



Illinois Department of Transportation

Memorandum

To: *

From: David Broviak

Subject: Special Provision Changes

Date: October 8, 2025

The following special provisions have been revised for the **January 16, 2026** and **February 27, 2026** lettings: Attached is the updated BDE Checklist for this letting.

Recurring Special Provisions

Adopted January 1, 2026

Interim Special Provisions (BDE)

ISP Number	Description
Alphabetic ISP Index (Revised)	Remove existing alphabetic index and insert revised index.
Numerical ISP Index (Revised)	Remove existing numeric index and insert revised index.
106.01 (Revised)	"Source of Supply and Quality Requirements (BDE)" Revised to include manufactured products as well as several definition changes to construction materials and manufactured project. To be inserted into all federal-aid contracts.
420.05 (New)	"Modified Longitudinal Construction Joint (BDE)" To be inserted into contracts that contain a modified longitudinal construction joint for PCC pavement.
420.09 (New)	"Longitudinal Tining (BDE)" To be inserted into contracts containing PCC pavement, continuously reinforced PCC pavement, and PCC shoulders.
530.00 (New)	"Deck Slab Repair (BDE)" To be inserted into contracts containing a bridge deck concrete overlay, HMA surface removal (deck), or deck slab repair.
531.00 (New)	"Bridge Deck Concrete Overlays (BDE)" To be inserted into contracts containing bridge deck microsilica, latex, HRM, GGBF or fly ash concrete overlays.
532.00 (New)	"Structural Repair of Concrete (BDE)" To be inserted into all contracts with Structural Repair of Concrete.
642.00 (New)	"Sinusoidal Rumble Strips (BDE)" To be inserted into contracts containing Sinusoidal Rumble Strips.

Interim Special Provisions (BDE) Continued

ISP Number	Description
701.03 (Revised)	“Work Zone Traffic Control Devices (BDE)” Revised to incorporate updates from the 11 th Edition MUTCD and to fix a typographical error. To be inserted into all contracts.
704.02 (New)	“Temporary Concrete Barrier (BDE)” To be inserted into contracts with temporary concrete barrier.
720.02 (Revised)	“Sign Panels and Appurtenances (BDE)” Revised to allow digital printing for processing highway signs in coordination with the ATSSA Illinois Chapter Sign Committee. To be inserted into all contracts.
1001.01 (Revised)	“Cement, Finely Divided Minerals, Admixtures Concrete, and Mortar (BDE)” Revised to update testing requirements, incorporate fibers into concrete mixes, and update material and/or equipment requirements for concrete repair mixes, water for concrete, and mobile mixers for concrete. To be inserted into all contracts.
1030.07 (Revised)	“Hot-Mix Asphalt (BDE)” Revised to alleviate potential for HMA binder courses left open over winter; address sample and specimen removal from District locations, quality control air voids target limits in the QC/QA quality management program, HMA test strip acceptance, and HMA performance testing. To be inserted into all HMA paving contracts.

District Special Provisions (BDE)

No Changes except for 1 st Page (000.00)	
000.00 (Revised)	“State of Illinois Special Provisions First Page” Revised adoption date revised to January 1, 2026. To be inserted into all contracts.

General Notes

No changes.

2026 Supplemental Specifications and Recurring Special Provisions

214.03	“Grading and Shaping Ditches” New Location(s): Articles 214.03 & 214.04
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DB:tdp:S:\MGR2\WINWORD\Special Provisions\PL_Completed SP\Special Provisions Memo Changes.docx

Attachment(s)

cc: * S&P Engineer Team 3 Team 7 Team 11 Local Roads (T. Sassine)
T. Phillips Team 4 Team 8 Team 12 Operations (M. Eckhoff)
L. Hayworth Team 5 Team 9 Geometrics (R. Julich) Materials (D. Parish)
Team 1 Team 6 Team 10 Bridges Hydraulics (J. Jochums)
Team 2

**First Page
&
Index for
Supplemental Specifications
and
Recurring Special Provisions**

Current Lettings

(January 16, 2026 & February 27, 2026)

STATE OF ILLINOIS

SPECIAL PROVISIONS

The following Special Provisions supplement the "Standard Specifications for Road and Bridge Construction, adopted January 1, 2022 (revised January 1, 2026)", the latest edition of the "Manual on Uniform Traffic Control Devices for Streets and Highways", and the "Manual of Test Procedures for Materials" in effect on the date of invitation for bids, and the "Supplemental Specifications and Recurring Special Provisions" indicated on the Check Sheet included herein, and the "Recommended Standards for Water Works", (Ten State Standards), latest edition, which apply to and govern the construction of

and in case of conflict with any part or parts of said Specifications, the said Special Provisions shall take precedence and shall govern.

LOCATION OF PROJECT

DESCRIPTION OF PROJECT

INDEX
FOR
SUPPLEMENTAL SPECIFICATIONS
AND RECURRING SPECIAL PROVISIONS

Adopted January 1, 2026

This index contains a listing of SUPPLEMENTAL SPECIFICATIONS, frequently used RECURRING SPECIAL PROVISIONS, and LOCAL ROADS AND STREETS RECURRING SPECIAL PROVISIONS.

ERRATA Standard Specifications for Road and Bridge Construction
 (Adopted 1-1-22) (Revised 1-1-26)

SUPPLEMENTAL SPECIFICATIONS

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109 Measurement and Payment	1
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204 Borrow and Furnished Excavation	3
207 Porous Granular Embankment	4
211 Topsoil and Compost	5
214 Grading and Shaping Ditches	6
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420 Portland Cement Concrete Pavement	10
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RECURRING SPECIAL PROVISIONS

The following RECURRING SPECIAL PROVISIONS indicated by an "X" are applicable to this contract and are included by reference:

<u>CHECK SHEET #</u>		<u>PAGE NO.</u>
1	Additional State Requirements for Federal-Aid Construction Contracts	87
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BDE Special Provisions Checklist

January 16, 2026 & February 27, 2026 Lettings

BDE SPECIAL PROVISIONS
For the January 16 and February 27, 2026 Lettings

The following special provisions indicated by a "check mark" are applicable to this contract and will be included by the Project Coordination and Implementation Section of the Bureau of Design & Environment (BDE).

File Name	#		Special Provision Title	Effective	Revised
	80099	1	<input type="checkbox"/> Accessible Pedestrian Signals (APS)	April 1, 2003	Jan. 1, 2022
	80274	2	<input type="checkbox"/> Aggregate Subgrade Improvement	April 1, 2012	April 1, 2022
	80192	3	<input type="checkbox"/> Automated Flagger Assistance Devices	Jan. 1, 2008	April 1, 2023
	80173	4	<input type="checkbox"/> Bituminous Materials Cost Adjustments	Nov. 2, 2006	Aug. 1, 2017
	80426	5	<input type="checkbox"/> Bituminous Surface Treatment with Fog Seal	Jan. 1, 2020	Jan. 1, 2022
	80475	6	<input type="checkbox"/> Bridge Deck Concrete Overlays	Jan. 1, 2026	
*	80241	7	<input type="checkbox"/> Bridge Demolition Debris	July 1, 2009	
*	50531	8	<input type="checkbox"/> Building Removal	Sept. 1, 1990	Aug. 1, 2022
*	50261	9	<input type="checkbox"/> Building Removal with Asbestos Abatement	Sept. 1, 1990	Aug. 1, 2022
	80460	10	<input checked="" type="checkbox"/> Cement, Finely Divided Minerals, Admixtures, Concrete, and Mortar	Jan. 1, 2025	Jan. 1, 2026
	80384	11	<input checked="" type="checkbox"/> Compensable Delay Costs	June 2, 2017	April 1, 2019
*	80198	12	<input type="checkbox"/> Completion Date (via calendar days)	April 1, 2008	
*	80199	13	<input type="checkbox"/> Completion Date (via calendar days) Plus Working Days	April 1, 2008	
	80461	14	<input type="checkbox"/> Concrete Barrier	Jan. 1, 2025	
	80453	15	<input type="checkbox"/> Concrete Sealer	Nov. 1, 2023	
	80261	16	<input type="checkbox"/> Construction Air Quality – Diesel Retrofit	June 1, 2010	Jan. 1, 2025
	80476	17	<input type="checkbox"/> Deck Slab Repair	Jan. 1, 2026	
*	80029	18	<input checked="" type="checkbox"/> Disadvantaged Business Enterprise Participation	Sept. 1, 2000	Jan. 2, 2025
	80467	19	<input type="checkbox"/> Erosion Control Blanket	Aug. 1, 2025	
	80229	20	<input type="checkbox"/> Fuel Cost Adjustment	April 1, 2009	Aug. 1, 2017
	80452	21	<input type="checkbox"/> Full Lane Sealant Waterproofing System	Nov. 1, 2023	
	80433	22	<input type="checkbox"/> Green Preformed Thermoplastic Pavement Markings	Jan. 1, 2021	Jan. 1, 2022
	80471	23	<input type="checkbox"/> Guardrail	Nov. 1, 2025	
	80472	24	<input type="checkbox"/> High Friction Surface Treatment	Nov. 1, 2025	
	80456	25	<input type="checkbox"/> Hot-Mix Asphalt	Jan. 1, 2024	Jan. 1, 2026
	80446	26	<input type="checkbox"/> Hot-Mix Asphalt - Longitudinal Joint Sealant	Nov. 1, 2022	Aug. 1, 2023
	80438	27	<input type="checkbox"/> Illinois Works Apprenticeship Initiative – State Funded Contracts	June 2, 2021	April 2, 2024
	80477	28	<input type="checkbox"/> Longitudinal Tining	Jan. 1, 2026	
	80450	29	<input type="checkbox"/> Mechanically Stabilized Earth Retaining Walls	Aug. 1, 2023	Aug. 1, 2025
	80478	30	<input type="checkbox"/> Modified Longitudinal Construction Joint	Jan. 1, 2026	
	80464	31	<input type="checkbox"/> Pavement Marking	April. 1, 2025	Nov. 1, 2025
	80468	32	<input type="checkbox"/> Pavement Patching	Aug. 1, 2025	
	80441	33	<input type="checkbox"/> Performance Graded Asphalt Binder	Jan. 1, 2023	
	80459	34	<input type="checkbox"/> Preformed Plastic Pavement Marking	June 2, 2024	
*	34261	35	<input type="checkbox"/> Railroad Protective Liability Insurance	Dec. 1, 1986	Jan. 1, 2022
	80473	36	<input type="checkbox"/> Raised Reflective Pavement Markers	Nov. 1, 2025	
	80455	37	<input checked="" type="checkbox"/> Removal and Disposal of Regulated Substances	Jan. 1, 2024	April 1, 2024
	80474	38	<input type="checkbox"/> Residential Driveway Temporary Signal	Nov. 1, 2025	
	80445	39	<input type="checkbox"/> Seeding	Nov. 1, 2022	
	80457	40	<input type="checkbox"/> Short Term and Temporary Pavement Markings	April 1, 2024	April 2, 2024
	80462	41	<input checked="" type="checkbox"/> Sign Panels and Appurtenances	Jan. 1, 2025	Jan. 1, 2026
	80479	42	<input type="checkbox"/> Sinusoidal Rumble Strips	Jan. 1, 2026	
	80469	43	<input type="checkbox"/> Slope Wall	Aug. 1, 2025	
	80448	44	<input type="checkbox"/> Source of Supply and Quality Requirements	Jan. 2, 2023	Jan. 1, 2026
	80340	45	<input type="checkbox"/> Speed Display Trailer	April 2, 2014	Jan. 1, 2022
	80127	46	<input type="checkbox"/> Steel Cost Adjustment	April 2, 2004	Nov. 1, 2025
	80480	47	<input type="checkbox"/> Structural Repair of Concrete	Jan. 1, 2026	
	80397	48	<input checked="" type="checkbox"/> Subcontractor and DBE Payment Reporting	April 2, 2018	
	80391	49	<input checked="" type="checkbox"/> Subcontractor Mobilization Payments	Nov. 2, 2017	April 1, 2019
	80463	50	<input checked="" type="checkbox"/> Submission of Bidders List Information	Jan. 2, 2025	Mar. 2, 2025
	80437	51	<input checked="" type="checkbox"/> Submission of Payroll Records	April 1, 2021	Nov. 2, 2023

	80435	52	<input type="checkbox"/>	Surface Testing of Pavements – IRI	Jan. 1, 2021	Jan. 1, 2023
	80465	53	<input checked="" type="checkbox"/>	Surveying Services	April 1, 2025	
	80481	54	<input type="checkbox"/>	Temporary Concrete Barrier	Jan. 1, 2026	
	80466	55	<input type="checkbox"/>	Temporary Rumble Strips	April 1, 2025	
	80470	56	<input type="checkbox"/>	Traffic Signal Backplate	Aug. 1, 2025	
*	20338	57	<input type="checkbox"/>	Training Special Provisions	Oct. 15, 1975	Sept. 2, 2021
	80429	58	<input type="checkbox"/>	Ultra-Thin Bonded Wearing Course	April 1, 2020	Jan. 1, 2022
	80439	59	<input checked="" type="checkbox"/>	Vehicle and Equipment Warning Lights	Nov. 1, 2021	Nov. 1, 2022
	80458	60	<input type="checkbox"/>	Waterproofing Membrane System	Aug. 1, 2024	
	80302	61	<input checked="" type="checkbox"/>	Weekly DBE Trucking Reports	June 2, 2012	Jan. 2, 2025
	80454	62	<input type="checkbox"/>	Wood Sign Support	Nov. 1, 2023	
	80427	63	<input checked="" type="checkbox"/>	Work Zone Traffic Control Devices	Mar. 2, 2020	Jan. 1, 2026
*	80071	64	<input type="checkbox"/>	Working Days	Jan. 1, 2002	

Highlighted items indicate a new or revised special provision for the letting.

An * indicates the special provision requires additional information from the designer, which needs to be submitted separately. The Project Coordination and Implementation Section will then include the information in the applicable special provision.

The following special provisions are in the 2026 Supplemental Specifications and Recurring Special Provisions.

<u>File Name</u>	<u>Special Provision Title</u>	<u>New Location(s)</u>	<u>Effective</u>	<u>Revised</u>
80447	Grading and Shaping Ditches	Articles 214.03 & 214.04	Jan. 1, 2023	

Special Provisions Generated Checklist
Generated – 10/06/2026 4:26 P.M.

January 16, 2026 & February 27, 2026 Lettings

SPECIAL PROVISIONS CHECK LIST

Generated - 10/6/25 4:26 P.M.

Designer: _____ **FAP/FAS/FAI/FAU:** _____
Contract No.: _____ **Section:** _____
Lettings: 1/16/2026 & 2/27/2026 **County(ies):** _____

√	Dir	File Name	Spec Title	Spec Dates
	BRG\	APSLRP-1.docx	Approach Slab Repair	E 3/13/97
	DES\	00000.docx	STATE OF ILLINOIS	E 1/1/22 R 1/1/26
	DES\	10500.docx	Construction Station Layout	E 7/30/10
	DES\	10501.docx	Construction Layout Responsibility	E 4/26/15 R 1/1/22
	DES\	10502.docx	Construction Layout Utilizing GPS Equipment	E 4/26/15 R 1/1/22
	DES\	10503.docx	Construction Layout Equipment	E 4/26/15 R 11/6/15
	DES\	10507.docx	Removal of Abandoned Underground Utilities	E 1/15/96 R 11/21/96
	DES\	10507a.docx	Status of Utilities/Utilities To Be Adjusted	E 1/21/05 R 1/1/22
	DES\	10507b.docx	Utilities - Locations/Information on Plans	E 11/8/13
	DES\	10712.docx	Requirements When Working with the Railroad	E 4/1/16 R 4/1/22
	DES\	10713a.docx	Protection of the Illinois River	E 8/1/22 R 10/1/22
	DES\	10713b.docx	Maintenance of Navigation	E 8/1/22 R 10/1/22
√	DES\	10731.docx	Location of Underground State Maintained Facilities	E 8/3/07 R 7/31/09
	DES\	10732.docx	Right-of-Way Restrictions	E 7/1/94
	DES\	10805a.docx	Date of Completion	E 3/1/90 R 4/25/08
	DES\	10805b.docx	Date of Completion (Plus Working Days)	E 3/1/90 R 8/3/18
	DES\	20500.docx	Geotechnical Reinforcement	E 6/10/93 R 1/1/07
	DES\	20504.docx	Embankment (Restrictions)	E 1/21/05 R 8/5/22
	DES\	25000.docx	Seeding, Minor Areas	E 7/1/90 R 4/1/19
	DES\	25006a.docx	Mowing	E 12/11/01 R 8/2/13
	DES\	25006b.docx	Mowing	E 12/11/01 R 8/2/13
	DES\	25300b.docx	Seedlings	E 5/5/00 R 8/1/19
	DES\	28100.docx	Grout for Use With Riprap	E 7/30/10
	DES\	30101.docx	Proof Rolling	E 4/23/04 R 1/1/07
	DES\	30103.docx	Subgrade Treatment	E 7/1/90 R 1/1/22
	DES\	30200.docx	Soil Modification	E 7/1/90 R 1/1/22
	DES\	31100.docx	Rock Fill	E 10/15/95 R 4/26/13
	DES\	35300.docx	Sawcutting of PCC Base Course and Base Course Widening	E 1/1/16
	DES\	35500d.docx	Temporary Pavement	E 10/1/95 R 4/24/20
	DES\	35600.docx	Temporary Base Course Widening ____ "	E 4/26/13 R 4/24/20
	DES\	40600.docx	Clean Existing Pavement Edge Joint	E 1/3/00 R 4/24/20
	DES\	40604a.docx	Hot-Mix Asphalt Surface Course Surface Tests	E 11/1/03 R 1/1/07
	DES\	40607.docx	Hot-Mix Asphalt -Tack Coat (Special) Options	E 8/1/19 R 11/8/19
	DES\	40713.docx	Grooved-In V Rumble Strip	E 11/16/07 R 7/30/10

SPECIAL PROVISIONS CHECK LIST

Generated - 10/6/25 4:26 P.M.

