



MILLENNIA PROFESSIONAL SERVICES

11 Executive Drive, Suite 12, Fairview Heights, Illinois 62208 618-624-8610

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Structural Geotechnical Report

IL Rte. 145 over Bay Creek (FAP Rte 132)

Section 103 (B-2)

Pope County, Illinois

PTB190-035

IDOT Job Number D-99-028-19

Existing Structure 076-0024

Proposed Structure 076-0033

Prepared For:

Oates Associates, Inc.

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Collinsville, Illinois 62234

618-345-2200

Prepared By:

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Millennia Project Number MG19034.09

August 17, 2021

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Structure Geotechnical Report
IL Route 145 over Bay Creek
FAP Route 132, Section 103(B-2)
Existing Structure 076-0024
Proposed Structure 076-0033
Pope County, Illinois

1.0 Project Description and Proposed Structure Information

1.1 Introduction

Structure geotechnical report for the proposed structure 076-0033 over Bay Creek, IL Route 145, FAP Route 132, Section 103(B-2). The existing structure 076-0024 was constructed in 1979. The proposed structure 076-0033 was designed in 2021. The structure is located in Pope County, Illinois.

1.2 Project Description

The structure is a concrete bridge structure over Bay Creek. The structure is located on IL Route 145, FAP Route 132, Section 103(B-2). The structure is 325 feet long and 32 feet wide. The structure is supported by two concrete piers. The structure is designed for a design speed of 35 mph.

1.3 Proposed Structure Information

The proposed structure is a concrete bridge structure over Bay Creek. The structure is located on IL Route 145, FAP Route 132, Section 103(B-2). The structure is 325 feet long and 32 feet wide. The structure is supported by two concrete piers. The structure is designed for a design speed of 35 mph.

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2.2 Laboratory Testing

3.0 Subsurface Conditions

3.1 Geology

3.2 Generalized Subsurface Profile

2021 Study:

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0 33

109 229.50 3 75 50 0

2019 Study:

24 42 5 15

16 334 26 28.5 321.5 324 0 1 M 55 164 3 1 2

6 24

1979 Study:

1979 332 322 2021 2019

1979 2019 20

3.3 Groundwater

2021 24 25 314.0 2019 36 313.9 37.5 312.6 333.3 333.6 330.1 330.3 337.0

4.0 Geotechnical Evaluations

4.1 Earthwork and Slope Stability

Millennia's geotechnical engineers performed global stability analyses for the proposed earthwork and slope stability conditions. The analyses were performed using the Morgenstern-Price method, which is a rigorous limit equilibrium method that accounts for the geometry of the failure surface, the distribution of pore water pressure, and the effects of seismicity. The analyses were performed for a range of failure surface shapes and locations, and the results are summarized in the table below.

The global stability analyses were performed using laboratory investigations, along with Millennia's experience in the area, and are shown on the following pages.

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Summary of Global Stability Results

Analysis Location Section	Assumed Conditions	Minimum Computed Factor of Safety		
		Short Term	Long Term	Seismic
Section 1	3:1 slope	2.1	1.6	1.1
Section 2	3:1 slope	2.1	1.7	1.1
Section 3	3:1 slope	1.9	1.5	1.1
Section 4	3:1 slope	2.1	1.5	1.2

The global stability analyses were performed using the Morgenstern-Price method, which is a rigorous limit equilibrium method that accounts for the geometry of the failure surface, the distribution of pore water pressure, and the effects of seismicity. The analyses were performed for a range of failure surface shapes and locations, and the results are summarized in the table below.

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4.2 Settlement

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4.3 Mining Activity

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4.4 Seismicity

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5.0 Foundation Evaluations and Design Recommendations

5.1 Drilled Shaft Foundations

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**Unfactored Side Resistance and Tip Resistance Values
for Boring 1-S (2019) North Abutment Station 898+26.00**

Elevation (ft)	Material Classification	Nominal Unit Side Resistance (psf)	Nominal Tip* Resistance (psf)	Nominal Liquefaction Downdrag Unit Values (psf)**
350-341				230
341-336.5		385		360
336.5-334		55		55
334-331.5		500		400
331.5-326.5		220		400
326.5-324		660		1200
324-321.5		385		700
321.5-314		190		350
314-296.5		1310		200
296.5-276.5		2000	36000	
276.5-255		1970	36000	

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**Unfactored Side Resistance and Tip Resistance Values
for Boring 2-S (2019) South Abutment Station 895+06.00**

Elevation (ft)	Material Classification	Nominal Unit Side Resistance (psf)	Nominal Tip* Resistance (psf)	Nominal Liquefaction Downdrag Unit Values (psf)**
350-344				130
344-336.5		275		445
336.5-334		110		200
334-324		430		670
324-321.5		385		690
321.5-316.5		220		400
316.5-311.5		730		200
311.5-298		2025	36000	
298-291.5		1600	28800	
291.5-260		1925	28800	

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Resistance Factors

Method and Soil Type	Resistance Factor
0.45	0.45
0.55	0.55
0.35	0.35
0.45	0.45
0.40	0.40
0.50	0.50
1.0	1.0

R_D 9

0.60

5.2 Driven Pile Foundations

spreadsheet "Pile Capacity and Length Estimates,". Steel H

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Estimated Pile Length Table – North Abutment (Boring 1-S 2019)
(Estimated Pile Cutoff Elevation: 345.18)

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls	201	110	49
	375	206	90
MS 14" w/0.25" walls	254	139	49
	451	248	90
MS 14" w/0.312" walls	252	139	49
	451	248	90
MS 16" w/0.312" walls	312	171	49
	531	292	90
MS 16" w/0.375" walls	312	171	49
	531	292	90
8x36	92	50	90
10x42	116	64	90
10x57	119	66	90
12x53	146	80	90
12x63	147	81	90
12x74	149	82	90
12x84	152	83	90
14x73	180	99	90
14x89	183	100	90
14x102	185	102	90
14x117	187	103	90

*= maximum nominal bearing and maximum estimated pile length below existing boring depth

Estimated Pile Length Table – South Abutment (Boring 2-S 2019)
(Estimated Pile Cutoff Elevation: 345.18)

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls	213	117	49
	392	213	82
MS 14" w/0.25" walls	259	143	49
	459	251	80
MS 14" w/0.312" walls	259	143	49
	479	263	84
MS 16" w/0.312" walls	308	169	49
	564	310	84
MS 16" w/0.375" walls	308	170	49
	564	310	84
836	93	51	84
1042	119	66	84
1057	122	67	84
1253	150	82	84
1263	152	83	84
1274	154	84	84
1284	156	86	84
1473	186	102	84
1489	189	104	84
14102	191	105	84
14117	194	107	84

***= maximum nominal bearing and maximum estimated pile length below existing boring depth**

**Estimated Pile Length Table – Pier 1 (Boring 4-S 2021)
(Pile Cutoff Elevation: 336.3)**

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls	230	125	47
	363	198	86
MS 14" w/0.25" walls	289	157	47
	435	237	91
MS 14" w/0.312" walls	289	157	47
	445	237	86
MS 16" w/0.312" walls	353	192	47
	510	279	86
MS 16" w/0.375" walls	353	192	47
	510	279	86
8x36	75	40	86
10x42	96	52	86
10x57	98	53	86
12x53	120	65	86
12x63	121	65	86
12x74	123	66	86
12x84	125	67	86
14x73	148	80	86
14x89	150	81	86
14x102	153	82	86
14x117	155	84	86

***= maximum nominal bearing and maximum estimated pile length below existing boring depth**

**Estimated Pile Length Table – Pier 2 (Boring 3-S 2021)
(Pile Cutoff Elevation: 327.0)**

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls	267	143	50
	392	211	79
MS 14" w/0.25" walls	325	174	50
	459	246	76
MS 14" w/0.312" walls	325	174	50
	570	275	89
MS 16" w/0.312" walls	387	207	50
	654	323	89
MS 16" w/0.375" walls	387	207	50
	782	394	95
836	270	146	102
1042	335	181	102
1057	454	250	103
1253	418	227	102
1263	497	273	103
1274	589	324	103
1284	664	365	103
1473	578	318	103
1489	705	388	103
14102	810	445	103
14117	929	511	103

***= maximum nominal bearing and maximum estimated pile length below existing boring depth**

**Estimated Pile Length Table – Pier 3 (Boring 3-S 2021)
(Pile Cutoff Elevation: 333.3)**

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls	320	174	50
	392	208	80
MS 14" w/0.25" walls	406	220	50
	459	231	79
MS 14" w/0.312" walls	406	220	50
	570	286	94
MS 16" w/0.312" walls	500	272	50
	654	333	94
M 16" w/0.375" walls	500	272	50
	782	409	101
836	286	156	107
1042	335	183	106
1057	454	250	108
1253	418	228	106
1263	497	271	107
1274	589	324	108
1284	664	365	108
1473	578	316	107
1489	705	388	108
14102	810	445	108
14117	929	511	108

***= maximum nominal bearing and maximum estimated pile length below existing boring depth**

5.3 Lateral Load Capacity Considerations

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Recommended Design Values for Deep Foundations
Parameters for Use in LPILE Analysis at Boring 1-S (2019)
North Abutment at Station 898+26.00

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K_{static} (pci)
350-341	CLAY	125	1000	0.009		350
341-336.5	CLAY	58	700	0.010		100
336.5-334	MUD	58	100	0.020		30
334-331.5	CLAY	58	900	0.009		350
331.5-326.5	MUD	53	400	0.020		30
326.5-321.5	CLAY	53	750	0.010		100
321.5-314	MUD	53	350	0.020		30
314-296.5	CLAY	58			31	41
296.5-276.5	CLAY	58			34	72
276.5-255	CLAY	58			33	60

Parameters for Use in LPILE Analysis at Boring 2-S (2019)
South Abutment at Station 895+06.00

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K_{static} (pci)
350-344	CLAY	125	900	0.009		350
344-336.5	CLAY	58	600	0.010		100
336.5-334	CLAY (M)	58	200	0.020		30
334-324	CLAY	58	780	0.010		100
324-321.5	CLAY	58	700	0.010		100
321.5-316.5	CLAY (M)	53	400	0.020		30
316.5-311.5	CLAY (R)	58			28	13
311.5-298	CLAY (R)	58			36	97
298-291.5	CLAY (R)	58			32	51
291.5-260.0	CLAY (R)	58			33	60

CLAY (M) CLAY (R) CLAY (M)

Parameters for Use in LPILE Analysis of Liquefiable Layers at Boring 1-S (2019)

North Abutment at Station 898+26.00

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K_{static} (pci)
318.9-301.4	MS	58	240	0.02		30

Parameters for Use in LPILE Analysis of Liquefiable Layers at Boring 2-S (2019)

South Abutment at Station 895+06.00

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K_{static} (pci)
319.1-314.1	MS	58	160	0.02		30

Parameters for Use in LPILE Analysis of Liquefiable Layers at Boring 4-S (2021)

Pier 1 at Station 895+76.00

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K_{static} (pci)
312.2-299.5	MS	58	300	0.02		30

Parameters for Use in LPILE Analysis of Liquefiable Layers at Boring 3-S (2021)

Pier 2 Station 896+66.00 and Pier 3 at Station 897+56.00

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K_{static} (pci)
316-301	MS	58	100	0.02		30

6.0 Construction Considerations

6.1 Temporary Sheeting and Soil Retention

Construction of the project will require temporary sheet piling and soil retention systems to be installed along the project alignment. The design of these systems will be based on the geotechnical data provided in the R... and will be subject to approval by the relevant authorities. The sheet piling will be installed in a trench excavated to a depth of 1.5 to 1.0 meters below the ground level. The soil retention system will consist of sheet piling supported by tie-backs or soldier piles. The design will take into account the soil strength, groundwater conditions, and the required excavation depth. The sheet piling will be installed in a trench excavated to a depth of 1.5 to 1.0 meters below the ground level. The soil retention system will consist of sheet piling supported by tie-backs or soldier piles. The design will take into account the soil strength, groundwater conditions, and the required excavation depth.

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6.2 Cofferdam

A cofferdam will be installed at the project site to allow for the construction of the foundation. The cofferdam will be excavated to a depth of 3.37 meters below the ground level. The cofferdam will be supported by sheet piling and tie-backs. The design will take into account the soil strength, groundwater conditions, and the required excavation depth. The cofferdam will be installed at the project site to allow for the construction of the foundation. The cofferdam will be excavated to a depth of 3.37 meters below the ground level. The cofferdam will be supported by sheet piling and tie-backs. The design will take into account the soil strength, groundwater conditions, and the required excavation depth.

6.3 Subgrade Water Protection

Subgrade water protection will be provided by the installation of a drainage system. The drainage system will consist of a drainage ditch and a drainage pipe. The drainage ditch will be excavated to a depth of 1.5 to 1.0 meters below the ground level. The drainage pipe will be installed in a trench excavated to a depth of 2.81 meters below the ground level. The drainage system will be designed to collect and remove water from the subgrade. Subgrade water protection will be provided by the installation of a drainage system. The drainage system will consist of a drainage ditch and a drainage pipe. The drainage ditch will be excavated to a depth of 1.5 to 1.0 meters below the ground level. The drainage pipe will be installed in a trench excavated to a depth of 2.81 meters below the ground level. The drainage system will be designed to collect and remove water from the subgrade.

6.4 Drilled Shaft Construction

Drilled shaft construction will be used for the foundation of the project. The drilled shafts will be installed in a trench excavated to a depth of 5.16 meters below the ground level. The drilled shafts will be supported by tie-backs or soldier piles. The design will take into account the soil strength, groundwater conditions, and the required excavation depth. The drilled shafts will be installed in a trench excavated to a depth of 5.16 meters below the ground level. The drilled shafts will be supported by tie-backs or soldier piles. The design will take into account the soil strength, groundwater conditions, and the required excavation depth.

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7.0 Closing

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Millennia Professional Services of Illinois, Ltd.

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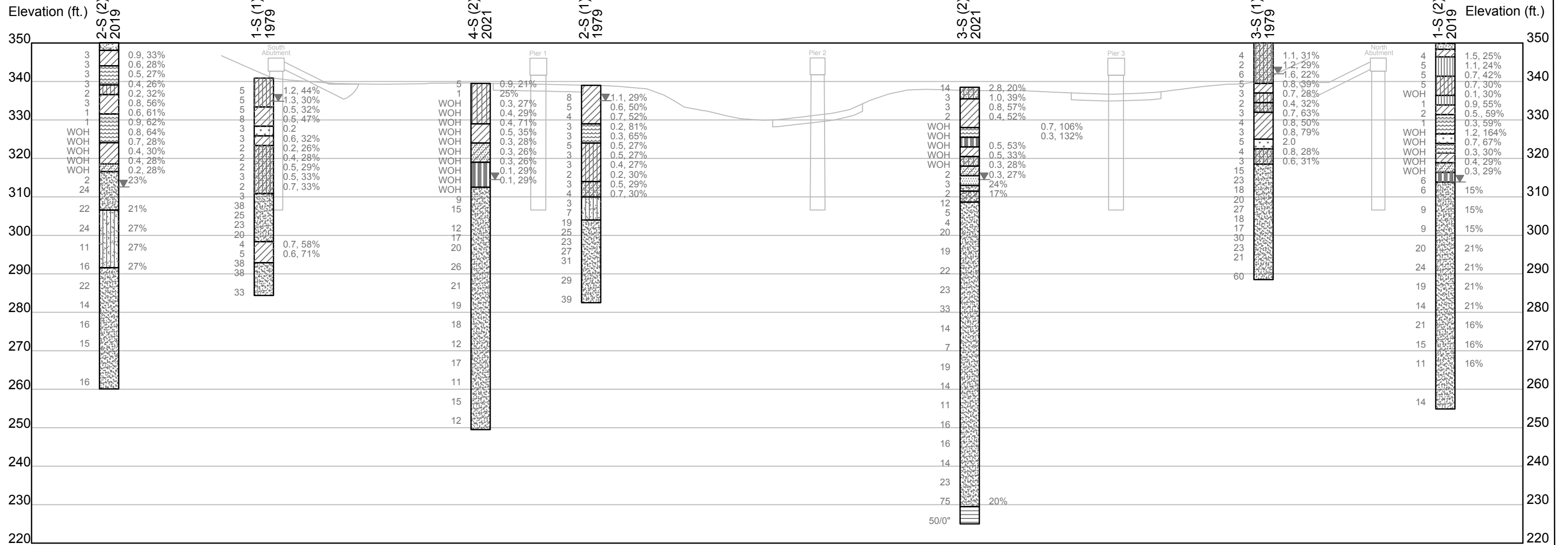
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STRUCTURE NO. 076-0033, -0024



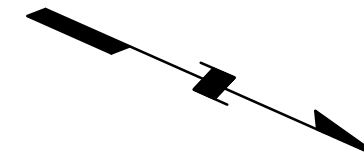
Note: Elevations are approximate. Actual conditions between borings are unknown, and are subject to change. Drilled shaft depths shown are only for reference.

STRATIGRAPHY

	CLAY		LOAM		SHALE
	CLAY LOAM		PEAT		SILT
	CLAY WITH ORGANICS		SAND		SILTY CLAY
	CONCRETE		SANDY CLAY		SILTY CLAY LOAM
			SANDY LOAM		SILTY LOAM

BORING DATA KEY:

	Boring ID
	N-Value (bpf)
	Stratigraphy
	Rimac Value (tsf), Moisture Content (%)
	Groundwater level encountered at boring



Vertical Scale: Approx. 1" = 25'
Horizontal Scale: IGNORE

FIGURE 3:

SUBSURFACE PROFILE

PROJECT NAME:

PROJECT No.:

DRAWN BY:

CHECKED BY:

Work Order No. 9

MG19034.09

B. FISHER

J. SCHAEFFER

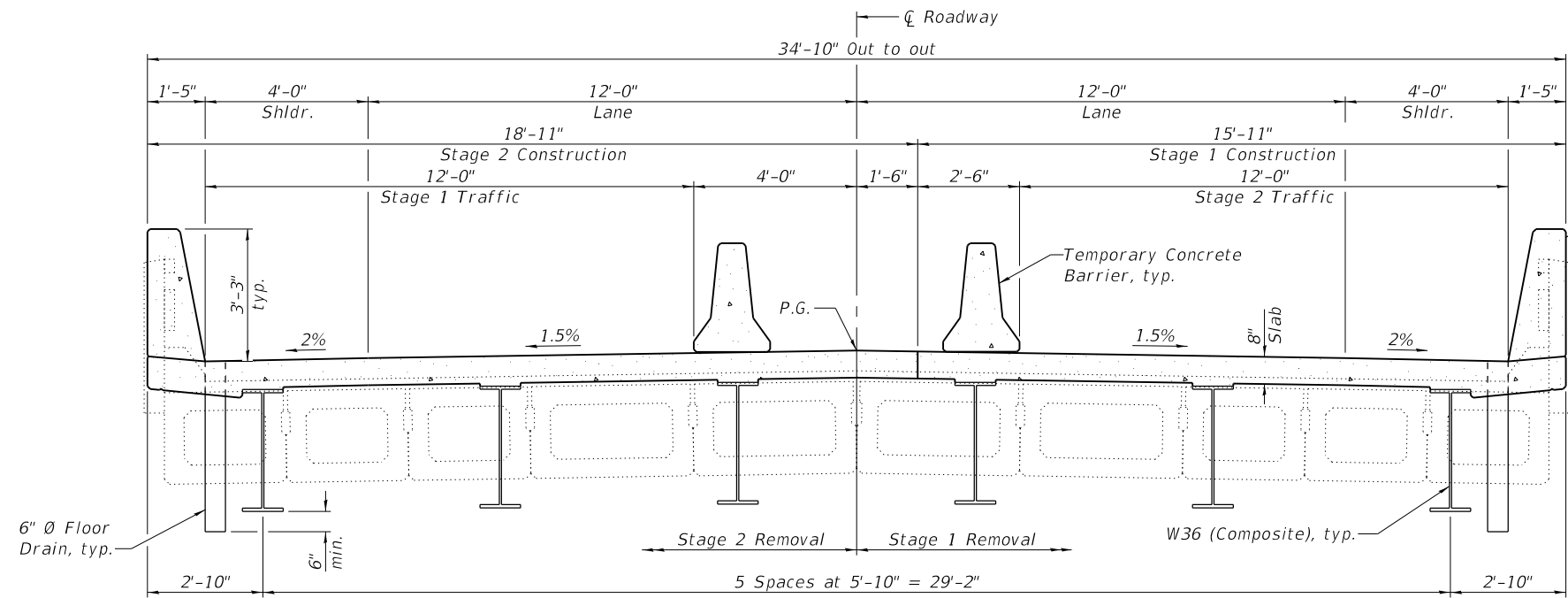
IL 145 over Bay Creek

08/05/2021

08/05/2021



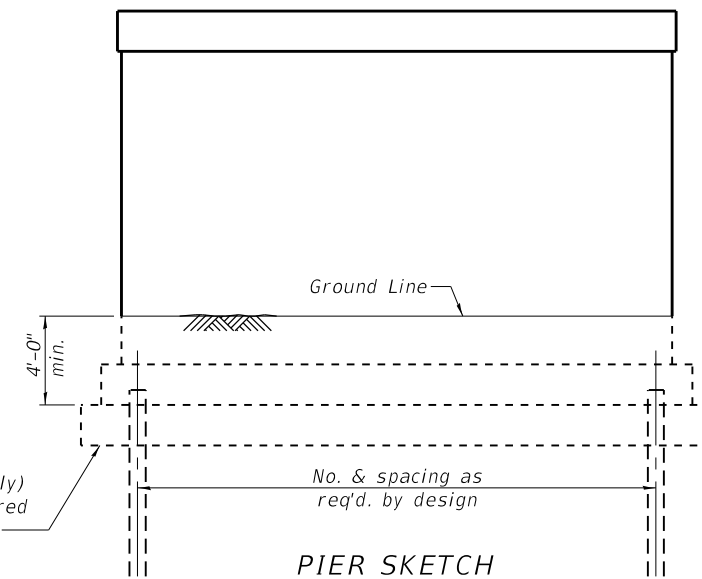
MILLENNIA
PROFESSIONAL
SERVICES



CROSS SECTION
(Looking North)

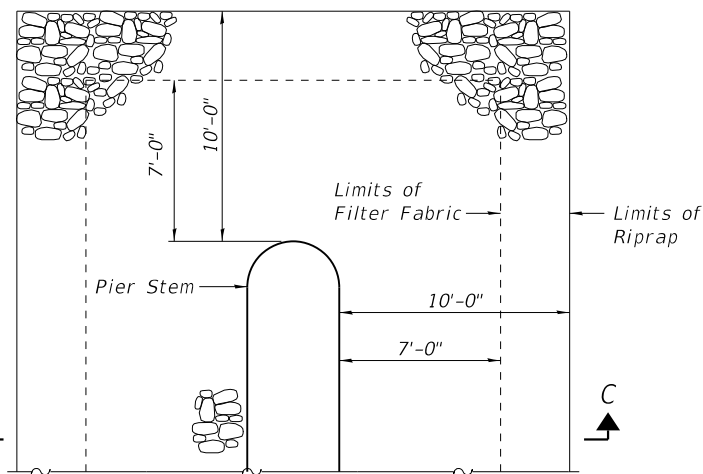
39" Constant Slope Parapet, typ.

Seal Coat (Pier 2 only) dimensions as required by design, 2'-6" min.

