

07-12-2019 LETTING ITEM 041

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(19, 117) BP	SCOTT	12	1
FAP 562, FAS 743		CONTRACT NO. 72K78		

FOR INDEX OF SHEETS, SEE SHEET NO. 2

# PROPOSED BRIDGE PAINTING

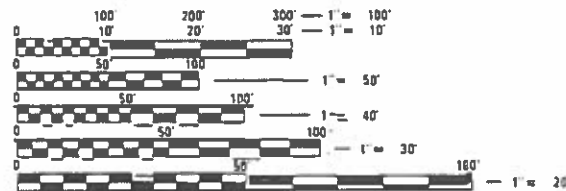
FAS 743, FAP 562 (HILLVIEW RD, IL 100)  
SECTION (19, 117) BP  
PROJECT STP-41TS(116)  
BRIDGE PAINTING  
SCOTT COUNTY

C-96-112-18

LOCATION #2  
SN 086-0039  
IL 100 OVER WALNUT CR  
1.5 MI N IL 106



LOCATION #1  
SN 066-0016  
HILLVIEW RD OVER SANDY CR  
5.0 MI S IL 106



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.  
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION  
1-800-892-0123  
OR 811

BRIDGE MAINTENANCE ENGINEER: BRANDON DUDLEY (217) 785-9290

GROSS LENGTH = x.xx FT. = x.xxx MILE  
NET LENGTH = x.xx FT. = x.xxx MILE

CONTRACT NO. 72K78

D-96-064-18



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

SUBMITTED 30 October 2018  
*[Signature]* REGIONAL ENGINEER  
June 14 2019  
*[Signature]* ENGINEER OF DESIGN AND ENVIRONMENT  
June 14 2019  
*[Signature]* DIRECTOR OF HIGHWAYS PROJECT IMPLEMENTATION 3

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OF THE STATE OF ILLINOIS





RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FAS 743	19BR	SCOTT	36	11

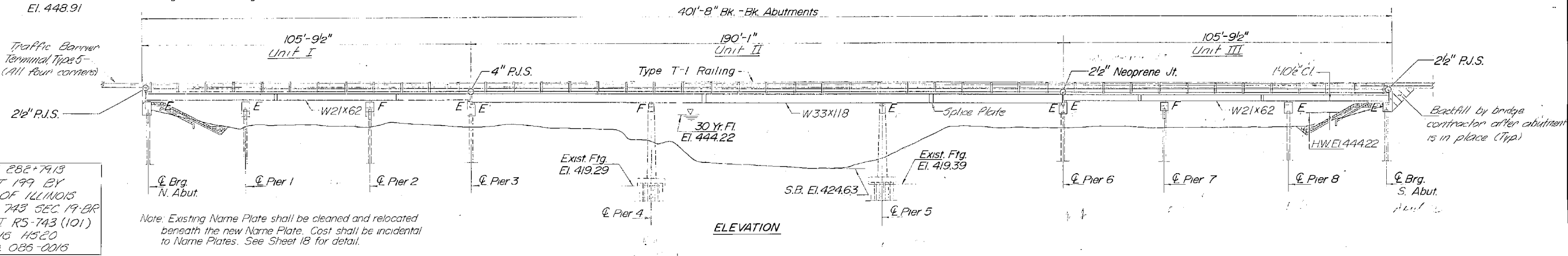
\* - BHF - 743 ( )

Sheet 1 of 24 Sheets

Existing Structure: Nine Span Reinforced Concrete Deck on wide flange deck beams, supported by six bent piers and two solid concrete piers. Open concrete abutments and concrete wingwalls. Bk-Bk Abutments 40'-8" Lengths: Spans 1 & 9 ±31'-9" Spans 2 & 8 ±40'-0", Spans 3 & 7 ±32'-4 1/2", Spans 4 & 6 ±58'-0 1/2", Span 5 ±74'-0". Clear Deck Width ±24'-0", 0° skew. Existing Structure No. 086-0016 Existing Structure to be used for Staging of Traffic during reconstruction.

