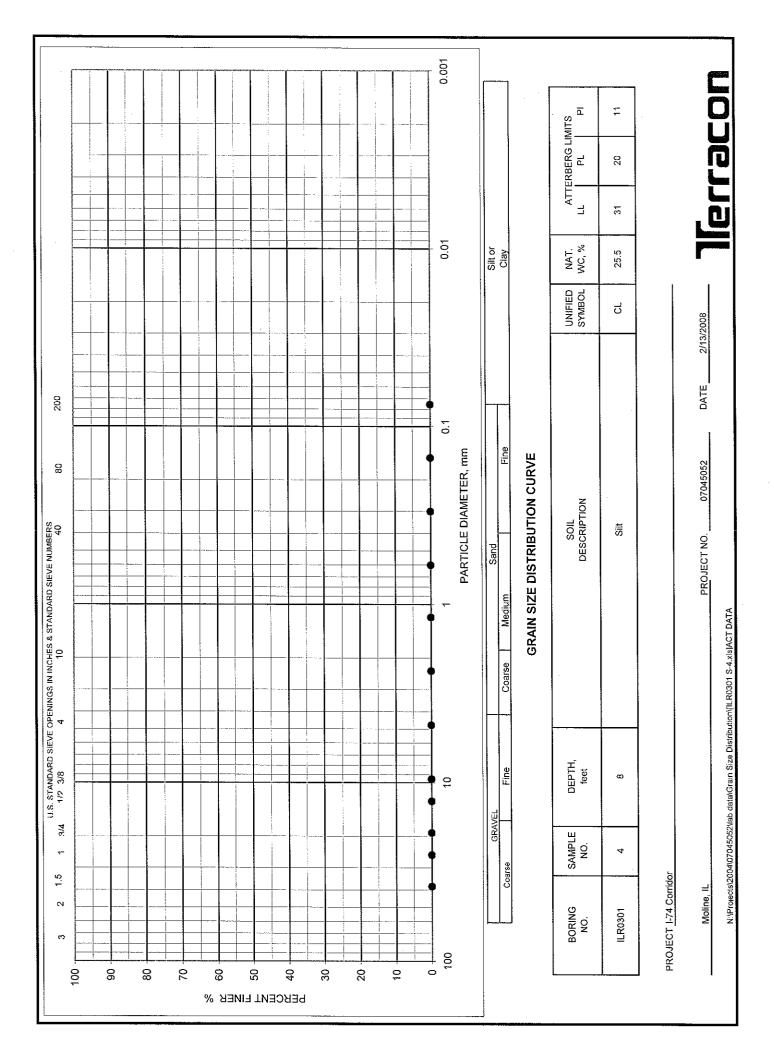
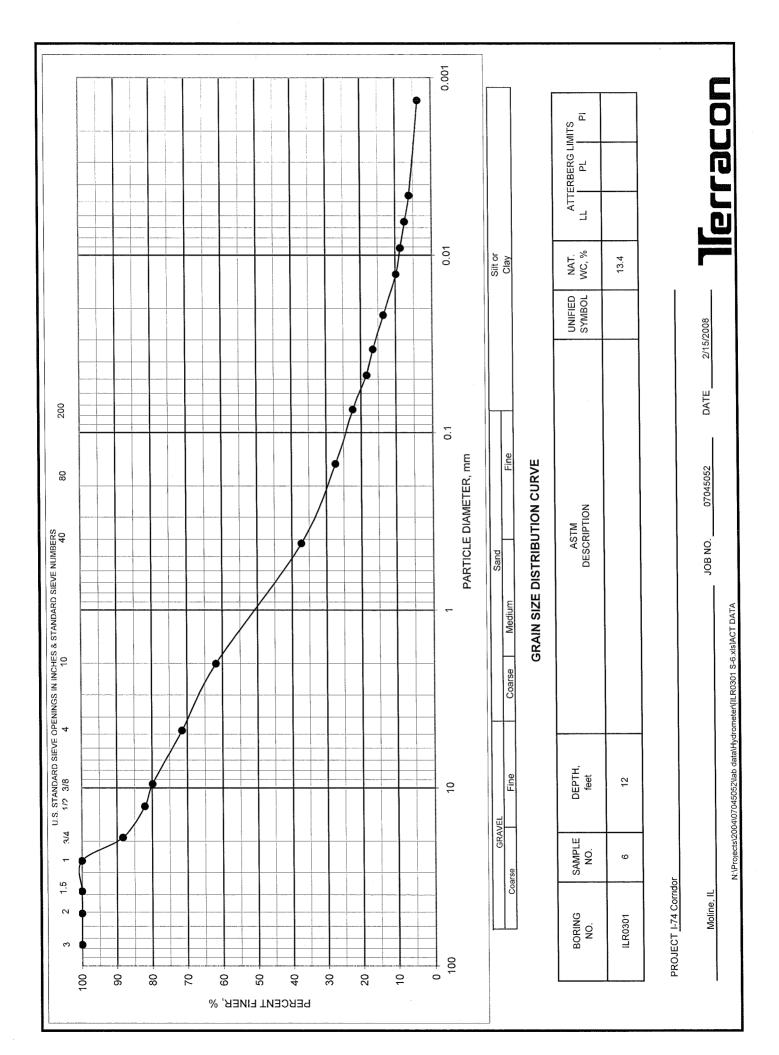
GEOTECHNICAL INVESTIGATION LABORATORY DATA

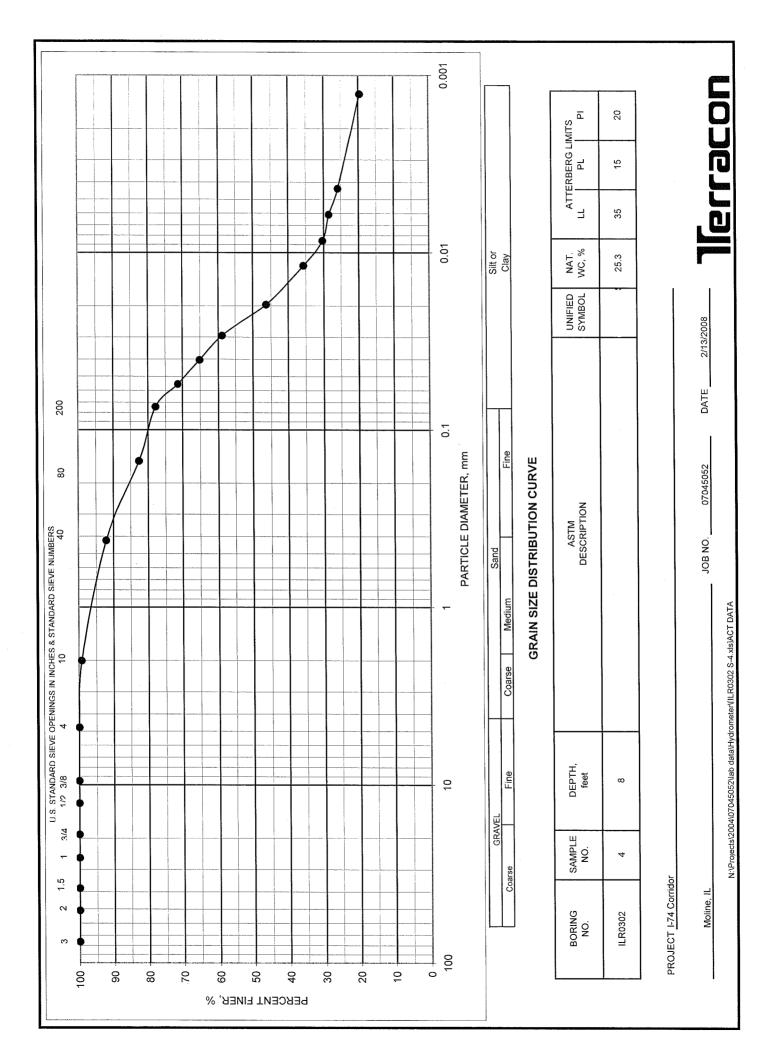
Description. The following data was collected during preliminary engineering and is attached herein for the Contractor's information.