Designer: _____ **FAP/FAS/FAI/FAU:** _____
Contract No.: _____ **Section:** _____
Lettings: 1/16/2026 & 2/27/2026 **County(ies):** _____

DES\	42401.docx	Sidewalk Drains	E 3/1/91 R 1/1/07
DES\	42402.docx	Temporary Sidewalks	E 3/1/91 R 2/1/96
DES\	44000.docx	Partial Depth Patching	E 4/26/13 R 11/6/20
DES\	44002.docx	Longitudinal Joint Repair	E 4/26/13 R 7/31/20
DES\	44003.docx	Protection of Frames and Lids of Utility Structures	E 3/6/91 R 1/1/07
DES\	44003a.docx	Hot-Mix Asphalt Surface Removal, *** (** mm)	E 3/1/93 R 1/1/22
DES\	44003b.docx	Hot-Mix Asphalt Surface Removal, *** (** mm)	E 2/5/93 R 1/1/22
DES\	44003d.docx	Pavement Drainage After Cold Milling	E 3/15/96 R 11/8/19
DES\	44003e.docx	Pavement Patching with Hot-Mix Asphalt Surface Removal	E 3/1/97 R 1/1/07
DES\	44004.docx	Hot-Mix Asphalt Joint Trimming	E 8/5/22
DES\	48205.docx	Hot-Mix Asphalt Shoulder Resurfacing Required to be Constructed Simultaneously with Mainline Paving	E 4/23/10 R 8/4/17
DES\	48206.docx	Hot-Mix Asphalt Shoulder Resurfacing Constructed Simultaneously with Mainline Paving	E 1/22/01 R 1/1/07
DES\	50103.docx	Concrete Headwall Removal	E 7/1/90
DES\	50104.docx	Concrete Handrail Removal	E 7/1/90 R 1/1/07
DES\	50301.docx	Granular Backfill for Structures	E 8/4/17 R 11/6/20
DES\	50302.docx	Surface Filler (Special)	E 4/23/10 R 10/1/23
DES\	50307.docx	PCC Placement by Pump Requirements	E 1/1/22
DES\	50312.docx	Plug Existing Deck Drains	E 1/1/96 R 11/6/20
DES\	50312a.docx	Floor Drain Extension	E 3/22/01 R 11/6/20
DES\	50319.docx	Protective Coat, Special	E 4/23/10 R 12/19/23
DES\	54200.docx	Seepage Collar	E 12/1/96
DES\	54201.docx	Remove and Relay Pipe Culvert (Special)	E 7/1/90 R 11/6/20
DES\	54202.docx	Pipe Culverts (Jacked)	E 1/1/14
DES\	54204e.docx	Backfill - Pipe Culverts	E 10/15/95 R 1/1/07
DES\	55000.docx	Storm Sewer, (Water Main Quality Pipe)	E 1/1/11 R 1/1/21
DES\	55007.docx	Backfill, Building Removal	E 8/20/91 R 1/1/07
DES\	55200.docx	Steel Pipe Culvert, Special (Jacked) * inches (* mm)	E 7/1/94 R 1/1/07
DES\	55201.docx	(*Storm Sewer/Pipe Culvert) Jacked in Place, ** inches (** mm)	E 7/1/94 R 1/1/07
DES\	56100.docx	Steel Casings * Inches	E 7/1/90 R 1/1/13
DES\	56101.docx	Steel Casings * Inches	E 7/1/90 R 1/1/13
DES\	59300.docx	Slope Wall Slurry Pumping	E 7/31/20 R 10/1/24
DES\	60200a.docx	Inlets, Type G-1	E 10/1/95 R 1/1/07
DES\	60200b.docx	Inlets, Type G-1, Special	E 10/1/95 R 1/1/07
DES\	60200c.docx	Inlets, Type G-1, Double, Special	E 10/1/95 R 1/1/07

SPECIAL PROVISIONS CHECK LIST

Generated - 10/6/25 4:26 P.M.

Designer: _____ **FAP/FAS/FAI/FAU:** _____
Contract No.: _____ **Section:** _____
Lettings: 1/16/2026 & 2/27/2026 **County(ies):** _____

	DES\	60200d.docx	Inlet Manhole, Type G-1, 4' (1.2 m) Diameter	E 10/1/95 R 1/1/07
	DES\	60200e.docx	Inlet-Manhole, Type G-1, 4' (1.2 m) Diameter, Special	E 10/1/95 R 1/1/07
	DES\	60200f.docx	Inlet-Manhole, Type G-1, 5' (1.5 m) Diameter	E 10/1/95 R 1/1/07
	DES\	60200g.docx	Inlet-Manhole, Type G-1, 5' (1.5 m) Diameter, Special	E 10/1/95 R 1/1/07
	DES\	60200h.docx	Inlet-Manhole, Type G-1, 5' (1.5 m) Diameter, Double, Special	E 10/1/95 R 1/1/07
	DES\	60200i.docx	Inlet-Manhole, Type G-1, 8' (2.4 m) Diameter, Double, Special	E 10/1/95 R 1/1/07
	DES\	60200j.docx	Manhole to be Adjusted with New Type G-1 Frame and Gate	E 10/1/95 R 1/1/07
	DES\	60200k.docx	Temporary Inlet Drainage Treatment	E 1/1/97
	DES\	60200l.docx	Inlets, Type G-2	E 11/1/03 R 1/1/07
	DES\	60200m.docx	Inlets, Type G-1, Double	E 7/31/09
	DES\	60200n.docx	Inlets, Type " * ", With Special Frame and Gate	E 8/2/13
	DES\	60200o.docx	Manhole, Type A, of the Diameter Specified with Special Frame and Gate	E 8/2/13
	DES\	60504.docx	Filling Existing Inlets	E 7/1/90 R 7/1/94
	DES\	60504a.docx	Filling Existing Culverts	E 10/15/95 R 4/1/17
	DES\	60504b.docx	Filling Drainage Structures	E 10/15/95 R 4/1/17
	DES\	60608.docx	Island Pavement Constructed on Existing Pavement	E 1/1/97 R 1/1/07
	DES\	60612.docx	Drainage Holes	E 7/1/90 R 1/1/07
	DES\	63001.docx	Guardrail Aggregate Erosion Control	E 2/1/93 R 1/1/07
	DES\	63111c.docx	Traffic Barrier Terminals	E 2/1/96 R 11/5/04
	DES\	63200.docx	Guard Post Removal	E 7/1/90 R 1/1/07
	DES\	63500.docx	Flexible Delineator Maintenance	E 5/5/92 R 1/1/94
	DES\	63501.docx	Flexible Delineators	E 10/1/95 R 1/1/07
	DES\	63502.docx	Recoverable Delineators	E 4/26/15 R 11/1/18
	DES\	67005.docx	Equipment Vault for Nuclear Testing Equipment	E 6/24/93 R 11/8/19
	DES\	68000.docx	Railroad Track Removal	E 11/1/94 R 1/1/07
	DES\	68000a.docx	Railroad Ties Removal and Disposal	E 11/1/94 R 10/1/95
	DES\	68300.docx	Mortared Stone Wall	E 3/1/91 R 1/1/07
√	DES\	70100.docx	Traffic Control Plan	E R
	DES\	70101.docx	Flaggers	E 8/3/18
	DES\	70108b.docx	Traffic Control and Protection Standard 701331 (Special)	E 10/15/95 R 7/31/09
√	DES\	70114.docx	Width Restriction Signing	E 11/1/07 R 1/1/19
	DES\	70120.docx	Traffic Control and Protection BLR 21	E 4/25/08 R 4/24/20
	DES\	70121.docx	Traffic Control and Protection BLR 22	E 4/25/08 R 4/24/20
	DES\	70400.docx	Temporary Concrete Barrier, State Owned	E 5/1/91 R 4/1/19

SPECIAL PROVISIONS CHECK LIST

Generated - 10/6/25 4:26 P.M.

Designer: _____ **FAP/FAS/FAI/FAU:** _____
Contract No.: _____ **Section:** _____
Lettings: 1/16/2026 & 2/27/2026 **County(ies):** _____

	DES\	70400a.docx	Temporary Concrete Barrier Reflectors	E 1/21/05 R 11/6/20
	DES\	73300.docx	Re-Tightening Anchor Bolts for Cantilever Sign Structures	E 4/25/14
	DES\	78201.docx	Linear Delineator Panels, 4 Inch	E 10/1/22
	DES\	81500.docx	Trench & Backfill, Special for Conduit Installation Beneath Bituminous Shoulders	E 3/21/94 R 11/6/20
	DES\	88600a.docx	Detector Loops, Type 1	E 3/1/96 R 11/6/20
	DES\	88601.docx	Adjust Existing Detector Loop Riser	E 11/7/14 R 11/6/20
	DES\	88602.docx	Miscellaneous Electrical Work	E 8/5/22
	DES\	100400.docx	PCC Slipform Paving Aggregate Optimization	E 8/3/12 R 1/1/22
	DES\	100402.docx	PCC Superstructure Aggregate Optimization	E 8/4/06 R 1/1/22
	DES\	100403b.docx	Coarse Aggregate for Bituminous Courses, Class A	E 6/29/93 R 1/1/07
	DES\	100404.docx	Aggregate Quality	E 7/1/90 R 4/26/13
	DES\	102013.docx	Membrane Curing Method	E 7/29/16 R 11/17/17
√	DES\	110300.docx	PCC QMP Electronic Report Submittals	E 1/13/22
√	DES\	110303.docx	PCC Automatic Batching Equipment	E 4/23/10 R 8/1/23

**Designer Notes
Recurring Special Provisions**

&

Guide Bridge Special Provisions (GBSP)

Designer Notes for January 1, 2026 Recurring Special Provisions
(January 16, 2026 & February 27, 2026 Lettings)

1. Designer Note: This check sheet is required in all contracts that involve Federal funds.
2. Designer Note: This check sheet is required in all Federal contracts.
3. Designer Note: This check sheet is required in all contracts.
4. Designer Note: This check sheet is required in all contracts involving State funds only.
5. Designer Note: This check sheet is required in all contracts involving State funds only.
6. Designer Note: Include in all contracts where Asbestos Bearing Pad Removal is part of the structure work.
7. Designer Note: Include in all contracts where the existing bridge deck HMA surface is to be removed and the waterproofing membrane contains asbestos and will be removed. The designer must have in the project files a completed "Asbestos Determination Certificate" for every bridge within the project limits. The District Bridge Maintenance Engineer and/or the District Hydraulics Engineer can provide copies of these certificates. If your project has any bridge deck containing asbestos, insert this special provision as well as the General Notes entitled, "Asbestos Bridge Wearing Surface Removal".
8. Designer Note: This check sheet will be required for those contracts that will involve Contractor work on haul road stream crossings, other temporary stream crossings, and in stream work pads. Contracts that would generally involve this type of work would be bridges/structures, new or rebuilt, and contracts involving earth excavation, embankment or borrow excavation. Discuss these types of work operations and any other stream related work with your Project Engineer. Any in-stream crossing or other work will require a 404 Permit from the Corps of Engineers. Be sure to let the Hydraulics Engineer know as soon as possible that a Corps permit will be needed.
9. Designer Note: Depending on IDOT manpower needs, this check sheet will be included as a pay item when the Contractor will be required to do all contract staking, including bridges. This check sheet should be used for a large box culvert or a multi pipe that will require a structure number. This would be a structure that will have a span length along survey line of more than 6 meters (20 feet). Discuss this check sheet with the Bureau of Project Implementation (Construction) as to what manpower sources are available.
10. Designer Note: This special provision specifies the requirements for geotextile fabric for use on railroad crossings. Include only on projects where the railroad crossing is a contract pay item. Also may be required for temporary crossings. Railroad crossings are generally (99%) handled by the Railroad through an agreement and not part of our contract. If in doubt as to how to handle, discuss with Project Support.
11. Designer Note: Use this check sheet where existing pavement is being reconstructed and voids are evident under the existing pavement that can be filled by grouting. Discuss with Maintenance Field Engineer responsible for the area. NOTE: A detail of the slab movement detection device is included in CADD and this drawing must be included in your contract plans.

12. Designer Note: This check sheet will be required on a contract where cold milling is required but where the cold milled area will not be overlaid. Include CADD Standard 440001 in your plans. If your contract is to be cold milled and the area overlaid, you should use one of the two District special provisions on this subject, **not** this check sheet.
13. Designer Note: This check sheet requires that once a lift of bituminous resurfacing is placed on a lane of pavement, any adjoining bituminous shoulder shall be resurfaced with an equal thickness before any other lane is resurfaced for each lift of resurfacing. Insert this special on resurfacing projects which meet the following criteria: All four lane interstates and freeways, all four lane expressways, four lane highways with ADT >25,000 or peak one-way VPH >1,700, two lane highways with ADT >10,000 or peak one-way VPH >800.
14. Designer Note: Intended to remove thick bituminous overlay so that the original pavement can be examined and then patched, if necessary. It also further defines specific pay items for work involved.
15. Designer Note: This check sheet was developed by Materials and Physical Research as an alternate to replacing Preformed Joint Sealer and Neoprene Expansion Joints up to 65 mm (2½" inches). Include with any projects that have "POLYMER CONCRETE" as a pay item.
16. Reserved.
17. Designer Note: This check sheet was developed to obtain the desired pipe coating on bike racks. Use on all projects with bike racks.
18. Designer Note: This special provision is for use on bridge contracts where staging is required, and the District wants the Contractor to have an option to post-mounting the temporary bridge and traffic signals. Discuss use with the District Traffic Control Technician.
19. Designer Note: This check sheet should be included for all projects containing roadway lighting. The designer should also include CADD Standard 701301-D4 in the plans.
20. Designer Note: This check sheet was developed to address difficulties with obtaining metric sized bolts. Include in all metric projects, which contain or could contain any type of bolted connection.
21. Designer Note: This special provision not to be used in District Four. Not recommended for use on recently constructed pavements or bridge decks. This is not recommended when there is steel in the patches due to the corrosion the calcium chloride causes.
22. Designer Note: Do not use Check Sheet #22 unless requested by Materials.
23. Designer Note: Use in all contracts involving cast-in-place concrete.
24. Reserved.
25. Reserved.
26. Designer Note: Insert into preventative maintenance contracts using cape seals or bituminous surface treatments.

27. Design Note: Insert into contracts using high-density expanding polyurethane foam or restoring the elevation of settled bridge approach pavements.
28. Designer Note: Insert into contracts using PCC inlays or overlays. Use in accordance with Chapter 53 of the *BDE Manual*.
29. Designer Note: Use on resurfacing projects to address areas which need repair, but do not warrant full depth repair. Joints and cracks, which exhibit environmental distresses, such as, spalling and "D" cracking or contains maintenance patching, are eligible for using this method of repair. Joints and cracks which exhibit load related stresses, such as pumping, alligator cracking, corner breaks, compression failures, subgrade failures, or punch-outs should not use this method on repair. Discuss use with your Project Engineer.
30. Designer Note: Consider using on contracts with longitudinal partial depth patching. There is a District Special Provision (Longitudinal Joint Repair, 440.02) that D4 prefers to use because it has different requirements. If using the BDE version and you cannot allow the milled trench to be left open overnight, specify the holes shall be filled every night.
31. Designer Note: Insert in projects with cast-in-place concrete. It is an interim measure to allow districts to transition from department mix designs to contractor mix designs.
32. Design Note: Use on all HMA overlay, Full-Dept HMA paving, and PCC pavement projects in District 4.

GUIDE BRIDGE SPECIAL PROVISION INDEX/CHECK SHEET
Effective as of the: January 16, 2026 & February 27, 2026 Lettings

√	File Name	Title	Effective	Revised
	GBSP4	Polymer Modified Portland Cement Mortar	June 7, 1994	April 1, 2016
	GBSP13	High-Load Multi-Rotational Bearings	Oct 13, 1988	June 28, 2024
	GBSP14	Jack and Remove Existing Bearings	Apr 20, 1994	April 13, 2018
	GBSP16	Jacking Existing Superstructure	Jan 11, 1993	April 13, 2018
	GBSP18	Modular Expansion Joint	May 19, 1994	Oct 27, 2023
	GBSP21	Cleaning and Painting Contact Surface Areas of Existing Steel Structures	Jun 30, 2003	Oct 23, 2020
	GBSP25	Cleaning and Painting Existing Steel Structures	Oct 2, 2001	April 15, 2022
	GBSP26	Containment and Disposal of Lead Paint Cleaning Residues	Oct 2, 2001	Apr 22, 2016
	GBSP28	Deck Slab Repair	May 15, 1995	Feb 2, 2024
	GBSP29	Bridge Deck Microsilica Concrete Overlay	May 15, 1995	April 30, 2021
	GBSP30	Bridge Deck Latex Concrete Overlay	May 15, 1995	April 30, 2021
	GBSP31	Bridge Deck High-Reactivity Metakaolin (HRM) Conc Overlay	Jan 21, 2000	April 30, 2021
	GBSP33	Pedestrian Truss Superstructure	Jan 13, 1998	Oct 27, 2023
	GBSP34	Concrete Wearing Surface	Jun 23, 1994	Oct 4, 2016
	GBSP45	Bridge Deck Thin Polymer Overlay	May 7, 1997	June 28, 2024
	GBSP53	Structural Repair of Concrete	Mar 15, 2006	Aug 9, 2019
	GBSP55	Erection of Curved Steel Structures	Jun 1, 2007	
	GBSP59	Diamond Grinding and Surface Testing Bridge Sections	Dec 6, 2004	April 15, 2022
	GBSP60	Containment and Disposal of Non-Lead Paint Cleaning Residues	Nov 25, 2004	April 22, 2016
	GBSP61	Slipform Parapet	Jun 1, 2007	April 15, 2022
	GBSP67	Structural Assessment Reports for Contractor's Means and Methods	Mar 6, 2009	Oct 5, 2015
	GBSP71	Aggregate Column Ground Improvement	Jan 15, 2009	Oct 15, 2011
	GBSP72	Bridge Deck Fly Ash or GGBF Slag Concrete Overlay	Jan 18, 2011	April 30, 2021
	GBSP78	Bridge Deck Construction	Oct 22, 2013	Dec 21, 2016
	GBSP79	Bridge Deck Grooving (Longitudinal)	Dec 29, 2014	Mar 29, 2017
	GBSP81	Membrane Waterproofing for Buried Structures	Oct 4, 2016	March 1, 2019
	GBSP82	Metallizing of Structural Steel	Oct 4, 2016	Oct 20, 2017
	GBSP83	Hot Dip Galvanizing For Structural Steel	Oct 4, 2016	June 28, 2024
	GBSP85	Micropiles	Apr 19, 1996	Oct 23, 2020
	GBSP86	Drilled Shafts	Oct 5, 2015	Oct 27, 2023
	GBSP87	Lightweight Cellular Concrete Fill	Nov 11, 2001	Apr 1, 2016
	GBSP88	Corrugated Structural Plate Structures	Apr 22, 2016	April 13, 2018
	GBSP89	Preformed Pavement Joint Seal	Oct 4, 2016	March 24, 2023
	GBSP90	Three Sided Precast Concrete Structure (Special)	Dec 21, 2016	March 22, 2024
	GBSP91	Crosshole Sonic Logging Testing of Drilled Shafts	Apr 20, 2016	March 24, 2023
	GBSP92	Thermal Integrity Profile Testing of Drilled Shafts	Apr 20, 2016	March 24, 2023
	GBSP93	Preformed Bridge Joint Seal	Dec 21, 2016	June 28, 2024
	GBSP94	Warranty for Cleaning and Painting Steel Structures	Mar 3, 2000	Nov 24, 2004
	GBSP96	Erection of Bridge Girders Over or Adjacent to Railroads	Aug 9, 2019	
	GBSP97	Folded/Formed PVC Pipeliner	April 15, 2022	
	GBSP98	Cured-in-Place Pipe Liner	April 15, 2022	
	GBSP99	Spray-Applied Pipe Liner	April 15, 2022	
	GBSP100	Bar Splicers, Headed Reinforcement	Sept 2, 2022	Oct. 27, 2023
	GBSP101	Noise Abatement Wall, Ground Mounted	Dec 9, 2022	June 28, 2024
	GBSP102	Noise Abatement Wall, Structure Mounted	Dec 9, 2022	June 28, 2024
	GBSP103	Noise Abatement Wall Anchor Rod Assembly	Dec 9, 2022	

