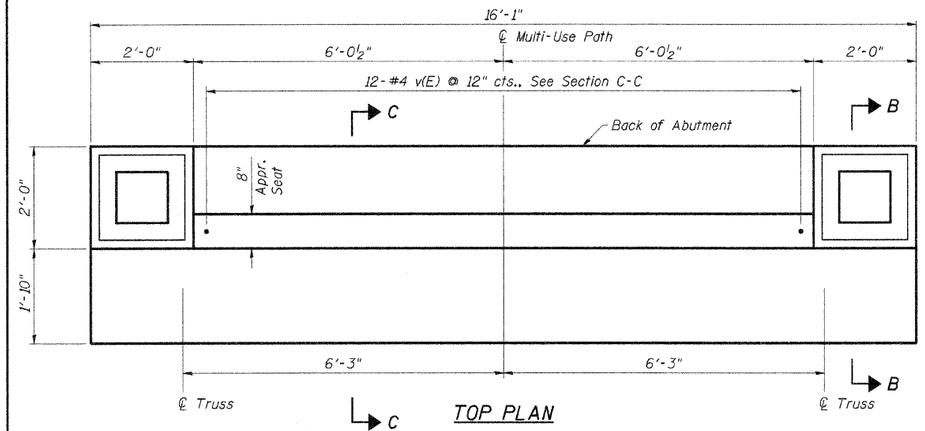
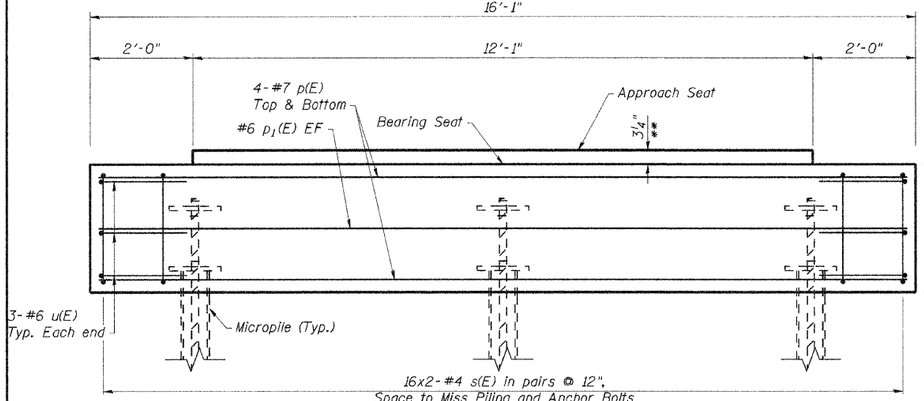


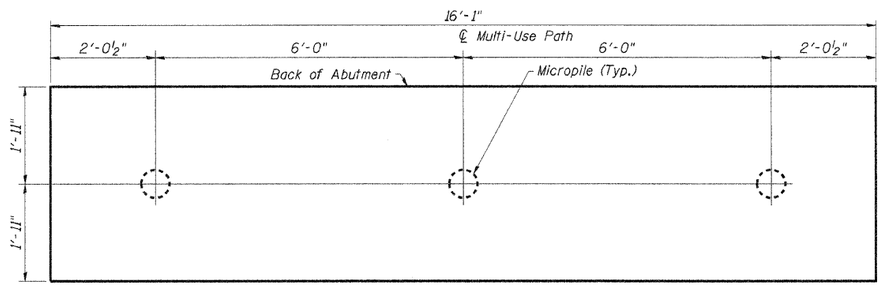
TOP PLAN

Note: Contractor to provide 8 - $\frac{3}{4}$" dia. x 18" long A36 anchor bolts with nuts and washers (galvanized), cost included in Pedestrian Truss Superstructure. Coordinate with Bridge Manufacturer's Anchor Bolt Layout

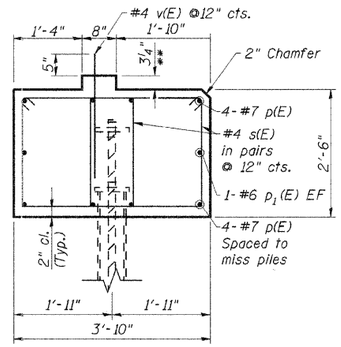


ELEVATION

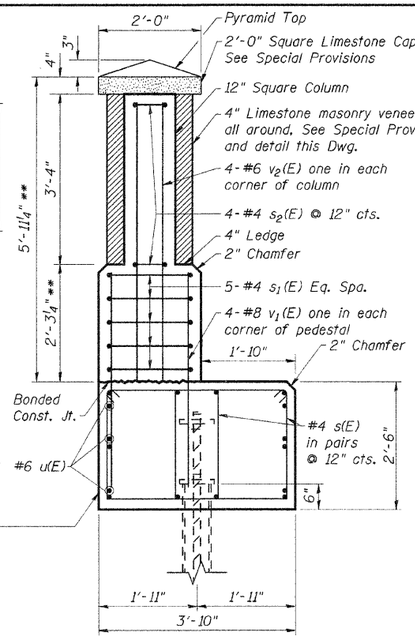
(Columns not shown for clarity)



PLAN - PILE CAP



SECTION C-C



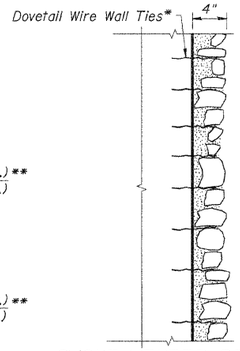
SECTION B-B

**Verify with truss superstructure design prior to construction of the substructure

Provide dovetail wire wall ties in abutment for block wall connection, cost included in Concrete Structures. Coordinate locations with Path Plans.