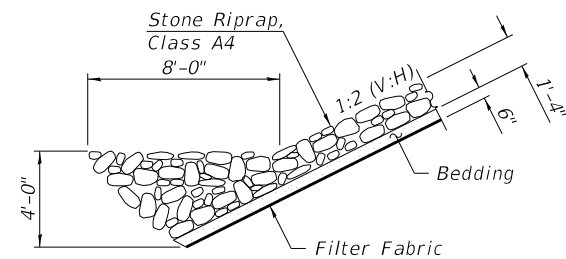


PIER SKETCH

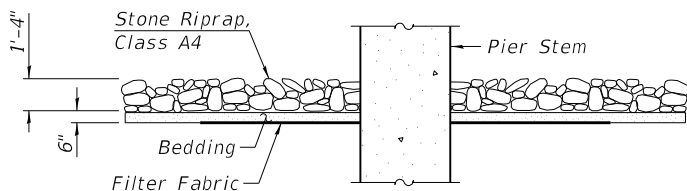
PRELIMINARY PLANS
NOT FOR REVIEW



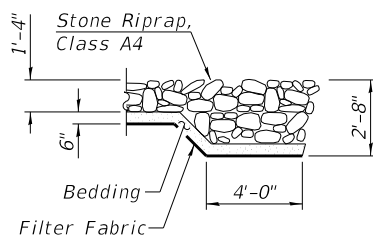
RIPRAP PROTECTION AT PIERS



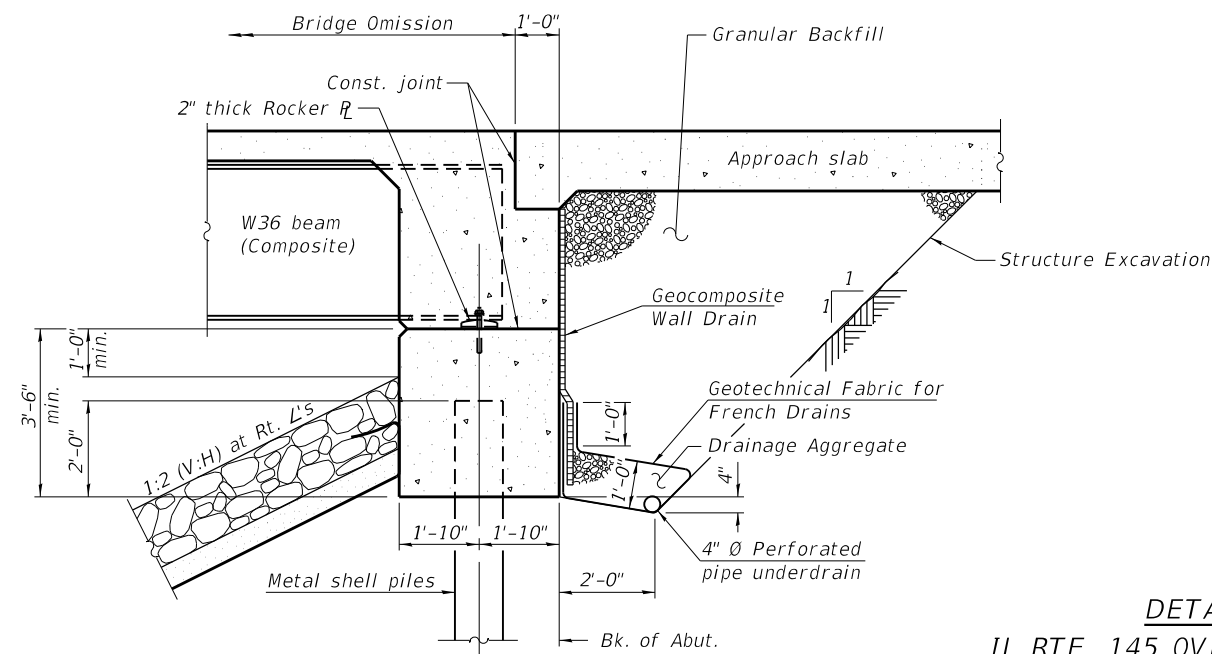
SECTION A-A



SECTION C-C



SECTION B-B



SECTION THRU INTEGRAL ABUTMENT

DETAILS
IL RTE. 145 OVER BAY CREEK
F.A.P. RTE. 132 - SEC. 103B-2
POPE COUNTY
STA. 896+66.00
STRUCTURE NO. 076-0033

FILE NAME: H:\P\181201\181201\09\Bidge\TSL\Microstation\0760033-78719-002-TSL.dgn



USER NAME =	DESIGNED - JAD	REVISED -
CHECKED - ETH	CHECKED - ETH	REVISED -
PLOT SCALE =	DRAWN - JAD	REVISED -
PLOT DATE = 8/16/2021	CHECKED - ETH	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SHEET 2 OF 2 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
132	103B-2	POPE		
CONTRACT NO. 78719				
ILLINOIS FED. AID PROJECT				



M r r d

11 Executive Drive, Suite 12

Fairview Heights, Illinois 62208

618-624-8610

Appendix B: Boring Logs

DESCRIPTION IL-145 over Bay Creek DISTRICT 9
LOCATION 2.3 miles South of IL 146 (North Abut.) CONSULTANT Millennia Professional Services
DRILLED BY Geotechnology LOGGED BY P. Adhikari RIG TYPE CME 550X
DRILLING METHOD Hollow Stem Auger HAMMER TYPE Automatic EFFICIENCY 75%

BORING NO.	Station	Offset	Northing	Easting	Ground Surface Elev.	ELEVATION (ft)	DEPTH (ft)	BLOWS /6"	UCS (tsf)	MOISTURE (%)	Surface Water Elev. (ft)	Stream Bed Elev. (ft)	Groundwater Elev.:	First Encounter (ft)	Upon Completion (ft)	After (Hrs.) (ft)	ELEVATION (ft)	DEPTH (ft)	BLOWS /6"	UCS (tsf)	MOISTURE (%)	
3-S	897+25	35 ft RT			338.5	338.30								314.5								
LITHOLOGY						LITHOLOGY						LITHOLOGY										
Topsoil (2.0")						Gray, soft, moist, SILTY CLAY LOAM (continued)						Gray, soft, moist, SILTY CLAY LOAM										
FILL: Brown, very stiff, SILTY CLAY LOAM						Gray, soft, moist, CLAY LOAM						Gray, soft, moist, CLAY LOAM										
335.50						315.50						313.00										
Gray, soft to medium-stiff, moist, CLAY						Gray, soft, moist, LOAM - Atterberg Limits: LL = 23, PI = 7						Gray, loose, SAND, fine- to medium-grained										
- trace organics below 6.5 ft. - Atterberg Limits: LL = 95, PI = 61						- Atterberg Limits: LL = 23, PI = 8						Gray, soft, SANDY CLAY, trace organics										
328.00						308.70						308.70										
Black, medium-stiff, moist, ORGANIC CLAY						Gray, loose to medium-dense, SAND, medium- to coarse-grained, with fine gravel - 4.1% passing No. 200 sieve, 15.0% retained on No. 4 sieve						Gray, soft, moist, CLAY, trace organics										
325.50						323.00						320.50										
Brownish gray, soft, moist, ORGANIC SILTY LOAM						Gray, soft, moist, CLAY, trace organics						Gray, soft, moist, CLAY, trace organics										
323.00						320.50						320.50										
Gray, soft, moist, CLAY, trace organics						Gray, soft, moist, CLAY, trace organics						Gray, soft, moist, CLAY, trace organics										
320.50						320.50						320.50										
Gray, soft, moist, SILTY CLAY LOAM						Gray, soft, moist, SILTY CLAY LOAM						Gray, soft, moist, SILTY CLAY LOAM										
- Atterberg Limits: LL = 41, PI = 20						- Atterberg Limits: LL = 41, PI = 20						- Atterberg Limits: LL = 41, PI = 20										
-20						-20						-20										

The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.

DESCRIPTION IL-145 over Bay Creek DISTRICT 9
LOCATION 2.3 miles South of IL 146 (South Abut.) CONSULTANT Millennia Professional Services
DRILLED BY Geotechnology LOGGED BY P. Adhikari RIG TYPE CME 550X
DRILLING METHOD Hollow Stem Auger HAMMER TYPE Automatic EFFICIENCY 75%

BORING NO.	Station	Offset	Northing	Easting	Ground Surface Elev.	ELEVATION (ft)	DEPTH (ft)	BLOWS (/6")	UCS (tsf)	MOISTURE (%)	Surface Water Elev. (ft)	Stream Bed Elev. (ft)	Groundwater Elev. (ft)	First Encounter (ft)	Upon Completion (ft)	After (Hrs.) (ft)	ELEVATION (ft)	DEPTH (ft)	BLOWS (/6")	UCS (tsf)	MOISTURE (%)	
4-S	895+65	37 ft RT			339.5 ft																	
LITHOLOGY						LITHOLOGY						LITHOLOGY										
Topsoil (2.0")						339.25							319.00									
Brown, soft to medium-stiff, SILTY CLAY								2														
								3	0.9	21												
								2	S													
- gray, soft, moist below 3.5 ft.								WOH														
								WOH		25												
						-5		1														
								WOH														
								WOH	0.3	27												
								WOH	B													
- trace organics below 8.5 ft.								WOH														
								WOH	0.4	29												
						-10		WOH	B													
Gray, soft, moist, CLAY, trace organics						329.00		WOH														
								WOH	0.4	71												
								WOH	B													
								WOH														
								WOH	0.5	35												
						-15		WOH	B													
Gray, soft, moist, CLAY LOAM, trace organics						324.00		WOH														
								WOH	0.3	28												
								WOH	B													
								WOH														
								WOH	0.3	26												
						-20		WOH	B													

The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.

DESCRIPTION IL-145 over Bay Creek DISTRICT 9
LOCATION 2.3 miles South of IL 146 (South Abut.) CONSULTANT Millennia Professional Services
DRILLED BY Geotechnology LOGGED BY P. Adhikari RIG TYPE CME 550X
DRILLING METHOD Hollow Stem Auger HAMMER TYPE Automatic EFFICIENCY 75%

BORING NO. <u>4-S</u>	ELEVATION	DEPTH	BLOWS	UCS	MOISTURE	Surface Water Elev. <u> </u> ft	ELEVATION	DEPTH	BLOWS	UCS	MOISTURE
Station <u>895+65</u>						Stream Bed Elev. <u> </u> ft					
Offset <u>37</u> ft RT						Groundwater Elev.: <u> </u>					
Northing <u> </u>						First Encounter <u>314.5</u> ft ▼					
Easting <u> </u>						Upon Completion <u> </u> ft					
Ground Surface Elev. <u>339.5</u> ft						After <u> </u> Hrs. <u> </u> ft					

LITHOLOGY	(ft)	(ft)	(/6")	(tsf)	(%)	LITHOLOGY	(ft)	(ft)	(/6")	(tsf)	(%)
Gray, very loose to medium-dense, SAND, fine- to medium-grained, trace gravel (continued) - with gravel below 41.0 ft.			5			Gray, very loose to medium-dense, SAND, fine- to medium-grained, trace gravel (continued)					
			8								
			9								
- fine- to medium-grained below 43.5 ft. - 3.5% passing No. 200 sieve			7						7		
			10						9		
			10						9		
	-45								-65		
			10								
			13			- medium- to coarse-grained below 68.5 ft.			3		
			13						6		
	-50								6		
			8								
- fine- to coarse-grained below 53.5 ft.			9						7		
			12						8		
	-55								9		
			12								
			9			- fine- to medium-grained below 78.5 ft.			6		
			10						5		
	-60								6		

The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.



SOIL BORING LOG

ROUTE IL 145 DESCRIPTION Structure over Bay Creek LOGGED BY L. Estel

SECTION 103(B-2) LOCATION 2.3 miles South of IL 146 (North Abut.), SEC. 29, TWP. 13S, RNG. 5E, PM

COUNTY Pope DRILLING METHOD Hollow stem auger (8" O.D., 3.25" I.D.) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO.	Station	DEPTH (ft)	BLOWS	UCS (tsf)	MOIST (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After Hrs.	DEPTH (ft)	BLOWS	UCS (tsf)	MOIST (%)
076-0024	896+49					333.6	330.1								
BORING NO.	1-S														
Station	898+44														
Offset	7.0ft Rt														
Ground Surface Elev.	349.9	ft													
Cored pavement, 9" HMA over 9" CONCRETE		348.40						Soft Grey, V. Moist CLAY with ORGANICS N.L. PI > 11 (Est.) (continued)					WOH		
Grey, Moist CLAY (visual ID from auger cuttings)		346.40											WOH	0.3	59
													1	B	
Stiff Grey, Moist SILT			1				326.40	Stiff Dark Brown and Black, V. Moist CLAYEY PEAT					WOH		
			2	1.5	25								WOH	1.2	164
			-5	B									WOH	B	
			1				323.90	M. Stiff Grey, V. Moist CLAY with specks of ORGANICS N.L. PI > 11 (Est.)					WOH		
			2	1.1	24								WOH	0.7	67
			3	B									WOH	B	
M. Stiff Grey, Moist SILTY CLAY with chunks of WOOD		341.40					321.40	Soft Grey, Moist CLAY N.L. PI > 11 (Est.)					WOH		
			1										WOH	0.3	30
			2	0.7	42								WOH	B	
			-10	B									WOH		
			1				318.90	Soft Grey, Moist CLAY LOAM with ORGANICS (medium plasticity) and a 12 inch layer of Grey, m. to c. SAND					WOH		
			2	0.7	30								WOH	0.4	29
			3	B									WOH	B	
V. Soft Grey, Moist SILT A-6(11) 0% SAND, 83% SILT, 17% CLAY, LL = 32, PI = 10 (Lab 41)		336.40					316.40	Soft Grey, Moist SILTY LOAM A-4(4) 25% m. SAND, 60% SILT, 15% CLAY, LL = 27, PI = 8 (Lab 42)					WOH		
													WOH	0.3	29
			-15	B									WOH	B	
M. Stiff Grey, V. Moist CLAY N.L. PI > 11 (Est.)		333.90					313.90	Loose Grey m. and c. SAND with f. GRAVEL 40% f. GRAVEL and c. SAND, 58% m. SAND, 1% SILT, 1% CLAY (based on visual ID and Lab 43)					4		
													3		
			1	0.9	55								3		
Soft Grey, V. Moist CLAY with ORGANICS N.L. PI > 11 (Est.)		331.40					311.40	(washed)					2		
													3		
			1	0.5	59								3		15
			-20	B									3		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE IL 145 DESCRIPTION Structure over Bay Creek LOGGED BY L. Estel

SECTION 103(B-2) LOCATION 2.3 miles South of IL 146 (South Abut.), SEC. 29, TWP. 13S, RNG. 5E, PM

COUNTY Pope DRILLING METHOD Hollow stem auger (8" O.D., 3.25" I.D.) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO.	Station	DEPTH (ft)	BLOWS	UCS (tsf)	MOIST (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After Hrs.	DEPTH (ft)	BLOWS	UCS (tsf)	MOIST (%)
076-0024	896+49					333.3	330.3								
2-S	894+54								312.6						
	7.0ft Lt														
	350.1														
Cored pavement, 13" HMA over 9" CONCRETE						N.L. PI > 11 (Est.)									
348.10						Dark Grey						WOH			
Grey, Moist CLAY (visual ID from auger cuttings)												WOH 0.9 62			
346.60												1 B			
M. Stiff Grey, Moist CLAY												WOH			
												WOH 0.8 64			
												-5 2 B			
344.10						M. Stiff Grey, Moist CLAY with thin layers of ORGANICS						WOH			
												WOH 0.7 28			
												1 0.6 28			
												2 B			
												WOH			
												WOH 0.4 30			
												1 0.5 27			
												-10 2 B			
339.10						Soft Grey, Moist SILTY CLAY						WOH			
												WOH 0.4 28			
												1 0.4 26			
												2 B			
336.60						V. Soft Grey, Moist CLAY N.L. PI > 11 (Est.)						WOH			
												WOH 0.2 32			
												-15 1 B			
334.10						M. Stiff Grey, V. Moist CLAY N.L. PI > 11 (Est.)						WOH			
												WOH 23			
												1 0.8 56			
												2 B			
331.60						M. Stiff Grey with specks of Brown, V. Moist CLAY with ORGANICS						WOH			
												WOH 8			
												12			
												1 0.6 61			
												12			
												-20 1 B			
												-40 12			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced By Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE IL 145 DESCRIPTION Structure over Bay Creek LOGGED BY L. Estel

SECTION 103(B-2) LOCATION 2.3 miles South of IL 146 (South Abut.), SEC. 29, TWP. 13S, RNG. 5E, PM

COUNTY Pope DRILLING METHOD Hollow stem auger (8" O.D., 3.25" I.D.) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. 076-0024
Station 896+49

BORING NO. 2-S
Station 894+54
Offset 7.0ft Lt
Ground Surface Elev. 350.1 ft

DEPTH H (ft)	BLOW S (tsf)	UCS Qu (%)	MOIST T
--------------------	--------------------	------------------	------------

Surface Water Elev. 333.3 ft
Stream Bed Elev. 330.3 ft

Groundwater Elev.:
▽ First Encounter 312.6 ft
▽ Upon Completion _____ ft
▼ After _____ Hrs. _____ ft

M. Dense Grey, Moist m. and c. SAND with rounded GRAVEL Non-plastic, 29% c. SAND and f. GRAVEL, 69% m. SAND, 1% SILT, 1% CLAY (based on visual ID and Lab 45) (washed) (continued)

-85

7
6
10

260.10 -90

Bottom of hole @ 90.0 ft

N.L. = Not Liquefiable

Elevation referenced to BM 804, Chiseled Square on the NE wingwall of SN 076-0024; EL. 350.05

To convert "N" values to "N60", multiply by 1.5

-95

-100

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

B.M. "□" Cut in N. end of East hubguard 10'-6" Rt Sta. 897+37
Elev. 351.36

Existing Structure: #076-0017 Built 1934 at Sta. 896+56 Section 103 A on S. B. I. Rte. 145 Seven I-beam spans with 4" Timber planks superstructure and timber pile bents substructure. Timber planks replaced with 6" concrete slab in 1959. Existing structure is 147'-3 1/2" long by 22'-0" wide Stage Construction shall be utilized so as to maintain one way traffic during reconstruction.

No Salvage

ROUTE	SECTION	COUNTY	TOTAL	SHEET
FA 132	103A-B	POPE	25	13
S.A. TO S.I.A.		ILLINOIS FED. AID PROJECT		

GENERAL NOTES

1 of 13

See Proposal for Boring Data.

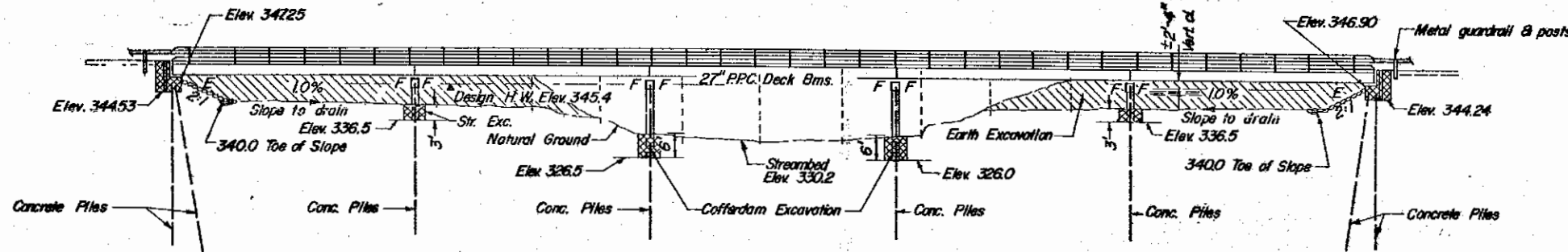
The top surface of the beams shall be finished in accordance with Article 505.06 of the Standard Specification except that the surface shall not be roughened by brooming. The finished surface shall be free of depressions or high spots with sharp corners.

Protective Coat shall not be applied to surfaces to which Waterproofing Membrane System is applied.

Reinforcement bars shall conform to the requirements of AASHTO M-31 or M-53 Grade 60, unless otherwise indicated.

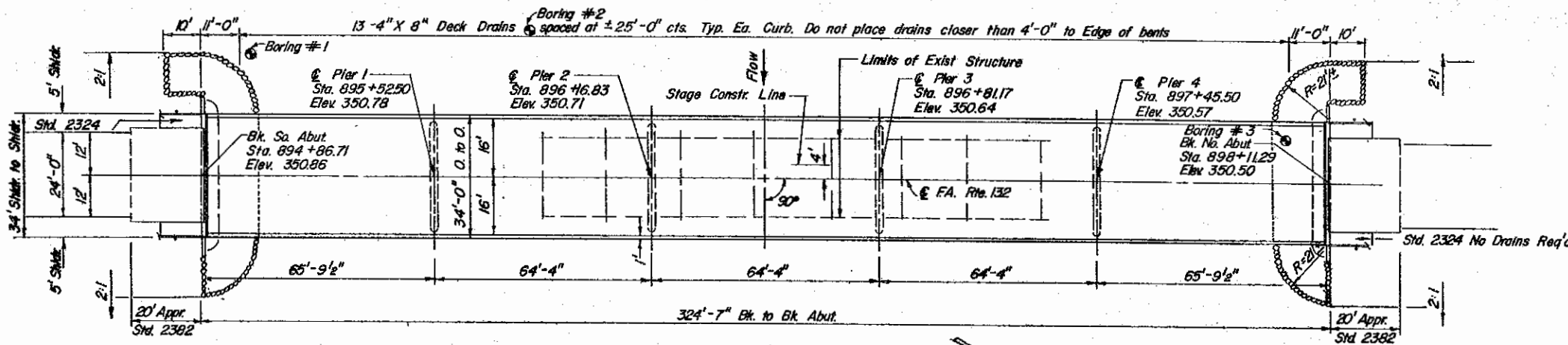
The Contractor shall drive one precast concrete test pile in a permanent location at Pier 2 and at Pier 4 as directed by the Engineer.

Bars designated "Grade 40" shall have a minimum yield strength of 35,000 psi and maximum yield strength of 45,000 psi.

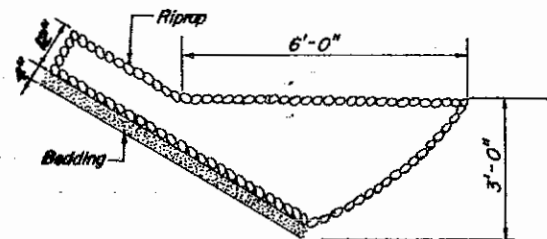


ELEVATION

NOTE: Hatched Area shows Earth Excavation which is included in the Earth work balances in the road plans.



PLAN



RIPRAP ANCHOR DETAIL

STATION 896+49
BUILT 198 BY
STATE OF ILLINOIS
FA. RT. 132 SEC. 103A-B
FA. PROJECT BR-F-132(49)
LOADING HS20
*STR. NO.

NAME PLATE

See Std. 213
*Structure Number to be supplied by District.

APPROVED
FOR STRUCTURAL ADEQUACY ONLY
Robert Bradford, Jr.
Engineer of Bridge & Traffic Structures

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Structure Excavation	Cu Yd.		195	195
Portland Cement Mortar Fairing Course	Lin. Ft.	2890		2890
Waterproofing Membrane System	Sq. Yd.	1145		1145
Protective Coat	Sq. Yd.	110		110
Bituminous Conc. Surf. Crse. Mixture D Class I	Ton	134		134
Concrete Piles	Lin. Ft.	1078		1078
Precast Concrete Piles, 14"	Lin. Ft.		2357	2357
Test Pile, Precast Concrete	Ea.		2	2
Neoprene Expansion Jt. 2"	Lin. Ft.	64		64
Precast Prestressed Concrete Deck Beams (27" Depth)	Sq. Ft.	10,923		10,923
Steel Railing, Type T	Lin. Ft.	644		644
Untreated Timber	FBM		624	624
Hardware	Pound		105	105
Name Plates	Ea.	1		1
Stone Riprap	Sq. Yd.		330	330
Removal of Existing Structures	Ea.		1	1
Class X Concrete	Cu. Yd.	18.2	1810	1922
Reinforcement Bars	Pound	1290	18,580	19,870
Temporary Bridge Rail	Lin. Ft.	365		365
Steel Sheet Piling, Temporary	Sq. Ft.		4750	4750
Cofferdam (Pier 2)	Ea.		1	1
Cofferdam (Pier 3)	Ea.		1	1
Cofferdam Excavation	Cu. Yds.		105	105

WATERWAY INFORMATION

Drainage Area 16466^{sq} mi. Low Grade Elev. 349.16' at Sta. 890+00

Flood	Freq. Yr.	Q. C.F.S.	Opening Sq. Ft.		Nat. H.W.E.	Head - Ft.		Headwater	
			Exist.	Prop.		Exist.	Prop.	Exist.	Prop.
Design	50	9900	1398	2360	345.4	2.0	0.47	347.4	345.9
Base	100	11700	1478	2528	346.0	2.5	0.60	348.5	346.6
Overlapping									
Max. Calc.	500	15515	1627	2842	347.12	3.0	0.75	350.1	347.9

DESIGN STRESSES

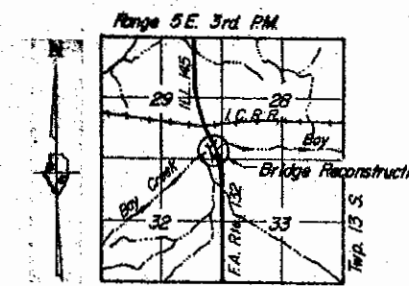
FIELD UNITS
f_c = 3,500 psi
f_y = 60,000 psi
Reinforcement

PRECAST PRESTRESSED UNITS
f_c = 5,000 psi
f_y = 4,000 psi
f_s = 270,000 psi (1/2" # Strands)
f_s = 189,000 psi (1/2" # Strands)

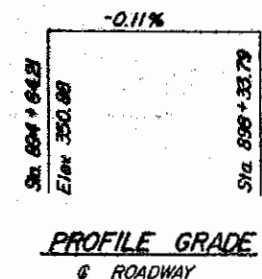
Design Specifications 1977 AASHTO & 1978, 1979 & 1980 Interims as applicable

Allow for 25 #/sq.ft. Future Wearing Surface

LOADING HS20-44



LOCATION SKETCH



PROFILE GRADE & ROADWAY

GENERAL PLAN AND ELEVATION

FA. Rte. 132 (ILL. 145) Over BAY CREEK
FA. Rte. 132 SECTION 103A-B
POPE COUNTY Sta. 896+49.00

GREENE & ELLIOTT, Ltd.

CONSULTING ENGINEERS

1819 STEVENSON DR. 217/529-6681 - SPRINGFIELD, ILL.

DRAWN	REVISION	DATE	PROJECT
L.V.R.			
CHECKED R.B.J.		OCT, 1980	

K.C.V. 009

BRIDGE FOUNDATION BORING LOG

STRUCTURE CARRYING F.A. ROUTE 132 OVER BAY CREEK
 SEC. 103A-B
 POPE COUNTY
 BORING NO. 1-S

Elevation	N	Qu t/s.	w (%)	Elevation	N	Qu t/s.	w (%)
-----------	---	---------	-------	-----------	---	---------	-------

SEE PREVIOUS SHEET	-45						
	5	0.6S	71				
292.9							
DENSE WET GREY COARSE GRAINED SAND & SOME SMALL GRAVEL	38		-	-75			
	-50						
	38	-	-				
	-55			-80			
284.4	33	-	-				
BOTTOM OF HOLE = 56.5'. DURING DRILLING OPERATIONS IT APPEARED THAT FREE WATER WAS ENCOUNTERED AT 30.0'. DURING DRILLING OPERATIONS WASH OUT PROCEDURES USED FROM 29.0' TO 56.5'. STRUCTURE LOCATION: SE 1/4, SE 1/4, SECTION 29, T13S, R5E, 3RD P.M., POPE COUNTY, ILLINOIS.	-60			-85			
	-65			-90			
	-70			-95			

BRIDGE FOUNDATION BORING LOG

PROJECT _____
ROUTE F.A. 132
SEC. 103A-B
COUNTY POPE

BRIDGE STRUCTURE CARRYING F.A.
ROUTE 132 OVER BAY CREEK
STA. 896+46

Date OCTOBER, 1979
Bored By JOEL CONGIARDO
Checked By GARY L. PULLEY

Boring No. 2-S
Station 895+83
Offset 41' LT. CL

Elevation	Z	Qu t/s.f.	w (%)	Surface Water El.	Groundwater El. at Completion	Groundwater El. After 24 Hours	Elevation	Z	Qu t/s.f.	w (%)
339.0	0			NONE	333.0	335.0				
Ground Surface										
STIFF MOIST TO VERY MOIST BROWN CLAY A-7-6								2	0.2B	30
							314.0	-25		
	8	1.1B	29					3	0.5B	29
334.0	-5									
MEDIUM VERY MOIST GREY CLAY A-6 TO A-7-6								4	0.7B	30
	5	0.6B	50				310.0			
	4	0.7B	52					3	-	-
329.0	-10									
SOFT TO VERY SOFT VERY MOIST GREY CLAY A-6 WITH ROTTEN WOOD MIXED								7	-	-
	3	0.2B	81							
	3	0.3B	65					19	-	-
324.0	-15									
MEDIUM WET GREY COARSE GRAINED SAND & SOME SMALL GRAVEL										
	5	0.5B	27					25	-	-
	3	0.5B	27					23	-	-
319.0	-20									
MEDIUM VERY MOIST GREY SILTY CLAY TO CLAY A-6 TO A-7-6										
	3	0.4B	27					27	-	-
294.0	-45									
SEE NEXT COLUMN										
SEE NEXT SHEET										

N - Standard Penetration Test - Blows per foot to drive 2" O.D. Split Spoon Sampler 12" with 140# hammer falling 30".