LIST ADDITIONAL SPECIAL PROVISIONS BELOW

The following Guide Bridge Special Provisions have been incorporated into other specifications:

File Name	Title	Location
GBSP12	Drainage System	SSRBC 523
GBSP15	Three Sided Precast Concrete Structure	Superseded by GBSP90
GBSP51	Pipe Underdrain for Structures	SSRBC 601
GBSP56	Setting Piles in Rock	SSRBC 512
GBSP75	Bond Breaker for Prestressed Concrete Bulb-T Beams	SSRBC 504

BDE Special Provisions

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106.01	10601	Source of Supply and Quality Requirements
107.01	10701	Construction Air Quality – Diesel Retrofit
107.11a	10711a	Railroad Protective Liability Insurance
107.19a	10719a	Building Removal with Asbestos Abatement
107.19d	10719d	Building Removal
107.38	10738	Bridge Demolition Debris
107.40	10740	Compensable Delay Costs
108.05	10805	Working Days
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus Working Days
108.06	10806	Training Special Provision
108.06a	10806a	Disadvantaged Business Enterprise Participation
108.06b	10806b	Weekly DBE Trucking Reports
108.06c	10806c	Illinois Works Apprenticeship Initiative – State Funded Contracts
109.00a	10900a	Steel Cost Adjustment
109.01	10901	Bituminous Materials Cost Adjustments
109.03	10903	Fuel Cost Adjustment
109.12	10912	Subcontractor Mobilization Payments
109.13	10913	Submission of Payroll Records
109.14	10914	Subcontractor and DBE Payment Reporting
250.07	25007	Seeding
251.00	25100	Erosion Control Blanket

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
303.00	30300	Aggregate Subgrade Improvement
403.00	40300	Bituminous Surface Treatment with Fog Seal
405.50	40550	Ultra-Thin Bonded Wearing Course
406.06	40606	Hot-Mix Asphalt – Longitudinal Joint Sealant
406.11	40611	Surface Testing of Pavements - IRI
409.00	40900	High Friction Surface Treatment
420.05	42005	Modified Longitudinal Construction Joint
420.09	42009	Longitudinal Tining
442.06	44206	Pavement Patching
511.00	51100	Slope Wall
530.00	53000	Deck Slab Repair
531.00	53100	Bridge Deck Concrete Overlays
532.00	53200	Structural Repair of Concrete
581.01	58101	Full Lane Sealant Waterproofing System
630.03	63003	Guardrail
637.12	63712	Concrete Barrier
642.00	64200	Sinusoidal Rumble Strips
667.04	66704	Surveying Services
669.04	66904	Removal and Disposal of Regulated Substances
701.00	70100	Automated Flagger Assistance Devices
701.03	70103	Work Zone Traffic Control Devices
701.08	70108	Vehicle and Equipment Warning Lights
701.15	70115	Speed Display Trailer
701.15a	70115a	Temporary Rumble Strips
701.18	70118	Residential Driveway Temporary Signal

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
704.02	70402	Temporary Concrete Barrier
720.02	72002	Sign Panels and Appurtenances
730.02	73002	Wood Sign Support
780.13	78013	Pavement Marking
780.14	78014	Green Preformed Thermoplastic Pavement Markings
781.03	78103	Raised Reflective Pavement Markers
888.00	88800	Accessible Pedestrian Signals (APS)
1001.01	100101	Cement, Finely Divided Minerals, Admixtures, Concrete, and Mortar
1003.07	100307	Mechanically Stabilized Earth Retaining Walls
1026.01	102601	Concrete Sealer
1030.07	103007	Hot-Mix Asphalt
1032.05	103205	Performance Graded Asphalt Binder
1061.03	106103	Waterproofing Membrane System
1078.03	107803	Traffic Signal Backplate
1095.03	109503	Preformed Plastic Pavement Marking
1095.06	109506	Short Term and Temporary Pavement Markings

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NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 100

Get a copy of the current check list from the Studies & Plans Secretary, indicate which ISP's are to be included in your set of special provisions, fill in any blanks as indicated on the check list, and include with your set of special provisions to be sent to Springfield where they will be inserted.

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
102.01	10201	Submission of Bidders List Information
106.01	10601	Source of Supply and Quality Requirements
107.01	10701	Construction Air Quality – Diesel Retrofit
107.11a	10711a	Railroad Protective Liability Insurance
107.19a	10719a	Building Removal with Asbestos Abatement
107.19d	10719d	Building Removal
107.38	10738	Bridge Demolition Debris
107.40	10740	Compensable Delay Costs
108.05	10805	Working Days
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus Working Days
108.06	10806	Training Special Provision
108.06a	10806a	Disadvantaged Business Enterprise Participation
108.06b	10806b	Weekly DBE Trucking Reports
108.06c	10806c	Illinois Works Apprenticeship Initiative – State Funded Contracts
109.00a	10900a	Steel Cost Adjustment
109.01	10901	Bituminous Materials Cost Adjustments
109.03	10903	Fuel Cost Adjustment
109.12	10912	Subcontractors Mobilization Payments
109.13	10913	Submission of Payroll Records
109.14	10914	Subcontractor and DBE Payment Reporting

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 200

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
250.07	25007	Seeding
251.00	25100	Erosion Control Blanket

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 300

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
303.00	30300	Aggregate Subgrade Improvement

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 400

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
403.00	40300	Bituminous Surface Treatment with Fog Seal
405.50	40550	Ultra-Thin Bonded Wearing Course
406.06	40606	Hot-Mix Asphalt – Longitudinal Joint Sealant
406.11	40611	Surface Testing of Pavements – IRE
409.00	40900	High Friction Surface Treatment
420.05	42005	Modified Longitudinal Construction Joint
420.09	42009	Longitudinal Tining
442.06	44206	Pavement Patching

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 500

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
511.00	51100	Slope Wall
530.00	53000	Deck Slab Repair
531.00	53100	Bridge Deck Concrete Overlays
532.00	53200	Structural Repair of Concrete
581.01	58101	Full Lane Sealant Waterproofing System

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 600

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
630.03	63003	Guardrail
637.12	63712	Concrete Barrier
642.00	64200	Sinusoidal Rumble Strips
667.04	66704	Survey Services
669.04	66904	Removal and Disposal of Regulated Substances

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 700

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
701.00	70100	Automated Flagger Assistance Devices
701.03	70103	Work Zone Traffic Control Devices
701.08	70108	Vehicle and Equipment Warning Lights
701.15	70115	Speed Display Trailer
701.15a	70115a	Temporary Rumble Strips
701.18	70118	Residential Driveway Temporary Signal
704.02	70402	Temporary Concrete Barrier
720.02	72002	Sign Panels and Appurtenances
730.02	73002	Wood Sign Support
780.13	78013	Pavement Marking
780.14	78014	Green Preformed Thermoplastic Pavement Markings
781.03	78103	Raised Reflective Pavement Markers

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 800

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
888.00	88800	Accessible Pedestrian Signals (APS)

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 900

Standard
Spec. No.PC
No.Item

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

SECTION 1000

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
1001.01	100101	Cement, Finely Divided Minerals, Admixtures, Concrete, and Mortar
1003.07	100307	Mechanically Stabilized Earth Retaining Walls
1026.01	102601	Concrete Sealer
1032.05	103205	Performance Graded Asphalt Binder
1061.03	106103	Waterproofing Membrane System
1078.03	107803	Traffic Signal Backplate
1095.03	109503	Preformed Plastic Pavement Marking
1095.06	109506	Short Term and Temporary Pavement Markings

BDE Special Provisions

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ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Get a copy of the current check list from the Program Development Secretary, indicate which ISP's are to be included in your set of special provisions, fill in any blanks as indicated on the check list, and include with your set of special provisions to be sent to Springfield where they will be inserted.

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
888.00	88800	Accessible Pedestrian Signals (APS)
303.00	30300	Aggregate Subgrade Improvement
701.00	70100	Automated Flagger Assistance Devices
109.01	10901	Bituminous Materials Cost Adjustment
403.00	40300	Bituminous Surface Treatment with Fog Seal
531.00	53100	Bridge Deck Concrete Overlays
107.38	10738	Bridge Demolition Debris
107.19a	10719a	Building Removal with Asbestos Abatement
107.19d	10719d	Building Removal
1001.01	100101	Cement, Finely Divided Minerals, Admixtures, Concrete, and Mortar
107.40	10740	Compensable Delay Costs
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus Working Days
637.12	63712	Concrete Barrier
1026.01	102601	Concrete Sealer
107.01	10701	Construction Air Quality – Diesel Retrofit
530.00	53000	Deck Slab Repair
108.06a	10806a	Disadvantaged Business Enterprise Participation
251.00	25100	Erosion Control Blanket
109.03	10903	Fuel Cost Adjustment
581.01	58101	Full Lane Sealant Waterproofing System

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ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
630.03	63003	Guardrail
780.14	78014	Green Preformed Thermoplastic Pavement Markings
409.00	40900	High Friction Surface Treatment
1030.07	103007	Hot-Mix Asphalt
406.06	40606	Hot-Mix Asphalt – Longitudinal Joint Sealant
108.06c	10806c	Illinois Works Apprenticeship Initiative – State Funded Contracts
420.09	42009	Longitudinal Tining
1003.07	100307	Mechanically Stabilized Earth Retaining Walls
420.05	42005	Modified Longitudinal Construction Joint
780.13	78013	Pavement Marking
442.06	44206	Pavement Patching
1032.05	103205	Performance Graded Asphalt Binder
1095.03	109503	Preformed Plastic Pavement Marking
107.11	10711a	Railroad Protective Liability Insurance
781.03	78103	Raised Reflective Pavement Markers
669.04	66904	Removal and Disposal of Regulated Substances
701.18	70118	Residential Driveway Temporary Signal
250.07	25007	Seeding
1095.06	109506	Short Term and Temporary Pavement Markings
720.02	72002	Sign Panels and Appurtenances
642.00	64200	Sinusoidal Rumble Strips
511.00	51100	Slope Wall

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ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
106.01	10601	Source of Supply and Quality Requirements
701.15	70115	Speed Display Trailer
109.00	10900a	Steel Cost Adjustment
532.00	53200	Structural Repair of Concrete
109.14	10914	Subcontractor and DBE Payment Reporting
109.12	10912	Subcontractor Mobilization Payments
102.01	10201	Submission of Bidders List Information
109.13	10913	Submission of Payroll Records
406.11	40611	Surface Testing of Pavements – IRI
667.04	66704	Surveying Services
704.02	70402	Temporary Concrete Barrier
701.15a	70115a	Temporary Rumble Strips
1078.03	107803	Traffic Signal Backplate
108.06	10806	Training Special Provision
405.50	40550	Ultra-Thin Bonded Wearing Course
701.08	70108	Vehicle and Equipment Warning Lights
1061.03	106103	Waterproofing Membrane System
108.06b	10806b	Weekly DBE Trucking Reports
730.02	73002	Wood Sign Support
108.05	10805	Working Days
701.03	70103	Work Zone Traffic Control Devices

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District Special Provisions

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SECTION 100

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
105.00	CONSTRUCTION STATION LAYOUT	10500
105.01	CONSTRUCTION LAYOUT RESPONSIBILITY	10501
105.02	CONSTRUCTION LAYOUT UTILIZING GPS EQUIPMENT	10502
105.03	CONSTRUCTION LAYOUT EQUIPMENT	10503
105.07	REMOVAL OF ABANDONED UNDERGROUND UTILITIES	10507
105.07a	STATUS OF UTILITIES/UTILITIES TO BE ADJUSTED	10507a
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BDE Special Provisions

Designer Note: Insert into all federal-aid contracts.

SOURCE OF SUPPLY AND QUALITY REQUIREMENTS (BDE)

Effective: January 2, 2023

Revised: January 1, 2026

Revise the third through ninth paragraphs of Article 106.01 of the Standard Specifications to read:

"Articles, materials, and supplies shall be classified into only one of the following categories.

- (a) Iron and Steel. All iron and steel products, which are to be incorporated into the work, shall be domestically manufactured or produced and fabricated, unless an exception is expressly permitted under Federal and/or State law and written permission is given by the Department. The Contractor shall obtain from the iron or steel producer and/or fabricator, in addition to the mill analysis, a certification that all iron or steel materials meet these domestic source requirements.

The applications of all coatings, epoxy, galvanizing, painting, etc. to iron and steel products shall be domestically applied.

- (b) Manufactured Products. Manufactured products shall include articles, materials or supplies that have been processed into a specific form or shape; or have been combined with other articles, materials, or supplies to create a product with different properties than the individual articles, materials, or supplies. Manufactured products incorporated into the work shall have the final assembly for the manufacturing process occur domestically.

A manufactured product may include components that are construction materials, iron or steel products, or exempt materials.

Precast concrete products and intelligent transportation systems (ITS) or other electronic hardware systems shall comply with the requirements of Article 106.01(a) in addition to the requirements of manufactured products.

- (c) Construction Materials. All manufacturing processes for construction materials shall occur within the United States. Construction materials shall include an article, material, or supply consisting of only one of the following.

- (1) Non-ferrous metals;

- (2) Plastic and polymer-based products (including polyvinylchloride, composite building materials, and polymers used in fiber optic cables);

- (3) Glass (including optic glass);

- (4) Fiber optic cable (including drop cable);

- (5) Optical fiber;

(6) Lumber;

(7) Drywall;

(8) Engineered wood.

Minor additions of articles, materials, supplies, or binding agents to a construction material do not change the categorization of the construction material.

- (d) Exempt Materials. Materials exempt from domestic production requirements are cement or cementitious materials, aggregates, aggregate binding agents or additives, or items not permanently incorporated into the work. Exempt materials may be combined with other materials into a final form to produce a manufactured product."

Designer Note: This special provision should be inserted into contracts that contain a modified longitudinal construction joint for PCC pavement.

MODIFIED LONGITUDINAL CONSTRUCTION JOINT (BDE)

Effective: January 1, 2026

Add the following to Article 420.05 to the Standard Specifications:

- "(f) Modified Longitudinal Construction Joint. Modified longitudinal construction joints shall be according to Article 420.05(b), except a 3/4 in. (19 mm) dowel bar shall be used in place of the tie bar and the minimum pull-out strength criteria and testing is not applicable. In addition, the face of the slab shall be coated with a bond breaking application of curing compound, and a light coating of oil shall be uniformly applied to the dowel bar."

Designer Note: This special provision should be inserted into contracts containing Portland cement concrete pavement, continuously reinforced Portland cement concrete pavement, and Portland cement concrete shoulders.

LONGITUDINAL TINING (BDE)

Effective: January 1, 2026

Revise the first three paragraphs of Article 420.09(e)(1) of the Standard Specifications to read:

- "(1) Type A. Type A final finish shall be obtained by the use of a carpet drag composed of an artificial turf followed immediately by a mechanically operated metal comb longitudinal tining device.

The artificial turf shall be made of molded polyethylene with synthetic turf blades approximately 0.85 in. (20 mm) long and contain approximately 7,200 individual blades per 1 sq. ft. (0.1 sq. m). The artificial turf shall be attached to a device that will permit control of the time and rate of texturing. The artificial turf carpet shall be full pavement width and of sufficient size that during the finishing operation, approximately 2 ft. (600 mm) of carpet in the direction of drag (i.e., parallel to the pavement centerline) will be in contact with the pavement surface over the entire pavement width. The drag shall be operated in a longitudinal direction to produce a uniform appearing finish. If necessary for maintaining contact with the pavement surface, the carpet may be weighted.

The metal comb shall consist of a single line of tempered spring steel tines uniformly spaced at 3/4 in. (19 mm). The tines shall be flat and of a size and stiffness sufficient to produce a groove of the specified dimensions in the plastic concrete without tearing of the pavement surface. The mechanically operated metal comb shall be either an exclusive piece of equipment which is mechanically self-propelled or shall be combined with the curing equipment. The artificial turf carpet drag may be attached to this piece of equipment provided a surface texture is produced satisfactory to the Engineer. The tining device shall be operated to produce a pattern of grooves, 1/8 to 3/16 in. (3 to 5 mm) deep and 1/10 to 1/8 in. (2.5 to 3 mm) wide along the pavement in a single pass. The tining shall be operated parallel to the longitudinal joint or edge of pavement and shall not deviate more than 1 in. (25 mm) in 25 ft. (8 m). Tining shall be withheld 1 to 1 1/2 in. (25 to 38 mm) from a longitudinal joint or pavement edge.

Hand tining or tining with a mechanically operated comb combined with the curing equipment specified in Article 1101.09 will be permitted where the specifications permit hand finishing or screeds, one lane construction up to 16 ft. (5 m) wide, gaps, projects with a net length of 1/2 mile (800 m) or less, and where the production rate on any paving day will be less than 1,500 cu. yd. (1,200 cu. m) per day. A foot bridge shall be provided for the hand tining operation for all pavement over 12 ft. (3.6 m) wide, unless it can be demonstrated that an alternate texturing operation produces satisfactory results."

This special provision should be inserted into contracts containing a bridge deck concrete overlay, HMA surface removal (deck), or deck slab repair.

DECK SLAB REPAIR

Effective: January 1, 2026

Add the following Section 530 to the Standard Specifications:

"SECTION 530. DECK SLAB REPAIR

530.01 Description. This work shall consist of the removal of existing hot-mix asphalt (HMA) surface and loose and deteriorated concrete from a bridge deck, and the replacement with new concrete to the existing top of deck.

Work will be classified as follows.

Removal Type	Description
HMA Surface Removal	Removal of the HMA surface from the concrete bridge deck.
Deck Slab Repair (Partial)	Removal and replacement of the concrete deck, at least 3/4 in. (20 mm) but not more than 1/2 of the concrete deck thickness.
Deck Slab Repair (Full Depth, Type I)	Removal and replacement of the concrete deck to full depth, for an area 1 ft. x 1 ft. (300 mm x 300 mm) to 5 sq. ft. (0.5 sq. m).
Deck Slab Repair (Full Depth, Type II)	Removal and replacement of the concrete deck to full depth, for an area greater than 5 sq. ft. (0.5 sq. m).

530.02 Materials. Materials shall be according to the following.

Item	Article/Section
(a) Portland Cement Concrete (Note 1)	1020
(b) Reinforcement Bars	508

Note 1. Class PP-1, PP-2, PP-3, PP-4, PP-5, or BS concrete shall be used unless otherwise noted on the plans.

