F-5	F-6
5'-11 1/2"	5'-6 1/2"	5'-15"	4'-0 5/8"	4'-3 3/8"	3'-10 1/2"



LIVE GROWN CATTLE				
Bar	No.	Size	Length	Stage
12	108	#6	5'-3"	—
33	42	#4	9'-8"	—
34	60	#4	7'-8"	—
32	24	#5	13'-4"	—
38	38	#4	3'-2"	—
12	412	#8	10'-9"	—
13	85	#5	10'-0"	—
110	24	#6	5'-9"	—
111	48	#6	6'-9"	—
112	24	#6	5'-9"	—
115	32	#6	20'-6"	—
Class X Cows			21-165	19.5
Reinforced Bars			18s	1110

*Min. Bar top = 20 diameter unless otherwise noted.
All exposed edges shall have Standard $\frac{3}{8}$ " chamfers.
Bend $\frac{3}{8}$, $\frac{1}{4}$, or the bar in field to fit if necessary.*



FOOTING AT HINGE 2
EAT. RT. 57 SEC. 41-34B
JEFFERSON COUNTY
57A 2387 D-06.95

[illegible]

1 FEB 1968 DIST J

3HB

<i>N</i> -Standard Penetration Test-Blast test to drive 2" O.D. Split Spoon Sampler 12" with 140 # hammer falling 30"	Qu-Unconfined Compressive Strength-461 w-Water Content- percentage of oven dry weight- %	Type failure
		B-Bubble Failure
		S-Shear Failure
		E-Estimated Value

BORING DATA
F.A.I. PT. 57 SEC. 41-3HB
JEFFERSON COUNTY
STATION 2387D+06.95

ALB THOR

Sgt I 1966

EXAMINED

Capt E D Pearson Jr

PASSED

A. J. Miller

APPROVED

N.E. Staff

RECEIVED