

11-06-2015 LETTING ITEM 066

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS

**PLANS FOR PROPOSED  
SURFACE TRANSPORTATION PROGRAM – BRIDGE**

**PROJECT BHS-2462(105)  
SECTION 12-00047-00-BR  
TAZEWELL COUNTY  
C.H. 11 / F.A.S. 2462 / TOWERLINE ROAD  
STRUCTURE NO. 090-3029  
JOB NO. C-94-011-14**

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
F.A.S. 2462	12-00047-00-BR	TAZEWELL	39	1
FED. ROAD DIST. NO.		ILLINOIS CONTRACT NO. 89653		

**INDEX OF SHEETS**

SHEET NO.	DESCRIPTION
1.	COVER SHEET
2.	SUMMARY OF QUANTITIES AND GENERAL NOTES
3.	TYPICAL CROSS SECTIONS
4.	SCHEDULE OF QUANTITIES
5.-6.	PLAN & PROFILE
7.	GUARDRAIL AND ENTRANCE DETAILS SHEET
8.-9.	DETOUR PLANS
10.-25.	BRIDGE PLANS
26.-34.	EXISTING BRIDGE PLANS
35.-39.	STATION CROSS SECTIONS

**HIGHWAY STANDARDS:**

000001-06	STANDARD SYMBOLS, ABBREVIATIONS, AND PATTERNS
630001-10	STEEL PLATE BEAM GUARDRAIL
630301-06	SHOULDER WIDENING FOR TYPE 1, (SPECIAL) GUARDRAIL TERMINALS
631032-08	TRAFFIC BARRIER TERMINAL, TYPE 6A
635006-03	REFLECTOR AND TERMINAL MARKER PLACEMENT
635011-02	REFLECTOR MARKER AND MOUNTING DETAILS
701901-04	TRAFFIC CONTROL DEVICES
780001-05	TYPICAL PAVEMENT MARKINGS
781001-03	TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS
BLR 21-9	TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES FOR CONSTRUCTION ON RURAL LOCAL HIGHWAYS

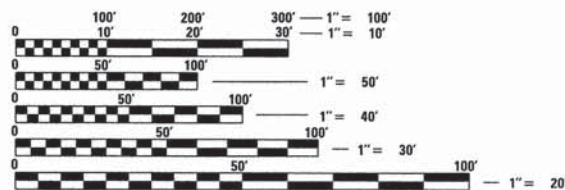


FUNCTIONAL CLASSIFICATION: MAJOR COLLECTOR (RURAL)  
DESIGN SPEED: 50 MPH  
DESIGN TRAFFIC: 1874 ADT (2013)

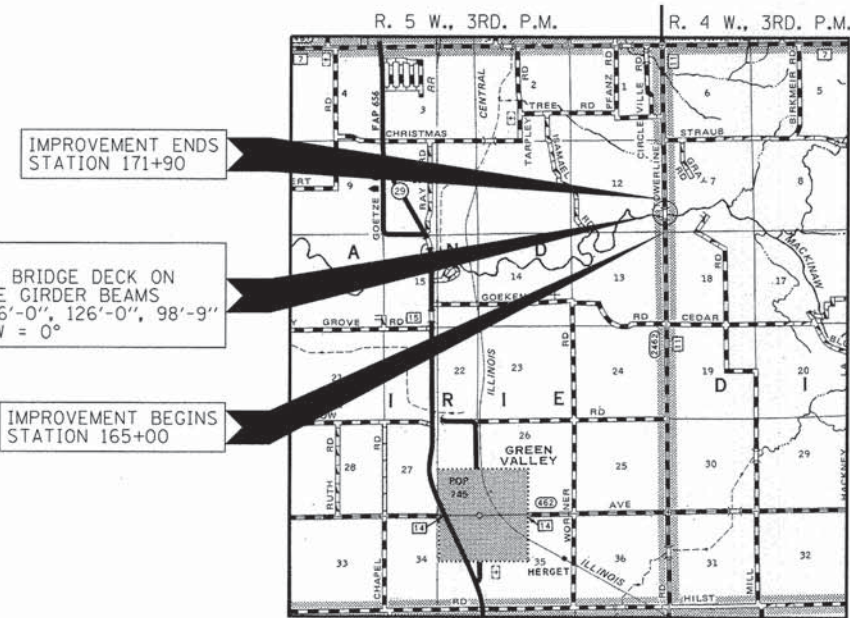
**UTILITIES**

FRONTIER COMMUNICATIONS  
109 E MARKET ST., 2ND FLOOR  
BLOOMINGTON, IL 61701

AT&T  
1000 COMMERCE DRIVE, 1ST FLOOR  
OAK BROOK, IL 60523



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.



**LOCATION MAP**

APPROXIMATE SCALE: 0 1 MILE  
NET LENGTH OF SECTION = 690 FEET = 0.131 MILES



ILLINOIS DEPARTMENT OF TRANSPORTATION	
APPROVED	August 19, 2015 Craig Fisher COUNTY ENGINEER
PASSED	August 24, 2015 [Signature] DISTRICT FOUR ENGINEER OF LOCAL ROADS & STREETS
Releasing For Bid Based on Limited Review	August 25, 2015 [Signature] DEPUTY DIRECTOR OF HIGHWAYS REGION THREE ENGINEER STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

CONTRACT NO. 89653



**HAMPTON, LENZINI AND RENWICK, INC.**  
CIVIL ENGINEERS • STRUCTURAL ENGINEERS • LAND SURVEYORS  
3085 STEVENSON DRIVE, SUITE 201  
SPRINGFIELD, ILLINOIS 62703  
217.548.3400 www.hfrenengineering.com

EXPIRES: 11/30/2015

PROJECT NUMBER: 11.0323.130

DATE: 08/14/15

SUMMARY OF QUANTITIES			
CODE NO.	ITEM	CONSTRUCTION CODE 0014	
		UNIT	TOTAL
20200100	EARTH EXCAVATION	CU YD	25
20400800	FURNISHED EXCAVATION	CU YD	30
25100115	MULCH, METHOD 2	ACRE	0.25
28100207	STONE RIPRAP, CLASS M	TON	270
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	13
28000400	PERIMETER EROSION BARRIER	FOOT	277
28200200	FILTER FABRIC	SQ YD	276
40600285	POLYMERIZED BITUMINOUS MATERIALS (PRIME COAT)	POUND	334
40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SQ YD	98
40603210	POLYMERIZED HOT-MIX ASPHALT BINDER COURSE, IL-9.5, N50	TON	3
40603535	POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, MIX "D", N50	TON	48
48101500	AGGREGATE SHOULDERS, TYPE B 6"	SQ YD	272
50102400	CONCRETE REMOVAL	CU YD	9.3
50104720	REMOVAL OF EXISTING CONCRETE DECK	EACH	1
50157300	PROTECTIVE SHIELD	SQ YD	722
50300225	CONCRETE STRUCTURES	CU YD	9.3
50300255	CONCRETE SUPERSTRUCTURE	CU YD	385.7
50300260	BRIDGE DECK GROOVING	SQ YD	1403
50300300	PROTECTIVE COAT	SQ YD	1637
50500405	FURNISHING AND ERECTING STRUCTURAL STEEL	POUND	1300
50500505	STUD SHEAR CONNECTORS	EACH	3330
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	86390
50901050	STEEL RAILING, TYPE SM	FOOT	906
51500100	NAME PLATES	EACH	1
52000110	PREFORMED JOINT STRIP SEAL	FOOT	60
52100020	ELASTOMERIC BEARING ASSEMBLY, TYPE II	EACH	10
52100510	ANCHOR BOLTS, 3/4"	EACH	20
58700300	CONCRETE SEALER	SQ FT	575
63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A 6 FOOT POSTS	FOOT	25
63100087	TRAFFIC BARRIER TERMINAL, TYPE 6A	EACH	2
63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	2
63200310	GUARDRAIL REMOVAL	FOOT	104
67100100	MOBILIZATION	L SUM	1
78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	1470
78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	3

^ SEE SPECIAL PROVISIONS

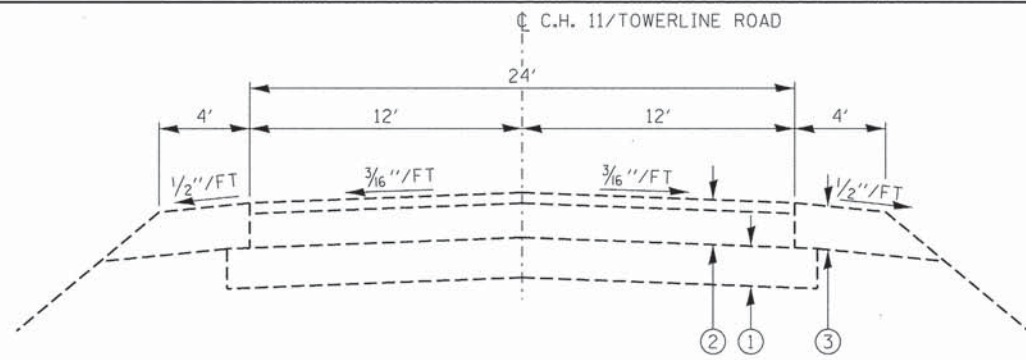
SUMMARY OF QUANTITIES			
CODE NO.	ITEM	CONSTRUCTION CODE 0014	
		UNIT	TOTAL
* 78200410	GUARDRAIL MARKERS, TYPE A	EACH	4
* 78200420	GUARDRAIL MARKERS, TYPE B	EACH	12
* 78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	2
* 78300200	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL	EACH	3
^ X2501000	SEEDING, CLASS 2 (SPECIAL)	ACRE	0.25
^ X4401198	HOT-MIX ASPHALT SURFACE REMOVAL, VARIABLE DEPTH	SQ YD	471
* ^ X6310088	TRAFFIC BARRIER TERMINAL TYPE 6A (SPECIAL)	EACH	2
^ XX008438	TRAFFIC CONTROL AND PROTECTION FOR TEMPORARY DETOUR	EACH	1
^ Z0001899	JACK AND REMOVE EXISTING BEARINGS	EACH	10
* ^ Z0007112	CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES	L SUM	1
* ^ Z0010501	CLEANING AND PAINTING STEEL BRIDGE NO. 1	L SUM	1
^ X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	36

^ SEE SPECIAL PROVISIONS

\* SPECIALTY ITEMS

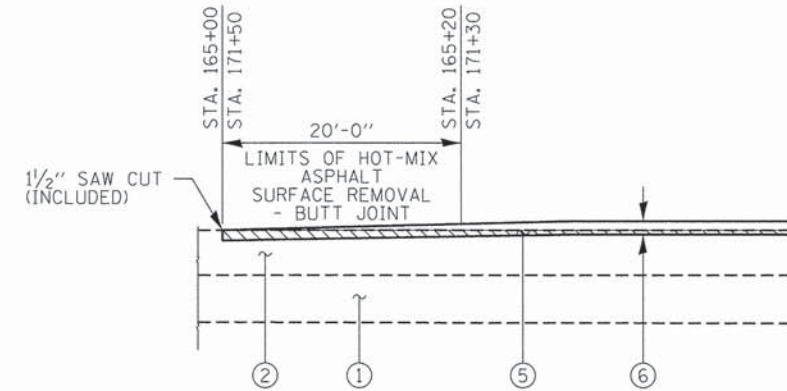
**GENERAL NOTES**

- ALL CONSTRUCTION SHALL BE DONE IN ACCORDANCE WITH THE STATE OF ILLINOIS "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION" ADOPTED JANUARY 1, 2012; THE "SUPPLEMENTAL SPECIFICATIONS AND RECURRING SPECIAL PROVISIONS" ADOPTED JANUARY 1, 2015; THE LATEST EDITION OF THE "ILLINOIS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS"; THE DETAILS IN THE PLANS AND THE SPECIAL PROVISIONS INCLUDED IN THE CONTRACT DOCUMENTS.
- ALL CLEARING AND GRUBBING, FENCE REMOVAL AND REMOVAL OF EXISTING DRAINAGE STRUCTURES SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION.
- THE LOCATION ON THE PLANS OF EXISTING DRAINAGE STRUCTURES, TELEPHONE LINES, ELECTRIC LINES, WATER SERVICE LINES, GAS MAINS, AND OTHER UTILITY FACILITIES AS SHOWN ON THE PLANS ARE BASED ON FIELD INVESTIGATIONS AND THE BEST INFORMATION AVAILABLE. BUT THE LOCATIONS ARE NOT GUARANTEED. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ASCERTAIN THEIR EXACT LOCATION FROM THE INDIVIDUAL UTILITY COMPANIES AND BY FIELD INSPECTION.
- WHERE SECTION OR SUBSECTION MONUMENTS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE REMOVED. THE CONTRACTOR SHALL PROTECT AND CAREFULLY PRESERVE ALL PROPERTY MARKS AND MONUMENTS UNTIL THE OWNER, AN AUTHORIZED SURVEYOR OR AGENT HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING AN AUTHORIZED SURVEYOR REESTABLISH ANY SECTION OR SUBSECTION MONUMENTS DISTURBED BY CONSTRUCTION OPERATIONS.
- THE REVISION NUMBER INDICATED FOR THE STANDARDS LISTED IN THE INDEX OF SHEETS SHALL BE USED IN THE CONSTRUCTION OF THIS SECTION.
- TREES WITHIN THE RIGHT-OF-WAY WHICH INTERFERE WITH CONSTRUCTION SHALL BE REMOVED ONLY AT THE DIRECTION OF THE ENGINEER.
- THE CONTRACTOR SHALL PROVIDE ACCESS TO ADJUTING PROPERTY AT ALL TIMES DURING CONSTRUCTION OF THE PROJECT.
- THE FOLLOWING RATES OF APPLICATION HAVE BEEN USED IN CALCULATING PLAN QUANTITIES.  
 AGGREGATE SHOULDERS 2.0 TON / CU YD  
 STONE RIPRAP 1.75 TON / CU YD  
 HOT-MIX ASPHALT 112 LB / SQ YD / IN  
 BITUMINOUS MATERIALS (RESIDUAL RATE)  
 ON PAVEMENT 0.05 LB/SQ.FT.  
 INTERMEDIATE LIFTS (FOG COAT) 0.025 LB/SQ.FT.  
 ON AGGREGATE SURFACE 0.25 LB/SQ.FT.
- THE AREA TO BE SEEDED SHALL CONSIST OF ALL DISTURBED EARTH SURFACES WITHIN THE RIGHT OF WAY OR AS DIRECTED BY THE ENGINEER. ESTIMATED QUANTITY SEEDING, CLASS 2 (SPECIAL) = 0.25 ACRES
- ALL ELEVATIONS SHOWN ON THE PLANS ARE ESTABLISHED FROM U.S.G.S. MEAN SEA LEVEL DATUM.
- PRIOR TO THE USE OF ANY PROPOSED BORROW AREAS, USE AREAS (TEMPORARY ACCESS ROADS, DETOURS, RUN-AROUNDS, ETC.) WASTE AREAS, THE CONTRACTOR SHALL FILE THE REQUIRED ENVIRONMENTAL RESOURCE REQUEST SURVEYS ACCORDING TO OF THE STANDARD SPECIFICATIONS. THESE SURVEYS ARE REQUIRED IN ORDER FOR THE DEPARTMENT TO CONDUCT CULTURAL AND BIOLOGICAL RESOURCE SURVEYS FOR THE PROPOSED SITE.  
  
 THE REQUIRED ENVIRONMENTAL RESOURCE DOCUMENTATION SHALL INCLUDE THE FOLLOWING:  
 BDE FORM 2289 (CULTURAL AND NATURAL RESOURCES REVIEW OF BORROW AREAS)  
 BDE FORM 2290 (WASTE/USE AREA REVIEW)  
 A LOCATION MAP SHOWING THE SIZE LIMITS AND LOCATION OF THE USE AREA  
 COLOR PHOTOGRAPHS DEPICTING THE USE AREA  
 BORROW AREA ENTRY AGREEMENT FORM - D4 P10101  
  
 PRIOR TO ANY WASTE MATERIALS BEING REMOVED FROM THE CONSTRUCTION SITE, THE REQUIRED ENVIRONMENTAL RESOURCE SHALL BE OBTAINED AND FILED BY THE CONTRACTOR. EXCESS WASTE PRODUCTS REMOVED FROM THE CONSTRUCTION SITE SHALL BE DISPOSED OF AS REQUIRED IN SECTION 202.03 OF THE STANDARD SPECIFICATIONS.  
  
 ANY PROTRUDING METAL BARS SHALL BE REMOVED PRIOR TO THE DISPOSAL OF BROKEN CONCRETE AT APPROVED DISPOSAL SITES.  
  
 PLEASE NOTE THAT A MINIMUM OF FOUR WEEKS SHALL BE ALLOWED FOR THE DISTRICT TO OBTAIN THE REQUIRED WASTE SITE ENVIRONMENTAL CLEARANCES AND SIX WEEKS FOR THE REQUIRED BORROW SITE ENVIRONMENTAL CLEARANCES.
- COMMITMENTS AS OF 6/15/2015:  
NONE

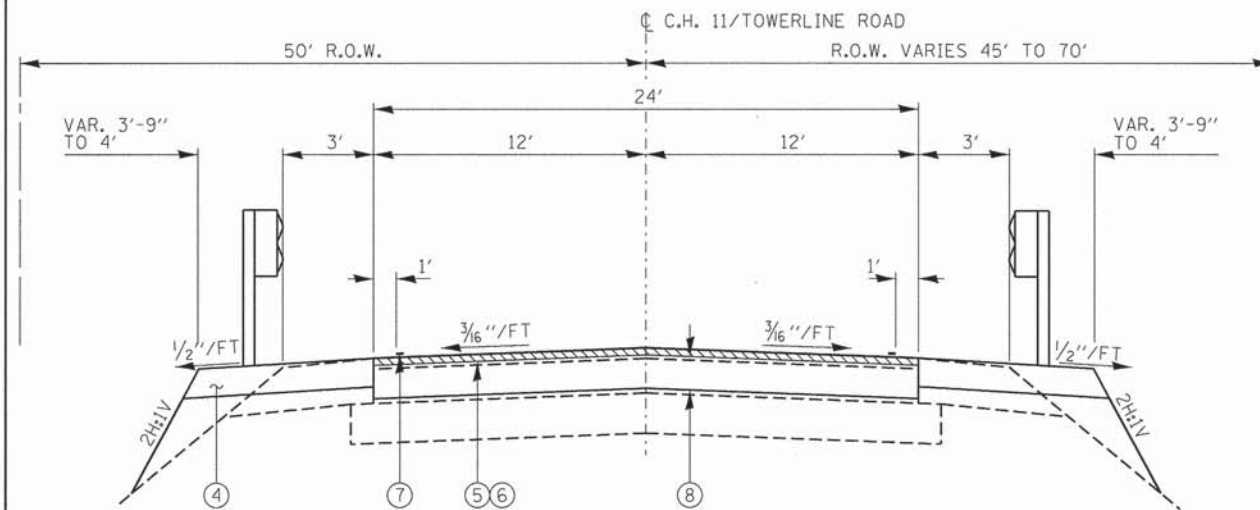


**EXISTING TYPICAL CROSS SECTION**

STA. 164+00 TO STA. 165+86  
STA. 170+40 TO STA. 171+90



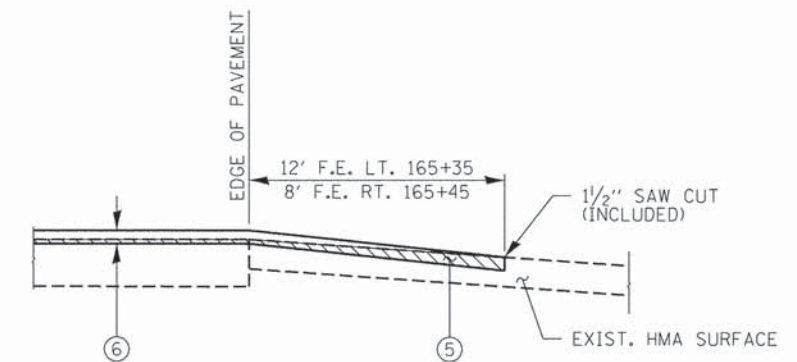
**BUTT JOINT DETAIL**



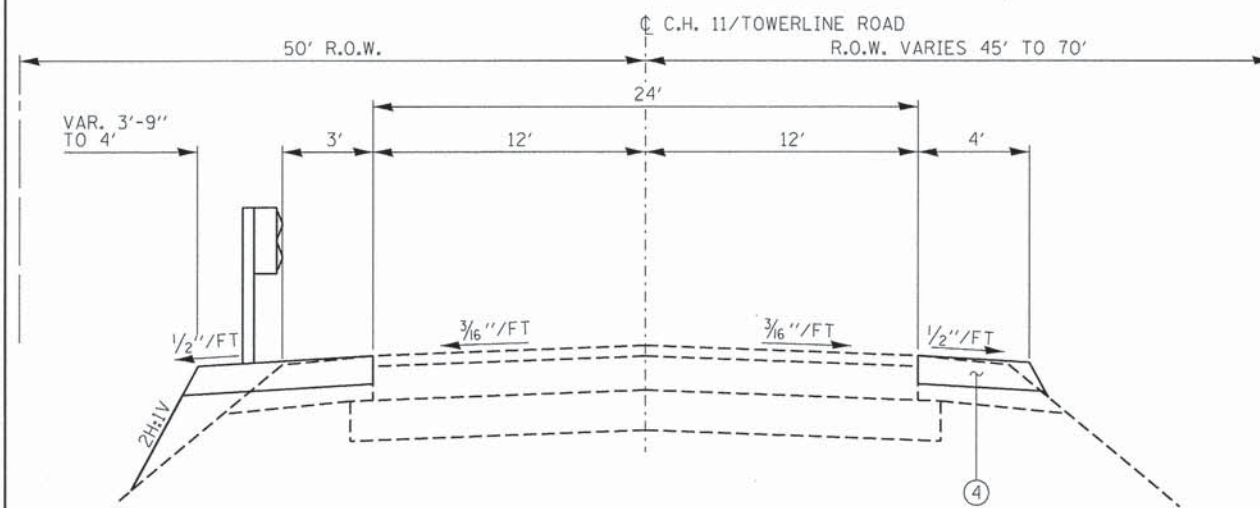
**PROPOSED TYPICAL CROSS SECTION**

STA. 165+00 TO STA. 165+86.39  
STA. 170+39.92 TO STA. 171+50

NOTE: HMA BINDER COURSE TO BE CONSTRUCTED FROM STA. 165+77.25 TO STA. 165+86.39 AND STA. 170+39.92 TO STA. 170+44.07



**ENTRANCE DETAIL**



**PROPOSED TYPICAL CROSS SECTION**

STA. 171+50 TO STA. 171+90

**LEGEND**

- ① EXISTING AGGREGATE BASE (8.5")
- ② EXISTING HMA PAVEMENT (8.5")
- ③ EXISTING AGGREGATE SHOULDER
- ④ PROPOSED AGGREGATE SHOULDERS, TYPE B 6"
- ⑤ HMA SURFACE REMOVAL VARIABLE DEPTH (1.5" MAX)
- ⑥ POLYMERIZED HMA SURFACE COURSE, MIX "D", N50 (1.5")
- ⑦ PAINT PAVEMENT MARKING LINE, (4")
- ⑧ POLYMERIZED HMA BINDER COURSE, IL-9.5 N50 (7")

**HMA MIXTURE REQUIREMENTS**

LOCATIONS(S)	CH 11 / FAS 2462 / TOWERLINE ROAD	CH 11 / FAS 2462 / TOWERLINE ROAD
MIXTURE USE(S):	POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE	POLYMERIZED HOT-MIX ASPHALT BINDER COURSE
PG:	SBS 76-22	SBS 76-22
DESIGN AIR VOIDS:	4% @ Ndes 50	4% @ Ndes 50
MIXTURE COMPOSITION: (MIXTURE GRADATION)	IL 9.5	IL 9.5
FRICTION AGGREGATE:	MIXTURE D	MIXTURE D
MIXTURE WEIGHT:	112 LBS. / SQ. YD. / INCH	112 LBS. / SQ. YD. / INCH
QUALITY CONTROL PROGRAM:	QC/QA	QC/QA

FILE NAME = 110323-sht-tpsections.dgn	USER NAME =	DESIGNED - J.W.F.	REVISED -
HAMPTON, LENZINI AND RENWICK, INC. 280 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	DRAWN - T.W.K.	REVISED -
ILLINOIS PROFESSIONAL DESIGN FIRM L3 / FLS / FSE COMP. 184-200689	PLOT DATE = 8/18/2015	CHECKED - M.D.C.	REVISED -
		DATE - 08/14/15	REVISED -

**STATE OF ILLINOIS  
TAZEWELL COUNTY HIGHWAY DEPARTMENT**

TYPICAL CROSS SECTIONS C.H. 11 / TOWNLINE ROAD			
SCALE:	SHEET NO.	OF SHEETS	STA. TO STA.

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
2462	12-00047-00-BR	TAZEWELL	39	3
				CONTRACT NO. 89653
[ILLINOIS] FED. AID PROJECT				

ROADWAY SCHEDULE						
LOCATION	POLYMERIZED BITUMINOUS MATERIALS (PRIME COAT)	HOT-MIX ASPHALT SURFACE REMOVAL BUTT JOINT	POLYMERIZED HOT-MIX ASPHALT BINDER COURSE IL-9.5, N50 (7")	POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE MIX "D", N50 (1.5")	AGGREGATE SHOULDERS TYPE B, 6"	HOT-MIX ASPHALT SURFACE REMOVAL VARIABLE DEPTH
	POUND	SQ YD	TON	TON	SQ YD	SQ YD
	STA 165+00 TO STA 165+86.39	147	49	2	18	68
STA 170+39.92 TO STA 171+90	147	49	1	23	204	220
ENTRANCES	40			7		89
<b>TOTAL</b>	<b>334</b>	<b>98</b>	<b>3</b>	<b>48</b>	<b>272</b>	<b>471</b>

EARTHWORK SUMMARY						
LOCATION	EARTH EXCAVATION	SHRINKAGE FACTOR	% USED	EARTH EXCAVATION ADJUSTED FOR SHRINKAGE	EMBANKMENT REQUIRED	EARTHWORK BALANCE WASTE (+) / SHORTAGE (-)
	CU YD			CU YD	CU YD	CU YD
STA 165+00 TO STA 165+86.39	7	25.00%	100.00%	5	2	3
STA 170+39.92 TO STA 171+90	17	25.00%	100.00%	13	45	-32
<b>TOTAL</b>	<b>24</b>			<b>18</b>	<b>47</b>	<b>-29</b>
<b>USE</b>	<b>25</b>					<b>30</b>

FURNISHED EXCAVATION 30 CU YD

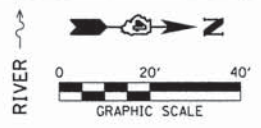
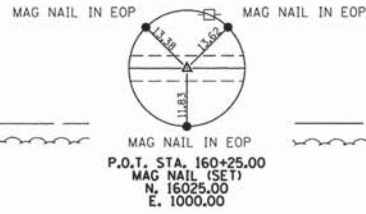
SEEDING SCHEDULE			
LOCATION	MULCH METHOD 2	TEMPORARY EROSION CONTROL SEEDING	SEEDING, CLASS 2 (SPECIAL)
	ACRE	POUND	ACRE
STA 165+00 TO STA 165+86.39	0.01	1.4	0.01
STA 170+39.92 TO STA 171+90	0.11	11.0	0.05
<b>TOTAL</b>	<b>0.12</b>	<b>12.4</b>	<b>0.06</b>
<b>USE</b>	<b>0.25</b>	<b>13</b>	<b>0.25</b>

PERIMETER EROSION BARRIER	
LOCATION	FOOT
LT. STA 170+40 TO STA 171+90	150
RT. STA 170+40 TO STA 171+67	127
<b>TOTAL</b>	<b>277</b>

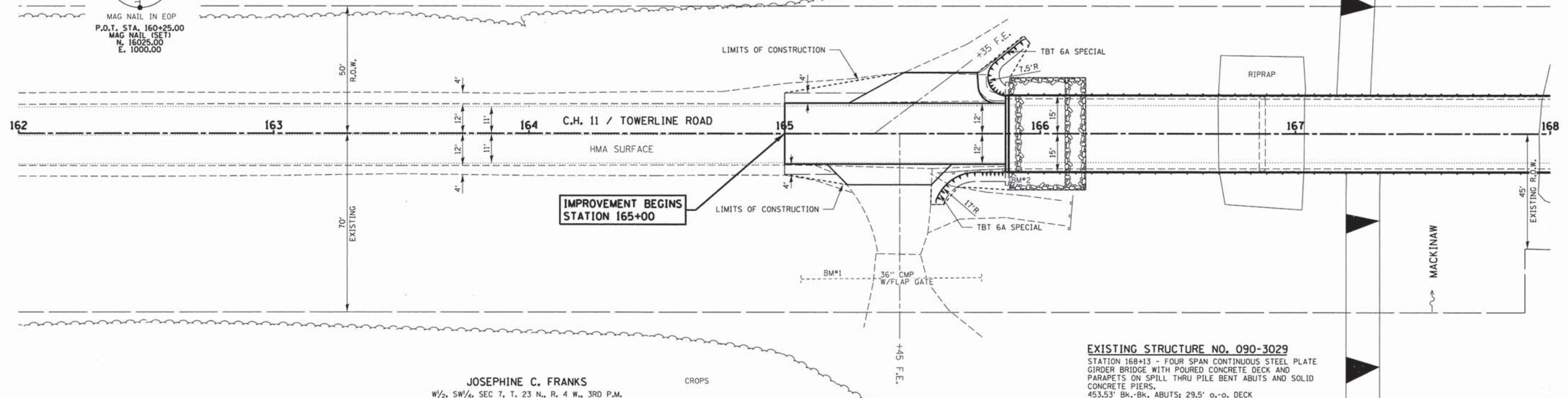
GUARDRAIL SCHEDULE								
LOCATION	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	TRAFFIC BARRIER TERMINAL TYPE 6A	TRAFFIC BARRIER TERMINAL TYPE 1 (SPECIAL) TANGENT	GUARDRAIL REMOVAL	GUARDRAIL MARKERS TYPE A	GUARDRAIL MARKERS TYPE B	TERMINAL MARKER - DIRECT APPLIED	TRAFFIC BARRIER TERMINAL TYPE 6A (SPECIAL)
	FOOT	EACH	EACH	FOOT	EACH	EACH	EACH	EACH
	LT. STA 165+80.38 TO STA 171+61.22	25	1	1	65	2	6	1
RT. STA 165+59.37 TO STA 171+36.22		1	1	39	2	6	1	1
<b>TOTAL</b>	<b>25</b>	<b>2</b>	<b>2</b>	<b>104</b>	<b>4</b>	<b>12</b>	<b>2</b>	<b>2</b>

PAVEMENT MARKING SCHEDULE				
LOCATION	PAINT PAVEMENT MARKING - LINE 4"		RAISED REFLECTIVE PAVEMENT MARKER	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL
	EDGE LINE SOLID WHITE	CENTERLINE SKIP DASH YELLOW	EACH	EACH
	FOOT	FOOT		
LT. STA 165+00 TO STA 171+50	650			
CL. STA 165+00 TO STA 171+50		170	3	3
RT. STA 165+00 TO STA 171+50	650			
<b>SUBTOTAL</b>	<b>1300</b>	<b>170</b>	<b>3</b>	<b>3</b>
<b>TOTAL</b>		<b>1470</b>	<b>3</b>	<b>3</b>

GEORGE M. BIRKMEIER  
NE 1/4, SE 1/4, SEC 12, T. 23 N., R. 5 W., 3RD P.M.



