



Luminaire Performance Table

Reset Form

Project

Date	Contract Number	Section Number	County
01/25/24	D264C24	(201-3)K	Winnebago
Marked Route Number		Municipality	
I-39 / US-20		VILLAGE OF CHERRY VALLEY	

Roadway

Lane Width	# of Lanes	Median Width	I.E.S. Surface Classification	Q-Zero Value
48'	4	27'	R3	0.07

Structure

Mounting Height	Arm Length	Set-Back	Number of Luminares (Highmast & Sign Lighting Only)
22.5'	N/A	1'	N/A

Luminaire

Description	I.E.S. Lateral Distribution	I.E.S. Distribution (Transverse)	
UNDERPASS LUM., LED, DESIGNATION E	MEDIUM	TYPE III	
Total Light Loss Factor (LLF)	B-U-G Rating	Shields	Dimming Protocol
0.7	U=0	0	0

Layout

Spacing (to Nearest 5 ft)	Configuration (Opposite, Staggered, 1 Sided, or Median)
60'	OPPOSITE

Performance

Average Illuminance, $E_{AVE}$ (fc)	Uniformity Ratio, $E_{AVE}/E_{MIN}$		
1.0	3.0		
Average Luminance, $L_{AVE}$ (cd/m <sup>2</sup> )	Uniformity Ratio, $L_{AVE}/L_{MIN}$	Uniformity Ratio, $L_{MAX}/L_{MIN}$	Veiling Luminance Ratio, $L_v/L_{AVE}$
N/A	N/A	N/A	N/A

Light Tresspass

Distance to ROW (behind pole)	Max. Horizontal Illuminance at ROW, $E_H$	Max. Vertical Illuminance at ROW, $E_v$
N/A	N/A	N/A

Notes

LED, UNDERPASS LUMINAIRE,OPPOSITE, WALL MOUNTED ON I-39 OVER UPRR UNDERPASS

Printed 01/25/24

BDE 5630 (04/10/19)

MODEL: UPRR Underpass Performance Table [Sheet]  
FILE NAME: c:\paword\benesch\_projects\projects\0173716\D264C24\_ML-shr-PerformanceTable2.dgn



USER NAME = Ifranceschina	DESIGNED - R. SCARIA	REVISED -
	DRAWN - R. SCARIA	REVISED -
PLOT SCALE = \$SCALE\$	CHECKED - G. THIESSE	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/14/2025	REVISED -

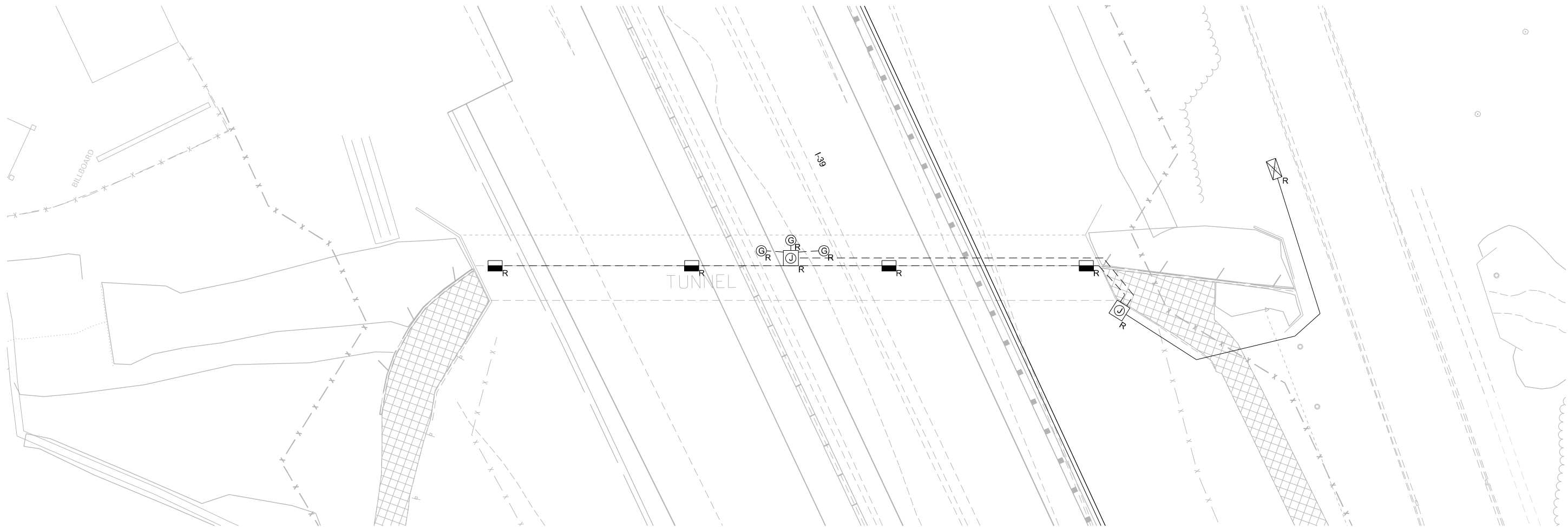
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

PROPOSED LIGHTING DETAILS  
PERFORMANCE TABLE

SCALE: SHEET 16 OF 21 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	501
		CONTRACT NO. 64C24		
		ILLINOIS	FED. AID PROJECT	

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)



LEGEND THIS SHEET ONLY

- PEDESTRIAN LUMINAIRE TO BE REMOVED
- GAS SENSOR TO BE REMOVED
- CONDUIT ATTACHED TO STRUCTURE WITH CABLE, TO BE REMOVED
- UNDERGROUND CONDUIT WITH CABLE, TO BE REMOVED
- EXISTING CONTROLLER AND FOUNDATION TO BE REMOVED



MODEL: CVP Lighting Removal - Plan 1 [Sheet]  
FILE NAME: c:\paword\benesch\_projects\projects\0173716\0264C24\_ML-sh-H\_GT-CVP-REM.dgn



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PLOT DATE	= 8/12/2024

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DRAWN	-
CHECKED	- G. THIESSE
DATE	- 02/14/2025

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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

CHERRY VALLEY PATH - LIGHTING  
REMOVAL PLAN

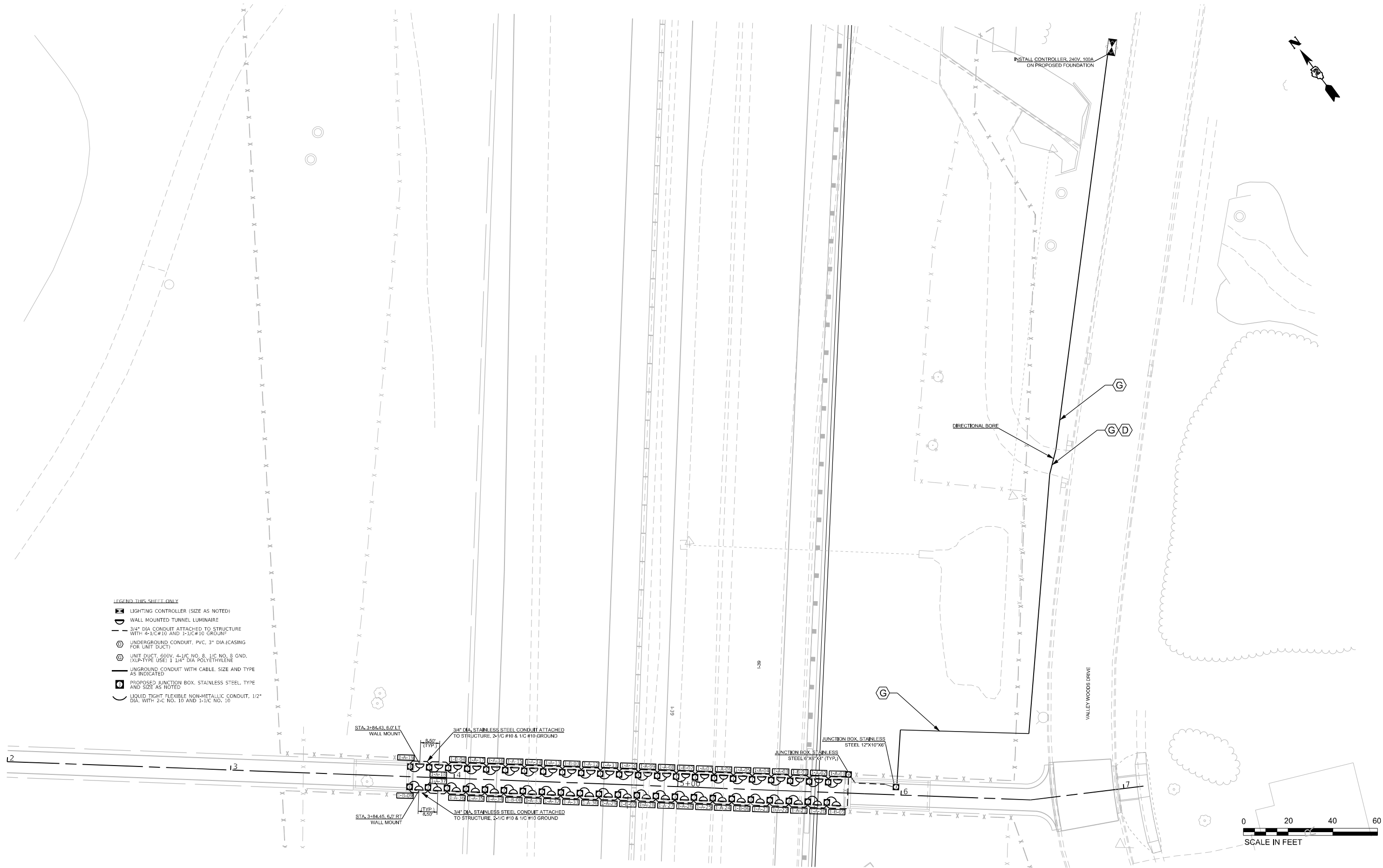
SCALE: 1"=20'	SHEET 17	OF 21	SHEETS	STA. 0+00.00	TO STA. 6+00.00
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F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	502
CONTRACT NO. 64C24				
		ILLINOIS	FED. AID PROJECT	

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) ▶ •



MODEL: P:\CYP-Plan 1 [Sheet]  
FILE NAME: c:\paword\benesch\projects\0173716\0264C24\_ML-sh-H-GT-CYP.dgn



<div><div><div></div><div>benesch</div></div><div><div>Affiliated Research &amp; Company</div><div>25 W. Wacker Drive, Suite 3000</div><div>Chicago, Illinois 60601</div><div>312-565-4150</div><div>Job No. 10900.00</div></div></div>	USER NAME = j.tardy		DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CHERRY VALLEY PATH - LIGHTING PROPOSED LIGHTING PLAN			F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	DRAWN -		CHECKED - G. THIESSE	REVISED -		SCALE: 1"=20'			*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	503
	PLOT SCALE = \$SCALE\$		DATE - 02/14/2025	REVISED -		SHEET 18 OF 21 SHEETS			CONTRACT NO. 64C24				
	PLOT DATE = 8/12/2024					STA. 2+00.00 TO STA. 8+00.00			ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

MODEL: CVP Wiring Diagram [Sheet]  
FILE NAME: c:\paward\benesch\_projects\projects\0173716\0264C24\_ML-shr-CVP-Wiring-Diagram.dgn



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35 W. Wacker Drive, Suite 3000  
Chicago, Illinois 60601  
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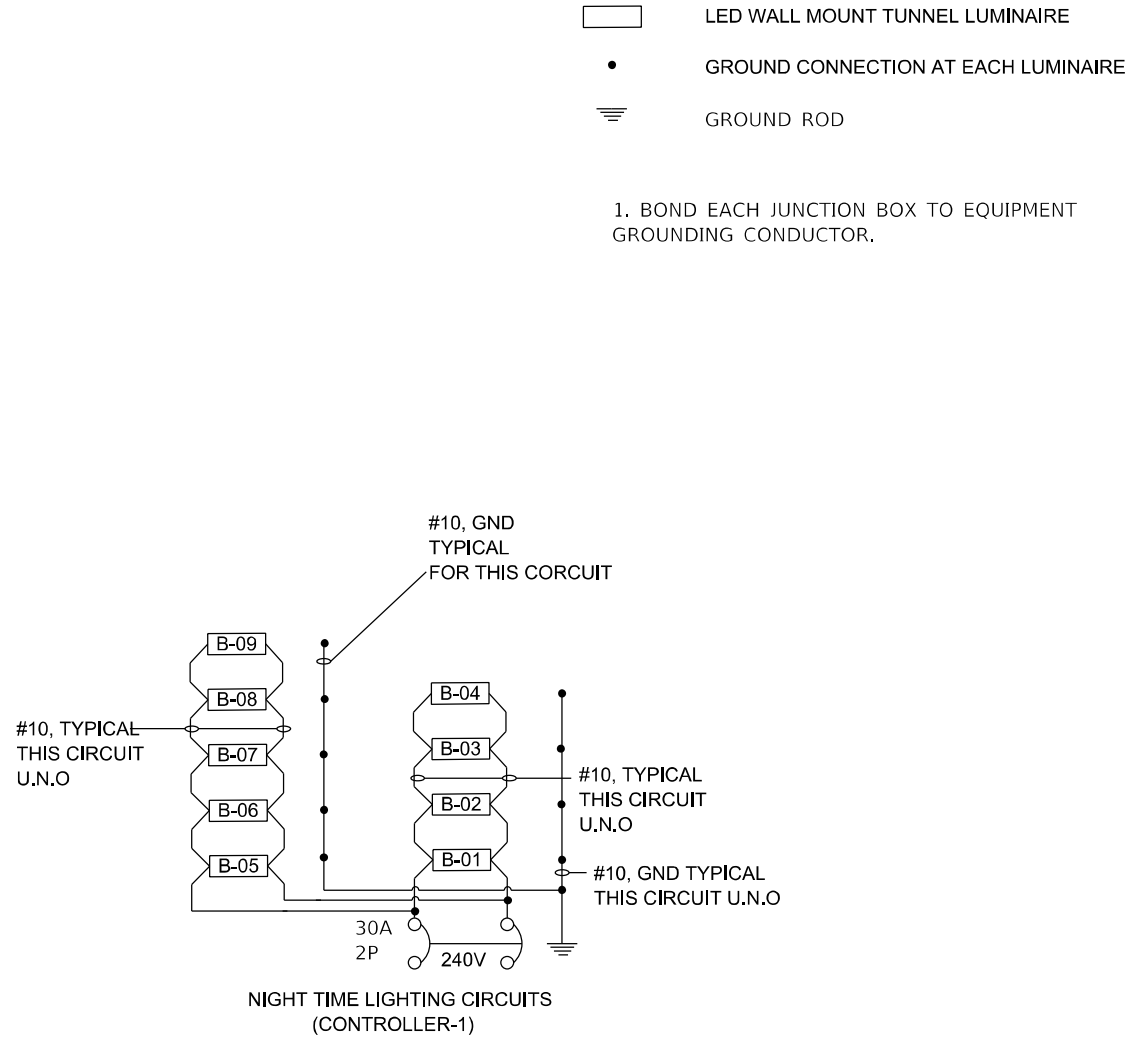
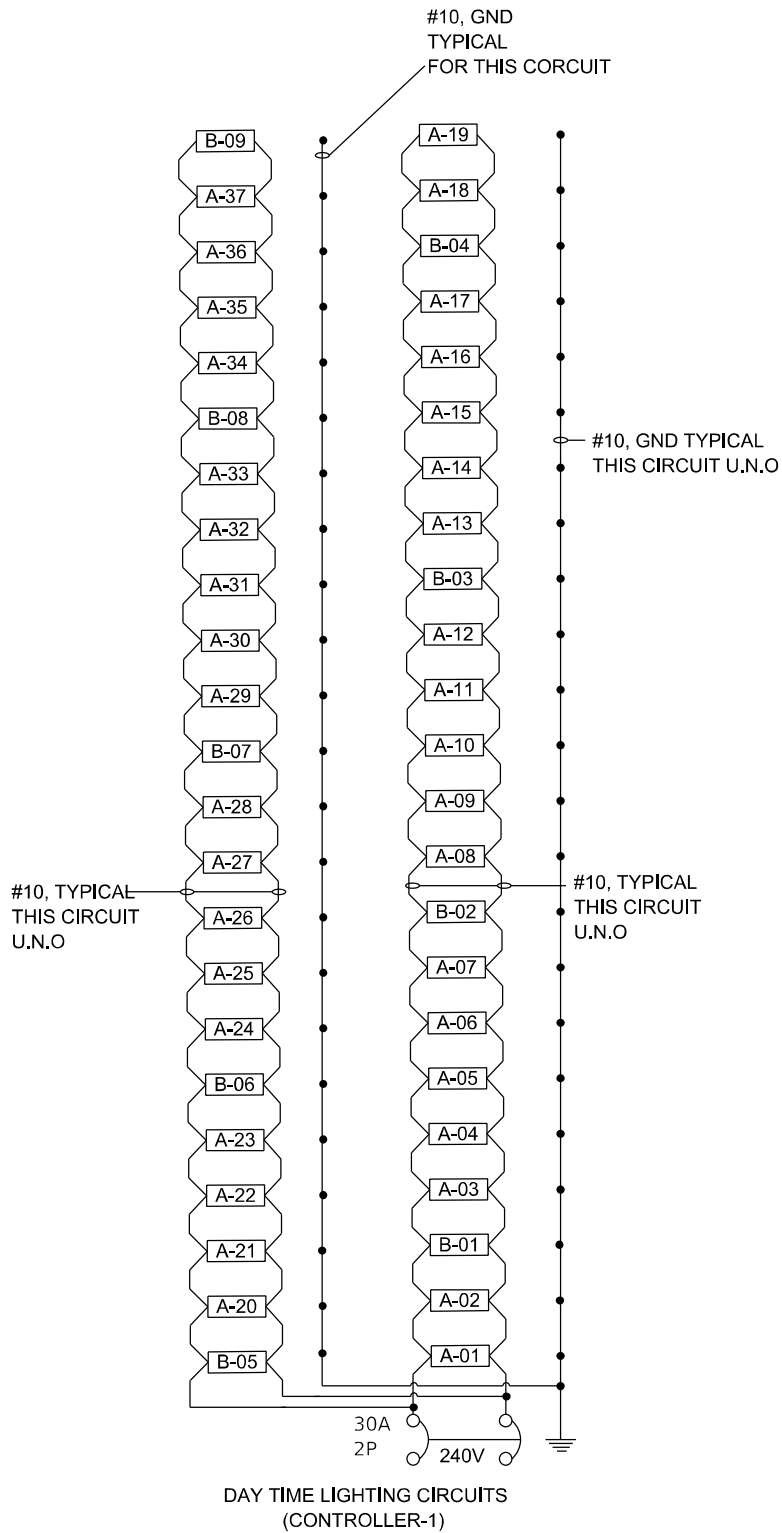
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

CHERRY VALLEY PATH - LIGHTING  
PROPOSED LIGHTING DETAILS

SCALE: SHEET 19 OF 21 SHEETS STA. TO STA.

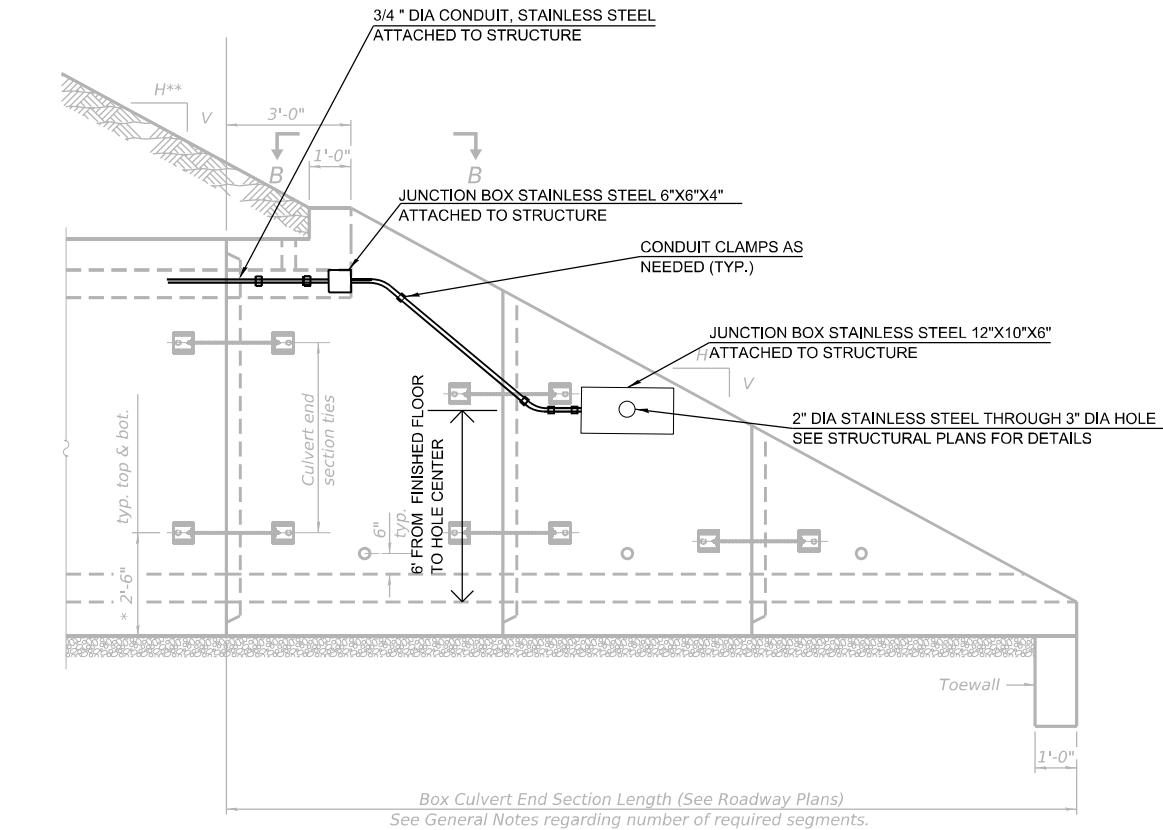
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*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	504
CONTRACT NO. 64C24				
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\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

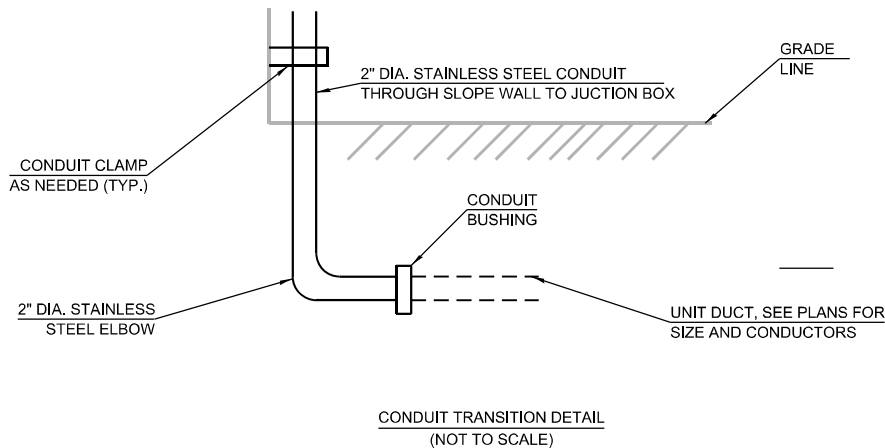
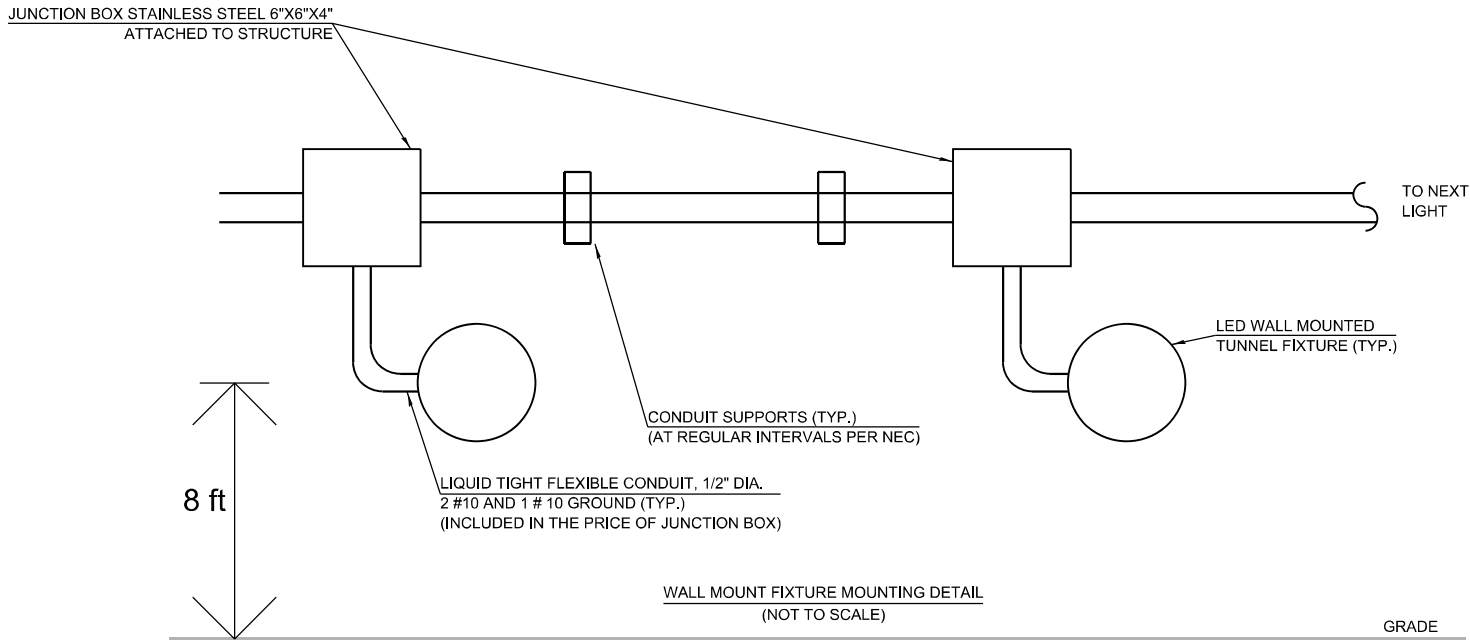
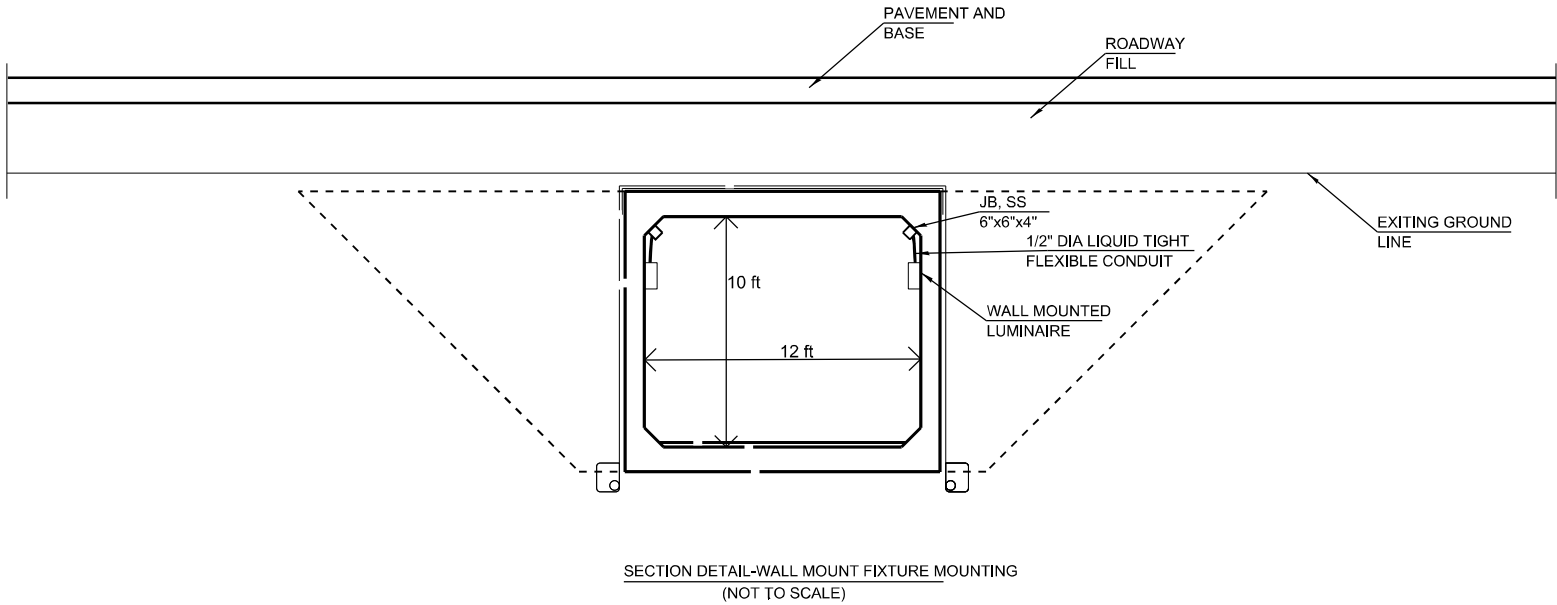
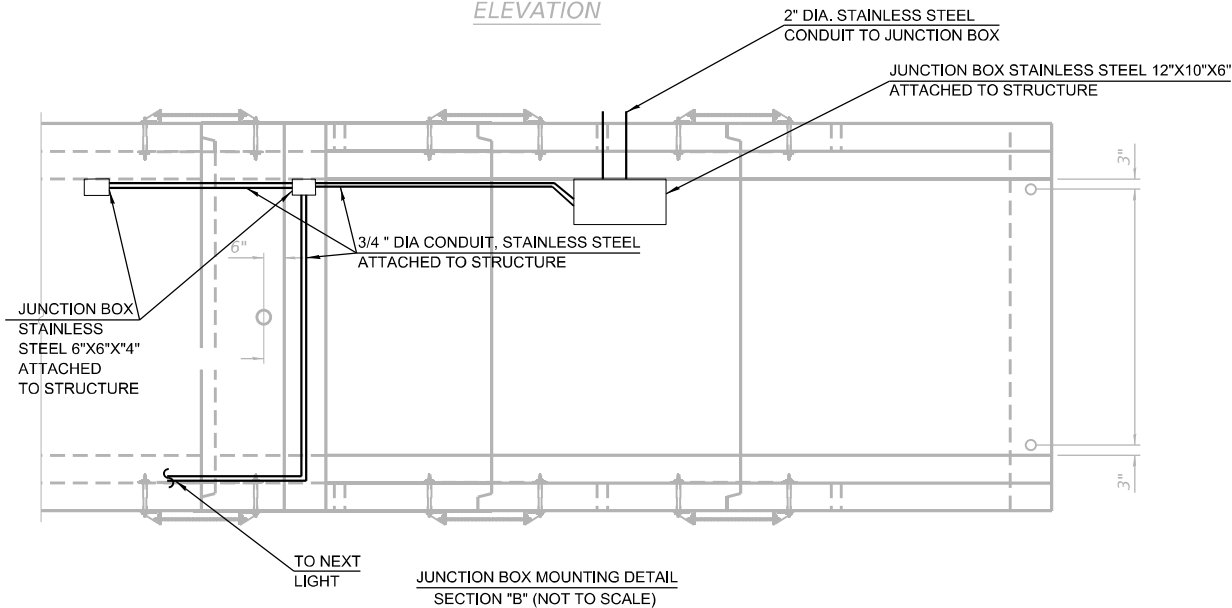


- LED WALL MOUNT TUNNEL LUMINAIRE
  - GROUND CONNECTION AT EACH LUMINAIRE
  - GROUND ROD
1. BOND EACH JUNCTION BOX TO EQUIPMENT GROUNDING CONDUCTOR.

MODEL: CVP Details-1 [Sheet]  
FILE NAME: c:\pawword\benesch projects\projects\0173716\0264C24\_ML-shr-CVP-Details-1.dgn



ELEVATION



USER NAME = Ifranceschina  
PLOT SCALE = \$SCALE\$  
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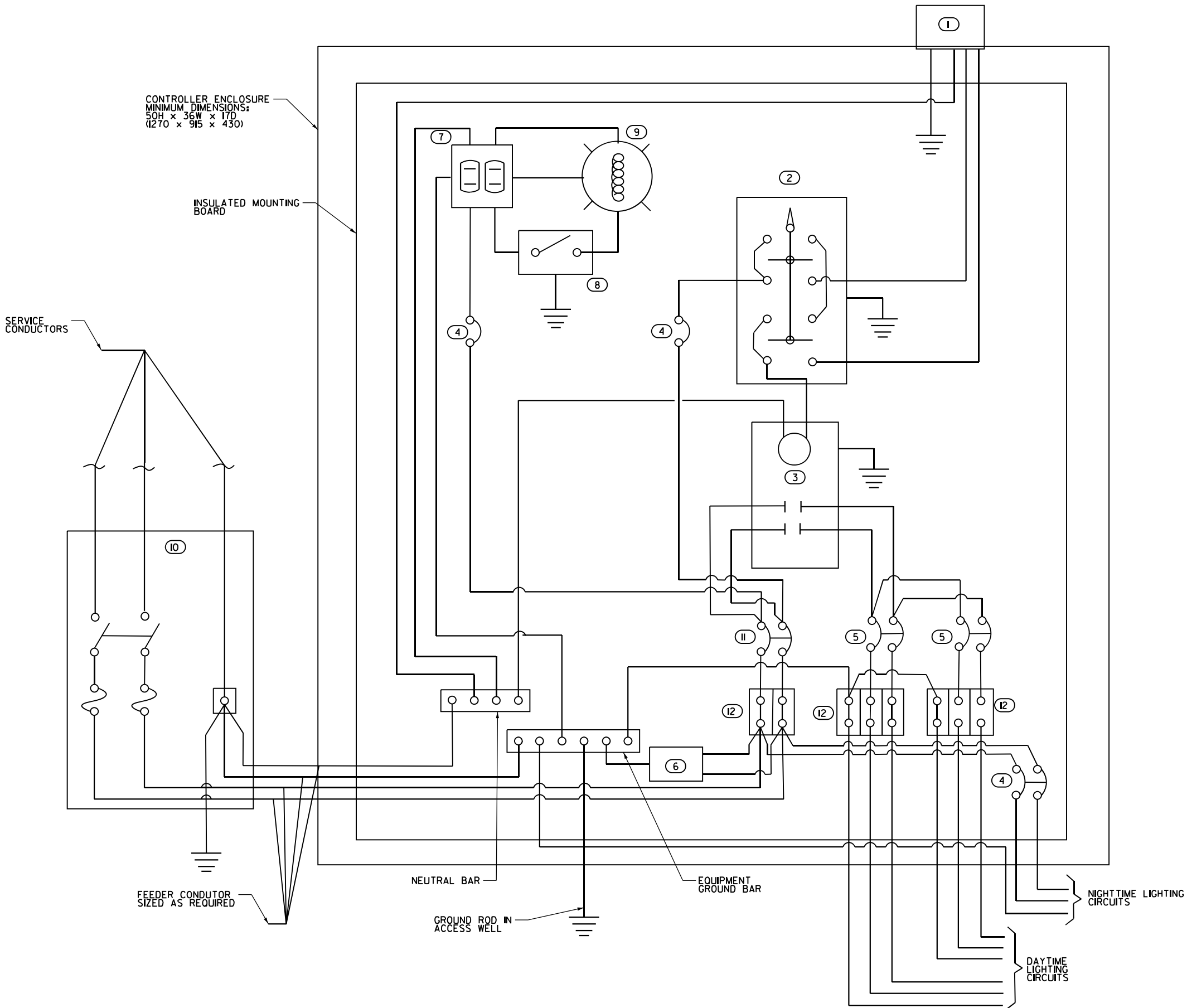
CHERRY VALLEY PATH - LIGHTING  
PROPOSED LIGHTING DETAILS

SCALE: SHEET 20 OF 21 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	505
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

- ① Photocell with integral surge arrester
- ② HAND-OFF-AUTO selector switch
- ③ 100 amp, electrically held contactor
- ④ 15 amp, 1-pole circuit breaker
- ⑤ 20 amp, 2P circuit breaker (two spares required but not shown)
- ⑥ Surge Arrester
- ⑦ GFCI Duplex receptacle
- ⑧ Single-pole, single throw switch
- ⑨ Incandescent luminaire, enclosed and gasketed with 100W lamp
- ⑩ Service disconnect switch-2 pole, 3 wire, 100A, fused at 100 amp, solid neutral in Nema 4X enclosure having lockable external handle
- ⑪ 100 amp, 2 pole circuit breaker
- ⑫ Terminal block sized for conductors as shown on plans



NOTE:  
1. PHOTOCELL AND CONTACTOR  
SHALL BE WIRED SO THAT  
CONTACTOR IS CLOSED DURING  
DAYTIME HOURS.

MODEL: CVP Details-2 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\0173716\0264C24\_ML-shr-CVP-Details-2.dgn



USER NAME = Ifranceschina  
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DATE - 02/14/2025

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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

CHERRY VALLEY PATH - LIGHTING  
PROPOSED LIGHTING DETAILS

SCALE: SHEET 21 OF 21 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	506
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

MODEL: FO GEN NOTES (Sheet)  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\0173716\0264C24\_ML-sh-FiberGeneralNotes.dgn

PAY ITEM NO.	DESCRIPTION	UNIT	TOTAL
81028260	UNDERGROUND CONDUIT, GALVANIZED STEEL, 6" DIA	FOOT	1349
81028750	UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, 2" DIA	FOOT	1470
81101205	CONDUIT ATTACHED TO STRUCTURE, 6"DIA., PVC COATED GALVANIZED STEEL	FOOT	400
81300948	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 24" X 24" X 10"	EACH	4
81400100	HANDHOLE	EACH	2
81702130	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO.6	FOOT	1470
81702140	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO.4	FOOT	2940
89502300	REMOVE ELECTRIC CABLE FROM CONDUIT	FOOT	120
89502380	REMOVE EXISTING HANDHOLE	EACH	1
X0323917	CABINET, MODEL 334	EACH	1
X0325485	TRUSS MOUNTED LED DYNAMIC MESSAGE SIGN	EACH	1
X0327616	MAINTAINING ITS DURING CONSTRUCTION	CAL MO	36
X1400282	POLYETHYLENE DUCT, 1 1/4" DIA	FOOT	53808
X1400459	DYNAMIC MESSAGE SIGN REMOVAL - IDOT	EACH	1
X6026627	VAULTS TO BE REMOVED	EACH	6
X8301804	REMOVAL OF TEMPORARY WOOD POLES AND FIBER OPTIC CABLE	LSUM	1
X8500102	MAINTENANCE OF EXISTING FIBER OPTIC CABLE (FOC) NETWORK	MONTH	32
X8710013	FIBER OPTIC CABLE IN CONDUIT, 12 FIBERS, SINGLE MODE	FOOT	170
X8710023	FIBER OPTIC CABLE IN CONDUIT, 96 FIBERS, SINGLE MODE	FOOT	45,195
X8710306	FIBER OPTIC CABLE SPLICE - MAINLINE	EACH	9
X8710314	FIBER OPTIC SPLICE-LATERAL	EACH	2
X8710318	FIBER OPTIC UTILITY MARKER	EACH	14
X8950510	REMOVE FIBER OPTIC CABLE FROM CONDUIT	FOOT	7,310
Z0010614	CLEANING EXISTING MANHOLE OR HANDHOLE	EACH	3
Z0033052	COMMUNICATIONS VAULT	EACH	14

PAY ITEM NO.	DESCRIPTION	UNIT	TOTAL
JS810826	UNDERGROUND CONDUIT, GALVANIZED STEEL, 6" DIA.	FOOT	180
JS817213	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 6	FOOT	839
JS817214	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 4	FOOT	1,678
JT132040	DYNAMIC MESSAGE SIGN - TYPE 1	EACH	1
JT132050	DYNAMIC MESSAGE SIGN -TYPE 1 (TRAINING)	L SUM	1
JT132060	DYNAMIC MESSAGE SIGN - TYPE 1 (SPARE PARTS)	EACH	1
JT132112	REMOVE DYNAMIC MESSAGE SIGN CONTROLLER FOUNDATION	EACH	1
JT132114	REMOVE DYNAMIC MESSAGE SIGN - TYPE 1	EACH	1
JT132621	DMS ELECTRICAL WORK - TYPE 1	EACH	1
JT132800	REMOVE ITS POLE MOUNTED ENCLOSURE	EACH	1
JT132810	ITS POLE MOUNTED ENCLOSURE (CCTV OR MVDS)	EACH	1
JT132814	ITS DISCONNECT SWITCH ASSEMBLY	EACH	2
JT132820	CCTV CAMERA, ITS ASSEMBLY	EACH	1
JT132830	FIBER OPTIC COMMUNICATIONS, ITS ASSEMBLY	EACH	2
JT134002	MAINTAIN INTELLIGENT TRANSPORTATION SYSTEMS	CAL MO	36
JT134037	ITS ELEMENT SITE GROUNDING - POLE MOUNTED ASSEMBLY	EACH	1
JT135097	REMOVE CCTV CAMERA, POLE MOUNTED	EACH	1
JT160099	HANDHOLE FOR SINGLE MODE FIBER OPTIC CABLE, TORSION ASSIST, 48"X72"X36"	EACH	1
JT160217	LOCATE POST FOR FIBER OPTIC CABLE	EACH	2
JT160226	SINGLE MODE FIBER OPTIC CABLE REMOVAL, NO SALVAGE	FOOT	267
JT810873	UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, SDR 11, 1 1/4" DIA.	FOOT	515
JT810876	UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, SDR 11, 2" DIA.	FOOT	839
JT830051	GROUND MOUNTED LIGHT POLE, GALVANIZED STEEL, 50 FT., WITHOUT MAST ARM	EACH	1
JT836018	ITS ELEMENT POLE FOUNDATION STEEL HELIX (10 FT)	EACH	1
JT836028	ITS CONCRETE SERVICE PAD, TYPE B	EACH	1
JT901056	LOCATE TRACER WIRE	FOOT	329

LEGEND

- (E)

EXISTING
- (P)

PROPOSED
- (R)

REMOVED
- (A)

ABANDON IN PLACE
- FO

PROPOSED UNDERGROUND FIBER OPTIC CABLE IN DUCT, TYPE AND SIZE AS NOTED
- FO

EXISTING UNDERGROUND FIBER OPTIC DUCT
- FO

EXISTING FIBER OPTIC CABLE, AERIAL, 3-96 FIBERS
- E

PROPOSED UNDERGROUND ELECTRICAL CABLE IN CONDUIT, TYPE AND SIZE AS NOTED
- TEMPORARY WOOD POLE, 50 FT., CLASS 4
- PROPOSED COMMUNICATION VAULT
- EXISTING COMMUNICATION VAULT
- GALVANIZED RIGID STEEL CONDUIT PUSHED (P), TYPE AND SIZE AS NOTED
- PROPOSED JUNCTION BOX, STAINLESS STEEL, TYPE AND SIZE AS NOTED
- H

PROPOSED ELECTRICAL HANDHOLE
- C

PROPOSED CCTV CAMERA
- DMSC

PROPOSED DMS CABINET
- ~~DMSC~~

DMS CABINET TO BE REMOVED
- DMS

PROPOSED DMS SIGN
- ~~DMS~~

EXISTING DMS SIGN TO BE REMOVED
- ~~C~~

CCTV CAMERA TO BE REMOVED
- PROPOSED DISCONNECT SWITCH
- EXISTING ELECTRICAL SERVICE

GENERAL NOTES

1. THE CONTRACTOR SHALL TAKE RESPONSIBILITY FOR THE MAINTENANCE OF THE FIBER OPTIC CABLE FOR THE DURATION OF THE PROJECT.
2. THE CONTRACTOR SHALL PROVIDE A 10 DAY NOTICE PRIOR TO ANY SERVICE INTERRUPTION FOR THE SPLICING OF NEW CABLE INTO THE SYSTEM.
3. THE CONTRACTOR SHALL VERIFY THE LOCATIONS OF HANDHOLES AND FIBER OPTIC CABLE PRIOR TO CONSTRUCTION OF THE JOB.
4. ALL SERVICE OUTAGES FOR SPLICING IN OF NEW CABLE SHALL BE LESS THAN 2 HOURS.
5. THERE EXISTS ENVIRONMENTALLY SENSITIVE AREAS ALONG THIS JOB. CARE MUST BE TAKEN NOT TO REMOVE PLANT LIFE FROM THESE AREAS.
6. THERE SHALL BE 200 FEET OF 96 SM FIBER OPTIC CABLE SLACK, PER CABLE, IN EACH COMMUNICATION VAULT. THERE SHALL BE 50 FEET OF FIBER OPTIC CABLE SLACK, PER CABLE, IN ALL JUNCTION BOXES.
7. THE CONTRACTOR SHALL EXERCISE CARE WITH THE INSTALLATION OF UNDERGROUND EQUIPMENT AS THERE ARE EXISTING PRIVATELY OWNED UTILITIES WITHIN THE PROJECT LIMITS. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO CONTACT ANY UTILITIES IN THE WORK ZONE AND REQUEST UTILITY LOCATES.
8. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL EXISTING STATE OWNED LIGHTING, TRAFFIC SIGNALS AND/OR FIBER OPTIC CABLE.
9. THE CONTRACTOR SHALL BE AWARE OF THE DOCUMENTATION REQUIREMENTS WHICH REQUIRE GPS DATA ACQUISITION.
10. ALL EXISTING FIBER OPTIC CABLE CALLED TO BE EITHER REMOVED OR ABANDONED SHALL REMAIN IN PLACE AND OPERATIONAL UNTIL PROPOSED UNDERGROUND FIBER OPTIC CABLE IS INSTALLED AND CUT OVER. TEMPORARY FIBER INSTALLATIONS SHALL ALSO REMAIN IN PLACE UNTIL UNDERGROUND FIBER OPTIC CABLES ARE INSTALLED AND FULLY OPERATIONAL.
11. ALL PROPOSED FIBER SHALL HAVE A MINIMUM BENDING RADIUS OF 24".
12. INCASE OF ANY ACCIDENTAL DISRUPTION IN SERVICE TO THE FIBER OPTIC CABLE DURING CONSTRUCTION, THE CONTRACTOR SHALL CONTACT THE 24 HOUR EMERGENCY CONTACT NUMBER AT 312-814-3648.



Affiliated Research & Company  
35 W. Wacker Drive, Suite 3000  
Chicago, Illinois 60601  
312-955-4150 Job No. 10900.00

USER NAME	= rscarla
PLOT SCALE	= \$SCALE\$
PLOT DATE	= 2/14/2025

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DATE	-	02/14/2025

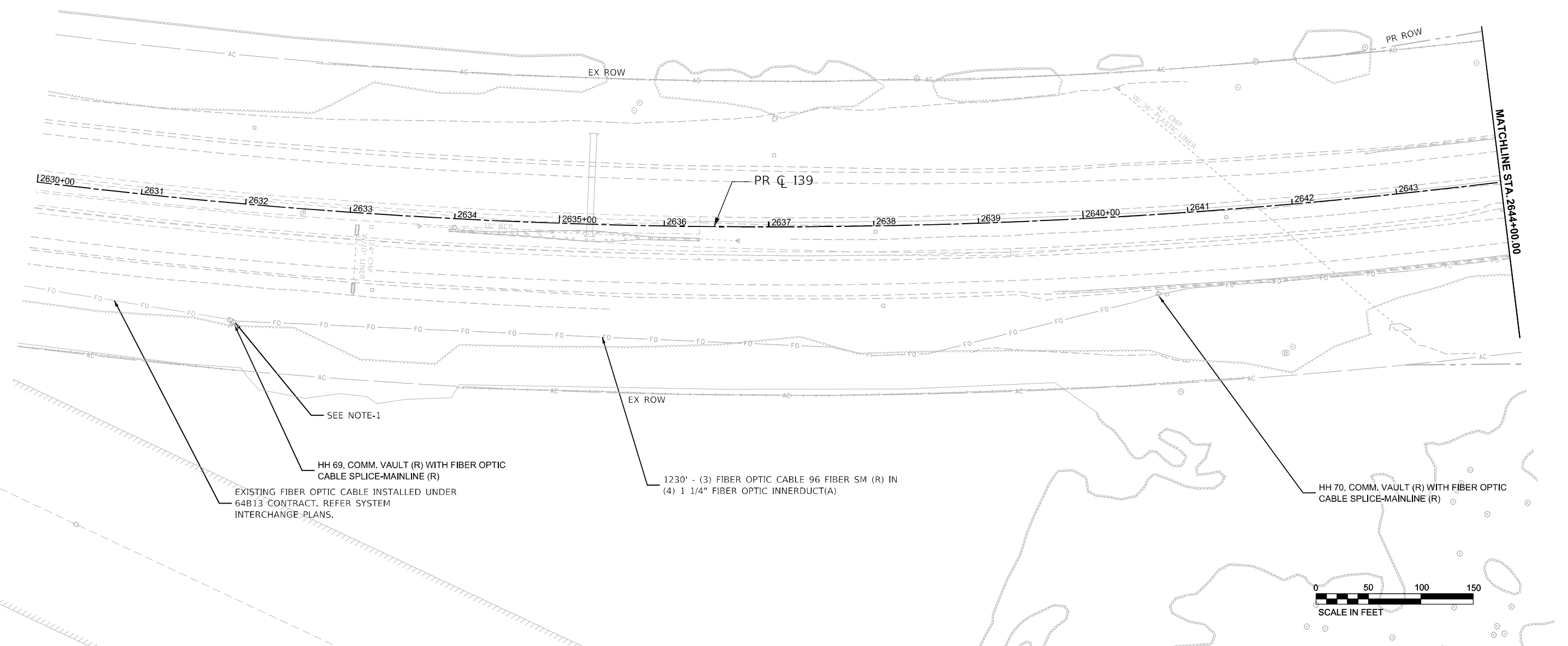
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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC GENERAL NOTES			
I-39			
SCALE:	SHEET 1	OF	SHEETS
STA.		TO STA.	

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	507
		CONTRACT NO. 64C24		
		ILLINOIS FED. AID PROJECT		

- NOTES:
- 1. REFER TO CONTRACT 64B13 FIBER PLANS FOR PROPOSED UNDERGROUND FIBER DUCT, SOUTH OF COMMUNICATION VAULT.
  - 2. CONTRACTOR SHALL MAKE SURE THAT THE PROPOSED FIBER IS INSTALLED AND OPERATIONAL BEFORE THE EXISTING FIBER IS REMOVED.(TYP.)



MODEL: 130 Wide - LGT REM Plan 6 (Sheet)  
FILE NAME: c:\pwworkdir\benesch\projects\0173716\264C24\_ML-shr-Fiber-Rem-139.dgn



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	DATE - 02/14/2025	REVISED -
PLOT DATE = 8/12/2024		

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC REMOVALS  
I-39

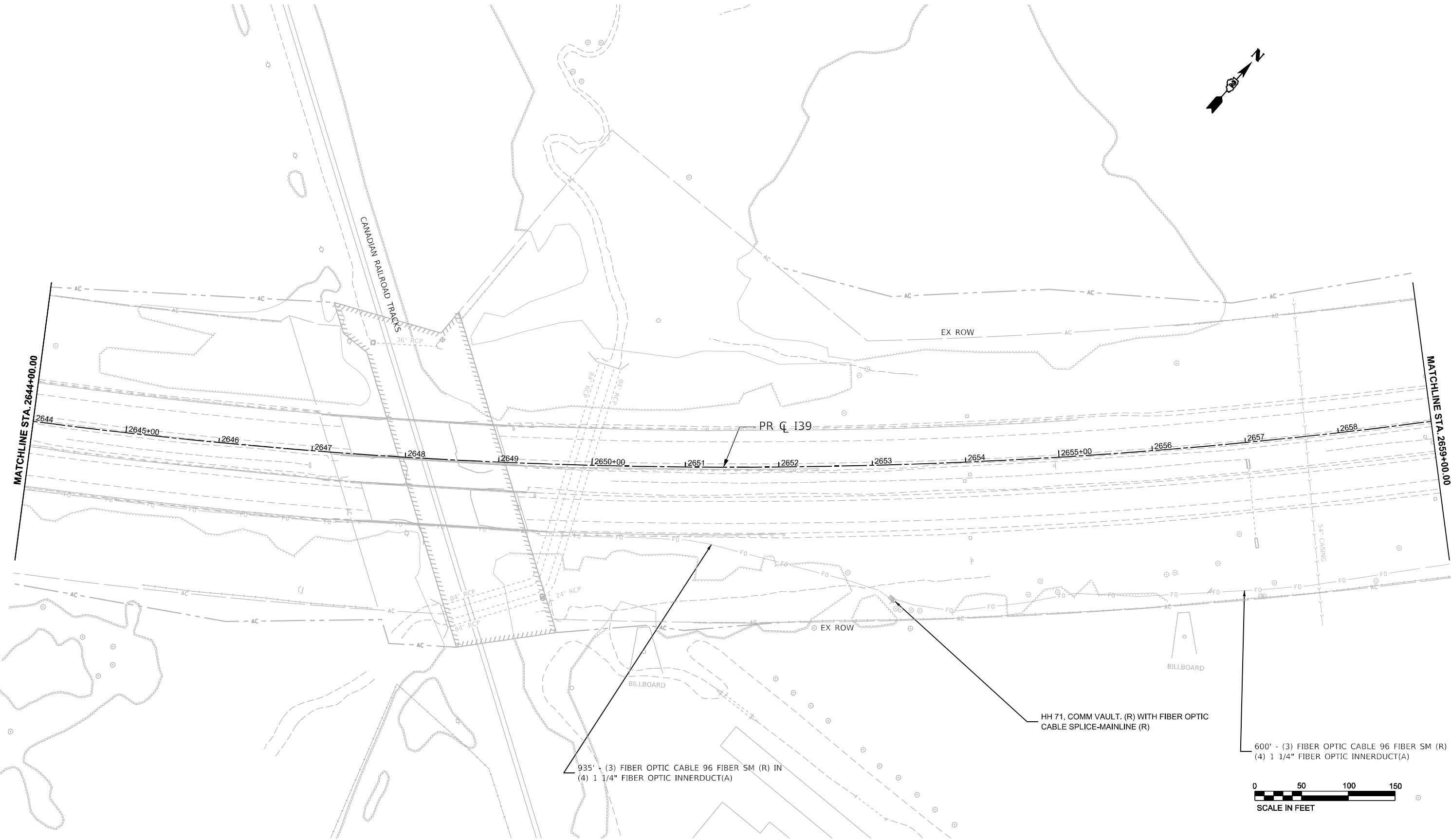
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F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	508
CONTRACT NO. 64C24				
ILLINOIS      FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



MODEL: I39 Wide - LGT REM Plan 7 (Sheet)  
FILE NAME: c:\pwworkdir\benesch\projects\i39\16\I264C24\_ML-sh-Fiber-Rem-I39.dgn



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		DRAWN -	R.SCARIA	REVISED -	
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PLOT DATE	= 2/6/2025	DATE	= 02/14/2025	REVISED -	

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DEPARTMENT OF TRANSPORTATION

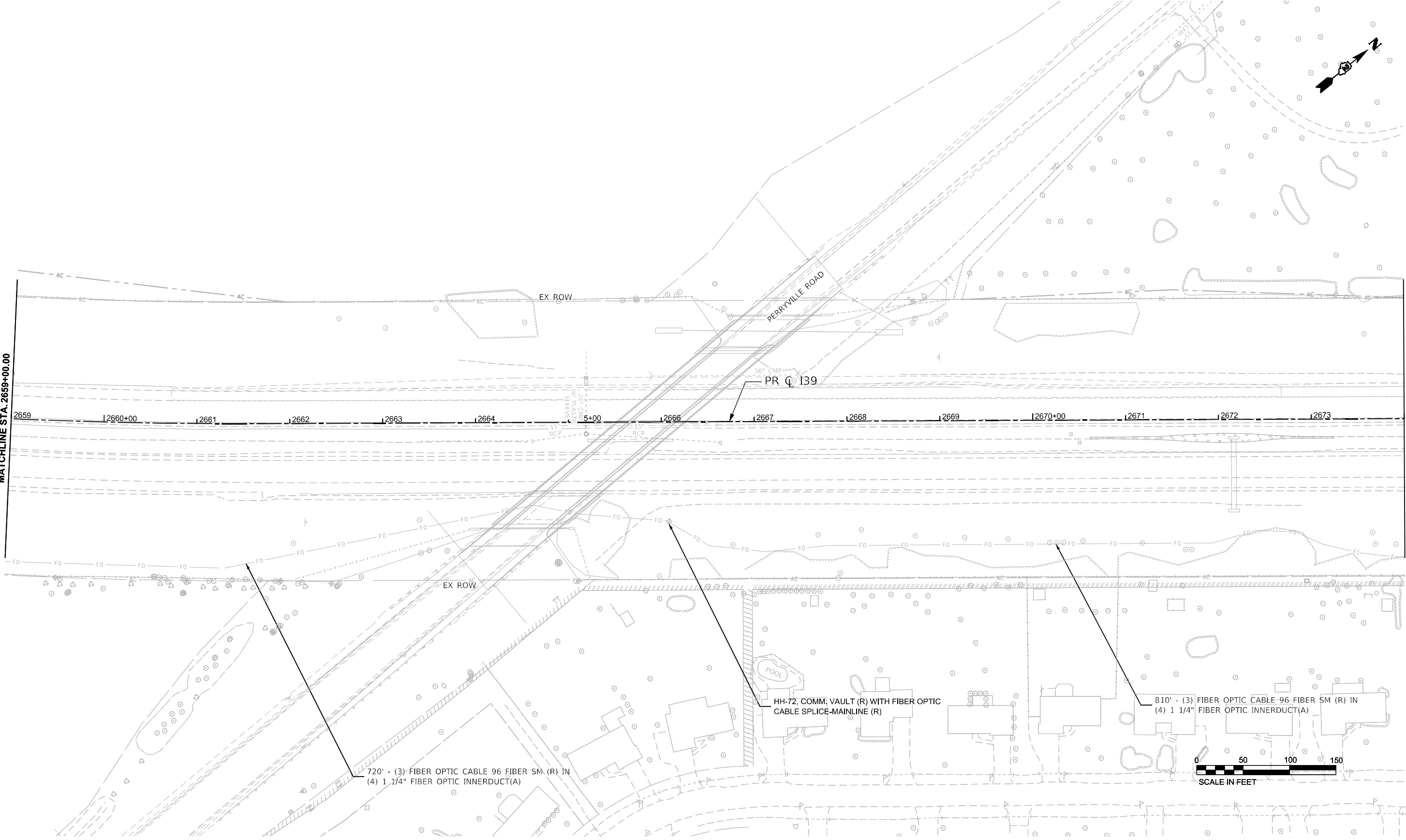
FIBER OPTIC REMOVALS  
I-39

SCALE: 1"=50'      SHEET 3      OF 11      SHEETS      STA. 2644+00.00      TO STA. 2659+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	509
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ILLINOIS   FED. AID PROJECT				

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MODEL: I39 Wide - LGT REM Plan 8 (Sheet)  
FILE NAME: c:\pwworkdir\benesch projects\projects\0173716\I264C24\_ML-sh-Fiber-Rem-I39.dgn



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STATE OF ILLINOIS  
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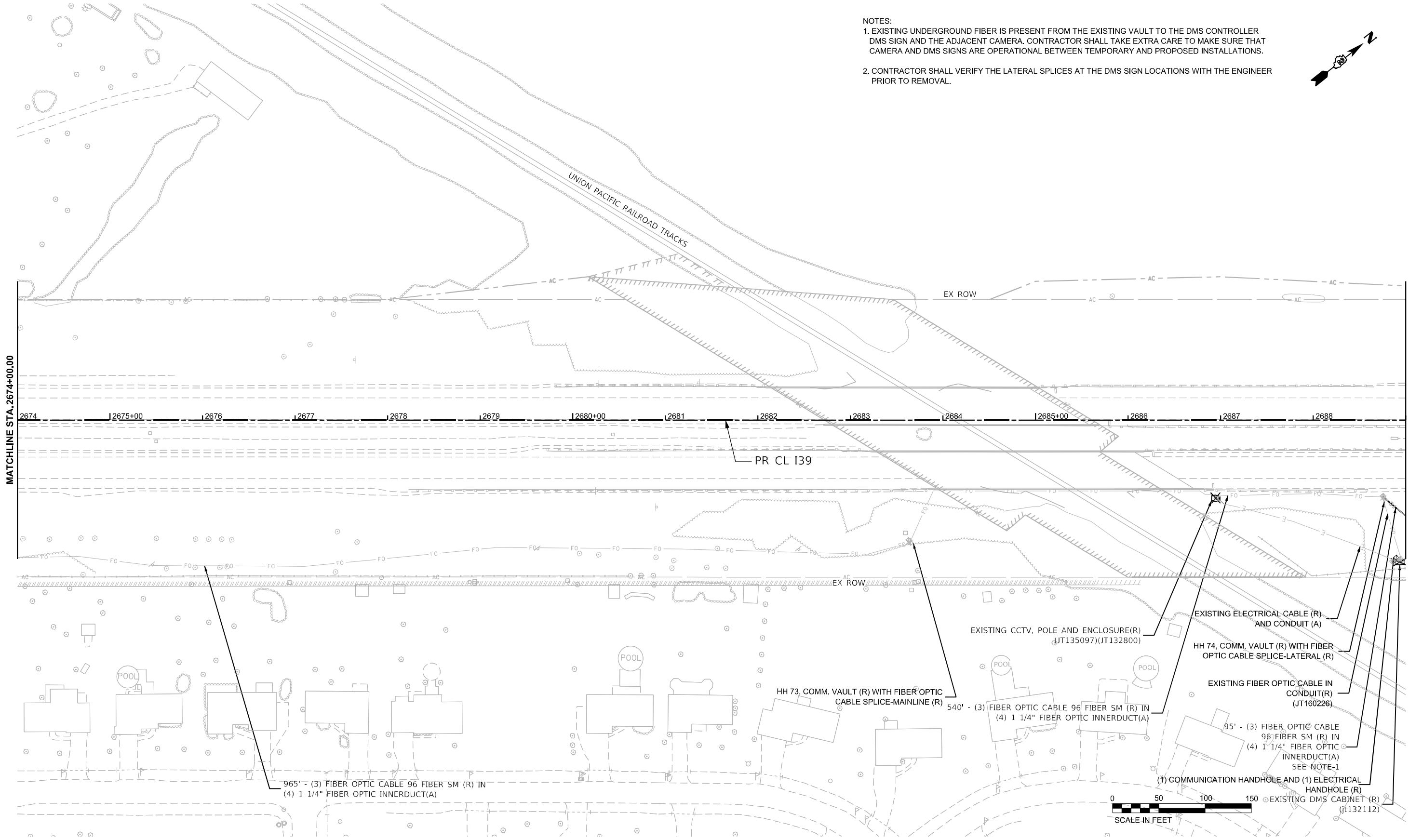
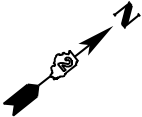
FIBER OPTIC REMOVALS  
I-39

SCALE: 1"=50'      SHEET 4      OF 11      SHEETS      STA. 2659+00.00      TO STA. 2674+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	510
CONTRACT NO. 64C24				
ILLINOIS    FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

- NOTES:
1. EXISTING UNDERGROUND FIBER IS PRESENT FROM THE EXISTING VAULT TO THE DMS CONTROLLER DMS SIGN AND THE ADJACENT CAMERA. CONTRACTOR SHALL TAKE EXTRA CARE TO MAKE SURE THAT CAMERA AND DMS SIGNS ARE OPERATIONAL BETWEEN TEMPORARY AND PROPOSED INSTALLATIONS.
  2. CONTRACTOR SHALL VERIFY THE LATERAL SPLICES AT THE DMS SIGN LOCATIONS WITH THE ENGINEER PRIOR TO REMOVAL.



MODEL: I39 Wide - LGT REM Plan 9 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\projects\0173716\0264C24\_IL-sh-Fiber-Rem-I39.dgn



USER NAME = agibson  
PLOT SCALE = \$SCALE\$  
PLOT DATE = 2/6/2025

DESIGNED - R.SCARIA  
DRAWN - R.SCARIA  
CHECKED - G.THIESSE  
DATE - 02/14/2025

REVISED -  
REVISED -  
REVISED -  
REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

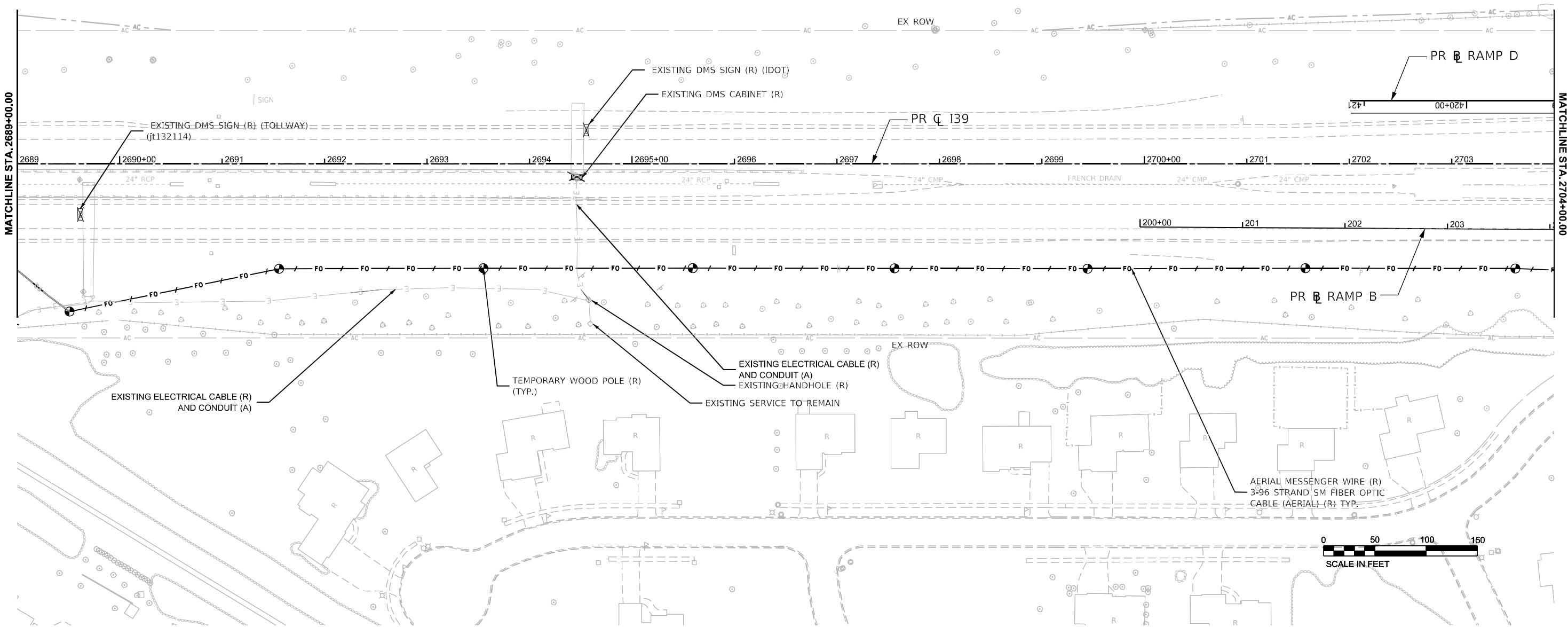
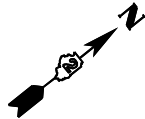
FIBER OPTIC REMOVALS  
I-39

SCALE: 1"=50' SHEET 5 OF 11 SHEETS STA. 2674+00.00 TO STA. 2689+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	511
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

- NOTES:
1. EXISTING UNDERGROUND FIBER IS PRESENT FROM THE EXISTING VAULT TO THE DMS CONTROLLER DMS SIGN AND THE ADJACENT CAMERA. CONTRACTOR SHALL TAKE EXTRA CARE TO MAKE SURE THAT CAMERA AND DMS SIGNS ARE OPERATIONAL BETWEEN TEMPORARY AND PROPOSED INSTALLATIONS.
  2. WHILE THE DMS SIGN IS REMOVED, THE DISCONNECT SWITCH ASSOCIATED WITH THE DMS SIGN SHALL ALSO BE REMOVED.
  3. CONTRACTOR SHALL VERIFY THE LATERAL SPLICES AT THE DMS SIGN LOCATIONS WITH THE ENGINEER PRIOR TO REMOVAL.