Qu - Unconfined Compressive Strength - t/sf
 w - Water Content - percentage of oven dry weight - %.

Type failure:
 B - Bulge Failure
 S - Shear Failure
 E - Estimated Value
 P - Penetrometer

BRIDGE FOUNDATION BORING LOG

STRUCTURE CARRYING F.A. ROUTE 132 OVER BAY CREEK	Elevation	Z	Qu t/s	w (%)	Elevation	Z	Qu t/s	w (%)
<p>SECTION 103A-B POPE COUNTY BORING NO. 3-S</p> <p style="margin-top: 20px;"><u>SEE PREVIOUS SHEET</u></p>	-45							
	18	-	-					
	17	-	-		-75			
	-50							
	30	-	-					
	23	-	-		-80			
	-55							
	21	-	-		-85			
	-60							
	289.5							
<p>VERY DENSE WET GREY COARSE GRAINED SAND</p>	288.5	60	-					
<p>BOTTOM OF HOLE = 61.5'</p>					-90			
<p>DURING DRILLING OPERATIONS IT APPEARED THAT FREE WATER WAS ENCOUNTERED AT 31.5'</p>	-65							
<p>DURING DRILLING OPERATIONS WASH OUT PROCEDURE USED FROM 31.5' TO 60.0'</p>								
	-70				-95			



MILLENNIA PROFESSIONAL SERVICES

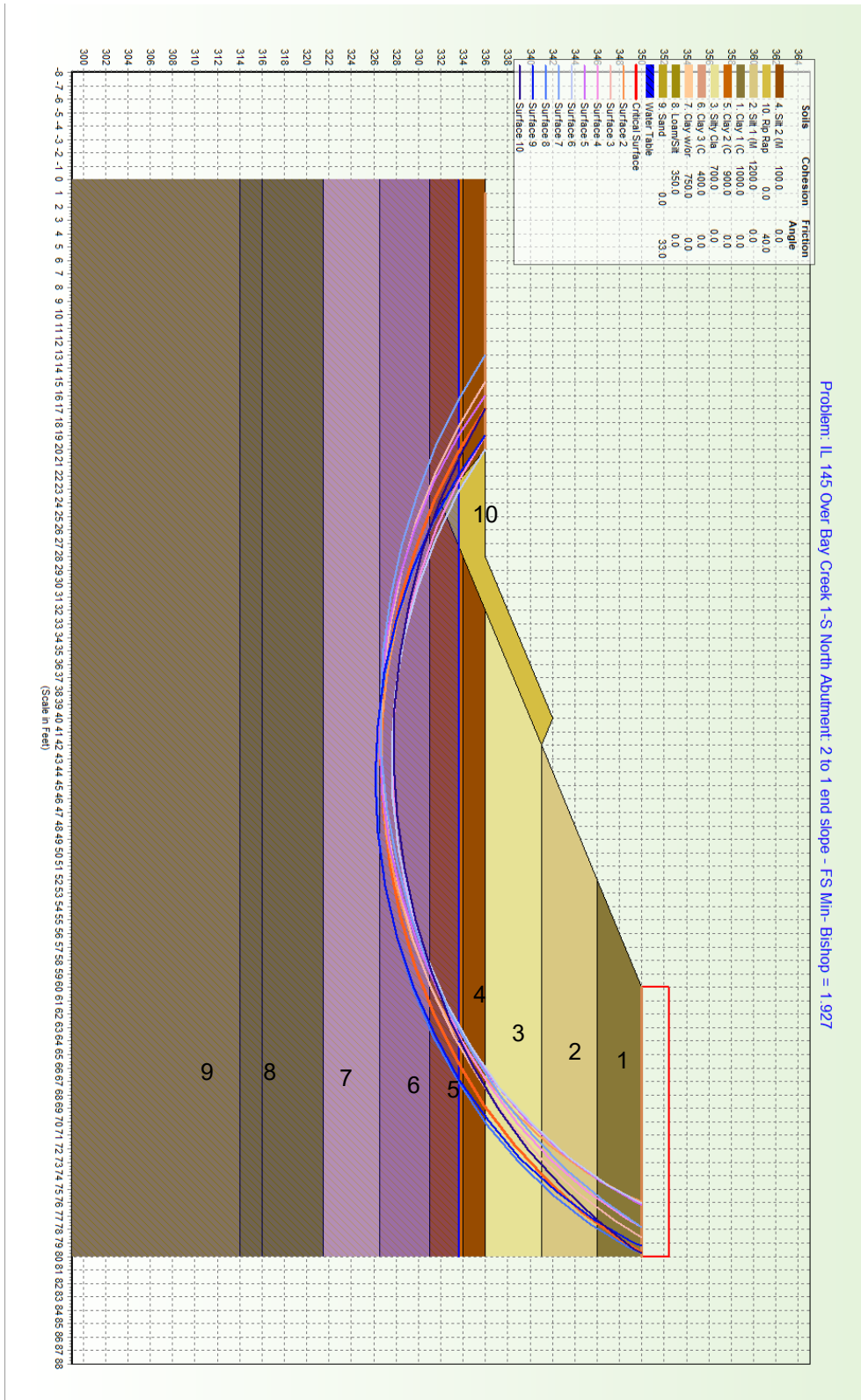
11 Executive Drive, Suite 12

Fairview Heights, Illinois 62208

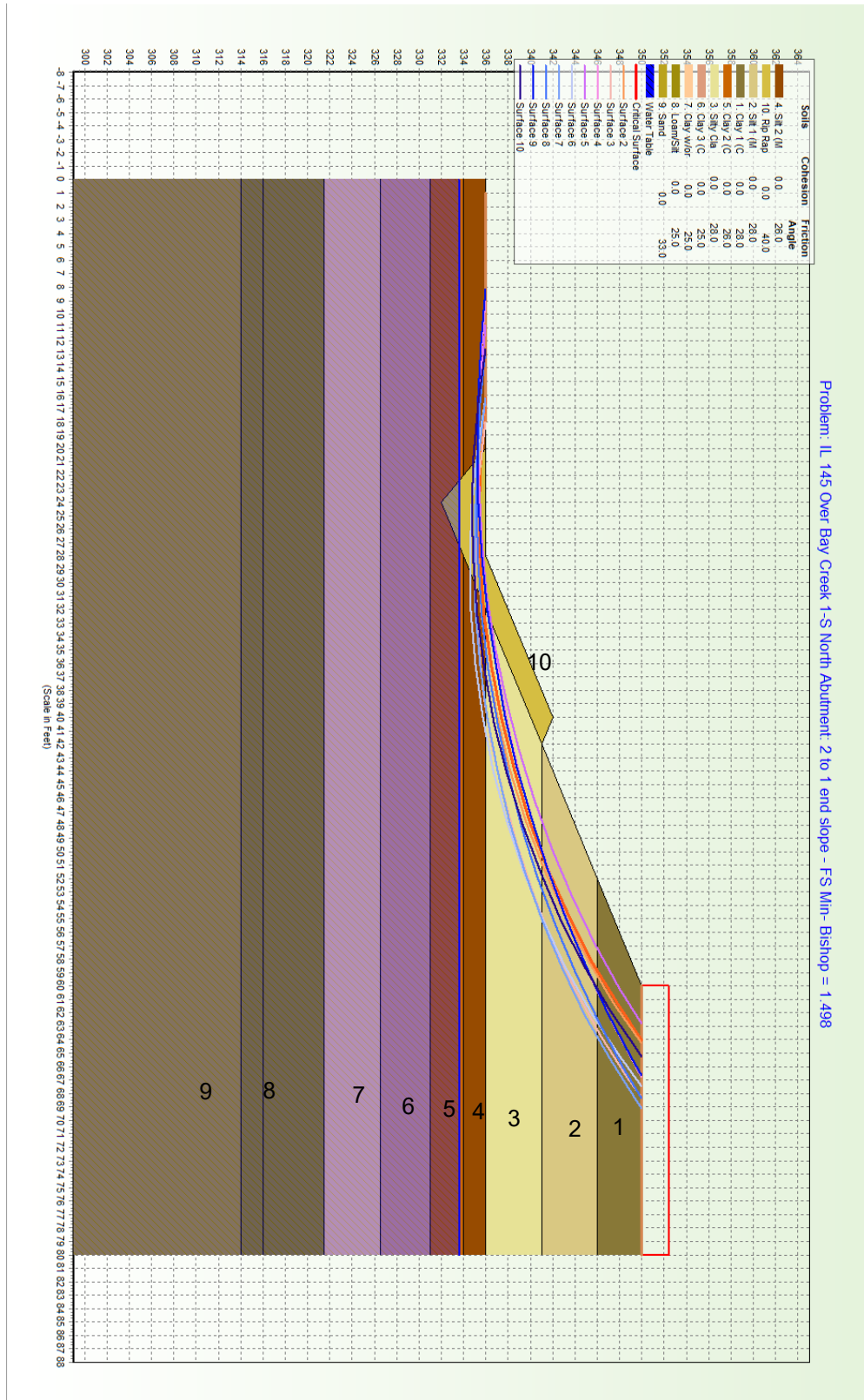
618-624-8610

Appendix C: Summary Stability Profiles

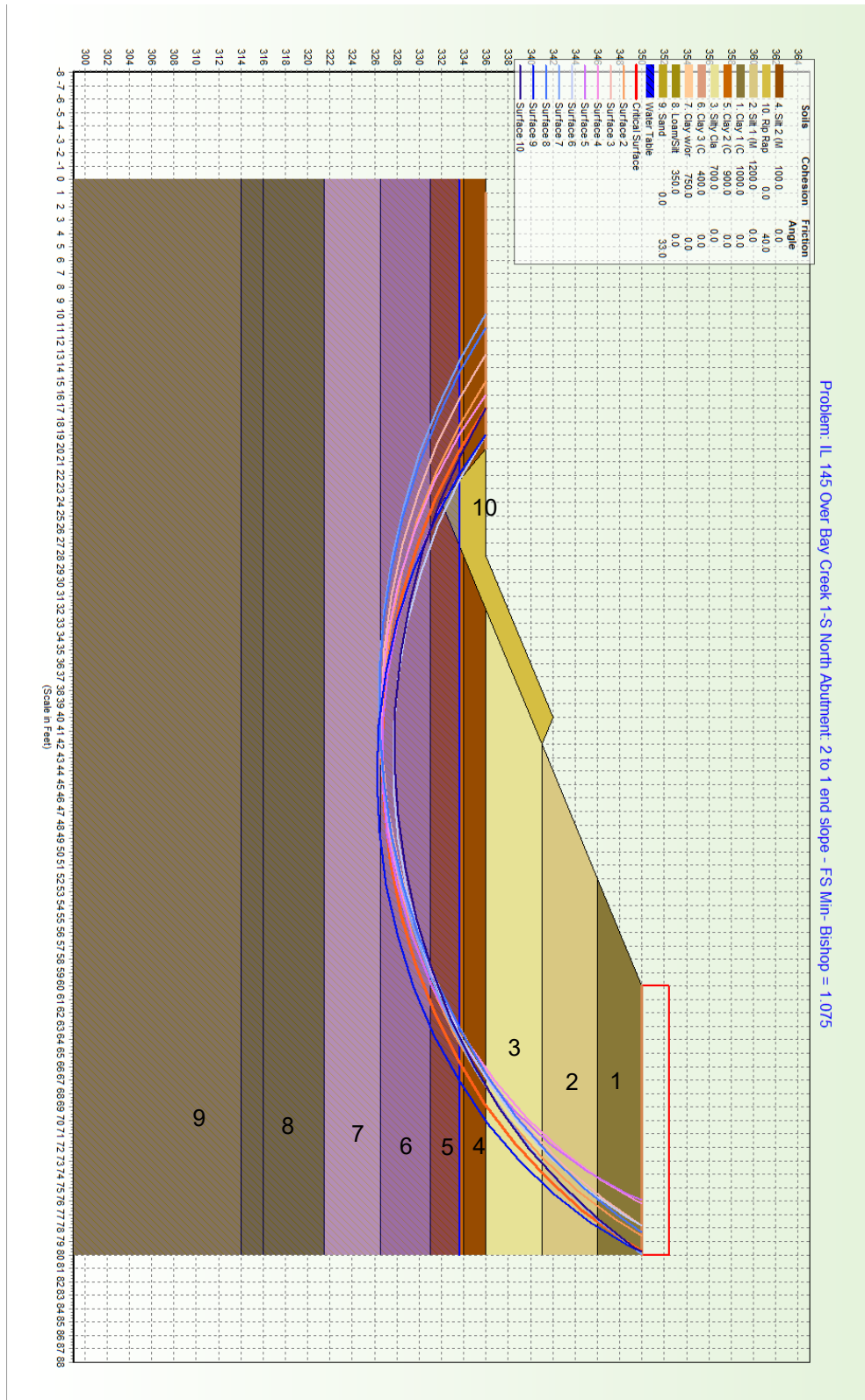
IL 145 over Bay Creek 1-S North Abutment- End Slope (undrained condition)



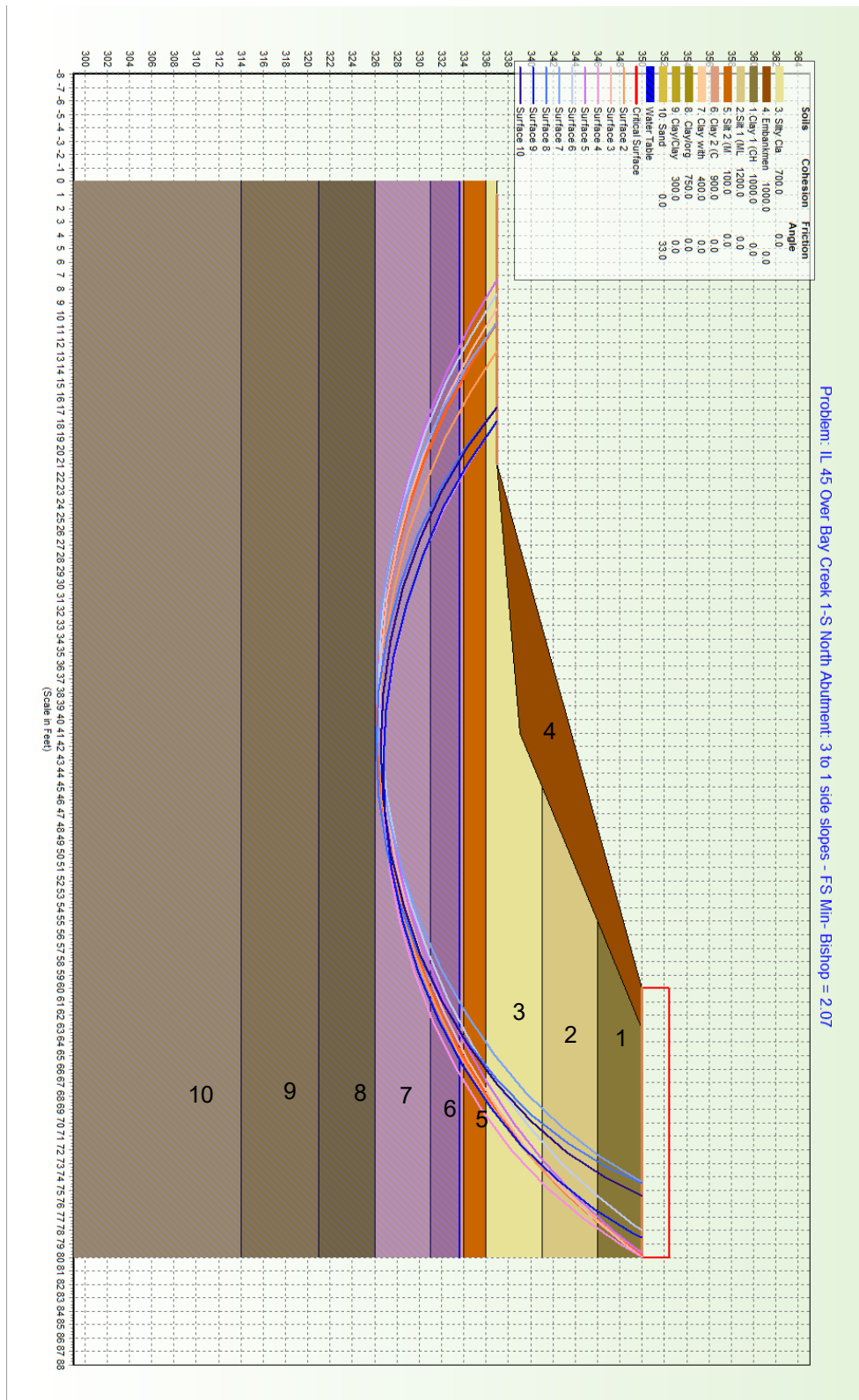
IL 145 over Bay Creek 1-S North Abutment- End Slope (drained condition)



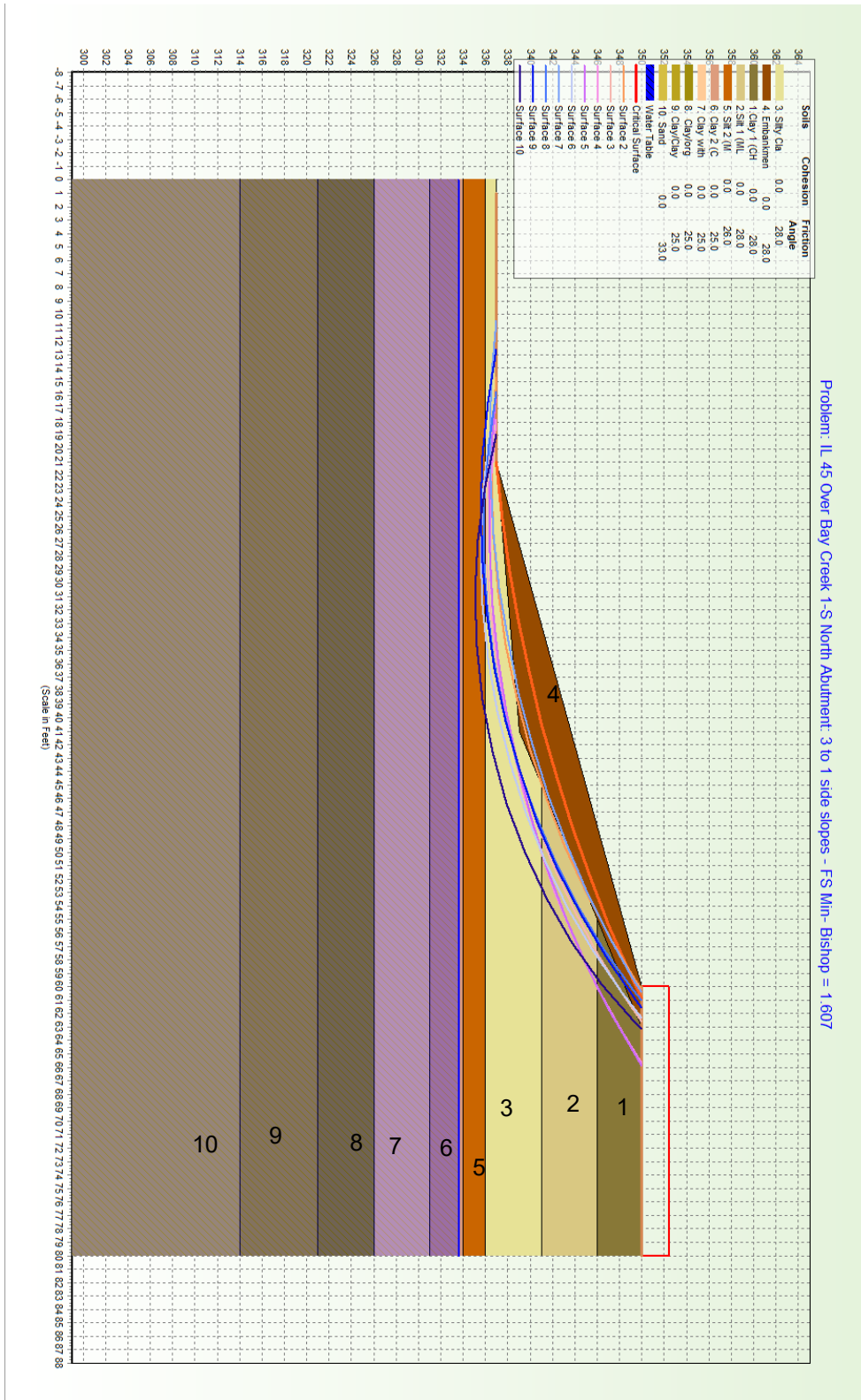
IL 145 over Bay Creek 1-S North Abutment- End Slope (seismic condition)



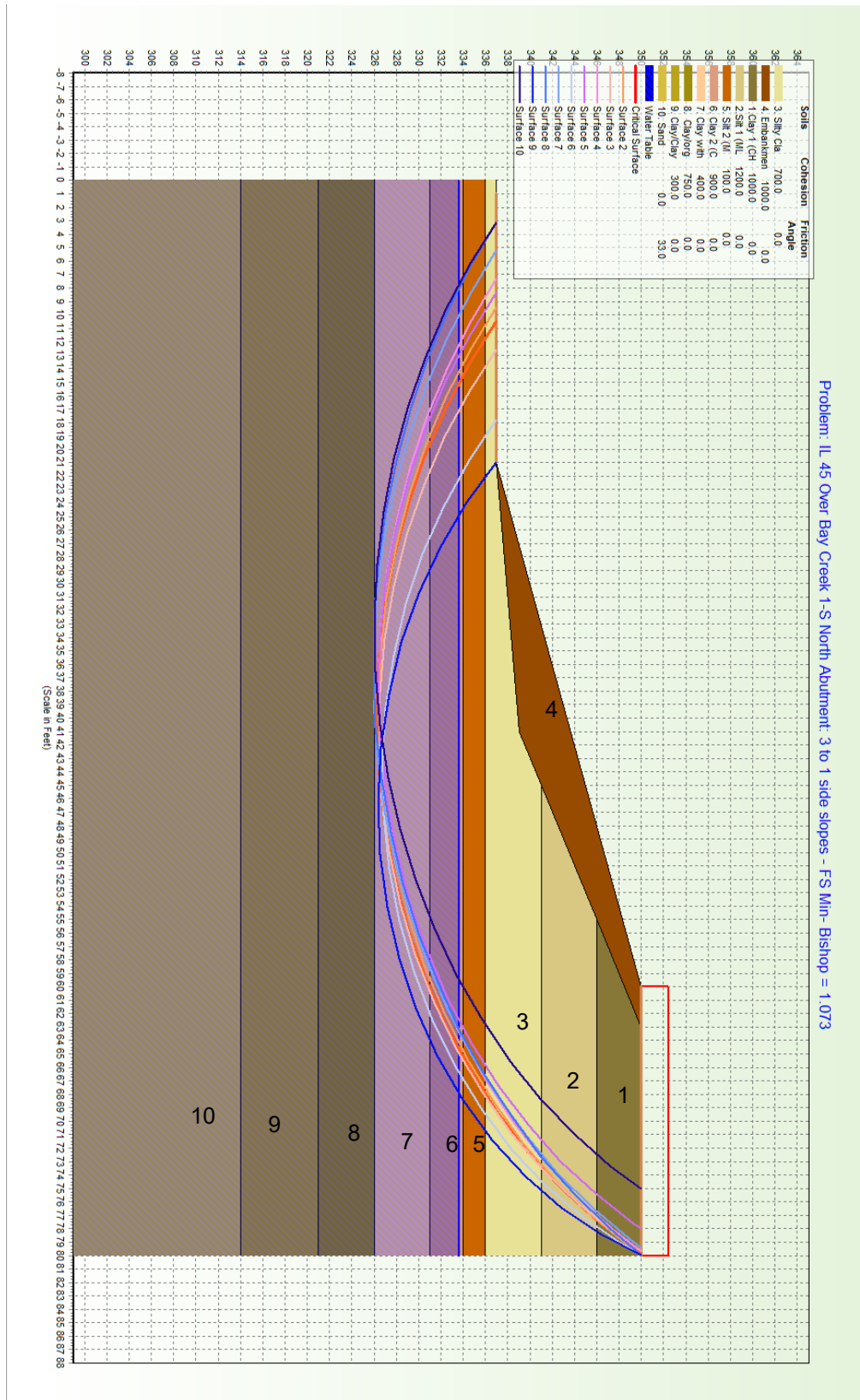
IL 145 over Bay Creek 1-S North Abutment – 3:1 Side Slope (undrained condition)