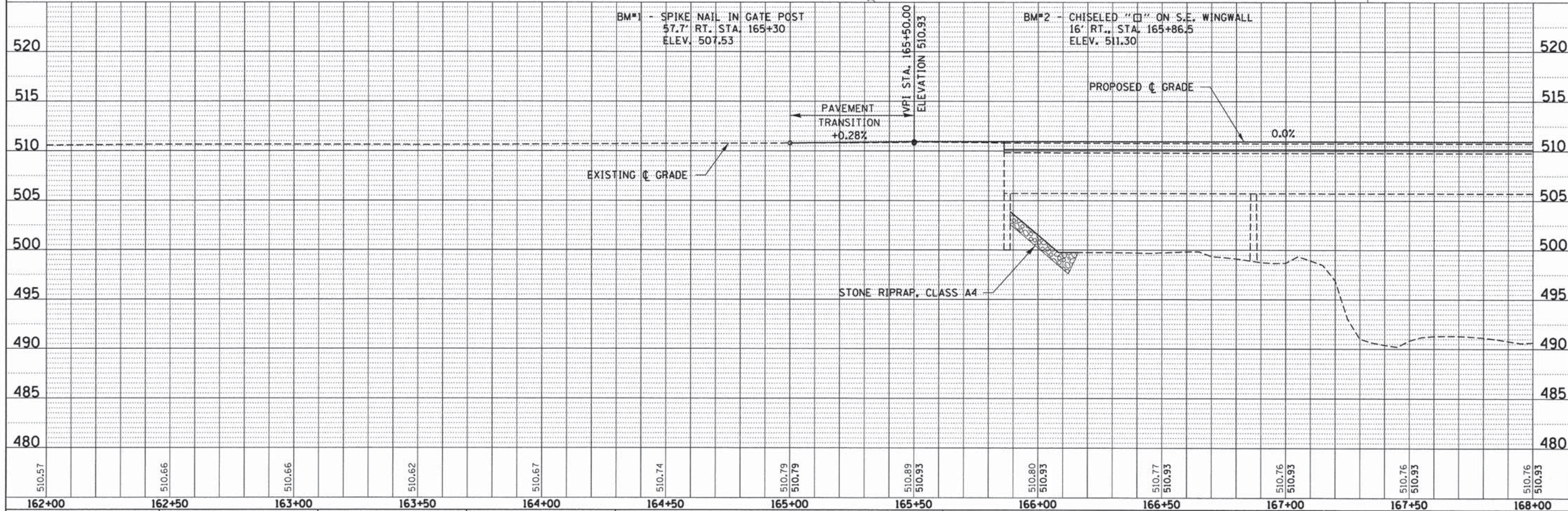
DATE	
BY	
SURVEYED	
ADJUSTED	
ALIGNED	
CHECKED	
RT. OF WAY	
CHECKED	
NO.	
PLANNING	
NO.	



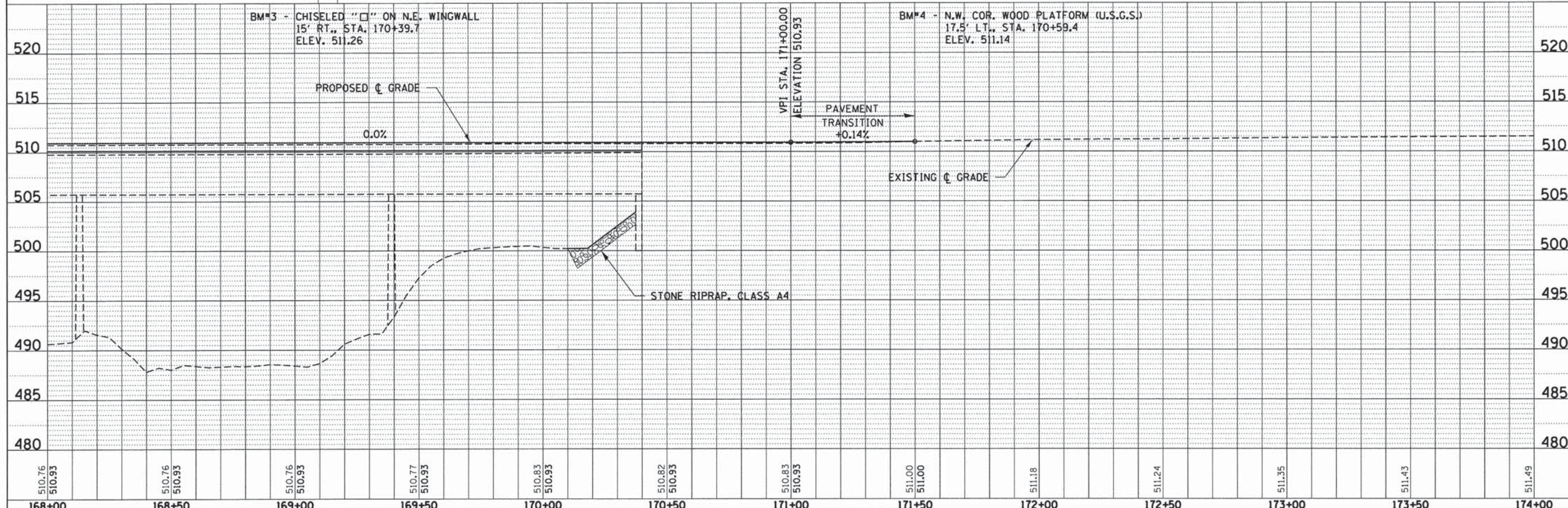
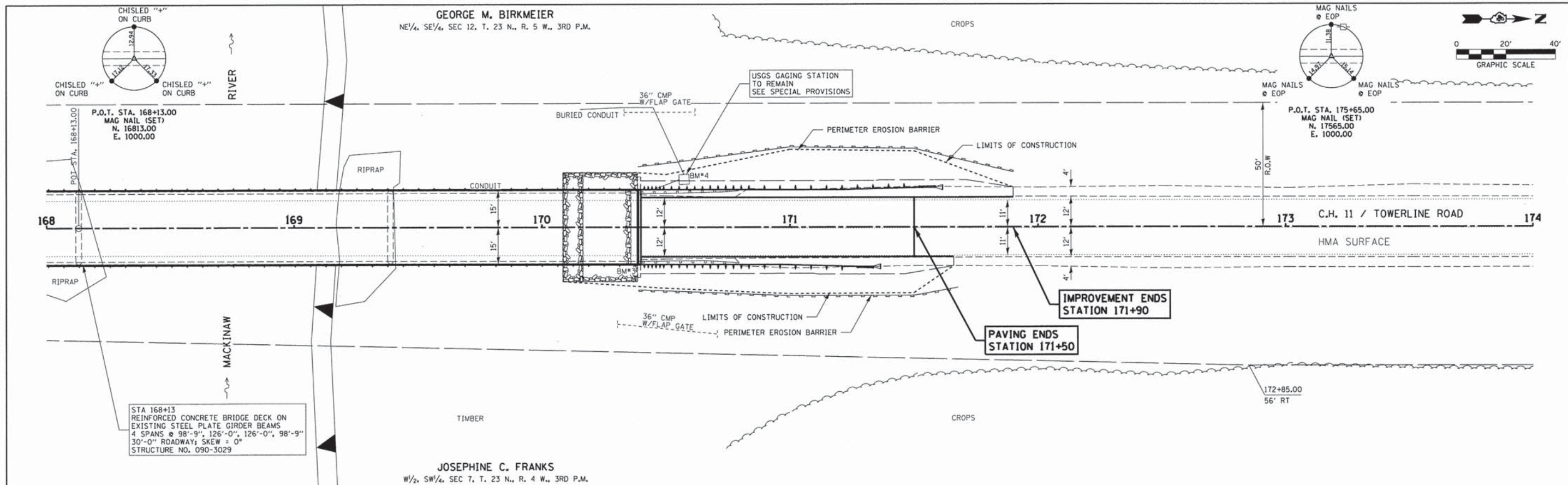
JOSEPHINE C. FRANKS  
W 1/2, SW 1/4, SEC 7, T. 23 N., R. 4 W., 3RD P.M.

EXISTING STRUCTURE NO. 090-3029  
STATION 168+13 - FOUR SPAN CONTINUOUS STEEL PLATE GIRDER BRIDGE WITH POURED CONCRETE DECK AND PARAPETS ON SPILL THRU PILE BENT ABUTS AND SOLID CONCRETE PIERS.  
453.53' BK.-BK. ABUTS; 29.5' o.-o. DECK

DATE	
BY	
SURVEYED	
GRADES	
CHECKED	
B.M. NOTED	
STRUCTURE	
NOTATING	
CHRD	
NO.	

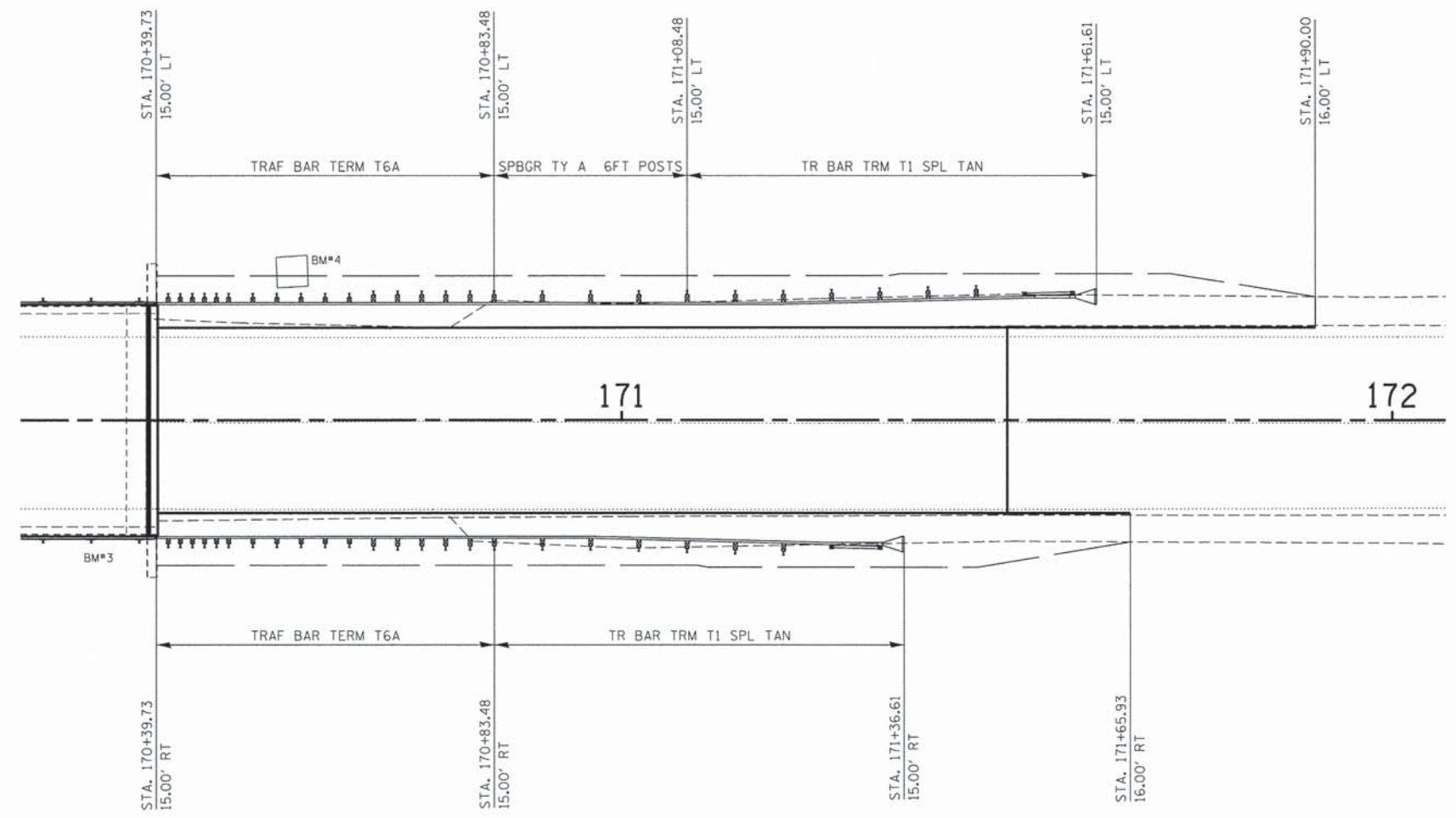
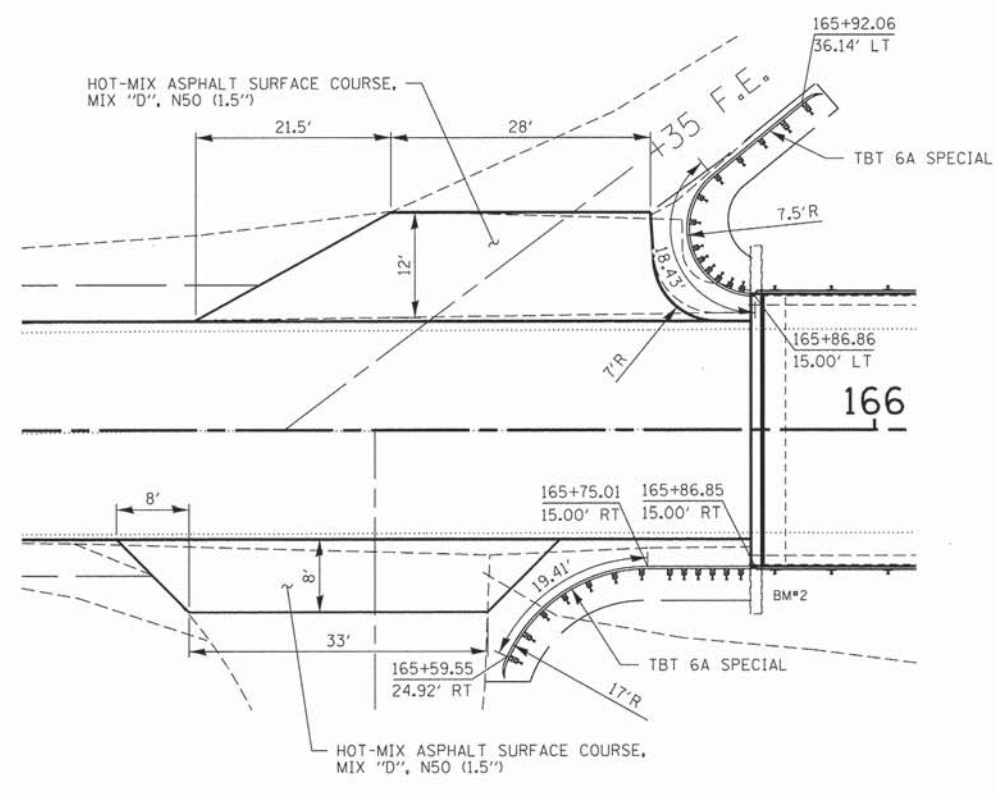
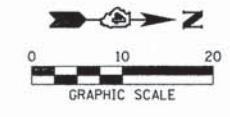


FILE NAME = 110323-sh1-p8p.dgn	USER NAME =	DESIGNED - J.W.F.	REVISED -	<p align="center"><b>STATE OF ILLINOIS</b> <b>TAZEWELL COUNTY HIGHWAY DEPARTMENT</b></p>	<p align="center"><b>PLAN &amp; PROFILE</b> <b>C.H. 11 / TOWERLINE ROAD</b></p>		F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3083 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L.S. / P.E. / SE. CORP. 184-000889	PLLOT SCALE =	DRAWN - L.G.C.	REVISED -		2462	12-00047-00-BR	TAZEWELL	39	5		
PLLOT DATE = 8/18/2015	DATE - 08/14/15	CHECKED - M.D.C.	REVISED -		SCALE: 20H:5V	SHEET NO. 1 OF 2 SHEETS	STA. 162+00.00 TO STA. 168+00.00	CONTRACT NO. 89653			
		DATE - 08/14/15	REVISED -		ILLINOIS FED. AID PROJECT						



DATE	
BY	
SURVEYED	
ALIGNED	
ADJUSTED	
CHECKED	
RT. OF WAY CHECKED	
NO. OF WAY CHECKED	
FILE NAME	
PLAIN	
NO.	

DATE	
BY	
SURVEYED	
ALIGNED	
ADJUSTED	
CHECKED	
RT. OF WAY CHECKED	
NO. OF WAY CHECKED	
FILE NAME	
PROF. FILE	
NO.	



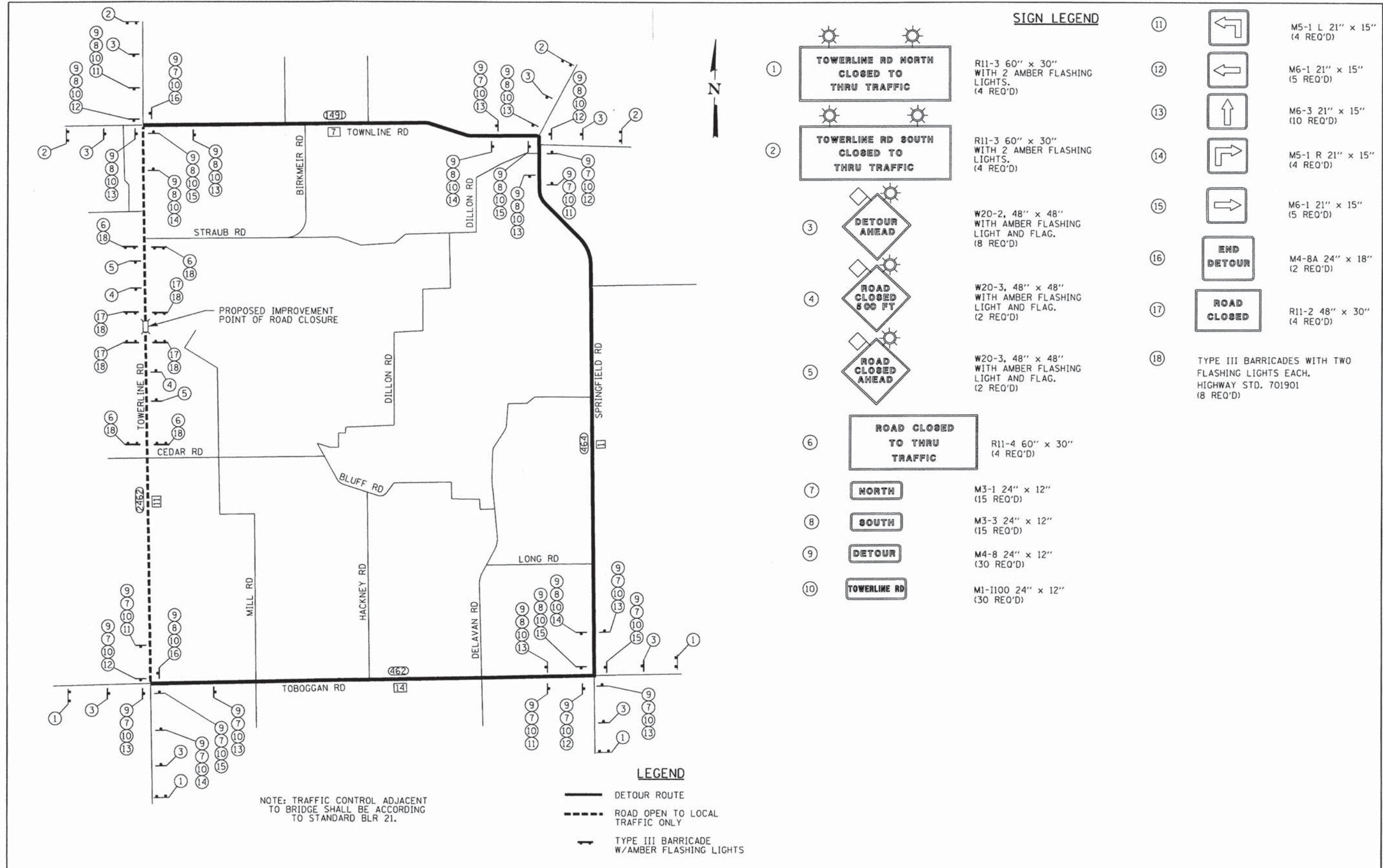
FILE NAME = 110323-sht-guardrail.dgn	USER NAME =	DESIGNED - J.W.F.	REVISED -
<b>HAMPTON, LENZINI AND RENWICK, INC.</b> 3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	DRAWN - T.W.K.	REVISED -
<b>HLR</b> ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.000959	PLOT DATE = 8/18/2015	CHECKED - M.D.C.	REVISED -
		DATE - 08/14/15	REVISED -

**STATE OF ILLINOIS  
TAZEWELL COUNTY HIGHWAY DEPARTMENT**

**GUARDRAIL AND ENTRANCE DETAILS  
C.H. 11 / TOWERLINE ROAD**

SCALE: 1:10      SHEET NO.    OF    SHEETS    STA.    TO STA.

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
2462	12-00047-00-BR	TAZEWELL	39	7
CONTRACT NO. 89653				
ILLINOIS FED. AID PROJECT				



**SIGN LEGEND**

- ① R11-3 60" x 30" WITH 2 AMBER FLASHING LIGHTS. (4 REQ'D)
- ② R11-3 60" x 30" WITH 2 AMBER FLASHING LIGHTS. (4 REQ'D)
- ③ W20-2, 48" x 48" WITH AMBER FLASHING LIGHT AND FLAG. (8 REQ'D)
- ④ W20-3, 48" x 48" WITH AMBER FLASHING LIGHT AND FLAG. (2 REQ'D)
- ⑤ W20-3, 48" x 48" WITH AMBER FLASHING LIGHT AND FLAG. (2 REQ'D)
- ⑥ R11-4 60" x 30" (4 REQ'D)
- ⑦ M3-1 24" x 12" (15 REQ'D)
- ⑧ M3-3 24" x 12" (15 REQ'D)
- ⑨ M4-8 24" x 12" (30 REQ'D)
- ⑩ M1-1100 24" x 12" (30 REQ'D)
- ⑪ M5-1 L 21" x 15" (4 REQ'D)
- ⑫ M6-1 21" x 15" (5 REQ'D)
- ⑬ M6-3 21" x 15" (10 REQ'D)
- ⑭ M5-1 R 21" x 15" (4 REQ'D)
- ⑮ M6-1 21" x 15" (5 REQ'D)
- ⑯ M4-8A 24" x 18" (2 REQ'D)
- ⑰ R11-2 48" x 30" (4 REQ'D)
- ⑱ TYPE III BARRICADES WITH TWO FLASHING LIGHTS EACH. HIGHWAY STD. 701901 (8 REQ'D)

**LEGEND**

- DETOUR ROUTE
- ROAD OPEN TO LOCAL TRAFFIC ONLY
- TYPE III BARRICADE W/AMBER FLASHING LIGHTS

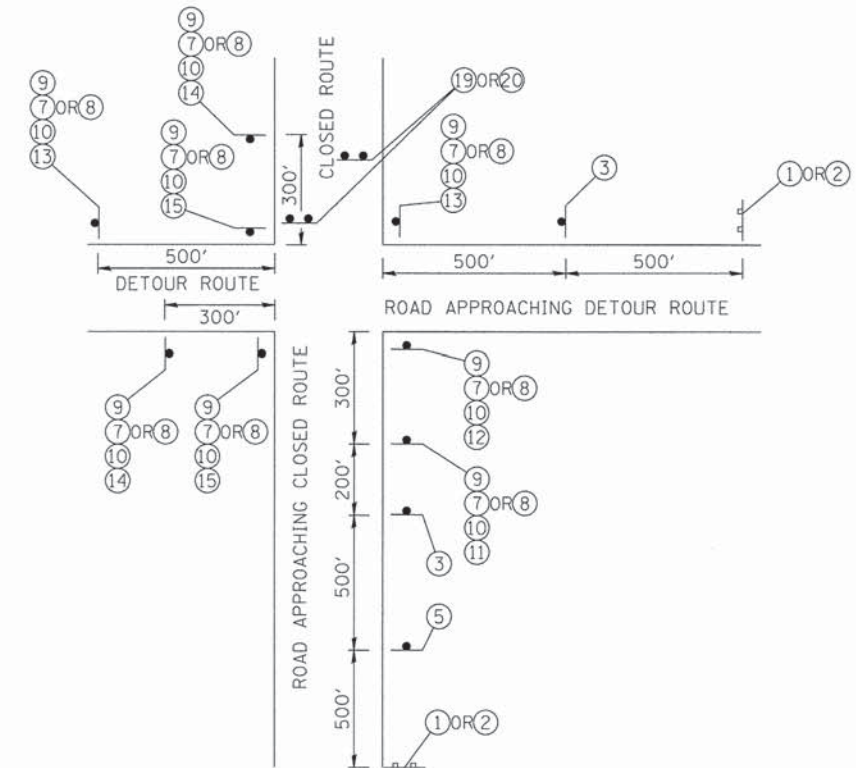
NOTE: TRAFFIC CONTROL ADJACENT TO BRIDGE SHALL BE ACCORDING TO STANDARD BLR 21.

FILE NAME = 110223-shs-detour.dgn	USER NAME =	DESIGNED - J.W.F.	REVISED -	<b>STATE OF ILLINOIS TAZEWELL COUNTY HIGHWAY DEPARTMENT</b>	<b>DETOUR PLAN C.H. 11 / TOWERLINE ROAD</b>		F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 300 STEVENSON DRIVE, SUITE 204 SPRINGFIELD, IL 62763 ILLINOIS PROFESSIONAL DESIGN FIRM L3/FRE/ISS CORP. 184.010981	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		SCALE:	SHEET NO. 1 OF 2 SHEETS	STA.	2462	12-00047-00-BR	TAZEWELL	39	8
PLOT DATE = 8/28/2015		CHECKED - M.D.C.	REVISED -				TO STA.					
		DATE - 08/14/15	REVISED -									CONTRACT NO. 89653
ILLINOIS FED. AID PROJECT												



**DETOUR GENERAL NOTES**

- ALL SIGNING SHALL BE IN ACCORDANCE WITH THE APPLICABLE PROVISIONS OF THE STATE OF ILLINOIS "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION ADOPTED JAN. 1, 2012", "THE QUALITY STANDARD FOR WORK ZONE TRAFFIC CONTROL DEVICES ADOPTED 2010", THE DETAILS IN THESE PLANS, THE LATEST EDITION OF THE STATE OF ILLINOIS "MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES", AND THE SPECIAL PROVISIONS FOR TRAFFIC CONTROL AND PROTECTION.
- THE CONTRACTOR SHALL SCHEDULE ALL WORK IN AN EXPEDIENT MANNER TO REDUCE THE LENGTH OF TIME THAT THE DETOUR NEEDS TO BE IN EFFECT.
- THE ENGINEER SHALL BE NOTIFIED IN WRITING AT LEAST THREE WEEKS PRIOR TO THE DAY THE DETOUR IS TO BE IN EFFECT. THE ENGINEER WILL CONTACT THE APPROPRIATE LOCAL AGENCIES AND INTERESTED PARTIES FOR APPROVAL OF SUCH DATE.
- IF REQUESTED BY THE ENGINEER A PRE-CONSTRUCTION MEETING WITH THE CONTRACTOR SHALL BE HELD AT LEAST TWO WEEKS PRIOR TO THE DAY THE DETOUR IS TO BE IN EFFECT.
- THE CONTRACTOR SHALL SUPPLY TO THE ENGINEER THE NAMES AND TELEPHONE NUMBERS OF HIS REPRESENTATIVES ON THE CONSTRUCTION SITE AND HIS REPRESENTATIVE RESPONSIBLE FOR THE DETOUR SIGNING PRIOR TO THE START OF THE WORK. THE TAZEWELL COUNTY HIGHWAY DEPARTMENT REPRESENTATIVE FOR THE DETOUR IS:  
  
CRAIG FINK  
TAZEWELL COUNTY HIGHWAY DEPARTMENT  
21308 IL RTE 9  
TREMONT, IL 61568  
309-925-5532
- THE ENGINEER WILL FIELD LOCATE THE POSITIONS OF ANY SIGNS IF REQUESTED BY THE CONTRACTOR IN WRITING AT LEAST THREE WEEKS PRIOR TO THE DAY THE DETOUR IS TO BE IN EFFECT.
- LONGITUDINAL DIMENSIONS SHOWN ON THESE PLANS MAY BE ADJUSTED TO FIT FIELD CONDITIONS.
- THE ROAD SHALL NOT BE CLOSED UNTIL ALL SIGNING IS ERECTED IN ACCORDANCE WITH THE DETOUR PLAN AND INSPECTED AND APPROVED BY THE ENGINEER.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT ALL BARRICADES, SIGNS, LIGHTS, AND OTHER DEVICES INSTALLED BY HIM ARE IN PLACE AND OPERATING 24 HOURS EACH DAY INCLUDING SUNDAYS AND HOLIDAYS DURING THE TIME THE DETOUR IS IN EFFECT.
- THE TRAFFIC CONTROL SHOWN ON THE DETOUR PLAN IS THE MINIMUM NECESSARY TO ENSURE THIS ROAD CLOSURE. THE CONTRACTOR SHALL MAKE ALL CHANGES IN TRAFFIC CONTROL DEEMED NECESSARY BY THE ENGINEER. ADDITIONS AND DELETIONS OF TRAFFIC CONTROL FOR THIS DETOUR SHALL BE INCLUDED IN THE PAY ITEM "TRAFFIC CONTROL AND PROTECTION FOR TEMPORARY DETOUR".
- ALL EXISTING SIGNING THAT IS NOT APPLICABLE WHILE THE DETOUR IS IN EFFECT SHALL BE COMPLETELY COVERED BY THE CONTRACTOR, IN A MANNER APPROVED BY THE ENGINEER.
- ALL DETOUR SIGNING SHALL BE POST MOUNTED.
- ALL DETOUR SIGNING EXCEPT REGULATORY SIGNS SHALL HAVE BLACK LEGENDS ON FLUORESCENT ORANGE SHEETING AND STANDARD BLACK BORDERS. THE FLUORESCENT ORANGE REFLECTIVE SHEETING SHALL MEET THE REQUIREMENTS OF ARTICLE 1106.01 OF THE STANDARD SPECIFICATIONS. ALL DETOUR SIGNING SHALL BE NEW OR LIKE NEW CONDITION. THE ENGINEER SHALL BE THE SOLE JUDGE OF THE CONDITION AND ACCEPTANCE OF THE SIGNS.
- THE SIZES OF ALL SIGNS NOT SPECIFIED IN THESE PLANS SHALL BE AS REQUIRED BY THE ILLINOIS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.
- AS A MINIMUM, ALL AMBER FLASHING LIGHTS THAT ARE REQUIRED FOR THIS DETOUR SHALL MEET THE REQUIREMENTS FOR TYPE A-LOW INTENSITY FLASHING LIGHTS IN ARTICLE 1106.02 OF THE STANDARD SPECIFICATIONS. ALL LIGHTS SHALL OPERATE DURING THE HOURS OF DARKNESS. ONLY LIGHTS THAT HAVE BEEN APPROVED BY THE ILLINOIS DEPARTMENT OF TRANSPORTATION SHALL BE USED.
- THE MINIMUM DIMENSIONS OF THE ORANGE WARNING FLAGS SHOWN IN THE PLANS ARE 18" BY 18".
- ALL BARRICADES SHALL HAVE REFLECTORIZED STRIPING ON BOTH SIDES OF THE BARRICADES. THE TYPE III BARRICADES USED AT THE POINT OF CLOSURE TO THRU TRAFFIC SHALL NOT EXCEED 8'-0" IN WIDTH EACH, FOR A SINGLE APPROACH LANE.
- THE "ROAD CLOSED" (R11-2), SIGNS SHALL BE MOUNTED ABOVE THE TOP OF THE BARRICADE. ALL TYPE III BARRICADES SHALL HAVE TWO (2) AMBER TYPE A-LOW INTENSITY FLASHING LIGHTS SPACED NEAR THE CENTERLINE OF THE SUPPORTS.
- THE ROAD NAME SIGN SHALL HAVE A BLACK LEGEND ON FLUORESCENT ORANGE REFLECTIVE SHEETING. THE SIGN BLANK SHALL BE A 9" BY VARIABLE OR A 12" BY VARIABLE WITH DESIGN SERIES C LETTERS. THE CAPITAL LETTERS SHALL BE 6" WITH 5" LOWER CASE.
- DURING NON-WORKING HOURS AT THE POINT OF ROAD CLOSURE TO ALL TRAFFIC THE CONTRACTOR SHALL PROVIDE A MEANS TO RESTRAIN THE BARRICADES FROM EASY MOVEMENT BY VANDALS. THE CHOSEN METHOD SHALL BE APPROVED BY THE ENGINEER.
- CONSTRUCTION EQUIPMENT SHALL NOT BE PARKED IMMEDIATELY BEHIND THE TYPE III BARRICADES DURING NON-WORKING HOURS. IN ANY EVENT ARTICLE 701.11 OF THE STANDARD SPECIFICATIONS SHALL APPLY.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE VISIBILITY OF ALL DETOUR AND CONSTRUCTION SIGNING, INCLUDING BRUSHING BACK VEGETATION IF DEEMED NECESSARY BY THE ENGINEER.
- THE FOLLOWING ILLINOIS DEPARTMENT OF TRANSPORTATION STANDARDS ARE APPLICABLE FOR THIS WORK: STANDARD 701901, BLR 21
- THE ENGINEER SHALL BE NOTIFIED AT LEAST TWO (2) DAYS BEFORE THE ROAD IS TO BE OPENED TO TRAFFIC. THE ENGINEER WILL CONTACT THE APPROPRIATE LOCAL AGENCIES AND INTERESTED PARTIES.