530.03 Equipment. Equipment shall be according to the following.

Item	Article/Section
(a) Concrete Equipment	1020.03
(b) Finishing Equipment	503.03
(c) Hydrodemolition Equipment	1101.11
(e) Hydro-Scarification Equipment (Note 1)	

Note 1. Hydro-scarification equipment shall consist of filtering and pumping units operating with a computerized, self-propelled robotic machine capable of removing unsound concrete in a single pass to the specified depth and operating at a 16,000 psi (110 MPa) minimum water pressure with a 55 gal./min. (208 L/min.) minimum water flow rate.

CONSTRUCTION REQUIREMENTS

530.04 General. Sidewalks, curbs, drains, reinforcement, existing transverse and longitudinal joints, and other appurtenances which are to remain in place shall be protected from damage during removal and cleaning operations.

The Contractor shall control the runoff water generated by the various construction activities in such a manner as to minimize, to the maximum extent practicable, the discharge of untreated effluent into adjacent waters, and shall properly dispose of the solids generated according to Article 202.03. The Contractor shall submit a water management plan to the Engineer specifying the control measures to be used. The control measures shall be in place prior to the start of runoff water generating activities. Runoff water shall not be allowed to constitute a hazard to adjacent or underlying roadways, waterways, drainage areas or railroads nor be allowed to erode existing slopes.

530.05 HMA Surface Removal. The existing HMA surface and waterproofing membrane shall be removed and disposed of according to Articles 440.04 and 440.06, except milling equipment will not be allowed if the deck is to receive a waterproofing membrane system. Removal of the HMA surface by the use of radiant or direct heat will not be permitted. If the HMA surface or waterproofing membrane contains asbestos fibers, removal shall be according to the Recurring Special Provision "Asbestos Waterproofing Membrane or Asbestos Hot-Mix Asphalt Surface Removal".

530.06 Concrete Removal. Concrete shall be removed, as determined by the Engineer, in areas of loose, disintegrated, and unsound concrete. Areas designated for partial depth repair in which unsound concrete is found to extend below half the concrete deck thickness shall be removed to full depth.

A concrete saw shall be used to provide vertical edges 3/4 in. (20 mm) deep around the perimeter of the area to be patched. Where reinforcement bars lack sufficient concrete cover, the depth shall be reduced as directed by the Engineer. A saw cut will not be required if the deck is to get an overlay, or along the face of the curb, parapet or joint, or when sharp vertical edges are provided by hydrodemolition.

- (a) Partial-Depth. The loose and unsound concrete shall be removed using power-driven hand tools or hydrodemolition equipment. Power-driven hand tools include jackhammers less than or equal to the nominal 45 lb. (20 kg) class. Chipping hammers heavier than the nominal 15 lb. (7 kg) class shall not be used for removing concrete from below reinforcing bars, or for removal within 1 ft. (300 mm) of existing beams, girders, or other structural members that are to remain in service. Jackhammers and chipping hammers shall not be operated at an angle less than 45 degrees measured from the surface of the deck.
- (b) Full Depth. Full depth removal shall be performed according to Article 501.05 except that hydraulic impact equipment may be permitted in areas of full depth removal more than 1 ft. (300 mm) away from the edges of existing beams, girders or other supporting structural members or more than 1 ft. (300 mm) from the boundaries of full depth repairs.

Forms for full depth repair may be supported by hangers with adjustable bolts or by blocking from the beams below. When approved by the Engineer, forms for Type I patches may be supported by No. 9 (3.8 mm) wires or other devices attached to the reinforcement bars.

Exposed reinforcement bars and newly exposed concrete shall be blast cleaned. If the Engineer determines the bond between existing concrete and reinforcement bars within the patch area has been destroyed, the concrete adjacent to the bar shall be removed to a depth that will permit new concrete to bond to the entire periphery of the exposed bar. A minimum of 1 in. (25 mm) clearance will be required. If the Engineer enlarges a designated removal area due to

deterioration beyond the limits previously designated, a new saw cut shall be made around the extended area before additional removal is begun. The removal area shall not be enlarged solely to correct debonded reinforcement or deficient lap lengths.

530.07 Reinforcement Treatment. Care shall be exercised during concrete removal to protect the reinforcement bars and structural steel from damage. Damage to the reinforcement bars or structural steel to remain in place shall be repaired or replaced. Existing reinforcement bars shall remain in place and loose bars shall be tied. Reinforcing bars which have been removed with the patch or lost at least 25 percent of their original cross-sectional area due to corrosion shall be supplemented by new in-kind reinforcement bars. New bars shall be lapped a minimum of 32 bar diameters to existing bars. A mechanical splicer capable of developing in tension at least 125 percent of the yield strength of the existing bar shall be used when it is not feasible to provide the minimum bar lap. No welding of bars will be permitted.

530.08 Cleaning. After concrete removal and reinforcement repairs, the repair areas shall be cleaned of dust and debris. Once the initial cleaning is completed, the repair areas shall be blast cleaned to a roughened appearance free from concrete fines and foreign matter.

Exposed reinforcement bars shall be free of dirt, detrimental scale, paint, oil, or other foreign substances which may reduce bond with the concrete. Loose, scaling rust shall be removed by rubbing with burlap, wire brushing, blast cleaning, or another method approved by the Engineer.

Dust, concrete fines, debris, and water resulting from the blast cleaning shall be confined and immediately removed. If concrete placement does not follow immediately after the final cleaning, the area shall be protected with anchored polyethylene sheeting.

530.09 Placing. Concrete placement shall be according to Articles 503.07, 503.16, 1020.14(b), and the following.

The patch area shall be cleaned to the satisfaction of the Engineer and shall be thoroughly wetted and maintained in a dampened condition with water for at least 12 hours before placement of the concrete. Excess water shall be removed by compressed air or by vacuuming prior to the beginning of concrete placement. Water shall not be applied to the surface within one hour before or at any time during placement of the concrete.

When an overlay system is not specified, the patches shall be finished according to Article 503.16(a), followed by a light brooming.

530.10 Curing and Protection. Concrete patches shall be cured according to Articles 1020.13(a)(3) or (a)(5). The curing period shall be three days for Class PP-1, PP-2, PP-3, PP-4, and PP-5 concrete, and seven days for Class BS concrete. When the air temperature is below 55°F (13°C), the Contractor shall cover the patch according to Article 1020.13(d)(1) with minimum R12 insulation.

A 72-hour minimum drying period shall be required before placing waterproofing or HMA surfacing.

530.11 Opening to Traffic. No traffic will be permitted on a patch until after the cure period and the concrete has obtained a minimum compressive strength of 4,000 psi (27.6 MPa) or flexural strength of 675 psi (4.65 MPa). Construction equipment will be permitted on a patch during the cure period if the concrete has obtained the minimum required strength when the specimens are cured with the patch.

530.12 Method of Measurement. HMA surface removal and deck slab repair will be measured for payment in place and the area computed in Square Yards (Square Meters).

530.13 Basis of Payment. HMA surface removal will be paid for at the contract unit price per Square Yard (Square Meter) for HOT-MIX ASPHALT SURFACE REMOVAL (DECK).

Areas removed and replaced up to and including a depth of half the concrete deck thickness will be paid for at the contract unit price per Square Yard (Square Meter) for DECK SLAB REPAIR (PARTIAL). Areas removed and replaced to a depth greater than half the concrete deck thickness will be paid for at the contract unit price per Square Yard (Square Meter) for DECK SLAB REPAIR (FULL DEPTH, of the type specified).

Concrete protected according to Article 1020.13(d) will be paid according to Article 503.22.

Reinforcement bars repaired in-kind, as approved by the Engineer, will be paid according to Article 109.04."

This special provision should be inserted into contracts containing a BRIDGE DECK MICROSILICA CONCRETE OVERLAY, BRIDGE DECK LATEX CONCRETE OVERLAY, BRIDGE DECK HRM CONCRETE OVERLAY, or BRIDGE DECK GGBF OR FLY ASH CONCRETE OVERLAY.

BRIDGE DECK CONCRETE OVERLAYS

Effective: January 1, 2026

Add the following Section 531 to the Standard Specifications:

"SECTION 531. BRIDGE DECK CONCRETE OVERLAYS

531.01 Description. This work shall consist of constructing a concrete overlay on an existing concrete bridge deck or approach slab.

531.02 Materials. Materials shall be according to the following:

Item	Article/Section
(a) Coarse Aggregate (Note 1)	1004.02
(b) Packaged, Dry, Rapid Hardening Mortar or Concrete	1018
(c) Portland Cement Concrete (Note 2) (Note 3)	1020
(d) Fibers for Concrete (Note 4).....	1014

Note 1. Coarse aggregate shall be crushed stone or crushed gravel.

Note 2. Concrete shall be Class BS with the following exceptions.

Overlay Type	Gradation
Fly Ash Ground Granulated Blast Furnace (GGBF) Slag High Reactivity Metakaolin (HRM) Microsilica	CA 11, CA 13, CA 14, or CA 16
Latex	CA 13, CA 14, or CA 16

Slump shall be 2-7 in. (50-175 mm). For latex overlays, the maximum slump may be increased if no visible segregation is observed.

For latex overlays, air content shall be a maximum of 7.0 percent and maximum water/cement ratio shall be 0.40.

For Class F fly ash overlays, fly ash shall replace cement at the maximum rate permitted according to Article 1020.05(c)(1)b. The replacement ratio shall be 1:1.

For Class C fly ash overlays, fly ash shall replace cement at a rate ranging from a minimum of 25 percent to the maximum rate permitted according to Article 1020.05(c)(1)c. The replacement ratio shall be 1:1.

For GGBF slag overlays, GGBF slag shall replace cement at a rate ranging from a minimum of 25 percent to the maximum rate permitted according to Article 1020.05(c)(2)b. The replacement ratio shall be 1:1.

For microsilica and HRM overlays, the cementitious factor (excluding microsilica or HRM) shall be 565 lb./cu. yd. (390 kg/cu. m). The minimum amount of microsilica or HRM added to the mix shall be 33 lb./cu. yd. (20 kg/cu. m) and the maximum shall be 37 lb./cu. yd. (22 kg/cu m).

For latex overlays, the cementitious factor shall be a minimum of 658 lbs./cu. yd. (335 kg/cu. m), the dosage of the latex admixture shall be 24.5 gal./cu. yd. (121.3 L/cu. m), the coarse aggregate shall be 42-50 percent by weight (mass) of total aggregate, no air entraining admixture shall be used, and water used shall be a maximum of 157 lb./cu. yd. (93.1 kg/cu. m) (including free moisture on the fine and coarse aggregates).

Note 3. For HRM and microsilica overlays, the Contractor shall submit a specific batching sequence for verification by the Engineer. The submittal shall include whether a truck or stationary mixer will be used. For HRM overlays, the submittal shall include whether bulk or bagged HRM will be used. For microsilica overlays, the submittal shall include whether the microsilica to be used is a water-based slurry, bulk densified, or bagged densified.

Note 4. Synthetic fibers, when required, shall be according to Article 1014.01, and 1014.04 or 1014.05, as applicable.

531.03 Equipment. Equipment shall be according to the following.

<u>Item</u>	<u>Article/Section</u>
(a) Concrete Equipment (Note 1)	1020.03
(b) Finishing Equipment	503.03
(c) Fogging Equipment	1103.17(k)
(d) Mechanical Scarifying Equipment (Note 2)	1101.16
(e) Hydro-Scarification Equipment (Note 3)	
(f) Hand-Held Blast Cleaning Equipment (Note 4)	
(g) Mechanical Blast Cleaning Equipment (Note 5)	
(h) Pull-Off Test Equipment (Note 6)	

Note 1. For latex concrete overlays, a mobile Portland cement concrete plant shall be used according to Articles 1020.12 and 1103.04.

Note 2. Mechanical scarifying equipment shall be capable of uniformly scarifying or removing the old concrete surface and new patches to the depth required. The minimum width of the equipment shall be 3 ft. (1 m).

Note 3. Hydro-scarification equipment shall consist of filtering and pumping units operating with a computerized, self-propelled robotic machine capable of removing sound concrete in a single pass to the specified depth and operating at a 16,000 psi (110 MPa) minimum water pressure with a 55 gal./min (208 L/min) minimum water flow rate.

Note 4. Hand-held blast cleaning shall be performed by high-pressure waterblasting or abrasive blasting. High-pressure waterblasting equipment shall have a minimum water pressure of 7,000 psi (48 MPa) and have oil traps.

Note 5. Mechanical blast cleaning shall be performed by high-pressure waterblasting or shotblasting capable of removing weak concrete at the surface, including the microfractured concrete surface layer remaining as a result of mechanical scarification, and shall have oil traps.

Mechanical high-pressure waterblasting equipment shall be mounted on a wheeled carriage, include multiple nozzles mounted on a rotating assembly, and shall be operated with a 7,000 psi (48 MPa) minimum water pressure. The distance between the nozzles

and the deck surface shall be kept constant and the wheels shall maintain contact with the deck surface during operation.

Note 6. Equipment used to perform pull-off testing shall be on the Department's qualified product list "Pull-Off Test Equipment for Bridge Deck Overlays".

CONSTRUCTION REQUIREMENTS

531.04 General. The Contractor shall control the runoff water generated by the various construction activities in such a manner as to minimize, to the maximum extent practicable, the discharge of untreated effluent into adjacent waters, and shall properly dispose of the solids generated according to Article 202.03. The Contractor shall submit a water management plan to the Engineer specifying the control measures to be used. The control measures shall be in place prior to the start of runoff water generating activities. Runoff water shall not be allowed to constitute a hazard to adjacent or underlying roadways, waterways, drainage areas or railroads nor be allowed to erode existing slopes.

531.05 Preparation. Sidewalks, curbs, drains, reinforcement, existing transverse and longitudinal joints, and other appurtenances which are to remain in place shall be protected from damage during scarification and cleaning operations.

- (a) Scarification. The scarification shall consist of removing the concrete using mechanical and hydro-scarifying equipment to the depth specified on the plans. The depth specified shall be measured from the existing concrete surface to the grout line between aggregates remaining after scarification. Power-driven hand tools shall be used for removal around areas to remain in place and areas not accessible to the scarifying equipment. Power-driven hand tools include jackhammers lighter than the nominal 45 lb. (20 kg) class. Jackhammers or chipping hammers shall be operated at an angle less than 45 degrees measured from the surface of the slab.

Mechanical scarification equipment shall be used to remove the initial 1/4 in. (25 mm) minimum depth of concrete, creating a uniform roughened concrete surface. If the Contractor's use of mechanical scarifying equipment results in exposing, snagging, or dislodging the top mat of reinforcing steel, the mechanical scarifying depth shall be reduced as necessary immediately. If the exposing, snagging, or dislodging the top mat of reinforcing steel cannot be avoided, the mechanical scarifying shall be stopped immediately, and the remaining removal shall be accomplished using the hydro-scarification equipment. All damage to the existing reinforcement resulting from the Contractor's operation shall be repaired or replaced at no additional cost to the Department. Replacement shall include the removal of any additional concrete required to position or splice the new reinforcing steel. Undercutting of exposed reinforcement bars shall only be as required to replace or repair damaged reinforcement. Repairs to existing reinforcement shall be according to Section 530.

- (b) Sounding. Just prior to performing hydro-scarification, the deck shall be sounded, with unsound areas marked on the deck by the Engineer. A trial section, in an area of sound concrete, on the existing deck surface will be designated by the Engineer to calibrate the equipment settings to remove sound concrete to the required depth, in a single pass, and provide a highly roughened bondable surface. The trial section shall consist of approximately 30 sq. ft. (3 sq. m). After calibration in an area of sound concrete, the equipment shall be moved to a second trial section, as designated by the Engineer, in an area containing unsound concrete to verify the calibrated settings are sufficient to remove the unsound concrete. If the calibrated settings are insufficient to remove the unsound concrete, the equipment may be moved back to an area of sound concrete and the calibration settings verified.

After the equipment settings are established, they shall be supplied to the Engineer and include the following.

- (1) Water pressure
- (2) Water flow rate
- (3) Nozzle type and size
- (4) Nozzle travel speed
- (5) Machine staging control (step/advance rate)

Hydro-scarification may begin after the calibration settings have been approved by the Engineer. At least the last 1/2 in. (13 mm) of removal shall be accomplished with hydro-scarification equipment.

The removal depth will be verified by the Engineer. If sound concrete is being removed below the desired depth, the equipment shall be recalibrated.

- (c) **Cleaning.** After hydro-scarification, the concrete shall be vacuum cleaned before the water and debris are allowed to dry and re-solidify to the deck. The vacuum shall be equipped with fugitive dust control devices capable of removing wet debris and water in the same pass. The vacuum shall also be capable of washing the deck with pressurized water prior to the vacuum operation to dislodge all debris and slurry from the deck surface. The use of alternative cleaning and debris removal methods to minimize driving heavy vacuum equipment over the exposed reinforcement may be used subject to the approval of the Engineer.

531.06 Patching. After scarification and cleaning, the Engineer will sound the scarified concrete and survey the existing reinforcement condition. All remaining unsound concrete and unacceptably corroded reinforcement bars will be marked for additional removal and/or repairs and shall be completed according to Section 530 except as noted below.

In areas where unsound concrete extends below the specified removal depth and hydro-scarification completely removes unsound concrete, a full-depth repair shall be required when the bottom mat of reinforcement is exposed.

Full-depth patches shall be struck off to the scarified surface and then roughened with a suitable stiff bristled broom or wire brush to provide a rough texture. Hand finishing of the patch surface shall be kept to a minimum to prevent overworking of the surface.

Any removal required or made below the specified depth for scarification, which does not result in full-depth repair, shall be filled with the overlay material at the time of the overlay placement.

Undercutting of exposed reinforcement bars shall only be as required to replace or repair damaged or corroded reinforcement, or to perform a full depth repair.

531.07 Final Surface Preparation. Final surface preparation shall include the cleaning of all dust, debris, concrete fines, and other foreign substances from the surface including vertical faces of curbs, previously placed adjacent overlays, barrier walls up to a height of 1 in. (25 mm) above the overlay, depressions, and beneath reinforcement bars using hand-held blast cleaning methods.

If spoils from the scarification operation are allowed to dry and re-solidify on the surface, the surface shall be cleaned with mechanical blast cleaning equipment. Areas determined by the Engineer to be inaccessible to scarifying equipment shall be blast cleaned with hand-held equipment.

The Engineer may require surface pull-off testing of areas inaccessible to scarifying equipment. The Contractor shall provide the testing equipment in accordance with Illinois Test Procedure 304 "Pull-Off Test (Surface Method)". Prior to the start of testing, the Contractor shall submit to the Engineer a technical data sheet and material safety data sheet for the epoxy used to perform the testing. For solvents used to clean the equipment, a material safety data sheet shall be submitted. The Engineer will determine each test location, and each individual test shall have a minimum strength of 175 psi (1,207 kPa). In the case of a failing test, the Contractor shall adjust the blast cleaning method and re-clean the area. Testing will be repeated until satisfactory results are attained.

Exposed reinforcement bars shall be free of dirt, detrimental scale, paint, oil, and other foreign substances which may reduce bond with the concrete. Loose, scaling rust shall be removed by rubbing with burlap, wire brushing, blast cleaning, or another method approved by the Engineer. Loose reinforcement bars, as determined by the Engineer, shall be retied.

