

NOTE

- SEE IDOT DISTRICT 1 STANDARD TC-12 "MULTI-LANE FREEWAY PAVEMENT MARKING DETAILS" FOR PLACEMENT AND TYPE OF RAISED REFLECTIVE PAVEMENT MARKERS.
- 2. FOR PROPOSED PAVEMENT MARKINGS IN THE PCC SECTIONS USE POLYUREA PAVEMENT MARKINGS ACCORDING TO STANDARD TC-12.

LEGEND

- THERMOPLASTIC PAVEMENT MARKING -LETTERS AND SYMBOLS (WHITE)
- THERMOPLASTIC PAVEMENT MARKING LINE 4" (YELLOW)
- THERMOPLASTIC PAVEMENT MARKING LINE 4" (WHITE) (10' DASH, 30' SKIP)
- THERMOPLASTIC PAVEMENT MARKING LINE 6" (WHITE) (2" DASH, 6" SKIP)
- 5 THERMOPLASTIC PAVEMENT MARKING LINE 6" (WHITE)
- 6 THERMOPLASTIC PAVEMENT MARKING LINE 12"
- POLYUREA PAVEMENT MARKING TYPE 1 -
- 8 POLYUREA PAVEMENT MARKING TYPE 1 LINE 4" (16) RAISED REFLECTIVE PAVEMENT MARKER (YELLOW)

- 9 POLYUREA PAVEMENT MARKING TYPE 1 LINE 4" (WHITE) (10' DASH, 30' SKIP)
- 10 POLYUREA PAVEMENT MARKING TYPE 1 LINE 6" (WHITE) (2' DASH, 6' SKIP), (3' DASH, 9' SKIP)
- POLYUREA PAVEMENT MARKING TYPE 1 LINE 6" (WHITE)
- 12 POLYUREA PAVEMENT MARKING TYPE 1 LINE 8"
- POLYUREA PAVEMENT MARKING TYPE 1 LINE 12" (YELLOW)
- 14 POLYUREA PAVEMENT MARKING TYPE 1 LINE 12"
- POLYUREA PAVEMENT MARKING TYPE 1 LINE 24"



EXISTING SIGN

SPEED LIMIT 40

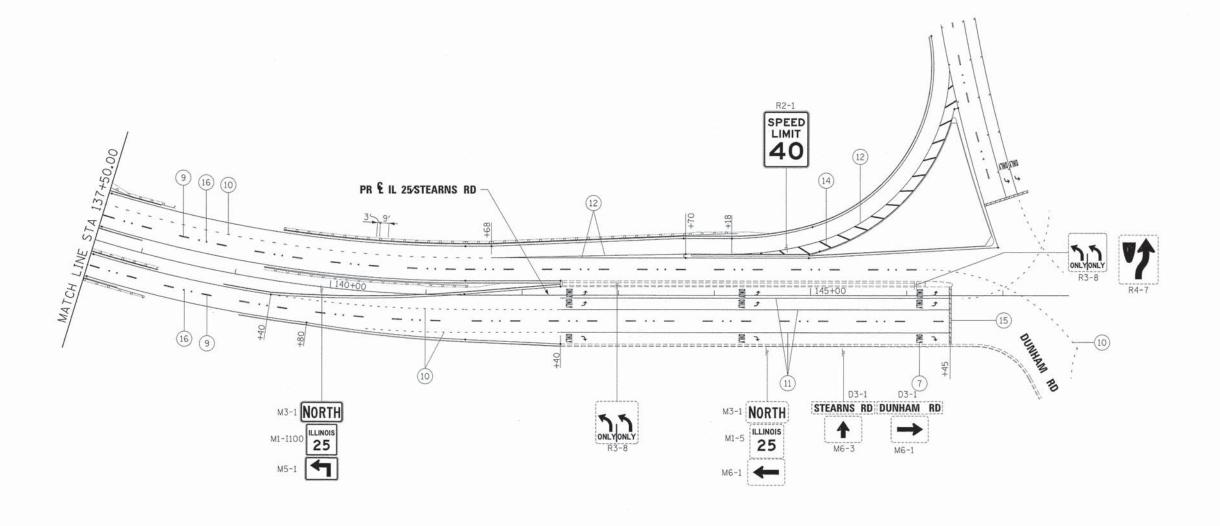
PROPOSED SIGN

OFFSETS ARE GIVEN TO SIGN POST

REFER TO SIGNING SCHEDULE PROPOSED SIGNS CAN EITHER BE NEW OR EXISTING SIGNS THAT HAVE BEEN RELOCATED

FILE NAME =	DESIGNED - AAA	REVISED -			PAVEMENT MARKING AND SIGNING PLAN	F.A.P.	SECTION	COUNTY	TOTAL SHEET
\D163598-sht-pmk-sign-03.dgn	DRAWN - TMB	REVISED -	benesch	STATE OF ILLINOIS		761	06 00014 10 00	VANE	AEI 204
USER NAME = tblank	CHECKED - JMM	REVISED -	engineers - scientists - planners	DIVISION OF TRANSPORTATION	IL 25/STEARNS RD	301	06-00214-10-RF	CONTRAC	T NO 63598
PLOT DATE = 1/17/2013	DATE - 01/18/2013	REVISED -			SCALE: 1"=50" SHEET NO. 3 OF 5 SHEETS STA. 125+50.00 TO STA. 137+50.00		ILLINOIS FED. A	AID PROJECT	1 10. 03330





NOTE

- SEE IDOT DISTRICT 1 STANDARD TC-12 "MULTI-LANE FREEWAY PAVEMENT MARKING DETAILS" FOR PLACEMENT AND TYPE OF RAISED REFLECTIVE PAVEMENT MARKERS.
- 2. FOR PROPOSED PAVEMENT MARKINGS IN THE PCC SECTIONS USE POLYUREA PAVEMENT MARKINGS ACCORDING TO STANDARD TC-12.

LEGEND

- 1 THERMOPLASTIC PAVEMENT MARKING LETTERS AND SYMBOLS (WHITE)
- 2 THERMOPLASTIC PAVEMENT MARKING LINE 4" (YELLOW)
- THERMOPLASTIC PAVEMENT MARKING LINE 4" (WHITE) (10' DASH, 30' SKIP)
- THERMOPLASTIC PAVEMENT MARKING LINE 6" (WHITE) (2" DASH, 6" SKIP)
- 5 THERMOPLASTIC PAVEMENT MARKING LINE 6" (WHITE)
- 6 THERMOPLASTIC PAVEMENT MARKING LINE 12"
- 7 POLYUREA PAVEMENT MARKING TYPE 1 LETTERS AND SYMBOLS (WHITE)
- 8 POLYUREA PAVEMENT MARKING TYPE 1 LINE 4" (YELLOW)

- 9 POLYUREA PAVEMENT MARKING TYPE 1 LINE 4" (WHITE) (10' DASH, 30' SKIP)
- POLYUREA PAVEMENT MARKING TYPE 1 LINE 6" (WHITE) (2" DASH, 6" SKIP), (3" DASH, 9" SKIP)
- 11 POLYUREA PAVEMENT MARKING TYPE 1 LINE 6" (WHITE)
- POLYUREA PAVEMENT MARKING TYPE 1 LINE 8"
- (13) POLYUREA PAVEMENT MARKING TYPE 1 LINE 12"
- 14 POLYUREA PAVEMENT MARKING TYPE 1 LINE 12"
- 15 POLYUREA PAVEMENT MARKING TYPE 1 LINE 24"

RAISED REFLECTIVE PAVEMENT MARKER (1-WAY CRYSTAL / OPAQUE)



EXISTING SIGN

LIMIT 40

PROPOSED SIGN

OFFSETS ARE GIVEN TO SIGN POST

REFER TO SIGNING SCHEDULE PROPOSED SIGNS CAN EITHER BE NEW OR EXISTING SIGNS THAT HAVE BEEN RELOCATED

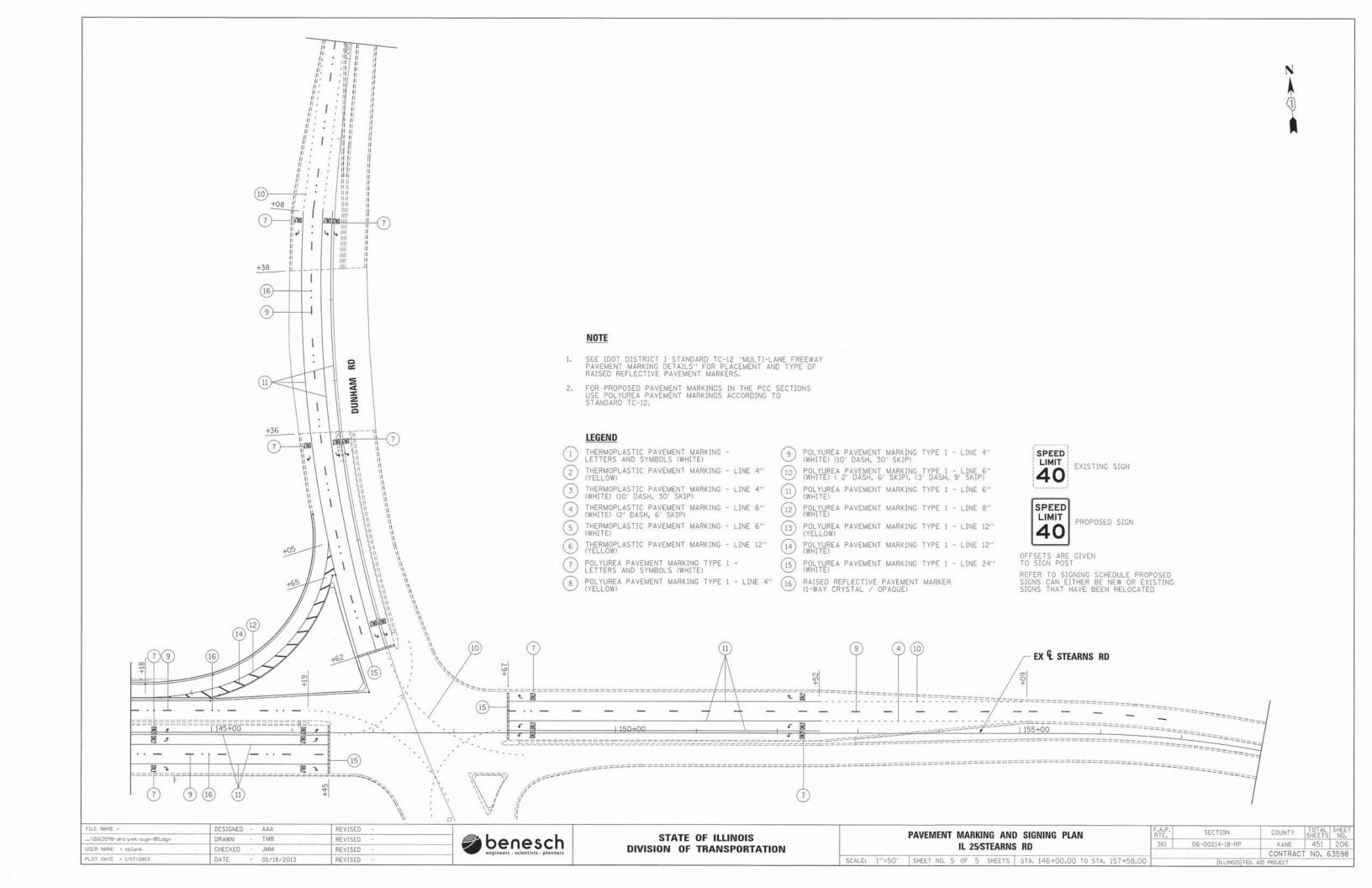
EN E MANE -	DESTRUED 111	DEWISE	
FILE NAME =	DESIGNED - AAA	REVISED -	
\D163598-sht-pmk-sign-04.dgn	DRAWN - TMB	REVISED -	4
USER NAME = tblank	CHECKED - JMM	REVISED -	
PLOT DATE = 1/17/2013	DATE - 01/1	8/2013 REVISED -	

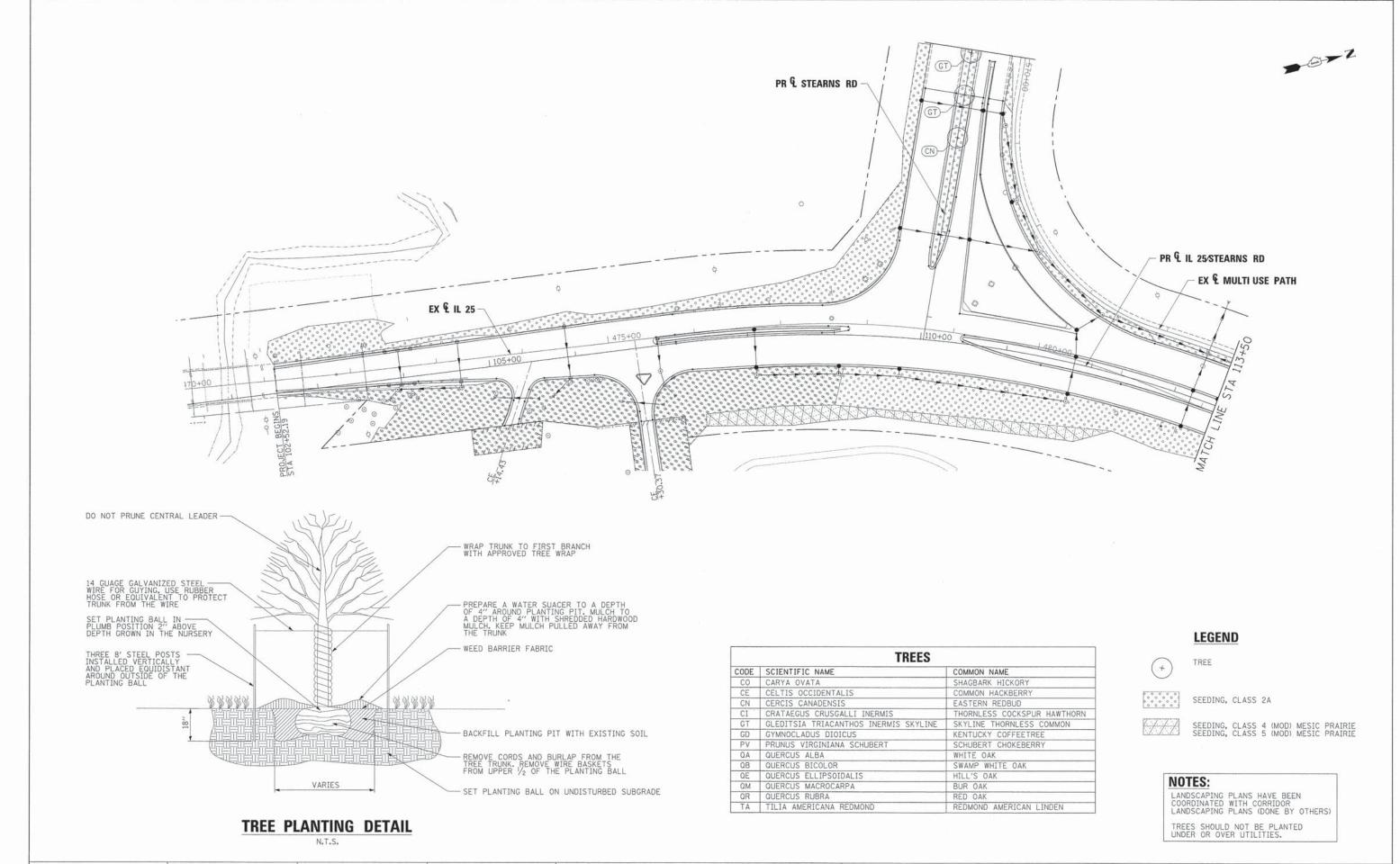
A DESTA	
	benesch
	Cellescii
	engineers - scientists - planners

STA	TE	OF ILLINOIS	
DIVISION	OF	TRANSPORTATION	

	PAVEMENT	MARKING AND	SIGNING PLAN	F.A.P. RTE.	
		IL 25/STEARNS	RD	361	C
-	CUEET NO A	OF E SUFFEE	ST4 177 (FO OO TO ST4 146 (OO OO	-	

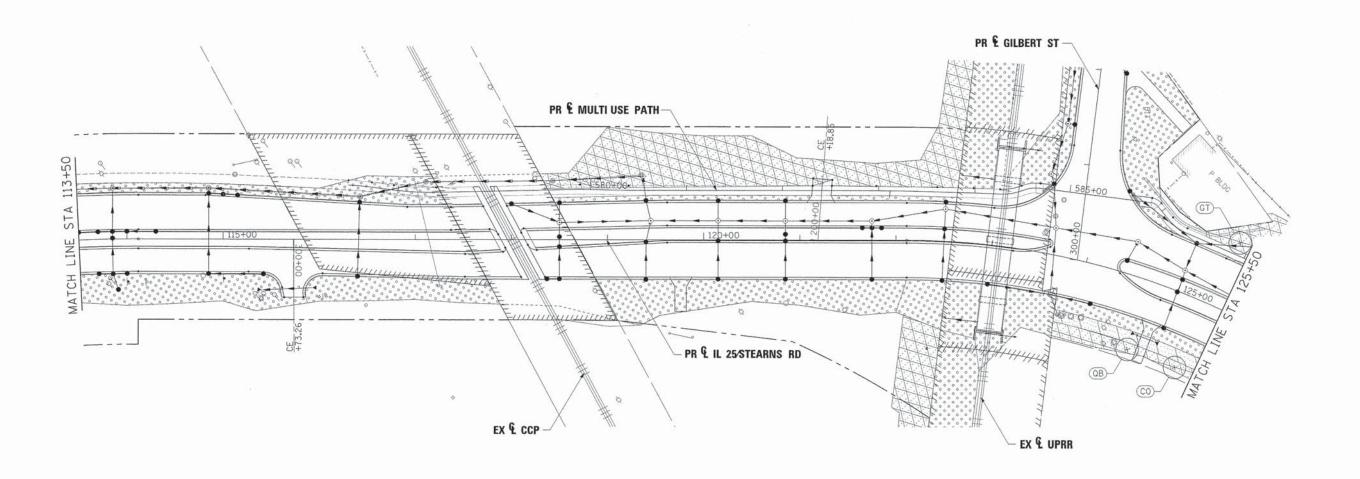
SECTION	COUNTY	TOTAL	SHEE NO.
06-00214-18-RP	KANE	451	205
	CONTRACT	NO. 6	3598
THE INOIS FED.	ATO PROJECT		





FILE NAME =	DESIGNED - AAF	REVISED -			LANDCCADING DIANG	F.A.P.	SECTION	COUNTY	TOTAL SHEET
\D163598-sht-1L25-stn-1s-01.dgn	DRAWN - TMB	REVISED -	benesch	STATE OF ILLINOIS	LANDSCAPING PLANS	RIE.	05 00014 10 00	KANE	SHEETS NO.
USER NAME a tblank	CHECKED - JMM	REVISED -	engineers - scientists - planners	DIVISION OF TRANSPORTATION	IL 25/STEARNS RD	361	06-00214-18-RP	CONTRAC	451 ZUT
PLOT DATE = 1/17/2013	DATE - 01/18/2013	REVISED -	- Congression Products		SCALE: 1"=50" SHEET NO. 1 OF 5 SHEETS STA. 102+52.19 TO STA. 113+50.00	J	ILLINOIS FED. A	ID PROJECT	1 10. 63336





TREES CODE | SCIENTIFIC NAME CO | CARYA OVATA COMMON NAME SHAGBARK HICKORY CE CELTIS OCCIDENTALIS COMMON HACKBERRY CN CERCIS CANADENSIS CI CRATAEGUS CRUSGALLI INERMIS EASTERN REDBUD THORNLESS COCKSPUR HAWTHORN GT GLEDITSIA TRIACANTHOS INERMIS SKYLINE SKYLINE THORNLESS COMMON GD GYMNOCLADUS DIOICUS PV PRUNUS VIRGINIANA SCHUBERT KENTUCKY COFFEETREE SCHUBERT CHOKEBERRY QA QUERCUS ALBA WHITE OAK QB QUERCUS BICOLOR SWAMP WHITE OAK QE QUERCUS ELLIPSOIDALIS HILL'S OAK QM QUERCUS MACROCARPA QR QUERCUS RUBRA BUR OAK RED OAK TA TILIA AMERICANA REDMOND REDMOND AMERICAN LINDEN

LEGEND



TREE



SEEDING, CLASS 2A



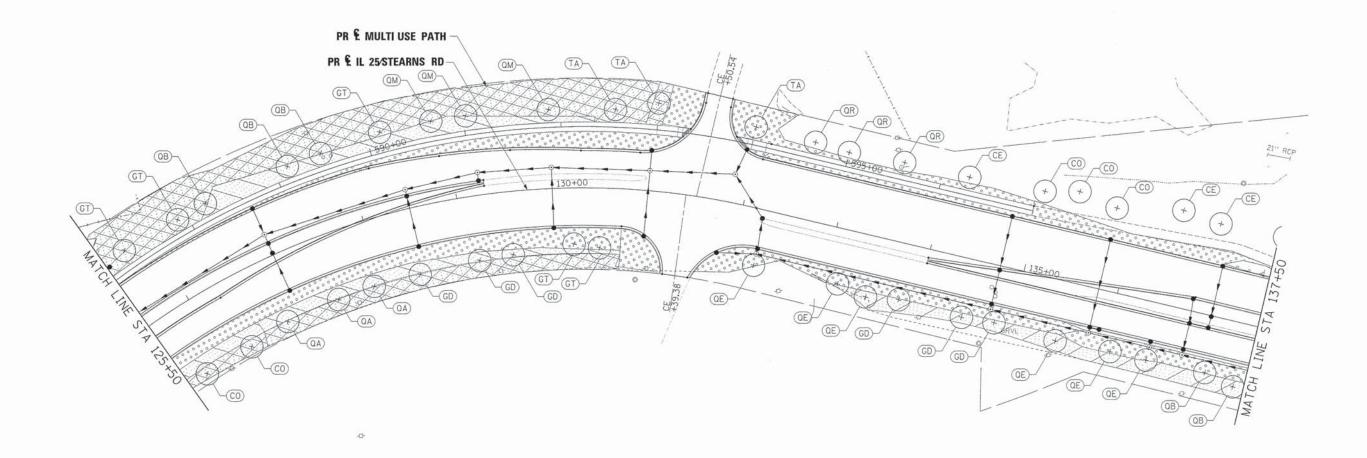
SEEDING, CLASS 4 (MOD) MESIC PRAIRIE SEEDING, CLASS 5 (MOD) MESIC PRAIRIE

NOTES:

LANDSCAPING PLANS HAVE BEEN COORDINATED WITH CORRIDOR LANDSCAPING PLANS (DONE BY OTHERS)

FILE NAME =	DESIGNED - AAF	REVISED -				LANDCCADING	DIANG	F.A.P.	SECTION	COUNTY	TOTAL SHE	ET
\D163598-sht-1L25-stn-1s-02.dgn	DRAWN - TMB	REVISED -	honosch	STATE OF ILLINOIS		LANDSCAPING		RIE.	00 00011 10 00	VANE	SHEETS NO	1.
USER NAME = tblank	CHECKED - JMM	REVISED -	benesch engineers - scientists - planners	DIVISION OF TRANSPORTATION		IL 25/STEARNS	S RD	361	06-00214-18-RP	CONTRAC	451 20	00
PLDT DATE = 1/17/2013	DATE - 01/18/2013	REVISED -			SCALE: 1"=50"	SHEET NO. 2 OF 5 SHEETS	STA. 113+50.00 TO STA. 125+50.00		ILLINOIS FED.	AID PROJECT	,1 NO. 6333	0





	TREES	
CODE	SCIENTIFIC NAME	COMMON NAME
CO	CARYA OVATA	SHAGBARK HICKORY
CE	CELTIS OCCIDENTALIS	COMMON HACKBERRY
CN	CERCIS CANADENSIS	EASTERN REDBUD
CI	CRATAEGUS CRUSGALLI INERMIS	THORNLESS COCKSPUR HAWTHORN
GT	GLEDITSIA TRIACANTHOS INERMIS SKYLINE	SKYLINE THORNLESS COMMON
GD	GYMNOCLADUS DIOICUS	KENTUCKY COFFEETREE
PV	PRUNUS VIRGINIANA SCHUBERT	SCHUBERT CHOKEBERRY
QA	QUERCUS ALBA	WHITE OAK
QB	QUERCUS BICOLOR	SWAMP WHITE OAK
QE	QUERCUS ELLIPSOIDALIS	HILL'S OAK
QM	QUERCUS MACROCARPA	BUR OAK
QR	QUERCUS RUBRA	RED OAK
TA	TILIA AMERICANA REDMOND	REDMOND AMERICAN LINDEN

LEGEND

(+)

INCE



SEEDING, CLASS 2A

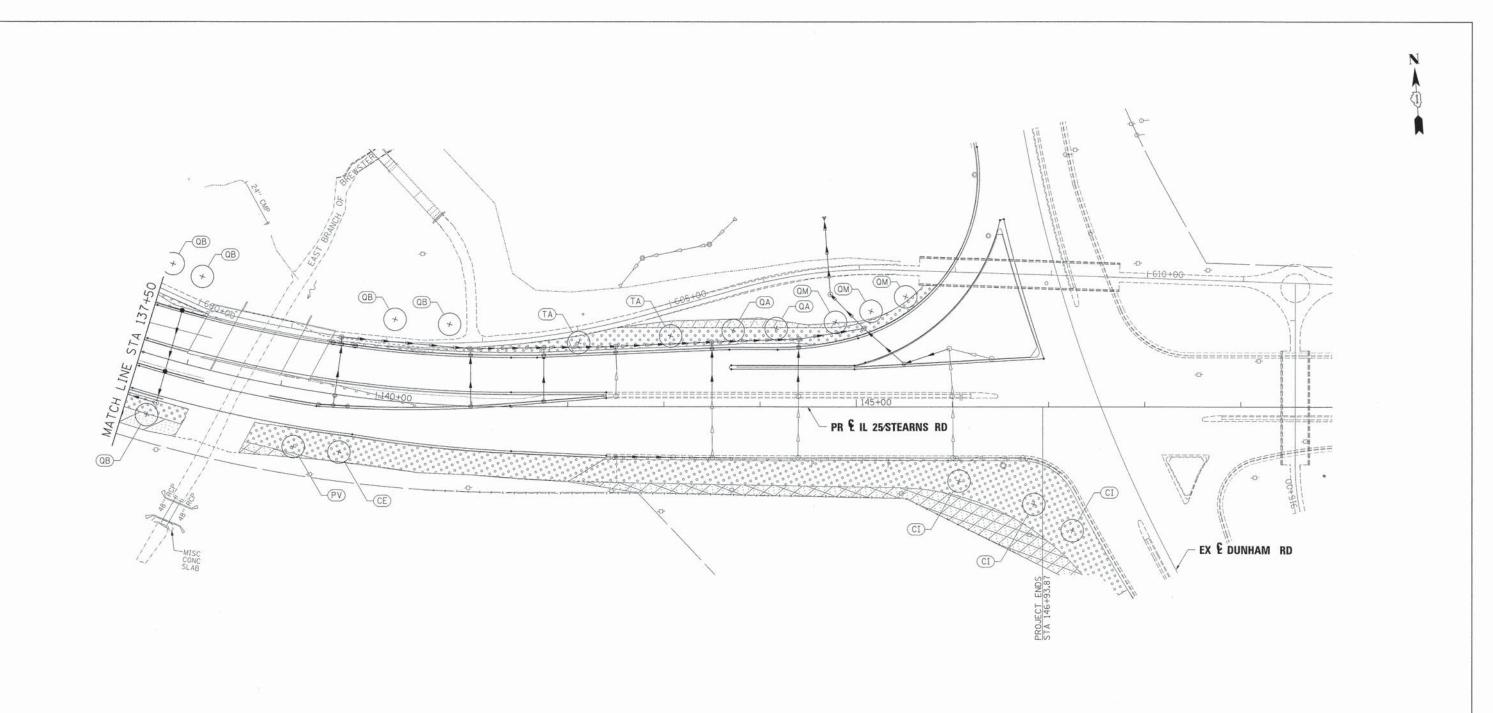


SEEDING, CLASS 4 (MOD) MESIC PRAIRIE SEEDING, CLASS 5 (MOD) MESIC PRAIRIE

NOTES:

LANDSCAPING PLANS HAVE BEEN COORDINATED WITH CORRIDOR LANDSCAPING PLANS (DONE BY OTHERS)

FILE NAME =	DESIGNED - AAF	REVISED -		CTATE OF HUMOIC	LANDSCAPING PLANS	F.A.P.	SECTION	COUNTY	SHEETS	SHEET NO.
\0163598-sht-IL25-stn-1s-03.dgn	DRAWN - TMB	REVISED -	benesch	STATE OF ILLINOIS	IL 25/STEARNS RD	361	06-00214-18-RP	KANE	451	209
USER NAME = tblank	CHECKED - JMM	REVISED -	engineers - scientists - planners	DIVISION OF TRANSPORTATION	IL ZJOTEMINO IID			CONTRAC	T NO. 6	3598
PLOT DATE = 1/17/2013	DATE - 01/18/2013	REVISED -			SCALE: 1"=50" SHEET NO. 3 OF 5 SHEETS STA. 125+50.00 TO STA. 137+50.00		ILLINOIS FED. A	ID PROJECT		



TREES CODE SCIENTIFIC NAME CO CARYA OVATA CE CELTIS OCCIDENTALIS COMMON NAME SHAGBARK HICKORY COMMON HACKBERRY CN CERCIS CANADENSIS EASTERN REDBUD CI CRATAEGUS CRUSGALLI INERMIS THORNLESS COCKSPUR HAWTHO GT GLEDITSIA TRIACANTHOS INERMIS SKYLINE SKYLINE THORNLESS COMMON THORNLESS COCKSPUR HAWTHORN GD GYMNOCLADUS DIOICUS PV PRUNUS VIRGINIANA SCHUBERT QA QUERCUS ALBA QB QUERCUS BICOLOR QE QUERCUS ELLIPSOIDALIS KENTUCKY COFFEETREE SCHUBERT CHOKEBERRY WHITE OAK SWAMP WHITE OAK HILL'S OAK QM QUERCUS MACROCARPA QR QUERCUS RUBRA BUR OAK RED OAK TA TILIA AMERICANA REDMOND REDMOND AMERICAN LINDEN

LEGEND

(+)

THEE



SEEDING, CLASS 2A



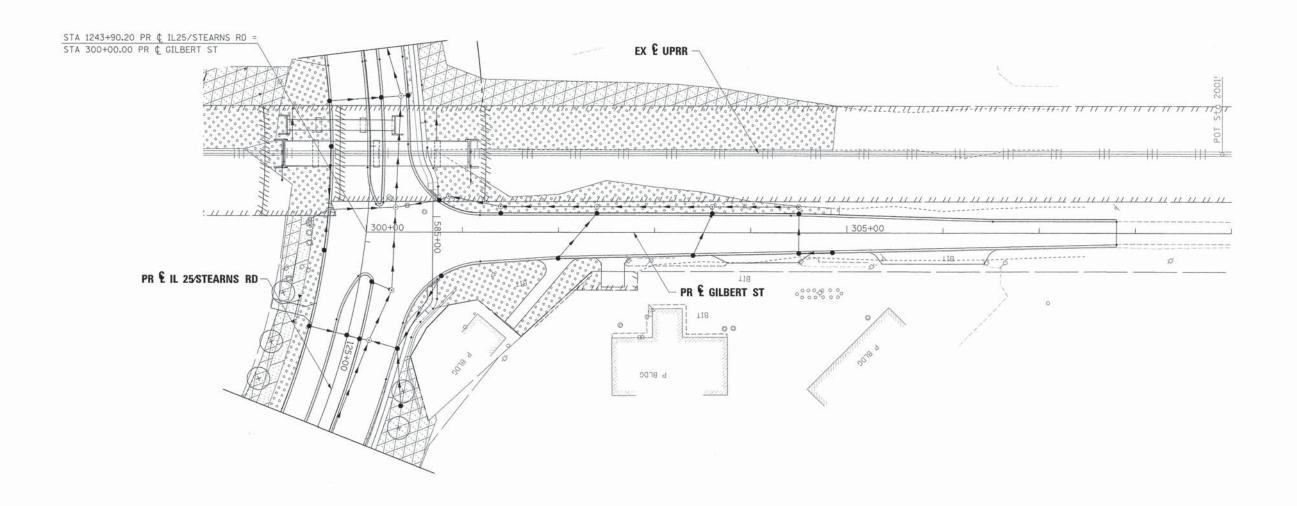
SEEDING, CLASS 4 (MOD) MESIC PRAIRIE SEEDING, CLASS 5 (MOD) MESIC PRAIRIE

NOTES:

LANDSCAPING PLANS HAVE BEEN COORDINATED WITH CORRIDOR LANDSCAPING PLANS (DONE BY OTHERS)

FILE NAME =	DESIGNED -	AAF	REVISED -		OTATE OF HUMOIO			1	ANDSC	APING	PLANS		F.A.P.	SECTION	COUNTY	TOTAL	SHEET	
\0163598~sht-1L25-stn-1s-04.dgn	DRAWN -	TMB	REVISED -	- benesch	STATE OF ILLINOIS			_		TEARN:			361	06-00214-18-RP	KANE	451	210	
USER NAME = tblank	CHECKED -	JMM	REVISED -	engineers - scientists - planners	DIVISION OF TRANSPORTATION				IL 230	, i LAIIII	J IID				CONTRA	CT NO.	63598	
PLOT DATE = 1/17/2013	DATE -	01/18/2013	REVISED -			SCALE: 1"=50"	' SHEE	T NO. 4	0F 5	SHEETS	STA. 137+50.00 TO STA. 14	6+93.87		ILLINOIS FED.	AID PROJECT			





	TREES	
CODE	SCIENTIFIC NAME	COMMON NAME
CO	CARYA OVATA	SHAGBARK HICKORY
CE	CELTIS OCCIDENTALIS	COMMON HACKBERRY
CN	CERCIS CANADENSIS	EASTERN REDBUD
CI	CRATAEGUS CRUSGALLI INERMIS	THORNLESS COCKSPUR HAWTHORN
GT	GLEDITSIA TRIACANTHOS INERMIS SKYLINE	SKYLINE THORNLESS COMMON
GD	GYMNOCLADUS DIOICUS	KENTUCKY COFFEETREE
PV	PRUNUS VIRGINIANA SCHUBERT	SCHUBERT CHOKEBERRY
QA	QUERCUS ALBA	WHITE OAK
QB	QUERCUS BICOLOR	SWAMP WHITE OAK
QE	QUERCUS ELLIPSOIDALIS	HILL'S OAK
QM	QUERCUS MACROCARPA	BUR OAK
QR	QUERCUS RUBRA	RED OAK
TA	TILIA AMERICANA REDMOND	REDMOND AMERICAN LINDEN

LEGEND

+

11/1



SEEDING, CLASS 2A



SEEDING, CLASS 4 (MOD) MESIC PRAIRIE SEEDING, CLASS 5 (MOD) MESIC PRAIRIE

NOTES:

LANDSCAPING PLANS HAVE BEEN COORDINATED WITH CORRIDOR LANDSCAPING PLANS (DONE BY OTHERS)

FILE NAME =\0163598-sht-g:l-1s-89.dgn	DESIGNED - DRAWN -	AAF TMB	REVISED -	⊘ benesch	STATE OF ILLINOIS	LANDSCAPING PLANS GILBERT ST	F.A.P. RTE. 361	SECTION 06-00214-18-RP	COUNTY	TOTAL SHEET NO. 451 211
USER NAME = tblank	CHECKED -	JMM	REVISED -	engineers - scientists - planners	DIVISION OF TRANSPORTATION	GEDENT OF			CONTRAC	CT NO. 63598
PLOT DATE = 1/17/2013	DATE -	01/18/2013	REVISED -			SCALE: 1"=50" SHEET NO. 5 OF 5 SHEETS STA. 300+00.00 TO STA. 307+80.3	8	ILLINOIS FED.	AID PROJECT	

SUMMARY OF QUANTITIES

CODE	DESCRIPTION	UNIT	TOTAL QUANTITY	IL-25 AT STEARNS RD	IL-25/STREANS AT GILBERT RD	IL-25/STEARNS RD AT DUNHAM	INTERCONNECT
80500020	SERVICE INSTALLATION - POLE MOUNTED	EACH	2	1	1		
81028200	UNDERGROUND CONDUIT, GALVANIZED STEEL, 2" DIA.	FOOT	2762	713	382	172	1495
81028210	UNDERGROUND CONDUIT, GALVANIZED STEEL, 2 1/2" DIA.	FOOT	1852	482	610	107	653
81028220	UNDERGROUND CONDUIT, GALVANIZED STEEL, 3" DIA.	FOOT	118	73	45		
81028240	UNDERGROUND CONDUIT, GALVANIZED STEEL, 4" DIA.	FOOT	1313	571	649	93	
81400100	HANDHOLE	EACH	21	9	8	1	3
81400200	HEAVY-DUTY HANDHOLE	EACH	3	11	2		
81400300	DOUBLE HANDHOLE	EACH	5	2	3		
86400100	TRANSCEIVER - FIBER OPTIC	EACH	2	1	1		
87300925	ELECTRIC CABLE IN CONDUIT, TRACER, NO. 141C	FOOT	4380			~~	4380
87301215	ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 142C 3494	FOOT	5471	1526	1968	(1977) 🔝	
87301225	ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 143C	FOOT	2215	798	1417	4	
87301245	ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 145C	FOOT	8370	4409	2962	999	
87301255	ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 147C	FOOT	607		607	~~	
87301305	ELECTRIC CABLE IN CONDUIT, LEAD-IN, NO. 141 PAIR 9229	FOOT	7252	4362	2890	{ 1977 }	
87301805	ELECTRIC CABLE IN CONDUIT, SERVICE, NO. 62C	FOOT	77	40	37	w	
87301900	ELECTRIC CABLE IN CONDUIT, EQUIPMENT GROUNDING CONDUCTOR, NO. 6 1C	FOOT	6854	3762	2125	967	
87502480	TRAFFIC SIGNAL POST, GALVANIZED STEEL 14 FT.	EACH	3	1		2	
87502500	TRAFFIC SIGNAL POST, GALVANIZED STEEL 16 FT.	EACH	1		1		
87502520	TRAFFIC SIGNAL POST, GALVANIZED STEEL 18 FT.	EACH	1		1		
87700230	STEEL MAST ARM ASSEMBLY AND POLE, 38 FT.	EACH	1	1			
87700250	STEEL MAST ARM ASSEMBLY AND POLE, 42 FT.	EACH	2	1	1		
87702910	STEEL COMBINATION MAST ARM ASSEMBLY AND POLE 36 FT.	EACH	1	1			
87702990	STEEL COMBINATION MAST ARM ASSEMBLY AND POLE 54 FT.	EACH	1		1		
87800100	CONCRETE FOUNDATION, TYPE A	FOOT	52	24	20	8	
87800150	CONCRETE FOUNDATION, TYPE C	FOOT	8	4	4		
87800415	CONCRETE FOUNDATION, TYPE E 36-INCH DIAMETER	FOOT	74	35	39		
87900200	DRILL EXISTING HANDHOLE	EACH	5	1		3	1
88030020	SIGNAL HEAD, LED, 1-FACE, 3-SECTION, MAST-ARM MOUNTED	EACH	15	8	7		
88030050	SIGNAL HEAD, LED, 1-FACE, 3-SECTION, BRACKET MOUNTED	EACH	9	4	5		
88030110	SIGNAL HEAD, LED, 1-FACE, 5-SECTION, MAST-ARM MOUNTED	EACH	1		1		
88030210	SIGNAL HEAD, LED, 2-FACE, 3-SECTION, BRACKET MOUNTED	EACH	3	2	1		
88030240	SIGNAL HEAD, LED, 2-FACE, 1-3 SECTION, 1-5 SECTION, BRACKET MOUNTED	EACH	1	-	1		
88102717	PEDESTRIAN SIGNAL HEAD, LED, 1-FACE, BRACKET MOUNTED WITH COUNTDOWN TIMER	EACH	2		2		
88200210	TRAFFIC SIGNAL BACKPLATE, LOUVERED, ALUMINUM	EACH	34	16	17	1	
88500100	INDUCTIVE LOOP DETECTOR	EACH	19	11	8	-	
88600700	PREFORMED DETECTOR LOOP	FOOT	1339	656	683		
88700200	LIGHT DETECTOR	EACH	5	2	3		
88700300	LIGHT DETECTOR AMPLIFIER	EACH	2	1	1		
88800100	PEDESTRIAN PUSH-BUTTON	EACH	2	-	2		
89000100	TEMPORARY TRAFFIC SIGNAL INSTALLATION	EACH	3	1	1	1	
89501150	RELOCATE EXISTING TRAFFIC SIGNAL POST	EACH	8	4	4		
89501300	RELOCATE EXISTING MAST ARM ASSEMBLY AND POLE	EACH	1	1			
89502105	REBUILD EXISTING SIGNAL HEAD, LED	EACH	2	•		2	
89502200	MODIFY EXISTING CONTROLLER	EACH	1			1	
89502300	REMOVE ELECTRIC CABLE FROM CONDUIT	FOOT	5362			665	4697
89502375	REMOVE EXISTING TRAFFIC SIGNAL EQUIPMENT	EACH	3	1	1	1	4037
89502376	REBUILD EXISTING HANDHOLE	EACH	1	-	*	1	
89502370	REMOVE EXISTING HANDHOLE	EACH	19	8	10	1	
89502385	REMOVE EXISTING CONCRETE FOUNDATION	EACH	20	9	10	1	
X0324085	EMERGENCY VEHICLE PRIORITY SYSTEM LINE SENSOR CABLE, NO. 20 3/C	FOOT	1362	798	564		
X7240311	RELOCATE EXISTING SIGN PANEL (SPECIAL)	EACH	6	3	3		
X8570226	FULL-ACTUATED CONTROLLER AND TYPE IV CABINET, SPECIAL	EACH	2	1	1		
X8620200	UNINTERRUPTABLE POWER SUPPLY, SPECIAL	EACH	2	1	1		
X8710031			4380	1	1		4200
	FIBER OPTIC CABLE 36 FIBERS, SINGLE MODE	FOOT					4380
Z0033056	OPTIMIZE TRAFFIC SIGNAL SYSTEM POST MOLINITED ELASHINIG REACON INSTALLATION (SPECIAL)	EACH	1	2	2		1
Z0033058	POST MOUNTED FLASHING BEACON INSTALLATION (SPECIAL)	EACH	4	2	2		
Z0073510	TEMPORARY TRAFFIC SIGNAL TIMING	EACH	21 *	1	1	1	

• NOTE: TOTAL INCLUDES SIGNALIZED INTERSECTIONS WITHIN DETOUR ROUTE.

FILE NAME =	DESIGNED - BC	REVISED - ADDENDUM /1\ 04/15/2013
\D163598-sht-ts-soq-01.dgn	DRAWN - TMB	REVISED -
USER NAME = toddblank	CHECKED - MPM	REVISED -
PLOT DATE = 4/15/2013	DATE - 01/18/2013	REVISED -



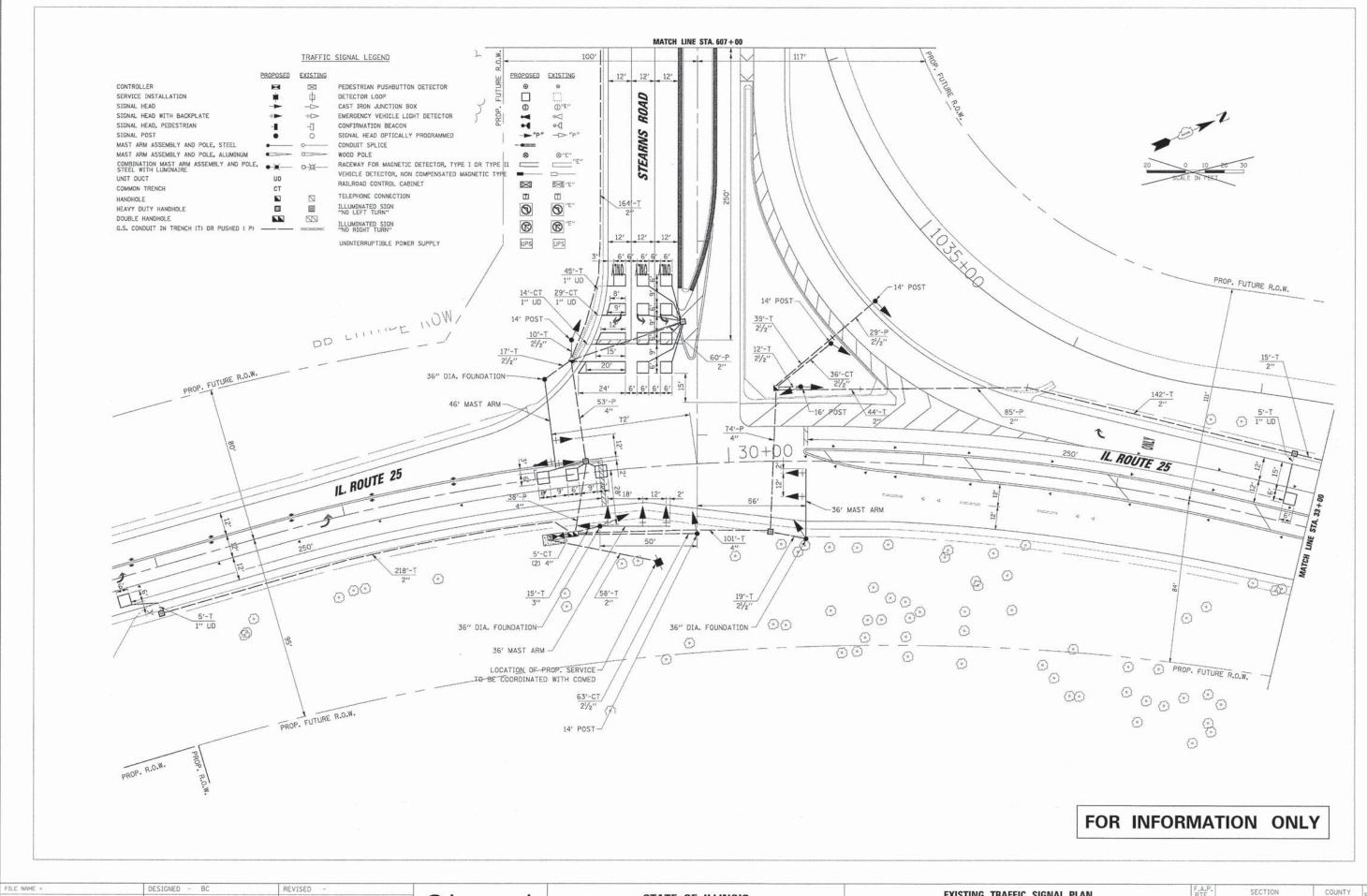
STATE OF ILLINOIS DIVISION OF TRANSPORTATION

			SUN	IM	AR	Y 0	FTRAF	FIC SIGNA	L	
						QU.	ANTITIE	S		
CALE:	NTS	SHEET	NO.	1	OF	49	SHEETS	STA.	TO	STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP	KANE	451	212
7.6		CONTRACT	NO. 6	3598
	ILLINOIS FED. A	AID PROJECT		

TRAFFIC SIGNAL LEGEND

	<u> </u>										
ITEM	REMOVAL	EXISTING	PROPOSED	ITEM	REMOVAL	EXISTING	PROPOSED	ITEM	REMOVAL	EXISTING	PROPOSED
CONTROLLER CABINET	R	\bowtie	\blacksquare	EMERGENCY VEHICLE LIGHT DETECTOR	R	\bowtie	-	ELECTRIC CABLE IN CONDUIT, TRACER, NO. 14 1/C, UNLESS NOTED OTHERWISE			
RAILROAD CONTROL CABINET		RX R	B F	CONFIRMATION BEACON	R_{o-0}	0—0	•4			-/	
COMMUNICATIONS CABINET	C C	ECC	CC	HANDHOLE	R			COAXIAL CABLE		—©—	—©—
MASTER CONTROLLER		EMC	MC	THOUSE			-	VENDOR CABLE FOR CAMERA			
MASTER MASTER CONTROLLER	R	[EMMC]	MMC	HEAVY DUTY HANDHOLE	RH	H	H				
UNINTERRUPTIBLE POWER SUPPLY	UPS	EUPS	UPS	DOUBLE HANDHOLE	R 📵		0	COPPER INTERCONNECT CABLE, NO. 18 3 PAIR TWISTED, SHIELDED		<u> </u>	-6-
SERVICE INSTALLATION, (P) POLE OR (G) GROUND MOUNT	-D-R	-D ^P	- ■ P	JUNCTION BOX GALVANIZED STEEL CONDUIT		(D)	e e	FIBER OPTIC CABLE NO. 62.5/125, MM12F		—(12F)—	
TELEPHONE CONNECTION (P) POLE OR (G) GROUND MOUNT	R	P	P	IN TRENCH (T) OR PUSHED (P) TEMPORARY SPAN WIRE, TETHER WIRE,		-		FIBER OPTIC CABLE		—(24F)—	—(24F)—
STEEL MAST ARM ASSEMBLY AND POLE	RO	0	•	AND CABLE	-К	-		NO. 62.5/125, MM12F SM12F		7	
ALUMINUM MAST ARM ASSEMBLY AND POLE	R	0		COMMON TRENCH			СТ	FIBER OPTIC CABLE NO. 62.5/125, (NUMBER OF FIBERS & TYPE TO BE		- Ø-	-
STEEL COMBINATION MAST ARM ASSEMBLY AND POLE WITH LUMINAIRE	R _{O-)Q}	0-×	• ×	COILABLE NONMETALLIC CONDUIT (EMPTY)			CNC	NOTED ON PLANS)			
STEEL COMBINATION MAST ARM	R	PIZI	PIZ	SYSTEM ITEM INTERSECTION ITEM		S	S IP	GROUND ROD AT (C) CONTROLLER, (H) HANDHOLE, (P) POST, (M) MAST ARM, OR (S) SERVICE		c _d —•	c _{il}
ASSEMBLY AND POLE WITH PTZ CAMERA	PED .	11 200 400		REMOVE ITEM	R		7.20	CONTROLLER CABINET AND	RCF		
SIGNAL POST TEMPORARY WOOD POLE (CLASS 5 OR	R O R_	⊗	•	RELOCATE ITEM	RL			FOUNDATION TO BE REMOVED	\boxtimes		
BETTER) 45 FOOT (13.7m) MINIMUM	$\stackrel{R}{\otimes}$	0	•	ABANDON ITEM	A			STEEL MAST ARM POLE AND FOUNDATION TO BE REMOVED	ORMF		
GUY WIRE	>R	>	>-	12" (300mm) TRAFFIC SIGNAL SECTION		R	R	ALUMINUM MAST ARM POLE AND	RMF		
SIGNAL HEAD	R →	>	-	12" (300mm) RED WITH 8" (200mm)		R		FOUNDATION TO BE REMOVED	0		
SIGNAL HEAD CONSTRUCTION STAGES (NUMBERS INDICATE THE CONSTRUCTION STAGE)			2	YELLOW AND GREEN TRAFFIC SIGNAL FACE		<u> </u>		STEEL COMBINATION MAST ARM ASSEMBLY AND POLE WITH LUMINAIRE AND	RMF O-X		
SIGNAL HEAD WITH BACKPLATE	+t⊳ ^R	+->	+-			R	R	FOUNDATION TO BE REMOVED			
SIGNAL HEAD OPTICALLY PROGRAMMED	_R →"P"	—□>"p"	→ "P"	SIGNAL FACE		Q	G ∢ Y	SIGNAL POST AND FOUNDATION TO BE REMOVED	RMF		
FLASHER INSTALLATION (S DENOTES SOLAR POWER)	O-C>"F"	0-0"F"	• ► "F"			• • •	∢ ∀	INTERSECTION & SAMPLING (SYSTEM) DETECTOR		[15]	IS
PEDESTRIAN SIGNAL HEAD	P	-0	-1			R	R	SAMPLING (SYSTEM) DETECTOR		$[\underline{s}]$	S
PEDESTRIAN PUSHBUTTON DETECTOR	R.	©	•	SIGNAL FACE WITH BACKPLATE, "P" INDICATES PROGRAMMED HEAD			G	EXISTING INTERSECTION LOOP DETECTOR PROPOSED INTERSECTION AND SAMPLING (SYSTEM) DETECT	ron	[P]	
ACCESSIBLE PEDESTRIAN PUSHBUTTON DETECTOR	R APS	@APS	APS			◆∀	← Y ← G	EXISTING PREFORMED INTERSECTION LOOP DETECTOR		[PP]	
ILLUMINATED SIGN "NO LEFT TURN"	R	(3)	9	12" (300mm) PEDESTRIAN SIGNAL HEAD		(SW)	"P"	PROPOSED INTERSECTION AND SAMPLING (SYSTEM) DETECT PREFORMED INTERSECTION AND SAMPLING	TOR		1 1
ILLUMINATED SIGN "NO RIGHT TURN"	R (S)	8	®	WALK/DON'T WALK SYMBOL 12" (300mm) PEDESTRIAN SIGNAL HEAD		(7)		(SYSTEM) DETECTOR		PIS Incl	PIS
DETECTOR LOOP, TYPE I	Nonethinkel C	[7]		INTERNATIONAL SYMBOL, OUTLINED				PREFORMED SAMPLING (SYSTEM) DETECTOR		ÎPSÎ	PS
PREFORMED DETECTOR LOOP		P.	P	12" (300mm) PEDESTRIAN SIGNAL HEAD INTERNATIONAL SYMBOL, SOLID		(*	RAILROAD	SYMBO	LS	
MICROWAVE VEHICLE SENSOR	R Mb	MD	 .	PEDESTRIAN SIGNAL HEAD, INTERNATIONAL SYMBOL, WITH COUNTDOWN TIMER		O C	₽ C			EXISTING	PROPOSED
VIDEO DETECTION CAMERA	R	₩ (V)	(V)	RADIO INTERCONNECT	- + ^R -0	###0	-•	RAILROAD CONTROL CABINET		B B	₽ ◀
VIDEO DETECTION ZONE					120	A		RAILROAD CANTILEVER MAST ARM	X	OX X	IOI X X
	R			RADIO REPEATER	RERR	ERR	RR	FLASHING SIGNAL	dash.	XOX	X-X
PAN, TILT, ZOOM CAMERA	erzh R			DENOTES NUMBER OF CONDUCTORS, ELECTRIC CABLE NO. 14, UNLESS NOTED OTHERWISE, ALL DETECTOR LOOP CABLE TO BE SHIELDED				CROSSING GATE		X0X>	X-X-
WIRELESS DETECTOR SENSOR	R R	W	W	GROUND CABLE IN CONDUIT		\prec		CROSSBUCK		<u>></u> ≺	*
WIRELESS ACCESS POINT				NO. 6 SOLID COPPER (GREEN)		Œ	(1)		16.751		
FILE NAME = USER NAME = bouerd1 oi\pw_work\PWIOOT\8AUERDL\d8108315\tx85dgn	DI	ESIGNED - DAG/BCK RAWN - BCK	REVISED -		E OF ILLINOIS			DISTRICT ONE	F.A.P. RTE.	SECTION 06-00214-18-RP	COUNTY TOTAL SHE SHEETS NO KANE 451 21
PLOT SCALE = 50.0000 '/ PLOT DATE = 11/4/2009		HECKED - DAD ATE - 10-28-09	REVISED -	DEPARTMENT	OF TRANSPO	DRTATION	SCALE: NO	STANDARD TRAFFIC SIGNAL DESIGN DETAILS NE SHEET NO. 6 OF 6 SHEETS STA. TO STA.		TS-05 DIST. NO. 1 ILLINOIS FED	CONTRACT NO. 6359



FILE NAME =	DESIGNED - BC	REVISED -	
\D163598-sht-IL25-stns-ex-ts-plan1.dgn	DRAWN - TMB	REVISED -	4
USER NAME = tblank	CHECKED - MPM	REVISED -	
PLOT DATE = 1/18/2013	DATE - 01/1	8/2013 REVISED -	

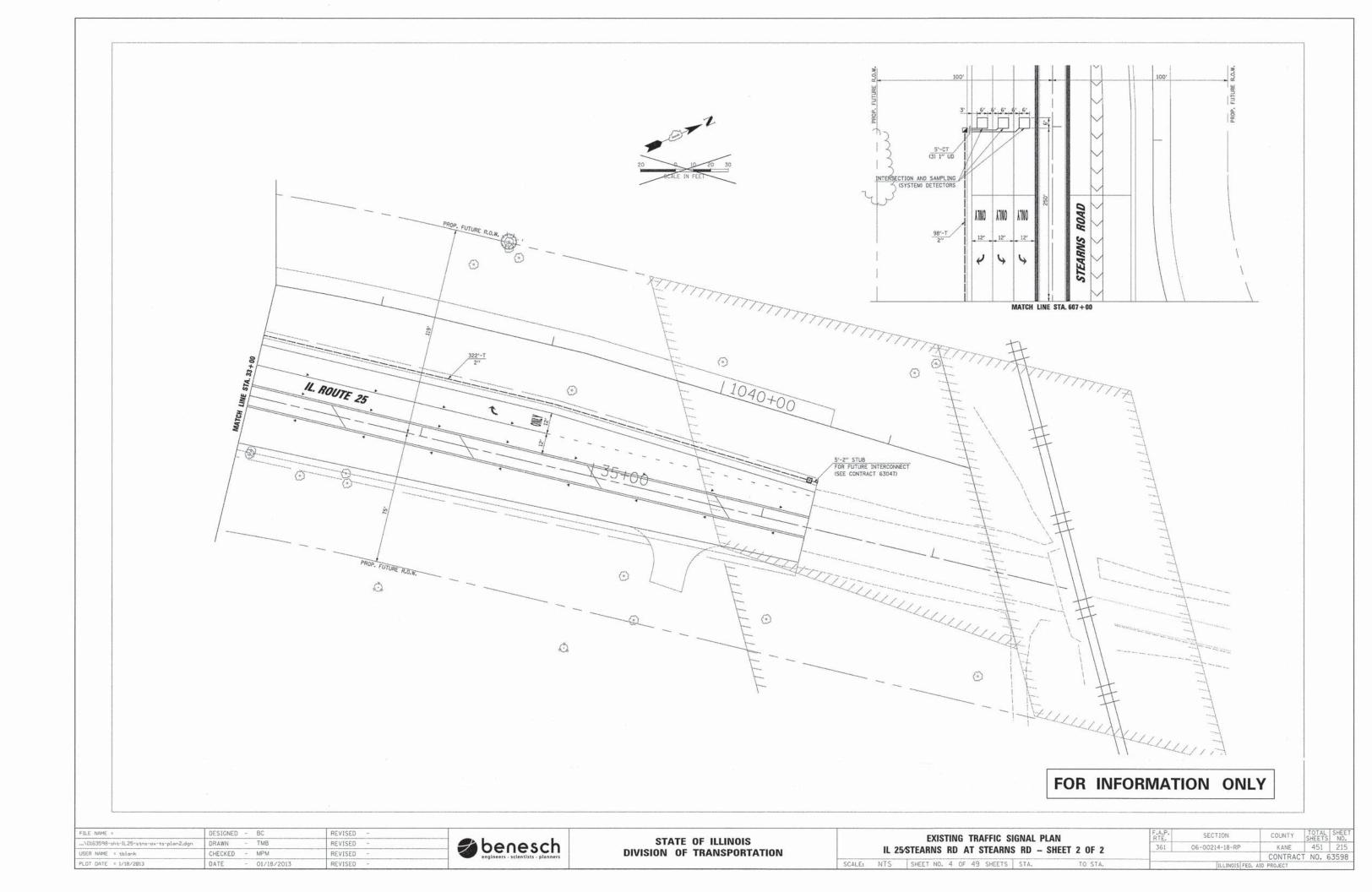


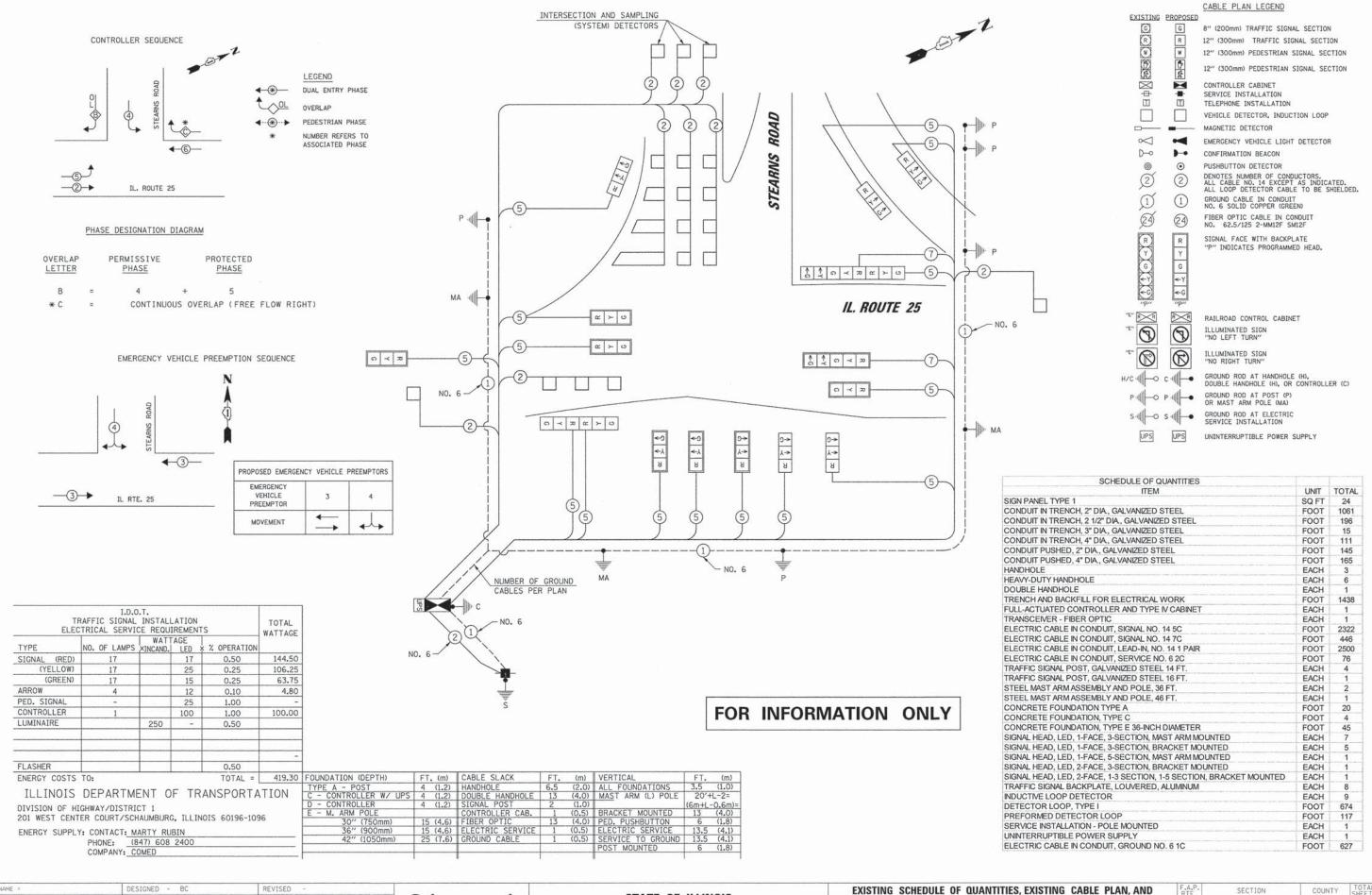
STATE OF ILLINOIS
DIVISION OF TRANSPORTATION

EXISTING TRAFFIC SIGNAL PLAN

IL 25/STEARNS RD AT STEARNS RD - SHEET 1 OF 2

SCALE: NTS SHEET NO. 3 OF 49 SHEETS STA. TO STA.