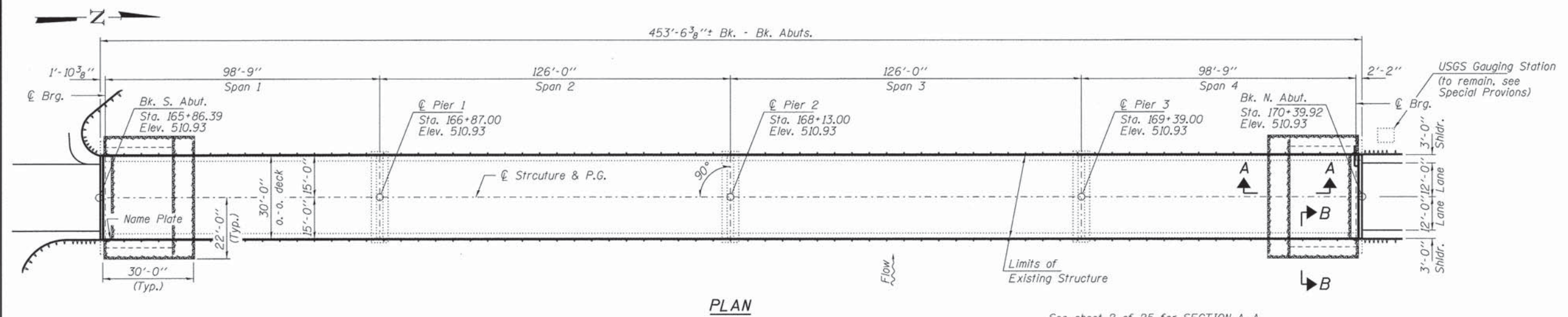
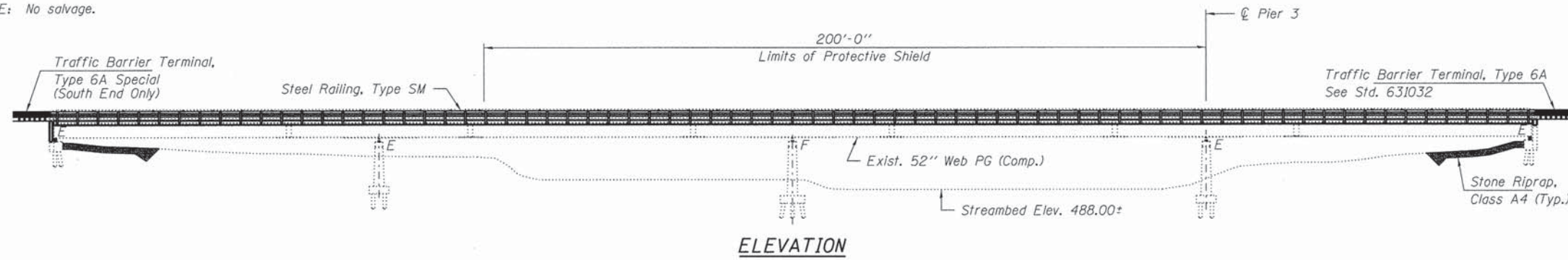
TYPICAL INTERSECTION  
AT POINT OF DETOUR

FILE NAME = 110323-sh1t-detour.dgn	USER NAME =	DESIGNED - J.W.F.	REVISED -	<b>STATE OF ILLINOIS TAZEWELL COUNTY HIGHWAY DEPARTMENT</b>	<b>DETOUR PLAN C.H. 11 / TOWERLINE ROAD</b>		F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
<b>HAMPTON, LENZINI AND RENWICK, INC.</b> 300 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		2462	12-00047-00-BR	TAZEWELL	39	9		
<b>ILLINOIS PROFESSIONAL DESIGN FIRM</b> L3 / PE / SE / CORP. 184-000009	PLOT DATE = 8/18/2015	CHECKED - S.W.M.	REVISED -		SCALE: SHEET NO. 2 OF 2 SHEETS STA. TO STA.		CONTRACT NO. 89653		ILLINOIS FED. AID PROJECT		
		DATE - 08/14/15	REVISED -								

BENCHMARK: B.M. #2 - Chiseled "□" on S.E. Wingwall. 16' Rt., Sta. 165+86.5, Elev. 511.30

EXISTING STRUCTURE: SN 090-3029 built in 1967 as C.H. 11 Section 47-1B. Structure consists of a four span continuous steel plate girder on solid wall piers and spill through abutments, 453'-6<sup>3</sup>/<sub>8</sub>" bk.-bk. and 29.5' o.-o. Existing deck to be removed and replaced using road closure. Traffic to be maintained using a detour route.

SALVAGE: No salvage.



See sheet 2 of 25 for SECTION A-A and SECTION B-B.

**INDEX OF STRUCTURE SHEETS**

1. General Plan & Elevation
2. General Details
- 3-5. Top of Slab Elevations
6. Superstructure
7. Superstructure Details
8. Steel Railing, Type SM
9. Preformed Joint Strip Seal
10. Structural Steel
11. Structural Steel Details
12. Bearing Details
13. South Abutment Removal Details
14. South Abutment
15. North Abutment Removal Details
16. North Abutment
- 17-25. Existing Plans

**DESIGN SPECIFICATIONS**

2002 AASHTO Standard Specifications for Highway Bridges (New Construction)

**LOADING HS-20 (NEW CONSTRUCTION)**

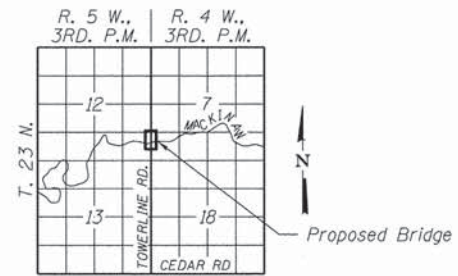
50 psf future wearing surface.

**DESIGN STRESSES**

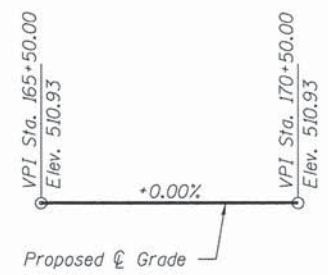
$f'_c = 5,000$  psi (Superstructure)  
 $f'_c = 3,500$  psi (Substructure)  
 $f_y = 60,000$  psi (Reinf.)  
 $f_y = 36,000$  psi (Existing Structural Steel)  
 $36,000$  psi (New Structural Steel M270 GR. 36)

**SEISMIC DATA**

Seismic Performance Category (SPC) = A  
 Horizontal Bedrock Acceleration Coefficient (A) = 0.043g  
 Site Coefficient (S) = 1.0



LOCATION SKETCH



PROFILE GRADE (along  $\phi$  roadway)

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current "AASHTO Standard Specifications for Highway Bridges."

*Michael D. Cima* 06/17/2015  
 ILLINOIS STRUCTURAL NO. 081-5984



Expires 11-30-2016

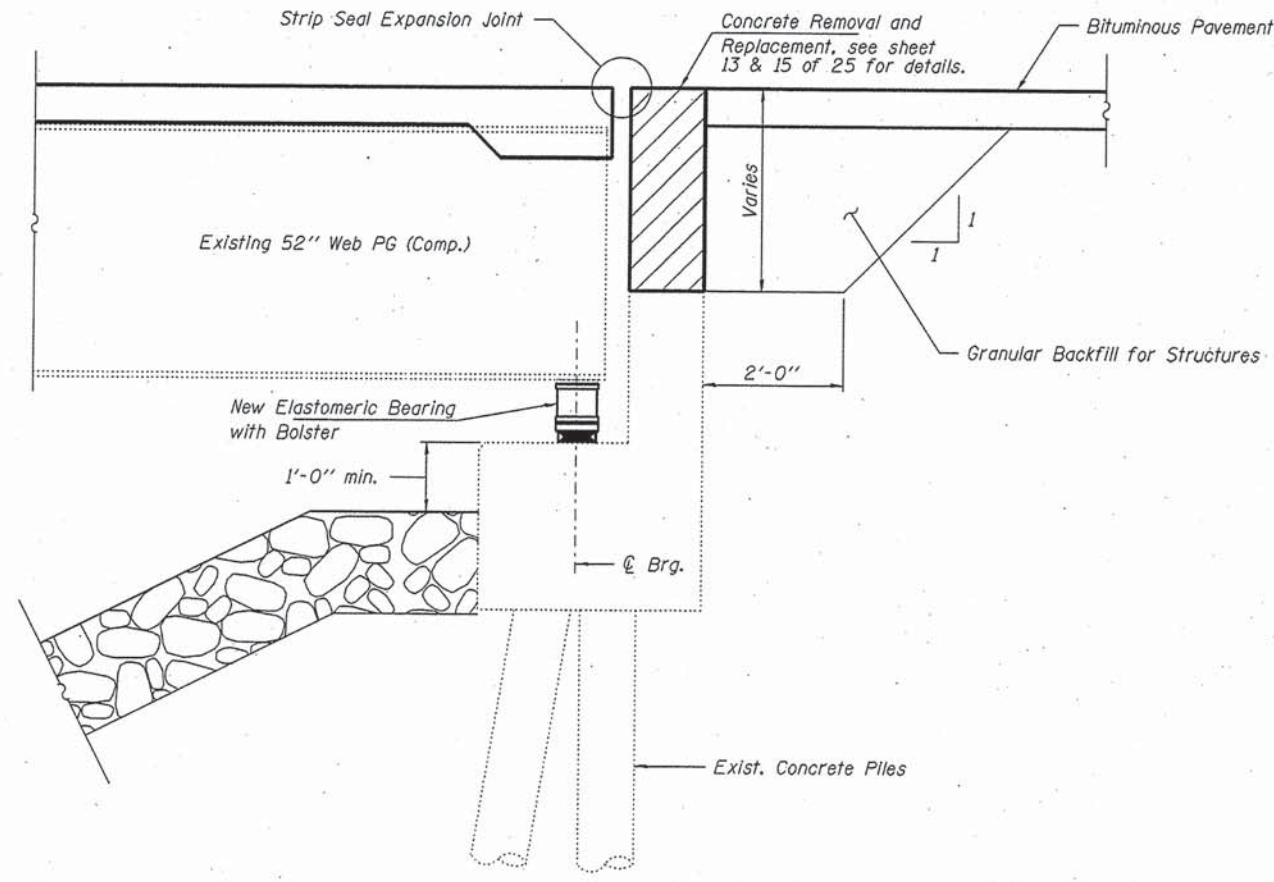
**GENERAL PLAN & ELEVATION**  
**C.H. 11 OVER**  
**MACKINAW RIVER**  
**FAS 2462 - SECTION 12-00047-00-BR**  
**TAZEWELL COUNTY**  
**STATION 168+13**  
**STRUCTURE NO. 090-3029**

FILE NAME = 110323-sht-br-bridge.dgn	USER NAME =	DESIGNED - J.R.T.	REVISED -	<b>STATE OF ILLINOIS</b> <b>TAZEWELL COUNTY HIGHWAY DEPARTMENT</b>	<b>GENERAL PLAN &amp; ELEVATION</b> <b>STRUCTURE NO. 090-3029</b> SHEET NO. 1 OF 25 SHEETS	C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 355 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	CHECKED - S.M.S.	REVISED -			11	12-00047-00-BR	TAZEWELL	39	10	
ILLINOIS PROFESSIONAL DESIGN FIRM LS 1 PE 1 SE CORP 144-00098	PLOT DATE = 8/18/2015	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 89653					
		CHECKED - M.D.C.	REVISED -			[ILLINOIS] FED. AID PROJECT					

**GENERAL NOTES**

Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts  $\frac{1}{2}$ "  $\phi$ , holes  $\frac{5}{16}$ "  $\phi$ , unless otherwise noted. No field welding is permitted except as specified in the contract documents. Reinforcement bars designated (E) shall be epoxy coated. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding  $\frac{1}{4}$  inch deep shall be identified and reported to the Engineer of Record for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work. Concrete Sealer shall be applied to the designated areas of the abutments. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. The Organic Zinc Rich Primer / Epoxy / Urethane Paint System shall be used for painting of new structural steel except where otherwise noted. The entire system shall be shop applied, with the exception of the exterior surface and the bottom of the bottom flange of fascia beams, masked off connection surfaces, field installed fasteners and damaged areas shall be touched up in the field. The color of the final finish coat for all steel surfaces shall be gray, Munsell No. 5B 7/1. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer. Cleaning and painting of the existing structural steel shall be as specified in the special provisions for "Cleaning and Painting Existing Steel Structures" and as specified below.

- All existing structural steel within 10 ft (measured along the beam) of either side of the deck joints shall be cleaned per Near White Blast Cleaning - SSPC-SP10. The exterior surfaces and bottom of the bottom flange of the fascia beams shall be cleaned per Commercial Grade Power Tool Cleaning - SSPC-SP15.
- All existing steel shall be painted according to the requirements of Paint System I - OZ/E/U. The color of the final finish coat for all steel surfaces shall be Gray Munsell No 5B 7/1.



**SECTION THRU ABUTMENT**  
(Horiz. Dimensions @ Rt. L's)

MACKINAW RIVER  
RE-BUILT 201 BY  
TAZEWELL COUNTY  
SEC. 12-00047-00-BR  
F.A.S. 2462 STA. 168+13  
STR. NO. 090-3029 LOADING HS-20

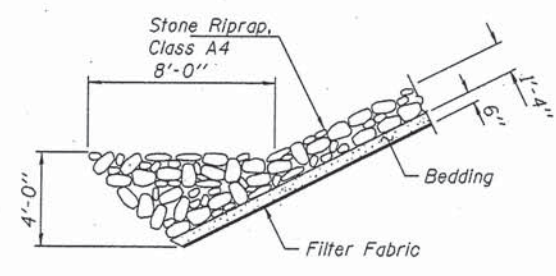
**NAME PLATE**  
See Std. 515001

Existing Name Plate shall be cleaned and relocated next to new Name Plate. Cost included with Name Plates.

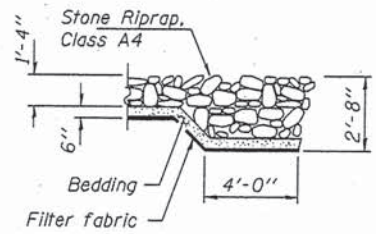
**TOTAL BILL OF MATERIAL**

ITEM	UNIT	SUPER	SUB	TOTAL
Stone Riprap, Class A4	Ton		270	270
Filter Fabric	Sq. Yd.		276	276
Concrete Removal	Cu. Yd.		9.3	9.3
Removal of Existing Concrete Deck	Each	1		1
Protective Shield	Sq. Yd.	722		722
Concrete Structures	Cu. Yd.		9.3	9.3
Concrete Superstructure	Cu. Yd.	385.7		385.7
Bridge Deck Grooving	Sq. Yd.	1,403		1,403
Protective Coat	Sq. Yd.	1,637		1,637
Furnishing and Erecting Structural Steel	Pound	1,300		1,300
Stud Shear Connectors	Each	3,330		3,330
Reinforcement Bars, Epoxy Coated	Pound	85,120	1,270	86,390
Steel Railing, Type SM	Foot	906		906
Name Plates	Each	1		1
Preformed Joint Strip Seal	Foot	60		60
Elastomeric Bearing Assembly, Type II	Each	10		10
Anchor Bolts, $\frac{3}{4}$ "	Each		20	20
Concrete Sealer	Sq. Ft.		575	575
Jack and Remove Existing Bearings	Each	10		10
Containment and Disposal of Lead Paint Cleaning Residues	L. Sum			1
Cleaning and Painting Steel Bridge No. 1	L. Sum			1
Granular Backfill for Structures	Cu. Yd.			36

\*Includes removal of existing parapet, railing and transverse joints.



**SECTION A-A**



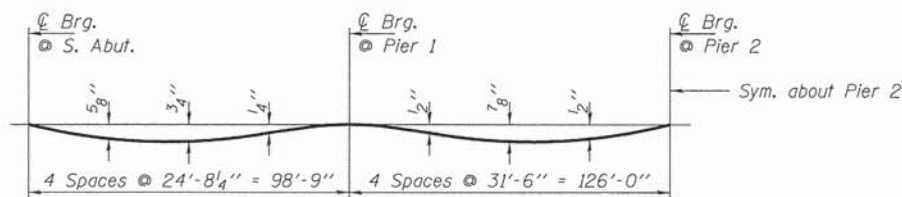
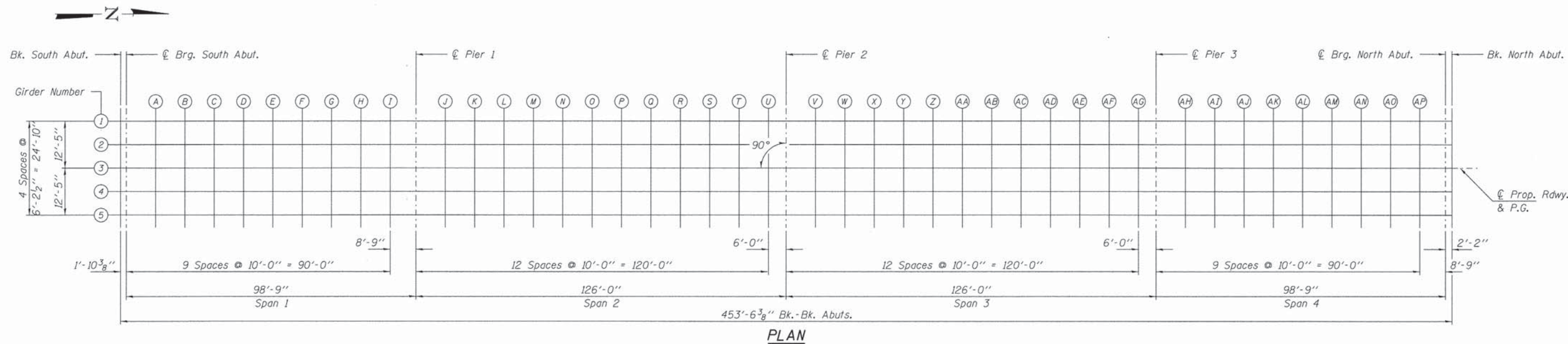
**SECTION B-B**

FILE NAME = 110323-ah-bridge.dgn	USER NAME = *USER*	DESIGNED - J.R.T.	REVISED -
HAMPTON, LENZINI AND RENWICK, INC. 3033 STEVENSON DRIVE, SUITE 101 SPRINGFIELD, ILLINOIS 62763 ILLINOIS PROFESSIONAL DESIGN FIRM L3/PE/SE CORP. 184020058	PLOT SCALE = *SCALE*	CHECKED - S.M.S.	REVISED -
	PLOT DATE = 9/8/2015	DRAWN - D.A.B.	REVISED -
		CHECKED - M.D.C.	REVISED -

STATE OF ILLINOIS  
TAZEWELL COUNTY HIGHWAY DEPARTMENT

GENERAL DETAILS  
STRUCTURE NO. 090-3029  
SHEET NO. 2 OF 25 SHEETS

C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
11	12-00047-00-BR	TAZEWELL	39	11
			CONTRACT NO. 89653	
[ILLINOIS] FED. AID PROJECT				

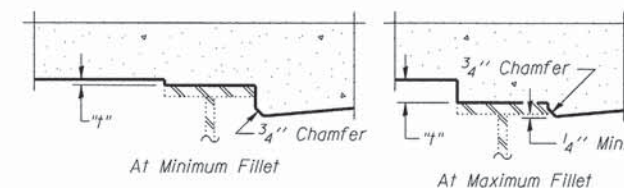


**DEAD LOAD DEFLECTION DIAGRAM**

(Includes weight of concrete only.)

Note:

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown on sheets 4 & 5 of 25.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown on sheets 4 & 5 of 25. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on sheets 4 & 5 of 25, minus slab thickness, equals the fillet heights "t" above top flange of beams.

**FILLET HEIGHTS**

FILE NAME = 110323-sht-brIDGE.dgn	USER NAME =	DESIGNED - J.R.T.	REVISED -	<b>STATE OF ILLINOIS TAZEWELL COUNTY HIGHWAY DEPARTMENT</b>	<b>TOP OF SLAB ELEVATIONS STRUCTURE NO. 090-3029</b>	C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		CHECKED - S.M.S.	REVISED -			11	12-00047-00-BR	TAZEWELL	39	12	
	PLOT SCALE =	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 89653					
	PLOT DATE = 8/18/2015	CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT					

**GIRDER 1**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	165+86.39	-12.42	510.74	510.74
☉ Brg. S. Abut.	165+88.25	-12.42	510.74	510.74
A	165+98.25	-12.42	510.74	510.76
B	166+08.25	-12.42	510.74	510.78
C	166+18.25	-12.42	510.74	510.79
D	166+28.25	-12.42	510.74	510.80
E	166+38.25	-12.42	510.74	510.79
F	166+48.25	-12.42	510.74	510.78
G	166+58.25	-12.42	510.74	510.77
H	166+68.25	-12.42	510.74	510.75
I	166+78.25	-12.42	510.74	510.74
☉ Pier 1	166+87.00	-12.42	510.74	510.74
J	166+97.00	-12.42	510.74	510.74
K	167+07.00	-12.42	510.74	510.76
L	167+17.00	-12.42	510.74	510.77
M	167+27.00	-12.42	510.74	510.79
N	167+37.00	-12.42	510.74	510.80
O	167+47.00	-12.42	510.74	510.81
P	167+57.00	-12.42	510.74	510.81
Q	167+67.00	-12.42	510.74	510.80
R	167+77.00	-12.42	510.74	510.78
S	167+87.00	-12.42	510.74	510.76
T	167+97.00	-12.42	510.74	510.75
U	168+07.00	-12.42	510.74	510.74
☉ Pier 2	168+13.00	-12.42	510.74	510.74
V	168+23.00	-12.42	510.74	510.74
W	168+33.00	-12.42	510.74	510.75
X	168+43.00	-12.42	510.74	510.77
Y	168+53.00	-12.42	510.74	510.79
Z	168+63.00	-12.42	510.74	510.80
AA	168+73.00	-12.42	510.74	510.81
AB	168+83.00	-12.42	510.74	510.81
AC	168+93.00	-12.42	510.74	510.80
AD	169+03.00	-12.42	510.74	510.79
AE	169+13.00	-12.42	510.74	510.77
AF	169+23.00	-12.42	510.74	510.75
AG	169+33.00	-12.42	510.74	510.74
☉ Pier 3	169+39.00	-12.42	510.74	510.74
AH	169+49.00	-12.42	510.74	510.74
AI	169+59.00	-12.42	510.74	510.75
AJ	169+69.00	-12.42	510.74	510.77
AK	169+79.00	-12.42	510.74	510.78
AL	169+89.00	-12.42	510.74	510.79
AM	169+99.00	-12.42	510.74	510.80
AN	170+09.00	-12.42	510.74	510.79
AO	170+19.00	-12.42	510.74	510.78
AP	170+29.00	-12.42	510.74	510.76
☉ Brg. N. Abut.	170+37.75	-12.42	510.74	510.74
Bk. N. Abut.	170+39.92	-12.42	510.74	510.74

**GIRDER 2**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	165+86.39	-6.21	510.83	510.83
☉ Brg. S. Abut.	165+88.25	-6.21	510.83	510.83
A	165+98.25	-6.21	510.83	510.86
B	166+08.25	-6.21	510.83	510.88
C	166+18.25	-6.21	510.83	510.89
D	166+28.25	-6.21	510.83	510.89
E	166+38.25	-6.21	510.83	510.89
F	166+48.25	-6.21	510.83	510.88
G	166+58.25	-6.21	510.83	510.86
H	166+68.25	-6.21	510.83	510.85
I	166+78.25	-6.21	510.83	510.84
☉ Pier 1	166+87.00	-6.21	510.83	510.83
J	166+97.00	-6.21	510.83	510.84
K	167+07.00	-6.21	510.83	510.85
L	167+17.00	-6.21	510.83	510.87
M	167+27.00	-6.21	510.83	510.89
N	167+37.00	-6.21	510.83	510.90
O	167+47.00	-6.21	510.83	510.91
P	167+57.00	-6.21	510.83	510.90
Q	167+67.00	-6.21	510.83	510.90
R	167+77.00	-6.21	510.83	510.88
S	167+87.00	-6.21	510.83	510.86
T	167+97.00	-6.21	510.83	510.84
U	168+07.00	-6.21	510.83	510.83
☉ Pier 2	168+13.00	-6.21	510.83	510.83
V	168+23.00	-6.21	510.83	510.84
W	168+33.00	-6.21	510.83	510.85
X	168+43.00	-6.21	510.83	510.87
Y	168+53.00	-6.21	510.83	510.89
Z	168+63.00	-6.21	510.83	510.90
AA	168+73.00	-6.21	510.83	510.91
AB	168+83.00	-6.21	510.83	510.91
AC	168+93.00	-6.21	510.83	510.90
AD	169+03.00	-6.21	510.83	510.88
AE	169+13.00	-6.21	510.83	510.86
AF	169+23.00	-6.21	510.83	510.85
AG	169+33.00	-6.21	510.83	510.84
☉ Pier 3	169+39.00	-6.21	510.83	510.83
AH	169+49.00	-6.21	510.83	510.84
AI	169+59.00	-6.21	510.83	510.85
AJ	169+69.00	-6.21	510.83	510.87
AK	169+79.00	-6.21	510.83	510.88
AL	169+89.00	-6.21	510.83	510.89
AM	169+99.00	-6.21	510.83	510.89
AN	170+09.00	-6.21	510.83	510.89
AO	170+19.00	-6.21	510.83	510.87
AP	170+29.00	-6.21	510.83	510.85
☉ Brg. N. Abut.	170+37.75	-6.21	510.83	510.83
Bk. N. Abut.	170+39.92	-6.21	510.83	510.83

**GIRDER 3, ☉ STRUCTURE, RDWY., & P.G.**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	165+86.39	0.00	510.93	510.93
☉ Brg. S. Abut.	165+88.25	0.00	510.93	510.93
A	165+98.25	0.00	510.93	510.95
B	166+08.25	0.00	510.93	510.97
C	166+18.25	0.00	510.93	510.99
D	166+28.25	0.00	510.93	510.99
E	166+38.25	0.00	510.93	510.99
F	166+48.25	0.00	510.93	510.98
G	166+58.25	0.00	510.93	510.96
H	166+68.25	0.00	510.93	510.94
I	166+78.25	0.00	510.93	510.93
☉ Pier 1	166+87.00	0.00	510.93	510.93
J	166+97.00	0.00	510.93	510.94
K	167+07.00	0.00	510.93	510.95
L	167+17.00	0.00	510.93	510.97
M	167+27.00	0.00	510.93	510.99
N	167+37.00	0.00	510.93	511.00
O	167+47.00	0.00	510.93	511.00
P	167+57.00	0.00	510.93	511.00
Q	167+67.00	0.00	510.93	510.99
R	167+77.00	0.00	510.93	510.98
S	167+87.00	0.00	510.93	510.96
T	167+97.00	0.00	510.93	510.94
U	168+07.00	0.00	510.93	510.93
☉ Pier 2	168+13.00	0.00	510.93	510.93
V	168+23.00	0.00	510.93	510.94
W	168+33.00	0.00	510.93	510.95
X	168+43.00	0.00	510.93	510.97
Y	168+53.00	0.00	510.93	510.98
Z	168+63.00	0.00	510.93	511.00
AA	168+73.00	0.00	510.93	511.00
AB	168+83.00	0.00	510.93	511.00
AC	168+93.00	0.00	510.93	510.99
AD	169+03.00	0.00	510.93	510.98
AE	169+13.00	0.00	510.93	510.96
AF	169+23.00	0.00	510.93	510.94
AG	169+33.00	0.00	510.93	510.93
☉ Pier 3	169+39.00	0.00	510.93	510.93
AH	169+49.00	0.00	510.93	510.93
AI	169+59.00	0.00	510.93	510.95
AJ	169+69.00	0.00	510.93	510.96
AK	169+79.00	0.00	510.93	510.98
AL	169+89.00	0.00	510.93	510.99
AM	169+99.00	0.00	510.93	510.99
AN	170+09.00	0.00	510.93	510.99
AO	170+19.00	0.00	510.93	510.97
AP	170+29.00	0.00	510.93	510.95
☉ Brg. N. Abut.	170+37.75	0.00	510.93	510.93
Bk. N. Abut.	170+39.92	0.00	510.93	510.93

**GIRDER 4**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	165+86.39	6.21	510.83	510.83
☉ Brg. S. Abut.	165+88.25	6.21	510.83	510.83
A	165+98.25	6.21	510.83	510.86
B	166+08.25	6.21	510.83	510.88
C	166+18.25	6.21	510.83	510.89
D	166+28.25	6.21	510.83	510.89
E	166+38.25	6.21	510.83	510.89
F	166+48.25	6.21	510.83	510.88
G	166+58.25	6.21	510.83	510.86
H	166+68.25	6.21	510.83	510.85
I	166+78.25	6.21	510.83	510.84
☉ Pier 1	166+87.00	6.21	510.83	510.83
J	166+97.00	6.21	510.83	510.84
K	167+07.00	6.21	510.83	510.85
L	167+17.00	6.21	510.83	510.87
M	167+27.00	6.21	510.83	510.89
N	167+37.00	6.21	510.83	510.90
O	167+47.00	6.21	510.83	510.91
P	167+57.00	6.21	510.83	510.90
Q	167+67.00	6.21	510.83	510.90
R	167+77.00	6.21	510.83	510.88
S	167+87.00	6.21	510.83	510.86
T	167+97.00	6.21	510.83	510.84
U	168+07.00	6.21	510.83	510.83
☉ Pier 2	168+13.00	6.21	510.83	510.83
V	168+23.00	6.21	510.83	510.84
W	168+33.00	6.21	510.83	510.85
X	168+43.00	6.21	510.83	510.87
Y	168+53.00	6.21	510.83	510.89
Z	168+63.00	6.21	510.83	510.90
AA	168+73.00	6.21	510.83	510.91
AB	168+83.00	6.21	510.83	510.91
AC	168+93.00	6.21	510.83	510.90
AD	169+03.00	6.21	510.83	510.88
AE	169+13.00	6.21	510.83	510.86
AF	169+23.00	6.21	510.83	510.85
AG	169+33.00	6.21	510.83	510.84
☉ Pier 3	169+39.00	6.21	510.83	510.83
AH	169+49.00	6.21	510.83	510.84
AI	169+59.00	6.21	510.83	510.85
AJ	169+69.00	6.21	510.83	510.87
AK	169+79.00	6.21	510.83	510.88
AL	169+89.00	6.21	510.83	510.89
AM	169+99.00	6.21	510.83	510.89
AN	170+09.00	6.21	510.83	510.89
AO	170+19.00	6.21	510.83	510.87
AP	170+29.00	6.21	510.83	510.85
☉ Brg. N. Abut.	170+37.75	6.21	510.83	510.83
Bk. N. Abut.	170+39.92	6.21	510.83	510.83