MODEL: I-39 Wide - LGT REM Plan 10 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\0173716\I-39-Fiber-Rem-I-39.dgn



USER NAME	= agibson	DESIGNED -	R.SCARIA	REVISED -	
DRAWN -	R.SCARIA	REVIS			
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PLOT DATE	= 2/6/2025	DATE	= 02/14/2025	REVISED -	

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC REMOVALS  
I-39

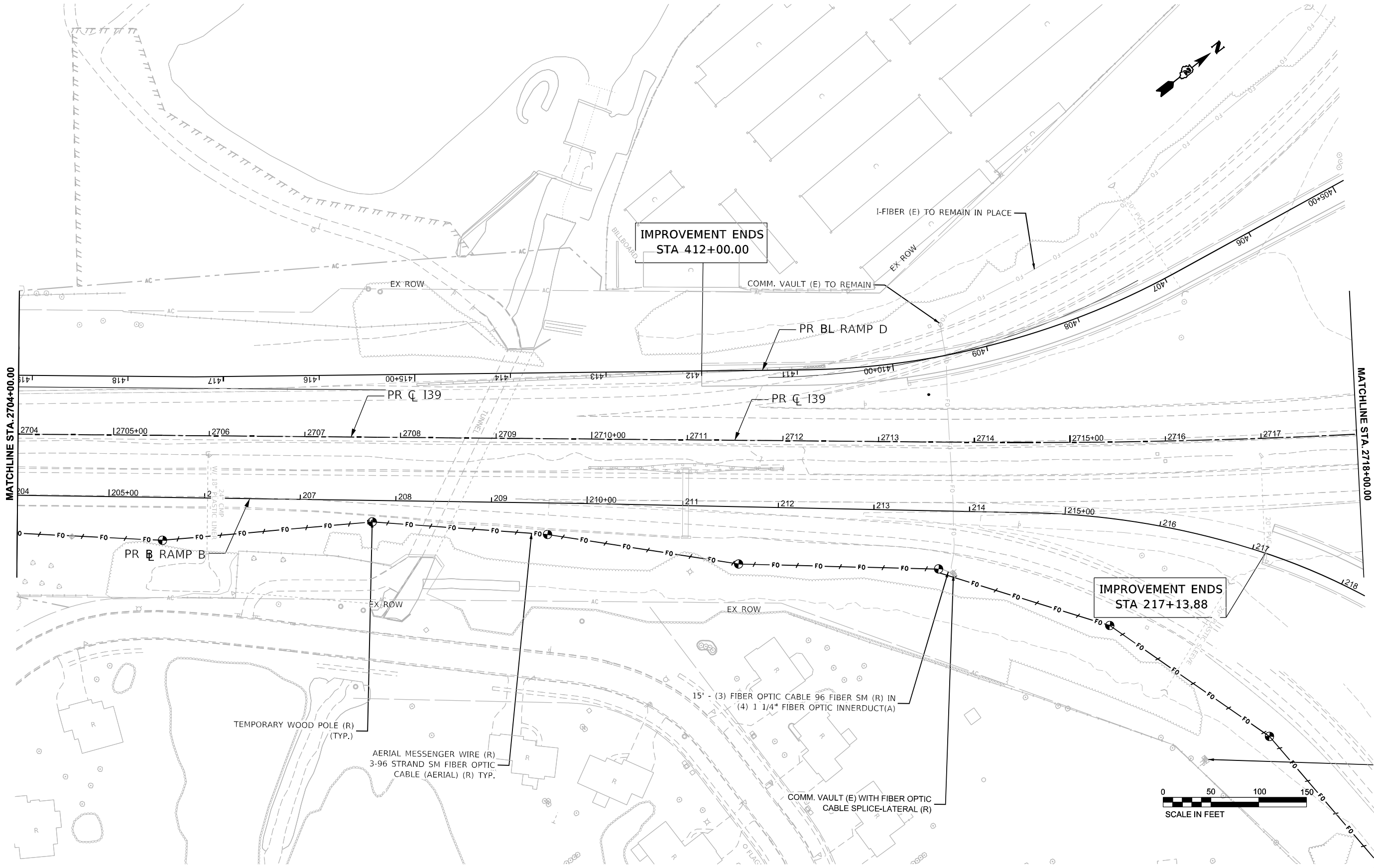
SCALE: 1"=50' SHEET 6 OF 11 SHEETS STA. 2689+00.00 TO STA. 2704+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	512
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



MODEL: I39 Wide - LGT REM Plan 11 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\projects\0173716\I284C24\_ML-sh-Fiber-Rem-I39.dgn



SEE FIBER OPTIC REMOVALS  
SHEET # 8 FOR CONTINUATION



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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

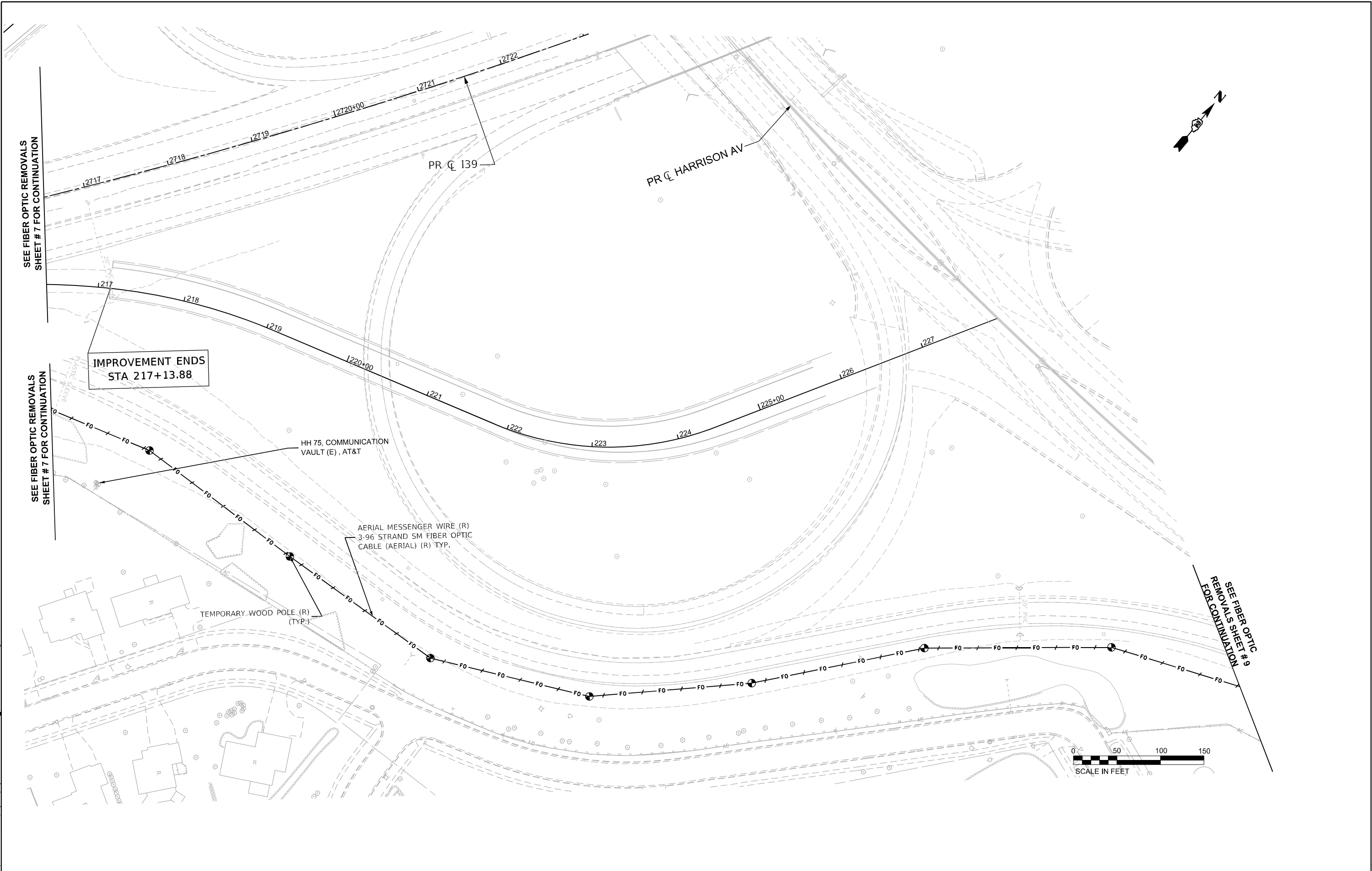
FIBER OPTIC REMOVALS  
I-39

SCALE: 1"=50' SHEET 7 OF 11 SHEETS STA. 2704+00.00 TO STA. 2718+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	513
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

MODEL: Ramp B Electrical - Plan 17 (Sheet)  
FILE NAME: c:\pwworkdir\benesch\projects\0173716\0264C24\_ML-shr-Fiber-Rem-I39.dgn



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PLOT SCALE	= \$SCALE\$	CHECKED -	G.THIESSE	REVISED -	
PLOT DATE	= 2/6/2025	DATE	= 02/14/2025	REVISED -	

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC REMOVALS  
I-39

SCALE: 1"=50'      SHEET 8      OF 11      SHEETS      STA. 0+00.00      TO STA. 0+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 64C24				
ILLINOIS   FED. AID PROJECT				

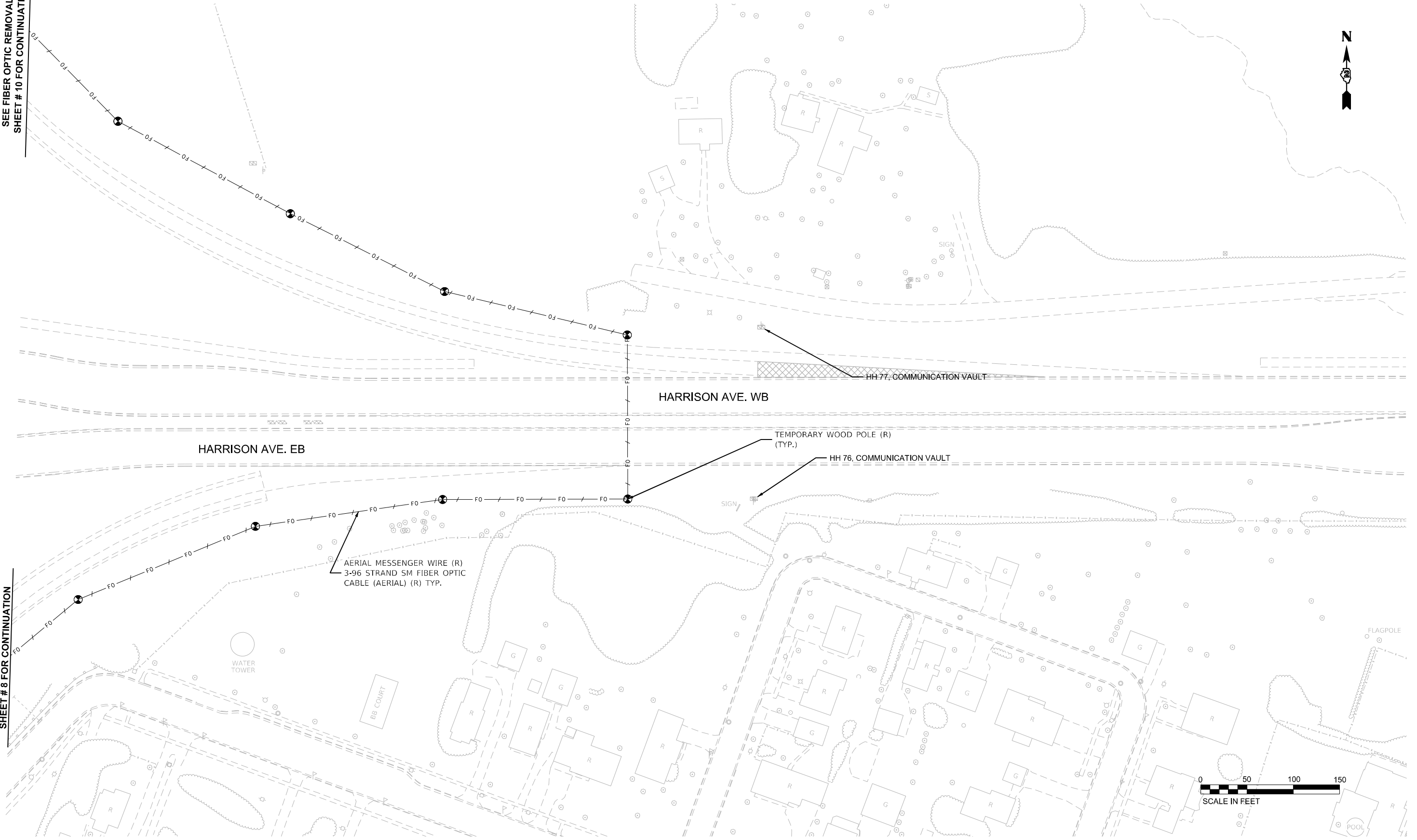
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



MODEL: Harrison Wide - HAR FO Plan 4 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\0173716\0264C24\_ML-sh-H-GT-PL-HAR.dgn

SEE FIBER OPTIC REMOVALS  
SHEET # 8 FOR CONTINUATION

SEE FIBER OPTIC REMOVALS  
SHEET # 10 FOR CONTINUATION



USER NAME = agibson	DESIGNED - R.SCARIA	REVISED -
	DRAWN - R.SCARIA	REVISED -
PLOT SCALE = \$SCALE\$	CHECKED - G.THIESSE	REVISED -
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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

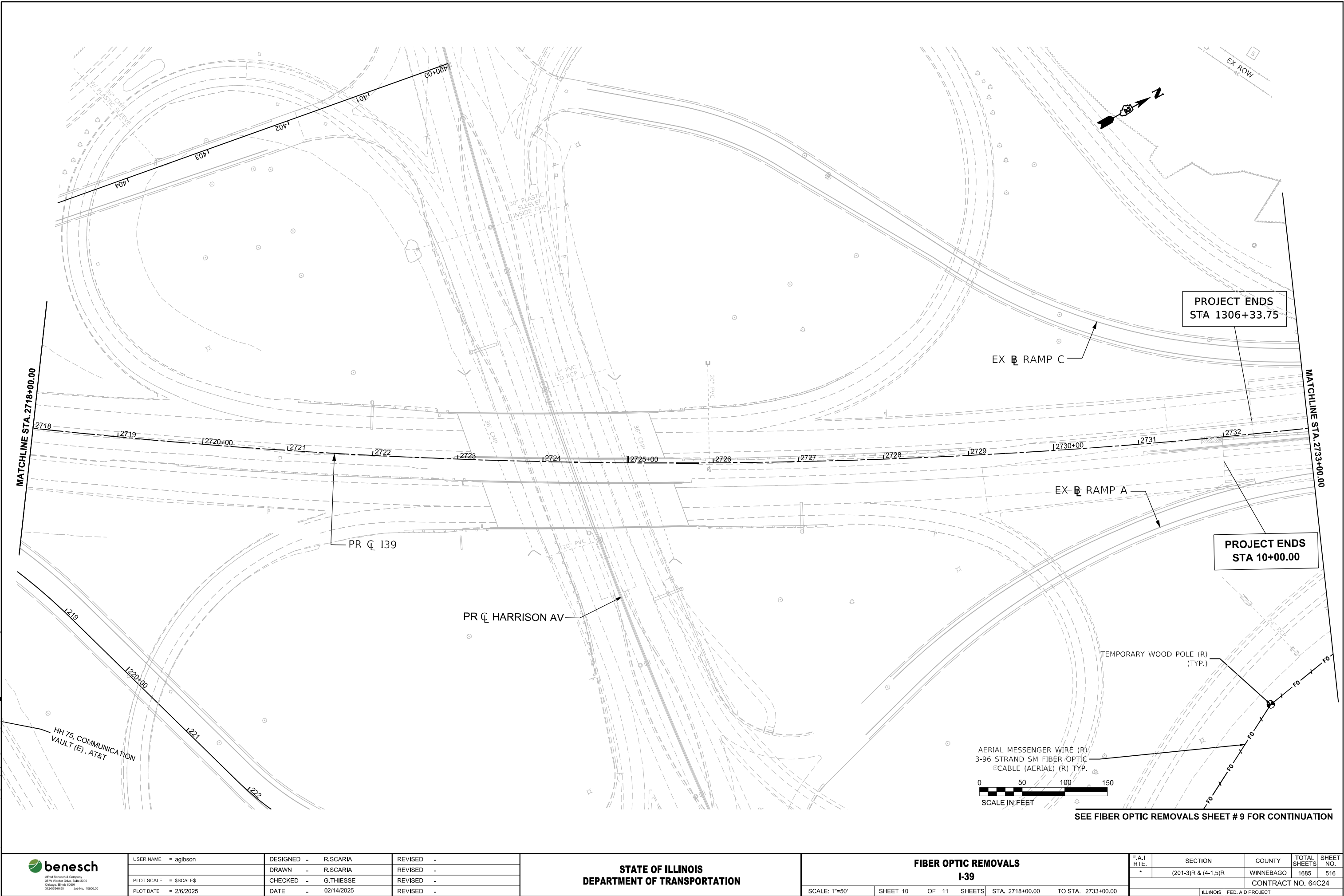
FIBER OPTIC REMOVALS  
HARRISON AVE

SCALE: 1"=50'	SHEET 9	OF 11 SHEETS	STA. 0+00.00	TO STA. 0+00.00
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F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	515
CONTRACT NO. 64C24				
ILLINOIS		FED. AID PROJECT		

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

MODEL: I39 Wide - LGT REM Plan 12 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\0173716\I39\I39-Fiber-Rem-I39.dgn



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PLOT SCALE	= \$SCALE\$	CHECKED -	G.THIESSE	REVISED -	
PLOT DATE	= 2/6/2025	DATE	= 02/14/2025	REVISED -	

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC REMOVALS  
I-39

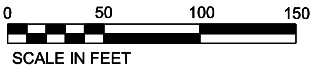
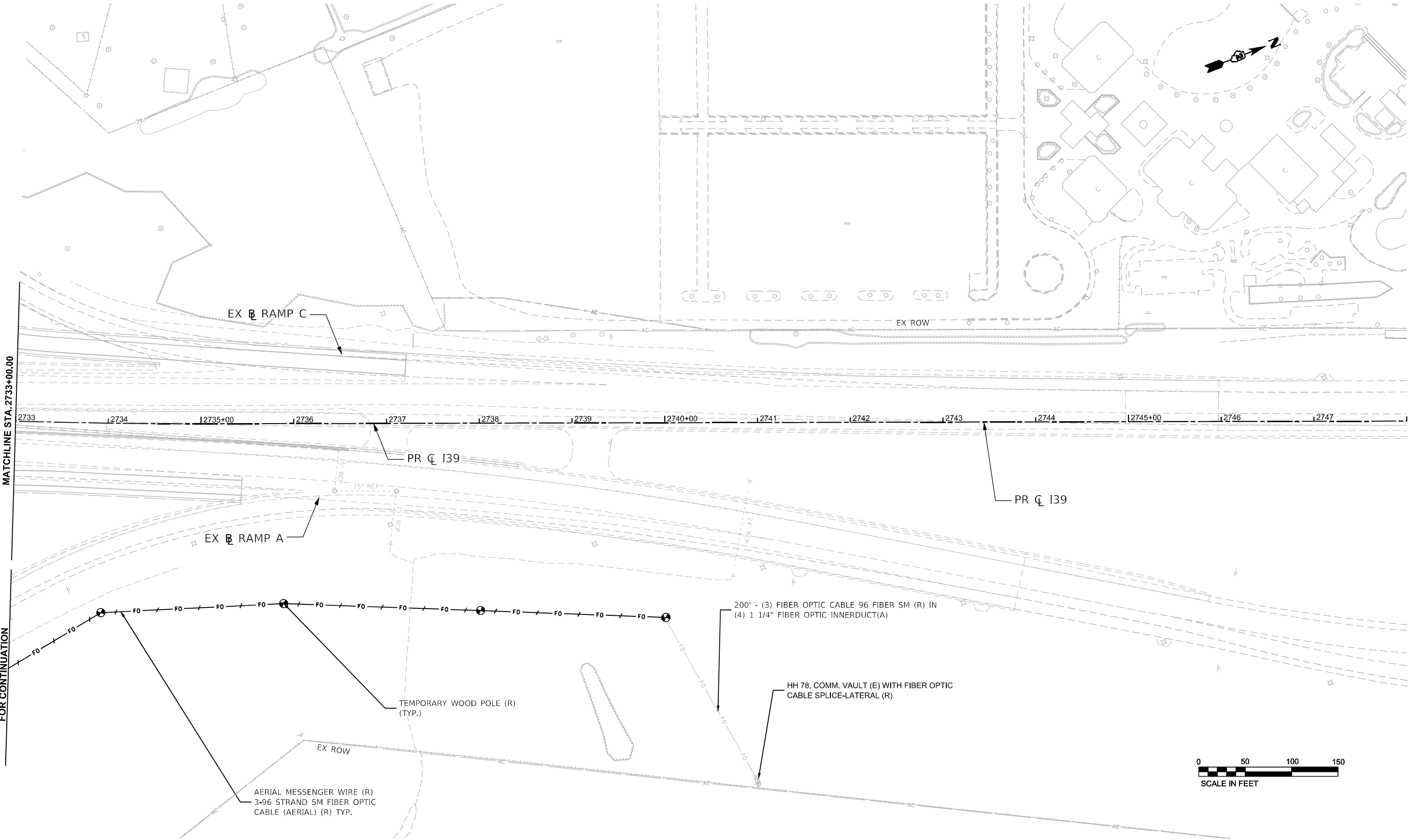
SCALE: 1"=50'    SHEET 10    OF 11    SHEETS    STA. 2718+00.00    TO STA. 2733+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	516
CONTRACT NO. 64C24				
ILLINOIS    FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

MODEL: I39 Wide - LGT REM Plan 13 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\0173716\I284C24\_ML-sh-FiberRem-I39.dgn

SEE FIBER OPTIC REMOVALS SHEET # 10  
FOR CONTINUATION



USER NAME = agibson  
PLOT SCALE = \$SCALE\$  
PLOT DATE = 2/6/2025

DESIGNED - R.SCARIA  
DRAWN - R.SCARIA  
CHECKED - G.THIESSE  
DATE - 02/14/2025

REVISED -  
REVISED -  
REVISED -  
REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

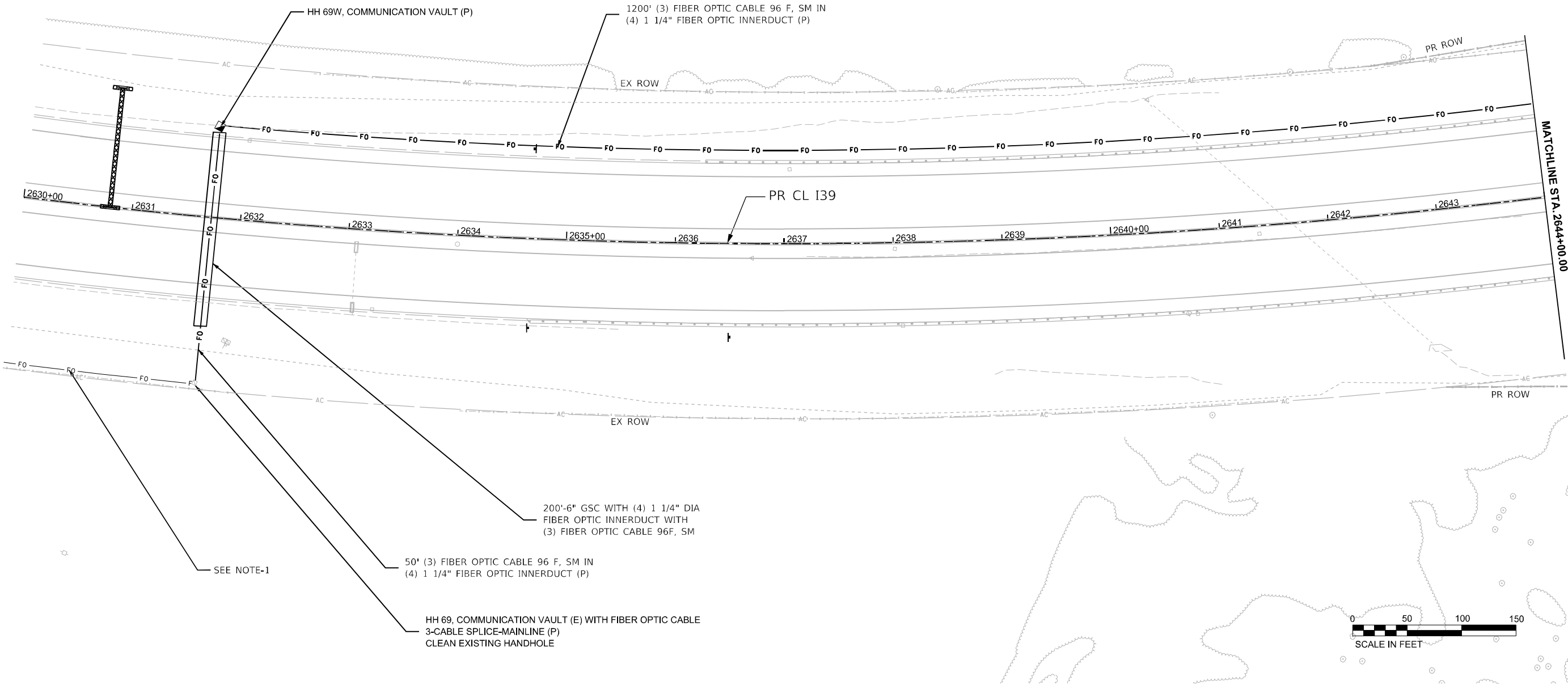
FIBER OPTIC REMOVALS  
I-39

SCALE: 1"=50' SHEET 11 OF 11 SHEETS STA. 2733+00.00 TO STA. 2748+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	517
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

NOTES:  
1. REFER TO CONTRACT 64B13 FIBER OPTIC PLANS FOR PROPOSED UNDERGROUND FIBER DUCTS SOUTH OF THE COMMUNICATION VAULT.



MODEL: 130 Wide - FO Plan 6 (Sheet)  
FILE NAME: c:\pwworkdir\benesch\projects\projects\173716\173716-D264C24\_ML-sh-Fiber-Proposed-139.dgn



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	DRAWN - R.SCARIA	REVISED -
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PLOT DATE = 9/3/2024	DATE - 02/14/2025	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

PROPOSED FIBER OPTIC PLANS  
I-39

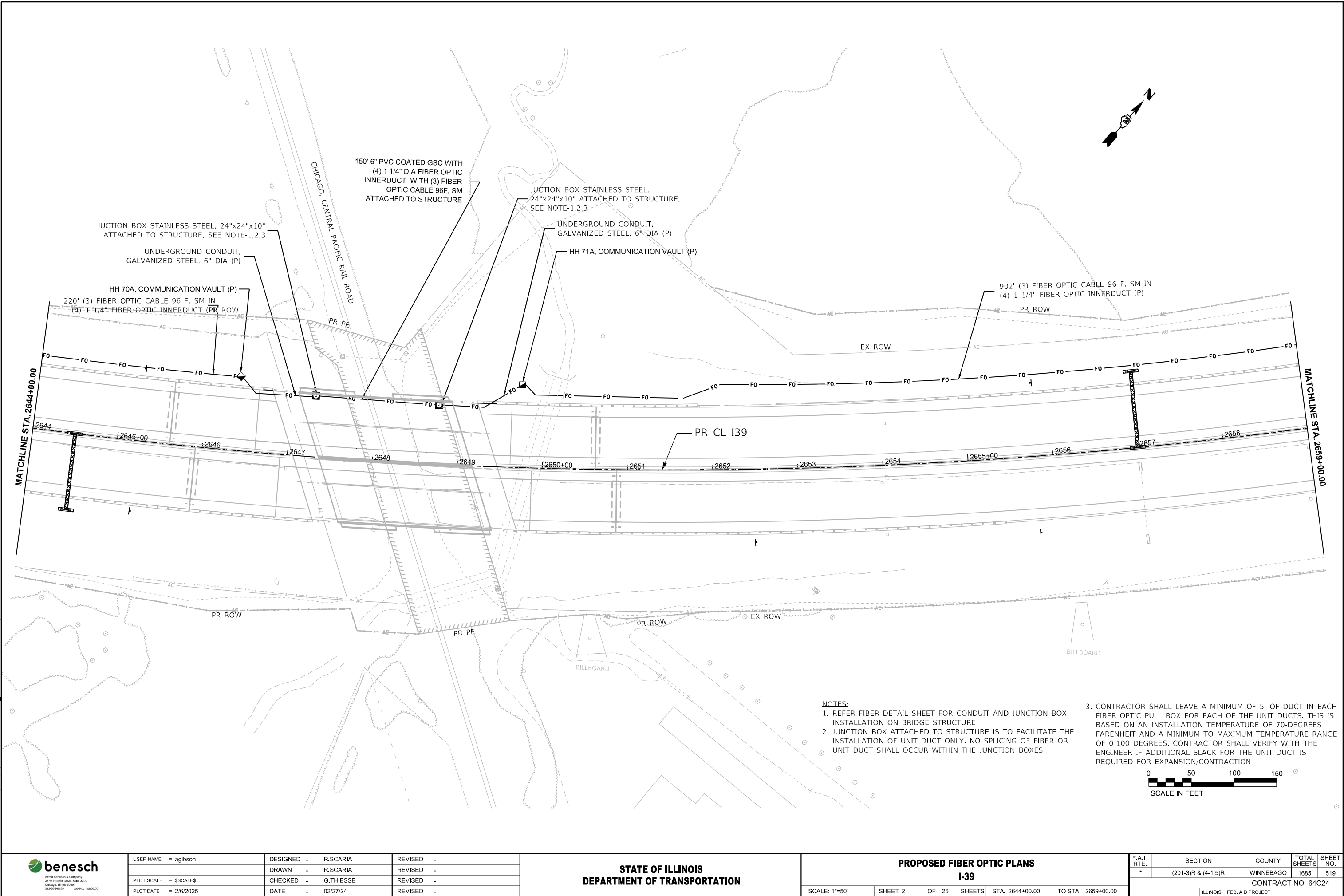
SCALE: 1"=50' SHEET 1 OF 26 SHEETS STA. 2630+00.00 TO STA. 2644+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	518
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



MODEL: I-39 Wide - FO Plan 7 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\projects\173716\I-39\I-39\Proposed-I39.dgn



- NOTES:**
1. REFER FIBER DETAIL SHEET FOR CONDUIT AND JUNCTION BOX INSTALLATION ON BRIDGE STRUCTURE
  2. JUNCTION BOX ATTACHED TO STRUCTURE IS TO FACILITATE THE INSTALLATION OF UNIT DUCT ONLY. NO SPLICING OF FIBER OR UNIT DUCT SHALL OCCUR WITHIN THE JUNCTION BOXES
  3. CONTRACTOR SHALL LEAVE A MINIMUM OF 5' OF DUCT IN EACH FIBER OPTIC PULL BOX FOR EACH OF THE UNIT DUCTS. THIS IS BASED ON AN INSTALLATION TEMPERATURE OF 70-DEGREES FARENHEIT AND A MINIMUM TO MAXIMUM TEMPERATURE RANGE OF 0-100 DEGREES. CONTRACTOR SHALL VERIFY WITH THE ENGINEER IF ADDITIONAL SLACK FOR THE UNIT DUCT IS REQUIRED FOR EXPANSION/CONTRACTION



USER NAME =	agibson	DESIGNED -	R.SCARIA	REVISED -	
		DRAWN -	R.SCARIA	REVISED -	
PLOT SCALE =	SSCALE\$	CHECKED -	G.THIESSE	REVISED -	
PLOT DATE =	2/6/2025	DATE -	02/27/24	REVISED -	

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

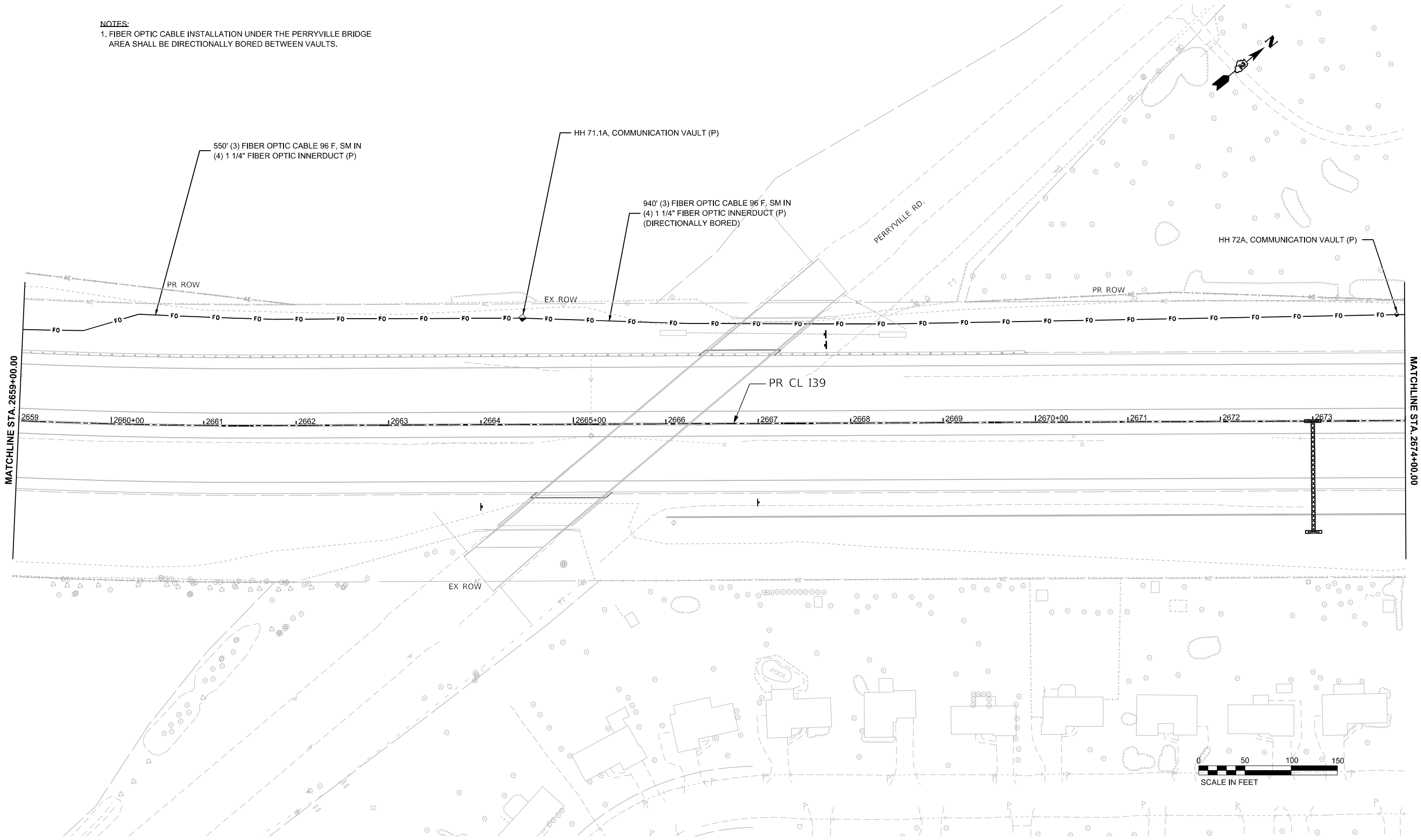
PROPOSED FIBER OPTIC PLANS  
I-39

SCALE: 1"=50' SHEET 2 OF 26 SHEETS STA. 2644+00.00 TO STA. 2659+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	519
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

NOTES:  
1. FIBER OPTIC CABLE INSTALLATION UNDER THE PERRYVILLE BRIDGE  
AREA SHALL BE DIRECTIONALLY BORED BETWEEN VAULTS.



MODEL: 130 Wide - FO Plan 8 (Sheet)  
FILE NAME: c:\paword\benesch projects\projects\0173716\0264C24\_ML-sh-Fiber-Proposed-139.dgn



USER NAME =	agibson	DESIGNED -	R.SCARIA	REVISED -	
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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

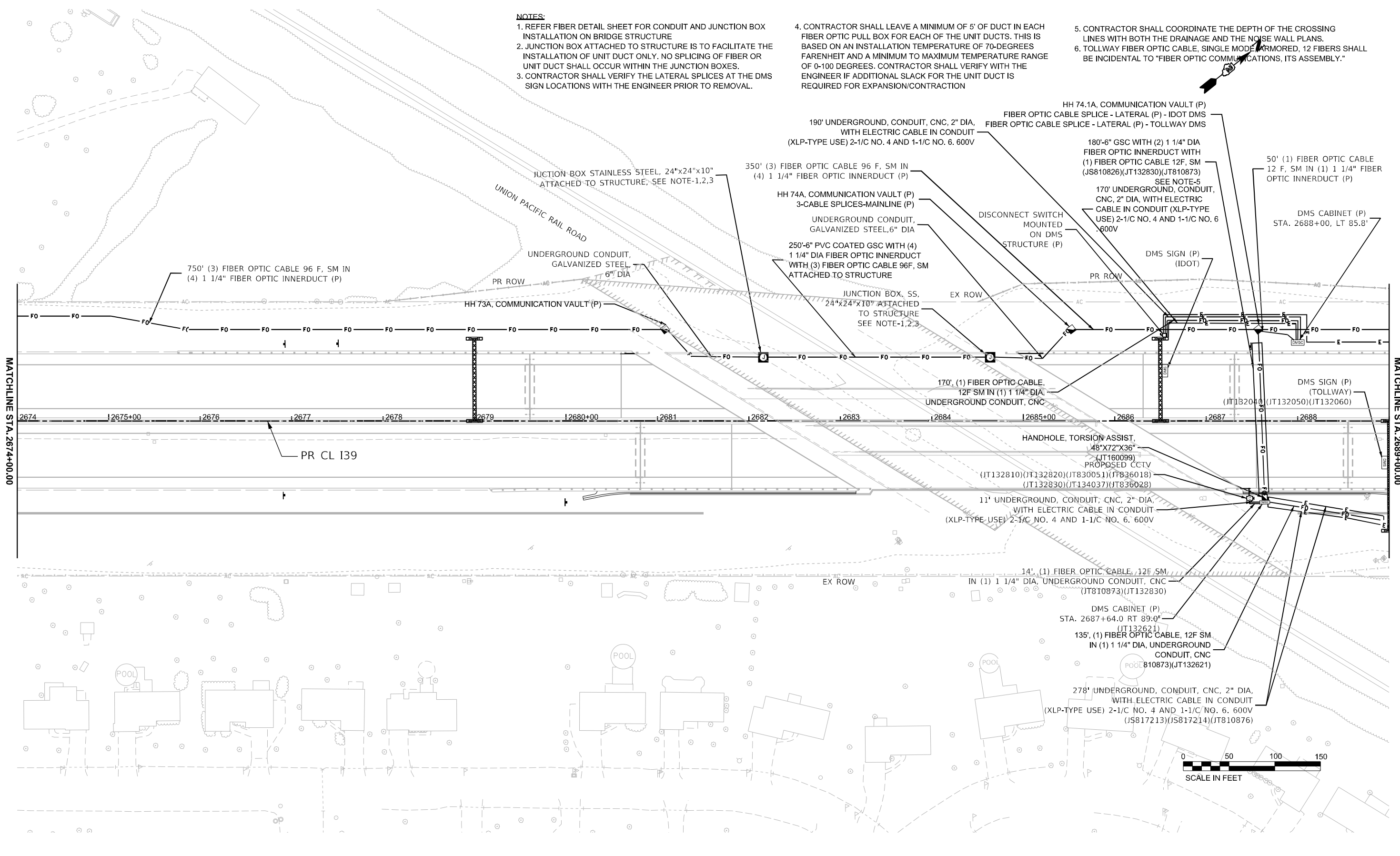
PROPOSED FIBER OPTIC PLANS  
I-39

SCALE: 1"=50' SHEET 3 OF 26 SHEETS STA. 2659+00.00 TO STA. 2674+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	520
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

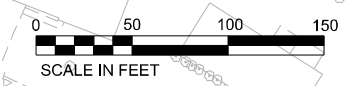




- NOTES:**
1. REFER FIBER DETAIL SHEET FOR CONDUIT AND JUNCTION BOX INSTALLATION ON BRIDGE STRUCTURE
  2. JUNCTION BOX ATTACHED TO STRUCTURE IS TO FACILITATE THE INSTALLATION OF UNIT DUCT ONLY. NO SPLICING OF FIBER OR UNIT DUCT SHALL OCCUR WITHIN THE JUNCTION BOXES.
  3. CONTRACTOR SHALL VERIFY THE LATERAL SPLICES AT THE DMS SIGN LOCATIONS WITH THE ENGINEER PRIOR TO REMOVAL.
  4. CONTRACTOR SHALL LEAVE A MINIMUM OF 5' OF DUCT IN EACH FIBER OPTIC PULL BOX FOR EACH OF THE UNIT DUCTS. THIS IS BASED ON AN INSTALLATION TEMPERATURE OF 70-DEGREES FARENHEIT AND A MINIMUM TO MAXIMUM TEMPERATURE RANGE OF 0-100 DEGREES. CONTRACTOR SHALL VERIFY WITH THE ENGINEER IF ADDITIONAL SLACK FOR THE UNIT DUCT IS REQUIRED FOR EXPANSION/CONTRACTION
  5. CONTRACTOR SHALL COORDINATE THE DEPTH OF THE CROSSING LINES WITH BOTH THE DRAINAGE AND THE NOISE WALL PLANS.
  6. TOLLWAY FIBER OPTIC CABLE, SINGLE MODE ARMORED, 12 FIBERS SHALL BE INCIDENTAL TO "FIBER OPTIC COMMUNICATIONS, ITS ASSEMBLY."

MATCHLINE STA. 2674+00.00

MATCHLINE STA. 2689+00.00



MODEL: I-39 Wide - FO Plan 9 (Sheet)  
FILE NAME: c:\pwworkdir\benesch\projects\projects\10173716\I264C24\_ML-sh-Fiber-Proposed-I39.dgn



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PLOT SCALE	= \$SCALE\$	CHECKED -	G.THIESSE	REVISED -	
PLOT DATE	= 2/6/2025	DATE	= 02/27/24	REVISED -	

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

PROPOSED FIBER OPTIC PLANS  
I-39

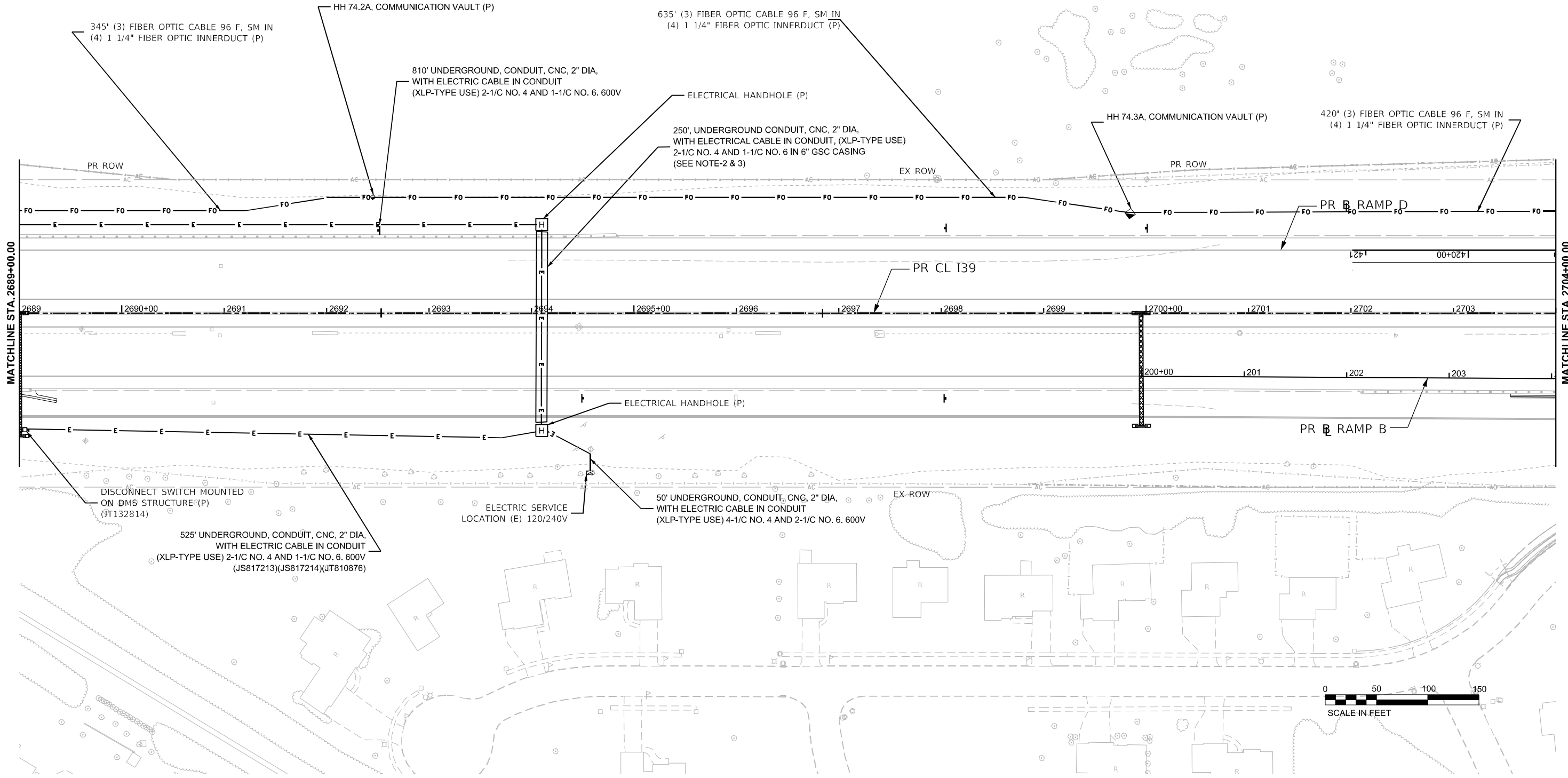
SCALE: 1"=50' SHEET 4 OF 26 SHEETS STA. 2674+00.00 TO STA. 2689+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	521
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

NOTES:

1. CONTRACTOR SHALL VERIFY THE LATERAL SPLICES AT THE DMS SIGN LOCATIONS WITH THE ENGINEER PRIOR TO REMOVAL.  
2. DIRECTIONAL BORE SHALL HAVE 2 FEET (MIN.) BETWEEN THE BASE OF THE NOISEWALL PANEL AND THE TOP OF THE BORE. SEE STRUCTURAL PLANS FOR ADDITIONAL INFORMATION.  
3. CONTRACTOR SHALL COORDINATE THE DEPTH OF THE CROSSING LINES WITH BOTH THE DRAINAGE AND THE NOISE WALL PLANS.



MODEL: I39 Wide - FO Plan 10 (Sheet)  
FILE NAME: c:\pwworking\benesch\_projects\projects\0173716\0264C24\_ML-sh-Fiber-Proposed-I39.dgn



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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

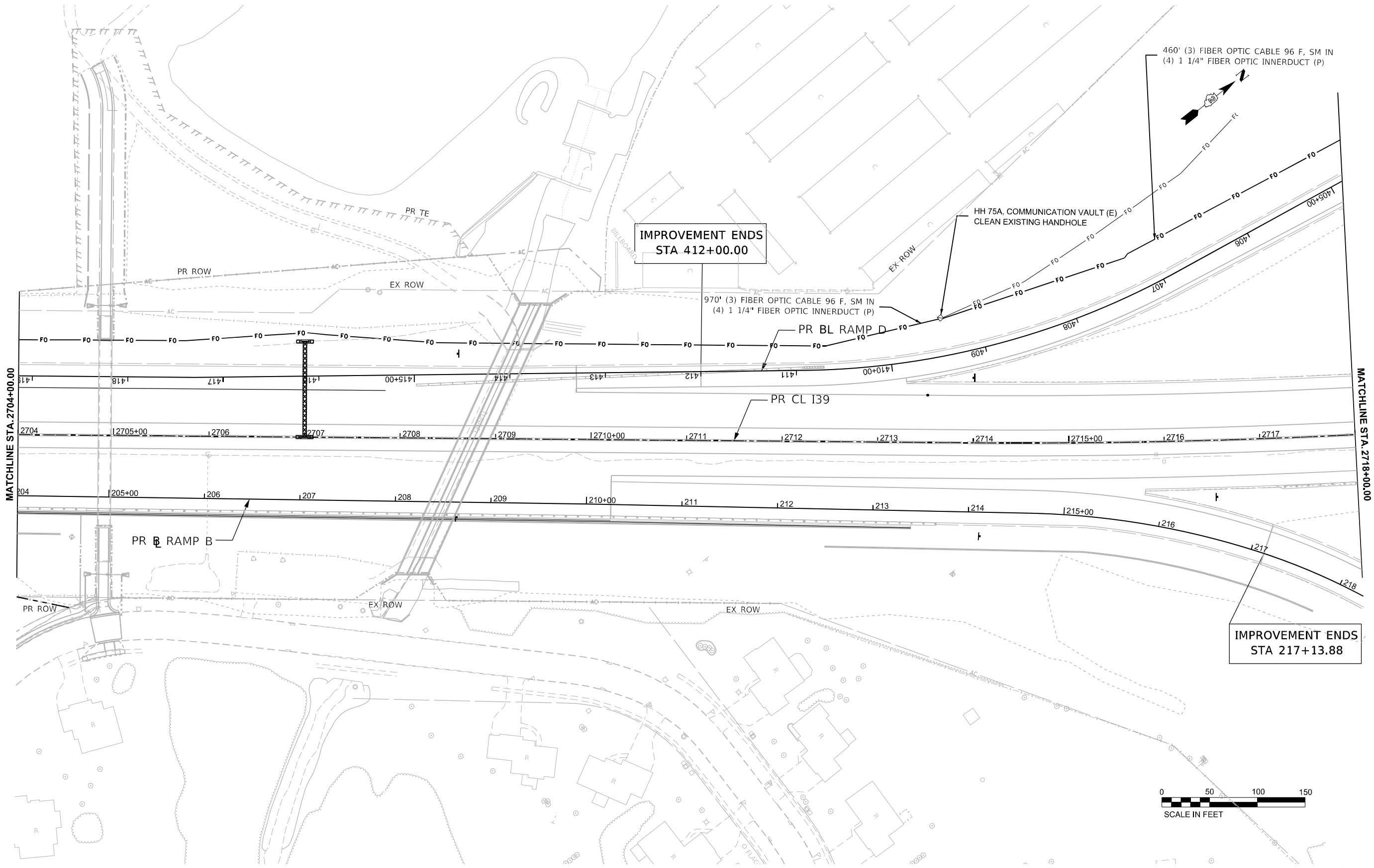
PROPOSED FIBER OPTIC PLANS  
I-39

SCALE: 1"=50'    SHEET 5    OF 26    SHEETS    STA. 2689+00.00    TO STA. 2704+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	522
CONTRACT NO. 64C24				
ILLINOIS    FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

MODEL: I39 Wide - FO Plan 11 (Sheet)  
FILE NAME: c:\pwworkdir\benesch\projects\projects\173716\I264C24\_ML-shr-Fiber-Proposed-I39.dgn



USER NAME	= jMajcher	DESIGNED	- R.SCARIA	REVISED	-
		DRAWN	- R.SCARIA	REVISED	-
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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

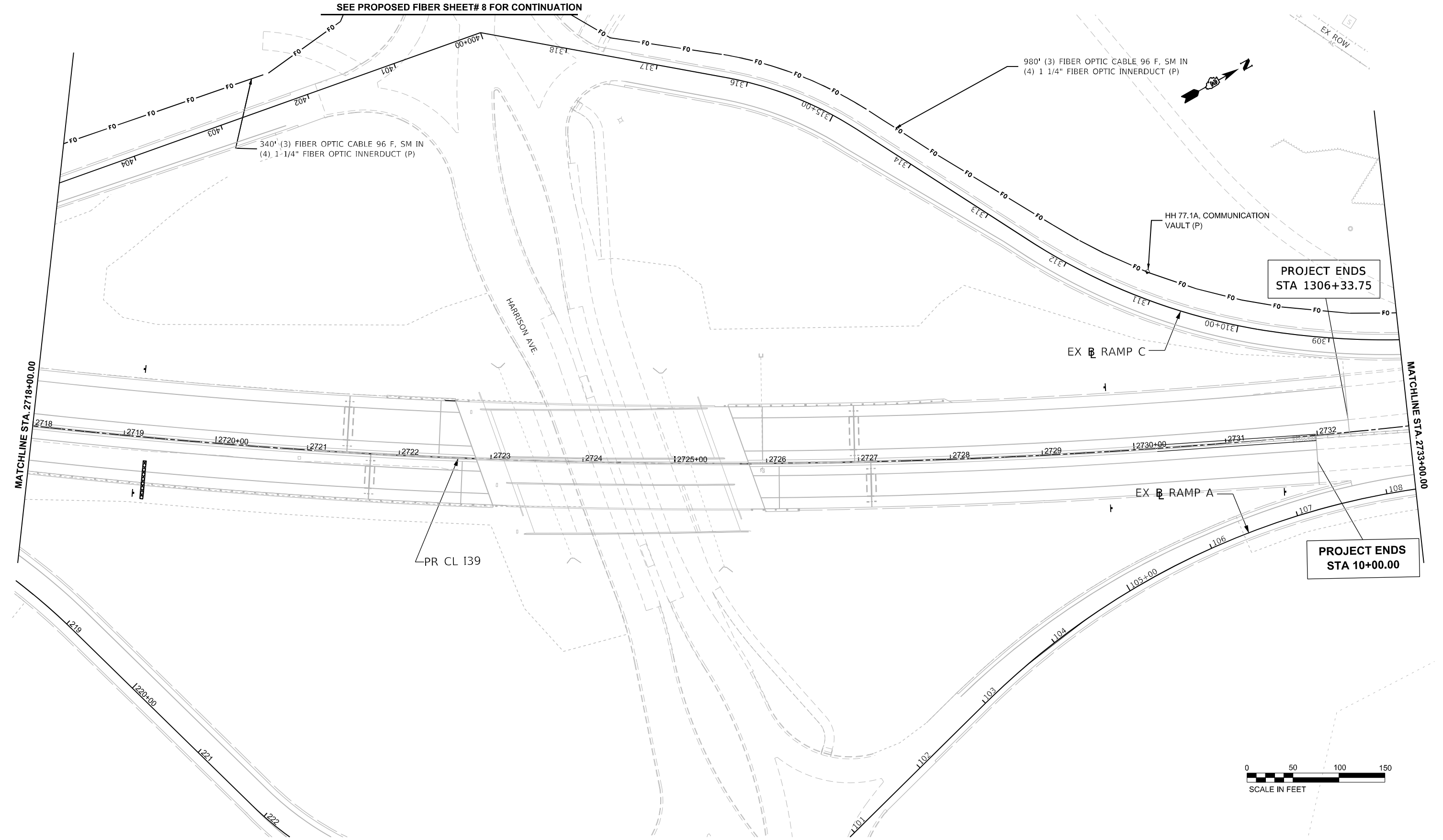
PROPOSED FIBER OPTIC PLANS  
I-39

SCALE: 1"=50' SHEET 6 OF 26 SHEETS STA. 2704+00.00 TO STA. 2718+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	523
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

MODEL: I-39 Wide - FO Plan 12 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\projects\173716\I-39 Wide - FO Plan 12 [Sheet].dgn



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PLOT DATE	= 2/6/2025

DESIGNED	- R.SCARIA
DRAWN	- R.SCARIA
CHECKED	- G.THIESSE
DATE	- 02/27/24

REVISED	-
REVISED	-
REVISED	-
REVISED	-

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

PROPOSED FIBER OPTIC PLANS  
I-39

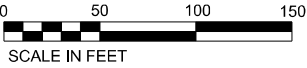
SCALE: 1"=50' SHEET 7 OF 26 SHEETS STA. 2718+00.00 TO STA. 2733+00.00

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	524
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



SEE PROPOSED FIBER SHEET# 9 FOR CONTINUATION



SEE PROPOSED FIBER SHEET# 7 FOR CONTINUATION

980' (3) FIBER OPTIC CABLE 96 F, SM IN  
(4) 1 1/4" FIBER OPTIC INNERDUCT (P)

200'-6" GSC WITH (4) 1 1/4" DIA FIBER OPTIC  
INNERDUCT WITH (3) FIBER OPTIC CABLE 96F, SM

HH 77A,  
COMM. VAULT (P)

102' (3) FIBER OPTIC  
CABLE 96 F, SM IN (4)  
1 1/4" FIBER OPTIC  
INNERDUCT (P)

HH 76A,  
COMM. VAULT (P)

85' (3) FIBER OPTIC CABLE  
96 F, SM IN (4) 1 1/4" FIBER  
OPTIC INNERDUCT (P)

340' (3) FIBER OPTIC CABLE 96 F, SM IN  
(4) 1 1/4" FIBER OPTIC INNERDUCT (P)

HARRISON AVE.

PR CL I39

MODEL: Harrison Wide - Plan 3-1 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\projects\projects\0173716\0264C24\_ML-sh-Fiber-Proposed-139.dgn



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PLOT DATE = 2/6/2025		

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

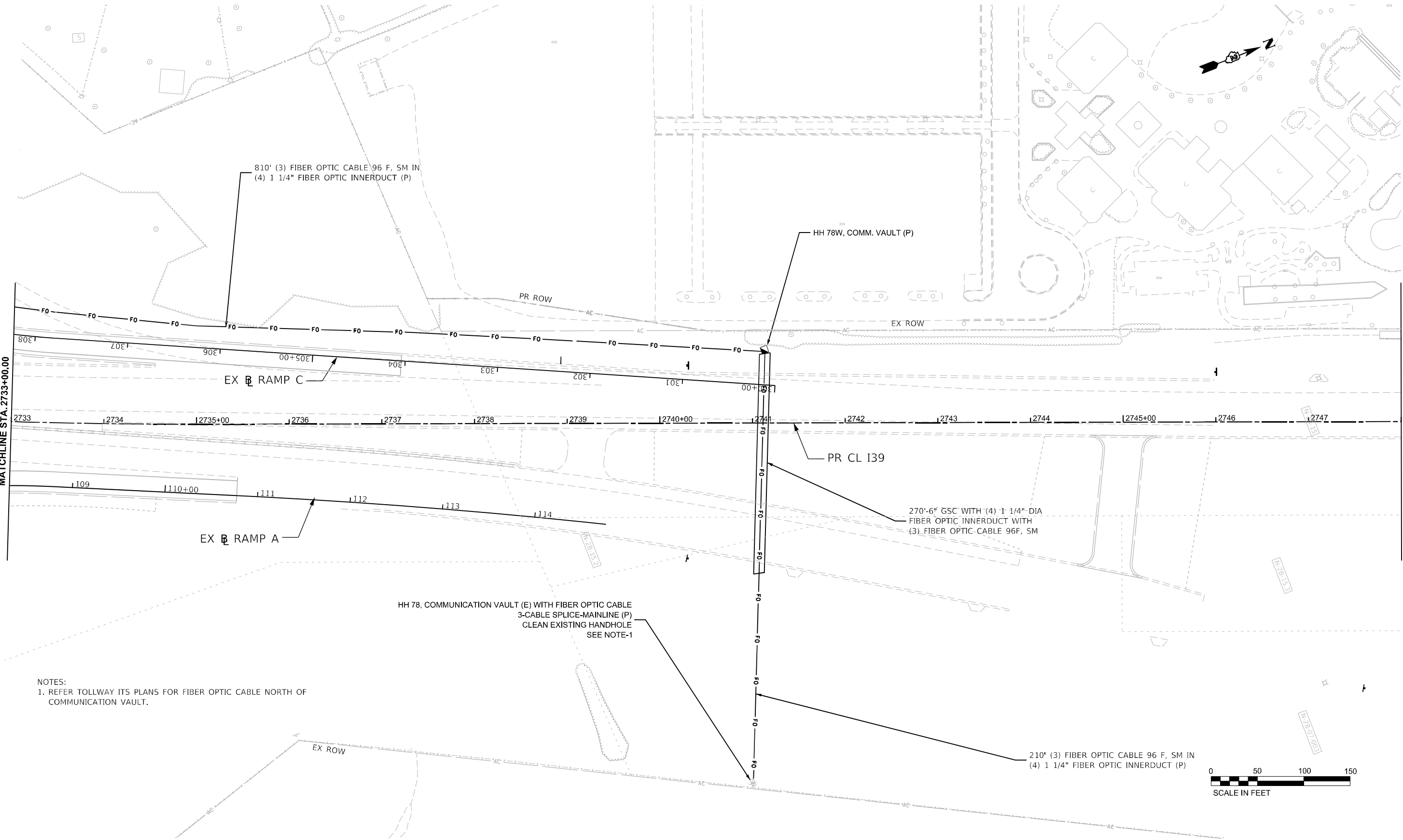
PROPOSED FIBER OPTIC PLANS  
HARRISON AVE

SCALE: 1"=50' SHEET 8 OF 26 SHEETS STA. 146+00.00 TO STA. A 1319+00.05

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	525
CONTRACT NO. 64C24				
ILLINOIS		FED. AID PROJECT		

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

MODEL: I-39 Wide - FO Plan 13 (Sheet)  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\i0173716\I284C24\_ML-sh-Fiber-Proposed-I39.dgn



NOTES:  
1. REFER TOLLWAY ITS PLANS FOR FIBER OPTIC CABLE NORTH OF COMMUNICATION VAULT.



USER NAME =	agibson	DESIGNED -	R.SCARIA	REVISED -	
		DRAWN -	R.SCARIA	REVISED -	
PLOT SCALE =	SSCALE\$	CHECKED -	G.THIESSE	REVISED -	
PLOT DATE =	2/6/2025	DATE -	02/14/2025	REVISED -	

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

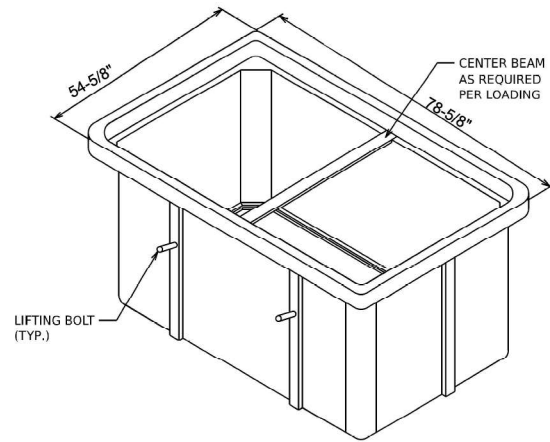
PROPOSED FIBER OPTIC PLANS  
I-39

SCALE: 1"=50' SHEET 9 OF 26 SHEETS STA. 2733+00.00 TO STA. 2748+00.00

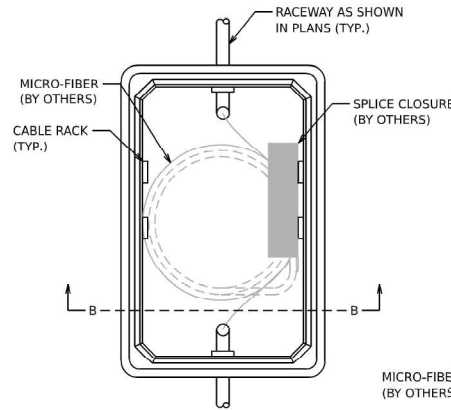
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*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	526
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

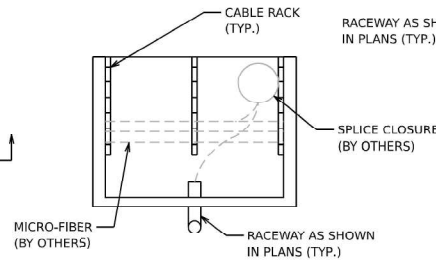




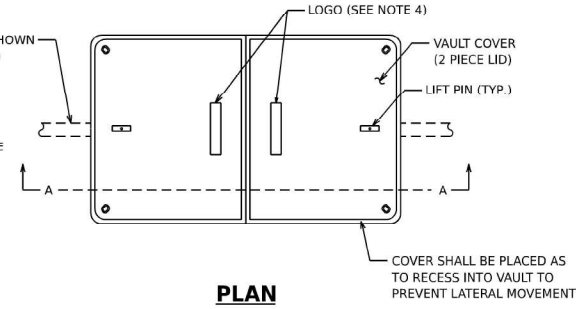
**VAULT BOX  
ISOMETRIC VIEW**



**TOP VIEW**

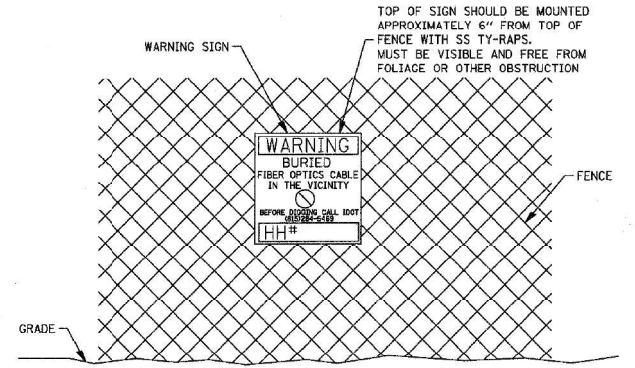


**SECTION B-B**



**PLAN**

- NOTES:
- GROUND ROD, 1-1/2" CONDUIT, #6 AWG GROUND WIRE, AND ASSOCIATED WORK ARE INCLUDED AS PART OF COMMUNICATIONS VAULT AND WILL NOT BE PAID FOR SEPARATELY. ALL MATERIALS FOR MECHANICAL CONNECTION SHALL BE UL LISTED AND INSTALLED PER NEC ARTICLE 250.
  - IDOT COMMUNICATIONS VAULTS SHALL HAVE A PERMANENTLY RECESSED LOGO THAT READS "IDOT" AND THIRD PARTY COMMUNICATIONS VAULTS SHALL HAVE A PERMANENTLY RECESSED LOGO THAT READS "IDOT - BB".
  - COMMUNICATIONS VAULT SHALL HAVE AN OPEN BASE. ALL CONDUITS AS SHOWN ON THE PLANS SHALL ENTER THE VAULT VIA THE OPEN BASE.
  - ALL DIMENSIONS ARE MINIMUM AND A LARGER SIZE VAULT MAY BE USED, WITH THE APPROVAL OF THE ENGINEER, TO FACILITATE USING A MANUFACTURER'S STANDARD PRODUCT.



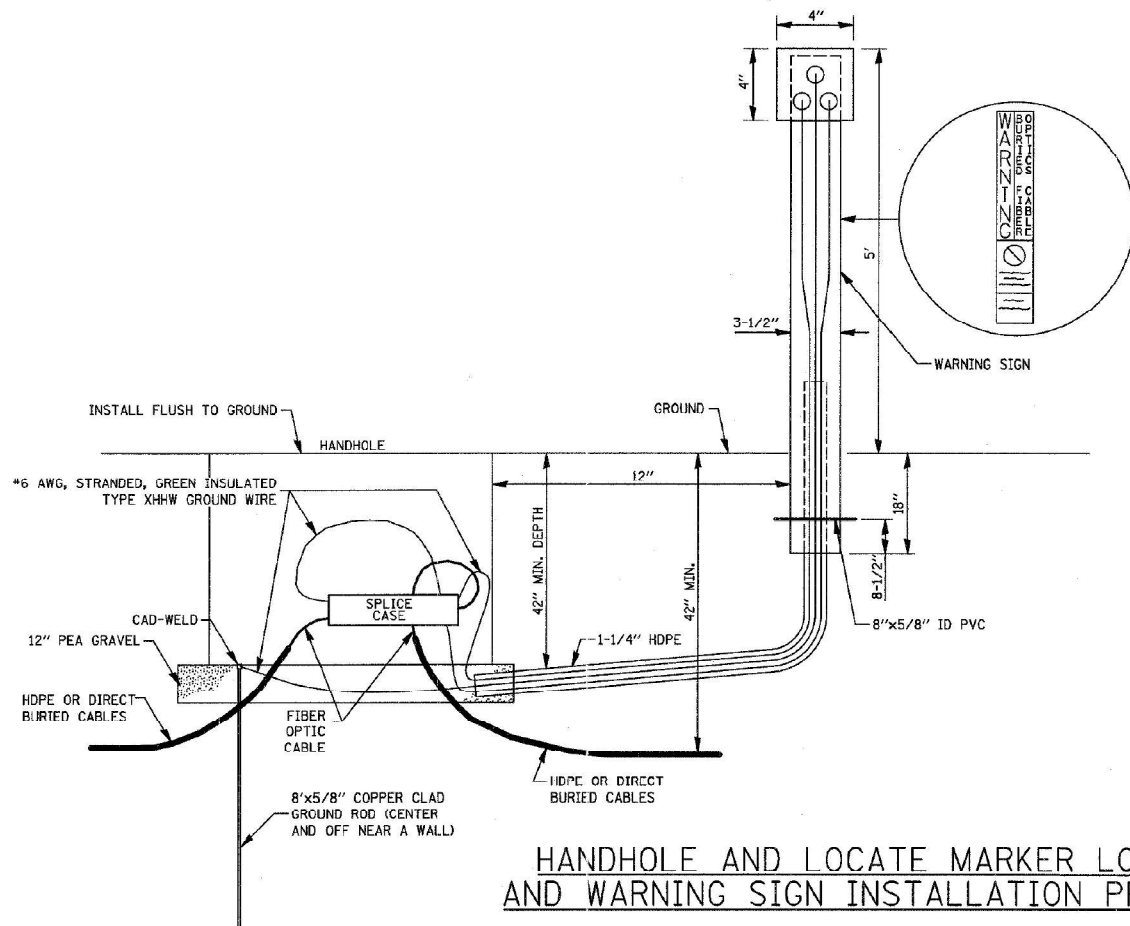
**FENCE MOUNTED WARNING SIGN  
NTS**

**NOTES**

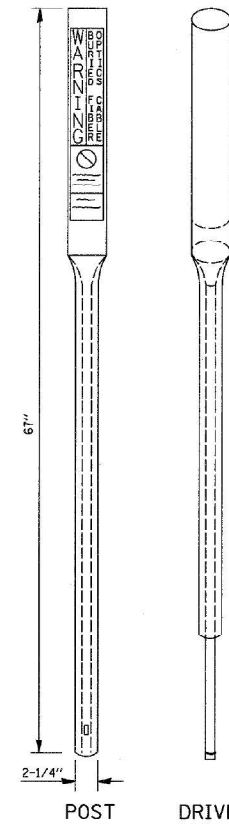
- ROUTE MARKER TO BE PLACED 1 FOOT FROM HANDHOLE OR AT FENCE LINE IF POSSIBLE.
- HANDHOLES SHALL BE BACKFILLED ONLY TO THE TOP OF THE BOX, FLUSH TO GROUND.
- COIL FIBER CABLE IN HANDHOLE ENSURING THAT THE BEND RADIUS DOES NOT EXCEED VALUES IN TABLE "A".
- INSTALL GROUND ROD & CAD-WELD AS PER MANUFACTURE'S INSTRUCTIONS. PLACE THE #6 GROUND WIRE (TYPE XHHW, STRANDED, GREEN INSULATED) THAT HAS BEEN ATTACHED TO THE GROUND ROD ON THE CENTER LUG OF THE WARNING SIGN. GROUND RODS AND GROUND WIRES INCLUDED IN THE COST OF HANDHOLE INSTALLATION.
- BACKFILL MATERIAL SHALL BE COMPACTED TO THE SATISFACTION OF THE ENGINEER.
- GROUND WIRE SHALL BE BONDED TO BOTH SHEATHS OF ARMORED FIBER OPTIC CABLE IN THE SPLICE ENCLOSURE USING #6 GROUND WIRE. EACH GROUND SHALL BE ISOLATED WITHIN THE ENCLOSURE.
- INSTALL 1-1/4" HDPE CONDUIT FROM HANDHOLE TO WARNING SIGN TO ALLOW GROUNDING CABLE TO BE INSTALLED.
- REFERENCE TYPICAL DRAWING FOR HANDHOLE INSTALLATION.
- PLACE 1-1/4" HDPE OVER FIBER OPTIC CABLE TO PROVIDE CRUSH PROTECTION, EXTEND HDPE 1' INSIDE HANDHOLE.
- NO HANDHOLES WILL BE ALLOWED IN PAVED ROADWAYS OR SHOULDERS.
- THE TOPS OF ALL HANDHOLES SHALL BE FLUSH WITH THE ADJACENT SLOPES.
- A WARNING SIGN/LOCATE SIGN IS REQUIRED AT ALL HANDHOLES, AND IS INCLUDED IN COST OF HANDHOLE INSTALLATION.
- FOR ALL SPLICE AND HANDHOLES, NUMBER DECALS WILL BE APPLIED AFTER INSTALLATION IS COMPLETED AND AT THE DIRECTION OF THE ENGINEER.

**TABLE "A"**

FIBER COUNT	MINIMUM BEND RADIUS (AT REST)
24F	6"
48F	6"
72F	8"
96F	8"
144F	10"
192F	10"
288F	10"
432F	10"
864F	11"



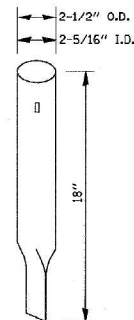
**HANDHOLE AND LOCATE MARKER LOCATION  
AND WARNING SIGN INSTALLATION PROCEDURE**



**NOTE: DAMAGE TO SOIL ANCHORS NOT  
INSTALLED W/ APPROVED DRIVER WILL  
BE THE RESPONSIBILITY OF  
CONTRACTOR FOR REPLACEMENT**

**INSTALLATION OF WARNING POST**

- USE THE SENTRY POST® DRIVER TO RAPIDLY INSTALL THE SENTRY POST® IN MOST SOIL CONDITIONS.
- THE SOIL ANCHOR IS DRIVEN UNTIL FLUSH WITH GROUND LEVEL OR SLIGHTLY BELOW THE SURFACE.
- PUSH THE SENTRY POST® DOWN INTO THE ANCHOR APPROXIMATELY 9", THEN PULL UP UNTIL IT LOCKS INTO PLACE.



**STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION**

**FIBER OPTIC DETAILS  
COMMUNICATION VAULT AND LOCATE POST**

SCALE: SHEET 10 OF 26 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	527
CONTRACT NO. 64C24				
ILLINOIS / FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

USER NAME = Ifranceschina	DESIGNED - R.SCARIA	REVISED -
PLOT SCALE = \$SCALES\$	DRAWN - R.SCARIA	REVISED -
PLOT DATE = 8/12/2024	CHECKED - G.THIESSE	REVISED -
	DATE - 02/14/2025	REVISED -

MODEL: HANDHOLE AND LOCATE MARKER [Sheet]  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\0173716\0284C24\_ML-sh-FiberDetails-17.dgn



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351 N. Wacker Drive, Suite 3000  
Chicago, Illinois 60601  
312-965-4150 Job No. 10900.00

USER NAME = Ifranceschina  
PLOT SCALE = \$SCALE\$  
PLOT DATE = 8/12/2024

DESIGNED -  
DRAWN -  
CHECKED - G. THIESSE  
DATE - 02/14/2025

REVISED -  
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REVISED -  
REVISED -

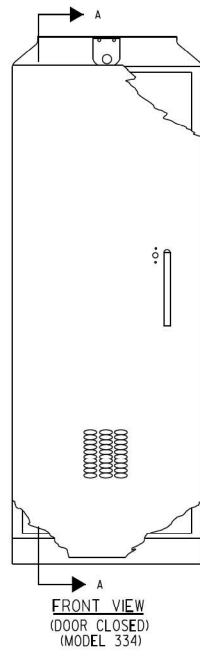
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC DETAILS  
CABINET MODEL 334 DETAIL

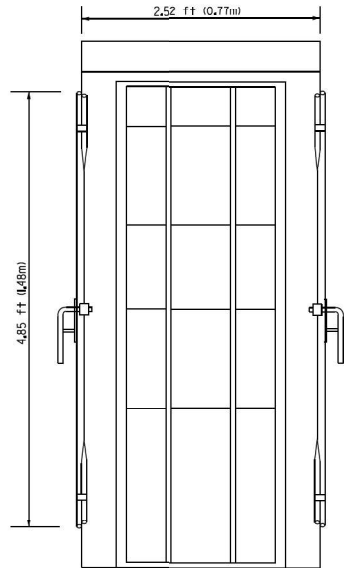
SCALE: SHEET OF 26 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	528
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

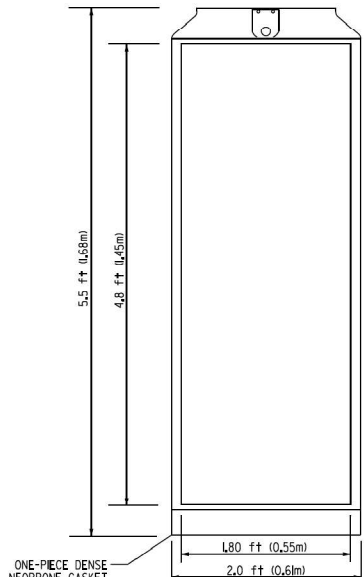
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



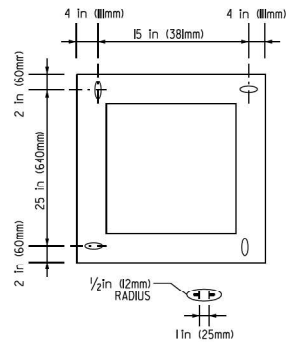
FRONT VIEW  
(DOOR CLOSED)  
(MODEL 334)



SECTION A-A  
FOUNDATION MOUNTED CABINET  
(MODEL 334)



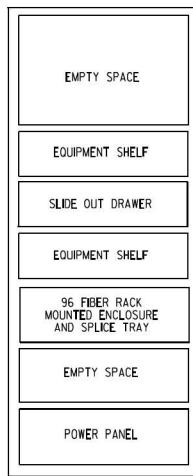
FRONT VIEW  
(DOOR REMOVED)  
(MODEL 334)



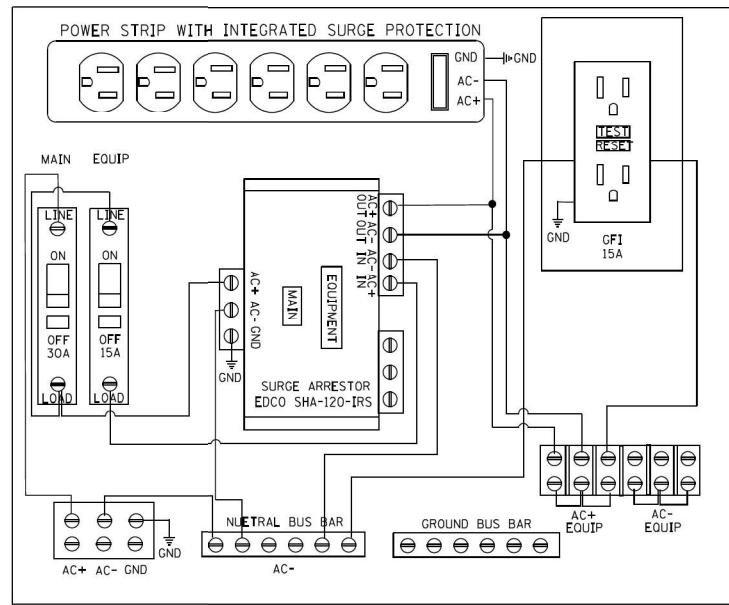
BOTTOM VIEW  
(MODEL 334)

NOTES (MODEL 334 CABINET):

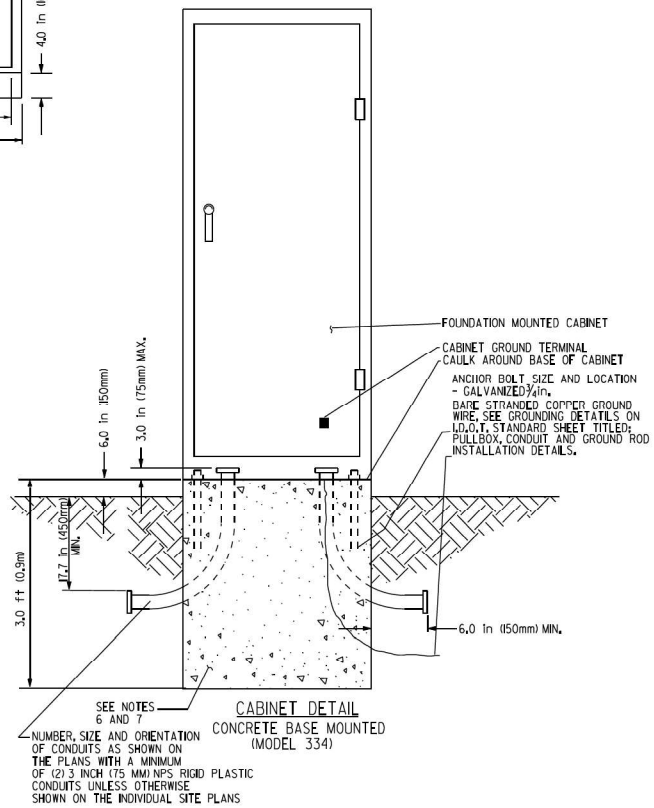
- REFER TO SPECIAL PROVISIONS FOR CABINET DETAILS AND ADDITIONAL REQUIREMENTS.
- CABINET ENTRIES INCLUDE VERTICAL ARRANGEMENT FOR MAJOR EQUIPMENT ITEMS ONLY.
- THERE WILL BE ADDITIONAL ITEMS INSTALLED IN THE CABINET ON SIDE AND BACK PANELS AS PER THE SPECIAL PROVISIONS.
- PROVIDE ALL UNUSED SPACE AT BOTTOM OF CABINET.
- THE CONTRACTOR SHALL INSTALL INSULATED BUSHINGS AND DUCT SEALANT AT ALL CONDUIT BEND TERMINATIONS IN FOUNDATIONS.
- CONCRETE BASE TO BE FORMED AT LEAST 6.0 in (150mm) BELOW THE GROUND SURFACE.
- CONCRETE BASE MUST BE CAST IN PLACE.
- THE CABINET SHALL BE EQUIPPED WITH A THERMOSTATICALLY CONTROLLED VENTILATION FAN, DELUXE PLEATED FILTER, POWER PANEL, 96 FIBER RACK MOUNTED ENCLOSURE WITH SPLICE TRAYS, 19" MOUNTING RAILS, EQUIPMENT SHELF, AND SLIDE OUT DRAWER WITH STORAGE.
- ALL WORK INDICATED SHALL BE PAID FOR UNDER ITEM CABINET, MODEL 334 EXCLUSIVE OF THE CONCRETE FOUNDATION.



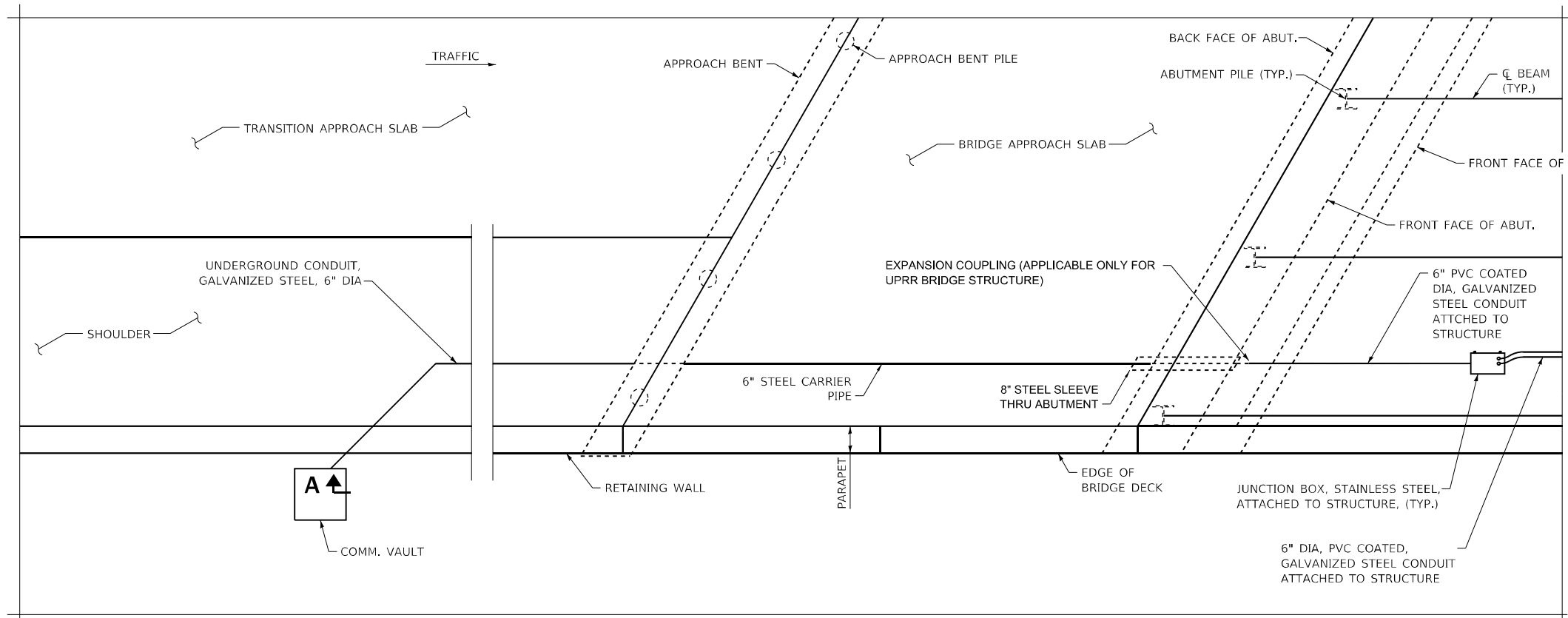
CONTROL CABINET  
LAYOUT (TYPICAL)  
(MODEL 334)



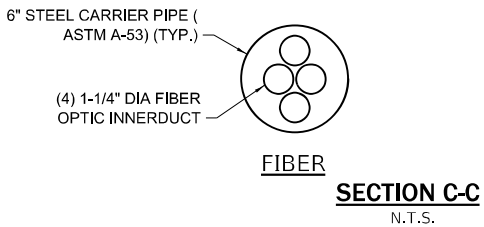
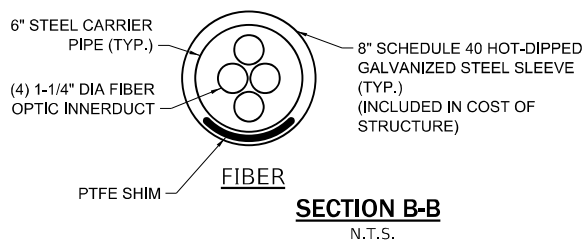
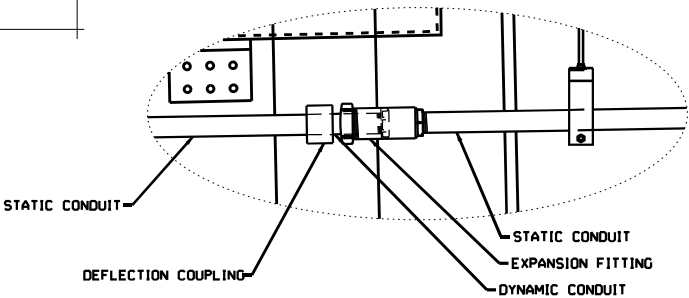
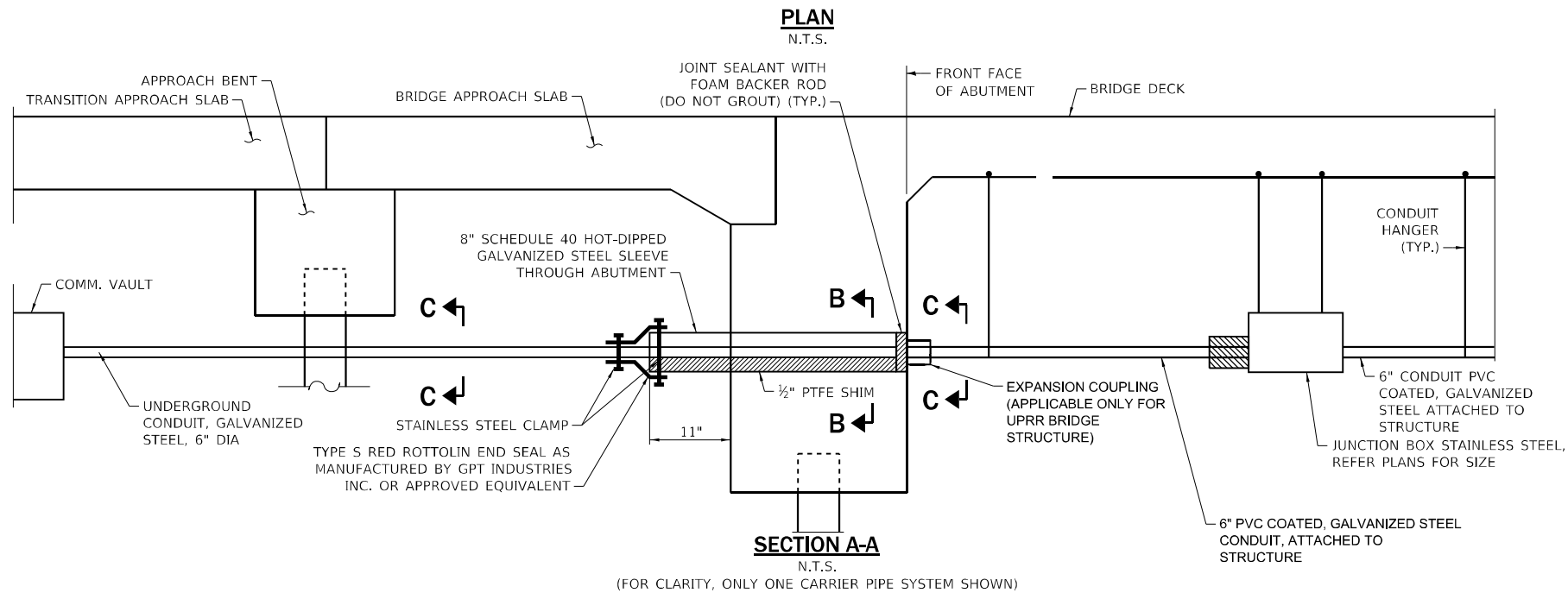
TYPE 334 CABINET POWER PANEL DETAIL  
(POWER PANEL TO BE EQUIPPED WITH PLEXI-GLASS SAFETY SHIELD)



CABINET DETAIL  
CONCRETE BASE MOUNTED  
(MODEL 334)



- NOTES:**
1. THE CONTRACTOR SHALL FURNISH & INSTALL A PULL TAPE THROUGH ALL CONDUITS INSTALLED AS PART OF THIS WORK.
  2. THE JUNCTION BOX SHALL MEET THE REQUIREMENTS OF ARTICLE 1088.04 OF THE STANDARD SPECIFICATIONS. A HINGED DOOR AND PROVISIONS FOR 3-POINT LOCK OR A PAD-LOCK ARE REQUIRED
  3. JUNCTION BOX SHALL BE LOCATED AT LEAST 2'-6" FROM CROSS FRAMES OR DIAPHRAMS.
  4. JUNCTION BOX DOORS SHALL BE POSITIONED FOR WORKERS TO FACE ONCOMING TRAFFIC WHEN ACCESSING THE BOX.
  5. MINIMUM BEND RADIUS FOR ALL CONDUIT SHALL BE PER MANUFACTURER'S REQUIREMENTS.
  6. THE FIBER INNERDUCT SHALL BE CONTINUOUS BETWEEN VAULTS.