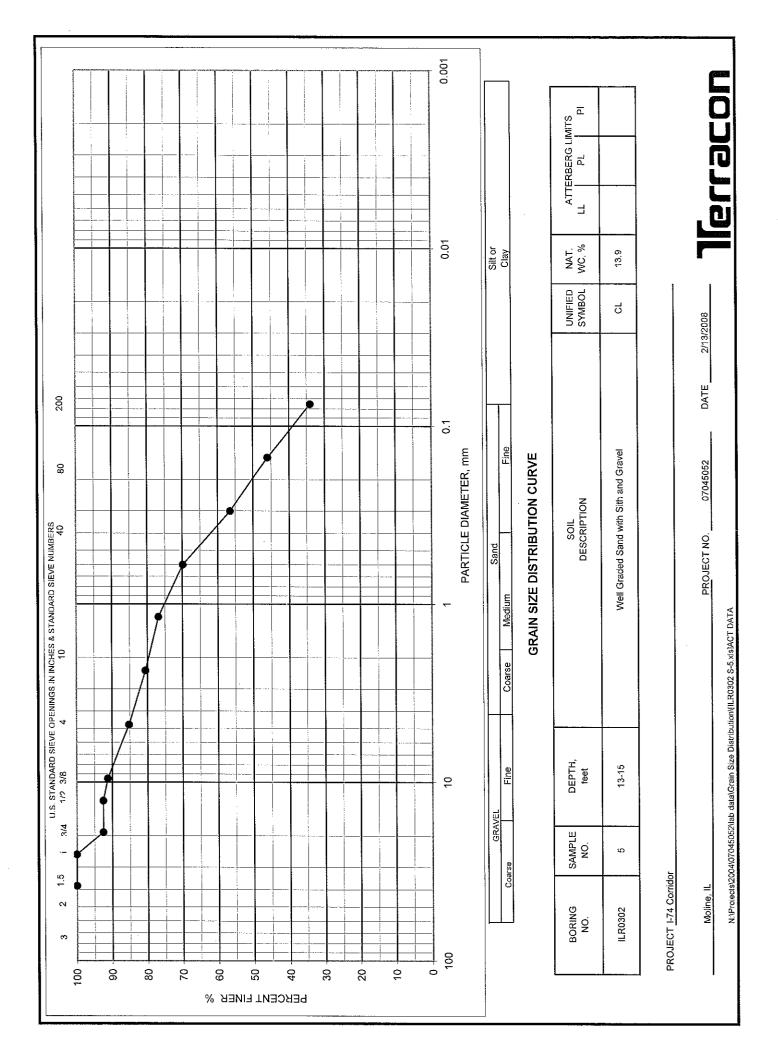
Data included is in reference to the following structures which require Aggregate Column Ground Improvements:

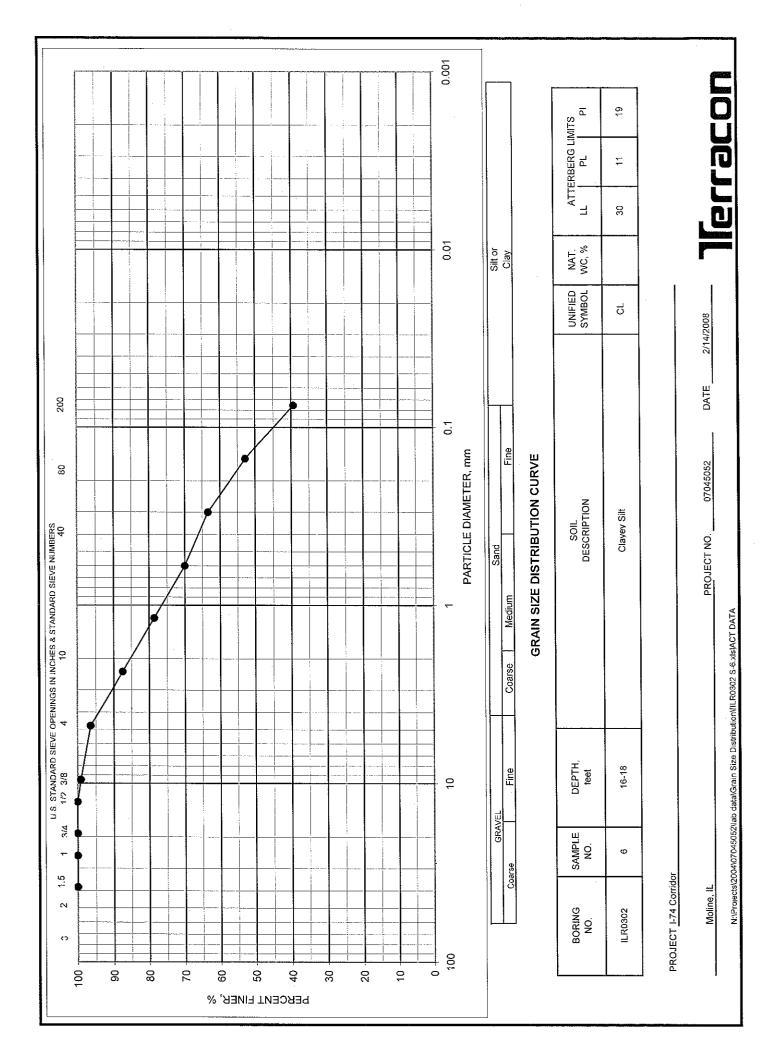
SN 081-6012 – Retaining Wall 03 SN 081-6019 – Retaining Wall 18