Dust, concrete fines, debris, and water resulting from the surface preparation shall be confined and immediately removed. If concrete placement does not follow immediately after the final cleaning, the area shall be protected with anchored white polyethylene sheeting.

531.08 Equipment Preparation. Before placing the overlay, the finishing machine shall be operated over the full length of bridge segment to be overlaid to check support rails for deflection and confirm the minimum overlay thickness. If adjustments are needed, another check shall be performed.

531.09 Placing. Concrete placement shall be according to Article 503.07, 503.16, and the following.

The surface shall be cleaned to the satisfaction of the Engineer and shall be thoroughly wetted and maintained in a dampened condition with water for at least 12 hours before placement of the overlay. Excess water shall be removed by compressed air or by vacuuming prior to the beginning of overlay placement. Water shall not be applied to the surface within one hour before or at any time during placement of the overlay.

Internal vibration shall be performed along edges, adjacent to bulkheads, and where the overlay thickness exceeds 3 in. (75 mm). Internal vibration along the longitudinal edges of a pour shall be performed with a minimum of two hand-held vibrators, one on each edge of the pour. Hand finishing shall be performed along the edges of the pour from sidewalks, curbs, or work bridges.

A construction dam or bulkhead shall be installed in case of a delay of 30 minutes or more in the concrete placement operation.

Construction joints shall be formed. When required by the Engineer, the previously placed overlay shall be sawed full depth to a straight and vertical edge before fresh concrete is placed. The Engineer will determine the extent of the removal. When longitudinal joints are not shown on the plans, the locations shall be subject to approval by the Engineer and shall not be located in the wheel paths.

The Contractor shall stencil the date of construction (month and year) and the appropriate letters into the overlay before it takes its final set, as follows.

- (a) MS, for MicroSilica
- (b) MSFA, when fly ash is used in the silica fume mix design, for MicroSilica Fly Ash
- (c) MSF, for MicroSilica with Fibers
- (d) MSFAF, when fly ash is used in the silica fume mix design, for MicroSilica Fly Ash with Fibers
- (e) LX, for LateX

- (f) LXF, for LateX with Fibers
- (g) HRM, for High-Reactivity Metakaolin
- (h) HRMFA, when fly ash is used in the HRM mix design, for High-Reactivity Metakaolin Fly Ash
- (i) HRMF, for High-Reactivity Metakaolin with Fibers
- (j) HRMFAF, when fly ash is used in the HRM mix design, for High-Reactivity Metakaolin Fly Ash with Fibers
- (k) FA, for Fly Ash
- (l) FAF, for Fly Ash with Fibers
- (m) GGBF, for Ground Granulated Blast Furnace slag
- (n) GGBFF, for Ground Granulated Blast Furnace slag with Fibers

The stencil shall be in a noticeable location, as determined by the Engineer, for each stage of construction outside of the grooving where possible and within 3 ft. (1 m) of an abutment joint. The characters shall be 3 in. to 4 in. (75 mm to 100 mm) in height, 1/4 in. (5 mm) in depth and face the centerline of the roadway.

531.10 Weather and Other Limitations. Temperature control for concrete placement shall be according to Article 1020.14(b). The concrete protection from low air temperatures during the curing period shall be according to Article 1020.13(d). Concrete shall not be placed when rain is expected during the working period. If night placement is required, illumination and placement procedures will be subject to approval by the Engineer.

Concrete delivery vehicles driven on the structure shall be limited to a maximum load of 6 cu. yd. (4.6 cu. m).

Truck mixers, concrete pumps, or other heavy equipment will not be permitted on any portion where the top reinforcing mat has been exposed. Conveyors, buggy ramps, and pump piping shall be installed in a way that will not displace undercut reinforcement bars. Air compressors may be operated on the deck only if located directly over a pier and supported off undercut reinforcement bars. Compressors will not be allowed to travel over undercut reinforcement bars.

Concrete removal may proceed during final cleaning and concrete placement on adjacent portions, provided the removal does not interfere with the cleaning or placement operations.

Water or contaminants from the hydro-scarification shall not be permitted in areas where the new overlay has been placed until the overlay has cured a minimum of 24 hours.

No concrete shall be removed within 6 ft. (1.8 m) of a newly placed overlay until the concrete has obtained a minimum compressive strength of 3,000 psi (20,700 kPa) or flexural strength of 600 psi (4,150 kPa).

531.11 Curing. For silica fume, HRM, GGBF, and fly ash concrete overlays, the surface shall be continuously wet cured for at least seven days according to Articles 1020.13(a)(5) or (a)(6).

For latex modified concrete overlays, the surface shall be continuously wet cured for at least 48 hours according to Articles 1020.13(a)(3), (a)(5), or (a)(6). After the wet cure is completed, all layers of covering materials shall be removed to allow for a dry cure for at least 48 hours. If the ambient temperature falls below 45°F (10°C) during either the wet or dry curing periods, the time below 45°F (10°C) will not be included in the 96-hour curing period. If there is sufficient rain to wet the surface of the overlay for more than one hour of the dry cure period, the wet time will not be included in the 48-hour dry cure period.

When the mats or blankets have been pre-dampened, excess water shall not be allowed to drip onto the overlay during placement.

531.12 Testing. The Engineer reserves the right to conduct pull-off tests on the overlay to determine if any areas are not bonded to the underlying concrete. The overlay will be tested according to the Illinois Test Procedure 305 "Pull-Off Test (Overlay Method)", and the Contractor shall provide the test equipment. Prior to the start of testing, the Contractor shall submit to the Engineer a technical data sheet and material safety data sheet for the epoxy used to perform the testing. For solvents used to clean the equipment, a material safety data sheet shall be submitted. Each individual test shall have a minimum strength of 150 psi (1,034 kPa). If the test results are unacceptable, the overlay shall be removed and replaced at the locations determined by the Engineer. When removing portions of an overlay, the saw cut shall be a minimum depth of 1 in. (25 mm).

If the overlay is to remain in place, all core holes due to testing shall be filled with a rapid set mortar or concrete. Only enough water to permit placement and consolidation by rodding shall be used, and the material shall be struck-off flush with the adjacent material.

For a rapid set mortar mixture, one part packaged rapid set cement shall be combined with two parts fine aggregate, by volume; or a packaged rapid set mortar shall be used. For a rapid set concrete mixture, a packaged rapid set mortar shall be combined with coarse aggregate according to the manufacturer's instructions; or a packaged rapid set concrete shall be used. Mixing of a rapid set mortar or concrete shall be according to the manufacturer's instructions.

531.13 Opening to Traffic. No traffic or construction equipment will be permitted on the overlay until after the cure period and the concrete has obtained a minimum compressive strength of 4,000 psi (27,500 kPa) or flexural strength of 675 psi (4,650 kPa).

531.14 Method of Measurement. Scarification will be measured for payment in Square Yards (Square Meters). No additional payment will be made for multiple passes of the equipment.

The concrete overlay will be measured for payment in Square Yards (Square Meters).

Additional concrete placed with the overlay, required to fill all depressions below the specified thickness will be measured for payment in Cubic Yards (Cubic Meters). The volume will be determined by subtracting the theoretical volume of the overlay from the ticketed volume of overlay delivered minus the volume estimated by the Engineer left in the last truck at the end of the overlay placement. The theoretical Cubic Yard (Cubic Meter) quantity for the overlay will be determined by multiplying the plan surface area of the overlay times the specified thickness of the overlay.

Partial depth removal in concurrence with hydro-scarification will not be measured for payment.

531.15 Basis of Payment. Scarification will be paid for at the contract unit price per Square Yard (Square Meter) for BRIDGE DECK SCARIFICATION, of the depth specified.

Silica fume concrete overlay will be paid for at the contract unit price per Square Yard (Square Meter) for BRIDGE DECK MICROSILICA CONCRETE OVERLAY, of the thickness specified. Latex modified concrete overlay will be paid for at the contract unit price per Square Yard (Square Meter) for BRIDGE DECK LATEX CONCRETE OVERLAY, of the thickness specified. HRM concrete overlay will be paid for at the contract unit price per Square Yard (Square Meter) for BRIDGE DECK HRM CONCRETE OVERLAY, of the thickness specified. GGBF or fly ash concrete overlay will be paid for at the contract unit price per Square Yard (Square Meter) for BRIDGE DECK GGBF OR FLY ASH CONCRETE OVERLAY, of the thickness specified.

The additional volume of overlay required to fill all depressions below the specified thickness and/or for grade adjustments will be paid for at the Contractor's actual material cost for the concrete per Cubic Yard (Cubic Meter) times an adjustment factor. For volumes 15 percent or

less over the theoretical volume of the overlay, the adjustment factor will be 1.15. For volumes greater than 15 percent, the adjustment factor will be 1.25 for that volume over 15 percent of the theoretical volume of the overlay.

Concrete protected according to Article 1020.13(d) will be paid according to Article 503.22.

Areas requiring additional partial depth or full depth removal of unsound concrete after hydro-scarification, as determined by the Engineer, will be paid according to Article 109.04.

Reinforcement bars repaired in-kind, as approved by the Engineer, will be paid according to Article 109.04.

When the Engineer conducts pull-off tests on the existing surface or overlay and they are acceptable, Contractor expenses incurred due to testing and filling core holes will be paid according to Article 109.04. Unacceptable pull-off tests will be at no additional cost to the Department."

Designer Note: This special provision should be inserted into all contracts with STRUCTURAL REPAIR OF CONCRETE.

STRUCTURAL REPAIR OF CONCRETE (BDE)

Effective: January 1, 2026

Add the following Section 532 to the Standard Specifications:

"SECTION 532. STRUCTURAL REPAIR OF CONCRETE

532.01 Description. This work shall consist of repairing concrete on a bridge substructure, culvert, retaining wall, or other structure.

532.02 Materials. Materials shall be according to the following.

<u>Item</u>	<u>Article/Section</u>
(a) Portland Cement Concrete (Note 1)	1020
(b) R1, R2, or R3 Concrete.....	1018
(c) Packaged, Dry, Combined Materials for Concrete.....	1017.02, 1017.03
(d) High Performance Shotcrete	1015
(e) Reinforcement Bars	1006.10
(f) Anchor Bolts.....	1006.09
(g) Water	1002
(h) Curing Compound	1022.01
(i) Cotton Mats.....	1022.02
(j) Protective Coat	1023
(k) Epoxy (Note 2)	1025
(l) Mechanical Bar Splicers.....	508.06(c)
(m) Polymer Modified Portland Cement Mortar (Note 3)	

Note 1. The concrete shall be Class SI, except the cement and finely divided minerals shall be a minimum 6.65 cwt/cu. yd. (395 kg/cu. m) with no reductions permitted, the cement shall not be below 4.70 cwt/cu. yd. (279 kg/cu. m), the coarse aggregate shall be a CA 16, the strength shall be a minimum 4,000 psi (27,500 kPa) at 28 days, and the slump shall be 5-10 in. (125-250 mm). A self-consolidating concrete mixture will also be permitted. Slump flow range, visual stability index, J-Ring, L-Box, and hardened visual stability index requirements shall be according to Article 1020.04.

Note 2. ASTM C 881, Type IV, Grade 2 or 3, Class A, B, or C may also be used.

Note 3. Polymer modified Portland cement mortar shall be a packaged product consisting of cementitious materials, fine aggregate, and a polymer modifier; and shall have successfully completed and remain current with the AASHTO Product Eval and Audit Rapid Hardening Concrete Patching Materials (RHCP) testing program. The mortar shall be a workable mix capable of bonding and holding its own plastic weight when mixed and placed according to manufacturer's instructions on vertical and overhead surfaces. It shall have a minimum compressive strength of 1,500 psi (10,300 kPa) at 24 hours, 4,000 psi (27,500 kPa) at 7 days, and 5,000 psi (34,500 kPa) at 28 days when tested according to ASTM C 109 or AASHTO T 106. In addition, the mortar shall have a minimum bond strength of 1,500 psi (10,300 kPa) at seven (7) days when tested according to ASTM C 882. For prestressed concrete applications, the mortar shall have a water-soluble chloride ion content of less than 0.06 percent by weight of cementitious material when tested according to ASTM C 1218 or AASHTO T 260; and for non-prestressed

concrete applications, the water-soluble chloride ion content shall be less than 0.15 percent by weight of cementitious material. The Department will maintain a Qualified Product List of Polymer Modified Portland Cement Mortar.

532.03 Equipment. Equipment shall be according to Article 503.03 and the following.

<u>Item</u>	<u>Article/Section</u>
(a) Hydrodemolition Equipment	1101.11
(b) High Performance Shotcrete Equipment (Note1)	

Note 1. The batching, mixing, pumping, hose, nozzle, and auxiliary equipment shall be for the wet-mix shotcrete method and meet the requirements of ACI 506R.

CONSTRUCTION REQUIREMENTS

532.04 General. The repair method shall be either formed concrete repair or shotcrete according to the following.

- (a) Formed Concrete Repair. For formed concrete repair, a subsequent patch to repair the placement point after initial concrete placement will not be allowed. As an example, this may occur in a vertical location located at the top of the repair.

Formed concrete repair shall not be used for overhead applications except as noted for polymer modified Portland cement mortar.

Polymer modified Portland cement mortar shall only be used for repairs between 3/8 in. (10 mm) and 2 in. (50 mm) deep on horizontal, vertical, and overhead surfaces.

- (b) Shotcrete. Shotcrete shall not be used for any repair greater than 6 in. (150 mm) in depth, except in horizontal applications where the shotcrete may be placed from above in one lift.

Shotcrete shall not be used for column repairs greater than 4 in. (100 mm) in depth, unless the shotcrete mixture contains 3/8 in. (9.5 mm) aggregate.

The Contractor shall provide a current copy of ACI 506R to the Engineer a minimum of one week prior to the start of construction.

The shotcrete personnel who perform the work shall have current American Concrete Institute (ACI) nozzle men certification for vertical wet and overhead wet applications, except one individual may be in training. This individual shall be adequately supervised by a certified ACI nozzle men as determined by the Engineer and a copy of the nozzle men certificate(s) provided to the Engineer.

532.05 Temporary Shoring or Cribbing. When a temporary shoring or cribbing support system is required, the Contractor shall provide details and computations, prepared and sealed by an Illinois Licensed Structural Engineer, to the Department for approval. Whenever possible, the support system shall be installed prior to starting the associated concrete removal. If no system is specified, but during the course of removal the need for temporary shoring or cribbing becomes apparent or is directed by the Engineer due to a structural concern, the Contractor shall not proceed with any further removal work until an approved support system is installed.

532.06 Concrete Removal. The Contractor shall provide ladders or other appropriate equipment for the Engineer to mark the removal areas. Repair configurations will be kept simple, and squared corners will be indicated whenever possible. The repair perimeter shall be sawed a depth of 1/2 in. (13 mm) or less, as required to avoid cutting the reinforcement. If the concrete is

broken or removed beyond the limits of the initial saw cut, the new repair perimeter shall be recut. The areas to be repaired shall have all loose, unsound concrete removed completely by the use of hydrodemolition equipment or chipping hammer with a 15 lb. (7 kg) maximum class. The concrete removal shall extend along the reinforcement bar until the reinforcement is free of bond inhibiting corrosion. Reinforcement bars with 50 percent or more exposed area shall be undercut to a depth of 3/4 in. (19 mm) or the diameter of the reinforcement bar, whichever is greater.

The minimum repair depth shall be 1 in. (25 mm). The substrate profile shall be $\pm 1/16$ in. (± 1.5 mm). The perimeter of the repair area shall have a vertical face.

If sound concrete is encountered before existing reinforcement bars are exposed, further removal of concrete shall not be performed unless the minimum repair depth is not met.

If a repair is located at the ground line, any excavation required below the ground line to complete the repair shall be included in this work.

Once concrete removal has started, the Contractor shall have 14 calendar days to complete each repair location.

The Engineer shall be notified of concrete removal that exceeds 6 in. (150 mm) in depth, one fourth the cross section of a structural member, more than half the vertical column reinforcement is exposed in a cross section, more than six consecutive reinforcement bars are exposed in any direction, within 1.5 in. (38 mm) of a bearing area, or other structural concern. Excessive deterioration or removal may require further evaluation of the structure or installation of temporary shoring and cribbing support system.

532.07 Surface Preparation. Prior to placing the concrete or shotcrete, the Contractor shall blast clean the repair area and exposed reinforcement to provide a surface that is free of oil, dirt, and loose material.

The repair area and perimeter vertical face shall have a rough surface. Just prior to concrete or shotcrete placement, the repair area shall be saturated with water to a saturated surface-dry condition. Any standing water shall be removed.

If a succeeding layer of shotcrete is to be applied, the initial shotcrete surface and remaining exposed reinforcement shall be free of curing compound, oil, dirt, loose material, rebound, and overspray. Preparation may be by lightly brushing or blast cleaning if the previous shotcrete surface was less than 36 hours prior. If more than 36 hours prior, the surface shall be prepared by blast cleaning.

532.08 Reinforcement. Exposed reinforcement bars shall be cleaned of concrete and corrosion by blast cleaning. After cleaning, exposed reinforcement will be evaluated to determine if replacement or additional reinforcement bars are required.

Reinforcing bars that have been cut or have lost 25 percent or more of their original cross-sectional area shall be supplemented by new in-kind reinforcement bars. New bars shall be lapped a minimum of 32 bar diameters to existing bars. A mechanical bar splicer shall be used when it is not feasible to provide the minimum bar lap. No welding of bars will be permitted.

Intersecting reinforcement bars shall be tightly secured to each other using 0.006 in. (0.15 mm) or heavier gauge tie wire and shall be adequately supported to minimize movement during concrete placement or application of shotcrete.

Where the depth of concrete removal is greater than 8 in. (205 mm) and there is no existing reinforcement extending into the repair area, the Contractor shall anchor the new concrete to the existing concrete with 3/4 in. (19 mm) diameter hook bolts spaced at 15 in. (380 mm) maximum

centers both vertically and horizontally, and a minimum of 12 in. (305 mm) away from the perimeter of the repair according to Section 584.

532.09 Repair Methods. Within 3 calendar days of the surface preparation and after the repair areas are approved by the Engineer, the concrete or shotcrete shall be placed according to the following.

- (a) **Formed Concrete Repair.** Falsework and forms shall be according to Articles 503.05 and 503.06. Formwork shall provide a smooth and uniform concrete finish and shall approximately match the existing concrete structure. Air vents may be provided to reduce voids and improve surface appearance. The Contractor may use exterior mechanical vibration, as approved by the Engineer, to release air pockets. The concrete shall be Class SI concrete; packaged R1, R2, or R3 concrete; packaged, dry, combined materials for concrete; or polymer modified Portland cement mortar. The concrete shall be placed and consolidated according to Article 503.07.

Curing shall be according to Article 1020.13.

The surfaces of the completed repair shall be finished according to Article 503.15.