MODEL: JB ATTACHED TO STRUCTURE DETAIL (Sheet)  
FILE NAME: c:\paword\benesch\_projects\projects\0173716\0264C24\_ML-sh-FiberDetails-2.dgn



USER NAME = Ifranceschina  
DESIGNED - R.SCARIA  
DRAWN - R.SCARIA  
CHECKED - G.THIESSE  
DATE - 02/14/2025

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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

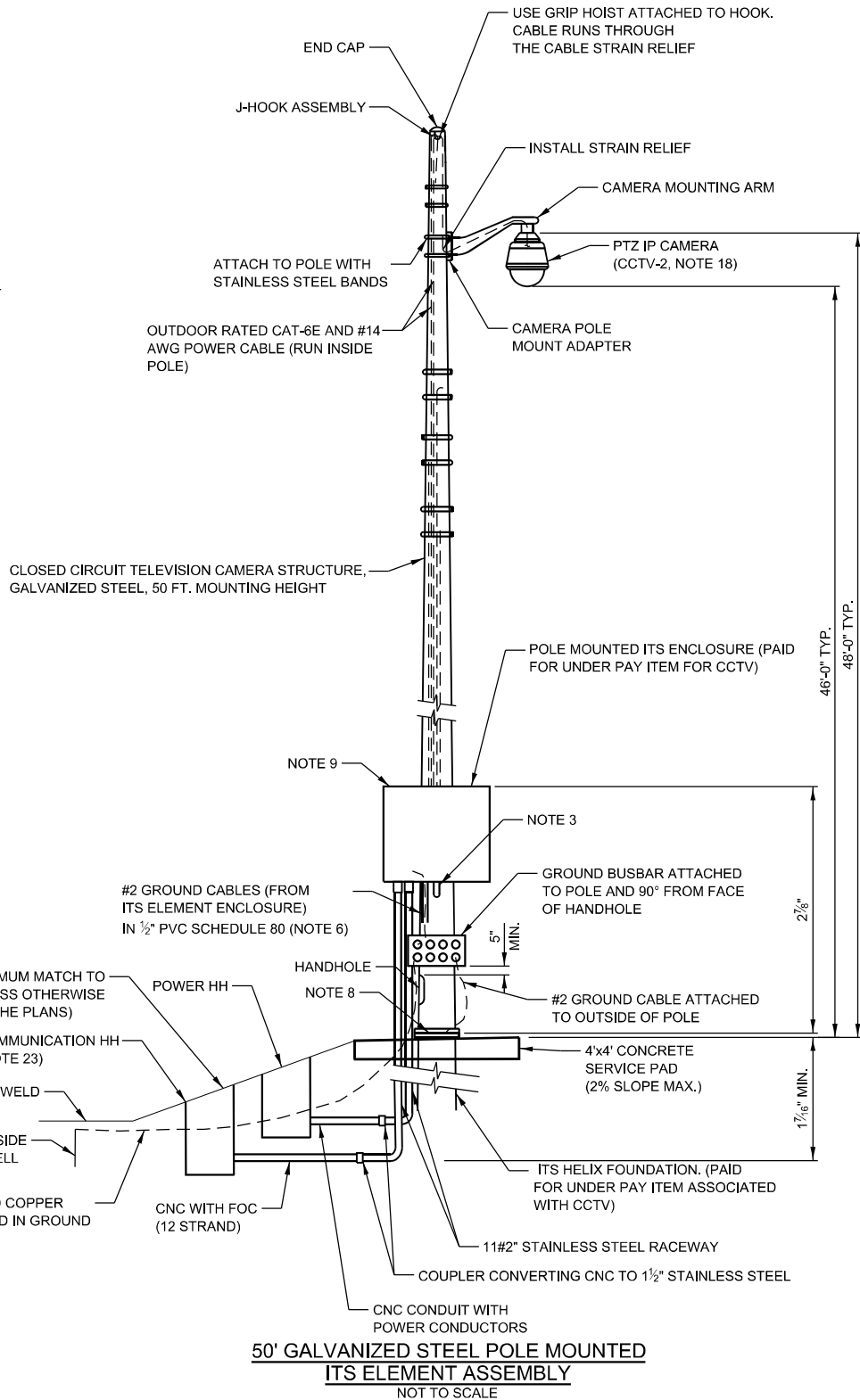
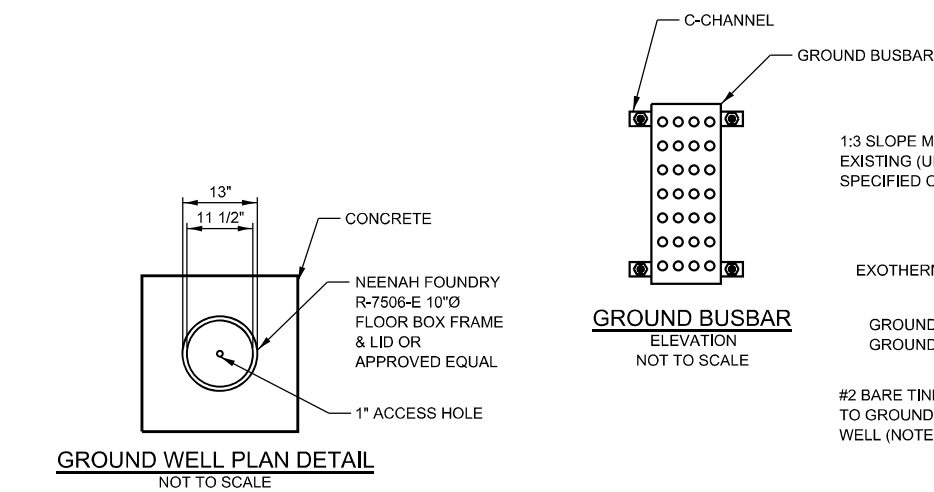
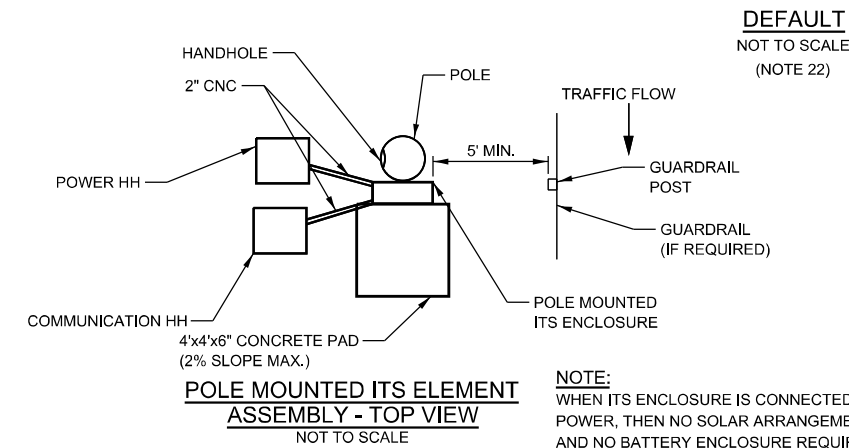
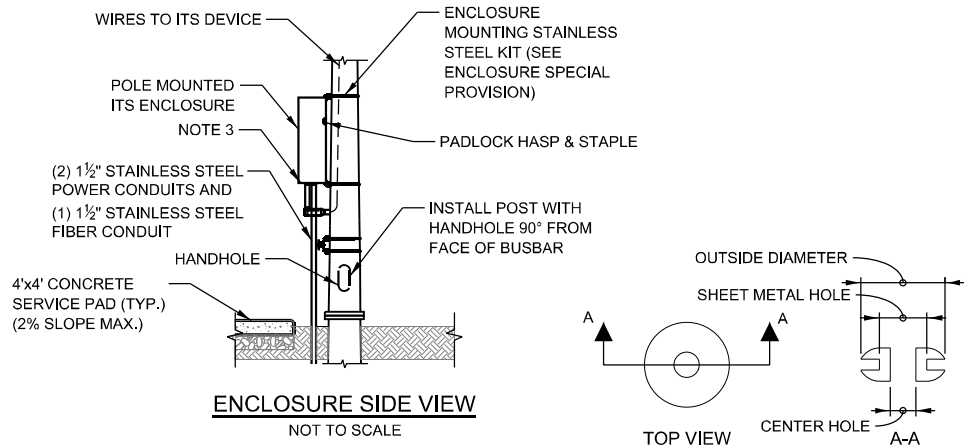
FIBER OPTIC DETAILS  
CONDUIT DETAILS AT ABUTMENT WITH WALLS

SCALE: SHEET 11 OF 26 SHEETS STA. TO STA.

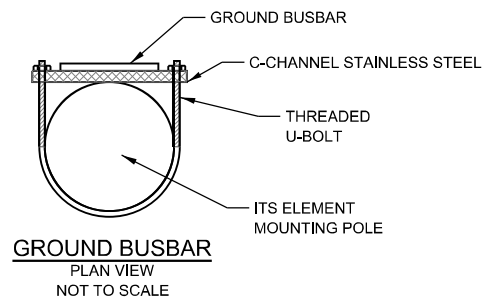
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	529
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

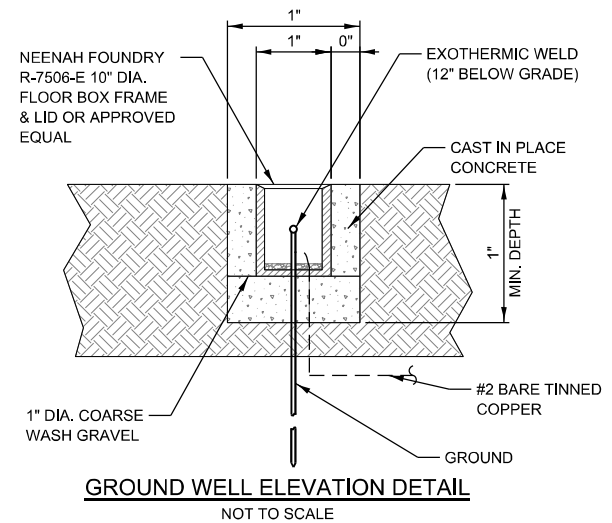
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	SITE				SUPPORT TYPE		
	MILEPOST	STATION	OFFSET	ORIENTATION	POLE	FOUNDATION	MOUNTING HEIGHT
CCTV-1	46.5	2687	84.5	NORTH WEST	STEEL	17 1/2" DIA. HELIX	50 FT.
CCTV-2							
WIRELESS MODEM							



**NOTE:**  
1. SEE FIBER OPTIC DETAILS SHEET-13 FOR NOTES.



USER NAME = Ifranceschina  
PLOT SCALE = \$SCALE\$  
PLOT DATE = 8/12/2024

DESIGNED - R.SCARIA  
DRAWN - R.SCARIA  
CHECKED - G.THIESSE  
DATE - 02/14/2025

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REVISED -  
REVISED -  
REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC DETAILS  
ELEVATION VIEWS POLE MOUNTED ITS ELEMENT ASSEMBLY

SCALE: SHEET 12 OF 26 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	530
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



MODEL: M-ITS-1001 [Sheet]  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\0173716\0264C24\_ML-sh-FiberDetails-4.dgn

GENERAL NOTES:

1. ITS ELEMENT POLES SHIELDED BY GUARDRAIL SHALL BE LOCATED A MINIMUM OF 5' TO A MAXIMUM OF 20' BEHIND THE GUARDRAIL POST. SEE ILLINOIS TOLLWAY GUARDRAIL STANDARD (SECTION C OF STANDARDS) FOR MORE INFORMATION. ALL OTHER POLES SHALL BE LOCATED OUTSIDE THE CLEAR ZONE. FINAL LOCATION TO BE APPROVED BY THE ENGINEER.
2. ANY GROUND CABLES ROUTED INSIDE THE ENCLOSURE SHALL BE GREEN INSULATED TYPE RHW CONDUCTORS. ANY GROUND CONDUCTORS THAT ARE BURIED SHALL BE BARE COPPER TINNED. ANY GROUND CONNECTED TO THE EXTERNAL GROUND BUSBAR SHALL BE CADWELDED TO THE BUSBAR. PVC SCH 80 CONDUIT SHOULD BE GROMMETTED ON END GOING TO BUSBAR TO PREVENT RODENTS AND INSECTS FROM ENTERING.
3. PROVIDE A 1½" ALUMINUM CONDUIT NIPPLE WITH LB FITTING FOR ROUTING ITS ELEMENT CABLES INSIDE THE POLE TO THE EQUIPMENT ENCLOSURE. DRILL AND TAP POLE FOR THE CONDUIT NIPPLE. CABLE SLACK SHALL BE PULLED AND FASTENED WITHIN THE TOP OF THE POLE. PROPER CABLE STRAIN RELIEF SHALL BE INSTALLED AND APPROVED BY THE ENGINEER. ALL CABLE RUN INSIDE THE POLE SHALL NOT HANG BELOW THE TOP OF THE HANDHOLE COVER ON THE POLE.
4. ALL CONDUITS ENTERING THE ENCLOSURE SHALL BE SEALED. SEE "ITS POLE MOUNTED ENCLOSURE, ITS ASSEMBLY (CCTV OR MVDS)" SPECIAL PROVISION FOR MORE DETAIL FOR RODENT PROTECTION.
5. CONTRACTOR TO PROVIDE ALL POWER, COMMUNICATIONS AND GROUND WIRING REQUIRED FOR SYSTEM OPERATION.
6. ATTACH PVC SCH 80 CONDUIT TO POLE FOR SUPPORT. USE METAL BUSHING WHEN CONNECTING PVC TO CABINET. USE GROMMETS AT BOTH ENDS OF CONDUIT TO SEAL CONDUIT BUT ALLOW GROUND CABLE TO RUN THROUGH BOTH ENDS.
7. GROUND ROD SHALL BE PLACED A MINIMUM OF 10' FROM THE FOUNDATION. A GROUND WELL SHALL BE INCLUDED TO PERMIT ACCESS TO THE GROUND ROD CONNECTION. CONNECTION TO THE GROUND BUSBAR AND THE GROUND ROD SHALL BE CADWELD.
8. A FLAT STEEL MESH PANEL ALONG WITH A COMMERCIALLY AVAILABLE HYDROPHOBIC LOW DENSITY COMPOSITE BACKFILL MATERIAL (KNOWN AS Q-SET 250) SHALL BE INSTALLED BETWEEN THE ANCHOR BASE AND THE POLE TO PREVENT THE ENTRY OF RODENTS INTO THE POLE. SEE SPECIAL PROVISIONS FOR MORE DETAILS.
9. THIS ITS ELEMENT ENCLOSURE DETAIL WILL BE UTILIZED FOR POLE MOUNTED APPLICATIONS ONLY, IT CANNOT BE UTILIZED FOR TOWER MOUNTED APPLICATION.
10. BACKFILL PER ILLINOIS TOLLWAY STANDARD H1. BACKFILL SHALL BE TO THE TOP OF THE POLE BASE ON ALL SIDES.
11. ALL CABLING (INCLUDING CABLING INSIDE THE ENCLOSURE) IS OUTDOOR RATED. CAMERA CABLE PART NUMBERS ARE: CAT-6E CABLE (BELDEN CATALOG NO. 7953A) AND #14 AWG 3/C CCTV POWER CABLE (BELDEN CATALOG NO. 9367). THE GROUND WIRE (WHITE) IN THE 3/C #14 AWG POWER CABLE SHALL BE TAPED GREEN. ANY OTHER ITS ELEMENT WILL USE SPECIFIC CABLE ASSOCIATED TO THAT ELEMENT.
12. THE J-HOOK SHALL BE WELDED IN PLACE TO THE SIDE OF THE POLE, NEAR THE TOP OF THE POLE. THE CONTRACTOR SHALL PROVIDE A CUSTOM FLAT TOP POLE CAP THAT WILL FIT THE POLE TOP WITH THE J-HOOK WELDED TO THE SIDE. THE POLE CAP SHALL BE SECURED TO THE POLE BY DRILLING AND INSERTING SET SCREWS.
13. THIS DRAWING IS A MULTI-PURPOSE DRAWING THAT INCLUDES TWO TYPES OF CONNECTIONS TO A SOLAR POWERED BATTERY ENCLOSURE. IF SOLAR POWER IS UTILIZED, THEN THE SPECIAL PROVISIONS WILL CALL OUT THE MATERIAL AND NECESSARY CONNECTIONS TO THE ITS ELEMENT ENCLOSURE.
14. CONSTRUCT A 4 FT. X 4 FT. CONCRETE SERVICE PAD 6-INCHES FROM THE POLE BASE ON THE SAME SIDE AS THE ITS ENCLOSURE, CENTERED WITH THE ITS ENCLOSURE.
15. THIRTY DAYS PRIOR TO INSTALLING ANY NEW CCTV CAMERA, MVDS, SWITCH, WIRELESS OR FIBER OPTIC MODEM, THE CONTRACTOR SHALL COORDINATE DEVICE CONFIGURATION WITH THE ENGINEER.
16. THE DISCONNECT SWITCH, SUPPORT, AND ASSOCIATED CONDUIT SHALL BE INSTALLED FOR ITS SITES WHERE THE UTILITY SERVICE INSTALLATION IS GREATER THAN 500 FEET FROM THE ITS SITE OR LOCATED ON THE OPPOSITE SIDE OF THE ROADWAY FROM THE ITS SITE.
17. ALL SLOPE RATIOS ARE EXPRESSED AS UNITS OF VERTICAL DISPLACEMENT TO UNITS OF HORIZONTAL DISPLACEMENT (V:H).
18. CABLES SHALL ENTER POLES THROUGH A GROMMET. GROMMET SIZE SHALL BE CHOSEN SO THAT THE CENTER HOLE FORMS A WATER TIGHT SEAL AROUND THE CABLES.
19. IF HANDHOLE IS INSTALLED NEAR THE BOTTOM OF A DITCH, THEN IT SHALL NOT BE INSTALLED BELOW THE FIFTY YEAR FLOOD ELEVATION.

CCTV NOTES:

18. FINAL PLACEMENT HEIGHTS OF THE CCTV CAMERAS SHALL BE BASED ON SITE CONDITIONS, ILLINOIS TOLLWAY OPERATIONAL NEEDS, AND AS PER MANUFACTURER'S MOUNTING RECOMMENDATIONS. THE HEIGHT SHALL BE APPROVED BY THE ENGINEER ONLY AFTER REVIEW BY ILLINOIS TOLLWAY ITS OPERATIONS. FOR SITE WHERE 2 CCTV TO BE INSTALLED ON SAME ITS POLE: KEEP A MINIMUM 24 INCHES HEIGHT DIFFERENCE BETWEEN THE 2 CCTV.



Affiliated Research & Company  
3510 Wacker Drive, Suite 3000  
Chicago, Illinois 60601  
312-565-4150 Job No. 10800.00

USER NAME	= Ifranceschina
PLOT SCALE	= \$\$SCALE\$
PLOT DATE	= 8/12/2024

DESIGNED	-	R.SCARIA
DRAWN	-	R.SCARIA
CHECKED	-	G.THIESSE
DATE	-	02/14/2025

REVISED	-
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REVISED	-

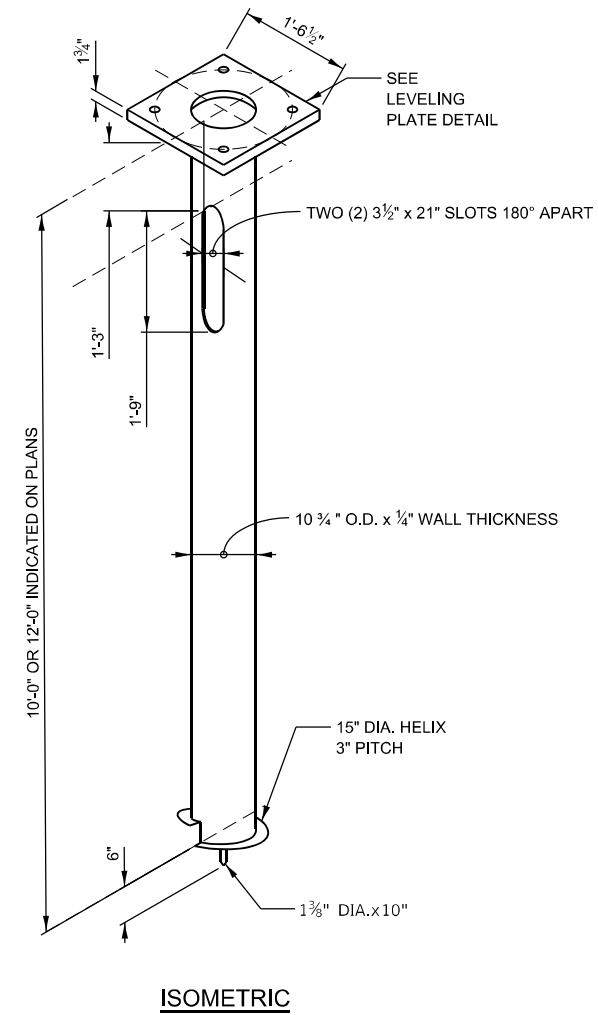
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC DETAILS  
GENERAL NOTES POLE MOUNTED ITS ELEMENT ASSEMBLY

SCALE:	SHEET 13	OF 26	SHEETS	STA.	TO STA.
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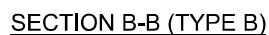
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	531
CONTRACT NO. 64C24				
		ILLINOIS	FED. AID PROJECT	





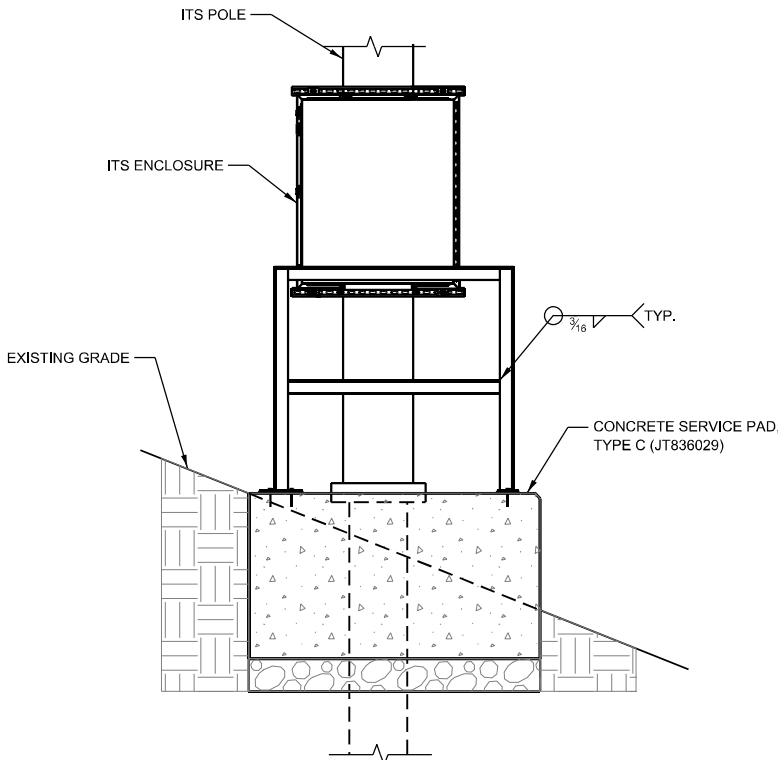


ASTM A615, GRADE 60 DEFORMED, OR	Fy = 60,000 PSI (EPOXY COATED)
WELDED WIRE MESH,	Fy = 60,000 PSI - 6x6 D10

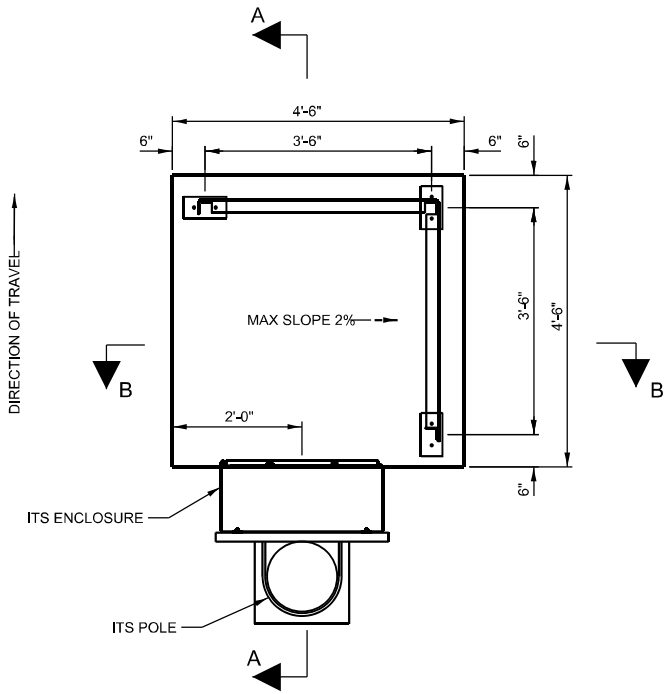


NOTES:

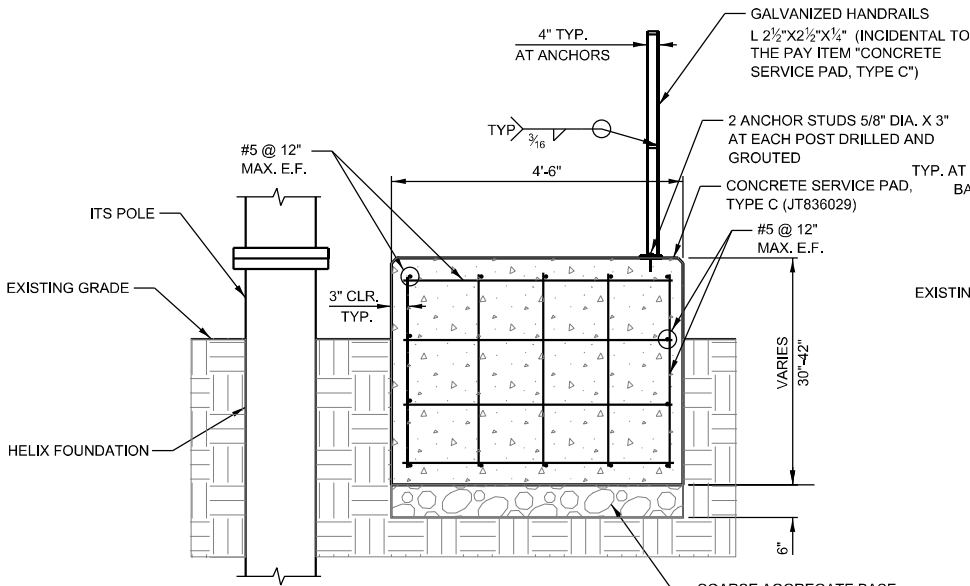
- TYPE A SERVICE PADS SHALL BE INSTALLED ON SLOPES UP TO AND INCLUDING 1:6 (V:H).
- TYPE B SERVICE PADS SHALL BE INSTALLED ON SLOPES GREATER THAN 1:6 AND LESS THAN OR EQUAL TO 1:3 WHEN WELL BEYOND THE CLEAR ZONE.
- TYPE C SERVICE PADS SHALL BE INSTALLED ON SLOPES GREATER THAN 1:3 AS SHOWN ON SHEET M-ITS-1003 SHEET 2 OF 2.
- ALL EXPOSED CONCRETE EDGES SHALL HAVE A 1" MINIMUM CHAMFER.
- CONTRACTOR SHALL TAKE PRECAUTIONS TO STABILIZE EXISTING ITS POLES AND HELIX FOUNDATIONS WHILE EXCAVATING SOIL FOR INSTALLATION OF CONCRETE SERVICE PADS.
- COMPACTED SOIL SHALL BE PLACED TO BE LEVEL WITH THE SERVICE PAD. CONTRACTOR MAY USE EXCAVATED SOIL FROM PLACING THE PAD'S AGGREGATE BASE FOR GRADING PURPOSES WITH APPROVAL OF THE ENGINEER. SEEDING AND EROSION CONTROL SHALL BE PER THE GENERAL NOTES ON SHEET GN-08.
- SOIL EXCAVATED FOR THE PURPOSE OF MAINTAINING A STABLE WORKING SLOPE WHILE INSTALLING THE SERVICE PAD SHALL BE REPLACED. BACKFILL SHALL BE EARTH WHICH IS FREE FROM DEBRIS, CINDERS, AND ROCKS MEASURING 2" OR GREATER IN DIAMETER. IN THE EVENT THAT EXCAVATED MATERIAL IS UNSUITABLE FOR USE AS BACKFILL, THE CONTRACTOR SHALL USE A CLEAN, NATURAL SAND. THIS SUBSTITUTE BACKFILL SHALL BE INCIDENTAL TO THE SERVICE PAD INSTALLATION AND WILL NOT BE PAID FOR SEPARATELY. ALL BACKFILL MATERIALS SHALL BE COMPACTED TO THE SATISFACTION OF THE ENGINEER.
- THE TOP SURFACE OF SOIL DISTURBED BY EXCAVATION FOR PLACING THE SERVICE PADS SHALL BE SEEDED AND PROTECTED WITH EROSION CONTROL MEASURES PER THE GENERAL NOTES ON SHEET GN-08.
- SURFACE OF SERVICE PADS SHALL BE BROOM FINISHED.
- ALL EMBEDDED LIFTING HARDWARE USED SHALL BE GALVANIZED.
- FOR LIFTING INSERT, INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATION INCLUDING MINIMUM EDGE DISTANCE AND SPACING REQUIREMENTS AND HAVE A 4:1 SAFETY FACTOR.
- FOR LIFTING PLATES, INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS AND HAVE A STANDARD 5:1 SAFETY FACTOR FOR THE LIFTING HARDWARE.
- PRECAST SERVICE PADS SHALL BE TRANSPORTED IN SUCH A MANNER THAT THE PAD WILL NOT BE DAMAGED DURING TRANSPORTATION. PLASTIC CORNER PIECES OR SHOCK-ABSORBING CUSHIONING MATERIAL SHALL BE USED AT ALL BEARING POINTS AND ALL EXPOSED CORNERS DURING TRANSPORTATION OF THE PRECAST ELEMENTS. PADS SHALL BE PROPERLY SUPPORTED DURING TRANSPORTATION SUCH THAT CRACKING OR DEFORMATION DOES NOT OCCUR. IF MORE THAN ONE PAD IS TRANSPORTED PER VEHICLE, PROPER SUPPORT AND SEPARATION MUST BE PROVIDED BETWEEN INDIVIDUAL PADS. PADS MUST BE LYING HORIZONTALLY DURING TRANSPORTATION, UNLESS OTHERWISE APPROVED.
- PRECAST ELEMENT DAMAGED DURING HANDLING AND STORAGE SHALL BE REPAIRED OR REPLACED AT NO COST TO THE ILLINOIS TOLLWAY.
- A PRECAST PAD SHALL NOT BE TRANSPORTED FROM THE CASTING YARD UNTIL A MINIMUM 5 DAY COMPRESSIVE STRENGTH SPECIFIED HAS BEEN ATTAINED.
- MATERIAL QUALITY AND CONDITION AFTER SHIPMENT WILL BE INSPECTED AFTER DELIVERY TO THE CONSTRUCTION SITE, WITH THIS AND ANY PREVIOUS INSPECTIONS CONSTITUTING ON PARTIAL ACCEPTANCE.
- REPAIR OF DAMAGE CAUSE TO THE PADS DURING FABRICATION, LIFTING AND HANDLING, OR TRANSPORTATION SHALL BE ADDRESSED ON A CASE-BY-CASE BASIS. DAMAGE WITHIN ACCEPTABLE LIMITS CAUSED TO THE TOP OF THE SURFACES OR TO KEYED EDGES OF THE PADS SHALL BE REPAIRED USING AN APPROVED REPAIR METHOD AT THE FABRICATION PLANT AT THE EXPENSE OF THE CONTRACTOR. REPETITIVE DAMAGE TO THE PADS SHALL BE CAUSE FOR STOPPAGE OF FABRICATION OPERATION UNTIL CAUSE OF DAMAGE CAN BE REMEDIED.
- THE CONTRACTOR SHALL RETAIN THE SOLE RESPONSIBILITY FOR THE MEANS, METHODS, AND TECHNIQUES OF CONSTRUCTION OF THE PADS AND FOR COMPLIANCE WITH LAWS, REGULATIONS, AND CODES, AND FOR THE SAFETY OF CONSTRUCTION APPLICABLE TO THIS WORK.



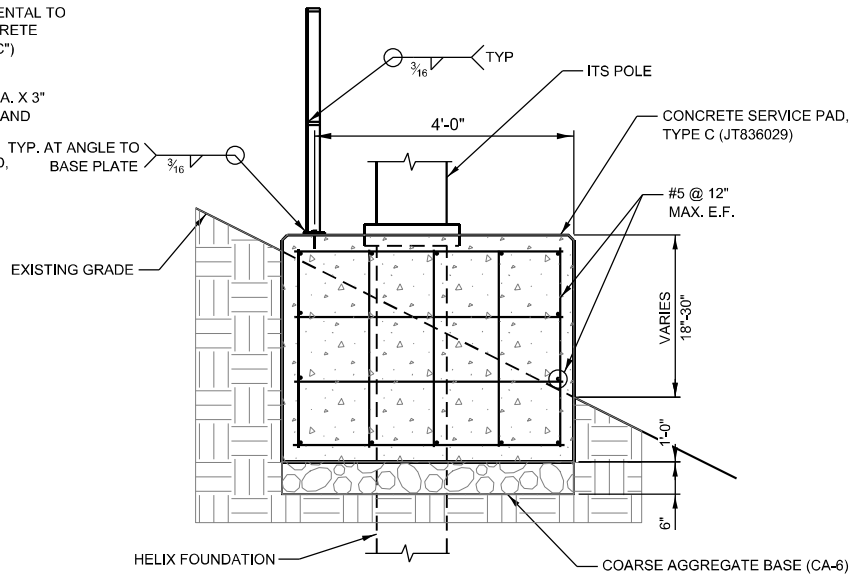
CONCRETE SERVICE PAD, TYPE C  
ELEVATION VIEW



CONCRETE SERVICE PAD, TYPE C  
PLAN VIEW



SECTION A-A



SECTION B-B

DESIGN STRESSES

CONCRETE

CAST-IN-PLACE:  $f_c = 3,500$  PSI AT 14 DAYS (CLASS SI)  
PRE-CAST:  $f_c = 3,500$  PSI AT 5 DAYS,  $f_c = 5,000$  PSI AT 28 DAYS (CLASS PC)

STEEL

ASTM A615, GRADE 60 DEFORMED,  $F_y = 60,000$  PSI (EPOXY COATED) OR  
WELDED WIRE MESH,  $F_y = 60,000$  PSI 6x6 D14

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FIBER OPTIC DETAILS  
ITS CONCRETE SERVICE PAD

SCALE: SHEET 16 OF 26 SHEETS STA. TO STA.

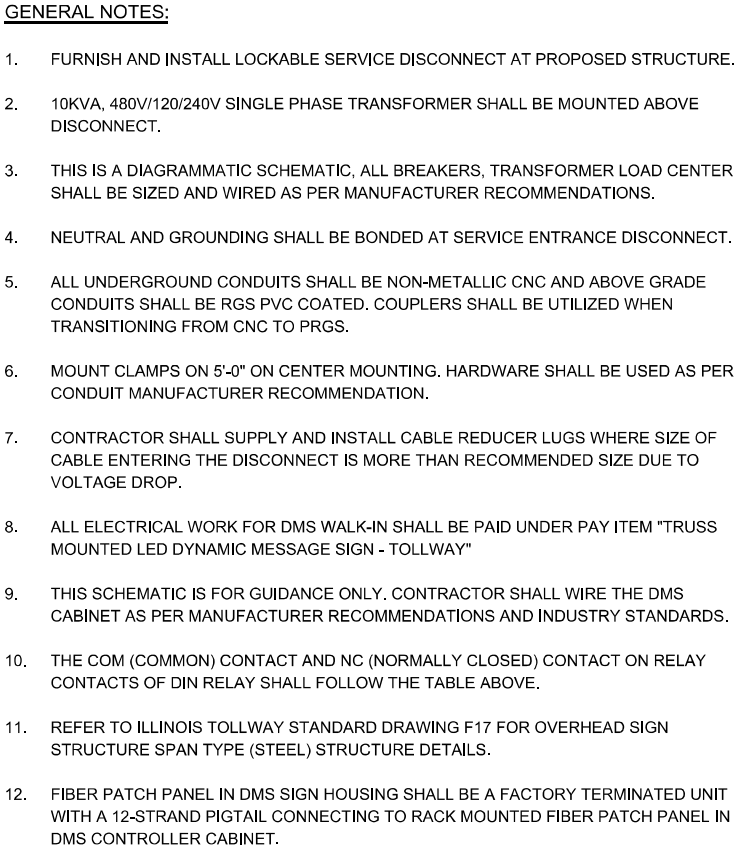
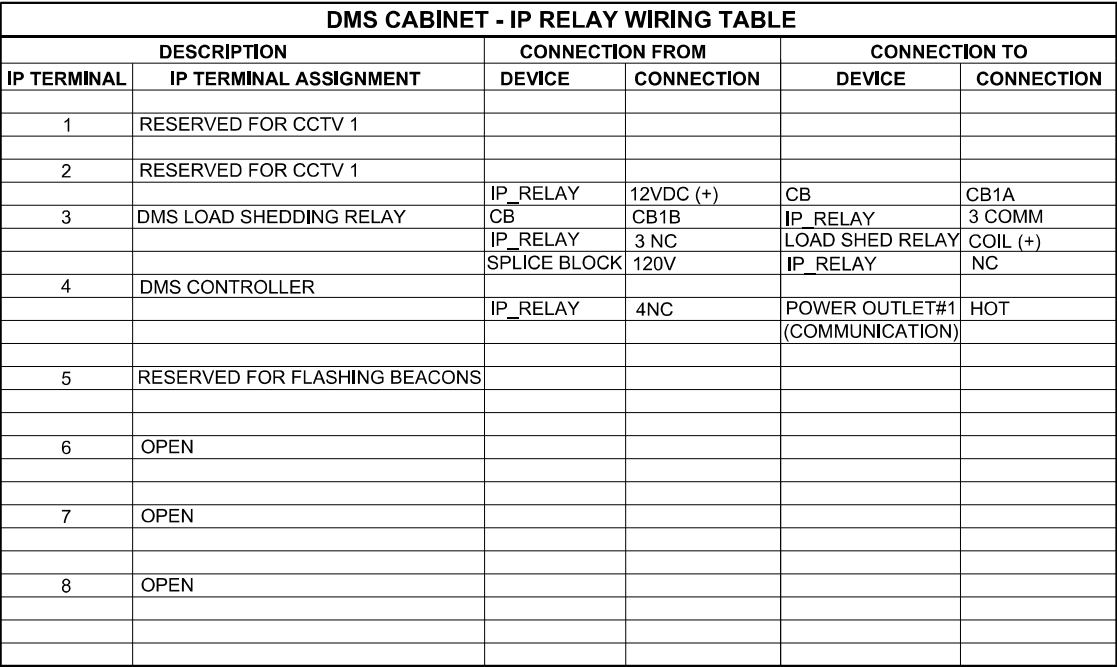
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	534
CONTRACT NO. 64C24				
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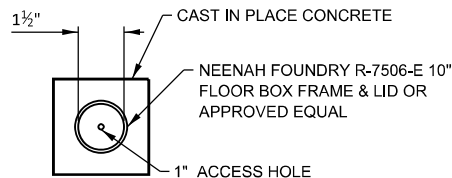
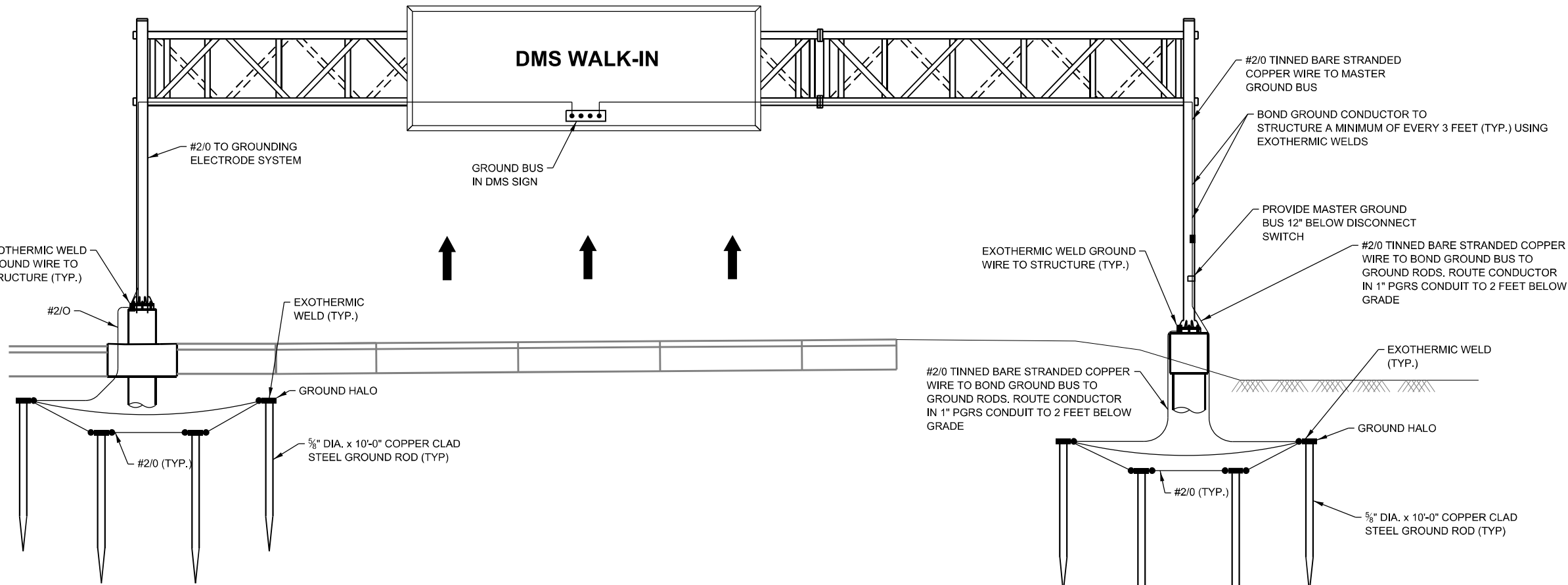
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

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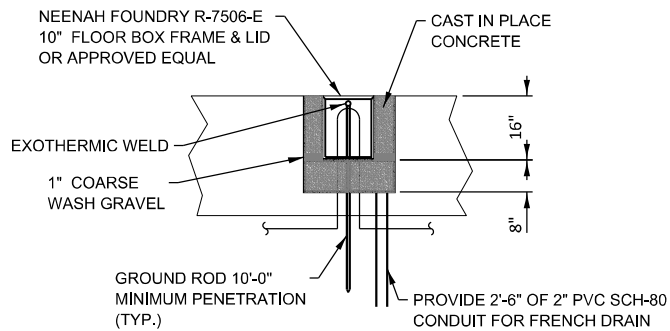


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	DRAWN - R.SCARIA	REVISED -
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PLOT DATE = 8/12/2024	DATE - 02/14/2025	REVISED -

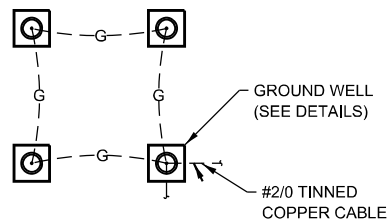




**GROUND WELL PLAN DETAIL**  
(NOT TO SCALE, NOTE 3)



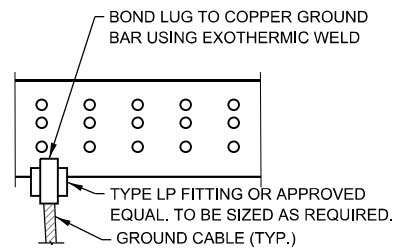
**GROUND WELL ELEVATION DETAIL**  
(NOT TO SCALE, NOTE 3)



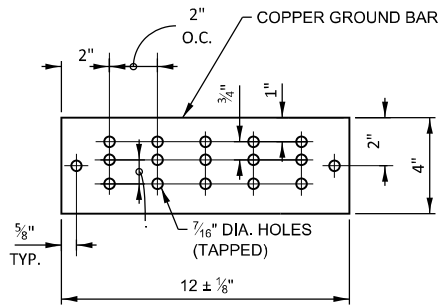
**GROUND HALO DETAIL**  
N.T.S.

**NOTES:**

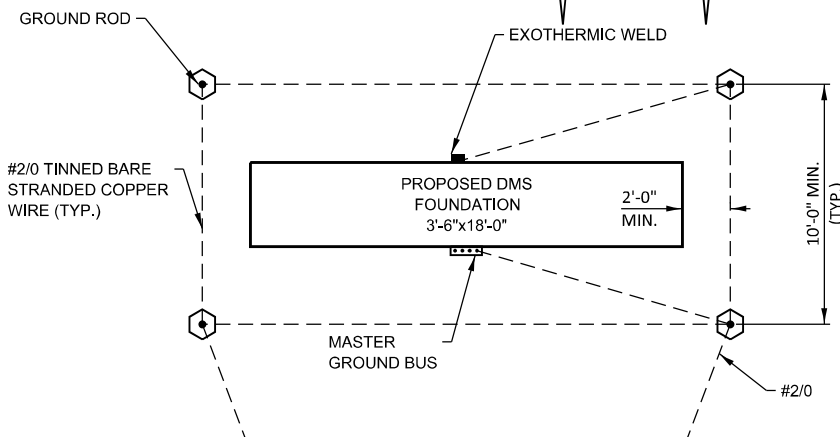
1. GROUND MOUNTED CONTROL CABINET SHALL BE PLACED UPSTREAM OF THE STRUCTURE AT THE LOCATION SHOWN ON THE PLAN VIEWS.
2. INSTALL MARKER TAPE DIRECTLY ABOVE GROUNDING ELECTRODE CONDUCTORS.
3. THE COST OF ALL MATERIALS, ALL GROUND BUSBARS, EXOTHERMIC WELDING GROUND WELL, GROUND RODS AND ALL OTHER ITEMS TO COMPLETE THE GROUNDING ELECTRODE SYSTEM SHALL BE INCLUDED IN THE PAY ITEM ASSOCIATED WITH DYNAMIC MESSAGE SIGN.
4. REFER FIBER OPTIC DETAIL SHEET-17 FOR DMS TYPICAL SITE WIRING.
5. GROUND RODS SHALL BE INSTALLED IN GROUND WELLS IN FINISHED GRADE UNLESS INSTALLED UNDER SHOULDERS OR PAVEMENT.



**MASTER GROUND BUSBAR CONNECTION DETAIL**  
(NOT TO SCALE)



**MASTER GROUND BUSBAR SUPPORT SPACING DETAIL**  
(NOT TO SCALE)



**GROUNDING SCHEMATIC**  
(NOT TO SCALE)

MODEL: M-JTS-101 [Sheet]  
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**FIBER OPTIC DETAILS  
DMS WALK-IN SITE GROUNDING PLAN**

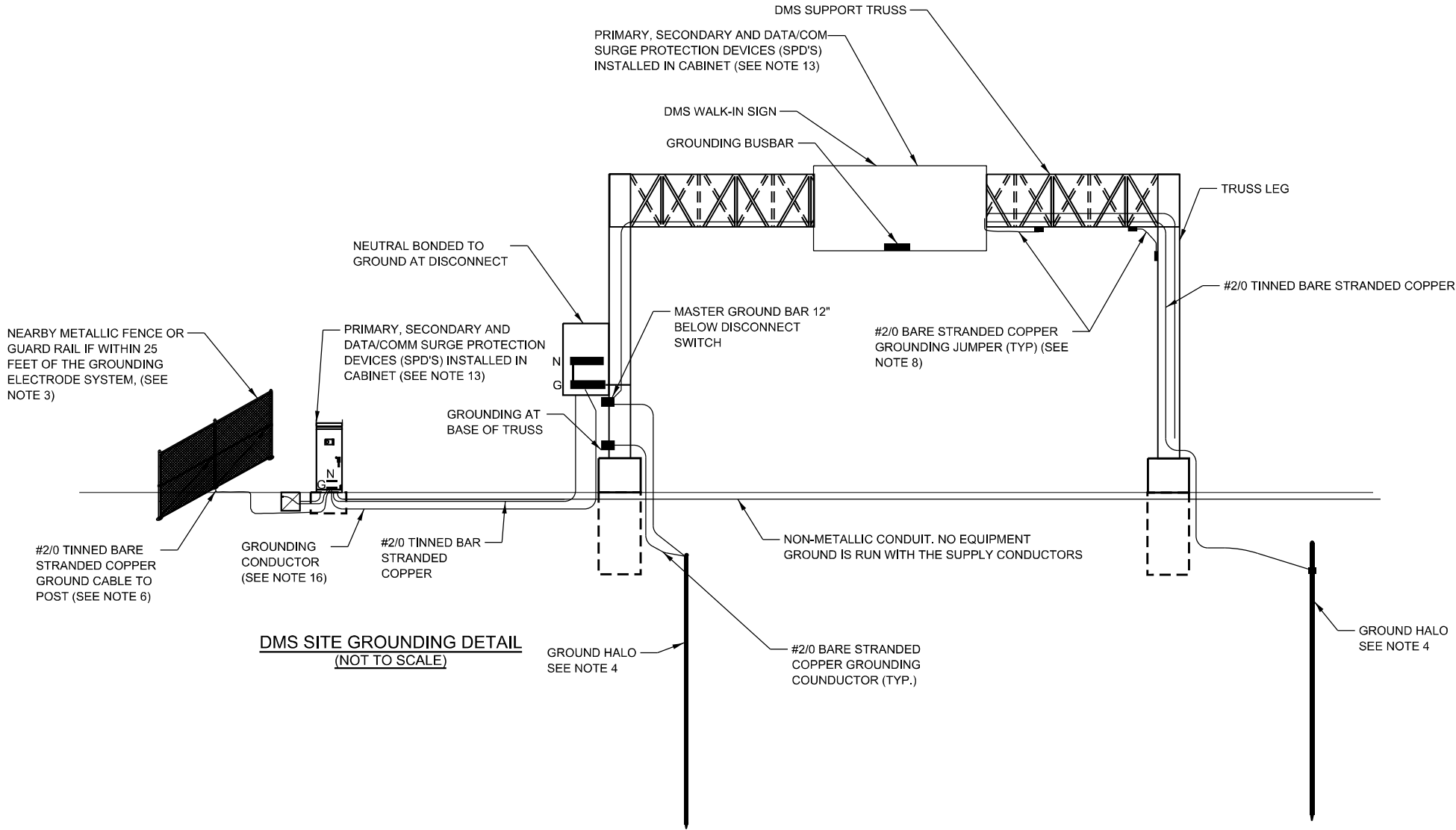
SCALE: SHEET 18 OF 26 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	536
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)



MODEL: M-ITS-1102 [Sheet]  
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**DMS SITE GROUNDING DETAIL**  
(NOT TO SCALE)

1. ADDITIONAL GROUND RODS SHALL BE ADDED TO GROUNDING ELECTRODE CONDUCTOR AS REQUIRED UNTIL RESISTANCE TO GROUND IS 5 OHMS OR LESS. FOR DEVICE AND POWER SERVICE LOCATIONS, IF ADDITIONAL GROUND ROD ELECTRODES ARE REQUIRED INORDER TO ACHEIVE REQUIRED RESISTANCE THEY SHALL RADIATE OUT FROM EXISTING GROUND ROD ELECTRODES. THESE SHALL BE CONNECTED WITH #2/0 TINNED BARE STRANDED CONDUCTORS AND SHALL BE 20' FROM CONNECTED GROUND ROD. ALL COMMUNICATION EQUIPMENT GROUNDING SITES SHALL BE TESTED FOR RESISTANCE TO GROUND USING THE THREE-POINT FALL OF POTENTIAL TEST PER ANSI/IEEE STD 81. ALL REQUIRED GROUNDING SHALL BE INCLUDED IN THE PAY ITEM ASSOCIATED WITH DMS SIGN.
2. GROUND RODS SHALL NOT BE ROUTED THROUGH FOUNDATIONS.
3. FENCES AND OTHER METALLIC STRUCTURES WITH PATHS TO GROUND SHALL BE CONNECTED TO EQUIPMENT GROUND IF THEY ARE LOCATED WITHIN 25' OF THE GROUNDING ELECTRODE SYSTEM OR ANY OBJECT GROUNDED TO THE GROUNDING ELECTRODE SYSTEM.
4. GROUND RODS SHALL BE INSTALLED IN GROUND WELLS IN FINISHED GRADE UNLESS INSTALLED UNDER SHOULDERS OR PAVEMENTS.
5. ALL EQUIPMENT GROUNDS SHALL BE PROPERLY CONNECTED TO A CHASSIS: ALL PAINT AND OTHER COATINGS, INCLUDING GALVANIZATION, SHALL BE REMOVED PRIOR TO TERMINATION OF A GROUND, AFTER THE GROUND IS TERMINATED A NON-OXIDIZING COATING SHALL BE PAINTED OVER THE EXPOSED METAL SURFACES.
6. GROUNDING ELECTRODE SYSTEM CONNECTIONS TO FENCING SHALL BE MADE USING HEAVY DUTY TINNED LISTED PIPE CLAMPS DESIGNED FOR GROUNDING AND STAINLESS STEEL HARDWARE.
7. ALL GROUNDING DIAGRAMS ARE SCHEMATIC ONLY.
8. ALL METALLIC MEMBERS OF THE DMS TRUSS AND THE DMS SIGN WITHIN 6 FEET OF EACH OTHER SHALL BE BONDED TOGETHER. WELDS SHALL BE CONSIDERED AN ACCEPTABLE BONDING METHOD. U-BOLT CONNECTIONS SHALL NOT BE CONSIDERED AN ACCEPTABLE BONDING METHOD.
9. AT LEAST AN 8 INCH MINIMUM BENDING RADIUS SHALL BE MAINTAINED ON ALL GROUNDING ELECTRODE CONDUCTORS. THE ANGLE OF ANY BENDING SHALL NOT BE LESS THAN 90 DEGREE.
10. GROUNDING CONDUCTORS SHALL ALWAYS ROUTE AS STRAIGHT AS POSSIBLE. "U" FORM JUMPERS SHALL BE ACCEPTABLE ONLY FOR GATES AND DOORS.
11. THE QUANTITY OF GROUNDING ELECTRODE CONDUCTORS CONNECTED TO A GROUND ROD ELECTRODE SHALL BE LIMITED TO THREE.
12. WHENEVER POSSIBLE, GROUND ROD ELECTRODES SHALL BE INSTALLED NO CLOSER THAN 11' FROM A FOUNDATION.
13. EVERY COPPER CONDUCTOR OR CABLE ENTERING OR LEAVING A DMS ENCLOSURE, THE DMS CONTROLLER, OR THE CCTV ELECTRONICS ENCLOSURE SHALL BE PROTECTED, WITH A SURGE PROTECTION DEVICE.
14. DIAGRAM OMITTS EQUIPMENT GROUNDING INSIDE ENCLOSURES.
15. GROUNDING CONDUCTOR SHALL BE #2/0 TINNED BARE STRANDED COPPER. CONTRACTOR SHALL INSTALL GROUND RODS AS NECESSARY TO ENSURE GROUND RESISTANCE AT DMS CABINET IS 5 OHMS OR LESS.
16. IF THERE IS A METAL HANDRAIL WITHIN 20 FEET OF CONTROL CABINET CONNECT HANDRAIL TO GROUNDING SYSTEM WITH #2/0 TINNED BARE STRANDED COPPER CONDUCTOR.



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Chicago, Illinois 60601  
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PLOT DATE	= 8/12/2024	DATE	- 02/14/2025	REVISED	-

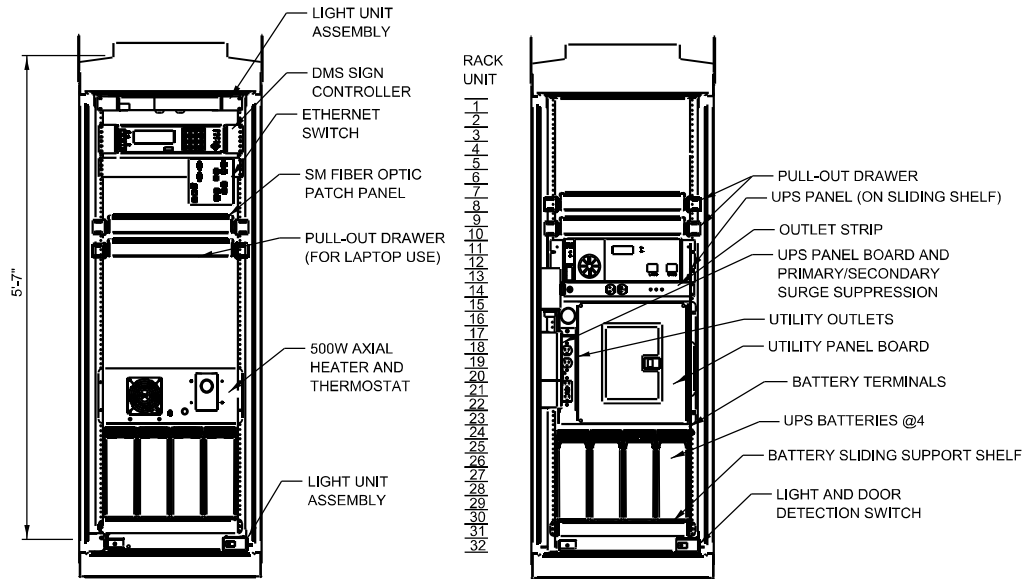
**STATE OF ILLINOIS**  
**DEPARTMENT OF TRANSPORTATION**

**FIBER OPTIC DETAILS**  
**DMS WALK-IN TYPICAL SITE WIRING DETAIL**

SCALE: SHEET 19 OF 26 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	537
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

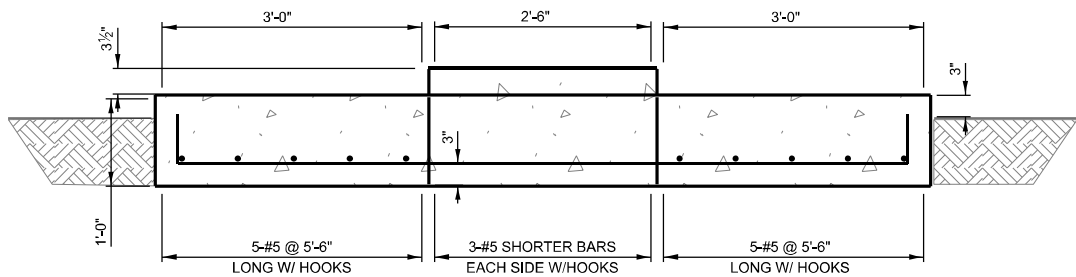


SECTION B-B FRONT VIEW  
(NO DOOR SHOWN)

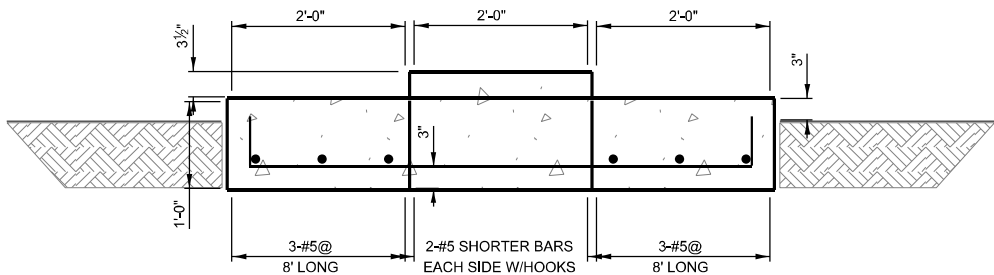
SECTION A-A REAR VIEW  
(NO DOOR SHOWN)

TYPE 334 DMS CABINET LAYOUT DETAILS

NOTE: SPLICE BLOCK, LOAD SHED RELAY, CONTROL POWER TRANSFORMER & DIN IP RELAY MOUNTED ON SIDE WALL ADJACENT UTILITY PANEL BOARD.



SECTION E-E

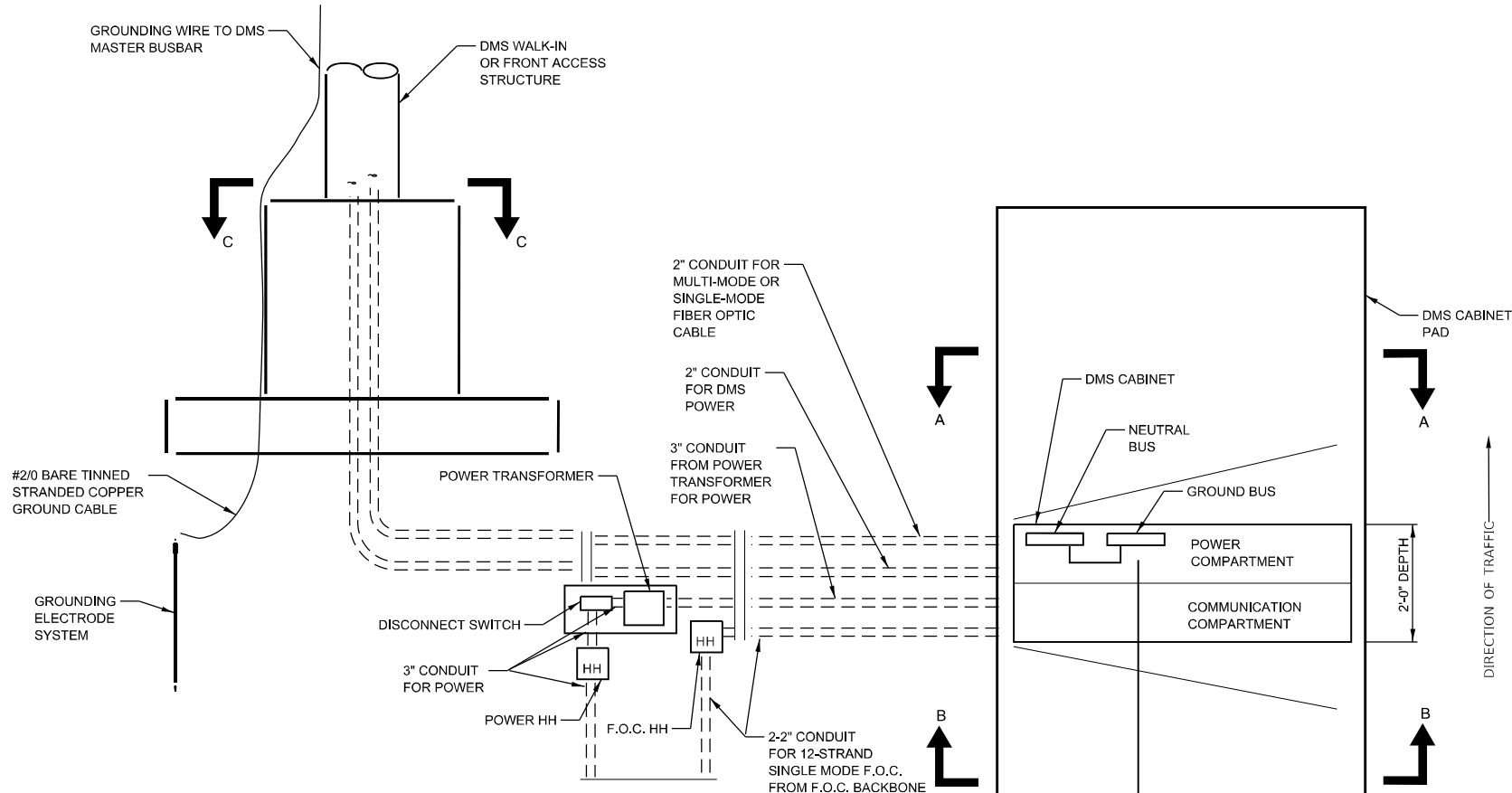


SECTION D-D  
NOT TO SCALE

DMS CONTROLLER FOUNDATION DETAILS

DMS CABINET FOUNDATION NOTES:

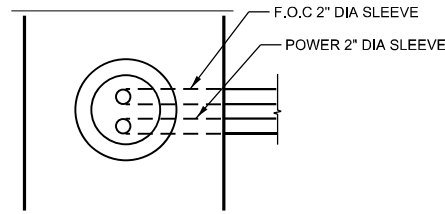
- COORDINATE SIZE OF OPENING WITH DMS CONTROLLER CABINET BOTTOM CONDUIT CUT-OUTS
- CONCRETE = 4,000 PSI (MIN.)
- REBAR EPOXY COATED FY=60,000 PSI (MIN.)
- PROVIDE SHOP DRAWINGS PRIOR TO CONSTRUCTION
- INCLUDE CONDUITS



PLAN/ELEVATION

DMS CABINET NOTES:

- PAD MOUNT CONFIGURATION
- 0.125" ALUMINUM 5052-H34 CONSTRUCTION WITH CONTINUOUSLY WELDED EXTERNAL SEAMS
- THREE POINT LATCH WITH SST HANDLE
- DOUBLE FLANGED DOOR SEAL WITH 1/2" X 2" CLOSED CELL NEOPRENE GASKET WITH CORBIN #2 LOCK ON EACH DOOR.
- FULL LENGTH EIA GAGE FOR 19" EQUIPMENT
- ADJUSTABLE PULL OUT DRAWER
- DOOR OPENING: 21.50" X 54.75"
- FULL LENGTH STAINLESS STEEL HINGE
- ALL STAINLESS STEEL HARDWARE
- CORBIN #2 LOCK
- NEMA 4X ENCLOSURE
- SHIPPED ON WOOD PALLET
- MOUNT LAYER 2 ETHERNET SWITCH (DIN-RAIL MOUNT) USING DIN-RAIL MOUNT
- BATTERIES AND UPS SHALL BE PLACED ON A SLIDING SHELF
- CABINET DIMENSION 24"X30"X67"



SECTION C-C  
POWER AND F.O.C WITHIN DMS FOUNDATION

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DEPARTMENT OF TRANSPORTATION

FIBER OPTIC DETAILS  
DMS CABINET LAYOUT DETAIL

SCALE: SHEET 20 OF 26 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	538
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

MODEL: M-ITS-108 [Sheet]  
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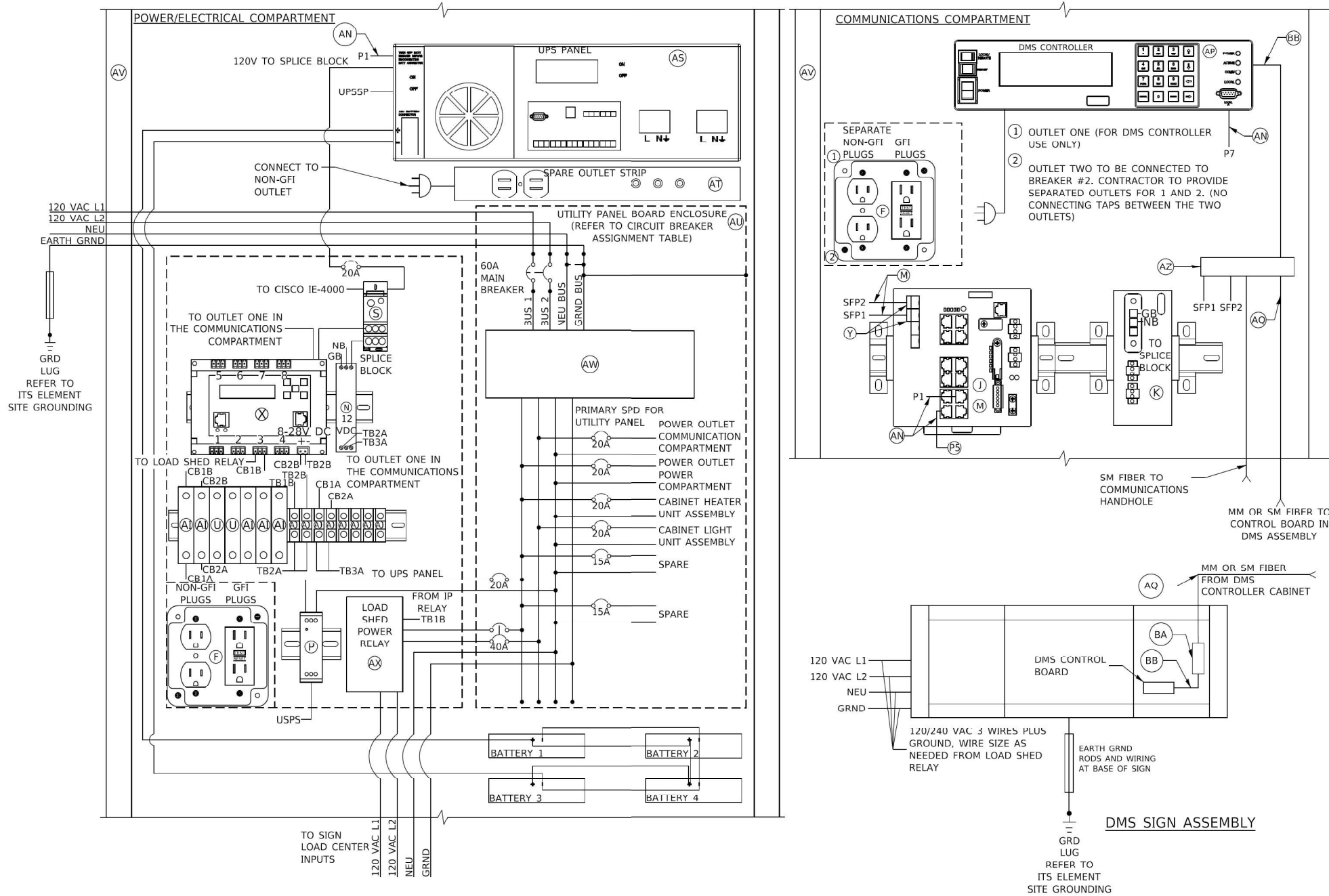
STATE OF ILLINOIS  
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FIBER OPTIC DETAILS  
DMS CABINET WIRING DIAGRAM

SCALE: SHEET 21 OF 26 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	539
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



LEGEND:

ITEM	DESCRIPTION
A-E	NOT USED
F	TWO DUPLEX 120V RECEPTACLES, ONE GFCI (HUBBELL GFR5362TR) AND ONE STANDARD (HUBBELL BR20WR)
G-I	NOT USED
J	NETWORK SWITCH CISCO IE-4000-8T4G-E
K	CISCO POWER SUPPLY, PWR-IE170W-PC-AC=
L	IP SERVICES LICENSE: L-IE4000-RTU=
M	2 METER - SMFO LC-SC DUPLEX JUMPERS, CORNING/047202R5120002M
N	AC/DC POWER SUPPLY, 12VDC, 10 WATTS, MEAN WELL/MDR-10-12
O	SMF PATCH PANEL WITH SC CONNECTORS FIBER CONNECTIONS G620U012 LAN-100-0
P	120VAC SURGE SUPPRESSOR, MOUNTED ON DIN RAIL COOPER CROUSE HINDS/MA15/D/1/SI OR APPROVED EQUAL
Q-R	NOT USED
S	SPLICE BLOCK, ALTECH/38041
T	NOT USED
U	5A CIRCUIT BREAKER, ALLEN BRADLEY/1492-SPM1B050
V-W	NOT USED
X	POWER CONTROLLER, 8-CHANNEL DIN ETHERNET RELAY DIGITAL LOGGERS/DIN 4
Y	(2) GLC-LX-SM-RGD = 1 GBPS SM SFP MODULES
Z	NOT USED
AA-AH	NOT USED
AI	2A CIRCUIT BREAKER, ALLEN BRADLEY/1492-SPM1D020
AJ	TERMINAL BLOCK, ALLEN BRADLEY/1492-CD8
AK-AM	NOT USED
AN	INDOOR/OUTDOOR RATED CAT6 (1000MBS, TEMPERATURE HARDENED) THESE ARE THE CAT6 CABLES ROUTED INSIDE CABINET
AO	NOT USED
AP	DMS CONTROLLER
AQ	12-STRAND MULTI-MODE OR SINGLE-MODE FIBER OPTIC CABLE
AR	NOT USED
AS	UPS PANEL ALPHA TECHNOLOGIES FXM1100 WITH BATTERIES
AT	OUTLET STRIP
AU	DMS MANUFACTURER UTILITY PANEL ENCLOSURE
AV	DMS CONTROL CABINET TYPE 334 NEMA 4X
AW	120/240VAC MTL ZONE DEFENDER MODEL ZD16100
AX	LOAD SHED POWER RELAY MAGNECRAFT MODEL 199X-12 WITH COVER
AZ	RACK MOUNTED FIBER PATCH PANEL
BA	STAND ALONE FIBER PATCH PANEL
BB	2 METER FIBER JUMPER, CORNING (TYPE AND CONNECTION PER DMS MANUFACTURER)

NOTES:

- FABRICATOR TO PROVIDE CABINET DRAWINGS SUBMITTAL FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
- ENTIRE COMPLETED SYSTEM SHALL BE GROUNDED AND BONDED IN ACCORDANCE WITH MOTOROLA R56 MANUAL AND THE APPLICABLE ARTICLES OF SECTION 250 OF THE NATIONAL ELECTRICAL CODE.
- DMS CONTROLLER SHOWN REPRESENTS A GENERIC DMS CONTROLLER. DMS CONTROLLERS ARE SUPPLIED BY THE DMS MANUFACTURER AND THEREFORE THE FRONT PANEL MAY DIFFER.

CIRCUIT BREAKER ASSIGNMENT TABLE  
(SEE UTILITY PANEL BOARD CIRCUIT BREAKER LOCATIONS)

MAIN	CIRCUIT BREAKER DESCRIPTION	AMPS	CIRCUIT BREAKER LOCATION
1	POWER OUTLET POWER COMPARTMENT	20	1
2	CABINET HEATER UNIT ASSEMBLY	20	2
3	POWER OUTLET COMMUNICATION COMPARTMENT	20	3
4	CABINET LIGHT UNIT ASSEMBLY	15	4
5	LOAD SHED RELAY	40	5
6	UPS PANEL	20	6
7	NOT USED	-	7
8	NOT USED	-	8



MODEL: M-ITS-2013 [Sheet]  
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1½" STAINLESS  
STEEL CONDUIT  
FOR FIBER  
(NOTE 13)

GROUND ROD  
CONNECTION USING A  
#2 AWG SOLID TINNED  
COPPER GROUND CABLE  
EXOTHERMICALLY  
WELDED TO THE  
EXTERNAL GROUND  
BUSBAR.

1½" STAINLESS STEEL CONDUIT FOR POWER (NOTE 13)



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Chicago, Illinois 60601  
312-565-4150 Job No. 10900.00

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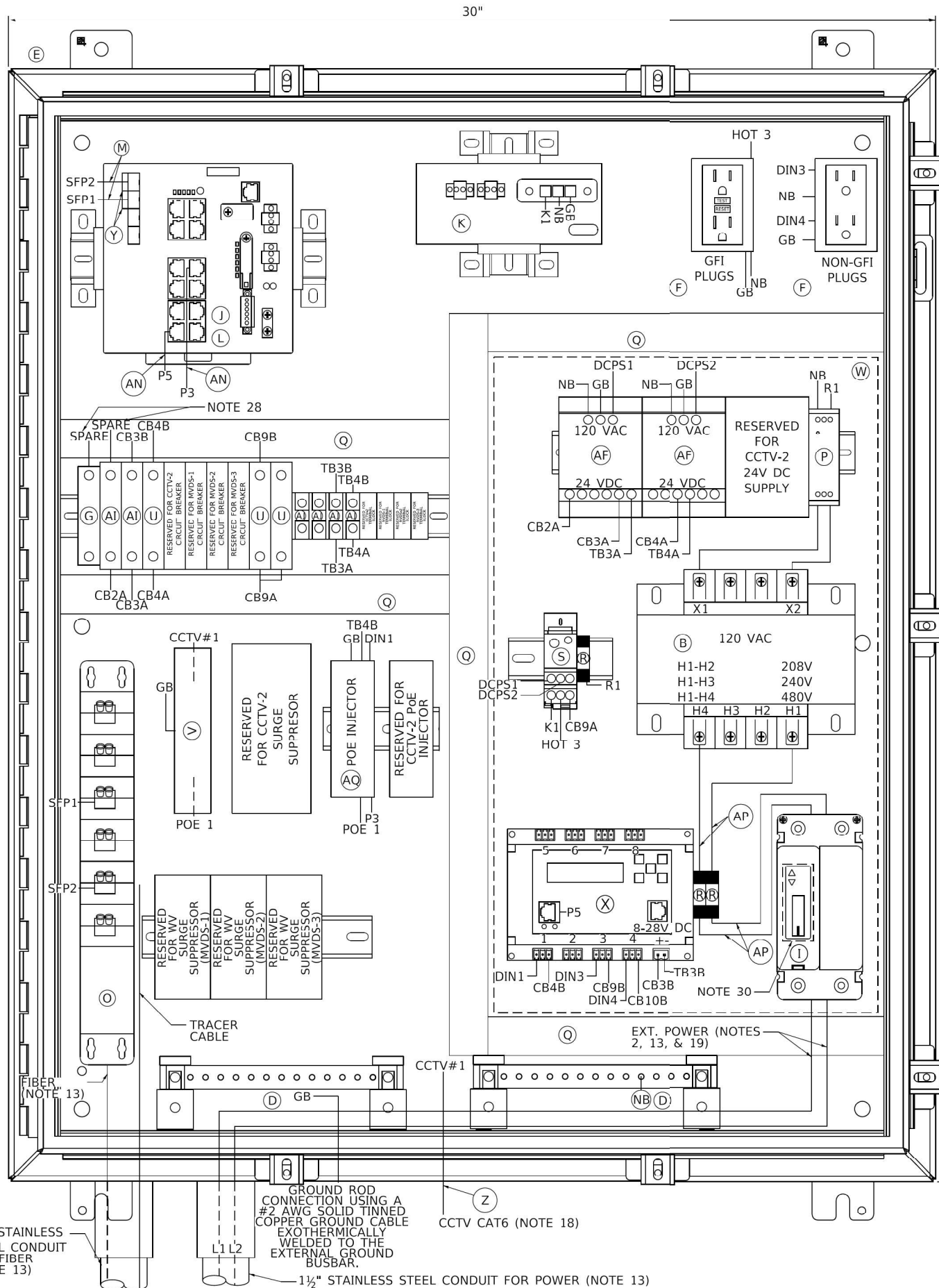
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DEPARTMENT OF TRANSPORTATION

FIBER OPTIC DETAILS  
CABINET LAYOUT AND WIRING ITS POLE MOUNTED ENCLOSURE

SCALE: SHEET 22 OF 26 SHEETS STA. TO STA.