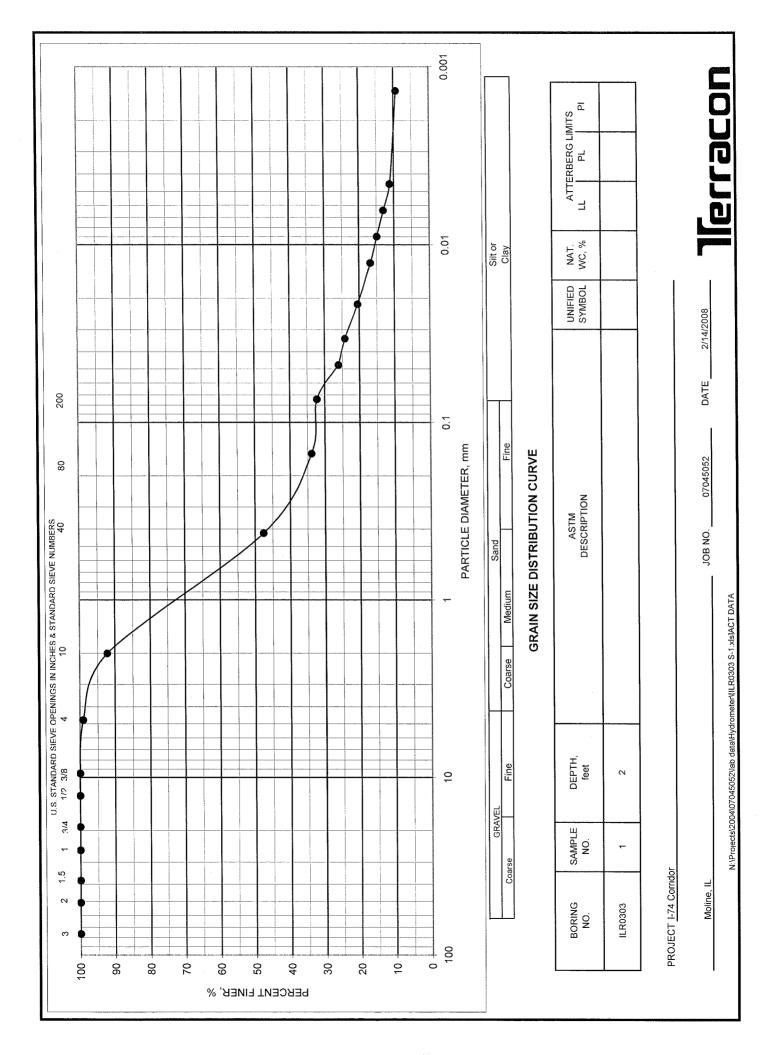


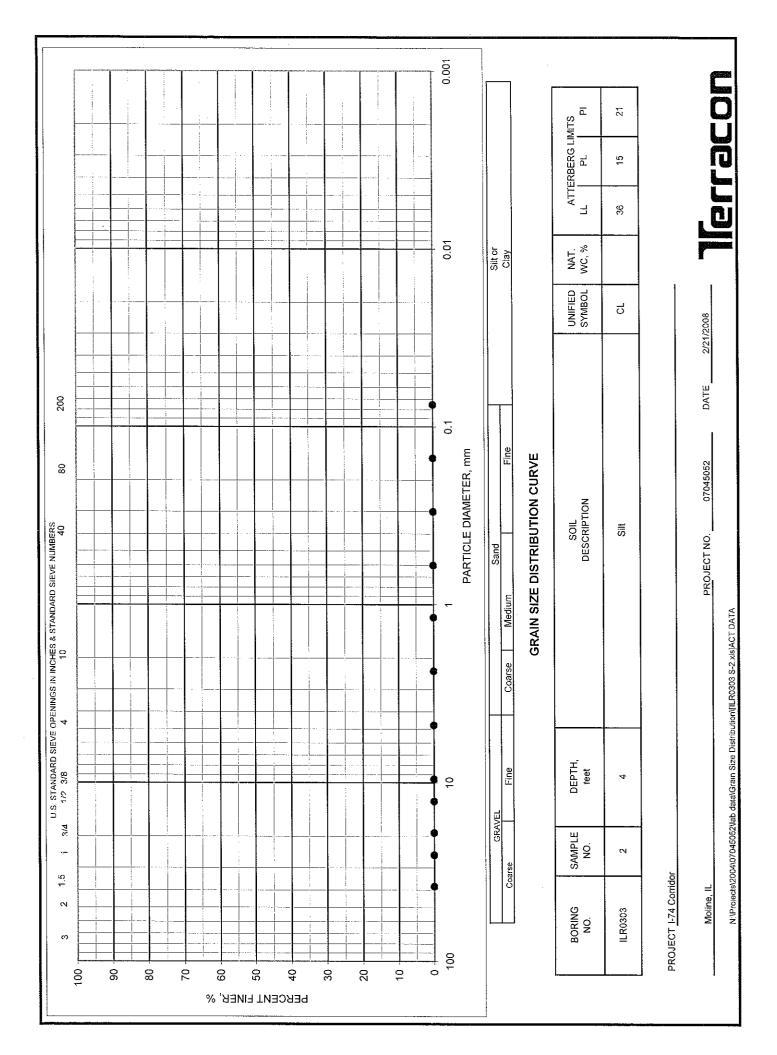


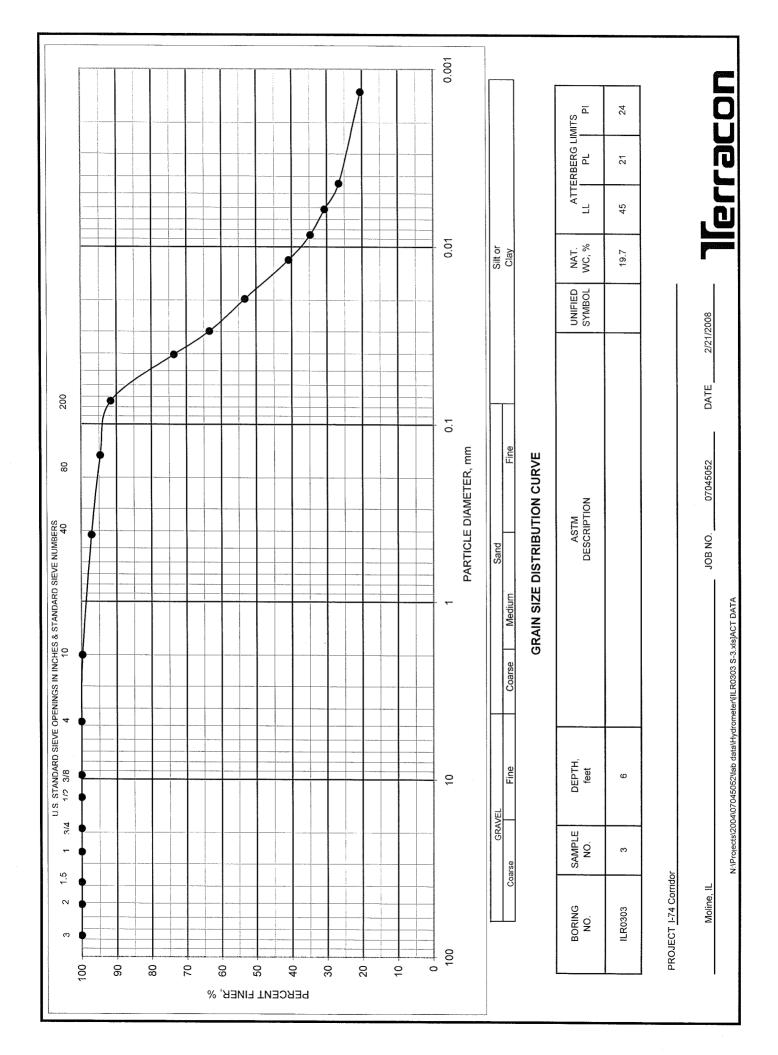


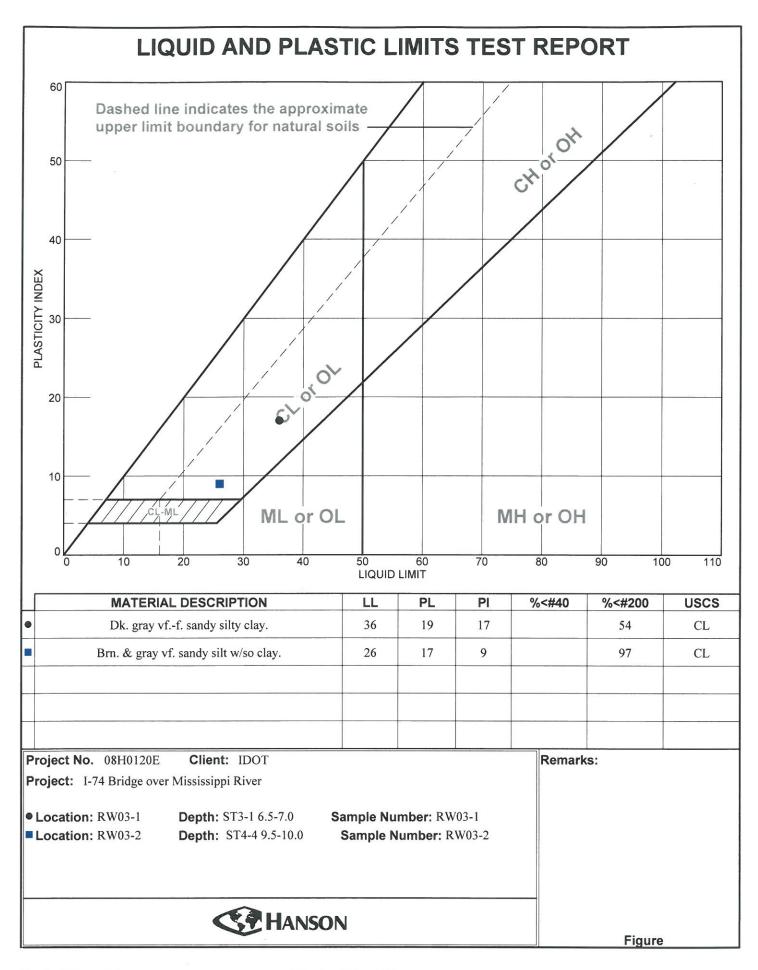












UNCONFINED COMPRESSIVE STRENGTH OF INTACT ROCK CORE

CLIENT: CH2M HILL

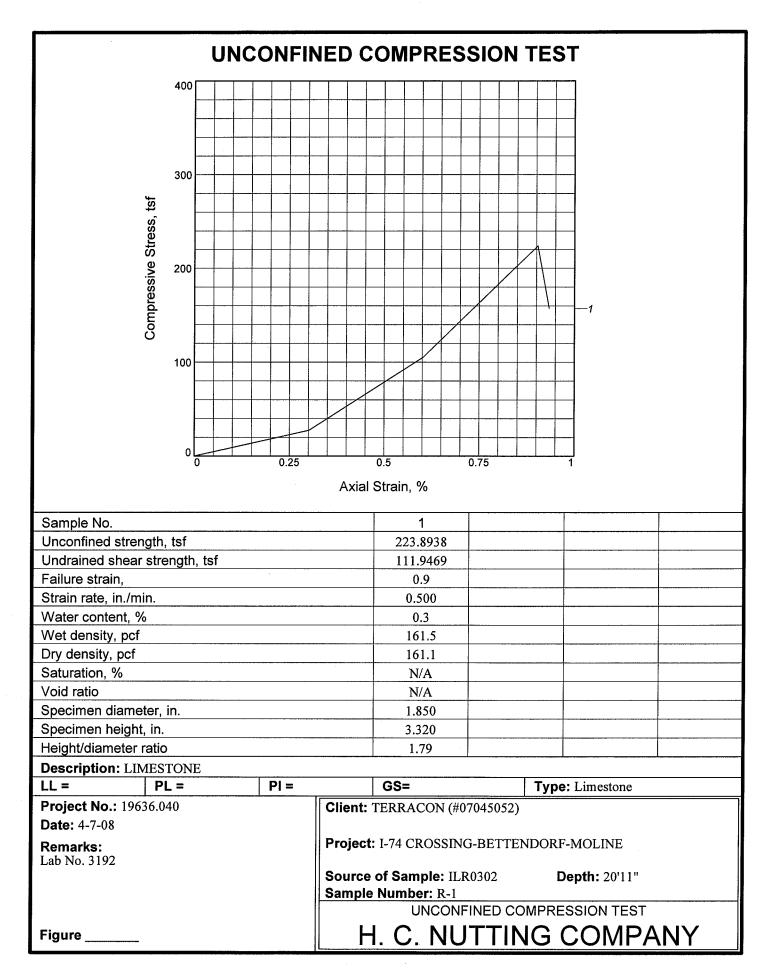
JOB NO.: 07045052

PROJECT: INTERSTATE I-74 IMPROVEMENTS DATE: 2/22/06 BETTENDORF, IOWA

TEST NO.	5	6	7	8			
BORING NO,	NG NO, PRMPD2		RW1401	RW1401			
RUN NO.	1	2	5	5			
DEPTH (FT.)	171/2 –18	25 – 25 1/2	38 1/2 – 39	39 – 39 ½			
PREPARED CORE (IN.)	4.50	4.52	3.06	3.07			
ROCK DESCRIPITION (Note 1)	LIMESTONE	LIMESTONE	SHALE SANDSTONE	SHALE SANDSTONE			
MOISTURE CONTENT %	0.2	0.1	2.2	7.0			
COMPRESSIVE STRENGTH TESTS							
