EXISTING BEARING REMOVAL DETAIL

(18 Required) 6 at each Abutment & 6 at Pier 2

PROCEDURE TO JACK AND REMOVE EXISTING BEARINGS

(North & South Abutments & Pier No. 2, Span 2 only) (Minimum Jack Capacity Required 60 tons at the Abutments and 110 tons at Pier 2)

- 1. Jack and Remove Existing Bearings shall be conducted according to the Bridge Special Provision "Jack and Remove Existing Bearings. See Interior Beam Reaction Table for loads.
- 2. Jacking and removing existing bearings shall be done after partial deck concrete removal and before new deck concrete is poured.
- 3. Three beams may be lifted simultaneously, as outlined in the stage construction layout.
- The existing anchor bolts shall be cut off flush with the existing bridge seat, the rockers, top and bottom plates shall be removed.
- Formwork and bearing seat construction shall occur.
- The new elastomeric bearings shall be placed and the jacks shall be lowered.
- 7. The new holes for the side retainers shall be drilled at the locations specified.
- 8. No Bearing Replacement is required at Pier 1 and (Pier 2 Span 3). Clean and paint as specified for Structural Steel.

DESIGNED B.B.

CHECKED C.J.F.

DRAWN

W.J.S.

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of

Anchor bolts for side retainers may be cast in place or installed in holes drilled before or after members are in place.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Side retainers and other steel members required at the expansion bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I. See Sheet 16 of 27 for Anchor Bolt location Details at South Abutment.

See Sheet 19 of 27 for Anchor Bolt location Details at North Abutment.

See Sheet 22 of 27 for Anchor Bolt location Details at Pier 1.

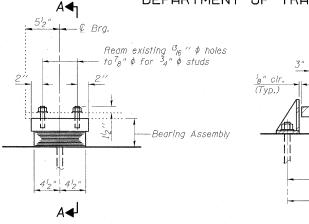
See Sheet 25 of 27 for Anchor Bolt location Details at Pier 2.

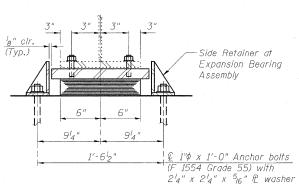
Side retainers, anchor holts, nuts, washers and bearing plates may be galvanized according to AASHTO M111 or M232 (as applicable).

Side Retainers at the Encased Fixed Bearings shall be paid for as Furnishing

& Erecting Structural Steel.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

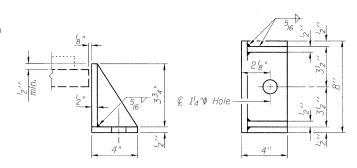




under nut SECTION A-A

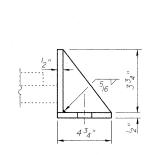
TYPE I ELASTOMERIC EXP. BRG.

(18 Required) 6 at each Abutment & 6 at Pier 2

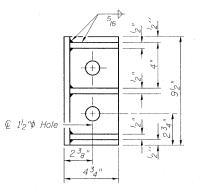


SIDE RETAINER AT EXPANSION BEARING ASSEMBLY

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

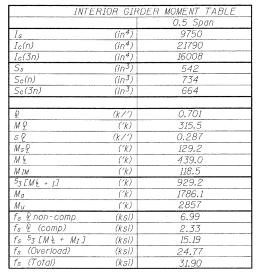


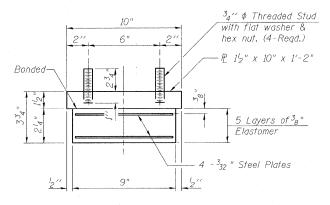
ELEVATION



SIDE RETAINER AT ENCASED FIXED BEARINGS

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.





BEARING ASSEMBLY

- $I_{\mathcal{S}}$, $S_{\mathcal{S}}$: Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads (in.4 and in.3).
- $I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_{ε} (Total and Overload) due to short-term composite live loads (in.4 and in.3).
- $I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads (in.4 and in.3).
 - Q: Un-factored non-composite dead load (kips/ft.).
 - $M\bar{\varrho}$: Un-factored moment due to non-composite dead load (kip-ft.).
 - $s\, ilde{arrho}:$ Un-factored long-term composite (superimposed) dead load (kips/ft.)
 - $M_S P$: Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).
 - Mt: Un-factored live load moment (kip-ft.).
 - Mi: Un-factored moment due to impact (kip-ft.).
 - Ma: Factored design moment (kip-ft.). 1.3 [$MQ + M_sQ + \frac{5}{3} (M_t + M_I)]$
 - Mu: Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).
- fs (Overload): Sum of stresses as computed from the moments below (ksi). $MQ + M_SQ + \frac{5}{3}(ML + M_I)$
- f (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).

1.3 [M2 + M_{s} 2 + $\frac{5}{3}$ (M½ + M_{I})]

	2½" Pier 1 ½" Pier 2	Side Retainer at Encased Bearing	RQ R4 Imp. RTotal
Encased Bearing	8 ⁵ g" 8 ⁵ g"	© 1½"\$\phi \times 1'-0" \text{ Anchor} \\ (F 1554 \text{ Grade 55}) \text{ with } \\ 2^3_4" \times 2^3_4" \times 2^5_{16}" \text{PL w} \\ under nut	
En	ENCASED FIXED BEAR	<u>INGS</u>	

INTERIOR BEAM REACTION TABLE						
		S. Abut.	Pier 1	Pier 2	N. Abut.	
R Q	(k)	29.6	59.0	58.2	28.7	
R4	(k)	37.0	68.3	68.2	36.8	
Imp.	(k)	10.0	18.4	18.4	9.9	
RTotal	(k)	76.6	145.8	144.7	75.5	

BILL OF MATERIAL

Item	Unit	Total				
Elastomeric Bearing Assembly Type I	Each	18				
Anchor Bolts, 1"	Each	. 36				
Anchor Bolts, 1 ¹ 4"	Each	72				
Jack & Remove Existing Bearings	Each	18				
Furnishing & Erecting Structural Steel	Pound	540				

BEARING DETAILS STRUCTURE NO. 025-0001

EFFINGHAM 1416

CONTRACT NO. 74296

COUNTY

TOTAL SHEET NO.

1368





PIERS 1 & 2

HEET NO.14	F.A.I. RTE.	SECTION				COUNT		
THEET NO. 14	57	(25-3HB)I-2				EFFINGH		
27 SHEETS SN 025-000			0001			CONTRA		
	FED. RO	DAD DIS	T. NO.	7	ILLINOIS	FED.	ΑI	D PROJECT

CHECKED C.J.F. & B.B.