FAI RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	540
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



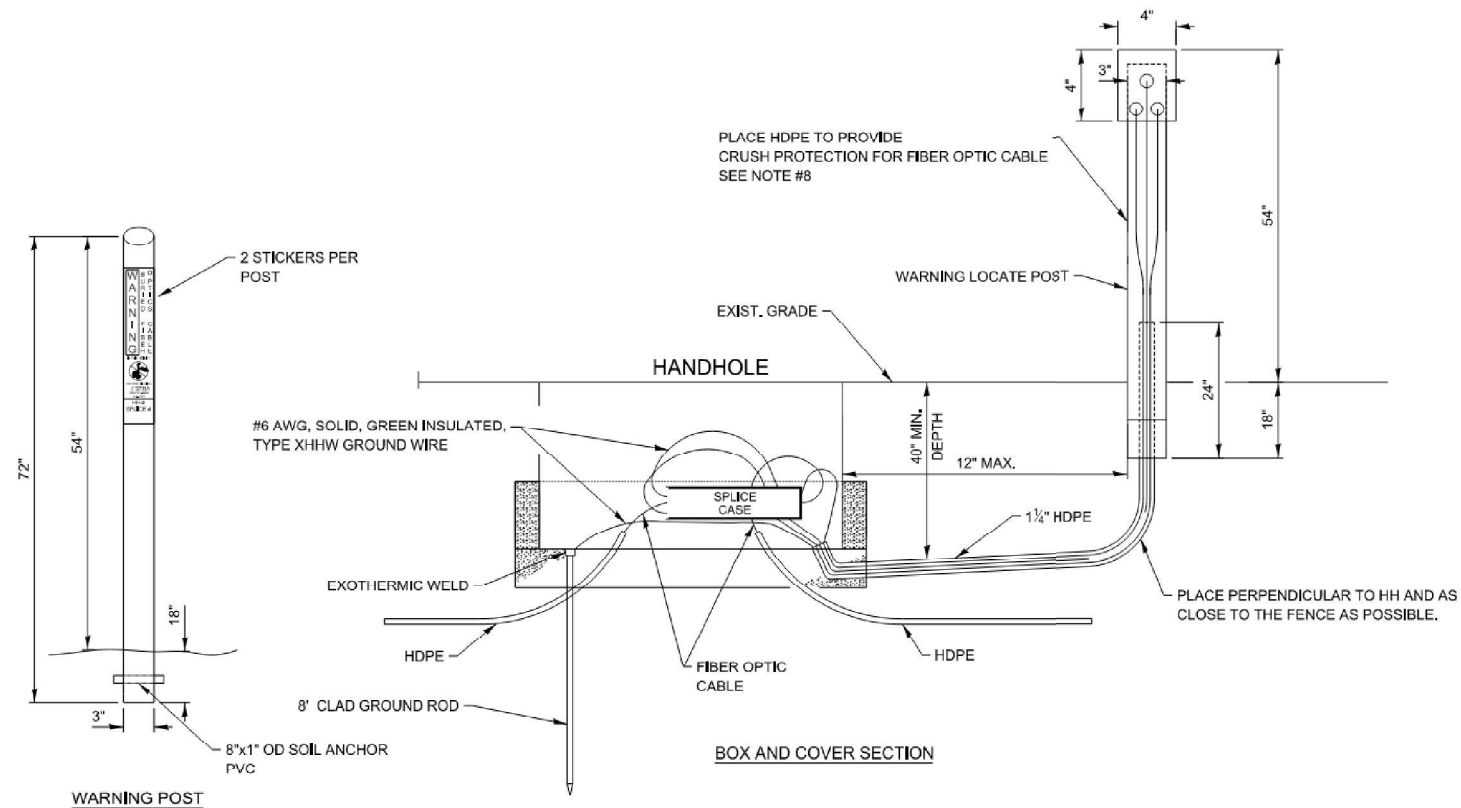
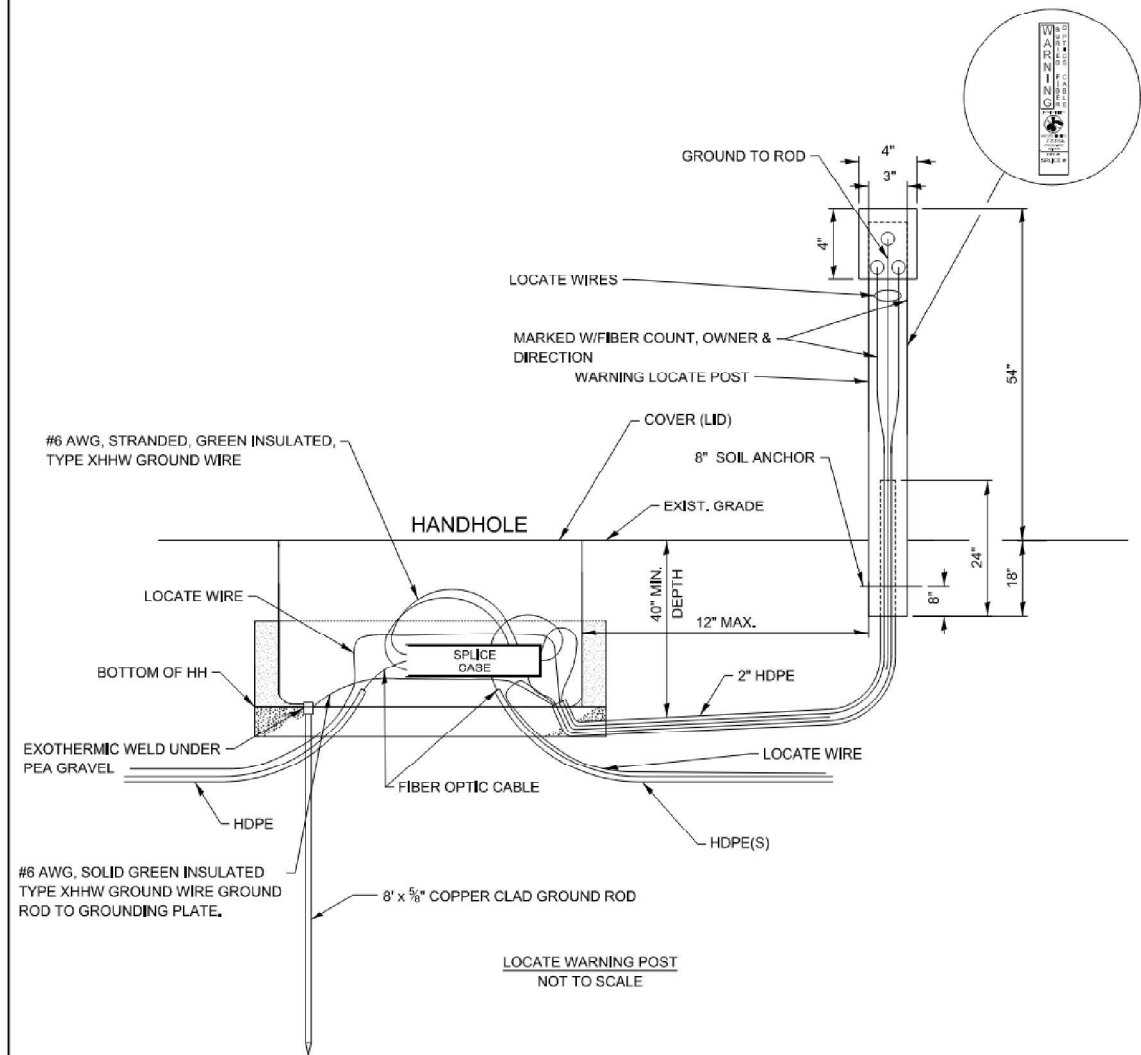
#### ITEM DESCRIPTION

- A NOT USED FOR THIS SHEET APPLICATION
- B CONTROL POWER TRANSFORMER, 1000VA, 208/240/480-120VAC, 1PH SQUARE D/CLASS 9070 - T1000 D95
- C NOT USED FOR THIS SHEET APPLICATION
- D TWO (2) GROUNDING BAR SYSTEM HOFFMAN/PGS2K, BONDED OR SEPARATED AS REQUIRED.
- E NEMA 4X STAINLESS STEEL, 36"H X 30"W X 12"D ENCLOSURE WITH 33"X27" PANEL, HOFFMAN/A36H3012556LP & A36P30
- F TWO DUPLEX 120V RECEPTACLES, ONE GFCI AND NON-GFI (SEE NOTE 9) HUBBELL/GFR5362 & BR20WR
- G 24VDC, 1P, 15A CIRCUIT BREAKER SCHNEIDER ELECTRIC/MGN61510
- H NOT USED FOR THIS SHEET APPLICATION
- I 480V, 2P, 30A CIRCUIT BREAKER WITH TERMINAL SHIELD EATON/HFD2030L & 625B220G07
- J NETWORK SWITCH CISCO IE-4000-8T4G-E
- K CISCO POWER SUPPLY, PWR-IE170W-PC-AC=
- L IP SERVICES LICENSE: L-IE4000-RTU=
- M 2 METER - SMFO LC-LC DUPLEX JUMPERS, CORNING/040402R5Z220002M
- N NOT USED FOR THIS SHEET APPLICATION
- O SMF PATCH PANEL WITH LC CONNECTORS, SEE SPECIAL PROVISIONS
- P 120VAC SURGE SUPPRESSOR, MOUNTED ON DIN RAIL COOPER CROUSE HINDS/MA15/D1/SI OR APPROVED EQUAL
- Q PANDUIT WIRING DUCT (OR EQUIVALENT) PANDUIT/F1X2LG6 WITH COVER-C1LG6
- R 10 AMP FUSE, GOULD (MERSEN)/ATM-10
- S SPLICE BLOCK, ALTECH/38041
- T NOT USED FOR THIS SHEET APPLICATION
- U 5A CIRCUIT BREAKER, ALLEN BRADLEY/1492-SPM1B050
- V CAT6 PoE+ SURGE SUPPRESSOR: USE AXIS T8061 FOR AXIS PoE CAMERA
- W CLEAR POLY METHYL METHACRYLATE (PMMA, PLEXIGLAS) SAFETY COVER ENCOMPASSING ITEMS AF, P, S, R, B, X, & I. (THE INSTALLER SHALL PERMANENTLY AFFIX A LABEL STATING "DANGER 480 VAC" OR "DANGER 240 VAC" OR "DANGER 120 VAC" FOR 120 VAC AS FIELD CONDITIONS WARRANT.)
- X POWER CONTROLLER, 8-CHANNEL DIN ETHERNET RELAY DIGITAL LOGGERS/DIN 4
- Y (2) CISCO GLC LX SM RGD = 1 GBPS SM SFP MODULES
- Z CATEGORY 6 CABLE, 23 AWG, OUTDOOR RATED CABLE BELDEN/7953A
- AA SENSOR SURGE SUPPRESSION, WAVETRONIX - CLICK-200 OR ISS ZONE BARRIER ZB24510
- AB NOT USED FOR THIS SHEET APPLICATION
- AC NOT USED FOR THIS SHEET APPLICATION
- AD NOT USED FOR THIS SHEET APPLICATION
- AE RS-232 / RS-485 TO ETHERNET CONVERTOR WAVETRONIX - CLICK-301 OR ISS-MOXA P5150A-T, DK-035T
- AF AC/DC POWER SUPPLY, 24VDC WAVETRONIX - CLICK-204 OR ISS LAMBDA DSP100-24
- AG NOT USED FOR THIS SHEET APPLICATION
- AH NOT USED FOR THIS SHEET APPLICATION
- AI 2A CIRCUIT BREAKER, ALLEN BRADLEY/1492-SPM1B020
- AJ TERMINAL BLOCK, ALLEN BRADLEY/1492-CD8
- AK MVDS ASSEMBLY (NOT SHOWN), SEE SPECIAL PROVISIONS
- AL TRANSFORMER COVERS, SQUARE D/9070FSC2
- AM 5-CONDUCTOR JUMPER (TX, RX, GND, RTS, CTS), RS-232 SERIAL COMMUNICATIONS (APPLICABLE TO ISS/MOXA)
- AN INDOOR/OUTDOOR RATED CAT6 (1000MBS, TEMPERATURE HARDENED) THESE ARE THE CAT6 CABLES ROUTED INSIDE CABINET
- AO MVDS CABLE, SEE SPECIAL PROVISIONS
- AP #10 AWG
- AQ PoE INJECTOR AXIS T8144 24VDC
- AR T-BUS CONNECTOR (WAVETRONIX)

#### NOTES:

- ALL POWER WIRING SHALL BE RHH/RHW WITH WIRE TERMINALS OR TINNED.
- CONTRACTOR TO VERIFY CORRECT TRANSFORMER TAPS ARE USED BASED ON INCOMING POWER SOURCE.
- ALL CABLES AND EQUIPMENT SHALL BE PROPERLY DRESSED AND LABELED. ALL CONDUITS SHALL BE PROPERLY PLUGGED WITH DUCT SEAL PUTTY (RAINBOW TECHNOLOGIES OR EQUIVALENT).
- NOT USED.
- EACH 120VAC OUTLET, P5 OR TRANSFORMER (ITEM F, K, L, & AF) SHALL BE FED FROM A SEPARATE INPUT LINE.
- THE DIN RAIL(S) FOR ITEMS J & K SHALL BE INSTALLED WITH THE CENTER LINE NO LESS THAN 5 INCHES FROM ANY OBSTACLE ABOVE AND NO LESS THAN 4 INCHES FROM ANY OBSTACLE BELOW. ALL DIN RAIL SHALL BE GROUNDED.
- ALL CABLES INSTALLED WITHIN THE CABINET AND POLE SHALL BE OUTDOOR RATED.
- WIFI COMMUNICATION SHALL BE DISABLED ON DIN ETHERNET RELAY.
- THE GFI OUTLETS LOAD SHALL NOT BE CONNECTED TO ANY OTHER LOAD IN THE ENCLOSURE. THE GFI'S ARE INTENDED TO BE UTILIZED FOR EXTERNAL EQUIPMENT ONLY. EACH OUTLETS TAB SHALL BE BROKEN SO THEY ARE INDEPENDENT.
- ALL BREAKERS SHALL BE LABELED (E.G. CAMERA-AC, CAMERA-DC, DIN RELAY-AC, DIN RELAY-DC, CELL MODEM-AC ETC.).
- NOT USED FOR THIS SHEET APPLICATION.
- USE THE MOUNTING TABS ON THE IP RELAY UNIT TO MOUNT THE UNIT DIRECTLY TO THE BACK PLATE. REFER TO THE IP RELAY WIRING TABLE FOR WIRING DETAILS.
- THE FIBER CABLE SHALL RUN STRAIGHT DOWN FROM THE GATOR PATCH THROUGH THE LEFT MOST CONDUIT. THE POWER CABLE SHALL BE PULLED THROUGH THE CONDUIT TO THE RIGHT OF THE FIBER CONDUIT. NO SLACK SHALL BE PLACED IN THE CABINET. ALL POWER AND COMMUNICATION CABLE SLACK SHALL BE PLACED IN THEIR RESPECTIVE HANDHOLES.
- POWER FEED TO THE CISCO IE4000 SWITCH SHALL BE FROM THE 120VAC INPUT WHEN THE ENCLOSURE IS AC POWERED.
- NOT USED FOR THIS SHEET APPLICATION.
- IF A SOLAR GENERATOR IS CONNECTED, THEN ITEM P AND THE SECONDARY SIDE OF ITEM B SHALL BE CONNECTED UNTIL A FINAL AC CONNECTION IS MADE.
- ITEM X IS USED TO CONTROL POWER TO THE CAMERAS AND DETECTORS. ALL 120VAC CONNECTIONS ON ITEM X SHALL BE PROTECTED.
- CABLES TO BE ROUTED THROUGH POLE.
- WHEN A 24VDC TO 120VAC POWER GENERATOR IS CONNECTED, THEN THE 480VAC TO 120VAC STEP DOWN TRANSFORMER IS BYPASSED.
- NOT USED FOR THIS SHEET APPLICATION.
- NOT USED FOR THIS SHEET APPLICATION.
- DIN RAIL SHALL BE INSTALLED AS ILLUSTRATED ON DRAWING. DIN RAIL SHALL BE GROUNDED TO THE GROUND BUS.
- BOND NEUTRAL AND GROUND BUSES TOGETHER, TIE THE ENCLOSURE INTO THE GROUND BUS.
- ITEM W SHALL BE FORMED AND MOLDED TO FIT AROUND THE AREA DENOTED BY THE DASHED LINE. THE PLEXIGLASS SHALL BE MOUNTED TO THE BACKPLATE WITH SUFFICIENT AIR HOLES TO ALLOW HEAT TO ESCAPE THE AREA. THERE SHALL ALSO BE OPENINGS ON THE BOTTOM TO ALLOW CABLES TO BE PASSED FROM THE AC SECTION TO THE OTHER SECTIONS OF THE ENCLOSURE.
- ITEM AL SHALL BE PLACED ON ITEM B.
- ALL INTERNAL ENCLOSURE ROUTED AND TERMINATED CAT6 CABLE SHALL BE TEMPERATURE RATED.
- ALL INTERNAL 24VAC, 120VAC (STARTING ON SECONDARY SIDE OF ITEM B) AND ANY DC VOLTAGE POWER FEEDS USE #16 AWG CABLE.
- SPARE BREAKER RESERVED.
- ALL CONDUIT EXITING THE BOTTOM OF THE CABINET SHALL BE INSTALL IN-LINE WITH THE EQUIPMENT IT IS CONNECTED TO. THE CABLES SHALL BE INSTALLED IN A NEAT AND PROFESSIONAL MANNER.
- PROVIDE WINDOW IN PMMA SHIELD FOR ACCESS TO BREAKER. MOUNT BREAKER FLUSH WITH PMMA SHIELD USING MOUNTING BRACKET.

FIBER HANDHOLE SITE DETAIL AND GROUNDING



GENERAL NOTES

- LOCATE WARNING POST SHALL BE PLACED 1 FOOT FROM HANDHOLE OR AT FENCE LINE OR RIGHT-OF-WAY LINE IF POSSIBLE.
- AREA AROUND THE HANDHOLE SHALL BE BACKFILLED ONLY TO THE TOP OF THE BOX, FLUSH TO EXISTING GRADE.
- COIL FIBER CABLE IN HANDHOLE ENSURING THAT THE BEND RADIUS SHALL NOT EXCEED VALUES LISTED IN TABLE A ON SHEET 8.
- INSTALL GROUND ROD & EXOTHERMIC WELD AS PER MANUFACTURER'S INSTRUCTIONS, PLACE THE #6 GROUND WIRE (TYPE XHHW, SOLID, GREEN INSULATED) THAT HAS BEEN ATTACHED TO THE GROUND ROD AND TO THE CENTER LUG OF THE LOCATE POST.
- BACKFILL MATERIAL SHALL BE COMPACTED TO THE SATISFACTION OF THE ENGINEER.
- GROUND WIRE SHALL BE BONDED TO BOTH SHEATHS OF ARMORED FIBER OPTIC CABLE IN THE SPLICE ENCLOSURE USING #6 GROUND STRANDED, GREEN INSULATED WIRE. EACH GROUND SHALL BE ISOLATED WITHIN THE ENCLOSURE.
- INSTALL 2" HDPE CONDUIT FROM HANDHOLE TO WARNING POST TO ALLOW GROUNDING CABLE AND LOCATE TRACE WIRES TO BE INSTALLED.
- NO HANDHOLES WILL BE ALLOWED IN PAVED ROADWAYS OR SHOULDERS.
- THE TOPS OF ALL HANDHOLES SHALL BE FLUSH WITH THE EXISTING GRADE.
- A WATER PROOF SEALING SIMPLEX DUCT PLUG SHALL BE INSTALLED AROUND THE FIBER OPTIC TO SEAL AROUND THE CONDUIT. A WATER PROOF SEALING PLUG SHALL BE INSTALLED IN ALL VACANT CONDUIT.
- ANY WORK IN AN EXISTING SINGLE MODE HANDHOLE OR INVOLVING AN EXISTING SINGLE MODE DUCT AND FIBER SHALL BE COORDINATED WITH THE TOLLWAY FIBER OPTIC CONTRACTOR, USING A-36 PROCESS.
- FOR ALL SPLICE AND HANDHOLE, NUMBER DECALS SHALL BE APPLIED AFTER INSTALLATION IS COMPLETED.
- PLACEMENT OF SIGNS IS PREFERRED OVER POSTS, SIGNS SHALL BE USED ON LOCATIONS WHERE FENCE IS VISIBLE FROM ROAD, POSTS SHALL ONLY BE USED WHERE SIGN WOULD NOT BE VISIBLE FROM ROAD.



FIBER OPTIC SYSTEM  
TYPICALS AND DRAWINGS

VERSION: 2024-03 STANDARD: L1-06 SHEET: 9 OF 15

APPROVED BY: *Manar Nashif*  
CHIEF ENGINEERING OFFICER  
DATE: 03/01/2024



USER NAME = Ifranceschina  
PLOT SCALE = \$SCALE\$  
PLOT DATE = 8/12/2024

DESIGNED - R. SCARIA  
DRAWN - R. SCARIA  
CHECKED - G. THIESSE  
DATE - 02/14/2025

REVISED -  
REVISED -  
REVISED -  
REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FIBER OPTIC DETAILS  
FIBER OPTIC SYSTEM TYPICALS AND DRAWINGS

SCALE: SHEET 23 OF 26 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	541
CONTRACT NO. 64C24				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)



MODEL: L1-06B [Sheet]  
FILE NAME: c:\paword\benesch\_projects\projects\0173716\0284C24\_ML-sh-FiberDetails-15.dgn

TYPES OF BURY  
CABLE AND CONDUIT  
BORED, TRENCHED, AND PLOWED

GENERAL NOTES:

1. UNDERGROUND CONDUIT SHALL BE PLACED AT 42" MINIMUM COVER UNLESS OTHERWISE SPECIFIED ON THE PLANS.

2. UNDERGROUND CONDUIT SHALL BE PLACED AT 48" MINIMUM COVER UNDER STREAM, CREEK AND DRAINAGE DITCHES UNLESS OTHERWISE SPECIFIED ON THE PLANS.

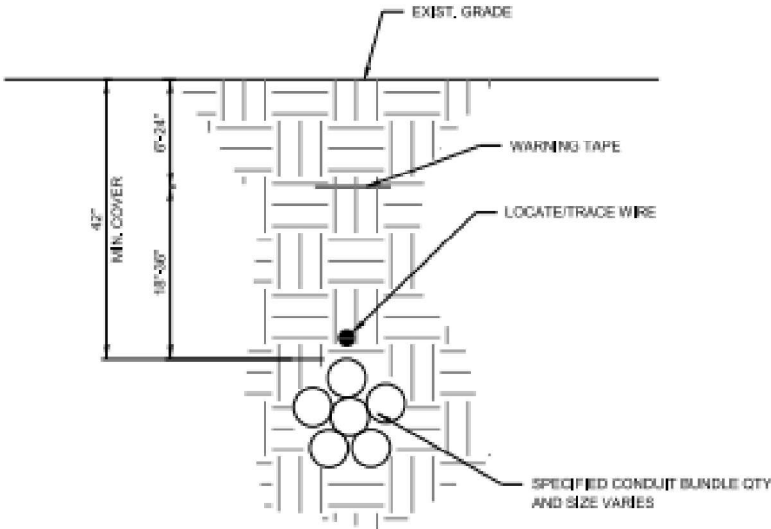
3. IF WHILE LOWERING THE CONDUIT THERE IS NOT ENOUGH SLACK, ADDITIONAL CONDUIT SHALL BE ADDED. EMPTY CONDUITS CAN BE CUT AND HAVE NEW CONDUIT FUSED OR COMPRESSION COUPLED ON. CONDUITS WITH FIBER INSTALLED SHALL BE RING CUT WITH A TUBE CUTTER SO AS NOT TO DAMAGE THE FIBER.

4. CONDUIT USED ABOVE GROUND SHALL BE STAINLESS STEEL OR FIBERGLASS REINFORCED EPOXY (FRE) CONDUIT. UNDERGROUND CASINGS SHALL BE FRE PER THE SPECIAL PROVISIONS OR HDPE.
5. LOCATE/TRACE WIRE SHALL BE DIRECT BURIED WITH EVERY CONDUIT BUNDLE PATH AS CLOSE TO THE CENTER OF THE CONDUITS AS POSSIBLE. LOCATE/TRACE WIRE SHALL NOT BE INSTALLED IN A CONDUIT WITHOUT APPROVAL OF THE ENGINEER.

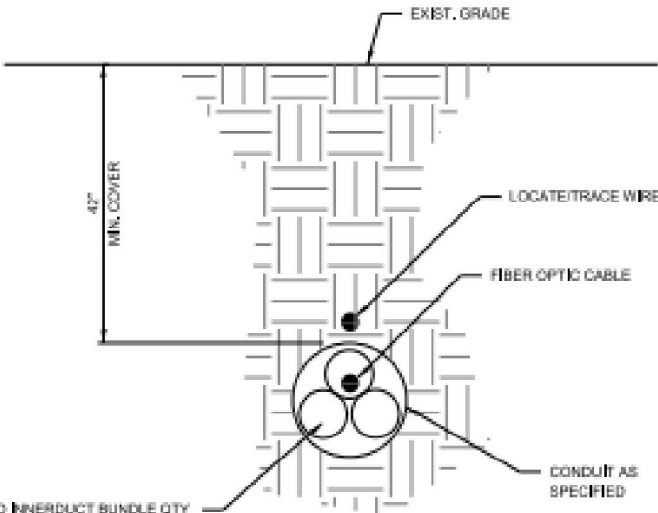
6. WHEN AN OPTIC FIBER CONDUIT SEPARATES FROM A CONDUIT BUNDLE OR DUCT BANK, AN ADDITIONAL LOCATE WIRE SHALL BE INSTALLED WITH THAT SEPARATE CONDUIT PATH GOING BACK TO THE PREVIOUS HANDHOLE.

7. ALL LOCATE/TRACE WIRE WILL BE TESTED PER SPECIFICATIONS PRIOR TO ANY FIBER BEING INSTALLED.

8. ALL UNUSED CONDUIT SHALL HAVE 1200 LB MULE TAPE INSTALLED FOR FUTURE USE.

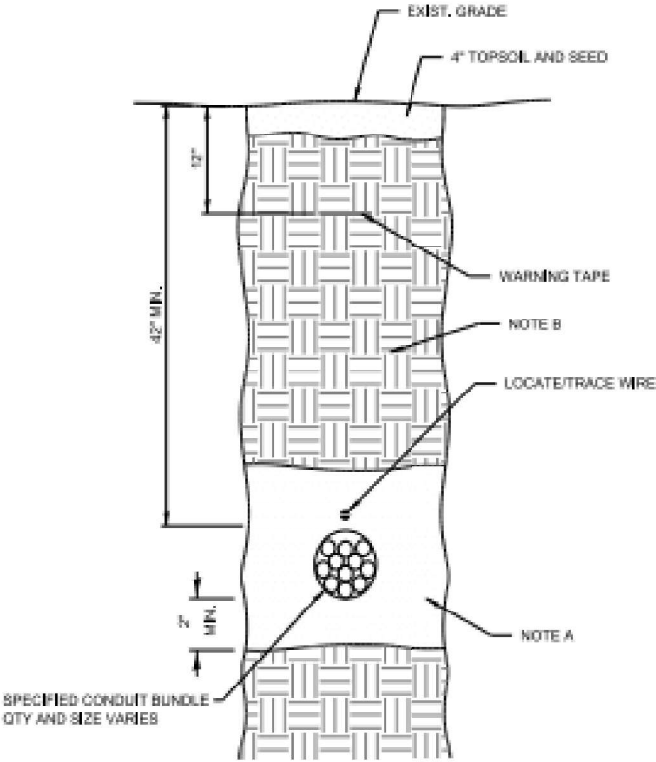


PLOWED CONDUIT BUNDLES  
QTY VARIES



BORED CONDUIT WITH FIBER OPTIC CABLE AND/OR MULTIPLE INNERDUCTS  
AS REQUIRED

NOTE:  
THE PICTURE ABOVE IS A CONCEPT LAYOUT.



CONSTRUCTION NOTES TRENCHED CONDUIT BUNDLES

- A. A MINIMUM OF 2" OF SAND SHALL BE PLACED UNDER THE CONDUIT. SAND SHALL TRANSITION TO BACKFILL ACCORDING TO NOTE B 4" ABOVE CONDUIT.
- B. BACKFILL SHALL BE ACCORDING TO ARTICLE 810.04 OF THE STANDARD SPECIFICATIONS.

TRENCHED CONDUIT BUNDLES

APPROVED BY:  
*Manar Nashif*  
CHIEF ENGINEERING OFFICER  
03/01/2024

REVISIONS	
DATE	DESCRIPTION
03-01-2024	MODIFIED PROFILE VIEW TO INCLUDE TRACE WIRE, MODIFIED SIDE VIEW TO INCLUDE A REFERENCE TO NOTE 2, REMOVED AND MODIFIED NOTES, UPDATED THE GENERAL NOTES SECTION, MODIFIED DETAILS.



FIBER OPTIC SYSTEM  
TYPICALS AND DRAWINGS

REVISION: 2024-03  
DRAWING: L1-06  
SHEET: 1 of 15



USER NAME = ifranceschina	DESIGNED - R. SCARIA	REVISED -
	DRAWN - R. SCARIA	REVISED -
PLOT SCALE = \$SCALE\$	CHECKED - G. THIESSE	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/14/2025	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

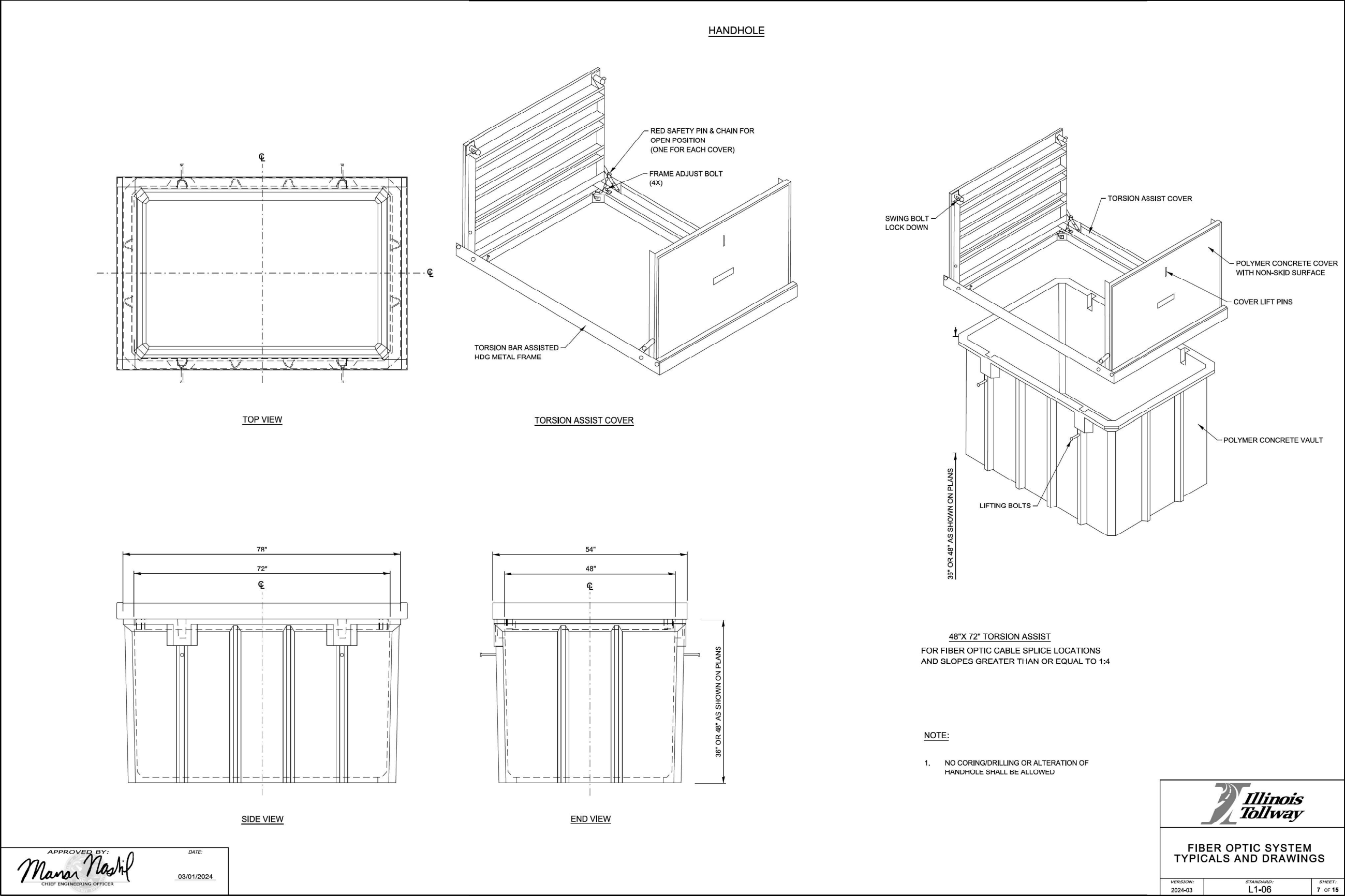
FIBER OPTIC DETAILS  
FIBER OPTIC SYSTEM TYPICALS AND DRAWINGS

SCALE: SHEET 24 OF 26 SHEETS STA. TO STA.

FAI RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	542
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

MODEL: COMMUNICATIONS VAULT COMPOSITE CONCRETE (Sheet)  
FILE NAME: c:\pwworking\benesch\_projects\projects\0173716\0264C24\_ML-shr-FiberDetails-16.dgn



USER NAME = Ifranceschina  
PLOT SCALE = \$SCALE\$  
PLOT DATE = 8/12/2024

DESIGNED - R. SCARIA  
DRAWN - R. SCARIA  
CHECKED - G. THIESSE  
DATE - 02/14/2025

REVISED -  
REVISED -  
REVISED -  
REVISED -

**STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION**

**FIBER OPTIC DETAILS  
COMMUNICATIONS VAULT, COMPOSITE CONCRETE**

SCALE: SHEET 25 OF 26 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	543
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

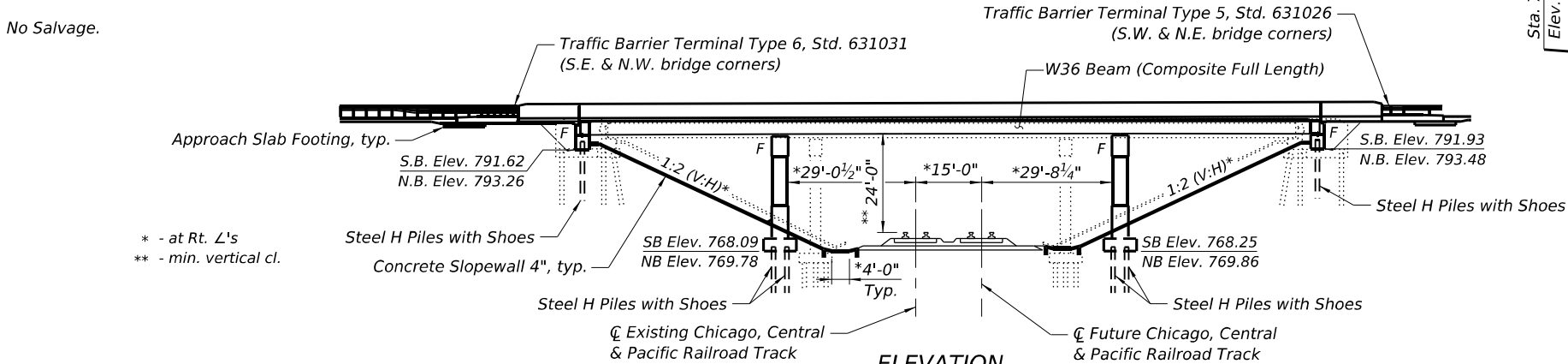
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

Bench Mark: B.M. 400-Cut "□" on southeast corner of S.N. 101-0067. Elevation = 800.52

Existing Structure: S.N. 101-0067 (N.B.) and S.N. 101-0068 (S.B.) built as F.A. Route 194, Section 4 VB at Station 796+28.52 in 1963. Bridges widened and deck replaced in 1991 as F.A.I. Route 39, Section 4 VBY. Each superstructure consists of 3 spans of continuous composite rolled steel beams. Each substructure consists of concrete stub abutments and concrete hammerhead piers supported on piles. Length = 169'-8" (back-to-back of abutments). Width of Eastbound Bridge = 42'-3" (out-to-out deck). Width of Westbound Bridge = 43'-2" (out-to-out deck).

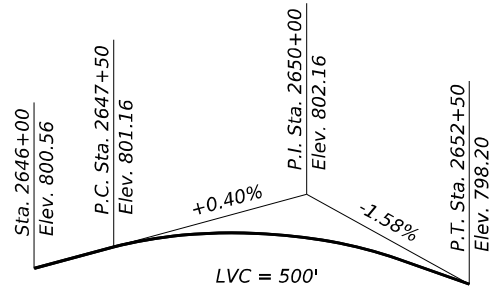
Traffic is to be maintained utilizing stage construction.

No Salvage.



Sta. 7+37.75	Elev. 772.45
Sta. 8+37.75	Elev. 771.95
Sta. 9+37.75	Elev. 771.49
Sta. 9+97.63	Elev. 771.14
Sta. 10+57.35	Elev. 770.93
Sta. 11+57.35	Elev. 770.49
Sta. 12+57.35	Elev. 770.12

TOP OF RAIL ELEVATIONS  
(Chicago, Central & Pacific Railroad)



PROFILE GRADE I-39/US 20

(Along the outer edge of the interior shoulders)  
(Up to 1/4" to be ground off the bridge deck and the bridge approach slabs.  
The Profile Grade shows the final grade after grinding.)

LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design  
Specifications, 9th Edition

DESIGN STRESSES

FIELD UNITS

f'c = 4,000 psi (Superstructure)

f'c = 3,500 psi (Substructure)

fy = 60,000 psi (Reinforcement)

fy = 50,000 psi (M270 Grade 50)\*\*\*

\*\*\* Structural steel to be galvanized.

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1

Design Spectral Acceleration at 1.0 sec. (SD1) = 0.056g

Design Spectral Acceleration at 0.2 sec. (SDS) = 0.101g

Soil Site Class = C

LEGEND

- FO — Exist. Fiber Optic Line
- FO — Prop. Fiber Optic Line
- — — — — Exist. Railroad
- — — — — Prop. Railroad
- ◆ Soil Boring
- ⊗ Prop. Light Pole

APPROVED

For Structural Adequacy Only

Engineer of Bridges & Structures

081-5894

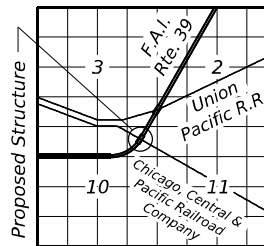
ARTHUR J. NOWAK

CHICAGO, ILLINOIS

STRUCTURAL ENGINEER

05/16/2025

Range 2E, 3rd P.M.



LOCATION SKETCH

GENERAL PLAN & ELEVATION

I-39/U.S. ROUTE 20 OVER CHICAGO, CENTRAL

& PACIFIC RAILROAD COMPANY

F.A.I. RTE. 39 - SECTION (4-1,5)R

WINNEBAGO COUNTY

STA. 2648+18.87

STRUCTURE NUMBER 101-0208 (NB)

STRUCTURE NUMBER 101-0209 (SB)

- Notes:
1. All transverse dimensions on the plan are radial dimensions.
  2. Elevations shown in Plan are elevations after grinding.
  3. Existing culverts to be removed and relocated, see Rdwy Plans.
  4. See Sheet 3 of 60 for Section A-A.

PLAN

Note A: Limits of temporary sheet piling

Bowman

10 S. LaSalle Street, Suite 210  
Chicago, Illinois 60603  
(312) 464-0300  
www.bowman.com

USER NAME = Ifranceschina

DESIGNED - JW

REVISED -

PLOT SCALE = \$SCALES\$

DRAWN - AT

REVISED -

PLOT DATE = 8/12/2024

CHECKED - AJN

REVISED -

DATE = 02/27/24

REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

SCALE: SHEET 1 OF 60 SHEETS STA. TO STA.

F.A.I. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO.

\* (201-3)R & (4-1,5)R WINNEBAGO 1685 544

CONTRACT NO. 64C24

ILLINOIS FED. AID PROJECT

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

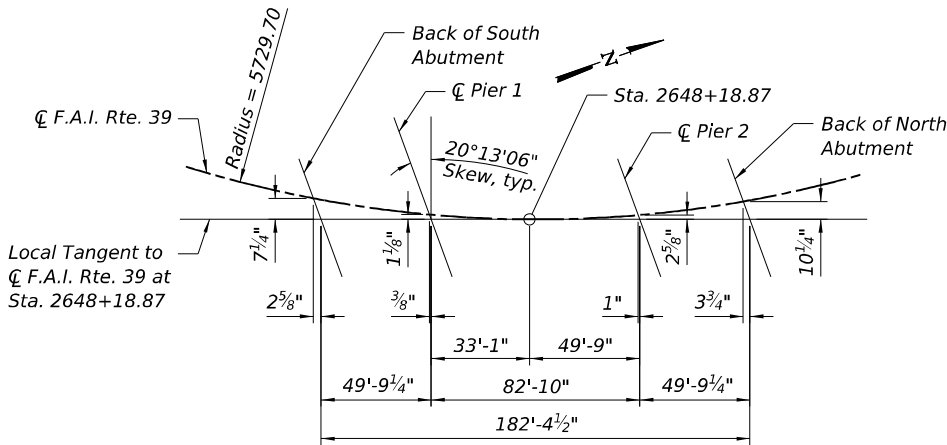


1. *These plans are for erection of the bridges. All work related to the Beam and Bearing Fabrication Contract (64U51) is for information only.*
2. *Fasteners shall be ASTM F 3125 Grade A325 Type 1. Fasteners shall be hot dip galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel." Bolts 7/8 in. diameter, holes 15/16 in. diameter, unless otherwise noted.*
3. *Calculated weight of Structural Steel (Grade 50)= 751,120 lb and Structural Steel (Grade 36)= 89,100 lb.*
4. *FOR INFORMATION ONLY - All structural steel shall be galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel".*
5. *No field welding is permitted except as specified in the contract documents.*
6. *Reinforcement bars designated (E) shall be epoxy coated.*
7. *If the Contractor elects to use cantilever forming brackets on the exterior beams or girders, the brackets shall be placed at the same locations as required for the hardwood blocks in Article 503.06(b) of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and first interior beam at each of these additional bracket locations.*
8. *Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8 in. (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.*
9. *The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.*
10. *The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to address the presence of lead on this project.*
11. *It shall be the Contractor's responsibility to verify the location of all utilities prior to starting construction.*
12. *It is anticipated that the structural steel and bearings will be fabricated by June 1, 2026 for Stage 1, June 1, 2027 for Stage 2, and June 1, 2028 for Stage 3. The delivery dates shall be coordinated with IDOT and the Contractor responsible for Contract No. 64U51.*

ITEM	UNIT	101-0208/0209		
		SUPER	SUB	TOTAL
REMOVAL OF EXISTING STRUCTURES NO. 1	EACH	0.5	0.5	1
REMOVAL OF EXISTING STRUCTURES NO. 2	EACH	0.5	0.5	1
SLOPE WALL REMOVAL	SQ YD	-	1,676	1,676
PROTECTIVE SHIELD	SQ YD	580	-	580
STRUCTURE EXCAVATION	CU YD	0	971	971
CONCRETE STRUCTURES	CU YD	-	1,208.9	1,208.9
CONCRETE SUPERSTRUCTURE	CU YD	967.0	-	967.0
PROTECTIVE COAT	SQ YD	4,476	-	4,476
CONCRETE SUPERSTRUCTURE (APPROACH SLAB)	CU YD	410.9	-	410.9
ERECTING STRUCTURAL STEEL	L SUM	0.22	-	0.22
STUD SHEAR CONNECTORS	EACH	21,240	-	21,240
REINFORCEMENT BARS, EPOXY COATED	POUND	433,040	135,780	568,820
BAR SPLICERS	EACH	1,762	276	2,038
SLOPE WALL 4 INCH	SQ YD	-	2,780	2,780
FURNISHING STEEL PILES HP12X63	FOOT	-	5,208	5,208
DRIVING PILES	FOOT	-	5,208	5,208
TEST PILE STEEL HP12X63	EACH	-	2	2
PILE SHOES	EACH	-	166	166
NAME PLATES	EACH	2	-	2
PREFORMED JOINT SEAL 2 1/2"	FOOT	241	-	241
ANCHOR BOLTS, 1"	EACH	80	-	80
ANCHOR BOLTS, 1¼"	EACH	80	-	80
TEMPORARY SHEET PILING	SQ FT	-	1,284	1,284
GRANULAR BACKFILL FOR STRUCTURES	CU YD	-	512	512
GEOCOMPOSITE WALL DRAIN	SQ YD	-	207.6	207.6
PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	-	354.8	354.8
BRIDGE DECK GROOVING (LONGITUDINAL)	SQ YD	2,564	-	2,564
BAR TERMINATORS	EACH	956	1,748	2,704
DIAMOND GRINDING (BRIDGE SECTION)	SQ YD	3,848	-	3,848

1. General Plan & Elevation
2. General Notes & Total Bill of Material
3. General Details
4. Foundation Layout
5. Temporary Concrete Barrier
6. Stage 1 Construction & Removal
7. Stage 2 Construction & Removal
8. Stage 3 Construction & Removal
9. Removal Plan and Elevation
0. Existing Southbound Abutments Removal
1. Existing Northbound Abutments Removal
2. Existing Southbound Piers Removal
3. Existing Northbound Piers Removal
4. Slab Elevations 1
5. Slab Elevations 2
6. Slab Elevations 3
7. Slab Elevations 4
8. Slab Elevations 5
9. Slab Elevations 6
0. Slab Elevations 7
1. Approach Slab Elevations - Southbound
2. Approach Slab Elevations - Northbound
3. Deck Slab - Southbound Structure
4. Deck Slab - Northbound Structure
5. Deck Slab Details - Miscellaneous & Bill of Material
6. Deck Slab Details - Southbound Parapets
7. Deck Slab Details - Northbound Parapets
8. Deck Slab Details - Parapet Light Pole
9. Concrete End Diaphragm - Southbound
0. Concrete End Diaphragm - Northbound

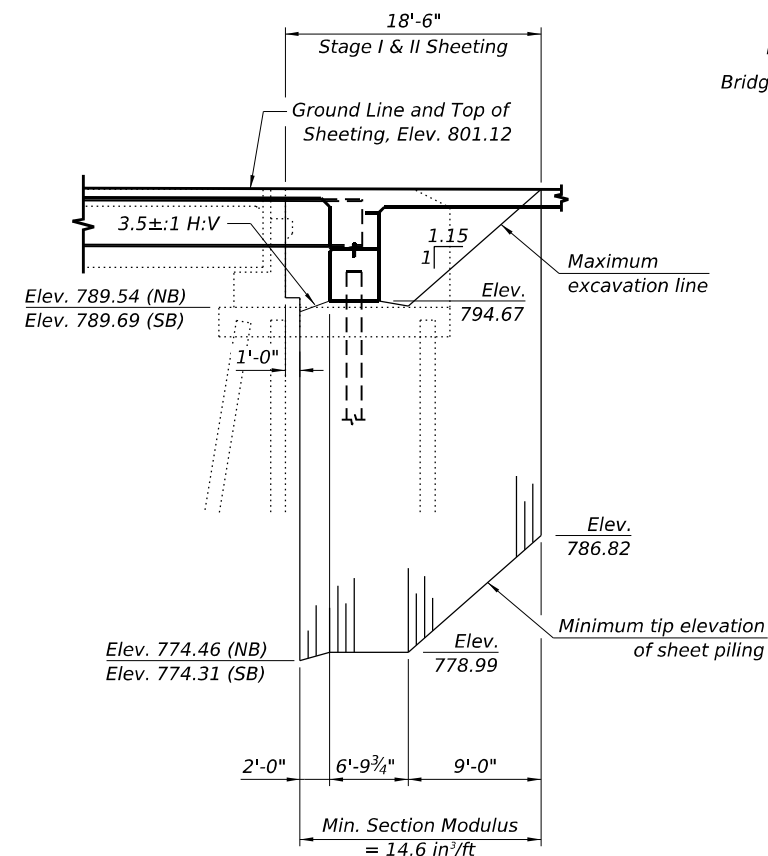
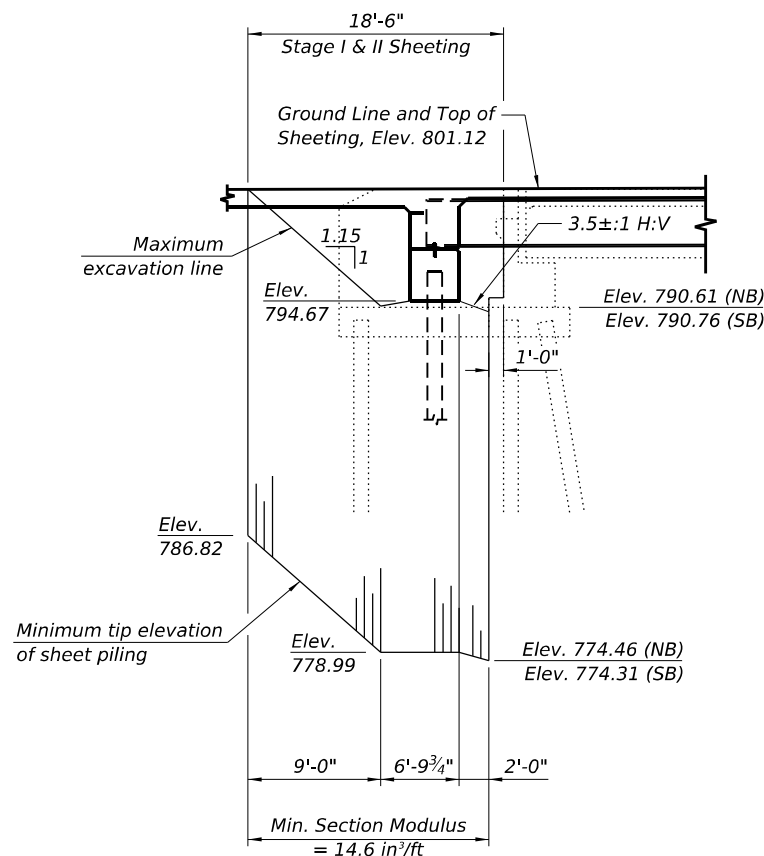
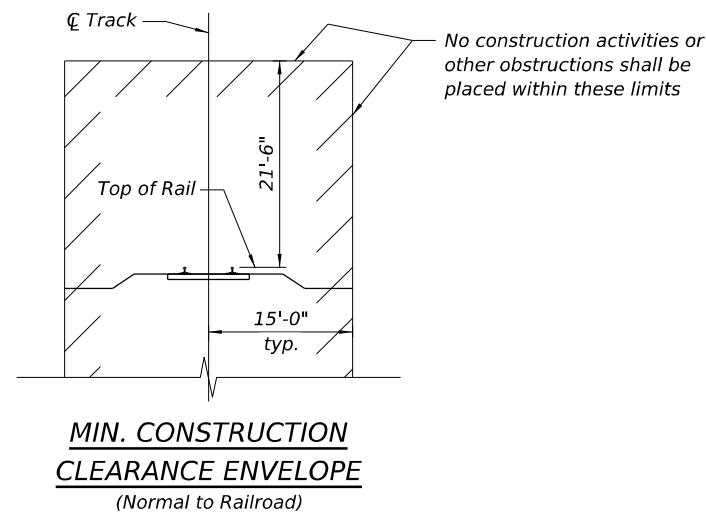
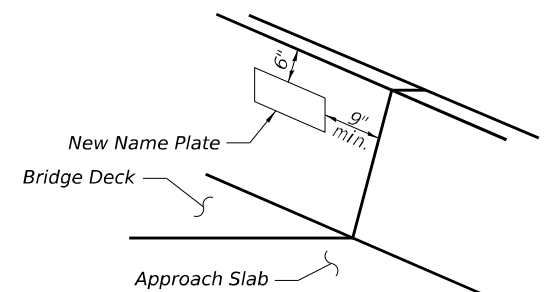
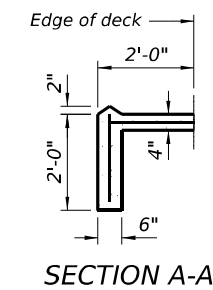
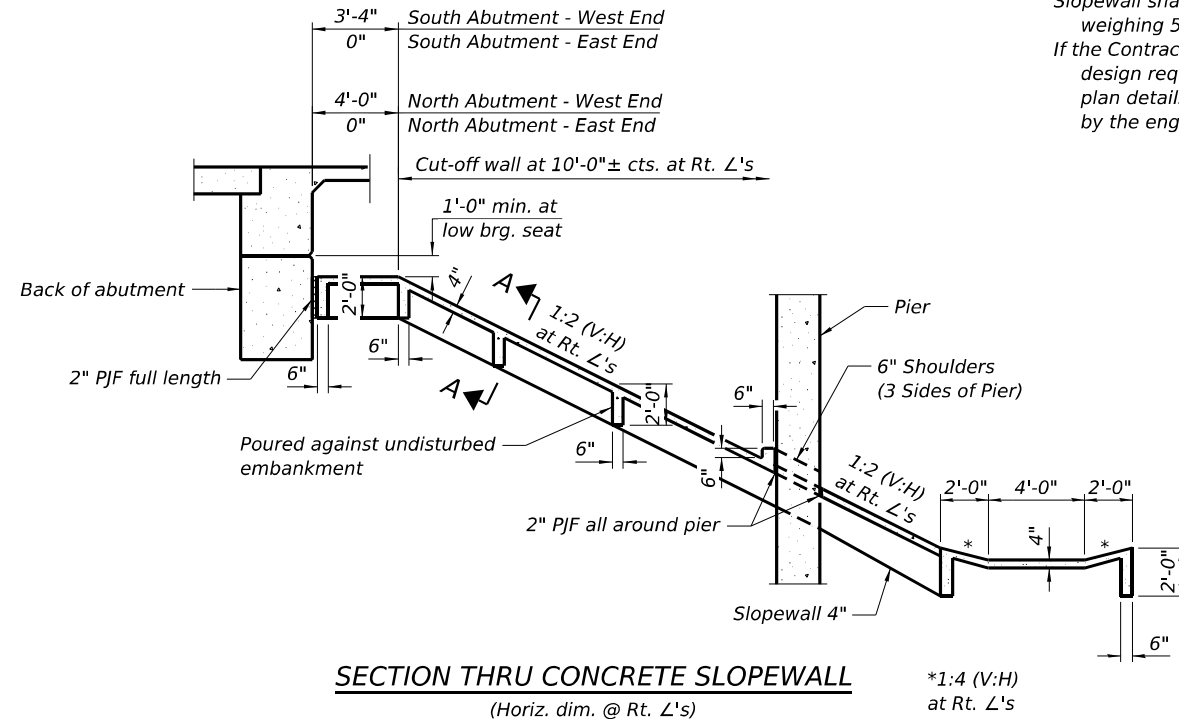
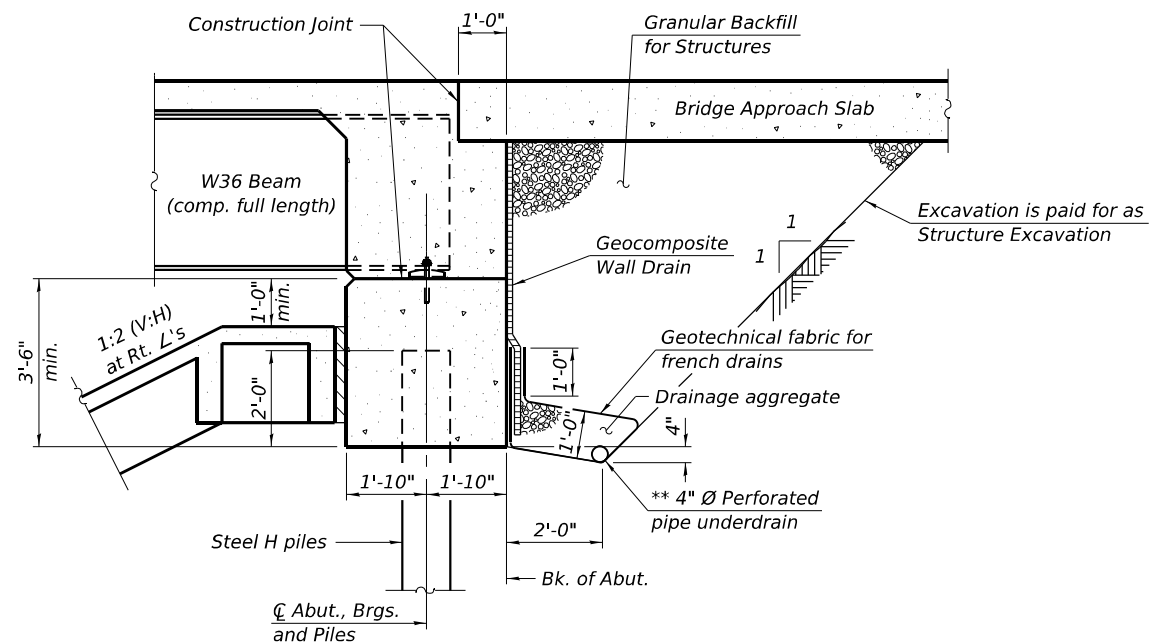
abut.	abutment	NE	north east
B.F.	back face	NW	north west
ℓ	baseline	No.	number
brg.	bearing	N.S.	near side
℄	centerline	O.F.	outside face
cl.	clearance	PJF	preformed joint filler
conc.	concrete	PJS	preformed joint sealer
cts.	centers	PG	profile grade
const.	construction	prop.	proposed
E.B.	expansion bearings	req'd	required
EA	east abutment	rte.	route
EB	east bound	SB	south bound
E.F.	each face	SE	south east
elev.	elevation	SW	south west
exist.	existing	sect.	section
F.B.	fixed bearings	spa.	spaces
F.F.	front face	spec.	specification
F.S.	far side	sta.	station
H.S.	high strength	std.	standard
I.F.	inside face	struct.	structure
jt.	joint	typ.	typical
long.	longitudinal	UNO	unless noted otherwise
max.	maximum	WA	west abutment
min.	minimum	WB	west bound
NB	north bound	WW	wingwall



CURVE DATA  
(@ I-39/US Rte 20)

P.I. Sta. = 2638+19.71  
 $\Delta = 50^{\circ}41'00''$  (LT)  
 $D = 1^{\circ}00'00''$   
 $R = 5,729.70'$   
 $T = 2713.52'$   
 $L = 5,068.44'$   
 $E = 610.07'$   
 $S.E. Run = 3.4\%$   
P.C. Sta. = 2611+06.19  
P.T. Sta. = 2661+74.63





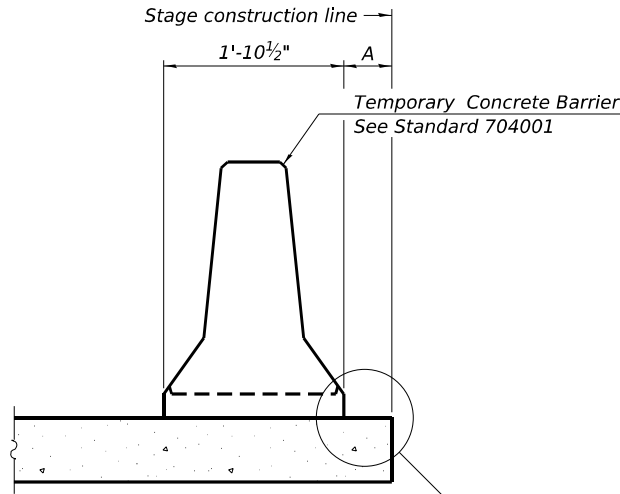
**NOTES:**

*Sloped wall shall be reinforced with welded wire fabric, 6" x 6" - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.*

*If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the engineer.*

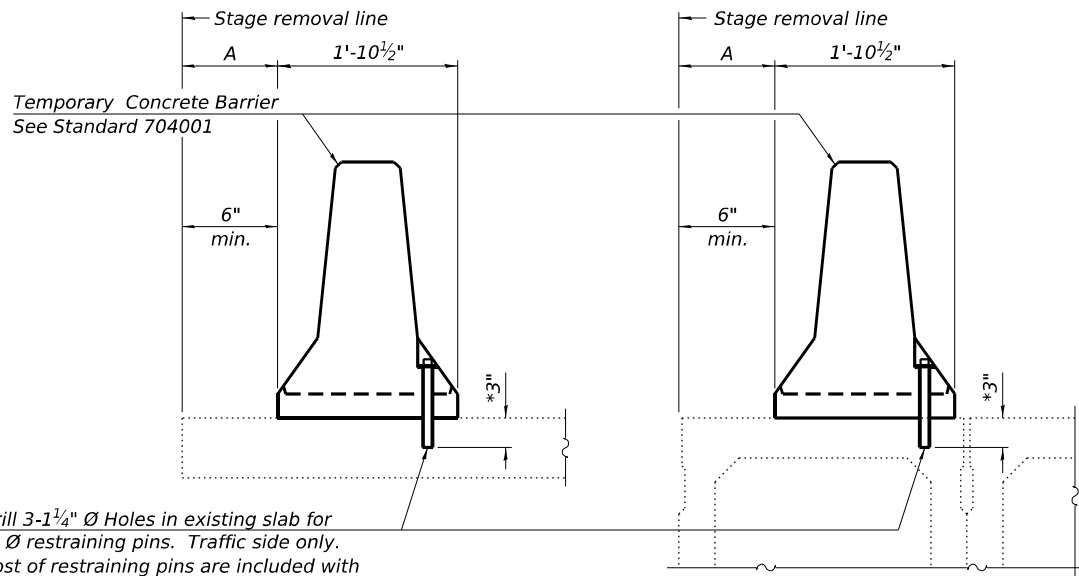


MODEL - SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\17387\1D264C24\_SHT\_0208-0209-CN\_TCB.dgn



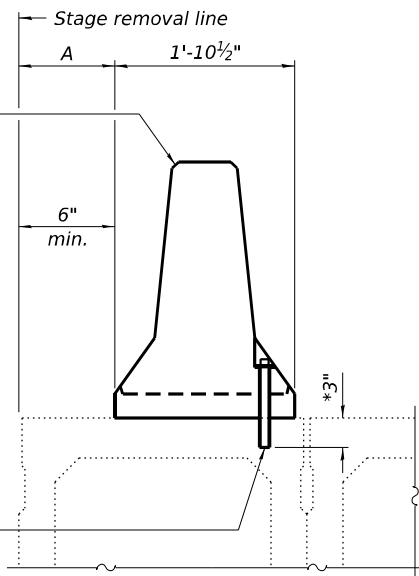
When "A" is 3'-1" or less, the temporary concrete barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

NEW SLAB OR NEW DECK BEAM



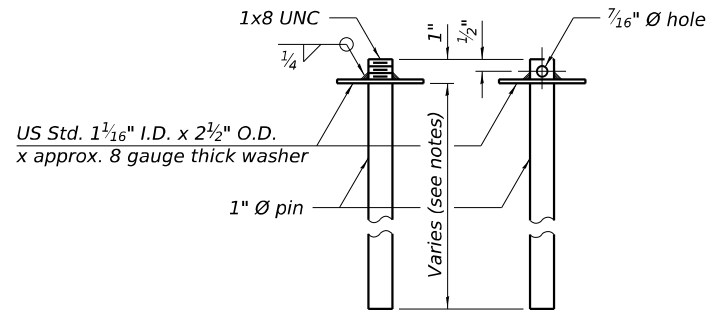
Drill 3-1 1/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint is required when "A" is greater than 3'-1".

EXISTING SLAB



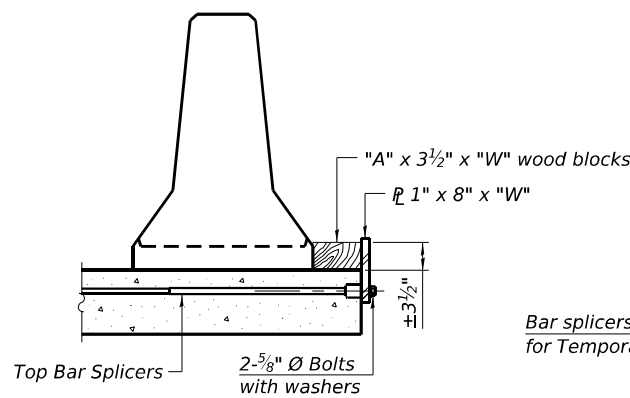
\* When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

EXISTING DECK BEAM

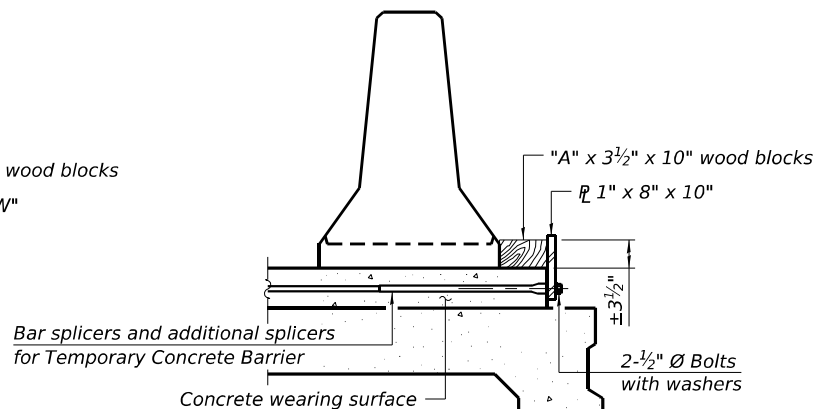


RESTRAINING PIN

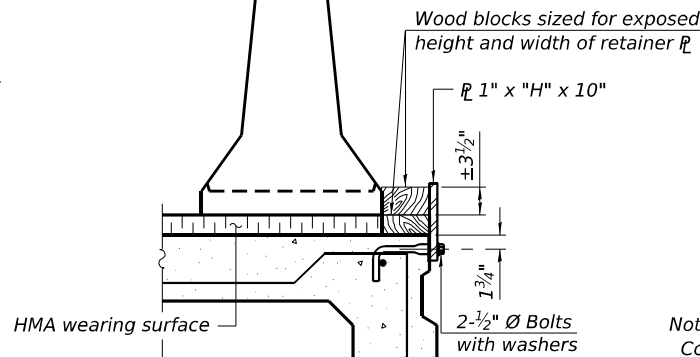
SECTIONS THRU SLAB OR DECK BEAM



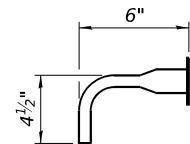
DETAIL I



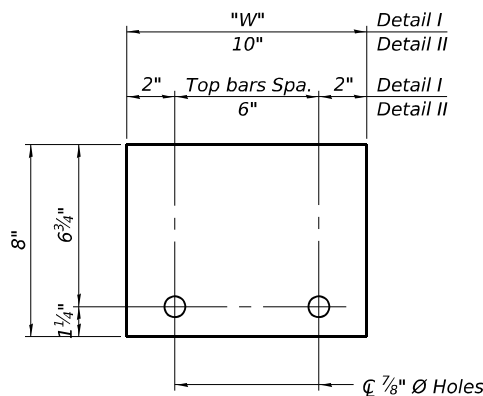
DETAIL II



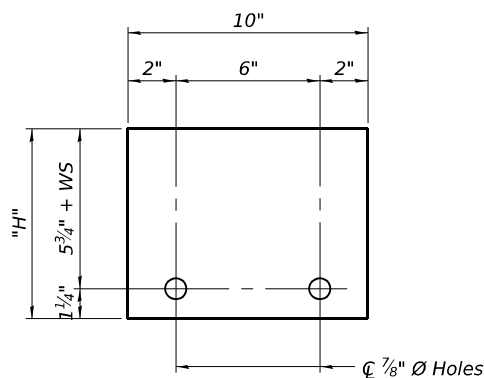
DETAIL III



BAR SPLICER FOR #4 BAR - DETAIL III



STEEL RETAINER 1" x 8" x "W"  
(Detail I and II)



STEEL RETAINER 1" x "H" x 10"  
(Detail III)

Notes:  
Cost of retainer assembly is included with Temporary Concrete Barrier.  
A retainer assembly shall be located at the approximate C of each temporary concrete barrier.  
The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.  
When the 'A' dimension is less than 1 1/2", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

Detail I - Installation for a new bridge deck or bridge slab.

Detail II - Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

RAILING CRITERIA

NCHRP 350 Test Level	3
Railing Weight (plf)	440

R-27

5-15-2023

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PLOT DATE	= 8/12/2024

DESIGNED	- JW
DRAWN	- AT
CHECKED	- AJN
DATE	- 02/27/24

REVISED	-
REVISED	-
REVISED	-
REVISED	-

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

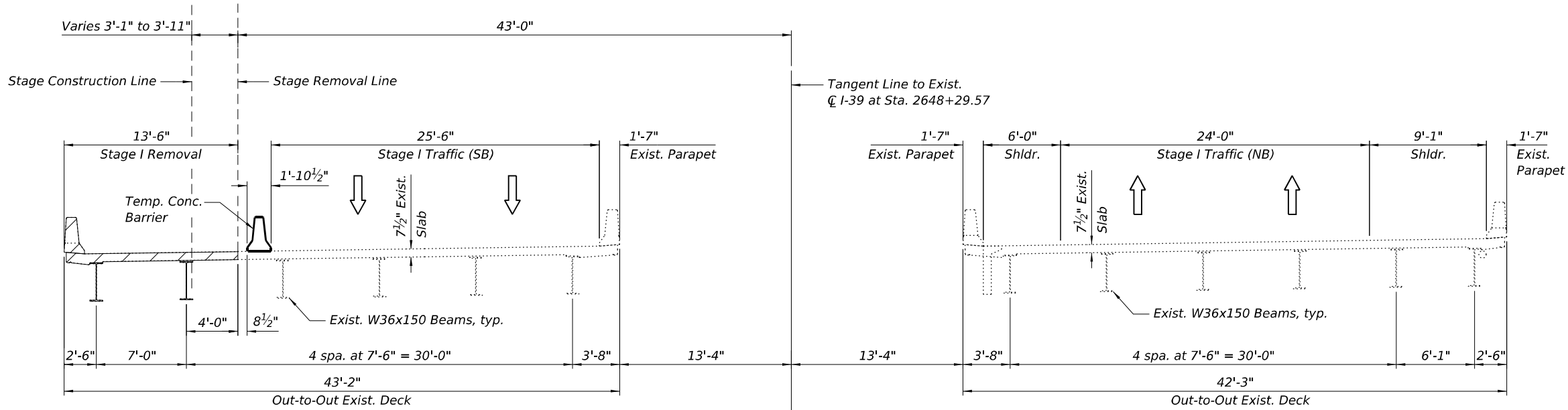
TEMPORARY CONCRETE BARRIER  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 5 OF 60 SHEETS STA. TO STA.

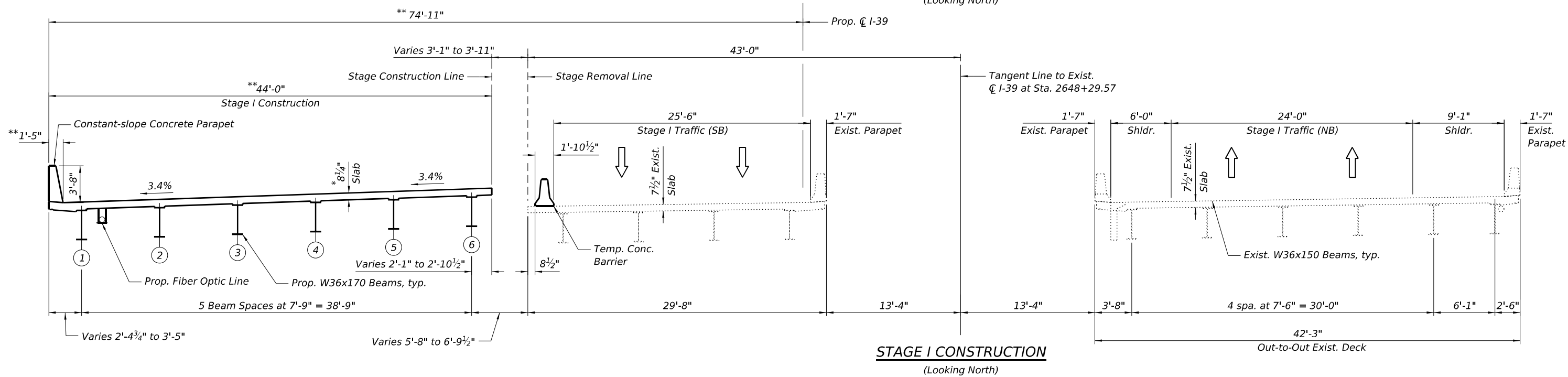
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	548
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

MODEL: SHEET  
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**STAGE I REMOVAL**  
(Looking North)



**STAGE I CONSTRUCTION**  
(Looking North)

**STAGE I REMOVAL**

1. Install temporary concrete barrier as shown to locate traffic on the east side of existing SB structure.
2. Install Temporary Sheet Piling at the approaches.
3. Remove existing concrete bridge decks, approaches, parapets, appurtenances, steel beams, bearings and substructure elements within the limits of Stage I Removal.

**STAGE I CONSTRUCTION**

1. Drive HP piles for abutments, and piers, as shown in the Substructure Layout Plan.
2. Construct Integral abutment caps, slopewalls, pier crash walls, pier columns and pier caps.
3. Install bearings at abutments and piers and erect Beams 1 thru 6 and associated permanent bracing.
4. Install deck forms and reinforcement.
5. Construct reinforced concrete decks, steel diaphragms, and parapets.

**STAGE I CONSTRUCTION - CONTINUED**

6. Construct the cast-in-place bridge approach slabs.
7. Perform diamond grinding and longitudinal bridge deck grooving for bridge deck and approaches.
8. Apply Protective Coat for bridge decks, approaches and tops and inside faces of parapets.
9. Place temporary pavement markings on the top of deck for Stage II traffic (See Roadway Plans).

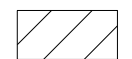
\* Prior to Grinding

\*\* Radial Dimension

**NOTE:**

1. For Quantity of Temporary Concrete Barrier, See Roadway Plans.
2. For temporary concrete barrier details, see sheet 5 of 60.

**LEGEND**



Removal of Existing Structures No. 2

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	DRAWN - AT	REVISED -
PLOT SCALE = \$SCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

**STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION**

**STAGE 1 CONSTRUCTION & REMOVAL  
STRUCTURE NO. 101-0208/0209**

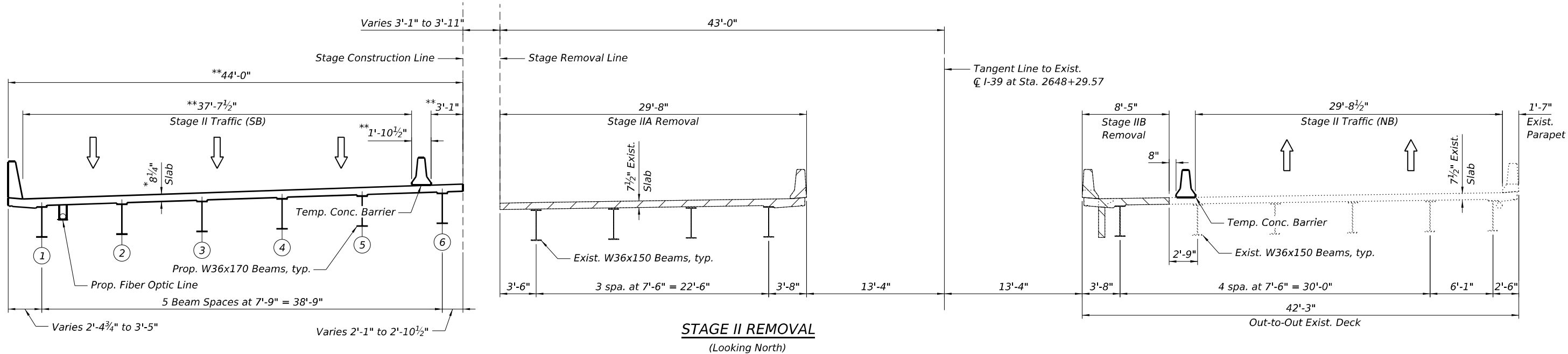
SCALE: SHEET 6 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	549
		CONTRACT NO. 64C24		
		ILLINOIS FED. AID PROJECT		

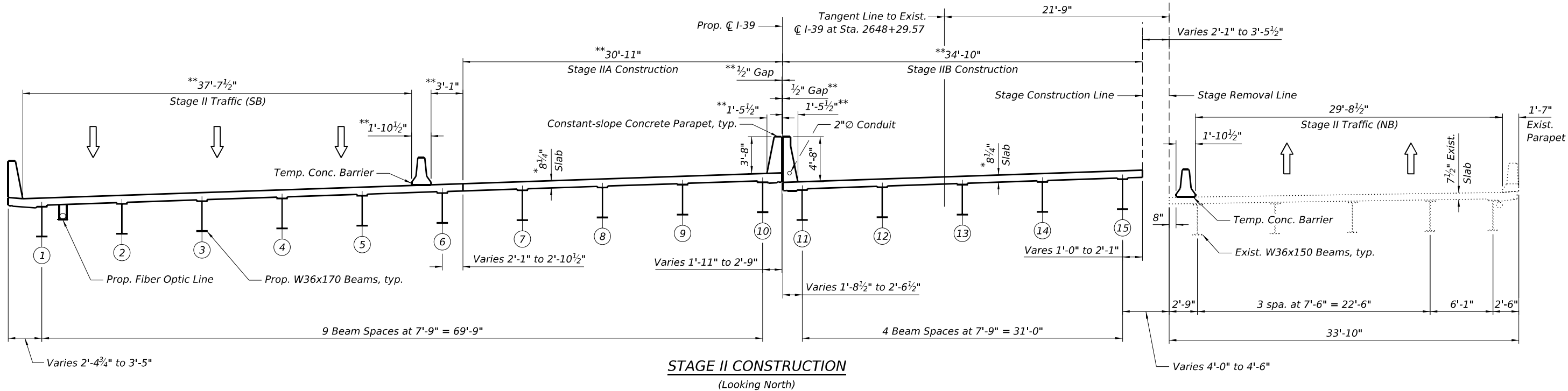
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



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**STAGE II REMOVAL**  
(Looking North)



**STAGE II CONSTRUCTION**  
(Looking North)



\* Prior to Grinding

\*\* Radial Dimension

**NOTE:**

1. For Quantity of Temporary Concrete Barrier, See Roadway Plans.
2. For temporary concrete barrier details, see sheet 5 of 60.

**LEGEND**

-  Removal of Existing Structures No. 1
-  Removal of Existing Structures No. 2

**STAGE II REMOVAL**

1. Install temporary concrete barrier as shown to locate traffic on the west side of proposed SB structure.
2. Install Temporary Sheet Piling at the approaches.
3. Remove existing concrete bridge decks, approaches, parapets, appurtenances, steel beams, bearings and substructure elements within the limits of Stage II Removal.

**STAGE II CONSTRUCTION**

1. Drive HP piles for abutments, and piers, as shown in the Substructure Layout Plan.
2. Construct Integral abutment caps, slopewalls, pier crash walls, pier columns and pier caps.
3. Install bearings at abutments and piers and erect Beams 7 thru 15 and associated permanent bracing.
4. Install deck forms and reinforcement.
5. Construct reinforced concrete decks, steel diaphragms, and parapets.

**STAGE II CONSTRUCTION - CONTINUED**

6. Construct the cast-in-place bridge approach slabs.
7. Perform diamond grinding and longitudinal bridge deck grooving for bridge deck and approaches.
8. Apply Protective Coat for bridge decks, approaches and tops and inside faces of parapets.
9. Place temporary pavement markings on the top of deck for Stage III traffic (See Roadway Plans).

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PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

**STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION**

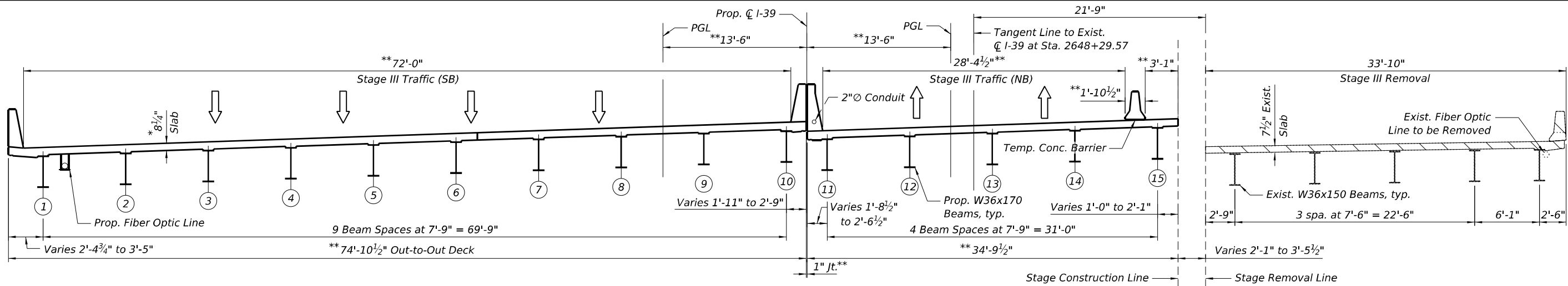
**STAGE 2 CONSTRUCTION & REMOVAL  
STRUCTURE NO. 101-0208/0209**

SCALE: SHEET 7 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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				CONTRACT NO. 64C24
ILLINOIS FED. AID PROJECT				

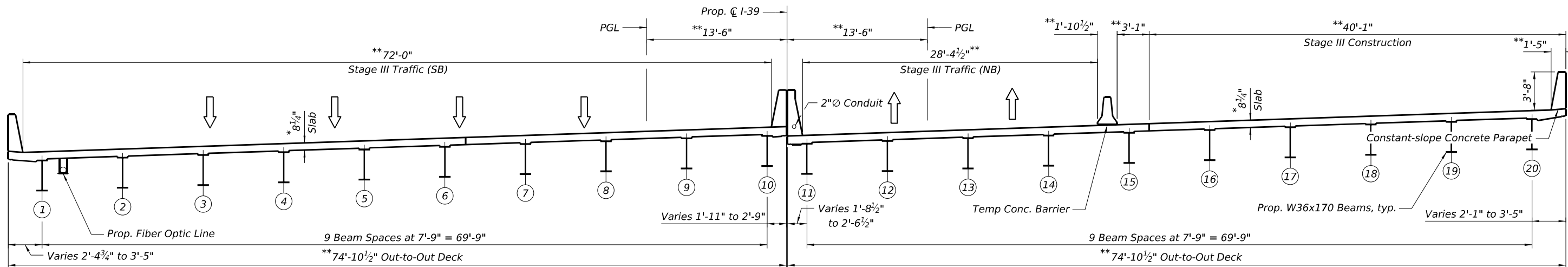
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

MODEL - SHEET  
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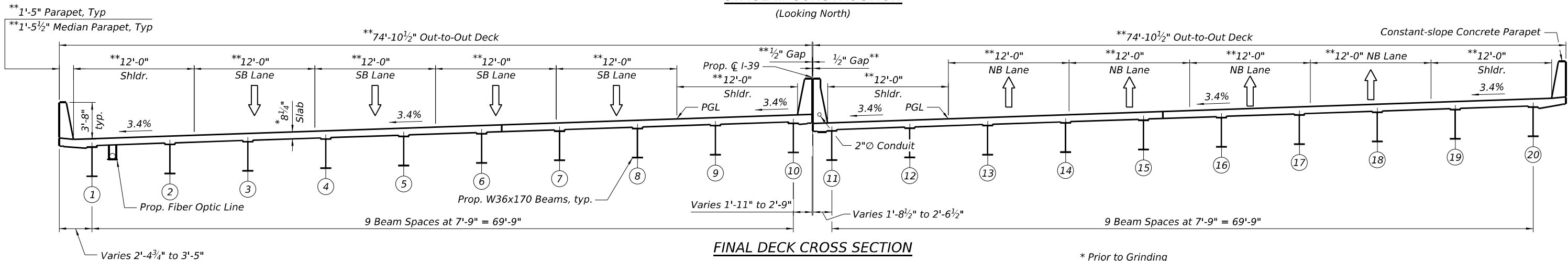
### STAGE III REMOVAL

(Looking North)



### STAGE III CONSTRUCTION

(Looking North)



### FINAL DECK CROSS SECTION

(Looking North)

#### STAGE III REMOVAL

1. Install temporary concrete barrier as shown to locate traffic on the west side of proposed NB structure.
2. Remove existing concrete bridge decks, approaches, parapets, appurtenances, steel beams, bearings and substructure elements within the limits of Stage III Removal.