Benchmark: U.S.G.S. tablet set in N.W. Wingwall of Existing Structure. El. 448.91

Traffic Barrier Terminal Type 5 (All four corners)



STA 282+74.3  
REBUILT 199 BY STATE OF ILLINOIS  
FAS RTE 743 SEC 19-BR  
FA PROJ RS-743 (101)  
LOADING HS20  
STR. NO. 086-0016

Note: Existing Name Plate shall be cleaned and relocated beneath the new Name Plate. Cost shall be incidental to Name Plates. See Sheet 18 for detail.

NAME PLATE (5th 2113)

WATERWAY INFORMATION

Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft.		Nat. H.W.E.	Head-Ft.		Headwater El.	
			Exlat.	Prop.		Exlat.	Prop.	Exlat.	Prop.
Design	30	12906	3043	3043	444.22	0.54	0.54	444.76	444.76
Base	100	16827	3667	3667	445.86	0.59	0.59	446.45	446.45
Overtopping	200	18800	3798	3798	446.43	0.61	0.61	447.24	447.24
Max. Calc.	500								

DESIGN STRESSES

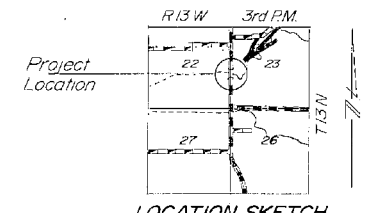
f'c = 3500 p.s.i.  
fy = 60000 p.s.i. (Reinf.)  
Fy = 50,000 p.s.i. (AASHTO M-223, Grade 50)

DESIGN SPECIFICATIONS

1989 A.A.S.H.T.O. Specifications & 1990 Interim Specifications. Seismic Retrofitting Guidelines for Highway Bridges

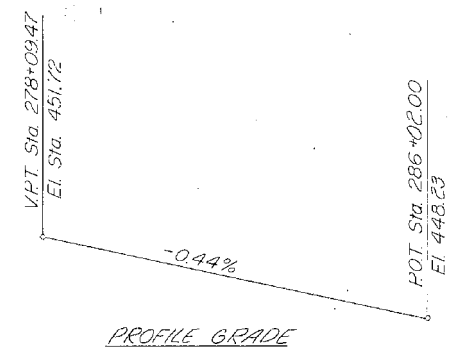
LOADING HS 20-44

No Allowance for Future Wearing Surface

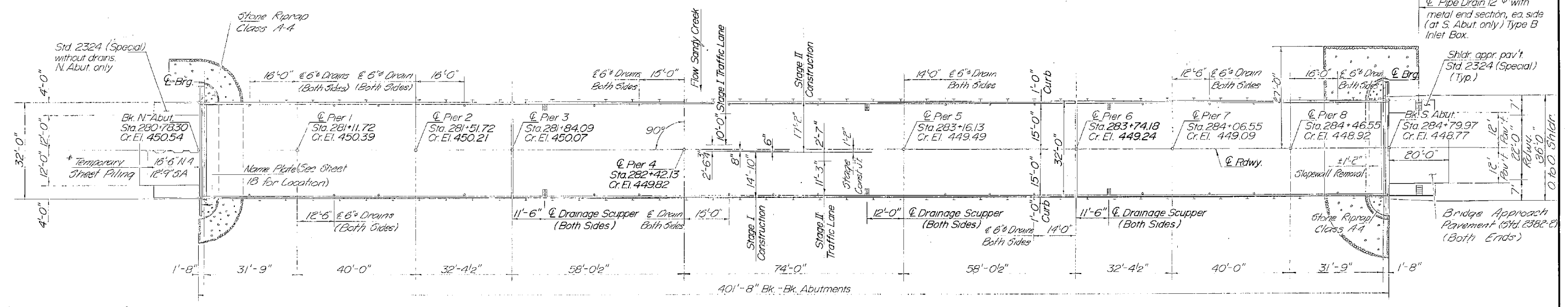


LOCATION SKETCH

12" Pipe Drain with metal end section, ea. side (at S. Abut. only) Type B Inlet Box.