**GIRDER 5**

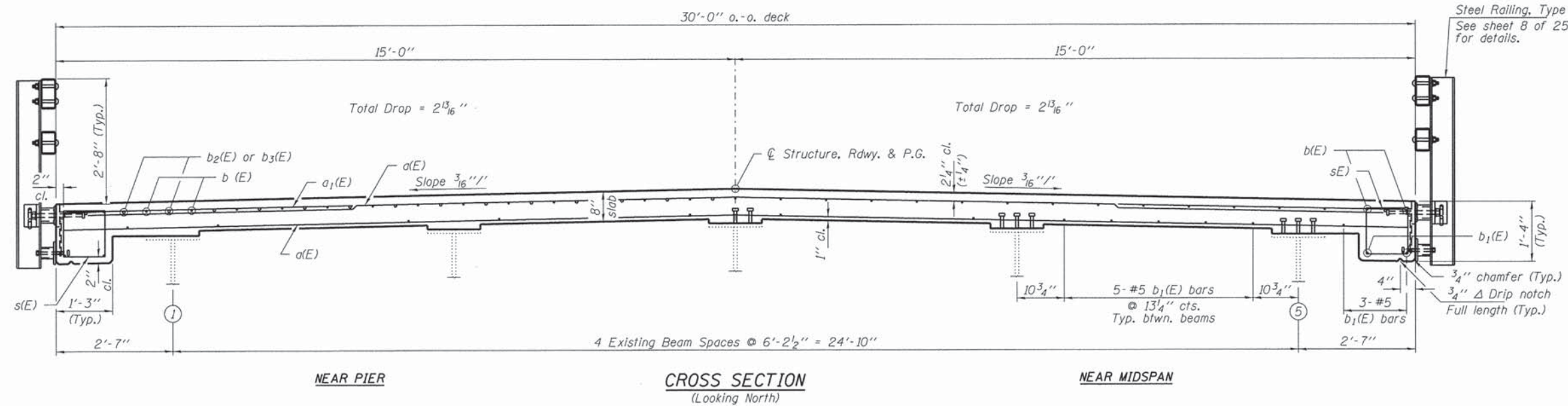
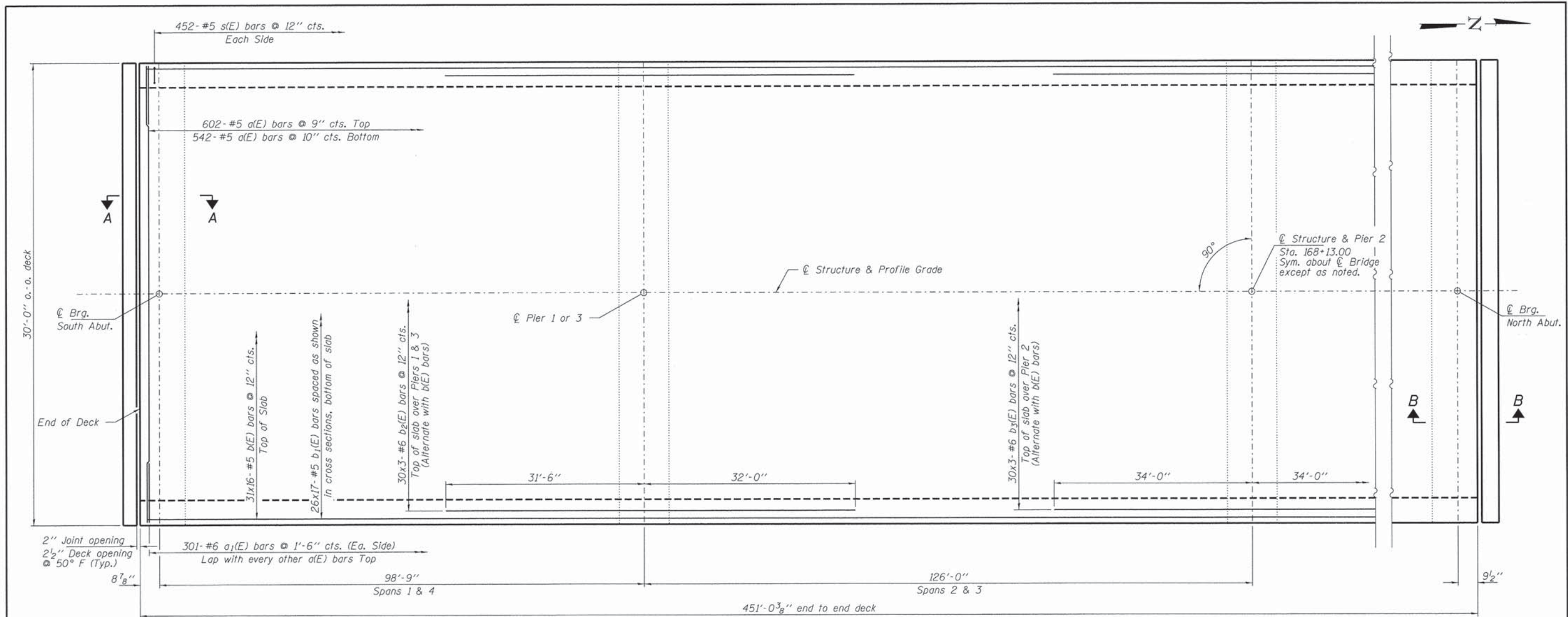
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	165+86.39	12.42	510.74	510.74
☉ Brg. S. Abut.	165+88.25	12.42	510.74	510.74
A	165+98.25	12.42	510.74	510.76
B	166+08.25	12.42	510.74	510.78
C	166+18.25	12.42	510.74	510.79
D	166+28.25	12.42	510.74	510.80
E	166+38.25	12.42	510.74	510.79
F	166+48.25	12.42	510.74	510.78
G	166+58.25	12.42	510.74	510.77
H	166+68.25	12.42	510.74	510.75
I	166+78.25	12.42	510.74	510.74
☉ Pier 1	166+87.00	12.42	510.74	510.74
J	166+97.00	12.42	510.74	510.74
K	167+07.00	12.42	510.74	510.76
L	167+17.00	12.42	510.74	510.77
M	167+27.00	12.42	510.74	510.79
N	167+37.00	12.42	510.74	510.80
O	167+47.00	12.42	510.74	510.81
P	167+57.00	12.42	510.74	510.81
Q	167+67.00	12.42	510.74	510.80
R	167+77.00	12.42	510.74	510.78
S	167+87.00	12.42	510.74	510.76
T	167+97.00	12.42	510.74	510.75
U	168+07.00	12.42	510.74	510.74
☉ Pier 2	168+13.00	12.42	510.74	510.74
V	168+23.00	12.42	510.74	510.74
W	168+33.00	12.42	510.74	510.75
X	168+43.00	12.42	510.74	510.77
Y	168+53.00	12.42	510.74	510.79
Z	168+63.00	12.42	510.74	510.80
AA	168+73.00	12.42	510.74	510.81
AB	168+83.00	12.42	510.74	510.81
AC	168+93.00	12.42	510.74	510.80
AD	169+03.00	12.42	510.74	510.79
AE	169+13.00	12.42	510.74	510.77
AF	169+23.00	12.42	510.74	510.75
AG	169+33.00	12.42	510.74	510.74
☉ Pier 3	169+39.00	12.42	510.74	510.74
AH	169+49.00	12.42	510.74	510.74
AI	169+59.00	12.42	510.74	510.75
AJ	169+69.00	12.42	510.74	510.77
AK	169+79.00	12.42	510.74	510.78
AL	169+89.00	12.42	510.74	510.79
AM	169+99.00	12.42	510.74	510.80
AN	170+09.00	12.42	510.74	510.79
AO	170+19.00	12.42	510.74	510.78
AP	170+29.00	12.42	510.74	510.76
☉ Brg. N. Abut.	170+37.75	12.42	510.74	510.74
Bk. N. Abut.	170+39.92	12.42	510.74	510.74

FILE NAME = 110323-shr-bridge.dgn	USER NAME =	DESIGNED - J.R.T.	REVISED -
<b>HAMPTON, LENZINI AND RENWICK, INC.</b> 2005 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	CHECKED - S.M.S	REVISED -
<b>HLR</b> ILLINOIS PROFESSIONAL DESIGN FIRM L3 / P.E. / S.E. COMP. 184-00089	PLOT DATE = 8/18/2015	DRAWN - D.A.B.	REVISED -
		CHECKED - M.D.C.	REVISED -

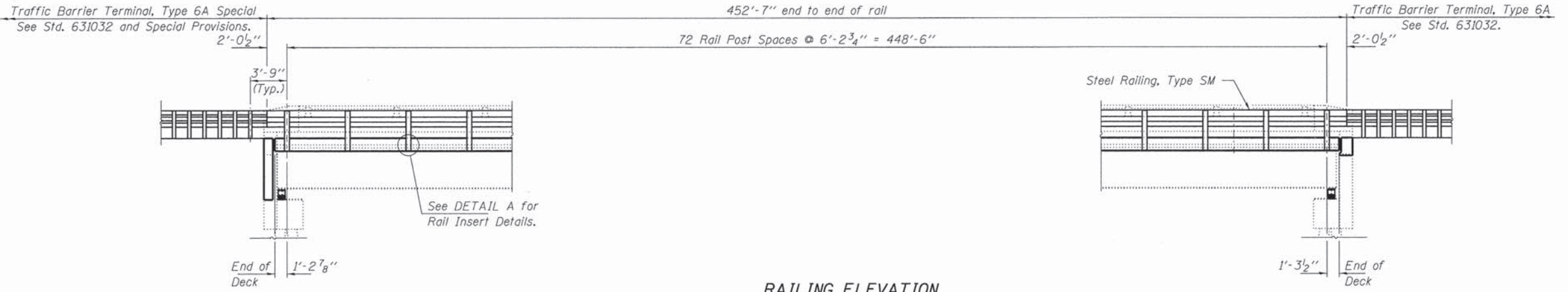
**STATE OF ILLINOIS  
TAZEWELL COUNTY HIGHWAY DEPARTMENT**

**TOP OF SLAB ELEVATIONS  
STRUCTURE NO. 090-3029**

C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
11	12-00047-00-BR	TAZEWELL	39	14
			CONTRACT NO. 89653	
ILLINOIS FED. AID PROJECT				

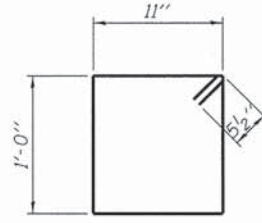


FILE NAME = 110323-sht-bridge.dgn	USER NAME =	DESIGNED - J.R.T.	REVISED -	<b>STATE OF ILLINOIS TAZEWELL COUNTY HIGHWAY DEPARTMENT</b>	<b>SUPERSTRUCTURE STRUCTURE NO. 090-3029</b>	C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
<b>HAMPTON, LENZINI AND RENWICK, INC.</b> 2009 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	CHECKED - S.M.S.	REVISED -			11	12-00047-00-BR	TAZEWELL	39	15	
<b>HLR</b> ILLINOIS PROFESSIONAL DESIGN FIRM 151 PE / SE CORP. 184 000099	PLOT DATE = 8/18/2015	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 89653					
		CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT					

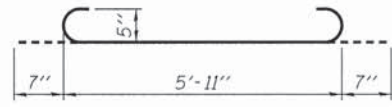


**RAILING ELEVATION**  
Showing Rail Post Spaces

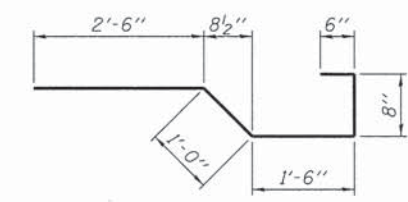
Notes:  
See sheets 14 and 16 of 25 for abutment backwall details.  
See sheet 8 of 25 for Railing Details.



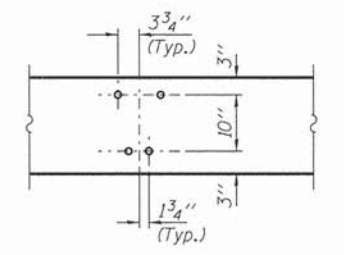
**BAR s(E)**



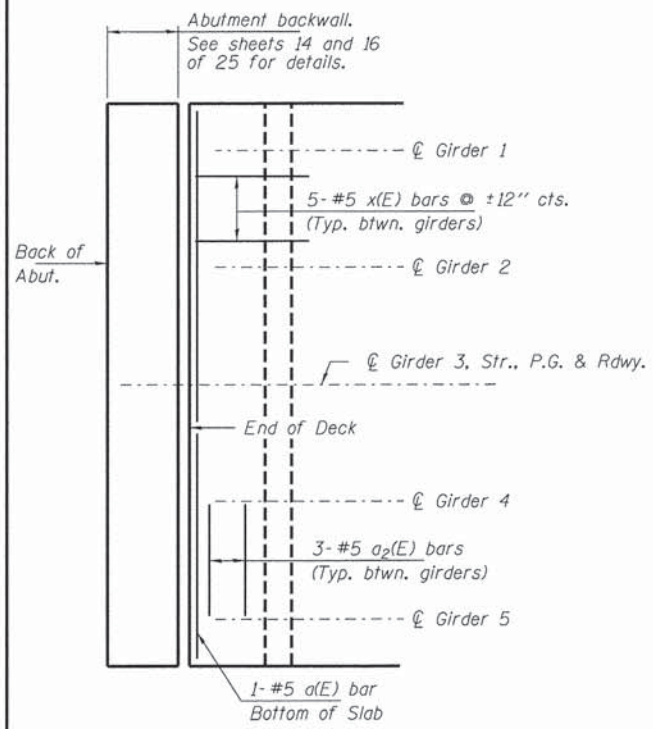
**BAR a2(E)**



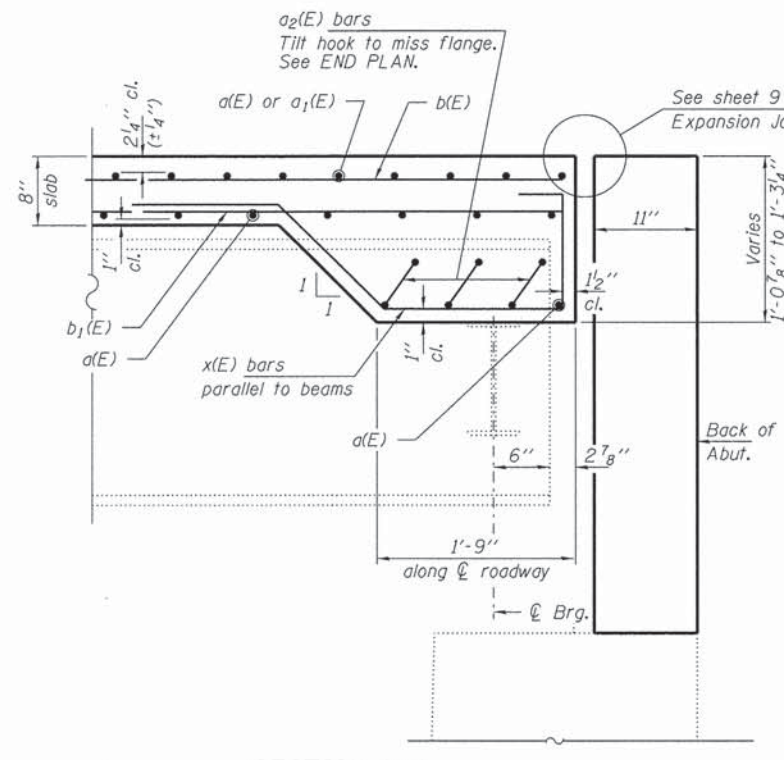
**BAR x(E)**



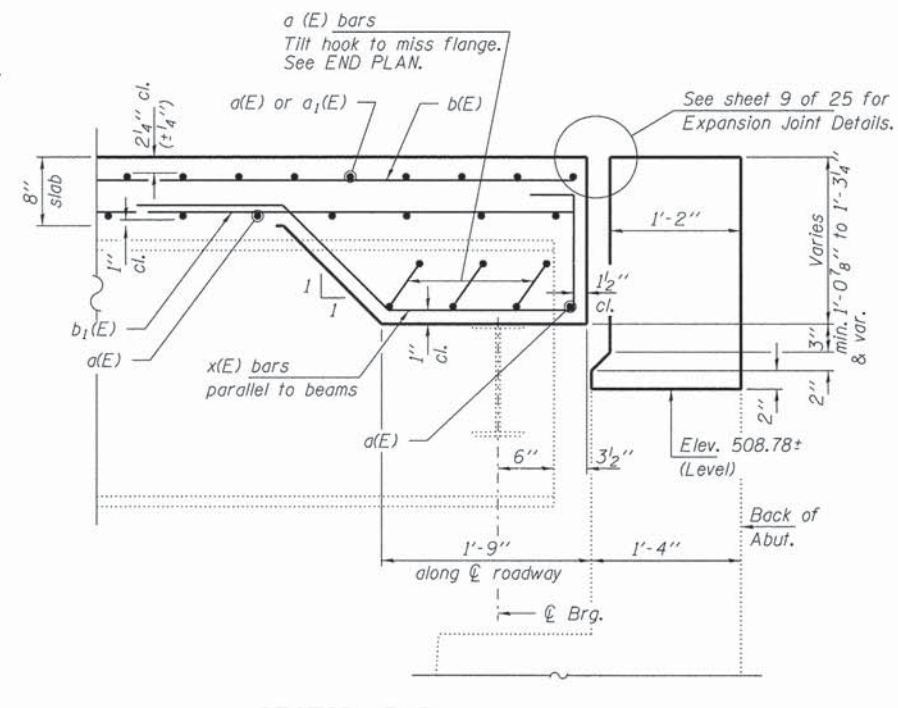
**DETAIL A**



**END PLAN**  
(Typ. Each End of Deck)



**SECTION A-A**  
(South Abut.)



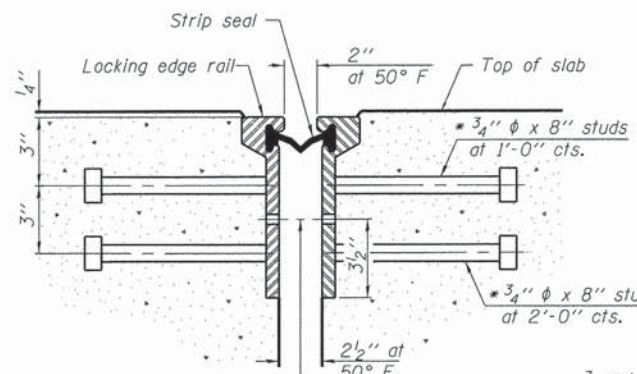
**SECTION B-B**  
(North Abut.)

**SUPERSTRUCTURE BILL OF MATERIAL**

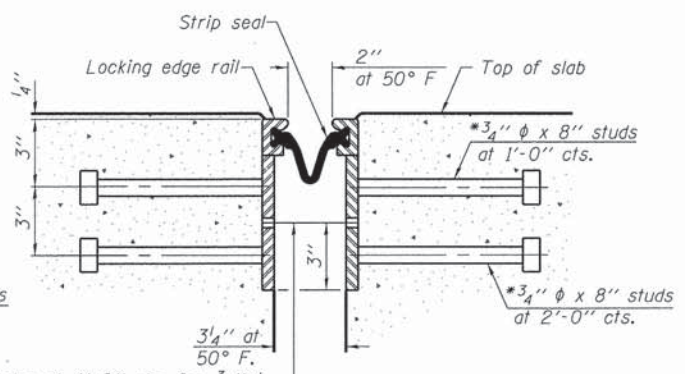
BAR	NO.	SIZE	LENGTH	SHAPE
a(E)	1,146	#5	29'-8"	—
a1(E)	602	#6	6'-6"	—
a2(E)	24	#5	7'-1"	—
b(E)	496	#5	30'-8"	—
b1(E)	442	#5	29'-0"	—
b2(E)	180	#6	23'-3"	—
b3(E)	90	#6	24'-9"	—
s(E)	904	#5	4'-9"	□
x(E)	40	#5	6'-2"	—
Concrete Superstructure			Cu. Yd.	385.7
Bridge Deck Grooving			Sq. Yd.	1,403
Protective Coat			Sq. Yd.	1,637
Reinforcement Bars, Epoxy Coated			Pound	85,120



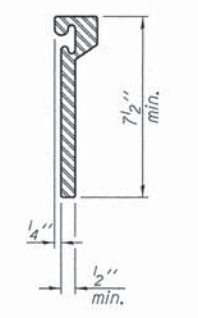




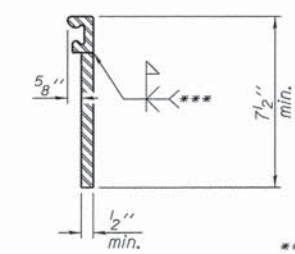
**SECTION THRU  
ROLLED RAIL JOINT**



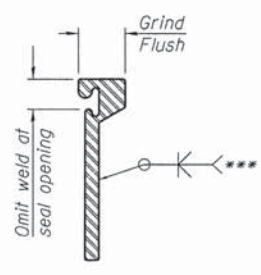
**SECTION THRU  
WELDED RAIL JOINT**



**ROLLED  
EXTRUDED RAIL**



**WELDED RAIL**



**LOCKING EDGE  
RAIL SPLICE**

The inside of the locking edge rail groove shall be free of weld residue.  
Rolled rail shown, welded rail similar.

\*\*\* Back gouge not required if complete joint penetration is verified by mock-up.

7/16"  $\phi$  holes at 4'-0" cts. for 3/8"  $\phi$  bolts. All bolts shall be burned, sawed, or chipped off flush with the plates after forms are removed, typ.

7/16"  $\phi$  holes at 4'-0" cts. for 3/8"  $\phi$  bolts. All bolts shall be burned, sawed, or chipped off flush with the plates after forms are removed, typ.

\* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

**Notes:**  
The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.  
The Locking Edge Rails depicted are conceptual only, except for the minimum dimensions shown. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities.  
The manufacturer's recommended installation methods shall be followed.  
The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.  
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications. Maximum space between rail segments shall be 3/16", sealed with a suitable sealant. Joints in rails within 10 ft. of curbs shall be welded.  
Parapet plates and anchorage studs for skews > 30° included in the cost of Preformed Joint Strip Seal.

**BILL OF MATERIAL**

Item	Unit	Total
Preformed Joint Strip Seal	Foot	60

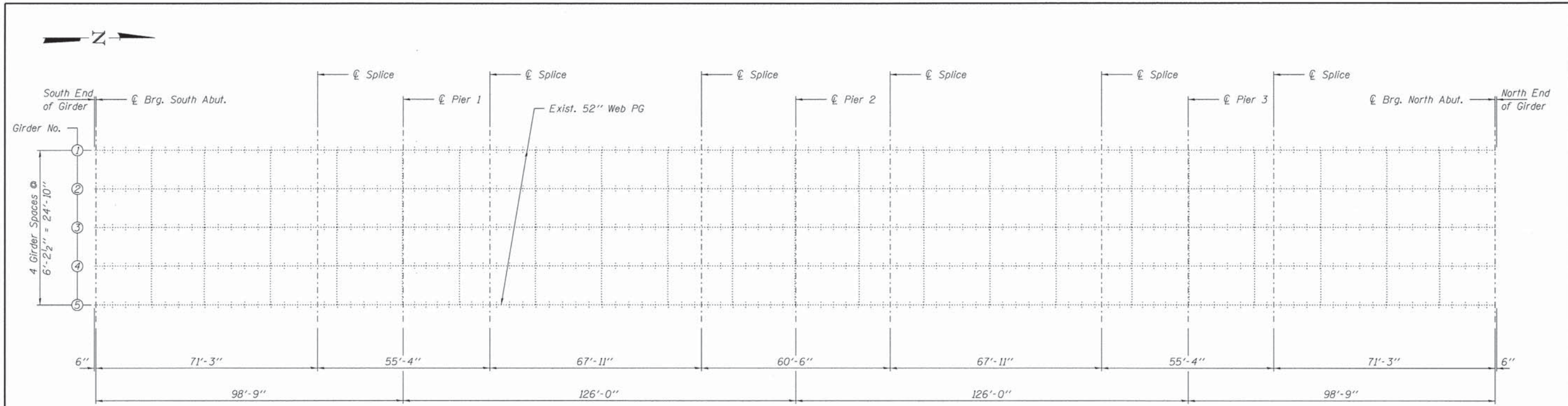
FILE NAME = 110323-sh1-bridge.dgn	USER NAME =	DESIGNED - J.R.T.	REVISED -
HAMPTON, LENZINI AND RENWICK, INC. 2005 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	CHECKED - S.M.S.	REVISED -
ILLINOIS PROFESSIONAL DESIGN FIRM L31/P/E/SE CORP. 184-000089	PLOT DATE = 8/18/2015	DRAWN - D.A.B.	REVISED -
		CHECKED - M.D.C.	REVISED -

**STATE OF ILLINOIS  
TAZEWELL COUNTY HIGHWAY DEPARTMENT**

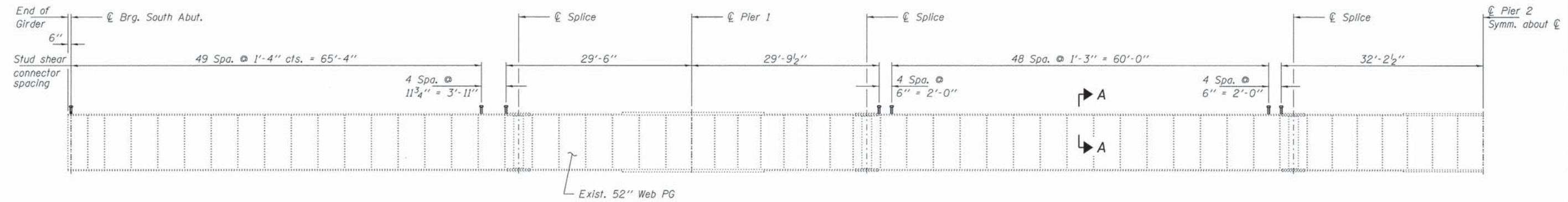
**PREFORMED JOINT STRIP SEAL  
STRUCTURE NO. 090-3029**

SHEET NO. 9 OF 25 SHEETS

C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
11	12-00047-00-BR	TAZEWELL	39	18
			CONTRACT NO. 89653	
ILLINOIS FED. AID PROJECT				

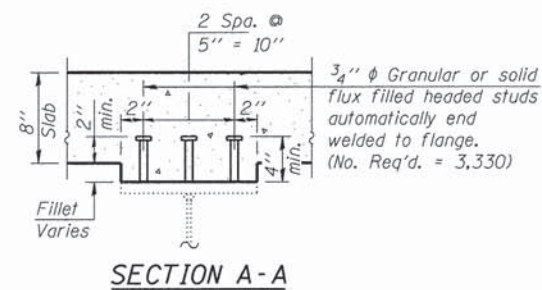


**EXISTING FRAMING PLAN**



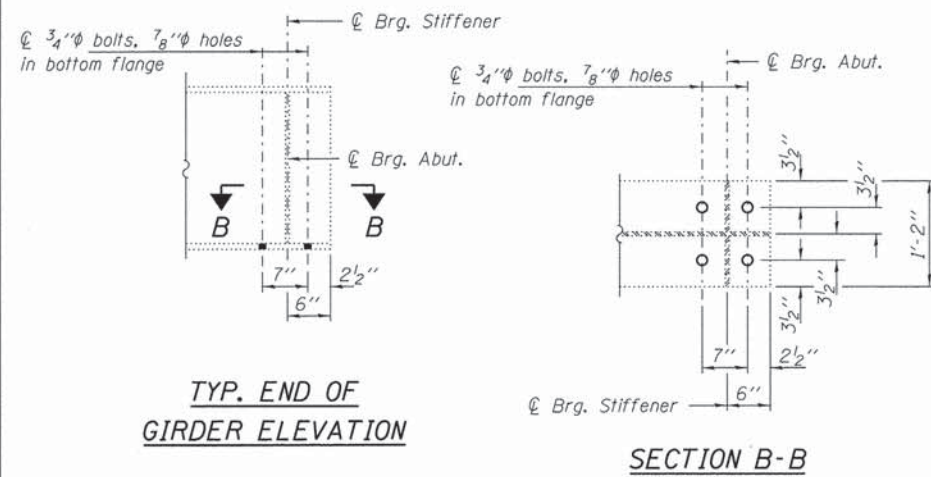
**EXISTING GIRDER ELEVATION**

Note: For additional structural steel details see sheets 11 of 25.



FILE NAME = 110323-sht-bridge.dgn	USER NAME =	DESIGNED - J.R.T.	REVISED -	<b>STATE OF ILLINOIS TAZEWELL COUNTY HIGHWAY DEPARTMENT</b>	<b>STRUCTURAL STEEL STRUCTURE NO. 090-3029</b>	C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
<b>HAMPTON, LENZINI AND RENWICK, INC.</b> 300 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	CHECKED - S.M.S.	REVISED -			11	12-00047-00-BR	TAZEWELL	39	19	
<b>ILLINOIS PROFESSIONAL DESIGN FIRM</b> L.S. / P.E. / S.E. CORP. 184-00099	PLOT DATE = 8/18/2015	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 89653					
		CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT					

SHEET NO. 10 OF 25 SHEETS



INTERIOR GIRDER MOMENT TABLE					
		0.4 Sp. 1 or 0.6 Sp. 4	Pier 1 or Pier 3	0.5 Sp. 2 or Sp. 3	Pier 2
$I_s$	(in <sup>4</sup> )	24,059	35,781	24,059	39,797
$I_c(n)$	(in <sup>4</sup> )	53,652	-	53,652	-
$I_c(3n)$	(in <sup>4</sup> )	40,630	-	40,630	-
$S_s$	(in <sup>3</sup> )	891	1,298	891	1,434
$S_c(n)$	(in <sup>3</sup> )	1,156	-	1,156	-
$S_c(3n)$	(in <sup>3</sup> )	1,072	-	1,072	-
$\phi$	(k/')	0.83	0.90	0.83	0.92
$M\phi$	(k)	526	1,119	494	1,201
$s\phi$	(k/')	0.32	0.32	0.32	0.32
$M_s\phi$	(k)	229	394	239	440
$M_t$	(k)	709	645	773	737
$M_i$	(k)	159	136	154	147
$\phi_s [M_t + i]$	(k)	1,447	1,302	1,545	1,473
$M_a$	(k)	2,862	3,659	2,961	4,049
$M_u$	(k)	4,602	-	4,602	-
$f_s \phi$ non-comp	(ksi)	7.1	10.3	6.7	10.1
$f_s \phi$ (comp)	(ksi)	2.6	3.6	2.7	3.7
$f_s \phi_s [M_t + M_i]$	(ksi)	15.0	12.0	16.0	12.3
$f_s$ (Overload)	(ksi)	24.7	26.0	25.4	26.1
$f_s$ (Total)	(ksi)	-	33.8	-	33.9
VR	(k)	48.5	-	48.0	-

INTERIOR GIRDER REACTION TABLE				
		Abut.	Pier 1 or 3	Pier 2
$R\phi$	(k)	42.0	146.3	150.5
$R_t$	(k)	35.9	61.9	65.1
$R_i$	(k)	8.0	13.1	13.0
$R_{Total}$	(k)	85.9	221.3	228.6

$I_s, S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$  (Total and Overload) due to non-composite dead loads (in<sup>4</sup> and in<sup>3</sup>).