#### STAGE III CONSTRUCTION

1. Drive HP piles for abutments, and piers, as shown in the Substructure Layout Plan.
2. Construct Integral abutment caps, slopewalls, pier crash walls, pier columns and pier caps.
3. Install bearings at abutments and piers and erect Beams 16 thru 20 and associated permanent bracing.
4. Install deck forms and reinforcement.
5. Construct reinforced concrete decks, steel diaphragms, and parapets.

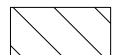
#### STAGE III CONSTRUCTION - CONTINUED

6. Construct the cast-in-place bridge approach slabs.
7. Perform diamond grinding and longitudinal bridge deck grooving for bridge deck and approaches.
8. Apply Protective Coat for bridge decks, approaches and tops and inside faces of parapets.
9. Place temporary pavement markings on the top of deck for final condition (See Roadway Plans).

\* Prior to Grinding

\*\* Radial Dimension

#### LEGEND



Removal of Existing Structures No. 1

#### NOTE:

1. For Quantity of Temporary Concrete Barrier, See Roadway Plans.
2. For temporary concrete barrier details, see sheet 5 of 60.

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PLOT SCALE = \$SCALES\$	CHECKED - AJN	REVISED -
PLOT DATE = 2/13/2025	DATE - 02/27/24	REVISED -

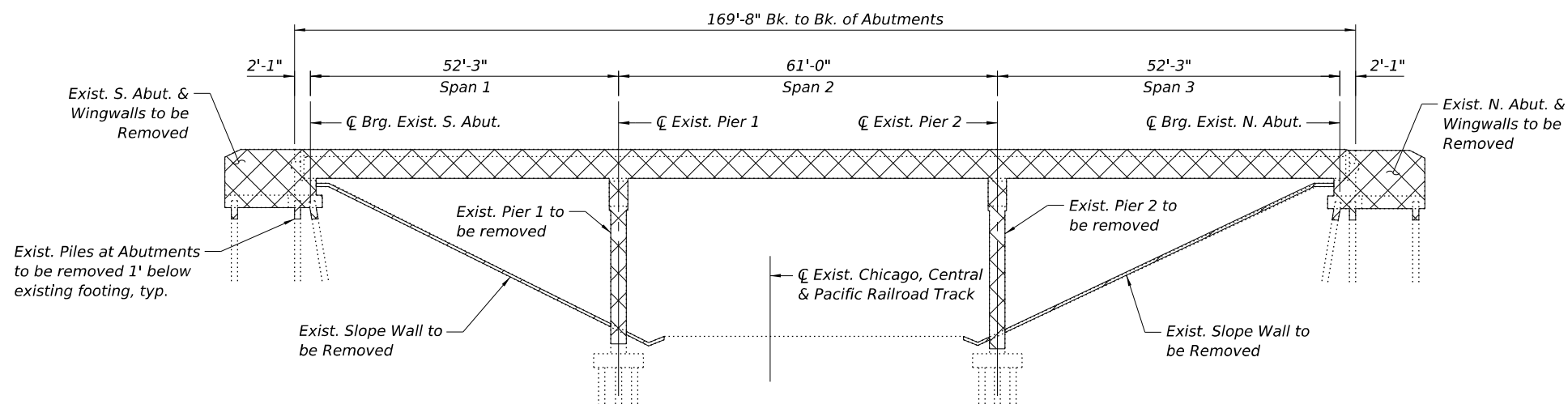
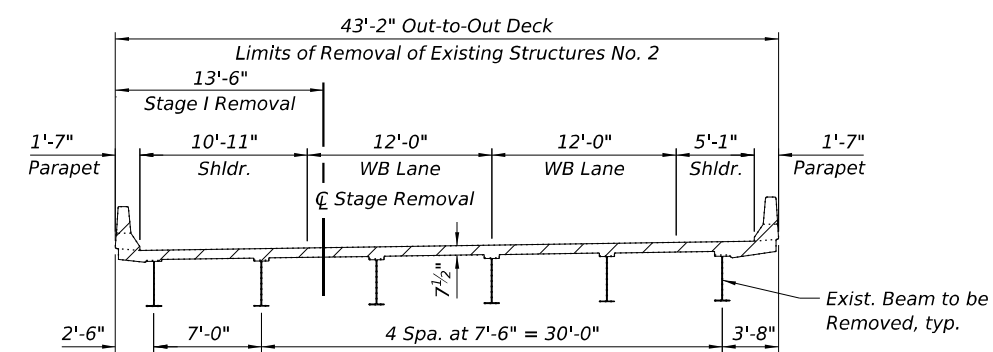
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

STAGE 3 CONSTRUCTION & REMOVAL  
STRUCTURE NO. 101-0208/0209

SCALE: \_\_\_\_\_ SHEET 8 OF 60 SHEETS STA. \_\_\_\_\_ TO STA. \_\_\_\_\_

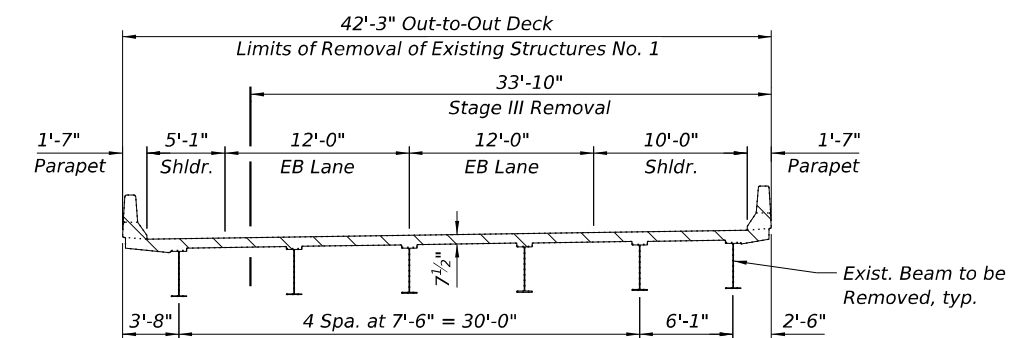
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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				CONTRACT NO. 64C24
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

REMOVAL ELEVATION

EXISTING SOUTHBOUND DECK CROSS SECTION

(Looking North)



EXISTING NORTHBOUND DECK CROSS SECTION

(Looking North)

*LEGEND*

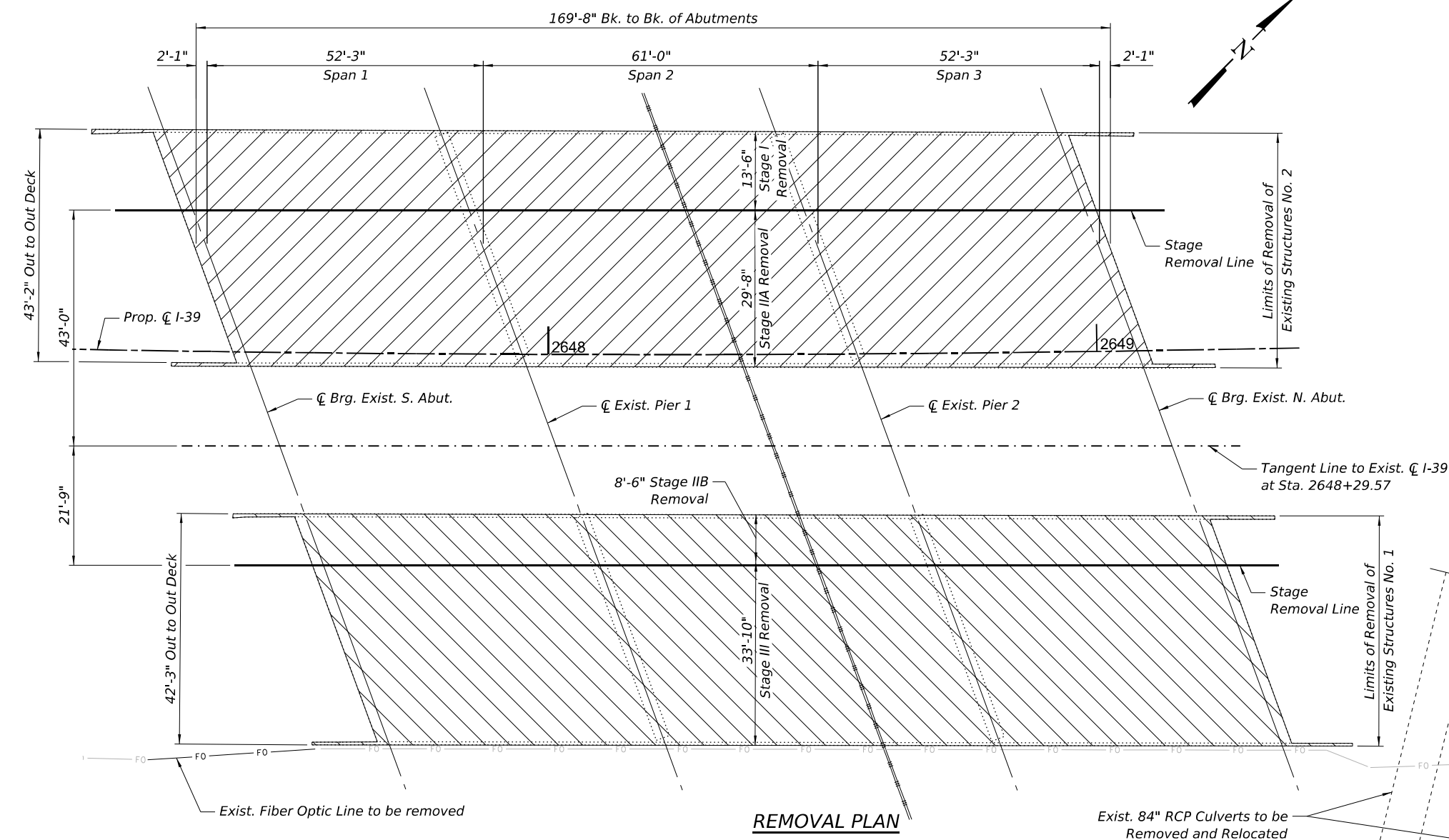
Removal of Existing Structures

Removal of Existing Structures No. 1

Removal of Existing Structures No. 2

NOTES

1. *For suggested stages of construction and sequencing requirements, see Roadway Plans and Special Provisions*
2. *For substructure removal details, see Sheets 10 thru 13 of 60.*
3. *For Temporary Sheet Piling limits and details, see Sheet 3 of 60.*
4. *The Contractor shall take all necessary precautions to protect existing utilities and adjacent structures during removal/construction of the bridge.*
5. *Existing Piers shall be removed according to Art. 501.05 of the Standard Specifications and as shown in the Plans.*
6. *For Approach Slab Removal, See Roadway Plans.*

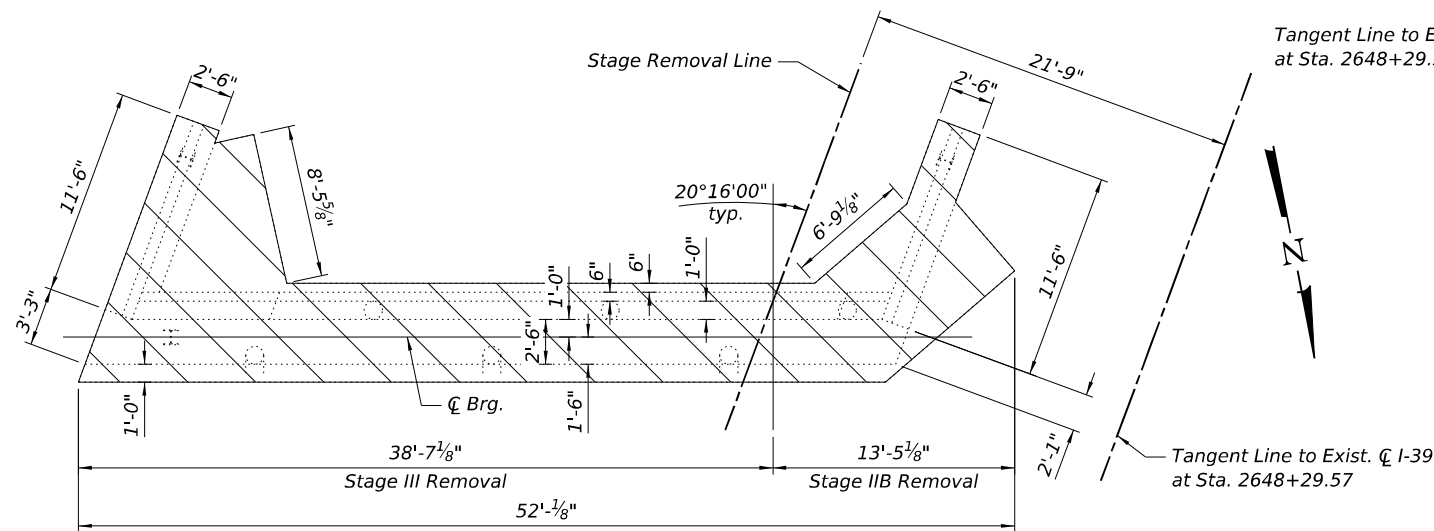


### REMOVAL PLAN

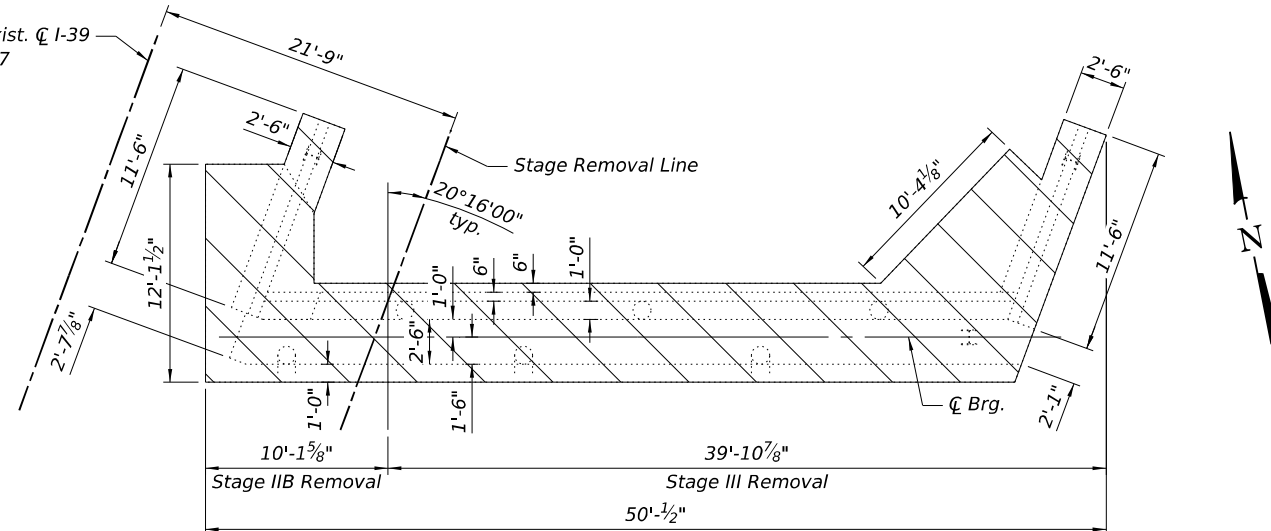
*Exist. 84" RCP Culverts to be  
Removed and Relocated*



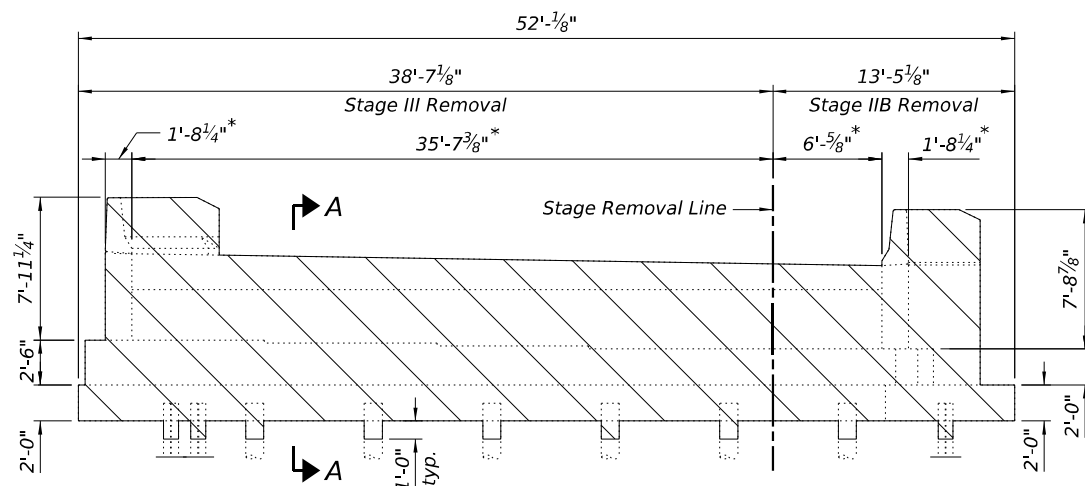




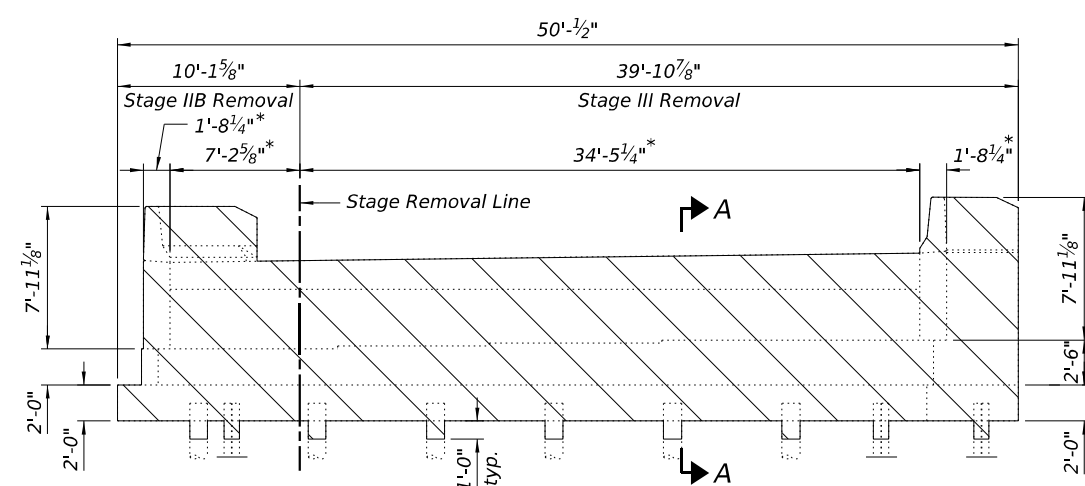
**SOUTH ABUTMENT - REMOVAL PLAN**



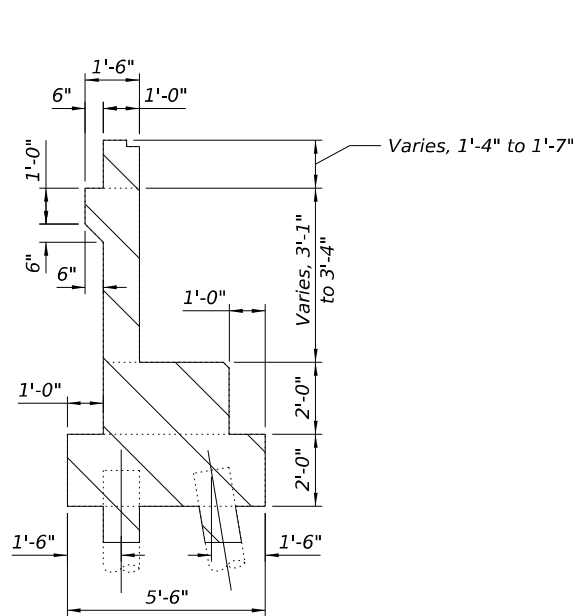
**NORTH ABUTMENT - REMOVAL PLAN**



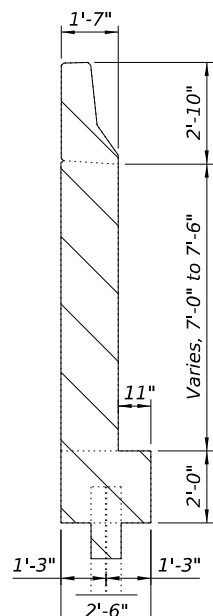
**SOUTH ABUTMENT - REMOVAL ELEVATION**  
(Looking South)



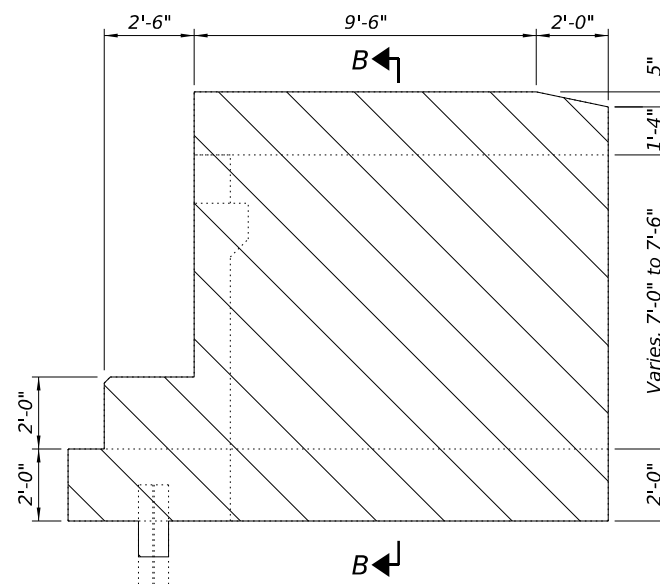
**NORTH ABUTMENT - REMOVAL ELEVATION**  
(Looking North)



**SECTION A-A**



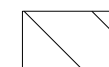
**SECTION B-B**



**TYPICAL WINGWALL - REMOVAL ELEVATION**

\* Dimensions measured along Front Face of Abutment

**LEGEND**



Removal of Existing Structures No. 1

MODEL: SHEET  
FILE NAME: c:\pwwork\benesch projects\projects\101\101-0208-0209-CN\_EXIST-ABUTS-NB.dgn

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PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

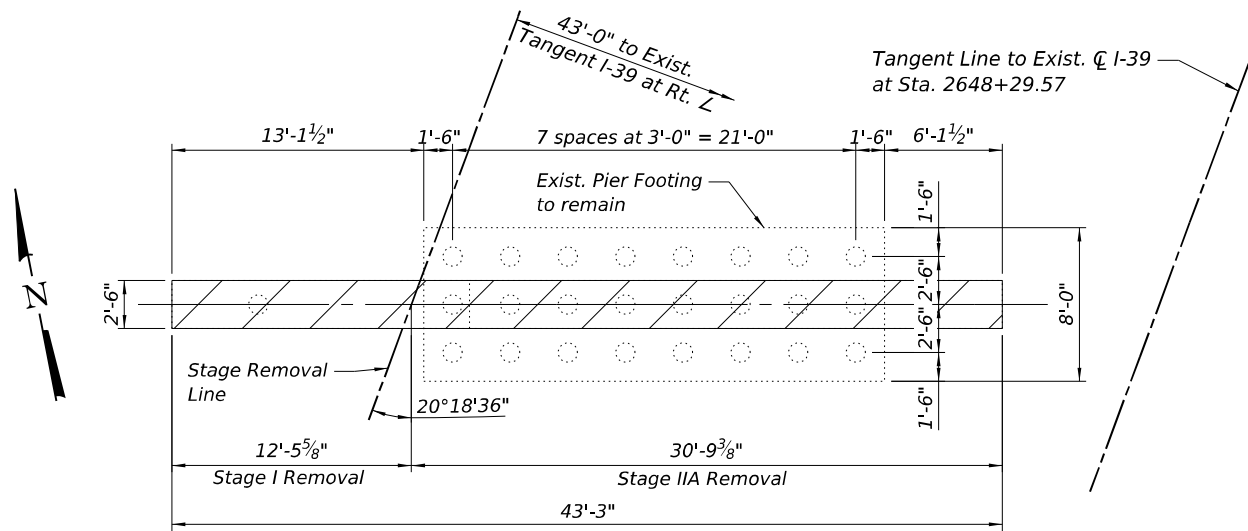
**STATE OF ILLINOIS**  
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**EXISTING NORTHBOUND ABUTMENTS REMOVAL**  
**STRUCTURE NO. 101-0208/0209**

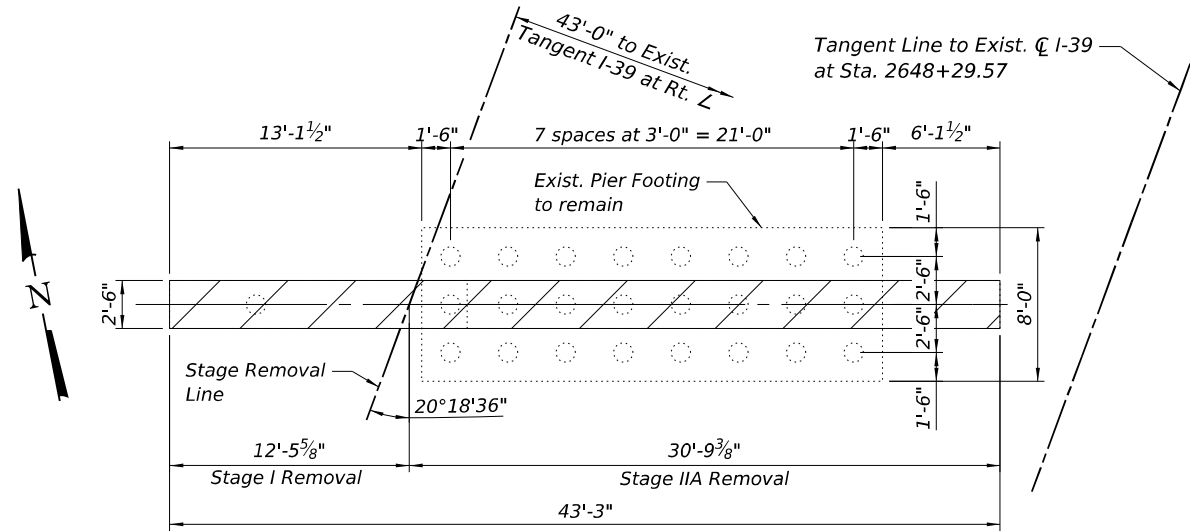
SCALE: SHEET 11 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	554
				CONTRACT NO. 64C24
		ILLINOIS	FED. AID PROJECT	

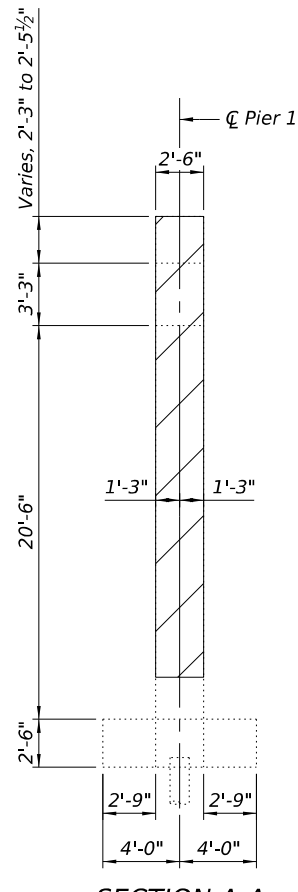
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



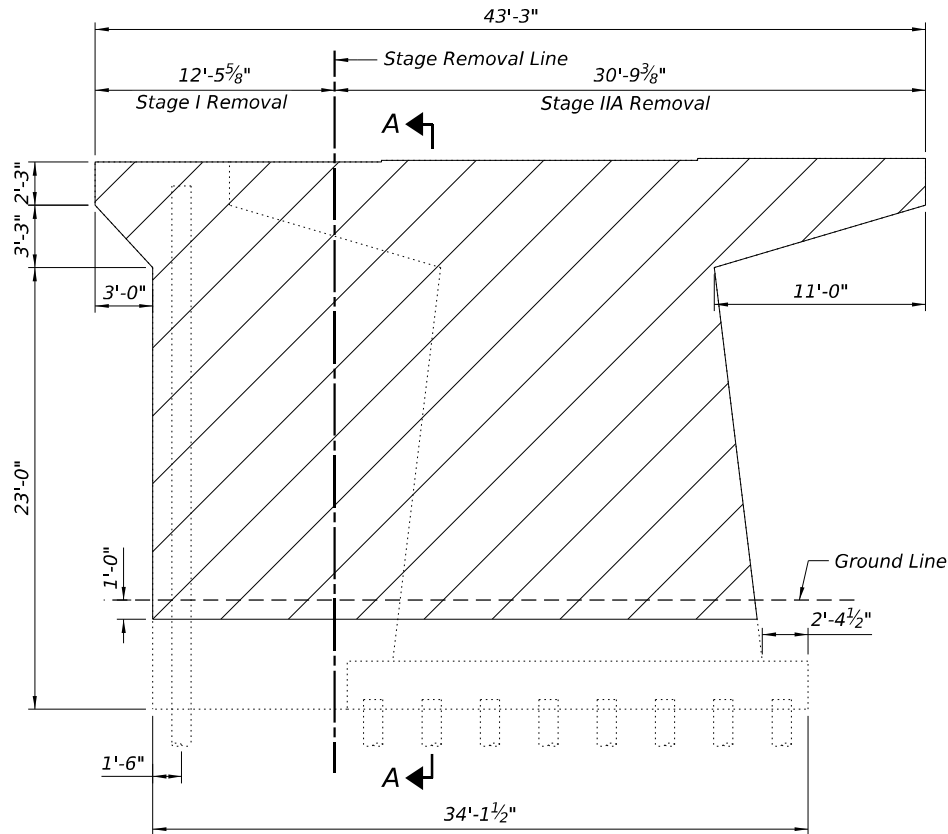
PIER 1 REMOVAL PLAN



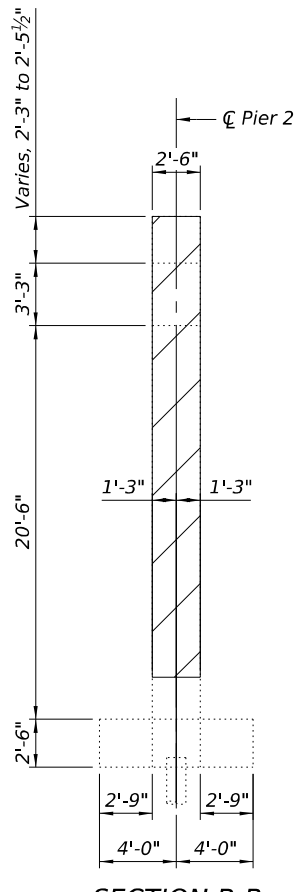
PIER 2 REMOVAL PLAN



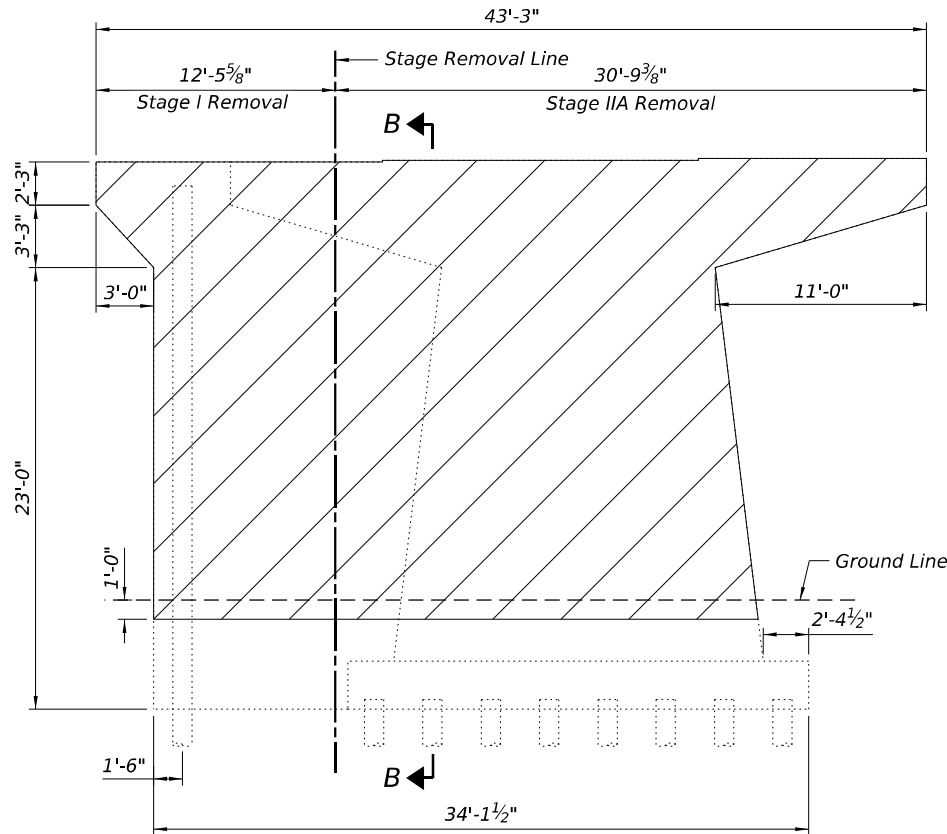
SECTION A-A



PIER 1 REMOVAL ELEVATION  
(Looking North)

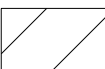


SECTION B-B



PIER 2 REMOVAL ELEVATION  
(Looking North)

LEGEND



Removal of Existing Structures No. 2

MODEL: SHEET  
FILE NAME: c:\paword\benesch projects\projects\0173871\0264C24\_SHT\_0208-0209-CN\_EXIST-PIERS-SB.dgn

**Bowman**

10 S. LaSalle Street, Suite 210  
Chicago, Illinois 60603  
(312) 464-0300  
www.bowman.com

USER NAME = Ifranceschina	DESIGNED - JW	REVISED -
DRAWN - AT	REVISED -	
PLOT SCALE = \$SCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

EXISTING SOUTHBOUND PIERS REMOVAL  
STRUCTURE NO. 101-0208/0209

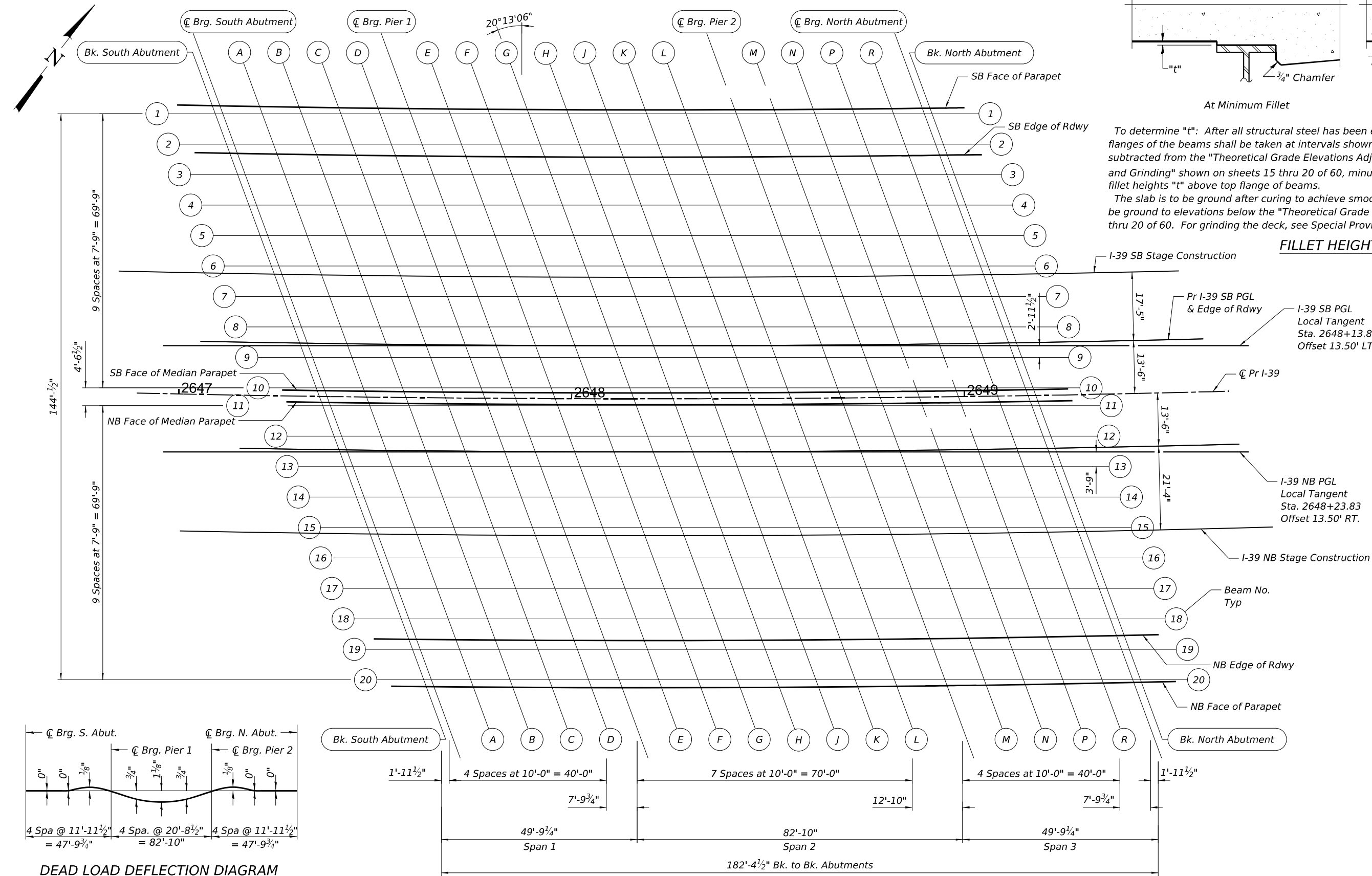
SCALE: SHEET 12 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	555
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



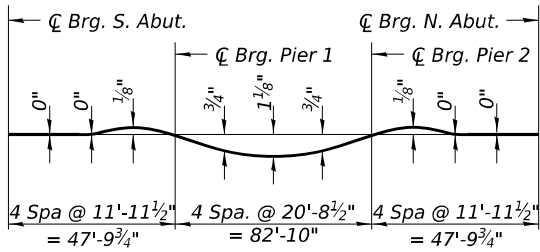
MODEL: SHEET  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\101\101-0208-0209-CN\_DECK-SLAB-ELEV-1.dgn



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown on this sheet. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection and Grinding" shown on sheets 15 thru 20 of 60, minus 8 1/4" deck thickness, equals the fillet heights "t" above top flange of beams.

The slab is to be ground after curing to achieve smoothness, but the slab is not to be ground to elevations below the "Theoretical Grade Elevations" shown on sheets 15 thru 20 of 60. For grinding the deck, see Special Provisions.

### FILLET HEIGHTS



### DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only)

Note:  
The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections and grinding as shown on sheets 15 thru 20 of 60.

### PLAN

**Bowman**

10 S. LaSalle Street, Suite 210  
Chicago, Illinois 60603  
312-454-0550  
www.bowman.com

USER NAME = Ifranceschina	DESIGNED - JW	REVISED -
	DRAWN - DSO	REVISED -
PLOT SCALE = \$SCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

SLAB ELEVATIONS 1  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 14 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	557
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)



*SOUTHBOUND FACE OF PARAPET*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+07.11	-73.50	798.88	798.90
└ Brg. S. Abutment	2647+09.11	-73.50	798.88	798.90
A	2647+19.31	-73.50	798.93	798.95
B	2647+29.50	-73.50	798.97	798.99
C	2647+39.69	-73.50	799.01	799.03
D	2647+49.87	-73.50	799.05	799.07
└ Brg. Pier 1	2647+57.82	-73.50	799.08	799.10
E	2647+67.99	-73.50	799.11	799.15
F	2647+78.15	-73.50	799.14	799.20
G	2647+88.30	-73.50	799.17	799.25
H	2647+98.45	-73.50	799.19	799.28
J	2648+08.59	-73.50	799.21	799.30
K	2648+18.72	-73.50	799.23	799.30
L	2648+28.85	-73.50	799.24	799.29
└ Brg. Pier 2	2648+41.83	-73.50	799.25	799.27
M	2648+51.94	-73.50	799.25	799.28
N	2648+62.05	-73.50	799.25	799.27
P	2648+72.15	-73.50	799.24	799.26
R	2648+82.24	-73.50	799.23	799.25
└ Brg. N. Abutment	2648+90.13	-73.50	799.22	799.24
Bk. N. Abutment	2648+92.10	-73.50	799.21	799.23

*BEAM 1*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+07.91	-71.49	799.02	799.04
☒ Brg. S. Abutment	2647+09.89	-71.52	799.03	799.05
A	2647+19.96	-71.70	799.06	799.08
B	2647+30.14	-71.87	799.10	799.12
C	2647+40.26	-72.02	799.13	799.15
D	2647+50.39	-72.14	799.17	799.19
☒ Brg. Pier 1	2647+58.31	-72.23	799.19	799.21
E	2647+68.43	-72.33	799.23	799.27
F	2647+78.56	-72.41	799.25	799.31
G	2647+88.69	-72.47	799.28	799.36
H	2647+98.82	-72.51	799.30	799.39
J	2648+08.95	-72.54	799.32	799.41
K	2648+19.07	-72.55	799.33	799.40
L	2648+29.20	-72.54	799.34	799.39
☒ Brg. Pier 2	2648+42.20	-72.50	799.35	799.37
M	2648+52.33	-72.45	799.36	799.39
N	2648+62.46	-72.38	799.36	799.38
P	2648+72.58	-72.30	799.35	799.37
R	2648+82.71	-72.20	799.35	799.37
☒ Brg. N. Abutment	2648+90.63	-72.10	799.34	799.36
Bk. N. Abutment	2648+92.61	-72.08	799.33	799.35

*BEAM 2*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+10.95	-63.79	799.29	799.31
☒ Brg. S. Abutment	2647+12.92	-63.83	799.30	799.32
A	2647+23.03	-64.01	799.34	799.36
B	2647+33.15	-64.16	799.37	799.39
C	2647+43.26	-64.30	799.41	799.43
D	2647+53.37	-64.43	799.44	799.46
☒ Brg. Pier 1	2647+61.28	-64.51	799.47	799.49
E	2647+71.39	-64.60	799.50	799.54
F	2647+81.50	-64.68	799.52	799.59
G	2647+91.62	-64.73	799.55	799.64
H	2648+01.73	-64.77	799.57	799.67
J	2648+11.85	-64.79	799.59	799.68
K	2648+21.96	-64.80	799.60	799.68
L	2648+32.08	-64.78	799.61	799.66
☒ Brg. Pier 2	2648+45.06	-64.74	799.62	799.64
M	2648+55.17	-64.68	799.62	799.65
N	2648+65.28	-64.61	799.62	799.65
P	2648+75.40	-64.52	799.61	799.63
R	2648+85.51	-64.41	799.61	799.63
☒ Brg. N. Abutment	2648+93.42	-64.32	799.60	799.62
Bk. N. Abutment	2648+95.39	-64.29	799.59	799.61

*SOUTHBOUND EDGE OF ROADWAY*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+11.85	-61.50	799.38	799.40
☒ Brg. S. Abutment	2647+13.84	-61.50	799.38	799.40
A	2647+24.02	-61.50	799.42	799.44
B	2647+34.18	-61.50	799.47	799.49
C	2647+44.35	-61.50	799.51	799.53
D	2647+54.50	-61.50	799.54	799.56
☒ Brg. Pier 1	2647+62.43	-61.50	799.57	799.59
E	2647+72.58	-61.50	799.61	799.65
F	2647+82.71	-61.50	799.63	799.70
G	2647+92.84	-61.50	799.66	799.75
H	2648+02.96	-61.50	799.68	799.78
J	2648+13.08	-61.50	799.70	799.79
K	2648+23.19	-61.50	799.71	799.79
L	2648+33.29	-61.50	799.72	799.77
☒ Brg. Pier 2	2648+46.24	-61.50	799.73	799.75
M	2648+56.33	-61.50	799.73	799.76
N	2648+66.41	-61.50	799.72	799.75
P	2648+76.49	-61.50	799.72	799.74
R	2648+86.56	-61.50	799.70	799.72
☒ Brg. N. Abutment	2648+94.42	-61.50	799.69	799.71
Bk. N. Abutment	2648+96.39	-61.50	799.69	799.71

### BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+13.98	-56.10	799.57	799.59
☒ Brg. S. Abutment	2647+15.95	-56.13	799.57	799.59
A	2647+26.05	-56.30	799.61	799.63
B	2647+36.15	-56.46	799.64	799.66
C	2647+46.24	-56.59	799.68	799.70
D	2647+56.34	-56.71	799.71	799.73
☒ Brg. Pier 1	2647+64.24	-56.79	799.74	799.76
E	2647+74.34	-56.88	799.77	799.81
F	2647+84.44	-56.95	799.79	799.86
G	2647+94.54	-57.00	799.82	799.91
H	2648+04.64	-57.03	799.84	799.94
J	2648+14.74	-57.05	799.85	799.94
K	2648+24.84	-57.04	799.87	799.95
L	2648+34.94	-57.03	799.88	799.93
☒ Brg. Pier 2	2648+47.90	-56.97	799.88	799.90
M	2648+58.00	-56.92	799.88	799.91
N	2648+68.10	-56.84	799.88	799.91
P	2648+78.20	-56.74	799.88	799.90
R	2648+88.30	-56.63	799.87	799.89
☒ Brg. N. Abutment	2648+96.20	-56.53	799.86	799.88
Bk. N. Abutment	2648+98.17	-56.50	799.85	799.87

*BEAM 4*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+17.00	-48.40	799.84	799.86
☒ Brg. S. Abutment	2647+18.97	-48.43	799.85	799.87
A	2647+29.05	-48.60	799.88	799.90
B	2647+39.14	-48.75	799.92	799.94
C	2647+49.22	-48.88	799.95	799.97
D	2647+59.31	-48.99	799.99	800.01
☒ Brg. Pier 1	2647+67.19	-49.07	800.01	800.03
E	2647+77.28	-49.15	800.04	800.08
F	2647+87.36	-49.21	800.06	800.13
G	2647+97.45	-49.26	800.09	800.18
H	2648+07.54	-49.29	800.11	800.21
J	2648+17.62	-49.30	800.12	800.21
K	2648+27.71	-49.29	800.13	800.21
L	2648+37.80	-49.27	800.14	800.19
☒ Brg. Pier 2	2648+50.74	-49.21	800.15	800.17
M	2648+60.83	-49.15	800.15	800.18
N	2648+70.91	-49.06	800.14	800.17
P	2648+81.00	-48.96	800.14	800.16
R	2648+91.09	-48.85	800.13	800.15
☒ Brg. N. Abutment	2648+98.97	-48.74	800.12	800.14
Bk. N. Abutment	2649+00.94	-48.71	800.11	800.13

*BEAM 5*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+20.01	-40.70	800.12	800.14
☒ Brg. S. Abutment	2647+21.98	-40.73	800.12	800.14
A	2647+32.05	-40.89	800.16	800.18
B	2647+42.12	-41.04	800.19	800.21
C	2647+52.19	-41.16	800.23	800.25
D	2647+62.26	-41.27	800.26	800.28
☒ Brg. Pier 1	2647+70.14	-41.34	800.28	800.30
E	2647+80.21	-41.42	800.31	800.35
F	2647+90.28	-41.48	800.33	800.40
G	2648+00.35	-41.52	800.36	800.45
H	2648+10.43	-41.54	800.37	800.47
J	2648+20.50	-41.55	800.39	800.48
K	2648+30.57	-41.54	800.40	800.48
L	2648+40.65	-41.51	800.41	800.46
☒ Brg. Pier 2	2648+53.57	-41.44	800.41	800.43
M	2648+63.65	-41.37	800.41	800.44
N	2648+73.72	-41.29	800.41	800.44
P	2648+83.79	-41.18	800.40	800.42
R	2648+93.86	-41.06	800.39	800.41
☒ Brg. N. Abutment	2649+01.73	-40.95	800.38	800.40
Bk. N. Abutment	2649+03.70	-40.92	800.37	800.39

*BEAM 6*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+23.02	-33.00	800.39	800.41
☒ Brg. S. Abutment	2647+24.98	-33.03	800.40	800.42
A	2647+35.04	-33.19	800.43	800.45
B	2647+45.10	-33.33	800.47	800.49
C	2647+55.15	-33.45	800.50	800.52
D	2647+65.21	-33.55	800.53	800.55
☒ Brg. Pier 1	2647+73.07	-33.62	800.55	800.57
E	2647+83.13	-33.69	800.58	800.62
F	2647+93.19	-33.74	800.60	800.67
G	2648+03.25	-33.78	800.63	800.72
H	2648+13.31	-33.80	800.64	800.74
J	2648+23.37	-33.80	800.66	800.75
K	2648+33.43	-33.78	800.67	800.75
L	2648+43.49	-33.75	800.67	800.72
☒ Brg. Pier 2	2648+56.40	-33.68	800.67	800.69
M	2648+66.46	-33.60	800.67	800.70
N	2648+76.51	-33.51	800.67	800.70
P	2648+86.57	-33.40	800.66	800.68
R	2648+96.63	-33.27	800.65	800.67
☒ Brg. N. Abutment	2649+04.49	-33.16	800.64	800.66
Bk. N. Abutment	2649+06.46	-33.13	800.63	800.65

## SOUTHBOUND STAGE CONSTRUCTION

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+23.83	-30.92	800.46	800.48
└ Brg. S. Abutment	2647+25.80	-30.92	800.47	800.49
A	2647+35.92	-30.92	800.51	800.53
B	2647+46.02	-30.92	800.55	800.57
C	2647+56.12	-30.92	800.59	800.61
D	2647+66.21	-30.92	800.63	800.65
└ Brg. Pier 1	2647+74.10	-30.92	800.65	800.67
E	2647+84.18	-30.92	800.68	800.72
F	2647+94.25	-30.92	800.70	800.77
G	2648+04.32	-30.92	800.73	800.82
H	2648+14.38	-30.92	800.74	800.84
J	2648+24.43	-30.92	800.76	800.85
K	2648+34.48	-30.92	800.76	800.84
L	2648+44.52	-30.92	800.77	800.82
└ Brg. Pier 2	2648+57.40	-30.92	800.77	800.79
M	2648+67.43	-30.92	800.76	800.79
N	2648+77.45	-30.92	800.75	800.78
P	2648+87.46	-30.92	800.74	800.76
R	2648+97.47	-30.92	800.73	800.75
└ Brg. N. Abutment	2649+05.29	-30.92	800.71	800.73
Bk. N. Abutment	2649+07.24	-30.92	800.71	800.73

*BEAM 7*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+26.01	-25.30	800.66	800.68
☒ Brg. S. Abutment	2647+27.97	-25.33	800.67	800.69
A	2647+38.02	-25.48	800.71	800.73
B	2647+48.06	-25.61	800.74	800.76
C	2647+58.11	-25.73	800.77	800.79
D	2647+68.15	-25.82	800.81	800.83
☒ Brg. Pier 1	2647+76.00	-25.89	800.83	800.85
E	2647+86.05	-25.95	800.85	800.89
F	2647+96.09	-26.00	800.88	800.95
G	2648+06.14	-26.03	800.89	800.98
H	2648+16.19	-26.05	800.91	801.01
J	2648+26.23	-26.04	800.92	801.01
K	2648+36.28	-26.02	800.93	801.01
L	2648+46.32	-25.98	800.94	800.99
☒ Brg. Pier 2	2648+59.21	-25.91	800.94	800.96
M	2648+69.26	-25.83	800.93	800.96
N	2648+79.30	-25.73	800.93	800.96
P	2648+89.35	-25.62	800.92	800.94
R	2648+99.39	-25.48	800.91	800.93
☒ Brg. N. Abutment	2649+07.24	-25.37	800.89	800.91
Bk. N. Abutment	2649+09.21	-25.34	800.89	800.91

*BEAM 8*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+29.00	-17.60	800.94	800.96
☐ Brg. S. Abutment	2647+30.96	-17.63	800.94	800.96
A	2647+40.99	-17.77	800.98	801.00
B	2647+51.02	-17.90	801.01	801.03
C	2647+61.05	-18.01	801.05	801.07
D	2647+71.08	-18.10	801.08	801.10
☐ Brg. Pier 1	2647+78.93	-18.16	801.10	801.12
E	2647+88.96	-18.22	801.12	801.16
F	2647+98.99	-18.26	801.15	801.22
G	2648+09.02	-18.29	801.16	801.25
H	2648+19.05	-18.30	801.18	801.28
J	2648+29.08	-18.29	801.19	801.28
K	2648+39.12	-18.26	801.20	801.28
L	2648+49.15	-18.22	801.20	801.25
☐ Brg. Pier 2	2648+62.02	-18.14	801.20	801.22
M	2648+72.05	-18.05	801.20	801.23
N	2648+82.08	-17.95	801.19	801.22
P	2648+92.12	-17.83	801.18	801.20
R	2649+02.15	-17.69	801.17	801.19
☐ Brg. N. Abutment	2649+09.99	-17.58	801.15	801.17
Bk. N. Abutment	2649+11.95	-17.54	801.15	801.17

*SOUTHBOUND PGL & EDGE OF ROADWAY*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+30.59	-13.50	801.08	801.10
☐ Brg. S. Abutment	2647+32.56	-13.50	801.09	801.11
A	2647+42.63	-13.50	801.13	801.15
B	2647+52.70	-13.50	801.17	801.19
C	2647+62.77	-13.50	801.21	801.23
D	2647+72.82	-13.50	801.24	801.26
☐ Brg. Pier 1	2647+80.68	-13.50	801.26	801.28
E	2647+90.73	-13.50	801.29	801.33
F	2648+00.77	-13.50	801.31	801.38
G	2648+10.80	-13.50	801.33	801.42
H	2648+20.82	-13.50	801.34	801.44
J	2648+30.84	-13.50	801.35	801.44
K	2648+40.86	-13.50	801.36	801.44
L	2648+50.86	-13.50	801.36	801.41
☐ Brg. Pier 2	2648+63.69	-13.50	801.36	801.38
M	2648+73.69	-13.50	801.35	801.38
N	2648+83.67	-13.50	801.34	801.37
P	2648+93.65	-13.50	801.32	801.34
R	2649+03.62	-13.50	801.31	801.33
☐ Brg. N. Abutment	2649+11.42	-13.50	801.29	801.31
Bk. N. Abutment	2649+13.36	-13.50	801.28	801.30

*BEAM 9*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+31.98	-9.89	801.21	801.23
☒ Brg. S. Abutment	2647+33.94	-9.92	801.22	801.24
A	2647+43.96	-10.06	801.25	801.27
B	2647+53.97	-10.18	801.29	801.31
C	2647+63.99	-10.29	801.32	801.34
D	2647+74.01	-10.37	801.35	801.37
☒ Brg. Pier 1	2647+81.84	-10.43	801.37	801.39
E	2647+91.86	-10.48	801.39	801.43
F	2648+01.87	-10.52	801.41	801.48
G	2648+11.89	-10.54	801.43	801.52
H	2648+21.91	-10.55	801.44	801.54
J	2648+31.93	-10.53	801.46	801.55
K	2648+41.95	-10.50	801.46	801.54
L	2648+51.97	-10.45	801.46	801.51
☒ Brg. Pier 2	2648+64.82	-10.36	801.46	801.48
M	2648+74.84	-10.27	801.46	801.49
N	2648+84.86	-10.17	801.45	801.48
P	2648+94.88	-10.04	801.44	801.46
R	2649+04.89	-9.90	801.43	801.45
☒ Brg. N. Abutment	2649+12.72	-9.78	801.41	801.43
Bk. N. Abutment	2649+14.68	-9.75	801.41	801.43

*BEAM 10*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+34.95	-2.18	801.49	801.51
☒ Brg. S. Abutment	2647+36.91	-2.21	801.49	801.51
A	2647+46.91	-2.35	801.53	801.55
B	2647+56.92	-2.46	801.56	801.58
C	2647+66.92	-2.56	801.59	801.61
D	2647+76.92	-2.64	801.62	801.64
☒ Brg. Pier 1	2647+84.74	-2.70	801.64	801.66
E	2647+94.75	-2.75	801.66	801.70
F	2648+04.75	-2.78	801.68	801.74
G	2648+14.76	-2.80	801.70	801.78
H	2648+24.76	-2.79	801.71	801.80
J	2648+34.77	-2.78	801.72	801.81
K	2648+44.77	-2.74	801.73	801.80
L	2648+54.78	-2.69	801.73	801.78
☒ Brg. Pier 2	2648+67.62	-2.59	801.73	801.75
M	2648+77.62	-2.50	801.72	801.75
N	2648+87.62	-2.39	801.71	801.73
P	2648+97.63	-2.26	801.70	801.72
R	2649+07.63	-2.11	801.68	801.70
☒ Brg. N. Abutment	2649+15.45	-1.98	801.67	801.69
Bk. N. Abutment	2649+17.40	-1.95	801.67	801.69

*SOUTHBOUND FACE OF MEDIAN PARAPET*

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+35.22	-1.50	800.62	800.64
☐ Brg. S. Abutment	2647+37.18	-1.50	800.63	800.65
A	2647+47.24	-1.50	800.67	800.69
B	2647+57.28	-1.50	800.71	800.73
C	2647+67.32	-1.50	800.74	800.76
D	2647+77.35	-1.50	800.77	800.79
☐ Brg. Pier 1	2647+85.19	-1.50	800.79	800.81
E	2647+95.21	-1.50	800.82	800.86
F	2648+05.23	-1.50	800.84	800.90
G	2648+15.24	-1.50	800.85	800.93
H	2648+25.24	-1.50	800.87	800.96
J	2648+35.23	-1.50	800.88	800.97
K	2648+45.22	-1.50	800.88	800.95
L	2648+55.21	-1.50	800.88	800.93
☐ Brg. Pier 2	2648+68.01	-1.50	800.87	800.89
M	2648+77.98	-1.50	800.87	800.90
N	2648+87.94	-1.50	800.85	800.87
P	2648+97.89	-1.50	800.84	800.86
R	2649+07.85	-1.50	800.82	800.84
☐ Brg. N. Abutment	2649+15.62	-1.50	800.80	800.82
Bk. N. Abutment	2649+17.56	-1.50	800.79	800.81

MODEL - SHEET  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\10173871\10264C24\_SHT\_0208-0209-CN\_DECK-SLAB-ELEV-5.dgn

**Bowman**  
10 S. LaSalle Street, Suite 210  
Chicago, Illinois 60603  
312-455-0360  
www.bowman.com

NORTHBOUND FACE OF MEDIAN PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+36.37	1.50	800.63	800.65
☒ Brg. S. Abutment	2647+38.34	1.50	800.63	800.65
A	2647+48.38	1.50	800.67	800.69
B	2647+58.42	1.50	800.71	800.73
C	2647+68.46	1.50	800.75	800.77
D	2647+78.48	1.50	800.77	800.79
☒ Brg. Pier 1	2647+86.32	1.50	800.79	800.81
E	2647+96.33	1.50	800.82	800.86
F	2648+06.34	1.50	800.84	800.90
G	2648+16.34	1.50	800.86	800.94
H	2648+26.34	1.50	800.87	800.96
J	2648+36.33	1.50	800.88	800.97
K	2648+46.31	1.50	800.88	800.95
L	2648+56.29	1.50	800.88	800.93
☒ Brg. Pier 2	2648+69.08	1.50	800.87	800.89
M	2648+79.05	1.50	800.87	800.90
N	2648+89.00	1.50	800.85	800.87
P	2648+98.95	1.50	800.84	800.86
R	2649+08.90	1.50	800.81	800.83
☒ Brg. N. Abutment	2649+16.67	1.50	800.80	800.82
Bk. N. Abutment	2649+18.61	1.50	800.79	800.81

NORTHBOUND PGL & EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+40.98	13.50	801.12	801.14
☒ Brg. S. Abutment	2647+42.94	13.50	801.13	801.15
A	2647+52.96	13.50	801.17	801.19
B	2647+62.98	13.50	801.21	801.23
C	2647+72.99	13.50	801.24	801.26
D	2647+82.99	13.50	801.27	801.29
☒ Brg. Pier 1	2647+90.80	13.50	801.29	801.31
E	2648+00.79	13.50	801.31	801.35
F	2648+10.78	13.50	801.33	801.40
G	2648+20.76	13.50	801.34	801.43
H	2648+30.73	13.50	801.35	801.45
J	2648+40.70	13.50	801.36	801.45
K	2648+50.66	13.50	801.36	801.44
L	2648+60.61	13.50	801.36	801.41
☒ Brg. Pier 2	2648+73.37	13.50	801.35	801.37
M	2648+83.31	13.50	801.34	801.37
N	2648+93.24	13.50	801.32	801.35
P	2649+03.17	13.50	801.31	801.33
R	2649+13.09	13.50	801.28	801.30
☒ Brg. N. Abutment	2649+20.84	13.50	801.26	801.28
Bk. N. Abutment	2649+22.78	13.50	801.26	801.28

BEAM 11

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+36.69	2.33	800.73	800.75
☒ Brg. S. Abutment	2647+38.65	2.31	800.73	800.75
A	2647+48.64	2.17	800.77	800.79
B	2647+58.64	2.06	800.80	800.82
C	2647+68.63	1.96	800.83	800.85
D	2647+78.63	1.89	800.86	800.88
☒ Brg. Pier 1	2647+86.44	1.84	800.88	800.90
E	2647+96.44	1.79	800.90	800.94
F	2648+06.44	1.76	800.92	800.98
G	2648+16.43	1.74	800.94	801.02
H	2648+26.43	1.75	800.95	801.04
J	2648+36.43	1.77	800.96	801.05
K	2648+46.42	1.81	800.96	801.03
L	2648+56.42	1.87	800.96	801.01
☒ Brg. Pier 2	2648+69.25	1.97	800.96	800.98
M	2648+79.25	2.06	800.96	800.99
N	2648+89.24	2.18	800.95	800.97
P	2648+99.24	2.31	800.93	800.95
R	2649+09.23	2.46	800.92	800.94
☒ Brg. N. Abutment	2649+17.04	2.59	800.90	800.92
Bk. N. Abutment	2649+19.00	2.62	800.90	800.92

BEAM 13

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+42.60	17.75	801.28	801.30
☒ Brg. S. Abutment	2647+44.55	17.73	801.28	801.30
A	2647+54.52	17.61	801.32	801.34
B	2647+64.49	17.50	801.35	801.37
C	2647+74.46	17.42	801.38	801.40
D	2647+84.43	17.35	801.40	801.42
☒ Brg. Pier 1	2647+92.22	17.31	801.42	801.44
E	2648+02.19	17.27	801.44	801.48
F	2648+12.16	17.25	801.46	801.53
G	2648+22.13	17.24	801.47	801.56
H	2648+32.10	17.26	801.48	801.58
J	2648+42.07	17.29	801.49	801.58
K	2648+52.04	17.34	801.49	801.57
L	2648+62.01	17.41	801.49	801.54
☒ Brg. Pier 2	2648+74.80	17.52	801.49	801.51
M	2648+84.77	17.62	801.48	801.51
N	2648+94.74	17.75	801.47	801.50
P	2649+04.71	17.89	801.45	801.47
R	2649+14.68	18.05	801.43	801.45
☒ Brg. N. Abutment	2649+22.47	18.18	801.42	801.44
Bk. N. Abutment	2649+24.42	18.22	801.41	801.43

BEAM 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+39.65	10.04	801.00	801.02
☒ Brg. S. Abutment	2647+41.60	10.02	801.01	801.03
A	2647+51.58	9.89	801.04	801.06
B	2647+61.57	9.78	801.08	801.10
C	2647+71.55	9.69	801.10	801.12
D	2647+81.53	9.62	801.13	801.15
☒ Brg. Pier 1	2647+89.34	9.57	801.15	801.17
E	2647+99.32	9.53	801.17	801.21
F	2648+09.30	9.50	801.19	801.26
G	2648+19.29	9.49	801.20	801.29
H	2648+29.27	9.50	801.22	801.32
J	2648+39.25	9.53	801.22	801.31
K	2648+49.24	9.57	801.23	801.31
L	2648+59.22	9.64	801.23	801.28
☒ Brg. Pier 2	2648+72.03	9.74	801.22	801.24
M	2648+82.01	9.84	801.22	801.25
N	2648+92.00	9.96	801.21	801.24
P	2649+01.98	10.10	801.19	801.21
R	2649+11.96	10.25	801.18	801.20
☒ Brg. N. Abutment	2649+19.76	10.38	801.16	801.18
Bk. N. Abutment	2649+21.71	10.42	801.16	801.18

BEAM 14

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+45.55	25.47	801.55	801.57
☒ Brg. S. Abutment	2647+47.49	25.44	801.56	801.58
A	2647+57.45	25.32	801.59	801.61
B	2647+67.40	25.23	801.62	801.64
C	2647+77.36	25.14	801.65	801.67
D	2647+87.32	25.08	801.67	801.69
☒ Brg. Pier 1	2647+95.10	25.04	801.69	801.71
E	2648+05.06	25.01	801.71	801.75
F	2648+15.01	25.00	801.72	801.79
G	2648+24.97	25.00	801.74	801.83
H	2648+34.93	25.02	801.75	801.85
J	2648+44.88	25.05	801.75	801.84
K	2648+54.84	25.11	801.75	801.83
L	2648+64.79	25.18	801.75	801.80
☒ Brg. Pier 2	2648+77.57	25.30	801.75	801.77
M	2648+87.53	25.41	801.74	801.77
N	2648+97.48	25.54	801.73	801.76
P	2649+07.44	25.68	801.71	801.73
R	2649+17.39	25.84	801.69	801.71
☒ Brg. N. Abutment	2649+25.17	25.98	801.68	801.70
Bk. N. Abutment	2649+27.11	26.02	801.67	801.69



MODEL - SHEET  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\10173871\10264C24\_SHT\_0208-0209-CN\_DECK-SLAB-ELEV-6.dgn

**Bowman**

10 S. LaSalle Street, Suite 210  
Chicago, Illinois 60603  
312-455-0360  
www.bowman.com

USER NAME = Ifranceschina	DESIGNED - JW	REVISED -
	DRAWN - DSO	REVISED -
PLOT SCALE = \$SCALES\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

SLAB ELEVATIONS 6  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 19 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	562
CONTRACT NO. 64C24				
		ILLINOIS	FED. AID PROJECT	

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

BEAM 15

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+48.49	33.18	801.82	801.84
☒ Brg. S. Abutment	2647+50.43	33.16	801.83	801.85
A	2647+60.37	33.04	801.86	801.88
B	2647+70.31	32.95	801.89	801.91
C	2647+80.25	32.87	801.92	801.94
D	2647+90.20	32.82	801.94	801.96
☒ Brg. Pier 1	2647+97.97	32.78	801.96	801.98
E	2648+07.91	32.75	801.98	802.02
F	2648+17.86	32.74	801.99	802.06
G	2648+27.80	32.75	802.01	802.10
H	2648+37.74	32.78	802.01	802.11
J	2648+47.69	32.82	802.02	802.11
K	2648+57.63	32.88	802.02	802.10
L	2648+67.57	32.95	802.02	802.07
☒ Brg. Pier 2	2648+80.33	33.08	802.01	802.03
M	2648+90.27	33.19	802.00	802.03
N	2649+00.21	33.32	801.99	802.02
P	2649+10.15	33.48	801.97	801.99
R	2649+20.09	33.64	801.95	801.97
☒ Brg. N. Abutment	2649+27.86	33.79	801.93	801.95
Bk. N. Abutment	2649+29.81	33.82	801.93	801.95

NORTHBOUND STAGE CONSTRUCTION

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+49.11	34.83	801.88	801.90
☒ Brg. S. Abutment	2647+51.06	34.83	801.89	801.91
A	2647+61.04	34.83	801.93	801.95
B	2647+71.02	34.83	801.96	801.98
C	2647+80.99	34.83	801.99	802.01
D	2647+90.95	34.83	802.01	802.03
☒ Brg. Pier 1	2647+98.73	34.83	802.03	802.05
E	2648+08.68	34.83	802.05	802.09
F	2648+18.62	34.83	802.06	802.13
G	2648+28.56	34.83	802.08	802.17
H	2648+38.49	34.83	802.08	802.18
J	2648+48.41	34.83	802.09	802.18
K	2648+58.33	34.83	802.09	802.17
L	2648+68.24	34.83	802.08	802.13
☒ Brg. Pier 2	2648+80.95	34.83	802.07	802.09
M	2648+90.85	34.83	802.05	802.08
N	2649+00.74	34.83	802.04	802.07
P	2649+10.63	34.83	802.01	802.03
R	2649+20.51	34.83	801.99	802.01
☒ Brg. N. Abutment	2649+28.23	34.83	801.97	801.99
Bk. N. Abutment	2649+30.15	34.83	801.96	801.98

BEAM 16

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+51.41	40.89	802.10	802.12
☒ Brg. S. Abutment	2647+53.35	40.87	802.10	802.12
A	2647+63.28	40.77	802.14	802.16
B	2647+73.21	40.68	802.16	802.18
C	2647+83.14	40.61	802.19	802.21
D	2647+93.07	40.55	802.21	802.23
☒ Brg. Pier 1	2648+00.83	40.52	802.23	802.25
E	2648+10.76	40.50	802.25	802.29
F	2648+20.69	40.49	802.26	802.33
G	2648+30.62	40.51	802.27	802.36
H	2648+40.55	40.54	802.28	802.38
J	2648+50.48	40.58	802.28	802.37
K	2648+60.41	40.65	802.28	802.36
L	2648+70.34	40.73	802.28	802.33
☒ Brg. Pier 2	2648+83.08	40.86	802.27	802.29
M	2648+93.01	40.98	802.26	802.29
N	2649+02.94	41.12	802.25	802.28
P	2649+12.86	41.27	802.23	802.25
R	2649+22.79	41.44	802.21	802.23
☒ Brg. N. Abutment	2649+30.55	41.59	802.19	802.21
Bk. N. Abutment	2649+32.49	41.63	802.19	802.21

BEAM 17

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+54.33	48.61	802.37	802.39
☒ Brg. S. Abutment	2647+56.27	48.59	802.38	802.40
A	2647+66.19	48.49	802.41	802.43
B	2647+76.10	48.40	802.43	802.45
C	2647+86.02	48.34	802.46	802.48
D	2647+95.93	48.29	802.48	802.50
☒ Brg. Pier 1	2648+03.69	48.26	802.50	802.52
E	2648+13.60	48.25	802.51	802.55
F	2648+23.52	48.25	802.53	802.60
G	2648+33.44	48.26	802.54	802.63
H	2648+43.35	48.30	802.54	802.64
J	2648+53.27	48.35	802.54	802.63
K	2648+63.18	48.42	802.54	802.62
L	2648+73.10	48.50	802.54	802.59
☒ Brg. Pier 2	2648+85.83	48.64	802.53	802.55
M	2648+95.74	48.76	802.52	802.55
N	2649+05.65	48.91	802.50	802.53
P	2649+15.57	49.07	802.49	802.51
R	2649+25.48	49.24	802.47	802.49
☒ Brg. N. Abutment	2649+33.23	49.39	802.45	802.47
Bk. N. Abutment	2649+35.17	49.43	802.44	802.46

BEAM 18

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+57.25	56.33	802.64	802.66
☒ Brg. S. Abutment	2647+59.18	56.31	802.65	802.67
A	2647+69.08	56.21	802.68	802.70
B	2647+78.99	56.13	802.70	802.72
C	2647+88.89	56.07	802.73	802.75
D	2647+98.79	56.03	802.75	802.77
☒ Brg. Pier 1	2648+06.53	56.01	802.77	802.79
E	2648+16.44	55.99	802.78	802.82
F	2648+26.34	56.00	802.79	802.86
G	2648+36.24	56.02	802.80	802.89
H	2648+46.15	56.06	802.81	802.91
J	2648+56.05	56.12	802.81	802.90
K	2648+65.95	56.19	802.81	802.89
L	2648+75.85	56.28	802.80	802.85
☒ Brg. Pier 2	2648+88.56	56.42	802.79	802.81
M	2648+98.46	56.55	802.78	802.81
N	2649+08.36	56.70	802.76	802.79
P	2649+18.26	56.86	802.75	802.77
R	2649+28.16	57.05	802.72	802.74
☒ Brg. N. Abutment	2649+35.90	57.20	802.70	802.72
Bk. N. Abutment	2649+37.84	57.24	802.70	802.72

NORTHBOUND EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+59.19	61.50	802.83	802.85
☒ Brg. S. Abutment	2647+61.13	61.50	802.83	802.85
A	2647+71.06	61.50	802.87	802.89
B	2647+80.98	61.50	802.89	802.91
C	2647+90.90	61.50	802.92	802.94
D	2648+00.81	61.50	802.94	802.96
☒ Brg. Pier 1	2648+08.55	61.50	802.96	802.98
E	2648+18.44	61.50	802.97	803.01
F	2648+28.34	61.50	802.98	803.05
G	2648+38.22	61.50	802.99	803.08
H	2648+48.10	61.50	802.99	803.09
J	2648+57.97	61.50	802.99	803.08
K	2648+67.84	61.50	802.99	803.07
L	2648+77.70	61.50	802.98	803.03
☒ Brg. Pier 2	2648+90.34	61.50	802.96	802.98
M	2649+00.19	61.50	802.95	802.98
N	2649+10.03	61.50	802.92	802.95
P	2649+19.86	61.50	802.90	802.92
R	2649+29.69	61.50	802.87	802.89
☒ Brg. N. Abutment	2649+37.37	61.50	802.85	802.87
Bk. N. Abutment	2649+39.29	61.50	802.84	802.86

MODEL: SHEET  
FILE NAME: c:\pwwork\benesch\_projects\projects\0173871\0264C24\_SHT\_0208-0209-CN\_DECK-SLAB-ELEV-L7.dgn

**Bowman**

10 S. LaSalle Street, Suite 210  
Chicago, Illinois 60603  
312-455-0300  
www.bowman.com

USER NAME = Ifranceschina	DESIGNED - JW	REVISED -
	DRAWN - DSO	REVISED -
PLOT SCALE = \$SCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

SLAB ELEVATIONS 7  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 20 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	563
CONTRACT NO. 64C24				
		ILLINOIS	FED. AID PROJECT	

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

BEAM 19

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+60.15	64.05	802.92	802.94
⌀ Brg. S. Abutment	2647+62.08	64.03	802.92	802.94
A	2647+71.97	63.94	802.95	802.97
B	2647+81.86	63.86	802.97	802.99
C	2647+91.75	63.81	803.00	803.02
D	2648+01.64	63.77	803.02	803.04
⌀ Brg. Pier 1	2648+09.37	63.75	803.03	803.05
E	2648+19.26	63.74	803.05	803.09
F	2648+29.15	63.75	803.06	803.13
G	2648+39.04	63.78	803.07	803.16
H	2648+48.93	63.82	803.07	803.17
J	2648+58.82	63.88	803.07	803.16
K	2648+68.71	63.96	803.07	803.15
L	2648+78.60	64.06	803.07	803.12
⌀ Brg. Pier 2	2648+91.29	64.21	803.05	803.07
M	2649+01.18	64.34	803.04	803.07
N	2649+11.07	64.49	803.02	803.05
P	2649+20.95	64.66	803.00	803.02
R	2649+30.84	64.85	802.98	803.00
⌀ Brg. N. Abutment	2649+38.57	65.01	802.96	802.98
Bk. N. Abutment	2649+40.50	65.05	802.96	802.98

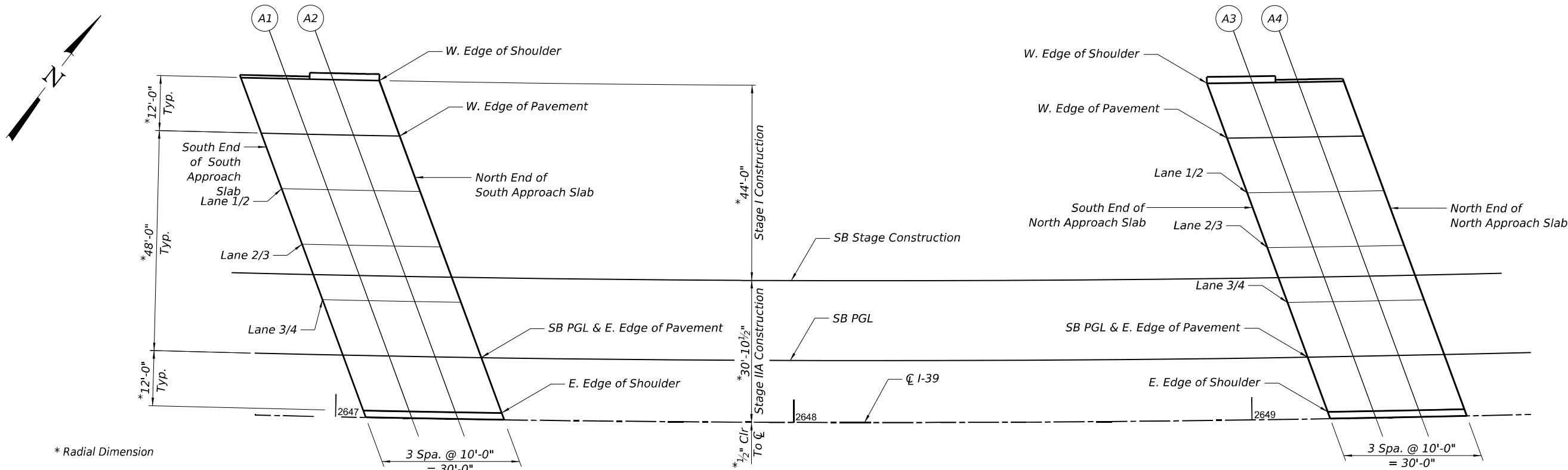
BEAM 20

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+63.05	71.77	803.19	803.21
⌀ Brg. S. Abutment	2647+64.98	71.75	803.20	803.22
A	2647+74.85	71.67	803.22	803.24
B	2647+84.73	71.60	803.25	803.27
C	2647+94.61	71.55	803.27	803.29
D	2648+04.48	71.51	803.29	803.31
⌀ Brg. Pier 1	2648+12.20	71.50	803.30	803.32
E	2648+22.08	71.49	803.32	803.36
F	2648+31.96	71.51	803.33	803.39
G	2648+41.83	71.54	803.33	803.41
H	2648+51.71	71.59	803.34	803.43
J	2648+61.59	71.66	803.34	803.43
K	2648+71.46	71.74	803.33	803.40
L	2648+81.34	71.84	803.33	803.38
⌀ Brg. Pier 2	2648+94.01	71.99	803.31	803.33
M	2649+03.89	72.13	803.30	803.33
N	2649+13.76	72.29	803.28	803.30
P	2649+23.64	72.46	803.26	803.28
R	2649+33.51	72.66	803.24	803.26
⌀ Brg. N. Abutment	2649+41.23	72.82	803.22	803.24
Bk. N. Abutment	2649+43.16	72.86	803.21	803.23