PROFILE GRADE



\* North Abutment - Top El. 450.6 Tip El. 430.0 Est. Quant. - 340 Sq. Ft.  
South Abutment - Top El. - 448.8 Tip El. - 430.0 Est. Quant. - 240 Sq. Ft.  
See General Notes on Sheet No. 3 of P4



Expiration Date: 11/30/192

APPROVED FOR STRUCTURAL ADEQUACY ONLY

Allen Henderson, Licensed Structural Engineer

Note: 6" Deck Drains shall be placed at 15" O" centers and shall be placed at a minimum of 10" O" from the face of all substructure units. Drainage Scuppers shall replace 6" Deck Drains as noted on the Plan view. Temporary Sheet Piling required behind each abutment for Stage Construction.

GENERAL PLAN & ELEVATION  
F.A.S. ROUTE 743  
SECTION 19BR  
SCOTT COUNTY  
S.N. 086-0016

MODEL: Definit FILE: \\MAILS\GROUPS\DESIGN\Bridges\Bridges\Bridges\CAD\72K78 - Scott County.pbltr 2019.dgn

USER NAME = dudleybm	DESIGNED -	REVISED -
PLOT SCALE = 100,0000' / in.	DRAWN -	REVISED -
PLOT DATE = 5/28/2019	CHECKED -	REVISED -
	DATE -	REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS, SN 086-0016  
(FOR INFORMATION ONLY)

SCALE: SHEET OF SHEETS STA. TO STA.

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(19, 117) BP	SCOTT	12	4
* FAP 562, FAS 743		CONTRACT NO. 72K78		
ILLINOIS FED. AID PROJECT				













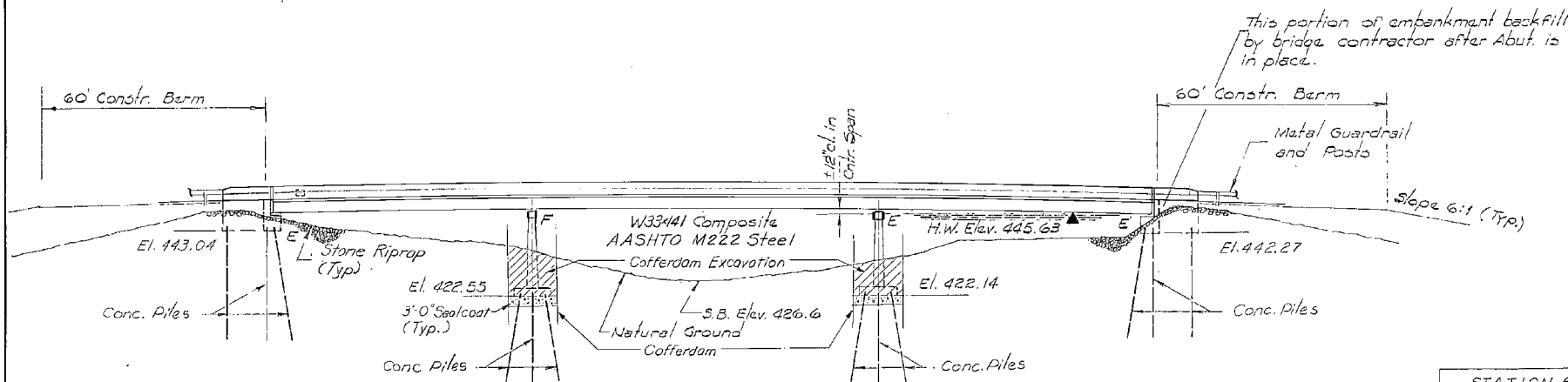
Bench Mark: USGS J245 Standard Bronze Disk in N.E. wingwall of walnut creek bridge, approx. 1.5 mi. north of intersection of U.S. Rte. 36 & S.B.1. Rte. 100 N. Elev. 447.29  
 Existing Structure: Single Steel Span thru Truss cnt. to cnt. brgs. 125'-0" on closed abuts and 20.8 ft. face to face curbs. Built as S.B.1. Rte. 100 Section 117-B2 in 1932 @ Sta. 687+94  
 Existing bridge will be used for traffic during the construction of the new bridge.  
 Existing Structure #086-0010, to be removed by bridge contractor after the completion of the new bridge.