$I_c(n), S_c(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing  $f_s$  (Total and Overload) due to short-term composite live loads (in<sup>4</sup> and in<sup>3</sup>).

$I_c(3n), S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Total and Overload) due to long-term composite (superimposed) dead loads (in<sup>4</sup> and in<sup>3</sup>).

$\phi$ : Un-factored non-composite dead load (kips/ft.).

$M\phi$ : Un-factored moment due to non-composite dead load (kip-ft.).

$s\phi$ : Un-factored long-term composite (superimposed) dead load (kips/ft.).

$M_s\phi$ : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).

$M_t$ : Un-factored live load moment (kip-ft.).

$M_i$ : Un-factored moment due to impact (kip-ft.).

$M_a$ : Factored design moment (kip-ft.).

$1.3 [M\phi + M_s\phi + \frac{2}{3} (M_t + M_i)]$

$M_u$ : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).

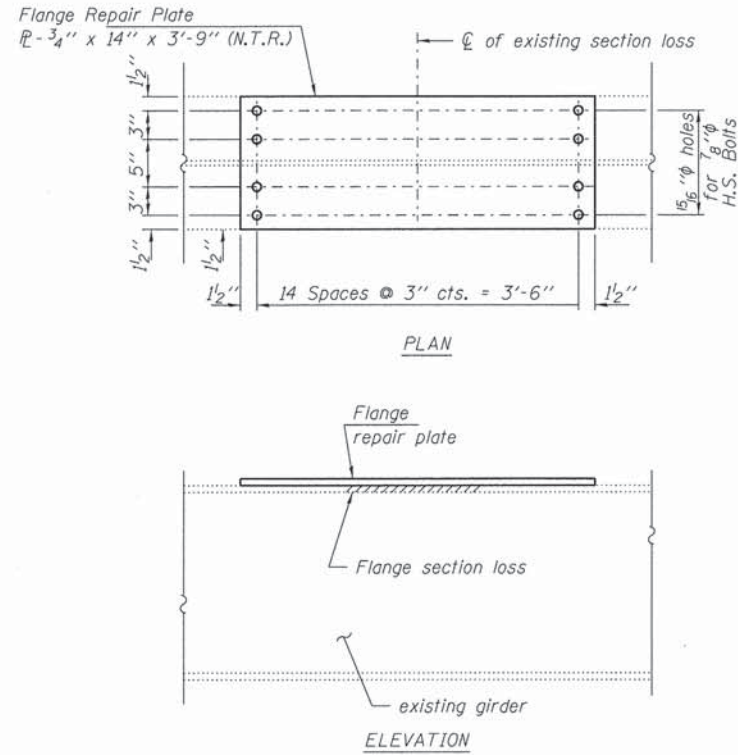
$f_s$  (Overload): Sum of stresses as computed from the moments below (ksi).

$M\phi + M_s\phi + \frac{2}{3} (M_t + M_i)$

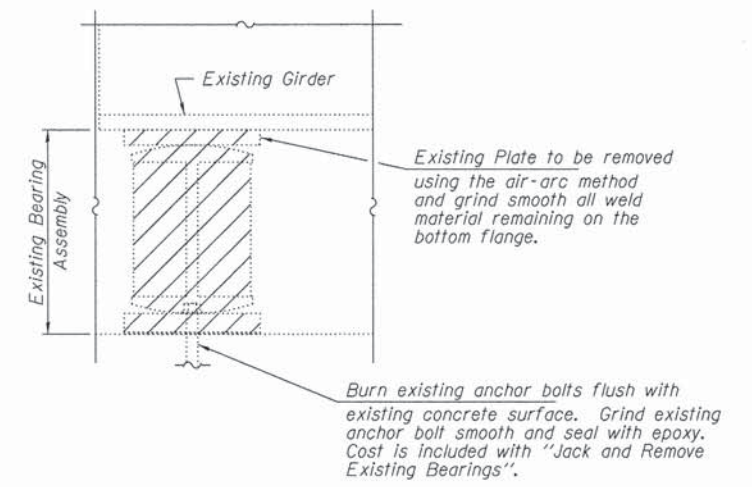
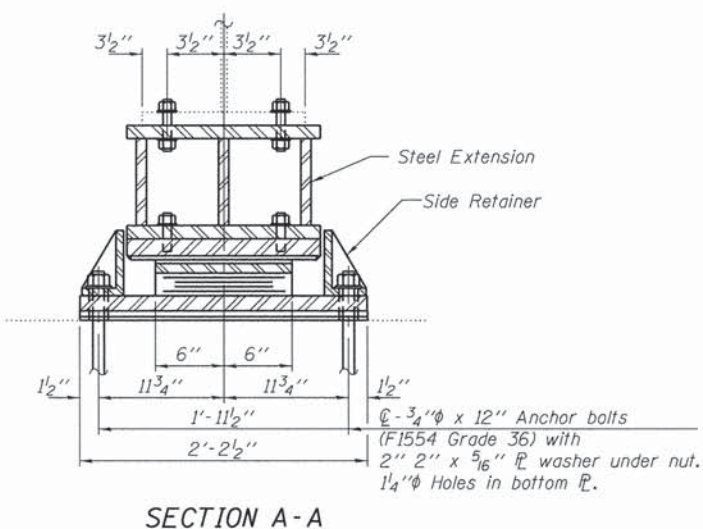
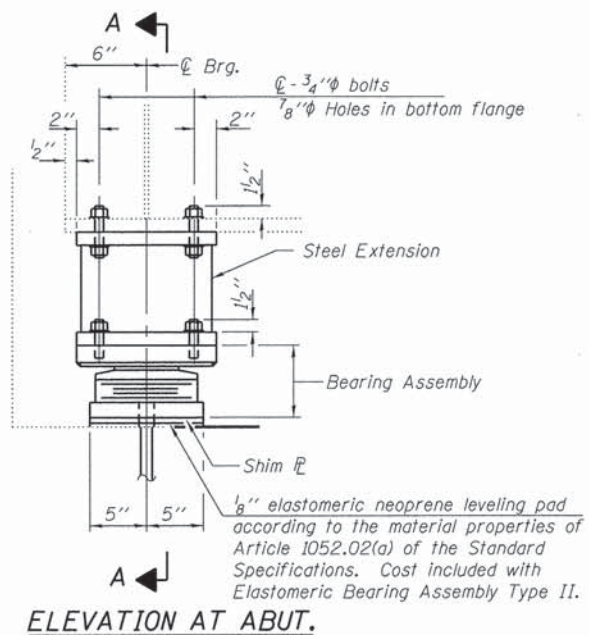
$f_s$  (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).

$1.3 [M\phi + M_s\phi + \frac{2}{3} (M_t + M_i)]$

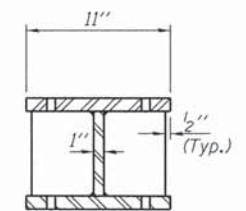
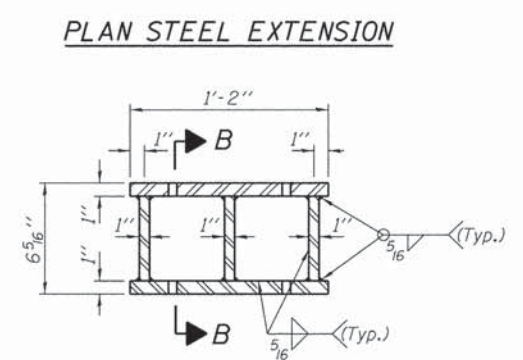
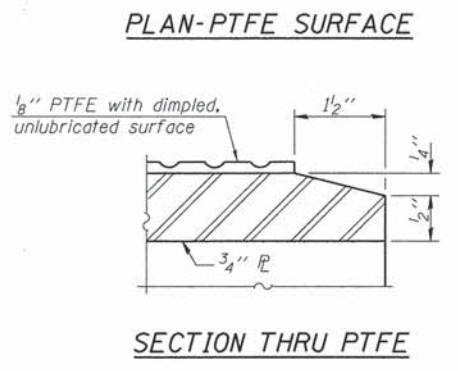
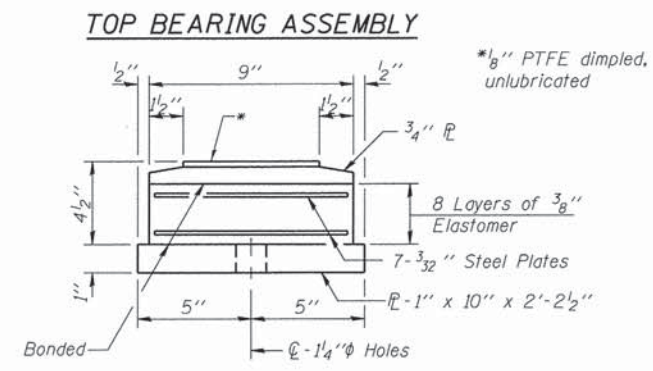
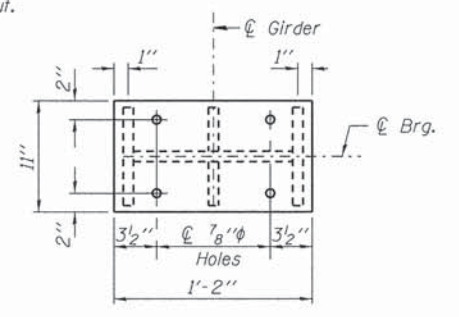
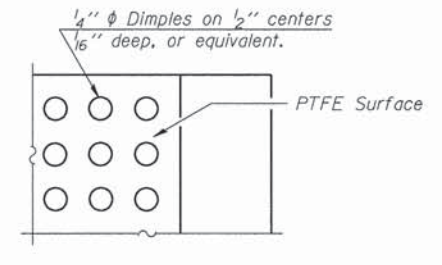
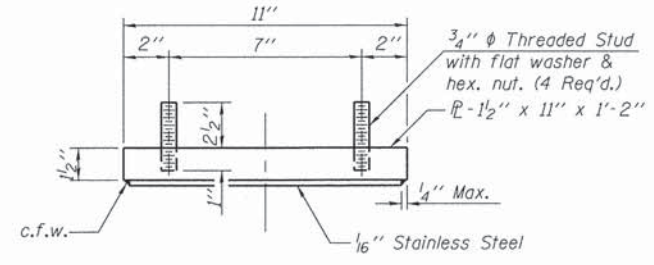
VR: Maximum  $\phi$  + impact shear range within the composite portion of the span for stud shear connector design (kips).



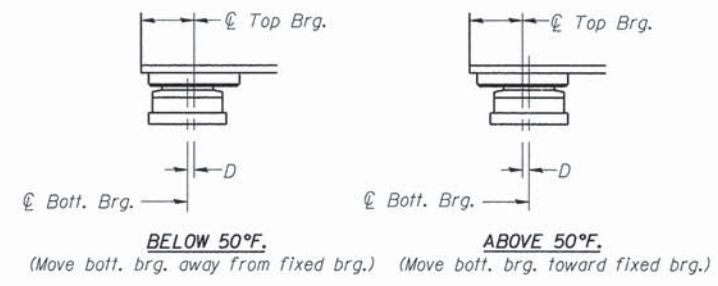
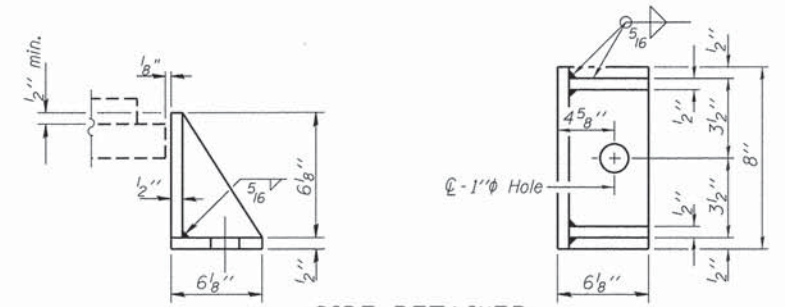
Notes:  
 For possible use at flange section loss locations as directed by the Engineer, See Special Provision.  
 HS bolts shall be 7/8"  $\phi$  ASTM A325.  
 If transverse stiffeners interfere with bolt placement, one row of bolts may be omitted from the repair plate.  
 Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.



**TYPE II ELASTOMERIC EXP. BRG.**  
(Typ. Both Abuts.)

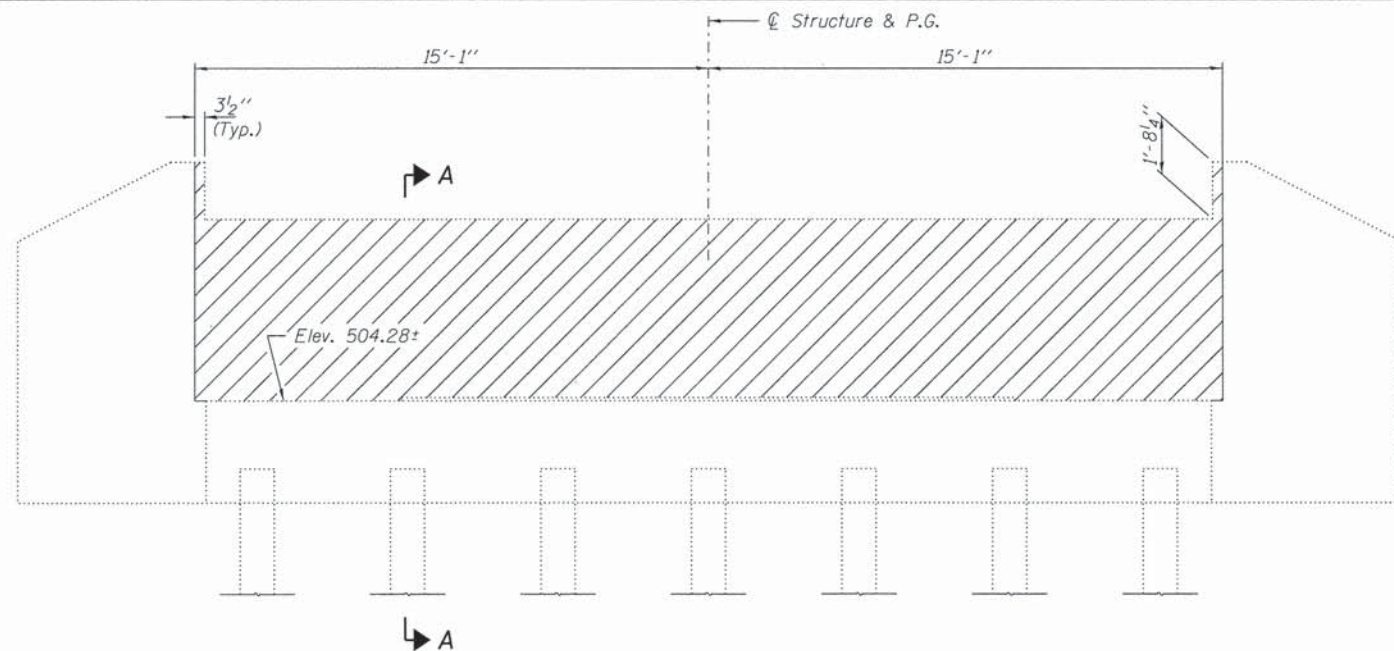


**Notes:**  
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.  
Anchor bolts for Type II bearings shall be placed in holes drilled in the concrete through holes in the bottom bearing plate after members are in place. Side retainers shall be placed after bolts are installed.  
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.  
Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type II.  
The 1/8" PTFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces.  
Bonding of 1/8" PTFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.  
Two 1/8" adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.  
Provide 3/8" shim plate under bearing assembly at Girder 3. Cost included with Elastomeric Bearing Assembly.  
Cost of Steel Extension is included with Elastomeric Bearing Assembly, Type II.

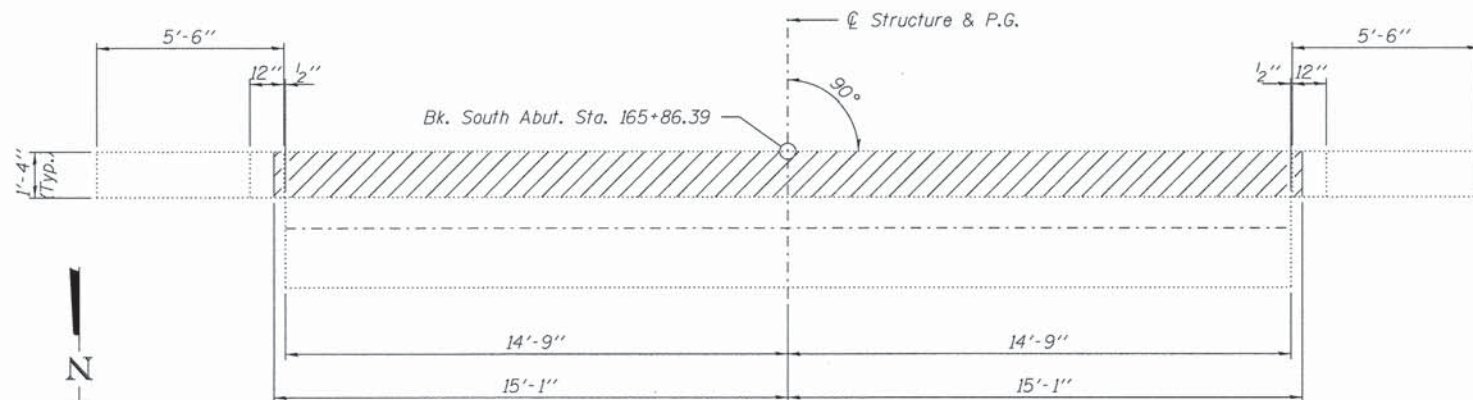


**BILL OF MATERIAL**

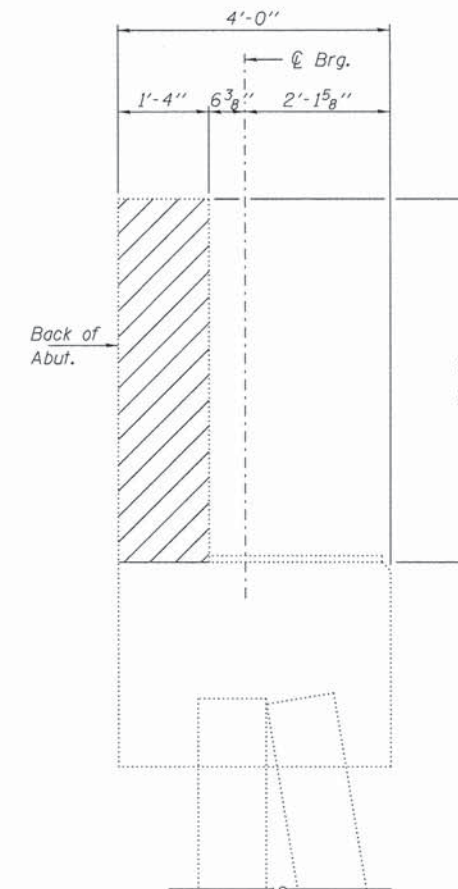
Item	Unit	Total
Elastomeric Bearing Assembly, Type II	Each	10
Anchor Bolts, 3/4"	Each	20
Jack and Remove Existing Bearings	Each	10



**ELEVATION**



**PLAN**



**SECTION A-A**

Note:  
Hatched areas indicate Concrete Removal.

**BILL OF MATERIAL**

Item	Unit	Quantity
Concrete Removal	Cu. Yd.	8.0

FILE NAME = 110323-sht-bridge.dgn  
 USER NAME =  
 DESIGNED - J.R.T.  
 CHECKED - S.M.S.  
 DRAWN - D.A.B.  
 CHECKED - M.D.C.  
 PLOT SCALE =  
 PLOT DATE = 8/18/2015

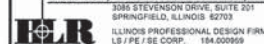
DESIGNED - J.R.T.  
 CHECKED - S.M.S.  
 DRAWN - D.A.B.  
 CHECKED - M.D.C.  
 REVISIONS:  
 REVISIONS:  
 REVISIONS:

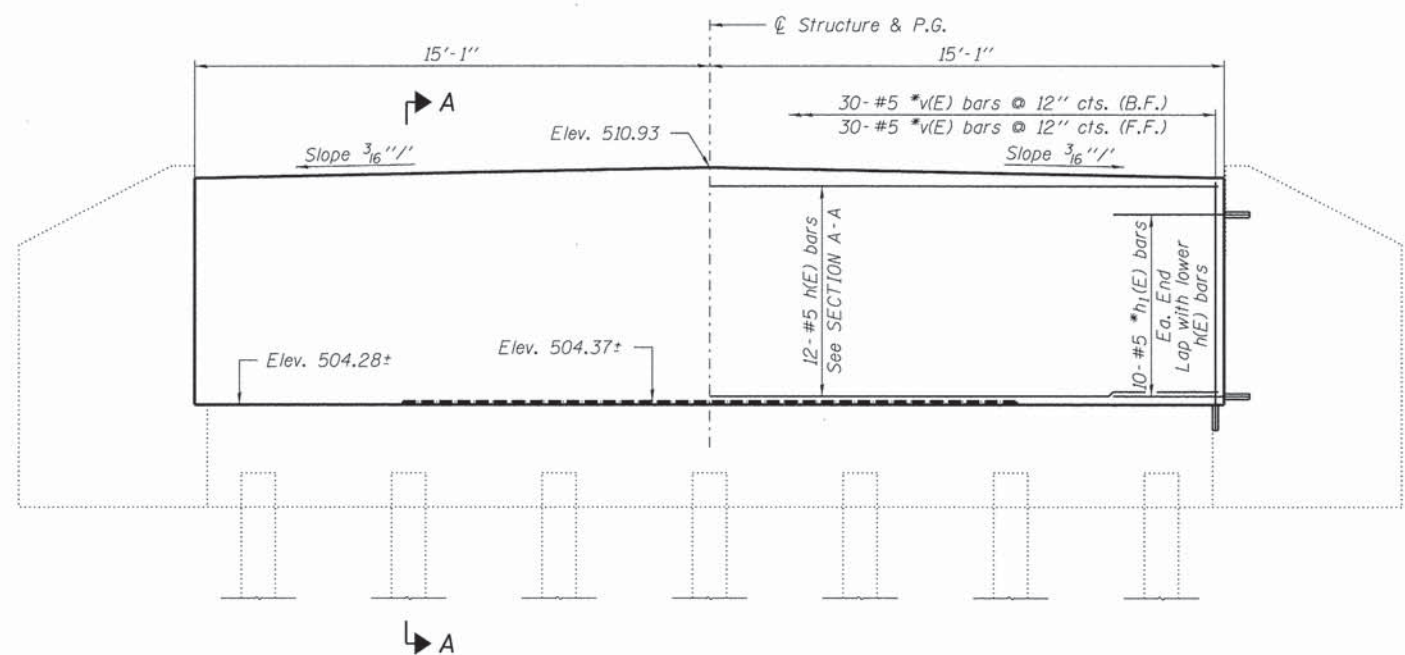
**STATE OF ILLINOIS**  
**TAZEWELL COUNTY HIGHWAY DEPARTMENT**

**SOUTH ABUTMENT REMOVAL DETAILS**  
**STRUCTURE NO. 090-3029**  
 SHEET NO. 13 OF 25 SHEETS

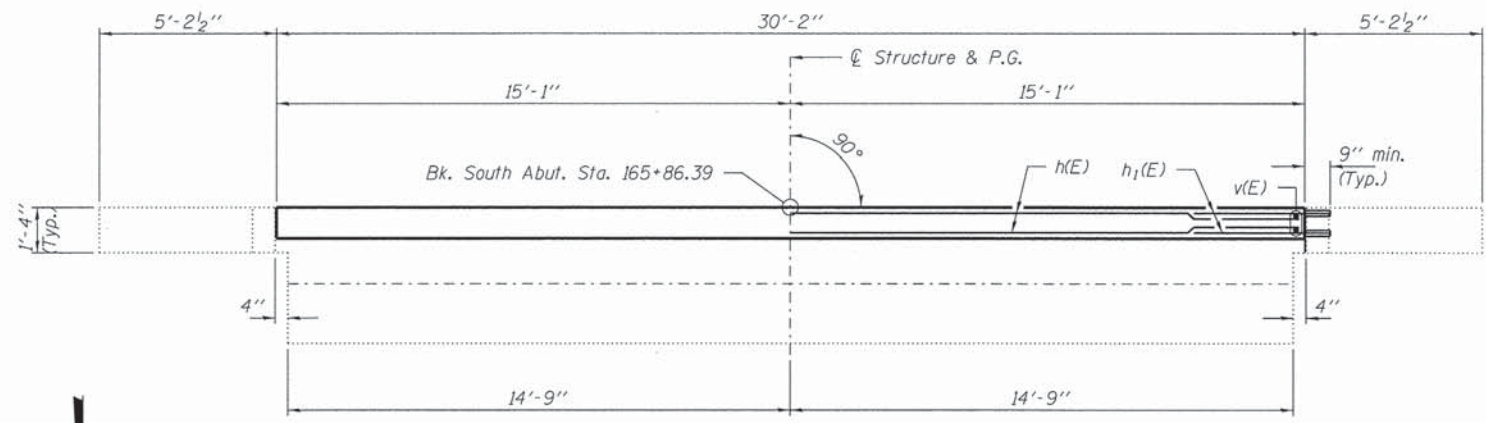
C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
11	12-00047-00-BR	TAZEWELL	39	22

CONTRACT NO. 89653  
 ILLINOIS FED. AID PROJECT

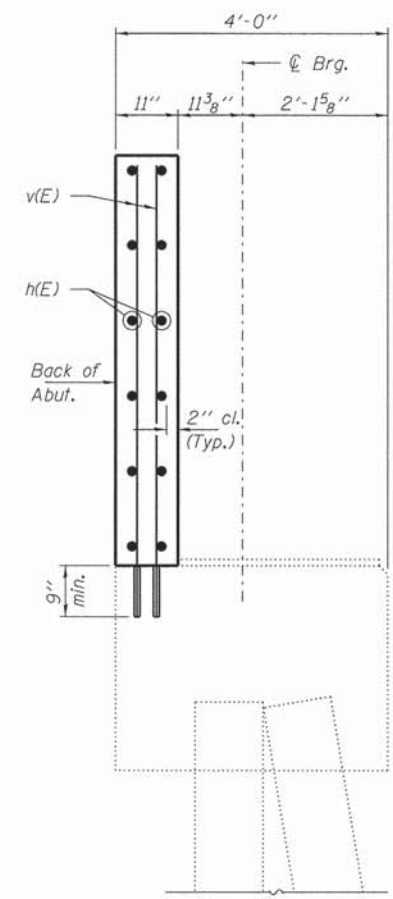




**ELEVATION**



**PLAN**



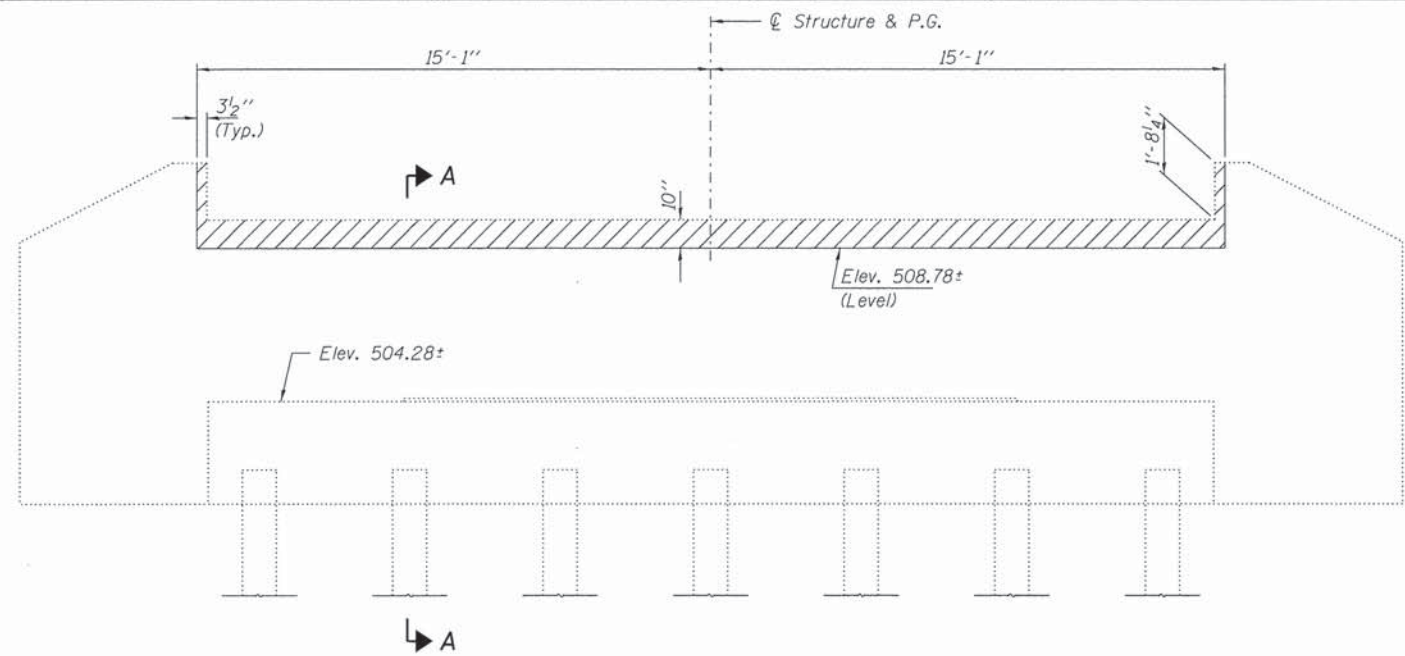
**SECTION A-A**

\*Epoxy grout v(E) and h<sub>1</sub>(E) bars in 9" min. drilled holes according to Section 584 of the Standard Specifications. Cost included with Concrete Structures.

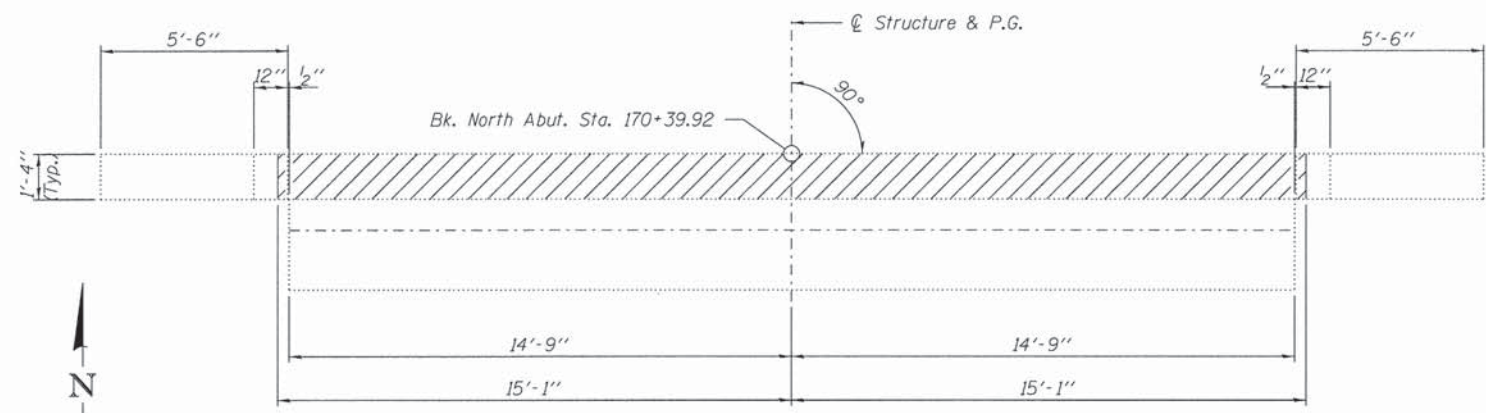
Notes:  
Existing reinforcement not used in new construction shall be cut off, ground smooth and sealed with epoxy. Cost is included in Concrete Removal.  
Concrete Sealer shall be applied to the bearing seat surface and inside face of the back of the abutment.