NORTHBOUND FACE OF PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abutment	2647+63.70	73.50	802.36	802.38
⌀ Brg. S. Abutment	2647+65.63	73.50	802.37	802.39
A	2647+75.54	73.50	802.40	802.42
B	2647+85.44	73.50	802.42	802.44
C	2647+95.33	73.50	802.45	802.47
D	2648+05.21	73.50	802.47	802.49
⌀ Brg. Pier 1	2648+12.94	73.50	802.48	802.50
E	2648+22.81	73.50	802.50	802.54
F	2648+32.68	73.50	802.51	802.57
G	2648+42.54	73.50	802.51	802.59
H	2648+52.39	73.50	802.51	802.60
J	2648+62.24	73.50	802.51	802.60
K	2648+72.09	73.50	802.50	802.57
L	2648+81.92	73.50	802.49	802.54
⌀ Brg. Pier 2	2648+94.54	73.50	802.47	802.49
M	2649+04.36	73.50	802.46	802.49
N	2649+14.18	73.50	802.43	802.45
P	2649+23.99	73.50	802.41	802.43
R	2649+33.80	73.50	802.38	802.40
⌀ Brg. N. Abutment	2649+41.46	73.50	802.35	802.37
Bk. N. Abutment	2649+43.37	73.50	802.34	802.36

MODEL: SHEET  
FILE NAME: c:\pwworkdir\benesch\_projects\10173871\10264C24\_SHT\_0208-0209-CN\_APPR-SLAB-ELEV-SB.dgn



**SB APPROACH SLABS PLAN**

**WEST EDGE OF SHOULDER**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2646+77.49	-73.50	798.83	798.85
A1	2646+87.71	-73.50	798.87	798.89
A2	2646+97.93	-73.50	798.91	798.93
N. End of S. Appr. Slab	2647+08.13	-73.50	798.95	798.97
S. End of N. Appr. Slab	2648+91.09	-73.50	799.29	799.31
A3	2649+01.17	-73.50	799.27	799.29
A4	2649+11.24	-73.50	799.25	799.27
N. End of N. Appr. Slab	2649+21.31	-73.50	799.22	799.24

**LANE 1/2**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2646+87.09	-49.50	799.69	799.71
A1	2646+97.26	-49.50	799.73	799.75
A2	2647+07.42	-49.50	799.77	799.79
N. End of S. Appr. Slab	2647+17.58	-49.50	799.81	799.83
S. End of N. Appr. Slab	2648+99.66	-49.50	800.09	800.11
A3	2649+09.69	-49.50	800.07	800.09
A4	2649+19.72	-49.50	800.04	800.06
N. End of N. Appr. Slab	2649+29.74	-49.50	800.01	800.03

**SB STAGE CONSTRUCTION**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2646+94.46	-30.92	800.35	800.37
A1	2647+04.59	-30.92	800.39	800.41
A2	2647+14.72	-30.92	800.43	800.45
N. End of S. Appr. Slab	2647+24.84	-30.92	800.47	800.49
S. End of N. Appr. Slab	2649+06.24	-30.92	800.71	800.73
A3	2649+16.24	-30.92	800.68	800.70
A4	2649+26.23	-30.92	800.66	800.68
N. End of N. Appr. Slab	2649+36.21	-30.92	800.63	800.65

**SB PGL & EAST EDGE OF PAVEMENT**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+01.32	-13.50	800.96	800.98
A1	2647+11.42	-13.50	801.01	801.03
A2	2647+21.51	-13.50	801.04	801.06
N. End of S. Appr. Slab	2647+31.59	-13.50	801.08	801.10
S. End of N. Appr. Slab	2649+12.37	-13.50	801.28	801.30
A3	2649+22.33	-13.50	801.26	801.28
A4	2649+32.29	-13.50	801.23	801.25
N. End of N. Appr. Slab	2649+42.24	-13.50	801.20	801.22

**WEST EDGE OF PAVEMENT**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2646+82.30	-61.50	799.26	799.28
A1	2646+92.50	-61.50	799.30	799.32
A2	2647+02.69	-61.50	799.34	799.36
N. End of S. Appr. Slab	2647+12.87	-61.50	799.38	799.40
S. End of N. Appr. Slab	2648+95.38	-61.50	799.69	799.71
A3	2649+05.44	-61.50	799.67	799.69
A4	2649+15.49	-61.50	799.65	799.67
N. End of N. Appr. Slab	2649+25.53	-61.50	799.62	799.64

**LANE 2/3**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2646+91.85	-37.50	800.11	800.13
A1	2647+02.00	-37.50	800.15	800.17
A2	2647+12.14	-37.50	800.19	800.21
N. End of S. Appr. Slab	2647+22.27	-37.50	800.23	800.25
S. End of N. Appr. Slab	2649+03.91	-37.50	800.49	800.51
A3	2649+13.92	-37.50	800.47	800.49
A4	2649+23.93	-37.50	800.44	800.46
N. End of N. Appr. Slab	2649+33.92	-37.50	800.41	800.43

**LANE 3/4**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2646+96.60	-25.50	800.54	800.56
A1	2647+06.72	-25.50	800.58	800.60
A2	2647+16.84	-25.50	800.62	800.64
N. End of S. Appr. Slab	2647+26.95	-25.50	800.66	800.68
S. End of N. Appr. Slab	2649+08.15	-25.50	800.89	800.91
A3	2649+18.14	-25.50	800.86	800.88
A4	2649+28.11	-25.50	800.83	800.85
N. End of N. Appr. Slab	2649+38.09	-25.50	800.80	800.82

**EAST EDGE OF SHOULDER**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+06.04	-1.46	801.39	801.41
A1	2647+16.11	-1.46	801.43	801.45
A2	2647+26.18	-1.46	801.48	801.50
N. End of S. Appr. Slab	2647+36.24	-1.46	801.52	801.54
S. End of N. Appr. Slab	2649+16.58	-1.46	801.68	801.70
A3	2649+26.52	-1.46	801.66	801.68
A4	2649+36.45	-1.46	801.63	801.65
N. End of N. Appr. Slab	2649+46.38	-1.46	801.59	801.61

**Bowman**

10 S. LaSalle Street, Suite 210  
Chicago, Illinois 60603  
312-455-0350  
www.bowman.com

USER NAME = Ifranceschina	DESIGNED - JW	REVISED -
	DRAWN - DSO	REVISED -
PLOT SCALE = SSCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

**STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION**

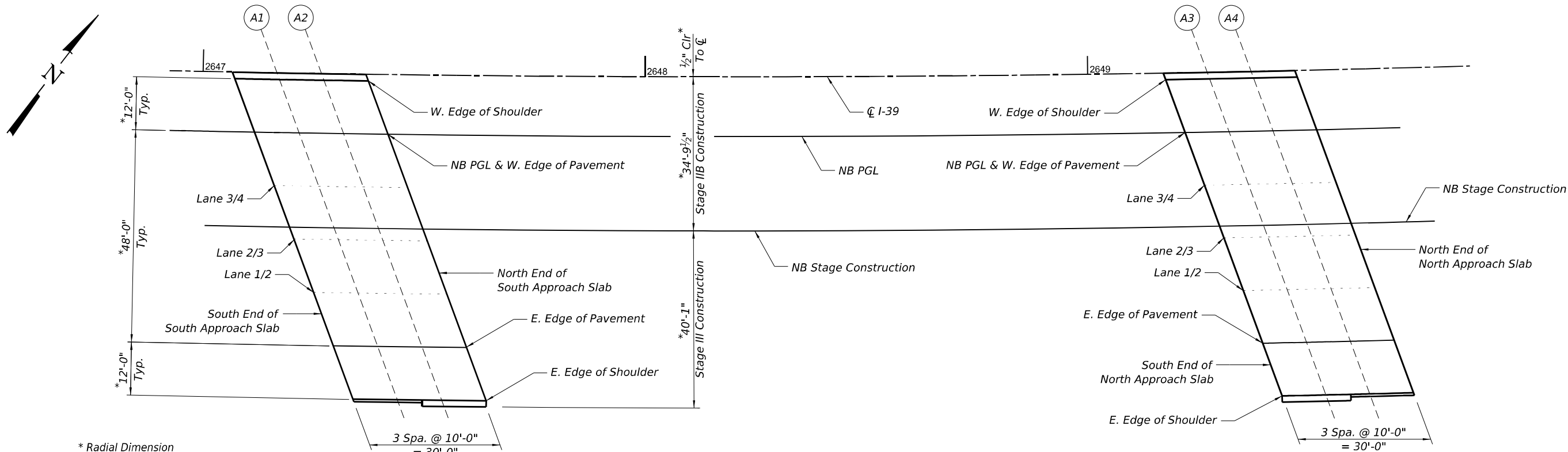
**APPROACH SLAB ELEVATIONS - SOUTHBOUND  
STRUCTURE NO. 101-0208/0209**

SCALE: SHEET 21 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	564
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

MODEL: SHEET  
FILE NAME: c:\pwworkdir\benesch projects\projects\101-0208-0209-CN\_APPR-SLAB-ELEV-NB.dgn



NB APPROACH SLABS PLAN

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+07.18	1.46	800.58	800.60
A1	2647+17.25	1.46	800.62	800.64
A2	2647+27.31	1.46	800.66	800.68
N. End of S. Appr. Slab	2647+37.36	1.46	800.70	800.72
S. End of N. Appr. Slab	2649+17.60	1.46	800.86	800.88
A3	2649+27.53	1.46	800.83	800.85
A4	2649+37.46	1.46	800.80	800.82
N. End of N. Appr. Slab	2649+47.38	1.46	800.77	800.79

LANE 3/4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+11.87	25.50	801.43	801.45
A1	2647+21.91	25.50	801.48	801.50
A2	2647+31.95	25.50	801.52	801.54
N. End of S. Appr. Slab	2647+41.98	25.50	801.55	801.57
S. End of N. Appr. Slab	2649+21.79	25.50	801.66	801.68
A3	2649+31.70	25.50	801.63	801.65
A4	2649+41.60	25.50	801.59	801.61
N. End of N. Appr. Slab	2649+51.50	25.50	801.55	801.57

LANE 2/3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+21.16	37.50	801.86	801.88
A1	2647+31.15	37.50	801.90	801.92
A2	2647+41.14	37.50	801.94	801.96
N. End of S. Appr. Slab	2647+51.12	37.50	801.98	802.00
S. End of N. Appr. Slab	2649+30.08	37.50	802.05	802.07
A3	2649+39.95	37.50	802.02	802.04
A4	2649+49.81	37.50	801.98	802.00
N. End of N. Appr. Slab	2649+59.66	37.50	801.94	801.96

EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+30.36	61.50	802.71	802.73
A1	2647+40.31	61.50	802.75	802.77
A2	2647+50.25	61.50	802.79	802.81
N. End of S. Appr. Slab	2647+60.19	61.50	802.83	802.85
S. End of N. Appr. Slab	2649+38.31	61.50	802.84	802.86
A3	2649+48.13	61.50	802.80	802.82
A4	2649+57.94	61.50	802.77	802.79
N. End of N. Appr. Slab	2649+67.74	61.50	802.72	802.74

NB PGL & WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+11.87	13.50	801.01	801.03
A1	2647+21.91	13.50	801.05	801.07
A2	2647+31.95	13.50	801.09	801.11
N. End of S. Appr. Slab	2647+41.98	13.50	801.13	801.15
S. End of N. Appr. Slab	2649+21.79	13.50	801.26	801.28
A3	2649+31.70	13.50	801.23	801.25
A4	2649+41.60	13.50	801.20	801.22
N. End of N. Appr. Slab	2649+51.50	13.50	801.16	801.18

NB STAGE CONSTRUCTION

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+20.13	34.83	801.77	801.79
A1	2647+30.13	34.83	801.81	801.83
A2	2647+40.12	34.83	801.85	801.87
N. End of S. Appr. Slab	2647+50.11	34.83	801.89	801.91
S. End of N. Appr. Slab	2649+29.17	34.83	801.96	801.98
A3	2649+39.04	34.83	801.93	801.95
A4	2649+48.90	34.83	801.90	801.92
N. End of N. Appr. Slab	2649+58.75	34.83	801.86	801.88

LANE 1/2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+25.77	49.50	802.29	802.31
A1	2647+35.74	49.50	802.33	802.35
A2	2647+45.71	49.50	802.37	802.39
N. End of S. Appr. Slab	2647+55.67	49.50	802.40	802.42
S. End of N. Appr. Slab	2649+34.21	49.50	802.45	802.47
A3	2649+44.05	49.50	802.41	802.43
A4	2649+53.88	49.50	802.37	802.39
N. End of N. Appr. Slab	2649+63.71	49.50	802.33	802.35

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
S. End of S. Appr. Slab	2647+34.93	73.50	803.14	803.16
A1	2647+44.86	73.50	803.18	803.20
A2	2647+54.78	73.50	803.22	803.24
N. End of S. Appr. Slab	2647+64.69	73.50	803.25	803.27
S. End of N. Appr. Slab	2649+42.39	73.50	803.23	803.25
A3	2649+52.19	73.50	803.20	803.22
A4	2649+61.98	73.50	803.16	803.18
N. End of N. Appr. Slab	2649+71.76	73.50	803.11	803.13

Bowman

10 S. LaSalle Street, Suite 210  
Chicago, Illinois 60603  
312-455-0350  
www.bowman.com

USER NAME = Ifranceschina  
DESIGNED - JW  
DRAWN - DSO  
PLOT SCALE = SSCALE\$  
PLOT DATE = 8/12/2024

REVISD -  
REVISD -  
REVISD -  
REVISD -

DATE - 02/27/24

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

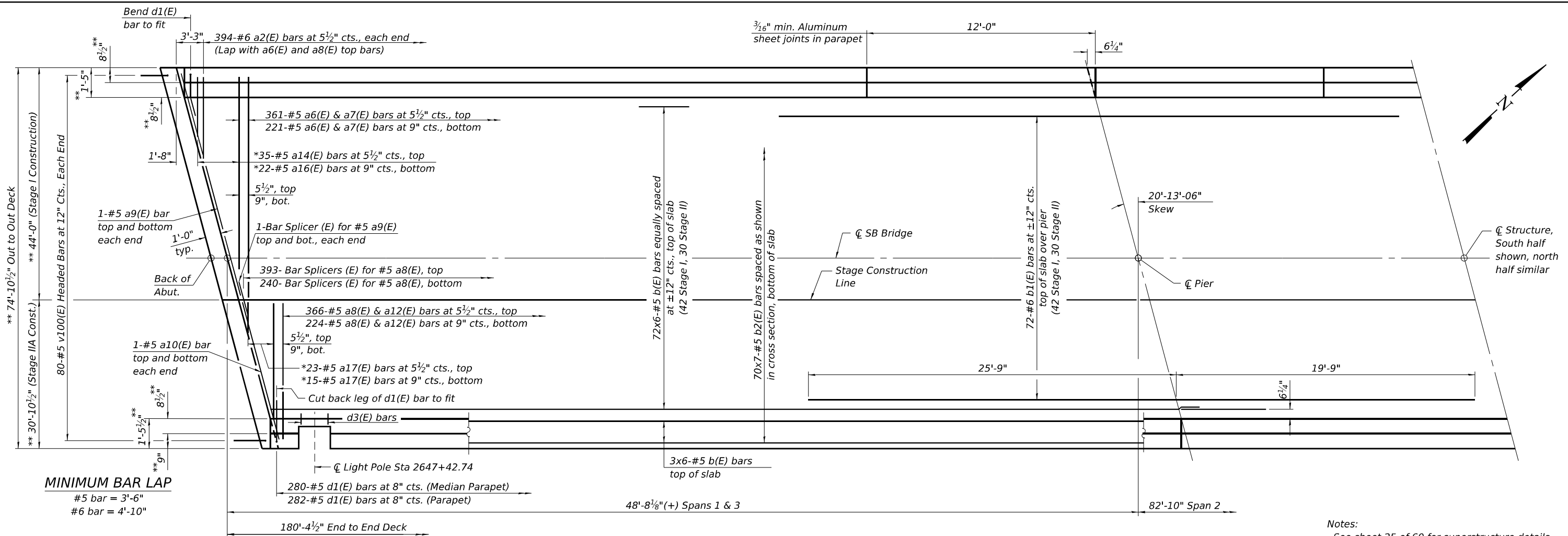
APPROACH SLAB ELEVATIONS - NORTHBOUND  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 22 OF 60 SHEETS STA. TO STA.

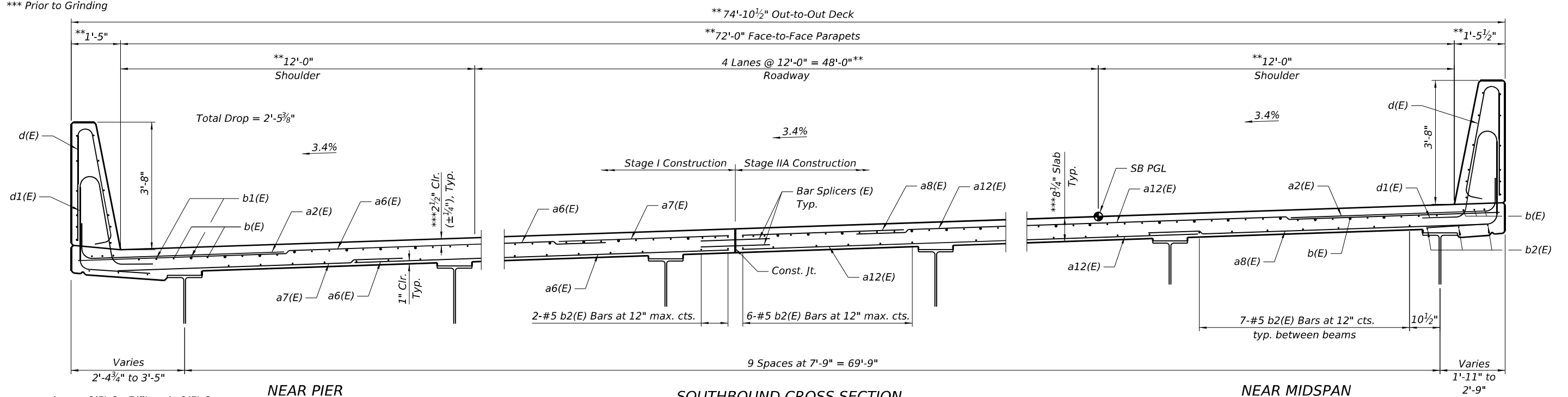
F.A.I. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO.  
\* (201-3)R & (4-1.5)R WINNEBAGO 1685 565  
CONTRACT NO. 64C24  
ILLINOIS FED. AID PROJECT

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*





Notes:  
See sheet 25 of 60 for superstructure details and Bill of Material.  
Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.



Note:  
All transverse bars, a6(E) & a7(E) and a8(E) & a12(E), shall be oriented with top bar laps opposite of bottom bar laps, as shown in cross-section.

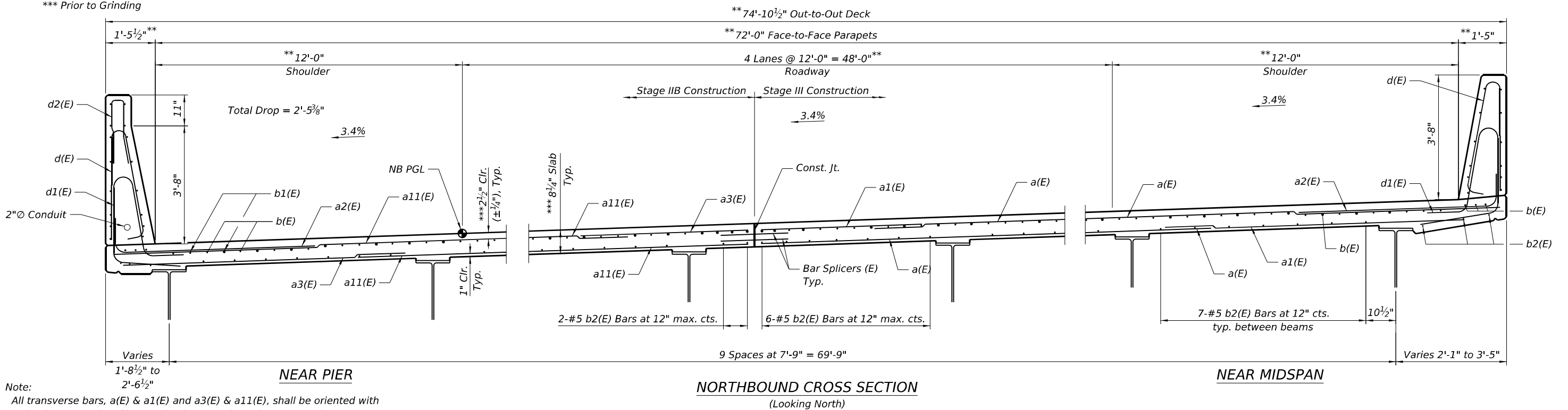
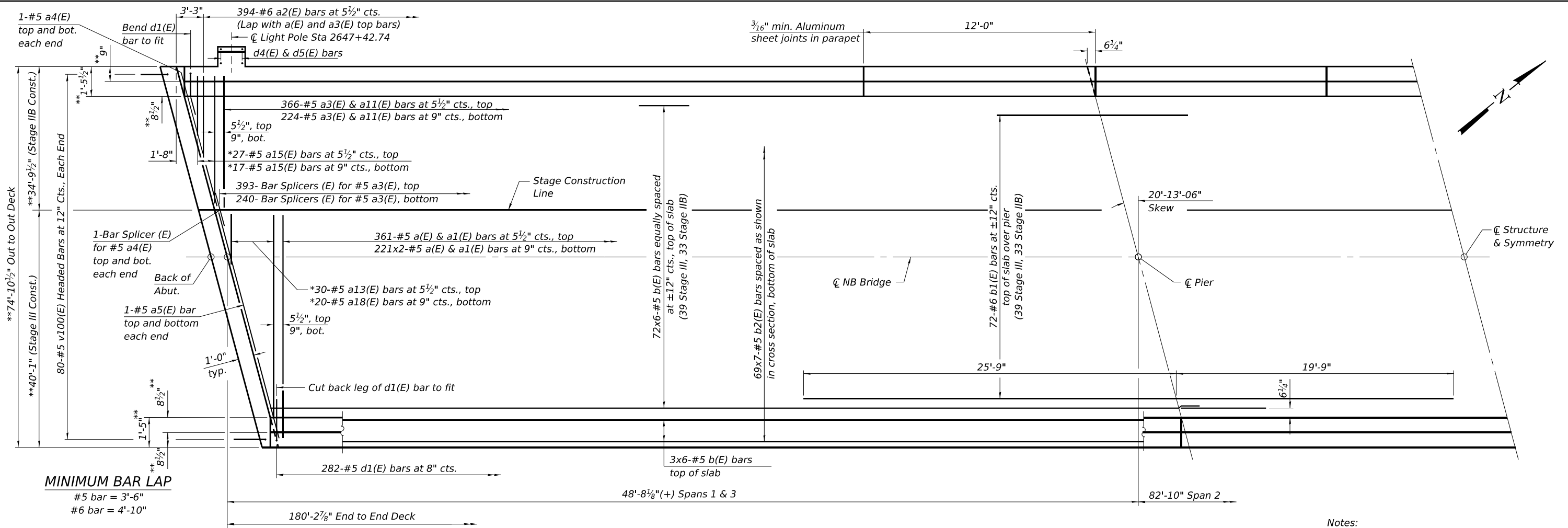
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	DRAWN = DSO	REVISED =
PLOT SCALE = \$\$CA\$	CHECKED = AJN	REVISED =
PLOT DATE = 8/12/2024	DATE = 02/27/24	REVISED =

**STATE OF ILLINOIS**  
**DEPARTMENT OF TRANSPORTATION**

**DECK SLAB - SOUTHBOUND STRUCTURE**  
**STRUCTURE NO. 101-0208/0209**

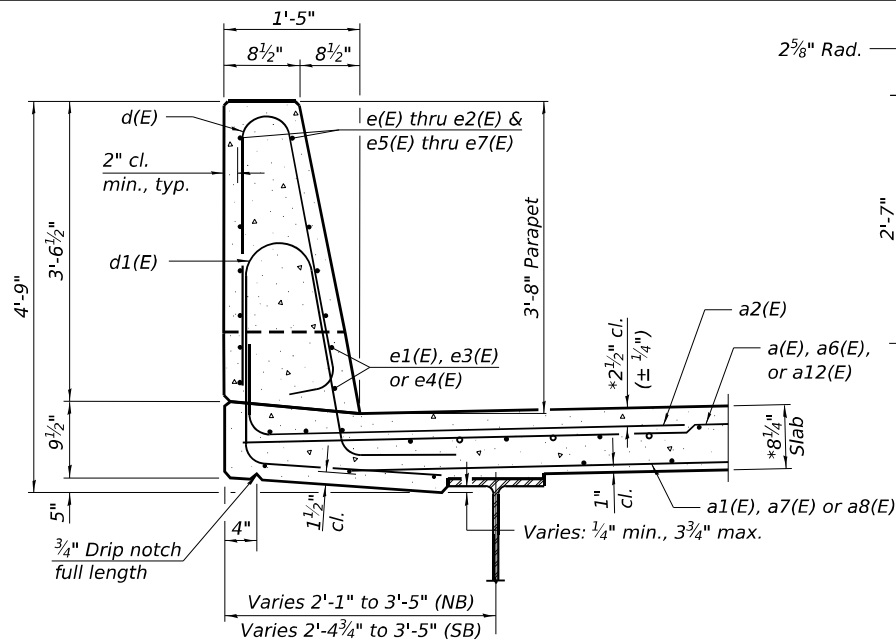
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	566
		CONTRACT NO. 64C24		
ILLINOIS		FED. AID PROJECT		

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

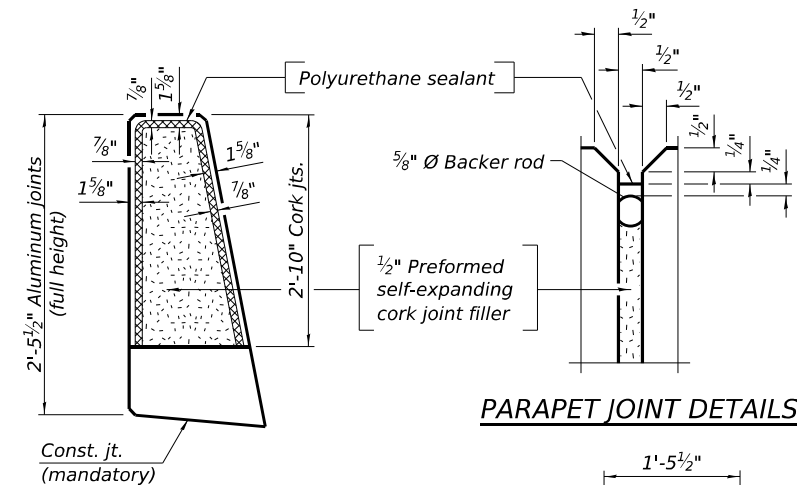


Note:  $2' \cdot 6\frac{1}{2}"$

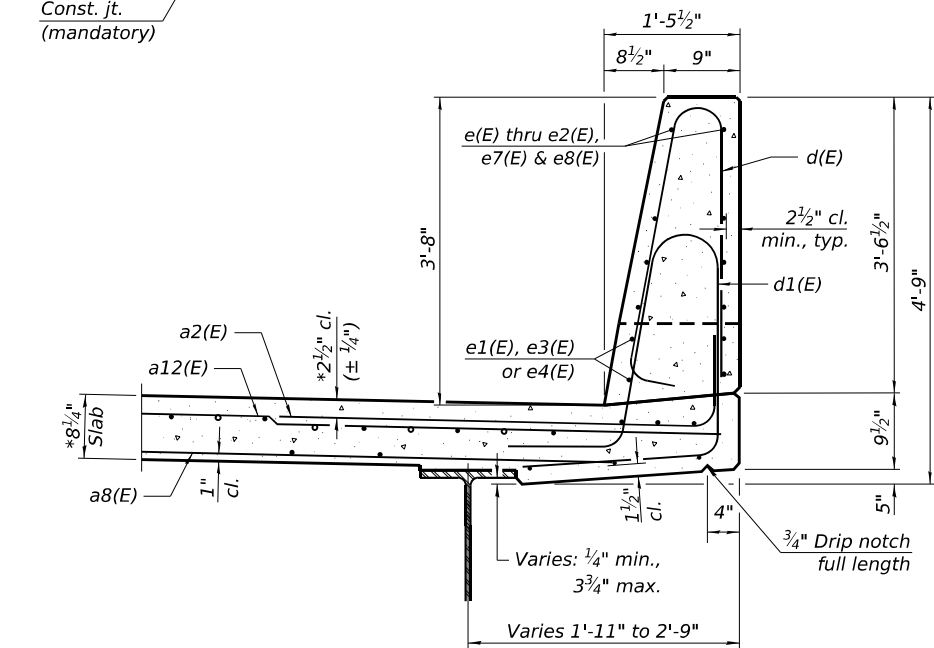
All transverse bars, a(E) & a1(E) and a3(E) & a11(E), shall be oriented with top bar laps opposite of bottom bar laps, as shown in cross-section.



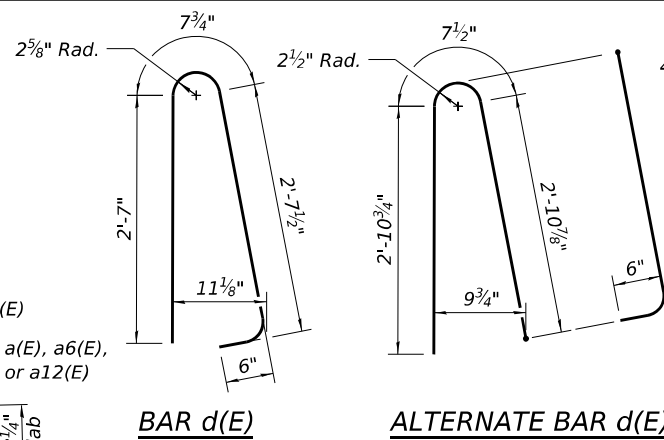
SECTION THRU NB & SB PARAPETS  
\* Prior to grinding.



PARAPET JOINT DETAILS

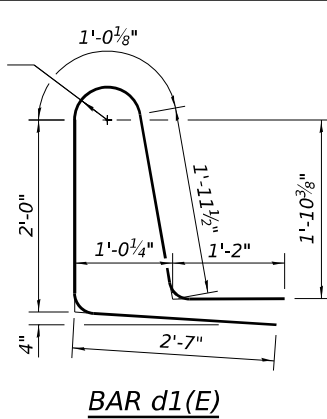


SECTION THRU SB MEDIAN PARAPET  
\* Prior to grinding.



BAR d(E)  
ALTERNATE BAR d(E)  
(For 44" constant slope parapet when conduit is present)

Notes:  
The 3/16" minimum aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated with 5 mils of either bitumen paint or epoxy paint to minimize reaction with wet concrete. Cost included with Concrete Superstructure.  
The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.  
Bar terminators, paid for separately. See Total Bill of Material.



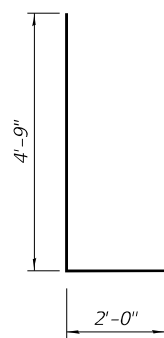
BAR d1(E)

BAR d2(E)

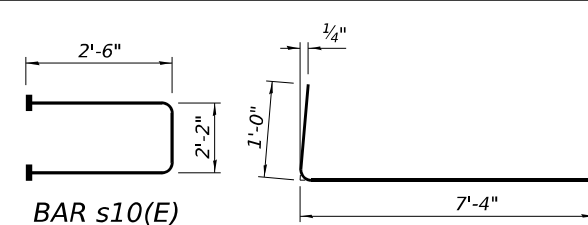
BAR d3(E)

BAR d4(E)

BAR d5(E)

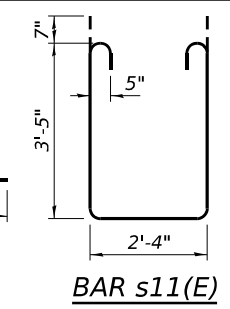


BAR v100(E)  
(Headed. 320-#5 Bar terminators)



BAR s10(E)  
(Headed. 636-#5 Bar terminators)

BAR a2(E)



BAR s11(E)

NB SUPERSTRUCTURE  
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a6(E)	686	#5	28'-6"	—
a1(E)	686	#5	15'-1"	—
a2(E)	788	#6	8'-4"	—
a3(E)	636	#5	13'-5"	—
a4(E)	4	#5	36'-9"	—
a5(E)	4	#5	42'-4"	—
a11(E)	636	#5	24'-11"	—
a13(E)	30	#5	42'-0"	—
a15(E)	44	#5	37'-8"	—
a18(E)	20	#5	41'-4"	—
a19(E)	10	#6	5'-1"	—
b(E)	468	#5	32'-11"	—
b1(E)	144	#6	45'-6"	—
b2(E)	525	#5	28'-9"	—
b3(E)	2	#5	2'-2"	—
d(E)	564	#5	6'-5"	—
d1(E)	564	#5	8'-8 1/2"	—
d2(E)	282	#5	4'-5"	—
d4(E)	6	#5	6'-9"	—
d5(E)	6	#5	3'-4"	—
e(E)	40	#4	17'-8"	—
e1(E)	112	#4	11'-8"	—
e2(E)	80	#4	14'-3"	—
e3(E)	32	#4	19'-10"	—
e4(E)	16	#4	28'-11"	—
e5(E)	8	#4	18'-9"	—
e6(E)	8	#4	17'-10"	—
e7(E)	12	#4	18'-5"	—
e8(E)	12	#4	18'-2"	—
e11(E)	4	#9	2'-2"	—
e12(E)	9	#6	8'-11"	—
m10(E)	20	#6	23'-8"	—
m11(E)	64	#6	7'-8"	—
m12(E)	4	#6	1'-8"	—
m13(E)	4	#6	2'-8"	—
m14(E)	4	#6	7'-0"	—
m15(E)	4	#6	6'-0"	—
m16(E)	20	#6	20'-9"	—
m17(E)	8	#6	2'-0"	—
m18(E)	8	#6	0'-6"	—
s10(E)	158	#5	7'-2"	—
s11(E)	158	#5	10'-4"	—
v100(E)	160	#5	3'-1"	—
Concrete Superstructure			Cu. Yd.	471.7
Protective Coat			Sq. Yd.	1,727
Reinforcement Bars, Epoxy Coated			Lb.	131,780

Bars indicated thus 1 x 2-#4 etc. indicates 1 line of bars with 2 lengths per line.

SB SUPERSTRUCTURE  
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a6(E)	698	#5	31'-1"	—
a7(E)	698	#5	16'-5"	—
a2(E)	788	#6	8'-4"	—
a8(E)	704	#5	12'-1"	—
a9(E)	4	#5	46'-10"	—
a10(E)	4	#5	32'-9"	—
a12(E)	704	#5	22'-3"	—
a14(E)	35	#5	46'-10"	—
a16(E)	22	#5	46'-2"	—
a17(E)	38	#5	32'-9"	—
b(E)	468	#5	32'-11"	—
b1(E)	144	#6	45'-6"	—
b2(E)	490	#5	28'-9"	—
b4(E)	4	#5	2'-0"	—
d(E)	562	#5	6'-5"	—
d1(E)	562	#5	8'-8 1/2"	—
d3(E)	4	#5	1'-10"	—
e(E)	32	#4	17'-8"	—
e1(E)	96	#4	11'-8"	—
e2(E)	64	#4	14'-3"	—
e3(E)	32	#4	19'-10"	—
e4(E)	16	#4	28'-11"	—
e7(E)	16	#4	18'-5"	—
e8(E)	8	#4	18'-2"	—
e9(E)	18	#4	10'-8"	—
e10(E)	9	#4	4'-9"	—
m11(E)	64	#6	7'-8"	—
m19(E)	8	#6	3'-2"	—
m20(E)	8	#6	2'-6"	—
m21(E)	24	#6	1'-10"	—
m22(E)	8	#6	1'-7"	—
m23(E)	8	#6	5'-5"	—
m24(E)	20	#6	26'-1"	—
m25(E)	20	#6	19'-0"	—
s10(E)	160	#5	7'-2"	—
s11(E)	160	#5	10'-4"	—
v100(E)	160	#5	3'-1"	—
Concrete Superstructure			Cu. Yd.	467.0
Protective Coat			Sq. Yd.	1,751
Reinforcement Bars, Epoxy Coated			Lb.	132,200
Preformed Joint Seal 2 1/2"			Foot	181

MODEL - SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\101-0208-0209-CN\_DECK-SLAB-DETAILS-MISC.dgn

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USER NAME	= Ifranceschina	DESIGNED	- JW	REVISED	-
PLOT SCALE	= \$SCALES	DRAWN	- DSO	REVISED	-
PLOT DATE	= 8/12/2024	CHECKED	- AJN	REVISED	-
		DATE	- 02/27/24	REVISED	-

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

DECK SLAB DETAILS - MISCELLANEOUS & BILL OF MATERIAL  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 25 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	568
				CONTRACT NO. 64C24
		ILLINOIS	FED. AID PROJECT	

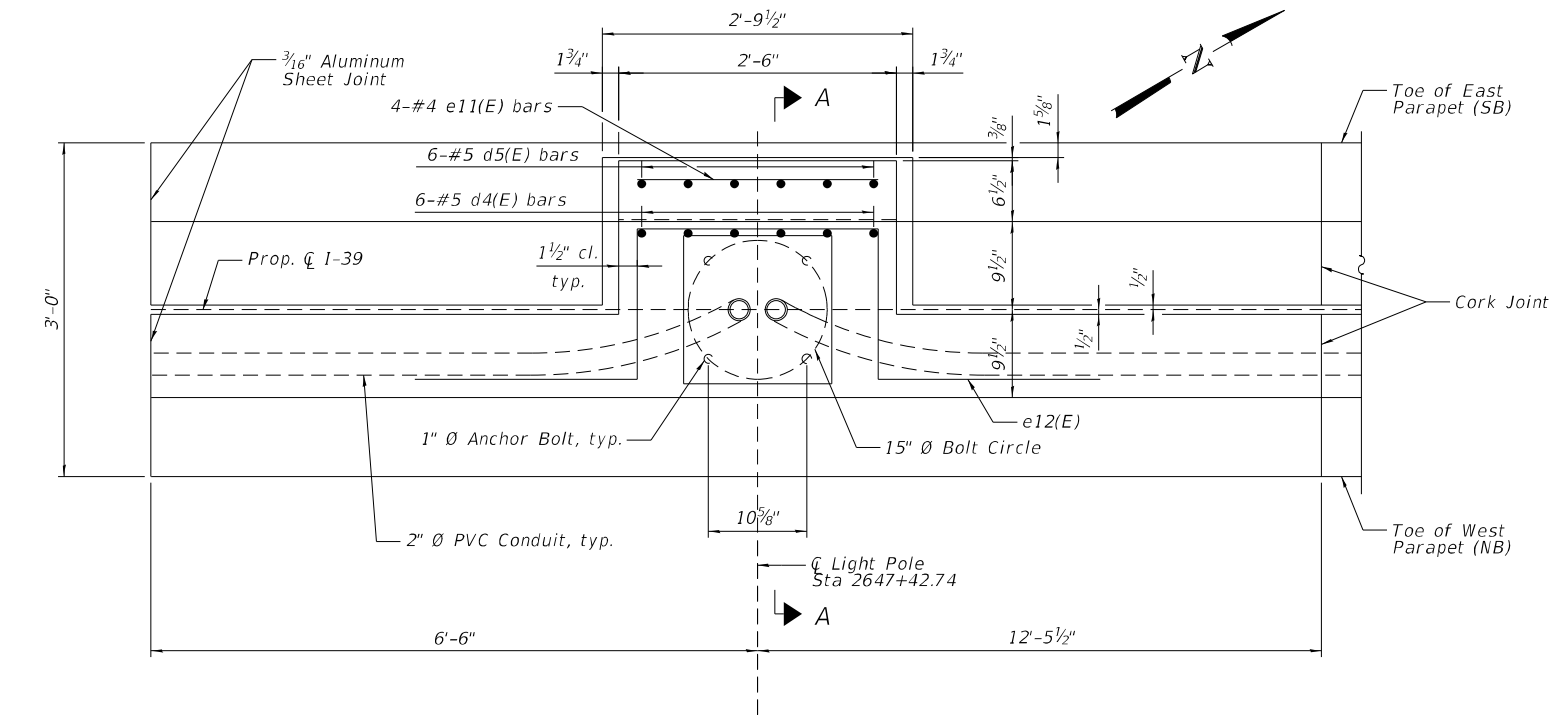
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)



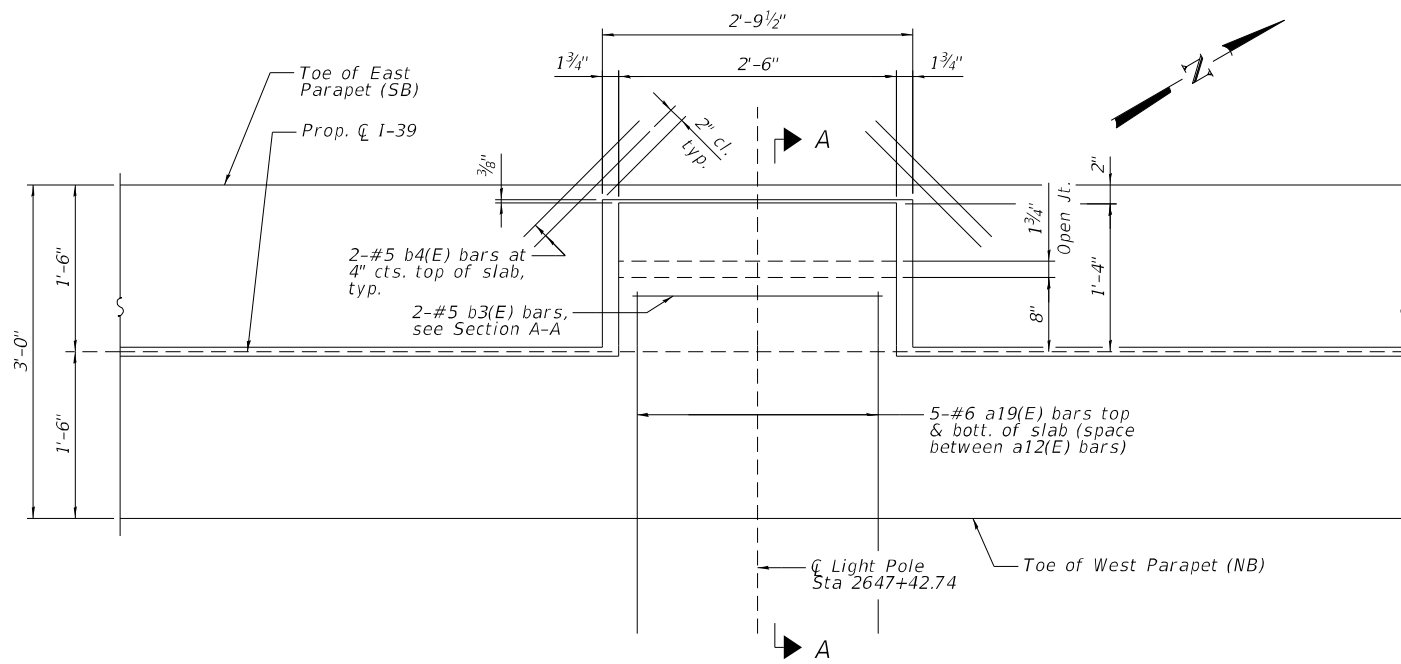




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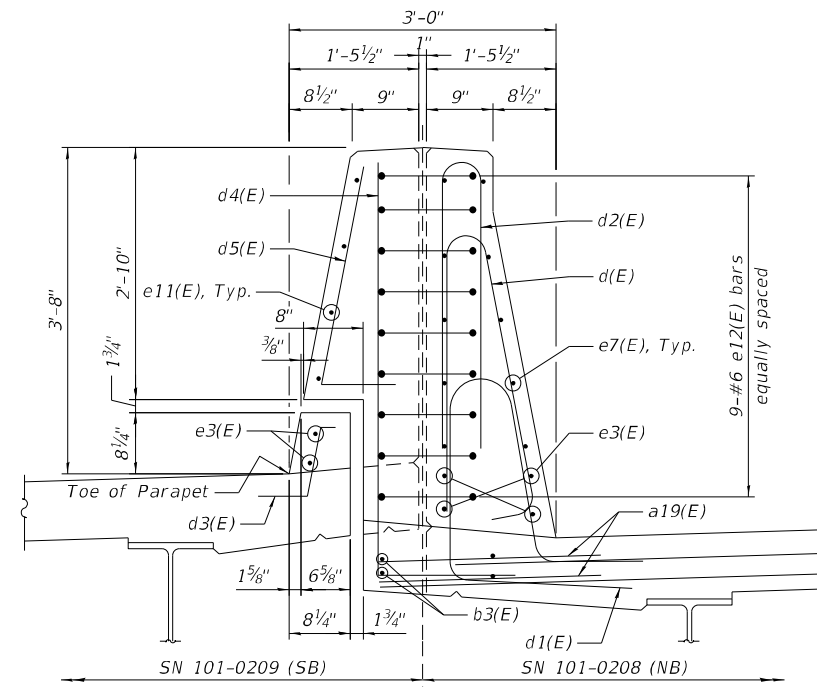


TOP PLAN



BOTTOM PLAN

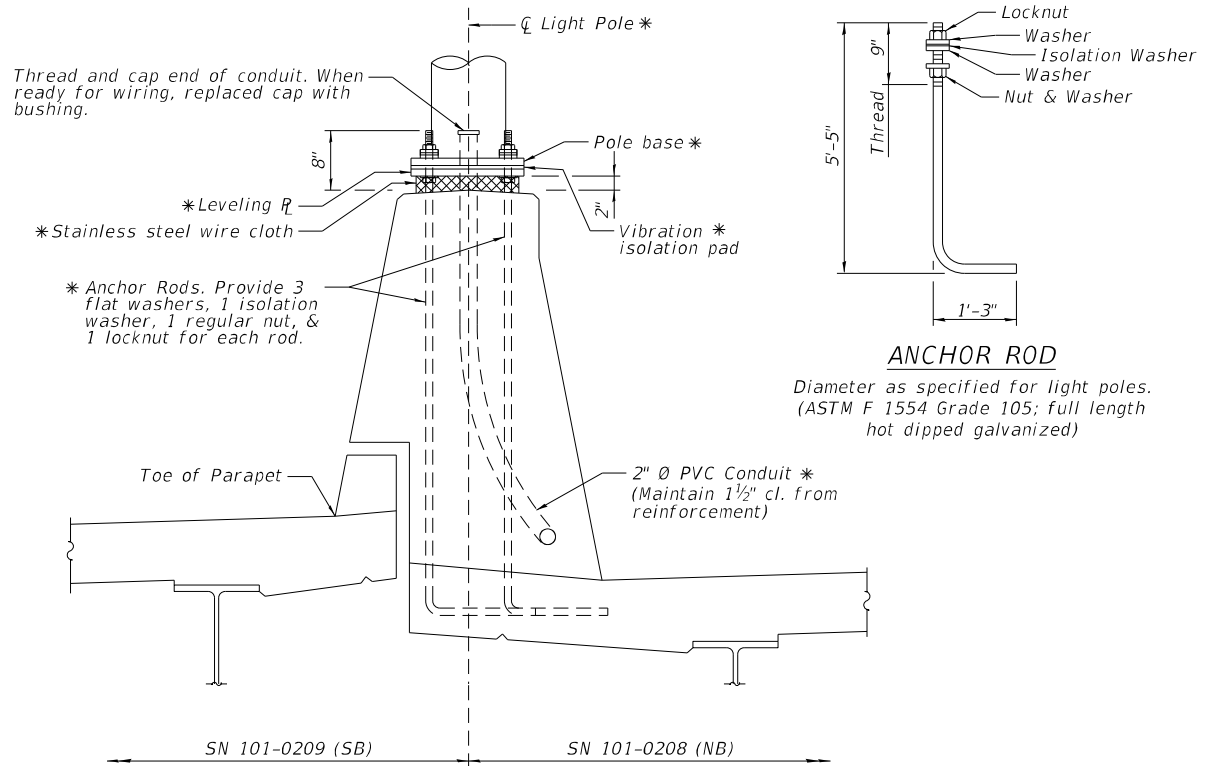
(Parapet joints not shown for clarity)



SECTION A-A

(Looking upstation)

(Light Pole and Conduit not shown for clarity)



\* See Lighting Plans for additional light pole details and pay items.

NOTES:

1. See Highway Standard 812001.
2. Cost of anchor rods is included with Concrete Superstructure.

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DRAWN - DSO	REVISED -	
PLOT SCALE = SSCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 05/23/2024	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

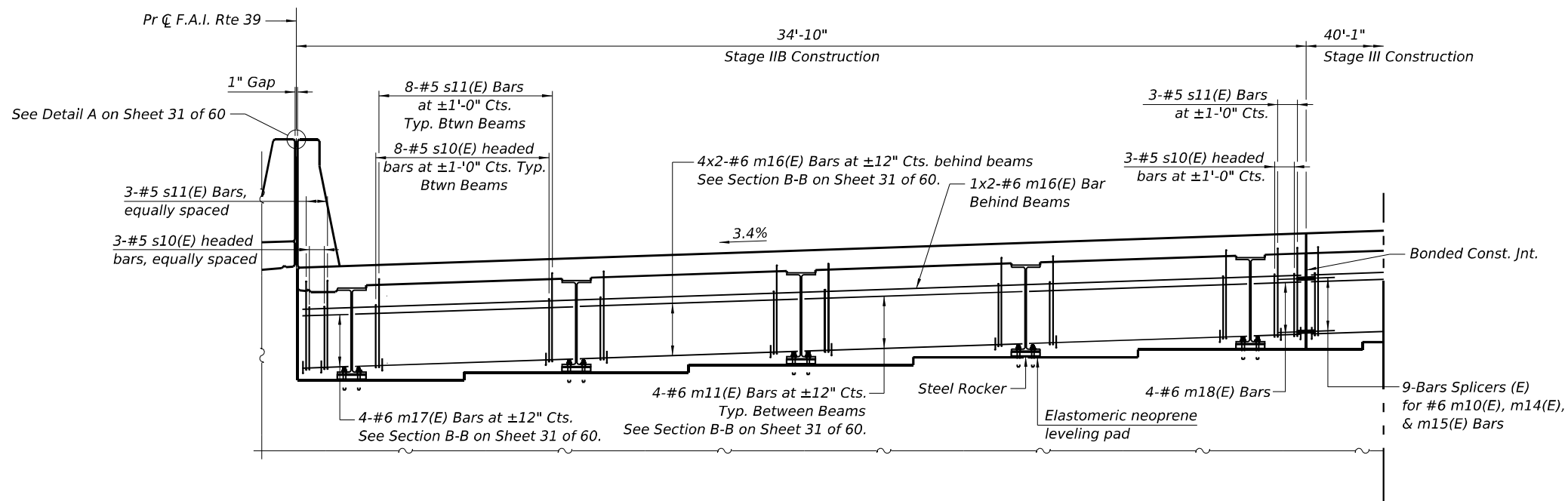
DECK SLAB DETAILS - PARAPET LIGHT POLE  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 28 OF 60 SHEETS STA. TO STA.

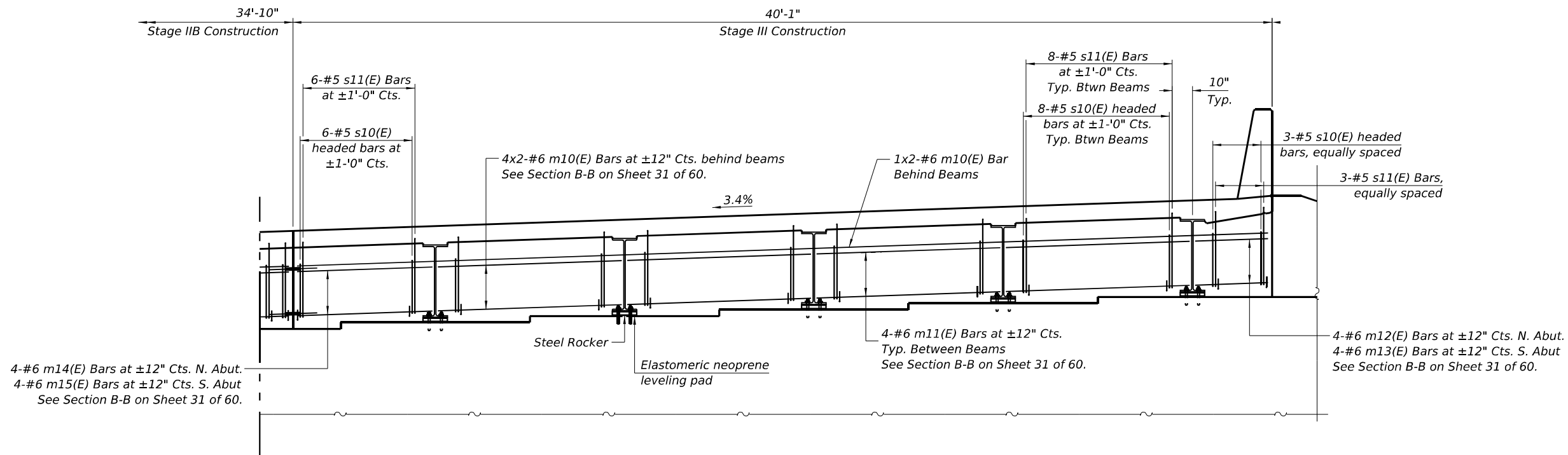
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	571
				CONTRACT NO. 64C24

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)





**NB DIAPHRAGM ELEVATION**  
(North Abutment diaphragm shown. South Abutment diaphragm similar.)  
(Looking North)

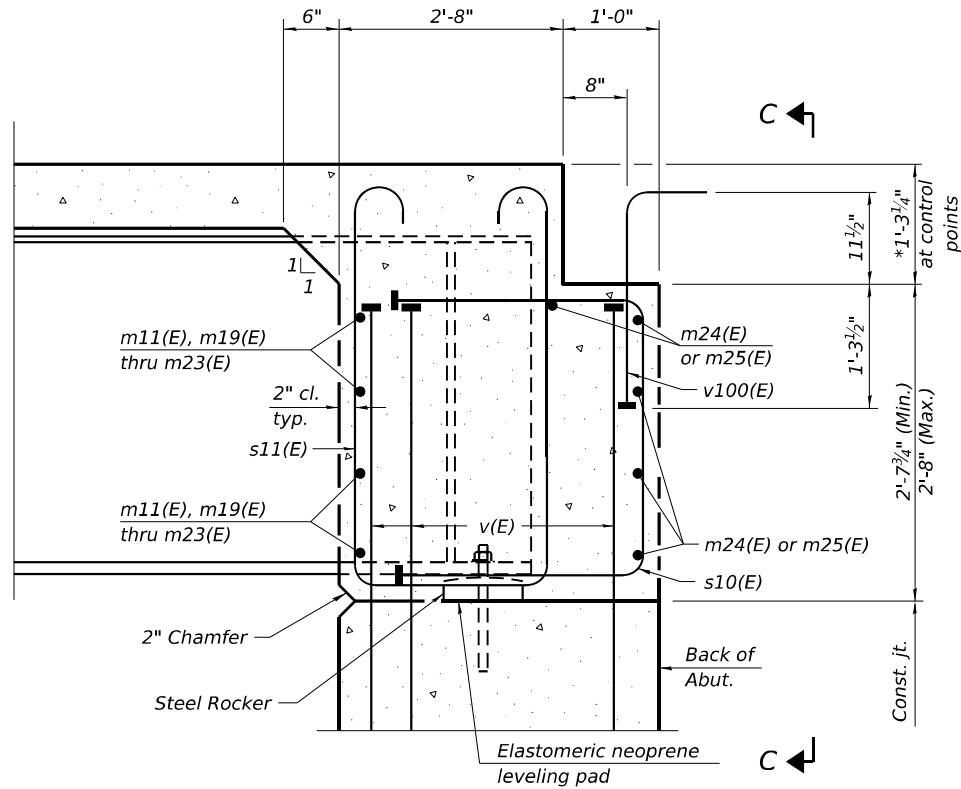


**NB DIAPHRAGM ELEVATION**  
(North Abutment diaphragm shown. South Abutment diaphragm similar.)  
(Looking North)

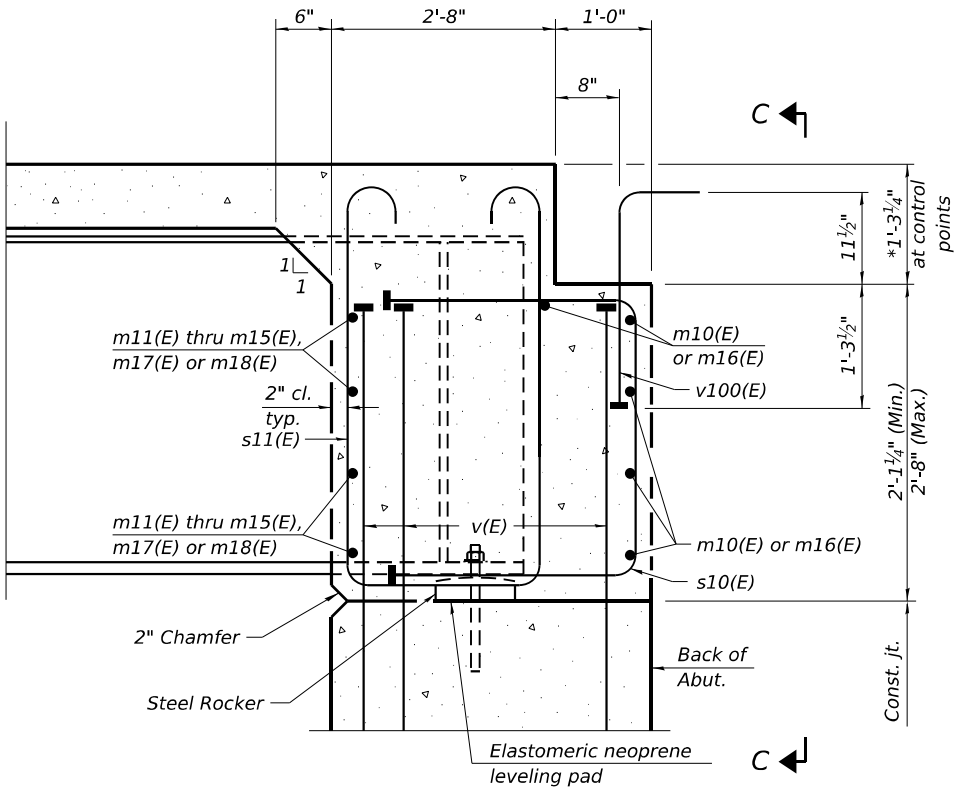
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<b>Bowman</b> <small>10 S. LaSalle Street, Suite 210 Chicago, Illinois 60603 (312) 455-0300 www.bowman.com</small>	USER NAME = Ifranceschina	DESIGNED - JW	REVISED -	<b>STATE OF ILLINOIS</b> <b>DEPARTMENT OF TRANSPORTATION</b>	<b>CONCRETE END DIAPHRAGM - NORTHBOUND</b> <b>STRUCTURE NO. 101-0208/0209</b>			F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	DRAWN - DSO	CHECKED - AJN	REVIS					*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	573
	PLOT SCALE = \$SCALE\$	DATE - 02/27/24	REVIS		SCALE: SHEET 30 OF 60 SHEETS STA. TO STA.			ILLINOIS FED. AID PROJECT				
	PLOT DATE = 8/12/2024							* FAI ROUTE 39 (I-39) & FAP 301 (US 20)				

Notes:  
See sheet 25 of 60 for superstructure details and Bill of Material.  
See sheet 25 of 60 for PJF details.  
See sheets 29 & 30 of 60 for Concrete End Diaphragm elevations.  
The s10(E) and s11(E) bars shall be placed parallel to the beams.  
Spacing for these bars shall be at right angles to the beams.  
The approach slab seat shall have a constant slope determined from the control points shown.

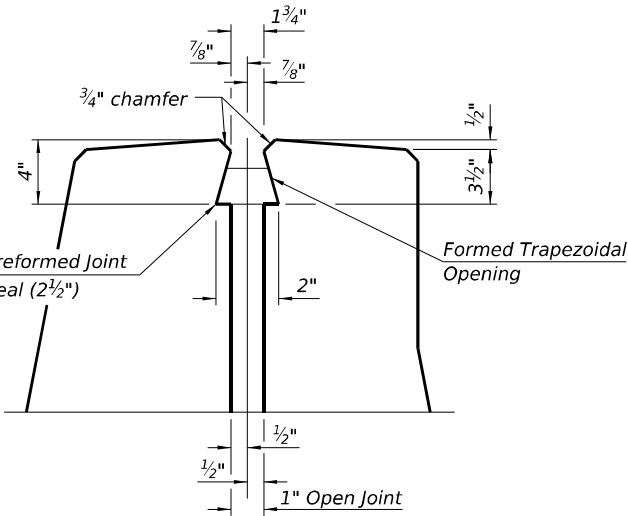


SECTION A-A  
(at Rt. L's)

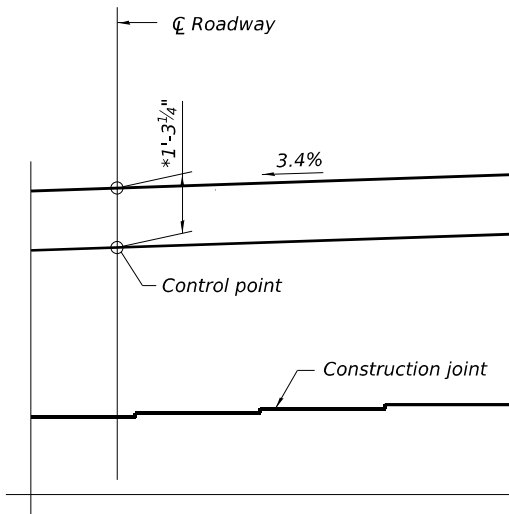


SECTION B-B  
(at Rt. L's)

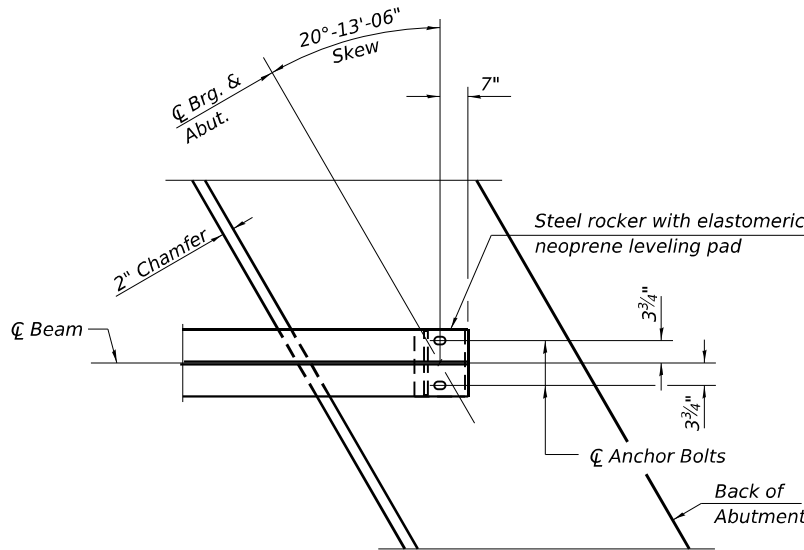
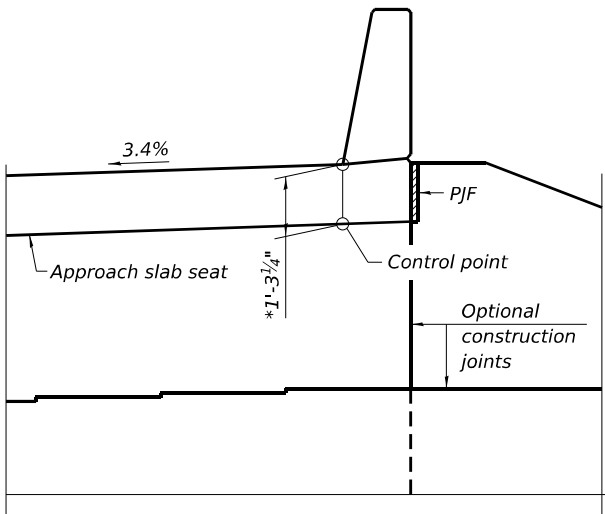
\* Prior to Grinding



DETAIL A



VIEW C-C



BOTTOM FLANGE PLAN AT ABUTMENT  
(Showing bottom flange of beam)

MODEL: SHEET  
FILE NAME: c:\pwworkdir\benesch projects\projects\101-0208-0209-CN\_END-DIAPHRAGM+DETAILS.dgn

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PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

CONCRETE END DIAPHRAGM - DETAILS  
STRUCTURE NO. 101-0208/0209

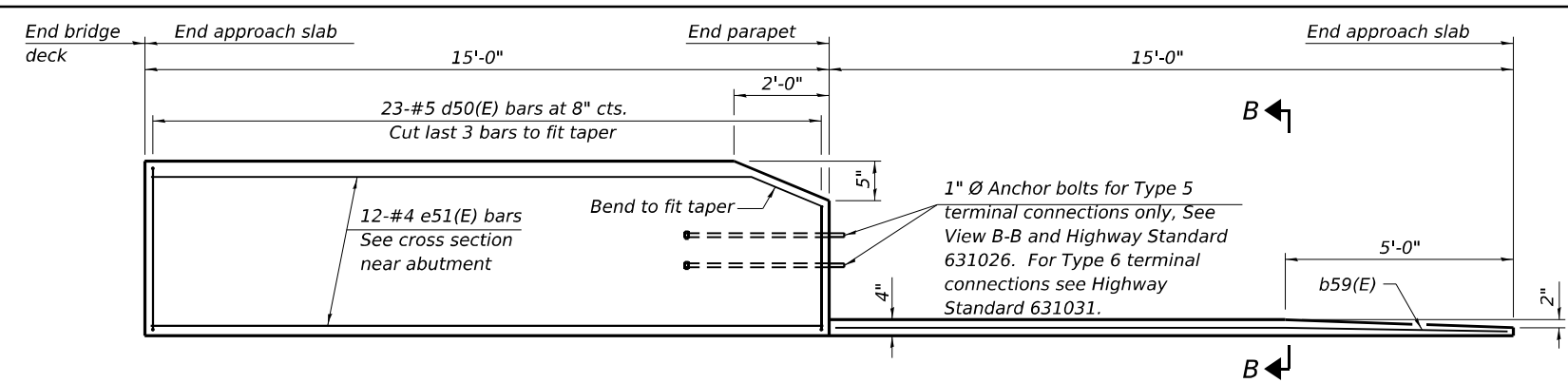
SCALE: SHEET 31 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	574
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

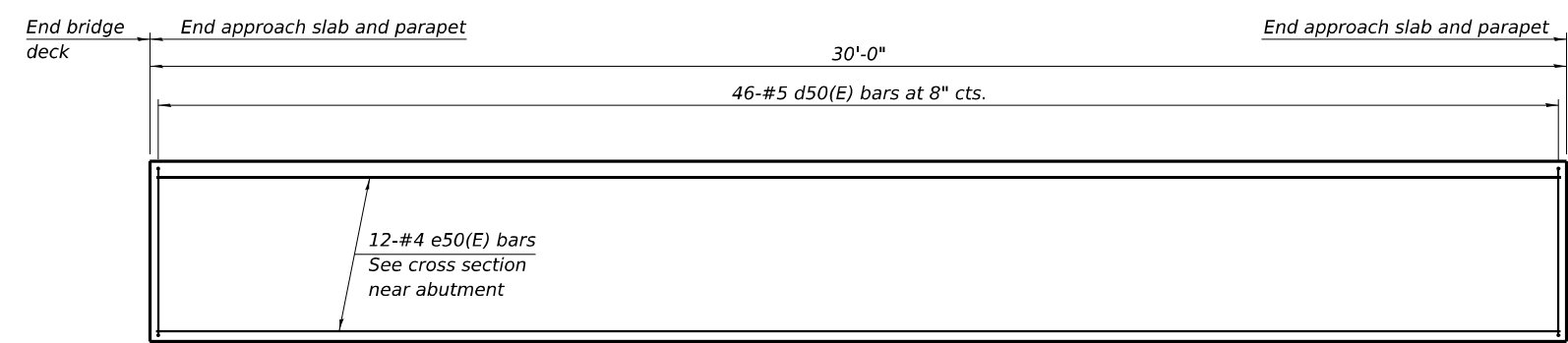
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)



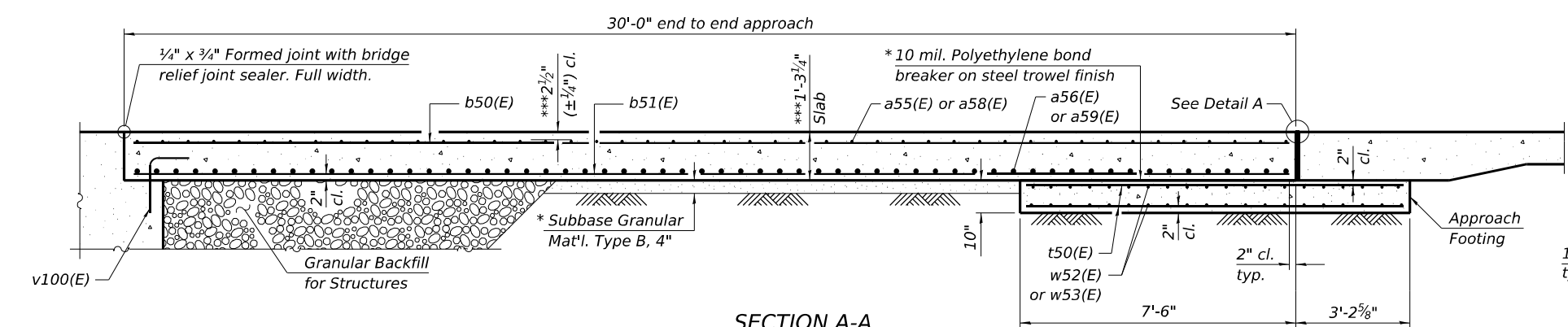




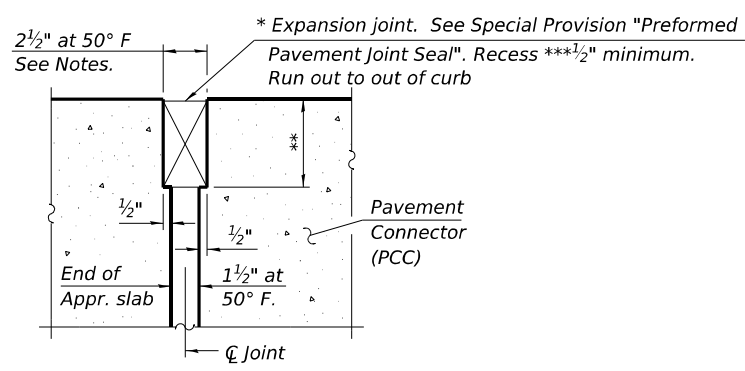
INSIDE ELEVATION OF OUTSIDE PARAPET AND CURB



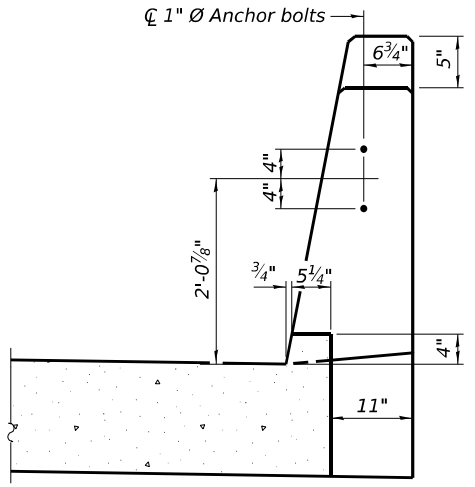
INSIDE ELEVATION OF MEDIAN PARAPET



SECTION A-A



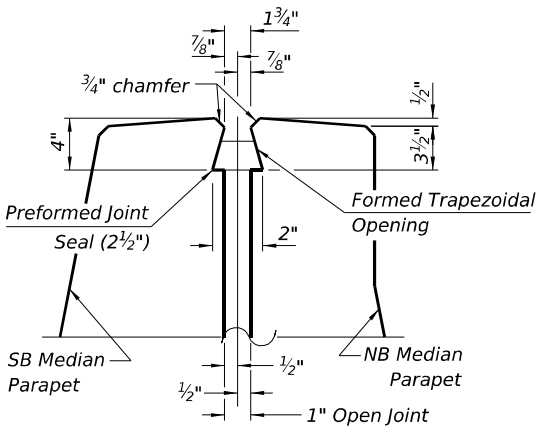
DETAIL A  
(at Rt. L's)



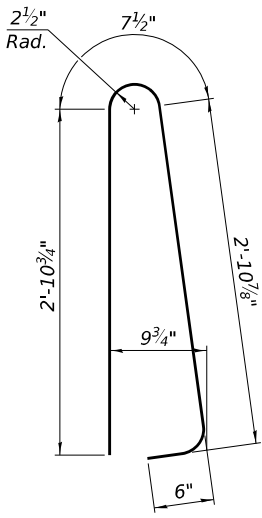
VIEW B-B

(Terminal 5 shown. For Terminal 6 see Highway Standard 631031.)

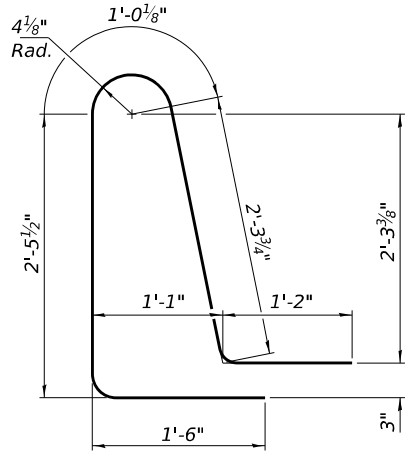
Notes:  
The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.  
Parapet concrete shall be paid for as Concrete Superstructure.  
Approach slab shall be paid for as Concrete Superstructure (Approach Slab).  
Approach footing concrete shall be paid for as Concrete Structures.  
The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.  
Cost of excavation for approach footing included with Concrete Structures.  
For Granular Backfill for Structures and drainage treatment details, see sheet 3 of 60.