DIAMETER (IN.)	1.87	1.86	1.87	1.88			
AREA (SQ.IN.)	2.74	2.72	2.74	2.77			
L/D RATIO	2.4	2.4	1.6	1.6			
TOTAL LOAD (LBS.)	18,420	25,830	7,540	11,300			
COMPRESSIVE STRENGTH (PSI) (Note 2)	6,720	9,500	2,750	4,080			
TYPE FRACTURE	VERTICAL FRACTURE	VERTICAL FRACTURE	VERTICAL FRACTURE	VERTICAL FRACTURE			
DATE TESTED	2/21/06	2/21/06	2/21/06	2/21/06			
DENSITY (PCF)	157	162	130	122			

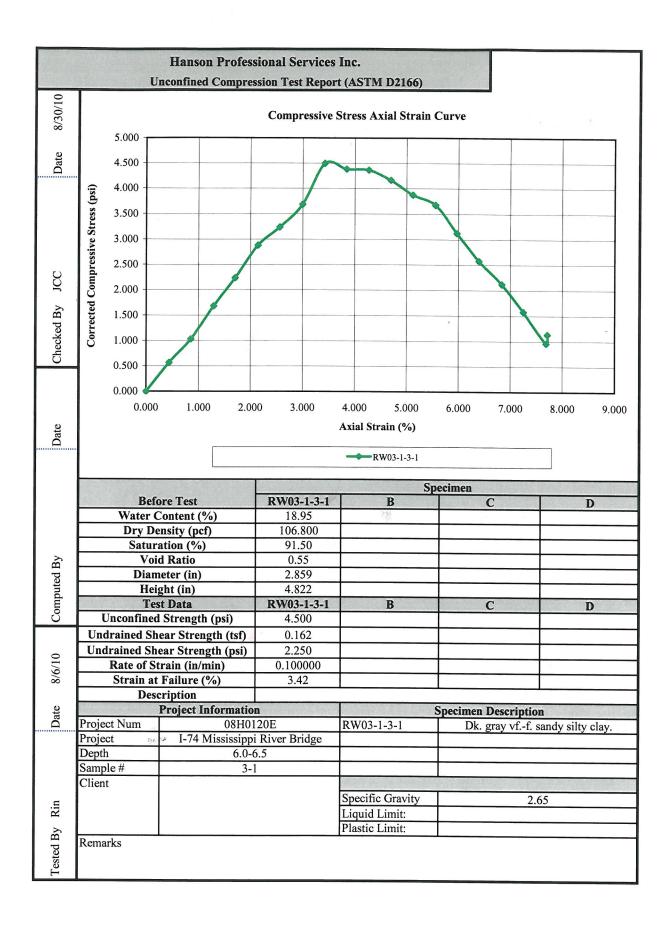
Note 1: Rock type based on visual and tactile observation of core.

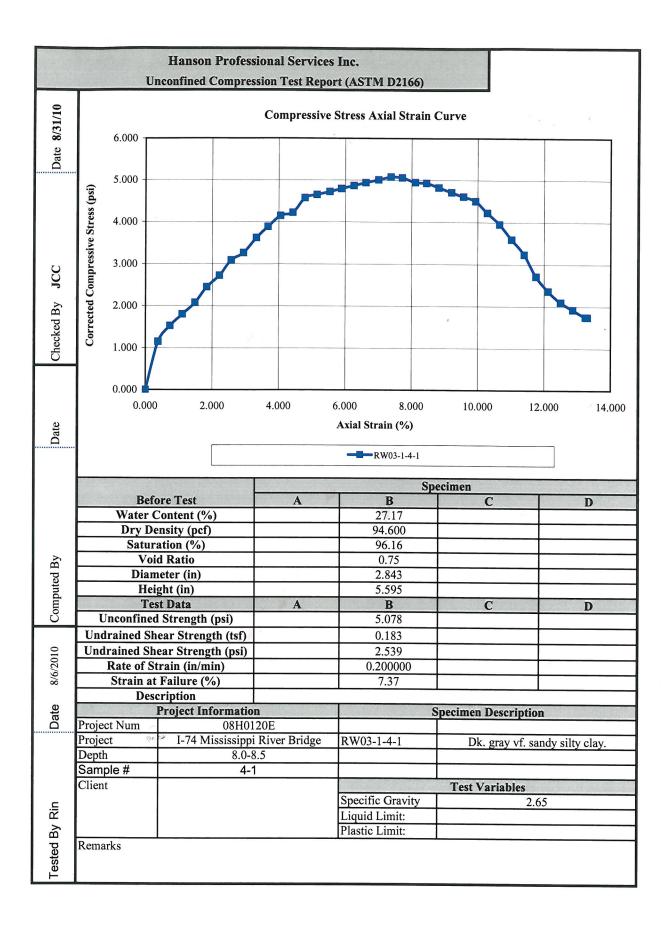
Note 2: Tests No. 7 and 8 are below the L/D ratio of 2.0 to 2.5 stated in the ASTM 4543 Standard, compressive strength values may not be representative.