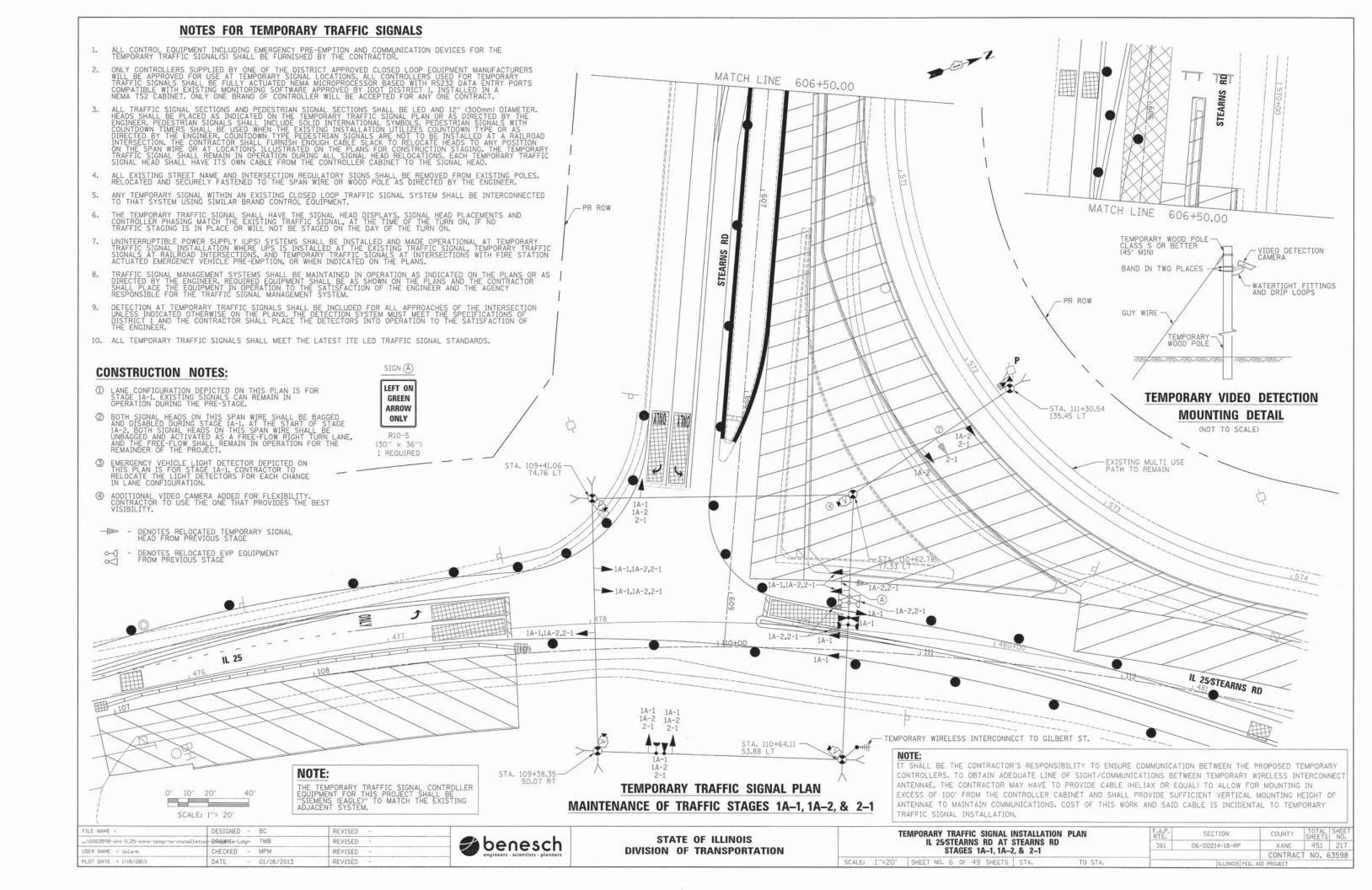
FILE NAME =	DESIGNED	-	BC	REVISED -	
\D163598-sht-1L25-stns-ex-ts-cable.dgn	DRAWN	-	TMB	REVISED -	4
USER NAME = tblonk	CHECKED	-	MPM	REVISED -	
PLOT DATE = 1/18/2013	DATE	-	01/18/2013	REVISED -	

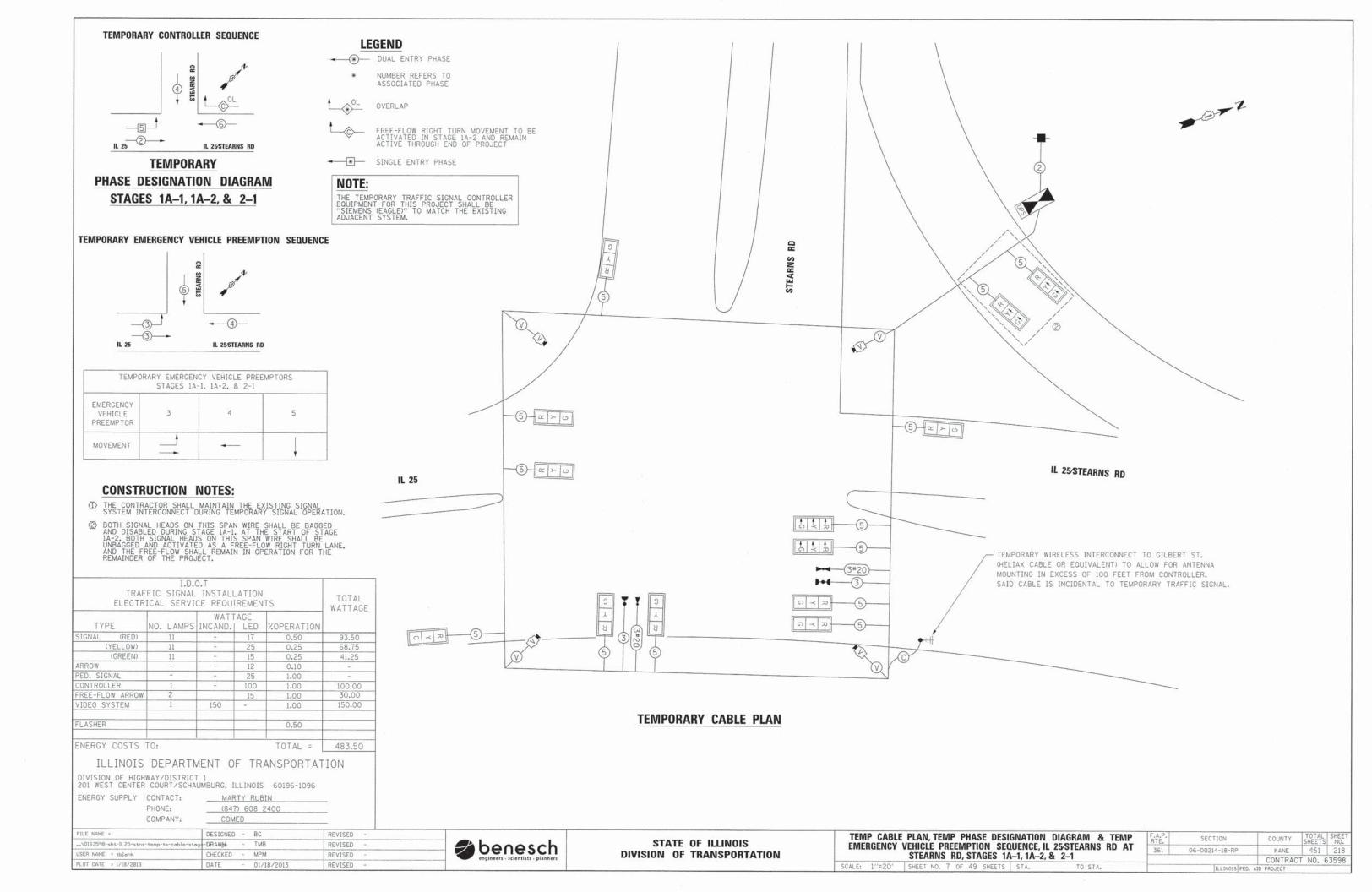


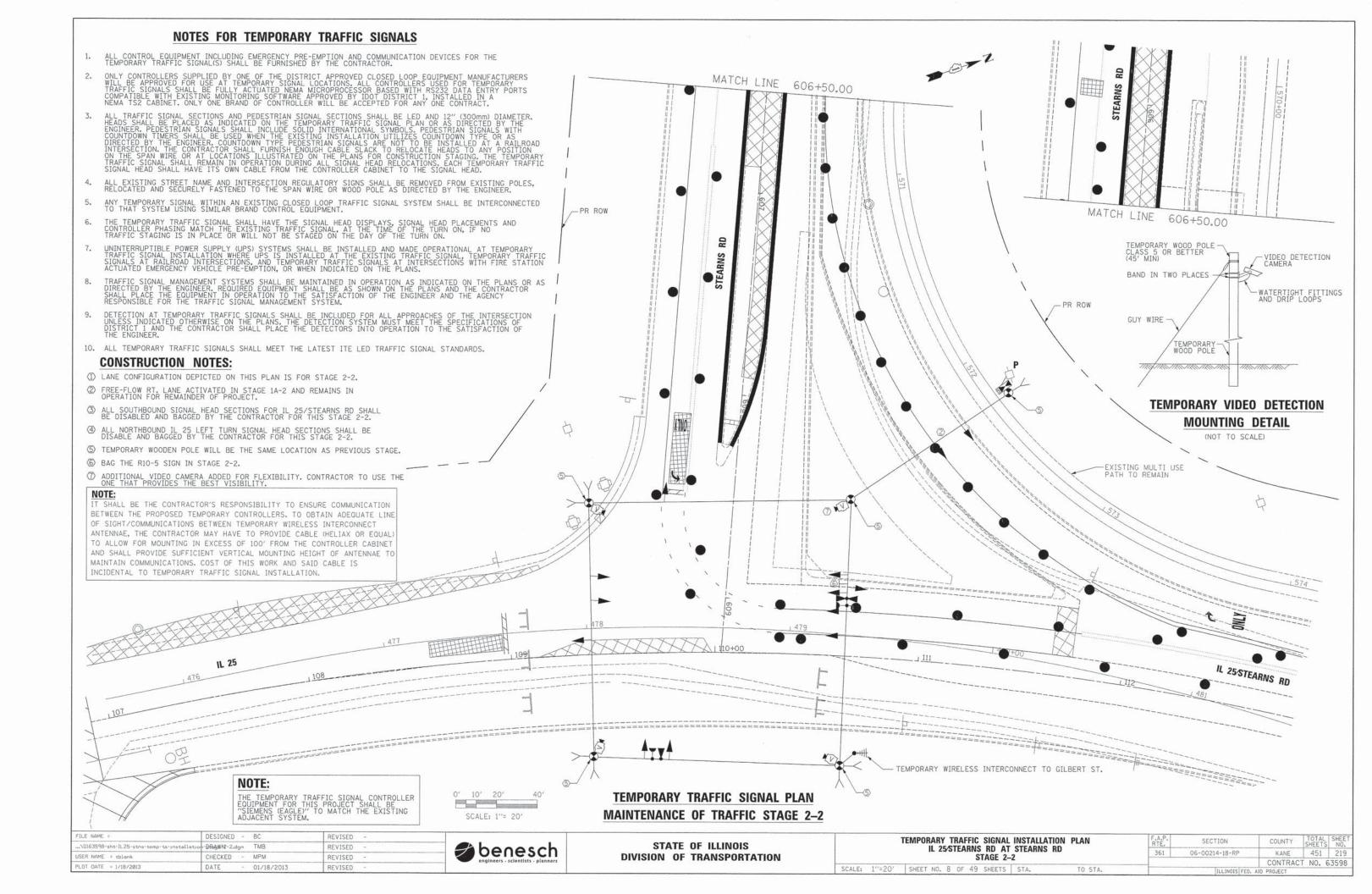
STATE OF ILLINOIS
DIVISION OF TRANSPORTATION

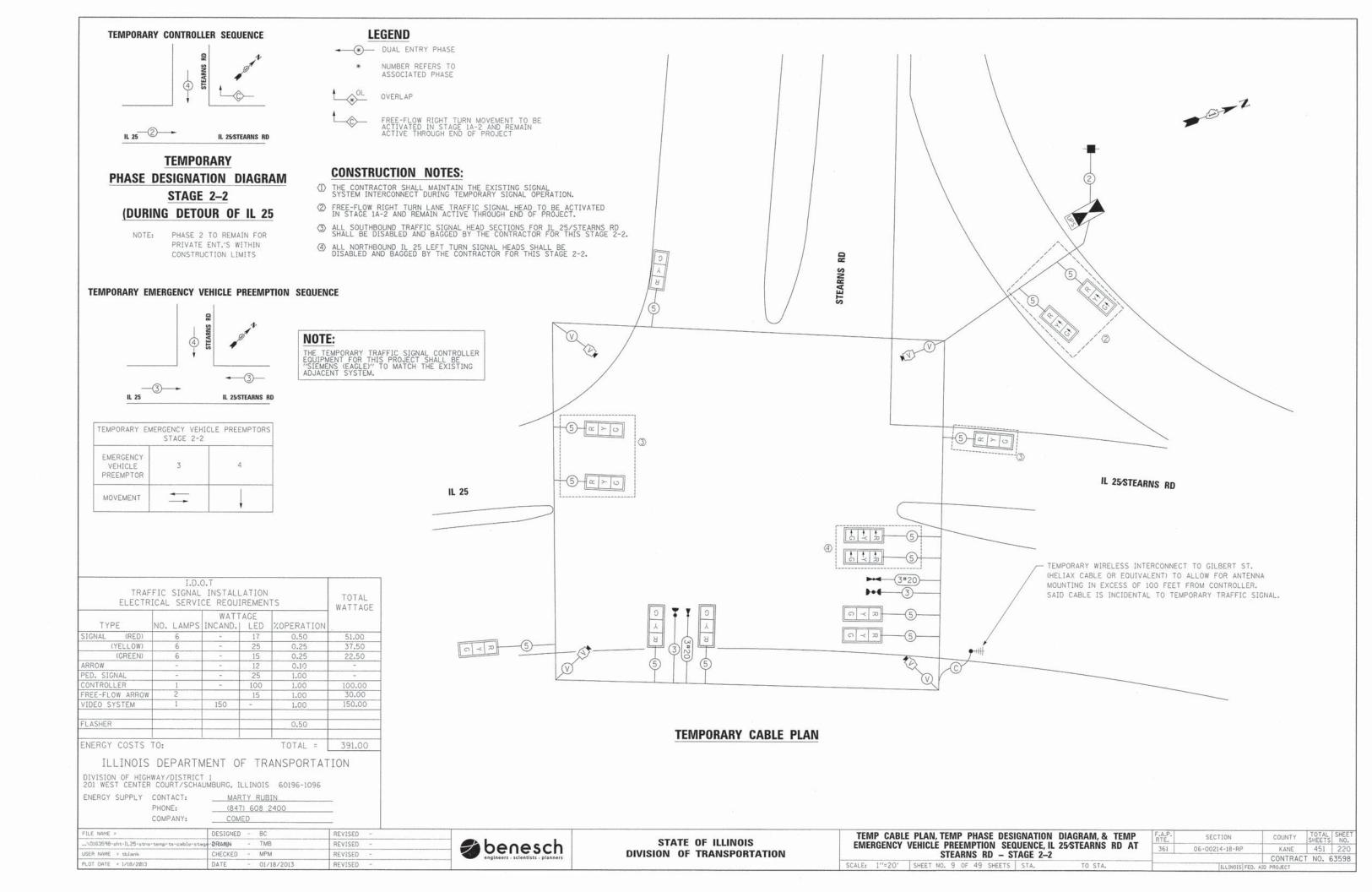
	EXISTING	EXISTIN	IG PH	IASE	DESIGN	EXISTING (ATION DIAC STEARNS		
SCAL	LE: 1"=20"	SHEET N	10. 5	OF 49	SHEETS	STA.	TO STA.	

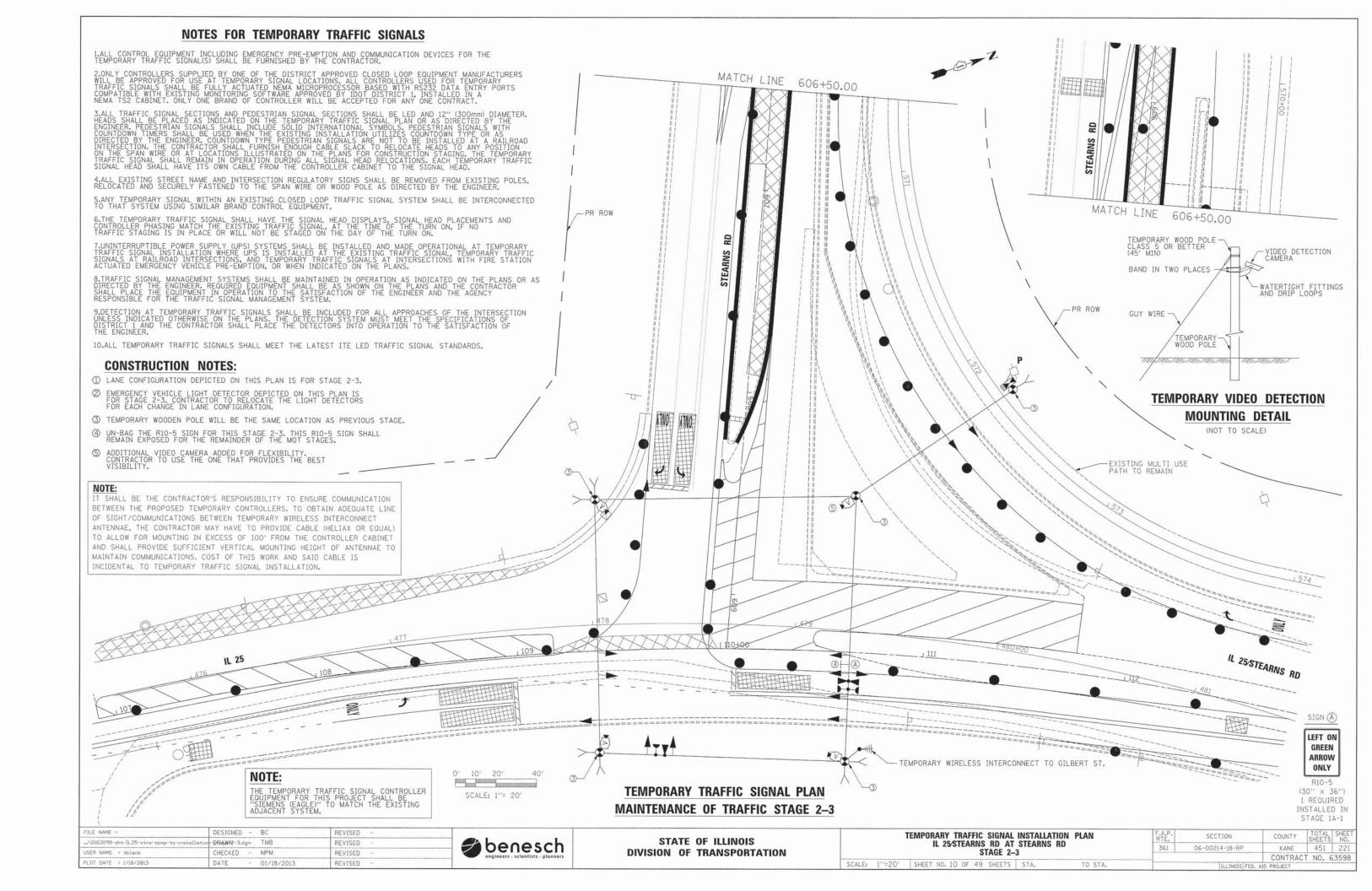
F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEE NO.
361	06-00214-18-RP	KANE	451	216
		CONTRAC	T NO. 6	3598
	ILLINOIS FED.	AID PROJECT		