MEDIAN JOINT DETAIL



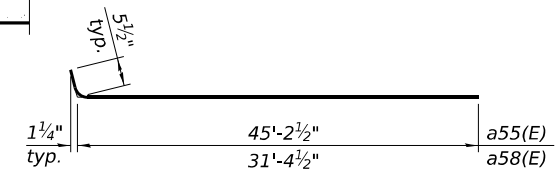
BAR d50(E)



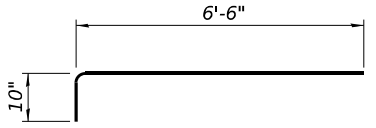
BAR d51(E)

SOUTHBOUND  
TWO APPROACHES  
BILL OF MATERIAL

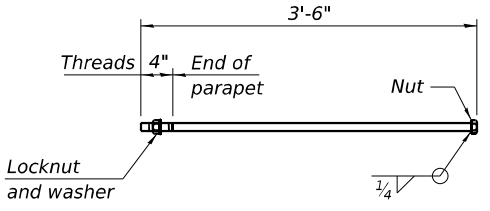
Bar	No.	Size	Length	Shape
a55(E)	92	#5	45'-8"	
a56(E)	248	#8	26'-7"	
a57(E)	138	#5	7'-4"	
a58(E)	92	#5	31'-10"	
a59(E)	248	#8	19'-7"	
b50(E)	222	#5	29'-8"	
b51(E)	294	#9	29'-8"	
b52(E)	8	#5	29'-8"	
b55(E)	8	#5	14'-7"	
b56(E)	2	#4	14'-4"	
d50(E)	138	#5	6'-5"	
d51(E)	138	#5	8'-6"	
e50(E)	56	#4	29'-8"	
e51(E)	56	#4	14'-7"	
t50(E)	152	#4	10'-2"	
w52(E)	40	#5	45'-4"	
w53(E)	40	#5	32'-3"	
Concrete Superstructure			Cu. Yd.	13.4
Concrete Superstructure (Approach Slab)			Cu. Yd.	205.6
Concrete Structures			Cu. Yd.	48.4
Reinforcement Bars, Epoxy Coated			Pound	84,050
Protective Coat			Sq. Yd.	499
Preformed Joint Seal 2 1/2"			Foot	30



BAR a55(E) or a58(E)



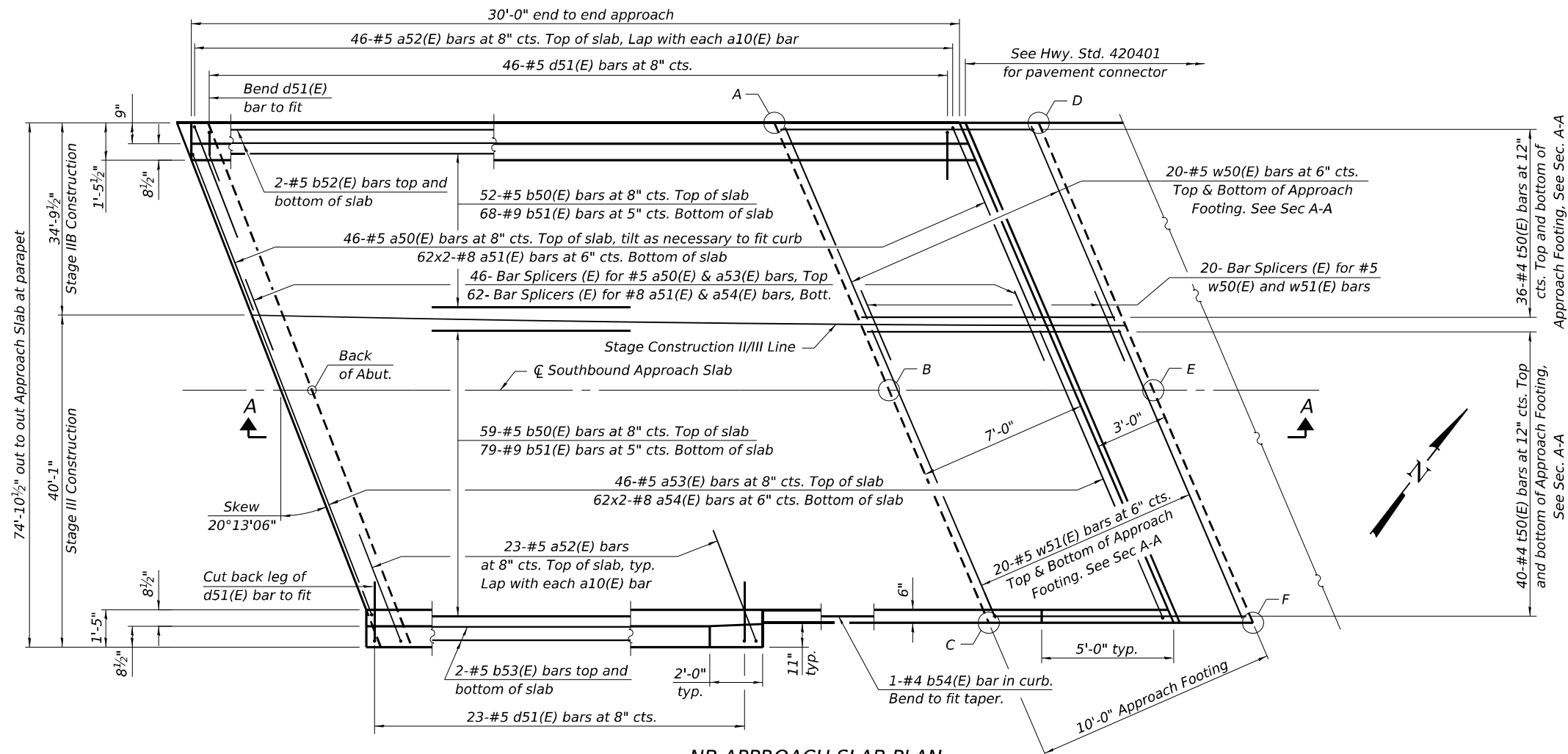
BAR a57(E)



\* 1" Ø ANCHOR BOLT

(Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications)

MODEL: SHEET  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\10173871\10264C24\_SHT\_0208-0209-CN\_APPR-SL-ABS-NB-1.dgn



**NB APPROACH SLAB PLAN**

(North approach slab shown; South approach slab similar by 180° rotation)

**START & END STATIONS OF  
NB APPROACH SLABS**

(Along NB PGL)

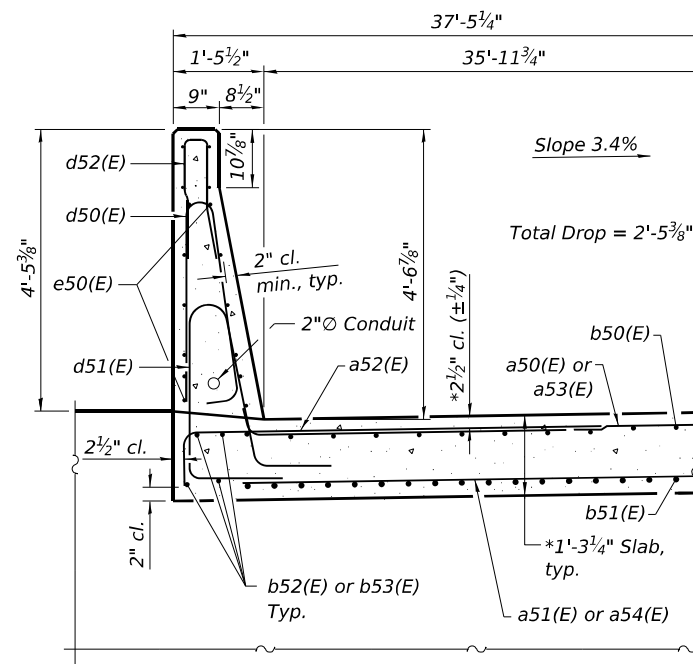
North Approach			South Approach		
	Station	Offset		Station	Offset
Start	2649+21.79	13.5'	Start	2647+41.98	13.5'
End	2649+51.50	13.5'	End	2647+11.87	13.5'

**TOP & BOTTOM ELEVATIONS  
FOR APPROACH FOOTING**

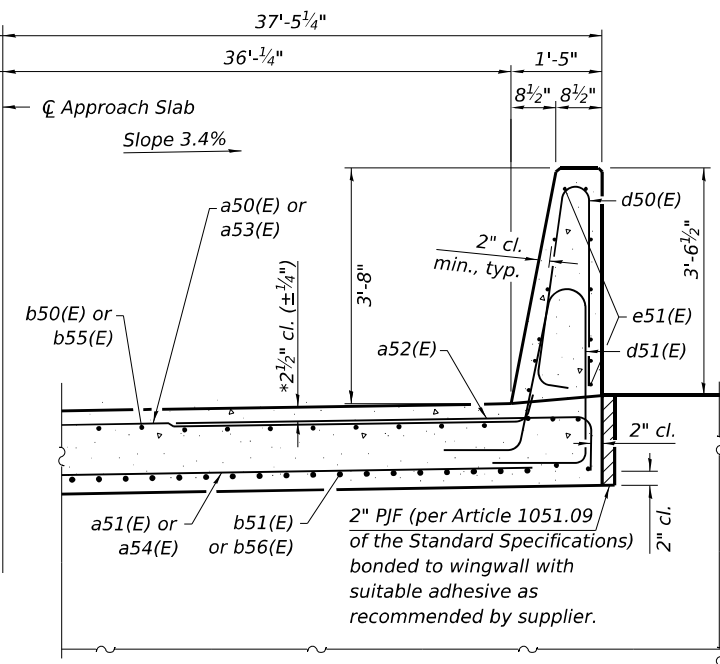
North Approach			South Approach		
Point/ Location	Top	Bottom	Point/ Location	Top	Bottom
A - SW	799.73	798.90	A - NE	799.47	798.64
B - S C	800.71	799.88	B - N C	800.63	799.80
C - SE	801.87	801.04	C - NW	801.90	801.07
D - NW	799.64	798.81	D - SE	801.85	801.02
E - N C	800.66	799.83	E - S C	800.58	799.75
F - NE	801.82	800.99	F - SW	799.48	798.65

**MINIMUM BAR LAP**

#8 bar = 7'-10"



**AT MEDIAN PARAPET**

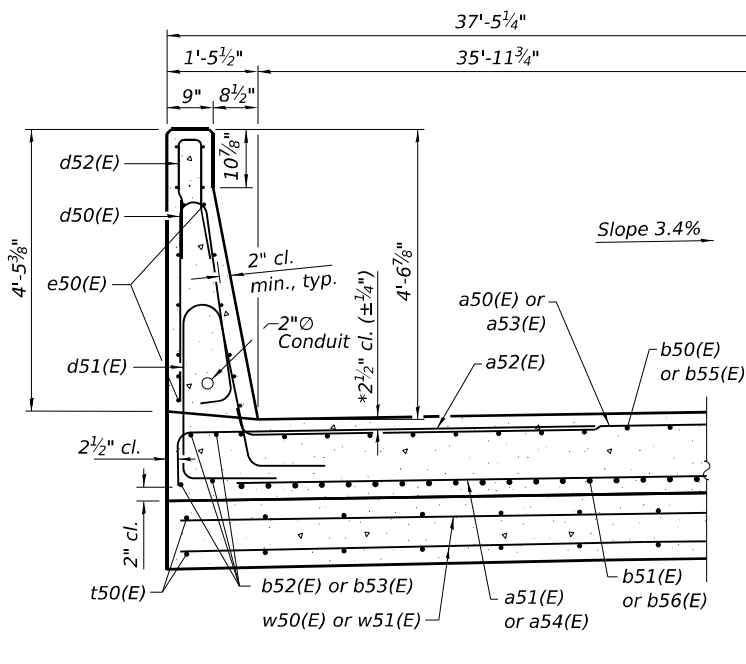


**AT OUTSIDE PARAPET**

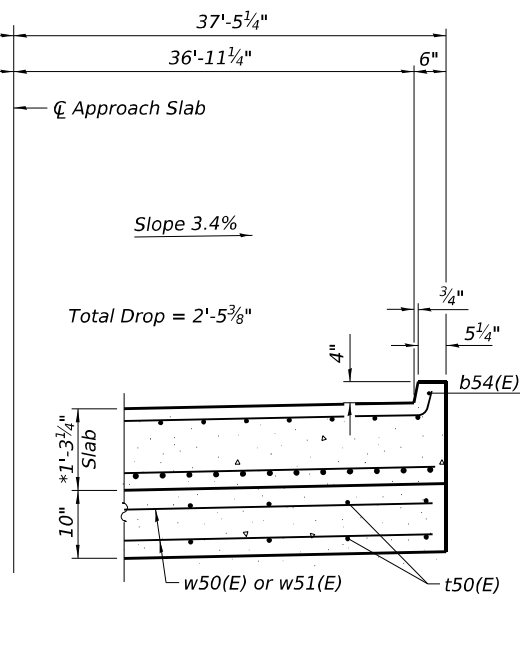
**NB APPROACH SLAB CROSS SECTION AT ABUTMENT**

(Looking North)

\*Prior to grinding



**AT MEDIAN PARAPET**



**AT OUTSIDE PARAPET**

**NB APPROACH SLAB CROSS SECTION AT APPROACH FOOTING**

(Looking North)

\*Prior to grinding

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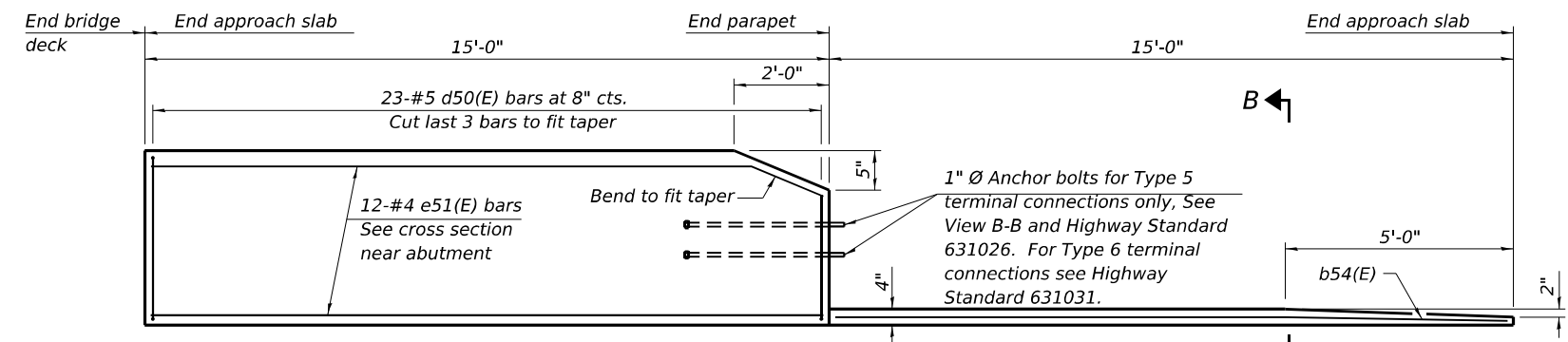
**STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION**

**APPROACH SLABS - NORTHBOUND  
STRUCTURE NO. 101-0208/0209**

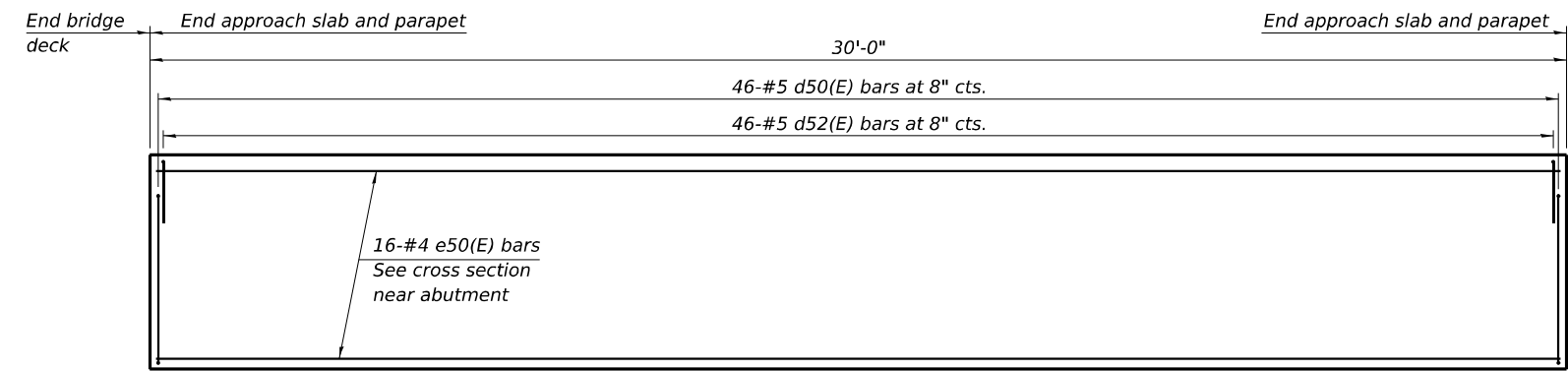
SCALE: SHEET 34 OF 60 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	577
CONTRACT NO. 64C24				

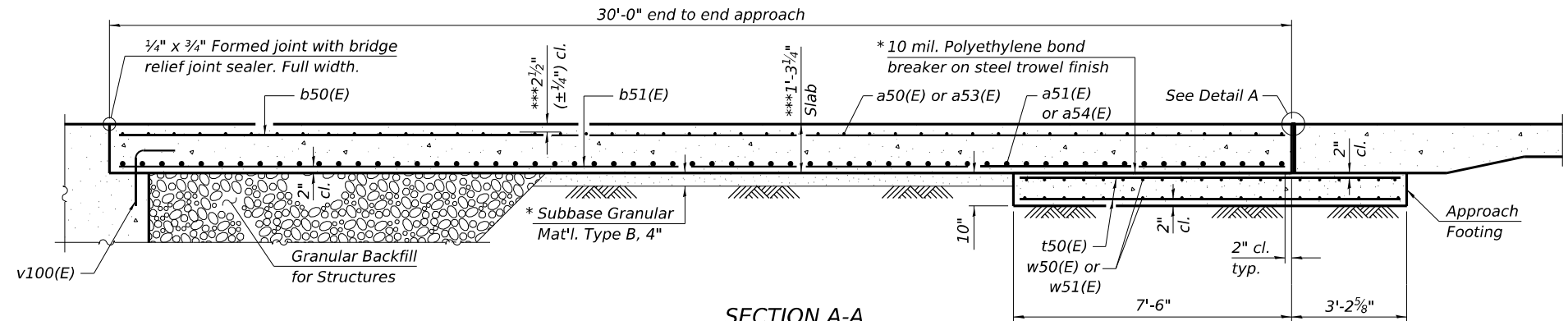
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



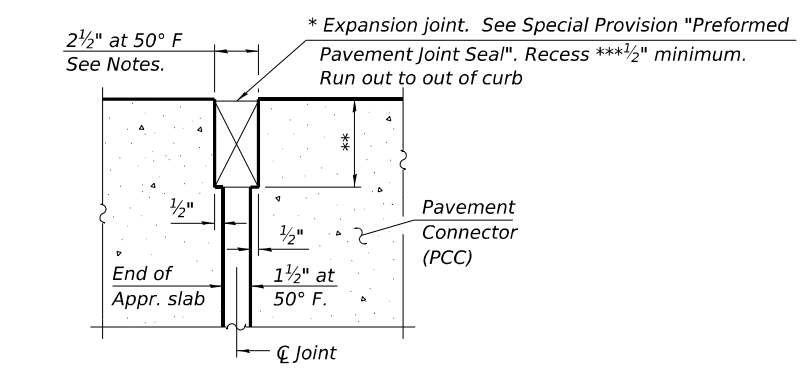
INSIDE ELEVATION OF OUTSIDE PARAPET AND CURB



INSIDE ELEVATION OF MEDIAN PARAPET

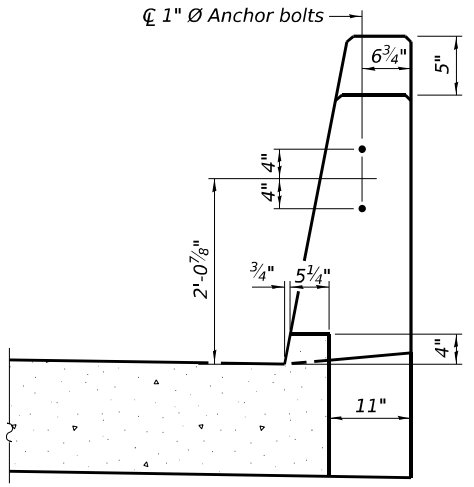


SECTION A-A



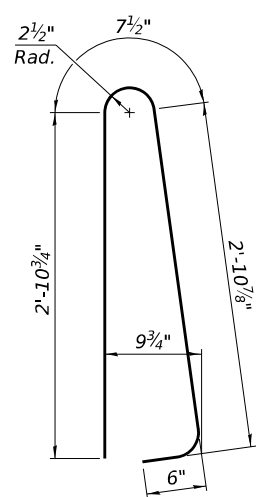
DETAIL A  
(at Rt. L's)

\* Cost included with Concrete Superstructure (Approach Slab).  
\*\* Per manufacturer recommendations  
\*\*\* Prior to grinding

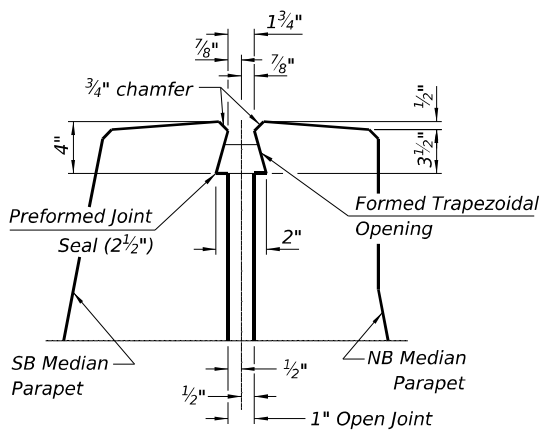


VIEW B-B

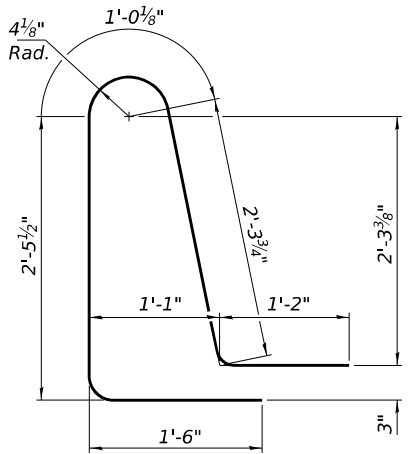
(Terminal 5 shown. For Terminal 6, see Highway Standard 631031.)



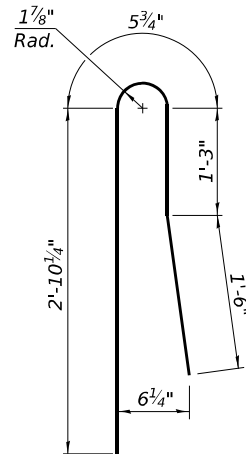
BAR d50(E)



MEDIAN JOINT DETAIL



BAR d51(E)

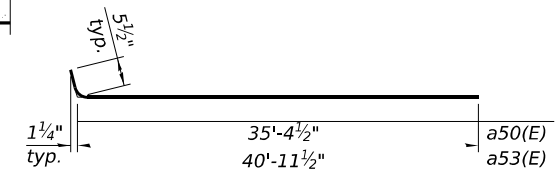


BAR d52(E)

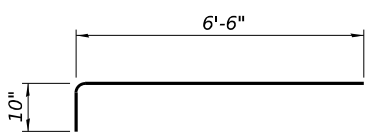
Notes:  
The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.  
Parapet concrete shall be paid for as Concrete Superstructure.  
Approach slab shall be paid for as Concrete Superstructure (Approach Slab).  
Approach footing concrete shall be paid for as Concrete Structures.  
The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.  
Cost of excavation for approach footing included with Concrete Structures.  
For Granular Backfill for Structures and drainage treatment details, see sheet 3 of 60.

NORTHBOUND  
TWO APPROACHES  
BILL OF MATERIAL

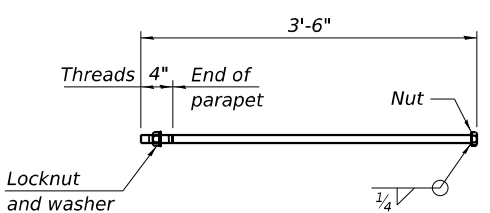
Bar	No.	Size	Length	Shape
a50(E)	92	#5	35'-11"	
a51(E)	248	#8	21'-8"	
a52(E)	138	#5	7'-4"	
a53(E)	92	#5	41'-0"	
a54(E)	248	#8	24'-8"	
b50(E)	222	#5	29'-8"	
b51(E)	294	#9	29'-8"	
b52(E)	8	#5	29'-8"	
b53(E)	8	#5	15'-3"	
b54(E)	2	#4	13'-8"	
d50(E)	138	#5	6'-5"	
d51(E)	138	#5	8'-6"	
d52(E)	138	#5	6'-2"	
e50(E)	56	#4	29'-8"	
e51(E)	56	#4	15'-2"	
t50(E)	152	#4	10'-2"	
w50(E)	40	#5	36'-5"	
w51(E)	40	#5	41'-1"	
Concrete Superstructure		Cu. Yd.	14.9	
Concrete Superstructure (Approach Slab)		Cu. Yd.	205.3	
Concrete Structures		Cu. Yd.	48.4	
Reinforcement Bars, Epoxy Coated		Pound	85,010	
Protective Coat		Sq. Yd.	499	
Preformed Joint Seal 2 1/2"		Foot	30	



BAR a50(E) or a53(E)



BAR a52(E)



\* 1" Ø ANCHOR BOLT  
(Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications)

MODEL: SHEET  
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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

APPROACH SLABS DETAILS - NORTHBOUND  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 35 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1-5)R	WINNEBAGO	1685	578
				CONTRACT NO. 64C24
		ILLINOIS		FED. AID PROJECT

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

MODEL: SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\1017387\10264C24\_SHT\_0208-0209-CN\_FRAMING-PLAN.dgn

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PLOT DATE = 8/12/2024	DATE -	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 36 OF 60 SHEETS STA. TO STA.

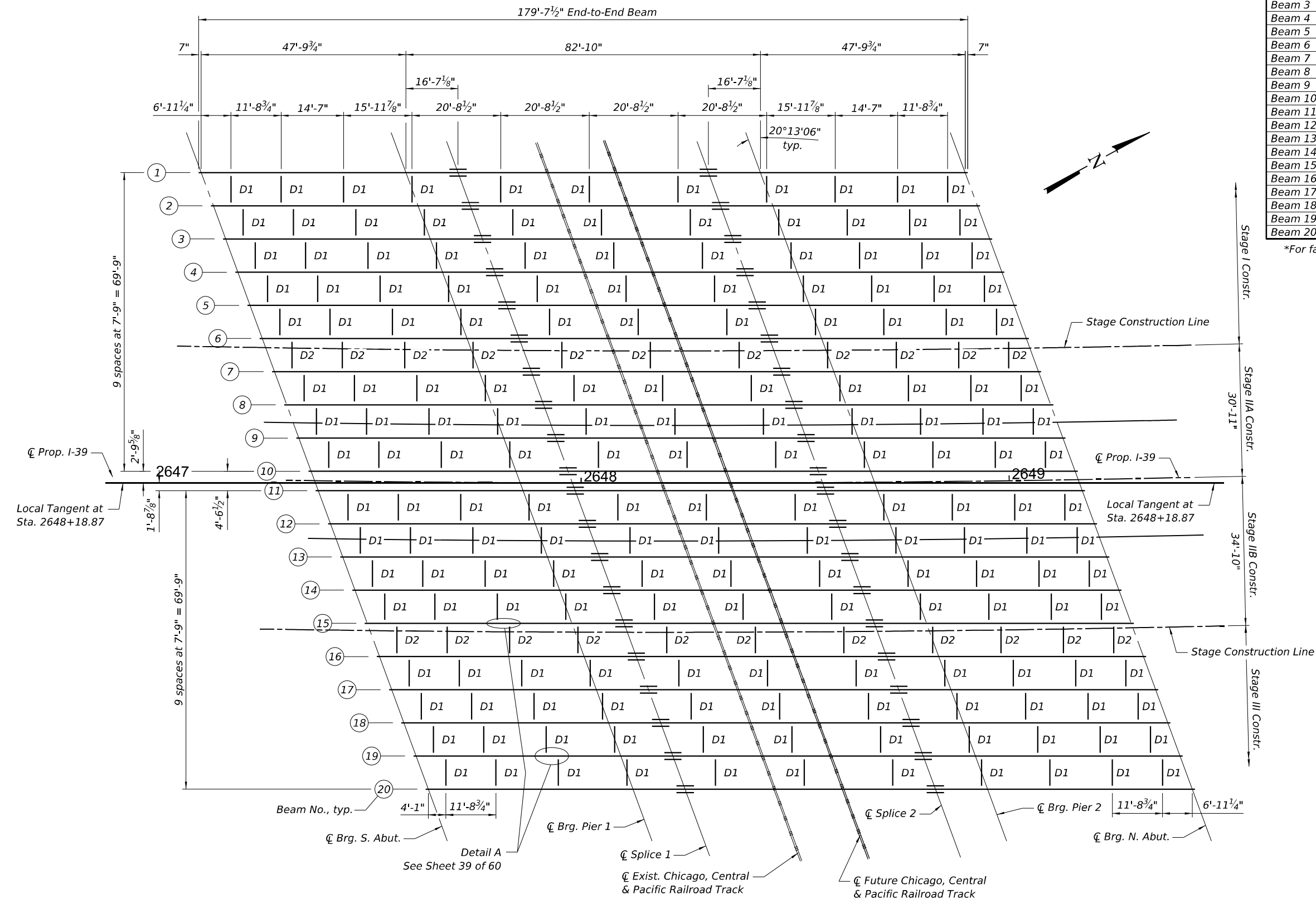
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	579
		CONTRACT NO. 64C24		
		ILLINOIS FED. AID PROJECT		

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

**\* TOP OF BEAM ELEVATIONS**

Location	℄ Brg. S. Abut.	℄ Brg. Pier 1	℄ Splice 1	℄ Splice 2	℄ Brg. Pier 2	℄ Brg. N. Abut.
Beam 1	798.28	798.44	798.42	798.52	798.60	798.59
Beam 2	798.55	798.72	798.69	798.79	798.87	798.85
Beam 3	798.82	798.99	798.96	799.05	799.13	799.11
Beam 4	799.10	799.26	799.23	799.32	799.40	799.37
Beam 5	799.37	799.53	799.50	799.59	799.66	799.63
Beam 6	799.65	799.80	799.77	799.85	799.92	799.89
Beam 7	799.92	800.08	800.04	800.11	800.19	800.14
Beam 8	800.19	800.35	800.31	800.38	800.45	800.40
Beam 9	800.47	800.62	800.58	800.64	800.71	800.66
Beam 10	800.74	800.89	800.85	800.91	800.98	800.92
Beam 11	799.98	800.13	800.09	800.14	800.21	800.15
Beam 12	800.26	800.40	800.36	800.41	800.47	800.41
Beam 13	800.53	800.67	800.63	800.67	800.74	800.67
Beam 14	800.81	800.94	800.90	800.93	801.00	800.93
Beam 15	801.08	801.21	801.17	801.20	801.26	801.18
Beam 16	801.35	801.48	801.43	801.46	801.52	801.44
Beam 17	801.63	801.75	801.70	801.72	801.78	801.70
Beam 18	801.90	802.02	801.97	801.98	802.04	801.95
Beam 19	802.17	802.28	802.24	802.25	802.30	802.21
Beam 20	802.45	802.55	802.50	802.51	802.56	802.47

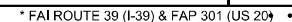
\*For fabrication only



Note:  
See sheet 39 of 60 for Detail A.

**FRAMING PLAN**





MODEL - SHEET  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\0173871\0284C24\_SHT\_0208-0209-CN\_STR-STEEL-DETAIL.s.dgn

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BEAM REACTION TABLE		
	S. Abut. or N. Abut.	Pier 1 or 2
LLDF	0.8550	0.8550
OCF	-	-
R <sub>DC1</sub>	(k) 14.63	75.22
R <sub>DC2</sub>	(k) 0.83	4.61
R <sub>DW</sub>	(k) 5.28	29.34
R <sub>ℓ</sub>	(k) 59.73	103.19
R <sub>IM</sub>	(k) 15.69	19.95
R <sub>Total</sub> (Strength I)(Impact)	(k) 159.23	359.29
R <sub>Total</sub> (Strength I)(No Impact)	(k) 131.77	324.38

INTERIOR BEAM MOMENT TABLE			
	0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or 2	0.5 Span 2
I <sub>s</sub>	(in <sup>4</sup> ) 10,500	10,500	10,500
I <sub>c</sub> (n)	(in <sup>4</sup> ) 30,304	-	30,304
I <sub>c</sub> (3n)	(in <sup>4</sup> ) 22,186	-	22,186
I <sub>c</sub> (cr)	(in <sup>4</sup> ) -	14,420	-
S <sub>s</sub>	(in <sup>3</sup> ) 581	581	581
S <sub>c</sub> (n)	(in <sup>3</sup> ) 889	-	889
S <sub>c</sub> (3n)	(in <sup>3</sup> ) 803	-	803
S <sub>c</sub> (cr)	(in <sup>3</sup> ) -	678	-
DC1	(k/°) 0.996	0.996	0.996
M <sub>DC1</sub>	(°k) 79	485	369
DC2	(k/°) 0.123	0.123	0.123
M <sub>DC2</sub>	(°k) 5	30	22
DW	(k/°) 0.36	0.36	0.36
M <sub>DW</sub>	(°k) 30	191	142
LLDF	0.702	0.702	0.637
M <sub>ℓ</sub> + IM	(°k) 558	684	735
f <sub>ℓ</sub> (Strength I)	(ksi) 0	0	0
M <sub>u</sub> + 1/3 f <sub>ℓ</sub> S <sub>x</sub>	(°k) 1,126	2,128	1,988
Φ <sub>f</sub> M <sub>n</sub>	(°k) 4,773	2,499	4,466
f <sub>s</sub> DC1	(ksi) 1.60	10.00	7.60
f <sub>s</sub> DC2	(ksi) 0.07	0.37	0.33
f <sub>s</sub> DW	(ksi) 0.45	2.37	2.12
f <sub>s</sub> (ℓ+IM)	(ksi) 15.2	26.4	26.8
f <sub>ℓ</sub> (Service II)	(ksi) 0.0	0.0	0.0
f <sub>s</sub> + f <sub>ℓ</sub> /2 (Service II)	(ksi) 21.9	47.1	44.9
Service II Resistance	(ksi) 47.5	47.5	47.5
f <sub>s</sub> + f <sub>ℓ</sub> /3 (Strength I)	(ksi) 29.4	62.8	60.1
Φ <sub>f</sub> F <sub>n</sub>	(ksi) -	-	-
V <sub>f</sub>	(k) 51.52	49.52	47.59

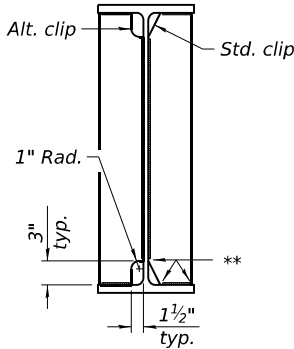
V<sub>f</sub>: Maximum factored shear range in span computed according to Article 6.10.10.  
OCF: Obtuse Correction Factor according to Article 4.6.2.2.3c or as further simplified by IDOT provisions.  
R<sub>DC1</sub>: Un-factored reaction due to non-composite dead load (kip).  
R<sub>DC2</sub>: Un-factored reaction due to long-term composite (superimposed excluding future wearing surface) dead load (kip).  
R<sub>DW</sub>: Un-factored reaction due to long-term composite (superimposed future wearing surface only) dead load (kip).  
R<sub>ℓ</sub>: Un-factored live load reaction (kip).  
R<sub>IM</sub>: Un-factored dynamic load allowance (impact) (kip).  
R<sub>Total</sub>(Strength I)(Impact): Strength I load combination of factored design reactions (kip).  
1.25 (R<sub>DC1</sub> + R<sub>DC2</sub>) + 1.5R<sub>DW</sub> + 1.75 (R<sub>ℓ</sub> + R<sub>IM</sub>)  
R<sub>Total</sub>(Strength I)(No Impact): Strength I load combination of factored design reactions, not including dynamic load allowance (Impact) (kip).  
1.25 (R<sub>DC1</sub> + R<sub>DC2</sub>) + 1.5R<sub>DW</sub> + 1.75 (R<sub>ℓ</sub>)

Note:  
M<sub>ℓ</sub> and R<sub>ℓ</sub> include the effects of centrifugal force and superelevation.

I<sub>s</sub>, S<sub>s</sub>: Non-composite moment of inertia and section modulus of the steel section used for computing f<sub>s</sub> (Total-Strength I, and Service II) due to non-composite dead loads (in.<sup>4</sup> and in.<sup>3</sup>).  
I<sub>c</sub> (n), S<sub>c</sub> (n): Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f<sub>s</sub> (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in.<sup>4</sup> and in.<sup>3</sup>).  
I<sub>c</sub> (3n), S<sub>c</sub> (3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f<sub>s</sub> (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.<sup>4</sup> and in.<sup>3</sup>).  
I<sub>c</sub> (cr), S<sub>c</sub> (cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f<sub>s</sub> (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.<sup>4</sup> and in.<sup>3</sup>).  
S<sub>x</sub>: Section modulus about the major axis of a section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in.<sup>3</sup>).  
DC1: Un-factored non-composite dead load (kips/ft.).  
M<sub>DC1</sub>: Un-factored moment due to non-composite dead load (kip-ft.).  
DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).  
M<sub>DC2</sub>: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).  
DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).  
M<sub>DW</sub>: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).  
LLDF: Live Load Distribution Factor for moment and shear computed according to Article 4.6.2.2 and further IDOT provisions.  
M<sub>ℓ</sub> + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).  
M<sub>u</sub>: Strength I load combination of factored design moments (kip-ft.).  
1.25 (M<sub>DC1</sub> + M<sub>DC2</sub>) + 1.5 M<sub>DW</sub> + 1.75 M<sub>ℓ</sub> + IM  
f<sub>ℓ</sub>: Factored calculated flange lateral bending stress as calculated using Article 6.10.1.6 and as further simplified by IDOT provisions (ksi).  
Φ<sub>f</sub> M<sub>n</sub>: Factored nominal flexural resistance of the section determined as specified in Article 6.10.7.1 or A6 as applicable (kip-ft.).  
f<sub>s</sub> DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).  
M<sub>DC1</sub> / S<sub>s</sub>  
f<sub>s</sub> DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).  
M<sub>DC2</sub> / S<sub>c</sub> (3n) or M<sub>DC2</sub> / S<sub>c</sub> (cr) as applicable.  
f<sub>s</sub> DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).  
M<sub>DW</sub> / S<sub>c</sub> (3n) or M<sub>DW</sub> / S<sub>c</sub> (cr) as applicable.  
f<sub>s</sub> (ℓ + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).  
M<sub>ℓ</sub> + IM / S<sub>c</sub> (n) or M<sub>ℓ</sub> + IM / S<sub>c</sub> (cr) as applicable.  
f<sub>s</sub> + f<sub>ℓ</sub> / 2 (Service II): Sum of stresses as computed below (ksi).  
f<sub>s</sub> DC1 + f<sub>s</sub> DC2 + f<sub>s</sub> DW + 1.3 f<sub>s</sub> (ℓ + IM) + f<sub>ℓ</sub> / 2  
Service II Resistance: Composite (0.95R<sub>n</sub>F<sub>yℓ</sub>) or noncomposite (0.80R<sub>n</sub>F<sub>yℓ</sub>) stress capacity according to Article 6.10.4.2 (ksi).  
f<sub>s</sub> + f<sub>ℓ</sub> / 3 (Strength I): Sum of stresses as computed below on non-compact sections (ksi).  
1.25 (f<sub>s</sub> DC1 + f<sub>s</sub> DC2) + 1.5 f<sub>s</sub> DW + 1.75 f<sub>s</sub> (ℓ + IM) + f<sub>ℓ</sub> / 3  
Φ<sub>f</sub> F<sub>n</sub>: Factored nominal flexural resistance of the section as specified in Article 6.10.7.2 or 6.10.8 as applicable (ksi).

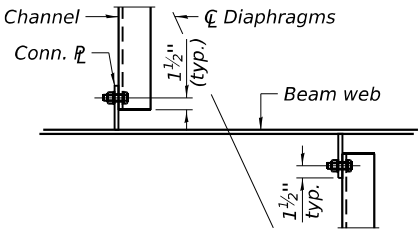
<div><div>Bowman</div><div>10 S. LaSalle Street, Suite 210 Chicago, Illinois 60602 312-674-0360 www.bowman.com</div></div>	USER NAME = Ifranceschina	DESIGNED - JW	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	STRUCTURAL STEEL DETAILS STRUCTURE NO. 101-0208/0209					F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		DRAWN - AT	REVISED -		*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	581					
	PLOT SCALE = \$SCALE\$	CHECKED - AJN	REVISED -		CONTRACT NO. 64C24									
	PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -		SCALE:	SHEET 38 OF 60 SHEETS	STA.	TO STA.	ILLINOIS FED. AID PROJECT					

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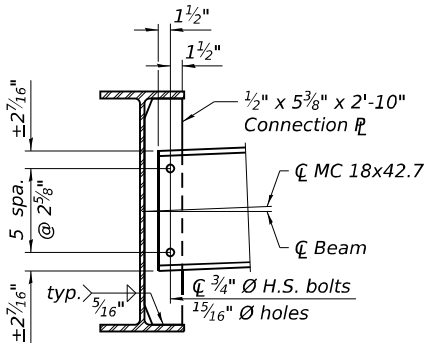


WELD LIMITS AND CLIP DETAILS

\*\* Stop welds 1/4" (±1/8") from edges as shown. Typical.



DETAIL A



DIAPHRAGM - D1

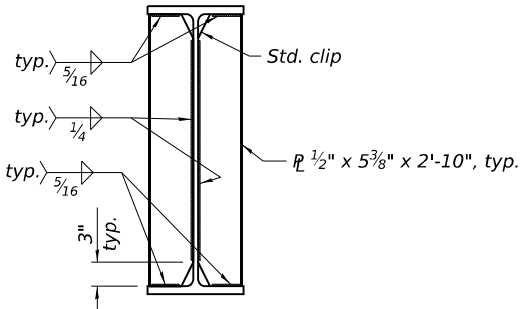
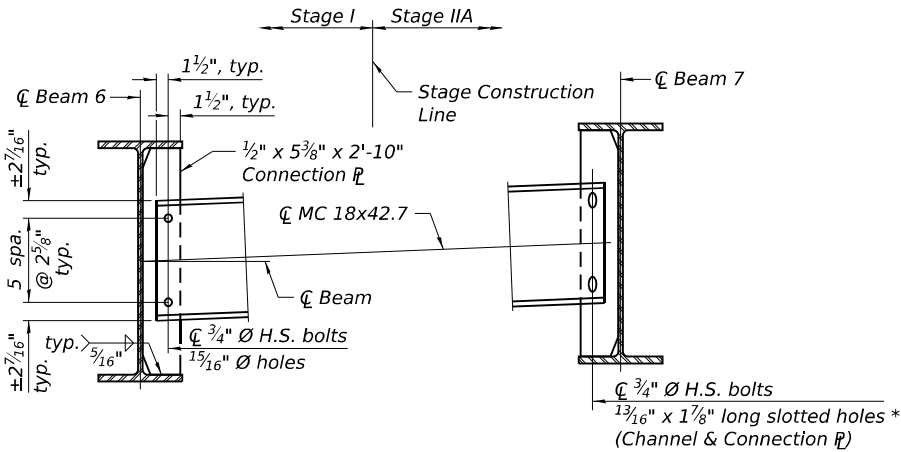


PLATE CONNECTION DETAIL  
FOR DIAPHRAGMS

\*\* Stop welds 1/4" (±1/8") from edges as shown. Typical.



DIAPHRAGM - D2

(Stage I/IIA Construction Line shown, Stage IIB/III similar)

\* Long slotted holes shall be at Beam 7 for Stage I/IIA diaphragms and at Beam 16 for Stage IIB/III diaphragms.

NOTES:

1. All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor bolts.
2. Two hardened washers required for each set of oversized holes.
3. Alternate channels of equal depth and larger weight are permitted to facilitate material acquisition. Alternate channels, if utilized, shall be provided at no additional cost to the Department.
4. Bolts in long-slots shall be finger tight until the second stage pour is complete, and position slots so bolts start at one end with no concrete load and finish near the opposite end under deck load, allowing maximum displacement without laterally stressing main members. All holes shall have appropriate hardened or plate washers.

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PLOT DATE = 8/12/2024	CHECKED - AJN	REVISED -
	DATE - 02/27/24	REVISED -

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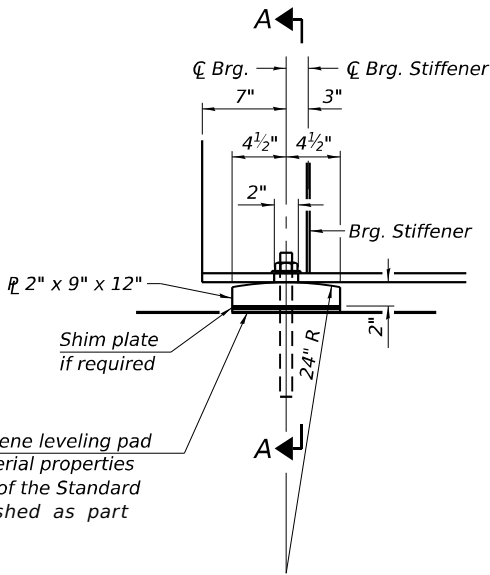
STRUCTURAL STEEL DIAPHRAGMS  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 39 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	582
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

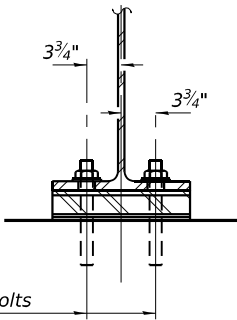
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

MODEL: SHEET  
FILE NAME: c:\pwword\benesch projects\projects\173871\284C24\_SHT\_0208-0209-CN\_BEARING DETAILS.dgn



1/8" Elastomeric neoprene leveling pad according to the material properties of Article 1052.02(a) of the Standard Specifications. Furnished as part of Contract 64U51.

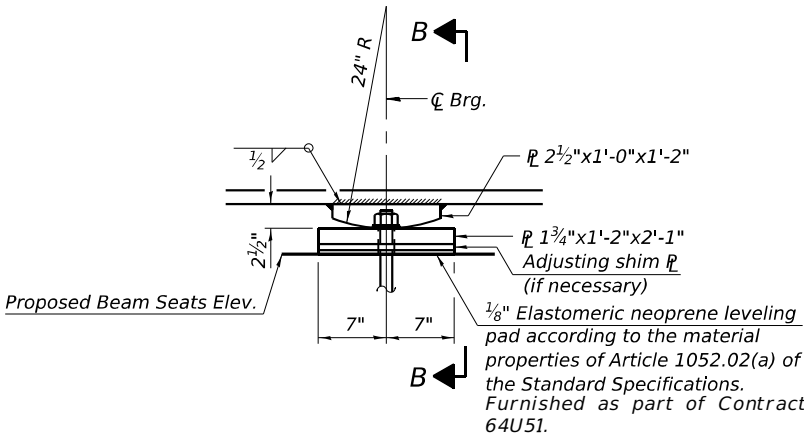
ELEVATION AT ABUTMENT



1"  $\varnothing$  x 12" All-thread anchor bolts (Grade 55) with 2 1/4" x 2 1/4" x 5/16"  $\varnothing$  washer under nut. 1 3/8" x 2" slotted hole in flange. 1 1/2"  $\varnothing$  Holes in bearing plate.

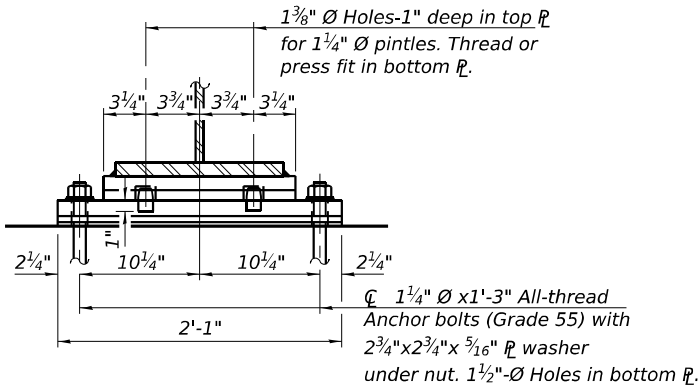
SECTION A-A

**FIXED BEARING**  
Typical at Each Beam End  
(40 Req'd)

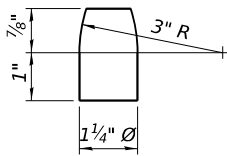


ELEVATION AT PIER

**FIXED BEARING**  
Typical at Each Beam at Piers  
(40 Req'd)



SECTION B-B



PINTLE

**NOTES:**

- Anchor bolts at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
- Two 1/8 in. adjusting shims shall be furnished as part of Contract 64U51 for each bearing in addition to all other plates or shims and placed as shown on bearing details.
- All (embedded and separate) bearing plates, side retainers, and anchor bolts, nuts, washers and pintles shall be galvanized according to AASHTO M111 or M232 as applicable.
- The structural steel plates and pintles of the bearing shall conform to the requirements of AASHTO M270 Grade 50.
- Installation of all bearing plates, shims, leveling pads, and pintles shall be included in the cost of Erecting Structural Steel.

**BILL OF MATERIAL**

Item	Unit	Total
Anchor Bolts, 1"	Each	80
Anchor Bolts, 1 1/4"	Each	80

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**DEPARTMENT OF TRANSPORTATION**

**BEARING DETAILS**  
**STRUCTURE NO. 101-0208/0209**

SCALE: SHEET 40 OF 60 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	583
				CONTRACT NO. 64C24
		ILLINOIS	FED. AID PROJECT	

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

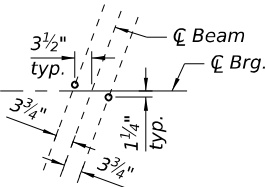
NOTES:

1. Pour steps monolithically with cap.  
2. Bar terminators, paid for separately. See Total Bill of Material.  
3. For details of piles see sheet 55 of 60.

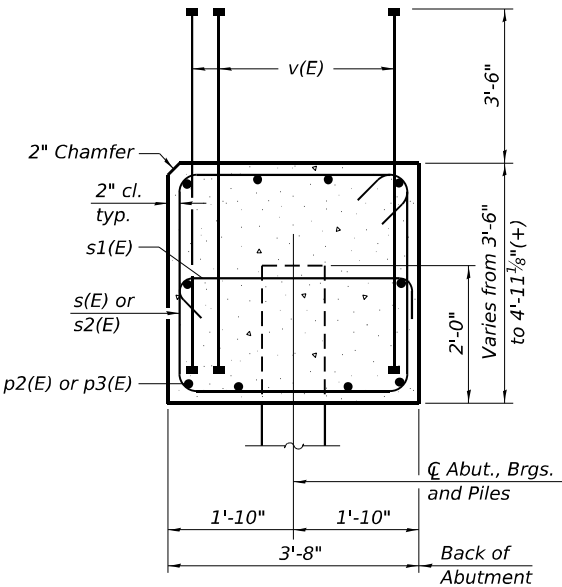
BEARING SEAT

ELEVATIONS

Beam	Elev.
1	795.12
2	795.39
3	795.66
4	795.94
5	796.21
6	796.49



ANCHOR BOLT LAYOUT



SEC. THRU ABUT.

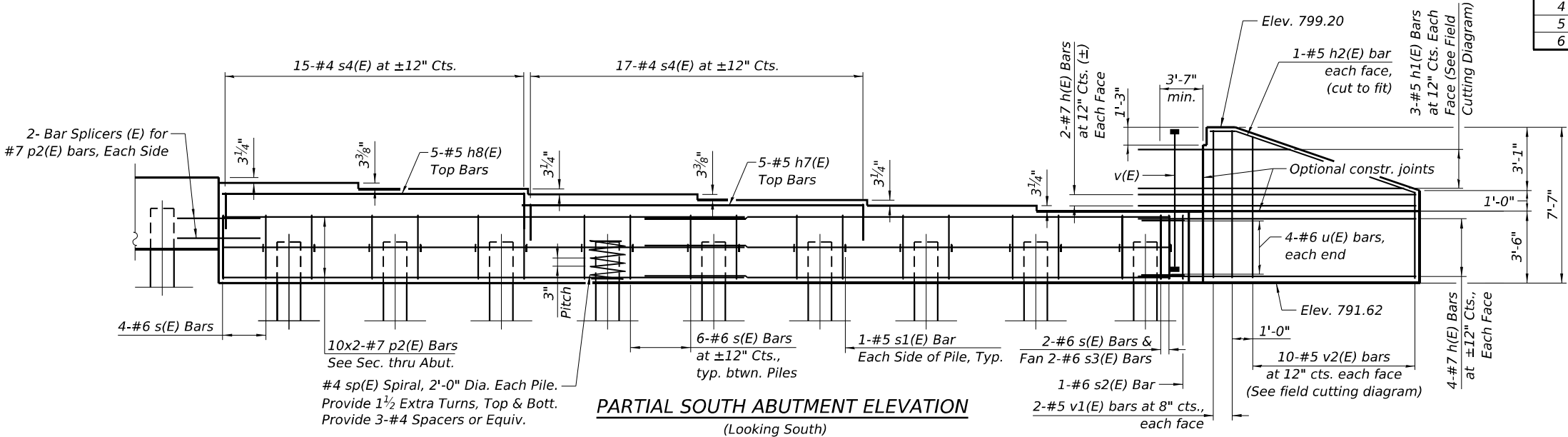
Dimensions at right angles to abutment.

MINIMUM BAR LAP

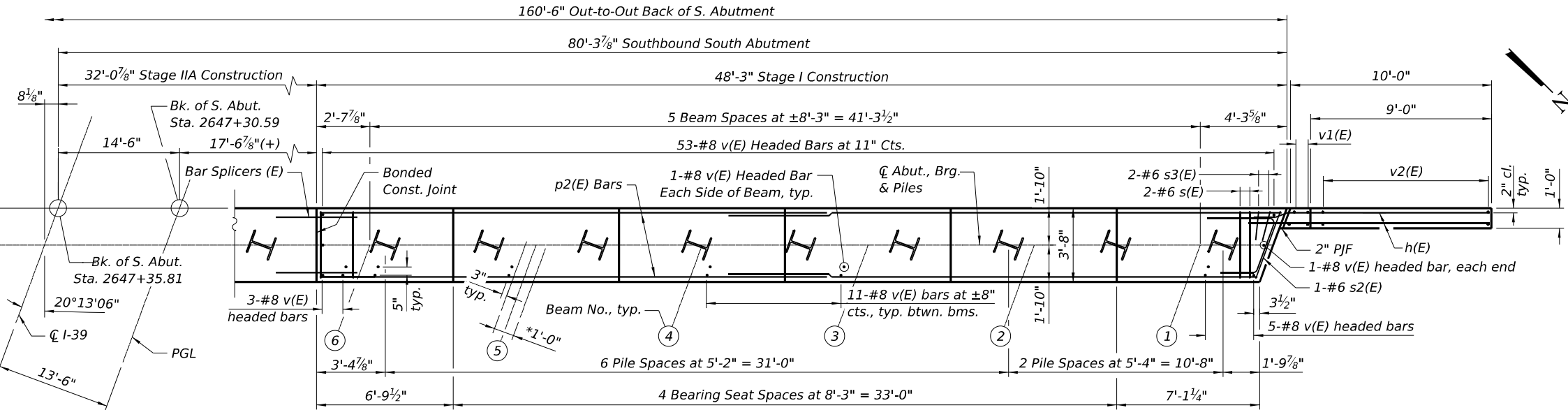
- #5 Bar = 3'-7"  
#6 Bar = 4'-4"  
#7 Bar = 5'-0"

PILE DATA

Type: HP12x63 with pile shoes  
Nominal Required Bearing: 497 kips  
Factored Resistance Available: 273 kips  
Est. Length: 42 Ft  
No. Production Piles: 15  
No. Test Piles: 1

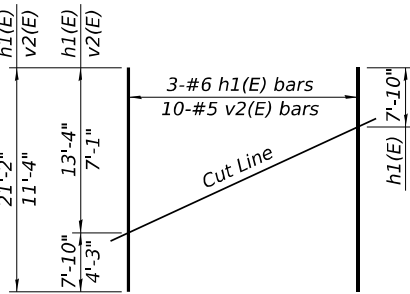


PARTIAL SOUTH ABUTMENT ELEVATION  
(Looking South)



PARTIAL SOUTH ABUTMENT PLAN

\*Limits of beam flange



FIELD CUTTING DIAGRAM

Order h1(E) and v2(E) full length. Cut as shown and use remainder of bars in opposite face of wingwall.

BAR v(E)

(Headed. 440-#8 Bar terminators)

BAR h2(E)

BAR s(E) & s2(E)

BAR s1(E)

BAR s3(E) & s4(E)

BAR u(E)

MODEL: SHEET  
FILE NAME: c:\paword\benesch\projects\101-0208-0209-CN-ABUT-SOUTH-S1.dgn

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SOUTH ABUTMENT - SOUTHBOUND STAGE 1  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 41 OF 60 SHEETS STA. TO STA.

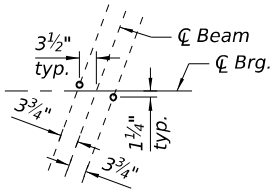
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	584
				CONTRACT NO. 64C24
		ILLINOIS	FED. AID PROJECT	

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

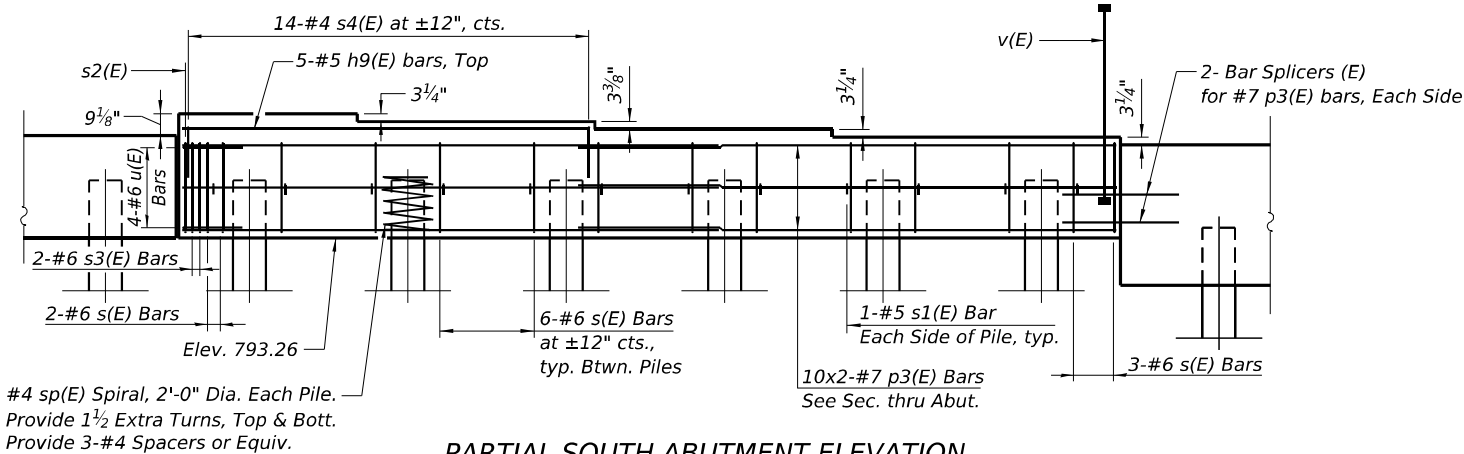


NOTES:

1. Pour steps monolithically with cap.  
2. Bar terminators, paid for separately. See Total Bill of Material.  
3. For details of piles see sheet 55 of 60.

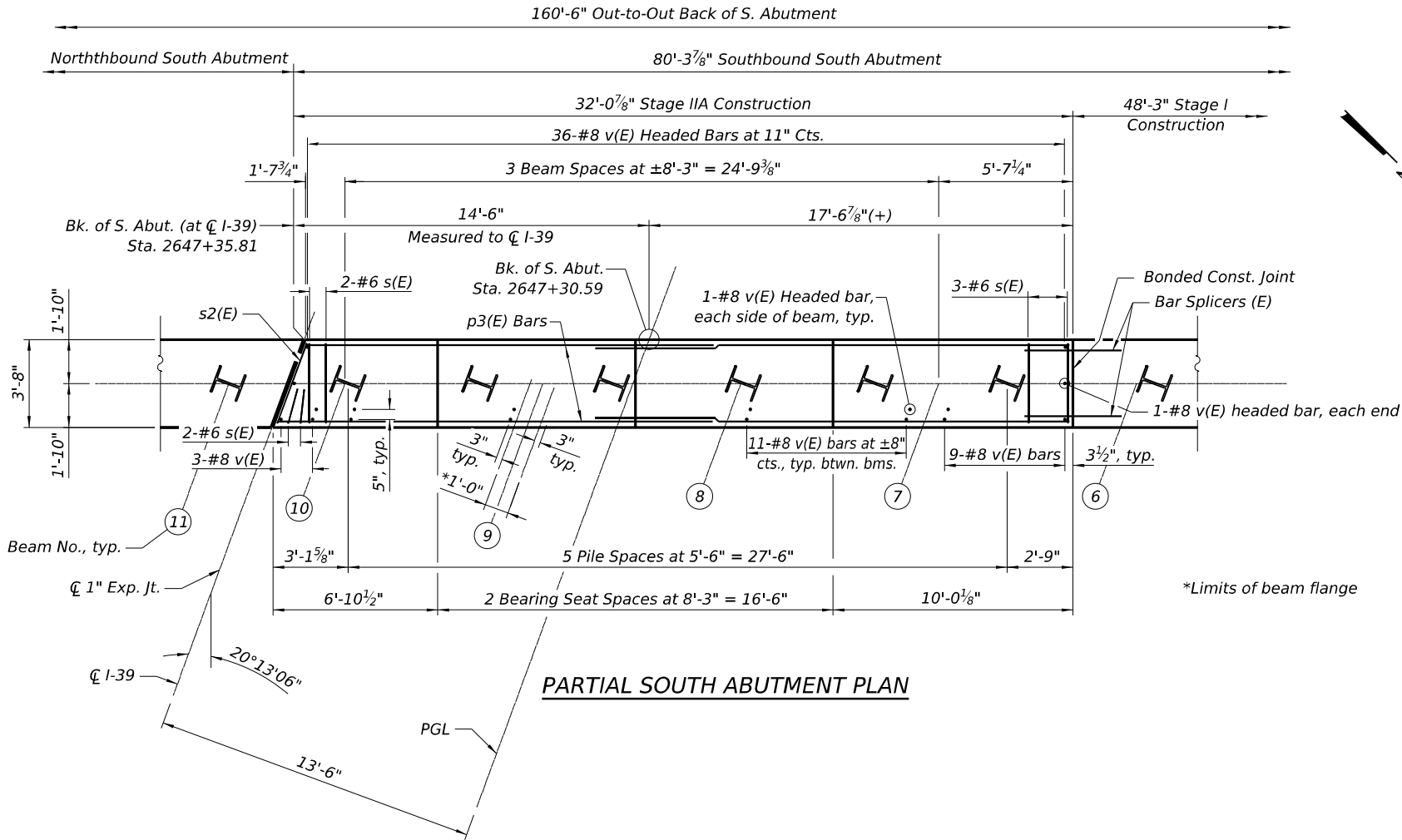


ANCHOR BOLT LAYOUT



BEARING SEAT  
ELEVATIONS

Beam	Elev.
7	796.76
8	797.03
9	797.31
10	797.58



SOUTHBOUND SOUTH ABUTMENT  
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	6	#7	14'-10"	
h1(E)	3	#6	21'-2"	
h2(E)	2	#5	9'-9"	
h7(E)	5	#5	16'-8"	
h8(E)	5	#5	14'-9"	
h9(E)	5	#5	14'-2"	
p2(E)	20	#7	26'-5"	
p3(E)	20	#7	19'-0"	
s(E)	89	#6	14'-4"	
s1(E)	30	#5	4'-4"	
s2(E)	2	#6	14'-8"	
s3(E)	4	#6	6'-2"	
s4(E)	46	#4	6'-10"	
**sp(E)	15	#4	2'-0"	
u(E)	8	#6	12'-0"	
v(E)	220	#8	6'-6"	
v1(E)	4	#5	7'-3"	
v2(E)	10	#5	11'-4"	
Structure Excavation			Cu. Yd.	92
Concrete Structures			Cu. Yd.	49.7
Reinforcement Bars, Epoxy Coated			Pound	9,900
Furnishing Steel				
Piles HP12x63			Foot	630
Driving Piles			Foot	630
Test Pile Steel HP12x63			Each	1
Pile Shoes			Each	15

\*\* Length is height of spiral

MODEL: SHEET  
FILE NAME: c:\paword\benesch projects\projects\173871\264C24\_SHT\_0208-0209-CN\_ABUT-SOUTH-S2A.dgn

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SOUTH ABUTMENT - SOUTHBOUND STAGE 2A  
STRUCTURE NO. 101-0208/0209

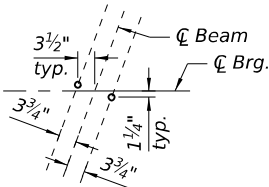
SCALE: SHEET 42 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	585
CONTRACT NO. 64C24				
ILLINOIS		FED. AID PROJECT		

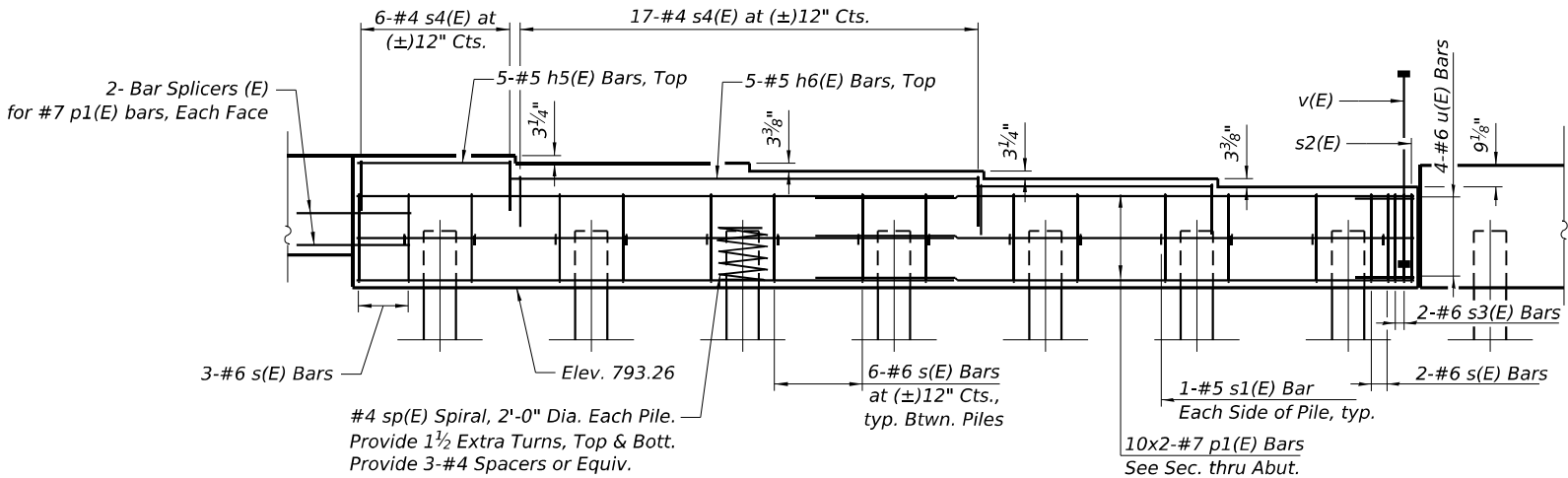
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

NOTES:

1. Pour steps monolithically with cap.  
2. Bar terminators, paid for separately. See Total Bill of Material.  
3. For details of piles see sheet 55 of 60.



ANCHOR BOLT LAYOUT



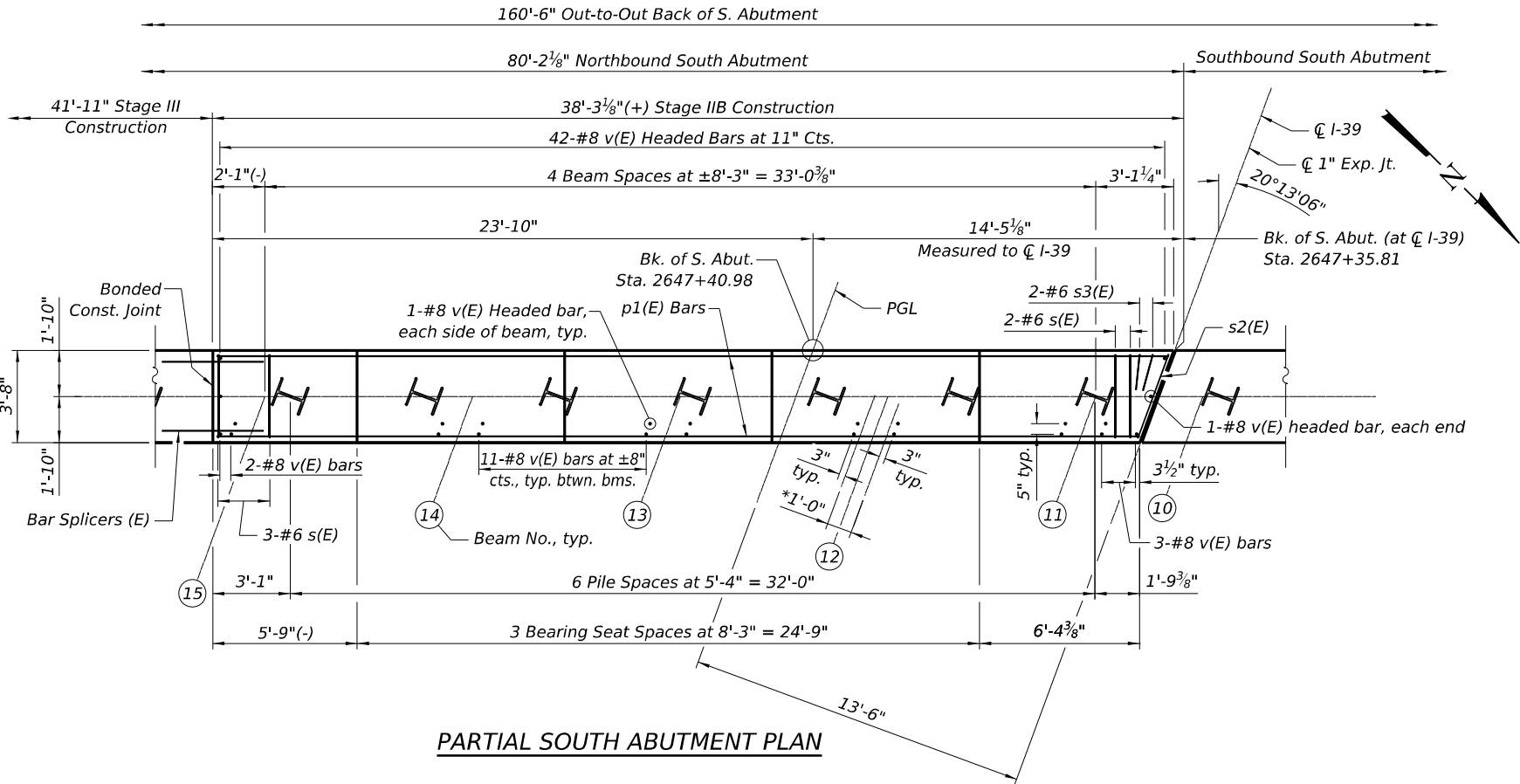
PARTIAL SOUTH ABUTMENT ELEVATION

(Looking South)

BEARING SEAT

ELEVATIONS

Beam	Elev.
11	796.82
12	797.10
13	797.37
14	797.65
15	797.92



PARTIAL SOUTH ABUTMENT PLAN

\*Limits of beam flange

NORTHBOUND SOUTH ABUTMENT

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	9	#7	14'-10"	
h1(E)	2	#6	21'-2"	
h2(E)	2	#5	9'-9"	
h3(E)	5	#5	15'-4"	
h4(E)	5	#5	17'-0"	
h5(E)	5	#5	5'-5"	
h6(E)	5	#5	16'-6"	
p(E)	20	#7	23'-11"	
p1(E)	20	#7	21'-5"	
s(E)	89	#6	14'-4"	
s1(E)	30	#5	4'-4"	
s2(E)	2	#6	14'-8"	
s3(E)	4	#6	6'-2"	
s4(E)	56	#4	6'-10"	
**sp(E)	15	#4	2'-0"	
u(E)	8	#6	12'-0"	
v(E)	217	#8	6'-6"	
v3(E)	4	#5	8'-9"	
v4(E)	10	#5	14'-6"	
Structure Excavation		Cu. Yd.	92	
Concrete Structures		Cu. Yd.	49.2	
Reinforcement Bars, Epoxy Coated		Pound	8,990	
Furnishing Steel Piles HP12x63		Foot	672	
Driving Piles		Foot	672	
Pile Shoes		Each	16	

\*\* Length is height of spiral

MODEL: SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\173871\264C24\_SHT\_0208-0209-CN\_ABUT-SOUTH-S2B.dgn

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SOUTH ABUTMENT - NORTHBOUND STAGE 2B  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 43 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	586
				CONTRACT NO. 64C24
		ILLINOIS	FED. AID PROJECT	

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

1. *Four steps monolithically with cap.*
2. *Bar terminators, paid for separately. See Total Bill of Material.*
3. *For details of piles see sheet 55 of 60.*



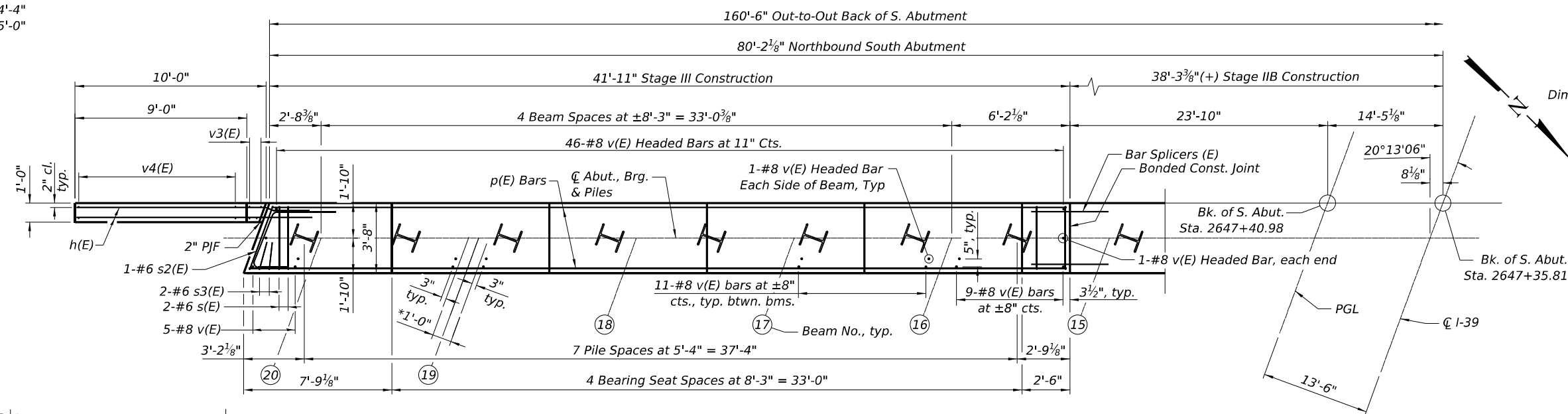
Beam	Elev.
15	797.92
16	798.19
17	798.47
18	798.74
19	799.01
20	799.29



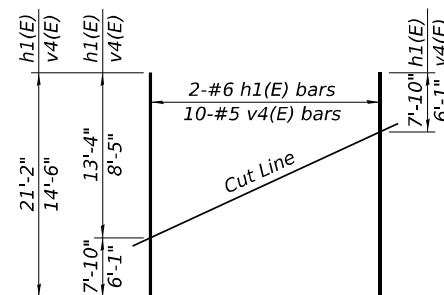
Dimensions at right angles to abutment.

#5 Bar = 3'-7"  
#6 Bar = 4'-4"  
#7 Bar = 5'-0"

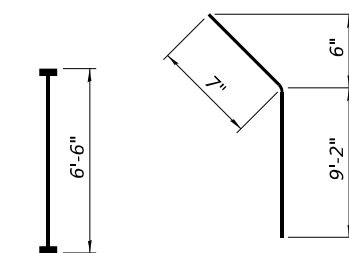
(Looking South)



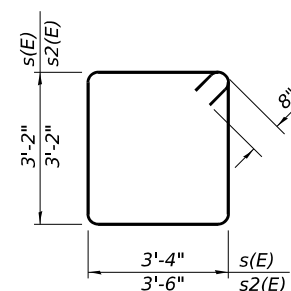
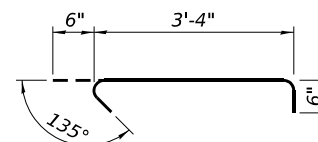
\*Limits of beam flange



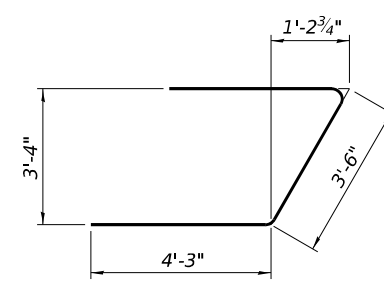
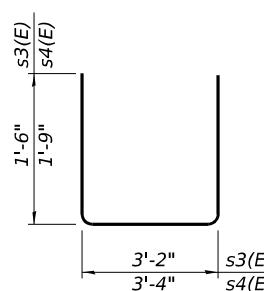
Order h1(E) and v4(E) full length. Cut as shown and use remainder of bars in opposite face of wingwall.



(Headed, 434-#8 Bar terminators)


$$BAR\ s1(E)$$


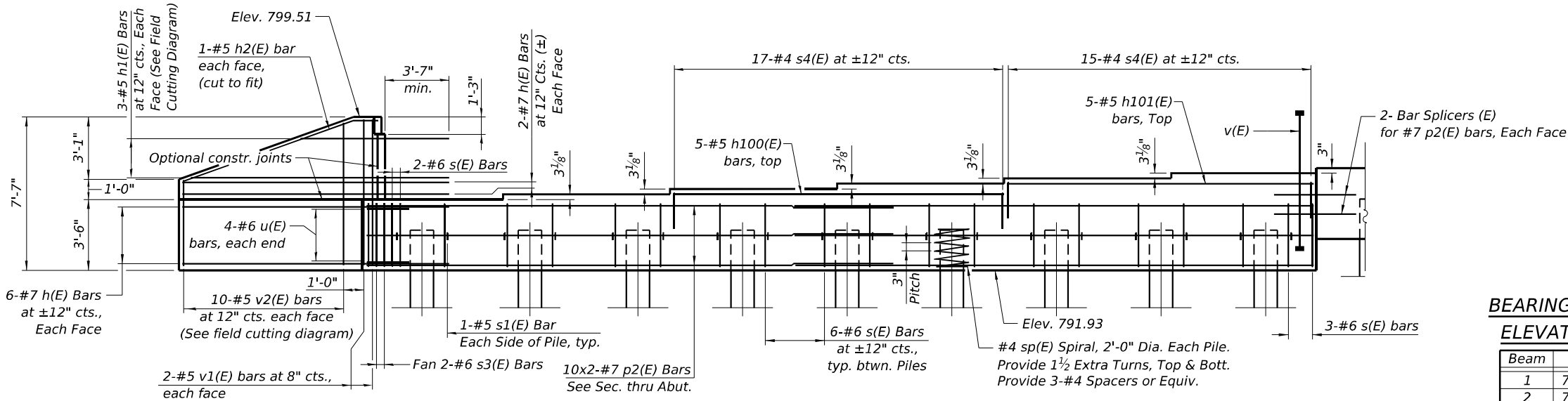
*BAR*  $s3(E)$  &  $s4(E)$


$$BAR \ u(E)$$

Type: HP12x63 with Pile Shoes  
Nominal Required Bearing: 497 kips  
Factored Resistance Available: 273 kips  
Est. Length: 42 Ft  
No. Production Piles: 16  
No. Test Piles: 0

NOTES:

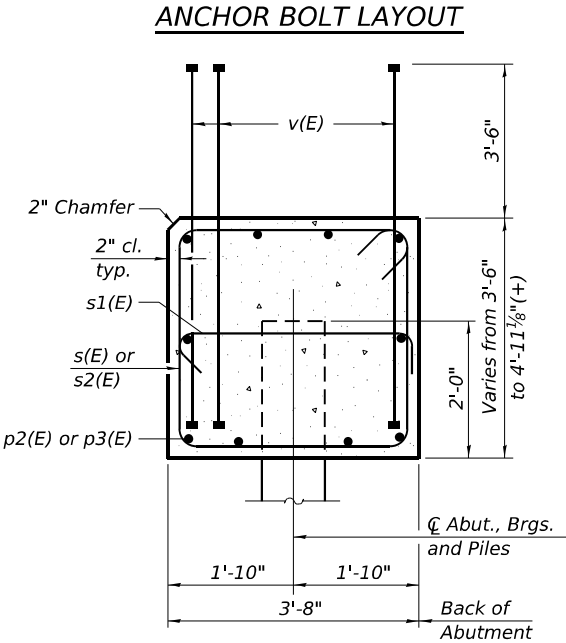
1. Pour steps monolithically with cap.  
2. Bar terminators, paid for separately. See Total Bill of Material.  
3. For details of piles see sheet 55 of 60.



PARTIAL NORTH ABUTMENT ELEVATION  
(Looking North)

BEARING SEAT  
ELEVATIONS

Beam	Elev.
1	795.43
2	795.69
3	795.95
4	796.21
5	796.47
6	796.73

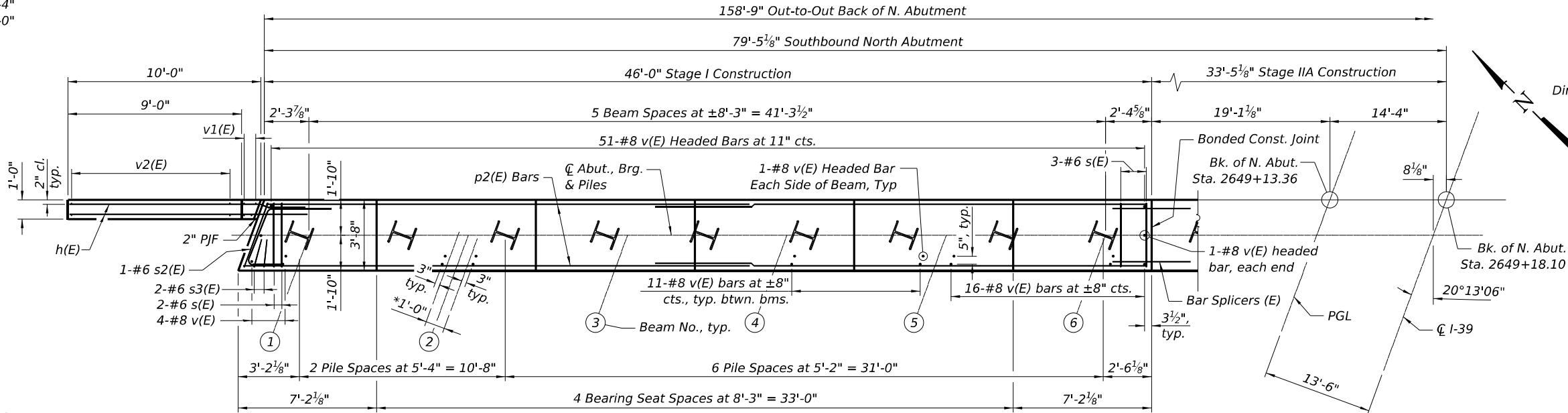


SEC. THRU ABUT.

Dimensions at right angles to abutment.

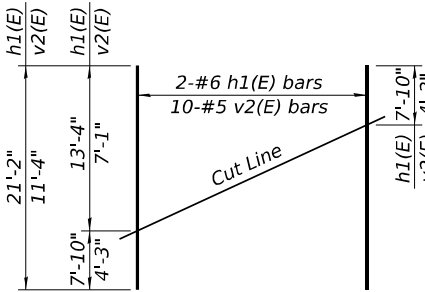
MINIMUM BAR LAP

- #5 Bar = 3'-7"  
#6 Bar = 4'-4"  
#7 Bar = 5'-0"



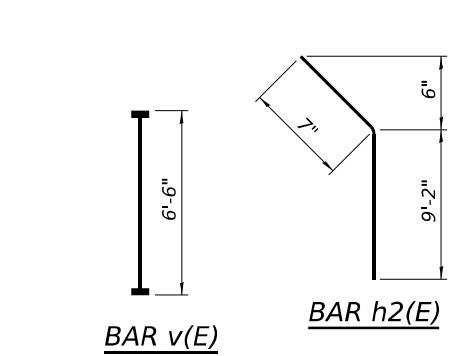
PARTIAL NORTH ABUTMENT PLAN

\*Limits of beam flange

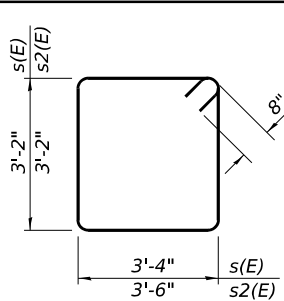


FIELD CUTTING DIAGRAM

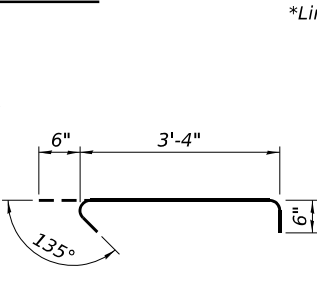
Order h1(E) and v2(E) full length. Cut as shown and use remainder of bars in opposite face of wingwall.