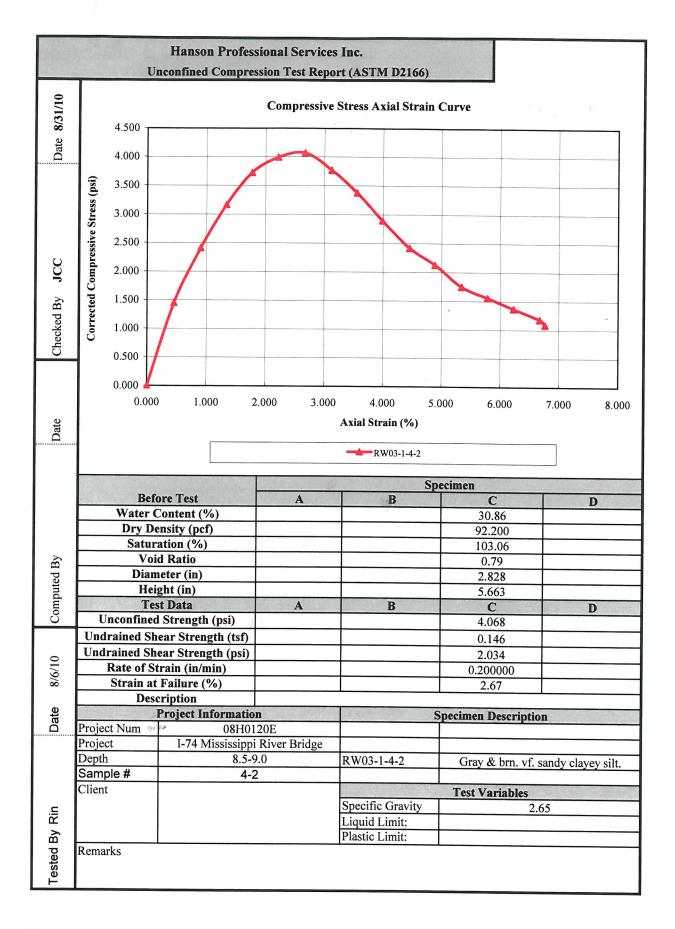


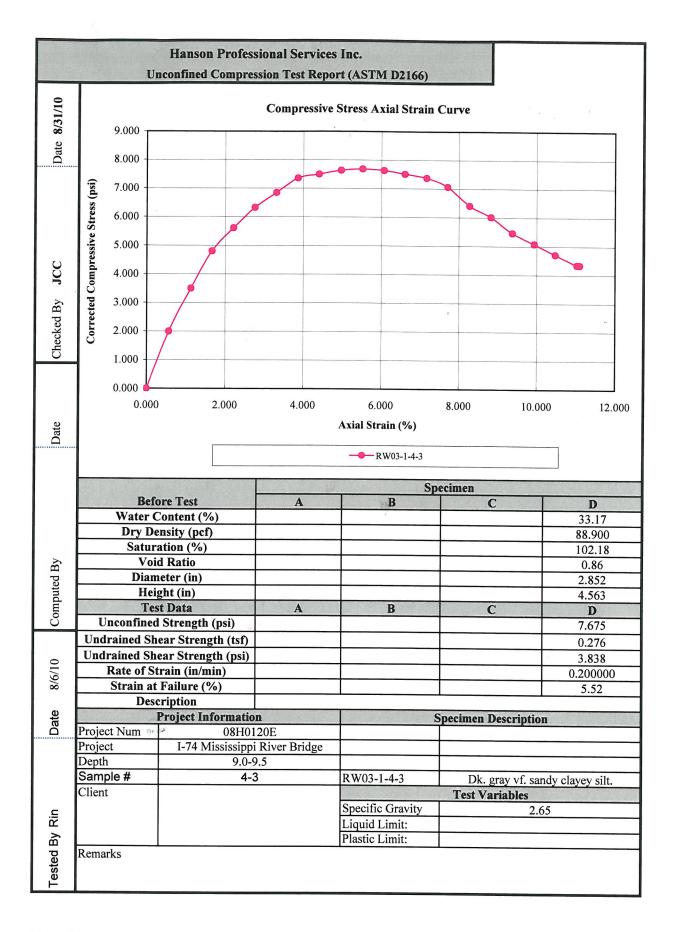
Tested By: DR

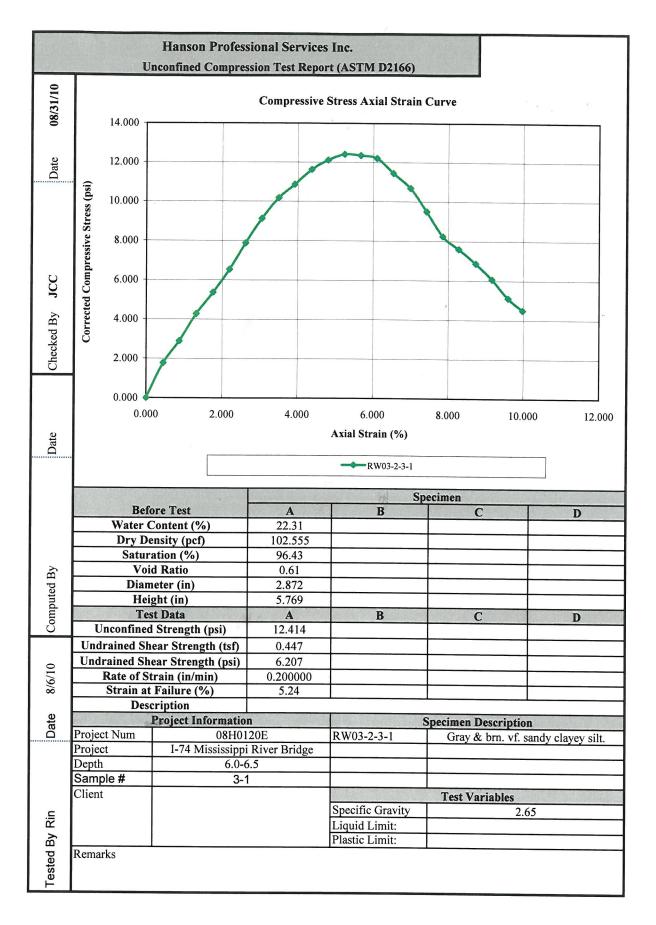
Checked By: GS

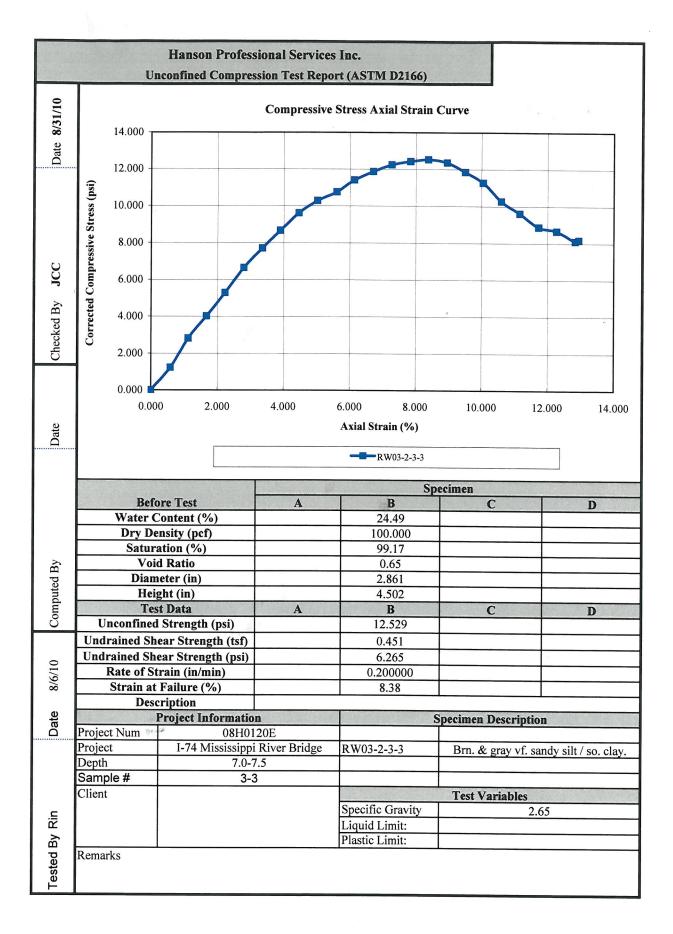


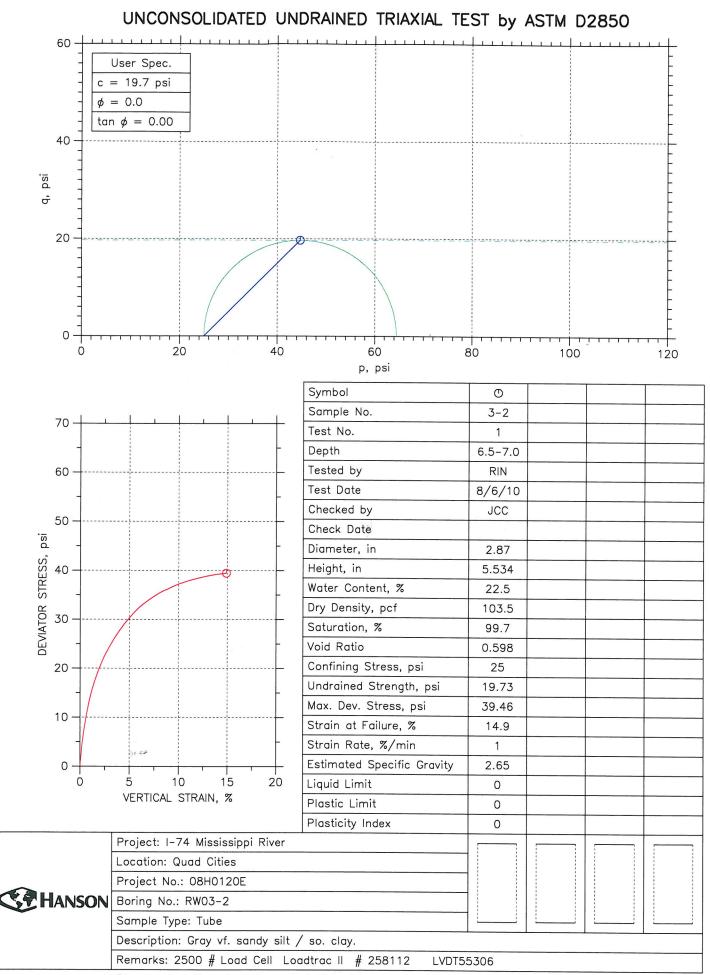




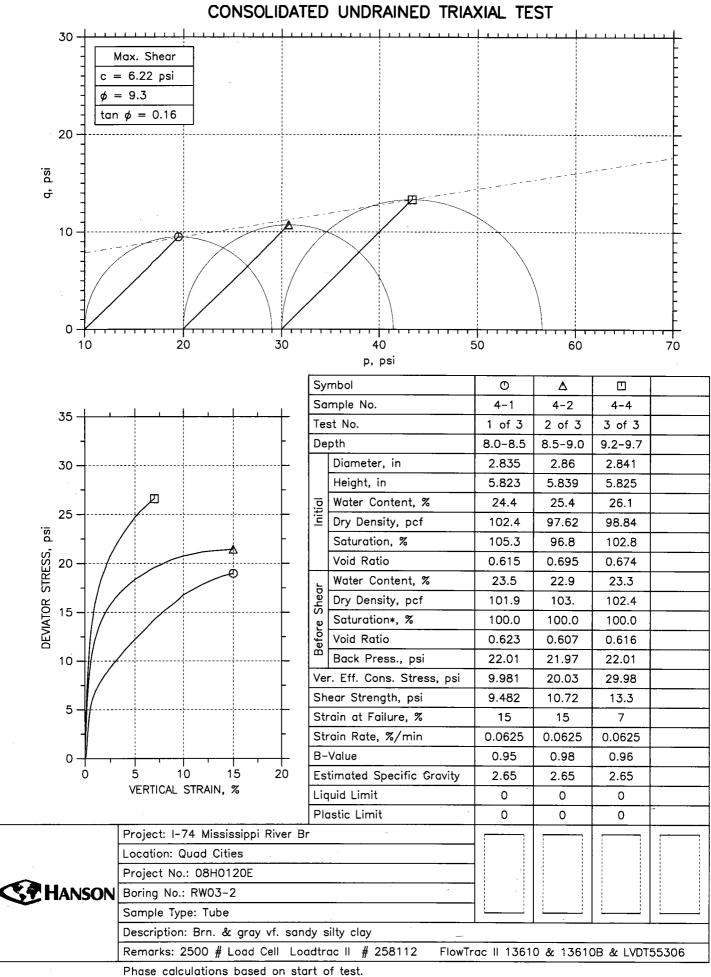