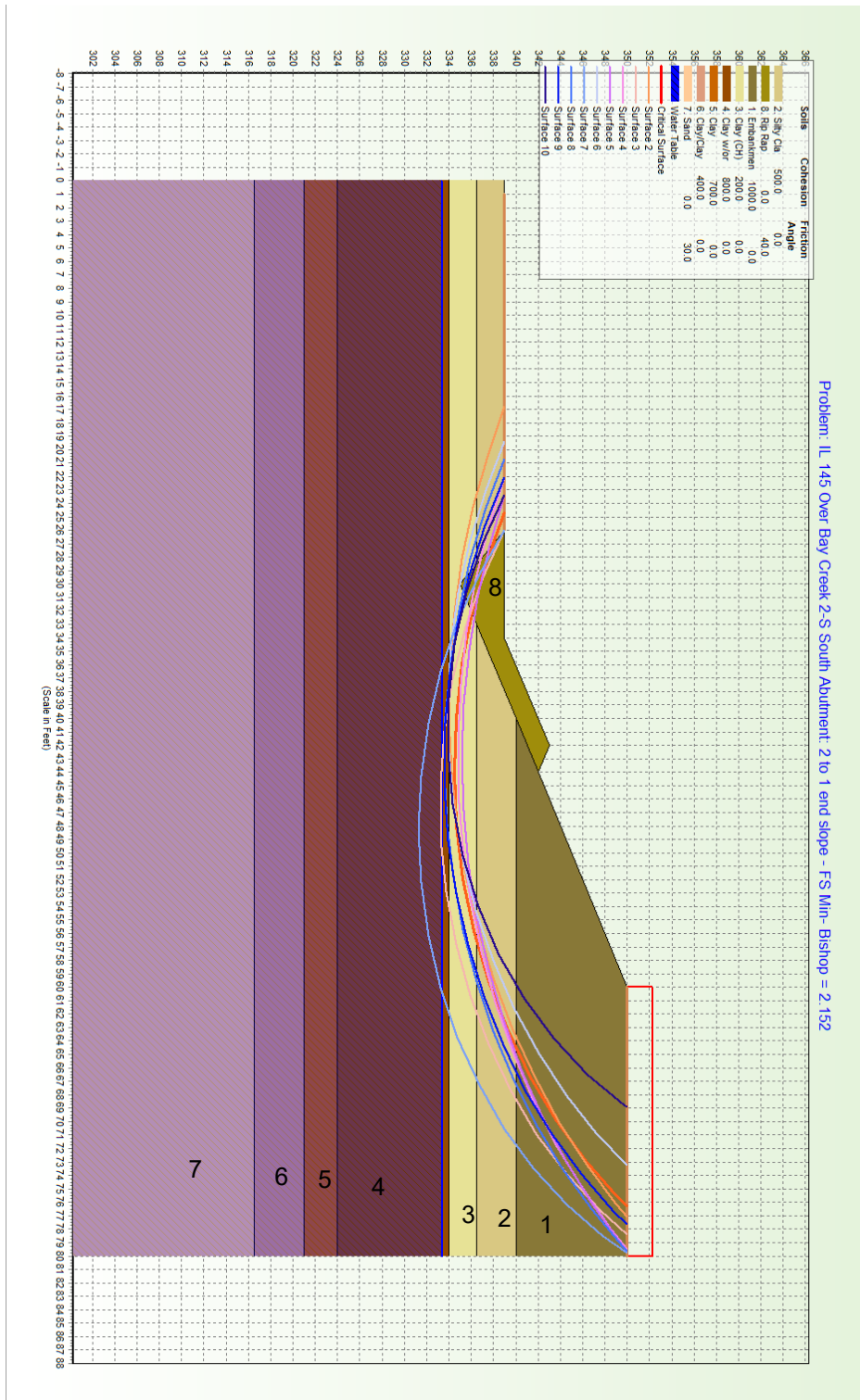
IL 145 over Bay Creek 1-S North Abutment – 3:1 Side Slope (drained condition)



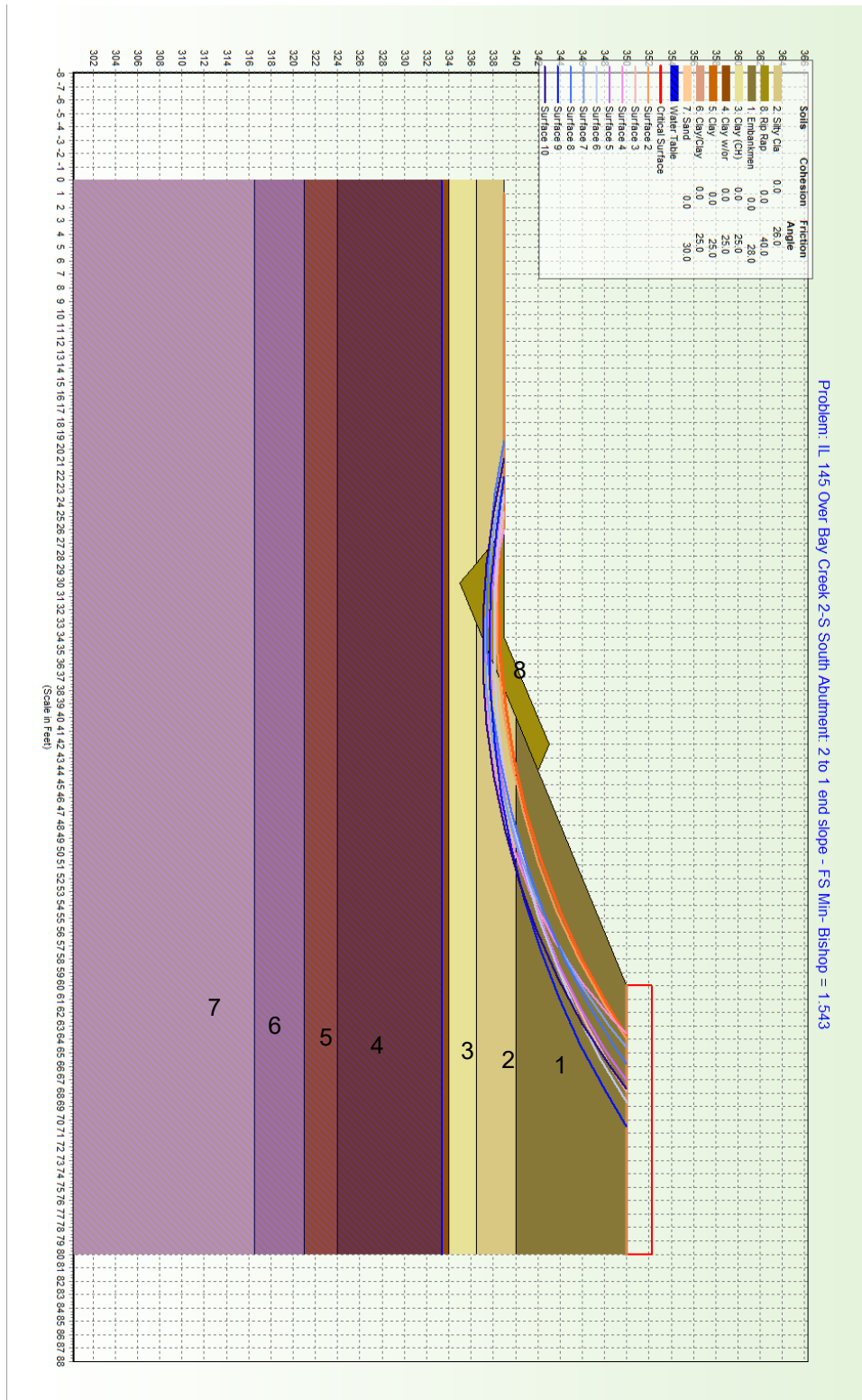
IL 145 over Bay Creek 1-S North Abutment – 3:1 Side Slope (seismic condition)



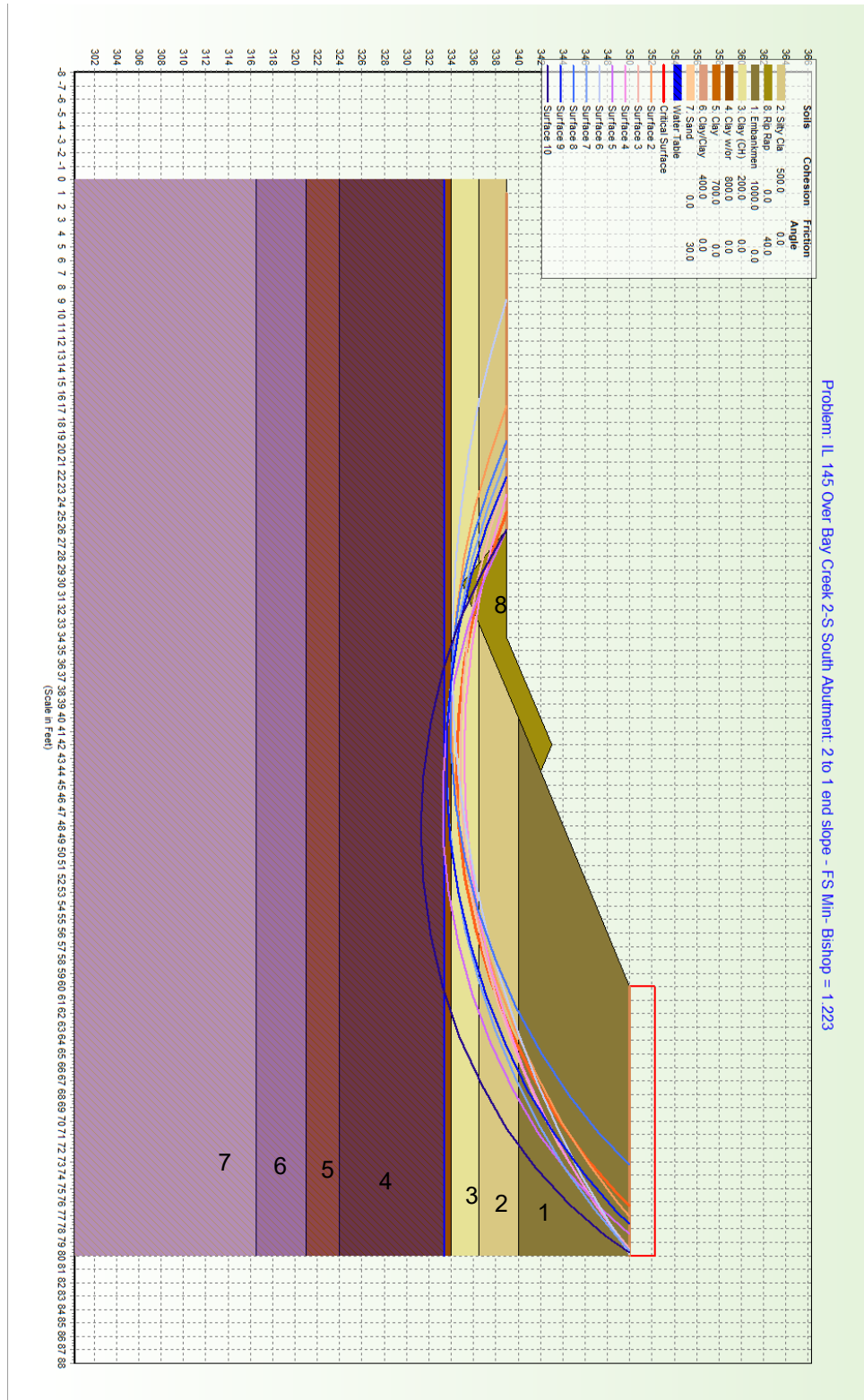
IL 145 over Bay Creek 2-S South Abutment- End Slope (undrained condition)



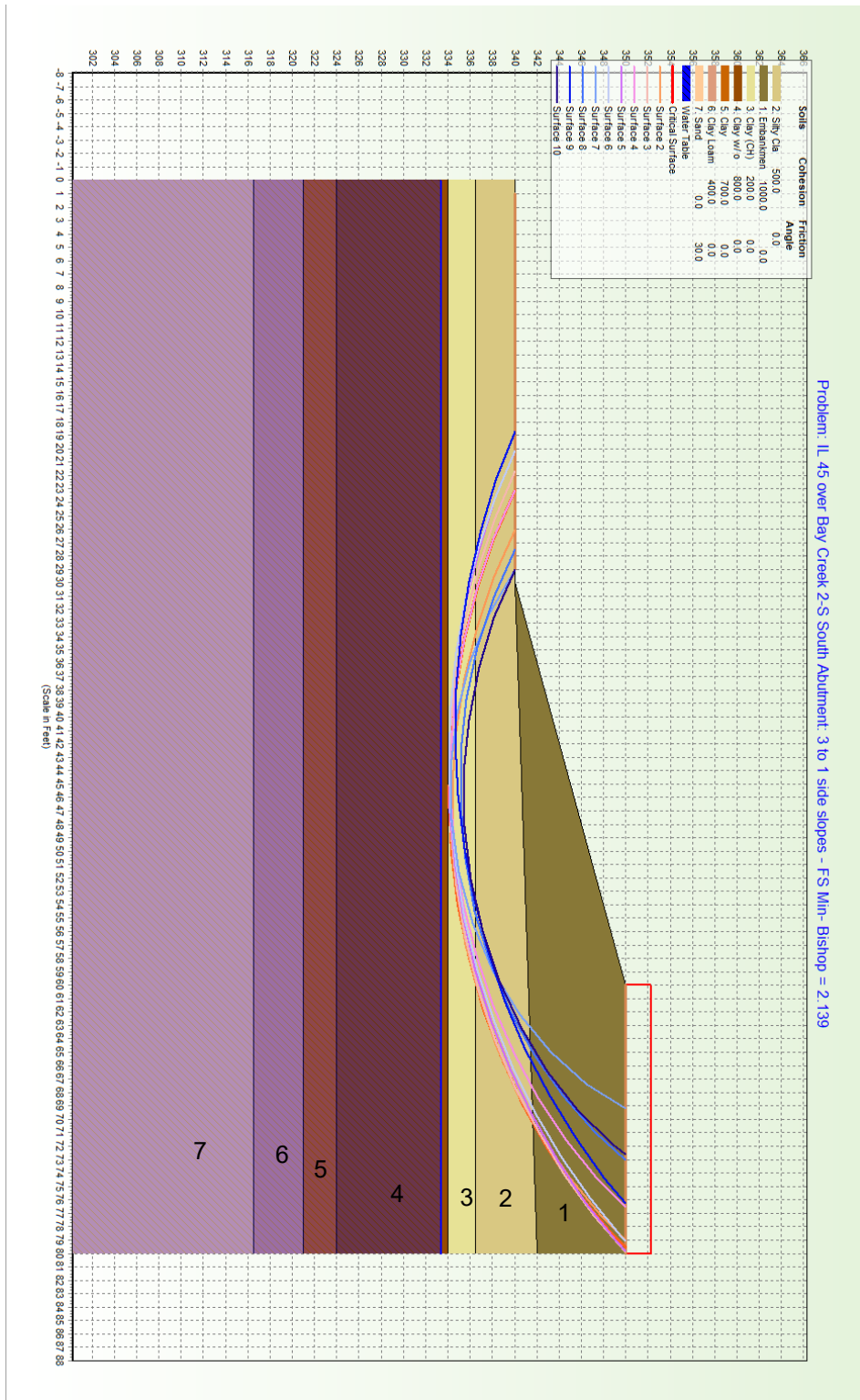
IL 145 over Bay Creek 2-S South Abutment- End Slope (drained condition)



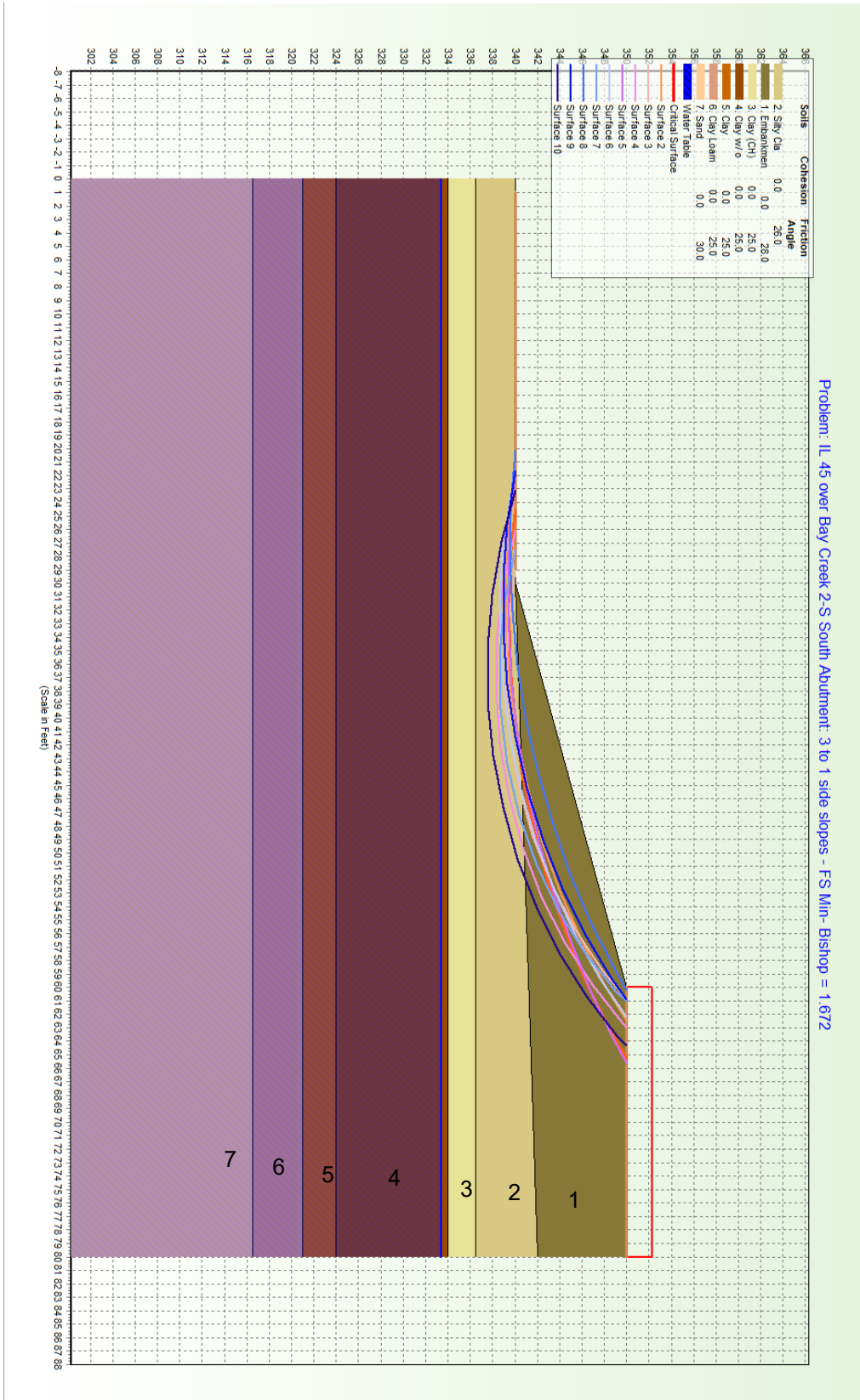
IL 145 over Bay Creek 2-S South Abutment- End Slope (seismic condition)



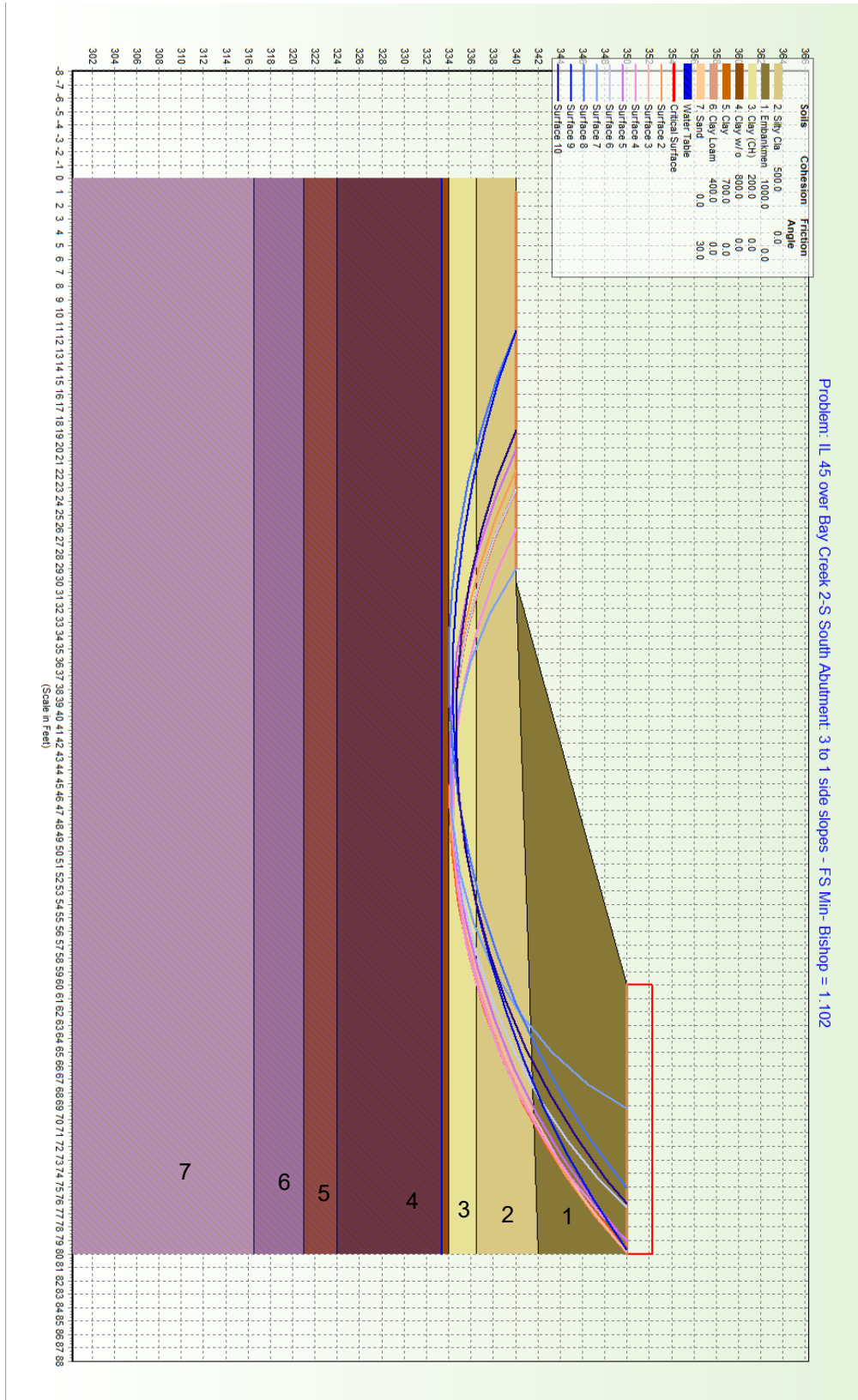
IL 145 over Bay Creek 2-S South Abutment 3:1 Side Slope (undrained condition)



IL 145 over Bay Creek 2-S South Abutment 3:1 Side Slope (drained condition)



IL 145 over Bay Creek 2-S South Abutment 3:1 Side Slope (seismic condition)





Mirrored

11 Executive Drive, Suite 12

Fairview Heights, Illinois 62208

618-624-8610

Appendix D: Seismic Site Class and Liquefaction Spreadsheets

SEISMIC SITE CLASS DETERMINATION

M.d.d. 12.10.10

R 145

R

Substructure 1

	342.88
	48
	1
	349.9
	318.88

Individual Site Class Definition:

8
13

Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick. (ft.)	N	Qu (tsf)	Layer Description Boundary
347.4	2.50	4	1.50		
344.9	2.50	4	1.50		
342.4	2.50	4	1.50		
341.4	1.00	4	1.50		
338.9	2.50	5	0.70		
336.4	2.50	5	0.70		
333.9	2.50	1	0.10		
331.4	2.50	1	0.90		
328.9	2.50	2	0.50		
326.4	2.50	1	0.30		
323.9	2.50	1	1.20		
321.4	2.50	1	0.70		
318.9	2.50	1	0.30		
316.4	2.50	1	0.40		
313.9	2.50	1	0.30		
311.4	2.50	6			
308.9	2.50	9			
306.4	2.50	9			
303.9	2.50	9			
301.4	2.50	9			
298.9	2.50	9			
296.4	2.50	9			
293.9	2.50	20			
291.4	2.50	20			
288.9	2.50	24			
286.4	2.50	24			
283.9	2.50	19			
281.4	2.50	19			
278.9	2.50	14			
276.4	2.50	14			
273.9	2.50	21			
271.4	2.50	21			
268.9	2.50	15			
266.4	2.50	15			
263.9	2.50	11			
261.4	2.50	11			
258.9	2.50	11			
256.4	2.50	11			
253.9	2.50	11			
251.4	2.50	14			
218.9	32.50	14			

Substructure 2

	342.88
	48
	2
	350.1
	318.88

Individual Site Class Definition:

10
16

Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick. (ft.)	N	Qu (tsf)	Layer Description Boundary
347.6	2.50	3	0.90		
345.1	2.50	3	0.90		
344.1	1.00	3	0.90		
341.6	2.50	3	0.60		
339.1	2.50	3	0.50		
336.6	2.50	3	0.40		R
334.1	2.50	2	0.20		
331.6	2.50	3	0.80		
329.1	2.50	1	0.60		
326.6	2.50	1	0.90		
324.1	2.50	1	0.80		
321.6	2.50	1	0.70		
319.1	2.50	1	0.40		
316.6	2.50	1	0.40		
314.1	2.50	1	0.20		
311.6	2.50	2			
309.1	2.50	24			
306.6	2.50	24			
304.1	2.50	22			
301.6	2.50	22			
299.1	2.50	24			
296.6	2.50	24			
294.1	2.50	11			
291.6	2.50	11			
289.1	2.50	16			
286.6	2.50	16			
284.1	2.50	22			
281.6	2.50	22			
279.1	2.50	14			
276.6	2.50	14			
274.1	2.50	16			
271.6	2.50	16			
269.1	2.50	15			
266.6	2.50	15			
264.1	2.50	15			
261.6	2.50	15			
259.1	2.50	15			
256.6	2.50	15			
254.1	2.50	16			
218.9	35.20	15			

Substructure 3

	336.3
	48
	3
	338.5
	312.3

Individual Site Class Definition:

16
16

Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick. (ft.)	N	Qu (tsf)	Layer Description Boundary
335.5	3.00	14	2.80		
333.0	2.50	3	1.00		
330.5	2.50	3	0.80		
328.0	2.50	2	0.40		
325.5	2.50	1	0.70		
323.0	2.50	1	0.30		
320.5	2.50	1	0.50		
318.0	2.50	1	0.50		
315.5	2.50	1	0.30		
313.0	2.50	2	0.30		
310.5	2.50	3			
308.0	2.50	2			
305.5	2.50	12			
303.0	2.50	5			
300.5	2.50	4			
298.0	2.50	20			
295.5	2.50	19			
293.0	2.50	19			
290.5	2.50	22			
288.0	2.50	22			
285.5	2.50	23			
283.0	2.50	23			
280.5	2.50	33			
278.0	2.50	33			
275.5	2.50	14			
273.0	2.50	14			
270.5	2.50	7			
268.0	2.50	7			
265.5	2.50	19			
263.0	2.50	19			
260.5	2.50	14			
258.0	2.50	14			
255.5	2.50	11			
253.0	2.50	11			
250.5	2.50	16			
248.0	2.50	16			
245.5	2.50	16			
243.0	2.50	16			
240.5	2.50	14			
238.0	2.50	14			
212.3	25.70	23			