STATE OF ILLINOIS  
 DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.B.1.	117B-2	SCOTT	52	14
13 SHEETS				

GENERAL NOTES

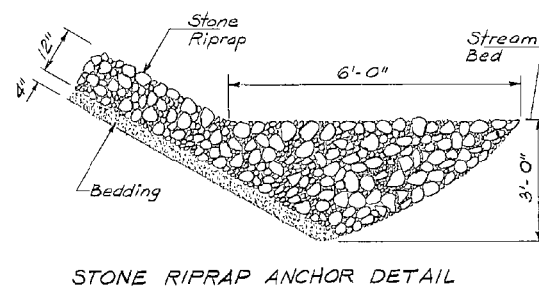
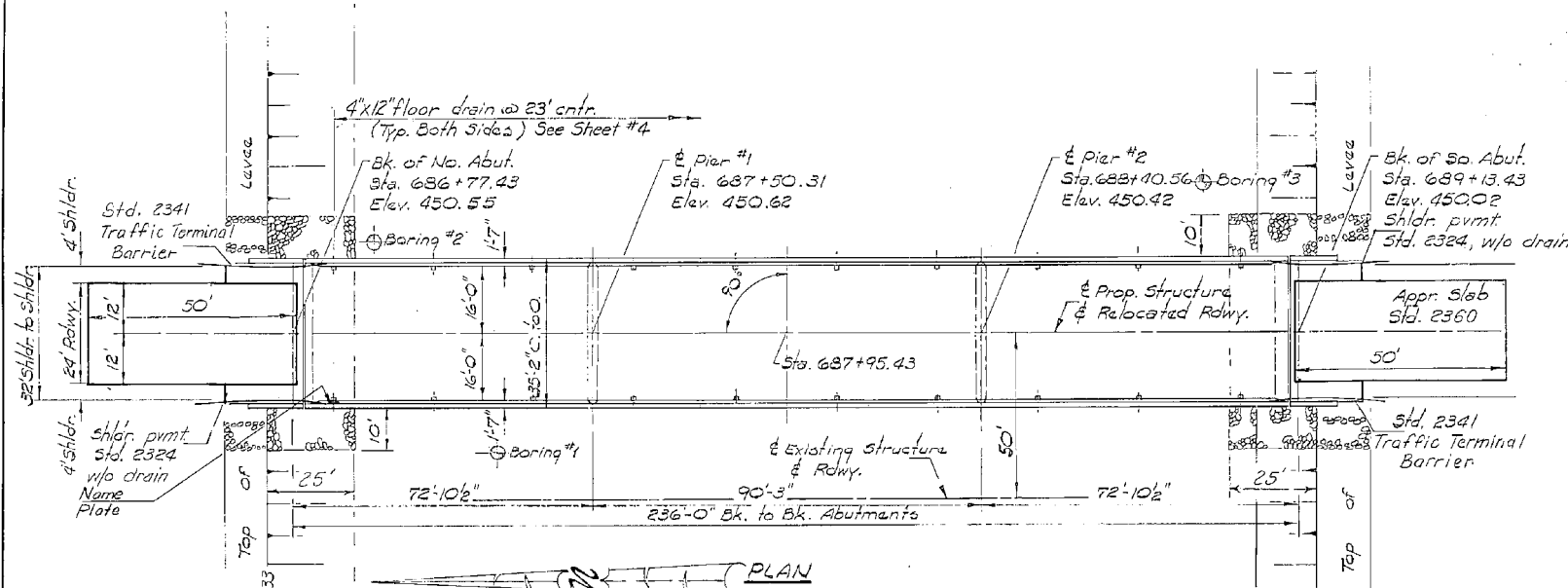
See Proposal for Boring Data.  
 Fasteners shall be high strength bolts (AASHTO M 164, Type 3). Bolts 3/4", open holes 1/2", unless otherwise noted.  
 Calculated weight of Structural Steel = 190,140 lbs.  
 The Zinc-Silicate and vinyl paint system shall be used for shop and field painting of Structural Steel except where otherwise noted.  
 All structural steel shall be AASHTO M 222 except expansion joint angles and attached bars which shall be AASHTO M 183.  
 Expansion joint angles and attached bars shall be shop painted with the Zinc-Silicate primer.  
 AASHTO M 222 structural steel shall not be painted except, that for a distance of three times the depth of the beams or girders (but not exceeding 10 feet) each way from deck joints, the AASHTO M 222 structural steel shall be cleaned and given one coat of the zinc-silicate primer and a dark maroon vinyl finish coat. Both coats may be applied in the shop with spot painting only in the field.  
 Field welding of construction accessories will not be permitted to the bottom flange of beams nor to the top flange for a distance equal to one-fourth the span length each way from the pier supports. Field welding in other areas will be permitted only when approved by the Engineer.  
 Anchor bolts shall be set before bolting diaphragms over supports.  
 The structural steel bearing plates of the Elastomeric Bearing Assembly shall conform to the requirements of AASHTO M 222.  
 The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Notch Toughness Zone 2. These Components are the wide flange beams and all splice plate material. Reinforcement bars shall conform to the requirements of AASHTO M-31 or M-53 Grade 60.  
 The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments.  
 Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8 inch. Adjustment shall be made either by grinding the surface or by shimming the bearing. Two 1/2" adjusting shims, of the dimensions of the bottom bearing plate, shall be provided for each bearing in addition to all other plates or shims. For Type I Elastomeric Bearings, shims of the dimensions of top plate shall be provided and placed as detailed.  
 The contractor shall drive one (1) concrete test pile in a permanent location at the North Abut. & Pier #2 as directed by the Engineer before ordering the remainder of piles.