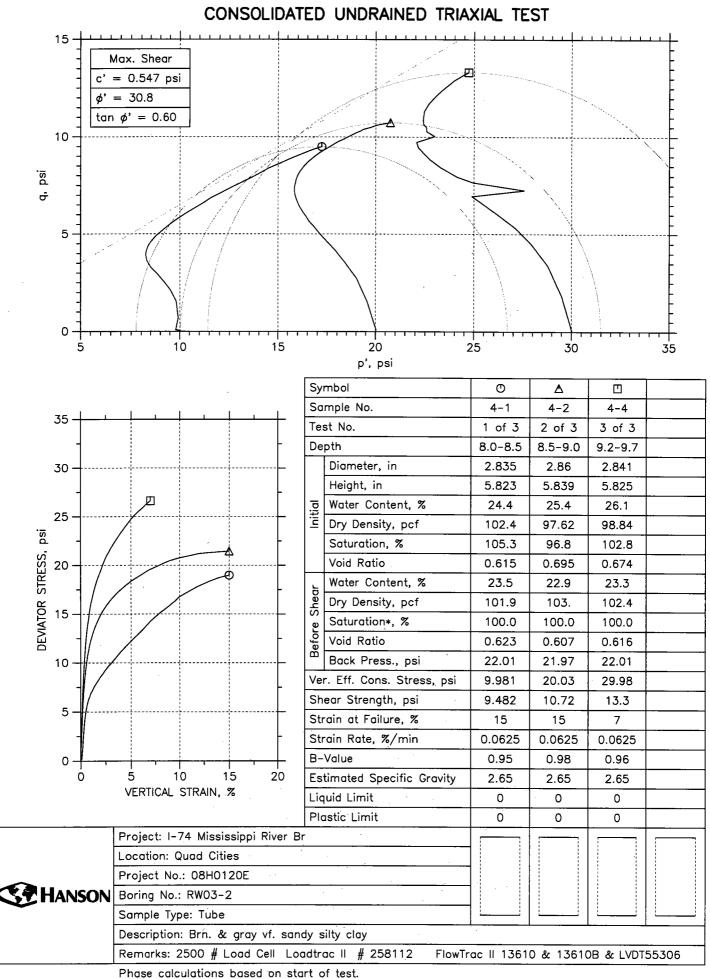


Phase calculations based on start of test.



Thu, 16-DEC-2010 14:53:35

* Saturation is set to 100% for phase calculations.

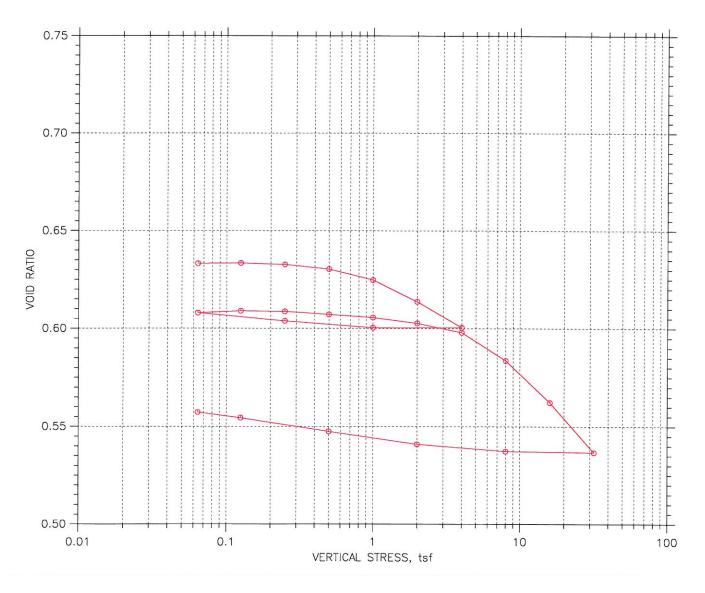


Thu, 16-DEC-2010 14:51:20

* Saturation is set to 100% for phase calculations.