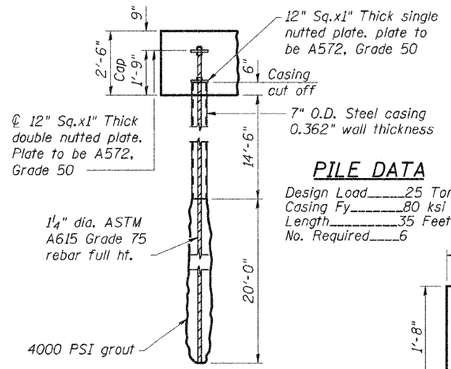
Elev. 692.93 (S. Abut.) **
Elev. 692.56 (N. Abut.)

Elev. 690.43 (S. Abut.) **
Elev. 690.06 (N. Abut.)



*Wall ties spaced so as to support no more than 2 square feet of wall. Ties shall be stainless steel. See Special Provisions.

STONE VENEER DETAIL



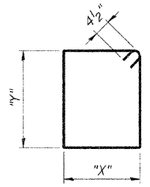
PILE DATA

Design Load_____25 Tons
Casing Fy_____80 ksi
Length_____35 Feet
No. Required____6

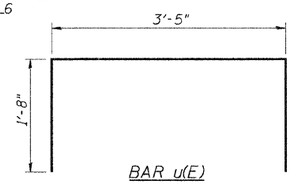
TYP. MICROPILE DETAIL

Note: The contractor shall verify the micropile design prior to construction, see special provisions.

Bar	"X"	"Y"
s(E)	2'-5"	2'-2"
s1(E)	1'-8"	1'-8"
s2(E)	8"	8"



BARS s(E), s1(E) and s2(E)



BAR u(E)

BAR BENDING DETAILS

TWO ABUTMENTS BILL OF MATERIAL

Bar	No.	Size	Length	Shape
p(E)	16	#7	15'-10"	——
p1(E)	4	#6	15'-10"	——
s(E)	64	#4	9'-11"	□
s1(E)	20	#4	7'-5"	□
s2(E)	16	#4	3'-5"	□
u(E)	12	#6	6'-9"	⊓
v(E)	24	#4	2'-6"	——
v1(E)	16	#8	4'-2"	——
v2(E)	16	#6	5'-5"	——
Structure Excavation			Cu. Yd.	21
Concrete Structures			Cu. Yd.	13.6
Reinforcement Bars, Epoxy Coated			Pound	1650
Micropiles			Each	6
Limestone Masonry Veneer			Sq. Ft.	105

MICROPILE TESTING REQUIREMENTS

1. Proof testing of at least 1 Micropile at each abutment shall be performed. Tension or compression testing may be utilized. The Micropile contractor shall use a test method which best suits the Geotechnical conditions.

2. Proof testing shall be in general conformance with ASTM D1143, Except as modified herein.

3. Test sequence shall be as follows:

Load	Hold Times (Minutes)
Alignment Load	0
0.15 Design Load	2.5
0.30 Design Load	2.5
0.45 Design Load	2.5
0.60 Design Load	2.5
0.75 Design Load	2.5
0.90 Design Load	2.5
1.00 Design Load	2.5
1.15 Design Load	2.5
1.30 Design Load	10
1.00 Design Load	4
0.75 Design Load	4
0.50 Design Load	4
0.25 Design Load	4
Alignment Load	4

4. Proof Load Acceptance Criteria:

   A) The pile shall sustain 1.0 Design load with no more than ½" total vertical movement at the top of the pile.

   B) Creep rate at the end of the 1.30 Design load shall be less than 0.04 in./Log Cycle Time.

   C) The slope of the load vs deflection curve at the end of the 1.30 Design load increment shall not exceed 0.025 in./kip.

5. The cost of testing and any additional reaction piles needed to facilitate the testing is included in "Micropiles, Each."

FILE NAME = 102000_S_02_HYB.dgn	USER NAME = RJT	DESIGNED - RDP 10/10	REVISED -	
		CHECKED - ELH 10/10	REVISED -	
	PLOT SCALE = 0.1 '/' / IN.	DRAWN - KAH 01/11	REVISED -	
	PLOT DATE = 1/24/2011 10:30:34 AM	CHECKED - RDP 01/11	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ABUTMENTS

SHEET NO. 2 OF 3 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	05-00416-00-BT	CHAMPAIGN	3	19
		CONTRACT NO.		
	ILLINOIS FED. AID PROJECT AID			