**BILL OF MATERIAL  
SOUTH ABUTMENT**

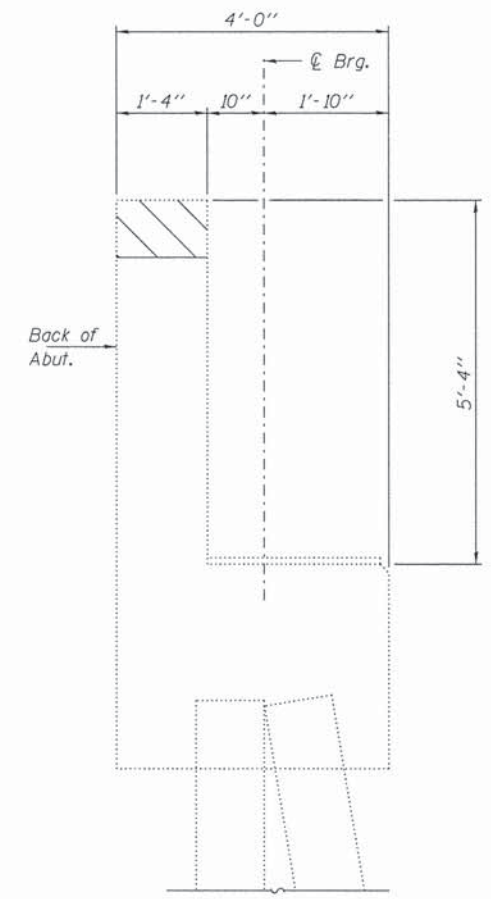
Bar	No.	Size	Length	Shape
h(E)	12	#5	29'-8"	————
h <sub>1</sub> (E)	20	#5	4'-0"	————
v(E)	60	#5	7'-0"	————
Concrete Structures		Cu. Yd.	6.6	
Reinforcement Bars, Epoxy Coated		Pound	890	
Concrete Sealer		Sq. Ft.	291	



**ELEVATION**



**PLAN**



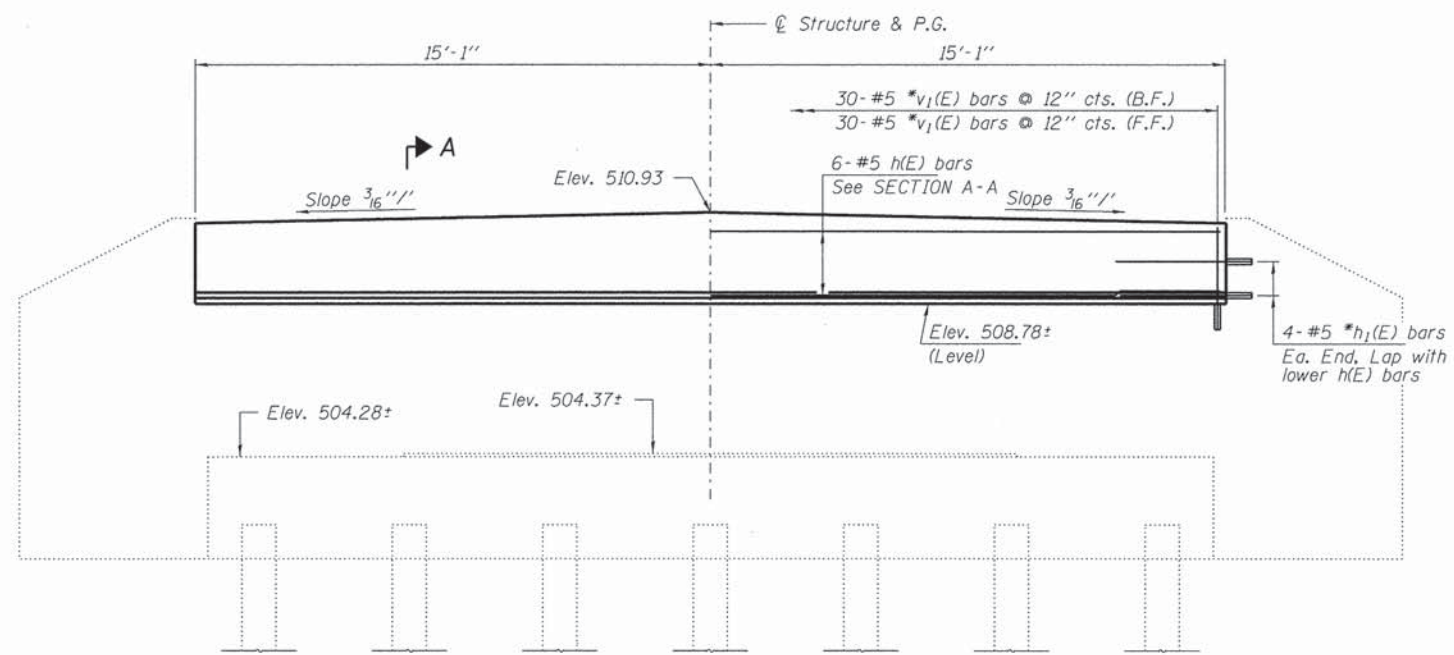
**SECTION A - A**

Note: Hatched areas indicate Concrete Removal.

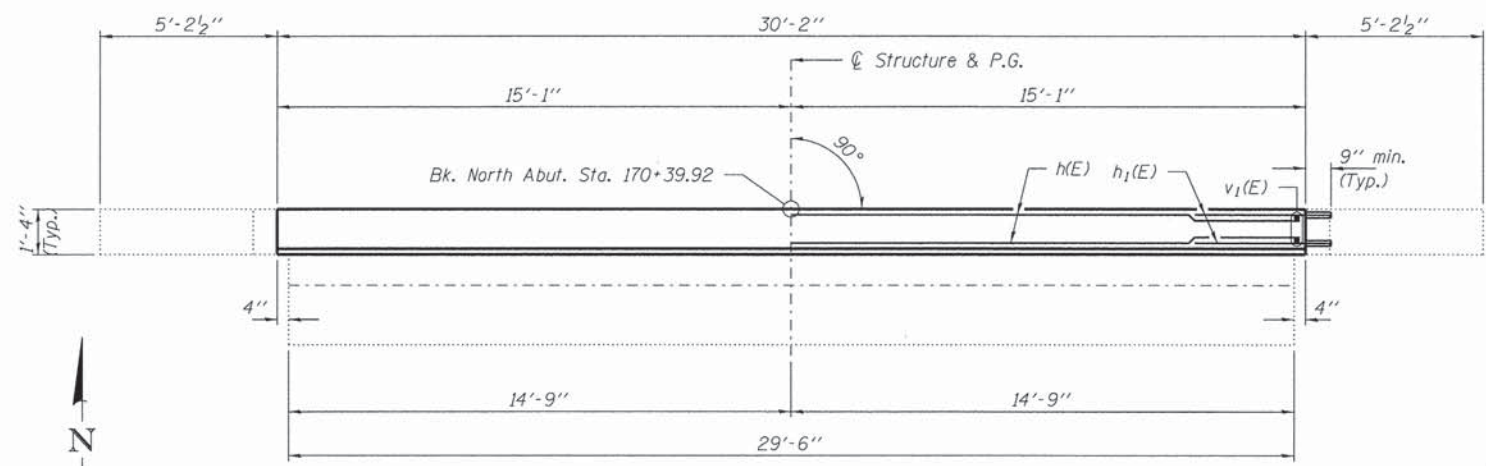
**BILL OF MATERIAL**

Item	Unit	Quantity
Concrete Removal	Cu. Yd.	1.3

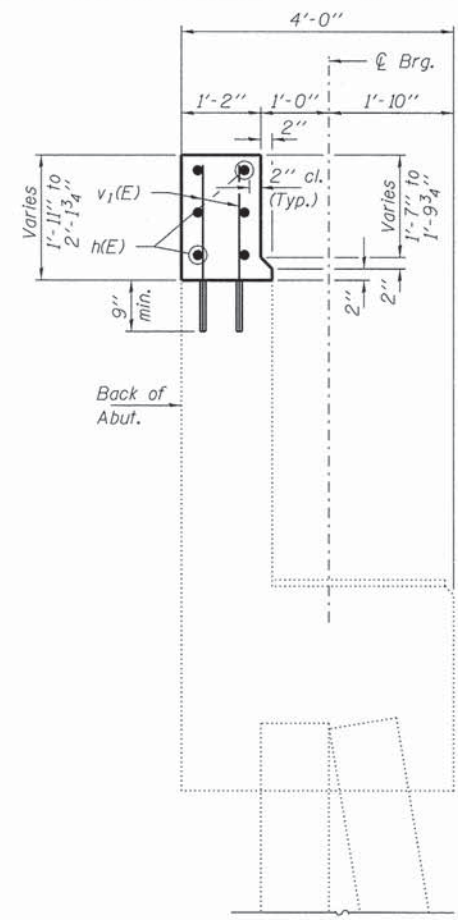




ELEVATION



PLAN



SECTION A-A

\*Epoxy grout v<sub>1</sub>(E) and h<sub>1</sub>(E) bars in 9" min. drilled holes according to Section 584 of the Standard Specifications. Cost included with Concrete Structures.

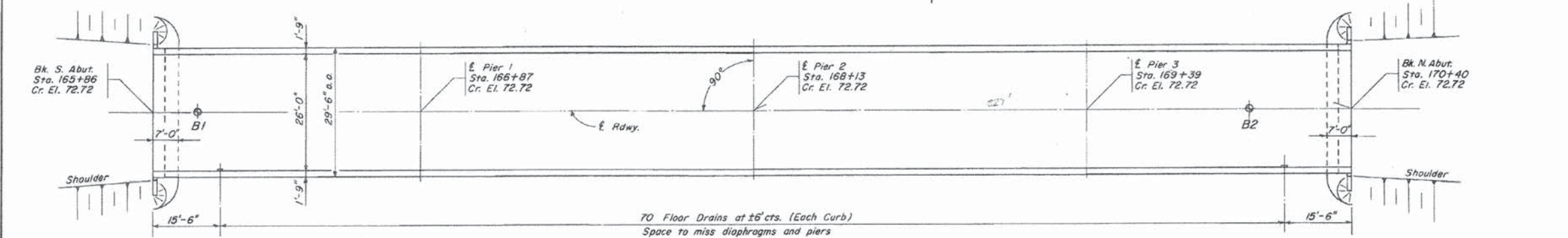
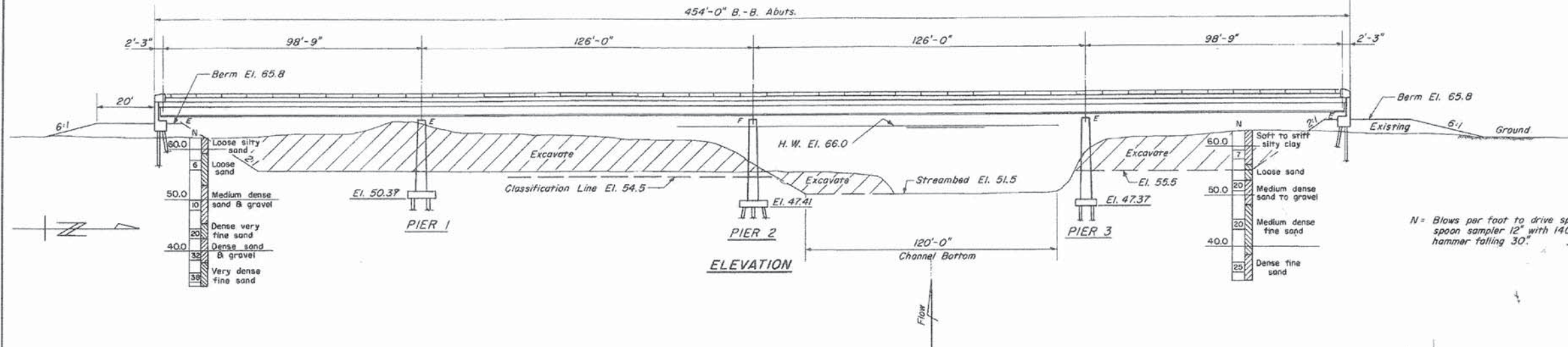
Notes:  
Existing reinforcement not used in new construction shall be cut off, ground smooth and sealed with epoxy. Cost is included in Concrete Removal.  
Concrete Sealer shall be applied to the bearing seat surface and inside face of the back of the abutment.

**BILL OF MATERIAL  
NORTH ABUTMENT**

Bar	No.	Size	Length	Shape
h(E)	6	#5	29'-8"	—
h <sub>1</sub> (E)	8	#5	4'-0"	—
v <sub>1</sub> (E)	60	#5	2'-6"	—
Concrete Structures			Cu. Yd.	2.7
Reinforcement Bars, Epoxy Coated			Pound	380
Concrete Sealer			Sq. Ft.	284

B.M. in tree Rt. Sta. 167+70  
El. 65.68.

C.H. ROUTE NO.	SEC.	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		



**GENERAL NOTES**

Class X Concrete shall be used throughout except in Piers. Class A Concrete shall be used in Piers.  
Coarse aggregate to be used in parapet handrails and end posts must be free of chert, flint, limonite, lignite and soft sandstone.  
The concrete floor slab shall be poured in one continuous operation within limits of construction joints shown and shall be finished in accordance with Article 51.19 of the Standard Specifications.  
Structural Steel shall conform to ASTM Designation A-36. Rivets  $\frac{3}{8}$ " open holes  $\frac{1}{16}$ " unless otherwise noted.  
All bearing material including lead plates and anchor bolts shall be furnished and installed in accordance with Article 51.15 of the Standard Specifications.  
Anchor bolts shall be set before riveting cross frames over supports.  
All structural steel shall receive one shop coat of red lead paint and two field coats of aluminum paint. See Art. 56.1 to 56.5 inclusive of the Standard Specifications.  
Shop inspection of structural steel by Illinois Division of Highways before painting.  
Permanent forms will not be permitted in forming the concrete floor.  
The Contractor shall drive one (1) test pile each of the type specified in permanent location at each Abutment and Pier 2, as directed by the Engineer before ordering the remainder of the piles.  
Embankment at the abutments shall be constructed to the lines shown before driving abutment piles.  
The Contractor shall excavate the channel to the lines shown within R.O.W. limits.

MACKINAW RIVER  
BUILT 1967 BY  
TAZEWELL COUNTY  
SEC. 47-1B M.F.T.  
LOADING HS-15

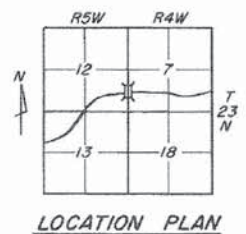
**NAME PLATE**  
See Standard 2113-1

**WATERWAY INFORMATION**  
Drainage Area 697,000 Acres  
Character Rolling, Wooded, Cultivated  
Required Opening (20 Year) 4800 S.F.  
Opening Provided 4800 S.F.

**DESIGN STRESSES**  
 $f_c = 1,400$  psi.  
 $f_s = 20,000$  psi. (Reinf.)  
 $f_s = 20,000$  psi. (Struct.)  
 $v = 75$  psi. (Ftg.)  
 $n = 10$   
Allowable L.L. deflection  $\frac{L}{1000}$

**TOTAL BILL OF MATERIAL**

Item	Super.	Substr.	Total
Class X Concrete	C.Y. 404.3	51.6	455.9
Class A Concrete	C.Y.	200.1	200.1
Structural Steel	Lbs. 466,590		466,590
Reinforcement Bars	Lbs. 86,400	12,940	99,340
Aluminum Handrail	L.F. 892		892
Protective Coat	S.Y. 1685		1685
Untreated Piles	L.F.	820	820
Concrete Piles	L.F.	360	360
Test Piles, Timber	Each	1	1
Test Piles, Concrete	Each	2	2
Channel Excavation	C.Y.		8190
Class A Excavation for Structures	C.Y.	44	44
Class B Excavation for Structures	C.Y.	266	266
Name Plates	Each	1	1



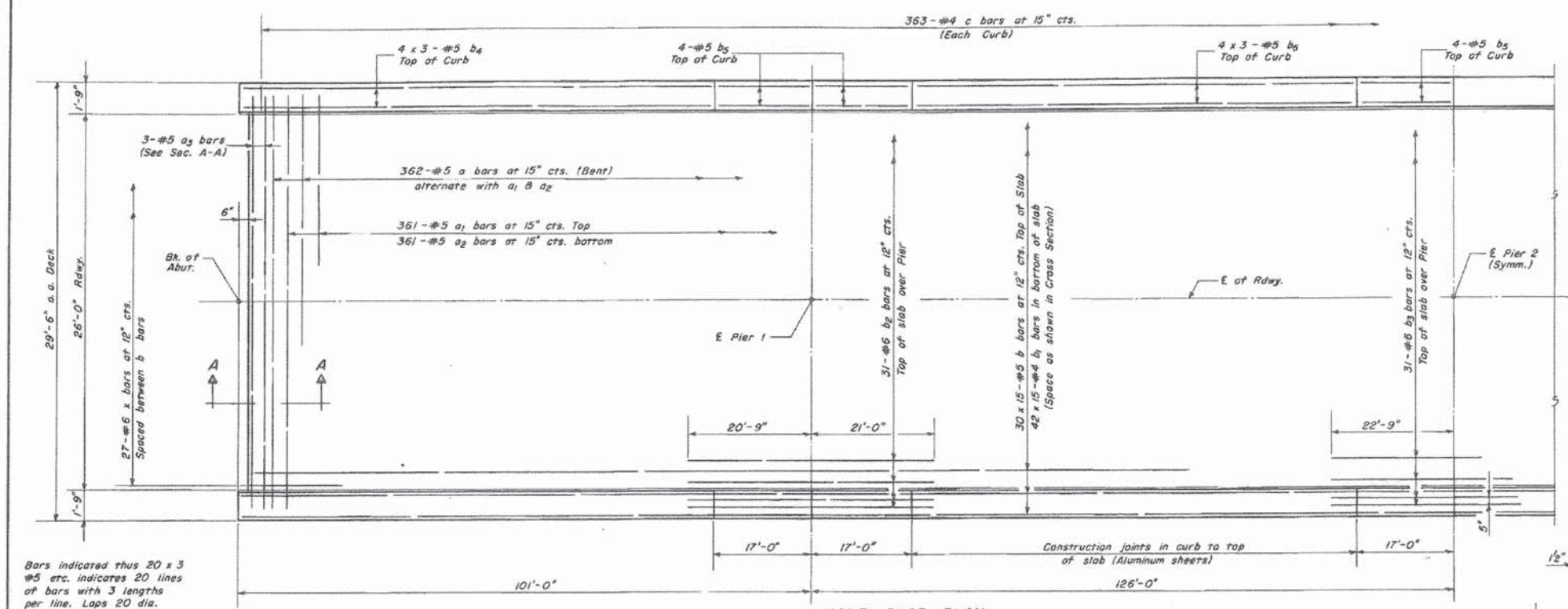
LOCATION PLAN



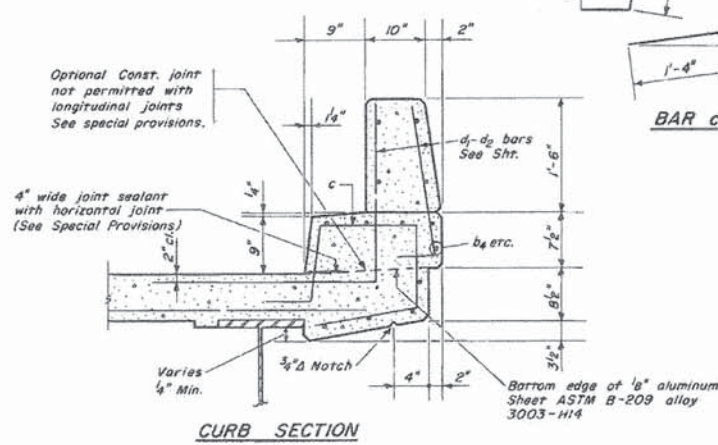
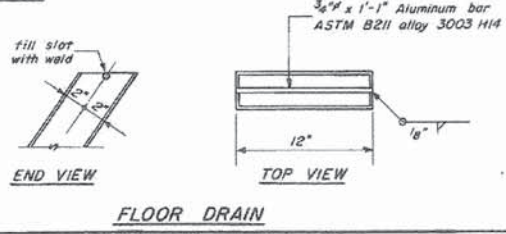
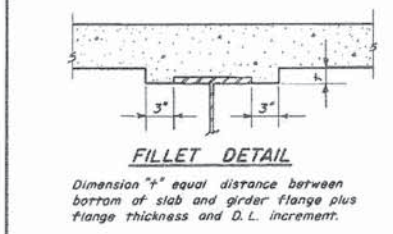
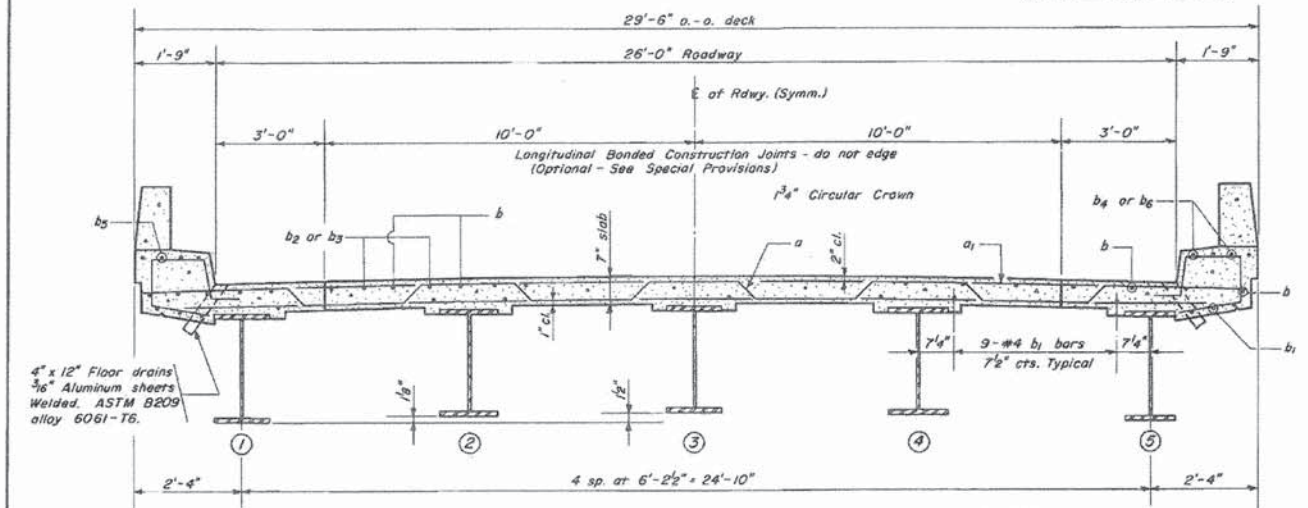
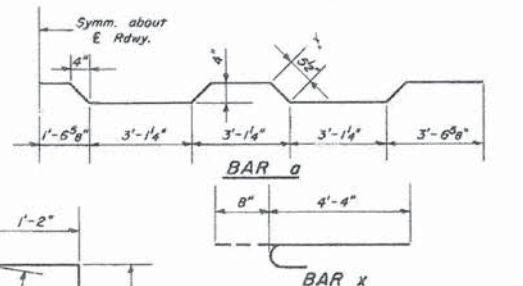
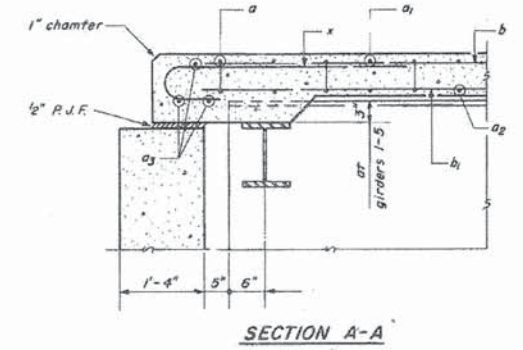
**MACKINAW RIVER BRIDGE**  
C.H. #11 SEC. 47-1B M.F.T.  
TAZEWELL COUNTY  
STA. 168+13

LOADING HS15-44

C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		
FED. ROAD DIST. NO. 7 ILLINOIS PROJECT				



Bars indicated thus 20 x 3 #5 etc. indicates 20 lines of bars with 3 lengths per line. Laps 20 dia.

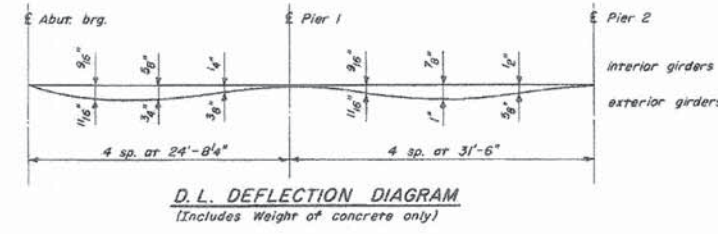


**BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a	362	#5	29'-10"	W
a1	361	5	28'-10"	—
a2	361	5	27'-5"	—
a3	6	5	26'-0"	—
b	450	5	31'-2"	—
b1	630	4	31'-0"	—
b2	62	6	41'-9"	—
b3	31	6	45'-6"	—
b4	60	5	28'-7"	—
b5	60	5	16'-8"	—
b6	60	5	31'-5"	—
c	726	4	4'-10"	—
x	54	6	5'-0"	—

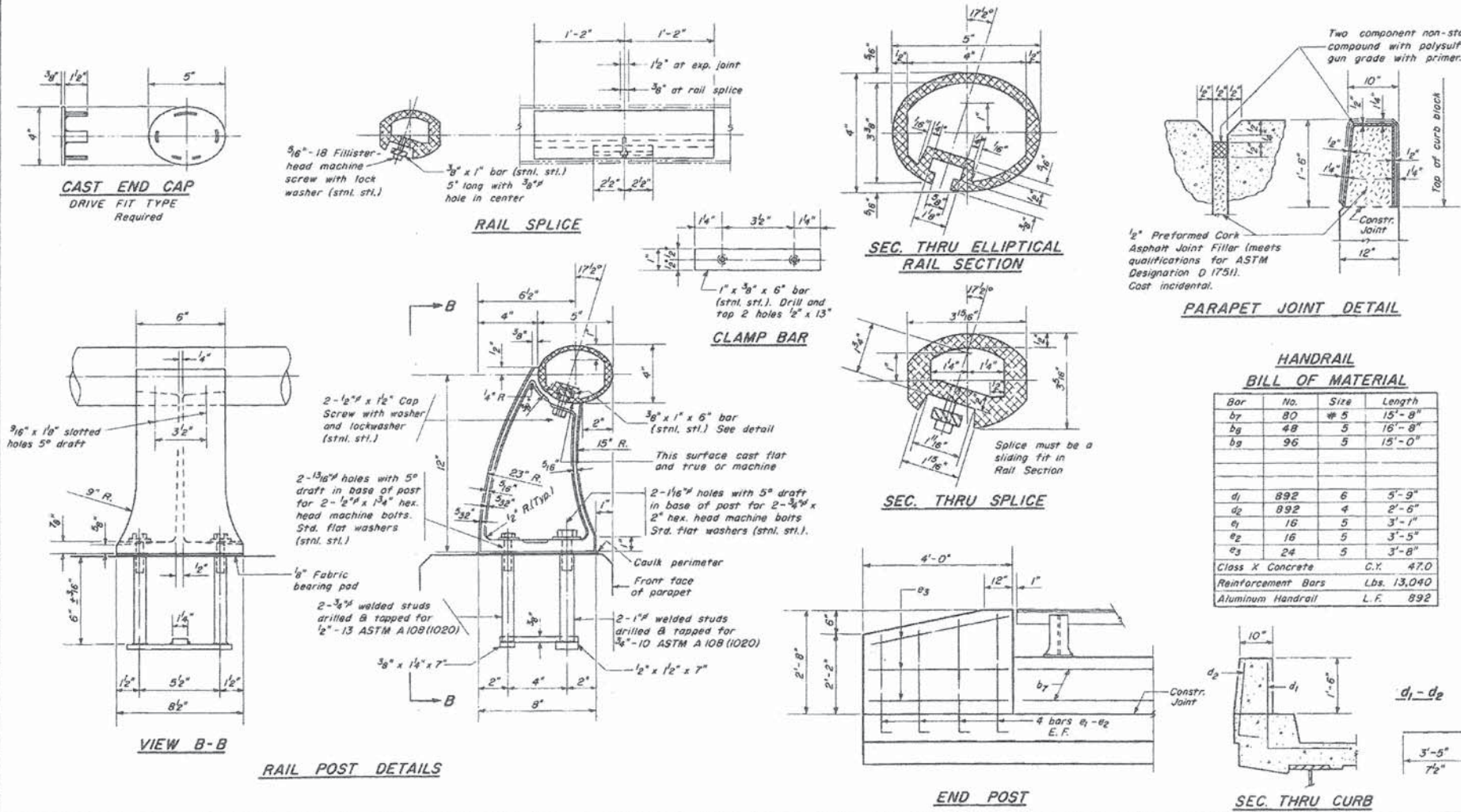
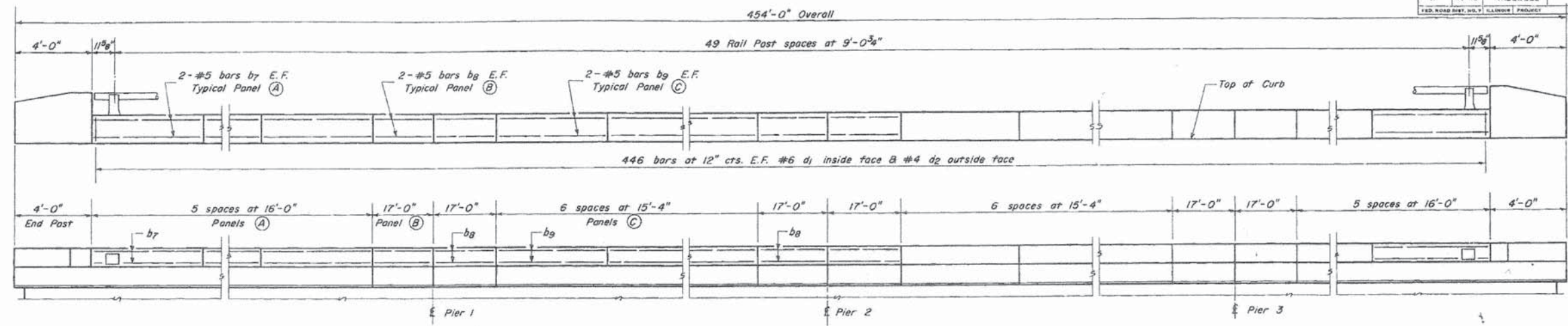
Class X Concrete C.Y. 357.2  
 Reinforcement Bars Lbs. 73,670  
 Structural Steel Lbs. 466,590

The length and quantities of longitudinal reinforcement and Class X Concrete in parapets are not included in super quantities. See Sheet



**MACKINAW RIVER BRIDGE**  
 C.H. #11 SEC. 47-1B M.F.T.  
 TAZEWELL COUNTY  
 STA. 168+13

C.H. DISTRICT NO.	REC.	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		



**HANDRAIL BILL OF MATERIAL**

Bar	No.	Size	Length
b7	80	#5	15'-8"
b8	48	#5	16'-8"
b9	96	#5	15'-0"
d1	892	#6	5'-9"
d2	892	#4	2'-6"
e1	16	#5	3'-1"
e2	16	#5	3'-5"
e3	24	#5	3'-8"
Class X Concrete	C.K.		47.0
Reinforcement Bars	Lbs.		13,040
Aluminum Handrail	L.F.		892

**NOTES**

All Posts shall be normal to parapet.

All Aluminum Alloy Extruded Rail shall conform to ASTM Specification B-221 alloy 6061-T6, and shall extend a minimum of 2 panel lengths (attached to minimum of 3 posts) except at ends or at open joints where a minimum of 1 panel length is required. All joints in railing must be spliced per detail.