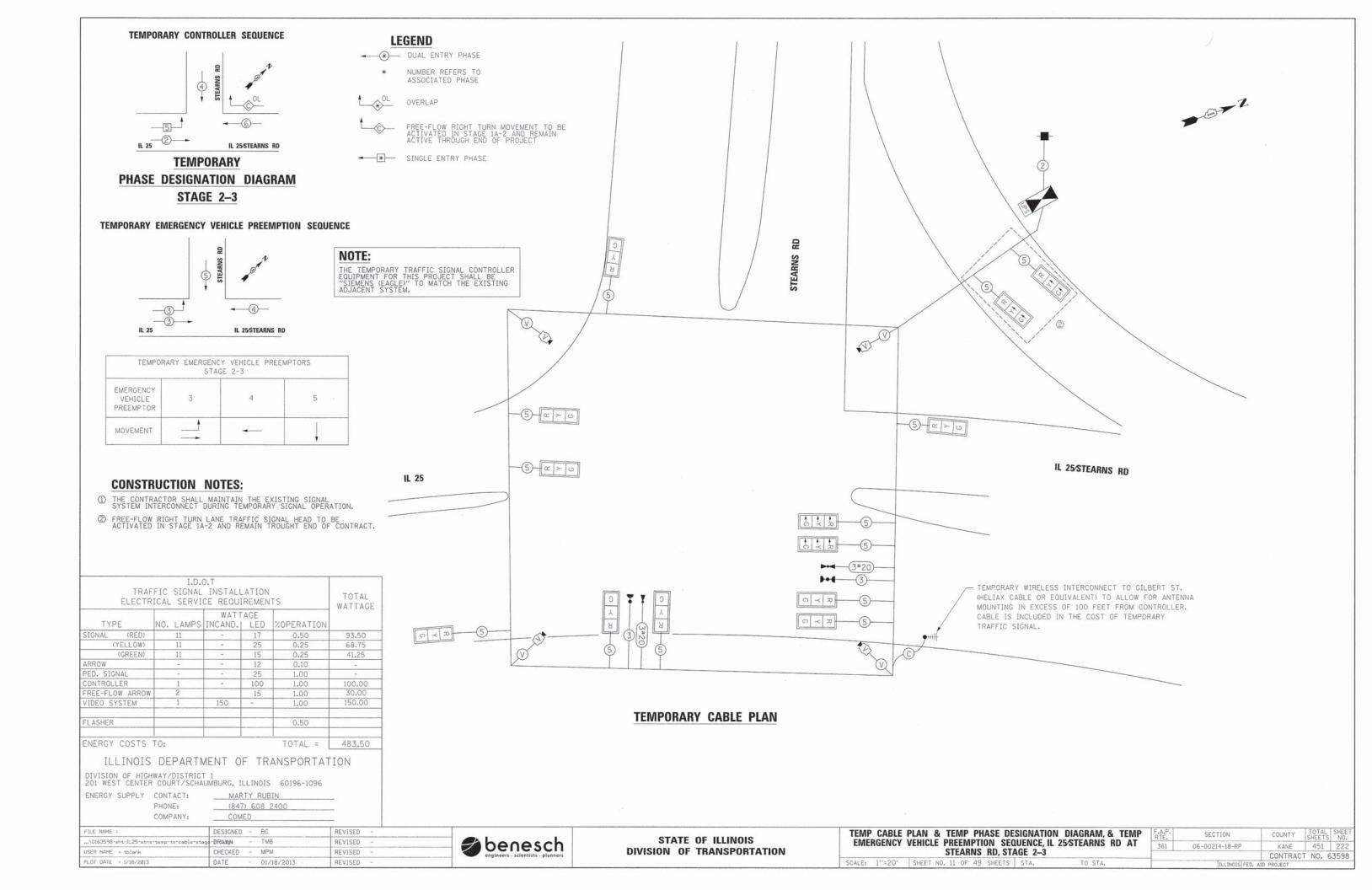


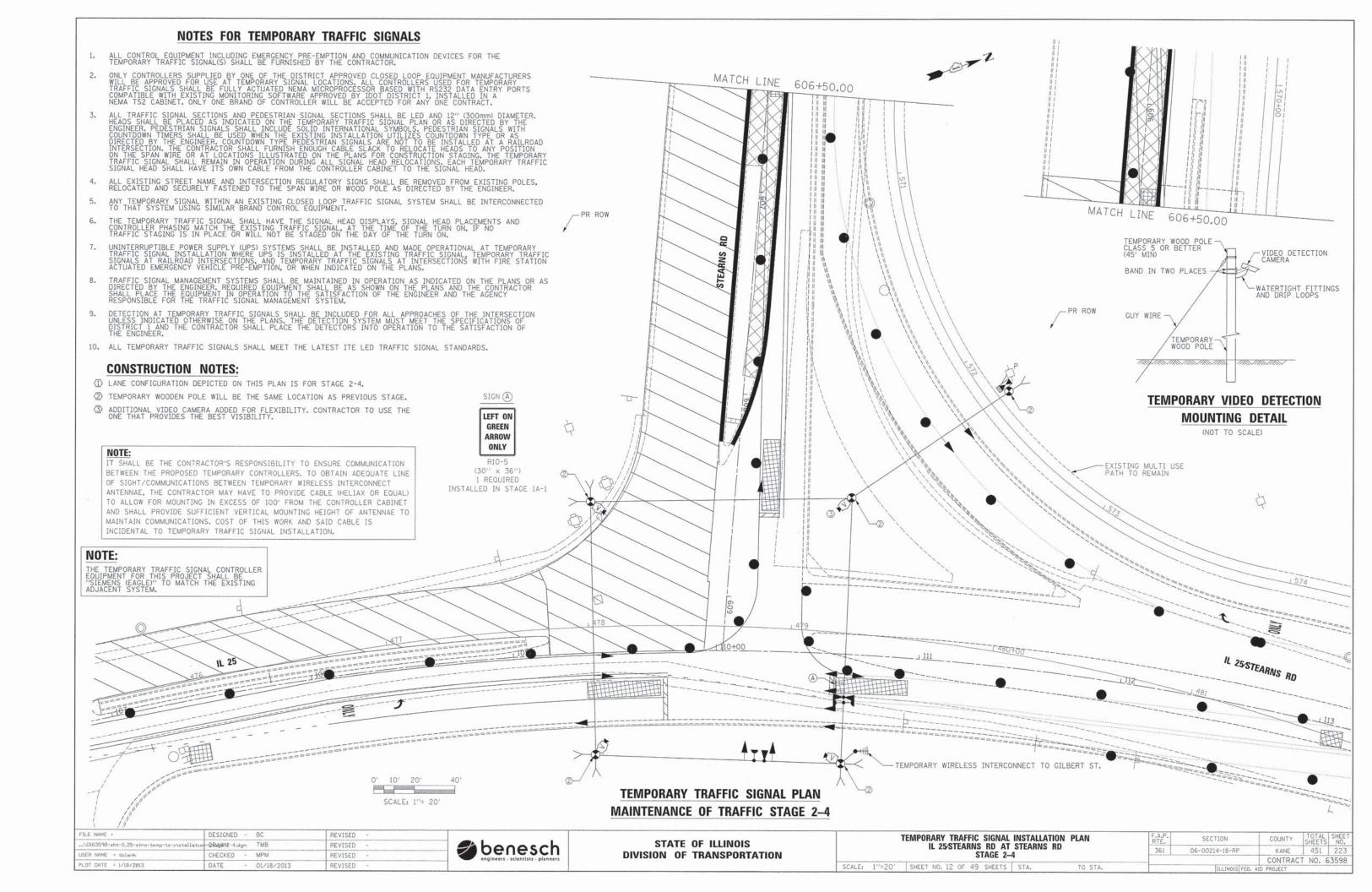


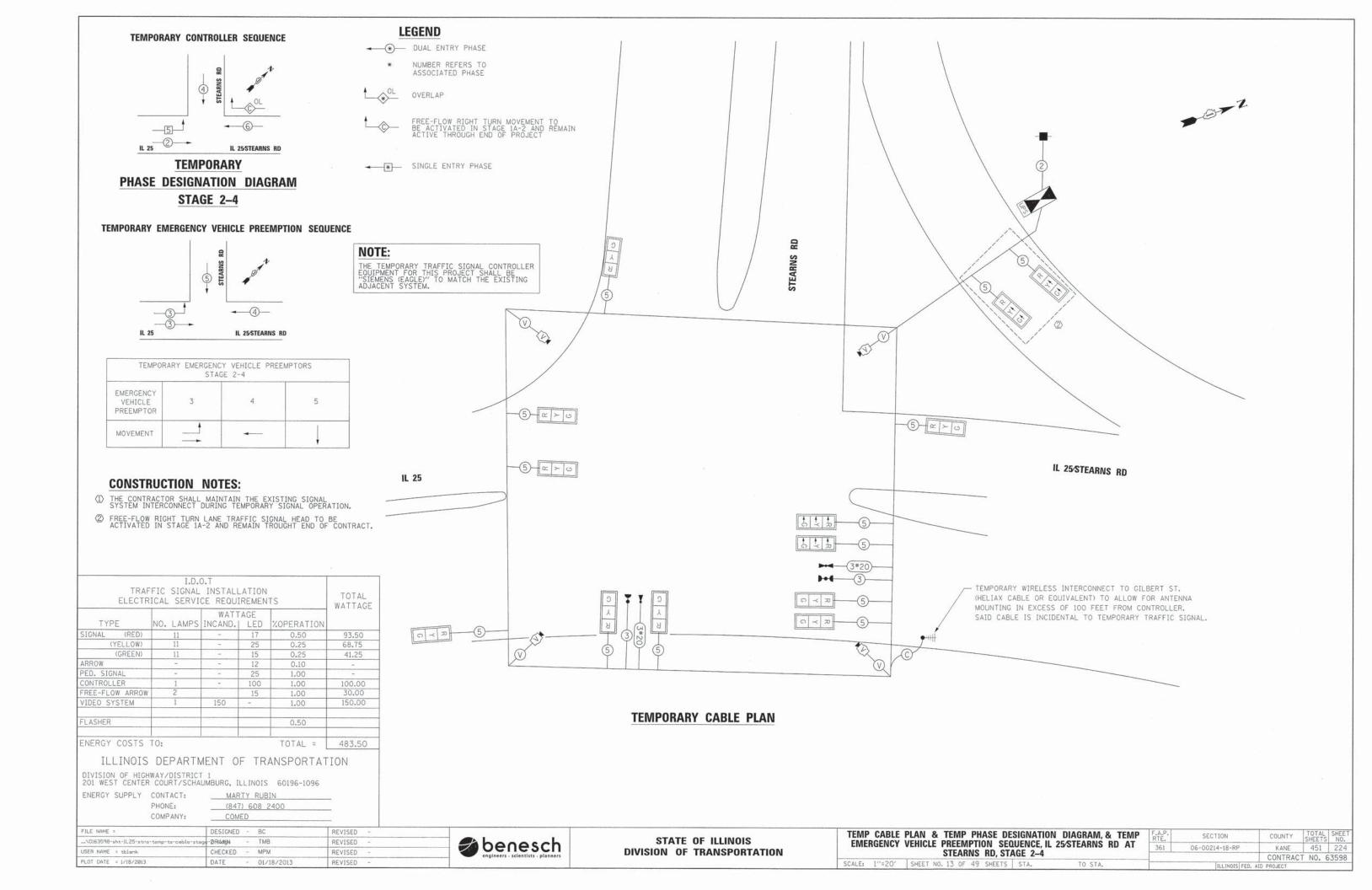


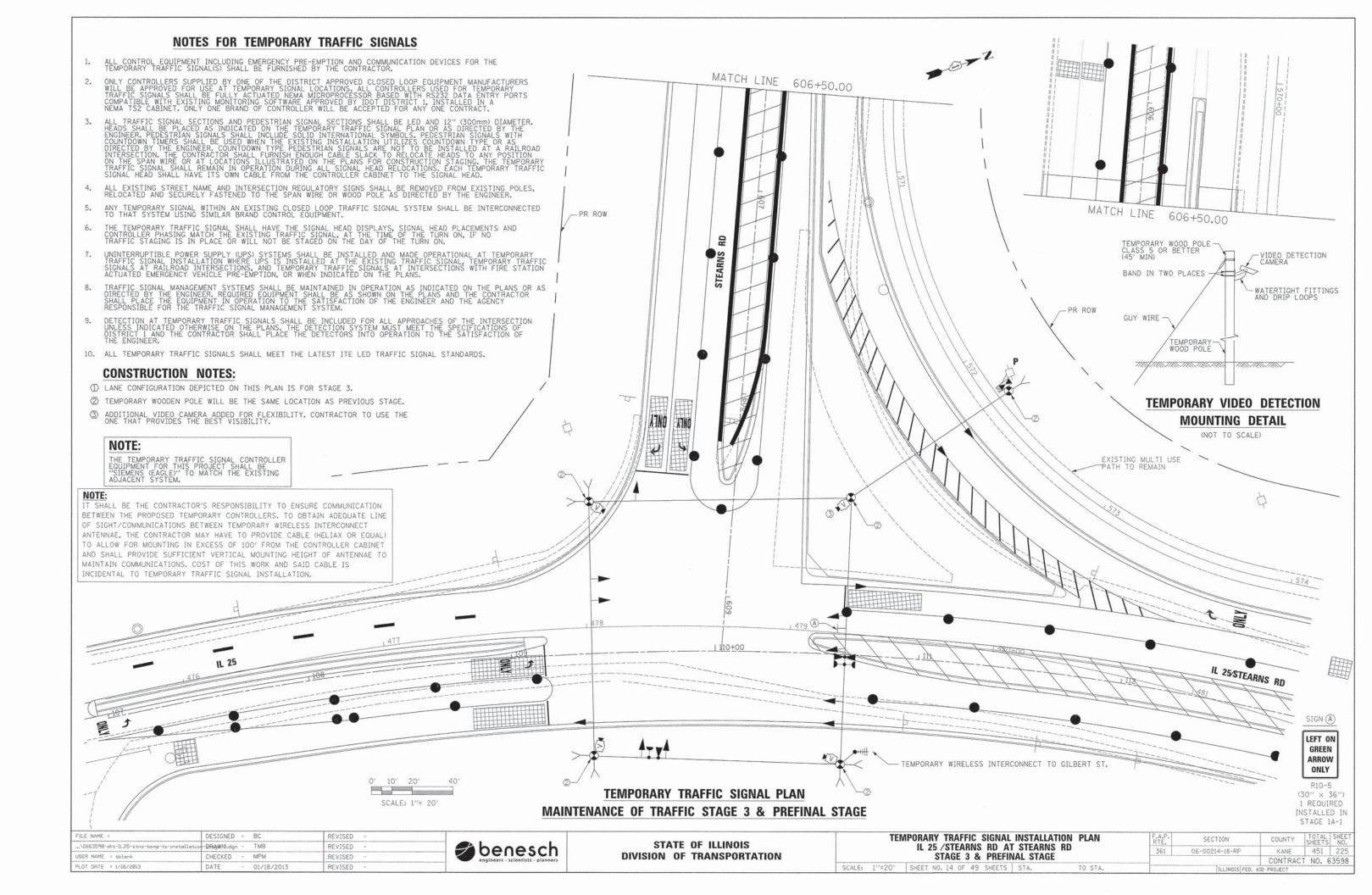


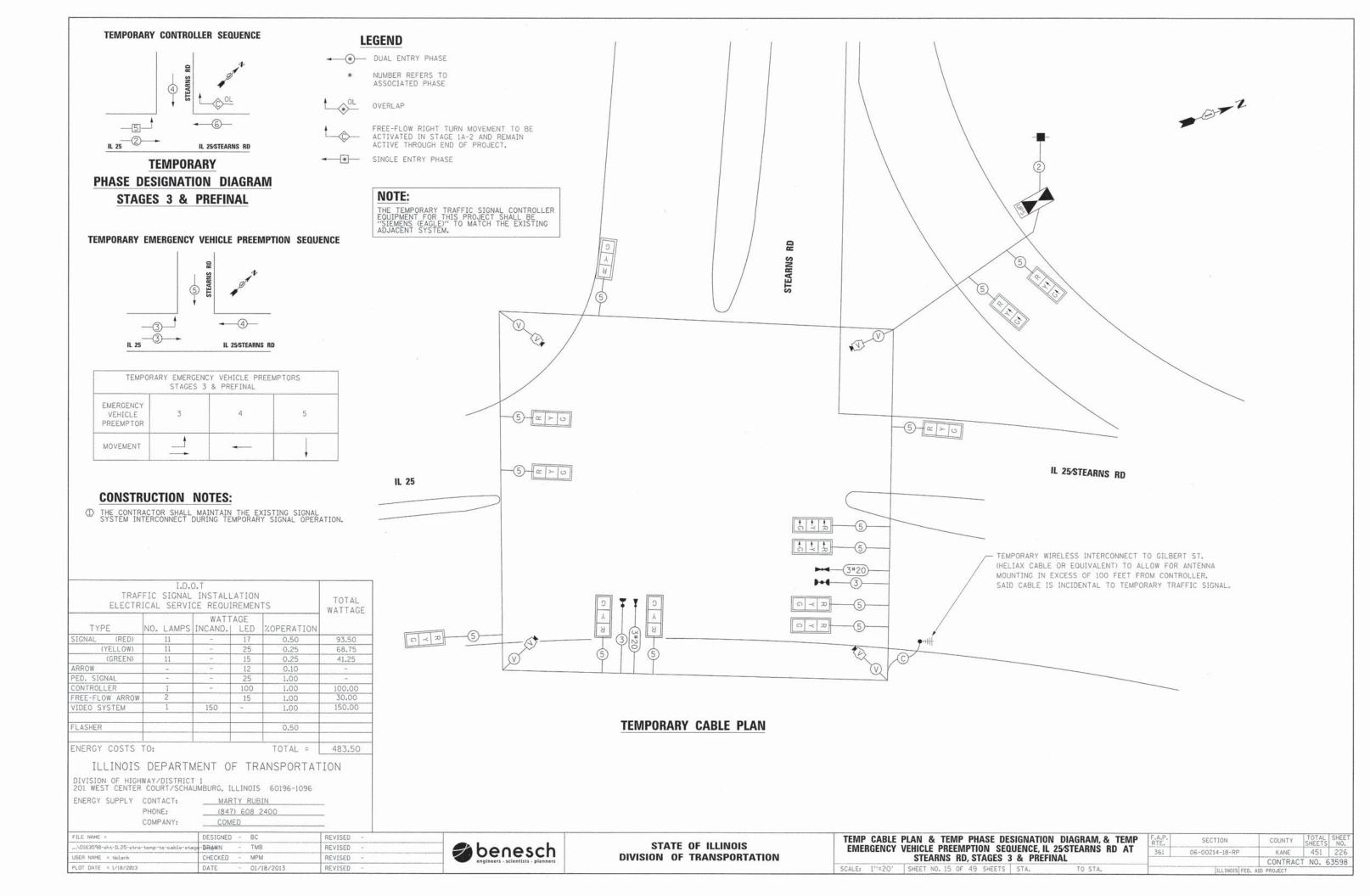


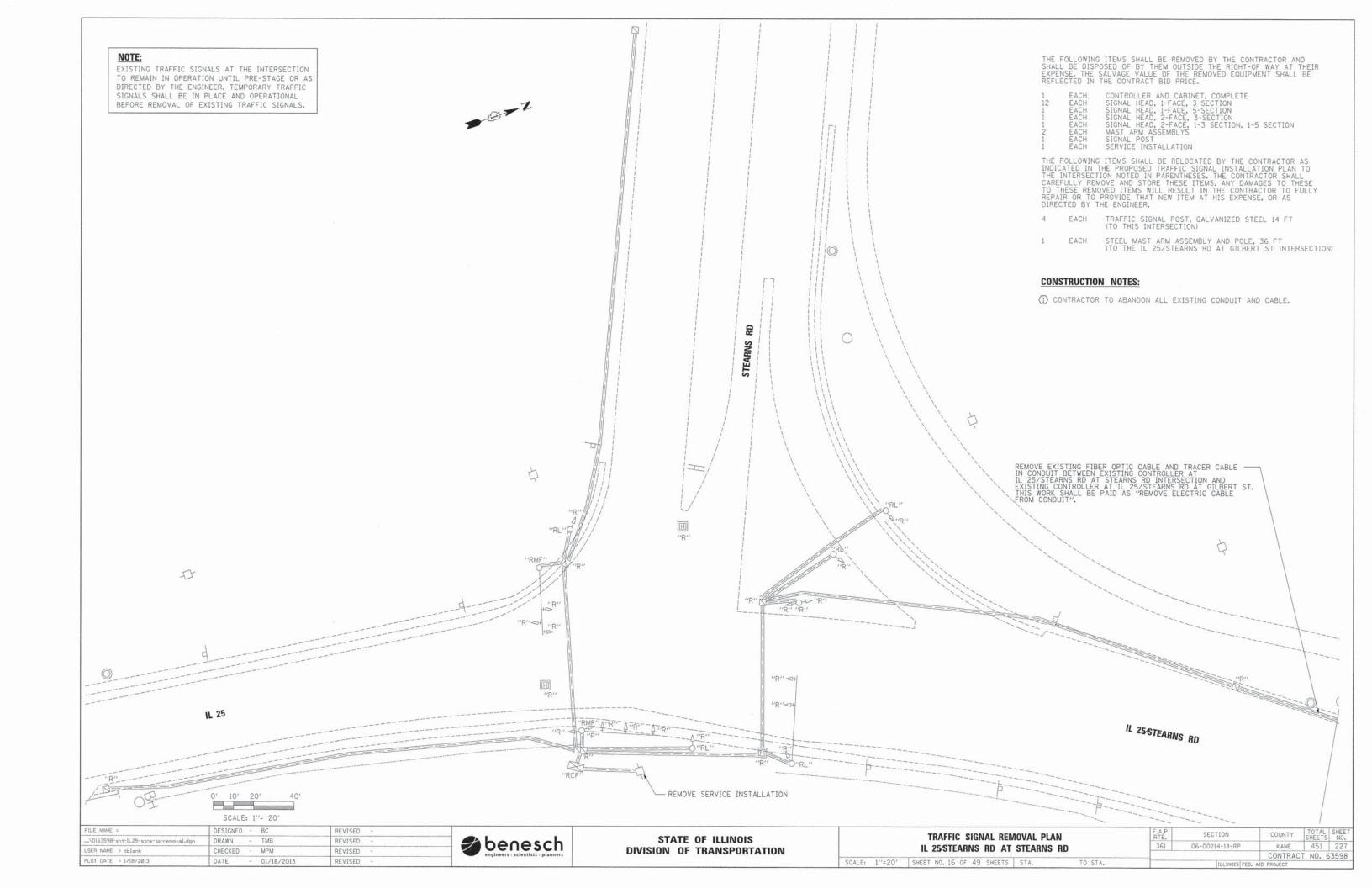


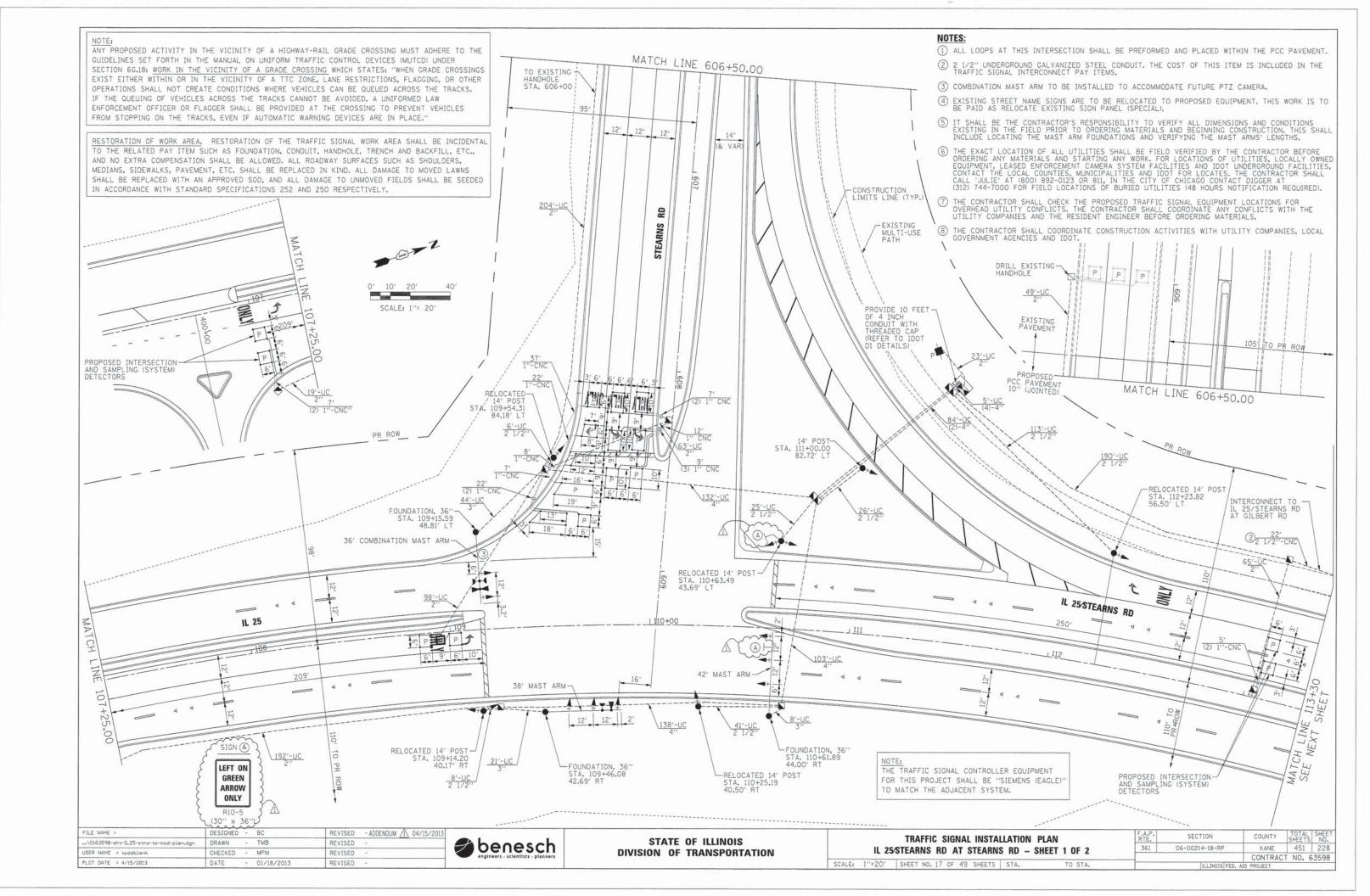


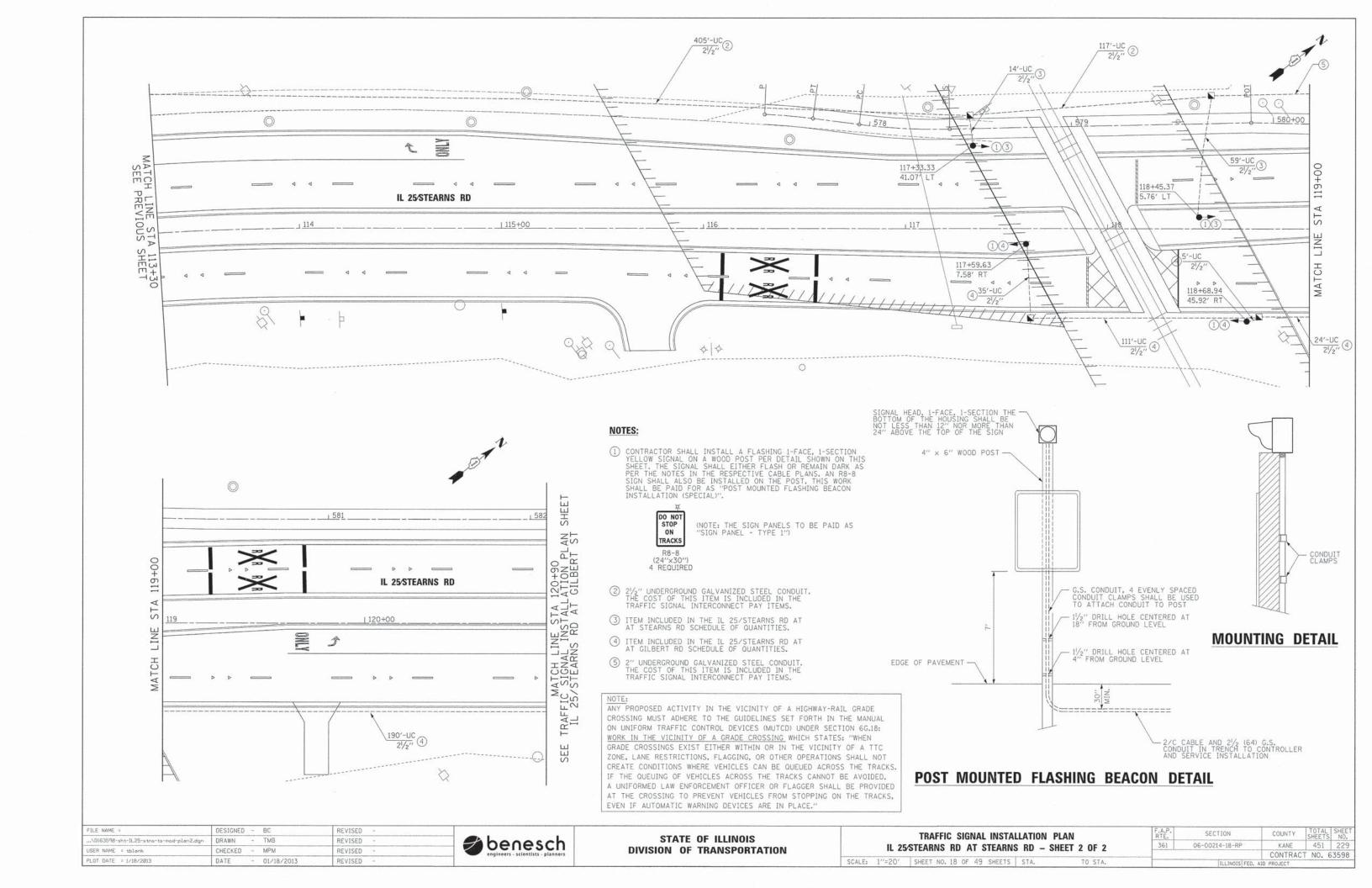


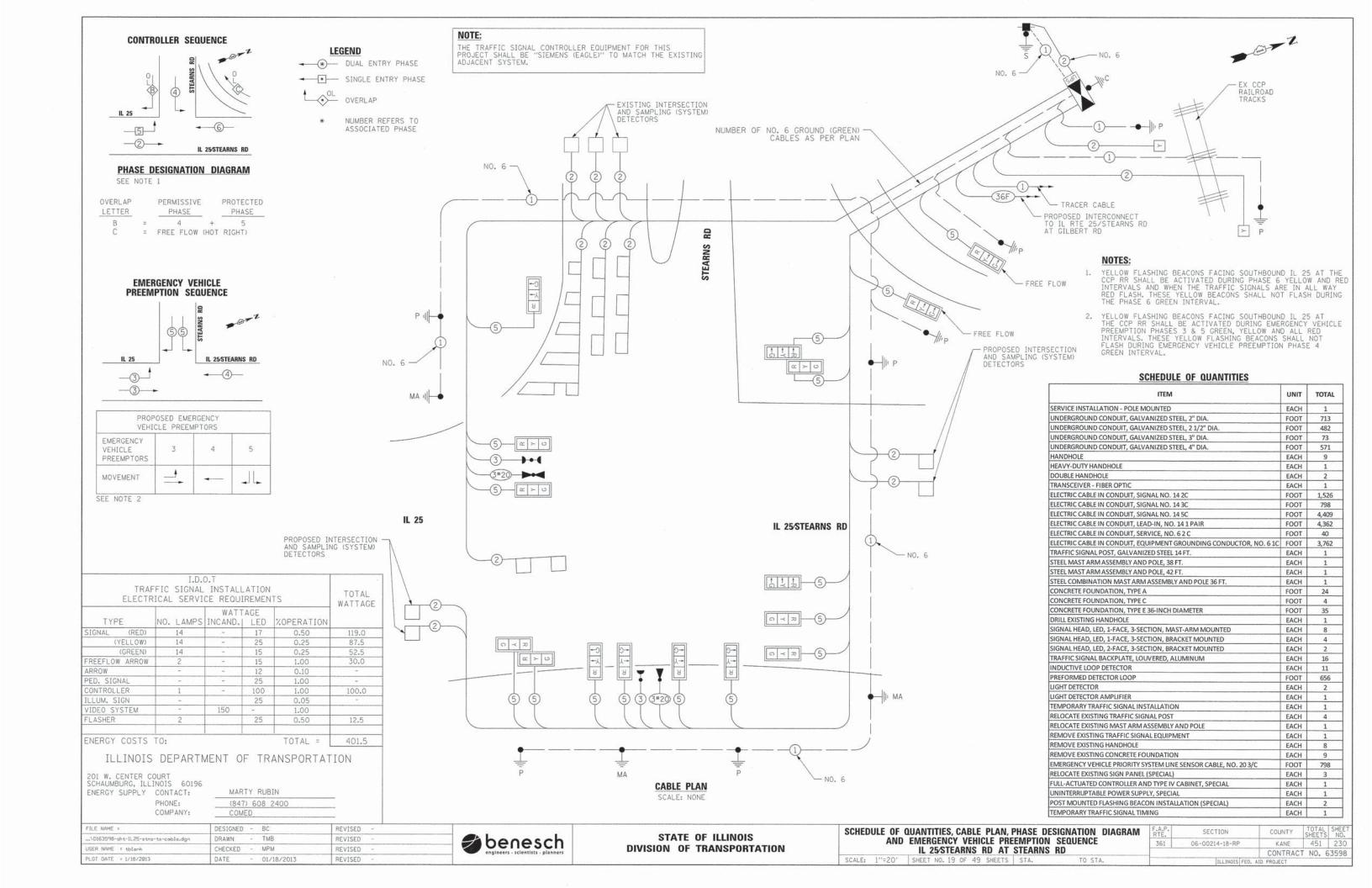


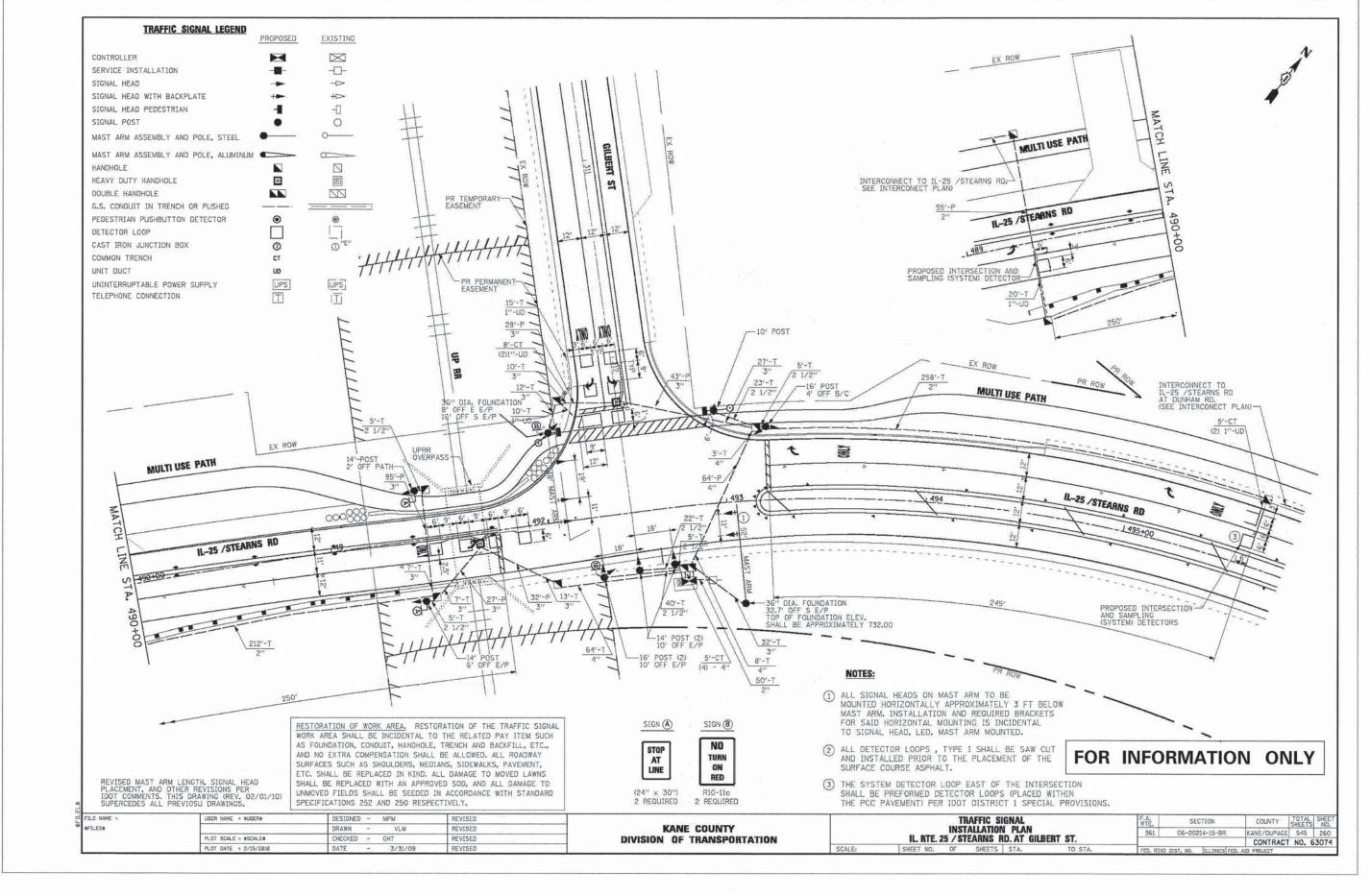












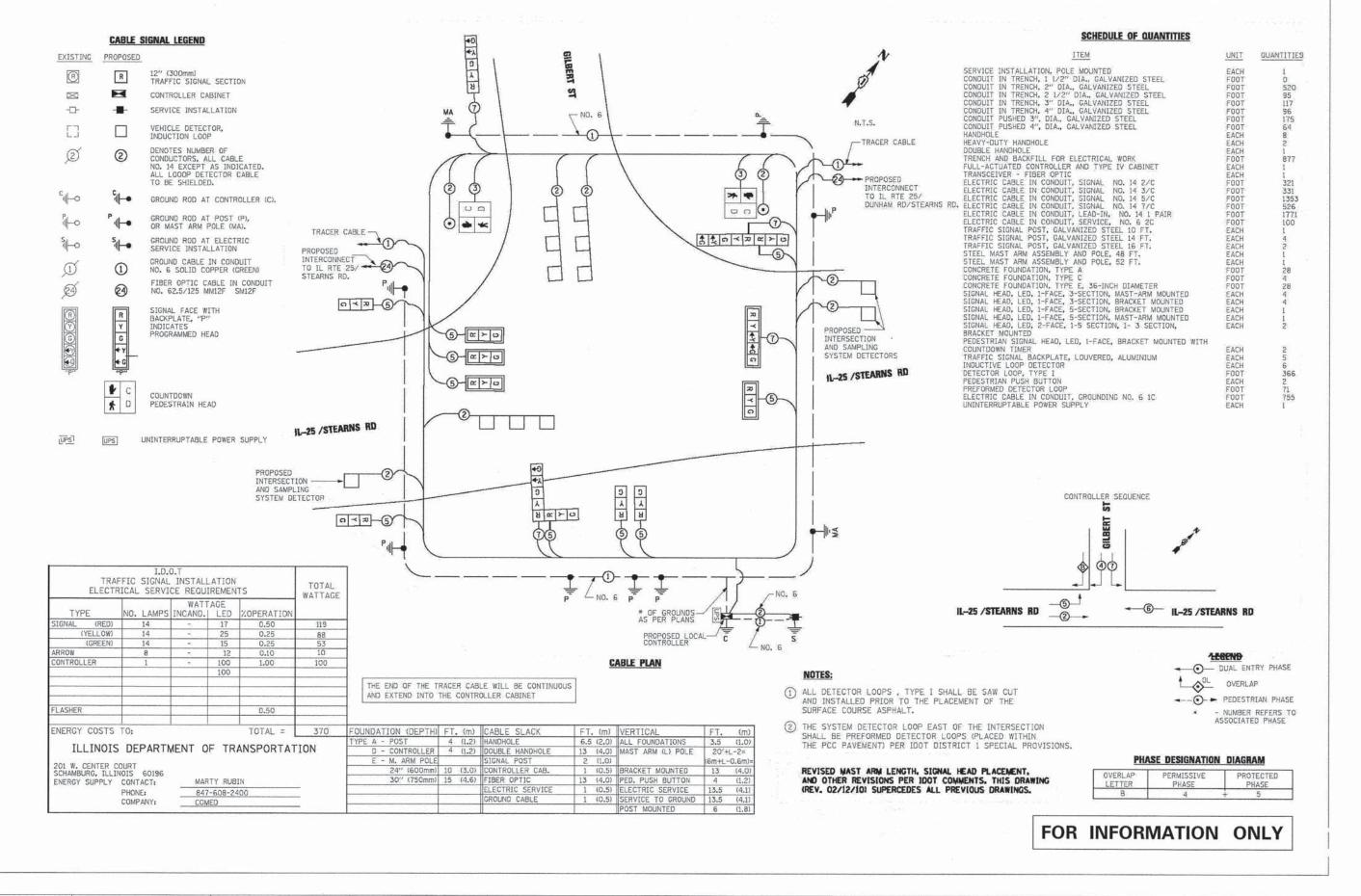
FILE NAME =	DESIGNED - BC	REVISED -
\D163598-sht-IL25-G:l-ex-ts-plan.dgn	DRAWN - TMB	REVISED -
USER NAME = tblank	CHECKED - MPM	REVISED -
PLOT DATE = 1/18/2013	DATE - 01/18/2013	REVISED -



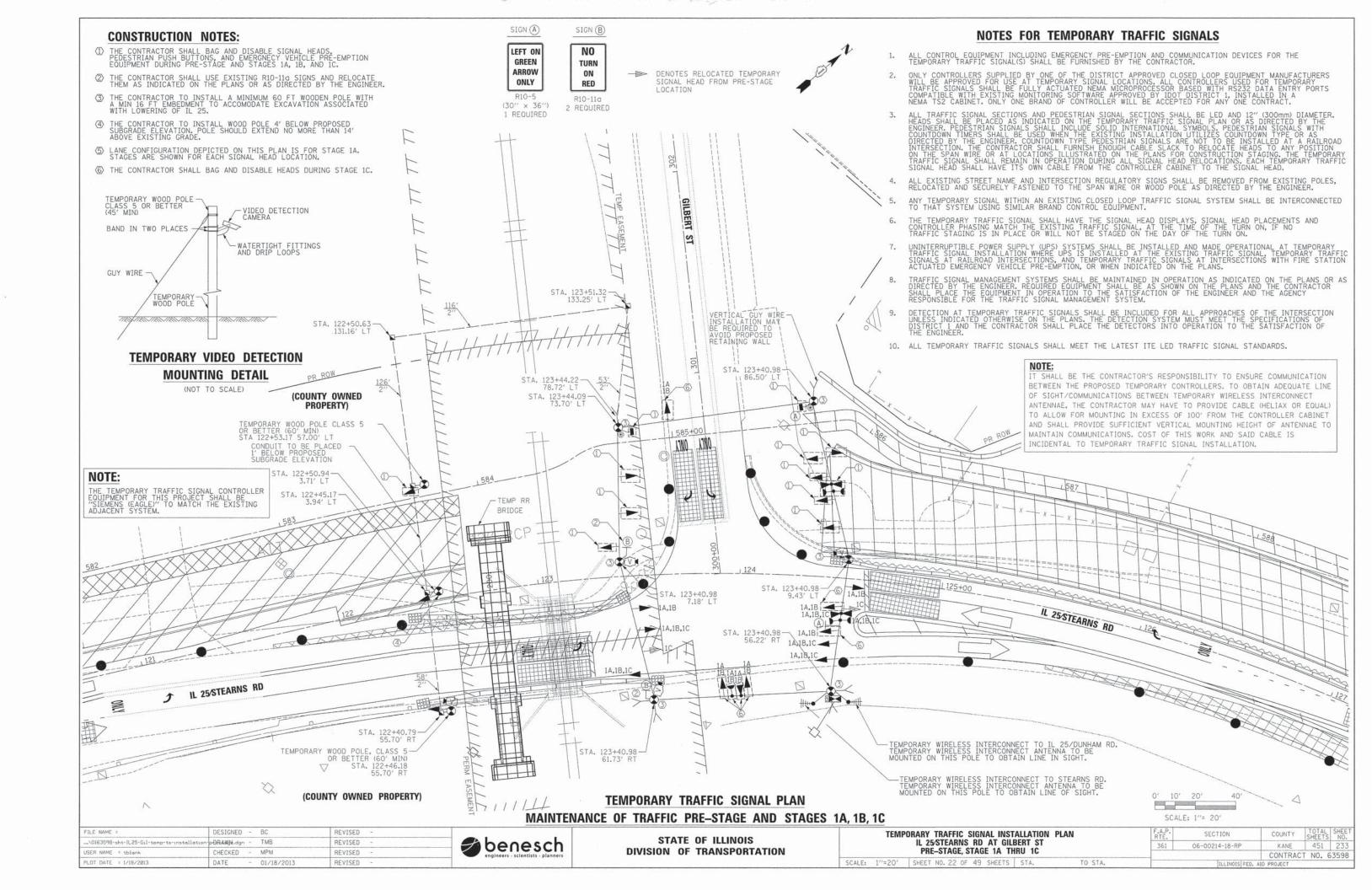
STA	TE	OF	ILLINOIS
DIVISION	OF	TR	ANSPORTATION

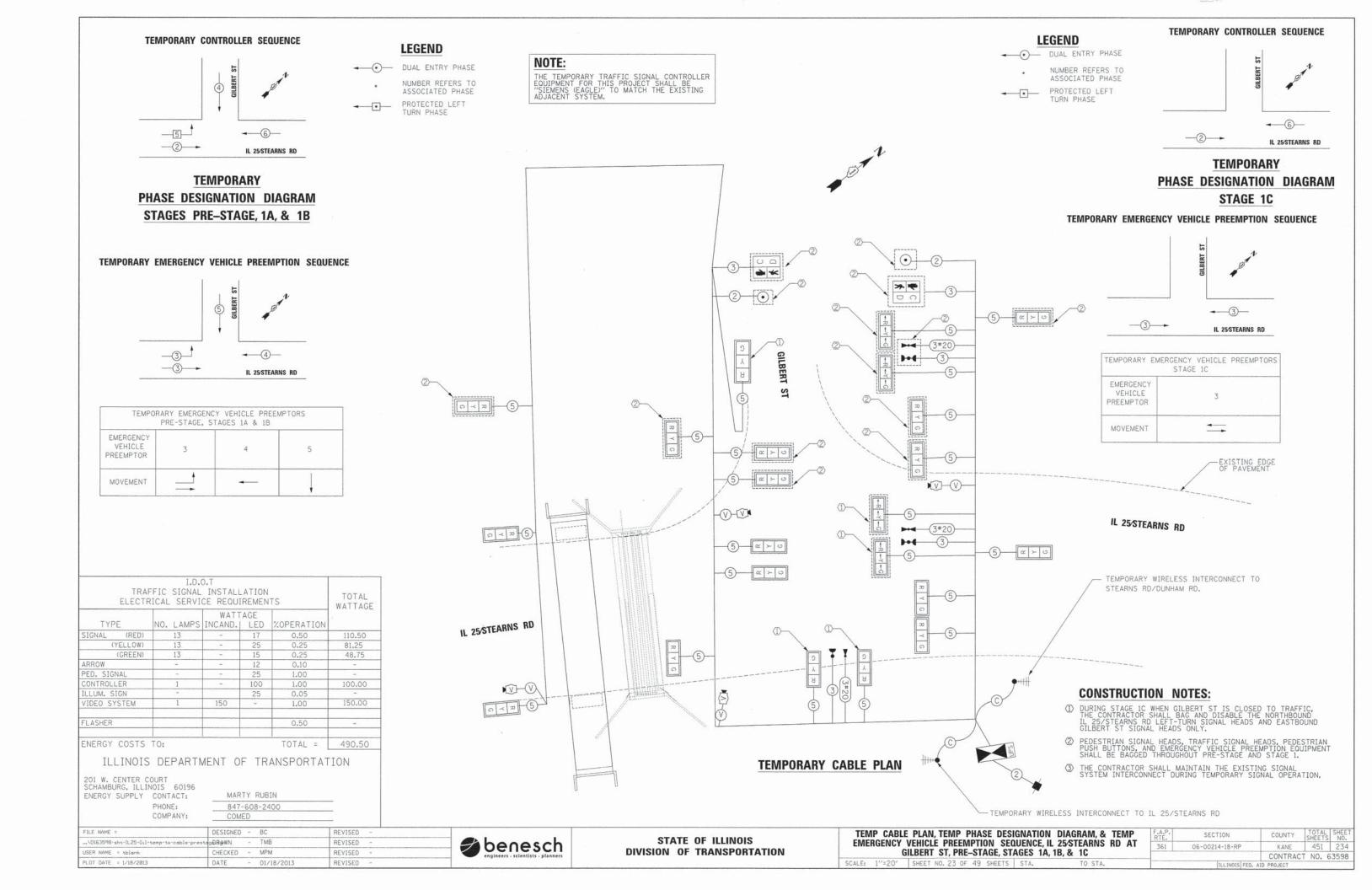
		EXISTING			TRAFFIC SIGNAL			GNAL PLA	PLAN	
		IL	25/S	TEA	RNS	RD .	AT	GILBERT	ST	
SCALE:	1"=20"	SHEET	NO. 2	O OF	49	SHEET	s	STA.		T

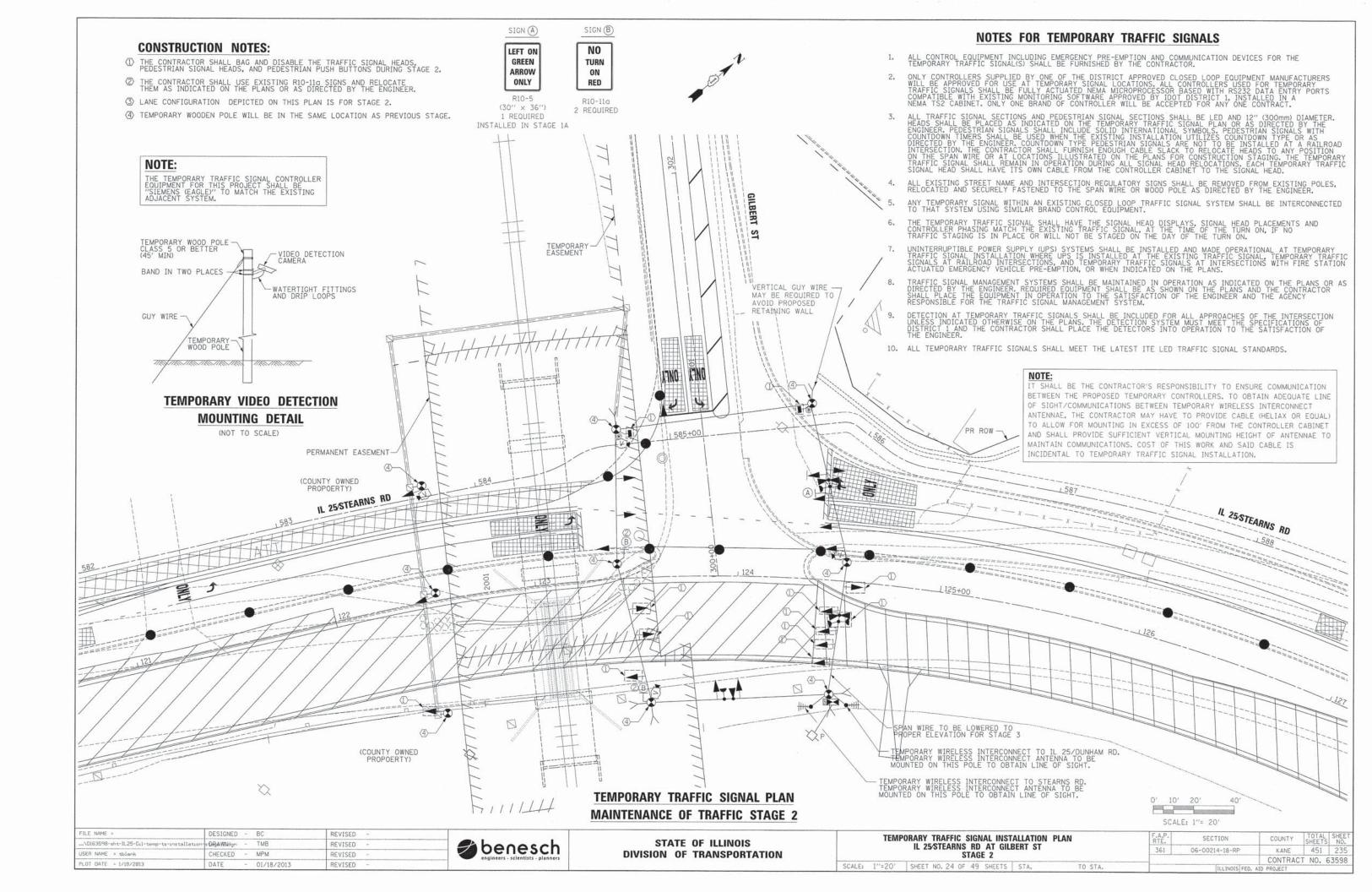
RTE.	SECTION	COUNTY	TOTAL	SHEE NO.	
361	06-00214-18-RP	KANE	451	231	
		CONTRACT	NO. 6	3598	
	ILLINOIS FED.	AID PROJECT			



FILE NAME =	DESIGNED - BC	REVISED -			EXISTING SCHEDULE OF QUANTITIES, EXISTING CABLE PLAN, AND	F.A.P.	SECTION	COUNTY	TOTAL SHEET
\D163598-sht-IL25-G:1-ex-ts-coble.dgn	DRAWN - TMB	REVISED -	Shoosch	STATE OF ILLINOIS	EXISTING PHASE DESIGNATION DIAGRAM	RIE.	22 2221 12 22	0001111	SHEETS NO.
USER NAME = tblank	CHECKED - MPM	REVISED -	benesch engineers - scientists - planners	DIVISION OF TRANSPORTATION	IL 25/STEARNS RD AT GILBERT RD	361	06-00214-18-RP	KANE	451 232
PLOT DATE = 1/18/2013	DATE - 01/18/2013	REVISED -			SCALE: 1"=20" SHEET NO. 21 OF 49 SHEETS STA. TO STA.		ILLINOIS FED. A	AID PROJECT	1 NO. 63598







TEMPORARY CONTROLLER SEQUENCE NOTE: **LEGEND** → DUAL ENTRY PHASE PROTECTED LEFT 4 TURN PHASE - NUMBER REFERS TO ASSOCIATED PHASE -6--2-IL 25/STEARNS RD (5) **TEMPORARY** PHASE DESIGNATION DIAGRAM 00 STAGE 2 TEMPORARY EMERGENCY VEHICLE PREEMPTION SEQUENCE PROPOSED EDGE OF PAVEMENT -3-4-4-IL 25/STEARNS RD 0 12 5 TEMPORARY EMERGENCY VEHICLE PREEMPTORS STAGE 2 EMERGENCY VEHICLE PREEMPTOR IL 25/STEARNS RD TEMPORARY WIRELESS INTERCONNECT TO DUNHAM RD. MOVEMENT I.D.O.T TRAFFIC SIGNAL INSTALLATION TOTAL ELECTRICAL SERVICE REQUIREMENTS WATTAGE WATTAGE NO. LAMPS INCAND. | LED %OPERATION 110.50 81.25 (YELLOW) (GREEN) 0.25 48.75 **CONSTRUCTION NOTES:** PED. SIGNAL ONTROLLER 100 1.00 LLUM. SIGN ② THE CONTRACTOR SHALL MAINTAIN THE EXISTING SIGNAL SYSTEM INTERCONNECT DURING TEMPORARY SIGNAL OPERATION. VIDEO SYSTEM 150.00 **TEMPORARY CABLE PLAN** FLASHER ENERGY COSTS TO: ILLINOIS DEPARTMENT OF TRANSPORTATION - TEMPORARY WIRELESS INTERCONNECT TO STEARNS RD. 201 W. CENTER COURT SCHAMBURG, ILLINOIS 60196

FILE NAME = DESIGNED - BC REVISED - ...\D183598\sht-IL25-Gil-temp-ts-coble-stoge 28BAWN - TMB REVISED - ...\USER NAME = tblonk CHECKED - MPM REVISED - ...\D171872013 DATE - .01/18/2013 REVISED - ...\D171872013 REVISED - .

MARTY RUBIN

847-608-2400

COMED

ENERGY SUPPLY CONTACT:

PHONE:

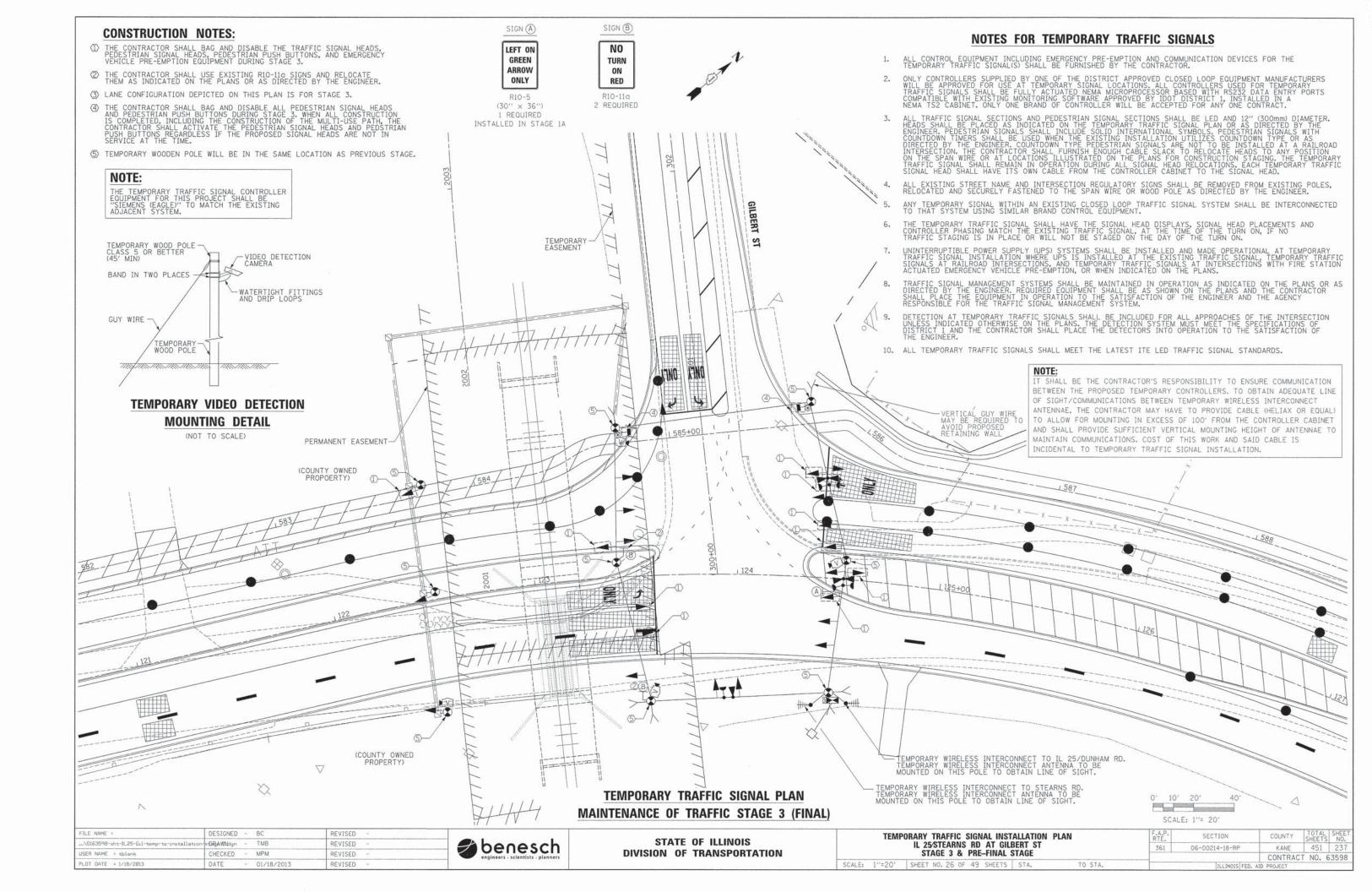
COMPANY:

benesch

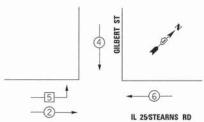
STATE OF ILLINOIS
DIVISION OF TRANSPORTATION

TEMP CABLE PLAN, TEMP PHASE DESIGNATION DIAGRAM, & TEMP EMERGENCY VEHICLE PREEMPTION SEQUENCE, IL 25/STEARNS RD AT GILBERT ST, STAGE 2

SCALE: 1"=20' SHEET NO. 25 OF 49 SHEETS STA.



TEMPORARY CONTROLLER SEQUENCE



LEGEND

- DUAL ENTRY PHASE NUMBER REFERS TO ASSOCIATED PHASE PROTECTED LEFT

NOTE:

PROPOSED EDGE OF PAVEMENT

THE TEMPORARY TRAFFIC SIGNAL CONTROLLER EQUIPMENT FOR THIS PROJECT SHALL BE "SIEMENS (EAGLE)" TO MATCH THE EXISTING ADJACENT SYSTEM.

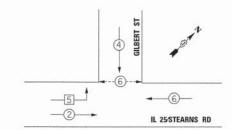
TEMPORARY CONTROLLER SEQUENCE

LEGEND

---- PEDESTRIAN PHASE

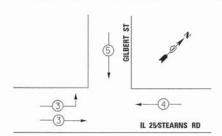
NUMBER REFERS TO ASSOCIATED PHASE PROTECTED LEFT

TURN PHASE



TEMPORARY PHASE DESIGNATION DIAGRAM PRE-FINAL STAGE

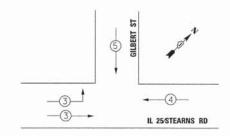
TEMPORARY EMERGENCY VEHICLE PREEMPTION SEQUENCE



TEMPORA		Y VEHICLE PREEN AL STAGE	MPTORS
EMERGENCY VEHICLE PREEMPTOR	3	4	5
MOVEMENT		-	

TEMPORARY PHASE DESIGNATION DIAGRAM STAGE 3

TEMPORARY EMERGENCY VEHICLE PREEMPTION SEQUENCE



TEMPORA		Y VEHICLE PREEM GE 3	IPTORS
EMERGENCY VEHICLE PREEMPTOR	3	4	5
MOVEMENT	=	S	ļ

I.D.O.T TRAFFIC SIGNAL INSTALLATION ELECTRICAL SERVICE REQUIREMENTS										
TYPE		NO. LAMPS	WATT INCAND.		%OPERATION	WATTAGE				
SIGNAL	(RED)	14	-	17	0.50	119.00				
(YELLOW)	14	-	25	0.25	87.50				
(GREEN)		14	-	15	0.25	52.50				
ARROW			-	12	0,10					
PED. SIG	NAL	2	12.	25	1,00	50.00				
CONTROL	LER	1	-	100	1.00	100.00				
ILLUM. S	IGN	-		25	0.05	-				
VIDEO SY	YSTEM	1	150	-	1,00	150,00				
FLASHER				- HV-100	0.50	-				
ENERGY	COSTS	TO:			TOTAL =	559.00				

ILLINOIS DEPARTMENT OF TRANSPORTATION

DESIGNED

CHECKED

DATE

201 W. CENTER COURT SCHAMBURG, ILLINOIS 60196 ENERGY SUPPLY CONTACT:

FILE NAME :

USER NAME = tblank

PLOT DATE = 1/18/2013

PHONE: COMPANY:

MARTY RUBIN 847-608-2400 COMED

01/18/2013

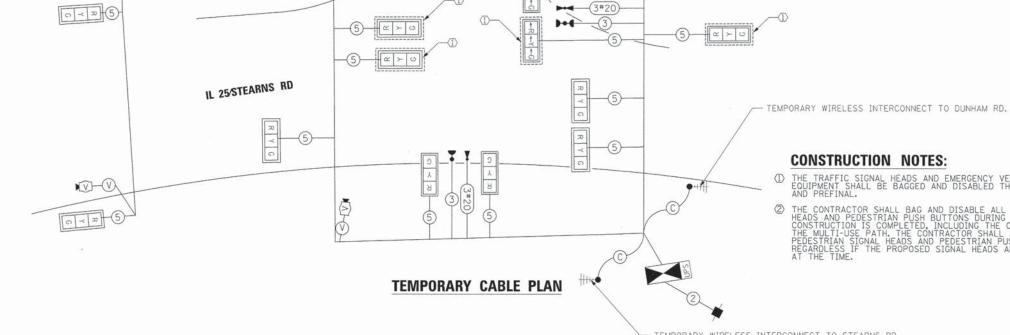
REVISED

REVISED

REVISED

REVISED

benesch



5 ~ 0

CONSTRUCTION NOTES:

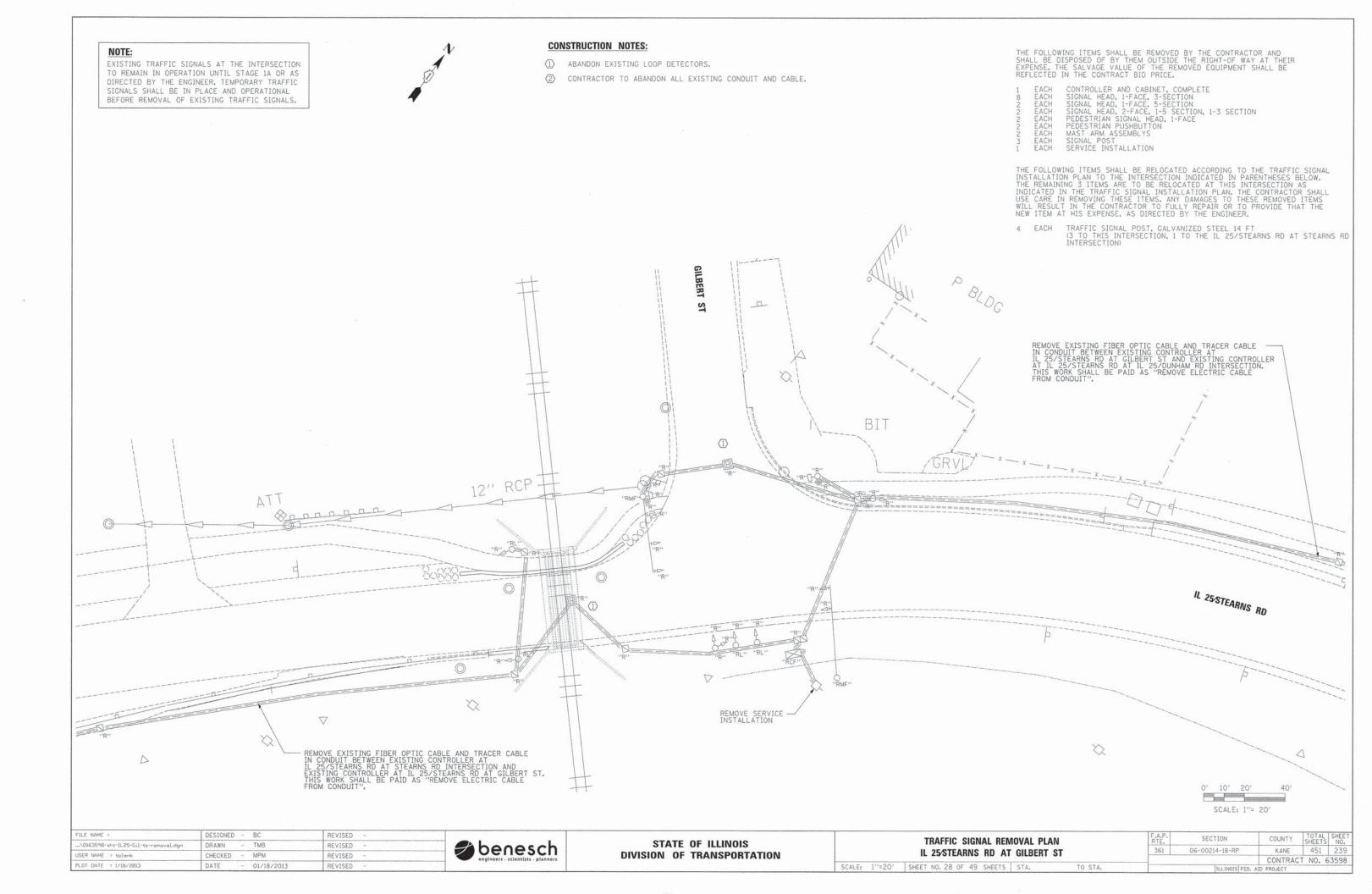
TEMPORARY WIRELESS INTERCONNECT TO STEARNS RD.

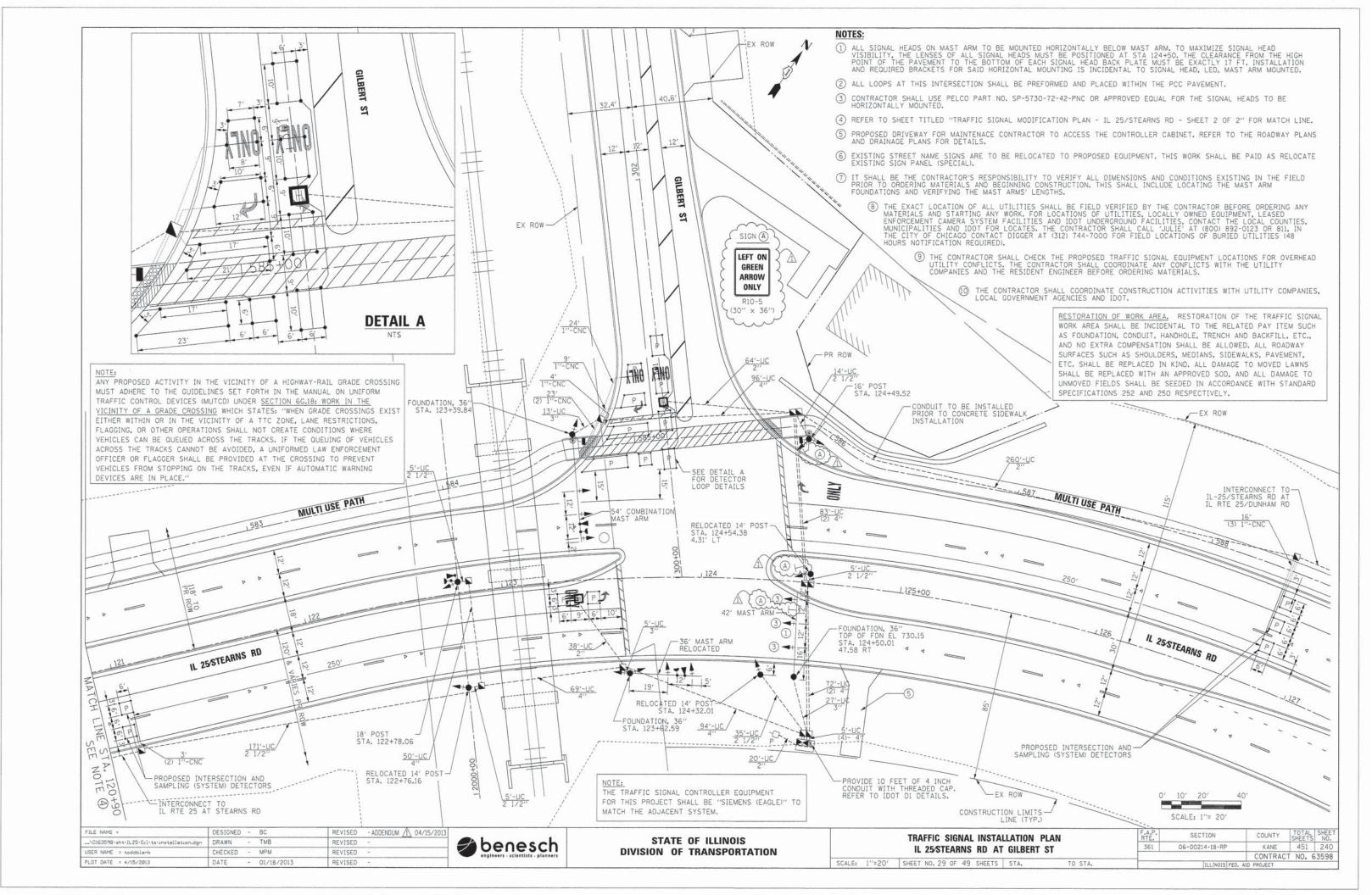
TEMP CABLE PLAN, TEMP PHASE DESIGNATION DIAGRAM, & TEMP EMERGENCY VEHICLE PREEMPTION SEQUENCE, IL 25'STEARNS RD AT GILBERT ST, STAGE 3 AND PREFINAL STAGE

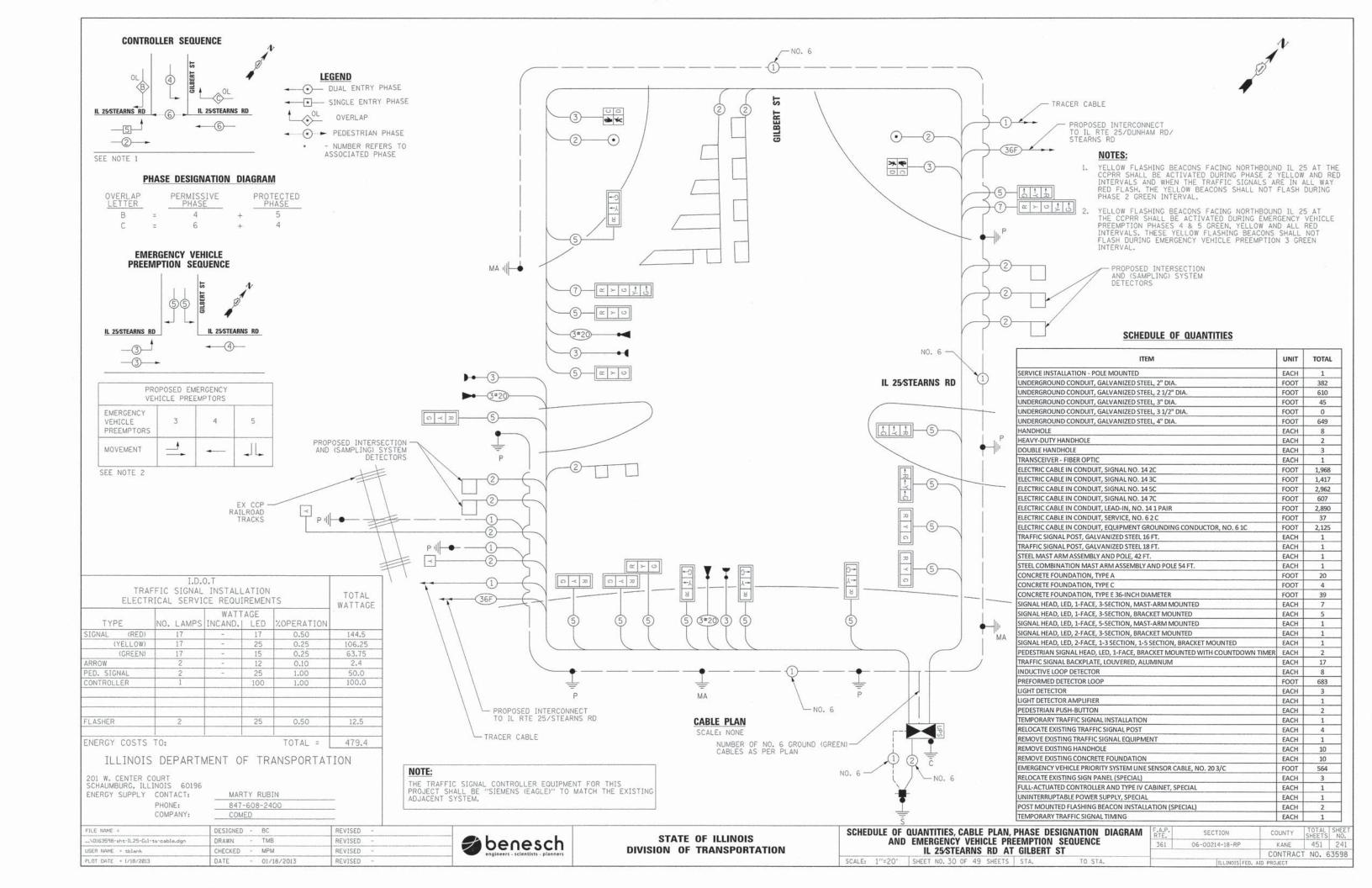
SCALE: 1"=20" SHEET NO. 27 OF 49 SHEETS STA.