ELEVATION  
 Design high water elevation for seal coat is ± 437.6.

STATION 687+95.43  
 BUILT 198 BY  
 STATE OF ILLINOIS  
 FA RT-562 SEC. 117B-2  
 LOADING HS 20  
 STR. NO. 086-0039

NAME PLATE  
 (See Std. 2113)

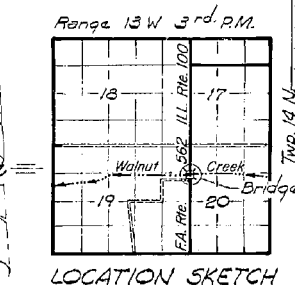


TOTAL BILL OF MATERIAL

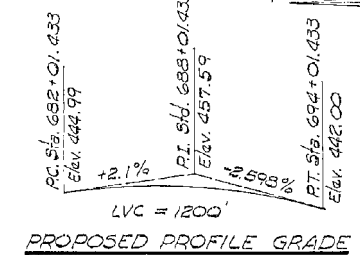
Item	Unit	Super	Sub.	Total
Protective Coat	Sq. Yd.	1,030		1,030
Removal of Existing Structures, No. 2	Each			1
Class X Concrete	Cu. Yd.	244.8	253.7	498.5
Stud Shear Connectors	Each	681		681
Reinforcement Bars	Pound		23,970	23,970
Reinf. Bars (Epoxy Coated)	Pound	66,060		66,060
Concrete Piles	Lin. Ft.		2,372	2,372
Test Piles (Concrete)	Each		2	2
Name Plates	Each	1		1
Stone Riprap	Tons		212	212
Neoprene Expansion Joint (2 1/2")	Lin. Ft.	33		33
Preformed Joint Seal (2 1/2")	Lin. Ft.	33		33
Seal Coat Concrete	Cu. Yd.		108.4	108.4
Cofferdams	Each		2	2
Floor Drains	Each	20		20
Cofferdam Excavation	Cu. Yd.		300	300
Structural Steel	L. Sum.			1
Structure Excavation	Cu. Yd.		140	140
Elastomeric Bearing Assembly Type I	Each	10		10
Elastomeric Bearing Assembly Type II	Each	5		5