Substructure 4

	339.3
	48
	4
	339.5
	315.3

Individual Site Class Definition:

10
15

Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick. (ft.)	N	Qu (tsf)	Layer Description Boundary
336.5	3.00	5	0.90		
334.0	2.50	1	0.30		
331.5	2.50	1	0.30		
329.0	2.50	1	0.40		
326.5	2.50	1	0.40		
324.0	2.50	1	0.50		
321.5	2.50	1	0.30		
319.0	2.50	1	0.30		
316.5	2.50	1	0.30		
314.0	2.50	1	0.10		
311.5	2.50	1	0.30		
309.0	2.50	1			
306.5	2.50	9			
304.0	2.50	15			
301.5	2.50	12			
299.0	2.50	12			
296.5	2.50	17			
294.0	2.50	20			
291.5	2.50	26			
289.0	2.50	26			
286.5	2.50	21			
284.0	2.50	21			
281.5	2.50	19			
279.0	2.50	19			
276.5	2.50	18			
274.0	2.50	18			
271.5	2.50	12			
269.0	2.50	12			
266.5	2.50	17			
264.0	2.50	17			
261.5	2.50	11			
259.0	2.50	11			
256.5	2.50	15			
254.0	2.50	15			
251.5	2.50	12			
249.0	2.00	12			
215.3	34.20	12			

Substructure 5

	342.88
	48
	1
	349.9
	318.88

Individual Site Class Definition:

8
13

Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick. (ft.)	N	Qu (tsf)	Layer Description Boundary
347.4	2.50	4	1.50		
344.9	2.50	4	1.50		
342.4	2.50	4	1.50		
341.4	1.00	4	1.50		
338.9	2.50	5	0.70		
336.4	2.50	5	0.70		
333.9	2.50	1	0.10		
331.4	2.50	1	0.90		
328.9	2.50	2	0.50		
326.4	2.50	1	0.30		
323.9	2.50	1	1.20		
321.4	2.50	1	0.70		
318.9	2.50	1	0.30		
316.4	2.50	1	0.40		
313.9	2.50	1	0.30		
311.4	2.50	6			
308.9	2.50	9			
306.4	2.50	9			
303.9	2.50	9			
301.4	2.50	9			
298.9	2.50	9			
296.4	2.50	9			
293.9	2.50	20			
291.4	2.50	20			
288.9	2.50	24			
286.4	2.50	24			
283.9	2.50	19			
281.4	2.50	19			
278.9	2.50	14			
276.4	2.50	14			
273.9	2.50	21			
271.4	2.50	21			
268.9	2.50	15			
266.4	2.50	15			
263.9	2.50	11			
261.4	2.50	11			
258.9	2.50	11			
256.4	2.50	11			
253.9	2.50	11			
251.4	2.50	14			
218.9	32.50	14			

Global Site Class Definition: Substructures 1 through 4

11
15

R 3
 338.50
 23.50
 5.00
 0.481
 7.5
 0.00
 75
 8

EQ MAGNITUDE SCALING FACTOR
 M 1.000

AVG. SHEAR WAVE VELOCITY (top 40')
 V₄₀ 262

PGA CALCULATOR
 M 7.5
 V₄₀ 30
 PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA							CONDITIONS DURING DRILLING					CONDITIONS DURING EARTHQUAKE							
	BORING DEPTH (FT.)	SPT N VALUE (BLOWS)	UNCONF. COMPR. STR., Q _u (TSF.)	% FINES < #200 (%)	PLAST. INDEX PI	LIQUID LIMIT LL	MOIST. CONTENT w _c (%)	EFFECTIVE UNIT WT. (KCF.)	CORR. VERT. STRESS (KSF.)	EQUIV. CLN. SPT N VALUE (N ₁) ₆₀	SAND SPT N VALUE (N ₁) _{60cs}	CRR RESIST. MAG 7.5 CRR 7.5	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	TOTAL VERT. STRESS (KSF.)	OVER-BURDEN CORR. FACT. (Ks)	CORR. RESIST. CRR 7.5 CRR	SOIL MASS PART. FACTOR (r _d)	EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR
	336	2.5	14	2.8				20	0.134	0.335	29.998	29.998	0.467	0.134	0.335	0.335	1.500	0.701	0.925	0.289
333.5	5	3	1				39	0.122	0.640	5.212	5.212	0.074	0.122	0.640	0.640	1.273	0.094	0.849	0.266	N.L. (1)
331	7.5	3	0.8		12	35	57	0.119	0.938	4.797	4.797	0.071	0.057	0.783	0.939	1.221	0.086	0.775	0.291	N.L. (2)
328.5	10	2	0.4		12	35	52	0.111	1.215	3.198	3.198	0.060	0.049	0.905	1.217	1.186	0.071	0.706	0.297	N.L. (2)
326	12.5	1	0.7		12	35	106	0.117	1.508	1.565	1.565	0.051	0.055	1.043	1.511	1.153	0.059	0.642	0.291	N.L. (2)
323.5	15	1	0.3		12	35	132	0.108	1.778	1.524	1.524	0.051	0.046	1.158	1.782	1.129	0.057	0.585	0.281	N.L. (2)
321	17.5	1	0.5		20	41	53	0.114	2.063	1.468	1.468	0.051	0.052	1.288	2.068	1.105	0.056	0.535	0.269	N.L. (2)
318.5	20	1	0.5		20	41	33	0.114	2.348	1.409	1.409	0.050	0.052	1.418	2.354	1.084	0.055	0.494	0.256	N.L. (2)
316	22.5	1	0.3		12	30	28	0.108	2.618	1.353	1.353	0.050	0.046	1.533	2.625	1.067	0.054	0.459	0.246	N.L. (2)
313.5	25	2	0.3		7	23	27	0.046	2.733	2.672	2.672	0.056	0.046	1.648	2.896	1.052	0.059	0.430	0.236	0.250 (C)
311	27.5	3						0.051	2.860	3.941	3.941	0.064	0.051	1.775	3.179	1.036	0.067	0.407	0.228	0.294 (C)
308.5	30	3	0.3		8	23	17	0.046	2.975	3.879	3.879	0.064	0.046	1.890	3.450	1.023	0.066	0.388	0.222	N.L. (2)
306	32.5	12						0.063	3.133	15.147	15.147	0.162	0.063	2.048	3.764	1.009	0.163	0.373	0.215	0.758 (D)
303.5	35	5						0.055	3.270	6.184	6.184	0.081	0.055	2.185	4.057	0.994	0.081	0.362	0.210	0.386 (C)
301	37.5	4						0.053	3.403	4.853	4.853	0.071	0.053	2.318	4.346	0.982	0.070	0.353	0.207	0.338 (C)
298.5	40	20						0.067	3.570	24.802	24.802	0.288	0.067	2.485	4.669	0.950	0.274	0.346	0.203	1.350 (D)
293.5	45	19						0.067	3.905	22.168	22.168	0.244	0.067	2.820	5.316	0.916	0.224	0.336	0.198	1.131 (D)
288.5	50	22						0.068	4.245	24.794	24.794	0.288	0.068	3.160	5.968	0.879	0.253	0.330	0.195	1.297 (D)
283.5	55	23						0.068	4.585	24.756	24.756	0.287	0.068	3.500	6.620	0.850	0.244	0.326	0.193	1.264 (D)
278.5	60	33						0.072	4.945	35.498	35.498	0.409	0.072	3.860	7.292	0.791	-0.324	0.324	0.191	N.L. (3)
273.5	65	14						0.064	5.265	13.187	13.187	0.142	0.064	4.180	7.924	0.843	0.120	0.321	0.190	0.632 (D)
268.5	70	7						0.058	5.555	6.358	6.358	0.083	0.058	4.470	8.526	0.856	0.071	0.314	0.187	0.380 (C)
263.5	75	19						0.067	5.890	16.658	16.658	0.177	0.067	4.805	9.173	0.800	0.142	0.307	0.183	0.776 (D)
258.5	80	14						0.064	6.210	11.792	11.792	0.129	0.064	5.125	9.805	0.807	0.104	0.300	0.179	0.581 (C)
253.5	85	11						0.062	6.520	8.950	8.950	0.104	0.062	5.435	10.427	0.809	0.084	0.293	0.176	0.477 (C)
248.5	90	16						0.065	6.845	12.568	12.568	0.136	0.065	5.760	11.064	0.781	0.107	0.286	0.172	0.622 (C)
243.5	95	16						0.065	7.170	12.147	12.147	0.133	0.065	6.085	11.701	0.772	0.102	0.279	0.168	0.607 (C)
238.5	100	14						0.064	7.490	10.086	10.086	0.114	0.064	6.405	12.333	0.774	0.088	0.272	0.164	0.537 (C)
233.5	105	23						0.068	7.830	15.237	15.237	0.162	0.068	6.745	12.985	0.737	0.120	0.265	0.159	0.755 (D)
228.5	110	75	5				20	0.079	8.225	49.954	49.954	0.303	0.079	7.140	13.692	0.615	0.186	0.258	0.154	N.L. (3)
225	113.5	50	5					0.079	8.502	27.568	27.568	0.355	0.079	7.417	14.187	0.652	0.232	0.253	0.151	N.L. (3)

1
 2
 3
 R
 D
 0.85
 25
 D

R 4
 339.50
 D 25.00
 D 5.00
 0.481
 7.5
 0.00
 75
 8

EQ MAGNITUDE SCALING FACTOR
 M 1.000

AVG. SHEAR WAVE VELOCITY (top 40')
 V₄₀ 215

PGA CALCULATOR
 M 7.5
 R 30
 M 0.535
 PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA							CONDITIONS DURING DRILLING					CONDITIONS DURING EARTHQUAKE							
	BORING SAMPLE DEPTH (FT.)	SPT N VALUE (BLOWS)	UNCONF. COMPR. STR., Q _u (TSF.)	% FINES < #200 (%)	PLAST. INDEX PI	LIQUID LIMIT LL	MOIST. CONTENT w _c (%)	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	CORR. SPT N VALUE (N ₁) ₆₀	EQUIV. CLN. SAND SPT N VALUE (N ₁) _{60cs}	CRR RESIST. MAG 7.5 CRR 7.5	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	TOTAL STRESS (KSF.)	OVER-BURDEN CORR. FACT. (Ks)	CORR. RESIST. CRR 7.5 CRR	SOIL MASS PART. FACTOR (r _d)	EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR
	337	2.5	5	0.9				21	0.120	0.300	9.724	9.724	0.111	0.120	0.300	0.300	1.500	0.166	0.907	0.284
334.5	5	1	0.1				25	0.098	0.545	1.791	1.791	0.052	0.098	0.545	0.545	1.312	0.068	0.818	0.256	N.L. (1)
332	7.5	1	0.3		12	35	27	0.108	0.815	1.657	1.657	0.051	0.046	0.660	0.816	1.263	0.065	0.735	0.284	N.L. (2)
329.5	10	1	0.4		12	35	29	0.111	1.093	1.653	1.653	0.051	0.049	0.783	1.095	1.221	0.063	0.659	0.288	N.L. (2)
327	12.5	1	0.4		20	40	71	0.111	1.370	1.620	1.620	0.051	0.049	0.905	1.373	1.186	0.061	0.593	0.281	N.L. (2)
324.5	15	1	0.5		20	40	35	0.114	1.655	1.569	1.569	0.051	0.052	1.035	1.659	1.154	0.059	0.536	0.269	N.L. (2)
322	17.5	1	0.3		12	35	28	0.108	1.925	1.514	1.514	0.051	0.046	1.150	1.930	1.130	0.057	0.489	0.256	N.L. (2)
319.5	20	1	0.3		12	31	26	0.108	2.195	1.454	1.454	0.051	0.046	1.265	2.201	1.109	0.056	0.449	0.244	N.L. (2)
317	22.5	1	0.3		12	31	26	0.108	2.465	1.394	1.394	0.050	0.046	1.380	2.472	1.090	0.055	0.417	0.234	N.L. (2)
314.5	25	1	0.1		12	31	29	0.098	2.710	1.342	1.342	0.050	0.160	1.780	3.028	1.036	0.052	0.391	0.208	N.L. (2)
312	27.5	1	0.1		12	31	29	0.035	2.798	1.329	1.329	0.050	0.035	1.868	3.272	1.026	0.051	0.370	0.203	N.L. (2)
309.5	30	1						0.043	2.905	1.309	1.309	0.050	0.043	1.975	3.535	1.014	0.051	0.354	0.198	0.258 (C)
307	32.5	9						0.060	3.055	1.1518	1.1518	0.127	0.060	2.125	3.841	0.999	0.127	0.341	0.193	0.658 (D)
304.5	35	15						0.065	3.218	1.9086	1.9086	0.204	0.065	2.288	4.160	0.978	0.200	0.331	0.188	1.064 (D)
299.5	40	12						0.063	3.533	14.290	14.290	0.153	0.063	2.603	4.787	0.948	0.145	0.317	0.182	0.797 (D)
297	42.5	17						0.066	3.698	20.252	20.252	0.219	0.066	2.768	5.108	0.924	0.202	0.312	0.180	1.122 (D)
294.5	45	20						0.067	3.865	23.648	23.648	0.267	0.067	2.935	5.431	0.902	0.241	0.308	0.178	1.354 (D)
289.5	50	26						0.069	4.210	30.181	30.181	0.481	0.069	3.280	6.088	0.855	0.411	0.303	0.176	N.L. (3)
284.5	55	21						0.068	4.550	22.470	22.470	0.249	0.068	3.620	6.740	0.847	0.211	0.300	0.175	1.206 (D)
279.5	60	19						0.067	4.885	19.130	19.130	0.205	0.067	3.955	7.387	0.835	0.171	0.298	0.174	0.983 (D)
274.5	65	18						0.066	5.215	17.191	17.191	0.183	0.066	4.285	8.029	0.823	0.151	0.296	0.173	0.873 (D)
269.5	70	12						0.063	5.530	10.934	10.934	0.121	0.063	4.600	8.656	0.832	0.101	0.289	0.170	0.594 (C)
264.5	75	17						0.066	5.860	14.890	14.890	0.159	0.066	4.930	9.298	0.802	0.127	0.282	0.166	0.765 (D)
259.5	80	11						0.062	6.170	9.308	9.308	0.107	0.062	5.240	9.920	0.814	0.087	0.275	0.162	0.537 (C)
254.5	85	15						0.065	6.495	12.239	12.239	0.133	0.065	5.565	10.557	0.789	0.105	0.268	0.159	0.660 (C)
249.5	90	12						0.063	6.810	9.461	9.461	0.108	0.063	5.880	11.184	0.792	0.086	0.261	0.155	0.555 (C)

R 0.85
 2 12 R 0.85
 3 25
 D

R 201979
 339.00
 4.00
 4.00
 0.481
 7.5
 0.00
 60
 8
 0.00

EQ MAGNITUDE SCALING FACTOR
M = 1.000

AVG. SHEAR WAVE VELOCITY (top 40')
V_{s40} = 369

PGA CALCULATOR
 M = 7.5
 V_{s40} = 369
 PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA							CONDITIONS DURING DRILLING					CONDITIONS DURING EARTHQUAKE					CORR. RESIST. CRR	SOIL MASS PART. FACTOR (r _d)	EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR
	BORING SAMPLE DEPTH (FT.)	SPT N VALUE (BLOWS)	UNCONF. COMPR. STR., Q _u (TSF.)	% FINES < #200 (%)	PLAST. INDEX PI	LIQUID LIMIT LL	MOIST. CONTENT w _c (%)	EFFECTIVE UNIT WT. (KCF.)	EFFECTIVE VERT. STRESS (KSF.)	CORR. SPT N VALUE (N ₁) ₆₀	EQUIV. CLN. SAND SPT (N ₁) _{60cs}	CRR MAG 7.5 CRR _{7.5}	EFFECTIVE UNIT WT. (KCF.)	EFFECTIVE VERT. STRESS (KSF.)	TOTAL VERT. STRESS (KSF.)	OVER-BURDEN CORR. FACT. (Ks)					
335.5	3.5	8	1.10		15	41	29	0.123	0.431	12.176	12.176	0.133	0.123	0.431	0.431	1.478	0.196	0.943	0.295	N.L. (1)	
333	6	5	0.60		15	41	50	0.053	0.563	7.121	7.121	0.089	0.053	0.563	0.688	1.327	0.118	0.897	0.343	N.L. (2)	
330.5	8.5	4	0.70		15	40	52	0.055	0.701	5.681	5.681	0.077	0.055	0.701	0.981	1.254	0.097	0.848	0.371	N.L. (2)	
328	11	3	0.20		15	35	81	0.042	0.806	4.410	4.410	0.068	0.042	0.806	1.242	1.214	0.082	0.796	0.384	N.L. (2)	
325.5	13.5	3	0.30		15	35	65	0.046	0.921	4.468	4.468	0.068	0.046	0.921	1.513	1.182	0.081	0.744	0.382	N.L. (2)	
323	16	5	0.50		11	40	27	0.051	1.048	7.422	7.422	0.091	0.051	1.048	1.797	1.164	0.106	0.693	0.371	N.L. (2)	
320.5	18.5	3	0.50		11	40	27	0.051	1.176	4.401	4.401	0.068	0.051	1.176	2.080	1.125	0.076	0.645	0.357	N.L. (2)	
318	21	3	0.40		12	35	27	0.049	1.298	4.331	4.331	0.067	0.049	1.298	2.359	1.103	0.074	0.601	0.341	N.L. (2)	
315.5	23.5	2	0.20		12	35	30	0.042	1.403	2.843	2.843	0.057	0.042	1.403	2.620	1.086	0.062	0.562	0.328	N.L. (2)	
313	26	3	0.50		12	35	29	0.051	1.531	4.165	4.165	0.066	0.051	1.531	2.903	1.067	0.070	0.528	0.313	N.L. (2)	
310.5	28.5	4	0.70		12	35	30	0.055	1.668	5.405	5.405	0.075	0.055	1.668	3.197	1.050	0.079	0.499	0.299	N.L. (2)	
308	31	3						0.051	1.796	3.953	3.953	0.065	0.051	1.796	3.480	1.034	0.067	0.475	0.288	0.233 (C)	
305.5	33.5	7						0.058	1.941	8.961	8.961	0.104	0.058	1.941	3.781	1.020	0.106	0.456	0.278	0.381 (C)	
303	36	19						0.067	2.108	25.462	25.462	0.301	0.067	2.108	4.105	1.002	0.302	0.440	0.268	N.L. (3)	
300.5	38.5	25						0.069	2.281	33.800	33.800	5.241	0.069	2.281	4.433	0.973	5.098	0.427	0.260	N.L. (3)	
298	41	23						0.068	2.451	29.474	29.474	0.435	0.068	2.451	4.759	0.950	0.413	0.417	0.253	N.L. (3)	
295.5	43.5	27						0.070	2.626	34.319	34.319	2.887	0.070	2.626	5.090	0.921	-2.660	0.409	0.248	N.L. (3)	
293	46	31						0.071	2.803	39.019	39.019	0.085	0.071	2.803	5.424	0.894	0.076	0.403	0.244	N.L. (3)	
289	50	29						0.071	3.087	33.963	33.963	27.189	0.071	3.087	5.957	0.867	23.560	0.395	0.239	N.L. (3)	
284	55	39						0.073	3.452	45.117	45.117	0.239	0.073	3.452	6.634	0.823	0.197	0.390	0.234	N.L. (3)	

12 R 0.85
 3 1.80 25
 ID

R 31979
 350.00
 8.00
 8.00
 0.481
 7.5
 0.00
 60
 8
 8

EQ MAGNITUDE SCALING FACTOR
M 1.000

AVG. SHEAR WAVE VELOCITY (top 40')
V_{s40} 366

PGA CALCULATOR
 M 7.5
 D 30
 PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA							CONDITIONS DURING DRILLING					CONDITIONS DURING EARTHQUAKE					CORR. RESIST. CRR 7.5	SOIL MASS PART. FACTOR (r _d)	EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR
	BORING SAMPLE DEPTH (FT.)	SPT N (BLOWS)	UNCONF. COMPR. STR., Q _u (TSF.)	% FINES < #200 (%)	PLAST. INDEX PI	LIQUID LIMIT LL	MOIST. CONTENT w _c (%)	EFFECTIVE UNIT WT. (KCF.)	EFFECTIVE VERT. STRESS (KSF.)	CORR. SPT N (N ₁) ₆₀	EQUIV. CLN. SAND SPT (N _s) _{60cs}	CRR MAG 7.5 CRR 7.5	EFFECTIVE UNIT WT. (KCF.)	EFFECTIVE VERT. STRESS (KSF.)	TOTAL VERT. STRESS (KSF.)	OVER-BURDEN CORR. FACT. (Ks)					
346.5	3.5	4	1.10		8	32	31	0.123	0.431	5.951	5.951	0.079	0.123	0.431	0.431	1.389	0.110	0.942	0.294	N.L. (1)	
344	6	2	1.20		8	32	29	0.124	0.741	2.694	2.694	0.057	0.124	0.741	0.741	1.234	0.070	0.895	0.280	N.L. (1)	
341.5	8.5	6	1.60		12	35	22	0.065	0.903	8.020	8.020	0.096	0.065	0.903	0.934	1.206	0.116	0.844	0.273	N.L. (2)	
339	11	5	0.80		12	35	39	0.057	1.046	6.859	6.859	0.087	0.057	1.046	1.233	1.162	0.101	0.792	0.292	N.L. (2)	
336.5	13.5	3	0.70		12	35	28	0.055	1.183	4.153	4.153	0.066	0.055	1.183	1.526	1.124	0.074	0.739	0.298	N.L. (2)	
334	16	2	0.40		8	38	32	0.049	1.306	2.770	2.770	0.057	0.049	1.306	1.805	1.102	0.063	0.688	0.297	N.L. (2)	
331.5	18.5	3	0.70		30	50	63	0.055	1.443	4.106	4.106	0.066	0.055	1.443	2.098	1.080	0.071	0.640	0.291	N.L. (2)	
329	21	4	0.80		30	50	50	0.057	1.586	5.372	5.372	0.075	0.057	1.586	2.397	1.061	0.079	0.596	0.282	N.L. (2)	
326.5	23.5	3	0.80		30	50	79	0.057	1.728	3.940	3.940	0.064	0.057	1.728	2.695	1.042	0.067	0.557	0.272	N.L. (2)	
324	26	5	2.00		12	32	32	0.067	1.896	6.371	6.371	0.083	0.067	1.896	3.019	1.024	0.085	0.524	0.261	N.L. (2)	
321.5	28.5	4	0.80		12	35	28	0.057	2.038	4.969	4.969	0.072	0.057	2.038	3.317	1.008	0.072	0.495	0.252	N.L. (2)	
319	31	3	0.60		12	35	31	0.053	2.171	3.639	3.639	0.062	0.053	2.171	3.606	0.995	0.062	0.472	0.245	N.L. (2)	
316.5	33.5	15						0.065	2.333	18.354	18.354	0.196	0.065	2.333	3.924	0.973	0.191	0.452	0.238	0.803 (D)	
314	36	23						0.068	2.503	28.936	28.936	0.407	0.068	2.503	4.250	0.944	0.384	0.437	0.232	N.L. (3)	
311.5	38.5	18						0.066	2.668	21.064	21.064	0.229	0.066	2.668	4.571	0.933	0.214	0.424	0.227	0.943 (D)	
309	41	20						0.067	2.836	22.985	22.985	0.257	0.067	2.836	4.895	0.913	0.234	0.414	0.224	1.045 (D)	
306.5	43.5	27						0.070	3.011	31.506	31.506	0.630	0.070	3.011	5.226	0.880	0.554	0.406	0.220	N.L. (3)	
304	46	18						0.066	3.176	19.120	19.120	0.205	0.066	3.176	5.547	0.890	0.182	0.400	0.218	0.835 (D)	
301.5	48.5	17						0.066	3.341	17.409	17.409	0.185	0.066	3.341	5.868	0.881	0.163	0.395	0.217	0.751 (D)	
299	51	30						0.071	3.518	32.415	32.415	0.866	0.071	3.518	6.201	0.828	0.718	0.392	0.216	N.L. (3)	
297	53	23						0.068	3.654	23.199	23.199	0.260	0.068	3.654	6.462	0.843	0.219	0.389	0.215	1.019 (D)	
294.5	55.5	21						0.068	3.824	20.308	20.308	0.219	0.068	3.824	6.788	0.840	0.184	0.387	0.215	0.856 (D)	
289.5	60.5	60						0.077	4.209	61.756	61.756	0.417	0.077	4.209	7.485	0.760	0.317	0.383	0.213	N.L. (3)	

12 R 0.85
 3 1.60 25
 R
 D

R: 2019
 R: 345.18
 R: 343.18
 R: 314.10
 R: 319.10

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M: 654	M: 564	M: 310	M: ***
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Metal Shell 16"φ w/.312" walls
 4: 189
 1: 396