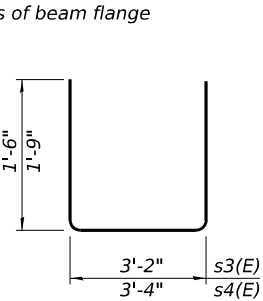
BAR v(E)  
(Headed. 438-#8 Bar terminators)



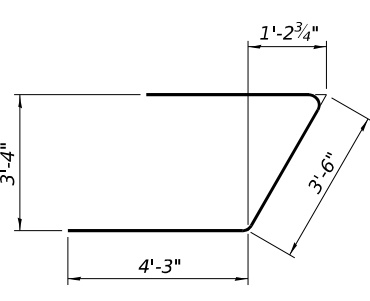
BAR s(E) & s2(E)



BAR s1(E)



BAR s3(E) & s4(E)



BAR u(E)

PILE DATA

Type: HP12x63 with Pile Shoes  
Nominal Required Bearing: 497 kips  
Factored Resistance Available: 273 kips  
Est. Length: 53 Ft  
No. Production Piles: 16  
No. Test Piles: 0

MODEL: SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\101-0208-0209-CN-ABUT-NORTH-S1.dgn

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NORTH ABUTMENT - SOUTHBOUND STAGE 1  
STRUCTURE NO. 101-0208/0209

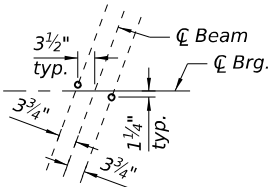
SCALE: SHEET 45 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	588
				CONTRACT NO. 64C24
		ILLINOIS	FED. AID PROJECT	

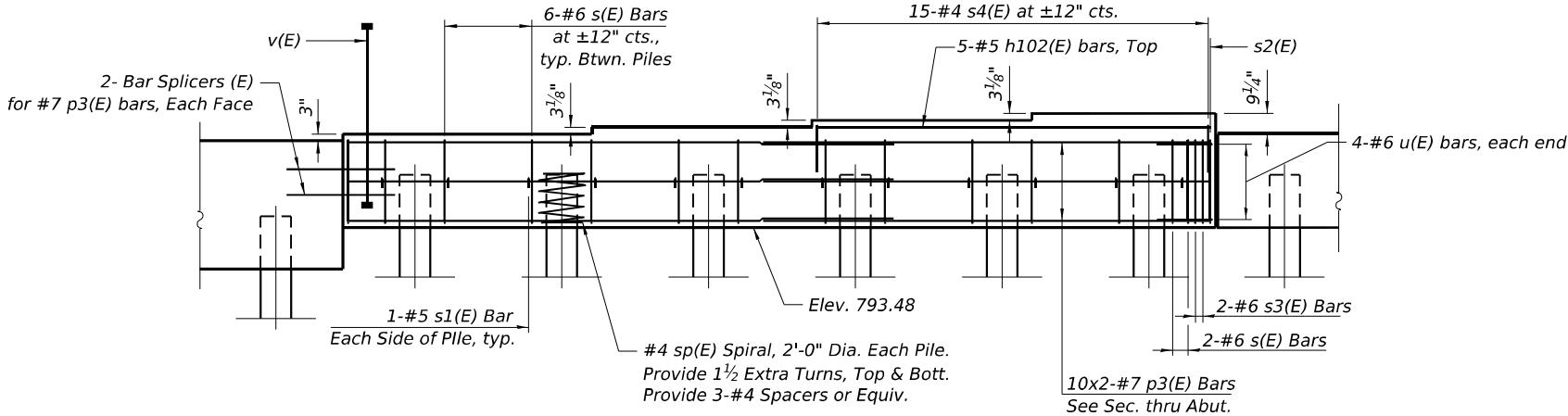
\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

NOTES:

1. Pour steps monolithically with cap.  
2. Bar terminators, paid for separately. See Total Bill of Material.  
3. For details of piles see sheet 55 of 60.



ANCHOR BOLT LAYOUT



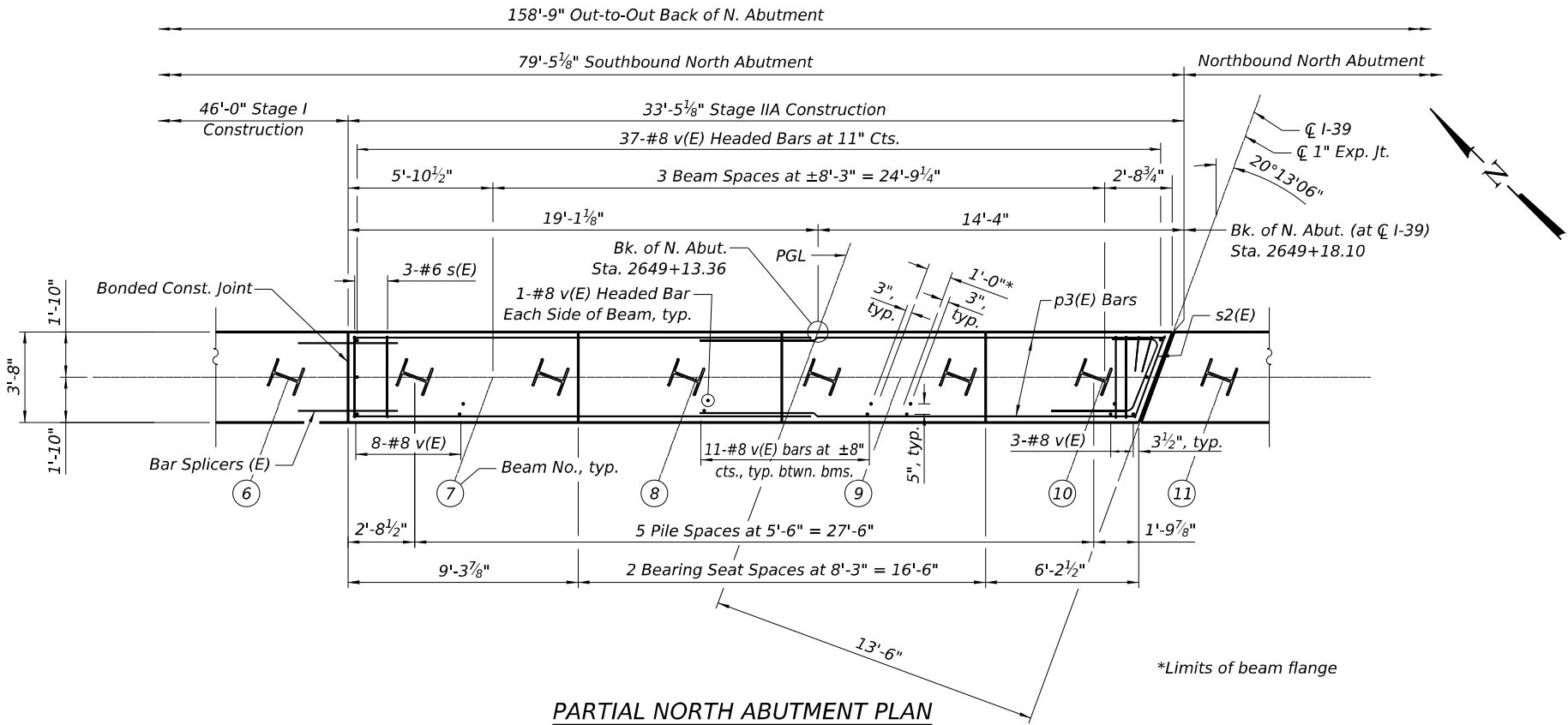
PARTIAL NORTH ABUTMENT ELEVATION

(Looking North)

BEARING SEAT

ELEVATIONS

Beam	Elev.
7	796.98
8	797.24
9	797.50
10	797.76



PARTIAL NORTH ABUTMENT PLAN

SOUTHBOUND NORTH ABUTMENT

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	9	#7	14'-10"	
h1(E)	2	#6	21'-2"	
h2(E)	2	#5	9'-9"	
h100(E)	5	#5	16'-8"	
h101(E)	5	#5	14'-9"	
h102(E)	5	#5	14'-2"	
p2(E)	20	#7	26'-5"	
p3(E)	20	#7	19'-0"	
s(E)	88	#6	14'-4"	
s1(E)	30	#5	4'-4"	
s2(E)	2	#6	14'-8"	
s3(E)	4	#6	6'-2"	
s4(E)	47	#4	6'-10"	
**sp(E)	15	#4	2'-0"	
u(E)	8	#6	12'-0"	
v(E)	219	#8	6'-6"	
v1(E)	4	#5	7'-3"	
v2(E)	10	#5	11'-4"	
Structure Excavation			Cu. Yd.	92
Concrete Structures			Cu. Yd.	49.2
Reinforcement Bars, Epoxy Coated			Pound	9,870
Furnishing Steel				
Piles HP12x63			Foot	848
Driving Piles			Foot	848
Pile Shoes			Each	16

\*\*Length is height of spiral

MODEL: SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\10173871\10284C24\_SHT\_0208-0209-CN\_ABUT-NORTH-S2A.dgn

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USER NAME = Ifranceschina	DESIGNED - JW	REVISED -
DRAWN - AT	REVISED -	
PLOT SCALE = SSCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT - SOUTHBOUND STAGE 2A  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 46 OF 60 SHEETS STA. TO STA.

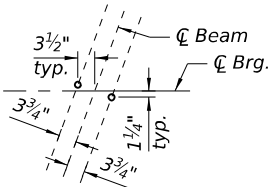
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	589
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

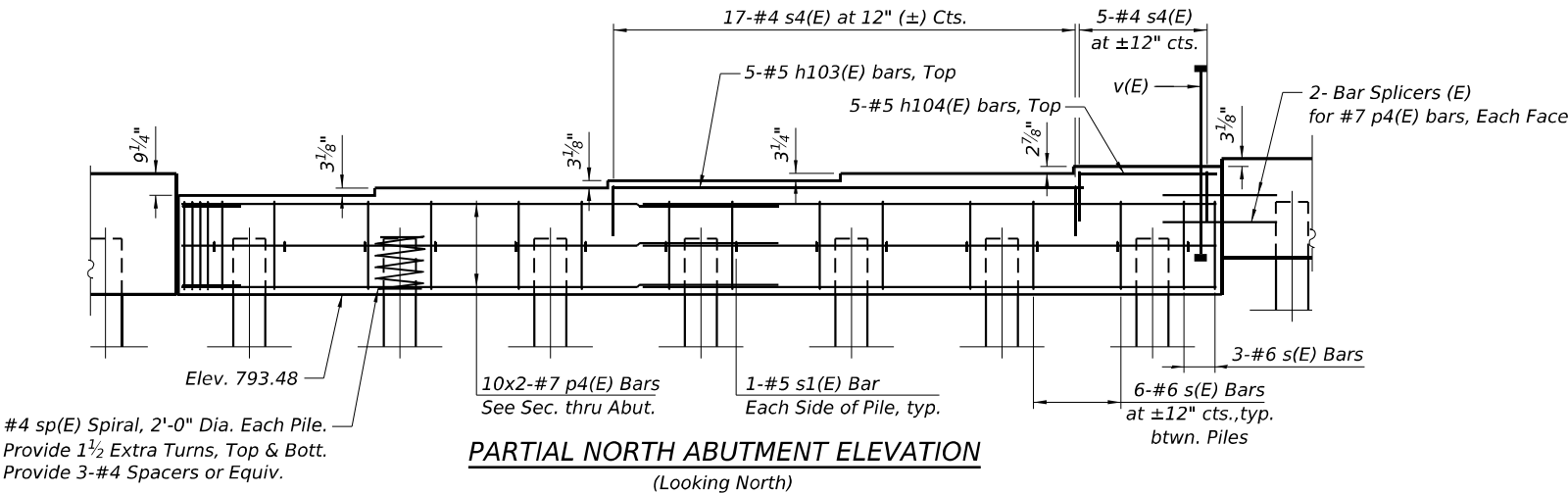


NOTES:

1. Pour steps monolithically with cap.  
2. Bar terminators, paid for separately. See Total Bill of Material.  
3. For details of piles see sheet 55 of 60.

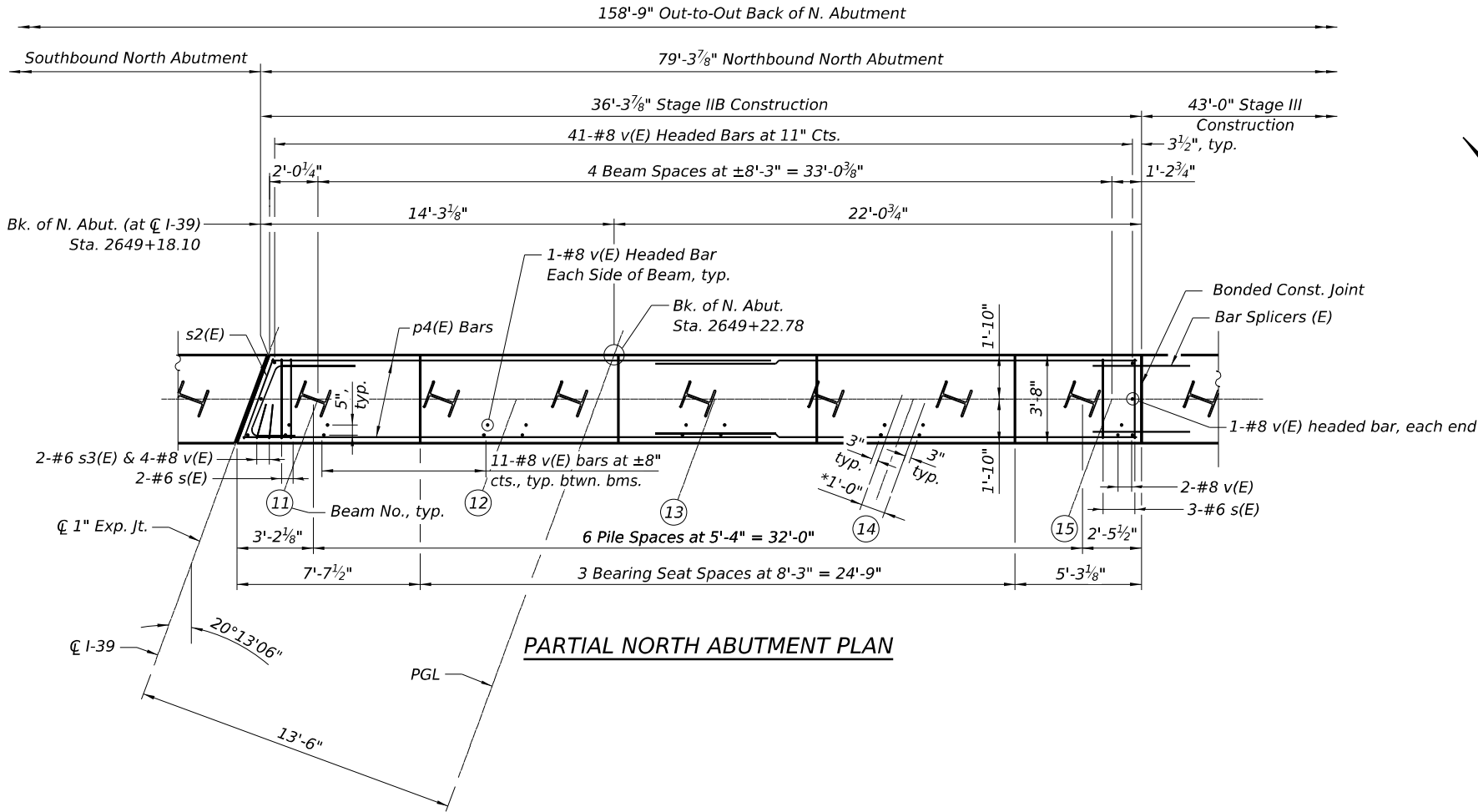


ANCHOR BOLT LAYOUT



BEARING SEAT ELEVATIONS

Beam	Elev.
11	796.99
12	797.25
13	797.51
14	797.78
15	798.02



PILE DATA

Type: HP12x63 with Pile Shoes  
Nominal Required Bearing: 497 kips  
Factored Resistance Available: 273 kips  
Est. Length: 53 Ft  
No. Production Piles: 16  
No. Test Piles: 0

MODEL: SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\10173871\10264C24\_SHT\_0208-0209-CN\_ABUT-NORTH-S2B.dgn

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DRAWN - AT	REVISED -	
PLOT SCALE = \$SCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT - NORTHBOUND STAGE 2B  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 47 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	590
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

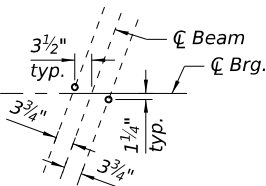
NOTES:

1. Pour steps monolithically with cap.  
2. Bar terminators, paid for separately. See Total Bill of Material.  
3. For details of piles see sheet 55 of 60.

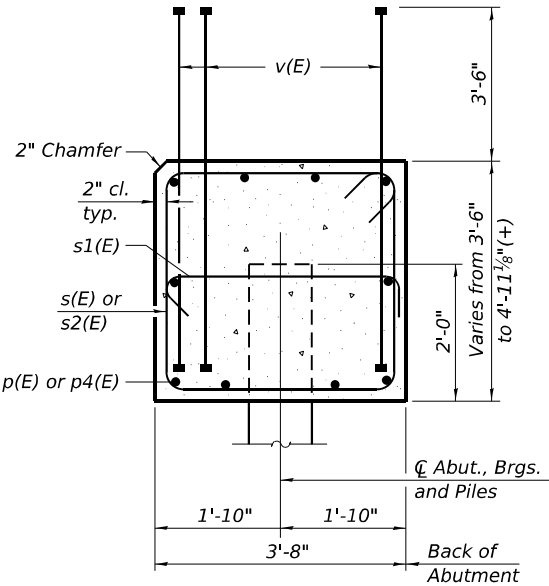
BEARING SEAT

ELEVATIONS

Beam	Elev.
16	798.28
17	798.54
18	798.79
19	799.05
20	799.31



ANCHOR BOLT LAYOUT



SEC. THRU ABUT.

Dimensions at right angles to abutment.

NORTHBOUND NORTH ABUTMENT

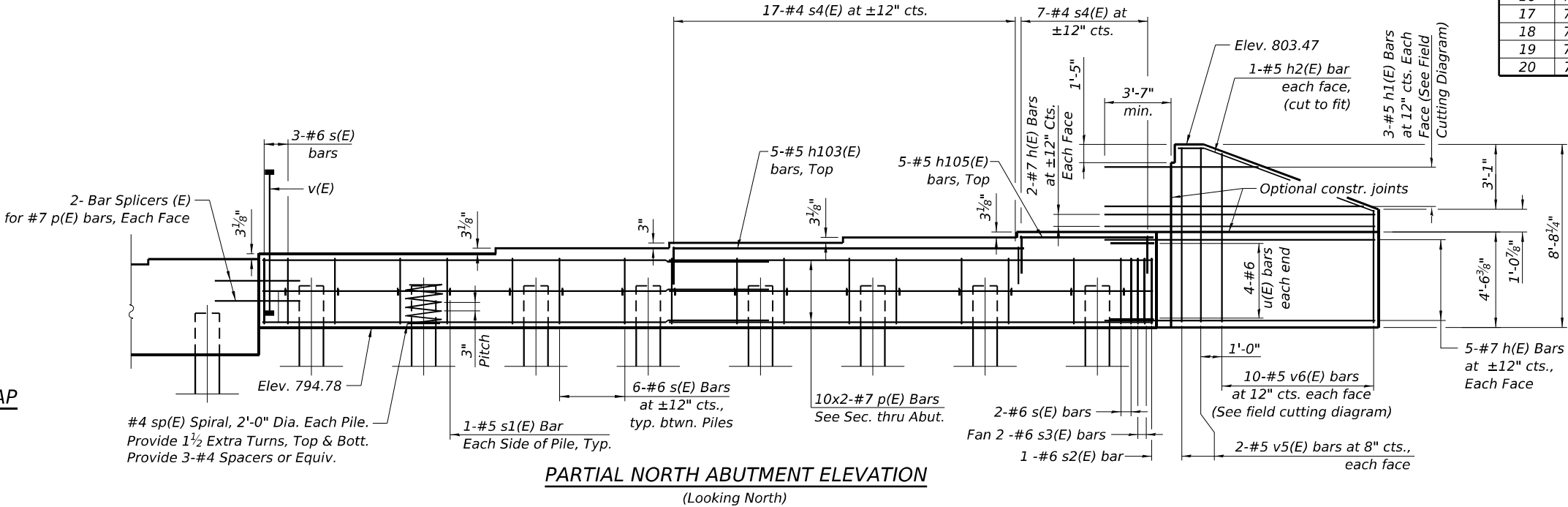
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	7	#7	14'-10"	
h1(E)	2	#6	21'-2"	
h2(E)	2	#5	9'-9"	
h103(E)	10	#5	16'-8"	
h104(E)	5	#5	4'-11"	
h105(E)	5	#5	6'-4"	
p(E)	20	#7	21'-2"	
p4(E)	20	#7	23'-10"	
s(E)	88	#6	14'-4"	
s1(E)	30	#5	4'-4"	
s2(E)	2	#6	14'-8"	
s3(E)	4	#6	6'-2"	
s4(E)	41	#4	6'-10"	
**sp(E)	15	#4	2'-0"	
u(E)	8	#6	12'-0"	
v(E)	218	#8	6'-6"	
v5(E)	4	#5	7'-5"	
v6(E)	10	#5	13'-4"	
Structure Excavation		Cu. Yd.	92	
Concrete Structures		Cu. Yd.	49.2	
Reinforcement Bars, Epoxy Coated		Pound	8,850	
Furnishing Steel				
Piles HP12x63		Foot	848	
Driving Piles		Foot	848	
Pile Shoes		Each	16	

\*\* Length is height of spiral

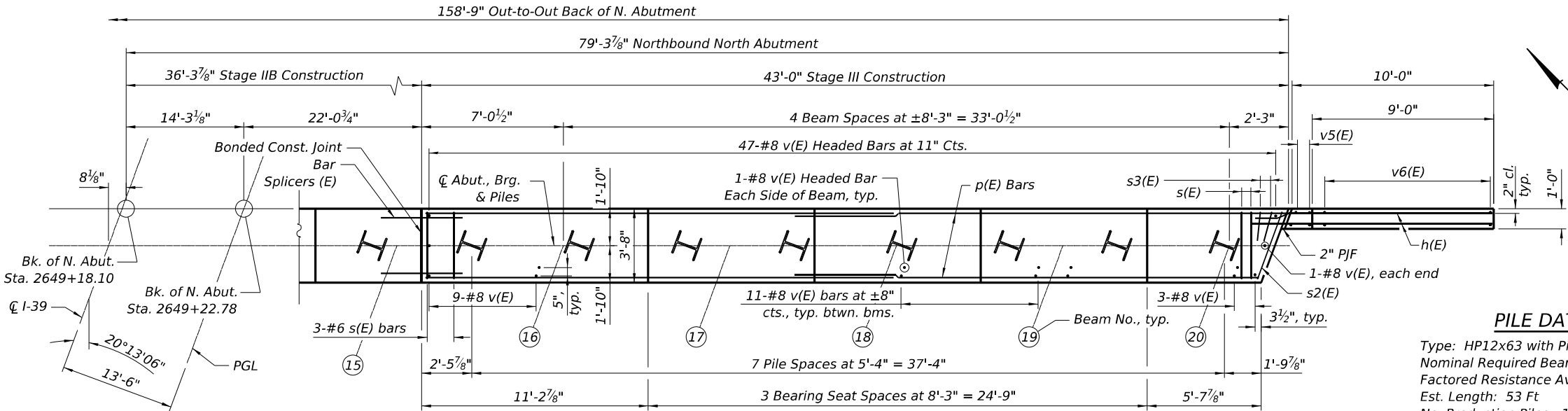
MINIMUM BAR LAP

- #5 Bar = 3'-7"  
#6 Bar = 4'-4"  
#7 Bar = 5'-0"



PARTIAL NORTH ABUTMENT ELEVATION

(Looking North)



PARTIAL NORTH ABUTMENT PLAN

PILE DATA

Type: HP12x63 with Pile Shoes  
Nominal Required Bearing: 497 kips  
Factored Resistance Available: 273 kips  
Est. Length: 53 Ft  
No. Production Piles: 16  
No. Test Piles: 0

FIELD CUTTING DIAGRAM

Order h1(E) and v6(E) full length. Cut as shown and use remainder of bars in opposite face of wingwall.

BAR v(E)

(Headed. 436-#8 Bar terminators)

BAR h2(E)

BAR s(E) & s2(E)

BAR s1(E)

BAR s3(E) & s4(E)

BAR u(E)

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USER NAME	= Ifranceschina
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PLOT DATE	= 8/12/2024

DESIGNED	- JW
DRAWN	- AT
CHECKED	- AJN
DATE	- 02/27/24

REVISED	-
REVISED	-
REVISED	-
REVISED	-

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT - NORTHBOUND STAGE 3  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 48 OF 60 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	591
			CONTRACT NO. 64C24	

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*





MODEL: SHEET  
FILE NAME: c:\paword\benesch projects\projects\173871\101-0208-0209-CN\_PIER-2-SB.dgn

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PLOT SCALE = SSCALE\$  
PLOT DATE = 8/12/2024

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REVISD -  
CHECKED - AJN  
DATE - 02/27/24

REVISD -  
REVISD -  
REVISD -  
REVISD -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

PIER 2 - SOUTHBOUND  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 51 OF 60 SHEETS STA. TO STA.

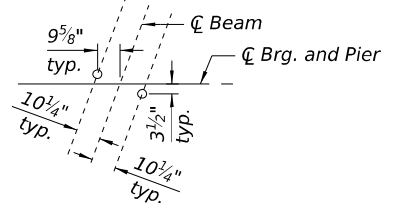
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	594
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

Notes:  
Space reinforcement in cap to miss anchor bolts.  
Pour steps monolithically with cap.  
Cost of PJJ included in Concrete Structures.  
For details of piles, see sheet 55 of 60.

#### PILE DATA

Type: HP 12x63 with Pile Shoes  
Nominal Required Bearing: 497 kips  
Factored Resistance Available: 273 kips  
Est. Length: 24 ft.  
No. Production Piles: 25  
No. Test Piles: 1

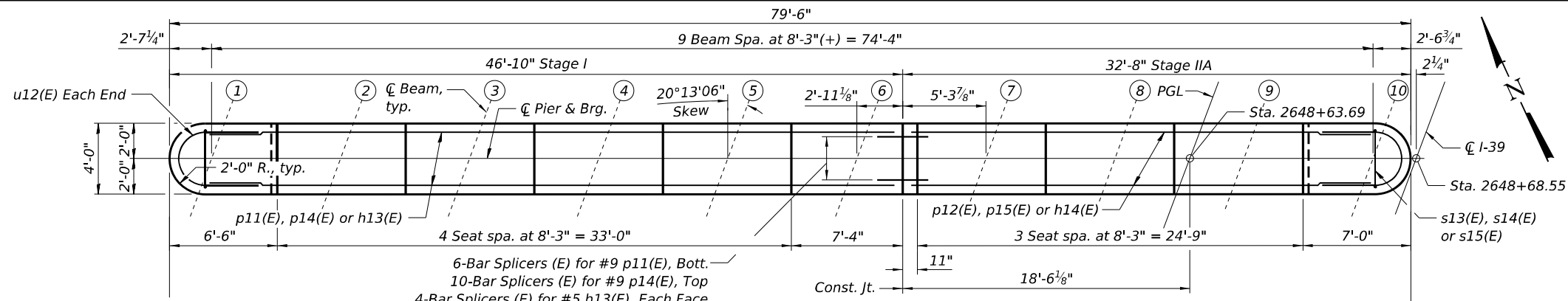
#### ANCHOR BOLT LAYOUT



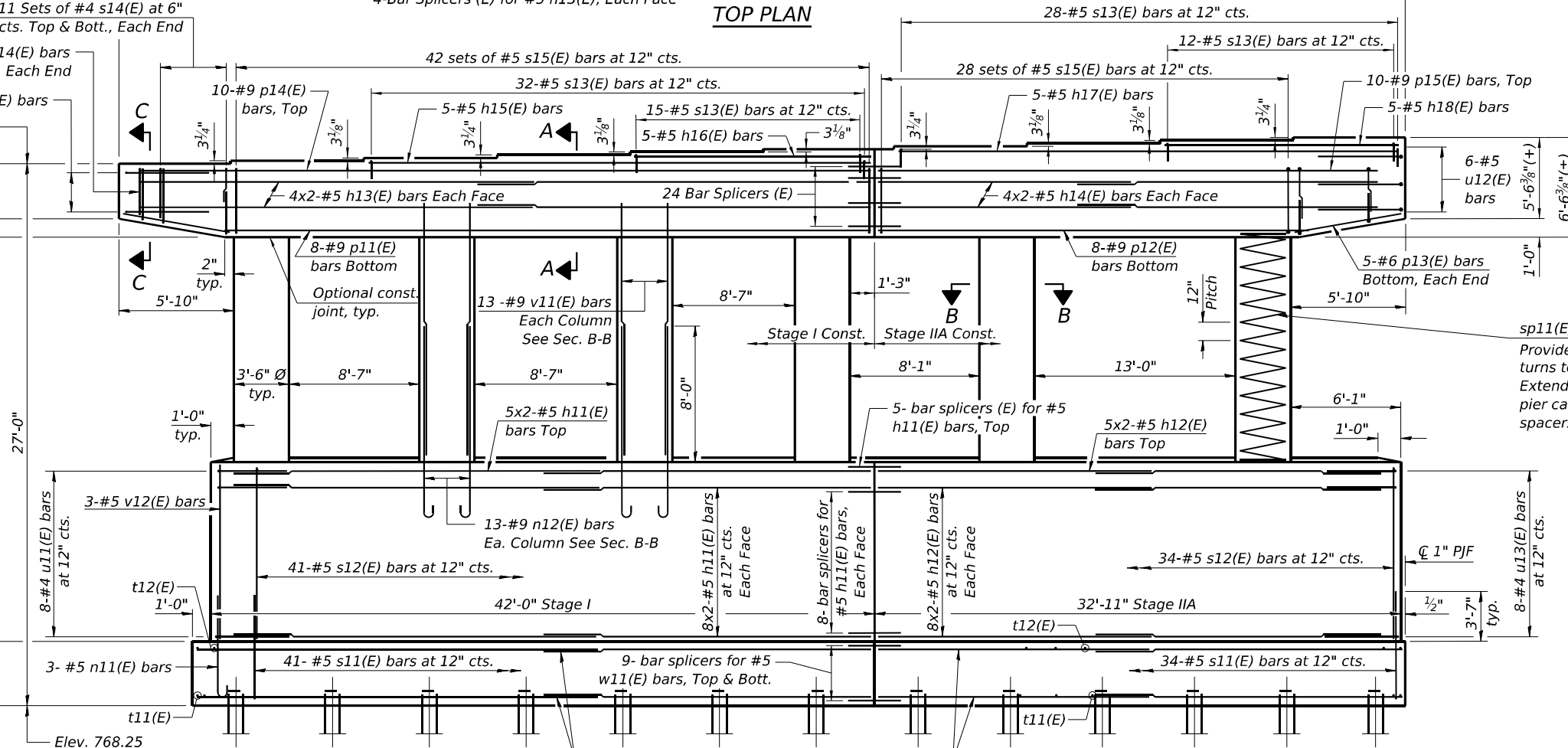
#### SEAT ELEVATIONS

Beam Line	Seat Elevation
1	795.25
2	795.52
3	795.78
4	796.05
5	796.31
6	796.57
7	796.84
8	797.10
9	797.36
10	797.63

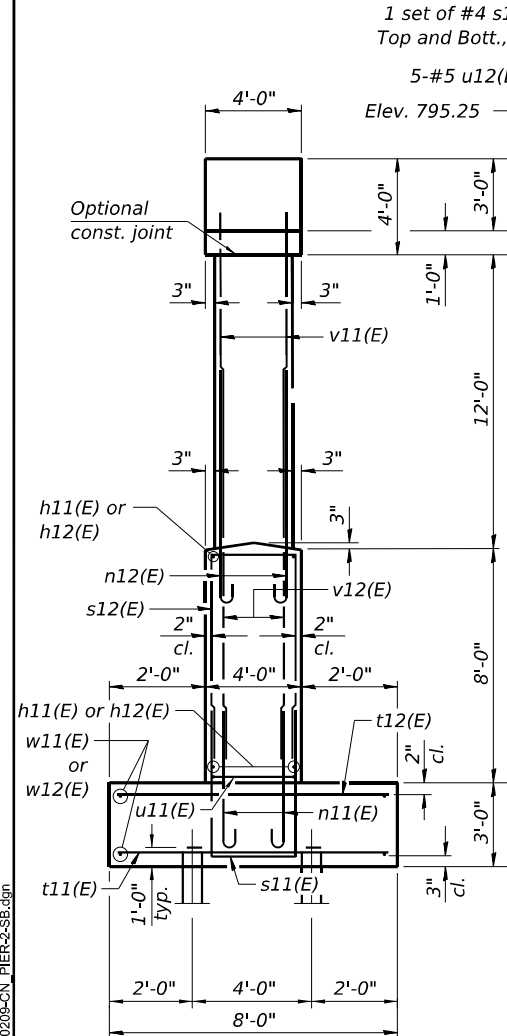
#### TOP PLAN



#### ELEVATION (Looking North)



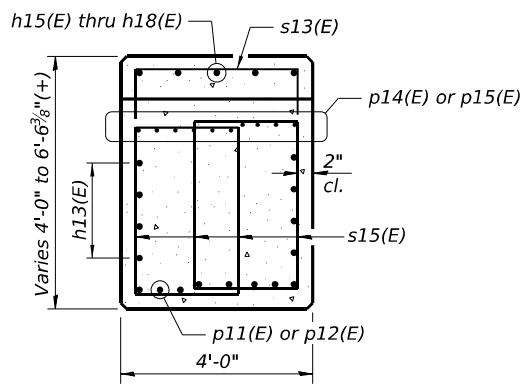
#### END VIEW



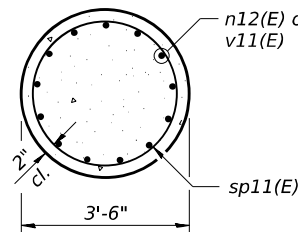
#### MIN. BAR LAP

#4 bar = 2'-11"  
#5 bar = 3'-7"  
#9 bar = 7'-1"

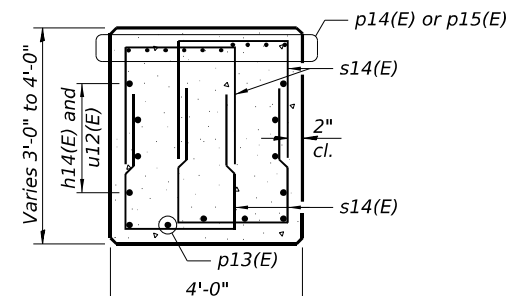
#### SEC. A-A



#### SEC. B-B



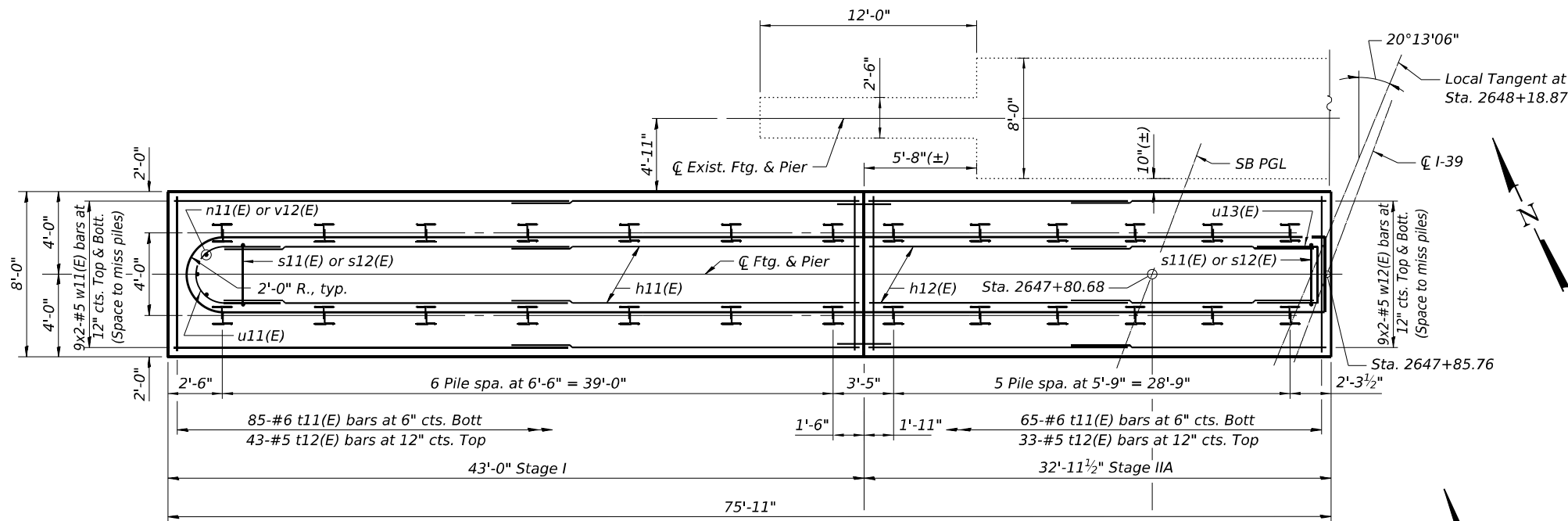
#### SEC. C-C



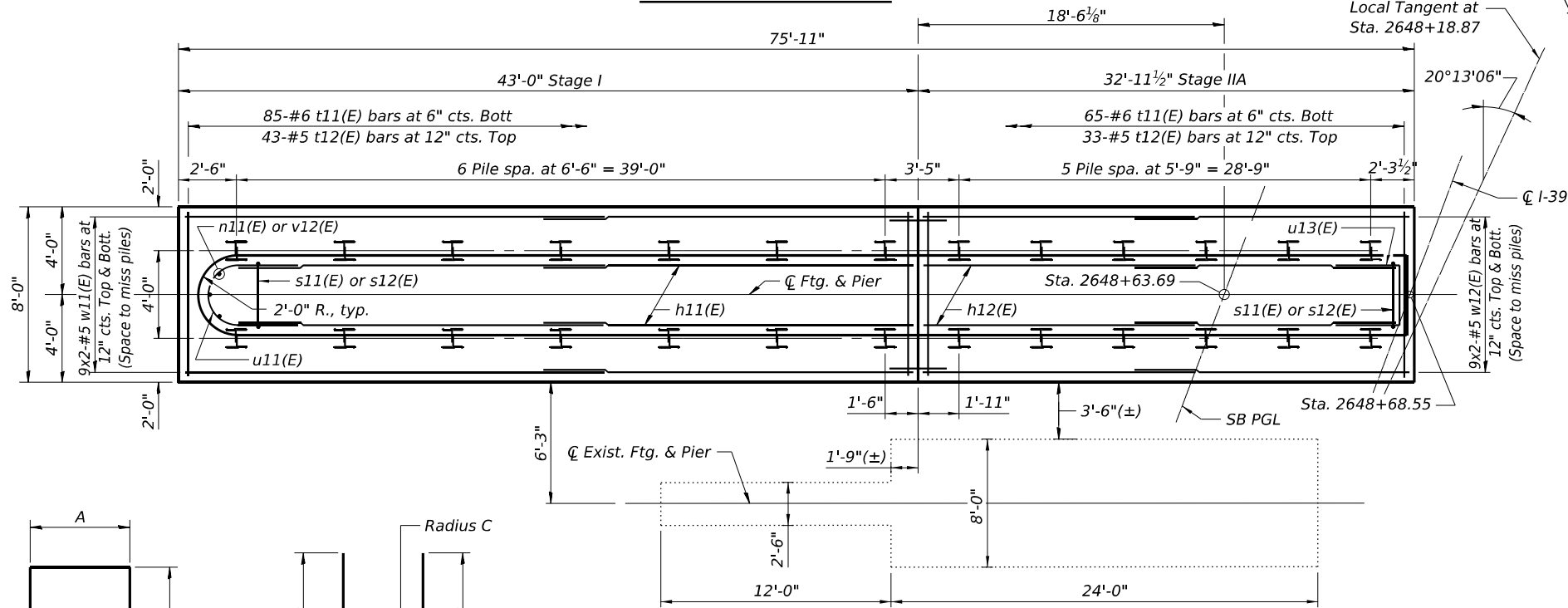




MODEL - SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\101-0208-0209-CN\_PIER-SB-DETAILS.dgn



FOOTING PLAN - PIER 1

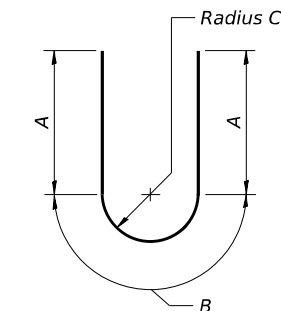


FOOTING PLAN - PIER 2

BARS s11(E), s12(E)  
THRU s14(E)

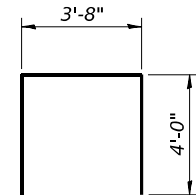
s11(E) THRU s14(E)  
DIMENSIONS

Bar	A	B
s11(E)	3'-8"	5'-9"
s12(E)	3'-8"	7'-8"
s13(E)	3'-6"	1'-0"
s14(E)	2'-3"	3'-1"

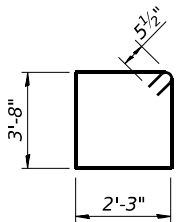


BAR u11(E) & u12(E)  
DIMENSIONS

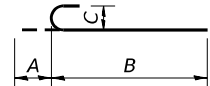
Bar	A	B	C
u11(E)	2'-4"	5'-9"	1'-10"
u12(E)	2'-11"	5'-6"	1'-9"



BAR u13(E)



BAR s15(E)



BARS n11(E) & n12(E)

BARS n11(E) & n12(E)  
DIMENSIONS

Bar	A	B	C
n11(E)	7"	5'-10"	5"
n12(E)	1'-3"	13'-6"	11 3/4"

BILL OF MATERIAL - PIER 1

Bar	No.	Size	Length	Shape
h11(E)	42	#5	22'-8"	—
h12(E)	42	#5	18'-1"	—
h13(E)	16	#5	24'-11"	—
h14(E)	16	#5	17'-10"	—
h15(E)	5	#5	31'-3"	—
h16(E)	5	#5	14'-10"	—
h17(E)	5	#5	22'-10"	—
h18(E)	5	#5	14'-8"	—
n11(E)	3	#5	6'-4"	U
n12(E)	78	#9	14'-9"	U
p11(E)	8	#9	41'-2"	—
p12(E)	8	#9	27'-1"	—
p13(E)	10	#6	5'-8"	—
p14(E)	10	#9	44'-8"	—
p15(E)	10	#9	30'-6"	—
s11(E)	75	#5	15'-2"	□
s12(E)	75	#5	19'-0"	□
s13(E)	87	#5	5'-6"	□
s14(E)	96	#4	8'-5"	□
s15(E)	140	#5	12'-9"	□
** sp11(E)	6	#4	12'-2"	W
t11(E)	150	#6	7'-8"	—
t12(E)	76	#5	7'-8"	—
u11(E)	8	#4	10'-5"	U
u12(E)	11	#5	11'-4"	U
u13(E)	8	#4	11'-8"	U
v11(E)	78	#9	13'-0"	—
v12(E)	3	#5	7'-8"	—
w11(E)	36	#5	23'-2"	—
w12(E)	36	#5	18'-1"	—
Structure Excavation		Cu. Yd.	150	
Concrete Structures		Cu. Yd.	229.1	
Reinforcement Bars, Epoxy Coated		Pound	24,640	
Furnishing Steel Piles HP12x63		Foot	494	
Driving Piles		Foot	494	
Pile Shoes		Each	26	

\*\* Length is height of spiral.

BILL OF MATERIAL - PIER 2

Bar	No.	Size	Length	Shape
h11(E)	42	#5	22'-8"	—
h12(E)	42	#5	18'-1"	—
h13(E)	16	#5	24'-11"	—
h14(E)	16	#5	17'-10"	—
h15(E)	5	#5	31'-3"	—
h16(E)	5	#5	14'-10"	—
h17(E)	5	#5	22'-10"	—
h18(E)	5	#5	14'-8"	—
n11(E)	3	#5	6'-4"	U
n12(E)	78	#9	14'-9"	U
p11(E)	8	#9	41'-2"	—
p12(E)	8	#9	27'-1"	—
p13(E)	10	#6	5'-8"	—
p14(E)	10	#9	44'-8"	—
p15(E)	10	#9	30'-6"	—
s11(E)	75	#5	15'-2"	□
s12(E)	75	#5	19'-0"	□
s13(E)	87	#5	5'-6"	□
s14(E)	96	#4	8'-5"	□
s15(E)	140	#5	12'-9"	□
** sp11(E)	6	#4	12'-2"	W
t11(E)	150	#6	7'-8"	—
t12(E)	76	#5	7'-8"	—
u11(E)	8	#4	10'-5"	U
u12(E)	11	#5	11'-4"	U
u13(E)	8	#4	11'-8"	U
v11(E)	78	#9	13'-0"	—
v12(E)	3	#5	7'-8"	—
w11(E)	36	#5	23'-2"	—
w12(E)	36	#5	18'-1"	—
Structure Excavation		Cu. Yd.	150	
Concrete Structures		Cu. Yd.	229.1	
Reinforcement Bars, Epoxy Coated		Pound	24,640	
Furnishing Steel Piles HP12x63		Foot	598	
Driving Piles		Foot	598	
Test Pile Steel HP12x63		Each	1	
Pile Shoes		Each	25	

\*\* Length is height of spiral.

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DATE - 02/27/24

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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

PIER DETAILS & BILL OF MATERIAL - SOUTHBOUND  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 53 OF 60 SHEETS STA. TO STA.

F.A.I. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO.

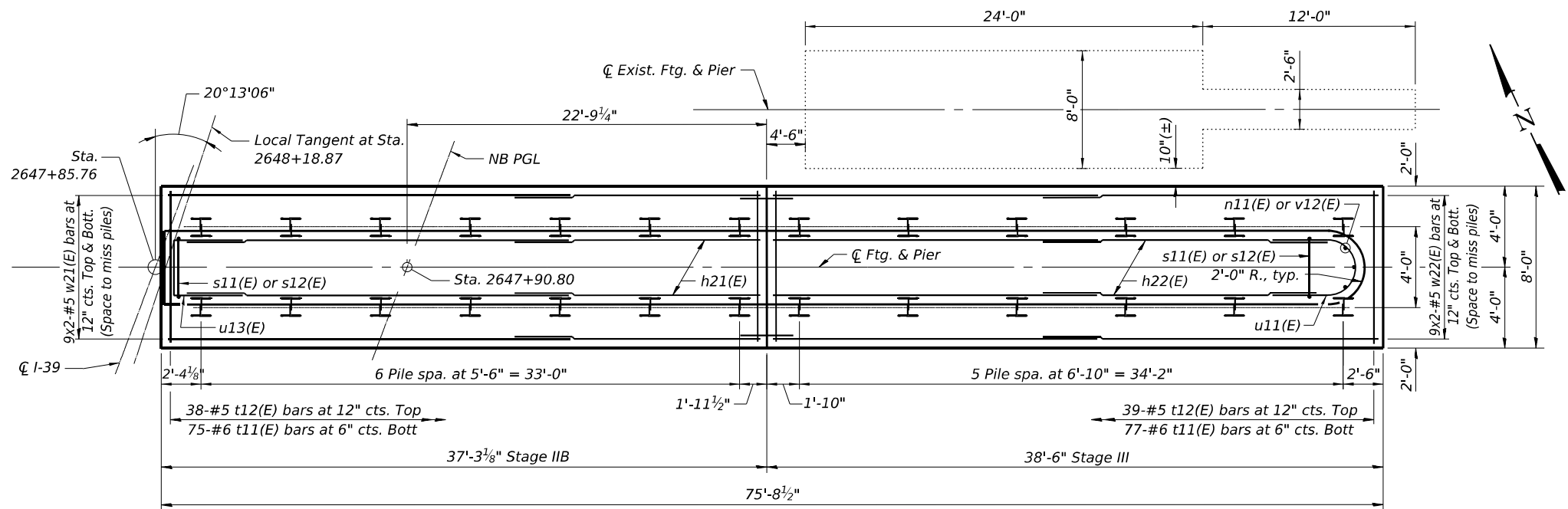
\* (201-3)R & (4-1.5)R WINNEBAGO 1685 596

CONTRACT NO. 64C24

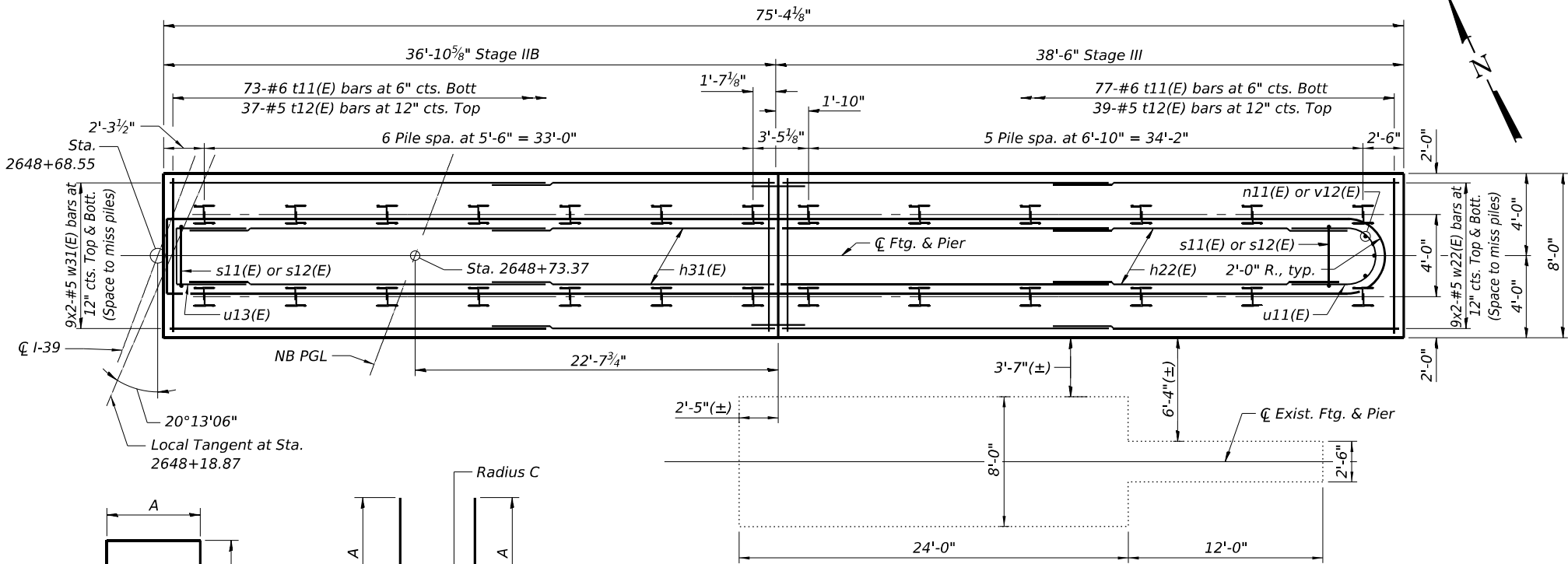
ILLINOIS FED. AID PROJECT

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

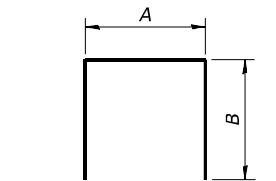
MODEL - SHEET  
FILE NAME: c:\paword\benesch\_projects\projects\101-0208-0209-CN\_PIER-S-NB-DETAILS.dgn



FOOTING PLAN - PIER 1



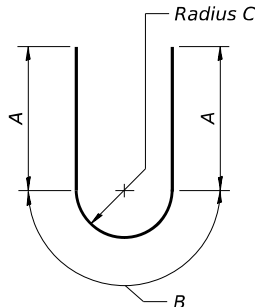
FOOTING PLAN - PIER 2



BARS s11(E), s12(E)  
THRU s14(E)

s11(E) THRU s14(E)  
DIMENSIONS

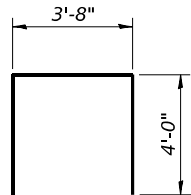
Bar	A	B
s11(E)	3'-8"	5'-9"
s12(E)	3'-8"	7'-8"
s13(E)	3'-6"	1'-0"
s14(E)	2'-3"	3'-1"



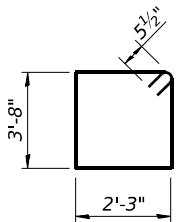
BAR u11(E) & u12(E)

u11(E) & u12(E)  
DIMENSIONS

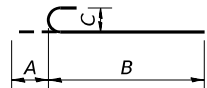
Bar	A	B	C
u11(E)	2'-4"	5'-9"	1'-10"
u12(E)	2'-11"	5'-6"	1'-9"



BAR u13(E)



BAR s15(E)



BARS n11(E) & n12(E)

BARS n11(E) & n12(E)  
DIMENSIONS

Bar	A	B	C
n11(E)	7"	5'-10"	5"
n12(E)	1'-3"	13'-6"	11 3/4"

BILL OF MATERIAL - PIER 1

Bar	No.	Size	Length	Shape
h21(E)	42	#5	21'-3"	—
h22(E)	42	#5	20'-5"	—
h23(E)	16	#5	19'-8"	—
h24(E)	16	#5	22'-4"	—
h25(E)	5	#5	22'-3"	—
h26(E)	5	#5	5'-5"	—
h27(E)	5	#5	37'-8"	—
h28(E)	5	#5	12'-11"	—
n11(E)	3	#5	6'-4"	U
n12(E)	78	#9	14'-9"	U
p13(E)	10	#6	5'-8"	—
p21(E)	8	#9	31'-6"	—
p22(E)	8	#9	36'-10"	—
p24(E)	10	#9	34'-10"	—
p25(E)	10	#9	40'-2"	—
s11(E)	74	#5	15'-2"	□
s12(E)	74	#5	19'-0"	□
s13(E)	76	#5	5'-6"	□
s14(E)	96	#4	8'-5"	□
s15(E)	142	#5	12'-9"	□
sp11(E)	6	#4	12'-2"	W
t11(E)	152	#6	7'-8"	—
t12(E)	77	#5	7'-8"	—
u11(E)	8	#4	10'-5"	U
u12(E)	11	#5	11'-4"	U
u13(E)	8	#4	11'-8"	U
v11(E)	78	#9	13'-0"	—
v12(E)	3	#5	7'-8"	—
w21(E)	36	#5	20'-3"	—
w22(E)	36	#5	20'-11"	—
Structure Excavation		Cu. Yd.	152	
Concrete Structures		Cu. Yd.	228.8	
Reinforcement Bars, Epoxy Coated		Pound	24,520	
Furnishing Steel Piles HP12x63		Foot	494	
Driving Piles		Foot	494	
Pile Shoes		Foot	26	

\*\* Length is height of spiral.

BILL OF MATERIAL - PIER 2

Bar	No.	Size	Length	Shape
h22(E)	42	#5	20'-5"	—
h24(E)	16	#5	22'-4"	—
h31(E)	42	#5	21'-1"	—
h32(E)	16	#5	19'-6"	—
h33(E)	5	#5	21'-7"	—
h34(E)	5	#5	5'-1"	—
h35(E)	5	#5	37'-4"	—
h36(E)	5	#5	12'-7"	—
n11(E)	3	#5	6'-4"	U
n12(E)	78	#9	14'-9"	U
p13(E)	10	#6	5'-8"	—
p22(E)	8	#9	36'-10"	—
p25(E)	10	#9	40'-2"	—
p31(E)	8	#9	31'-2"	—
p32(E)	10	#9	34'-6"	—
s11(E)	73	#5	15'-2"	□
s12(E)	73	#5	19'-0"	□
s13(E)	75	#5	5'-6"	□
s14(E)	96	#4	8'-5"	□
s15(E)	140	#5	12'-9"	□
sp11(E)	6	#4	12'-2"	W
t11(E)	150	#6	7'-8"	—
t12(E)	76	#5	7'-8"	—
u11(E)	8	#4	10'-5"	U
u12(E)	11	#5	11'-4"	U
u13(E)	8	#4	11'-8"	U
v11(E)	78	#9	13'-0"	—
v12(E)	3	#5	7'-8"	—
w31(E)	36	#5	20'-1"	—
w22(E)	36	#5	20'-11"	—
Structure Excavation		Cu. Yd.	151	
Concrete Structures		Cu. Yd.	227.8	
Reinforcement Bars, Epoxy Coated		Pound	24,370	
Furnishing Steel Piles HP12x63		Foot	624	
Driving Piles		Foot	624	
Pile Shoes		Foot	26	

\*\* Length is height of spiral.

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USER NAME = lfranceschina  
PLOT SCALE = SSCALE\$  
PLOT DATE = 8/12/2024

DESIGNED - JW  
DRAWN - AT  
CHECKED - AJN  
DATE - 02/27/24

REVISED -  
REVISED -  
REVISED -  
REVISED -

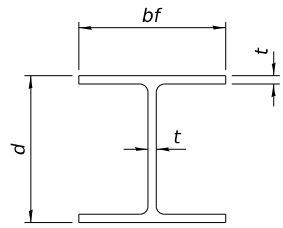
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

PIER DETAILS & BILL OF MATERIAL - NORTHBOUND  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 54 OF 60 SHEETS STA. TO STA.

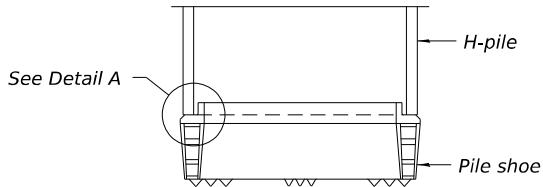
F.A.I. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO.  
\* (201-3)R & (4-1.5)R WINNEBAGO 1685 597  
CONTRACT NO. 64C24  
ILLINOIS FED. AID PROJECT

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*

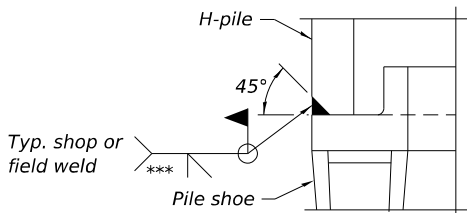


STEEL PILE TABLE

Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	1 3/16"	30"
x102	14"	14 3/4"	1 1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1 1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



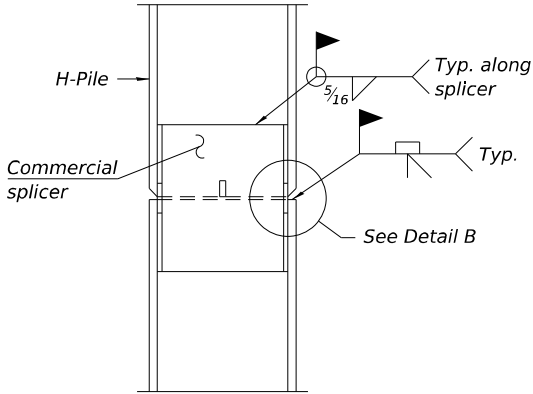
ELEVATION



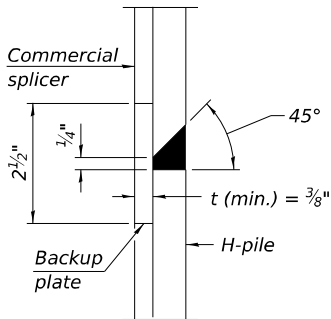
DETAIL A

SHOE ATTACHMENT

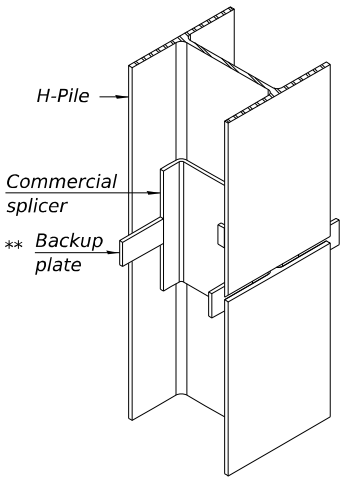
Note:  
The steel H-piles shall be according to  
AASHTO M270 Grade 50.



ELEVATION

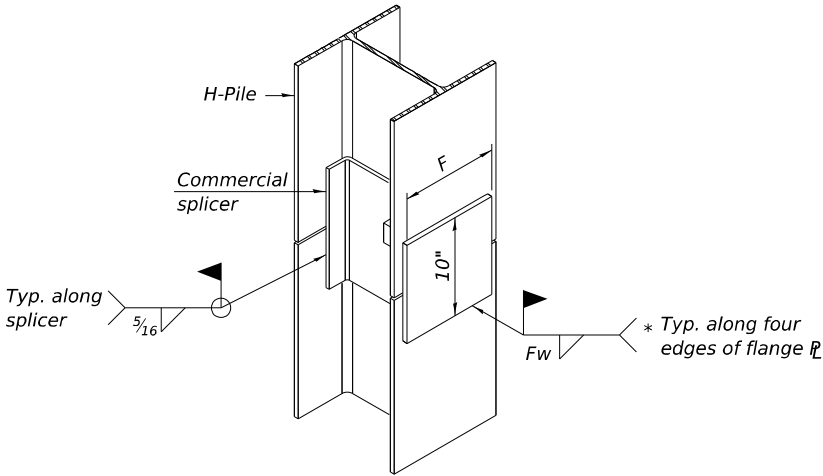


DETAIL "B"



ISOMETRIC VIEW

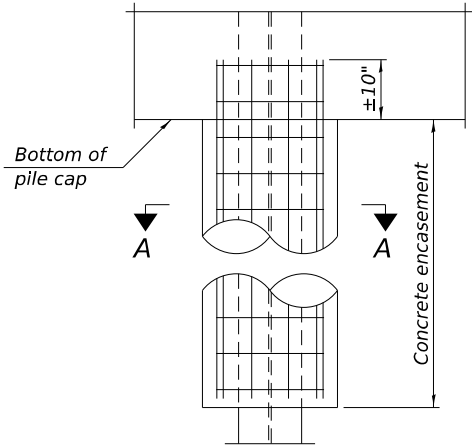
WELDED COMMERCIAL SPLICE



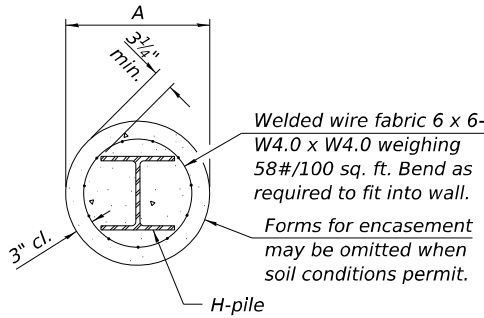
ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- \* Interrupt welds 1/4" from end of web and/or each flange.
- \*\* Remove portions of backup plates that extend outside the flanges.
- \*\*\* Weld size per pile shoe manufacturer (5/16" min.).

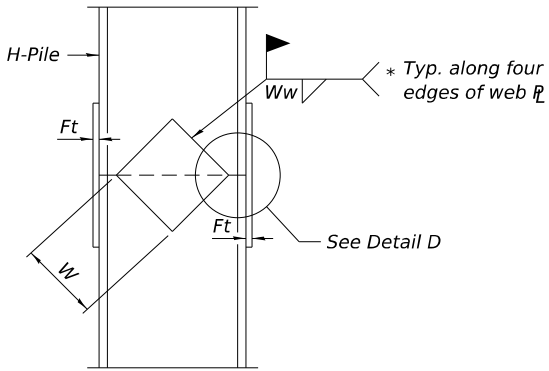


ELEVATION

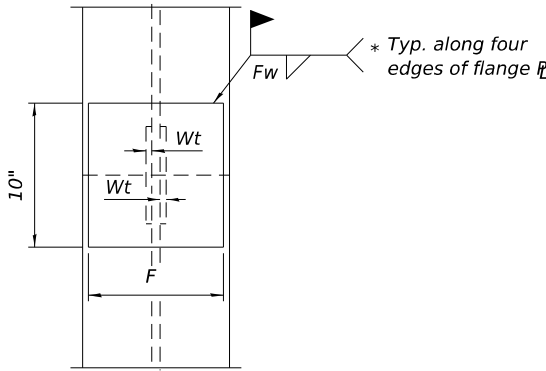


SECTION A-A

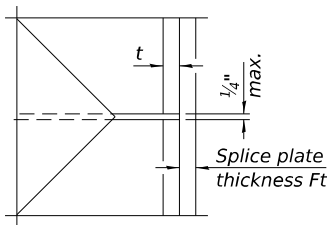
INDIVIDUAL PILE  
CONCRETE ENCASEMENT  
(when specified)



ELEVATION



END VIEW



DETAIL D

WELDED PLATE FIELD SPLICE

Designation	F	Ft	Fw	W	Wt	Ww
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5/8"	1/2"
x89	12 1/2"	3/4"	11/16"	7 3/4"	5/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5/8"	1/2"
HP 12x84	10"	7/8"	11/16"	6 1/2"	5/8"	1/2"
x74	10"	7/8"	11/16"	6 1/2"	5/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

MODEL - SHEET  
FILE NAME: c:\pwworkdir\benesch\_projects\projects\10173871\10284C24\_SHT\_0208-0209-CN\_PILE-DETAILS.dgn

F-HP

5-15-2023

Bowman

10 S. LaSalle Street, Suite 210  
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312-454-0360  
www.bowman.com

USER NAME	= Ifranceschina
PLOT SCALE	= \$SCALES
PLOT DATE	= 8/12/2024

DESIGNED	- JW
DRAWN	- DSO
CHECKED	- AJN
DATE	- 02/27/24

REVISED	-
REVISED	-
REVISED	-
REVISED	-

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

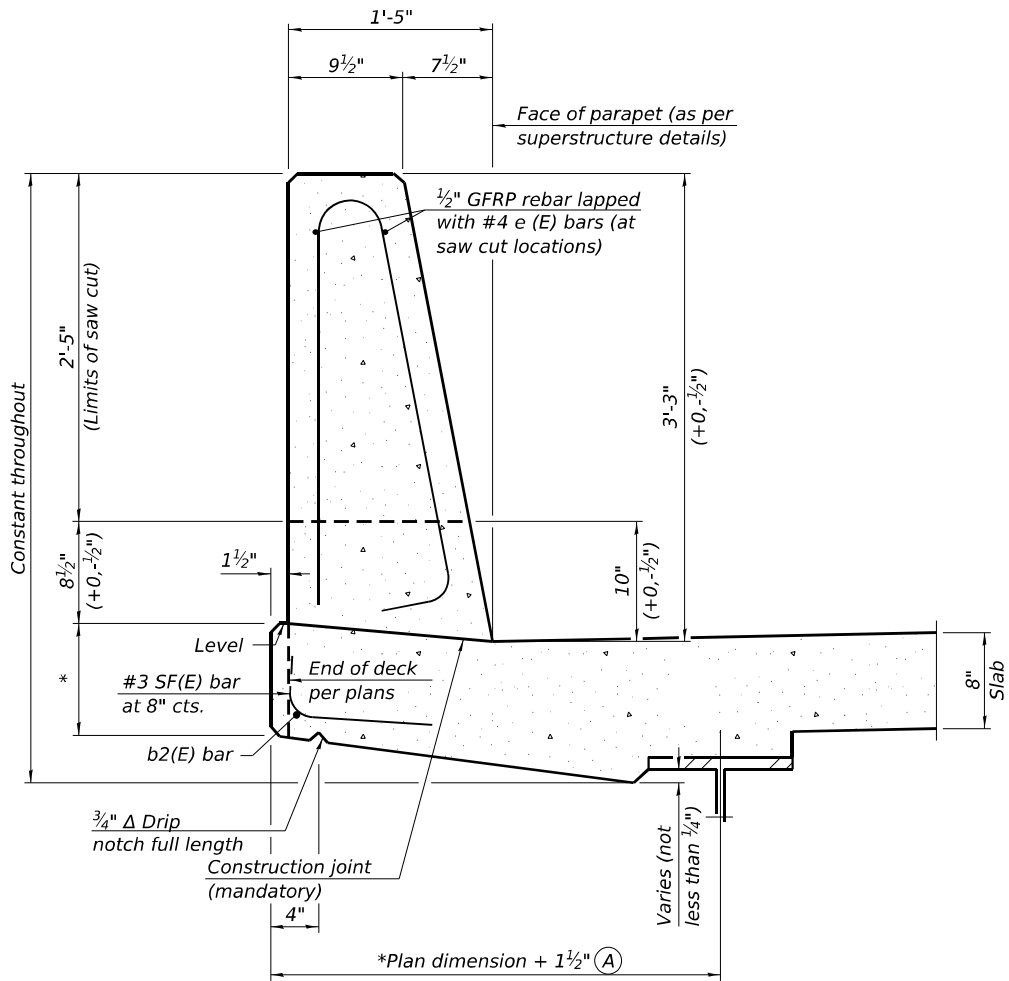
PILE DETAILS  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 55 OF 60 SHEETS STA. TO STA.

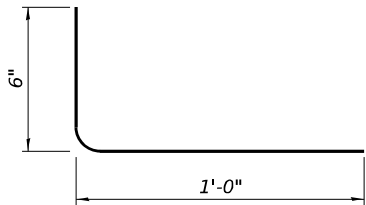
F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	598
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20)

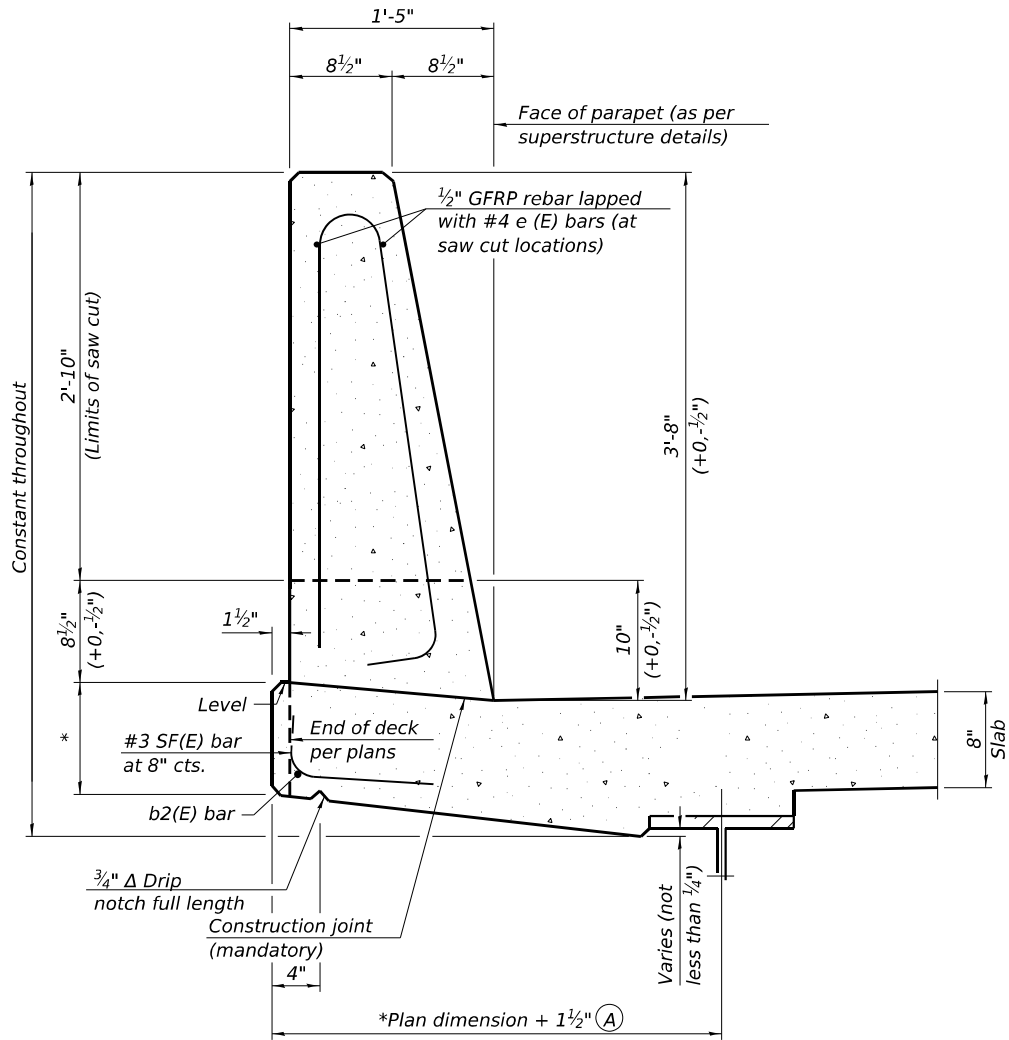
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FILE NAME: c:\paword\benesch projects\projects\0173871\0284C24\_SHT\_0208-0209-CN\_PARAPET-SLIPFORMING.dgn



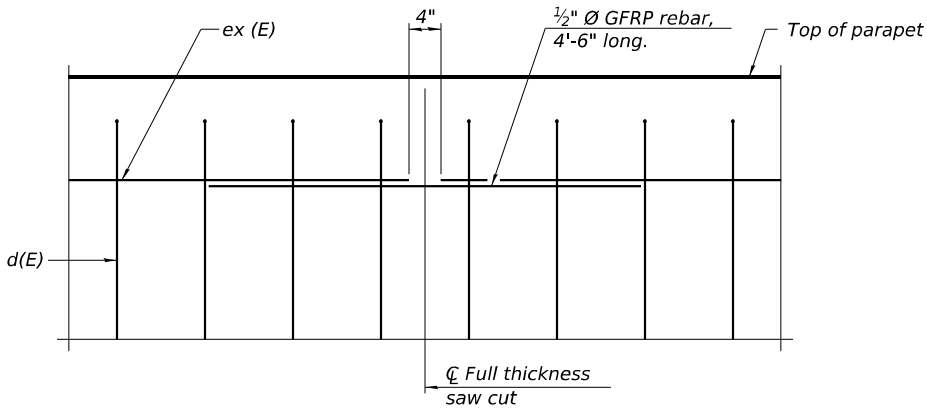
**39" CONSTANT-SLOPE  
PARAPET SECTION**  
(Showing dimensions, d(E), and 1/2" Ø GFRP rebar)



**SF(E) BAR**



**44" CONSTANT-SLOPE  
PARAPET SECTION**  
(Showing dimensions, d(E), and 1/2" Ø GFRP rebar)



**GFRP REBAR STIFFENING DETAIL**  
(Place as shown in parapet section  
at each parapet joint location.)

Notes:  
All dimensions shall remain the same as shown on  
superstructure details, except dimension A which is  
to be revised as shown. Additional concrete  
needed to revise dimension A = 0.00348 cu. yds./ft.  
for 39" and 44" parapets.  
Place full depth aluminum sheets as shown on  
superstructure details.  
Replace all cork joint filler locations with a full  
thickness saw cut.  
Steel superstructure shown. Other superstructure  
types similar.

SFP 39-44

5-15-2023

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PLOT SCALE = \$SCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION**

**CONCRETE PARAPET SLIPFORMING  
STRUCTURE NO. 101-0208/0209**

SCALE: SHEET 56 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	599
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) •

MODEL: SHEET  
FILE NAME: c:\paword\benesch projects\projects\10173871\10264C24\_SHT\_0208-0209-CN\_BAR-SPLICERS.dgn

BSD-1

5-15-2023

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USER NAME = Ifranceschina	DESIGNED - JW	REVISED -
	DRAWN - DSO	REVISED -
PLOT SCALE = \$SCALE\$	CHECKED - AJN	REVISED -
PLOT DATE = 8/12/2024	DATE - 02/27/24	REVISED -

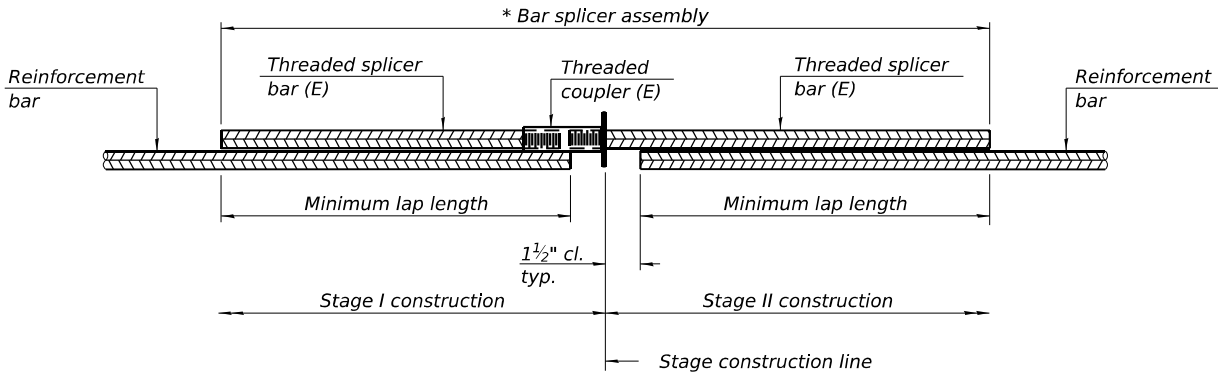
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

BAR SPLICER DETAILS  
STRUCTURE NO. 101-0208/0209

SCALE: SHEET 57 OF 60 SHEETS STA. TO STA.

F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(201-3)R & (4-1.5)R	WINNEBAGO	1685	600
CONTRACT NO. 64C24				
ILLINOIS FED. AID PROJECT				

\* FAI ROUTE 39 (I-39) & FAP 301 (US 20) \*



#### STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length + 1 1/2" + thread length

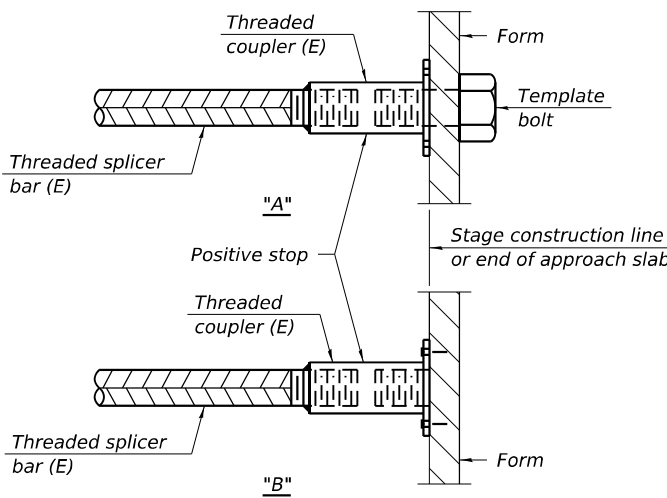
\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

#### NORTHBOUND STRUCTURE

Location	Bar size	No. assemblies required	Minimum lap length
Approach Slabs	#5	92	3'-2"
	#8	124	5'-1"
Deck Slab	#5	665	3'-2"
Abutments	#7	8	4'-5"
Piers	#5	94	3'-2"
	#9	36	5'-8"

#### SOUTHBOUND STRUCTURE

Location	Bar size	No. assemblies required	Minimum lap length
Approach Slabs	#5	92	3'-2"
	#8	124	5'-1"
Deck Slab	#5	665	3'-2"
Abutments	#7	8	4'-5"
Piers	#5	94	3'-2"
	#9	36	5'-8"

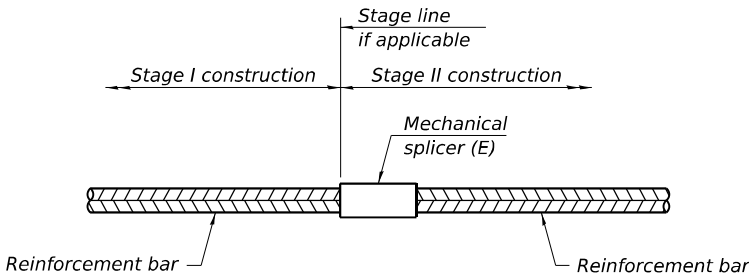


#### INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.

"B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.



#### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:  
Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.  
All reinforcement shall be lapped and tied to the splicer bars.  
Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.  
See approved list of bar splicer assemblies and mechanical splicers for alternatives.