DESIGN STRESSES  
 $f_c = 3,500$  psi  
 $f_y = 60,000$  psi (Reinforcement)  
 $f_y = 50,000$  psi (Structural AASHTO M222 Steel)

LOADING HS 20-44  
 Design Specifications: 1977 AASHTO and applicable Interims (1978 thru 1982 Interim Specifications) as applicable.  
 Allow 25#/sq.ft. for future wearing surface.



GENERAL PLAN  
 FA Rte. 562 Over WALNUT CREEK  
 FA Rte. 562 SECTION 117 B-2  
 SCOTT COUNTY  
 Sta. 687+95.43



WATERWAY INFORMATION

Drainage Area 56.2 sq. mi. Low grade Elev. 429.0' @ Sta.

Flood	Freq. Yr.	Q C.F.S.	Opening sq. ft.	Nat. H.W.E.	Head - Ft.	Headwater El.
			Exist. Prop.	Exist. Prop.	Exist. Prop.	Exist. Prop.
	50	7,500	1137 2220	445.63 0.38	0.04	446.01 445.67
Base	100	7670		445.83 0.38	0.04	446.21 445.87
Overtopping	25	7850		445.63 0.38	0.04	446.01 445.67

Leveed Channel - Levees are over topped @ Channel Sta. 5+00 the discharge is 7,500 cfs. The roadway design on the north approach is a 2 yr frequency.

DESIGNED	Bar Line
CHECKED	Patrick M. Petrone
DRAWN	Bar Line
CHECKED	Patrick M. Petrone