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
339.10	4.08	0.50	3		10.4		16.6	17	0	0	9	6
336.60	2.50	0.40	3		5.2	6.1	18.7	19	0	0	10	9
334.10	2.50	0.20	2		2.7	3.1	30.6	31	0	0	17	11
331.60	2.50	0.80	3		9.7	12.3	37.2	37	0	0	20	14
329.10	2.50	0.60	1		7.5	9.2	49.3	49	0	0	27	16
326.60	2.50	0.90	1		10.7	13.8	58.5	58	0	0	32	19
324.10	2.50	0.80	0		9.7	12.3	66.6	67	0	0	37	21
321.60	2.50	0.70	0		8.6	10.7	70.7	71	0	0	39	24
319.10	2.50	0.40	0		5.2	6.1	75.9	76	0	0	42	26
316.60	2.50	0.40	0		5.2	6.1	74.9	75	0	0	41	29
314.10	2.50		0		0.0	0.0	92.5	93	0	0	51	31
311.60	2.50		2		1.8	17.6	287.7	288	0	0	158	34
306.60	5.00		24	M: 463	46.3	210.9	316.4	316	0	0	174	39
301.60	5.00		22	M: 424	42.4	193.4	376.4	376	0	0	207	44
296.60	5.00		24	M: 463	46.3	210.9	308.4	308	0	0	170	49
291.60	5.00		11	M: 21.2	21.2	96.7	373.6	374	0	0	205	54
290.60	1.00		16	M: 6.2	6.2	140.6	379.7	380	0	0	209	55
286.60	4.00		16	M: 24.7	24.7	140.6	457.2	457	0	0	251	59
281.60	5.00		22	M: 42.4	42.4	193.4	429.3	429	0	0	236	64
276.60	5.00		14	M: 27.0	27.0	123.1	473.8	474	0	0	261	69
275.60	1.00		16	M: 6.2	6.2	140.6	480.0	480	0	0	264	70
274.60	1.00		16	M: 6.2	6.2	140.6	486.2	486	0	0	267	71
273.60	1.00		16	M: 6.2	6.2	140.6	492.3	492	0	0	271	72
272.60	1.00		16	M: 6.2	6.2	140.6	498.5	499	0	0	274	73
271.60	1.00		16	M: 6.2	6.2	140.6	495.9	496	0	0	273	74
270.60	1.00		15	M: 5.8	5.8	131.8	501.7	502	0	0	276	75
269.60	1.00		15	M: 5.8	5.8	131.8	507.5	507	0	0	279	76
268.60	1.00		15	M: 5.8	5.8	131.8	513.2	513	0	0	282	77
267.60	1.00		15	M: 5.8	5.8	131.8	519.0	519	0	0	285	78
266.60	1.00		15	M: 5.8	5.8	131.8	533.6	534	0	0	293	79
265.60	1.00		16	M: 6.2	6.2	140.6	539.8	540	0	0	297	80
264.60	1.00		16	M: 6.2	6.2	140.6	545.9	546	0	0	300	81
263.60	1.00		16	M: 6.2	6.2	140.6	552.1	552	0	0	304	82
262.60	1.00		16	M: 6.2	6.2	140.6	558.3	558	0	0	307	83
261.60	1.00		16	M: 6.2	6.2	140.6	564.5	564	0	0	310	84
260.60	1.00		16	M: 140.6		140.6						

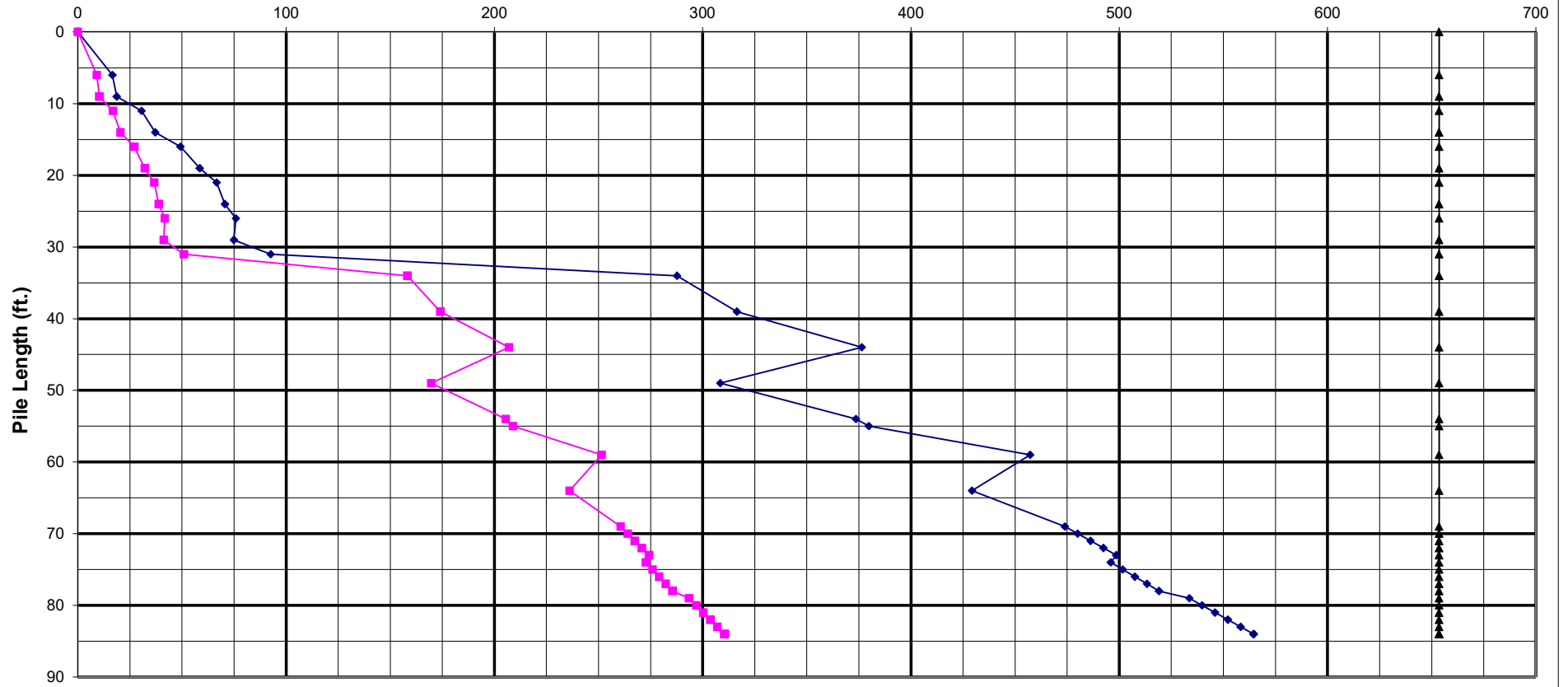
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

NOMINAL REQ'D BEARING

FACTORED RESISTANCE AVAILABLE

Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



Pile Design Table for South Abutment utilizing Boring #2-0 2019

R10000				R10000				R10000				R10000			
R10000				R10000				R10000				R10000			
R10000				R10000				R10000				R10000			
Metal Shell 12"Ø w/25" walls				Steel HP 10 X 42				Steel HP 12 X 84				Steel HP 12 X 84			
11	6	5	10	5	6	6	10	5	6	6	10	5	6	6	10
13	7	9	11	10	5	9	11	13	7	9	11	13	7	9	11
21	11	11	14	15	8	11	14	19	10	11	14	19	10	11	14
26	14	14	18	20	11	14	18	24	14	14	18	24	14	14	18
34	19	16	21	26	15	16	21	34	19	16	21	34	19	16	21
42	23	19	26	31	17	19	26	41	22	19	26	41	22	19	26
48	28	21	31	36	19	21	31	47	26	20	31	47	26	20	31
52	29	24	34	36	20	24	34	46	25	24	34	46	25	24	34
56	31	26	36	38	21	26	36	47	26	26	36	47	26	26	36
56	31	29	37	41	23	31	37	52	29	31	37	52	29	31	37
66	36	31	41	61	34	34	41	79	43	34	41	79	43	34	41
176	97	34	71	71	39	39	49	91	50	39	49	91	50	39	49
201	111	39	78	78	43	49	52	102	56	49	52	102	56	49	52
213	117	49	89	89	49	54	54	119	65	54	54	119	65	54	54
254	140	54	90	90	50	55	55	121	66	55	55	121	66	55	55
258	142	55	98	98	54	64	64	128	71	64	64	128	71	64	64
299	164	64	105	105	58	69	69	139	76	69	69	139	76	69	69
329	181	69	106	106	58	70	70	140	77	70	70	140	77	70	70
334	183	70	107	107	59	71	71	141	78	71	71	141	78	71	71
338	186	71	108	108	58	72	72	142	78	72	72	142	78	72	72
343	189	72	108	108	60	74	74	143	79	75	75	143	79	75	75
347	191	74	109	109	60	75	75	144	79	76	76	144	79	76	76
352	193	75	110	110	61	76	76	145	80	77	77	145	80	77	77
356	196	76	111	111	61	77	77	147	81	78	78	147	81	78	78
360	198	77	112	112	62	78	78	150	83	79	79	150	83	79	79
365	201	78	114	114	63	79	79	152	83	80	80	152	83	80	80
374	206	79	115	115	63	80	80	153	84	81	81	153	84	81	81
378	208	80	116	116	64	81	81	154	85	82	82	154	85	82	82
383	211	81	117	117	65	82	82	155	85	83	83	155	85	83	83
388	213	82	118	118	65	83	83	156	86	84	84	156	86	84	84
110	6	6	110	66	84	84	84								
Metal Shell 14"Ø w/25" walls				Steel HP 10 X 57				Steel HP 14 X 73				Steel HP 14 X 73			
16	9	9	11	8	5	6	6	15	8	9	9	15	8	9	9
25	14	11	11	10	6	6	6	21	12	11	11	21	12	11	11
32	17	14	14	15	8	11	11	31	17	14	14	31	17	14	14
42	23	16	16	20	11	14	14	40	22	16	16	40	22	16	16
50	27	19	19	25	14	15	15	49	27	19	19	49	27	19	19
57	31	21	21	32	17	19	19	54	29	21	21	54	29	21	21
61	34	24	24	36	20	21	21	54	30	29	29	54	30	29	29
66	36	26	26	37	20	24	24	61	33	31	31	61	33	31	31
79	43	31	31	38	21	29	29	89	49	34	34	89	49	34	34
229	126	34	42	42	23	31	31	104	57	39	39	104	57	39	39
256	141	36	63	63	35	34	34	118	65	44	44	118	65	44	44
259	143	49	73	73	40	39	39	122	67	49	49	122	67	49	49
311	171	54	80	80	44	49	49	138	76	54	54	138	76	54	54
317	174	54	82	82	50	54	54	140	77	55	55	140	77	55	55
362	199	64	93	93	51	55	55	150	83	64	64	150	83	64	64
389	220	69	100	100	55	64	64	153	84	64	64	153	84	64	64
405	223	70	108	108	59	69	69	166	91	69	69	166	91	69	69
410	226	71	109	109	60	70	70	167	92	70	70	167	92	70	70
415	228	72	110	110	60	71	71	169	93	71	71	169	93	71	71
419	231	74	111	111	61	72	72	169	93	74	74	169	93	74	74
425	234	75	111	111	61	74	74	171	94	75	75	171	94	75	75
430	236	76	112	112	61	75	75	172	95	76	76	172	95	76	76
435	239	77	113	113	62	76	76	173	95	77	77	173	95	77	77
440	242	78	114	114	62	77	77	175	96	78	78	175	96	78	78
452	248	79	115	115	63	78	78	179	99	79	79	179	99	79	79
457	251	80	117	117	64	79	79	181	99	80	80	181	99	80	80
14	8	6	119	119	66	81	81	182	100	81	81	182	100	81	81
16	9	9	120	120	66	82	82	185	102	83	83	185	102	83	83
25	14	11	121	121	67	83	83	186	102	84	84	186	102	84	84
32	17	14	122	122	67	84	84								
42	23	16	127	127	71	88	88								
50	27	19	130	130	74	91	91								
57	31	21	132	132	77	94	94								
61	34	24	134	134	77	94	94								
66	36	26	135	135	78	95	95								
79	43	31	137	137	80	97	97								
229	126	34	139	139	82	99	99								
256	141	36	141	141	84	101	101								
259	143	49	142	142	84	101	101								
311	171	54	145	145	86	103	103								
317	174	54	150	150	88	106	106								
362	199	64	154	154	91	109	109								
389	220	69	156	156	93	111	111								
405	223	70	158	158	94	111	111								
410	226	71	158	158	94	111	111								
415	228	72	159	159	94	111	111								
419	231	74	160	160	94	111	111								
425	234	75	161	161	94	111	111								
430	236	76	162	162	94	111	111								
435	239	77	163	163	94	111	111								
440	242	78	164	164	94	111	111								
452	248	79	165	165	94	111	111								
457	251	80	166	166	94	111	111								
462	254	81	167	167	94	111	111								
468	257	82	168	168	94	111	111								
473	260	83	169	169	94	111	111								
479	263	84	170	170	94	111	111								
Metal Shell 16"Ø w/31.2" walls				Steel HP 12 X 63				Steel HP 14 X 89				Steel HP 14 X 89			
17	9	6	146	146	81	81	81	12	7	6	6	12	7	6	6
19	10	9	148	148	81	81	81	15	8	9	9	15	8	9	9
31	17	11	149	149	82	83	83	22	12	11	11	22	12	11	11
37	20	14	150	150	82	84	84	32	17	14	14	32	17	14	14
49	27	16	151	151	83	84	84	40	22	16	16	40	22	16	16
56	32	19	152	152	84	84	84	49	27	19	19	49	27	19	19
67	37	21	153	153	84	84	84	54	29	21	21	54	29	21	21
71	39	24	154	154	84	84	84	61	34	24	24	61	34	24	24
75	41	29	155	155	84	84	84	62	34	31	31	62	34	31	31
93	51	31	156	156	84	84	84	82	43	31	31	82	43	31	31
288	158	34	157	157	84	84	84	94	52	34	34	94	52	34</	

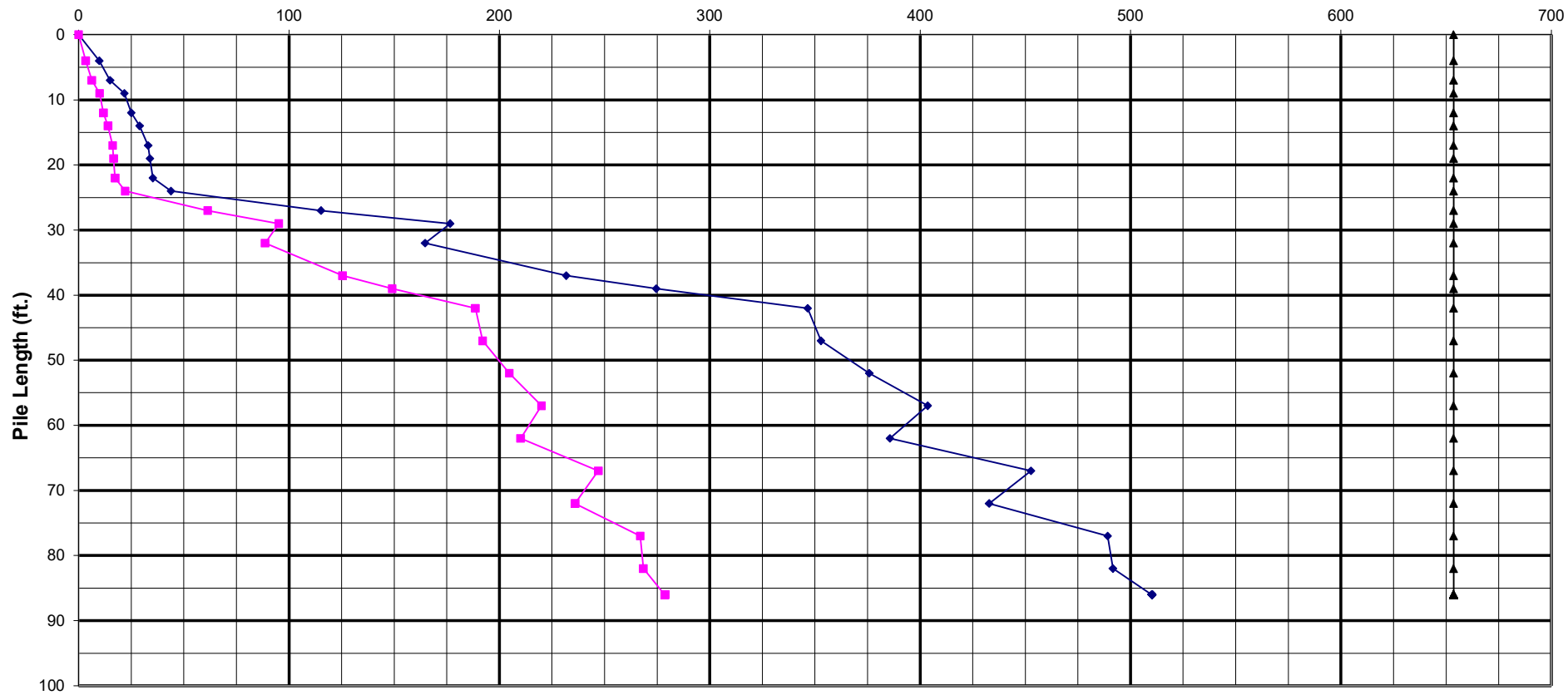
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

NOMINAL REQ'D BEARING

FACTORED RESISTANCE AVAILABLE

Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



File Design Table for Pier 1 utilizing Boring #4-S 2021

Metal Shell 12"Ø w/.25" walls			Steel HP 10 X 42			Steel HP 12 X 84		
Depth (ft)	Capacity (kips)	Capacity (kN)	Depth (ft)	Capacity (kips)	Capacity (kN)	Depth (ft)	Capacity (kips)	Capacity (kN)
8	2	4	3	1	4	4	1	4
10	4	7	7	3	7	9	4	7
15	7	9	11	5	9	14	6	9
18	8	12	13	6	12	17	8	12
21	10	14	15	7	14	20	10	14
24	12	17	17	8	17	22	11	19
25	12	19	17	8	19	23	11	22
28	13	22	18	9	22	25	12	24
31	16	24	19	10	24	36	18	27
72	38	27	28	14	27	41	21	29
104	56	32	31	16	29	44	23	32
146	79	37	34	18	32	52	28	37
173	94	39	40	21	37	58	31	39
217	118	42	45	24	39	66	35	42
230	125	47	51	27	42	78	42	47
250	136	52	61	33	47	89	47	52
269	147	62	70	37	52	98	53	57
306	167	72	76	41	62	101	54	62
342	187	77	83	45	72	109	59	72
349	190	82	93	50	82	121	65	82
363	198	86	96	52	86	125	67	86
Metal Shell 14"Ø w/.25" walls			Steel HP 10 X 57			Steel HP 14 X 73		
8	3	4	3	1	4	5	1	4
12	5	7	7	3	7	10	4	7
18	8	9	11	5	9	16	7	9
21	10	12	13	6	12	20	10	12
25	12	14	16	7	14	23	11	14
28	14	17	18	9	17	25	12	19
29	14	19	18	9	19	26	13	22
31	15	22	18	9	22	29	14	24
37	19	24	20	10	24	41	21	27
92	49	27	29	15	27	46	24	29
133	71	32	33	17	29	50	26	32
186	101	37	35	18	32	59	31	37
221	120	39	42	22	37	66	35	39
278	151	42	47	25	39	74	39	42
289	157	47	53	28	42	89	48	47
311	169	52	63	34	47	102	54	52
326	177	62	72	38	52	113	61	57
368	201	72	78	42	62	121	65	62
414	226	77	85	46	72	130	70	72
419	228	82	95	51	82	144	78	82
435	237	86	98	53	86	148	80	86
Metal Shell 14"Ø w/.312" walls			Steel HP 12 X 53			Steel HP 14 X 89		
8	3	4	4	1	4	5	1	4
12	5	7	8	3	7	11	4	7
18	8	9	13	6	9	16	8	9
21	10	12	17	8	12	21	10	12
25	12	14	19	9	14	24	12	14
28	14	17	21	10	19	25	13	19
29	14	19	22	11	22	26	13	22
31	15	22	24	12	24	29	14	24
37	19	24	34	17	27	42	22	27
92	49	27	38	20	29	48	25	29
133	71	32	41	21	32	51	27	32
186	101	37	49	25	37	61	32	37
221	120	39	54	28	39	68	35	39
278	151	42	61	32	42	77	41	42
289	157	47	73	39	47	91	49	47
311	169	52	84	45	52	104	56	52
326	177	62	93	50	57	116	62	57
368	201	72	97	52	62	123	66	62
414	226	77	105	56	72	132	71	72
419	228	82	116	63	82	146	79	82
435	237	86	120	65	86	150	81	86
Metal Shell 16"Ø w/.312" walls			Steel HP 12 X 63			Steel HP 14 X 102		
10	3	4	4	1	4	5	1	4
15	6	7	8	4	7	11	4	7
22	10	9	14	6	9	17	8	9
25	12	12	17	8	12	21	10	12
29	14	14	19	9	14	24	12	14
33	16	17	21	10	19	26	13	19
34	17	19	22	11	22	27	13	22
35	17	22	24	12	24	29	15	24
44	22	24	35	16	27	43	22	27
115	61	27	39	20	29	49	25	29
165	89	32	42	22	32	52	27	32
232	125	37	50	26	37	62	33	37
275	149	39	56	29	39	69	37	39
347	189	42	63	33	42	79	42	42
353	192	47	75	40	47	93	50	47
376	205	52	86	46	52	106	57	52
386	210	62	96	51	57	117	63	57
433	236	72	98	52	62	124	67	62
469	267	77	106	57	72	133	72	72
492	268	82	118	63	82	148	80	82
510	279	86	121	65	86	153	82	86
Metal Shell 16"Ø w/.375" walls			Steel HP 12 X 74			Steel HP 14 X 117		
10	3	4	4	1	4	5	1	4
15	6	7	8	4	7	11	5	7
22	10	9	14	6	9	17	8	9
25	12	12	17	8	12	21	10	12
29	14	14	19	9	14	24	12	14
33	16	17	21	11	19	26	13	19
34	17	19	22	11	22	27	13	22
35	17	22	24	12	24	30	15	24
44	22	24	35	16	27	44	23	27
115	61	27	40	21	29	50	26	29
165	89	32	43	22	32	53	28	32
232	125	37	51	27	37	64	34	37
275	149	39	57	30	39	71	38	39
347	189	42	65	34	42	81	43	42
353	192	47	77	41	47	95	51	47
376	205	52	87	47	52	108	58	52
386	210	62	97	52	57	120	64	57
433	236	72	99	53	62	126	68	62
469	267	77	107	58	72	135	73	72
492	268	82	119	64	82	150	81	82
510	279	86	123	66	86	155	84	86
Steel HP 8 X 36								
3	1	4						
6	2	7						
9	4	9						
10	5	12						
12	6	14						
14	7	17						
14	7	19						
14	7	22						
16	8	24						
23	12	27						
25	13	29						
27	14	32						
33	17	37						
36	19	39						
41	22	42						
49	26	47						
56	30	52						
59	32	62						
65	35	72						
72	39	77						
73	39	82						
75	40	86						

R 1
 R 4/2021
 R D M 336.30
 R D R 334.30
 R D R 299.50
 R D R 312.00

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M	M	M	M
R	R	R	R
654	510	389	***

M R D
 M R R
 R D R 8
 R D R 3
 D Metal Shell 16"φ w/.312" walls
 R 4/189
 R 1/396

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	ULTIMATE			NOMINAL REQ'D BEARING (KIPS)	NOMINAL GEOTECH. LOSS FROM LIQUEF. & DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	SEISMIC RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
332.00	2.30	0.30			3.7		9.8	10	4	4	2	4
329.50	2.50	0.40			5.2	6.1	15.0	15	9	10	4	7
327.00	2.50	0.40			5.2	6.1	21.7	22	14	15	8	9
324.50	2.50	0.50			6.4	7.7	25.0	25	20	23	8	12
322.00	2.50	0.30			4.0	4.6	29.0	29	24	27	22	14
319.50	2.50	0.30			4.0	4.6	33.0	33	28	31	27	17
317.00	2.50	0.30			4.0	4.6	33.9	34	32	36	34	19
314.50	2.50	0.10			1.4	1.5	35.3	35	34	37	36	22
312.00	2.50	0.10			1.4	1.5	43.9	44	35	39	30	24
309.50	2.50		1	M	1.0	8.8	115.2	115	36	39	40	27
307.00	2.50		9	M	8.7	79.1	176.6	177	45	39	93	29
304.50	2.50		15	M	14.5	131.8	164.7	165	59	39	67	32
299.50	5.00		12	M	23.1	105.5	231.8	232	82	39	111	37
297.00	2.50		17	M	16.4	149.4	274.5	275	82	39	153	39
294.50	2.50		20	M	19.3	175.8	346.5	347	82	39	226	42
289.50	5.00		26	M	50.2	228.5	352.8	353	82	39	232	47
284.50	5.00		21	M	40.5	184.6	375.7	376	82	39	255	52
279.50	5.00		19	M	36.6	167.0	403.6	404	82	39	283	57
274.50	5.00		18	M	34.7	158.2	385.6	386	82	39	265	62
269.50	5.00		12	M	23.1	105.5	452.6	453	82	39	332	67
264.50	5.00		17	M	32.8	149.4	432.7	433	82	39	312	72
259.50	5.00		11	M	21.2	96.7	489.1	489	82	39	368	77
254.50	5.00		15	M	28.9	131.8	491.6	492	82	39	371	82
250.50	4.00		12	M	18.5	105.5	510.1	510	82	39	389	86
249.50	1.00		12	M		105.5						

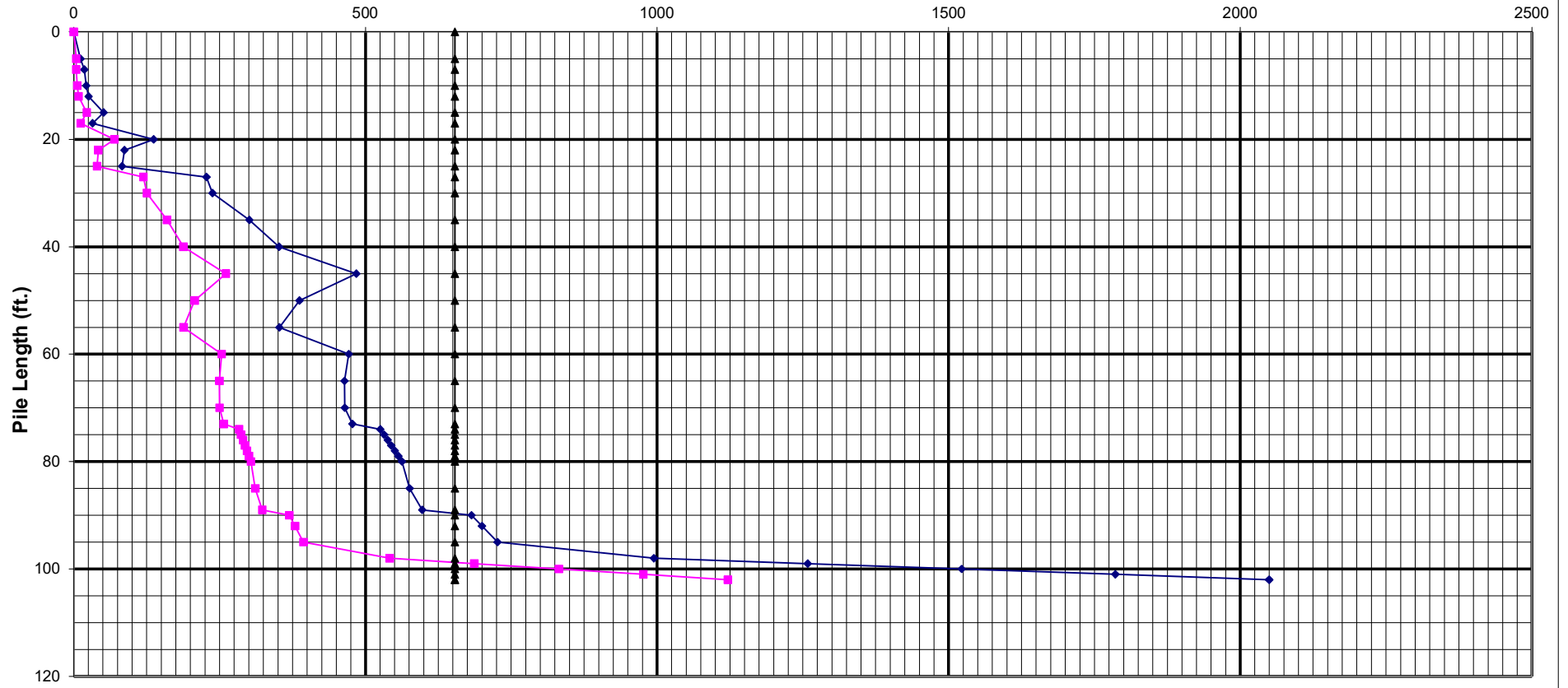
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