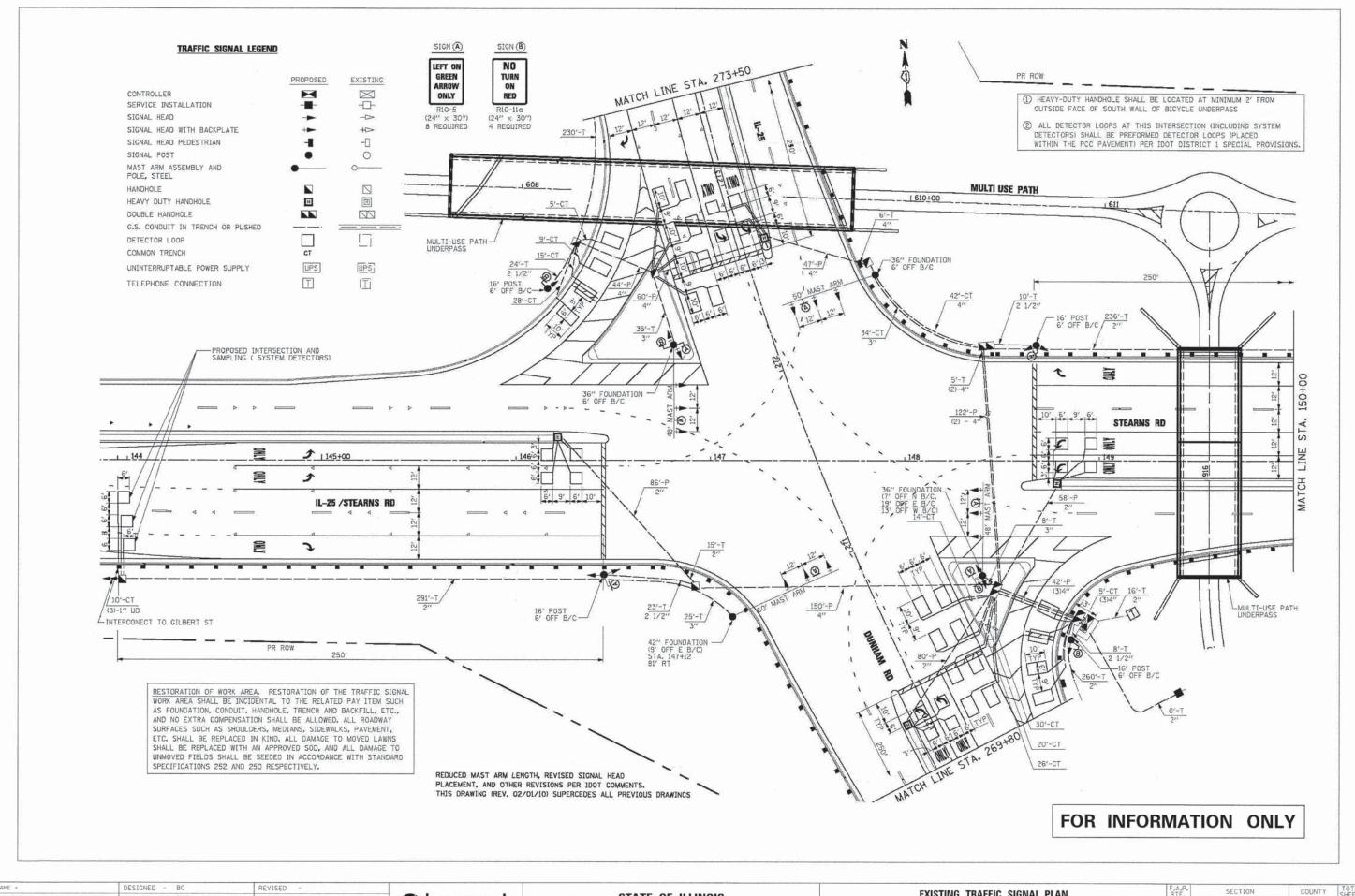
SECTION KANE 451 238 06-00214-18-RP CONTRACT NO. 63598

STATE OF ILLINOIS **DIVISION OF TRANSPORTATION**

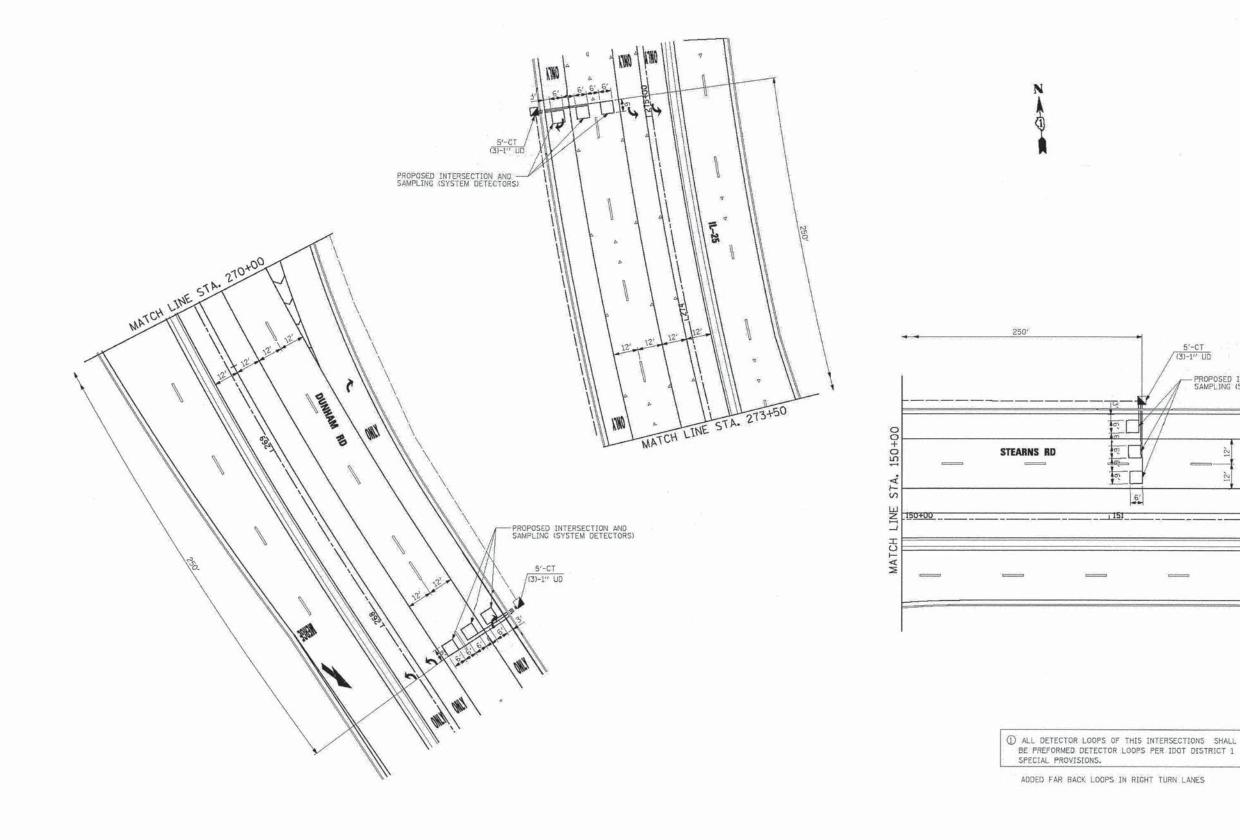








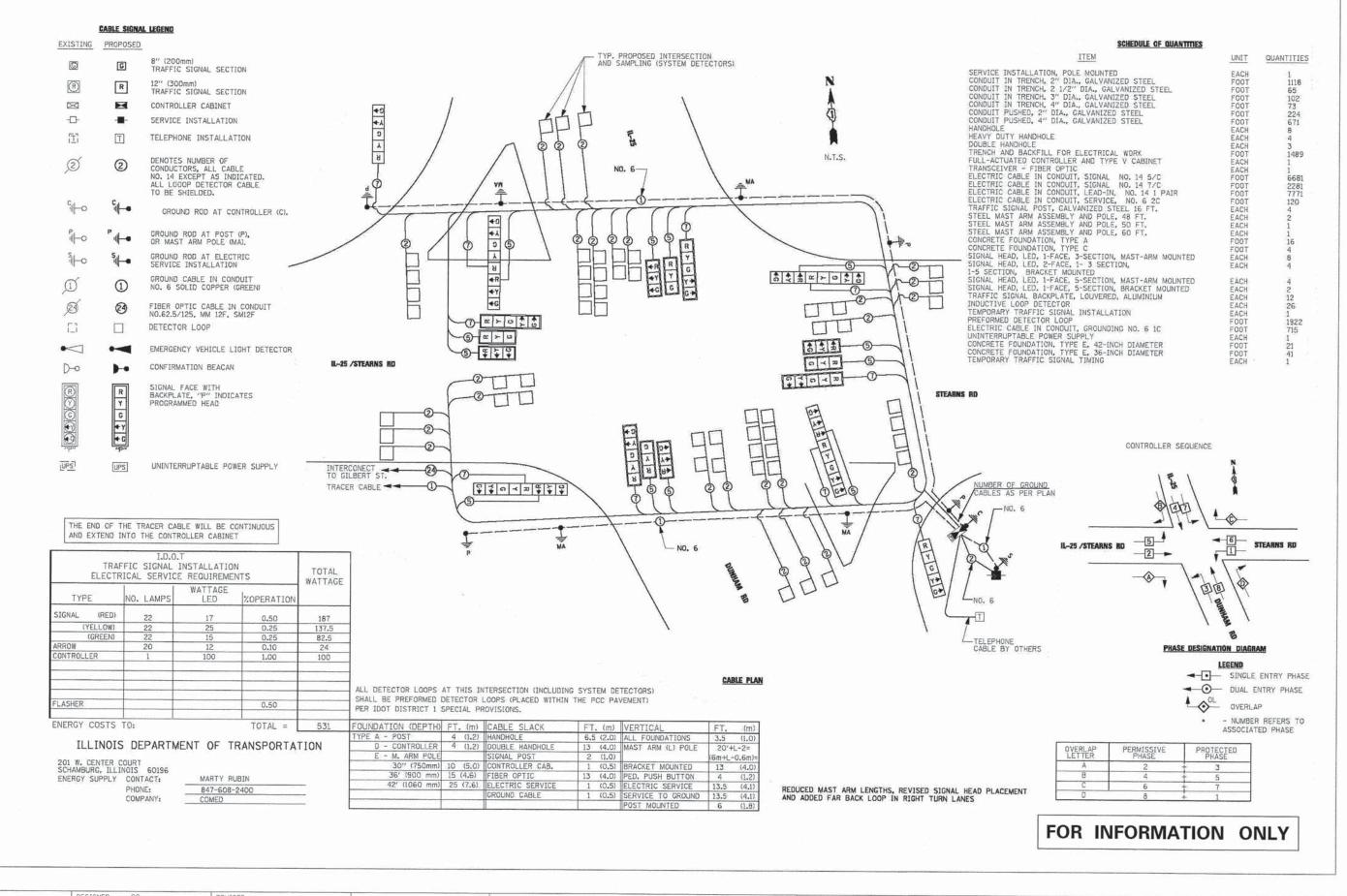
THE PERSON A	DESIGNED - DC	WEATOED -	Author Scales No. 100
163598-sht-1L25-Oun-ex-ts-planl.dgn	DRAWN - TMB	REVISED -	Shoosch
R NAME = tblank	CHECKED - MPM	REVISED -	benesch engineers - scientists - planners
T DATE = 1/18/2013	DATE - 01/18/2013	REVISED -	



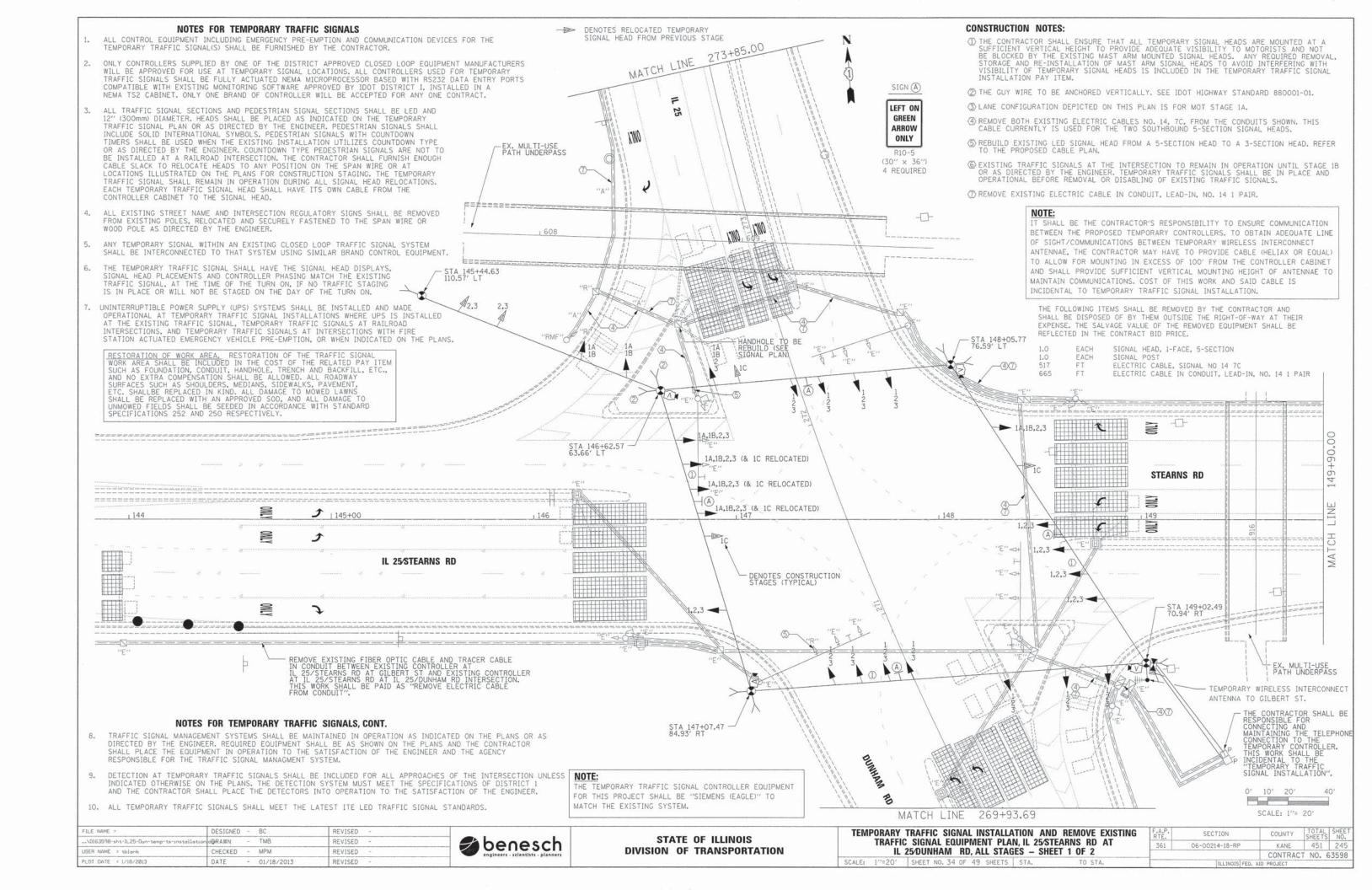
FOR INFORMATION ONLY

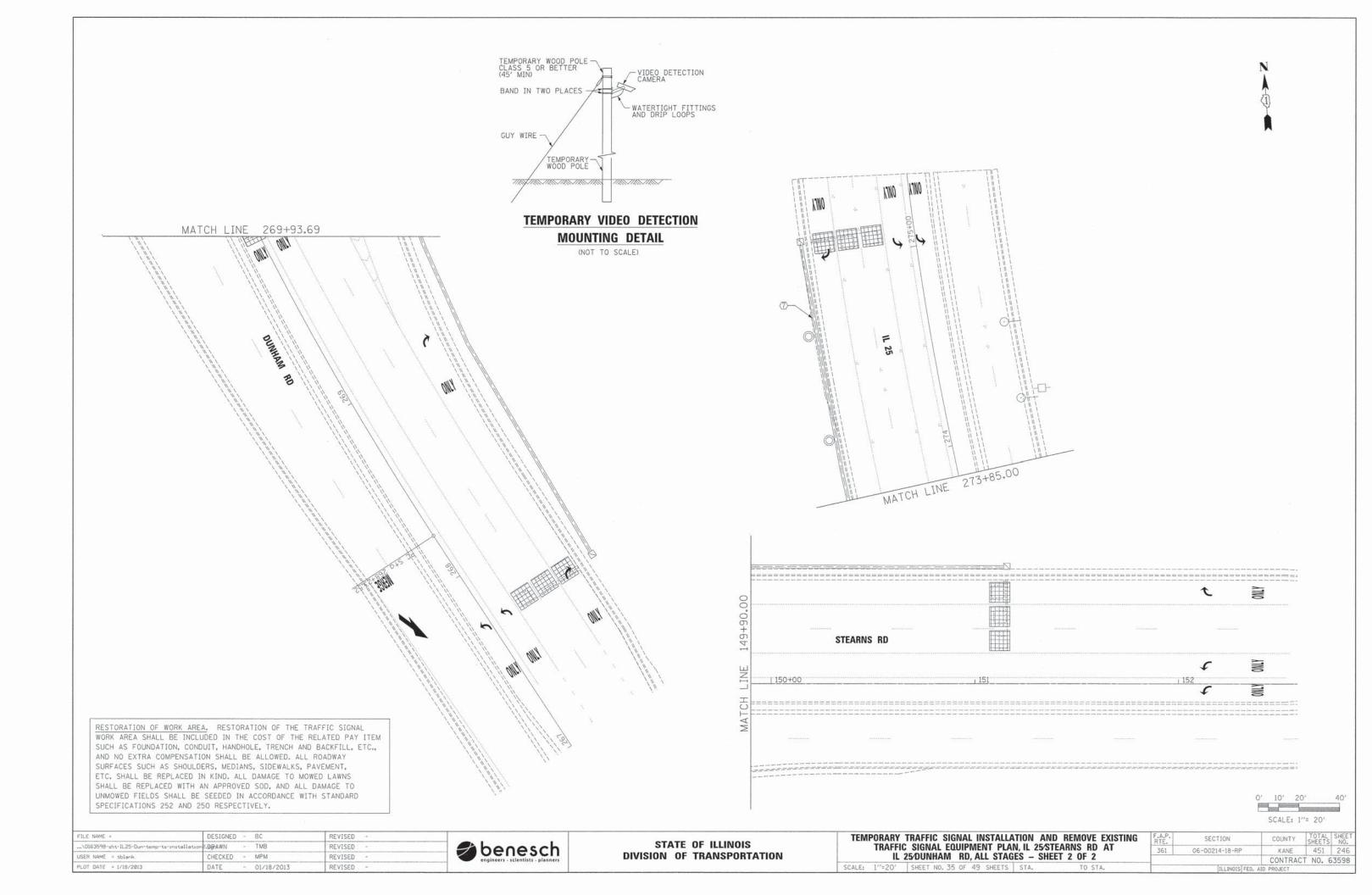
PROPOSED INTERSECTION AND SAMPLING (SYSTEM DETECTORS)

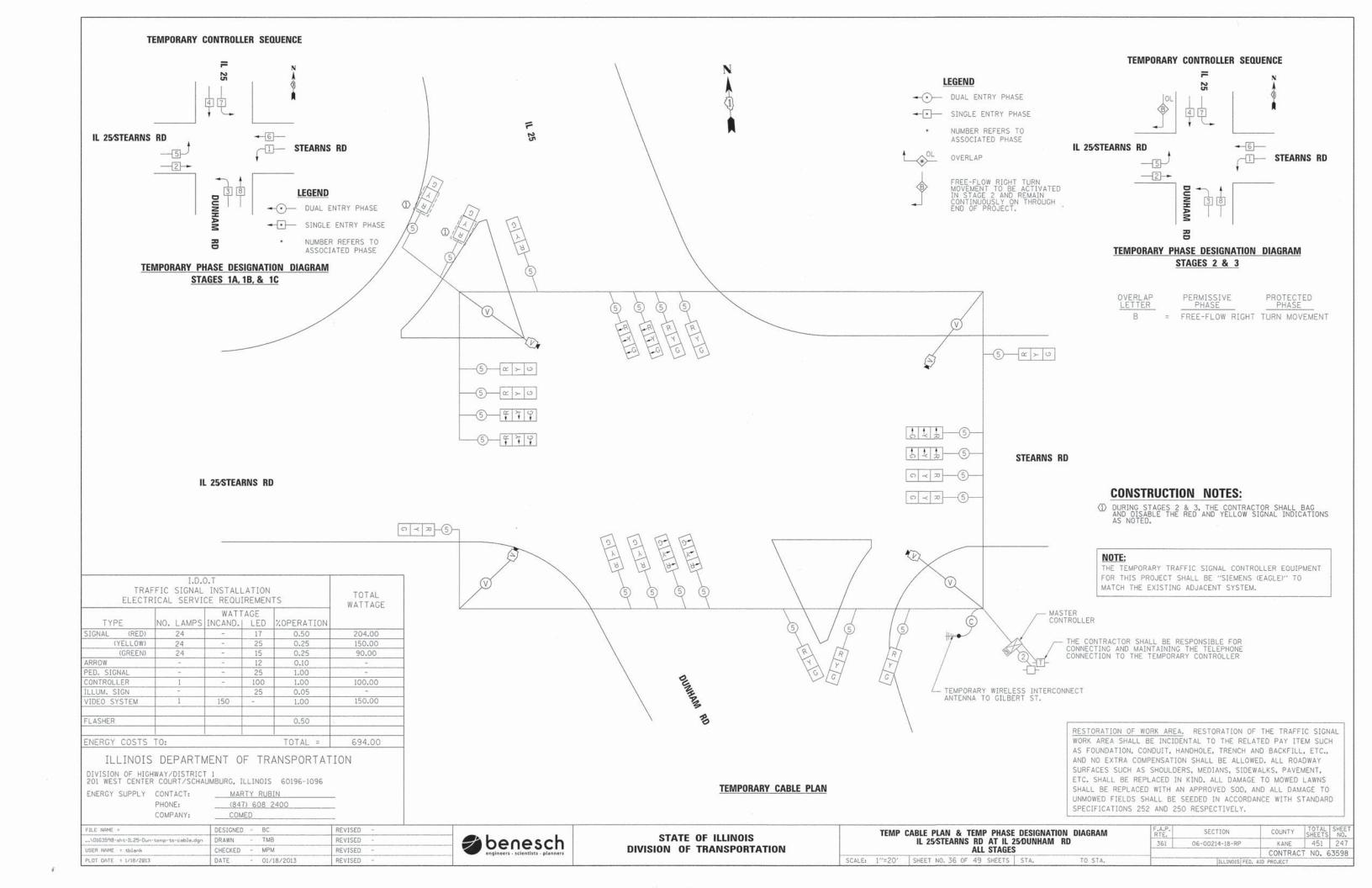
REVISED SECTION **EXISTING TRAFFIC SIGNAL PLAN** benesch STATE OF ILLINOIS ..\D163598-sht-|L25-Dun-ex-ts-plan2.dgn REVISED DRAWN 14-18-RP KANE 451 243
CONTRACT NO. 63598
ILLINOIS FED. AID PROJECT IL 25/STEARNS RD AT GILBERT ST **DIVISION OF TRANSPORTATION** USER NAME = tblank CHECKED MPM REVISED 01/18/2013 REVISED SCALE: 1"=20' SHEET NO. 32 OF 49 SHEETS STA.

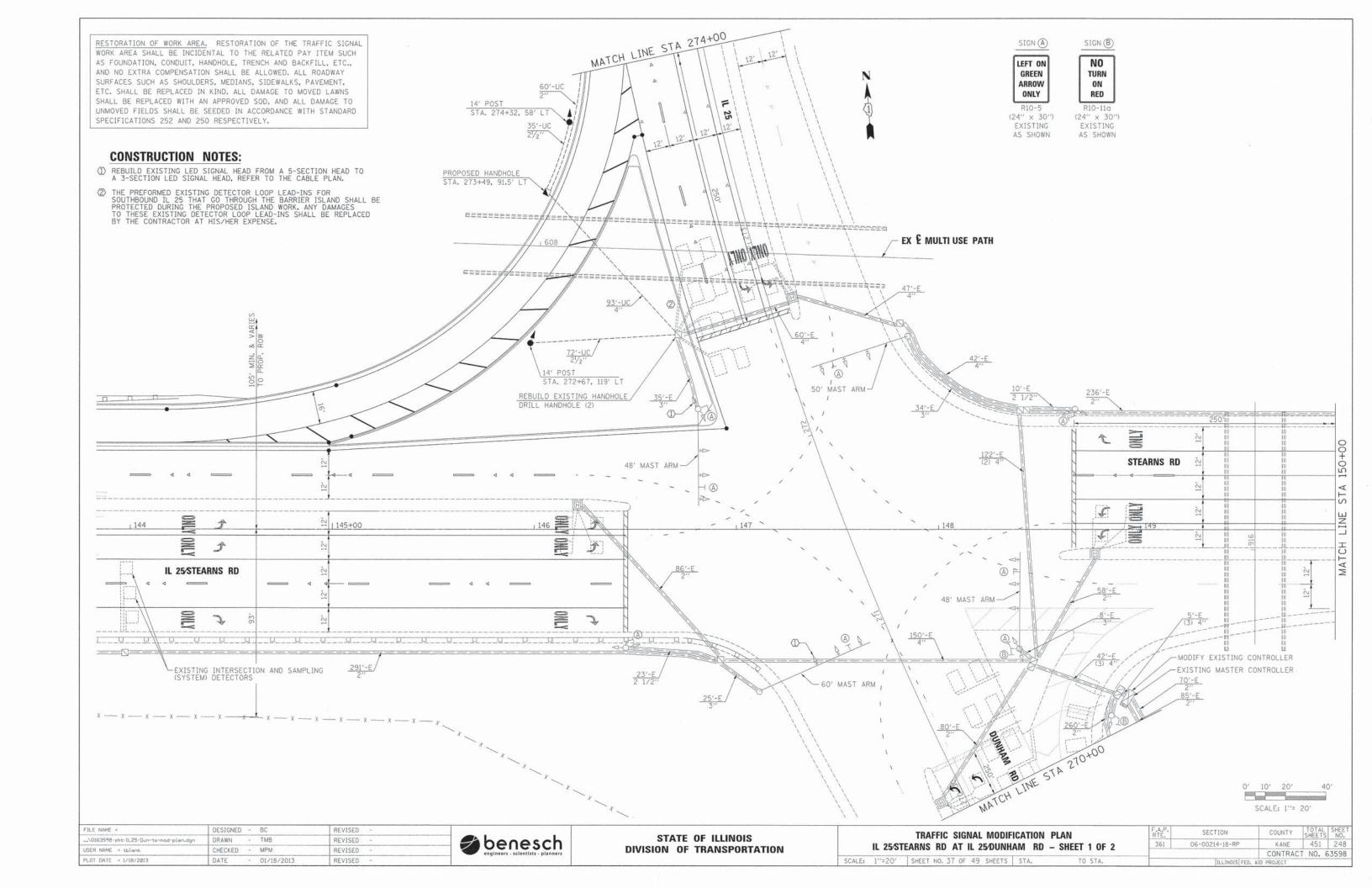


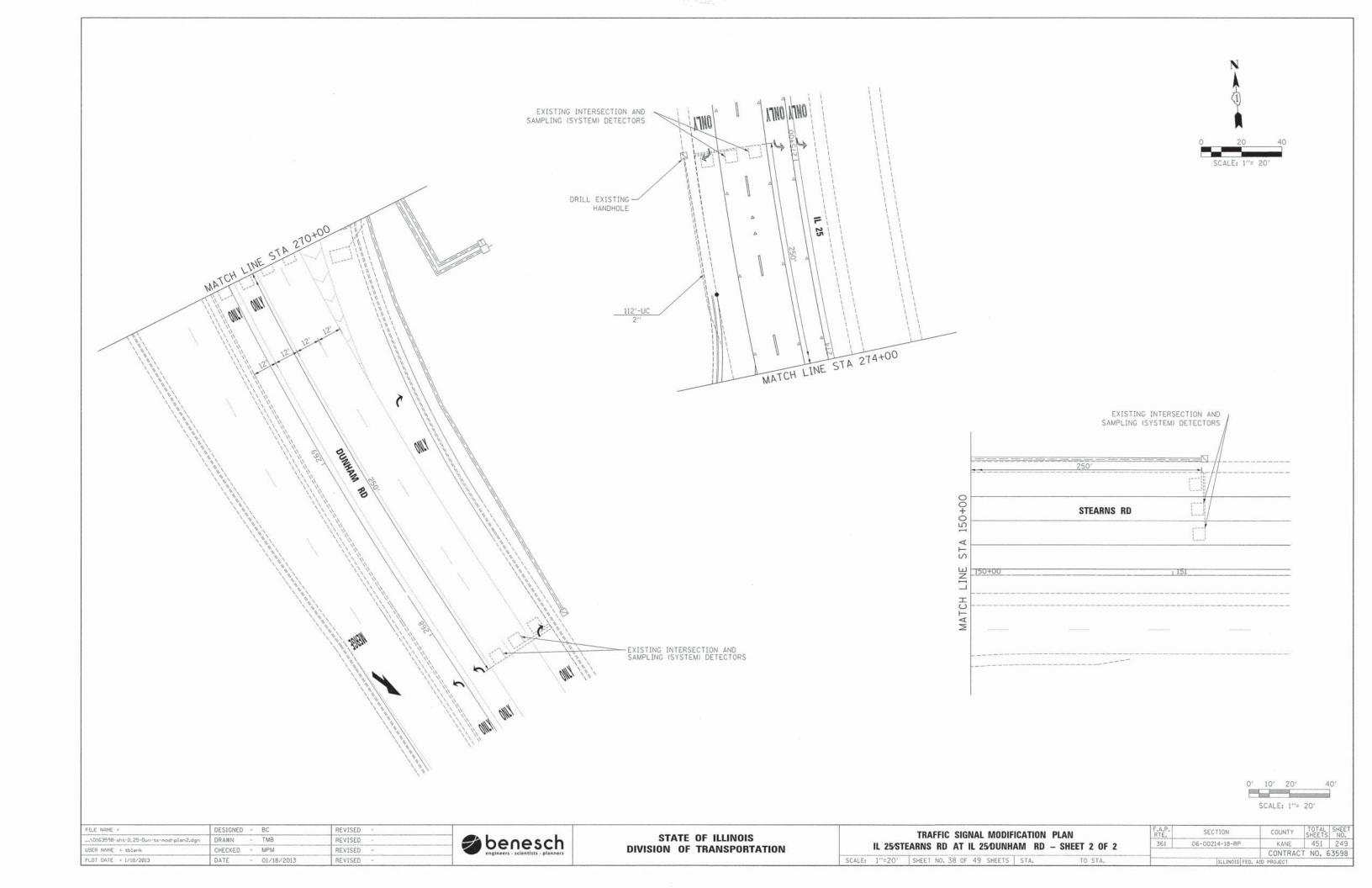
\D163598-sht-IL25-Dun-ex-ts-cable.dgn	DRAWN - TMB	REVISED -	S hoooseh	STATE OF ILLINOIS	EXISTING SCHEDULE OF QUANTITIES, EXISTING CABLE PLAN, AND EXISTING PHASE DESIGNATION DIAGRAM	F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
USER NAME = tblank	CHECKED - MPM	REVISED -	benesch DIVISION OF TRANSPORTATION	IL 25'STEARNS RD AT IL 25'DUNHAM RD	361	06-00214-18-RP	KANE	451	244	
PLOT DATE = 1/18/2013	DATE - 01/18/2013	REVISED -	engineers - sciencists - planners		SCALE: 1"=20" SHEET NO. 33 OF 49 SHEETS STA. TO STA.		TILL INOTS FED. A	CONTRACT	T NO. 6	3598

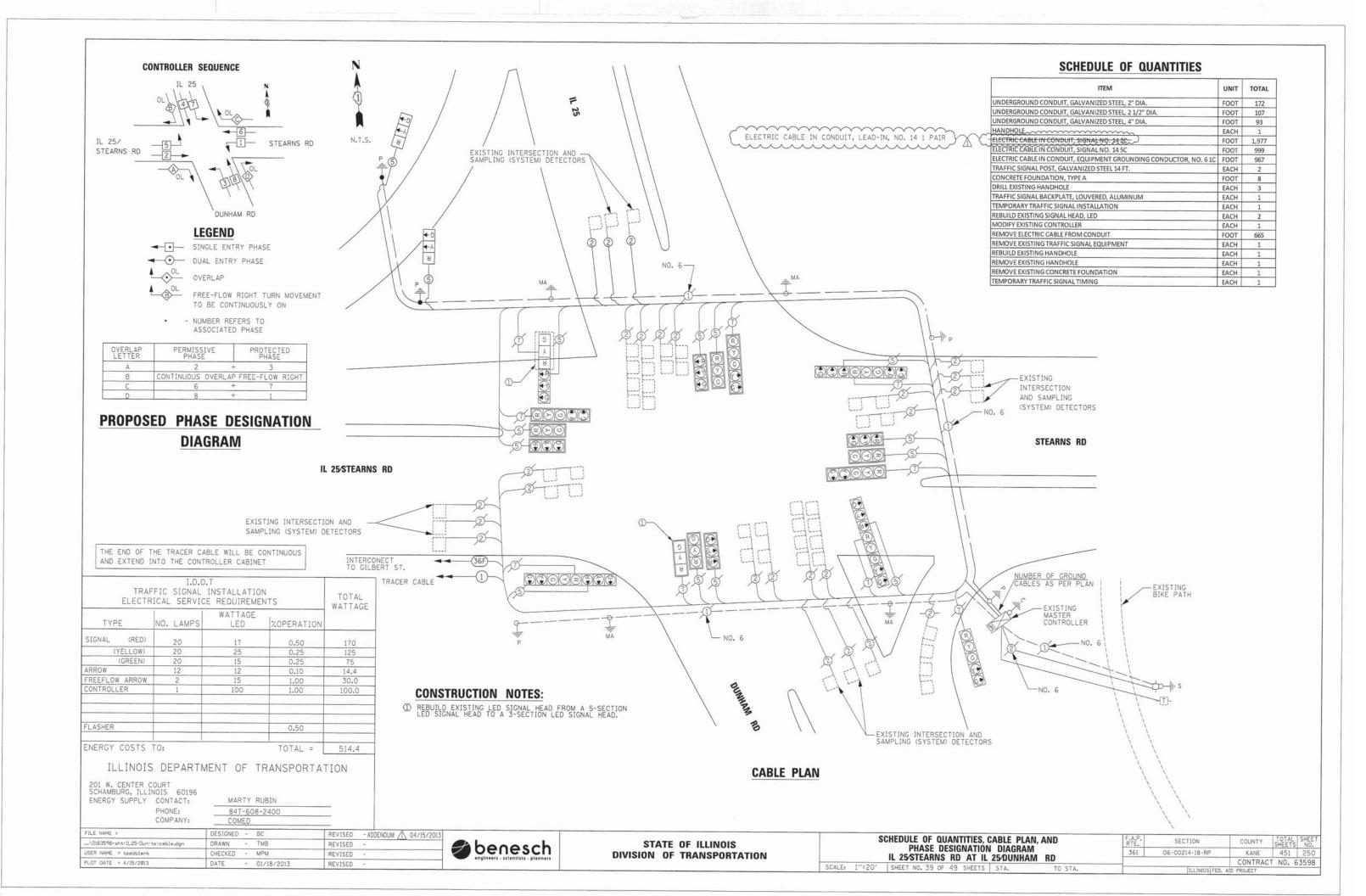


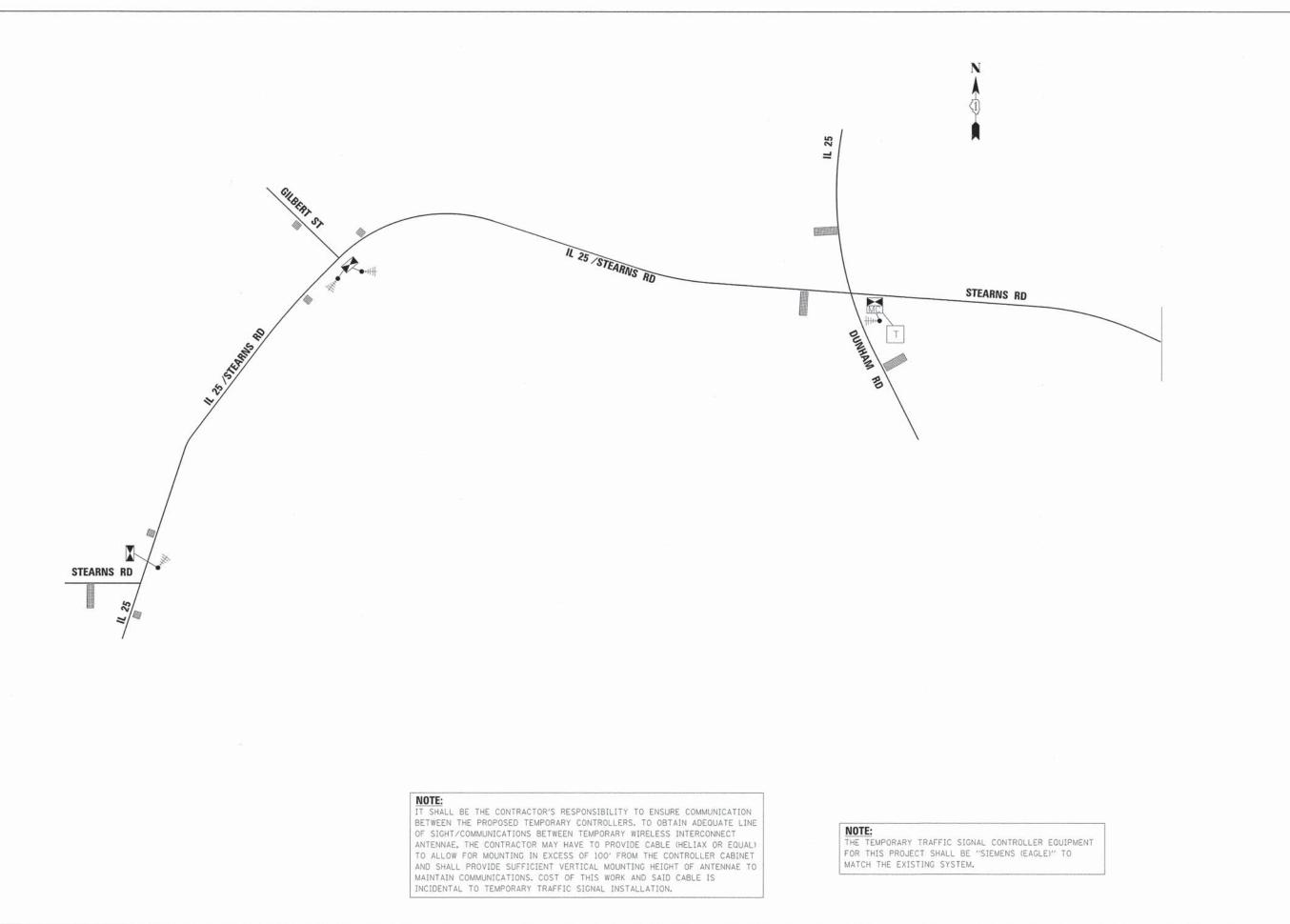












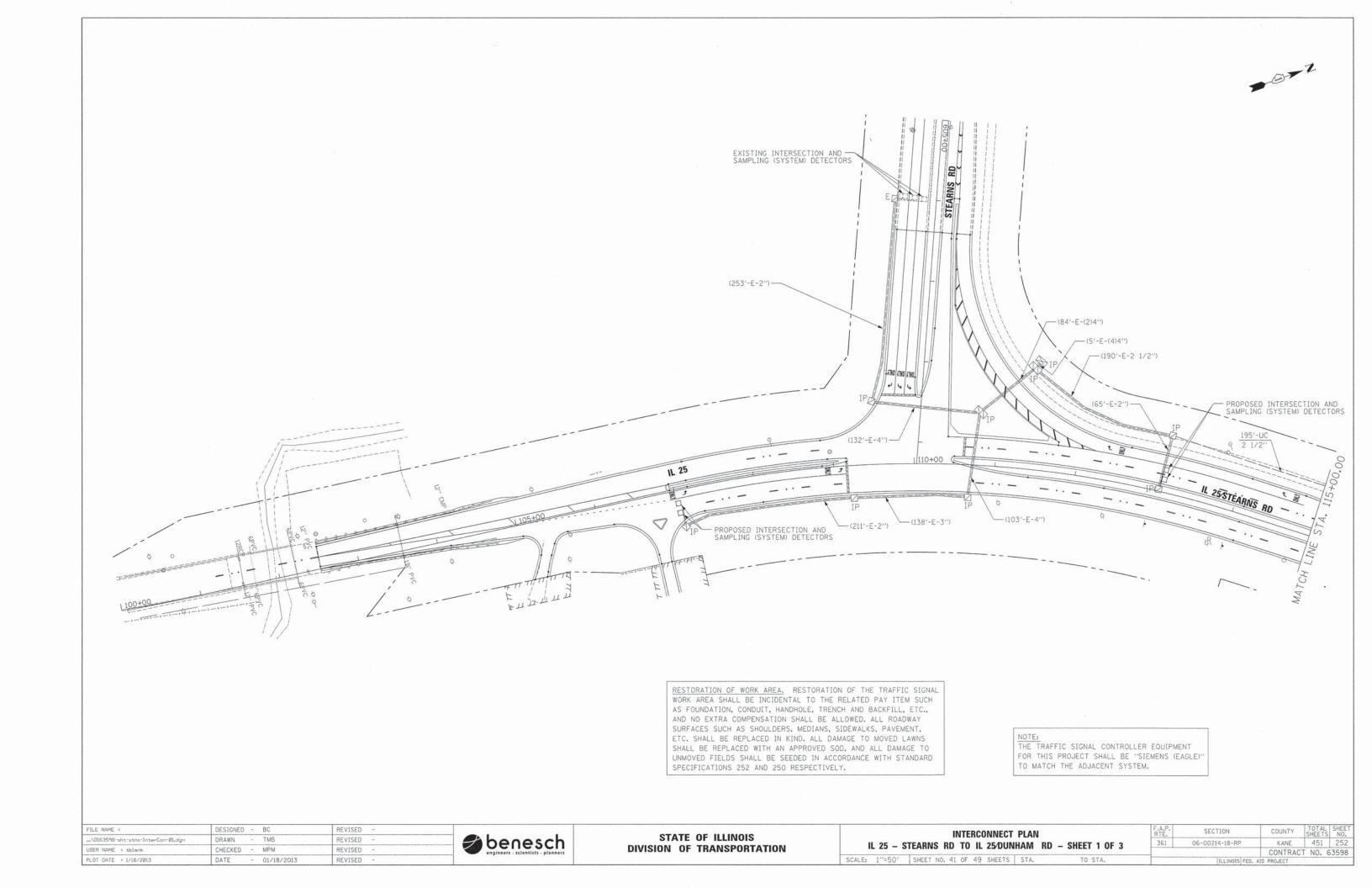
FILE NAME #	DESIGNED - BC	REVISED -	
\D163598-sht-InterCon-schem-temp.dgn	DRAWN - TMB	REVISED -	
USER NAME = tblank	CHECKED - MPM	REVISED -	
PLOT DATE = 1/18/2013	DATE - 01/18/2013	REVISED -	

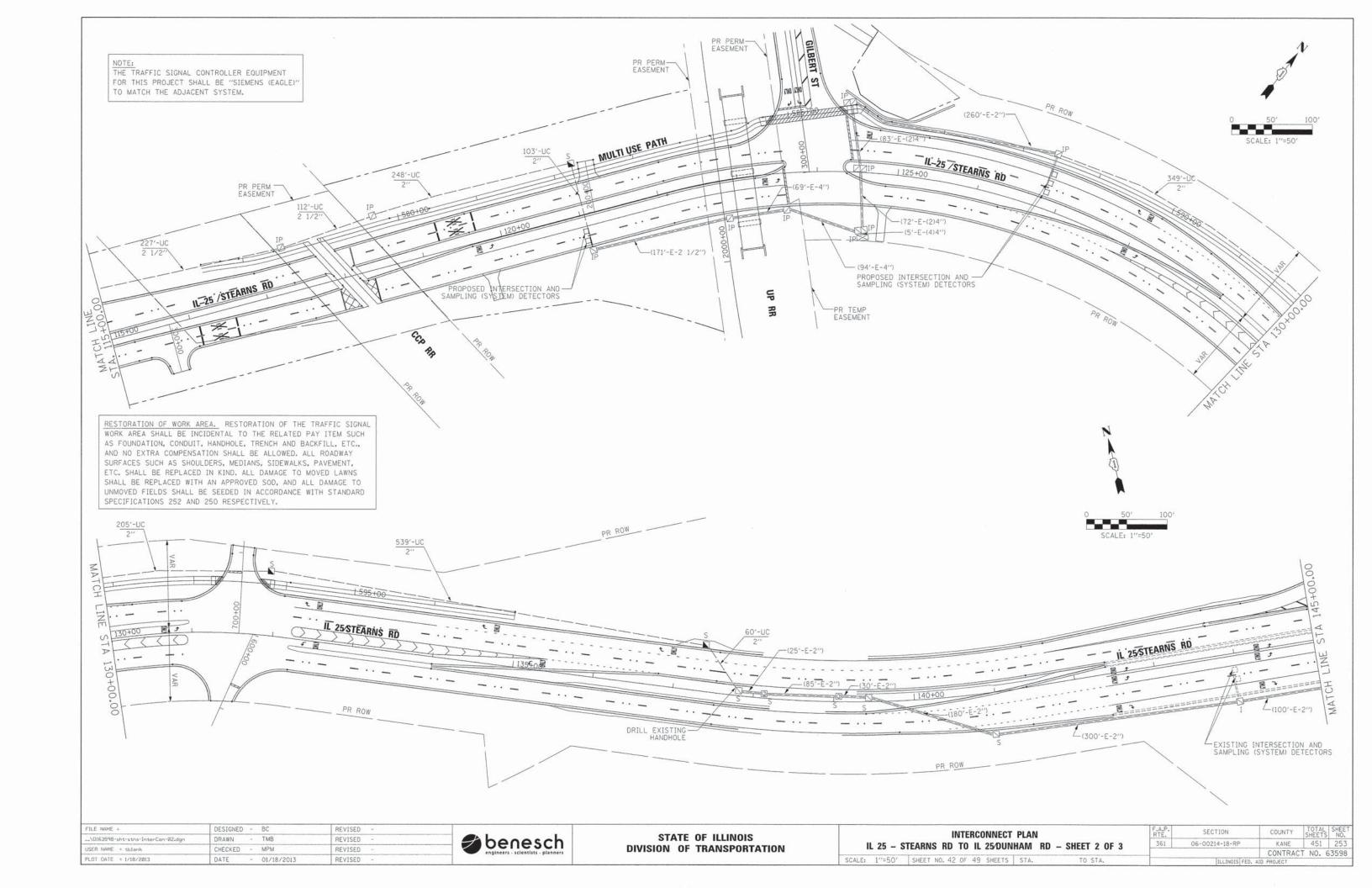


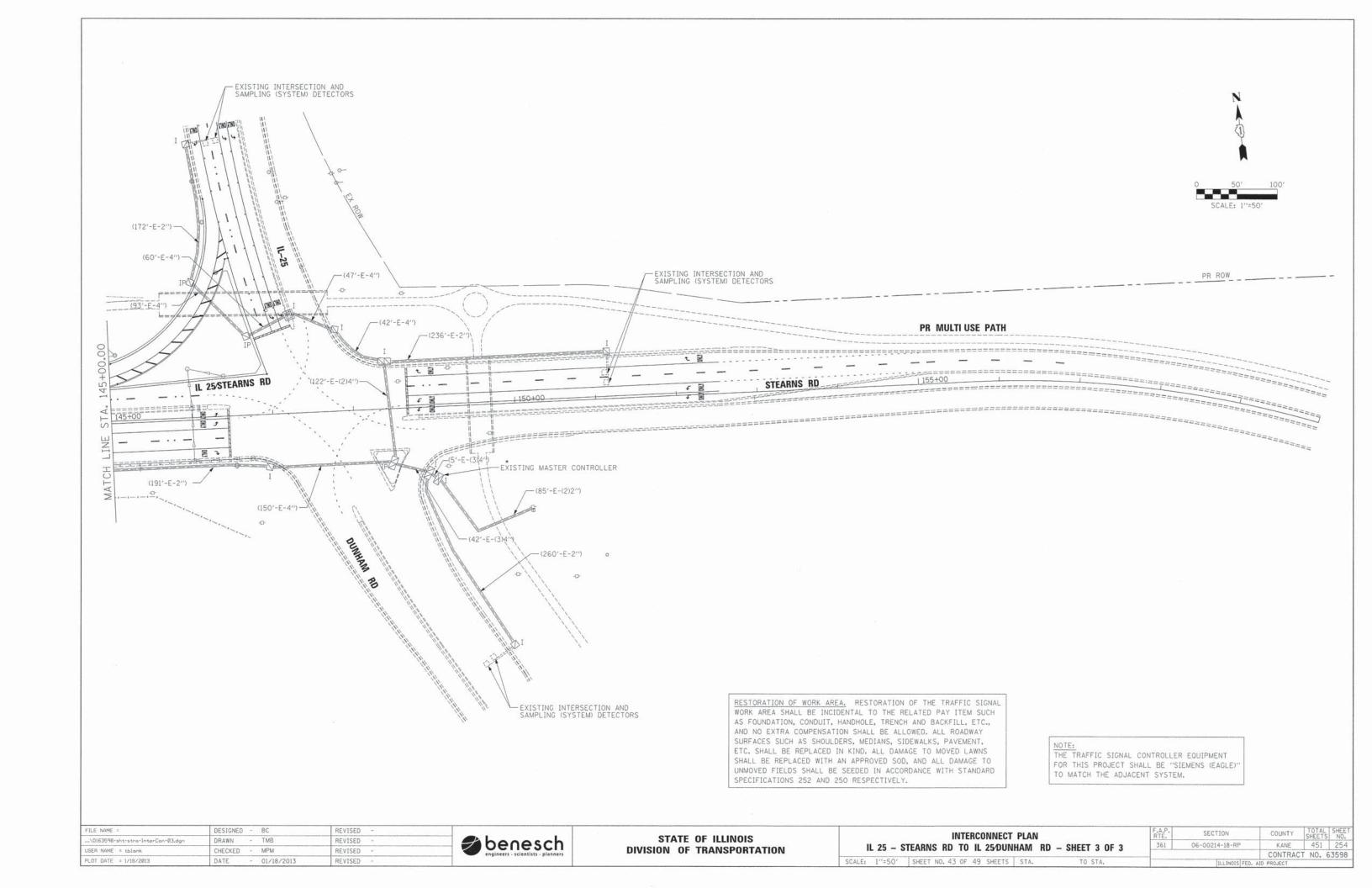
STATE OF ILLINOIS
DIVISION OF TRANSPORTATION

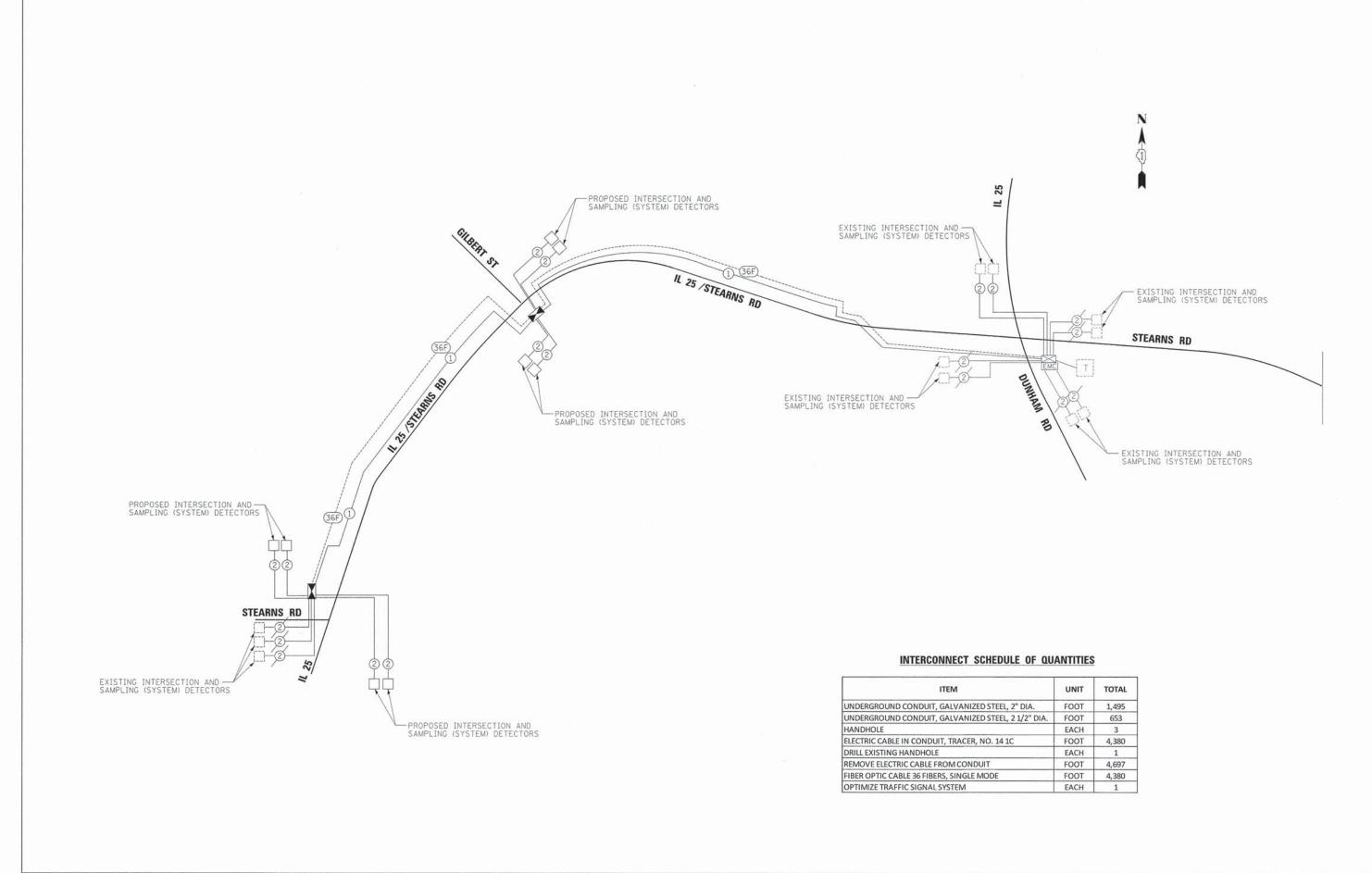
			TEN	1P	OR	AR	YI	NT	R	CONI	VE	CT SCHEMAT	TIC	
		IL	25	-	ST	EA	RN	S F	lD	TO	IL	25/DUNHAM	RD	
SCALE:	1"=200"		SHEE	Т	NO.	40	OF	49	SI	HEETS		STA.	TO	STA

F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP	KANE	451	251
		CONTRACT	NO. 6	3598
	ILLINOIS FED.	AID PROJECT		









FILE NAME =	DESIGNED - BC	REVISED -	
\D163598-sht-InterCon-schem.dgn	DRAWN - TMB	REVISED -	
USER NAME = tblank	CHECKED - MPM	REVISED -	
PLOT DATE = 1/18/2013	DATE - 01/18/2013	REVISED -	



STATE OF ILLINOIS
DIVISION OF TRANSPORTATION

INTERC	ON	INEC	T	SCH	EM/	TIC	A	ND	S	CHEDULE	0F	QUANTITIES	
	IL	25	-	STE	ARN	SR	D	TO	IL	25/DUNH	AM	RD	
NTS		SHEE	Т	NO. 4	4 OF	49	SH	FETS	T	STA.		TO STA.	

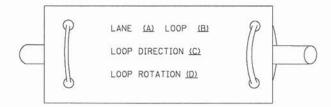
SCALE:

F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEE NO.
361	06-00214-18-RP	KANE	451	255
		CONTRACT	NO. 6	3598
	ILLINOIS FED. AI	D PROJECT		

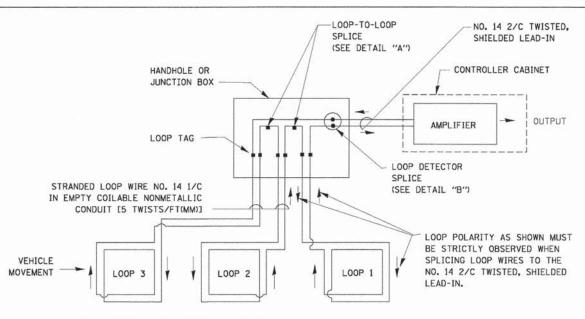
LOOP DETECTOR NOTES

- EACH PAIR OF LOOP WIRES SHALL BE PLACED IN A SEPARATE EMPTY COILABLE NONMETALLIC CONDUIT FROM THE EDGE OF PAVEMENT TO THE HANDHOLE. SPACING BETWEEN THE HOLES DRILLED IN THE PAVEMENT SHALL NOT BE LESS THAN 6" (150 mm). EMPTY COILABLE NONMETALLIC CONDUIT SHALL BE INCLUDED IN THE COST OF THE LOOP WIRE.
- 2. THE NUMBER OF LOOP TURNS SHALL BE AS RECOMMENDED BY THE AMPLIFIER MANUFACTURER. ALL ADJACENT SIDES OF THE LOOPS SHALL BE INSTALLED IN SUCH A WAY THAT THE CURRENT FLOW IS IN THE SAME DIRECTION TO REINFORCE ITS MAGNETIC FIELDS FOR SMALL VEHICLE DETECTION.
- 3. EACH LOOP LEAD-IN SHALL BE IDENTIFIED AND PERMANENTLY TAGGED IN THE HANDHOLE. EACH LEAD-IN CABLE TAG SHALL INDICATE THE LOCATION OF THE LOOP, LOOP ROTATION (CLOCKWISE/COUNTERCLOCKWISE), LOOP LEAD-IN DIRECTION (IN OR OUT), LOOP CABLE NUMBER AND LOCATION IN CABINET, AND NUMBER OF TURNS IN THE DETECTOR LOOPS IN WATER PROOF INK AS INDICATED ON THE DISTRICT 1 STANDARD TRAFFIC SIGNAL DESIGN DETAIL. THE CONTRACTOR SHALL MARK LOOP LOCATIONS ON RECORD DRAWINGS AND PRESENT TO THE ENGINEER AFTER FINAL INSPECTION. LOOPS SHALL BE MARKED BY LANE AND LOOP NUMBER. SEE DETAIL BELOW.
- 4. ALL LOOP CABLE SHALL BE FASTENED WITH PLASTIC TIE WRAP TO THE HANDHOLE HOOKS.
- 5. IN ASPHALT PAVEMENT, LOOPS SHOULD BE PLACED IN THE BINDER AND DIVEHOLES MARKED AT THE CURB WITH A SAW-CUT. THE SAW-CUT SHALL BE CUT IN ACCORDANCE WITH LOCAL AND E.P.A. DUST CONTROL REQUIREMENTS. DETECTOR LOOP(S) SHALL NOT BE INSTALLED IN WET CONDITIONS AND THE SAW-CUTS MUST BE FREE OF DEBRIS AND RESIDUE SUCH AS DUST AND WATER WHICH IS TO BE ACHIEVED BY THE USE OF COMPRESSED AIR, WIRE BRUSHING AND HEAT DRYING ACCORDING TO SEALANT MANUFACTURER REQUIREMENTS. THE DETECTOR WIRE SHALL BE HELD IN PLACE BY THE USE OF FORM WEDGES. WEDGES SHALL BE SPACED NO MORE THAN 18" (450 mm) APART.
- 6. LOOP SPLICES SHALL BE SOLDERED USING A SOLDERING IRON. BLOW TORCHES OR OTHER DEVICES WHICH OXIDIZE COPPER CABLE SHALL NOT BE ALLOWED FOR SOLDERING OPERATIONS. SEE DETAIL BELOW RIGHT.
- 7. PREFORMED DETECTOR LOOPS SHALL BE USED, AS SHOWN ON THE PLANS, WHERE NEW CONCRETE PAVEMENT IS PROPOSED. THE INSTALLATION OF PREFORMED LOOPS SHALL BE IN ACCORDANCE WITH THE DISTRICT 1 SPECIFICATIONS OR AS DIRECTED BY THE ENGINEER.

LOOP LEAD-IN CABLE TAG

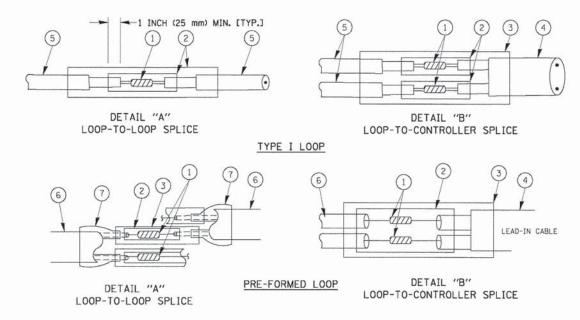


- A. LANE 1 IS THE LANE CLOSEST TO THE CENTERLINE OF THE ROADWAY
- B. LOOP #1 IS THE LOOP IN THE LANE CLOSEST TO THE INTERSECTION.
- C. LABEL LOOP CABLE "IN" OR LOOP CABLE "OUT".
- D. LABEL LOOP CABLE CLOCKWISE OR LOOP CABLE COUNTERCLOCKWISE.



DETECTOR LOOP WIRING SCHEMATIC

- LOOPS SHALL BE SPLICED IN SERIES.
- SAW-CUTS SHALL BE A MINIMUM WIDTH OF 5/16" (8 mm).
- SAW-CUT DEPTHS SHALL BE 3" (75 mm). IF IN CONCRETE, THE SAW-CUT DEPTH SHALL BE TO THE TOP OF THE REINFORCEMENT.
- LOOP CORNERS SHALL BE DRILLED WITH A 2" (50 mm) DIAMETER CORE.



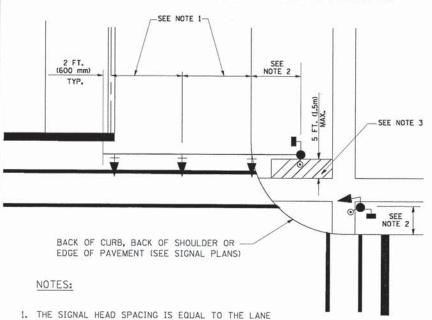
LOOP DETECTOR SPLICE

- $\begin{tabular}{llll} \hline \end{tabular}$ WESTERN UNION SPLICE SOLDERED WITH ROSIN CORE FLUX. ALL EXPOSED SURFACES OF THE SOLDER SHALL BE SMOOTH.
- (2) WCSMW 30/100 HEAT SHRINK TUBE, MINIMUM LENGTH 3" (75 mm), UNDERWATER GRADE.
- 3 WCS 200/750 HEAT SHRINK TUBE, MINIMUM LENGHT 6" (150 mm), UNDERWATER GRADE.
- (4) NO. 14 2/C TWISTED. SHIELDED CABLE.
- (5) LOOP CONDUCTOR WITH FLEXIBLE PLASTIC TUBE.
- 6 PRE-FORMED LOOP
- 7 XL POLYOLEFIN 2 CONDUCTOR BREAKOUT SEALS. TYCO CBR-2 OR APPROVED EQUAL

FILE NAME =	USER NAME = bauerdl	DESIGNED -	DAD	REVISED -			DISTRICT OF	ie.		F.A.P.	SECTION	COUNTY	TOTAL	SHEET
c:\pw_work\PWIDOT\BAUERDL\dØ108315\ts05	dgn	DRAWN -	BCK	REVISED -	STATE OF ILLINOIS					361	06-00214-18-RP	KANF	451	256
	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	DAD	REVISED -	DEPARTMENT OF TRANSPORTATION	STANDARD TRAFFIC SIGNAL DESIGN DETAILS				TS-05		CONTRACT	[NO. 6	3598
	PLOT DATE = 11/4/2009	DATE -	10-28-09	REVISED -		SCALE: NONE SHEET NO. 1 OF 6 SHEETS STA. TO STA.					D DIST. NO. 1 ILLINOIS FED. A	ID PROJECT		

TRAFFIC SIGNAL MAST ARM AND SIGNAL POST

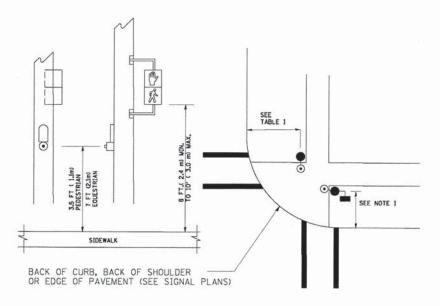
MAST ARM MOUNTED SIGNALS IN EXISTING, PROPOSED OR FUTURE SIDEWALK/BICYCLE PATH AREA. INTERSECTION SHOWN WITH PEDESTRIAN SIGNALS AND PEDESTRIAN PUSHBUTTON DETECTORS.