- (b) **Shotcrete.** In the field, shotcrete shall be tested for air content according to Illinois Modified AASHTO T 152 or AASHTO T 152. The sample shall be obtained from the discharge end of the nozzle by shooting a pile large enough to scoop a representative amount for filling the air meter measuring bowl. Shotcrete shall not be shot directly into the measuring bowl for testing.

Compressive strength shall be according to ASTM C 1140, except the test panel shall be cured according to Article 1020.13(a)(3) or (5) while stored at the jobsite and during delivery to the laboratory. At the discretion of the Engineer, the test panel may be reduced to 18x18x3.5 in. (457x457x89 mm).

The method of alignment control (i.e., ground wires, guide strips, depth gages, depth probes, and formwork) to ensure the specified shotcrete thickness and reinforcing bar cover is obtained shall be according to ACI 506R. Ground wires shall be removed after completion of cutting operations. Guide strips and formwork shall be of dimensions and a configuration that do not prevent proper application of shotcrete. Metal depth gauges shall be cut 1/4 in. (6 mm) below the finished surface. All repaired members shall be restored as close as practicable to their original dimensions.

Air temperature limits according to the first paragraph of Article 1020.14(b) shall apply in cold weather. Shotcrete shall not be applied when the air temperature is greater than 90 F (32 C). The applied shotcrete shall have a minimum temperature of 50°F (10°C) and a maximum temperature of 90°F (32 C). The shotcrete shall not be applied during periods of rain unless protective covers or enclosures are installed. If necessary, lighting shall be provided to provide a clear view of the shooting area.

The shotcrete shall be applied according to ACI 506R and shall be done in a manner that does not result in cold joints, laminations, sandy areas, voids, sags, or separations. In addition, the shotcrete shall be applied in a manner that results in maximum densification of the shotcrete. Shotcrete which is identified as being unacceptable while still plastic shall be removed and re-applied.

The nozzle shall normally be at a distance of 2-5 ft. (0.6-1 m) from the receiving surface and shall be oriented at right angles to the receiving surface. Exceptions to this requirement will be permitted to fill corners, encase large diameter reinforcing bars, or as approved by the Engineer. For any exception, the nozzle shall never be oriented more than 4 degrees from the surface. Care shall be taken to keep the front face of the

reinforcement bar clean during shooting operations. Shotcrete shall be built up from behind the reinforcement bar. Accumulations of rebound and overspray shall be continuously removed prior to application of new shotcrete. Rebound material shall not be incorporated in the work.

Whenever possible, shotcrete shall be applied to the full thickness in a single layer. When two or more layers are required, the minimum number shall be used and shall be done in a manner without sagging or separation. A flash coat (i.e. a thin layer of up to 1/4 in. (6 mm) applied shotcrete) may be used as the final lift for overhead applications.

Prior to application of a succeeding layer of shotcrete, the initial layer of shotcrete shall be prepared according to the surface preparation and reinforcement bar cleaning requirements. Upon completion of the surface preparation and reinforcement bar treatment, water shall be applied according to the surface preparation requirements unless the surface is moist. The second layer of shotcrete shall then be applied within 30 minutes.

Shotcrete shall be cut back to line and grade using trowels, cutting rods, screeds or other suitable devices. The shotcrete shall be allowed to stiffen sufficiently before cutting. For depressions, cut material may be used for small areas. Rebound material shall not be incorporated in the work. For the final finish, a wood float or a manufacturer approved finishing aid shall be used to approximately match the existing concrete texture.

Contractor operations for curing shall be continuous with shotcrete placement and finishing operations. Curing shall be accomplished using wetted cotton mats, membrane curing, or a combination of both. Cotton mats shall be applied according to Article 1020.13(a)(5), except the exposed layer of shotcrete shall be covered within 10 minutes after finishing and wet curing shall begin immediately. Curing compound shall be applied according to Article 1020.13(a)(4), except the curing compound shall be applied as soon as the shotcrete has hardened sufficiently to prevent marring the surface, and each of the two separate applications shall be applied in opposite directions to ensure coverage. Note 5 of the Index Table in Article 1020.13 shall also apply.

When a shotcrete layer is to be covered by a succeeding shotcrete layer within 36 hours, the repair area shall be protected by wet curing with either burlap or cotton mats until the succeeding layer is applied. Intermittent hand fogging may be used for the first hour and a half if wet curing with mats begins within 10 minutes after fogging has ceased.

The curing period shall be for seven (7) days, except when there is a succeeding layer of shotcrete. In this instance, the initial shotcrete layer shall be cured until the surface preparation and reinforcement bar treatment is started.

The concrete or shotcrete shall not be placed when frost is present on the surface of the repair area, or the surface temperature of the repair area is less than 40 F (4 C). All repaired members shall be restored as close as practicable to their original dimensions.

If temperatures below 45°F (7 C) are forecast during the curing period, Protection Method I according to Article 1020.13(d)(1) or Protection Method II according to Article 1020.13(d)(2) shall be used.

532.10 Protective Coat. Protective coat shall be applied to the completed repair at reinforcement bar locations with less than 3/4 in. (19 mm) of cover according to Article 503.19, except blast cleaning shall be performed to remove curing compound.

532.11 Inspection. After curing but no sooner than 28 days after placement of concrete or shotcrete, the Contractor shall provide ladders or other necessary equipment for the Engineer to inspect the repaired areas.

A repaired area with cracks, voids, or delamination shall be considered as nonconforming. Any of the following shall be cause for removal and replacement of a repaired area.

- (a) A single surface crack greater than 0.01 in. (0.25 mm) in width and greater than 12 in. (300 mm) in length.
- (b) Two or more surface cracks greater than 0.01 in. (0.25 mm) in width that total greater than 24 in. (600 mm) in length.
- (c) Map cracking in one or more regions totaling 15 percent or more of the gross surface area of the repair.
- (d) Two or more surface voids with least dimension of 3/4 in. (19 mm) each.
- (e) A repaired area not within 1/4 in. (6 mm) of the original dimensions.

Cracks greater than 0.007 in. (0.2 mm) in width shall be repaired with epoxy according to Section 590. For cracks less than or equal to 0.007 in. (0.2 mm) in width, the epoxy shall be applied to the surface of the crack as determined by the Engineer. Voids shall be repaired according to Article 503.15.

532.12 Method of Measurement. This work will be measured for payment in place and the area computed in Square Feet (Square Meters). For a repair at a corner, both sides will be measured.

532.13 Basis of Payment. This work will be paid for at the contract unit price per Square Foot (Square Meter) for STRUCTURAL REPAIR OF CONCRETE (DEPTH GREATER THAN 5 INCHES or STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES.

Concrete protected according to Article 1020.13(d) will be paid according to Article 503.22.

When not specified to be paid for elsewhere, the work to design, install, and remove the temporary shoring and cribbing will be paid according to Article 109.04.

With the exception of reinforcement damaged by the Contractor, supplemental reinforcement bars, mechanical bar splicers, hook bolts, and protective coat will be paid according to Article 109.04."

Designer Note: This special provision should be inserted into contracts containing SINUSOIDAL RUMBLE STRIPS, 14 INCH.

SINUSOIDAL RUMBLE STRIPS (BDE)

Effective: January 1, 2026

Revise Section 642 of the Standard Specifications to read:

"SECTION 642. RUMBLE STRIPS

642.01 Description. This work shall consist of constructing rumble strips into new or existing pavement.

642.02 Equipment. The equipment shall be a self-propelled milling machine with a rotary-type cutting head(s). The cutting head(s) shall be suspended from the machine such that it can align itself with the slope of the pavement and any irregularities in the pavement surface. The teeth of the cutting head(s) shall be arranged to provide a smooth cut, with no more than a 1/8 in. (3 mm) difference between peaks and valleys.

Prior to commencement of the work, the Contractor shall demonstrate the ability of the equipment to achieve the desired results without damaging the pavement.

CONSTRUCTION REQUIREMENTS

642.03 General. The rumble strips shall be cut to the dimensions shown on the plans.

Rumble strips shall be omitted within the limits of structures, entrances, side roads, entrance ramps, and exit ramps. In Portland cement concrete, rumble strips shall not be placed within 6 in. (150 mm) of transverse joints.

Cuttings resulting from this operation shall be disposed of according to Article 202.03 and the pavement swept clean.

642.04 Method of Measurement. This work will be measured for payment in place in Feet (Meters) along the edge of pavement for shoulder applications and along the centerline of pavement for centerline applications. Measurement will include both the cut and uncut sections of the rumble strips with exceptions for bridge decks, approach slabs, turn lanes, entrances, and other sections where rumble strips have been omitted.

642.05 Basis of Payment. This work will be paid for at the contract unit price per Foot (Meter) for SHOULDER RUMBLE STRIPS, 8 INCH; SHOULDER RUMBLE STRIPS, 16 INCH; or SINUSOIDAL RUMBLE STRIPS, 14 INCH."

Designer Note: Insert into all contracts.

WORK ZONE TRAFFIC CONTROL DEVICES (BDE)

Effective: March 2, 2020

Revised: January 1, 2026

Add the following to Article 701.03 of the Standard Specifications:

"(q) Temporary Sign Supports 1106.02"

Revise Article 701.03(p) of the Standard Specifications to read:

"(p) Detectable Pedestrian Channelizing Barricades 1106.02(m)"

Revise the third paragraph of Article 701.14 of the Standard Specifications to read:

"For temporary sign supports, the Contractor shall provide a FHWA eligibility letter for each device used on the contract. The letter shall provide information for the set-up and use of the device as well as a detailed drawing of the device. The signs shall be supported within 20 degrees of vertical. Weights used to stabilize signs shall be attached to the sign support per the manufacturer's specifications."

Revise the first paragraph of Article 701.15 of the Standard Specifications to read:

"701.15 Traffic Control Devices. For devices that must meet crashworthiness standards, the Contractor shall provide a manufacturer's self-certification or a FHWA eligibility letter for each Category 1 device and a FHWA eligibility letter for each Category 2 and Category 3 device used on the contract. The self-certification or letter shall provide information for the set-up and use of the device as well as a detailed drawing of the device."

Revise the first six paragraphs of Article 1106.02 of the Standard Specifications to read:

"1106.02 Devices. Work zone traffic control devices and combinations of devices shall meet crashworthiness standards for their respective categories. The categories are as follows.

Category 1 includes small, lightweight, channelizing and delineating devices that have been in common use for many years and are known to be crashworthy by crash testing of similar devices or years of demonstrable safe performance. These include cones, tubular markers, plastic drums, and delineators, with no attachments (e.g. lights). Category 1 devices shall be MASH compliant.

Category 2 includes devices that are not expected to produce significant vehicular velocity change but may otherwise be hazardous. These include vertical panels with lights, barricades, temporary sign supports, and Category 1 devices with attachments (e.g. drums with lights). Category 2 devices shall be MASH compliant.

Category 3 includes devices that are expected to cause significant velocity changes or other potentially harmful reactions to impacting vehicles. These include crash cushions (impact attenuators), truck mounted attenuators, and other devices not meeting the definitions of

Category 1 or 2. Category 3 devices manufactured after December 31, 2019, shall be MASH compliant. Category 3 devices manufactured on or before December 31, 2019, and compliant with NCHRP 350, may be used on contracts let before December 31, 2029. Category 3 devices shall be crash tested for Test Level 3 or the test level specified.

Category 4 includes portable or trailer-mounted devices such as sign supports, speed feedback displays, arrow boards, changeable message signs, temporary traffic signals, and area lighting supports. It is preferable for Category 4 devices manufactured after December 31, 2019, to be MASH-16 compliant; however, there are currently no crash tested devices in this category, so it remains exempt from the NCHRP 350 or MASH compliance requirement.

For each type of device, when no more than one MASH compliant is available, an NCHRP 350 compliant device may be used, even if manufactured after December 31, 2019."

Revise the first paragraph of Section 1106.02(a) of the Standard Specifications to read:

"(a) Lights. Lights shall meet the requirements of Chapter 13 of the "Equipment and Materials Standards of the Institute of Transportation Engineers," 1998, Institute of Transportation Engineers, and shall be visible on a clear night from a distance of 3000 ft. (900 m). Lights are classified as follows."

Revise Articles 1106.02(g), 1106.02(k), 1106.02(l), and 1106.02(m) of the Standard Specifications to read:

"(g) Truck Mounted/Trailer Mounted Attenuators. The attenuator shall be approved for use at Test Level 3. Test Level 2 may be used for normal posted speeds less than or equal to 45 mph.

(k) Temporary Water Filled Barrier. The water filled barrier shall be a lightweight plastic shell designed to accept water ballast and be on the Department's qualified product list.

Shop drawings shall be furnished by the manufacturer and shall indicate the deflection of the barrier as determined by acceptance testing; the configuration of the barrier in that test; and the vehicle weight, velocity, and angle of impact of the deflection test. The Engineer shall be provided one copy of the shop drawings.

(l) Movable Traffic Barrier. The movable traffic barrier shall be on the Department's qualified product list.

Shop drawings shall be furnished by the manufacturer and shall indicate the deflection of the barrier as determined by acceptance testing; the configuration of the barrier in that test; and the vehicle weight, velocity, and angle of impact of the deflection test. The Engineer shall be provided one copy of the shop drawings. The barrier shall be capable of being moved on and off the roadway on a daily basis.

(m) Detectable Pedestrian Channelizing Barricades. The top panel or handrail shall be continuous and there should be at least a 2 in. (50 mm) gap between the hand trailing edge and its support. When visible to vehicular traffic, the top rail shall have alternating white and orange retroreflective stripes sloping at 45 degrees. The bottom panel shall be continuous and have alternating white and orange retroreflective stripes sloping at 45 degrees. Barricade stripes shall be 6 in. (150 mm) in width. The predominant color for other barricade components shall be white, orange, or silver."

This special provision should be inserted into contracts with temporary concrete barrier.

TEMPORARY CONCRETE BARRIER (BDE)

Effective: January 1, 2026

Add the following to Article 704.02 of the Standard Specifications:

"(f) Type C Reflector 1097.02(c)"

Designer Note: This special provision should be inserted into all contracts.

SIGN PANELS AND APPURTENANCES (BDE)

Effective: January 1, 2025

Revised: January 1, 2026

Add Article 720.02(c) of the Standard Specifications to read:

"(c) Aluminum Epoxy Mastic

1008.03"

Revise the second and third paragraphs of Article 720.02 of the Standard Specifications to read:

"The sign mounting support channel shall be manufactured from steel or aluminum and shall be according to Standard 720001.

Steel support channels shall be according to ASTM A 1011 (A 1011M), ASTM A 635 (A 635M), ASTM A 568 (A 568M), or ASTM A 684 (A 684M), and shall be galvanized. Galvanizing shall be according to ASTM A 653 (A 653M) when galvanized before fabrication, and AASHTO M 111 (M 111M) when galvanized after fabrication. Field or post fabricated drilled holes shall be spot painted with one coat of aluminum epoxy mastic paint prior to installation."

Revise the fifth paragraph of Article 720.02 of the Standard Specifications to read:

"The stainless-steel banding for mounting signs or sign support channels to light or signal standards shall be according to ASTM A 240 (A 240M) Type 302 stainless steel."

Revise the first sentence of the tenth paragraph of Article 720.03 of the Standard Specifications to read:

"The backs of all sign panels shall be marked in a manner designed to last as long as the sign face material, in letters and numerals at least 3/8 in. (9.5 mm) but no more than 3/4 in. (19 mm) in height with the month and year of manufacture, the name of the sign manufacturer, the name of the sign sheeting manufacturer, the method of manufacture ("screened", "EC film", "direct applied", or "digital print"), and the initials IDOT."

Revise the first sentence of the fourth paragraph of Article 1091.03(a)(10) of the Standard Specifications to read:

"Transparent colors screened, or transparent acrylic electronic cutting films, or digital printing on white sheeting, shall meet the minimum initial coefficient of retroreflection values of the 0.2-degree observation angle, -4.0-degree entrance angle values as listed in the previous tables for the color being applied."

Add the following after the fourth paragraph of Article 1091.03(a)(10) of the Standard Specifications:

"Digitally printed signs shall be produced using digital print technologies and ink systems, products and processes that comply with the sheeting manufacturer's

recommendation. The digitally printed signs shall be fabricated with a full sign protective overlay film designed to provide a smooth surface needed for retroreflectivity, and to protect the sign from fading and UV degradation. The overlamine shall comply with the sheeting manufacturer's recommendations to ensure proper adhesion and transparency."

Add the following after the third paragraph of Article 1106.01 of the Standard Specifications:

"Digitally printed signs may omit protective overlay film."

Designer Note: This special provisions should be inserted into all contracts.

CEMENT, FINELY DIVIDED MINERALS, ADMIXTURES, CONCRETE, AND MORTAR (BDE)

Effective: January 1, 2025

Revised: January 1, 2026

Revise the first paragraph of Article 285.05 of the Standard Specifications to read:

"285.05 Fabric Formed Concrete Revetment Mat. The grout shall consist of a mixture of cement, fine aggregate, and water so proportioned and mixed as to provide a pumpable slurry. Fly ash or ground granulated blast furnace (GGBF) slag, and concrete admixtures may be used at the option of the Contractor. The grout shall have an air content of not less than 6.0 percent nor more than 9.0 percent of the volume of the grout. The mix shall obtain a compressive strength of 2,500 psi (17,000 kPa) at 28 days according to Article 1020.09."

Revise Article 302.02 of the Standard Specifications to read:

"302.02 Materials. Materials shall be according to the following.

<u>Item</u>	<u>Article/Section</u>
(a) Cement	1001
(b) Water	1002
(c) Hydrated Lime	1012.01
(d) By-Product, Hydrated Lime	1012.02
(e) By-Product, Non-Hydrated Lime	1012.03
(f) Lime Slurry	1012.04
(g) Fly Ash	1010
(h) Soil for Soil Modification (Note 1)	1009.01
(i) Bituminous Materials (Note 2)	1032

Note 1. This soil requirement only applies when modifying with lime (slurry or dry).

Note 2. The bituminous materials used for curing shall be emulsified asphalt RS-2, CRS-2, HFE 90, or HFE 150; rapid curing liquid asphalt RC-70; or medium curing liquid asphalt MC-70 or MC-250."

Revise Article 312.07(c) of the Standard Specifications to read:

"(c) Cement 1001"

Add Article 312.07(i) of the Standard Specifications to read:

"(i) Ground Granulated Blast Furnace (GGBF) Slag 1010"

Revise the first paragraph of Article 312.09 of the Standard Specifications to read:

"312.09 Proportioning and Mix Design. At least 60 days prior to start of placing CAM II, the Contractor shall submit samples of materials to be used in the work for proportioning and testing.

The mixture shall contain a minimum of 200 lb. (120 kg) of cement per cubic yard (cubic meter). Cement may be replaced with fly ash or ground granulated blast furnace (GGBF) slag according to Article 1020.05(c)(1) or 1020.05(c)(2), respectively, however the minimum cement content in the mixture shall be 170 lbs./cu. yd. (101 kg/cu. m). Blends of coarse and fine aggregates will be permitted, provided the volume of fine aggregate does not exceed the volume of coarse aggregate. The Engineer will determine the proportions of materials for the mixture according to the "Portland Cement Concrete Level III Technician Course" manual. However, the Contractor may substitute their own mix design. Article 1020.05(a) shall apply, and a Level III PCC Technician shall develop the mix design."