—◆— NOMINAL REQ'D BEARING

—■— FACTORED RESISTANCE AVAILABLE

—▲— Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



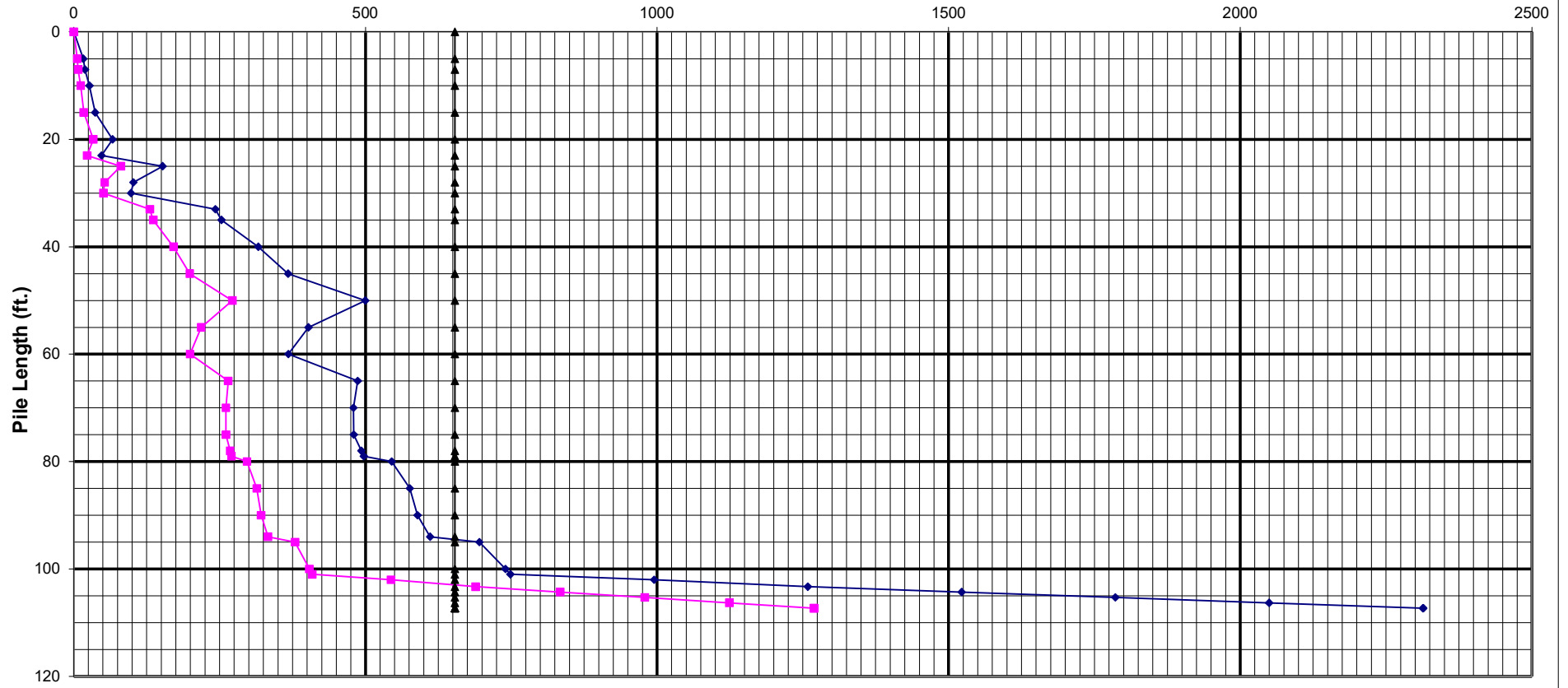
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

—◆— NOMINAL REQ'D BEARING

—■— FACTORED RESISTANCE AVAILABLE

—▲— Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



R 1 2019
 R 345.18
 R 343.18
 R 301.40
 R 318.90

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M 654	M 531	M 292	M ***
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Metal Shell 16"φ w/.312" walls
 4 189
 1 396

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
341.38	1.80	1.10			9.0		19.8	20	0	0	11	4
336.38	5.00	0.70			17.3	10.7	27.8	28	0	0	15	9
333.88	2.50	0.10			1.4	1.5	41.5	41	0	0	23	11
331.38	2.50	0.90			10.7	13.8	46.0	46	0	0	25	14
328.88	2.50	0.50			6.4	7.7	49.3	49	0	0	27	16
326.38	2.50	0.30			4.0	4.6	67.1	67	0	0	37	19
323.88	2.50	1.20			13.4	18.4	72.9	73	0	0	40	21
321.38	2.50	0.70			8.6	10.7	75.4	75	0	0	41	24
318.88	2.50	0.30			4.0	4.6	80.9	81	0	0	44	26
316.38	2.50	0.40			5.2	6.1	84.5	85	0	0	47	29
313.88	2.50	0.30			4.0	4.6	136.7	137	0	0	75	31
311.38	2.50		6	M	5.8	52.7	142.4	142	0	0	78	34
306.38	5.00		6	M	11.6	52.7	180.4	180	0	0	99	39
301.38	5.00		9	M	17.4	79.1	197.7	198	0	0	109	44
296.38	5.00		9	M	17.4	79.1	311.8	312	0	0	171	49
291.38	5.00		20	M	38.6	175.8	385.5	385	0	0	212	54
290.38	1.00		24	M	9.3	210.9	394.7	395	0	0	217	55
289.38	1.00		24	M	9.3	210.9	404.0	404	0	0	222	56
288.38	1.00		24	M	9.3	210.9	413.3	413	0	0	227	57
287.38	1.00		24	M	9.3	210.9	422.5	423	0	0	232	58
286.38	1.00		24	M	9.3	210.9	387.8	388	0	0	213	59
281.38	5.00		19	M	36.6	167.0	380.5	381	0	0	209	64
276.38	5.00		14	M	27.0	123.1	469.0	469	0	0	258	69
271.38	5.00		21	M	40.5	184.6	456.8	457	0	0	251	74
266.38	5.00		15	M	28.9	131.8	450.6	451	0	0	248	79
265.38	1.00		11	M	4.2	96.7	454.8	455	0	0	250	80
264.38	1.00		11	M	4.2	96.7	459.0	459	0	0	252	81
263.38	1.00		11	M	4.2	96.7	463.3	463	0	0	255	82
262.38	1.00		11	M	4.2	96.7	467.5	468	0	0	257	83
261.38	1.00		11	M	4.2	96.7	498.1	498	0	0	274	84
260.38	1.00		14	M	5.4	123.1	503.5	504	0	0	277	85
259.38	1.00		14	M	5.4	123.1	508.9	509	0	0	280	86
258.38	1.00		14	M	5.4	123.1	514.3	514	0	0	283	87
257.38	1.00		14	M	5.4	123.1	519.7	520	0	0	286	88
256.38	1.00		14	M	5.4	123.1	525.1	525	0	0	289	89
255.38	1.00		14	M	5.4	123.1	530.5	531	0	0	292	90
254.38	1.00		14	M		123.1						

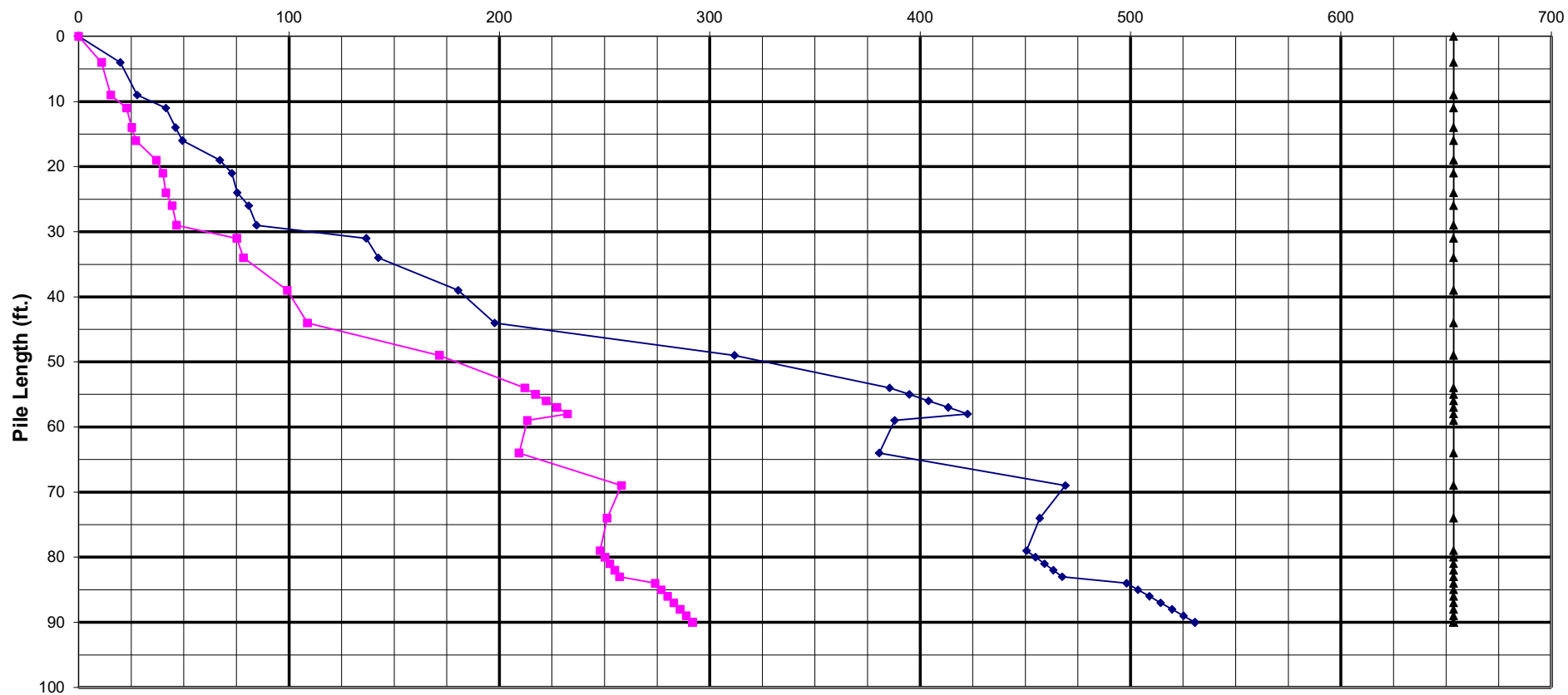
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

—◆— NOMINAL REQ'D BEARING

—■— FACTORED RESISTANCE AVAILABLE

—▲— Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



Pile Design Table for North Abutment utilizing Boring #1-S 2019

Steel HP 12 X 48				Steel HP 12 X 42				Steel HP 12 X 36					
Capacity (kips)	Capacity (kN)	Capacity (kips)	Capacity (kN)	Capacity (kips)	Capacity (kN)	Capacity (kips)	Capacity (kN)	Capacity (kips)	Capacity (kN)	Capacity (kips)	Capacity (kN)	Capacity (kips)	Capacity (kN)
13	7	4	10	5	4	4	10	5	4	10	5	4	10
21	11	9	14	8	9	18	10	9	18	10	9	18	10
29	16	11	22	12	11	27	15	11	31	17	14	31	17
33	18	14	24	13	14	31	17	14	35	19	16	35	19
36	20	16	25	14	16	32	18	16	36	20	18	36	20
47	26	19	36	20	19	47	26	19	54	30	23	54	30
52	29	21	38	21	21	49	27	21	57	32	24	57	32
56	31	24	38	21	24	53	29	24	61	34	26	61	34
60	33	26	41	23	26	54	30	26	63	35	28	63	35
63	34	29	43	24	29	58	32	29	68	37	31	68	37
93	51	31	52	29	31	69	38	34	81	45	34	81	45
97	53	34	53	29	34	79	44	36	94	52	36	94	52
120	66	39	60	33	39	83	46	44	101	56	44	101	56
133	73	44	63	35	44	99	55	49	117	64	56	117	64
201	110	49	78	43	49	112	61	54	132	73	61	132	73
250	137	54	88	48	54	114	63	55	141	77	63	141	77
257	141	55	90	50	55	117	64	56	146	80	64	146	80
260	143	59	92	51	59	119	66	57	150	82	67	150	82
262	144	64	93	51	64	122	67	58	154	84	71	154	84
317	174	69	103	57	69	123	67	59	160	88	75	160	88
318	175	74	104	57	74	124	68	64	164	90	79	164	90
320	176	79	104	57	81	134	73	79	173	96	84	173	96
323	178	80	105	58	82	134	74	80	174	96	85	174	96
326	179	81	106	58	83	135	74	81	175	96	86	175	96
329	181	82	111	61	84	136	75	82	176	97	87	176	97
333	183	83	112	62	85	137	75	83	177	97	88	177	97
351	193	84	113	62	86	146	80	84	187	103	90	187	103
355	195	85	114	63	87	147	81	85	188	103	91	188	103
359	197	86	115	63	88	148	81	86	189	103	92	189	103
363	199	87	116	64	89	149	82	87	190	103	93	190	103
367	202	88	116	64	90	150	82	88	191	103	94	191	103
371	204	89	116	64	90	151	83	89	192	103	95	192	103
375	206	90	116	64	90	152	83	90	193	103	96	193	103
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24	13	9	25	14	14	21	11	9	27	15	11	27	15
35	19	11	36	20	16	31	17	11	37	20	14	37	20
39	22	14	37	20	19	32	18	14	38	21	16	38	21
43	23	16	39	21	21	33	18	16	39	21	16	39	21
57	31	19	50	28	24	54	30	19	61	34	23	61	34
63	34	21	42	23	26	57	31	24	65	36	26	65	36
65	36	24	44	24	29	61	34	26	68	37	28	68	37
70	39	26	44	24	31	63	35	29	71	39	30	71	39
73	40	29	55	30	34	81	44	31	82	44	34	82	44
114	63	31	62	34	39	92	51	34	99	54	38	99	54
119	66	34	64	35	44	95	52	39	103	57	41	103	57
149	82	39	80	44	49	99	54	44	114	63	47	114	63
164	90	44	90	50	54	114	63	49	128	70	54	128	70
254	139	49	93	51	55	128	70	54	131	72	55	131	72
314	173	54	95	52	56	134	74	56	134	74	56	134	74
319	176	64	95	52	64	134	74	64	137	75	67	137	75
384	211	79	105	58	79	137	75	67	140	77	68	140	77
387	213	80	106	58	80	140	77	68	141	78	69	141	78
391	215	81	107	59	81	141	78	69	142	78	70	142	78
395	217	82	107	59	82	147	81	74	147	81	74	147	81
399	219	83	108	59	83	148	81	75	148	81	75	148	81
422	232	84	114	63	84	159	87	80	159	87	80	159	87
427	235	85	115	63	85	160	88	81	160	88	81	160	88
432	238	86	116	64	86	161	88	82	161	88	82	161	88
437	240	87	117	64	87	162	89	83	162	89	83	162	89
441	243	88	117	65	88	173	95	84	173	95	84	173	95
446	245	89	118	65	89	174	96	85	174	96	85	174	96
451	248	90	119	66	90	175	96	86	175	96	86	175	96
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24	13	9	17	10	9	17	9	9	17	9	9	17	9
35	19	11	26	14	11	26	14	11	26	14	11	26	14
39	22	14	30	17	14	30	17	14	30	17	14	30	17
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57	31	19	45	25	19	45	25	19	45	25	19	45	25
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65	36	24	51	28	26	51	28	26	51	28	26	51	28
70	39	26	53	29	29	53	29	29	53	29	29	53	29
73	40	29	65	36	31	65	36	31	65	36	31	65	36
114	63	31	67	37	34	67	37	34	67	37	34	67	37
119	66	34	70	42	39	70	42	39	70	42	39	70	42
149	82	39	79	44	44	79	44	44	79	44	44	79	44
164	90	44	94	52	49	94	52	49	94	52	49	94	52
254	139	49	105	58	54	105	58	54	105	58	54	105	58
314	173	54	108	59	55	108	59	55	108	59	55	108	59
319	176	64	111	61	56	111	61	56	111	61	56	111	61
384	211	79	113	62	57	113	62	57	113	62	57	113	62
387	213	80	116	64	58	116	64	58	116	64	58	116	64
391	215	81	117	64	59	117	64	59	117	64	59	117	64
395	217	82	118	65	64	118	65	64	118	65	64	118	65
399	219	83	128	71	79	128	71	79	128	71	79	128	71
422	232	84	129	71	80	129	71	80	129	71	80	129	71
427	235	85	130	71	81	130	71	81	130	71	81	130	71
432	238	86	131	72	82	131	72	82	131	72	82	131	72
437	240	87	131	72	83	131	72	83	131	72	83	131	72
441	243	88	140	77	84	140	77	84	140	77	84	140	77
446	245	89	141	77	85	141	77	85	141	77	85	141	77
451	248	90	142	78	86	142	78	86	142	78	86	142	78
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24	13	9	14	7	8	14	7	8	14	7	8	14	7
35	19	11	14	7	8	14	7	8	14	7	8	14	7
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57	31	19	14	7	8	14	7	8	14	7	8	14	7
63	34	21	14	7	8	14	7	8	14	7	8	14	7
65	36	24	14	7	8	14	7	8	14	7	8	14	7
70	39	26	14	7	8	14	7	8	14	7	8	14	7
73	40	29	14	7	8	14	7	8	14	7	8	14	7
114	63	31	14	7	8	14	7	8	14	7	8	14	7
119	66	34	14	7	8	14	7	8	14	7	8	14	7
149	82	39	14	7	8	14	7	8	14	7	8	14	7
164	90	44	14	7	8	14	7	8	14	7	8	14	7
254	139	49	14	7	8	14	7	8	14	7	8	14	7
314	173	54	14	7	8	14	7	8	14	7	8	14	7
319	176	64	14	7	8	14	7	8	14	7	8	14	7
384	211	79	14	7	8	14	7	8	14	7	8	14	7
387	213	80	14	7	8	14	7	8	14	7	8	14	7
391	215	81	14	7	8	14	7	8	14	7	8	14	7
395	217	82	14	7	8	14	7	8	14	7	8	14	7
399	219	83	14	7	8	14	7	8	14	7	8	14	7
422	232	84	14	7	8	14							

R 1 2019
 345.18
 343.18
 301.40
 318.90

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M	M	M	M
R	R	R	Dr
654	531	351	***

Metal Shell 16"φ w/.312" walls
 4 189
 1 396

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	ULTIMATE			NOMINAL REQ'D BEARING (KIPS)	NOMINAL GEOTECH. LOSS FROM LIQUEF. & DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	SEISMIC RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
341.38	1.80	1.10			9.0		19.8	20	9	10	1	4
336.38	5.00	0.70			17.3	10.7	27.8	28	26	29	27	9
333.88	2.50	0.10			1.4	1.5	41.5	41	28	31	17	11
331.38	2.50	0.90			10.7	13.8	46.0	46	38	42	35	14
328.88	2.50	0.50			6.4	7.7	49.3	49	45	49	35	16
326.38	2.50	0.30			4.0	4.6	67.1	67	49	54	35	19
323.88	2.50	1.20			13.4	18.4	72.9	73	62	69	58	21
321.38	2.50	0.70			8.6	10.7	75.4	75	71	78	73	24
318.88	2.50	0.30			4.0	4.6	80.9	81	75	78	72	26
316.38	2.50	0.40			5.2	6.1	84.5	85	80	78	73	29
313.88	2.50	0.30			4.0	4.6	136.7	137	84	78	25	31
311.38	2.50		6	M	5.8	52.7	142.4	142	90	78	25	34
306.38	5.00		6	M	11.6	52.7	180.4	180	101	78	1	39
301.38	5.00		9	M	17.4	79.1	197.7	198	101	78	18	44
296.38	5.00		9	M	17.4	79.1	311.8	312	101	78	132	49
291.38	5.00		20	M	38.6	175.8	385.5	385	101	78	206	54
290.38	1.00		24	M	9.3	210.9	394.7	395	101	78	215	55
289.38	1.00		24	M	9.3	210.9	404.0	404	101	78	225	56
288.38	1.00		24	M	9.3	210.9	413.3	413	101	78	234	57
287.38	1.00		24	M	9.3	210.9	422.5	423	101	78	243	58
286.38	1.00		24	M	9.3	210.9	387.8	388	101	78	209	59
281.38	5.00		19	M	36.6	167.0	380.5	381	101	78	201	64
276.38	5.00		14	M	27.0	123.1	469.0	469	101	78	290	69
271.38	5.00		21	M	40.5	184.6	456.8	457	101	78	277	74
266.38	5.00		15	M	28.9	131.8	450.6	451	101	78	271	79
265.38	1.00		11	M	4.2	96.7	454.8	455	101	78	275	80
264.38	1.00		11	M	4.2	96.7	459.0	459	101	78	280	81
263.38	1.00		11	M	4.2	96.7	463.3	463	101	78	284	82
262.38	1.00		11	M	4.2	96.7	467.5	468	101	78	288	83
261.38	1.00		11	M	4.2	96.7	498.1	498	101	78	319	84
260.38	1.00		14	M	5.4	123.1	503.5	504	101	78	324	85
259.38	1.00		14	M	5.4	123.1	508.9	509	101	78	330	86
258.38	1.00		14	M	5.4	123.1	514.3	514	101	78	335	87
257.38	1.00		14	M	5.4	123.1	519.7	520	101	78	340	88
256.38	1.00		14	M	5.4	123.1	525.1	525	101	78	346	89
255.38	1.00		14	M	5.4	123.1	530.5	531	101	78	351	90
254.38	1.00		14	M		123.1						



Sieve Analysis Data Sheet

□□□M□D422□63

Project Name: M□19034□09
 Location: □□145□□□r□□□□□r□□□
 Boring No: 3□
 Sample Depth: 31□32.5□

Tested By: □□□□□□□□
 Checked By: □□□□□□□□r
 Sample ID: □□13

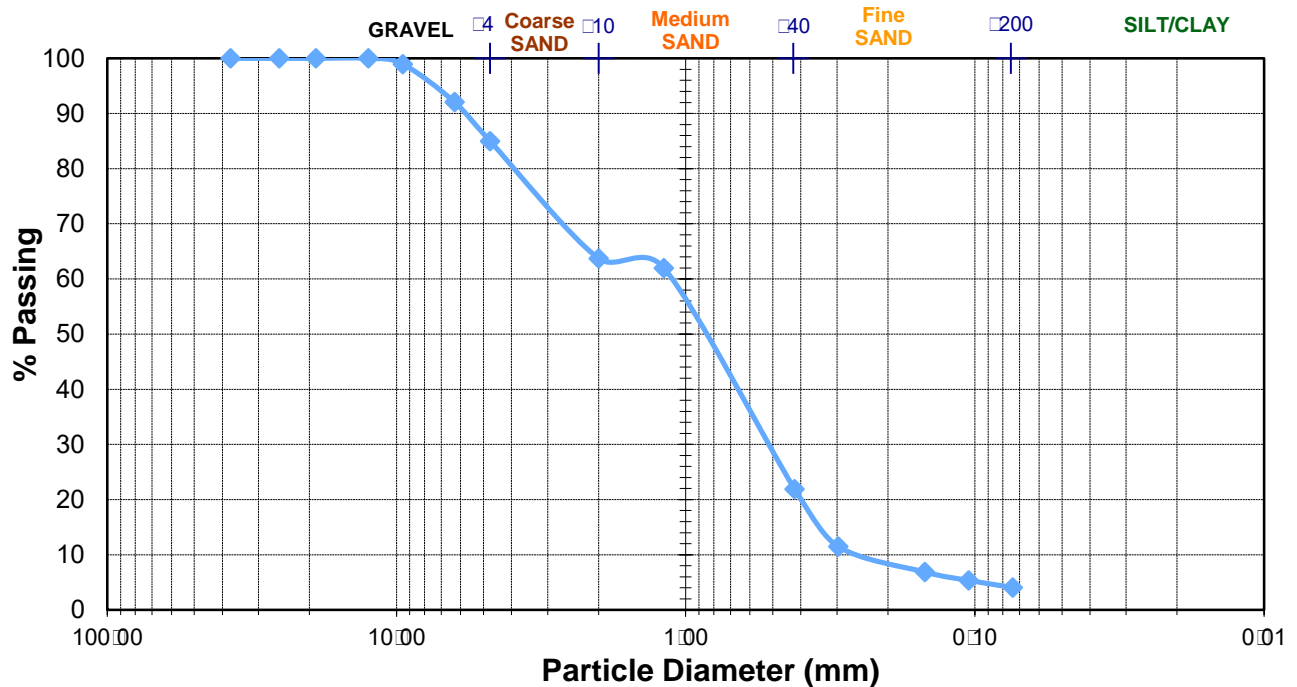
Date: 7□7□2021
 Date: 7□8□2021

IDH Soil Classification: Sand with Gravel

Weight of Container (g): 152□3
 Weight of Dry Sample (g): 196□8

Weight of Container & Soil (g): 349□1

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15□□□	37.50			0.0	0.0	100.0
1□□□□	25.40			0.0	0.0	100.0
34□□□	19.00			0.0	0.0	100.0
12□□□	12.50			0.0	0.0	100.0
38□□□	9.50	527.90	530	2.1	1.1	98.9
14□□□	6.30	513.00	526.5	13.5	6.9	92.1
□4	4.75	523.70	537.6	13.9	7.1	85.0
□10	2.00	678.10	720	41.9	21.3	63.7
□16	1.19	421.20	424.6	3.4	1.7	62.0
□40	0.420	564.400	643.3	78.9	40.1	21.9
□50	0.297	497.800	518.3	20.5	10.4	11.5
□100	0.149	518.300	527.4	9.1	4.6	6.9
□140	0.105	333.000	336	3.0	1.5	5.3
□200	0.074	326.100	328.6	2.5	1.3	4.1
Pan		371.1	378.8	7.7	3.9	0.2
TOTAL:				196.5	99.8	





