

COUNTY

* (205,57,105)RS-2 ** CHAMPAIGN & DOUGLAS

Build tops of headwalls parallel to the grade lines.

All construction joints shall be bonded according to Article 503.09 of the Standard Specifications.

Special Provisions, All bars should be rounded and conform to the requirements of Article 1006.10 of the

Reinforcement bars shall conform to the requirements of ASTM A706 Gr. (IL Modified). See

When lapping sheets of welded wire fabric, the overlap measured between the outermost cross wires of each fabric sheet shall not be less than 8"

End Sections will be paid for at the contract unit price per each for BOX CULVERT END SECTIONS, as outlined in Section 540 of the Standard Specifications.

Class SI Concrete shall be used throughout.

<u>General Notes</u>

Standard Specifications.

Concrete, Rebar, and Welded Wire Fabric quantities and lengths calculated for the cast-in-place End Sections may vary based on the precast box culverts supplied.

Drain holes shall be provided in accordance with Article 503.11 of the Standard

The precast manufacturer shall design and detail a connection/construction joint between the precast concrete box sections and the cast-in-place apron and wingwall. The minimum area of reinforcement passing through these construction joints shall be 0.20 sq. in./lineal ft. of welded wire fabric. The design shall be detailed in the shop drawings. The cost of the connection is included in the cost of the end section.

The box culvert end section shall be built in the field and a precast option is not allowed except the cut-off wall may be precast. If the contractor elects to use a precast cut-off wall, shop drawings and a proposed construction sequence shall be submitted to the Engineer for approval.

The ends of the precast box sections adjacent to the end section shall be formed without the male and female shapes specified in Article 8.1 of AASHTO M273. See Section D-D on

The design fill height for this box is less than 2 feet. The Precast Concrete Box Culvert Sections shall conform to the requirements of AASHTO M 273.

The joints between precast box sections shall be sealed, all voids filled with a mastic joint sealer. In addition, the joints shall be externally sealed on all four sides with a 13 inch wide external sealing band. The seal shall be centered over the joint, secured in place and protected during the backfilling process.

All dimensions are in FEET (') - INCHES (") unless otherwise noted.

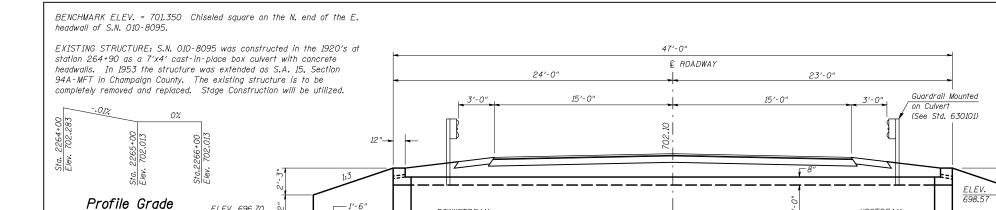
Drawings not to scale.

TOTAL BILL OF MATERIAL

Item	Unit	Total
Removal of Existing Structures	Each	1
Precast Concrete Box Culverts 8'x4'	Foot	88
Box Culvert End Sections	Each	2
Name Plates	Each	1
Storm Sewer Protected, Class A 12"	Foot	28
Storm Sewer Protected, Class A 15"	Foot	4
Concrete Collar	Cu Yd	0.5

SHEET 1 OF 6

GENERAL PLAN AND ELEVATION DOUBLE 8'x4' PRECAST BOX CULVERT F.A.P. ROUTE 808 - SECTION (205,57,105)RS-2 **CHAMPAIGN COUNTY** STATION 2264+91.00, S.N. 010-8137 CULVERT NO. 13



DOWNSTREAM

ELEV. 695.20

STATION 2264+91.00 BUILT 20__ BY STATE OF ILLINOIS F.A.P. RT. 808 SEC. (205,57,105)RS-2 LOADING HS 20 STRUCTURE NO. 010-8137

ELEV. 696.70

NAME PLATE

INDEX OF SHEETS

- General Plan and Elevation 2, 3 Box Culvert End Section Details
- Staging Details
- Porous Granular Detail
- Existing Structure Information

DESIGN SPECIFICATIONS

2002 AASHTO

<u>LOADING HS20-44</u>

Allow 50#/sq.ft. for future wearing surface

DESIGN STRESSES

FIFID UNITS

f'c = 3.500 psi

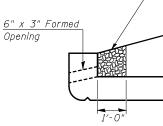
fy = 60,000 psi (reinforcement) fy = 65,000 psi (welded wire fabric)

PRECAST UNITS

f'c = 5,000 psi

fy = 65,000 psi (welded wire fabric)

Coarse aggregate full length of both headwalls. To be placed by Grading Contractor. Cost included with Concrete Box Culverts.



DRAIN DETAIL

ELEVATION (DIMENSIONS AT RIGHT ANGLES TO ¢ OF ROADWAY) 24'-0" 23'-0" @ ROADWAY 47'-0" **PLAN**

UPSTREAM

ELEV. 695.40

11'-212'

WATERWAY INFORMATION

Drainage Area = 0.361 sq. mi. Low Grade Elev. 702.10 @ Sta. 2264+91										
Flood	Freq.	Q	Opening Sq. Ft.		Nat.	Head - Ft.		Headwater El.		
	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.	
	10	166	28	48				699.89	698.30	
Design	50	282	28	48				702.10	699.09	
Base	100	337	28	48				Over	699.47	
Overtopping										
Max. Calc.	500	472	28	48				Over	700.45	

Note: Information provided using the USGS 2004-5103 Method.