- WIDTH OR AS SHOWN ON THE TRAFFIC SIGNAL PLAN.

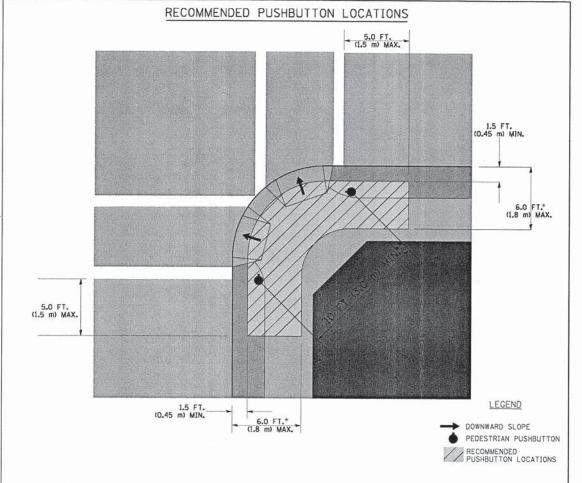
 2. REFER TO THE TRAFFIC SIGNAL EQUIPMENT OFFSET TABLE.
- 3. PROVIDE A LEVEL ALL-WEATHER SURFACE (CONCRETE SIDEWALK, ASPHALT BICYCLE PATH SURFACE OR MATCHING MATERIAL TO THE ADJACENT SURFACE) UP TO THE MAST ARM SHAFT OR THE SIGNAL POST.
- 4. THE FACE OF THE PEDESTRIAN PUSHBUTTON SHALL BE PARALLEL TO THE CROSSWALK TO BE USED.
- 5. THE LOCATIONS AND INSTALLATION OF PEDESTRIAN SIGNAL HEADS AND PEDESTRIAN PUSHBUTTONS SHALL MEET THE REQUIREMENTS OF THE MUTCD AND INFORMATION FOUND IN THE "AMERICANS WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES FOR BUILDINGS AND FACILITIES."

PEDESTRIAN SIGNAL POST AND PEDESTRIAN PUSH BUTTON POST



NOTES:

- 1. REFER TO THE TRAFFIC SIGNAL EQUIPMENT OFFSET TABLE.
- PROVIDE A LEVEL ALL-WEATHER SURFACE (CONCRETE SIDEWALK, ASPHALT BICYCLE PATH SURFACE OR MATCHING MATERIAL TO THE ADJACENT SURFACE) UP TO THE PEDESTRIAN SIGNAL POST OR THE PEDESTRIAN PUSH BUTTON POST.
- THE FACE OF THE PEDESTRIAN PUSHBUTTON SHALL BE PARALLEL TO THE CROSSWALK TO BE USED.
- 4. THE LOCATIONS AND INSTALLATION OF PEDESTRIAN SIGNAL HEADS AND PEDESTRIAN PUSHBUTTONS SHALL MEET THE REQUIREMENTS OF THE MUTCD AND INFORMATION FOUND IN THE "AMERICANS WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES FOR BUILDINGS AND FACILITIES."



- WHERE THERE ARE CONSTRAINTS THAT MAKE IT IMPRACTICAL TO PLACE THE PEDESTRIAN PUSHBUTTON BETWEEN 1.5 FT (0.45 m) AND 6 FT (1.8 m) FROM THE EDGE OF THE CURB, SHOULDER, OR PAVEMENT, IT SHOULD NOT BE FURTHER THAN 10 FT (3 m) FROM THE EDGE OF CURB, SHOULDER, OR PAVEMENT.
- ** WHERE THERE ARE CONSTRAINTS ON A PARTICULAR CORNER THAT MAKE IT IMPRACTICAL TO PROVIDE THE 10 FT (3 m) SEPERATION BETWEEN THE TWO PEDESTRIAN PUSHBUTTONS, THE PUSHBUTTONS MAY BE PLACED CLOSER TOGETHER OR ON THE SAME POLE,

NOTES:

- PEDESTRIAN SIGNAL HEADS SHALL BE MOUNTED WITH THE BOTTOM OF THE SIGNAL HOUSING INCLUDING BRACKETS NOT LESS THAN 8 FT (2.4 m) OR MORE THAN 10 FT (3 m) ABOVE SIDEWALK LEVEL, AND SHALL BE POSITIONED AND ADJUSTED TO PROVIDE MAXIMUM VISIBILITY AT THE BEGINNING OF THE CONTROLLED CROSSWALK.
- 2. THE BOTTOM OF THE SIGNAL HOUSING (INCLUDING BRACKETS) OF A VEHICULAR SIGNAL FACE THAT IS NOT LOCATED OVER A HIGHWAY SHALL BE AT LEAST 8 FT (2.4 m) BUT NOT MORE THAN 19 FT (5.8 m) ABOVE THE SIDEWALK OR, IF THERE IS NO SIDEWALK, ABOVE THE PAVEMENT GRADE AT THE CENTER OF THE ROADWAY.
- 3. THE BOTTOM OF THE SIGNAL HOUSING AND ANY RELATED ATTACHMENTS TO A SIGNAL FACE LOCATED OVER ANY PORTION OF A HIGHWAY SHALL BE ACCORDING TO CURRENT STATE STANDARDS 877001, 877002, 877006, 877011 AND 877012 WITH A MINIMUM OF 16 FT (5.0 m) AND A MAXIMUM OF 18 FT. (5.5 m) FROM THE HIGHEST POINT OF PAVEMENT.
- 4. THE BOTTOM OF THE TEMPORARY SPAN WIRE MOUNTED SIGNAL HOUSING AND ANY RELATED ATTACHMENTS TO A SIGNAL FACE LOCATED OVER ANY PORTION OF A HIGHWAY SHALL BE ACCORDING TO CURRENT STATE STANDARD 880001 WITH A MINIMUM OF 17 FT (5.18 m) FROM THE HIGHEST POINT OF PAVEMENT.
- 5. THE TOP OF THE SIGNAL HOUSING OF A SIGNAL FACE LOCATED OVER ANY PORTION OF A HIGHWAY SHALL NOT BE MORE THAN 25.6 FT (7.8 m) ABOVE THE PAVEMENT.

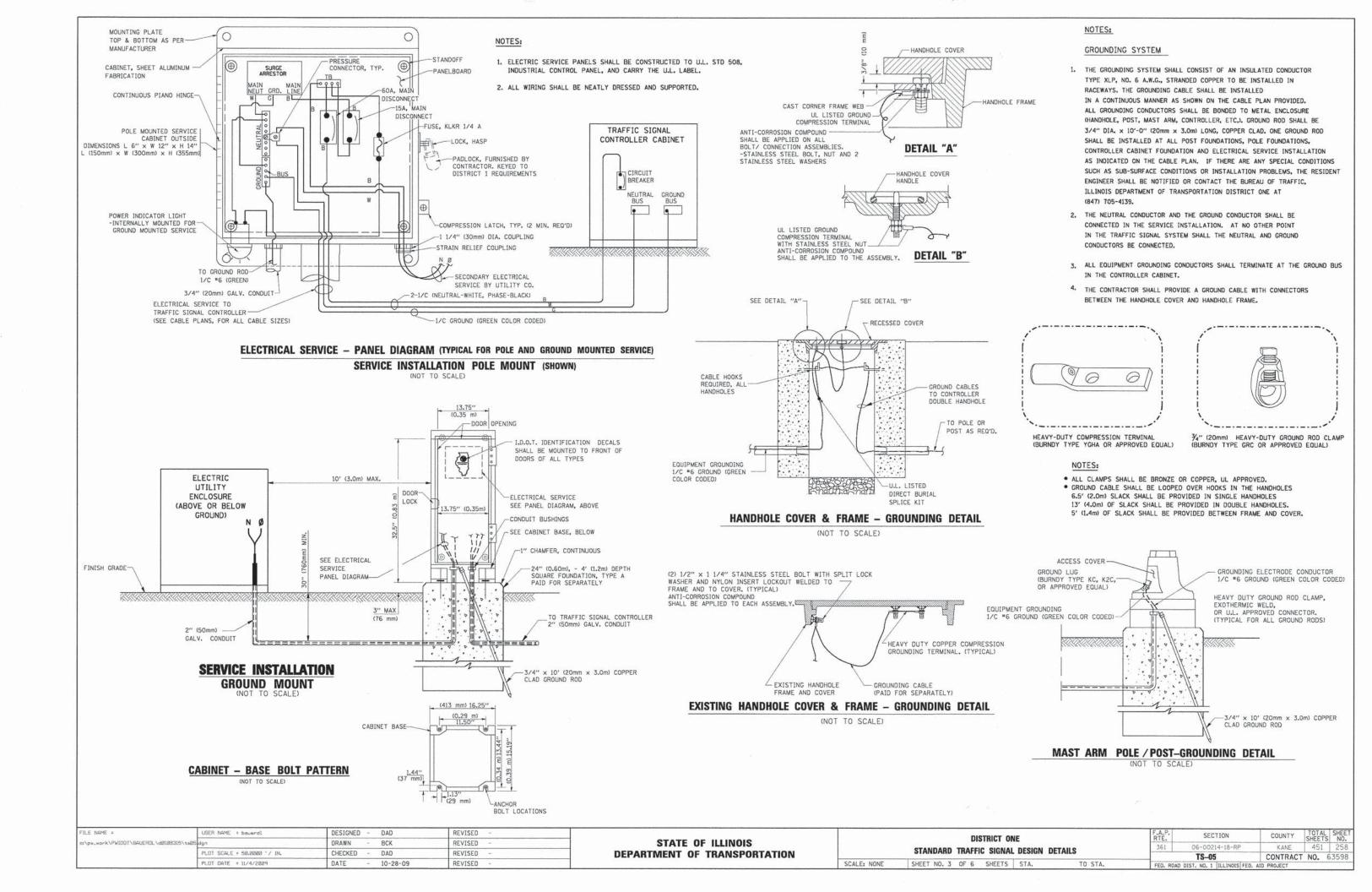
TRAFFIC SIGNAL EQUIPMENT OFFSET

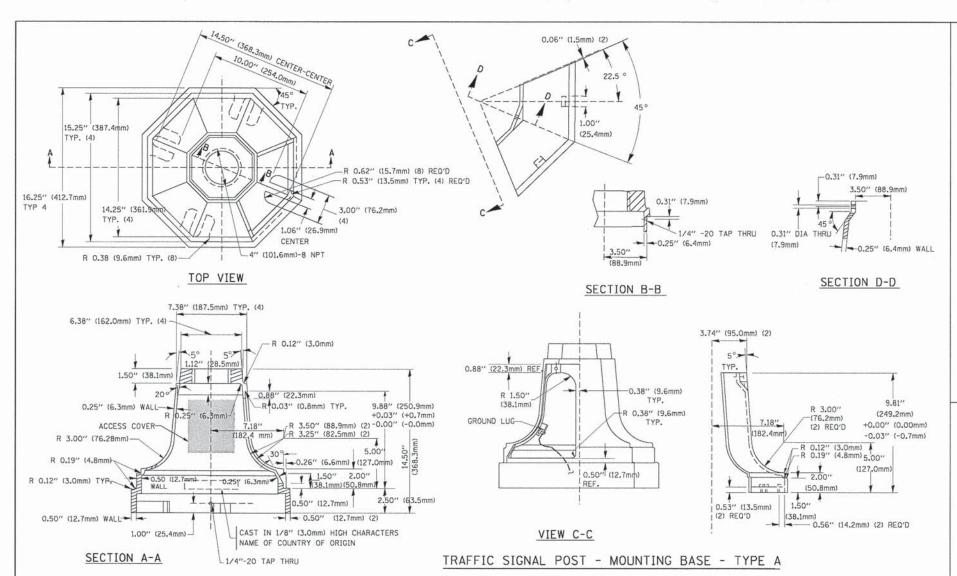
TRAFFIC SIGNAL EQUIPMENT	COMBINATION CONCRETE CURB AND GUTTER (MINIMUM DISTANCE FROM BACK OF CURB TO CENTERLINE OF FOUNDATION)	SHOULDER/NON-CURBED AREA (MINIMUM DISTANCE FROM EDGE OF PAVEMENT TO CENTERLINE OF FOUNDATION)
TRAFFIC SIGNAL MAST ARM POLE	6 FT (1.8m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
TRAFFIC SIGNAL POST	4 FT (1.2m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
PEDESTRIAN SIGNAL POST	4 FT (1.2m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
PEDESTRIAN PUSHBUTTON POST	4 FT (1.2m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
TEMPORARY WOOD POLE	6 FT (1.8m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
CONTROLLER CABINET	6 FT (1.8m) MINIMUM DISTANCE SEE NOTE 2	SHOULDER WIDTH + 6 FT (1.8m), MINIMUM 16 FT (4.9m) SEE NOTE 3.
SERVICE INSTALLATION, GROUND MOUNT	6 FT (1.8m) MINIMUM DISTANCE SEE NOTE 2	SHOULDER WIDTH + 6 FT (1.8m), MINIMUM 16 FT (4.9m) SEE NOTE 3.

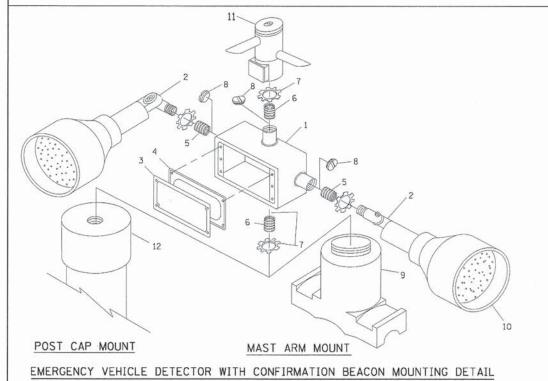
NOTES:

- CONTACT THE "AREA TRAFFIC SIGNAL MAINTENANCE AND OPERATIONS ENGINEER" FOR ASSISTANCE IN LOCATING THE TRAFFIC SIGNAL EQUIPMENT WHEN THERE ARE CONFLICTS WITH DITCHES OR THE MINIMUM OFFSET DISTANCES CANNOT BE MET.
- 2. MINIMUM DISTANCE FROM THE BACK OF CURB TO THE ROADWAY SIDE OF THE FOUNDATION.
- 3. MINIMUM DISTANCE FROM THE EDGE OF PAVEMENT TO THE ROADWAY SIDE OF THE FOUNDATION.
- 4. ANY CHANGES TO THE OFFSETS OF THE FOUNDATIONS, FROM THE MINIMUM DISTANCES LISTED IN THE "TRAFFIC SIGNAL EQUIPMENT OFFSET" CHART AND THE TRAFFIC SIGNAL INSTALLATION PLAN, COULD EFFECT THE PLACEMENT OF THE SIGNAL HEADS, PEDESTRIAN SIGNAL HEADS AND THE PEDESTRIAN PUSHBUTTONS. THE SIGNAL HEAD PLACEMENT ON THE MAST ARMS SHALL REMAIN AS PER THE TRAFFIC SIGNAL INSTALLATION PLAN AND THE "TRAFFIC SIGNAL MAST ARM AND SIGNAL POST" DETAIL ABOVE. THE PROPOSED MAST ARM LENGTHS MAY NEED TO BE REVISED TO MEET THE ABOVE REQUIREMENTS. THE PEDESTRIAN SIGNAL HEADS AND PEDESTRIAN PUSHBUTTONS MUST MEET THE REQUIREMENTS UNDER THE DETAILS ON THIS SHEET.

- LA DEHODE AND DE LA MINOS	OSEN WHITE = Develop	DESIGNED - DAD	KENIZED -			DISTRICT ONE	F.A.P.	SECTION	COUNTY	TOTAL	SHEET
CI/DW-MORK /LMIDOL/REGIENDE/40/10831	D\tsl/D dgn	DRAWN - BCK	REVISED -	STATE OF ILLINOIS			INIE.			SHEET	, NU.
	PLOT SCALE = 50.0000 1/ IN.	CHECKED - DAD	REVISED -	DEPARTMENT OF TRANSPORTATION	STANDARD TRAFFIC SIGNAL DESIGN DETAILS			06-00214-18-RP	KANE	451	257
	PLOT DATE = 11/4/2809	DATE - 10-28-09	REVISED -	DEFAITMENT OF THANSFORTATION	CON C. NONE	SHEET NO 2 OF 6 SHEETS STA TO STA		TS-05	CONTRACT	T NO.	63598
		10 20 03	110100		SCALE: NONE	SHEET NO. 2 OF 6 SHEETS STA. TO STA.	FED. RO	DAD DIST. NO. 1 ILLINOIS FED.	AID PROJECT		1.000



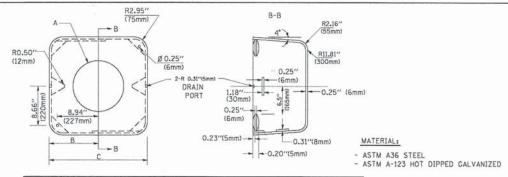




ITEM	NO. IDENTIFICATION
1	OUTLET BOX- GALV. 21 CU.IN. (0.000344 CU-M)
2	LAMP HOLDER AND COVER
3	OUTLET BOX COVER
4	RUBBER COVER GASKET
5	REDUCING BUSHING
6	3/4"(19 mm) CLOSE NIPPLE
7	3/4"(19 mm) LOCKNUT
8	3/4"(19 mm) HOLE PLUG
9	SADDLE BRACKET - GALV.
10	6 WATT PAR 38 LED FLOOD LAMP
11	DETECTOR UNIT
12	POST CAP [18 FT. (5.4 m) POST MIN.]

NOTES:

- ALL ELECTRICAL ITEMS, EXCEPT ITEMS "2 AND "11 SHALL BE ALUMINUM OR GALVANIZED
- 2. ITEM #1- OZ/GEDNEY FSX-1-50 OR EQUIVALENT
 ITEM #2- MULBERRY CON-O-SHADE LAMP SHIELD OR EQUIVALENT
 ITEM #9- "BAND-IT" SADDLE BRACKET OR EQUIVALENT
- 3. WHEN POST MOUNTING IS SPECIFIED, ITEM *9 SHALL NOT BE REQUIRED. THE DETECTION UNIT SHALL BE MOUNTED DIRECTLY ON TOP OF THE CAP BY DRILLING AND TAPPING A 3/4"(19 mm) HOLE WITH PIPE THREADS. THE POST CAP SHALL EITHER BE SCREWED TO THE TOP OF THE POST OR A MINIMUM OF 3 TIGHTENING SCREWS SHALL BE REQUIRED ON EACH CAP.

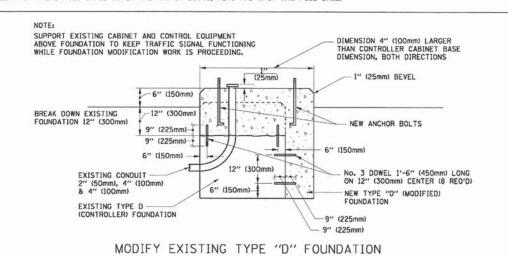


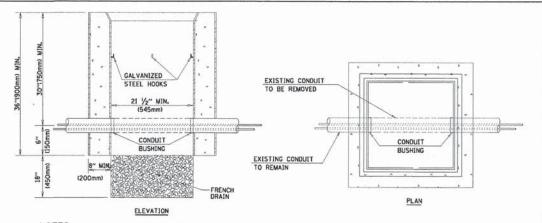
Α	В	С	HEIGHT	WEIGHT
VARIES	9.5"(241mm)	19"(483mm)	7" (178mm) - 12" (300mm)	53 lbs (24kg)
VARIES	10.75"(273mm)	21.5"(546mm)	7" (178mm) - 12" (300mm)	68 lbs (31 kg)
VARIES	13.0"(330mm)	26"(660mm)	7" (178mm) - 12" (300mm)	81 lbs (37 kg)
VARIES	18.5"(470mm)	37"(940mm)	7" (178mm) - 12" (300mm)	126 lbs (57 kg)

SHROUD

NOTES:

- DIMENSION "A" IS EQUAL TO THE DIAMETER OF THE MAST ARM POLE AT THE TOP OF THE SHROUD.
 THE SHROUD SHALL BE TIGHT TO THE MAST ARM POLE.
- 2. THE SUPPLIER SHALL VERIFIED THE ABOVE DIMENSIONS BASED ON MAST ARM REQUIREMENTS.
- 3. THE HEIGHT OF THE SHROUD SHALL COVER THE ANCHOR BOLTS, NUTS AND MAST ARM POLE BASE.





NOTES:

SCALE: NONE

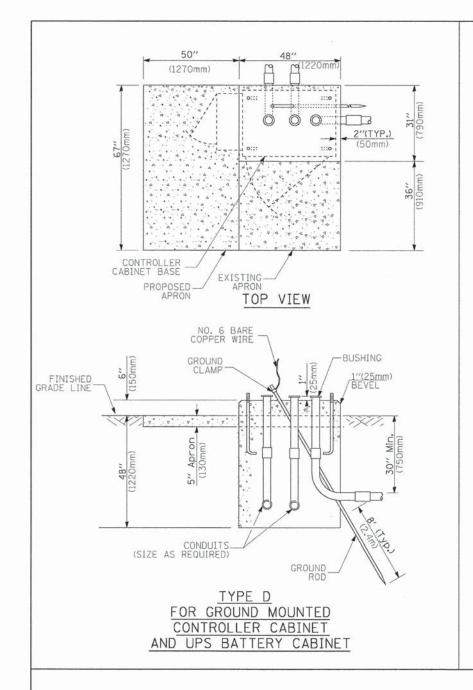
- 1. HANDHOLE CONSTRUCTED PER STATE STANDARD 814001.
- 2. REMOVAL OF THE EXISTING CONDUIT FROM THE HANDHOLE AND THE INSTALLATION OF THE CONDUIT BUSHINGS SHALL BE INCIDENTAL TO THE HANDHOLE.

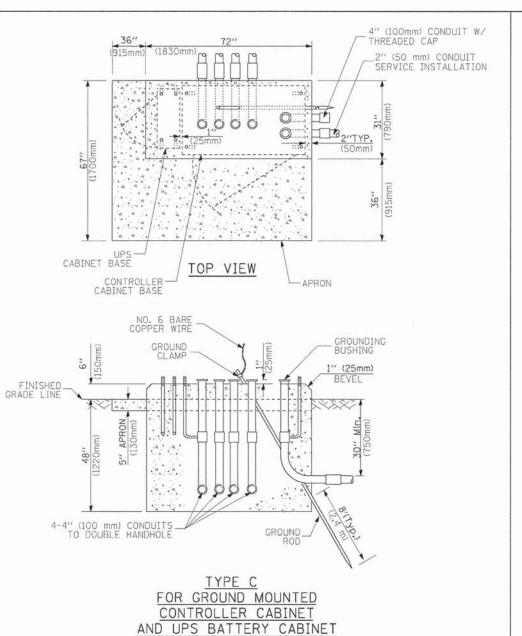
HANDHOLE TO INTERCEPT EXISTING CONDUIT

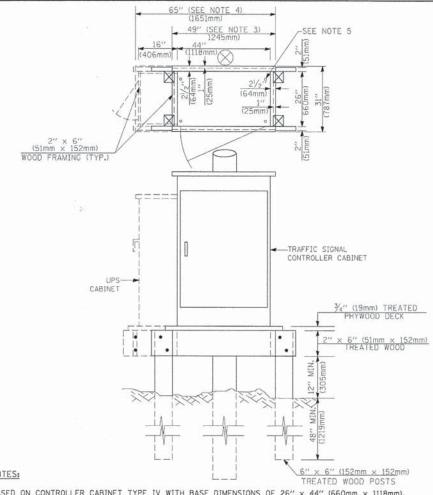
FILE NAME =	USER NAME = bouerd1	DESIGNED	-	DAD	REVISED	741	
c:\pw_work\PWIDDT\BAUERDL\dØ108315\tsØ5	dgn	DRAWN	-	BCK	REVISED	TQ:	
	PLDT SCALE = 50.0000 '/ IN.	CHECKED	1	DAD	REVISED	(#)	
	PLOT DATE = 11/4/2009	DATE	-	10-28-09	REVISED	-	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DISTRICT ONE	F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
STANDARD TRAFFIC SIGNAL DESIGN DETAILS	361	06-00214-18-RP	KANE	451	259
STANDARD TRAFFIC SIGNAL DESIGN DETAILS		TS-05	CONTRACT	NO.	63598
SHEET NO. 4 OF 6 SHEETS STA. TO ST	A. FED BOA	D DIST NO 1 THE INOIS SED	AID DDO IECT		







- BASED ON CONTROLLER CABINET TYPE IV WITH BASE DIMENSIONS OF 26" x 44" (660mm x 1118mm).
 ADJUST PLATFORM SIZE TO FIT CABINET BASE DIMENSIONS BEING SUPPLIED.
- BASED ON UNINTERRUPTIBLE POWER SUPPLY CABINET WITH BASE DIMENSIONS OF 16" x 25" (406mm x 635mm).
 ADJUST PLATFORM SIZE TO FIT CABINET BASE DIMENSIONS BEING SUPPLIED.
- 3. PLATFORM SIZE FOR CONTROLLER CABINET TYPE IV.
- 4. PLATFORM SIZE FOR CONTROLLER CABINET TYPE IV AND UNINTERRUPTIBLE POWER SUPPLY CABINET.
- 5. DRILLED HOLES THROUGH THE PLATFORM BASE TO MATCH THE CONTROLLER CABINET BOLT TEMPLATE. FASTEN THE CONTROLLER CABINET TO THE PLATFORM WITH CARRIAGE BOLTS, WASHERS AND NUTS.
- 6. FASTEN ALL SUPPORT WOOD FRAMING TO THE WOOD POSTS WITH 2 LAG SCREWS FOR EACH CONNECTION.

TEMPORARY SIGNAL CONTROLLER WOOD SUPPORT PLATFORM

CABLE SLACK LENGTH	FEET	METER
HANDHOLE	6.5	2.0
DOUBLE HANDHOLE	13.0	4.0
SIGNAL POST	2.0	0.6
MAST ARM	2.0	0.6
CONTROLLER CABINET	1.5	0.5
FIBER OPTIC AT CABINET	13.0	4.0
ELECTRIC SERVICE AT (CABINET OR SERVICE LOCATION)	1.5	0.5
GROUND CABLE (SIGNAL POST, MAST ARM, CABINET)	1.5	0.5
GROUND CABLE (BETWEEN FRAME AND COVER)	5.0	1.6

CABLE SLACK

VERTICAL CABLE LENGTH	FEET	METER
MAST ARM POLE (MAST ARM MOUNTED SIGNAL HEAD)		
(L = MAST ARM LENGTH - DISTANCE TO SIGNAL HEAD FROM END OF ARM)	20.0+L	6.0+L
BRACKET MOUNTED (MAST ARM POLE OR SIGNAL POLE)	13.0	4.0
PEDESTRIAN PUSH BUTTON	6.0	2.0
SERVICE INSTALLATION POLE MOUNT TO SERVICE DROP	13.5	4.1
SERVICE INSTALLATION POLE MOUNT TO GROUND	13.5	4.1
SERVICE INSTALLATION GROUND MOUNT	6.0	2.0
FOUNDATION (SIGNAL POST, MAST ARM POLE, CONTROLLER CABINET, SERVICE-GROUND MOUNT)	3.0	1.0

DEPTH OF FOUNDATION VERTICAL CABLE LENGTH

FOUNDATION	DEPTH
TYPE A - Signal Post	4'-0" (1.2m)
TYPE C - CONTROLLER W/ UPS	4'-0" (1.2m)
TYPE D - CONTROLLER	4'-0" (1.2m)
SERVICE INSTALLATION, GROUND MOUNT, TYPE A - SQUARE	4'-0" (1.2m)

Mast Arm Length	① Foundation Depth	Foundation Diameter	Spiral Diameter	Quantity of Rebars	Size of Rebars
Less than 30' (9.1 m)	10'-0" (3.0 m)	30" (750mm)	24" (600mm)	8	6(19)
Greater than or equal to	13'-6" (4.1 m)	30" (750mm)	24" (600mm)	8	6(19)
30' (9.1 m) and less than 40' (12.2 m)	11'-0" (3.4 m)	36" (900mm)	30" (750mm)	12	7(22)
Greater than or equal to 40' (12.2 m) and less than 50' (15.2 m)	13'-0" (4.0 m)	36" (900mm)	30" (750mm)	12	7(22)
Greater than or equal to 50' (15.2 m) and up to 55' (16.8 m)	15'-0" (4.6 m)	36" (900mm)	30" (750mm)	12	7(22)
Greater than or equal to 56' (16.8 m) and less than 65' (19.8 m)	21'-0" (6.4 m)	42" (1060mm)	36" (900mm)	16	8(25)
Greater than or equal to 65' (19.8 m) and up to 75' (22.9 m)	25'-0" (7.6 m)	42" (1060mm)	36" (900mm)	16	8(25)

- These foundation depths are for sites which have cohesive soils (clayey sit, sandy clay, etc.) along
 the length of the shaft, with an average Unconfined Compressive Strength (Qu) > 1.0 tsf (100 kpa).
 This strength shall be verified by boring data prior to construction or with testing by the Engineer
 during foundation drilling. The Bureau of Bridges & structures should be contacted for a revised
 design if other conditions are encountered.
- 2. Combination mast arm assemblies under 55 feet (16.8 m) shall use 36" (900 mm) diameter foundations.
- 3. Combination mast arm assemblies under 56 feet (16.8 m) through 75 feet (22.9 m) shall use 42" (1060 mm) diameter foundations.
- 4. For most arm assemblies with dual arms refer to state standard 878001.

DEPTH OF MAST ARM FOUNDATIONS, TYPE E

- 48												
	FILE NAME =	USER NAME = bauerdl	DESIGNED - DAG	REVISED -			DISTRICT ONE	F.A.P.	SECTION	COUNTY	TOTAL S	HEET
- 1	Ot New Work \PVIDOT\BAUFBOL\d0108315\±x05	dan	DRAWN - BCK	REVISED -	STATE OF ILLINOIS		DISTRICT ONE	MIE.			SHEETS	NO.
- 1	3, 3, 2, 2, 3, 4, 4, 4, 4, 4, 4, 4, 4, 4, 4, 4, 4, 4,	991	5000	TILTAGED	에 가지 3 MB 12 MB 1		CTANDARD TRAFFIC CICHAL DECICAL DETAILS	361	06-00214-18-RP	KANE	451	260
- 1		PLOT SCALE = 50.0000 '/ IN.	CHECKED - DAD	REVISED -	DEPARTMENT OF TRANSPORTATION	STANDARD TRAFFIC SIGNAL DESIGN DETAILS			TS-05	CONTRACT	NO GT	E00
- 1	1	CALCULAR DEPOSIT OF A CONTRACTOR	D175 10 00 00	net recen					13-03	CONTRACT	NO. 03	220
- 1		PLDT DATE = 11/4/2009	DATE - 10-28-09	REVISED -		SCALE: NONE	SHEET NO. 5 OF 6 SHEETS STA. TO STA.	FFD ROA	AD DIST NO 1 THE INDIS EED AT	O PROJECT		1

GENERAL NOTES:

- I. All construction, workmanship and materials shall be in accordance with the Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, dated January 1, 2012, and the 2011 American Railway Engineering and Maintenance-of-way Association (AREMA) manual for Railway Engineering. In the event of conflicts between specifications, the more restrictive will apply.
- The proposed superstructure and substructure are designed in accordance with the 2011 AREMA Manual, Chapter 8 - Concrete Structures and Foundations, and Chapter 15, Steel Structures.
- 3. The proposed superstructure and substructure have been designed for Cooper E80 Live Loading plus Impact and 30 inches total depth of ballast, (or Alternate Live Load where applicable).
- All information shown on these plans regarding location of the existing track and existing ground elevations are based on information provided by survey including preliminary general arrangement.

DIVISION OF RESPONSIBILITY:

- A. Railroad
- Remove rail and other track material as required, and perform cut-overs for proposed track and Temporary Shoo-fly track.
- 2. Provide and install bridge marker signs at each end of bridge.
- B. Contractor
- 1. Coordinate all construction activities with the railroad.
- Provide and install new ballast, ties, rail and OTM for proposed track and Temporary Shoo-fly track.
- 3. Before ordering any material, the contractor shall make a detailed field inspection of the site, verifying all pertinent dimensions and elevations. Any variations in dimensions or elevations from those shown on the plans shall be reported immediately to the Engineer.
- 4. Verify the location, relocation, abandonment, and/or temporary support of all utilities affected by the construction of the structure and embankment and coordinate these activities with the appropriate utility companies, agencies and/or authorities. For information on, and relocation of, fiber optic cable, call 1-800-336-9193.
- 5. Apply for and obtain all construction permits necessary to perform the work.
- Provide the railroad with a detailed construction plan defining the activity, schedule and procedure for each aspect of the work, Construction shall not begin until the construction plan has been approved by the railroad.
- 7. Supply all material and perform all work not supplied or performed by the railroad.
- 8. Provide all temporary shoring and/or bracing required to support and protect the existing embankments and track affected by the work. Provide the railroad with details, design and procedure for all temporary shoring and/or bracing. All temporary shoring and bracing shall be designed, signed and sealed by a Structural Engineer licensed in the State of Illinois. All temporary shoring and bracing must be submitted to and approved by the UPRR AVP Design and Construction Design prior to beginning construction. The provisions of UPRR Guidelines for Temporary Shoring shall be met.
- Construction submittals to be designed per UPRR and AREMA requirements and reviewed by the Engineer and UPRR.
- 10. Accomplish activities within the schedule specified in the approved construction plan.

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601

CAST-IN-PLACE CONCRETE NOTES:

- All concrete material, placement and workmanship shall be in accordance with Chapter 8 of the 2011 edition of AREMA.
- 2. Minimum compressive strength 4000 lb. per square inch at 28 days.
- 3. Exposed surfaces shall be formed in a manner that will produce a smooth and uniform appearance without rubbing or plastering. Exposed edges of 90 degrees or less are to be chamfered ³₄" x ³₄". Top surface to have a smooth finish, free of all float or trowel marks with the exception that a broom finish be used on all walkway surfaces.
- 4. Concrete shall be proportioned such that the water-cementitious material ratio (by weight) does not exceed the values in AREMA Table 8-1-9. Cast-in-place concrete must contain a minimum of 565 pounds of cementitious material per cubic yard of concrete. If fly ash is used with cement it shall be limited to 15% of cementitious material.
- 5. Cement shall be Type I, II or III Portland Cement per ASTM C150.
- 6. Course aggregate shall be Size No. 67.
- 7. Fine aggregate shall be natural sand.
- 8. Aggregates shall be graded in accordance with ASTM C33.
- 9. Air content shall be between 5% and 7% (by volume).
- 10. Admixtures, other than air entrainment, shall not be used without approval by the Engineer,
- 11. Membrane curing compound shall conform to ASTM C309 Type 2.
- Apply Thoroc Epoxy Adhesive 24LPL or approved alternate before placing new concrete against hardened surfaces.
- 13. Construction joints are permitted only where shown on the drawings.
- 14. Concrete for permanent bridge shall have an architectural treatment as shown on the plans.
- 15. The Contractor shall use self-consolidating concrete (SCC) for all bent encasement concrete.

 The self-consolidating concrete shall conform to all requirements as specified in the Special Provisions. Cost of SCC shall be included with the cost of Concrete Structures (Special).
- 16. The Contractor shall provide adequate forms to contain the increased hydraulic pressure of the self-consolidating concrete.

REINFORCING STEEL

- Reinforcing steel for abutment, bents and wingwalls for the permanent bridge shall be epoxy-coated, deformed, new billet bars per current ASTM A615 specifications and meet the requirements of AREMA Section 1.6. Reinforcement bars designated (E) shall be epoxy coated.
- Reinforcing steel for the temporary bridge shall be plain (black), deformed, new billet bars per current ASTM A615 specifications and meet the requirements of AREMA Section 1.6.
- Reinforcing steel requiring field welding or bending shall conform to ASTM A706
 Specifications, Grade 60. Fabrication of reinforcing steel shall be per Chapter 7 of the
 CRSI Manual of Standard Practice. Dimensions of bending details shall be out to out of bars.
- 4. Reinforcing steel is to be blocked to proper location and securely wired against displacement. Tack welding of reinforcing is prohibited. Minimum concrete cover not otherwise noted shall meet 2011 AREMA requirements.

PRECAST CONCRETE NOTES:

- All concrete material, placement and workmanship shall be in accordance with Chapter 8 of the 2011 edition of AREMA.
- 2. Minimum compressive strength 5,000 lb. per square inch at 28 days.
- 3. Exposed surfaces shall be formed in a manner that will produce a smooth and uniform appearance without rubbing or plastering. Exposed edges of 90 degrees or less are to be chamfered ³₄" x ³₄". Top surface to have a smooth finish, free of all float or trowel marks.
- 4. Concrete shall be proportioned such that the water-cementitious material ratio (by weight) does not exceed the values in AREMA Table 8-1-9. Precast concrete must contain a minimum of 610 pounds of cementitious material per cubic yard of concrete.
- 5. Cement shall be either Type I or Type III Portland Cement.
- 6. Aggregates shall be graded in accordance with ASTM C33.
- 7. Coarse aggregate shall be Size No. 67.
- 8. Fine aggregate shall be natural sand.
- 9. Air content shall be between 5% and 7% (by volume).
- 10. Admixtures shall not be used without approval by the Engineer.
- ${\it II.}$ Curing shall be accomplished by wet curing or the application of a Type 2 Membrane.
- 12. For precast elements, the fabricator shall stencil the fabricator's name, date of fabrication, the bridge number, lifting weight and piece mark on each component.
- 13. The production facility must be pre-certified. Production procedures for the manufacture of precast members shall be in accordance with AREMA and the current edition of the Precast Concrete Institute's Manual MNL 116 for quality control.
- 14. Dimensional tolerances governing the manufacture of precast members shall conform to Division VI, Section 6.4.6 of the Precast Concrete Institute's Manual MNL 116 for quality control. Tolerance for location of lifting devices shall be ±1/2".
- 15. The Fabricator will be responsible for the design of the lifting loops or lift anchors for the erection of the precast members. Required details to be coordinated with the Contractor and approved by the Engineer. The area around all lifting loops shall be recessed so that the loops can be removed to a depth of 3/4" and grouted. Properly designed lift anchors are acceptable in lieu of lifting loops.
- 16. The fabricator will be responsible for the loading and properly securing the precast concrete members for shipment. All concrete components shall be made available, at the railroad's discretion, for inspection by the Engineer-of-Record and the railroad at the fabricator's plant prior to shipment.
- 17. For notes regarding Precast/Prestressed Fascia Beam, see Sheet UP-31.

benesch

Perm_Br_802_Constnotesl.don

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168 CONSTRUCTION NOTES (1 OF 2)

SHEET NO. UP-2 OF UP-52 SHEETS

STRUCTURAL STEEL NOTES:

Structural Steel: ASTM A709, Grade 36

DESIGN STRESSES:

Design stresses for the following materials are in accordance with AREMA. Structural Steel: ASTM A709, 50W Fy = 50,000 psi

DESIGN LOADING:

* Live Load, Longitudinal Beams = Cooper E-80 or Alternative Live Load Chapter 15
Impact = 2011 AREMA Chapter 15
Chapter 15
Other Loads = 2011 AREMA Chapter 15
Ballast = 30" deep (accounts for future raises)

Fy = 36.000 psi

* Beams were analyzed and designed to satisfy deflection criteria under E-80 or Alternate Loading, Impact and Dead Loads without any composite action.

STRUCTURAL STEEL:

- 1. Fabrication and shop assembly shall comply with 2011 AREMA Chapter 15, Part 3: "Fabrication".
- Fabrication of structural steel shall be performed by a fabricator certified under AISC Certification Program, Category CBR, Major Steel Bridges, Fracture Critical Endorsement.
- 3. Structural steel shall conform to the following requirements:

Permanent Bridge:	
Beams	ASTM A709, Grade 50WT2
Ballast curbs, diaphragms, diaphragm connectors,	
deck support brackets and sole plates	ASTM A588
Deck plates, cover plates, backwall plates and	
flashing plates	ASTM A709, Grade 36
Fence posts	ASTM A500 Grade B
Anchor rods (anchor bolts, swedge bolts)	ASTM F1554, Grade 36
Embedded structural steel and sole plates	
(Fascia Beam)	ASTM A588
Anchor Bolts (Fascia Beam)	ASTM F1554, Grade 105
Temporary Bridge:	
Beams, diaphragms, stiffeners and sole plates.	ASTM A709, Grade 36
Handrail	ASTM A709, Grade 36
Anchor Bolts	ASTM F1554, Grade 36
Pile Plates	ASTM A36

4. Calculated weight of Structural Steel =

Permanent Bridge:	
ASTM A709, Grade 50WT2	336.000 lbs.
ASTM A588	70.035 lbs.
ASTM A36	
ASTM F1554 Grade 36	
Misc. Steel (Drain Pipe)	76 lbs.
Total =	499,840 lbs.
Temporary Bridge:	
ASTM A709, Grade 36	114.720 lbs.
ASTM F1554 Grade 36	
ASTM A709, Grade 36 (Handrail)	
ASTM A709, Grade 36 (Pile Plates)	9,170 lbs.
ASTM A709, Grade 36 (Bent bracing)	2,892 lbs.
ASTM A709, Grade 36 (Misc. Steel)	
Total =	128,406 lbs.

- Structural steel shall be of the type and quality as designated on the plans. Material supplied shall meet the longitudinal Charpy V-notch requirements as specified in the AREMA provisions.
- 6. The wide flange beams for the permanent and temporary structures are noted as conforming to the supplemental requirements for non-fracture critical impact test requirements noted in Table 1-2, Section 1.2.1, Chapter 15 of the AREMA specifications for Zone 2 service temperature. These components are noted on the plans as notch toughness requirements (N.T.R.).
- 7. All shop and field bolted connections shall use high strength bolts (including nuts and washers) conforming to ASTM A325 Type 3. All bolts shall be \$^3_4\$" diameter unless otherwise noted. Holes for bolts shall be \$^1_6\$" diameter larger than nominal bolt diameter unless otherwise noted. All high strength steel bolts shall have one hardened steel washer per bolt under the element to be turned and shall be installed using the "turn of the nut method". Alternative bolt fastening methods are subject to approval of the Engineer.

STRUCTURAL STEEL NOTES:

- 8. Bolts shall be placed so that the bolt heads are on the outside (exposed) surface of member unless shown otherwise on the plans. Threads shall be excluded from the shear plane in all connections.
- 9. Center to center dimensions for all bolts shall be as noted.
- 10. Machine bolts for shipment shall be ASTM A307.
- II. Welding: All welding shall be in accordance with the current edition of the Bridge Welding Code, AWS D1.5 and shall also conform to the applicable provisions of AREMA, Chapter 15. Welding to be allowed only as shown on design drawings and approved shop drawings.
- 12. Welded joints are to be AWS prequalified. All welding shall be done to minimize distortion. The welding sequence and procedures to be used shall be submitted for approval to the Engineer.
- 13. Fully automatic submerged arc welding shall be required for this project. Manual shielded arc welding or semiautomatic submerged arc welding will be allowed only if fully automatic submerged arc welding is not practical.
- 14. The fabricator shall submit copies of welder's certificates for semi-automatic welding process. If overhead welding is planned during fabrication, certificates for overhead semi-automatic welding process shall also be submitted to the Engineer.
- 15. In addition to the requirements of AREMA Chapter 15, Sections 1.14 and 3.5, nondestructive testing of welds shall be performed in accordance with the detailed specifications.
- 16. Grinding shall be in the longitudinal direction.
- 17. The fabricator shall submit three (3) copies of detailed shop drawings prior to beginning fabrication. Fabrication shall not begin until shop drawings are approved.
- 18. The fabricator shall shop assemble the beam span prior to shipping. During assembly and reaming all bolts shall be placed in holes as the work progresses to assure proper fit.
- 19. Shop assembled span shall be inspected by the Engineer before the steel is disassembled and shipped to the erection site.
- 20. All steel components shall be inspected by the Engineer before shipment.

WATERPROOFING:

- Waterproofing and protective asphalt panels shall comply with the recommendations of Chapter 8, Part 29 of AREMA. The waterproofing shall consist of one layer of 332" Butyl Rubber Membrane waterproofing system. Protective asphalt panels shall be placed in two - ½" layers with total thickness not less than 1" and shall be laid with joints staggered. Protective asphalt panels shall be bonded to the membrane and each other.
- Four inches of ballast shall be placed over waterproofing and protective asphalt panels immediately upon acceptance by the Engineer. No construction traffic is allowed on waterproofing until the ballast covering is in place. Waterproofing installation shall be observed and approved by the manufacturer's representative.

PILE DRIVING

- All piles shall be driven to the capacity shown on the plans. If any numbered pile cannot be driven to the capacity shown, the Engineer must be notified.
- Estimated capacity of driven piles shall be calculated using the Modified ENR Formula, with F.S. = 5 unless otherwise noted. Qualified Contractor QC personnel to verify nominal bearing capacity of piles and submit to the Engineer. Direct questions to the Engineer.
- 3. In lieu of the hammer selection criteria and use of the Modified ENR Formula, with F.S. = 5 specified in Section 02455 of the Specifications, the contractor shall conduct a wave equation analysis to establish the driving criteria at all pile foundations which specify a nominal required bearing above 600 kips. The analysis and calculations shall be submitted to the Engineer for approval.
- 4. The contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at the substructures specified or approved by the Engineer before ordering the remainder of piles.
- Mark every pile with a dimension indicating the pile depth from cut-off to point of the pile.
 The dimension should be rounded to the nearest foot.
- 6. The mark shall be welded on to outside face, low mile post side on the pile face, approximately 1' below the bottom of the cap, and in numbers of approximately 3" in height. If pile is not exposed, no mark is required.
- 7. All steel H-piles shall conform to ASTM A572, Grade 50.

FIELD WELDING

- 1. Welding must be accomplished with the SMAW or FCAW process. Welding must be in compliance with the requirements specified in AWS D1.5, except $^{5}_{16}$ " fillet welds may be made with a single pass. Welding electrodes must be E7018 for SMAW or E70T-1 or E-70T-5 for FCAW.
- Welders must possess valid certification. Welding must be performed by operators who have been qualified previously by tests as prescribed by the American Welding Society's standard qualification procedure to perform the work required. The qualifications of the personnel must be submitted to the Engineer in advance of the work.

MISCELLANEOUS STEEL SPECIFICATIONS:

- 1. Design and Workmanship Per 2011 AREMA Manual for Railway Engineering.
- 2. Miscellaneous Steel Per current ASTM A588 specifications unless otherwise noted.
- 3. Steel Coating None.
- Welding Arc process per 2011 AREMA Manual for Railway Engineering and AWS D1.1 Structural Welding Code.

MISCELLANEOUS MATERIAL NOTES:

- 1. Bearing pads for beam spans shall be polyurethane pads (80 Durometer).
- 2. Bearing pads shall be shipped flat.
- Bearing pads shall meet the requirements of Table 15-10-2 of the AREMA Manual for Railway Engineering.

benesch

erm_Br_003_Constnotes2.dgn

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450 Joh No. 10074

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

M.P. 37.71 BELVIDERE SUBDIVISION – STRUCTURE NO. 045–3168
CONSTRUCTION NOTES (2 OF 2)

F.A.P. RTE. SECTION COUNTY SHEETS NO. 168
INSTRUCTION NOTES (2 OF 2)

SHEET NO. UP-3 OF UP-52 SHEETS

F.A.P. RTE. SECTION COUNTY SHEETS NO. 169
361 06-00214-18-RP KANE 451 263
CONTRACT NO. 63598

DESCRIPTION

General Plan and Elevation

Construction Notes (1 of 2)

End Bent Details (1 of 2)

End Bent Details (2 of 2)

Deck Cross Section

Waterproofing Details

Assembly Details (1 of 2)

Assembly Details (2 of 2)

Deck Plate and Curb Layout

Bridge Fence Railing Details

Fascia Beam Details (1 of 3)

Fascia Beam Details (2 of 3)

Fascia Beam Details (3 of 3)

Temporary Bridge Deck Plan

Temporary Bridge General Plan and Elevation

Temporary Bridge Deck Sections, Details and Material Schedule

Temporary Bridge Pile Layout and Details

Temporary Bridge Bents 2 and 3 Details

Temporary Bridge Precast End Bent Details

Temporary Bridge Precast Bent Cap Details

Temporary Bridge Precast End Bent Wingwall Details

Temporary Bridge End Bent Details

Temporary Bridge Handrail Details

Temporary Bridge Framing Plan

Temporary Bridge Beam Details

Existing Plans Sheet (1 of 7)

Existing Plans Sheet (2 of 7)

Existing Plans Sheet (3 of 7)

Existing Plans Sheet (4 of 7)

Existing Plans Sheet (5 of 7)

Existing Plans Sheet (6 of 7)

Existing Plans Sheet (7 of 7)

Beam Framing Plan

Bent 3 - Plan and Elevation

Beam Elevation and Design Data

Structural Steel Details (1 of 2)

Structural Steel Details (2 of 2)

Log of Borings

Construction Notes (2 of 2)

Foundation Plan and Pile Details

Construction Staging - Stage 1A

Temporary Support System Details

Bents 2 and 4 - Plan and Elevation

Index of Drawings and Summary of Quantities

Construction Staging - Stages 1B and 1C

SHEET NO.

UP-1

UP-2

UP-3

UP-4

UP-13

UP-14

UP-15

UP-16

UP-17

UP-18

UP-19

UP-20

UP-21

UP-22

UP-23

UP-24

UP-25

UP-26

UP-27

UP-28

UP-29

UP-30

UP-31

UP-32

UP-33

UP-34

UP-35

UP-36

UP-37

UP-38

UP-39

UP-40

UP-41

UP-42

UP-43

UP-44

UP-45

UP-46

UP-47

UP-48

UP-49

UP-50

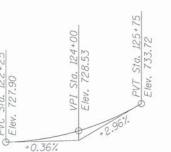
UP-51

UP-52

UP-5-12



PROFILE GRADE TOP OF RAIL TEMPORARY SHOO-FLY TRACK



PROFILE GRADE PROPOSED IL25/STEARNS RD.

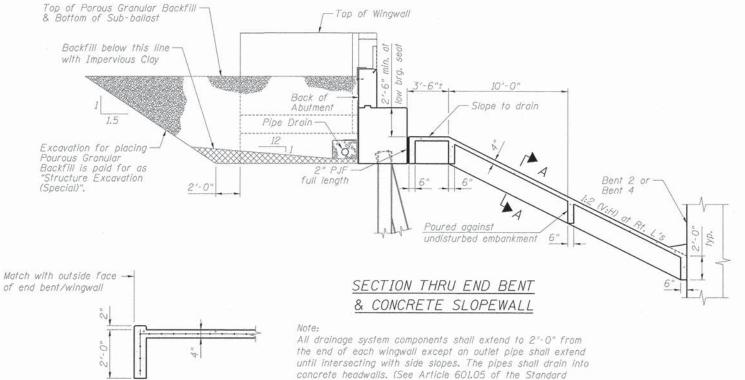
U.P.R.R. BRIDGE 37.71 BUILT 201_ BY STATE OF ILLINOIS OVER F.A.P. 361 ROUTE IL 25/STEARNS RD. SECTION 06-00214-18-RP STATION 20010+31.68 LOADING COOPER E-80 STRUCTURE NO. 045-3168

NAME PLATE

SUMMARY OF BILL OF MATERIALS

PAY ITEN	DESCRIPTION	UNIT	QUANTIT
20900110	Porous Granular Backfill	Cu. Yd.	228
50100300	Removal of Existing Structures No. 1	Each	1
50100400	Removal of Existing Structures No. 2	Each	1
50500105	Furnishing and Erecting Structural Steel	L. Sum	1
5080020	Reinforcement Bars, Epoxy Coated	Pound	27,300
50901730	Bridge Fence Railing	Foot	428
51100100	Slope Wall 4 Inch	Sq. Yd.	194
51201900	Furnishing Steel Piles HP14x89	Foot	5,705
51203900	Test Pile Steel HP14x89	Each	5
51500100	Name Plates	Each	1
58000100	Membrane Waterpoofing	Sq. Ft.	3.932
58700300	Concrete Sealer	Sq. Ft.	5,350
Z000230) Ballast	Cu. Yd.	40
Z0018905	Drill and Grout Bars	Each	96
Z0073500	Temporary Support System	L. Sum	1
X502020	Structure Excavation (Special)	Cu. Yd.	1,019
X503022	Concrete Structures (Special)	Cu. Yd.	388.2
X5120005	Driving Piles (Special)	Foot	5,705
X6013600	Pipe Underdrains 4" (Modified)	Foot	90
XX006847	Vinyl Fence, 4'	Foot	774
XX008386	Precast Concrete Box Segments	L. Sum	1
XX008628	Temporary Bridge	L. Sum	1
	Pile Shoes (Special)	Each	60
	Structure Marker Signs	Each	2

*Estimated quantity for Removal of Existing Structures No. 1: Concrete = 579 Cu. Yd. Steel = 104,420 Pounds



Specifications and Highway Standard 601101).

SECTION A-A

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601

USER NAME = rgrimm	DESIGNED - JLS	REVISED -	
	CHECKED - LRB	REVISED -	
PLOT SCALE =	DRAWN - RMG	REVISED -	
PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168 INDEX OF DRAWINGS AND SUMMARY OF QUANTITIES SHEET NO. UP-4 OF UP-52 SHEETS

		CONTRAC	T NO. 6	3598
1	06-00214-18-RP	KANE	451	264
Ρ.	SECTION	COUNTY	TOTAL	SHEET NO.