EXAMINED	JANUARY 27 1981
PASSED	
APPROVED	

STATE OF ILLINOIS  
 DEPARTMENT OF TRANSPORTATION

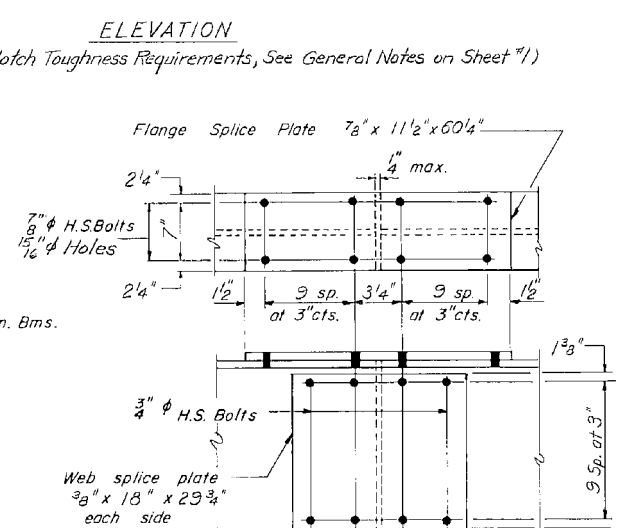
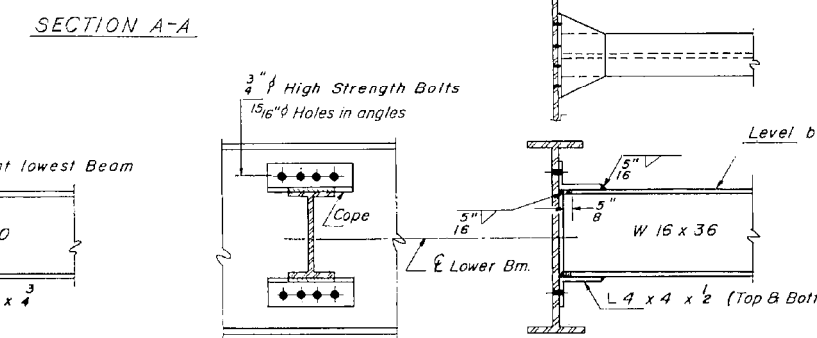
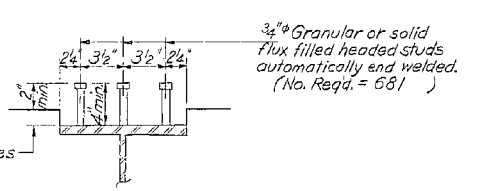
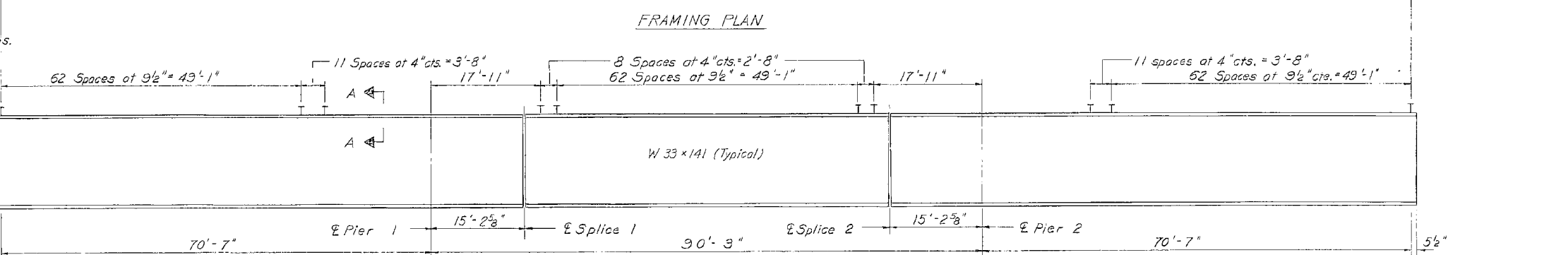
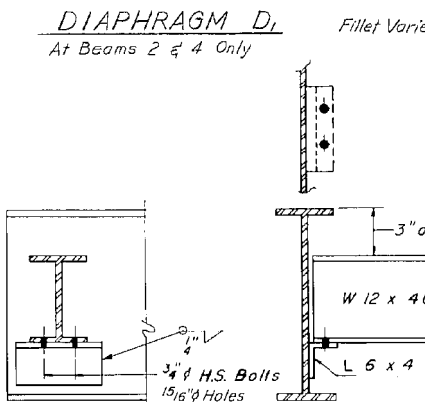
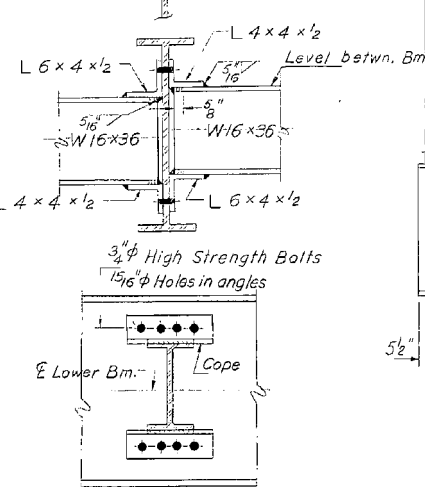
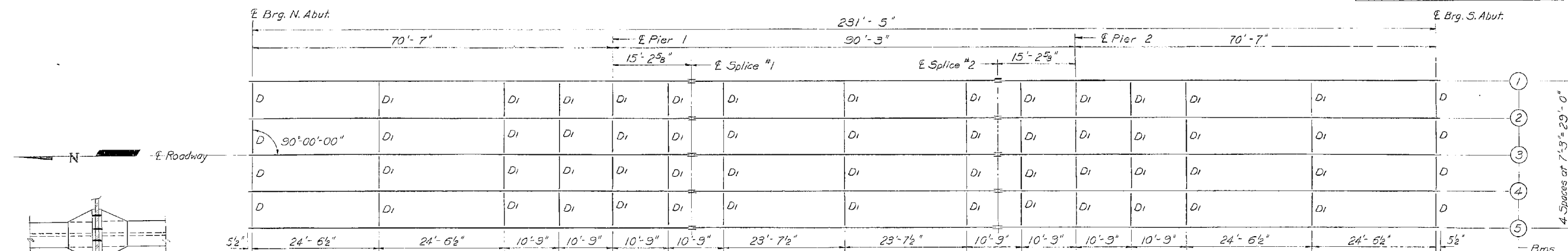
EXISTING PLANS, SN 086-0039  
 (FOR INFORMATION ONLY)