See Special Provisions for following Material Specifications:  
 Cast Aluminum Alloy Bridge Post - Alloy A344-T4  
 Stainless Steel Bars, Cap Screws, Washers, Lockwashers and Fabric Bearing Pads.

**METHOD of MEASUREMENT** Aluminum handrail shall be measured in lineal feet. The length paid for shall be the over all length along the top longitudinal railing member thru all posts and gaps.

**BASIS of PAYMENT** Aluminum handrail shall be paid for at the contract unit price per lineal foot for ALUMINUM HANDRAIL, measured as specified, which price shall be payment in full for all materials, fabrication, transportation, and erection.

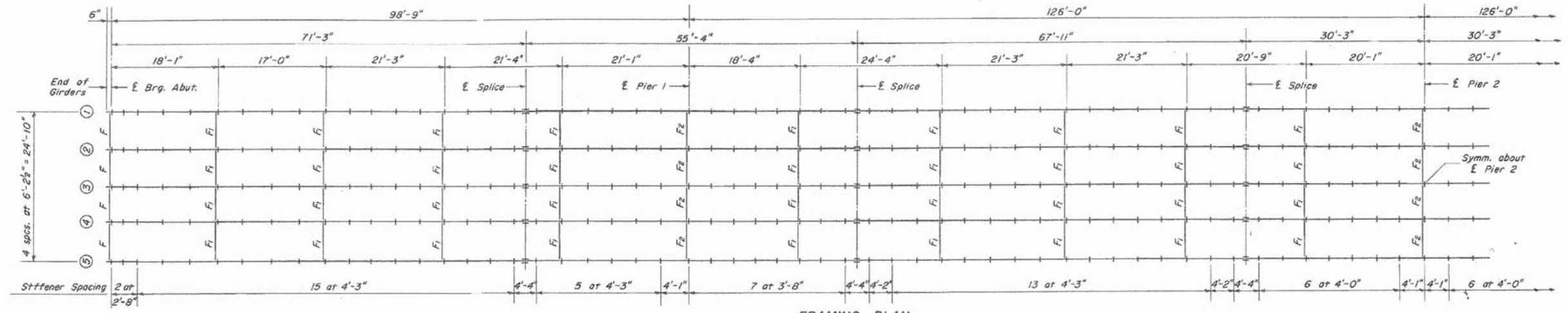
Cost of rail splice, and caps, and hardware to be incidental to item ALUMINUM HANDRAIL.

Seal perimeter of base of post to parapet with two component non-staining gray sealing compound with polysulfide liquid polymers gun grade with primer.

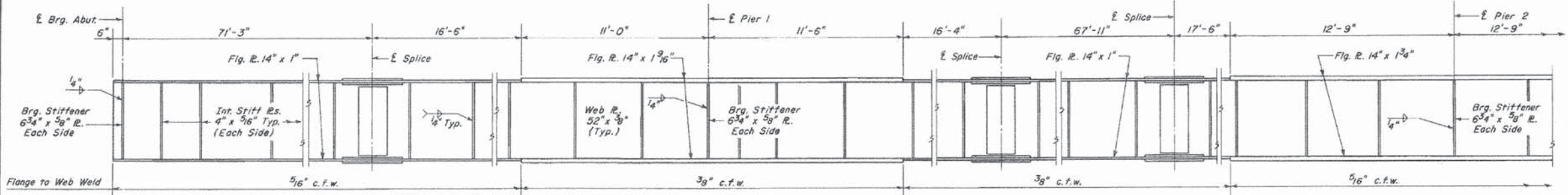
Provide 1-1/8" and 2-1/8" Aluminum shims for 25% of the Posts. Rail element shall be parallel to grade - high spots shall be ground and low spots shimmed.

**MACKINAW RIVER BRIDGE**  
 C.H. #11 SEC. 47-1B M.F.T.  
 TAZEWELL COUNTY  
 STA. 168+13

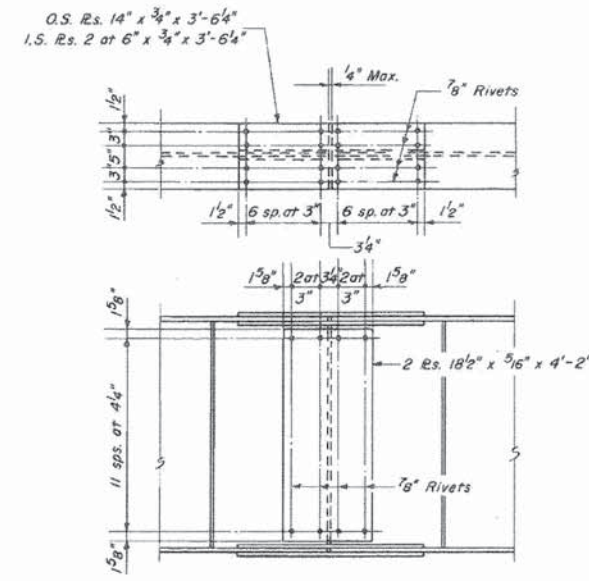
C.H. DIST. NO.	SEC.	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		



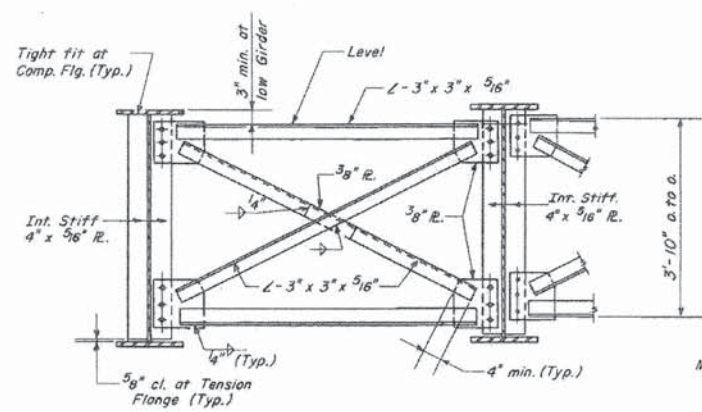
FRAMING PLAN



HALF GIRDER ELEVATION

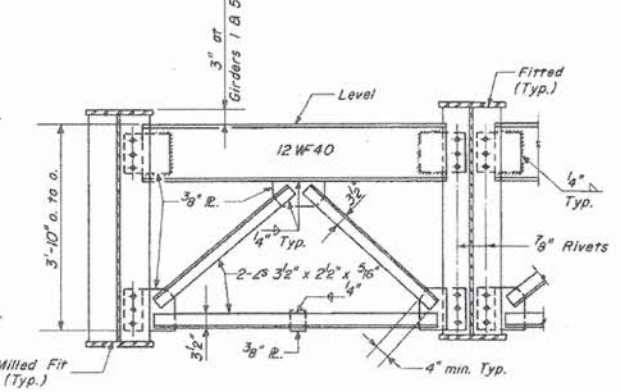


FIELD SPLICE DETAIL

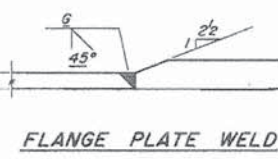


CROSS FRAME F1

72 Required  
12-F2 Required  
Cross Frame F2 similar to F1  
Stiffeners at Piers shall be similar to stiffeners shown in Cross Frame F detail.



CROSS FRAME F

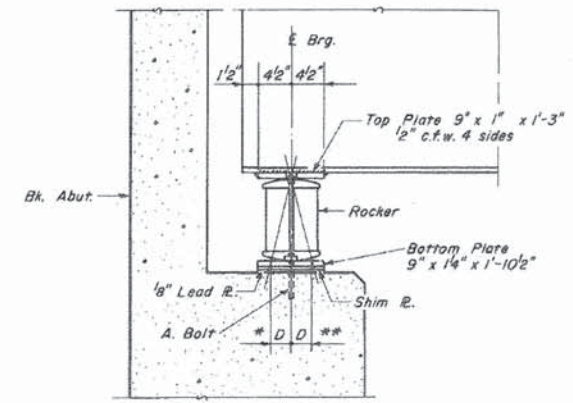


FLANGE PLATE WELD

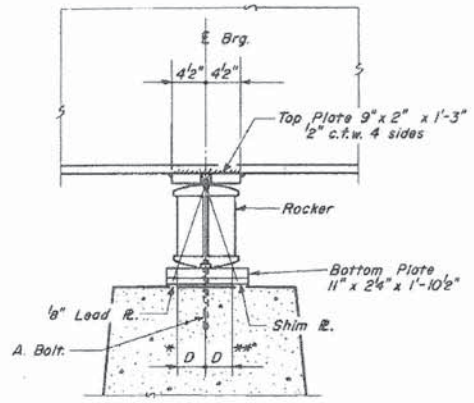
MACKINAW RIVER BRIDGE  
C.H. #11 SEC. 47-1B M.F.T.  
TAZEWELL COUNTY  
STA. 168+13

FILE NAME = 110323-ehc-brIDGE.dgn	USER NAME =	DESIGNED - J.R.T.	REVISED -	STATE OF ILLINOIS TAZEWELL COUNTY HIGHWAY DEPARTMENT	EXISTING PLANS STRUCTURE NO. 090-3029 SHEET NO. 20 OF 25 SHEETS	C.H.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 200 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	CHECKED - S.M.S.	REVISED -			11	12-00047-00-BR	TAZEWELL	39	29
IL/RS PROFESSIONAL DESIGN FIRM 1311 PE/15E CORP. 184-000093	PLOT DATE = 8/18/2015	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 89653				
		CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT				

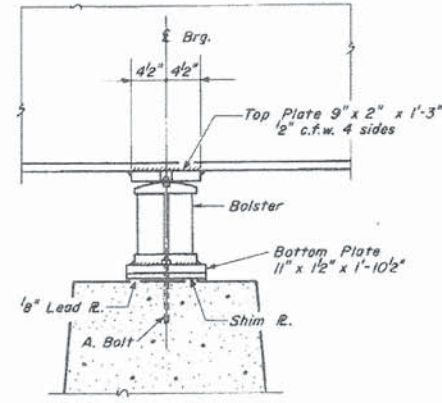
CONTRACT NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		
FED. ROAD DIST. NO. 7 CLINCH PROJECT				



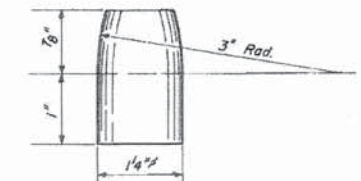
SEC. AT ABUT.



PIERS 1 & 3

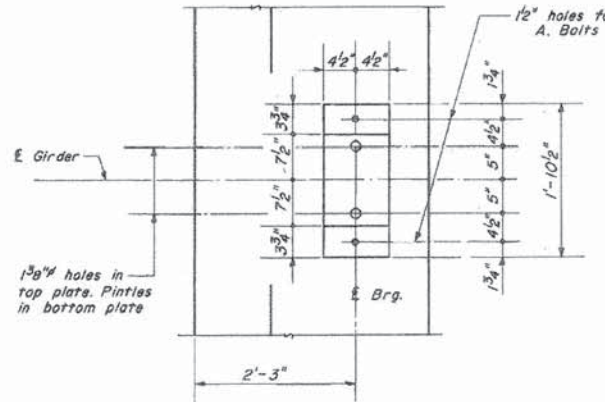


PIER 2

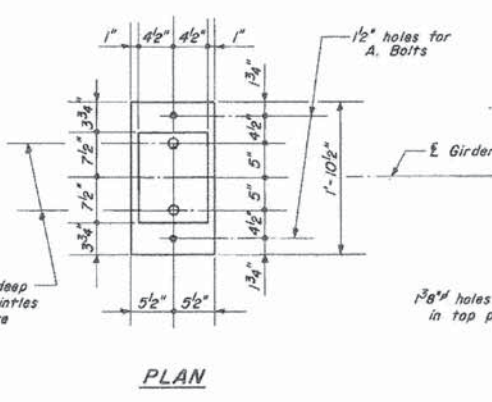


PINTLE DETAIL

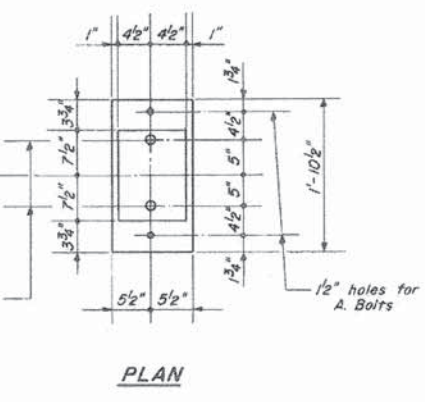
\*D = 1/8" / 100 ft. of expansion for every 15° below the normal temp. of 50° F.  
 \*\*D = 1/8" / 100 ft. of expansion for every 15° above the normal temp. of 50° F.



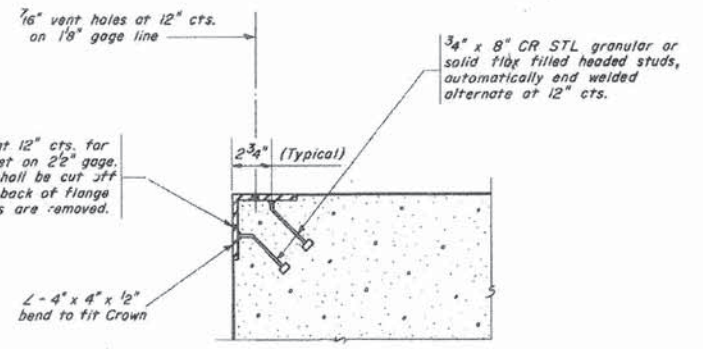
PLAN



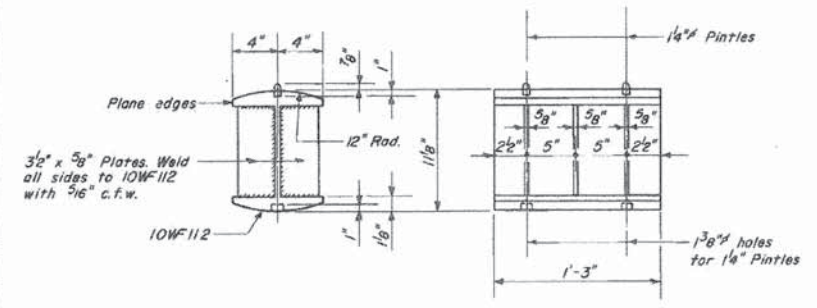
PLAN



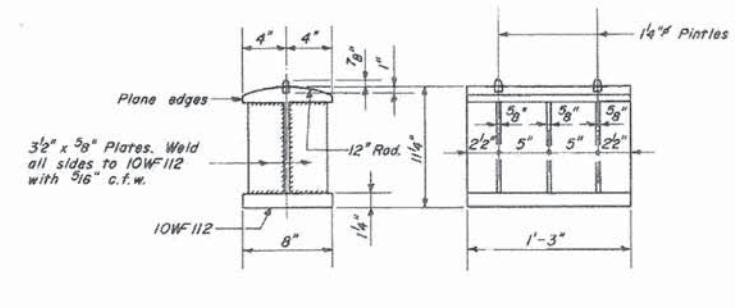
PLAN



DETAIL AT END OF SLAB



ROCKER

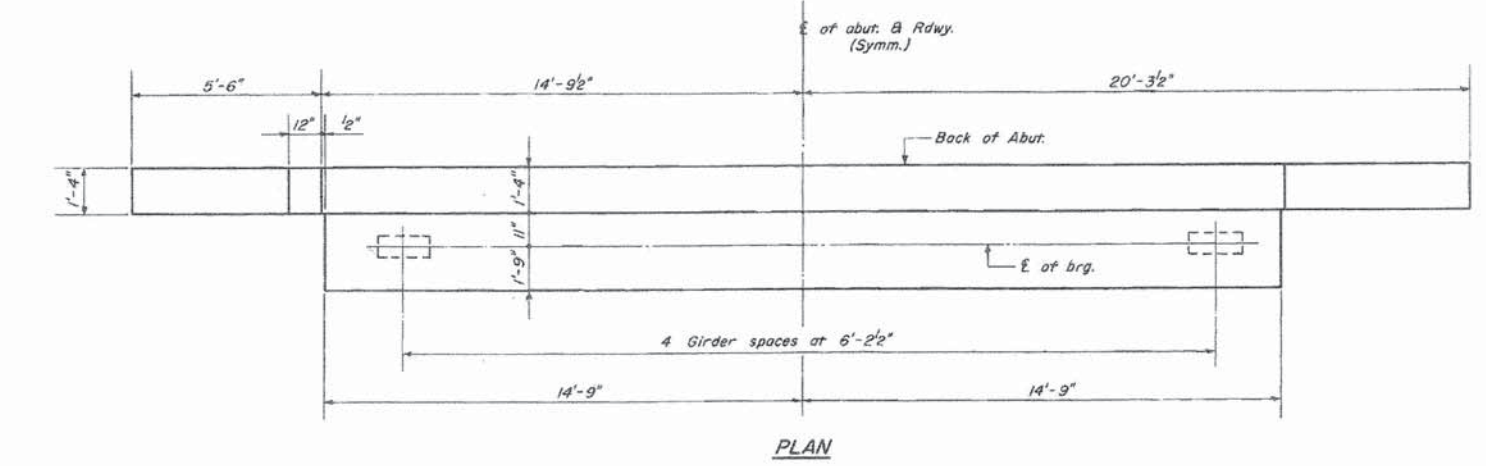


BOLSTER

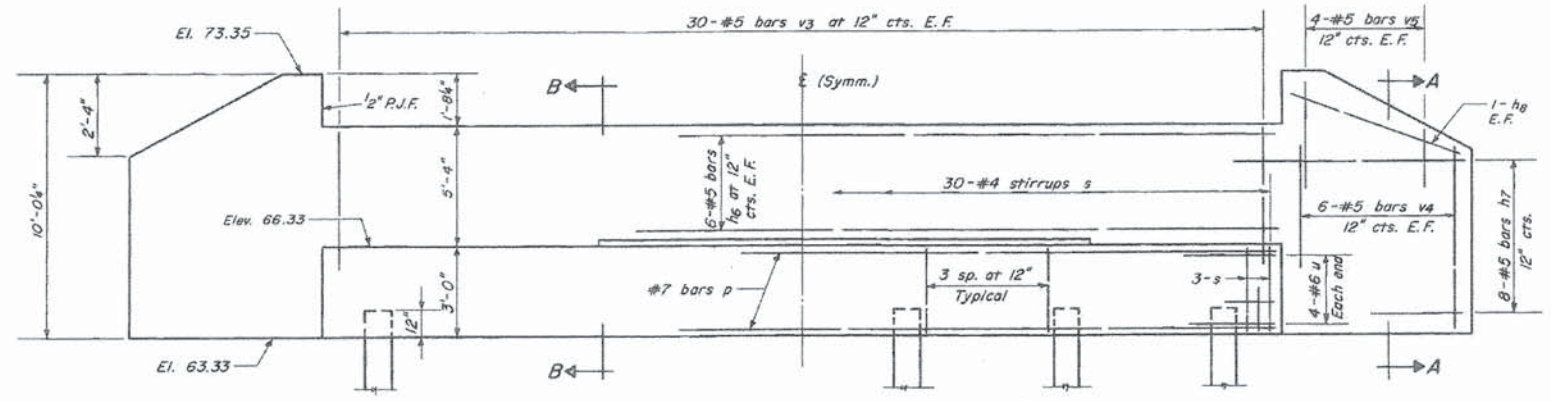
**NOTES**  
 Field splices shall be punched 1/16" and reamed to size with all Girders of a unit assembled and match marked.  
 All Anchor Bolts 1 1/2" x 12". Provide 2 1/2" x 2 1/2" x 5/16" plate washer under nut.  
 Pintles may be threaded or press fit into rockers, bolsters and bearing plates.  
 Provide 3/8" shim plate under bearing assembly at all bearings of beam 3.  
 Anchor Bolts to be grouted into drilled holes after beams are in place. At fixed pier Anchor Bolts may be built into the masonry.  
 All bearing material including Anchor Bolts and lead plates included for payment as Structural Steel. Est. Wt. 9790 Lbs.

MACKINAW RIVER BRIDGE  
 C. H. #11 SEC. 47-1B M.F.T.  
 TAZEWELL COUNTY  
 STA. 168+13

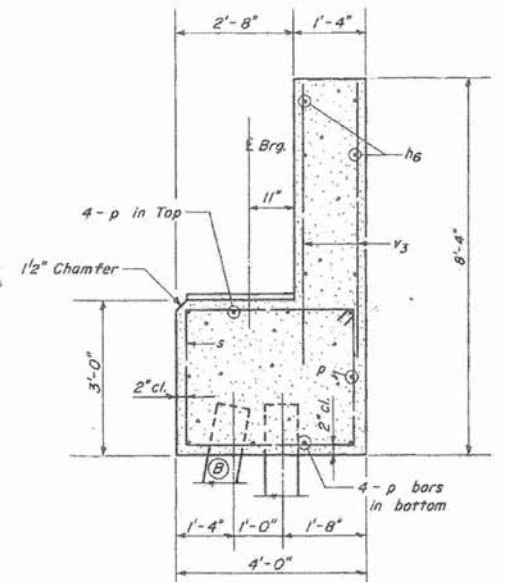
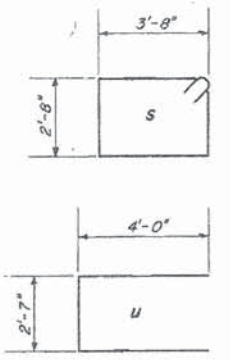
ROUTE NO.	SEC.	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		



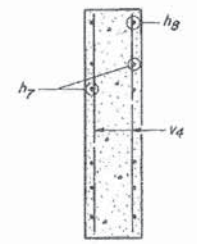
PLAN



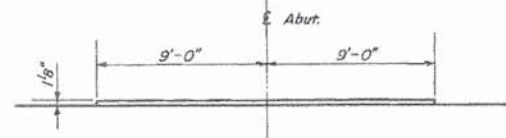
ELEVATION



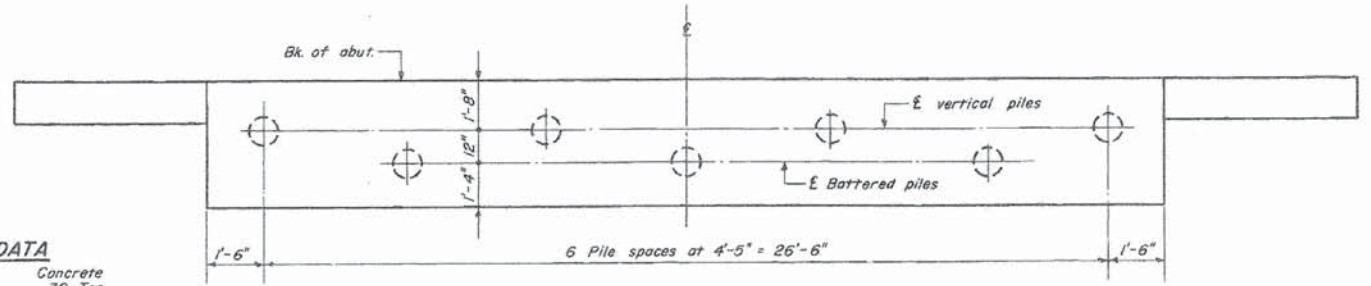
SECTION B-B



SEC. A-A



BRIDGE SEAT STEP DETAIL



PILE LAYOUT

**PILE DATA**

Type	Concrete
Capacity	30 Ton
Est. Length	30 Ft.
No. Req'd. (2 abuts.)	14
Includes 2 Test piles	

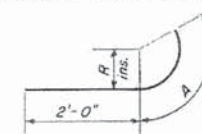
**BILL OF MATERIAL TWO ABUTMENTS**

Bar	No.	Size	Length	Shape
h6	24	#5	29'-6"	—
h7	64	5	6'-9"	—
h8	8	5	5'-6"	—
p	20	7	29'-3"	—
s	60	4	13'-5"	□
u	16	6	10'-7"	□
v3	120	5	6'-6"	—
v4	48	5	7'-6"	—
v5	32	5	3'-0"	—
Class X Concrete			C.Y.	51.6
Reinforcement Bars			Lbs.	4510
Concrete Piles			L.F.	360
Test Piles			Each	2

**NOTES**  
 Battered piles in front row 2" in 12".  
 Minimum concrete cover over reinforcement bars 1/2" except as noted.  
 Dimensions on bar bending details are out-out.

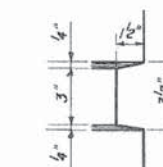
**MACKINAW RIVER BRIDGE**  
 C.H. #11 SEC. 47-1B M.F.T.  
 TAZEWELL COUNTY  
 STA. 168+13

C.H. DISTRICT NO.	SEC.	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		

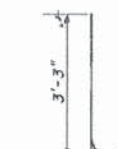


Bar	R	A
$h_1$	1'-0"	2'-0"
$h_2$	1'-2"	2'-7"
$h_3$	1'-4"	3'-0"

BARS  $h_1 - h_2 - h_3$



NOTCH DETAIL



BAR  $n$

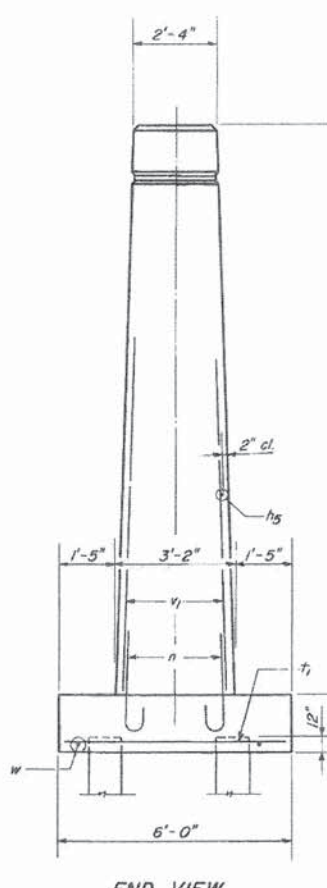
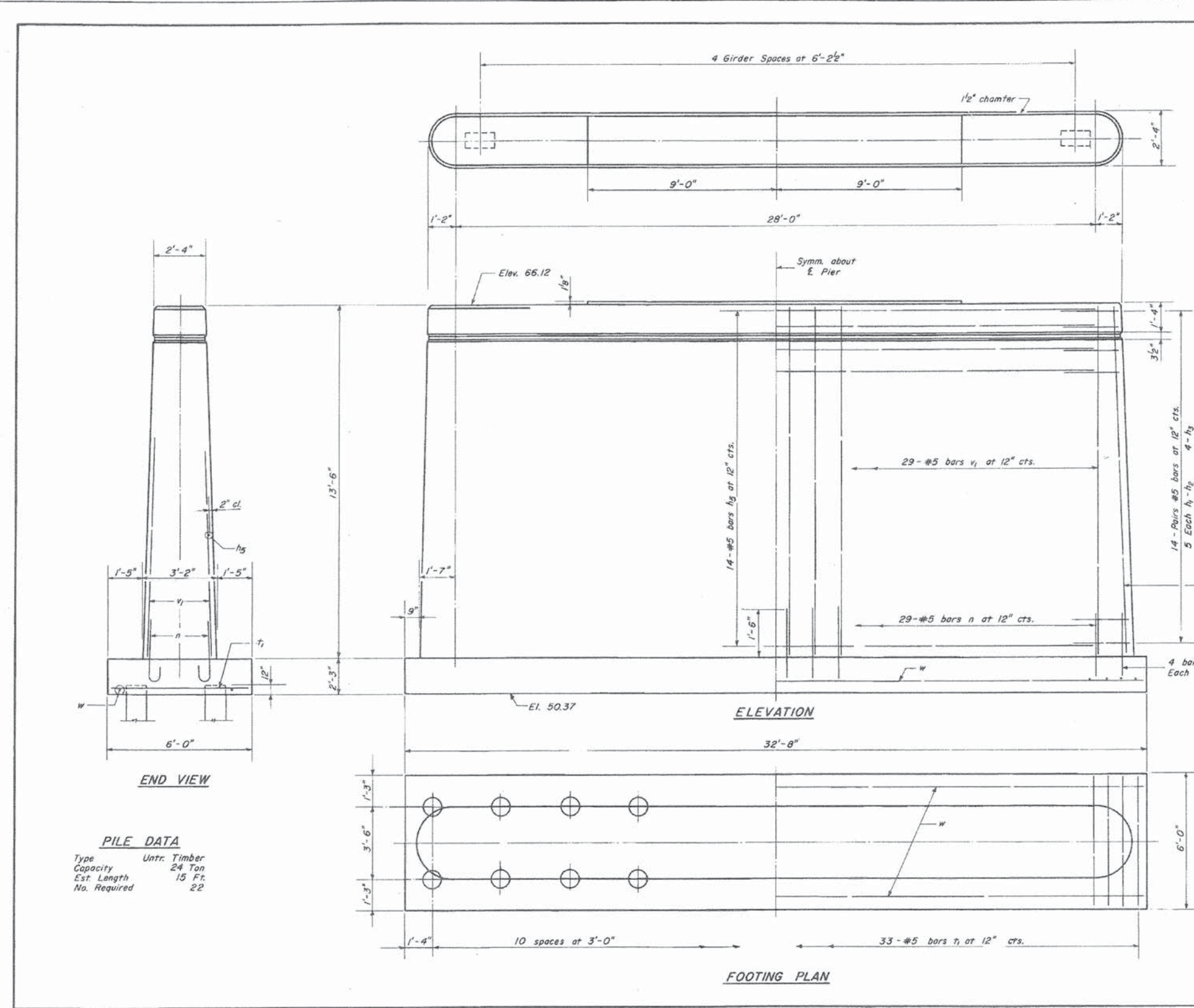
**BILL OF MATERIAL**

Bar	No	Size	Length	Shape
$h_1$	20	#5	4'-0"	U
$h_2$	20	5	4'-7"	U
$h_3$	16	5	5'-0"	U
$h_5$	28	5	28'-0"	—
$n$	66	5	3'-10"	C
$r_1$	33	5	5'-8"	—
$v_1$	66	5	13'-3"	—
$w$	4	4	17'-0"	—