	C
	01
	0
	-
	(O
	O.
	5
	1
	0
	8
	IO.
	0
	0
	3
	m
	1
	E
	2
	0
	/
	3
	7
	4
	a.
	1
	0
	90
	10
	L
	m
	a.
	Œ.
	0
	2
	0
	0
	3
	ìí
	10
	4
	3
	z
	S
	1
	200
	60
	0
	5
	-
	.00
	2
	ē
	Ē
	5
	0
	2
	- 1
	D
	C
	1
	0
	9
	7
	O
	1.5
ш	9
	A
	1
	0
	0
	1
	S
	00
	00
	36
	-
	1
	×

Geo Services Inc.	ring	SOI	L E	BOF	RING LOG DATE	4/25/2011		
Geo SErvices Inc. Geotechnical, Environmental & Givil Enginee 805 Amberst-Caurt Acate 204 Naperville Milingla 60565 (630-3554-2838						D BY RJ		-
	DESC	DIDTION	Stan	rne	GSI JC Road (IL. Route 25) From Dunham R	08 No. <u>08032</u>	1.9.5	
					1/4, TWP. 40 N., RNG. 8 E., 3rd P.M.		N.	_
					ow Stem Auger/Rotary HAMMER TYP		matic	_
STRUCT. NO045-3168	Г		T	I	Surface Water Elevn/a	I I I	T	
Station BORING NO. UP-01		D B L P O T W	U C S	M 0 - S	Stream Bed Elev. <u>n/a</u> Groundwater Elevation:	D B L P O T W	U C S	20-0
Station: <u>2000+08 UPRR</u> Offset: <u>31.5' Left</u>		H S	Qu	T	First Encounter Dry to 15's Upon Completion n/a	HS	Qu	Ţ
Ground Surface Elev. 7	32.2	(ft) (/6°	(tsf)	(%)	After Hrs 7	Z (ft) (/6")	(tsf)	(%
18.0" TOPSOIL-black		- AS	_	18		-		
	730.7	2				- 8		
	_	5 9	l un			11	NP	2
CAND & CDAVEL		- 9	NP	6	SAND & GRAVEL-brown- medium dense (A-1)	11	NP	2
SAND & GRAVEL-brown- medium dense (A-1)					mediani delise (A-1)		- 1	
median dense (A 1.)	-	8	\vdash			11	\rightarrow	_
	12	-5 11	NP	4		-25 15	NP	
	726.7		Г			706.7		
	_	5					- 1	
SAND with Gravel-brown-		9			SAND-brown-medium dense (A-3)	12	\neg	_
medium dense (A-1-b)	_	11	NP	14	0, 110 O O O O O O		NP	2
	724.2	-				703.7		
		2				10		
	-	5				10		П
		-10 8	NP	4		-30 15	1.25P	1
						- 1	- 1	
		_ 2		_			_	_
SAND & GRAVEL-brown- medium dense (A-1)	_	- 4 13	NP			-		
modali donoc (// //		13	I NP	4	CLAY-gray-stiff (A-6)		_	-
	-	1	-			8 8	-	1.
		-15 1	NP	4		-35 10	1.0B	1.
	_	_						
	-							
		10					\dashv	
	-	13	NP	4				
	_							
		8				10		11
	-	12				8		
The Uncertified Company of the Company		-20 12	NP	20	ge, S—Shear, P—Penetrometer) ST—Shelby Tube	-40 5 Sample VS=Vane	1.08	17

							_
Geo Services Inc.	SO	IL E	BOF	RING LOG DATE 4	/25/2011		_
Geo SEPVICES Inc. Geotechnical, Editingmental & divil Engineering 805 Artherit Caput Logie 204 Noperville, lithely 5 05055 (630) 555-528-8				LOGGED	BY RJ		
(630) 355+2888				GSI JOB	No. <u>08032</u>		
ROUTE F.A. 361 DE	ESCRIPTION	N Steo	rns			5.11.1	
SECTION <u>98-00214-02-BR</u> LC	CATION _	SEC. 1-	-SW	1/4, TWP. 40 N., RNG. 8 E., 3rd P.M.			
COUNTY Kane Di	RILLING MI	ETHOD .	Holle	ow Stem Auger/Rotary HAMMER TYPE _	Diedrich Aut	omatic	0
STRUCT. NO. <u>045-3168</u>	D B	3 U	м	Surface Water Elev. <u>n/a</u>	р в	U	,
Station	E L	C	0	Stream Bed Elev. <u>n/a</u>	E L	C	(
BORING NO. UP-01 Station: 2000+08 UPRR	TW	V	S	Groundwater Elevation:	TW		5
Offset: 31.5' Left	HS	Qu	T	First Encounter Dry to 15' V Upon Completion n/a ∇	H S	Qu	
Ground Surface Elev. 732.2	(ft) (/e	6") (tsf)	(%)	The first of the second of the	(ft) (/6")	(tsf)	(
	-				_		
	-	1			-		Г
	- I			CLAY LOAM—gray—medium stiff (A-6)	1		_
	-			Term gray meanin em (it e)	-		
CLAY-gray-stiff (A-6)	5	5			4		
CLAT-gray-still (A-6)	7				3		
	<u>-45</u> 9	-	18		-65 4	0.5P	1
					\Box		
	-	+	_		5.2	-	-
				60	5.2		
	- 4		108		- 24		
	6		100		26		
	-50 B	1.25B	21	and the same and the same same same same same same same sam	-70 26	NP	1
	\dashv			SANDY LOAM-gray- dense to very dense (A-2)	-		
				delise to very delise (A 2)			
680	.2						
	-	+			-	-	-
	9		114		20		_
	1°	0.6B	17	65	26 7.2 -7550/3	" NP	1
CLAY LOAM-gray-medium stiff (A-6)		0.00		End Of Boring @ -75.0'	7.00073	140	Ť
CLAT LOAM-gray-medium still (A-0)	_			Hollow Stem Augers To -15.0° Rotary Drilling To Completion			
	-	+		Diedrich Automatic Hammer	-		-
	- 8		121		-		
	_ 5		121				
- Annual Control of the Control of t	-60 4		13	ge, S—Shear, P—Penetrometer) ST—Shelby Tube Sa	-80 nple VS=Vane		

benesch Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinolis 06001 312-565-0450 Job No. 10074

FILE NAME =	USER NAME = rgrimm	DESIGNED - JLS	REVISED -
		CHECKED - LRB	REVISED -
Perm_Br_005_Borings_Ldgn	PLOT SCALE =	DRAWN - RMG	REVISED -
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

| COUNTY | TOTAL SHEET | STEET | SOLUTION | SHEETS | SOLUTION | SHEETS | SOLUTION | SHEET | SHEET | SOLUTION | SHEET | SHEET | SHEET | SHEET | SOLUTION | SHEET | SHEET | SHEET | SOLUTION | SHEET F.A.P. SECTION 361 06-00214-18-RP M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168 LOG OF BORINGS (1 OF 8) SHEET NO. UP-5 OF UP-52 SHEETS

1	
- 1	-
1	ō
1	0
-1	N
-1	10
-1	95
1	C
- 1	7
-1	0
-1	B
- 1	9
- 1	20
- 1	0
- 1	7
	8
- 1	E
	7
1	Pe
1	7
- 1	5
- 1	5
- 1	7
-1	0
-1	10
1	6
1	TO
- 1	2
- 1	m
1	œ.
	Œ
1	9
1	-1
	00
- 1	16
- 1	3
- 1	10
- 1	4
- 1	0
	-
- 1	S
- 1	<
- 1	=
-1	0
- 1	(1)
- 1	2
- 1	0
- 1	100
- 1	43
- 1	C
- 1	H e
- 1	5
1	0
- 1	ŏ
-1	-51
	9
- 1	7
J	ē
1	0
1	5
1	9
	L
-	7
١	4
- 1	10
- 1	00
- 1	7
-1	S
	0
1	00
- 1	00
- 1	7
1	×
1	
-	
-	

89:81 PM		X:\10
M9:01 PM		
10:60	1	_
3:09:		Ø1 PN
		3:09:

S	10	0.00		and a first and a second secon				_
	,01	L E	30 F	RING LOG DATE 4/3	20/201	1		
				LOGGED B	Y RJ			_
				GSI JOB N	lo. <u>08</u>	032		
DESCRIP	TION	Sted	rns	Road (IL. Route 25) From Dunham Road	To CC	&P F	RR	
OCATIO	N S	EC. 1-	-SW	1/4, TWP. 40 N., RNG. 8 E., 3rd P.M.				
ORILLING	MET	HOD .	Holl	ow Stem Auger/Rotary HAMMER TYPE D	liedrich	Auto	omati	c
	1		T.,	Surface Water Elevn/a	Τ,	0		Γ,
E	L	C	0	Stream Bed Elev. <u>n/a</u>	E	L	C	1
T	W		S	Groundwater Elevation:	T	W		
Н	S	Qu	T	First Encounter Dry to 10	H	S	Qu	
(ft)	(/6")	(tsf)	(%)	After Hrs	(ft)	(/6")	(tsf)	(
-			25		_			
8.8	12	_	25	SAND with Gravel-brown-	-	2		
	12			very loose to medium dense (A-I-b)	_	2		
-	12	NP	6	702	7.3	1	NP	2
	1				_			l
-	6	-	-		-	3		╀
-5	9	NP	7		-25	2	NP	2
_	1							Г
	5			SAND & GRAVEL-brown-	-	5		
_	11			loose to medium dense (A-1)		8		Г
100	11	NP	6		_	8	NP	1
<u> </u>								
-	3		_			2		L
-10	1	NP	5		-30	1	NP	١,
			-			Ť	-111	Γ
-	1.0							
	12			698	1.3			T
-	12	NP	10	1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	_			H
*	1				-			
	6					3		1
-15	12	ND	7		_35	3	1 1D	١,
	117	- ixi	<u> </u>	CLAY-gray-stiff (A-6)		3	1.10	Ť
-	-							
27.	6				-			-
_	11	NP	19					L
2.3	1				A			
	7					3		1
_	7				-	5		
	DRILLING DE P T H (ft) (ft)	DOCATION SI DOCATION SI DOCATION SI DOCATION MET DOCATION MET DOCATION MET DOCATION MET DOCATION MET DOCATION MET AS 48.8 12 12 12 12 12 13 5 -10 11 11 10 12 12 12 12 14 10 12 12 12 14 17 7 7 7 Mode is indicate i	DOCATION SEC. 1- DRILLING METHOD D B U E L C P O S T W G Q U (ft) (/6") (tsf) AS - 12 12 12 12 12 19 6 9 -5 9 NP -5 11 11 11 NP -10 12 12 12 NP -10 11 NP -10 12 12 12 12 NP -10 11 NP -10 12 12 12 12 12 11 NP -10 11 NP -10 10 12 12 11 11 NP -10 10 12 11 11 NP -10 10 12 11 11 11 NP -10 10 12 12 12 11 NP -10 11 NP -10 10 11 NP -10 11 NP -10 10 11 NP -10 10 11 NP -10 10 11 NP -10 10 11 NP -10 11 NP -10 10 11 NP -10 10 11 NP -10 10 11 NP -10 10 10 11 NP -10 10 10 10 10 10 10 10 10 1	DRILLING METHOD Holio D B U M E L C O P O S I T W Qu T (ft) (/6") (tsf) (%) AS - 25 8.8 12 12 12 12 12 12 12 17 10 11 11 11 11 11 11 11 11	DORILLING METHOD Hollow Stem Auger/Rotary HAMMER TYPE DORILLING METHOD Hollow Stem Auger/Rotary HAMMER TYPE DORING	DRILLING METHOD Hollow Stern Auger/Rotary HAMMER TYPE Diedrich	Description SEC. 1-SW 1/4, TWP. 40 N. RNG. 8 E. 3rd P.M.	DRILLING METHOD Hollow Stem Auger/Rotary HAMMER TYPE Diedrich Automative Diedrich Diedr

						PAGE 2	of _	2	
Geo Septices Inc.	S	OIL	В	OF	RING LOG	DATE _4/20/20	011		
Geotechnical, Environmental & Givil Engineering 805 Amberst Court Syste 204						LOGGED BY R			
Geo Services Inc. Geotechnical, Environmental & Givil Engineering 805 Artherst Court Syste 204 Nopeyville, Wilhold 60365 (630) 35512838						GSI JOB No			
ROUTE F.A. 361	DESCRIPT	ION	Stee	roe l			The species	was:	
		11100	interest.				COCP	VIV.	_
SECTION <u>98-00214-02-BR</u>								- 4	9
COUNTY Kane		METH	HOD _	Holle		R TYPE <u>Diedric</u>	h Aut	omatio	C
STRUCT. NO. <u>045-3168</u>	101	В	u	М	Surface Water Elev. n/a Stream Bed Elev. n/a	D	В	U	,
Station		C	C	0	tal	p	0	C	1
Station: 2001+22 UPRR	TH	WS	Qu	S	Groundwater Elevation: First Encounter <u>Dry to</u>	10' H		Qu	1
Offset: 29.5' Left		10.7840	00000	120	Upon Completion n/a	∇			
Ground Surface Elev. 730.3	(ft) (/6")	(tsf)	(%)	After Hrs	▼ (fi) (/6")	(tsf)	(
						-	-		
						-	+		
						127			Γ
	1	-	-			12	-		H
	-				CLAY LOAM-gray- medium dense (A-6)	-	1		
		3		108	,	-	7		L
	-45	5	1.1B	20		-	13	r	١,
01.471:# /4 0)	-43	1	1.10	20			16		ľ
CLAY-gray-stiff (A-6)							7		
	-	+	-			663.3	+		┝
						000.0			
							-		
		3		106		0.7	38		
	-	4	-	700		-	45		T
	-50	4	1.0B	21			7050/3	" NP	L
	-				Clayey SAND, GRAVEL & FRA	CTURED -	-		
					ROCK-gray-very dense (A-1,	/A-2) —			
	_					4	_		
	-	+	_	-		-	+		H
						-			
	-	3	-	106			27		-
	-55	6	1.0B	22		655.3 -7	30 75 35	NP	1
	_				End Of Boring @ -75.0'				Γ
	_				Hollow Stem Augers To −10.0 Rotary Drilling To Completion	_	-		
6	73.3	\forall			Diedrich Automatic Hammer	-			-
		_					1		
							-		
CLAY LOAM—gray— medium dense (A—6)		7				13-			
		8				-	30		

benesch Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-855-0450 Job No. 10074

- Clighters Strends 912-000-000 00010. 10014							
FILE NAME =	USER NAME = rgr:mm	DESIGNED - JLS	REVISED -				
		CHECKED - LRB	REVISED -				
Perm_Br.006_Borings_2.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -				
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -				

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION M.P. 37.71 BELVIDERE SUBDIVISION — STRUCTURE NO. 045-3168 LOG OF BORINGS (2 OF 8) SHEET NO. UP-6 OF UP-52 SHEETS

A.P.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
61	06-00214-18-RP	KANE	451	266
		CONTRACT	NO. 6	3598
	ILLINOIS FED. A	AID PROJECT		

ı		
L		
i	1	
3	1	
c		
3	1	
ì	1	
3	1	
3		
	1	
2		
0		
1		
a	1	
8	1	
1	1	
0		
U	1	
3	1	
Ę	ı	
0		
1	J	
7		
3	П	
a		
a	1	
9	1	
Ξ		
α	1	
C	1	
("	1	
Ш	1	
ZZ	1	
	1	
101		
1	1	
-	1	
1	1	
1	1	
6	1	
-		
+	1	
-	ı	
1	ı	
i	1	
3	- 1	
-		
è	1	
1	1	
9	1	
è		
1	1	
.0	1	
1	1	
7.4	1	
0	١	
101	1	
1		
MC	1	
N	1	
200		
5	1	
3	1	
100	1	
	1	
	1	
	-1	
	J	

BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168	F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
LOG OF BORINGS (3 OF 8)	361	06-00214-18-RP	KANE	451	267
			CONTRAC	T NO. 6	3598
SHEET NO. UP-7 OF UP-52 SHEETS		ILLINOIS FED.	AID PROJECT		

PAGE 2 of 2

B U C S W S Qu

DATE 4/21/2011 LOGGED BY RJ GSI JOB No. 08032

Geo SERVICES Inc. Gootechnical, Environmental & Givil Engineering 805 Ariberts - Court Legale 204 Napervile, limber 60:565 (630) 355-2838	S	011	L E	BOF	RING LOG	PAGE 1 DATE 4/21/	2011		
Napetvile) Illinois 60565 (630) 355 2888						GSI JOB No.		72-11-1	
ROUTE F.A. 361	_ DESCRIPT	NOF	Steo	rns	Road (IL. Route 25) From Dur	nham Road To	CC&P	RR	
SECTION <u>98-00214-02-BR</u>	_ LOCATION	SE	C. 1-	-SW	1/4, TWP. 40 N., RNG. 8 E.,	3rd P.M.			
COUNTY Kane	_ DRILLING	MET	HOD .	Holle	ow Stem Auger/Rotary HAMM	ER TYPE Died	rich A	tomati	c_
STRUCT. NO. 045-3168 Station — BORING NO. UP-03 Station: 2001+50 UPRR Offset: 29.5' Left Ground Surface Elev. 729.	D E P T H	B L O W S	U C S Qu (tsf)	M O - S T (%)	Upon Completion n/a	to 15'▼	D E L L P C Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y	CS	M O I S T
12.0" TOPSOIL-black	728.4						-		
SANDY CLAY-gray- medium dense (A-4/A-6)	726.4	1 3 4	0.7B	16 109	SAND & GRAVEL-brown- medium dense (A-1)	706.4			13
Clayey SAND-brown- very loose (A-2)		2 2 2	-	12	GRAVEL with Sand-brown- medium dense (A-1-a)		8 25_1	3	14
	_	9 9 11	NP	6	Thedam dense (A T d)	701.4	_		10
SAND & GRAVEL-brown- medium dense (A-1)		7 14 10	NP	8	SAND with Gravel-brown- medium dense (A-1-b)	-	8 8 30 10		20
	_	7 7 11	NP	7		697.4			
		10	NP			-	6		110
		6	NP	A	CLAY-groy-stiff (A-6)	-	-33 9	1.88	19
		6 8	NP	9	*	-	=		
	-20	3 8 12	NP	10			8 40_ 8	1.18	108

The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) The Unit Dry Weight (pcf) is noted in Italics above moist (%) NR-No Recovery

	engineers - scientists - planners	312-565-0450	Job No.	1007
7	benesch	Alfred Benesch & Compa 205 North Michigan Aver Chicago, Illinois 60601	nue, Suit	

gineers - scientists .	planners 312-303-0430 300 No. 100	14		
	USER NAME = rgrimm	DESIGNED - JLS	REVISED -	
		CHECKED - LRB	REVISED -	
Borings_3.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -	
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	

STATE DEPARTMENT OF

A.P.	SECTION	COUNTY	TOTAL	SHEET NO.
61	06-00214-18-RP	KANE	451	267
		CONTRACT	NO. 6	3598
	ILLINOIS FED. A	ID PROJECT		

OF ILLINOIS OF TRANSPORTATION	M.P. 37.71 BELVIDERE SUBDIVISION — STRUCTUR LOG OF BORINGS (3 OF 8)
	SHEET NO. UP-7 OF UP-52 SHEETS

CLAY LOAM-gray-stiff to very stiff (A-6)

SECTION 98-00214-02-BR

STRUCT. NO. 045-3168

BORING NO. UP-03 Station: 2001+50 UPRR

COUNTY Kane

Station ___

Station: 2001+50 UPRR Offset: 29.5' Left Ground Surface Elev. 729.4	H	S	Qu (tsf)	T (%)	First Encounter Upon Completion $\frac{Dry \ to \ 15}{n/a}$	H	S	Qu (tsf)	T (%)
Fround Surface Elev, 729.4		170	((31)	(70)	After Hrs	_	(/6)	((81)	(70)
	_	_			? `	-			
	-				CLAY LOAM-gray-		_		
	-	3		108	stiff to very stiff (A-6)	-	6		
	-45	5 9	1.25B	21) 	_ -65	10 11	2.5P	12
AY—gray—stiff (A—6)	_				· ·	-			
	_	\vdash			662.4	-			
	_	\vdash				-			
	_	3		109	Clayey SAND-brown-	-	7		
	_50	6	1.3B	20	medium dense (A-2)	_ -70	7 10	_	18
	-				9. -				
	_				657.4				
	=		***		SAND, GRAVEL & FRACTURED ROCK-				
	-	4			gray—medium dense (A—1)		10 9		
	-55	-		26	654.4 End Of Boring @ -75.0'	-75		NP	9
	-				Hollow Stem Augers To -15.0' Rotary Drilling To Completion Diedrich Automatic Hammer				
672	2.4				=				
							_		

SOIL BORING LOG

D B U

DESCRIPTION Stearns Road (IL. Route 25) From Dunham Road To CC&P RR

Surface Water Elev. <u>n/a</u>

Stream Bed Elev. n/a

DRILLING METHOD Hollow Stem Auger/Rotary HAMMER TYPE Diedrich Automatic

First Encounter Dry to 15'

LOCATION SEC. 1-SW 1/4, TWP. 40 N., RNG, 8 E., 3rd P.M.

The Unconfined Compressive Strength (UCS) Follure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) The Unit Dry Weight (pcf) is noted in italics above moist (%)
NR-No Recovery

	ı
	7
	C
	30
	0
	c
	010
	C
	10
	ANI
	c
	1
	0
	0
	1.6
	C
	ċ
	UN
	0
	í
	TIV
	č
	4
	-
	-
	-
	,
	1
	L
	7 1
	000
	3000
	SCAC
	110
	>
- 1	

TON	COUNTY	TOTAL IS

						1	of _	2	
Geo Services Inc.	S	10	L E	301	RING LOG DATE _	4/26/201	1		_
Geo Services Inc. Geotechnical, Environmental & Givil Engineering 805 Amberts - Caurt Josephe 204 Naperville, Jimbis - 60565 (630) 35542838					LOGGED	BY RJ		-	_
(630) 355+2838					GSI JOE	No08	3032		_
ROUTE F.A. 361	DESCRIP	TION	Steo	rns	Road (IL. Route 25) From Dunham Ro	od To CC	&P F	RR	_
SECTION <u>98-00214-02-BR</u>	LOCATION	N S	EC. 1-	-SW	1/4, TWP. 40 N., RNG. 8 E., 3rd P.M.				
COUNTY Kane	DRILLING	MET	HOD .	Holl	ow Stem Auger/Rotary HAMMER TYPE	Diedrich	Aut	omatic	c
STRUCT. NO. <u>045-3168</u>	D	В	U	м	Surface Water Elev. <u>n/a</u>	- D	В	u	М
Station	E	L	C	0	Stream Bed Elev. <u>n/a</u>	- E	L	C	0
BORING NO. UP-04	P	O W	S	S	Groundwater Elevation:	P	O W	S	S
Station: 2002+00 UPRR Offset: 76.5' Right	Н	S	Qu	1	First Encounter Dry to 15' Upon Completion n/a	10.00	S	Qu	I
Ground Surface Elev. 735.5	(ft)	(/6")	(tsf)	(%)	After Hrs		(/6")	(tsf)	(%
7.0" Clayey SAND	34.8 —					_			Г
	_	AS 4	_	11	SAND & GRAVEL-brown-	-	13		
SAND-brown-loose (A-3)	_	4			medium dense to dense (A-1)	-	16		Г
220	–	4	NP	11			18	NP	11
73	32.5					12.5			
		7		_	FRACTURED ROCK-brown & gray-		17		L
	-	9	NP	5	very dense (A-1)	26	35 50/3	- 110	١.
		13	NP	2	1	10.0	50/3	NP	8
	-	5	-	-	SAND & GRAVEL-brown-	-	50/3		⊢
	7 NP 5 very dense (A-1)	-	50/5	NP	10				
SAND & GRAVEL-brown-	-					07.5			
medium dense to dense (A-1)	-	5				¥ II.	6		
		7					8		
	10	9	NP	5		30	10	-	16
	_				-	_			
	_	5	_			-			L
	-	6	NP	6	CLAY LOAM-gray- medium dense (A-6)	-	1		
			- 111	Ť		-			
	_					-			
	-	4		\vdash			7		\vdash
	-15	6	NP	4		35	9	1.0P	20
	_						1		
		11				-			
		14							
	-	16	NP	6		_		-	-
		16	_	_		-	5		-
	-20	18	NP	R		-40	5	420	25

Geo Services Inc.		102	L E	BOF	RING LOG DATE	=0.00		
Geo Strices Inc. Geotechnical, Environmental & divil Engineering 805 Arbherst Court Style 204 Noperville, lilingta 50565 (630) 3551-2838					LOGGED BY			
Naperville) Illinois 60565 (630) 355-2888					GSI JOB No.			
	_ DESCRIF	MOITS	Steo	irns	Road (IL. Route 25) From Dunham Road To	************		
SECTION _98-00214-02-BR	LOCATIO	N S	EC. 1-	-SW	1/4, TWP. 40 N., RNG. 8 E., 3rd P.M.			
COUNTY Kane	DRILLING	G MET	THOD _	Holl	ow Stem Auger/Rotary HAMMER TYPE Diedri	ch Au	tomati	C
STRUCT. NO. <u>045-3168</u> Station BORING NO. <u>UP-04</u> Station: <u>2002+00 UPRR</u> Offset: <u>76.5' Right</u> Ground Surface Elev. <u>735.</u>	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. n/a Stream Bed Elev. n/a Groundwater Elevation: First Encounter Dry to 15' Upon Completion n/a	D B L O T W S	UCS	M O I S T
	_				SILTY LOAM-gray- medium dense (A-4)	1		
	-	_				丄		
CLAY LOAM-gray- medium dense (A-6)	_	4			-	14	_	
medium dense (A=0)	-4	7 5 9	-	21	_	18 65 24	1000000	18
	_				SANDY LOAM to LOAM-gray- medium dense to dense (A-2/A-4) —			
		6 10				6 8		
	<u>-5</u>	0 14		21	=	70 10	NP	17
	683.5	-			663.5	-		
CLAY-gray-very stiff (A-6)	-	7 9		103				
	5 	9	2.08	20	End Of Boring @ -75.0' Hollow Stem Augers To -15.0' Rotary Drilling To Completion Diedrich Automatic Hammer	75 22	1.5P	12
SILTY LOAM-gray- medium dense (A-4)		7 5						

The Unconfined Compressive Strength (UCS) Foliure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) The Unit Dry Weight (pcf) is noted in Italics above moist (%)
NR-No Recovery

benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 Chicago, Illinois 60501 Job No. 10074

engineers - scientists -	prainters 312-300-0430 300 No. 1007	*	
FILE NAME =	USER NAME = rgrimm	DESIGNED - JLS	REVISED -
		CHECKED - LRB	REVISED -
Perm_Br_008_Borings_4.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168	F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
LOG OF BORINGS (4 OF 8)	361	06-00214-18-RP	KANE	451	268
			CONTRACT	NO. 1	63598
SHEET NO. UP-8 OF UP-52 SHEETS		ILLINOIS FED. A	ID PROJECT		

ANSVD- B AND B- E T-
ANS A DE MAG DE LE PLANTE DE LA COMPANSION DE LA COMPANSI
ANS 10 - D. MAG D
AMOND D. MAG D.
AMOND D. MAG D.
ANICY D D. 000
ANICY D D. 000
ANICY D D. 000
ANCY D D. OR
ANICA
ANICA
ANICA
ANICA
DI ANCA
DI ANIG
ā
15
- 13
0
0
001
-
0
16
u
5
2
0
-
- 13
d
2.5
- 13
(m)
74/5-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0
74/5-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0
74/5-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0
74/5-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0
74/5-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0
10000011007475
3/1/2000/1000/100/100/100/100/100/100/100
3/1/2000/1000/100/100/100/100/100/100/100
10000011007475
lo lo

SOIL BORING LOG DATE 10/3/2012 LOGGED BY MD GROWN MD MD GROWN MD MD MD MD MD MD MD M		PAGE _1 of _4	
Cooking Cook	Can Spring Inc	5명을 마시되었다 그렇게 남자 마음에는 나는데 그리고 되었다	
ROUTE F.A. 381 DESCRIPTION Steams Road (II. Route 25) From Dunham Road To CC&P RR	Geotechnical, Environmental & Givil Engineering 805 Amberst Court 180te 204		
ROUTE F.A. 381 DESCRIPTION Steams Road (IL Route 25) From Dunham Road To CC&P RR	Naperville) Winds 60565 (630) 355 2838		
SECTION 98-00214-02-BR	ROUTE FA 361		
DRILLING METHOD Hollow Stem Auger/Rotory HAMMER TYPE CME Automatic	AND THE CONTRACT OF THE CONTRA		
STRUCT. NO. <u>045–3168</u> Station — B	[] : 사용 () - (
Station	Percentage and the specific property of the second property of the s		\equiv
Station 2001+38 UPRR Other 27.9 Left Cround Surface Elev. 731.5 (ft) /e* (tsf) (3)		DIBLUIMI a. D. G. T. T. DIBLU	
H S Qu T	BORING NO. UP-05		1.1
Cround Surface Elev. 731.5 (ft) 1/6" (taf) (%) After Hrs.		_ H S Qu T First Encounter <u>Dry to 10.0'</u> H S Qu	
		[(c) k (-m/s) 0 (cm/s)	(%)
			П
			
			П
	1	+++	+
	Blind Drill To -68.5'		
			Н
The Unconfined Compressive Strength (UCS) Failure Made is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test		5	
The Unconfined Compressive Strength (UCS) Failure Made is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			
The Unconfined Compressive Strength (UCS) Failure Made is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			
The Unconfined Compressive Strength (UCS) Failure Made is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			
The Unconfined Compressive Strength (UCS) Failure Made is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			+
The Unconfined Compressive Strength (UCS) Failure Made is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test		7111 71	
The Unconfined Compressive Strength (UCS) Failure Made is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			+
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			Н
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			Ш
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test		-	
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			П
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			П
The Unconfined Compressive Strength (UCS) Failure Mode is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test		_1535	+
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test		++++	\vdash
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (8-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			
The Unconfined Compressive Strength (UCS) Fallure Made is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test		-	
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Sheiby Tube Sample VS=Vane Shear Test	×		
The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test			
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) The Unit Dry Weight (pcf) is noted in italics above moist (%)	The Unconfined Compressive Strength (UCS) Fallu	-2U -40 -40 - ure Made is Indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear	

PAGE 2 of 4 SOIL BORING LOG DATE 10/3/2012 LOGGED BY MD GSI JOB No. 08032 ROUTE F.A. 361 DESCRIPTION Stearns Road (IL. Route 25) From Dunham Road To CC&P RR SECTION 98-00214-02-BR LOCATION SEC. 1-SW 1/4, TWP. 40 N., RNG. 8 E., 3rd P.M. DRILLING METHOD Hollow Stem Auger/Rotary HAMMER TYPE CME Automatic COUNTY Kane STRUCT. NO. <u>045-3168</u> Surface Water Elev. n/aUCS Stream Bed Elev. <u>n/a</u> Station __ E L C P O S BORING NO. UP-05 Groundwater Elevation: T W H S Qu W S Qu
 Station:
 2001+38
 UPRR

 Offset:
 27.9'
 Left
 First Encounter <u>Dry to 10.0'</u> Upon Completion n/a(ft) (/6") (tsf) (%) After _ Ground Surface Elev. 731.5 (ft) (/6") (tsf) (%) ____ Hrs. Blind Drill To -68.5' -45 663.0 -50 -70 16 SANDY CLAY LOAM with Gravel-grayhard (A-6) 50/5 654.5 CLAYEY SAND & GRAVEL-grayvery dense (A-2) -60

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS-Vane Shear Test
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) The Unit Dry Weight (pcf) is noted in italics above moist (%)
NR-No Recovery.



Perm_Br_009_Borings_5.dgn

USER NAME = rgrimm	DESIGNED - JLS	REVISED -	
	CHECKED - LRB	REVISED -	
PLOT SCALE =	DRAWN - RMG	REVISED -	
PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	

	0
	5
	00
	011
	BA
	MINA
	3-1
	- w
	Day
	101
	N N
	A/P
	do
	ä
	PPR
	311 2
	3169
	OA
	IVEN MAF
	11
	000
9	Ad a
	0+0
	e mi
	0
	200
	Š
	000
	000
	1/5
	1017
	110
	DIDIC
	100
	×

- 1	2
1	۵.
1	0
1	rů.
1	10
1	ä
1	
1	
1	
1	
-1	

						PAGE 3	0	f _4		
Geo Services Inc.	S	01	L E	BOF	RING LOG	DATE 10/3	/2012			_
Geo Services Inc. Geotechnical, Environmental & Givil Engineering 805 Antherial Court Forthe 204 Naperville, Winjust 60565 (630) 355-2838						LOGGED BY	MD			_
(630) 355+2638						GSI JOB No.	080	32		_
ROUTE F.A. 361 DE	SCRIP	TION	Stea	rns	Road (IL. Route 25) From Dur	ham Road T	o CC&	P RF		_
SECTION <u>98-00214-02-BR</u> LOG	CATION	N _SE	EC. 1-	-SW	1/4, TWP. 40 N., RNG. 8 E.,	3rd P.M.				
COUNTY Kane DR	LLING	MET	HOD _	Holle	ow Stem Auger/Rotary HAMM	ER TYPE CM	E Auto	mati	c	_
STRUCT. NO. <u>045-3168</u>	D	В	U	М	Surface Water Elev. <u>n/a</u>		D	В	11	М
Station	E	LO	Č	Ö	Stream Bed Elev. <u>n/a</u>		E	L	C	0
Station: 2001+38 UPRR	T	W	1 1 1 1	S	Groundwater Elevation: First Encounter Dry to	o 10.0' T	1 7 1	w		S
Offset: 27.9' Left	П П	3	Qu	Т	Upon Completion n/a	∇	111		2000 2000	Т
Ground Surface Elev. 731.5	(ft)	(/6")	(tsf)	(%)	After Hrs		(ft) (/	′6") (tsf)	(%)
CLAYEY SAND & GRAVEL-gray-	-						-			
very dense (A-2)										
649.	5						-			
	_						士	+	\neg	
	_						\neg	L		
AND-gray-very dense (A-3) 29 32 -85 34 NP 16 -105	-	32					50	2/4		-
	-	6								
	SANDY GRAVELLY CLAY LOAM with									
	_				Fractured Rock-gray-very d		コ	_	U C S Qu "") (tsf)	
644.	5				g () is the control of the control		-			
	_						士	\top		
	_						-	1		
		35 50/5	-		Ī			0/3	\dashv	
	-90		-	8			-110	+	-	6
	_						\dashv			
		_						4	_	
							-			
SANDY GRAVELLY CLAY LOAM with Fractured Rock-gray-very dense (A-2)	_							\neg		
	=	23					-	1/2		
		31					Ť	74	\neg	
	<u>-95</u>	45	-	8			-115	+	-	8
	_									
	_			_			+	+	\dashv	
	_									
							=			
	_	50/4						/2		
						92/25/00				
The Unconfined Compressive Strength (UCS) Failure Mo	-100	ndicate	ed by (7 B-Buid	ge. S-Shear, P-Penetrometer) ST-Sh	611.5 elby Tube Sample		ane S	- Shear	7 Test

1	benesch engineers - scientists - planners	A 2 0
DEVENOUS YE		

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450 Job No. 10074

FILE NAME =	USER NAME = rgrimm	DESIGNED - JLS	REVISED -	
		CHECKED - LRB	REVISED -	
Perm_Br_010_Borings_6.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -	
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168 LOG OF BORINGS (6 OF 8) SHEET NO. UP-10 OF UP-52 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEE'
361	06-00214-18-RP	KANE	451	270
		CONTRACT	NO. 6	3598
	ILLINOIS FED.	AID PROJECT		1000

Geo SERVICES Inc. Geotechnical, Environmental & Givil Engineering 805 Antherit Court Light 204 Noperville, Hirlylar 6,0565 (680) 355 2838	S	SOI	L B	BOF	RING LOG	PAGE _4 DATE _10/3 LOGGED BY GSI JOB No	MD		
ROUTE F.A. 361	DESCRIP	TION	Stea	rns	Road (IL. Route 25) Fr	rom Dunham Road	To CC&P	RR	
SECTION <u>98-00214-02-BR</u> I	LOCATIO	N S	EC. 1-	-SW	1/4, TWP. 40 N., RNG.	8 E., 3rd P.M.			
COUNTY Kane	DRILLING	MET	HOD _	Holle	ow Stem Auger/Rotary	HAMMER TYPE CM	IE Automo	ntic	
STRUCT. NO. <u>045-3168</u> Station BORING NO. <u>UP-05</u> Station: <u>2001+38 UPRR</u> Offset: <u>27.9' Left</u>	E P T H	B L O W S	DCN G	MOIST	Groundwater Elevation First Encounter Upon Completion	n/a : Dry to 10.0' ▼ n/a ▼		C S Qu	N C
Ground Surface Elev. 731.5 RUN 1 (-120.0' to -120.4')	(ft)	(/6")	(tsf)	(%)	After Hrs.		(ft) (/6")	(tsf)	(;
SILTY CLAY LOAM RUN 1 (-120.4' to -130.') Silurian System, Niagaran Series Dolor Gray & fine grained with horizontal bedding, Soft & argillaceous with som thin clay partings. Tight horizontal fractures throughout. Becoming light gray with wavy bedding & some cheri nodules @ -127.2'. Recovery=98.5% R.Q.D.=50.2%	mite —		RUN 1						
End Of Boring @ -130.0' Hollow Stem Augers To -10.0' Rotary Drilling To Completion 10.0' Of 4.0"Ø Casing Used CME Automatic Hammer									

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) ST-Shelby Tube Sample VS=Vane Shear Test
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) The Unit Dry Weight (pcf) is noted in italics above moist (%)
NR-No Recovery

				PA	GE _	1	_	of _1		_
Geo Services Inc.	ROCK	CORE	LOG	DA	TE _	10/8	/201	2		_
Geo SERVICES Inc. Geotechnical, Environmental & divil Engineering 805 Amberst Court Bothe 204 Nopelville, Jimber 50565 (830) 355 2838				LC	GGED	BY	DR			_
(630) 355 2638				GS	I JOE	No.	_08	3032		
ROUTE F.A. 361	DESCRIPTION Stee	rns Road (II	Route 25) Fro	m Dunha	n Ro	ad T	o CC	&P R	R	
SECTION <u>98-00214-02-BR</u>	LOCATION SEC. 1-	-SW 1/4, TV	VP. 40 N., RNG.	8 E., 3rd	P.M.					
COUNTY Kane	CORING METHOD	Rotary Was	1				_	-		
STRUCT. NO. <u>045-3168</u>			NX Double Sw	ivel-10 ft		0	R	R	CO	
Station	Core Diameter _ Top of Rock Elev	2.0 in			T	R	E C O	Q	R	
BORING NO. UP-05 Station: 2001+38 UPRR	- Begin Core Elev.	611.5			н		V E	D	M	1
Offset: 27.9' Left							R		E (min	i
Ground Surface Elev					(ft)	(#)	(%)	(%)	/ft)	(t
RUN 1 (-120.0' to -120.4') SILTY (CLAY LOAM-gray-h	ard			-	1	98.5	50.2	n/a	n,
RUN 1 (–120.4' to –130.') SILURIAN SYSTEM, NIAGARAN SERIES	DOLONITE									
Gray & fine grained with horizontal	bedding. Soft & ar				-			H		
partings. Tight horizontal fractures some chert nodules @ -127.2'.	throughout. Becomi	ing light gro	y with wavy bed	lding &				-		
					-					
					<u>_125</u>					
					-					
					-					
					470					
					-130	_				_
	TOP	6 + 130								
Color pictures of the cores Yes	Cores will be uniaxial compressive		xamination for		職					_

		25	. =	1 (2.5)		PAGE 1	of _	3	
Geo Services Inc.	S	01	L E	BOE	RING LOG	DATE _10/3	/2012		_
Geo SEPVICES Inc. Geotechnical, Efficient and Septime Services (Section 1997) Roperville, Mindis 60585 (830) 555-2888						LOGGED BY	MD		_
(630) 355+2888						GSI JOB No.	08032		_
ROUTE F.A. 361	DESCRIP	ПОП	Stea	rns	Road (IL. Route 25) From D	unham Road T	o CC&P	RR	_
SECTION <u>98-00214-02-BR</u> I	LOCATION	N S	C. 1-	-SW	1/4, TWP. 40 N., RNG. 8 E.	3rd P.M.			
COUNTY Kane	DRILLING	MET	HOD _	Holle	ow Stem Auger/Rotary HAM	MER TYPE CM	E Automo	atic	_
STRUCT. NO. <u>045-3168</u>	D	В	U	м	Surface Water Elev. <u>n/a</u>		D B	U	м
Station	E	LO	CS	0	Stream Bed Elev. <u>n/a</u>	-	E L P O	cs	Ö
BORING NO. UP-06 Station: 200+44 UPRR	T	W		S	Groundwater Elevation: First Encounter Dru	to 10.0'	TW		s
Offset: 123.7' Right	Н	S	Qu	Т	Upon Completion n/a	∇	H S	Qu	Т
Ground Surface Elev. 732.2	(ft)	(/6")	(tsf)	(%)	After Hrs		(ft) (/6*	(tsf)	(%)
							-		
	F						_		
							-		
	-						+	-	
Blind Drill To -68.5'	_						\neg		
	-	-					-	-	-
							-25		
	-						-		
	-						_		
	-						-		
	_								-
	_								
	-			⊢			_		\vdash
	_10						-30		
	-						-		- 72
							_		
							-		
	1,0	-	_	-			-		-
	_						\exists		
	_	\vdash		-	1			-	-
	-15						-35		
	-						_		
	3								
	19								
	200	-		-			-		-
							\exists		
	-	_		-			_		_
	-20						-40		

benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-585-0450
Job No. 10074

Perm_Br_011_Borings_7.dgn

sts - planners	312-505-0450 Job No. 100/			
USE	R NAME = rgrsmm	DESIGNED - JLS	REVISED -	
		CHECKED - LRB	REVISED -	
PLO	T SCALE =	DRAWN - RMG	REVISED -	garage available of
PLO	T DATE # 12/13/2012	CHECKED - LRB	REVISED -	

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION M.P. 37.71 BELVIDERE SUBDIVISION — STRUCTURE NO. 045-3168 LOG OF BORINGS (7 OF 8) SHEET NO. UP-11 OF UP-52 SHEETS

.A.P.	SECTION	COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP	KANE	451	271
		CONTRAC	T NO. 6	3598
	ILLINOIS FED. A	AID PROJECT		

	6
	ō
	70
- 4	00
	1
	ď
	č
	7
	Õ
	m
	0
	0
	0
	8
	-
	1 5
	ã
	۵,
	16
	15
	a
	0
	10
	lő
	TÕ
	ISB UPRR Brid
	m
	155
	H.
	100
	čč
	=
	N 045 31
	LC
	4
	0
	12
	11/5
	12
	Thase II
	0
	0
	1.8
	0.
	Q o
	1 +
	0
	8
	String
	Ĭč
	10
	1-
	1.0
	C
	1.7
	â
	d
	1
	10
	1.0
	14
	10
	1
	0
	0
	12
	00
	100
	100
	18
	17
	13
	1>

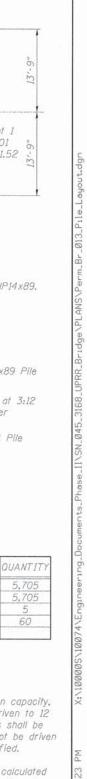
					PAGE 2	of _:	3	
Geo Services Inc.	S	OIL	BOF	RING LOG	DATE 10/3	/2012		_
Geo SERVICES Inc. Geotechnical, Environmento & Givil Engineer 805 Amberst Court Jorde 204 Nopelville, Julinio & 60565 (630) 355-2558	Ing				LOGGED BY	MD		_
(630) 355+2838					GSI JOB No.	08032		_
ROUTE F.A. 361	DESCRIPT	ON St	earns I	Road (IL. Route 25) From	Dunham Road T	CC&P F	R	_
SECTION <u>98-00214-02-BR</u>								_
COUNTY Kane						E Automa	tic	_
STRUCT. NO. <u>045-3168</u>				Surface Water Elevn/	'a			
Station	D E P	B L	0	Stream Bed Elev. <u>n/</u>	΄α	D B	C	М
BORING NO. UP-06	P	0 S	S	Groundwater Elevation:		P O T W	S	S
Station: 200+44 UPRR Offset: 123.7' Right	Ĥ	SQ	u T	First Encounter \underline{Dr} Upon Completion \underline{n}	y to 10.0' \(\nabla\)	H S	Qu	T
Ground Surface Elev. 7	32.2 (ft)	/6") (ts	f) (%)	After Hrs		(ft) (/6")	(tsf)	(%)
						\dashv		
	-	+	+			+		_
	-					\neg		
Blind Drill To -68.5'						_		
Billia Brill 10 - 30.5	-					\dashv		
			\top					
	45	_	+-			-65		
	-					\dashv		
						_		
	-	+	+					
					663.	7		
		-	_			29		_
	-50					38 -70 45	NP	15
	-	+	+					
				CLAYEY SAND with Grave	l-gray-	\neg		
			1	very dense (A-2)		-		
	_					43		
						50/5 -75	of the last	
	55	+	+			-/5	NP	13
						\exists		
	_	_	-			-	\vdash	_
	-					\dashv		
			\top	1		\exists		
	-							
	77	-	+	-		50/5	-	
						-80	NP	16

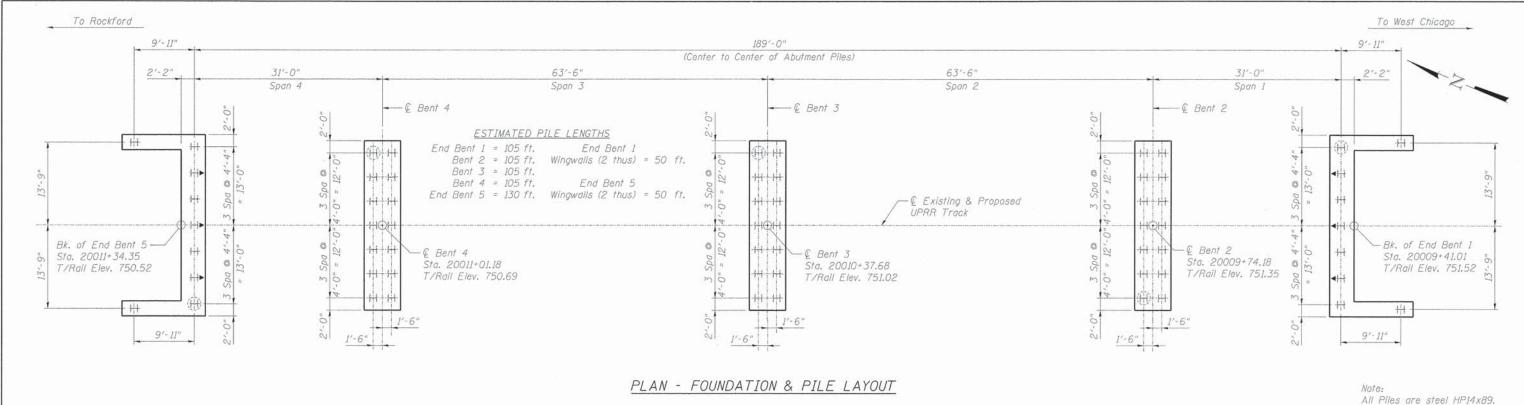
		>2200		35.00		PAGE 3	0	of _	3	
Geo Services Inc.	S	Oll	L B	OF	RING LOG	DATE _10/3	/2012	_	-	_
Geo SPENICES Inc. Geotechnical, Efviragmental & divi Engineering Annual County of the						LOGGED BY	MD			_
						GSI JOB No.				-
ROUTE F.A. 361 D								P F	RR	
SECTION <u>98-00214-02-BR</u> LO										
COUNTY Kane D	RILLING	MET	HOD _	Holle			E Auto	oma	tic	
STRUCT. NO. <u>045-3168</u> Station <u>-</u>	D	В	U	М	Surface Water Elev. n/a Stream Bed Elev. n/a			В	U	МО
BORING NO. UP-06	E P	L	C S	0	Groundwater Elevation:		P	LO	C	1
Station: 200+44 UPRR	T H	W	Qu	S	First Encounter <u>Dru</u>	to 10.0'	H	W S	Qu	S
Offset: 123.7' Right Ground Surface Elev. 732.2	(ft)	(/6")	(tsf)	(%)	Upon Completion n/a After Hrs	$\overline{\nabla}$	(ft) (/6"\	(tsf)	(%
Ground Surface Elev. 732.2	(10)	/ /	(101)	(7.5)	CLAYEY COBBLES, BOULDER		100	, - ,	()	
					FRACTURED ROCK-very den	se (A-2) 63	1.2			
CLAYEY SAND with Gravel-gray- very dense (A-2)	-	-	_	-	End Of Boring ◎ -101.0' Hollow Stem Augers To -10	0.0'	+	-		
,					Rotary Drilling To Completic	on	コ			
648	0.2				10.0' Of 4.0"ø Casing Used CME Automatic Hammer		\dashv			
	_	50/0	-		CONTROL SHEET MATERIAL SHEET S					
	-						105			
	85	1		NR			<u>-105</u>			-
COBBLES, BOULDERS & FRACTURED	_	1					_			
ROCK-gray-very dense (A-1)	-		-	H			+	-	_	-
										L
		1					\dashv			
		40					二			
	-	22					110			
	_90	150/1	* NP	6			-110			T
]								
64	0.2	\vdash	-	\vdash	1					
	_	1		_			\neg		_	┡
	-	1					_			
	_	44		_			\exists		_	1
	_05	30		8			-115			
CLAYEY COBBLES, BOULDERS &		1		Ť	1					Г
FRACTURED ROCK-gray- very dense (A-2)	-	1					-			
	_				1					T
	_	-	-	-	-		-	_	-	+
		1					\equiv			
	-	48		-			_		-	+
	-100	50/3	1_	R			-120			

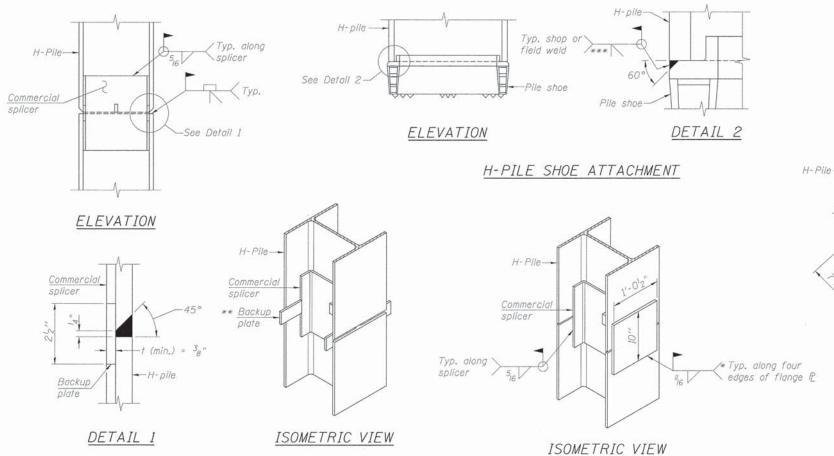
Ine unconfined compressive strength (UCS) reliure Mode is indicated by (8-Buige, S-Shear, P-Penetrometer). ST-Shelby Tube Sample VS-Vane Shear Test.
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206). The Unit Dry Weight (pcf) is noted in italics above moist (%) NR-No Recovery.

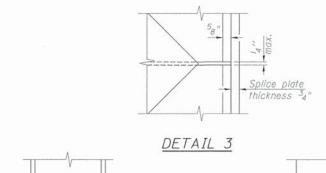
benesch Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-965-0450
Job No. 10074

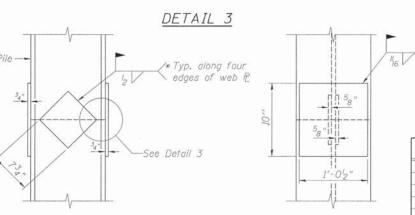
USER NAME = rgrimm	DESIGNED - JLS	REVISED -	
	CHECKED - LRB	REVISED -	
PLOT SCALE =	DRAWN - RMG	REVISED -	
PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	
	PLOT SCALE =	CHECKED - LRB	CHECKED - LRB REVISED - PLDT SCALE = DRAWN - RMG REVISED -











LEGEND

- → Vertical HP14x89 Pile
- HP14x89 Pile at 3:12 Forward Batter
-) HP14x89 Test Pile

BILL OF MATERIAL

DESCRIPTION	UNIT	QUANTITY
Furnishing Steel Piles HP14x89	Foot	5,705
Driving Piles (Special)	Foot	5,705
Test Pile Steel HP14x89	Each	5
Pile Shoes (Special)	Each	60

WELDED PLATE FIELD SPLICE

- * Interrupt welds 4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (516" min.).

ELEVATION

PILE DRIVING NOTES:

/*Typ. along four

edges of flange P

- All End Bent piles shall be driven to 50 ton capacity, all four End Bent Wingwall piles shall be driven to 12 ton capacity, and all intermediate bent piles shall be driven to 70 ton capacity. If any pile cannot be driven to this capacity, the Engineer must be notified.
- Estimated capacity of driven piles shall be calculated using the Modified ENR Formula with F.S. = 5, unless otherwise noted. Direct questions to the Engineer.
- The steel H-piles shall be according to ASTM A572, Grade 50.

engineers - scientists - planners 312-565-0450 Job No. 10074						
FILE NAME =	USER NAME = rgrimm	DESIGNED - JLS	REVISED -			
		CHECKED - LRB	REVISED -			
Perm_Br_013_Pile_Layout.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -			
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -			

WELDED COMMERCIAL SPLICE

benesch Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

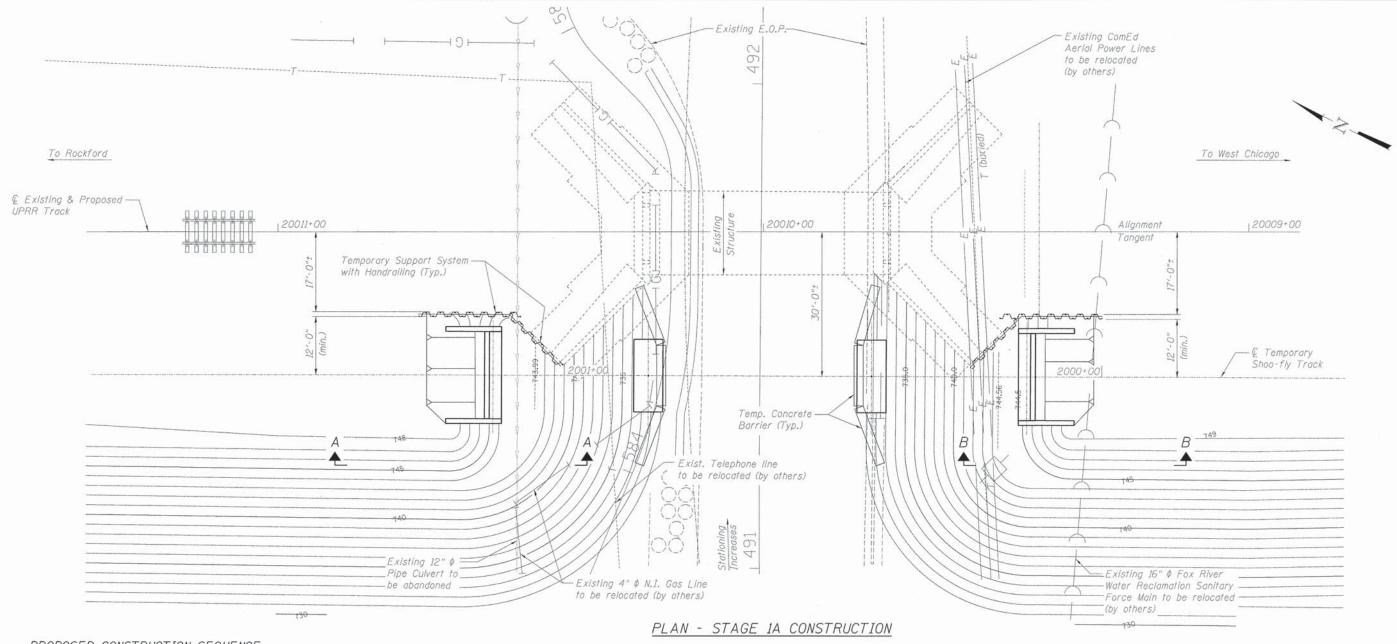
WELDED COMMERCIAL SPLICE ALTERNATE

M.P. 37.71 BELVIDERE SUBDIVISION — STRUCTURE NO. 045-3168 FOUNDATION PLAN AND PILE DETAILS

SHEET NO. UP-13 OF UP-52 SHEETS

END VIEW

.P.	SECTION	COUNTY	TOTAL	SHEET NO.
51	06-00214-18-RP	KANE	451	273
		CONTRAC	T NO. 6	3598
	ILLINOIS FED. A			



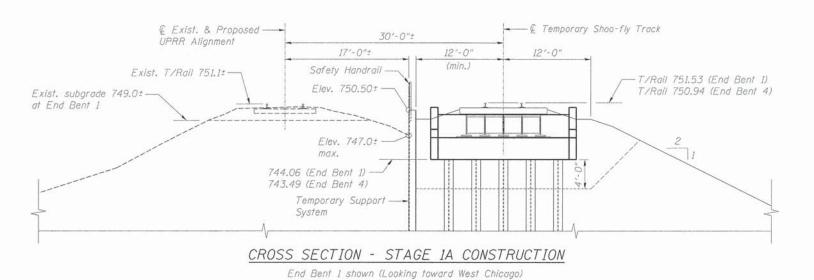
PROPOSED CONSTRUCTION SEQUENCE

Stage IA - Construct Temporary Shoo-fly bridge and embankment.

Maintain rail traffic on existing UPRR track.

Maintain existing vehicular traffic patterns on IL 25/Stearns Road except for night time closures to erect the temporary span over the roadway, and for removal of the existing bridge span.

- Construct portion of shoo-fly embankment to 4 feet below the bottom of cap elevation of the new abutments for the temporary
- 2. Install Temporary Support System as shown to protect existing UPRR track.
- 3. Drive steel pipe piling and construct the abutments and bents for the temporary bridge.
- 4. Complete construction of the shoo-fly embankment. 5. Complete construction of the shoo-fly bridge.
- 6. During a track outage, cut-over UPRR track to the shoo-fly
- 7. Remove existing bridge superstructure.



See sheet UP-16 for Views A-A and B-B.