FA RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(19, 117) BP	SCOTT	12	10
* FAP 562, FAS 743		CONTRACT NO. 72K78		

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
562	117B-2	SCOTT	52	19
FED. ROAD DIST. NO. 3	ILL. ROAD	FED. AID PROJECT		

SHEET NO. 6  
13 SHEETS



3 SPAN CONTINUOUS SYMMETRICAL UNIT  
(COMPOSITE IN POSITIVE MOMENT AREAS ONLY)  
INTERIOR BEAM MOMENT TABLE

	4 Sp. or 6 Sp. 3	PIERS	5 Sp. 2
$I_s$ (in <sup>4</sup> )	7450	7450	7450
$I_c$ (in <sup>4</sup> )	18188		18188
$S_s$ (in <sup>3</sup> )	448	448	448
$S_c$ (in <sup>3</sup> )	624.4		624.4
$Z_s$ (in <sup>3</sup> )	514	514	514
$R$ (k/ft)	.867	.867	.867
$S_{\Phi}$ (k/ft)	.345	.345	.345
$M_{\Phi}$ (ik)	290	572	311
$M_{s\Phi}$ (ik)	134	182	170
$M_{\Phi}$ (ik)	564	370	637
$M_{Imp.}$ (ik)	144	90	148
$M$ (k+Imp.)	708	460	785
$M_{max}$ (ik)	2088	1979	2330
$M_u$ (ik)	3662	2141	3662
$VR$ (k)	55.4		47.1

$I_s$  and  $S_s$  are the moment of inertia and section modulus of the steel section.  
 $Z_s$  is the plastic section modulus of the steel section.  $I_c$  and  $S_c$  are the moment of inertia and section modulus of the composite section.  
 $VR$  is the maximum  $\Phi$  + Imp. shear range in span used to determine shear connector spacing.  $M_{max}$  is the maximum moment indicated by the maximum design load =  $1.3 \times [M_{\Phi} + M_{s\Phi} + S_s(M_{\Phi} + I)]$ .  
 $M_u$  is the maximum moment capacity.

TOP OF BEAM ELEVATIONS

	Bms #1 & #5	Bms #2 & #4	Bm #3
E Brg. N. Abut.	449.65	449.78	449.89
E Pier #1	449.63	449.76	449.87
E Splice #1	449.63	449.76	449.87
E Splice #2	449.49	449.62	449.73
E Pier #2	449.43	449.56	449.67
E Brg. S. Abut.	449.13	449.26	449.37

INTERIOR BEAM REACTION TABLE

	ABUTS.	PIERS
$R_{\Phi}$ (k)	32.1	108.1
$R_k$ (k)	40.2	55.7
Imp. (k)	10.3	13.6
$R$ Total (k)	82.6	177.4

STRUCTURAL STEEL  
F.A. RTE. 562 SECTION 117 B-2  
SCOTT COUNTY  
STA. 687+95.43

DESIGNED: Bao Liu  
CHECKED: Patrick M. Petrone  
DRAWN: Bao Liu  
CHECKED: Patrick M. Petrone

EXAMINED: [Signature] 1981  
PASSED: [Signature]  
APPROVED: [Signature]  
UNDER SECRETARY, CHIEF TRANSPORTATION ENGINEER

Note:  
All contact surfaces of joints for the diaphragms shall be free of paint or lacquer.  
Fasteners shall be high strength bolts AASHTO M 164, Type 3.

Note: Two Hardened washers shall be required over all 15/16 inch holes.

I-2-D 4-15-73

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