Class A Concrete	C.Y.	58.0
Reinforcement Bars	Lbs.	2500
Untreated Piles	L.F.	330

PIER 1  
MACKINAW RIVER BRIDGE  
C.H. #11 SEC. 47-1B M.F.T.  
TAZEWELL COUNTY  
STA. 168+13



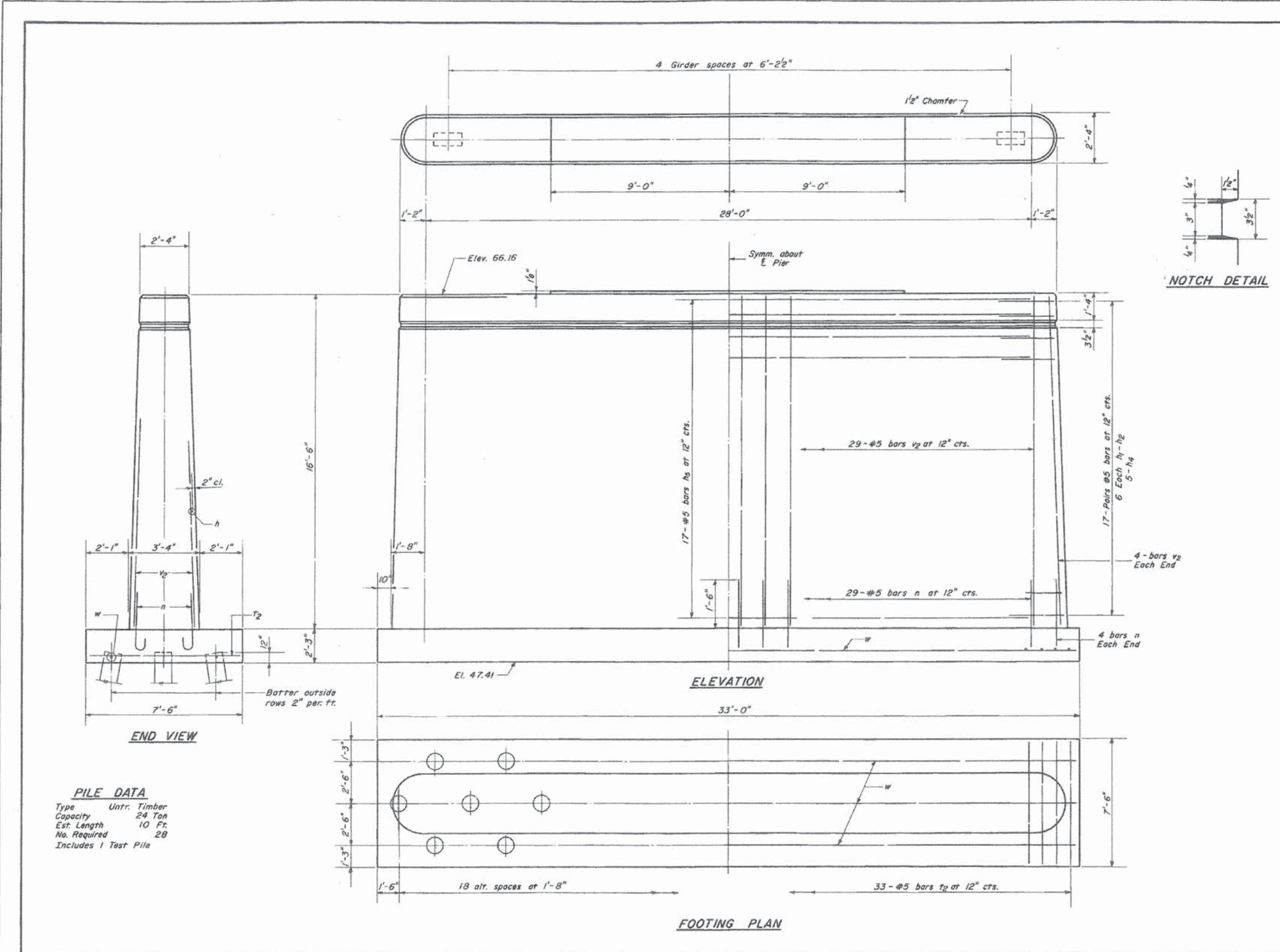
END VIEW

**PILE DATA**

Type	Untr. Timber
Capacity	24 Ton
Est. Length	15 Ft.
No. Required	22

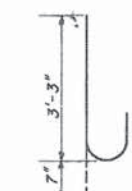


C.H. ROUTE NO.	SEC.	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		



Bar	R	A
h1	1'-0"	2'-0"
h2	1'-2 1/2"	2'-7"
h4	1'-5"	3'-2"

BARS h<sub>1</sub> - h<sub>2</sub> - h<sub>4</sub>



BAR n

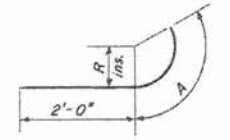
**BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h1	24	#5	4'-0"	U
h2	24	5	4'-7"	U
h4	20	5	5'-2"	U
h5	34	5	28'-0"	—
n	66	5	3'-10"	C
t2	33	5	7'-2"	—
v2	66	5	16'-3"	—
w	6	4	17'-0"	—
Class A Concrete			C.Y.	73.1
Reinforcement Bars			Lbs.	2990
Untreated Piles			L.F.	270
Test Pile			Each	1

**PILE DATA**  
 Type Untr. Timber  
 Capacity 24 Ton  
 Est. Length 10 Ft.  
 No. Required 28  
 Includes 1 Test Pile

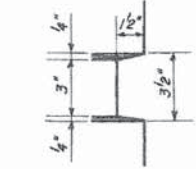
**PIER 2**  
 MACKINAW RIVER BRIDGE  
 C.H. #11 SEC. 47-1B M.F.T.  
 TAZEWELL COUNTY  
 STA. 168+13

C.H. DIST. NO.	SEC.	COUNTY	TOTAL SHEETS	SHEET NO.
11	47-1B	TAZEWELL		

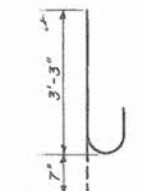


Bar	R	A
h <sub>1</sub>	1'-0"	2'-0"
h <sub>2</sub>	1'-2 1/2"	2'-7"
h <sub>4</sub>	1'-5"	3'-2"

BARS h<sub>1</sub> - h<sub>2</sub> - h<sub>4</sub>



NOTCH DETAIL



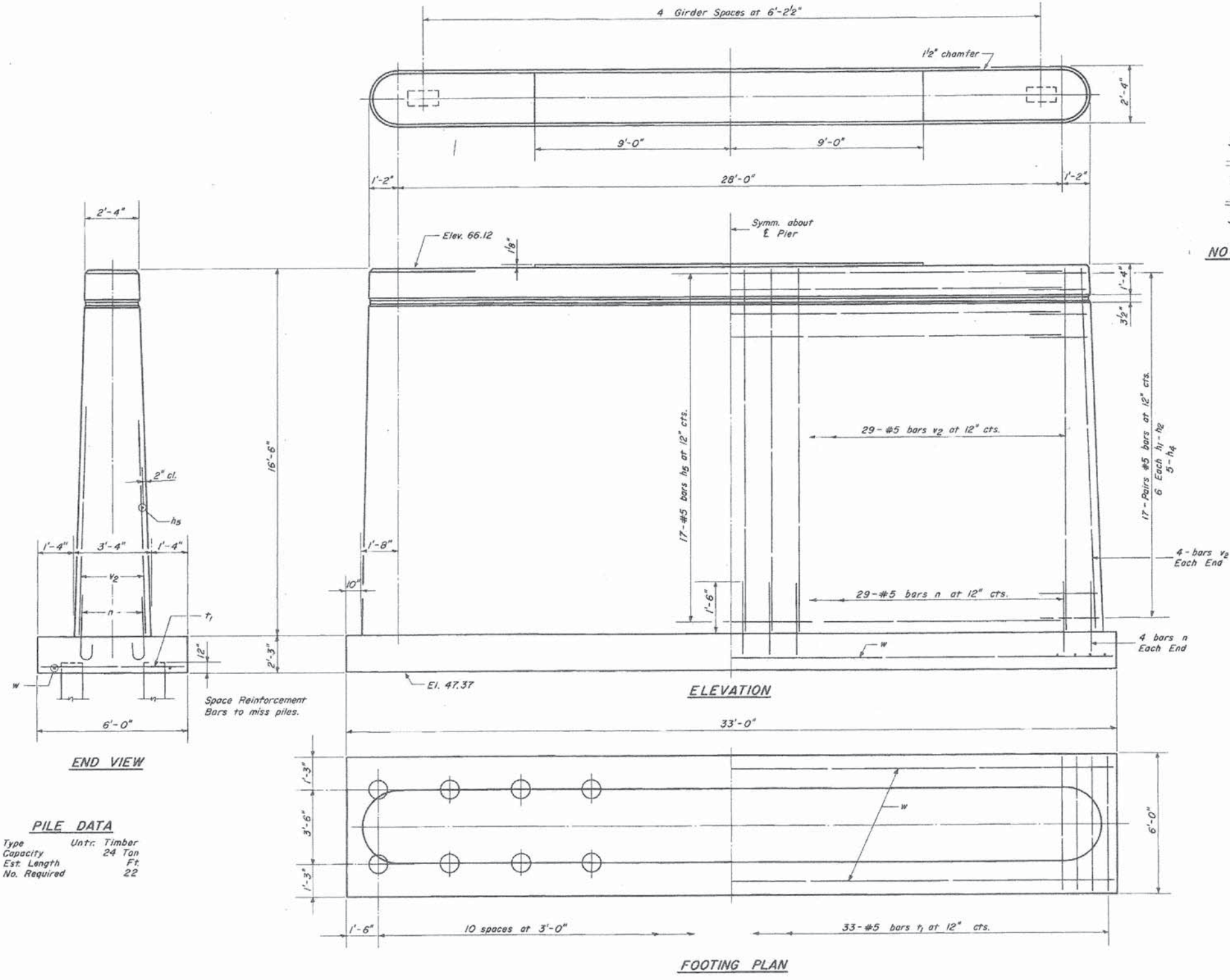
BAR n

BILL OF MATERIAL

Bar	No	Size	Length	Shape
h <sub>1</sub>	24	#5	4'-0"	U
h <sub>2</sub>	24	5	4'-7"	U
h <sub>4</sub>	20	5	5'-2"	U
h <sub>5</sub>	34	5	28'-0"	—
n	66	5	3'-10"	C
t <sub>1</sub>	33	5	5'-8"	—
v <sub>2</sub>	66	5	16'-3"	—
w	4	4	17'-0"	—

Class A Concrete C.Y. 69.0  
Reinforcement Bars Lbs. 2940  
Untreated Piles L.F. 220

PIER 3  
MACKINAW RIVER BRIDGE  
C.H. #11 SEC. 47-1B M.F.T.  
TAZEWELL COUNTY  
STA. 168+13

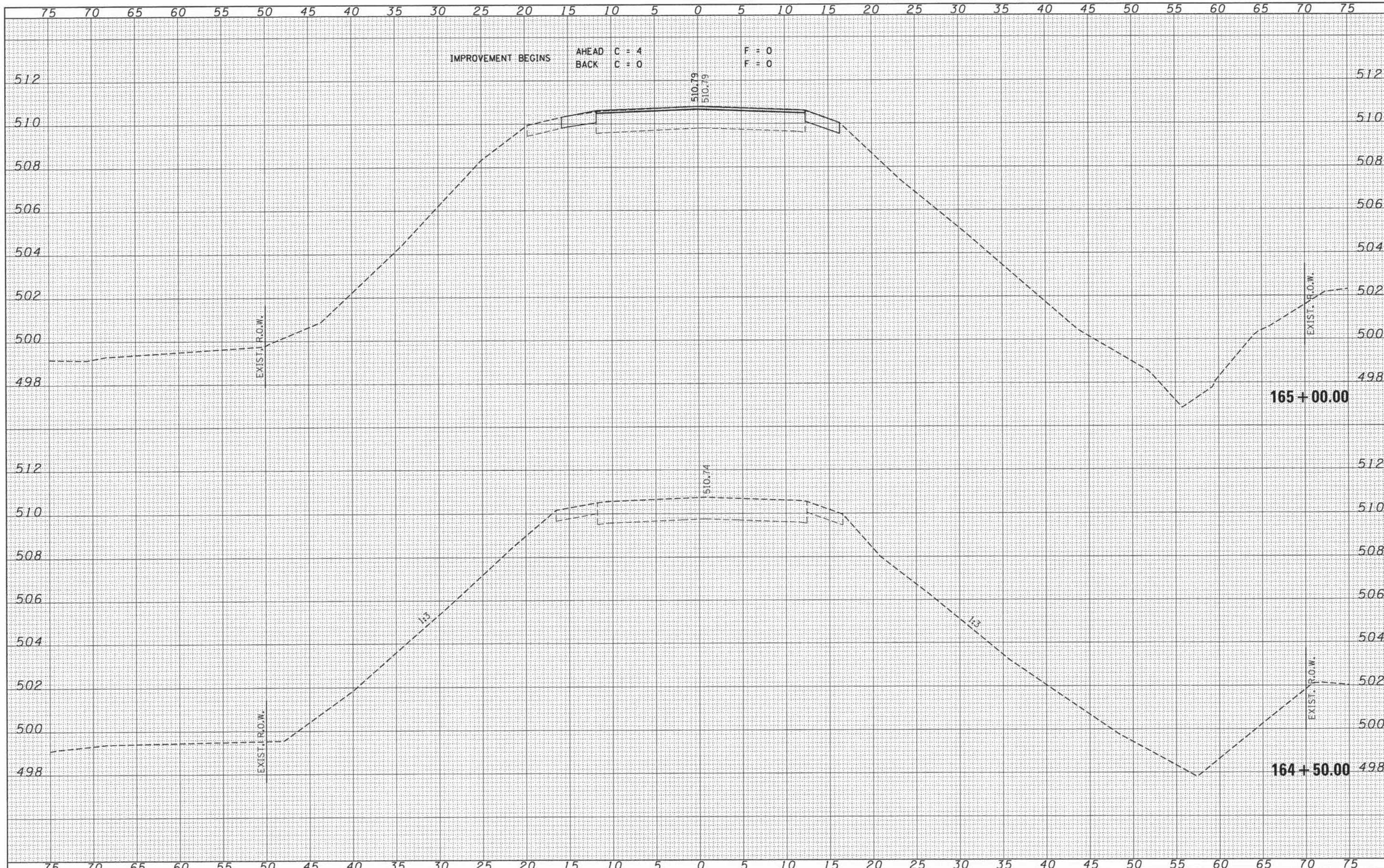


PILE DATA

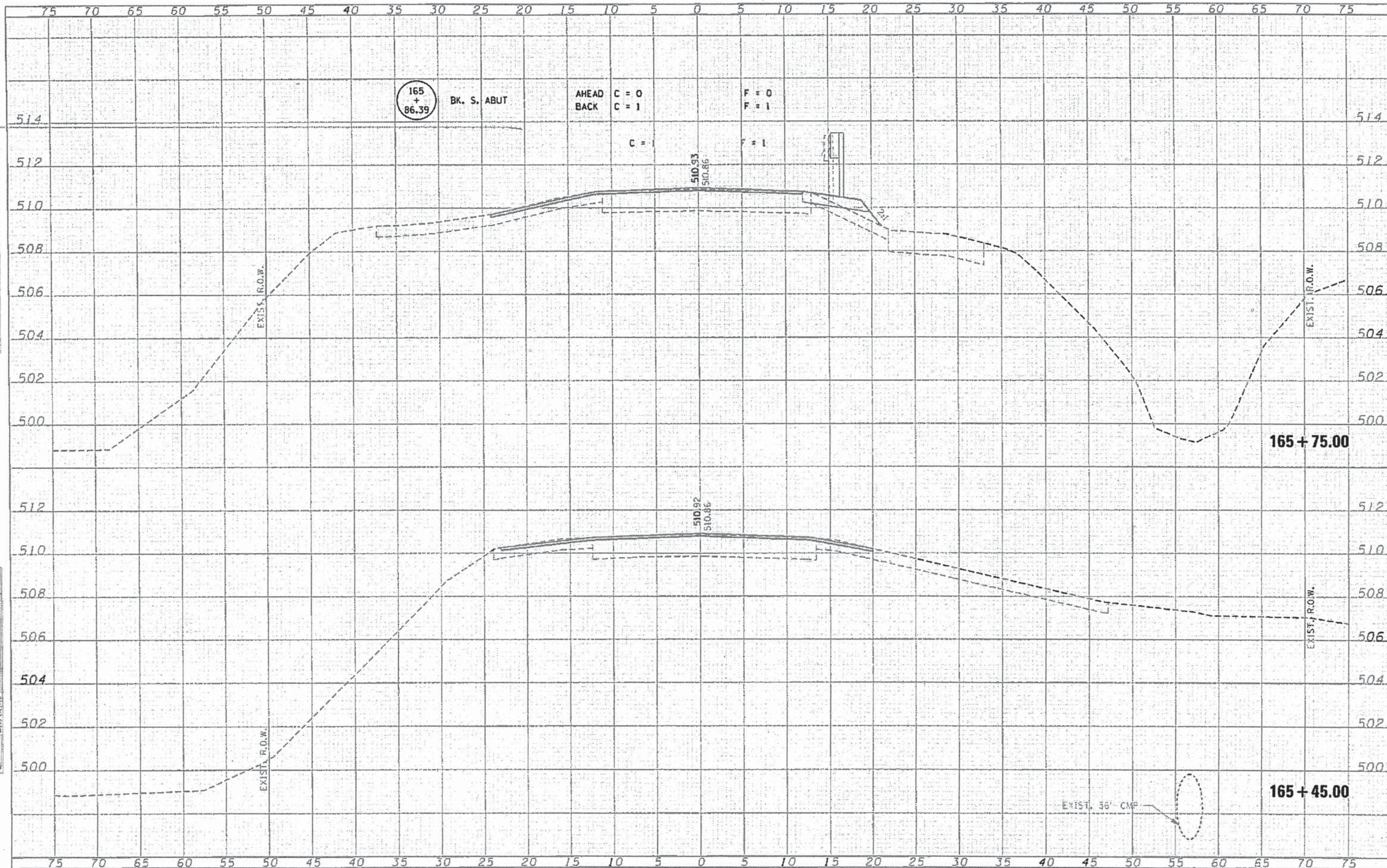
Type	Untr. Timber
Capacity	24 Ton
Est. Length	Ft.
No. Required	22

DATE	
BY	
SUBMITTED	
PLOTTED	
TEMPLATE	
NOTE BOOK	
AREAS CHECKED	

DATE	
BY	
SUBMITTED	
PLOTTED	
TEMPLATE	
NOTE BOOK	
AREAS CHECKED	



FILE NAME = 118323-shr-sss.dgn	USER NAME =	DESIGNED - J.W.F.	REVISED -	<b>STATE OF ILLINOIS</b> <b>TAZEWELL COUNTY HIGHWAY DEPARTMENT</b>	<b>CROSS SECTIONS</b> <b>C.H. 11 / TOWERLINE ROAD</b>		F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
<b>HAMPTON, LENZINI AND RENWICK, INC.</b>	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		2462	12-00047-00-BR	TAZEWELL	39	35		
200 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT DATE = 8/18/2015	CHECKED - M.D.C.	REVISED -		SCALE:		SHEET NO. OF SHEETS		CONTRACT NO. 89653		
ILLINOIS PROFESSIONAL DESIGN FIRM L.S. / P.E. / S.E. CORP. 184-000994	DATE = 08/14/15	DATE = 08/14/15	REVISED -		STA. 164+50.00 TO STA. 165+00.00		ILLINOIS FED. AID PROJECT				

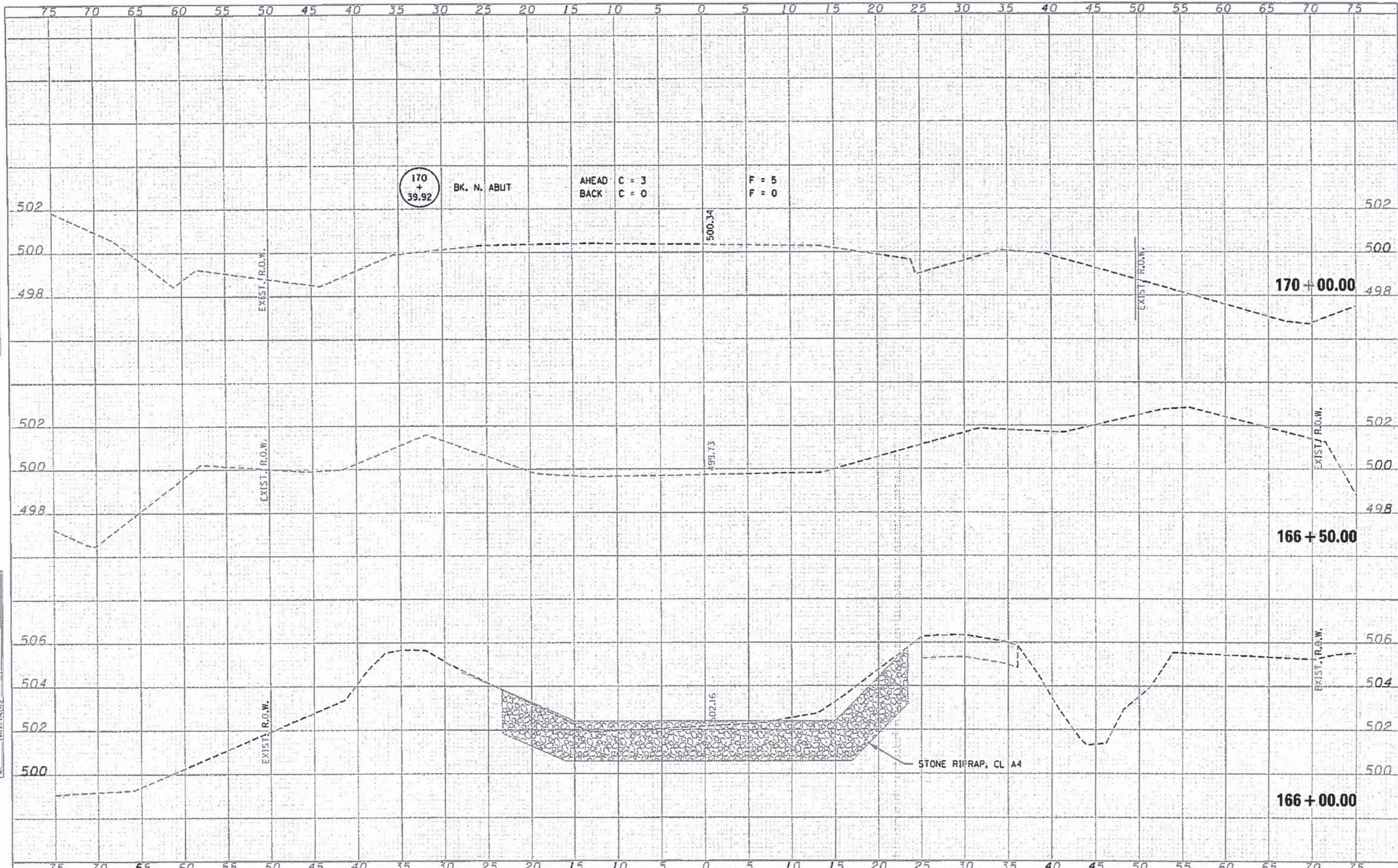


DATE	
BY	
REVISIONS	
NO.	
DATE	
BY	
REVISIONS	
NO.	
DATE	
BY	
REVISIONS	
NO.	

DATE	
BY	
REVISIONS	
NO.	
DATE	
BY	
REVISIONS	
NO.	
DATE	
BY	
REVISIONS	
NO.	

DATE	
BY	
DESIGNED	
DRAWN	
CHECKED	
DATE	

DATE	
BY	
DESIGNED	
DRAWN	
CHECKED	
DATE	



170 + 39.92

BK. N. ABUT

AHEAD C = 3  
BACK C = 0

F = 5  
F = 0

500.34

499.73

502.16

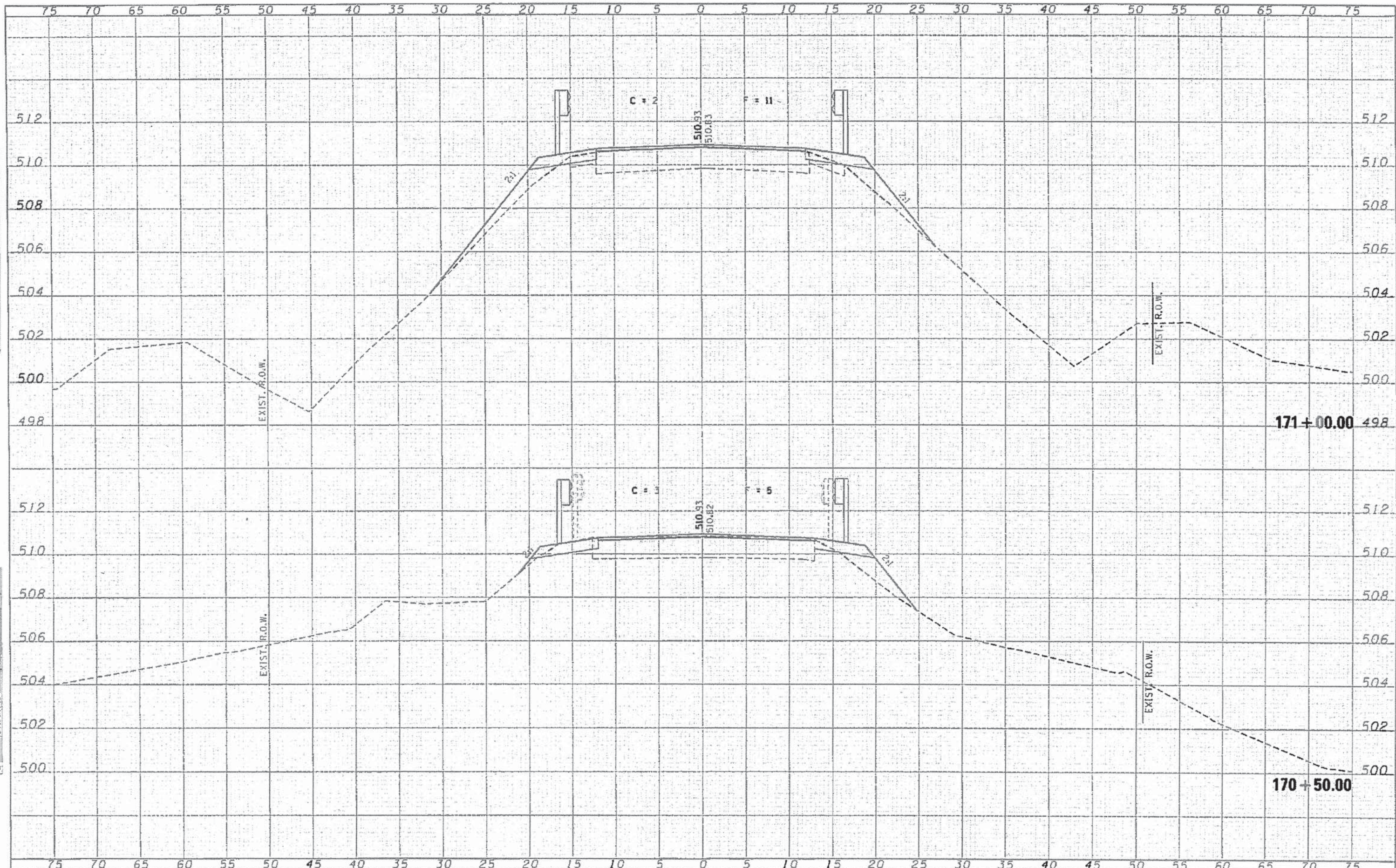
STONE RIPRAP, CL A4

170 + 00.00

166 + 50.00

166 + 00.00

FILE NAME = 110323.dwg	USER NAME = MUSEWA	DESIGNED - J.W.F.	REVISED -	<b>STATE OF ILLINOIS</b> <b>TAZEWELL COUNTY HIGHWAY DEPARTMENT</b>	<b>CROSS SECTIONS</b> <b>C.H. 11 / TOWERLINE ROAD</b>		F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZHI AND RENYICK, INC.	1000 N. WASHINGTON ST. SUITE 201	DRAWN - T.W.K.	REVISED -		2462	12-0004T-00-BR	TAZEWELL	39	37	CONTRACT NO. 89653	
PLT SCALE = HSCALE	PLT DATE = 8/14/2015	CHECKED - M.D.C.	REVISED -		SCALE:	SHEET NO.	OF SHEETS	STA. 166+00.00	TO STA. 170+00.00	ILLINOIS FED. AID PROJECT	
DATE = 08/14/15	REVISED -										



DATE	
BY	
DESIGNED	
DRAWN	
CHECKED	
DATE	

DATE	
BY	
DESIGNED	
DRAWN	
CHECKED	
DATE	

FILE NAME	120022-001-111.dgn
USER	MUSER
DESIGNED	J.W.F.
DRAWN	T.W.X.
CHECKED	M.D.C.
DATE	08/14/15

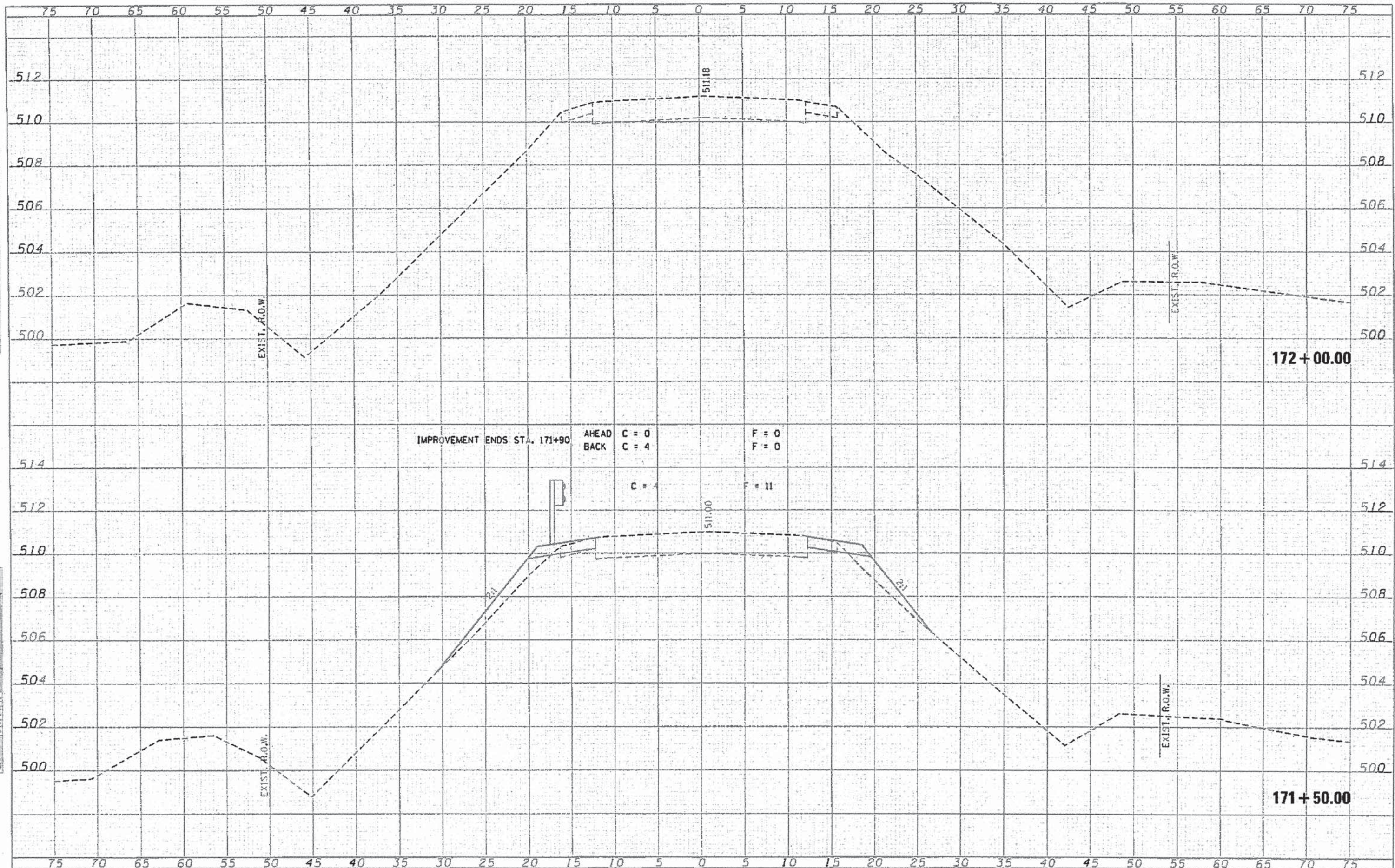
REVISION	
REVISION	
REVISION	
REVISION	

**STATE OF ILLINOIS  
TAZEWELL COUNTY HIGHWAY DEPARTMENT**

SCALE:		SHEET NO.	OF	SHEETS	STA. 170+50.00	TO	STA. 171+00.00
--------	--	-----------	----	--------	----------------	----	----------------

**CROSS SECTIONS  
C.H. 11 / TOWERLINE ROAD**

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
2462	12-00047-00-BR	TAZEWELL	39	38
CONTRACT NO. 89653				
ILLINOIS FED. AID PROJECT				



DATE	
BY	
NO.	
REVISIONS	
DATE	
BY	
NO.	

DATE	
BY	
NO.	
REVISIONS	
DATE	
BY	
NO.	