Revise Article 352.02 of the Standard Specifications to read:

"352.02 Materials. Materials shall be according to the following.

<u>Item</u>	<u>Article/Section</u>
(a) Cement (Note 1)	1001
(b) Soil for Soil-Cement Base Course	1009.03
(c) Water	1002
(d) Bituminous Materials (Note 2)	1032

Note 1. Bulk cement may be used for the traveling mixing plant method if the equipment for handling, weighing, and spreading the cement is approved by the Engineer.

Note 2. The bituminous materials used for curing shall be emulsified asphalt RS-2, CRS-2, HFE 90, or HFE 150; rapid curing liquid asphalt RC-70; or medium curing liquid asphalt MC-70 or MC-250."

Revise Article 404.02 of the Standard Specifications to read:

"404.02 Materials. Materials shall be according to the following.

<u>Item</u>	<u>Article/Section</u>
(a) Cement	1001
(b) Water	1002
(c) Fine Aggregate	1003.08
(d) Bituminous Material (Tack Coat)	1032.06
(e) Emulsified Asphalts (Note 1) (Note 2)	1032.06
(f) Fiber Modified Joint Sealer	1050.05
(g) Additives (Note 3)	

Note 1. When used for slurry seal, the emulsified asphalt shall be CQS-1h according to Article 1032.06(b).

Note 2. When used for micro-surfacing, the emulsified asphalt shall be CQS-1hP according to Article 1032.06(e).

Note 3. Additives may be added to the emulsion mix or any of the component materials to provide the control of the quick-traffic properties. They shall be included as part of the mix design and be compatible with the other components of the mix."

Revise the last sentence of the fourth paragraph of Article 404.08 of the Standard Specifications to read:

"When approved by the Engineer, the sealant may be dusted with fine sand, cement, or mineral filler to prevent tracking."

Revise Note 2 of Article 516.02 of the Standard Specifications to read:

"Note 2. The sand-cement grout mix shall be according to Section 1020 and shall be a 1:1 blend of sand and cement comprised of a Type I, IL, or II cement at 185 lb./cu. yd. (110 kg/cu. m). The maximum water cement ratio shall be sufficient to provide a flowable mixture with a typical slump of 10 in. (250 mm)."

Revise Note 2 of Article 543.02 of the Standard Specifications to read:

"Note 2. The grout mixture shall be 6.50 hundredweight/cu. yd. (385 kg/cu. m) of cement plus fine aggregate and water. Fly ash or ground granulated blast furnace (GGBF) slag may replace a maximum of 5.25 hundredweight/cu. yd. (310 kg/cu. m) of the cement. The water/cement ratio, according to Article 1020.06, shall not exceed 0.60. An air-entraining admixture shall be used to produce an air content, according to Article 1020.08, of not less than 6.0 percent nor more than 9.0 percent of the volume of the grout. The Contractor shall have the option to use a water-reducing or high range water-reducing admixture."

Revise Article 583.01 of the Standard Specifications to read:

"583.01 Description. This work shall consist of placing cement mortar along precast, prestressed concrete bridge deck beams as required for fairing out any unevenness between adjacent deck beams prior to placing of waterproofing membrane and surfacing."

Revise Article 583.02(a) of the Standard Specifications to read:

"(a) Cement 1001"

Revise the first paragraph of Article 583.03 of the Standard Specifications to read:

"583.03 General. This work shall only be performed when the air temperature is 45°F (7°C) and rising. The mixture for cement mortar shall consist of three parts sand to one part cement by volume. The amount of water shall be no more than that necessary to produce a workable, plastic mortar."

Revise Article 606.02(h) of the Standard Specifications to read:

"(h) Fibers (Note 1) 1014"

Revise Note 1 in Article 606.02(h) of the Standard Specifications to read:

"Note 1. Fibers, when required, shall only be used in the concrete mixture for slipform applications."

Revise the third paragraph in Article 606.10 of the Standard Specifications to read:

"Welded wire fabric shall be 6 x 6 in. (150 x 150 mm) mesh, #4 gauge (5.74 mm), 58 lb. (26 kg) per 100 sq. ft. (9 sq. m)."

Revise Article 1001.01(d) of the Standard Specifications to read:

- "(d) Rapid Hardening Cement. Rapid hardening cement shall be according to the Bureau of Materials Policy Memorandum "Portland or Blended Cement Acceptance Procedure for Qualified and Non-Qualified Plants", and ASTM C 1600, Type URH, Type VRH, or Type RH-CAC. It shall be used according to Article 1020.04 or when approved by the Engineer. The Contractor shall submit a report from the manufacturer or an independent lab that contains results for testing according to ASTM C 1600 which shows the cement meets the requirements of either Type URH, Type VRH, or Type RH-CAC. Test data shall be less than 1 year old from the date of submittal."

Revise Article 1001.01(e) of the Standard Specifications to read:

- "(e) Other Cements. Other cements shall be according to the Bureau of Materials Policy Memorandum "Portland or Blended Cement Acceptance Procedure for Qualified and Non-Qualified Plants", and ASTM C 1157 or ASTM C 1600, as applicable. Other cements shall be used according to Article 1020.04 or when approved by the Engineer. For cements according to ASTM C 1157, the Contractor shall submit a report from the manufacturer or an independent lab that contains results of tests which shows the cement meets the requirements Type GU, HE, MS, MH, or LH. For cements according to ASTM C 1600, the Contractor shall submit a report from the manufacturer or an independent lab that contains results of tests which shows the cement meets the requirements Type MRH or GRH. Test data shall be less than one (1) year old from the date of submittal."

Revise Article 1002.02 of the Standard Specifications to read:

"1002.02 Quality. Water used with cement in concrete or mortar and water used for curing concrete shall be clean, clear, and free from sugar. In addition, water shall be tested and evaluated for acceptance according to one of the following options.

OPTION 1.

- (a) Acceptable limits for acidity and alkalinity when tested according to ITP T 26.

- (1) Acidity -- 0.1 Normal NaOH 2 ml max.*
(2) Alkalinity -- 0.1 Normal HCl 10 ml max.*

*To neutralize 200 ml sample.

- (b) Acceptable limits for solids when tested according to the following.

- (1) Organic (ITP T 26)..... 0.02% max.
(2) Inorganic (ITP T 26) 0.30% max.
(3) Sulfate (SO₄) (ASTM D 516-82)..... 0.05% max.
(4) Chloride (ASTM D 512) 0.06% max.

- (c) The following tests shall be performed on the water sample and on deionized water. The same cement and sand shall be used for both tests.

- (1) Unsoundness (ASTM C 151).
(2) Initial and Final Set Time (ASTM C 266).
(3) Strength (ASTM C 109).

The test results for the water sample shall not deviate from the test results for the deionized water, except as allowed by the precision in the test method.

OPTION 2. Water shall meet the requirements ASTM C 1602 Tables 1 and 2 as outlined in Sections 5.1, 5.2, and 5.4."

Revise Note 2/ in Article 1003.01(b) of the Standard Specifications to read:

"2/ Applies only to sand. Sand exceeding the colorimetric test standard of 11 (Illinois Modified AASHTO T 21) will be checked for mortar making properties according to Illinois Modified ASTM C 87 and shall develop a compressive strength at the age of 14 days when using Type I, IL, or II cement of not less than 95 percent of the comparable standard."

Revise the second sentence of Article 1003.02(e)(1) of the Standard Specifications to read:

"The test will be performed with Type I, IL, or II Portland cement having a total equivalent alkali content ($\text{Na}_2\text{O} + 0.658\text{K}_2\text{O}$) of 0.90 percent or greater."

Revise the first sentence of the second paragraph of Article 1003.02(e)(3) of the Standard Specifications to read:

"The ASTM C 1293 test shall be performed with Type I, IL, or II Portland cement having a total equivalent alkali content ($\text{Na}_2\text{O} + 0.658\text{K}_2\text{O}$) of 0.80 percent or greater."

Revise the second sentence of Article 1004.02(g)(1) of the Standard Specifications to read:

"The test will be performed with Type I, IL, or II Portland cement having a total equivalent alkali content ($\text{Na}_2\text{O} + 0.658\text{K}_2\text{O}$) of 0.90 percent or greater."

Add the following Section to the Standard Specifications.

"SECTION 1014. FIBERS FOR CONCRETE

1014.01 General. Fibers used in concrete shall be Type II or Type III (polyolefin or carbon) according to ASTM C 1116. The testing required for Type II fibers or Type III polyolefin fibers shall be performed by an independent lab a minimum of once every five years, and the test results provided to the Department. Manufacturers of Type III carbon fibers shall provide materials certification documentation not more than six (6) years old a minimum of once every five (5) years to the Department. The Department will maintain a qualified product list. The method of inclusion of fibers into concrete mixtures shall be according to the manufacturer's specifications.

At the discretion of the Engineer, the concrete mixture shall be evaluated in a field demonstration for fiber clumping, ease of placement, and ease of finishing. The field demonstration shall consist of a minimum 2 cu. yd. (1.5 cu. m) trial batch placed in a 12 ft. x 12 ft. (3.6 m x 3.6 m) slab.

1014.02 Concrete Gutter, Curb, Median and Paved Ditch. Fibers shall be Type III. Fibers shall have a minimum length of 1/2 in. (13 mm) and a maximum length of 0.75 in. (19 mm). The maximum dosage rate in the concrete mixture shall not exceed 1.5 lb./cu. yd. (0.9 kg/cu. m). The minimum dosage rate shall be per the manufacturer's recommendation.

1014.03 Concrete Inlay or Overlay. Fibers shall be Type III. Fibers shall have a minimum length of 1.0 in. (25 mm), a maximum length of 2 1/2 in. (63 mm), and a maximum aspect ratio (length divided by the equivalent diameter of the fiber) of 150. The maximum dosage rate shall not exceed 5.0 lb./cu. yd. (3.0 kg/cu. m). The minimum dosage rate shall be per the manufacturer's recommendation.

1014.04 Bridge Deck Fly Ash, Ground Granulated Blast Furnace (GGBF) Slag, High Reactivity Metakaolin, or Microsilica (Silica Fume) Concrete Overlay. Fibers shall be Type III. The dosage rate shall be a minimum of 3.0 lb./cu. yd. (1.8 kg/cu. m), unless a field demonstration according to Article 1014.01 indicates that a lower dosage rate is necessary. Based on the results of the field demonstration, the Department has the option to reduce the dosage rate of fibers, but the dosage will not be reduced to less than 2.0 lb./cu. yd. (1.2 kg/cu. m).

1014.05 Bridge Deck Latex Concrete Overlay. Fibers shall be Type II or III. Fibers shall have a minimum length of 0.75 in. (19 mm), a maximum length of 1.75 in. (45 mm), and an aspect ratio (length divided by the equivalent diameter of the fiber) of between 70 and 100. The dosage rate shall be a minimum of 3.0 lb./cu. yd. (1.8 kg/cu. m), unless a field demonstration according to Article 1014.01 indicates that a lower dosage rate is necessary. Based on the results of the field demonstration, the Department has the option to reduce the dosage rate of fibers, but the dosage will not be reduced to less than 2.0 lb./cu. yd. (1.2 kg/cu. m)."

Add the following Section to the Standard Specifications:

"SECTION 1015. HIGH PERFORMANCE SHOTCRETE

1015.01 Packaged Shotcrete with Aggregate. The packaged shotcrete with aggregate shall be a pre-blended dry combination of materials for the wet-mix shotcrete method according to ASTM C 1480, Type FA or CA, Grade FR, Class I. The fibers shall be Type III according to Article 1014.01. The cement and finely divided minerals in the mixture shall be a minimum 6.65 cwt/cu. yd. (395 kg/cu. m), and the Portland cement shall not be below 4.70 cwt/cu. yd. (279 kg/cu. m). Microsilica is required in the mixture and shall be a minimum of 5 percent by weight (mass) of cementitious material, and a maximum of 10 percent. Strength requirements shall be according to ASTM C 1480 except that the strength at 28 days shall be at least 4,000 psi (27,500 kPa). Strength testing shall be according to ASTM C 1140. The air content as shot shall be 4.0 – 8.0 percent when tested according to AASHTO T 152, and the coarse aggregate shall be a maximum size of 1/2 in. (12.5 mm).

The packaged shotcrete shall have a water-soluble chloride ion content of less than 0.15% by weight of cementitious material when tested according to ASTM C 1218 or AASHTO T 260.

The testing according to ASTM C 1480, ASTM C 1140, AASHTO 152, and ASTM C 1218 or AASHTO T 260 shall be performed by an independent lab a minimum of once every five (5) years, and the test results shall be provided to the Department. The Department will maintain a qualified product list. Batching and mixing shall be per the manufacturer's recommendations.

1015.02 Packaged Shotcrete Without Aggregate. The packaged shotcrete that does not include pre-blended aggregate shall be according to Article 1015.01, except the added aggregate shall be according to Articles 1003.02 and 1004.02. The aggregate gradation shall be according to the manufacturer. The Department will maintain a qualified product list. Batching and mixing shall be per the manufacturer's recommendations."

Revise Section 1017 of the Standard Specifications to read:

"SECTION 1017. PACKAGED, DRY, COMBINED MATERIALS FOR MORTAR AND CONCRETE

1017.01 Mortar. The mortar shall be high strength according to ASTM C 387 and shall have a minimum 80.0 percent relative dynamic modulus of elasticity when tested according to AASHTO T 161. For prestressed concrete applications, the mortar shall have a water-soluble chloride ion content of less than 0.06 percent by weight of cementitious material when tested according to ASTM C 1218 or AASHTO T 260; and for non-prestressed concrete applications, the water-soluble chloride content shall be less than 0.15 percent by weight of cementitious material. The testing according to ASTM C 387, AASHTO T 161, and either ASTM C 1218 or AASHTO T 260 shall be performed by an independent lab a minimum of once every five (5) years, and the test results shall be provided to the Department. The Department will maintain a qualified product list. Mixing of the high-strength mortar shall be according to the manufacturer's specifications.

1017.02 Concrete. The materials, testing, and preparation of aggregate for the "high slump" packaged concrete mixture shall be according to ASTM C 387. The mixture shall be air entrained, the slump shall be 5-10 in. (125-250 mm), and the coarse aggregate shall be a maximum size of 1/2 in. (12.5 mm). Strength requirements shall be according to ASTM C 387 except that the strength at 28 days shall be at least 4,000 psi (27,500 kPa). The "high slump" packaged concrete mixture shall have a water-soluble chloride ion content of less than 0.15% by weight of cementitious material when tested according to ASTM C 1218 or AASHTO T 260. The testing according to ASTM C 387, and either ASTM C 1218 or AASHTO T 260 shall be performed by an independent lab a minimum of once every five (5) years, and the test results shall be provided to the Department. The Department will maintain a qualified product list. Mixing shall be per the manufacturer's recommendations.

1017.02 Self-Consolidating Concrete. The materials, testing, and preparation of aggregate for the "self-consolidating concrete" packaged concrete mixture shall be according to ASTM C 387. The mixture shall be air entrained, it should be uniformly graded, and the coarse aggregate shall be a maximum size of 1/2 in. (12.5 mm). Strength requirements shall be according to ASTM C 387 except that the strength at 28 days shall be at least 4,000 psi (27,500 Pa). Slump flow range shall be 22 in. (550 mm) minimum to 28 in. (700 mm) maximum when tested according to AASHTO T 347. The visual stability index shall be a maximum of 1 when tested according to AASHTO T 351. At the option of the manufacturer, either the J-Ring value shall be a maximum of 2 in. (50 mm) when tested according to AASHTO T 347 or the L-Box blocking ratio shall be a minimum of 80 percent when tested according AASHTO T 419. The hardened visual stability index shall be a maximum of 1 when tested according to AASHTO R 81.

The "self -consolidating concrete" packaged concrete mixture shall have a water-soluble chloride ion content of less than 0.15 percent by weight of cementitious material when tested according to ASTM C 1218 or AASHTO T 260.

The testing according to ASTM C 387, AASHTO T 347, AASHTO T 351, AASHTO T 419, AASHTO R 81, ASTM C 1218 and AASHTO T 260 shall be performed by an independent lab a minimum of once every five (5) years, and the test results shall be provided to the Department. The Department will maintain a qualified product list. Mixing shall be per the manufacturer's recommendations."

Revise Article 1018.01 of the Standard Specifications to read:

"1018.01 Requirements. The rapid hardening mortar or concrete shall be according to ASTM C 928 and shall have successfully completed and remain current with the AASHTO Product Eval and Audit Rapid Hardening Concrete Patching Materials (RHCP) testing program. R1, R2, or R3 concrete shall be air entrained, the slump shall be 5-10 in. (125-250 mm), and the coarse aggregate shall be a maximum size of 1/2 in. (12.5 mm). For prestressed concrete applications, the mortar or concrete shall have a water-soluble chloride ion content of less than 0.06 percent by weight of cementitious material when tested according to ASTM C 1218 or AASHTO T 260; and for non-prestressed concrete applications, the water-soluble chloride content shall be less than 0.15 percent by weight of cementitious material. The Department will maintain a qualified product list. Mixing of the mortar or concrete shall be according to the manufacturer's specifications."

Revise Article 1019.02 of the Standard Specifications to read:

"1019.02 Materials. Materials shall be according to the following.

Item	Article/Section
(a) Cement	1001
(b) Water	1002
(c) Fine Aggregate for Controlled Low-Strength Material (CLSM)	1003.06
(d) Fly Ash	1010
(e) Ground Granulated Blast Furnace (GGBF) Slag	1010
(f) Admixtures (Note 1)	

Note 1. The air-entraining admixture may be in powder or liquid form. The air content produced by the admixture shall be 15-25 percent when incorporated into Mix 2 or an equivalent mixture as determined by the Department and tested according to AASHTO T 121 or AASHTO T 152. The testing according to AASHTO T 121 or AASHTO T 152 shall be performed by an independent lab a minimum of once every five years, and the test results shall be provided to the Department. The Department will maintain a qualified product list."

Revise the third paragraph of Article 1019.04 of the Standard Specifications to read:

"The Engineer will instruct the Contractor to adjust the proportions of the mix design in the field as needed to meet the design criteria, provide adequate flowability, maintain proper solid suspension, or other criteria established by the Engineer."

Revise Article 1019.05 of the Standard Specifications to read:

"1019.05 Department Mix Design. The Department mix design shall be Mix 1, 2, or 3 and shall be proportioned to yield approximately one cubic yard (cubic meter).