Perm_Br_014_Const_Stage1.dgn

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601

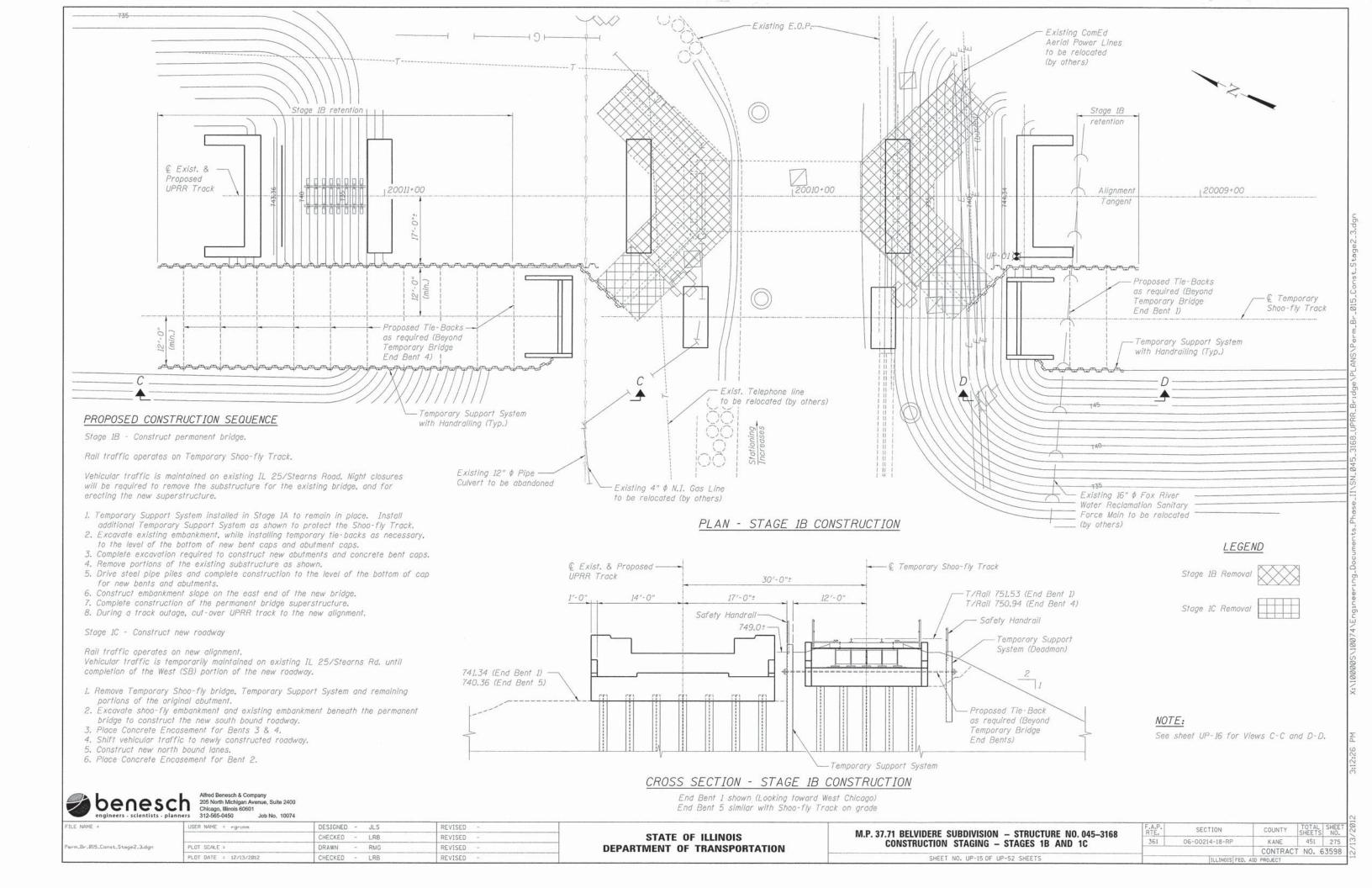
an	ners 312-565-0450 Job No. 100	74		
	USER NAME = rgrimm	DESIGNED - JLS	REVISED -	
		CHECKED - LRB	REVISED -	
	PLOT SCALE =	DRAWN - RMG	REVISED -	
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	

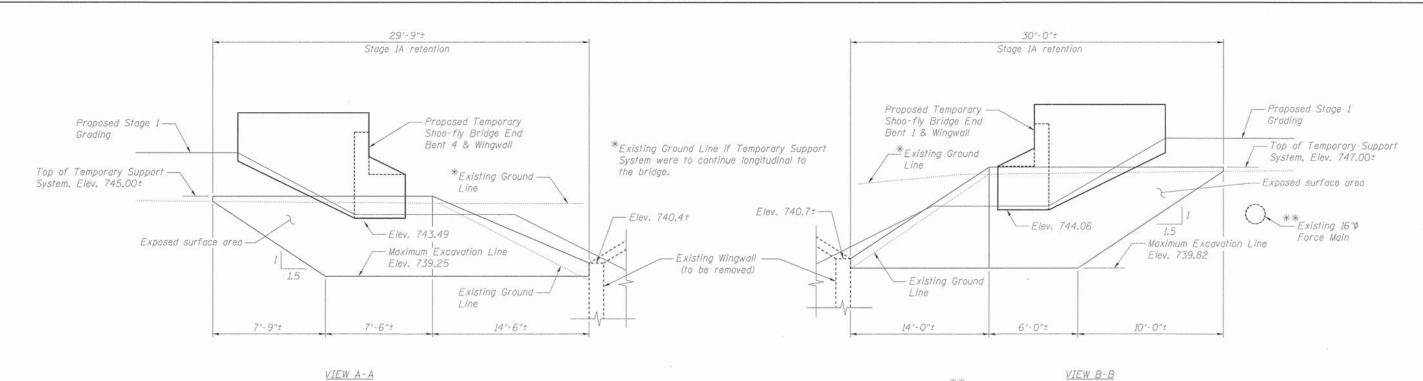
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

End Bent 4 similar

M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168 CONSTRUCTION STAGING - STAGE 1A SHEET NO. UP-14 OF UP-52 SHEETS

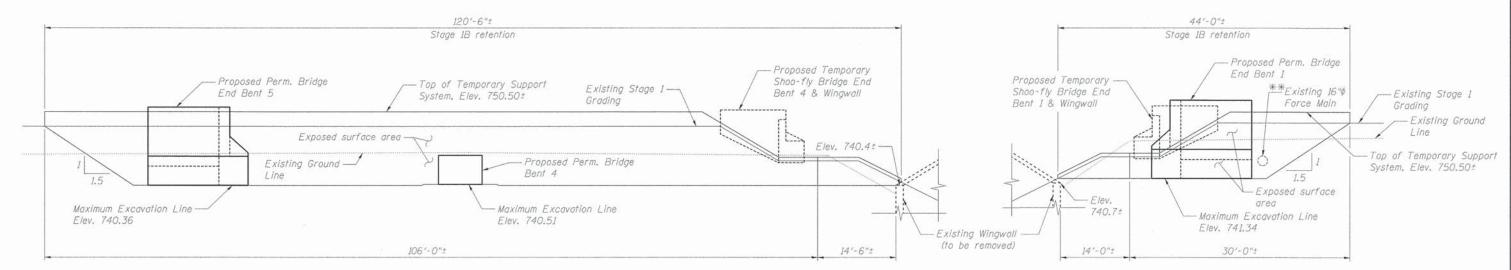
F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP	KANE	451	274
		CONTRAC	T NO. 6	3598
	ILLINOIS FED. A	ID PROJECT		-





TEMPORARY SUPPORT SYSTEM - ELEVATION
(Stage IA)

**(Existing 16" Force Main to be relocated by others, invert elevation unknown)



BILL OF MATERIAL

 (Stage IA)

 ITEM
 UNIT
 TOTAL

 *** Temporary Support System
 Sq. Ft. 214

VIEW C-C

TEMPORARY SUPPORT SYSTEM - ELEVATION (Stage 1B)

VIEW D-D

**(Existing 16" Force Main to be relocated by others, invert elevation unknown)

BILL OF MATERIAL

(Stage 1B)

	ITEM	UNIT	TOTAL
***	Temporary Support System	Sq. Ft.	990
			per de la

***For Information Only. See Note 3.

NOTES:

- 1. See sheet UP-14 for location of Views A-A and B-B.
- 2. See sheet UP-15 for location of Views C-C and D-D.
- The area measured for the Temporary Support System is the vertical exposed surface area envelope of the excavation supported by the Temporary Support System (For Information Only).
- All dimensions for the Temporary Support System are measured along the face of the wall.

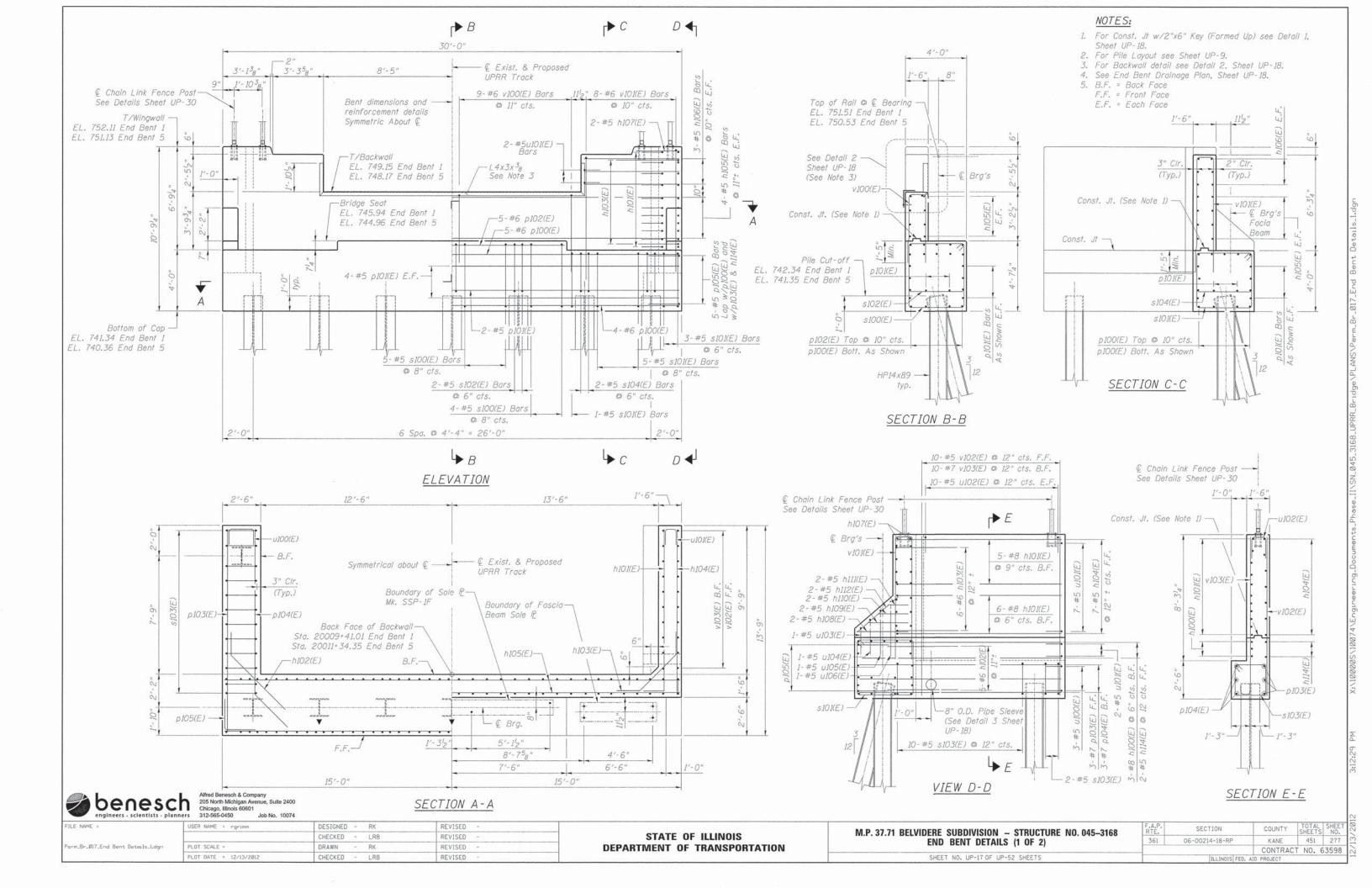
A DE	h h
	benesch
	engineers - scientists - planners

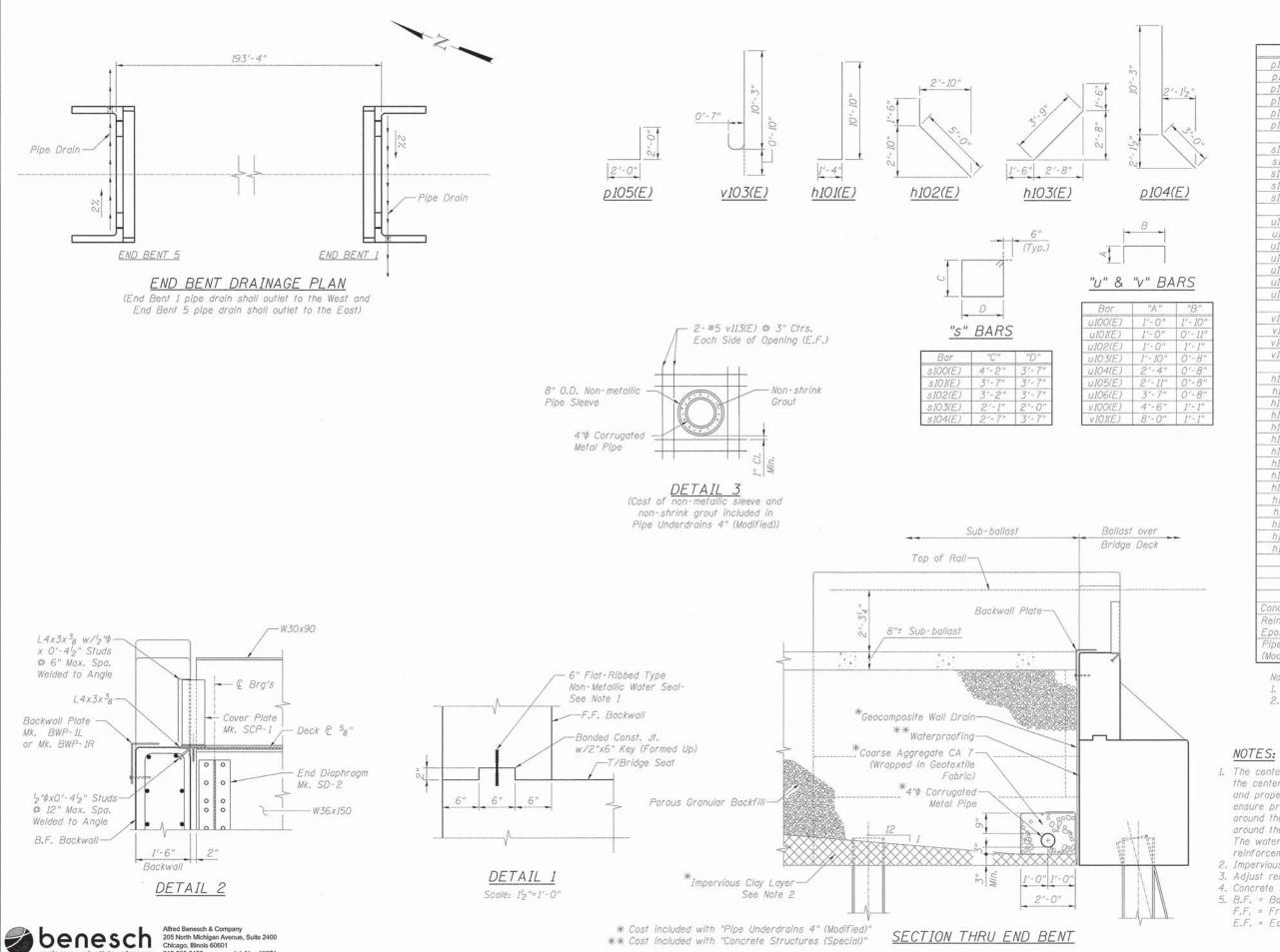
Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450 Job No. 10074

USER NAME = rgrimm	DESIGNED - JLS	REVISED -	
	CHECKED - LRB	REVISED -	
PLOT SCALE =	DRAWN - RMG	REVISED -	
PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	
	PLOT SCALE =	CHECKED - LRB PLOT SCALE = DRAWN - RMG	CHECKED - LRB REVISED - PLOT SCALE ** DRAWN - RMG REVISED -

M.P. 37.71	BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168 TEMPORARY SUPPORT SYSTEM DETAILS
	SHEET NO. UP-16 OF UP-52 SHEETS

RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP	KANE	451	276
		CONTRAC	T NO. 6	3598
	ILLINOIS FED.	AID PROJECT		





END BENT BILL OF BARS

Bar	No.	Size	Length	Shape
p100(E)	9	#6	29'-6"	
p101(E)	10	#5	29'-6"	-
p102(E)	5	#6	14'-8"	
p103(E)	6	#7	13'-3"	
p104(E)	6	#7	13'-3"	-
p105(E)	10	#5	4'-0"	
s100(E)	18	#5	16'-6"	
s101(E)	18	#5	15'-4"	
s102(E)	6	#5	14'-6"	
s103(E)	24	#5	9'-2"	
s104(E)	8	#5	13'-4''	
u100(E)	6	#5	3'-10''	
u101(E)	22	#5	2'-11"	
u102(E)	20	#5	3'-1"	n
u103(E)	2	#5	4'-4"	The second
u104(E)	2	#5	5'-4"	
u105(E)	2	#5	6'-6"	
u106(E)	2	#5	7'-10''	
O1001L/	fa.		7 20	
v100(E)	18	#6	10'-1"	
v101(E)	16	#6	17'-1"	
v102(E)	20	#5	10'-4"	
v103(E)	20	#7	11'-5"	
h100(E)	6	#8	13'-3"	
h101(E)	22	#8	12'-2"	1
h102(E)	10	#6	6'-6"	
h103(E)	12	#6	6'-9"	
h104(E)	14	#5	10'-10''	
h105(E)	8	#5	29'-8"	
h106(E)	12	#5	6'-3"	
h107(E)	4	#5	2'-10"	
h108(E)	4	#5	3'-4"	
h109(E)	4	#5	2'-8"	
h110(E)	4	#5	2'-0"	
h111(E)	4	#5	1'-4"	
h112(E)	4	#5	3'-6"	
h113(E)	16	#5	1'-6"	
h114(E)	4	#5	13'-3"	
**************************************	- 1		10 0	
Concrete Str	uctures (S	Special)	Cu. Yd.	40.9
Reinforcemen		-5-1011	Pound	5,560
Epoxy Coated Pipe Underdrains 4"			Foot	45
				64.7

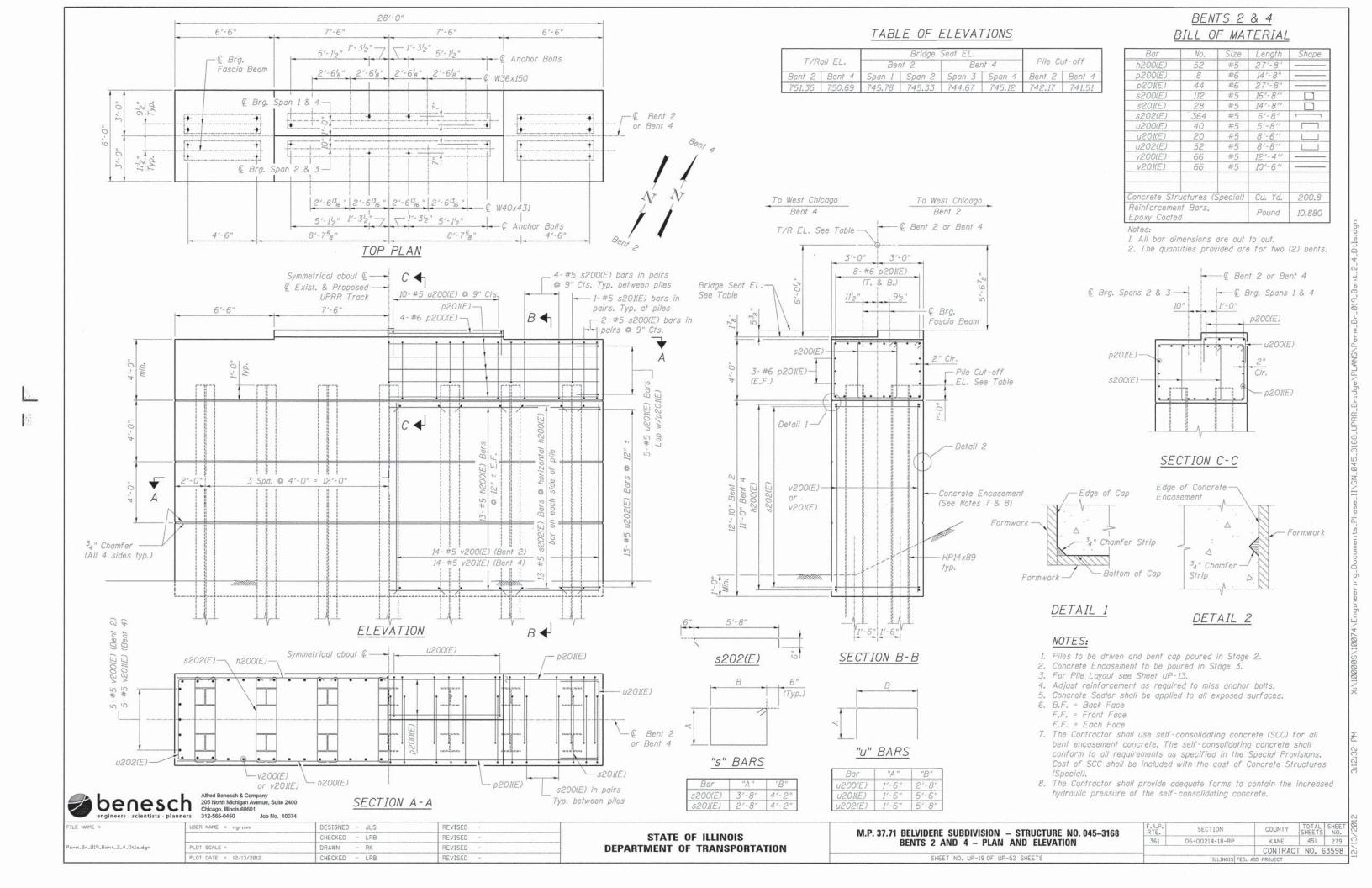
- 1. All bar dimensions are out to out.
- 2. The quantities provided are for one (1) end bent.

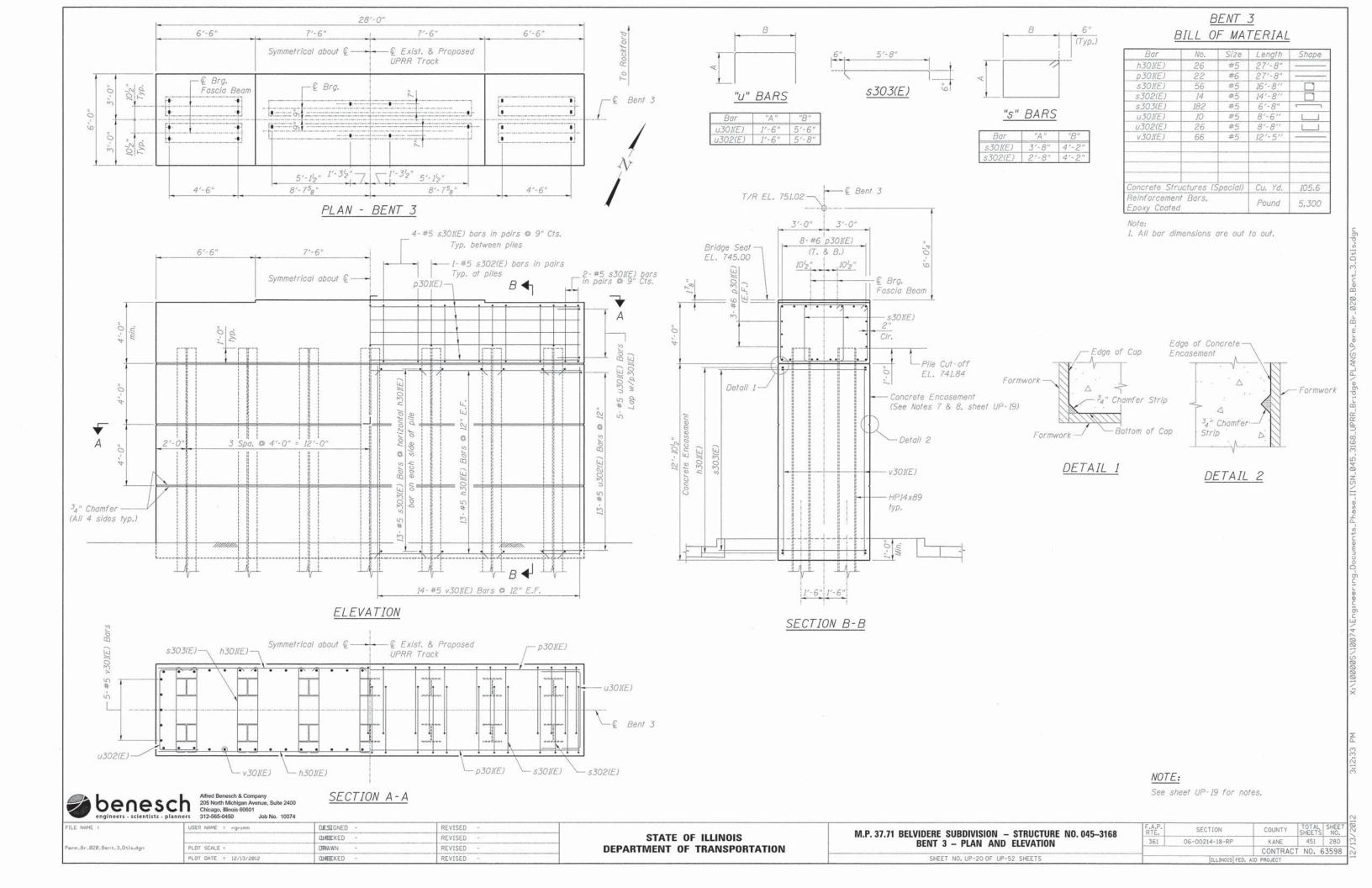
- 1. The centerline of the waterstop should be aligned with the center of the joint. The waterstop must be installed and properly secured prior to concrete placement to ensure proper positioning and concrete consolidation around the waterstop. Thoroughly consolidate the concrete around the waterstop to prevent voids or honeycombing. The waterstop shall not be cut or modified to allow reinforcement to pass through the waterstop.
- 2. Impervious clay layer required to maintain slope of subdrain.
- 3. Adjust reinforcement as required to miss anchor bolts.
- 4. Concrete Sealer shall be applied to all exposed surfaces.
- 5. B.F. = Back Face F.F. = Front Face
- E.F. = Each Face

STATE	OF	ILLINOIS	
DEPARTMENT O	FT	RANSPORTATION	

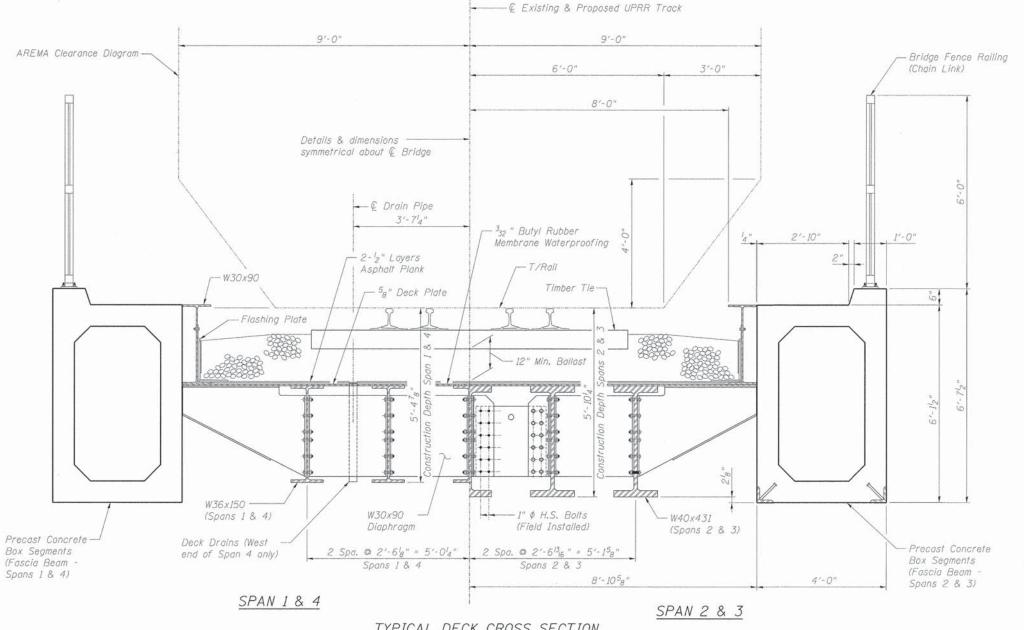
M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168	F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
END BENT DETAILS (2 OF 2)	361	06-00214-18-RP	KANE	451	278
			CONTRAC	T NO. 6	3598
SHEET NO. UP-18 OF UP-52 SHEETS	ILLINOIS FED. AID PROJECT				

FILE NAME = Perm_Br.818_End Bent Details_2.dgn	USER NAME = rgrimm	DESIGNED - RK	REVISED -
		CHECKED - LRB	REVISED -
	PLOT SCALE =	DRAWN - RK	REVISED -
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -

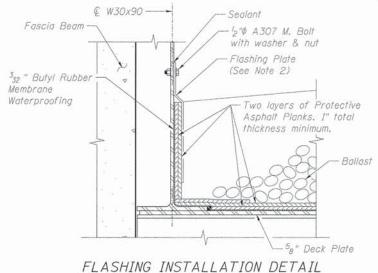








TYPICAL DECK CROSS SECTION



Construction Depth - Spans 1 & 4

Rail Tie P

Ballast (Min.)

2-1/2" Layers Asphalt Plank

Butyl Rubber Membrane Waterproofing

Deck Plate

35 ⁷₈" Beam W36x150

5'-478" ± Construction Depth (Min.)

Construction Depth - Spans 2 & 3

Rail

Tie P

Ballast (Min.)

2 1/2" Layers Asphalt Plank

332" Butyl Rubber Membrane Waterproofing Deck Plate

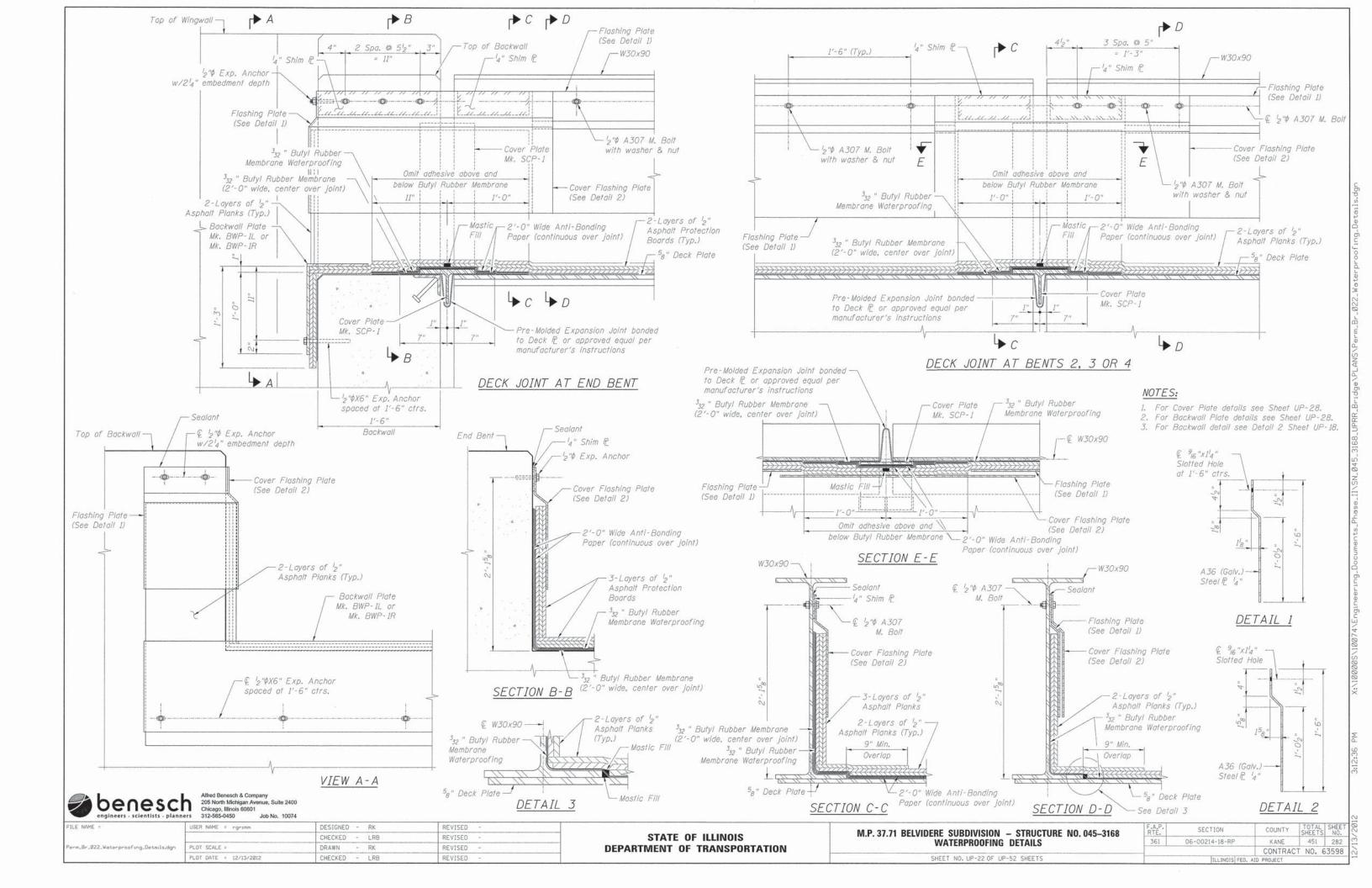
41'4" Beam W40x431

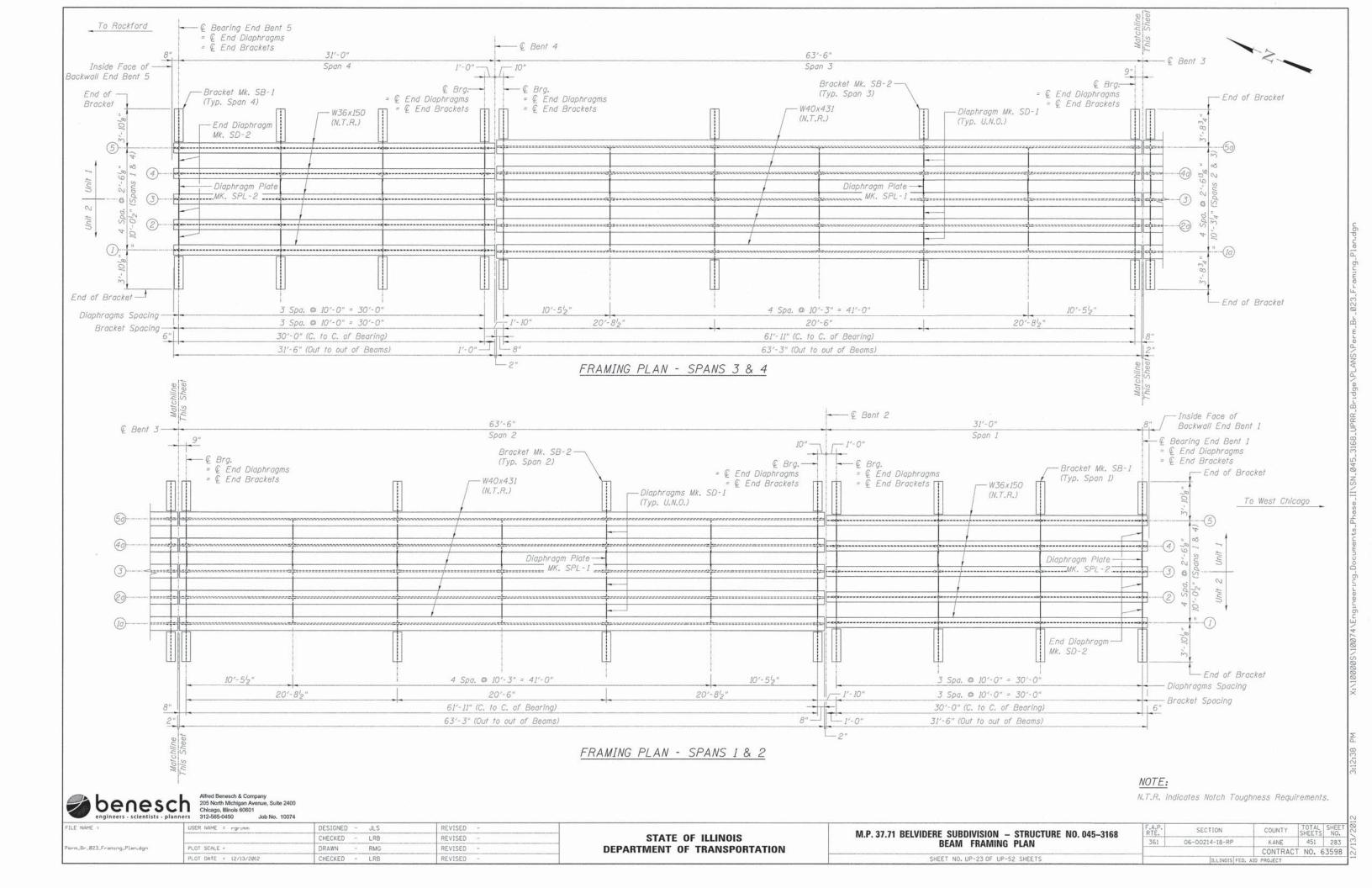
5'-10'4" ± Construction Depth (Min.)

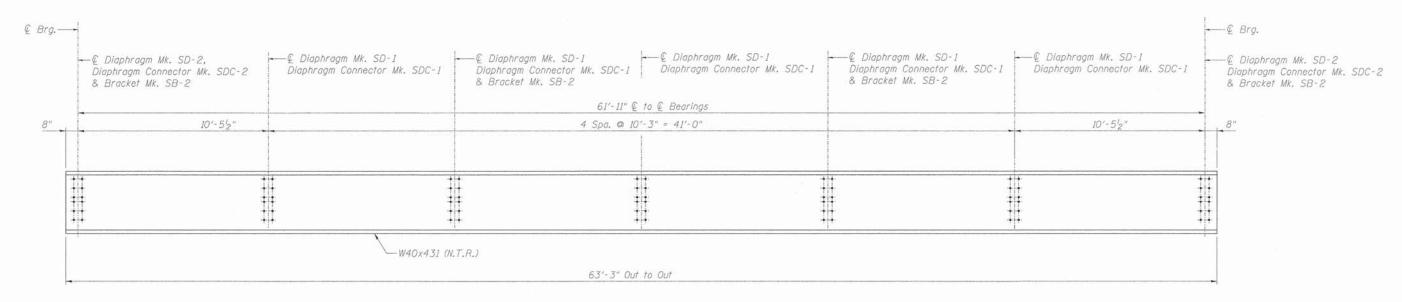
- 1. T/Rail to $T/Tie = 8\frac{1}{4}$ ". Dimension includes $7\frac{5}{16}$ " height of 136 lb. rail and a 15 " tie plate.
- 2. For Waterproofing Details see Sheet UP-22.

benes engineers - scientists - p			
FILE NAME :	USER NAME = rgrimm	DESIGNED - JLS	REVISED -
		CHECKED - LRB	REVISED -
Perm_Br_021_Deck_Section.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -

M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168	F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
DECK CROSS SECTION	361	06-00214-18-RP	KANE	451	281
			CONTRACT	NO. 6	33598
SHEET NO. UP-21 OF UP-52 SHEETS	ILLINOIS FED. AID PROJECT				

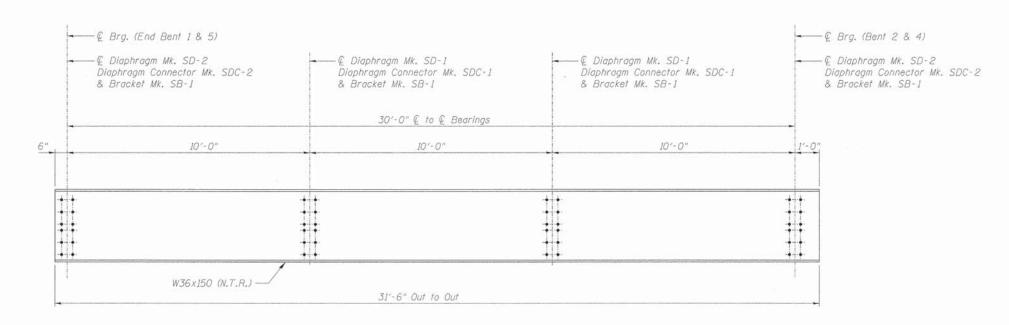






ELEVATION - BEAMS 1a, 2a, 3, 4a & 5a (SPANS 2 & 3)

10 Reg'd Est. Wt. = 27,270 LB, Each



ELEVATION - BEAMS 1 THRU 5 (SPANS 1 & 4)

10 Req'd Est. Wt. = 4,730 LB. Each

BEARING ON MASONRY

	Spans 1 & 4	Spans 2 & 3
Total Reaction	111 k	173 k
Net Bearing Area Furnished	168 in ²	219 in ²
Average Bearing Stress	661 psi	790 psi

1. N.T.R. indicates Notch Toughness Requirements. 2. Holes are $^{13}{16}$ " ϕ for $^{3}{4}$ " ϕ H.S. Bolts.

MOMENT AND SHEAR PER BEAM

		MOMENT			
		Spans I	8 4	S	pans 2 & 3
Dead Load		170 k	-f†		857 k-ft
Live Load		Alternate LL	394 k-ft	E80	1104 k-ft
Impact		195 k	-ft		487 k-ft
	Total	759 k	-ft		2,448 k-ft
Section - A709	Gr. 50W	W36x.	150		W40x431
Gross I Furnishe	ed	9,040	in ⁴		34,800 in ⁴
Net I Furnished		8,735	in ⁴		34,150 in ⁴
Net Section Modu	ulus Furnished	487	in 3		1.654 in ³
Allowable Max. S	tress in Flange	27.5	ksi		27.5 ksi
Actual stress	Tension	18.7 ksi		17.8 ksi	
in Flange	Compression	18.1 ksi			17.5 ksi
Allowable Max. D	Deflection	0.5	7"		1.16"
Actual Max. Def Live Load + Imp		0.3	7"		1.09"
		SHEAR			
Dead Load		23	k		56 k
Live Load		Alternate LL	59 k	E80	81 k
Impact		29	k		36 k
	Total	111	k		173 k
Web Shear		4.9 /	(si		3.1 ksi

Total estimated weight of structural steel = 499,840 Lbs. Quantity provided for information only.

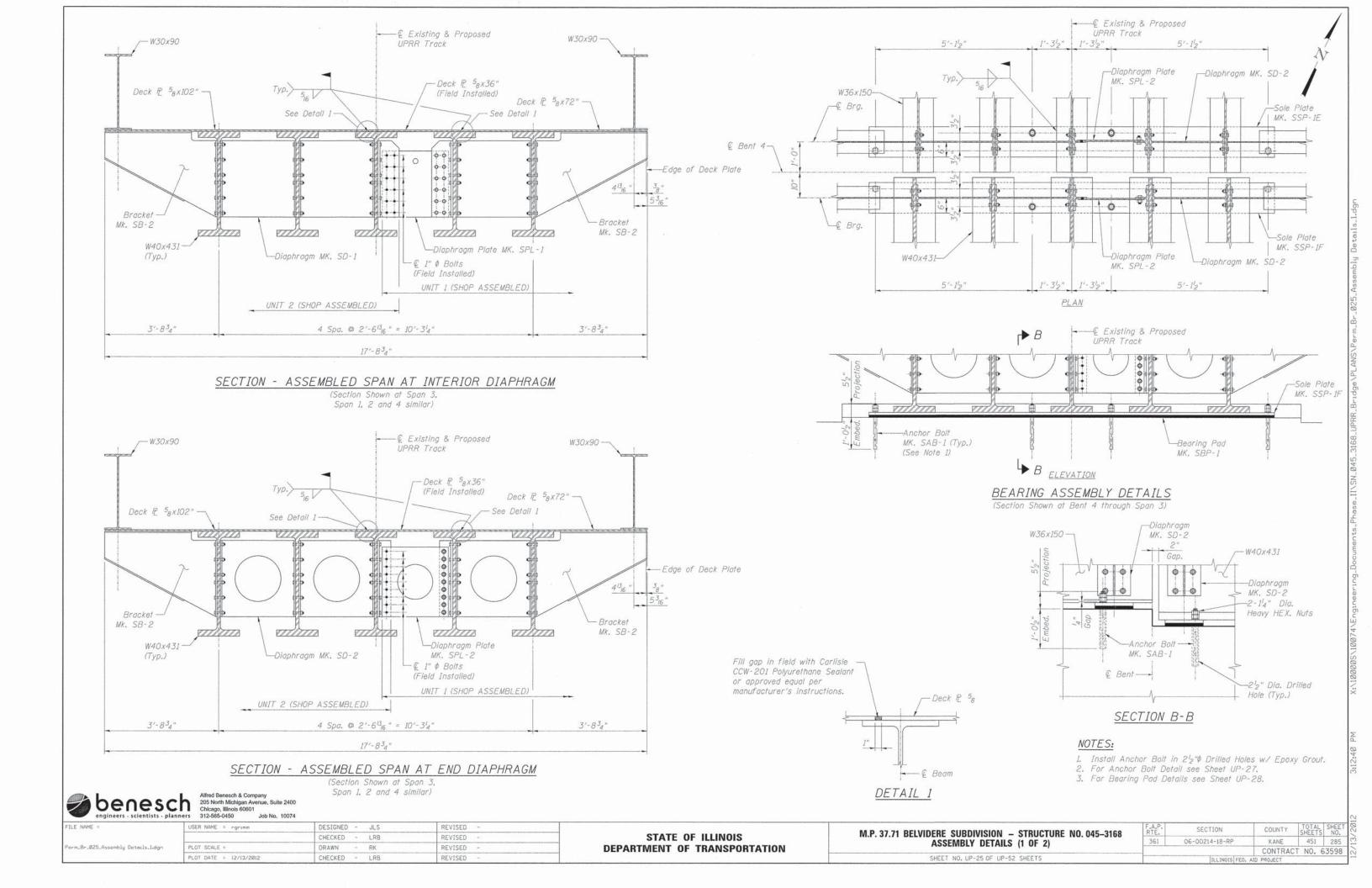
Perm_Br_024_Beam_Details.dgn

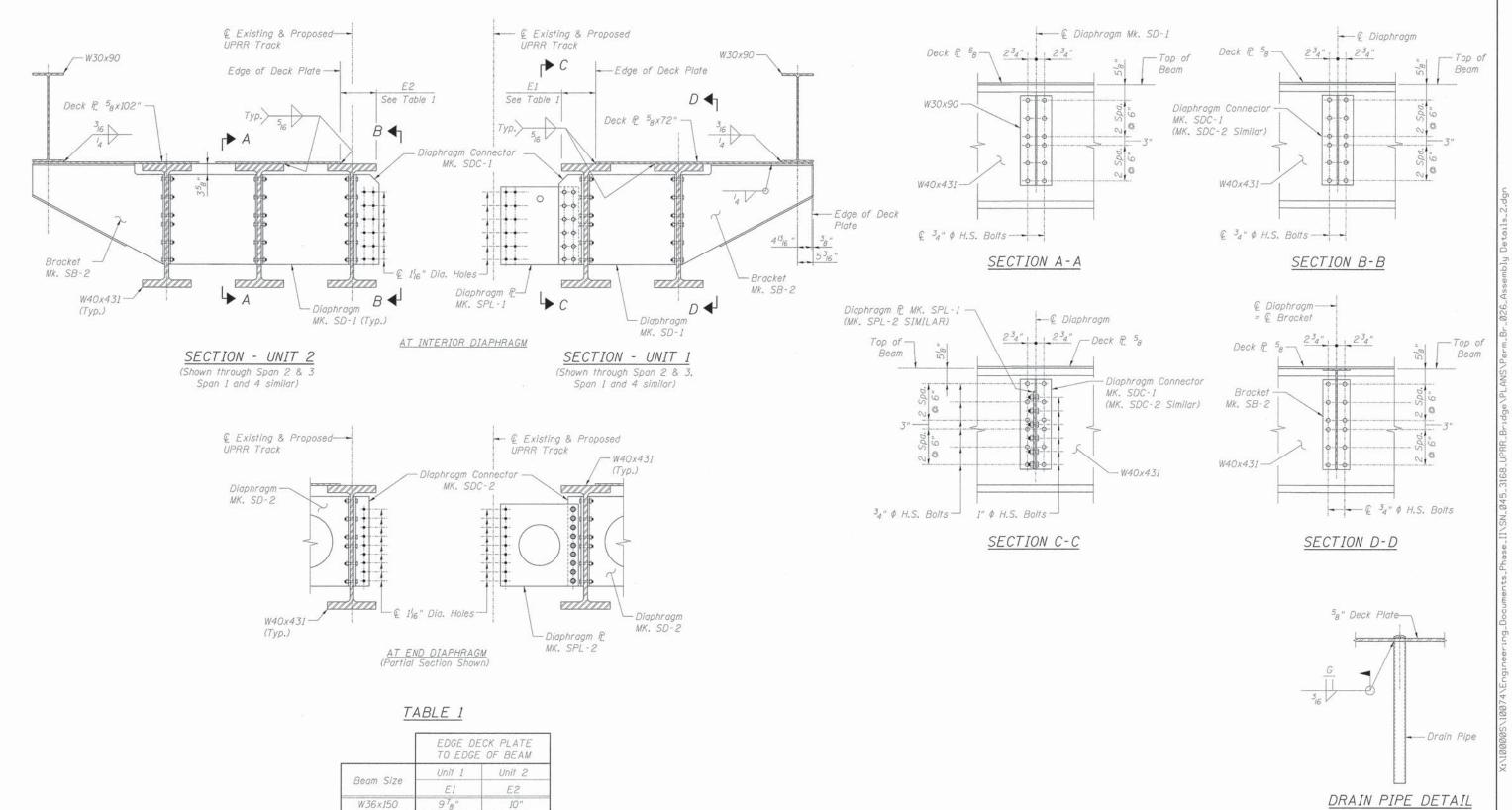
benesch Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-585-0450
Job No. 10074

 1613 512-505-0450 505 140. 100	1.4		
USER NAME = rgrimm	DESIGNED - JLS	REVISED +	
	CHECKED - LRB	REVISED -	
PLOT SCALE =	DRAWN - RMG	REVISED -	
PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	

M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168	F.A.P. RTE.	SECTIO
BEAM ELEVATION AND DESIGN DATA	361	06-00214-1
SHEET NO. LIP-24 OF LIP-52 SHEETS		10.1

A.P.	SECTION	COUNTY	TOTAL	SHEET NO.
361 06-00214-18-RP		KANE	451	284
		CONTRAC	T NO. 6	3598
	ILLINOIS FED. A	AID PROJECT		





L L	Α
benesch	21 C
engineers - scientists - planners	3

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 Job No. 10074

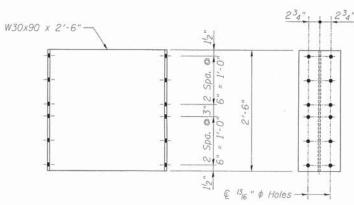
FILE NAME :	USER NAME = rgrimm	DESIGNED - JLS	REVISED -	
		CHECKED - LRB	REVISED -	
Perm_Br_026_Assembly Details_2.dgn	PLOT SCALE =	DRAWN - RK	REVISED -	
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	

W36x150 W40x431

1'-08'

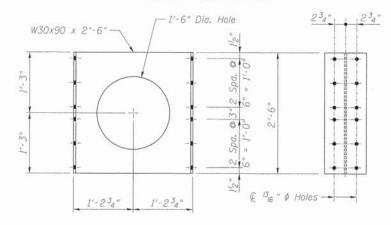
M.P. 37.71	BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168	
	ASSEMBLY DETAILS (2 OF 2)	
	CUEET NO UP OF OF UP SO CUEETS	

A.P. TE.	SECTION	COUNTY	TOTAL	SHEET NO.
61	06-00214-18-RP	KANE	451	286
		CONTRAC	T NO. 6	3598
	ILLINOIS FED. A			



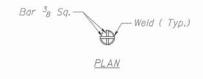
DIAPHRAGM MK. SD-1

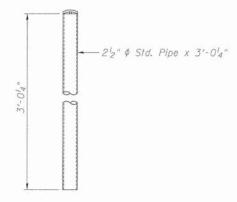
Est. Wt. = 225 LB. Each



DIAPHRAGM MK. SD-2

24 Req'd Est. Wt. = 225 LB. Each





ELEVATION

DRAIN PIPE

4 Req'd Est. Wt. = 18 LB. Each

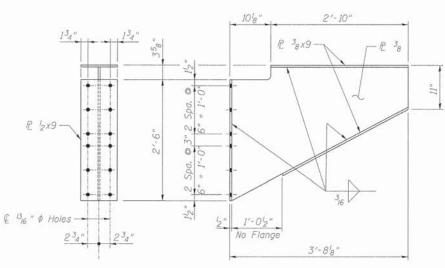
Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450 Job No. 10074

USER NAME = rgrimm DESIGNED - JLS REVISED CHECKED - LRB REVISED PLOT SCALE = DRAWN RMG REVISED

2'-111/6" 000 € 13₁₆ " \$\phi\$ Holes ------No Flange 3'-91316

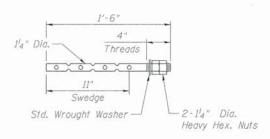
BRACKET MK. SB-1

16 Reg'd Est. Wt. = 212 LB. Each



BRACKET MK. SB-2

16 Req'd Est. Wt. = 205 LB. Each

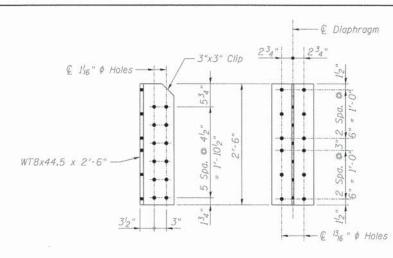


ANCHOR BOLT MK. SAB-1

32 Req'd Est. Wt. = 8.1 LB. Each

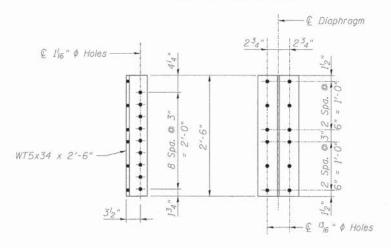
STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION



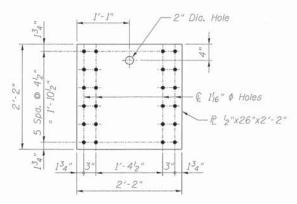
DIAPHRAGM CONNECTOR MK. SDC-1

28 Reg'd Est. Wt. = 112 LB. Each



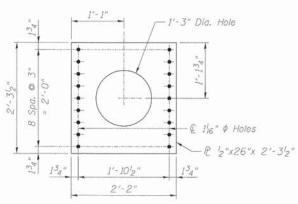
DIAPHRAGM CONNECTOR MK. SDC-2

Est. Wt. = 85 LB. Each



DIAPHRAGM PLATE MK. SPL-1

14 Req'd Est. Wt. = 96 LB. Each



DIAPHRAGM PLATE MK. SPL-2

8 Reg'd Est. Wt. = 102 LB. Each

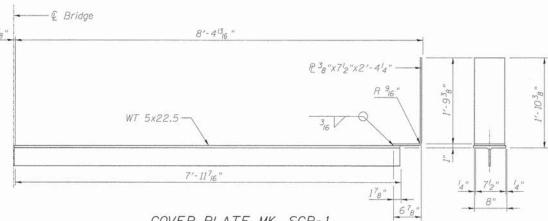
NOTE:

For Material Specification, see Structural Steel Notes Sheet UP-3.