CONSOLIDATION TEST DATA

SUMMARY REPORT



				*	Before Test	After Test
Overburden Pressure: 0 tsf			Water Content, %	24.59 21		
Preconsolidation Pressure: 0 tsf			Dry Unit Weight, pcf	101.3	106.2	
Compressi	Compression Index: 2.54639e-313			Saturation, %	103.06	103.78
Diameter: 2.499 in Height: 0.996 in		996 in	Void Ratio	0.63	0.56	
LL: 0	PL: 0	PI: 0	GS: 2.65			

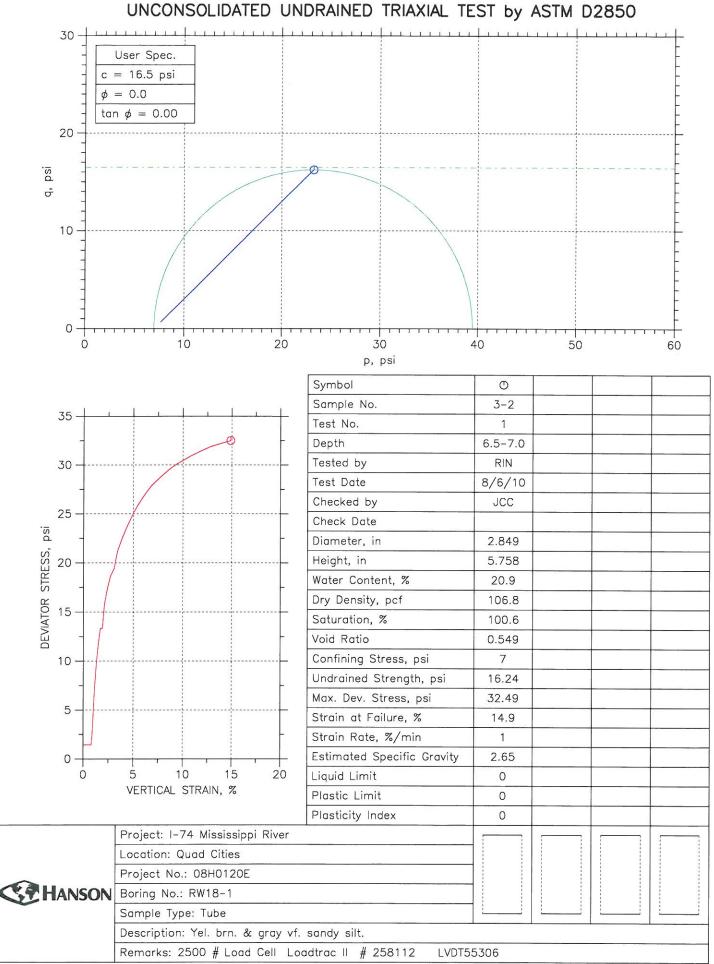
HANSON	Project: 174	Location: Quad Cities	Project No.: 08H0120E		
	Boring No.: RW03-2 Tested By: Rin		Checked By: JCC		
	Sample No.: 4-3	Test Date: 8/3/10	Depth: 9.0-9.2		
	Test No.: 1	Elevation:			
	Description: Red. & gray vf. sandy silty clay.				
	Remarks:				

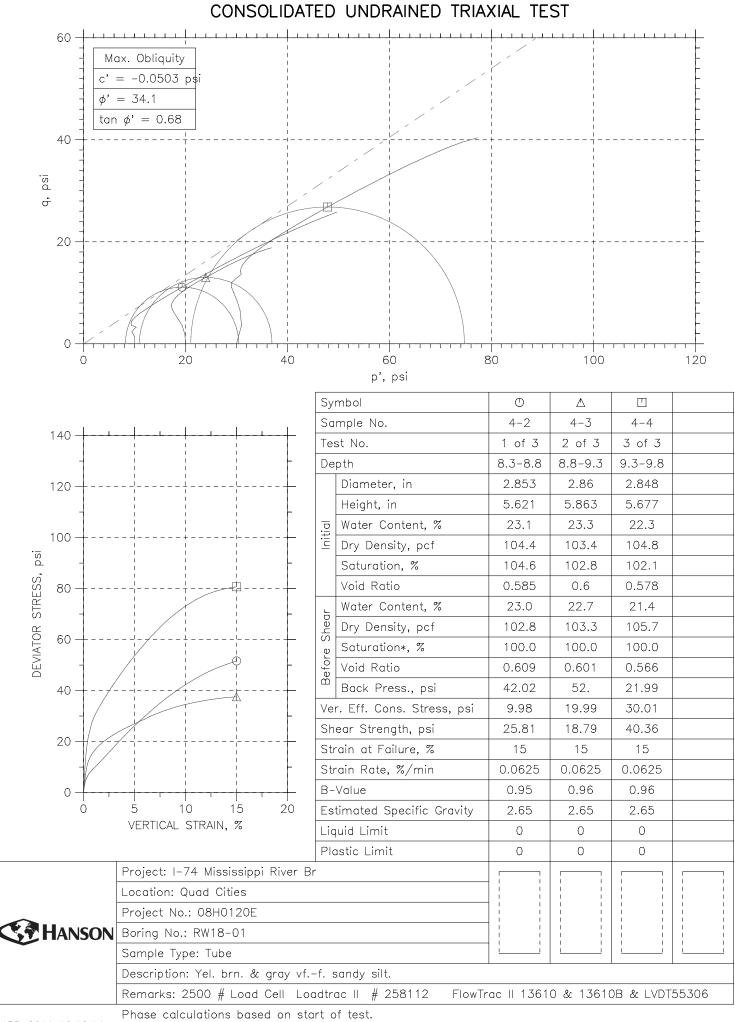
Project: I74Location: QuBoring No.: RW03-2Tested By: FSample No.: 4-3Test Date: FTest No.: 1Sample Type:

Location: Quad Cities Tested By: Rin Test Date: 8/3/10 Sample Type: Tube Project No.: 08H0120E Checked By: JCC Depth: 9.0-9.2 Elevation:

Soil Description: Red. & gray vf. sandy silty clay. Remarks:

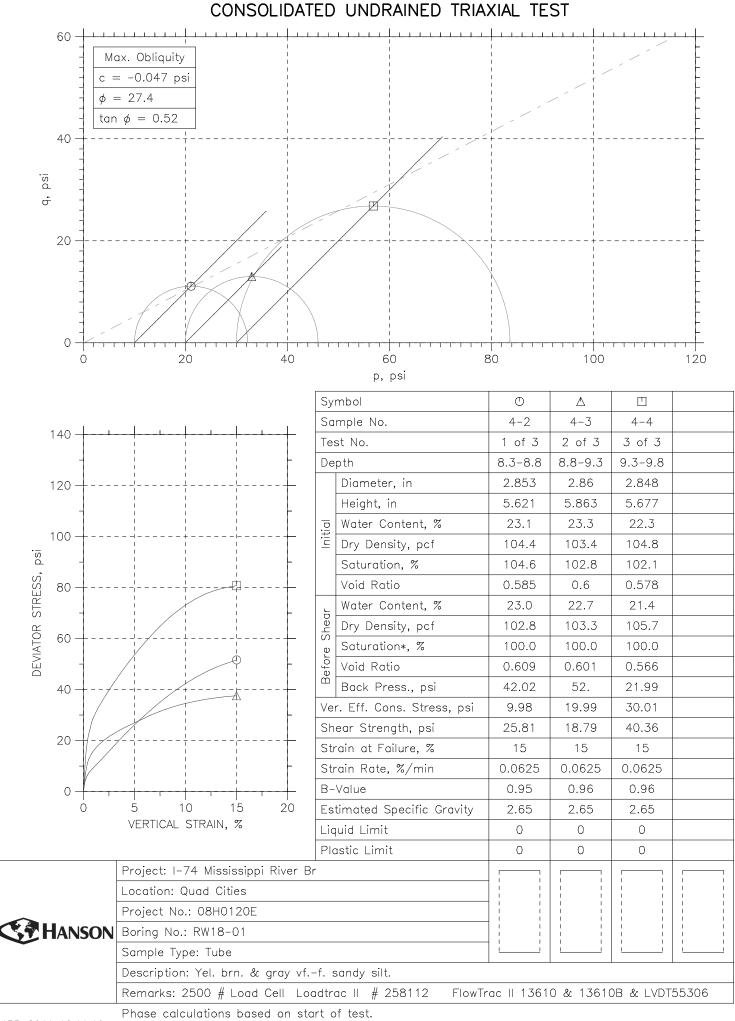
	Applied	Final	Final Void Strain T50		T50	Fitting	Coefficient of Consolidati			
	Stress	Displacement	Ratio	at End	Sq.Rt.	Log	Sq.Rt.	Log	Ave.	
	tsf	in		do	min	min	in^2/sec	in^2/sec	in^2/sec	
1	0.064	-0.0004536	0.633	-0.05	0.0	1.4	0.00e+000	5.83e-004	5.83e-004	
2	0.125	-0.0005591	0.633	-0.06	14.4	0.0	5.67e-005	0.00e+000	5.67e-005	
3	0.25	-0.0001238	0.633	-0.01	1.9	0.0	4.25e-004	0.00e+000	4.25e-004	
4	0.5	0.001302	0.630	0.13	1.8	1.3	4.53e-004	6.38e-004	5.30e-004	
5	1	0.004732	0.625	0.48	0.9	0.5	8.87e-004	1.61e-003	1.14e-003	
6	2	0.01146	0.614	1.15	0.5	0.5	1.76e-003	1.66e-003	1.71e-003	
7	4	0.01949	0.600	1.96	0.4	0.3	2.15e-003	2.46e-003	2.30e-003	
8	1	0.01957	0.600	1.97	0.1	0.1	6.76e-003	1.50e-002	9.32e-003	
9	0.25	0.01744	0.604	1.75	1.8	0.5	4.46e-004	1.70e-003	7.07e-004	
10	0.064	0.01493	0.608	1.50	3.5	3.2	2.23e-004	2.46e-004	2.34e-004	
11	0.125	0.01432	0.609	1.44	0.2	0.0	4.84e-003	0.00e+000	4.84e-003	
12	0.25	0.01451	0.609	1.46	0.4	0.0	2.21e-003	0.00e+000	2.21e-003	
13	0.5	0.01543	0.607	1.55	0.3	0.5	2.31e-003	1.64e-003	1.92e-003	
14	1	0.01642	0.605	1.65	0.5	0.3	1.58e-003	2.55e-003	1.95e-003	
15	2	0.01816	0.603	1.82	0.3	0.0	2.43e-003	0.00e+000	2.43e-003	
16	4	0.02103	0.598	2.11	0.3	0.1	2.30e-003	8.23e-003	3.59e-003	
17	8	0.02978	0.584	2.99	0.5	0.3	1.70e-003	2.39e-003	1.99e-003	
18	16	0.04281	0.562	4.30	0.3	0.3	2.19e-003	2.82e-003	2.47e-003	
19	32	0.05842	0.537	5.87	0.2	0.2	3.17e-003	4.02e-003	3.55e-003	
20	8	0.05805	0.537	5.83	0.0	0.0	3.65e-002	8.64e-002	5.13e-002	
21	2	0.05587	0.541	5.61	0.2	0.2	3.19e-003	4.48e-003	3.73e-003	
22	0.5	0.05185	0.547	5.21	1.8	1.4	4.01e-004	5.31e-004	4.57e-004	
23	0.125	0.04762	0.554	4.78	6.5	0.0	1.14e-004	0.00e+000	1.14e-004	
24	0.064	0.04585	0.557	4.60	36.4	28.1	2.04e-005	2.64e-005	2.30e-005	