Sieve Analysis Data Sheet

□□□M□D422□63

Project Name: M□19034□09
 Location: □□145□□□r□□□□□r□□□
 Boring No: 3□□
 Sample Depth: 36□□37□5□

Tested By: □□□□□□□□
 Checked By: □□□□□□□□r
 Sample ID: □□15

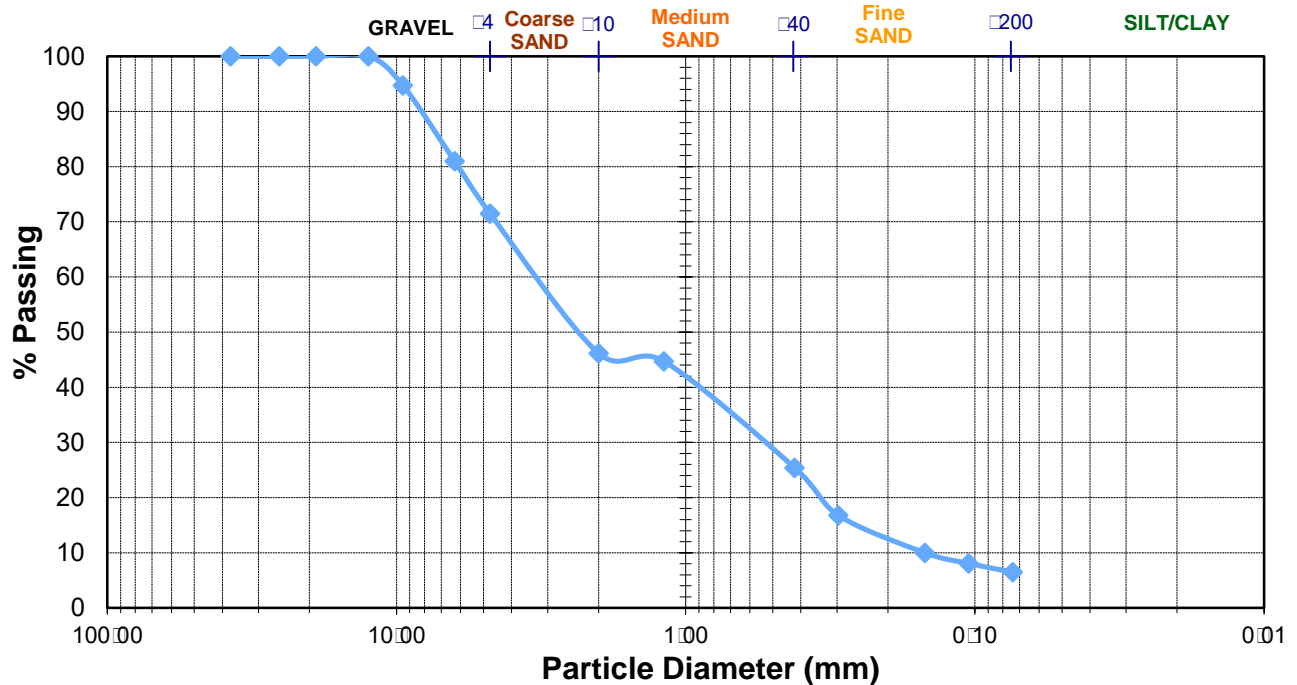
Date: 7/7/2021
 Date: 7/8/2021

IDH Soil Classification: Sand with Gravel

Weight of Container (g): 151□2
 Weight of Dry Sample (g): 159□0

Weight of Container & Soil (g): 310□2

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15□□□	37.50			0.0	0.0	100.0
1□□□□	25.40			0.0	0.0	100.0
34□□□	19.00			0.0	0.0	100.0
12□□□	12.50			0.0	0.0	100.0
38□□□	9.50	527.80	536.2	8.4	5.3	94.7
14□□□	6.30	513.00	534.8	21.8	13.7	81.0
□4	4.75	523.60	538.7	15.1	9.5	71.5
□10	2.00	676.80	717.1	40.3	25.3	46.2
□16	1.19	420.80	423.1	2.3	1.4	44.7
□40	0.420	573.000	603.7	30.7	19.3	25.4
□50	0.297	497.700	511.4	13.7	8.6	16.8
□100	0.149	518.400	529.2	10.8	6.8	10.0
□140	0.105	333.100	336.2	3.1	1.9	8.1
□200	0.074	326.300	328.8	2.5	1.6	6.5
Pan		480.2	485.7	5.5	3.5	3.0
TOTAL:				154.2	97.0	





Sieve Analysis Data Sheet

□□□M□D422□63

Project Name: M□19034□09
 Location: □□145□□□r□□□□□r□□□
 Boring No: 3□
 Sample Depth: 48.5□50□

Tested By: □□□□□□□□
 Checked By: □□□□□□□□r
 Sample ID: □□18

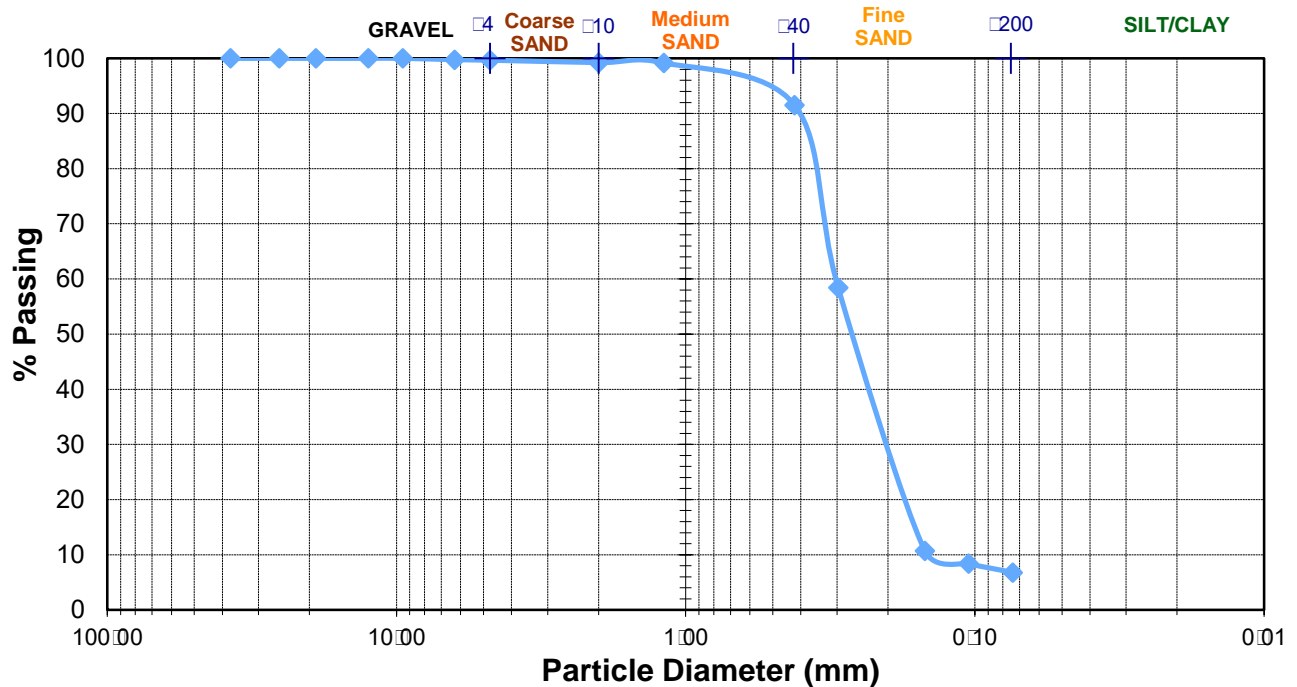
Date: 7/7/2021
 Date: 7/8/2021

IDH Soil Classification: Fine Sand

Weight of Container (g): 151.2
 Weight of Dry Sample (g): 206.2

Weight of Container & Soil (g): 357.4

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15	37.50			0.0	0.0	100.0
1	25.40			0.0	0.0	100.0
3/4	19.00			0.0	0.0	100.0
1/2	12.50			0.0	0.0	100.0
3/8	9.50			0.0	0.0	100.0
1/4	6.30	513.00	513.5	0.5	0.2	99.8
4	4.75	523.60	523.8	0.2	0.1	99.7
10	2.00	676.80	677.7	0.9	0.4	99.2
16	1.19	420.80	420.9	0.1	0.0	99.2
40	0.420	573.000	588.8	15.8	7.7	91.5
50	0.297	497.700	565.9	68.2	33.1	58.4
100	0.149	518.400	616.8	98.4	47.7	10.7
140	0.105	333.100	337.9	4.8	2.3	8.4
200	0.074	326.300	329.6	3.3	1.6	6.8
Pan		480.2	494	13.8	6.7	0.1
TOTAL:				206	99.9	





Sieve Analysis Data Sheet

□□□M□D422□63

Project Name: M□19034□09

Tested By: □□□□□□□□

Date: 7□7□2021

Location: □□145□□□r□□□□□r□□□

Checked By: □□□□□□□□r

Date: 7□8□2021

Boring No: 4□

Sample ID □□17

Sample Depth: 43.5□45□

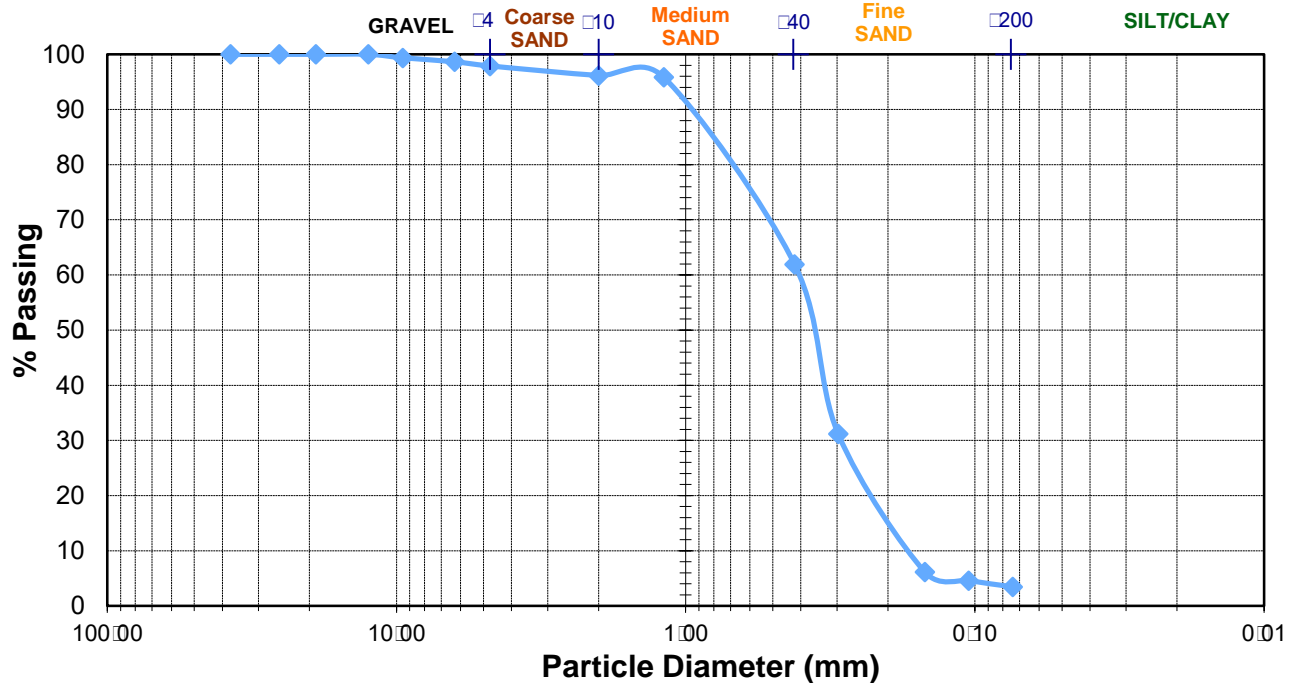
IDH Soil Classification: Fine SAND trace gravel

Weight of Container (g): 151□2

Weight of Container & Soil (g): 388□5

Weight of Dry Sample (g): 237□3

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15□□□	37.50			0.0	0.0	100.0
1□□□□	25.40			0.0	0.0	100.0
34□□□	19.00			0.0	0.0	100.0
12□□□	12.50			0.0	0.0	100.0
38□□□	9.50	527.80	529.3	1.5	0.6	99.4
14□□□	6.30	513.00	514.7	1.7	0.7	98.7
□4	4.75	523.60	525.4	1.8	0.8	97.9
□10	2.00	676.80	680.9	4.1	1.7	96.2
□16	1.19	420.80	421.6	0.8	0.3	95.8
□40	0.420	573.000	653.5	80.5	33.9	61.9
□50	0.297	497.700	570.6	72.9	30.7	31.2
□100	0.149	518.400	577.8	59.4	25.0	6.2
□140	0.105	333.100	336.8	3.7	1.6	4.6
□200	0.074	326.300	329	2.7	1.1	3.5
Pan		480.2	487.7	7.5	3.2	0.3
TOTAL:				236.6	99.7	





Sieve Analysis Data Sheet

□□□M□D422□63

Project Name: M□19034□09
 Location: □□145□□□r□□□□□r□□□
 Boring No: 4□
 Sample Depth: 31□32□5□

Tested By: □□□□□□□□
 Checked By: □□□□□□□□r
 Sample ID: □□13

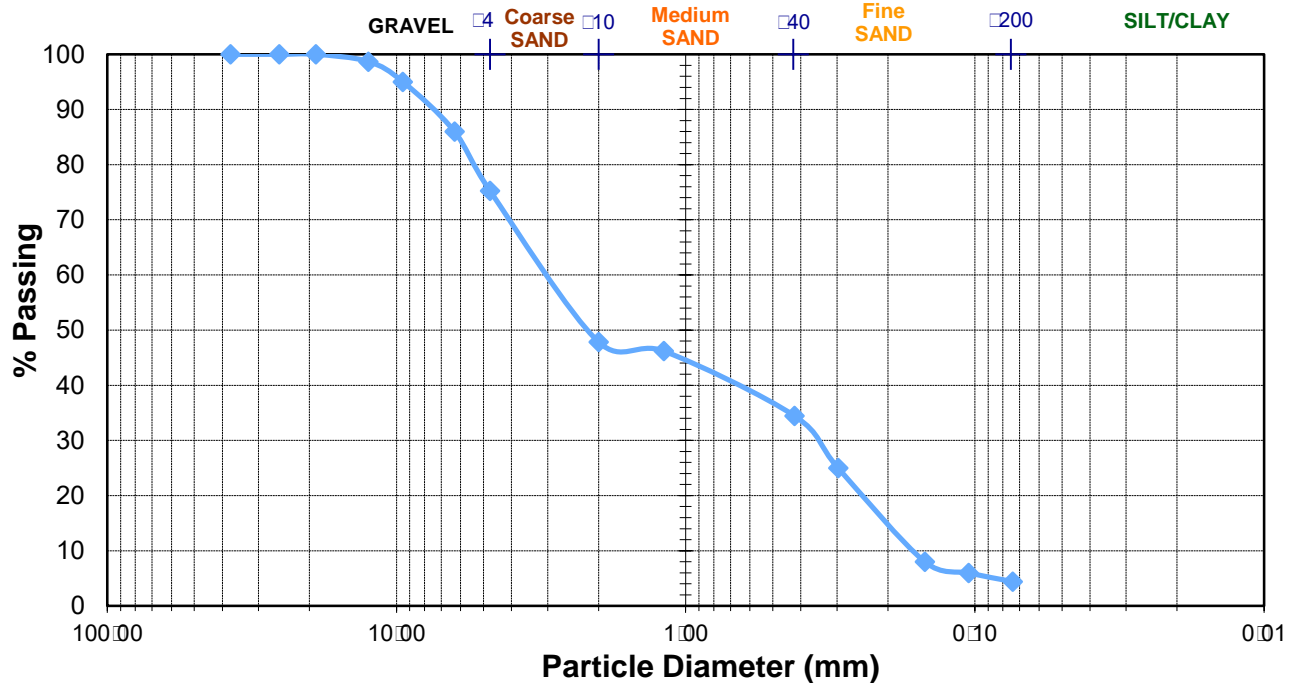
Date: 7□7□2021
 Date: 7□8□2021

IDH Soil Classification: Fine to coarse SAND trace gravel

Weight of Container (g): 151□2
 Weight of Dry Sample (g): 206□5

Weight of Container & Soil (g): 357□7

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15□□□	37.50			0.0	0.0	100.0
1□□□□	25.40			0.0	0.0	100.0
34□□□	19.00			0.0	0.0	100.0
12□□□	12.50	551.90	554.6	2.7	1.3	98.7
38□□□	9.50	527.80	535.4	7.6	3.7	95.0
14□□□	6.30	513.00	531.6	18.6	9.0	86.0
□4	4.75	523.60	545.8	22.2	10.8	75.3
□10	2.00	676.80	733.4	56.6	27.4	47.8
□16	1.19	420.80	424.2	3.4	1.6	46.2
□40	0.420	573.000	597.3	24.3	11.8	34.4
□50	0.297	497.700	517.2	19.5	9.4	25.0
□100	0.149	518.400	553.5	35.1	17.0	8.0
□140	0.105	333.100	337.3	4.2	2.0	6.0
□200	0.074	326.300	329.5	3.2	1.5	4.4
Pan		480.2	488.9	8.7	4.2	0.2
TOTAL:				206.1	99.8	



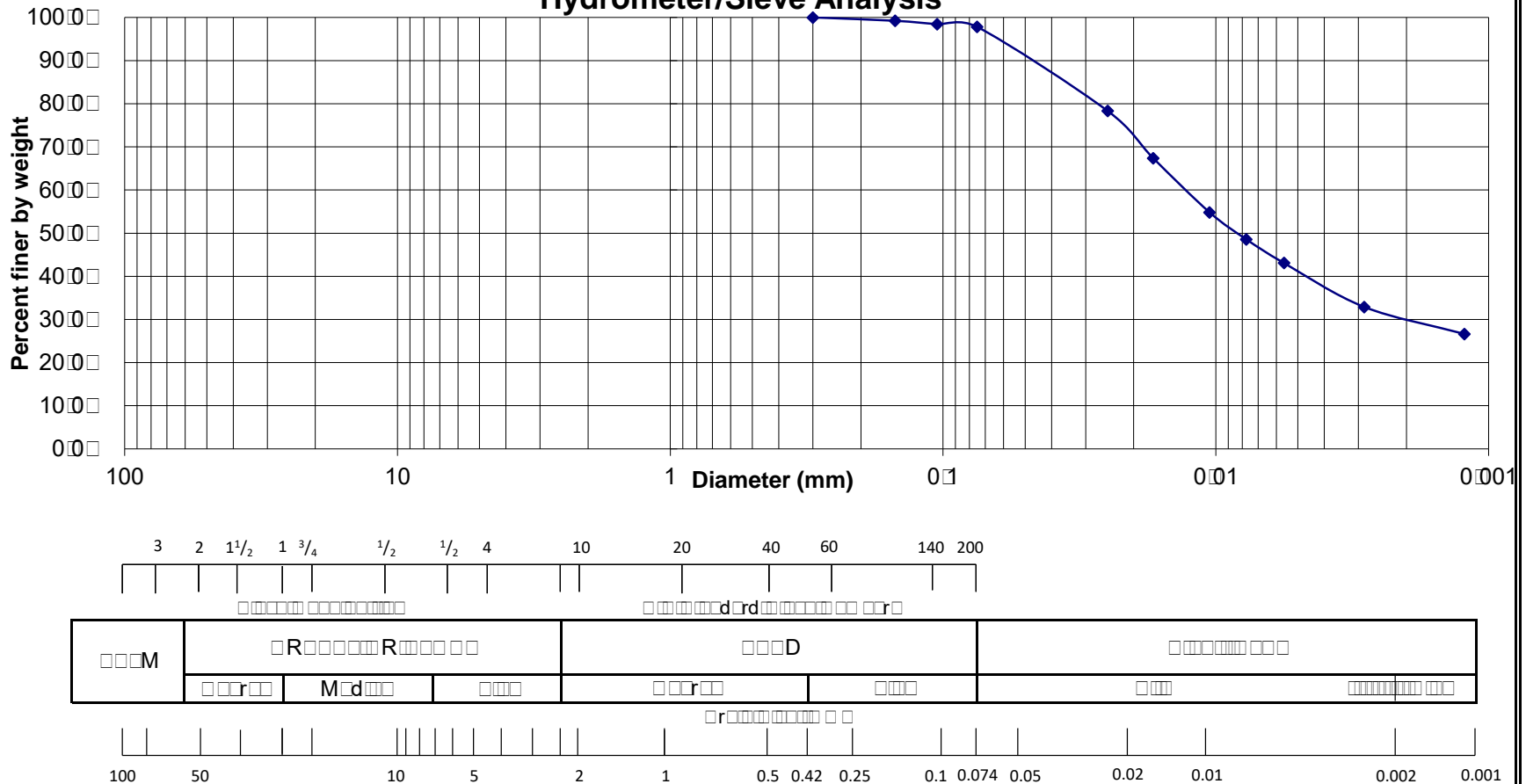
Sieve / Hydrometer Analysis (ASTM D 422)



Gravel:	0.0
Sand:	2.2
Silt:	68.2
Clay:	29.6

USCS Classification:

Hydrometer/Sieve Analysis



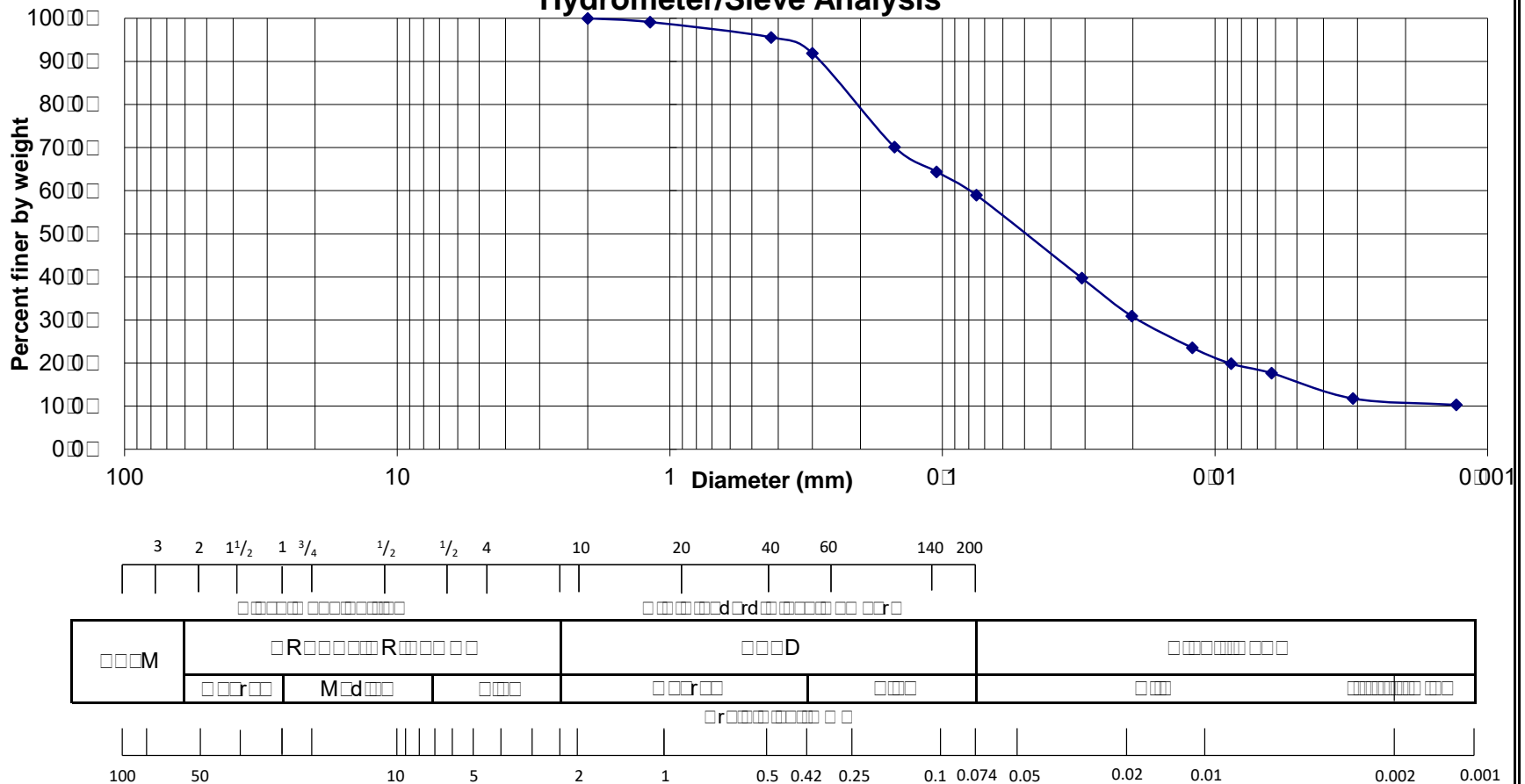
Sieve / Hydrometer Analysis (ASTM D 422)



Gravel:	0.0
Sand:	41.0
Silt:	48.1
Clay:	10.9

USCS Classification:

Hydrometer/Sieve Analysis



Sieve / Hydrometer Analysis (ASTM D 422)



Gravel:	0.0
Sand:	6.6
Silt:	74.7
Clay:	18.7

USCS Classification:

Hydrometer/Sieve Analysis