M.P. 37.71	BELVIDERE SUBDIVISION — STRUCTURE NO. 045-3168 STRUCTURAL STEEL DETAILS (1 OF 2)

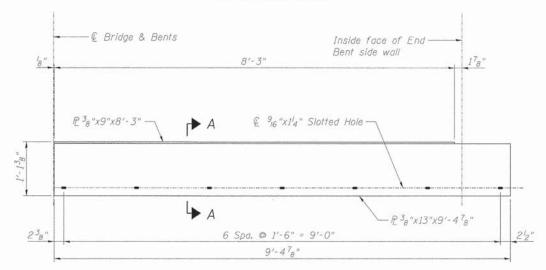
F.A.P. RTE.	SECTION		COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP		KANE	451	287
			CONTRACT	NO. 6	3598
	ILLINOIS	FED. AI	D PROJECT		

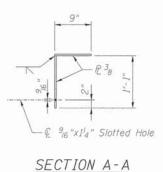
erm_Br_027_Steel_Details_1.dgn PLOT DATE = 12/13/2012 CHECKED REVISED



COVER PLATE MK. SCP-1

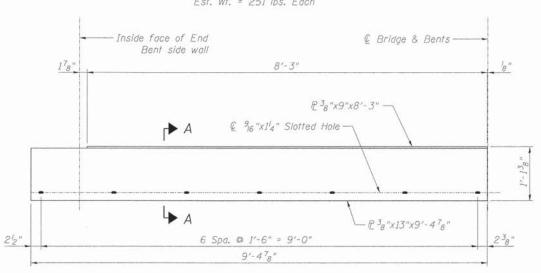
IO Req'd Est. Wt. = 202 lbs. Each





BACKWALL PLATE MK. BWP-1L

2 Reg'd Est. Wt. = 251 lbs. Each



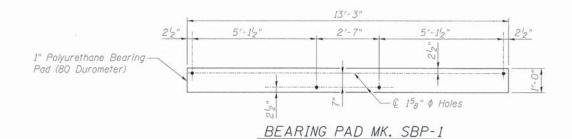
BACKWALL PLATE MK. BWP-1R

2 Reg'd Est. Wt. = 251 lbs. Each

benesch Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
Chicago, Illinois 60601

FILE NAME =	USER NAME = rgrzmm	DESIGNED - JLS	REVISED -	
		CHECKED - LRB	REVISED -	10.777
Perm_Br_028_Steel_Details_2.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -	
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -	

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION



13'-3" 5'-12" 2'-7" 5'-12" 158" ♦ Holes for SSP-1F P 1"x12"x13'-3" --

SOLE PLATES MK. SSP-1E & SSP-1F

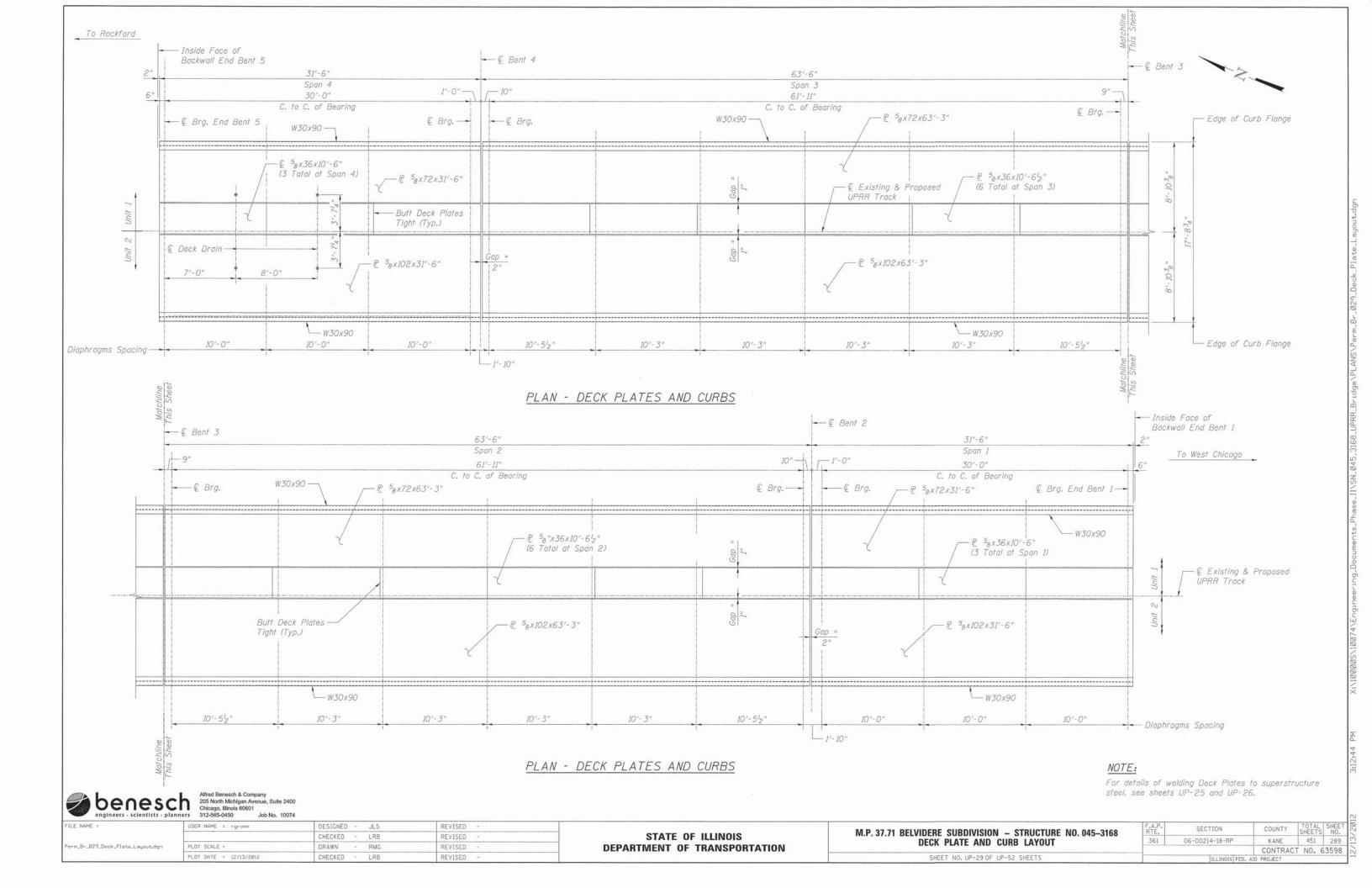
MK. SSP-IE Shown 4 Reg'd MK. SSP-IF 4 Req'd Est. Wt. = 541 lbs. Each

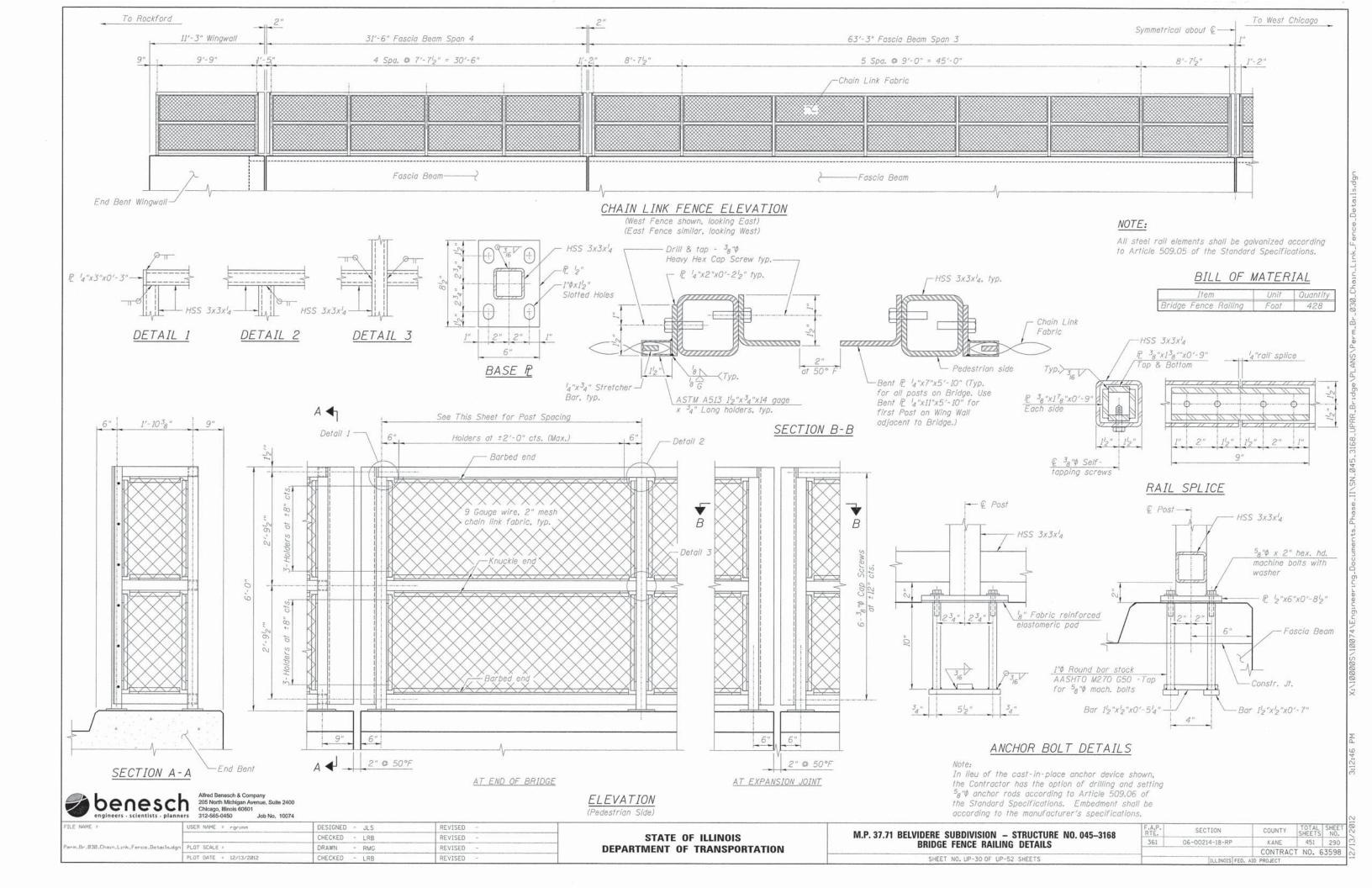
₹ 158" \$ x 3" Slotted Holes for SSP-1E

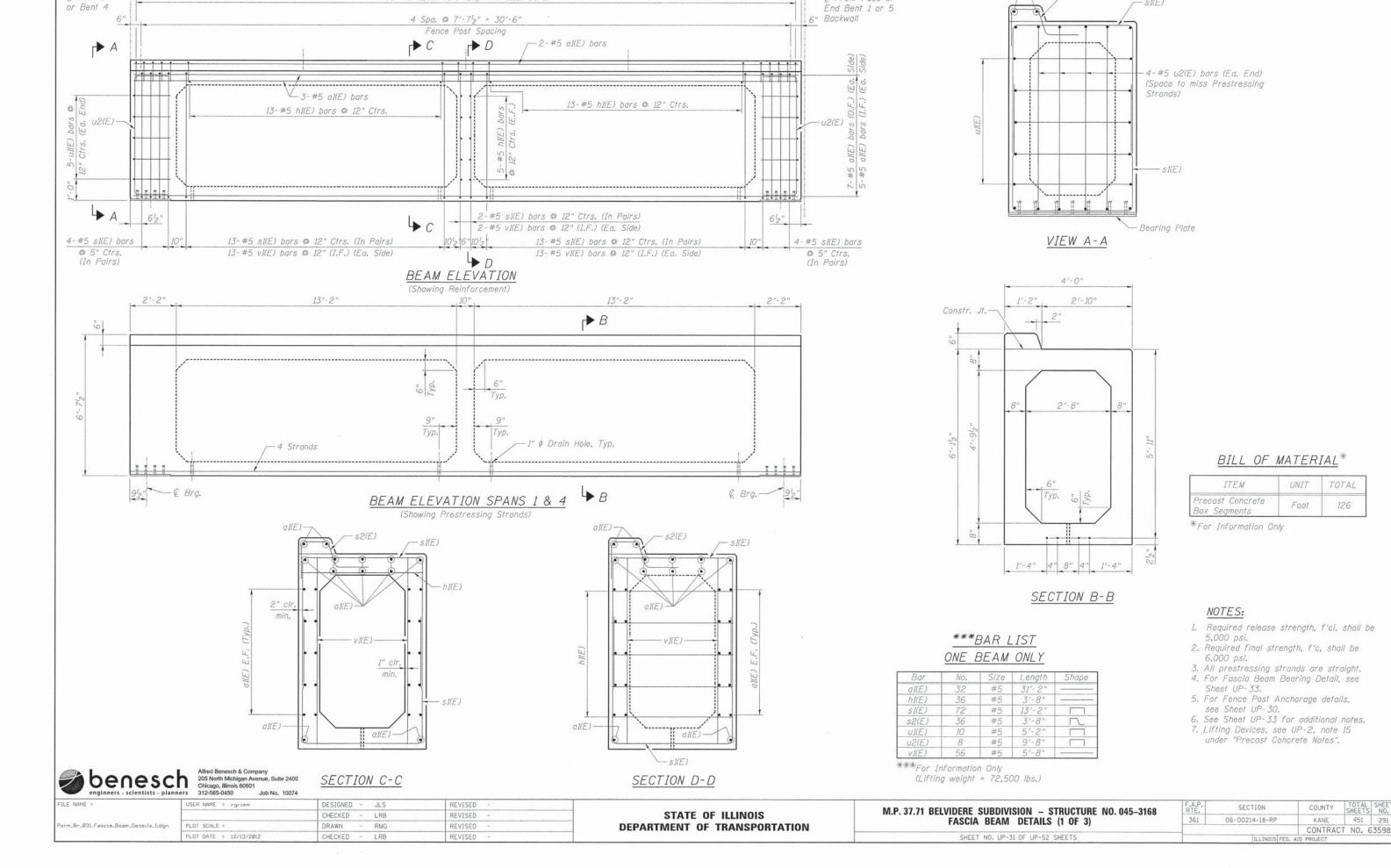
NOTES:

- 1. For Steel Material Specification, see Structural Steel Notes Sheet UP-3.
- 2. For Bearings Pad Material Specification, see Miscellaneous Material Notes Sheet UP-3.

SECTION M.P. 37.71 BELVIDERE SUBDIVISION - STRUCTURE NO. 045-3168 STRUCTURAL STEEL DETAILS (2 OF 2) 06-00214-18-RP KANE CONTRACT NO. 63598 SHEET NO. UP-28 OF UP-52 SHEETS







a1(E) -

- s1(E)

TOTAL

126

TOTAL SHEE NO.

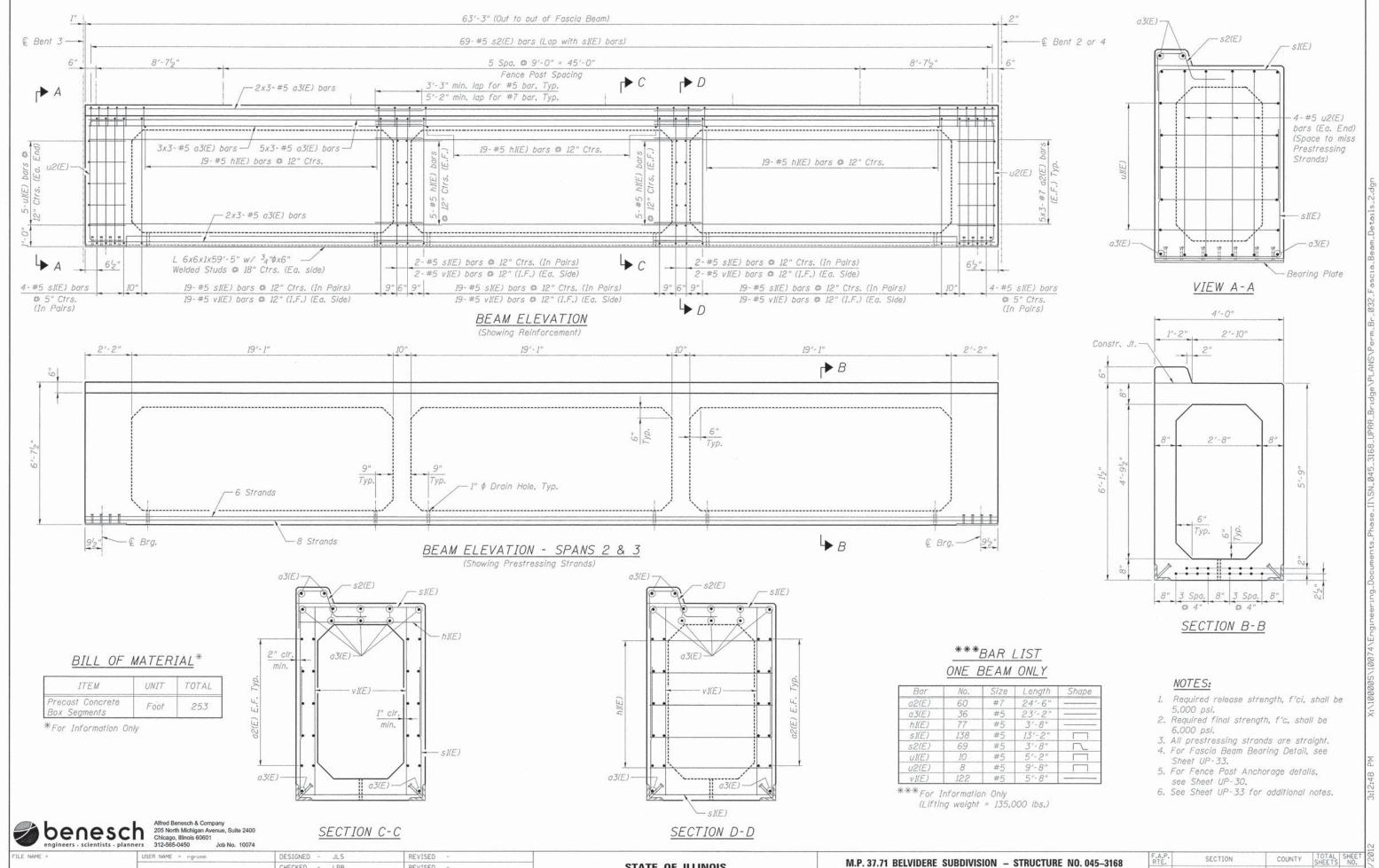
451 291

— € Front Face of

31'-6" (Out to out of Fascia Beam)

36-#5 s2(E) bars (Lap with sI(E) bars)

@ Bent 2 -



STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

CHECKED - LRB

RMG

DRAWN

erm_Br_032_Fascia_Beam_Details_2.dgn

PLOT SCALE :

PLOT DATE = 12/13/2012

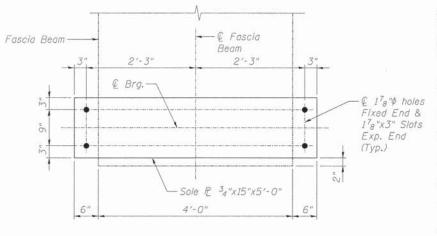
REVISED

REVISED

SECTION SHEETS NO. COUNTY KANE 06-00214-18-RP 451 292 CONTRACT NO. 63598 ILLINOIS FED. AID PROJECT

FASCIA BEAM DETAILS (2 OF 3)

SHEET NO. UP-32 OF UP-52 SHEETS



FASCIA BEAM BEARING DETAIL

38

Drill 21/2" \$ Holes

and set Anchor Bolts

in Non-Shrink Grout

--- € Fascia Beam

Polyurethane Bearing Pad

1"x15"x5'-0"

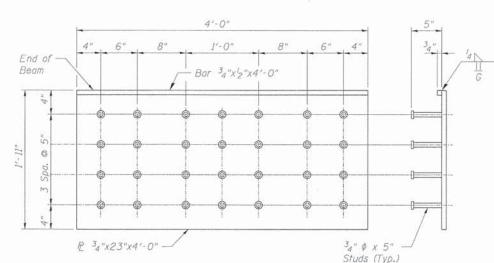
(80 Durometer)

- Anchor Bolt

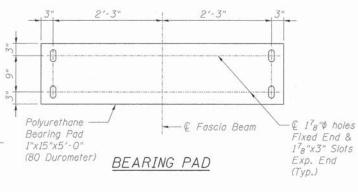
Mk. SAB-2

(See Detail)

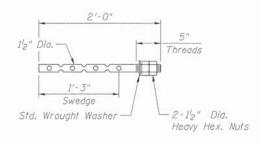
34"x15"x5'-0"



BEARING PLATE DETAIL

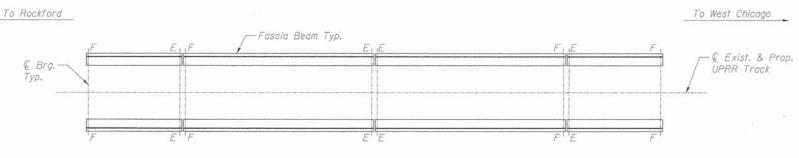


SOLE PLATE DETAIL



ANCHOR BOLT MK. SAB-2

64 Req'd Est. Wt. = 10.3 LB. Each



LOCATION PLAN



GENERAL NOTES:

Design, materials and construction of prestressed concrete beams shall be in accordance with the Current UPRR Standard Specifications and the current A.R.E.M.A. Manual for Railway Engineering, Chapter 8, Part 17 - Prestressed Concrete Design.

Ultimate compressive cylinder strength of beam concrete shall be not less than 5,000 p.s.i. at transfer of prestressing force, and 6,000 p.s.i. in 28 days.

Fixed End & Ultimate compressive cylinder strength of curb concrete shall be not less than 3,500 p.s.i. in 28 days.

Concrete shall be air-entrained containing 7% plus or minus 1% air by volume.

Maximum size of coarse aggregate shall be $^3\!_4$ inch.

Minimum concrete cover on reinforcement shall be 1'2 inches, except as noted.

All exposed edges shall be chamfered $^{3}_{4}$ inch.

All prestressing strands shall be $^{1}Z^{"}$ ϕ , 7 wire uncoated, low-relaxation, with minimum f's = 270,000 p.s.i. and otherwise meeting the requirements of the current A.S.T.M. designation: A416.

Initial prestress shall be 0.75 f's = 31,000 lbs. per strand.

Non-prestressing reinforcement shall be deformed bars meeting the current A.S.T.M. designation: A615, Grade 60. Fabrication of reinforcing steel shall be as per Chapter 7 of the current C.R.S.I. Manual of Standard Practice.

Dead Load: (Assumed - Ibs. per lin. ft. of Beam)

 Curb and Bridge Fence Railing
 135

 Beams
 2100

 Total Dead Load
 2235

Live Load: 100 psf over walkway width.

Impact: Not Used.

SPECIAL NOTES TO MANUFACTURER:

Production procedures and dimensional tolerances for the manufacture of precast, prestressed beams shall be in accordance with the Prestressed Concrete Institute's current manual MNL-116 for Quality Control.

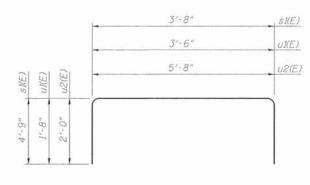
An alternate strand pattern better suited to the manufacturer's facilities, which has the same eccentricity as the pattern shown on this plan will be considered for approval prior to casting upon submission by the manufacturer of plans and computations.

If reinforcing bar supports are used, they shall be Class 1, plastic protected, in accordance with Chapter 3 of the C.R.S.I. Manual of Standard Practice,

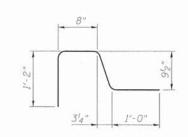
Manufacturer shall cut prestressing strands flush with ends of concrete beams and paint.

If lifted with slings instead of lifting loops, slings must not be placed more than 3'-0" from ends of Beams.

The walk surface on the top side of the Fascia Beam shall receive a non-slip broom finish in the transverse direction of the Beam (perpendicular to Beam length).



BARS s1(E), u1(E) & u2(E)



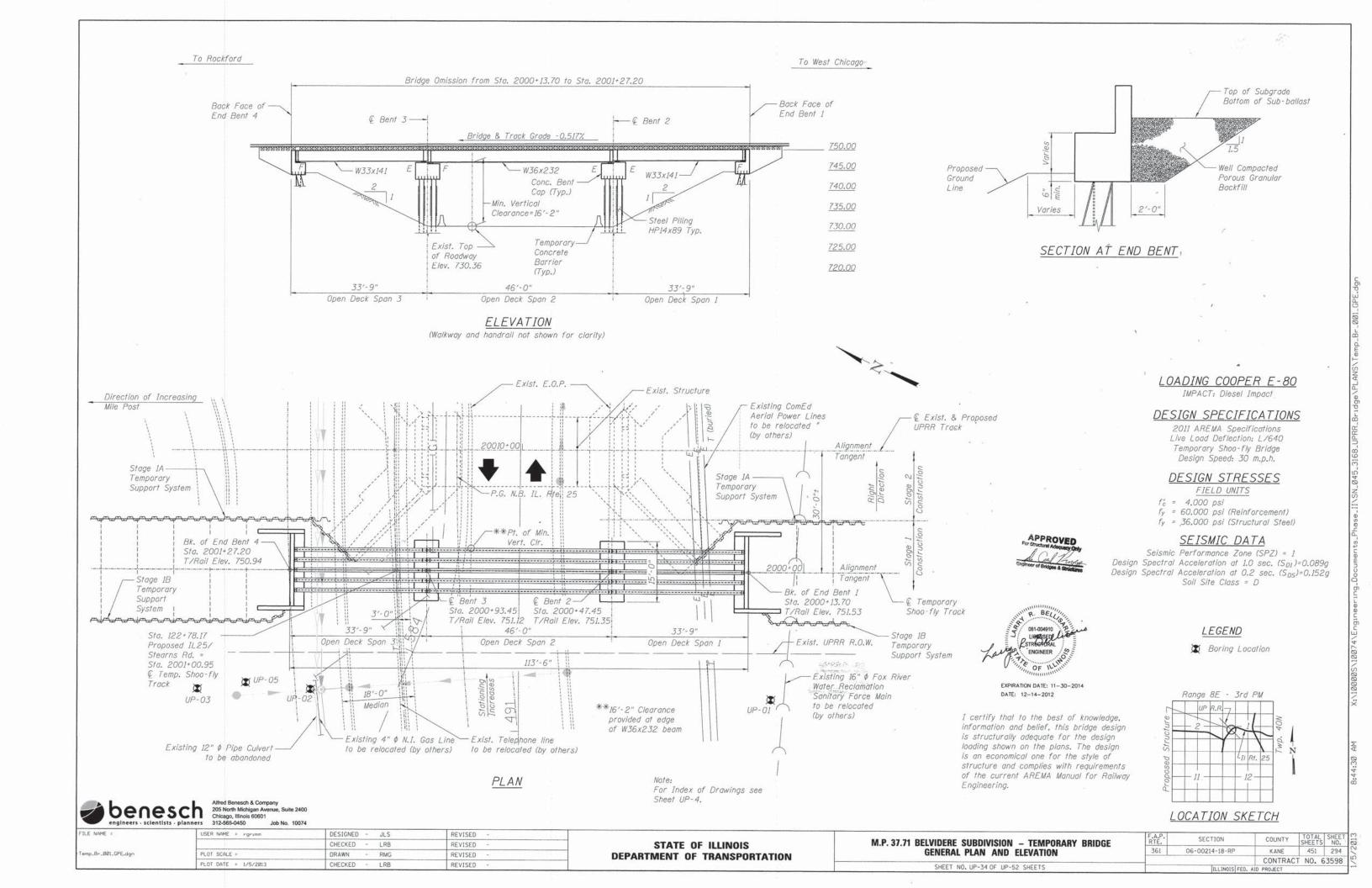
BAR s2(E)

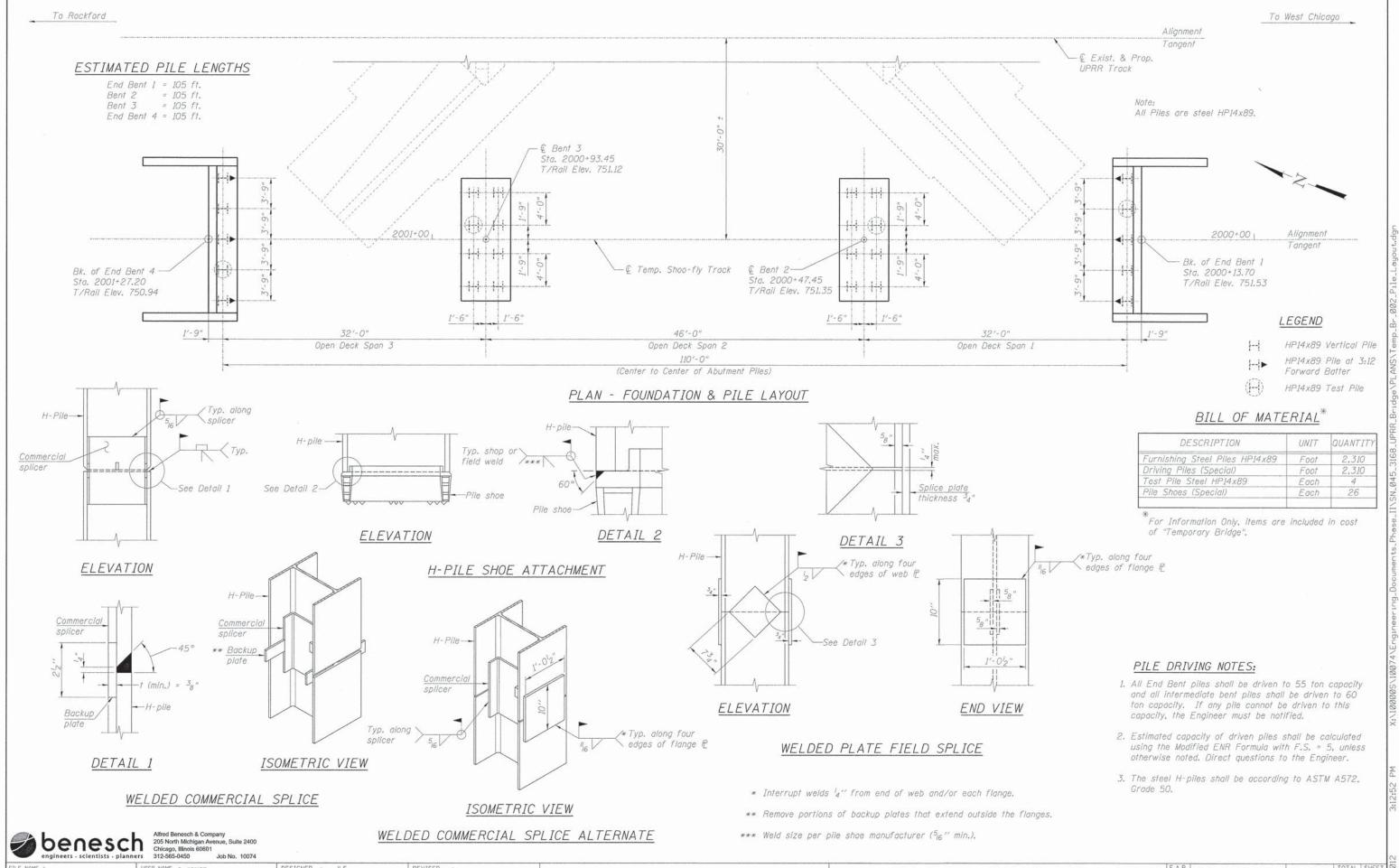
engineers - scientists - plann	ers 312-363-0430 Job No. 10074		
FILE NAME =	USER NAME = rgrimm	DESIGNED - JLS	REVISED -
		CHECKED - LRB	REVISED -
Perm_Br_033_Fescie_Beem_Details_3.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -

benesch Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601

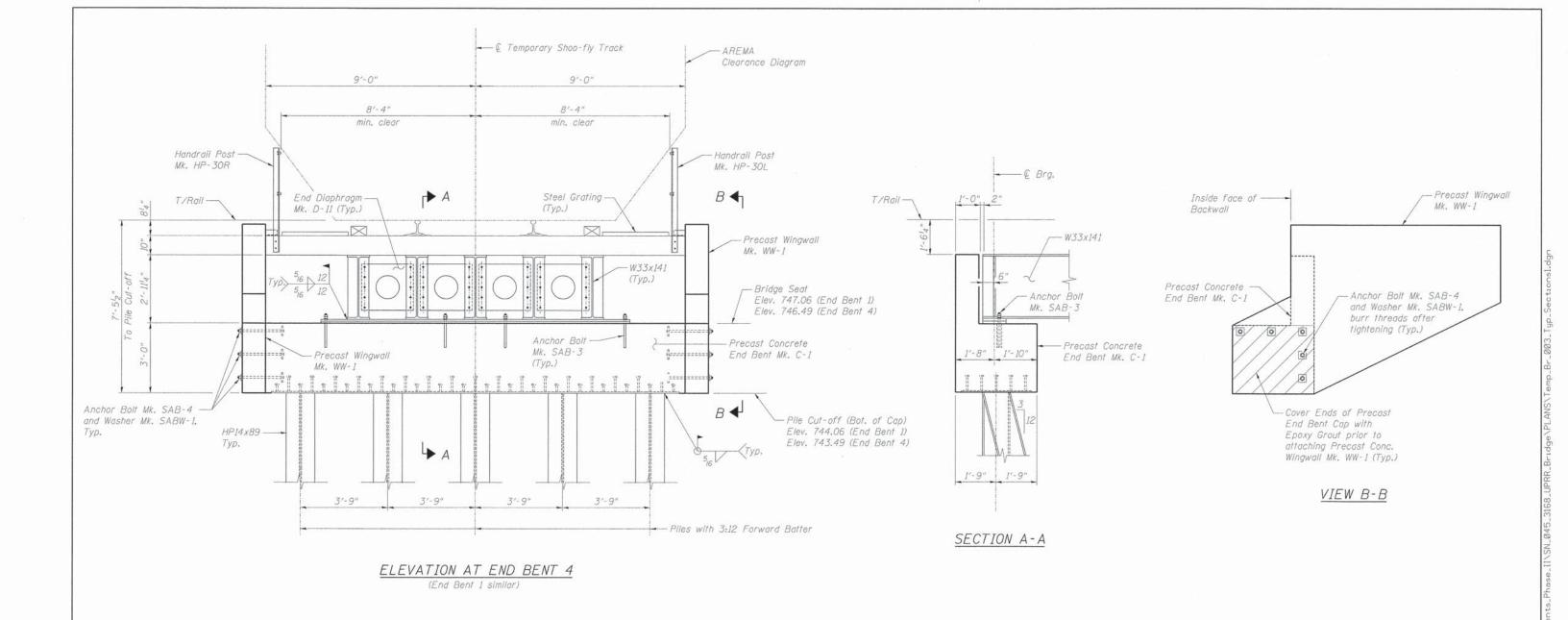
M.P. 37.71		SUBDIVISION – STRUCTURE NO. 045–3168 BEAM DETAILS (3 OF 3)
	SHEET	NO. UP-33 OF UP-52 SHEETS

.A.P.	SECTION	COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP	KANE	451	293
		CONTRAC	T NO. 6	3598
	ILLINOIS FED. A	ID PROJECT		





USER NAME = rgrime DESIGNED JLS REVISED TOTAL SHEE NO. SECTION COUNTY M.P. 37.71 BELVIDERE SUBDIVISION - TEMPORARY BRIDGE STATE OF ILLINOIS CHECKED -REVISED PILE LAYOUT AND DETAILS 361 06-00214-18-RP KANE 451 295 Temp_Br_002_Pile_Layout.dgn PLOT SCALE = DRAWN RMG REVISED DEPARTMENT OF TRANSPORTATION CONTRACT NO. 63598 PLOT DATE = 12/13/2012 SHEET NO. UP-35 OF UP-52 SHEETS LLINOIS FED. AID PROJECT



NOTES:

- 1. For Precast Concrete End Bent details see sheet UP-43.
- 2. T/Rail to T/Tie = $8\frac{I}{4}$ ". Dimension includes $7\frac{5}{16}$ " height for 136 lb. rail and a $\frac{15}{16}$ " tie plate.

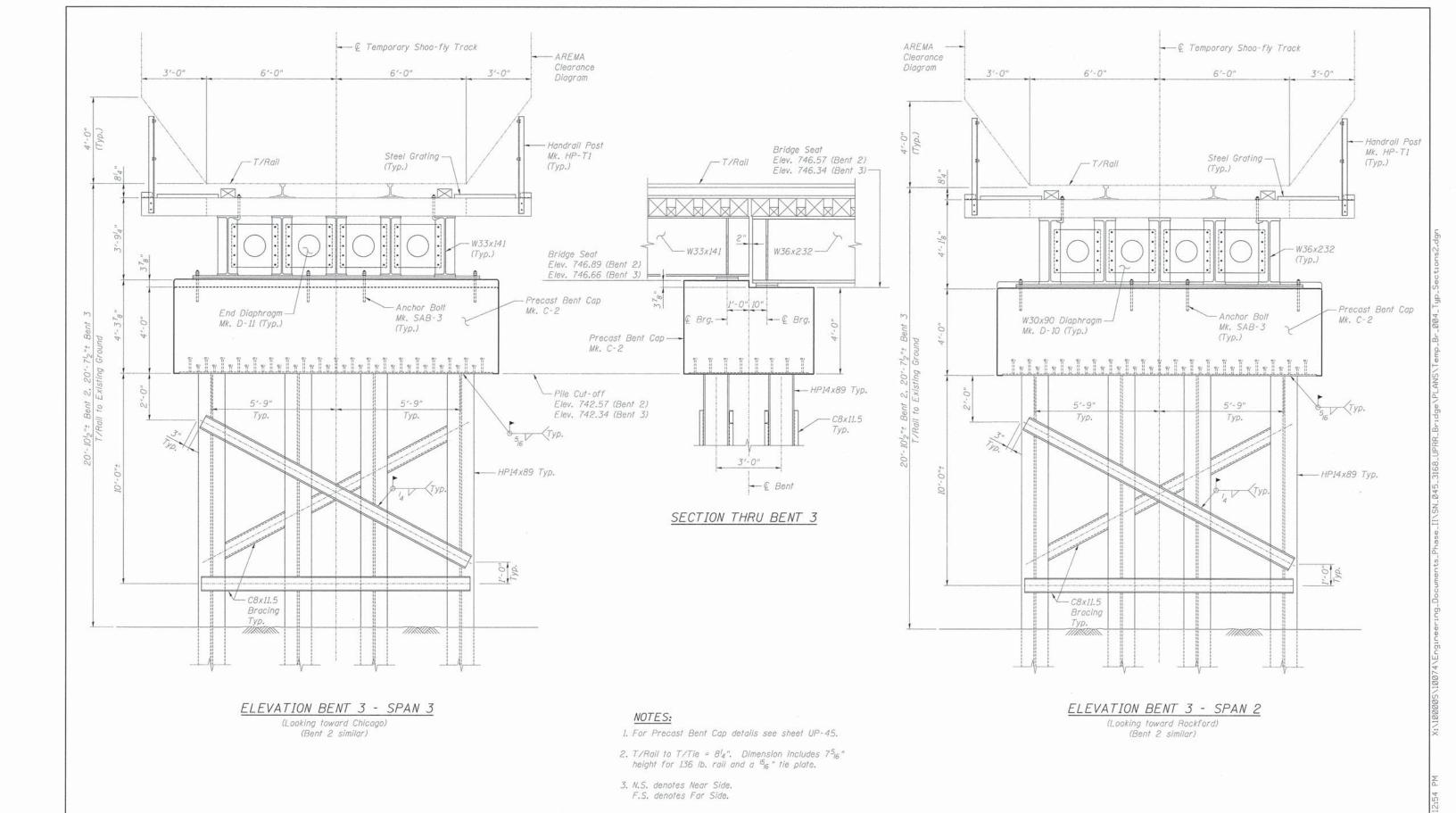
engineers · scientists · p		074	
FILE NAME =	USER NAME = rgrimm	DESIGNED - JLS	REVISED -
		CHECKED - LRB	REVISED -
Temp_Br_003_Typ_Sectionsl.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION M.P. 37.71 BELVIDERE SUBDIVISION - TEMPORARY BRIDGE END BENT DETAILS 06-00214-18-RP SHEET NO. UP-36 OF UP-52 SHEETS

CONTRACT NO. 63598

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400



benesch engineers - scientists - planners

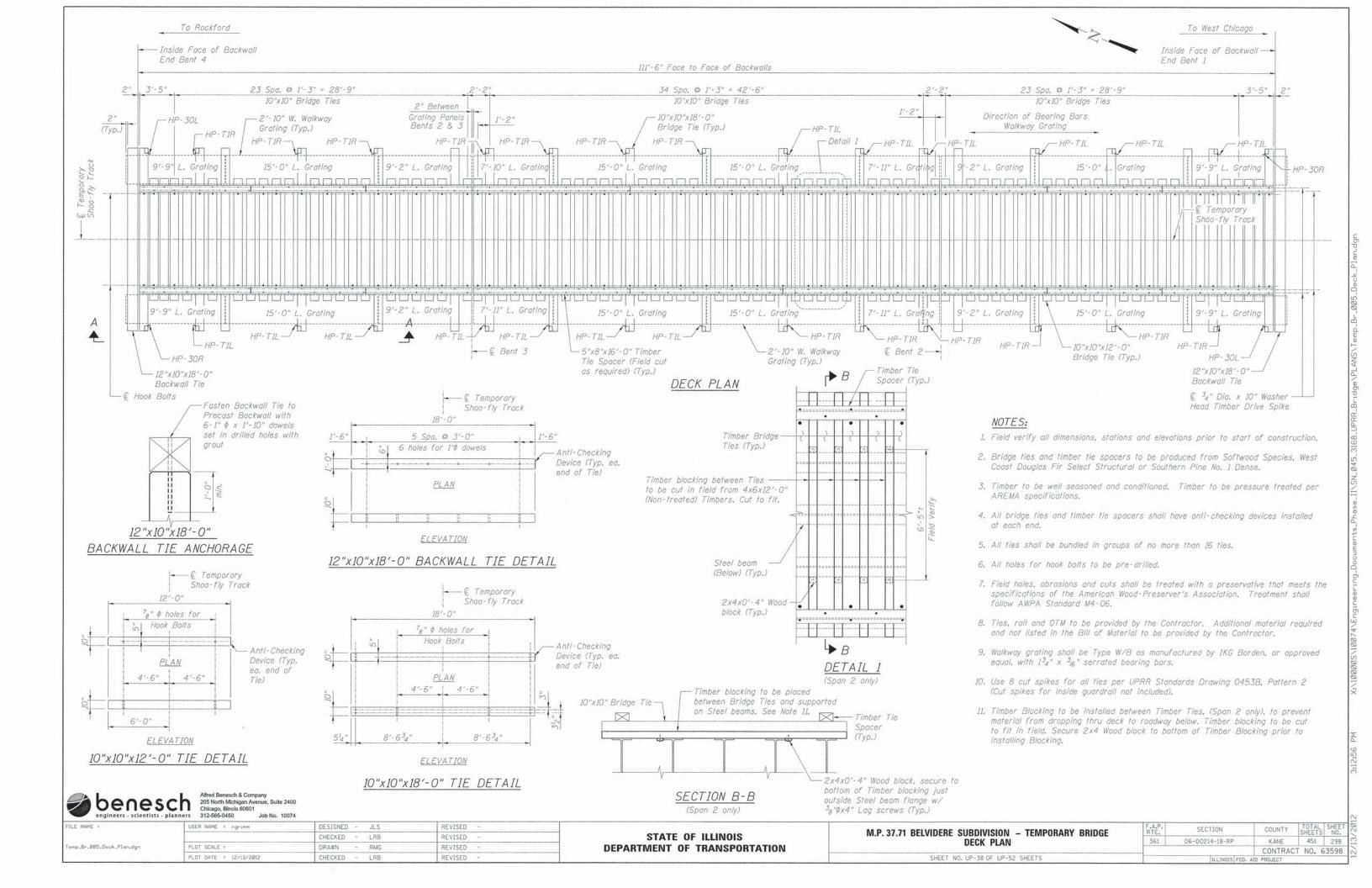
Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450 Job No. 10074

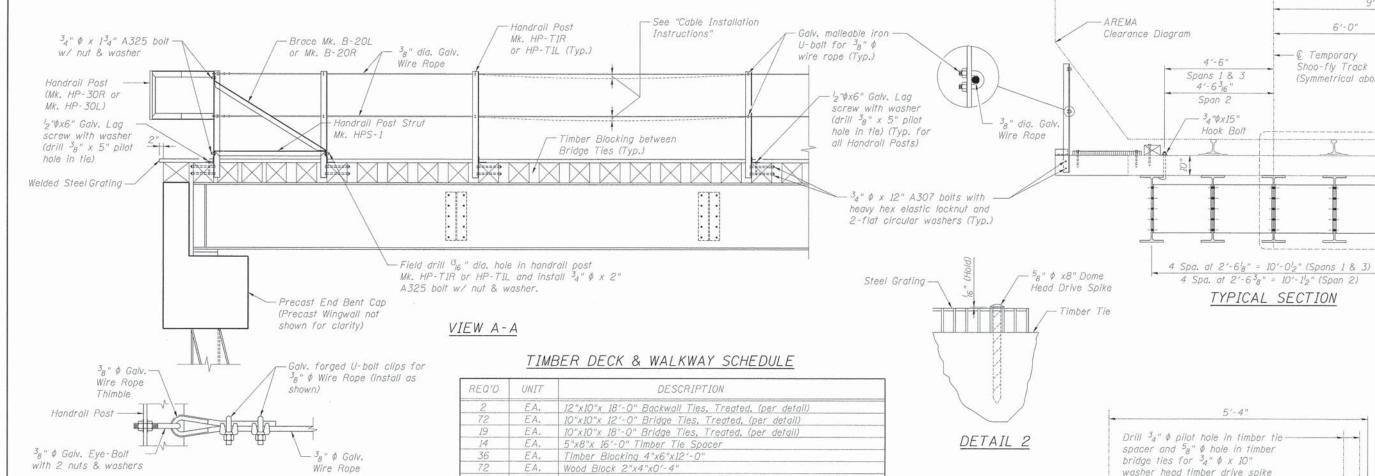
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

M.P. 37.71 BELVIDERE SUBDIVISION — TEMPORARY BRIDGE BENTS 2 AND 3 DETAILS

SHEET NO. UP-37 OF UP-52 SHEETS

.A.P.	SECTION	COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP	KANE	451	297
		CONTRAC	T NO. 6	3598
	ILLINOIS FED. A			





CABLE END INSTALLATION DETAIL

CABLE INSTALLATION INSTRUCTIONS (PER SPAN)

- 1. Prior to installing cable, make sure all Handrails Posts, Braces and Struts are installed with connections complete.
- 2. Thread Cable through all U-bolt clips and secure at End Posts. (Do not stretch)
- 3. Hang a minimum of 15 lbs. on cable between 2 Posts near mid-span, Span 2, and stretch cable to remove sag. Allow 2" max, sag to remain.
- 4. Tighten U-bolt clips and Eye-Bolts at End Posts.
- 5. Remove hanging weight(s).
- 6. Tighten U-bolt clips at intermediate Handrail Posts.

WALKWAY GRATING SCHEDULE

REQ'D	UNIT	DESCRIPTION
4	EA.	Welded Steel Grating (1^3_4 " x^3_{16} " serrated bars) 34" wide x 7'-11" long (galvanized)
4	EA.	Welded Steel Grating (1^3_4 " x^3_{16} " serrated bars) 34" wide x 9'-2" long (galvanized)
4	EA.	Welded Steel Grating (1^3_4 " x^3_{16} " serrated bars) 34" wide x 9'-9" long (galvanized)
8	EA.	Welded Steel Grating (1^3_4 " x^3_{16} " serrated bars) 34" wide x 15'-0" long (galvanized)

Estimated weight of walkway grating = 8,150 lb.

REO'D	UNIT	DESCRIPTION
2	EA.	12"x10"x 18'-0" Backwall Ties, Treated, (per detail)
72	EA.	10"x10"x 12'-0" Bridge Ties, Treated, (per detail)
19	EA.	10"x10"x 18'-0" Bridge Ties, Treated, (per detail)
14	EA.	5"x8"x 16'-0" Timber Tie Spacer
36	EA.	Timber Blocking 4"x6"x12'-0"
72	EA.	Wood Block 2"x4"x0'-4"
13	EA.	Handrail Post Mk, HP-T1R (Per detail)
13	EA.	Handrail Post Mk, HP-T1L (Per detail)
2	EA.	Handrail Post Mk. HP-30R (Per detail)
2	EA.	Handrail Post Mk. HP-30L (Per detail)
2	EA.	Brace Mk. B-20R (Per detail)
2	EA.	Brace Mk. B-20L (Per detail)
4	EA.	Handrail Post Strut Mk. HPS-1 (Per detail)
470	FEET	38" Nominal Diameter Wire Rope, 7 wire, galv. steel strand,
		Siemens Martins Grade, A-coating
16	EA.	Galv. Malleable Iron U-bolt Clips w/ 2 elastic locknuts
		(MIL-N-25027), zinc plated, for $\frac{3}{8}$ " dia. Wire Rope
52	EA.	Galv. Malleable Iron U-bolts w/ 2 elastic locknuts
	P-2-2-11/19	(MIL-N-25027), zinc plated, for $\frac{3}{8}$ " dia. Wire Rope
8	EA.	³ ₈ " φ Galv. Eye-Bolt with 2 nuts & 2 washers
8	EA.	³ ₈ " φ Galv. Wire Rope Thimble for ³ ₈ " dia. Wire Rope
8	EA.	$\frac{3}{4}$ " ϕ x $1\frac{3}{4}$ " A325 H.S. Bolt, Type 1 w/ Heavy Hex
		Nut (A563, lubricated) and flat circular washer (F436), each
		component hot dip or mechanically zinc coated
4	EA.	$^{3}4" \phi \times 2"$ A325 H.S. Bolt, Type 1 w/ Heavy Hex
		Nut (A563, lubricated) and flat circular washer (F436), each
		component hot dip or mechanically zinc coated
60	EA.	3 ₄ " ♦ x 12" A307 Heavy Hex Grade A Bolt, w/ elastic locknut
		(MIL-N-25027) & 2 - flat circular washer (F436), each
		component hot dip or mechanically zinc coated
30	EA.	I_2 " ϕ x 6" lag screw and flat circular washer, each component
		hot dip or mechanically zinc coated
182	EA.	³ ₄" ¢ x 10" washer head timber drive spike
104	EA.	$^{5}8" \phi \times 8"$ dome head drive spike (AREMA spec) (Galv.)
106	EA.	$\frac{3}{4}$ " ϕ x 15" hook bolt w/ 3" thread
106	EA.	#10 malleable washer for 34" dia. bolt
106	EA.	3 ₄ " #10 security locknut part SH 515
1	KEG	Spikes for wood ties per UPRR Standards Drawing No. 0451
1	LOT	1^3 4" \times^3 16" WB Serrated Walkway Grating (per schedule, this sheet
72	EA.	³ ₈ " φ x 4" Lag Screws
12	EA.	I" Φ x 1'-10" Steel Dowels

washer head timber drive spike (1 per tie, staggered) € Temporary — Shoo-fly Track Security locknut -136# rail -Malleable washer 5"x8" Timber Tie Spacer with 1516 " - Steel Grating tie plate (Typ.) 34" dia. Hook Bolt DETAIL 2

9'-0"

3'-0"

- See Detail 2

6'-0"

(Symmetrical about ©)

-⊈ Temporary

Shoo-fly Track

NOTES:

- 1. See sheet UP-40 for Handrail Details.
- 2. Install miscellaneous timber under ends of grating as required (Typ.).

benesch

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450 Job No. 10074 DESIGNED -JLS REVISED CHECKED LRB REVISED

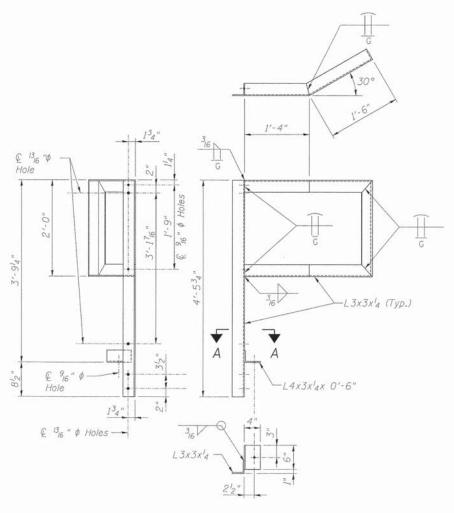
STATE OF ILLINOIS

M.P. 37.71 BELVIDERE SUBDIVISION - TEMPORARY BRIDGE DECK SECTIONS, DETAILS AND MATERIAL SCHEDULE SHEET NO. UP-39 OF UP-52 SHEETS

SECTION	COUNTY	TOTAL	SHEET NO.
06-00214-18-RP	KANE	451	299
	CONTRAC	T NO. 6	3598
ILLINOIS FED. AI	D PROJECT		
	06-00214-18-RP	06-00214-18-RP KANE	06-00214-18-RP KANE 451 CONTRACT NO. 6

USER NAME = rgrimm emp_Br_006_Deck_Section.dgr REVISED PLOT DATE = 12/13/2012 CHECKED LRB REVISED

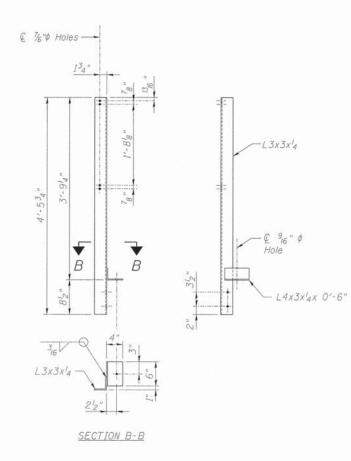
DEPARTMENT OF TRANSPORTATION



SECTION A-A

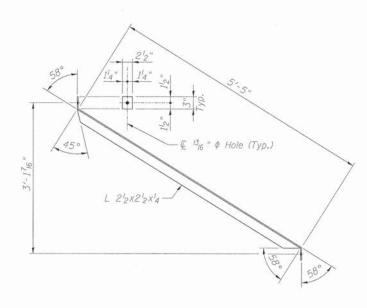
HANDRAIL POST MK. HP-30R

Scale: 1" = 1'-0" Est. Wt. = 62.0 lb. Ea. HANDRAIL POST MK. HP-30L (Opposite Hand) (2 Reg'd. each Mk. HP-30R/L)



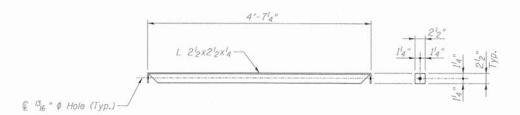
HANDRAIL POST MK. HP-TIR

Scale: 1" = 1'-0" Est. Wt. = 24.4 lb. Ea. HANDRAIL POST MK. HP-T1L (Opposite Hand) (13 Req'd. each Mk. HP-T1R/L)



HANDRAIL BRACE MK. B-20R

Scale: 1" = 1'-0" Est. Wt. = 24.4 lb. Ea. HANDRAIL BRACE MK. B-20L (Opposite Hand) (4 Reg'd)



HANDRAIL POST STRUT MK. HPS-1

Scale: 1" = 1'-0" Est. Wt. = 20.6 lb. Ea. (4 Req'd)



engineers - scientists - p	lanners 312-565-0450 Job No. 100	174	
FILE NAME =	USER NAME = rgrimm	DESIGNED - JLS	REVISED -
		CHECKED - LRB	REVISED -
Temp_Br_007_Handrail_Dtls.dgn	PLOT SCALE =	DRAWN - RMG	REVISED -
	PLOT DATE = 12/13/2012	CHECKED - LRB	REVISED -

M.P. 37.71 BELVIDERE SUBDIVISION – TEMPORARY BRIDGE	F.A
HANDRAIL DETAILS	
CUEST NO UP 40 OF UP 50 CUESTS	

F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
361	06-00214-18-RP	KANE	451	300
		CONTRAC	T NO. 6	3598
	ILLINOIS FED.		-	