Fri, 01-APR-2011 16:12:11

aat ta 10007 f Caturation in



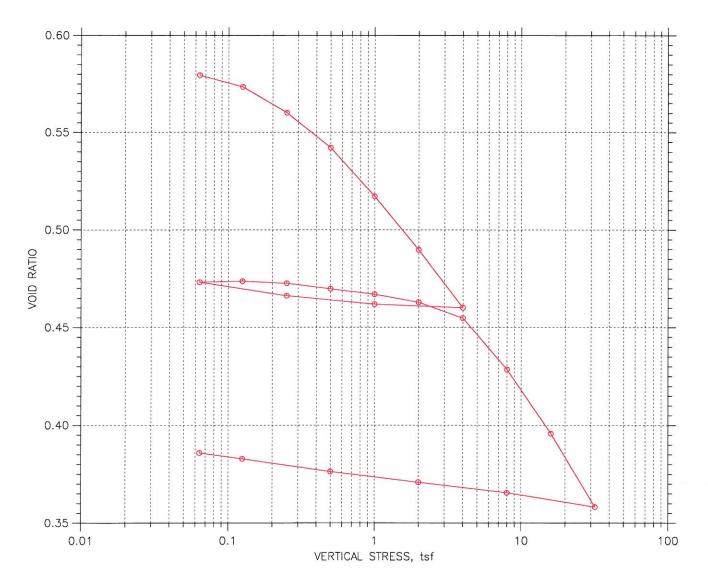
Fri, 01-APR-2011 16:11:19

Fildse culculations based on start of test.

* Saturation is pat to 100% for phase calculation

CONSOLIDATION TEST DATA

SUMMARY REPORT



					Before Test	After Test
Overburden Pressure: 0 tsf			Water Content, %	20.74	15.75	
Preconsolidation Pressure: 0 tsf			Dry Unit Weight, pcf	104.6	119.4	
Compression Index: 2.54639e-313			Saturation, %	94.51	108.15	
Diameter: 2.5 in Height: 0.998 in		998 in	Void Ratio	0.58	0.39	
LL: 0	PL: 0	PI: 0	GS: 2.65			

	Project: 174	Location: Quad Cities	Project No.: 08H0120E		
	Boring No.: RW18-01 Tested By: RIN		Checked By: JCC		
~	Sample No.: 4-1	Test Date: 8/3/10	Depth: 8.0-8.3		
HANSON	Test No.: 1	Elevation:			
	Description: Yel. brn. & gray vff. sandy silt.				
	Remarks: LT107 2000# 2009 Calibration				

Location: Quad Cities Tested By: RIN Test Date: 8/3/10 Sample Type: Tube

Project: I74 Boring No.: RW18-01 Sample No.: 4-1 Test No.: 1

Soil Description: Yel. brn. & gray vf.-f. sandy silt. Remarks: LT107 2000# 2009 Calibration Project No.: 08H0120E Checked By: JCC Depth: 8.0-8.3 Elevation:

	Applied	Final Void Strain T50 Fitting			cient of Con				
	Stress	Displacement	Ratio	at End	Sq.Rt.	Log	Sq.Rt.	Log	Ave.
	tsf	in		00	min	min	in^2/sec	in^2/sec	in^2/sec
1	0.064	0.00139	0.579	0.14	0.1	0.1	6.71e-003	9.51e-003	7.87e-003
2	0.125	0.005207	0.573	0.52	3.5	0.0	2.32e-004	0.00e+000	2.32e-004
2				1.36	3.5		2.32e-004 2.37e-004	0.00e+000	2.37e-004
	0.25	0.01361	0.560			0.0			
4	0.5	0.02494	0.542	2.50	2.0	1.2	3.90e-004	6.50e-004	4.87e-004
5	1	0.04072	0.517	4.08	0.9	0.0	8.42e-004	0.00e+000	8.42e-004
6	2	0.058	0.490	5.81	0.5	0.0	1.64e-003	0.00e+000	1.64e-003
7	4	0.07668	0.460	7.68	0.5	0.0	1.57e-003	0.00e+000	1.57e-003
8	1	0.07557	0.462	7.57	0.0	0.0	7.73e-002	7.88e-002	7.80e-002
9	0.25	0.07283	0.466	7.30	0.2	0.1	3.05e-003	1.38e-002	4.99e-003
10	0.064	0.06845	0.473	6.86	1.9	0.0	3.78e-004	0.00e+000	3.78e-004
11	0.125	0.06816	0.474	6.83	0.1	0.1	8.04e-003	1.32e-002	9.99e-003
12	0.25	0.06882	0.473	6.90	0.2	0.0	3.15e-003	0.00e+000	3.15e-003
13	0.5	0.07059	0.470	7.07	0.2	0.0	3.01e-003	2.60e-002	5.39e-003
14	1	0.07236	0.467	7.25	0.1	0.0	1.10e-002	3.84e-002	1.71e-002
15	2	0.07492	0.463	7.51	0.0	0.0	2.23e-002	6.58e-002	3.33e-002
16	4	0.08004	0.455	8.02	0.1	0.0	6.11e-003	5.86e-002	1.11e-002
17	8	0.09658	0.429	9.68	0.2	0.0	3.96e-003	4.22e-002	7.24e-003
18	16	0.1173	0.396	11.75	0.1	0.0	1.12e-002	5.91e-002	1.88e-002
19	32	0.1409	0.358	14.12	0.1	0.0	1.06e-002	5.06e-002	1.75e-002
20	8	0.1363	0.366	13.66	0.0	0.0	6.27e-002	0.00e+000	6.27e-002
21	2	0.133	0.371	13.32	0.0	0.0	3.68e-002	0.00e+000	3.68e-002
22	0.5	0.1295	0.376	12.98	0.5	0.0	1.36e-003	0.00e+000	1.36e-003
22	0.125	0.1254	0.383	12.50	1.9	0.0	3.32e-004	0.00e+000	3.32e-004
24	0.064	0.1235	0.386	12.38	13.4	0.0	4.69e-005	0.00e+000	4.69e-005