Mix 1	
Cement	50 lb. (30 kg)
Fly Ash – Class C or F, and/or GGBF Slag	125 lb. (74 kg)
Fine Aggregate – Saturated Surface Dry	2900 lb. (1720 kg)
Water	50-65 gal. (248-322 L)
Air Content	No air is entrained

Mix 2	
Cement	125 lb. (74 kg)
Fine Aggregate – Saturated Surface Dry	2500 lb. (1483 kg)
Water	35-50 gal. (173-248 L)
Air Content	15-25 %

Mix 3	
Cement	40 lb. (24 kg)
Fly Ash – Class C or F, and/or GGBF Slag	125 lb. (74 kg)
Fine Aggregate – Saturated Surface Dry	2500 lb. (1483 kg)
Water	35-50 gal (179-248 L)
Air Content	15-25 %"

Revise Article 1020.04, Table 1, Note (8) of the Standard Specifications to read:

"(8) In addition to the Type III Portland cement, 100 lb./cu. yd. of ground granulated blast-furnace slag and 50 lb./cu. yd. of microsilica (silica fume) shall be used. For an air temperature greater than 85 F, the Type III Portland cement may be replaced with Type I, IL, or II Portland cement."

Revise Article 1020.04, Table 1 (Metric), Note (8) of the Standard Specifications to read:

"(8) In addition to the Type III Portland cement, 60 kg/cu. m of ground granulated blast-furnace slag and 30 kg/cu. m of microsilica (silica fume) shall be used. For an air temperature greater than 30 C, the Type III Portland cement may be replaced with Type I, IL, or II Portland cement."

Revise Note 9 of Table 1 of Article 1020.04 of the Standard Specifications to read:

"(9) The cement shall be a rapid hardening according to Article 1001.01(d). Minimum or maximum cement factor may be adjusted when approved by the Engineer."

Revise the second paragraph of Article 1020.05(a) of the Standard Specifications to read:

"For a mix design using a Portland-pozzolan cement, Portland blast-furnace slag cement, Portland-limestone cement, or replacing Portland cement with finely divided minerals per Articles 1020.05(c) and 1020.05(d), the Contractor may submit a mix design with a minimum Portland cement content less than 400 lbs./cu. yd. (237 kg/cu. m), but not less than 375 lbs./cu. yd. (222 kg/cu. m), if the mix design is shown to have a minimum relative dynamic modulus of elasticity of 80 percent determined according to AASHTO T 161. Testing shall be performed by an independent laboratory accredited by AASHTO resource for Portland Cement Concrete."

Revise the first sentence of the first paragraph of Article 1020.05(b) of the Standard Specifications to read:

"Corrosion inhibitors and concrete admixtures shall be according to the qualified product lists."

Delete the fourth and fifth sentences of the second paragraph of Article 1020.05(b) of the Standard Specifications.

Revise Article 1020.05(b)(5) of the Standard Specifications to read:

"(5) For Class PP-4 concrete, a high range water-reducing admixture, retarder, and/or hydration stabilizer may be used in addition to the air-entraining admixture. The Contractor also has the option to use a water-reducing admixture with the high range water-reducing admixture. An accelerator shall not be used. A mobile Portland cement concrete plant shall be used to produce the patching mixture.

For PP-5 concrete, a non-chloride accelerator, high range water-reducing admixture, retarder, hydration stabilizer, and/or air-entraining admixture may be used. The accelerator, high range water-reducing admixture, retarder, hydration stabilizer, and/or air-entraining admixture shall be per the Contractor's recommendation and dosage. The qualified product list of concrete admixtures shall not apply. A mobile Portland cement concrete plant shall be used to produce the patching mixture."

Revise second paragraph of Article 1020.05(b)(10) of the Standard Specifications to read:

"When calcium nitrite is used, it shall be added at the rate of 4 gal./cu. yd. (20 L/cu. m) and shall be added to the mix immediately after all compatible admixtures have been introduced to the batch. Other corrosion inhibitors shall be added per the manufacturer's specifications."

Delete the third paragraph of Article 1020.05(b)(10) of the Standard Specifications.

Revise Article 1020.15(b)(1)c. of the Standard Specifications to read:

"c. The minimum Portland cement content in the mixture shall be 375 lbs./cu. yd. (222 kg/cu. m). When the total of organic processing additions, inorganic processing additions, and limestone addition exceed 5.0 percent in the cement, the minimum Portland cement content in the mixture shall be 400 lbs./cu. yd. (237 kg/cu. m). For a drilled shaft, foundation, footing, or substructure, the minimum Portland cement may be reduced to as low as 330 lbs./cu. yd. (196 kg/cu. m) if the concrete has adequate freeze/thaw durability. The Contractor shall provide freeze/thaw test results according to AASHTO T 161, and the relative dynamic modulus of elasticity of the mix design shall be a minimum of 80 percent. Testing shall be performed by an independent laboratory accredited by AASHTO resource for Portland Cement Concrete. Freeze/thaw testing will not be required for concrete that will not be exposed to freezing and thawing conditions as determined by the Engineer."

Revise Article 1021.01 of the Standard Specifications to read:

"1021.01 General. Admixtures shall be furnished in liquid or powder form ready for use. The admixtures shall be delivered in the manufacturer's original containers, bulk tank trucks or such containers or tanks as are acceptable to the Engineer. Delivery shall be accompanied by a ticket which clearly identifies the manufacturer, the date of manufacture, and trade name of the material. Containers shall be readily identifiable as to manufacturer, the date of manufacture, and trade name of the material they contain.

Concrete admixtures shall be on one of the Department's qualified product lists. Unless otherwise noted, admixtures shall have successfully completed and remain current with the

AASHTO Product Eval and Audit Concrete Admixture (CADD) testing program. For admixture submittals to the Department: the product brand name, manufacturer name, admixture type or types, an electronic link to the product's technical data sheet, and the NTPEP testing number which contains an electronic link to all test data shall be provided. In addition, a letter shall be submitted certifying that no changes have been made in the formulation of the material since the most current round of tests conducted by AASHTO Product Eval and Audit. After 28 days of testing by AASHTO Product Eval and Audit, air-entraining admixtures may be provisionally approved and used on Departmental projects. For all other admixtures, unless otherwise noted, the time period after which provisionally approved status may be earned is 6 months.

The manufacturer shall include the following in the submittal to the AASHTO Product Eval and Audit CADD testing program: the manufacturing range for specific gravity, the midpoint and manufacturing range for residue by oven drying, and manufacturing range of pH. The submittal shall also include an infrared spectrophotometer trace no more than five years old.

For air-entraining admixtures according to Article 1021.02, the specific gravity allowable manufacturing range established by the manufacturer shall be according to AASHTO M 194. For residue by oven drying and pH, the allowable manufacturing range and test methods shall be according to AASHTO M 194.

For admixtures according to Articles 1021.03, 1021.04, 1021.05, 1021.06, 1021.07, and 1021.08, the pH allowable manufacturing range established by the manufacturer shall be according to ASTM E 70. For specific gravity and residue by oven drying, the allowable manufacturing range and test methods shall be according to AASHTO M 194.

All admixtures, except chloride-based accelerators, shall contain a maximum of 0.3 percent chloride by weight (mass) as determined by an appropriate test method. To verify the test result, the Department will use Illinois Modified AASHTO T 260, Procedure A, Method 1.

Prior to final approval of an admixture, the Engineer reserves the right to request a sample for testing. The test and reference concrete mixtures tested by the Engineer will contain a cement content of 5.65 cwt/cu. yd. (335 kg/cu. m). For freeze-thaw testing, the Department will perform the test according to Illinois Modified AASHTO T 161. The flexural strength test will be performed according to AASHTO T 177. If the Engineer decides to test the admixture, the manufacturer shall submit AASHTO T 197 water content and set time test results on the standard cement used by the Department. The manufacturer may select their lab or an independent lab to perform this testing. The laboratory is not required to be accredited by AASHTO.

Random field samples may be taken by the Department to verify an admixture meets specification. A split sample will be provided to the manufacturer if requested. Admixtures that do not meet specification requirements or an allowable manufacturing range established by the manufacturer shall be replaced with new material."

Revise Article 1021.03 of the Standard Specifications to read:

"1021.03 Retarding and Water-Reducing Admixtures. The admixture shall be according to the following.

- (a) Retarding admixtures shall be according to AASHTO M 194, Type B (retarding) or Type D (water-reducing and retarding).
- (b) Water-reducing admixtures shall be according to AASHTO M 194, Type A.

- (c) High range water-reducing admixtures shall be according to AASHTO M 194, Type F (high range water-reducing) or Type G (high range water-reducing and retarding)."

Revise Article 1021.05 of the Standard Specifications to read:

"1021.05 Self-Consolidating Admixtures. Self-consolidating admixture systems shall consist of either a high range water-reducing admixture only or a high range water-reducing admixture combined with a separate viscosity modifying admixture. The one or two component admixture system shall be capable of producing a concrete that can flow around reinforcement and consolidate under its own weight without additional effort and without segregation.

High range water-reducing admixtures shall be according to AASHTO M 194, Type F.

Viscosity modifying admixtures shall be according to AASHTO M 194, Type S (specific performance)."

Revise Article 1021.06 of the Standard Specifications to read:

"1021.06 Rheology-Controlling Admixture. Rheology-controlling admixtures shall be capable of producing a concrete mixture with a lower yield stress that will consolidate easier for slipform applications used by the Contractor. Rheology-controlling admixtures shall be according to AASHTO M 194, Type S (specific performance)."

Revise Article 1021.07 of the Standard Specifications to read:

"1021.07 Corrosion Inhibitor. The corrosion inhibitor shall be according to one of the following.

- (a) Calcium Nitrite. Corrosion inhibitors shall contain a minimum 30 percent calcium nitrite by weight (mass) of solution and shall comply with either the requirements of AASHTO M 194, Type C (accelerating) or the requirements of ASTM C 1582. The corrosion inhibiting performance requirements of ASTM C 1582 shall not apply.

- (b) Other Materials. The corrosion inhibitor shall be according to ASTM C 1582.

For submittals requiring testing according to ASTM M 194, Type C (accelerating), the admixture shall meet the requirements of the AASHTO Product Eval and Audit CADD testing program according to Article 1021.01.

For submittals requiring testing according to ASTM C 1582, a report prepared by an independent laboratory accredited by AASHTO resource for Portland cement concrete shall be provided. The report shall show the results of physical tests conducted no more than five years prior to the time of submittal, according to applicable specifications. However, ASTM G 109 test information specified in ASTM C 1582 is not required to be from an independent accredited lab. All other information in ASTM C 1582 shall be from an independent accredited lab. Test data and other information required to be submitted to AASHTO Product Eval and Audit according to Article 1021.01, shall instead be submitted directly to the Department."

Add Article 1021.08 of the Standard Specifications as follows:

"1021.08 Other Specific Performance Admixtures. Other specific performance admixtures shall, at a minimum, be according to AASHTO M 194, Type S (specific performance). The Department also reserves the right to require other testing, as determined by the Engineer, to show evidence of specific performance characteristics.

Initial testing according to AASHTO M 194 may be conducted under the AASHTO Product Eval and Audit CADD testing program according to Article 1021.01, or by an independent laboratory accredited by AASHTO resource for Portland Cement Concrete. In either case, test data and other information required to be submitted to AASHTO Product Eval and Audit according to Article 1021.01, shall also be submitted directly to the Department. The independent accredited lab report shall show the results of physical tests conducted no more than five years prior to the time of submittal, according to applicable specifications."

Add Article 1021.09 of the Standard Specifications as follows:

"1021.09 Latex Admixtures. The latex admixture shall be a uniform, homogeneous, non-toxic, film-forming, polymeric emulsion in water to which all stabilizers have been added at the point of manufacture. The latex admixture shall not contain any chlorides and shall contain 46-49 percent solids.

In lieu of meeting the requirements of Article 1021.01, the Contractor shall submit a manufacturer's certification that the latex emulsion meets the requirements of FHWA Research Report RD-78-35, Chapter VI. The certificate shall include the date of manufacture of the latex admixture, batch or lot number, quantity represented, manufacturer's name, and the location of the manufacturing plant. The latex emulsion shall be sampled and tested in accordance with RD-78-35, Chapter VII, Certification Program.

The latex admixture shall be packaged and stored in containers and storage facilities which will protect the material from freezing and from temperatures above 85°F (30°C). Additionally, the material shall not be stored in direct sunlight and shall be shaded when stored outside of buildings during moderate temperatures."

Revise Article 1024.01 of the Standard Specifications to read:

"1024.01 Requirements for Grout. The grout shall be proportioned by dry volume, thoroughly mixed, and shall have a minimum temperature of 50°F (10°C). Water shall not exceed the minimum needed for placement and finishing.

Materials for the grout shall be according to the following.

Item	Article/Section
(a) Cement	1001
(b) Water	1002
(c) Fine Aggregate	1003.02
(d) Fly Ash	1010
(e) Ground Granulated Blast Furnace (GGBF) Slag	1010
(f) Concrete Admixtures	1021"

Revise Note 1 of Article 1024.02 of the Standard Specifications to read:

"Note 1. Nonshrink grout shall be according to ASTM C 1107.

For prestressed concrete applications, the nonshrink grout shall have a water-soluble chloride ion content of less than 0.06 percent by weight of cementitious material when tested according to ASTM C 1218 or AASHTO T 260; and for non-prestressed concrete applications, the water-soluble chloride ion content shall be less than 0.15 percent by weight of cementitious material. The testing according to ASTM 1107, and either ASTM C 1218 or AASHTO T 260 shall be performed by an independent lab a minimum of once every five years, and the test results shall be provided to the Department. The Department will maintain a qualified product list. Mixing of the nonshrink grout shall be according to the manufacturer's specifications."

Revise Article 1029.02 of the Standard Specifications to read:

"**1029.02 Materials.** Materials shall be according to the following.

Item	Article/Section
(a) Cement	1001
(b) Fly Ash.....	1010
(c) Ground Granulated Blast Furnace (GGBF) Slag	1010
(d) Water	1002
(e) Fine Aggregate	1003
(f) Concrete Admixtures.....	1021
(g) Foaming Agent (Note 1)	

Note 1. The manufacturer shall submit infrared spectrophotometer trace and test results indicating the foaming agent meets the requirements of ASTM C 869 in order to be on the Department's qualified product list. Submitted data/results shall not be more than five years old."

Revise the second paragraph of Article 1103.03(a)(4) the Standard Specifications to read:

"The dispenser system shall provide a visual indication that the liquid admixture is actually entering the batch, such as via a transparent or translucent section of tubing or by independent check with an integrated secondary metering device. If approved by the Engineer, an alternate indicator may be used for admixtures dosed at rates of 25 oz/cwt (1,630 mL/100 kg) or greater, such as accelerating admixtures, corrosion inhibitors, and viscosity modifying admixtures."

Revise Article 1103.04 of the Standard Specifications to read:

"**1103.04 Mobile Portland Cement Concrete Plants.** The mobile concrete plant shall be according to AASHTO M 241 and the Bureau of Materials Policy Memorandum "Approval of Volumetric Mobile Mixers for Concrete". The mixer shall be capable of carrying sufficient unmixed materials to produce not less than 6 cu. yd. (4.6 cu. m) of concrete."

Revise the first two sections of Check Sheet #11 "Subsealing of Concrete Pavements" of the Recurring Special Provisions to read:

"Description. This work shall consist of filling voids beneath rigid and composite pavements with cement grout.

Materials. Materials shall be according to the following Articles/Sections of the Standard Specifications:

<u>Item</u>	<u>Article/Section</u>
(a) Cement	1001
(b) Water	1002
(c) Fly Ash	1010
(d) Ground Granulated Blast Furnace (GGBF) Slag	1010
(e) Admixtures	1021
(f) Packaged Rapid Hardening Mortar or Concrete	1018"

Revise the Materials section of Check Sheet #28 "Portland Cement Concrete Inlay or Overlay" of the Recurring Special Provisions to read:

"Materials. Materials shall be according to the following Articles/Sections of the Standard Specifications.

<u>Item</u>	<u>Article/Section</u>
(a) Portland Cement Concrete (Note 1)	1020
(b) Fibers for Concrete	1014
(c) Protective Coat	1023.01

Note 1. Class PV concrete shall be used, except the cement factor for central mixed concrete shall be 6.05 cwt/cu. yd. (360 kg/cu. m). A cement factor reduction according to Article 1020.05(b)(8) of the Standard Specifications will be permitted. CA 5 shall not be used, and CA 7 may only be used for overlays that are a minimum of 4.5 in. (113 mm) thick. The Class PV concrete shall have a minimum flexural strength of 550 psi (3,800 kPa) or a minimum compressive strength of 3,000 psi (20,700 kPa) at 14 days."

Designer Note: This special provision should be inserted into all HMA paving contracts.

HOT-MIX ASPHALT (BDE)

Effective: January 1, 2024

Revised: January 1, 2026

Add the following to the end of Article 406.06(c) of the Standard Specifications:

"The amount of HMA binder course placed shall be limited to that which can be surfaced during the same construction season."

Revise the fifteenth through eighteenth paragraphs of Article 406.14 of the Standard Specifications to read:

"The mixture used in constructing acceptable HMA test strips will be paid for at the contract unit price. Unacceptable HMA test strips shall be removed and replaced at no additional cost to the Department."

Revise the first and second paragraphs of Articles 1030.06(c)(2) of the Standard Specifications to read:

"(2) Personnel. The Contractor shall provide a QC Manager who shall have overall responsibility and authority for quality control. This individual shall maintain active certification as a Hot-Mix Asphalt Level II technician.

In addition to the QC Manager, the Contractor shall provide sufficient personnel to perform the required visual inspections, sampling, testing, and documentation in a timely manner. Mix designs shall be developed by personnel with an active certification as a Hot-Mix Asphalt Level III technician. Technicians performing mix design testing and plant sampling/testing shall maintain active certification as a Hot-Mix Asphalt Level I technician. The Contractor may provide a technician trainee who has successfully completed the Department's "Hot-Mix Asphalt Trainee Course" to assist in the activities completed by a Hot-Mix Asphalt Level I technician for a period of one year after the course completion date. The Contractor may also provide a Gradation Technician who has successfully completed the Department's "Gradation Technician Course" to run gradation tests only under the supervision of a Hot-Mix Asphalt Level II Technician. The Contractor shall provide a Hot-Mix Asphalt Density Tester who has successfully completed the Department's "Nuclear Density Testing" course to run all nuclear density tests on the job site."

Add Article 1030.06(d)(3) to the Standard Specifications as follows:

"(3) The Contractor shall take possession of any Department HMA mixture samples or density specimens upon notification by the Engineer. The Contractor shall collect the HMA mixture samples or density specimens from the location designated by the Engineer and may add these materials to RAP stockpiles according to Section 1031."

Revise the second paragraph of Articles 1030.07(a)(11) and 1030.08(a)(9) of the Standard Specifications to read:

"When establishing the target density, the HMA maximum theoretical specific gravity (G_{mm}) will be based on the running average of four available Department test results for that project. If less than four G_{mm} test results are available, an average of all available Department test results for that project will be used. The initial G_{mm} will be the last available Department test result from a QMP project. If there is no available Department test result from a QMP project, the Department mix design verification test result will be used as the initial G_{mm} ."

Revise the Quality Control Limits table in Article 1030.09(c) to read:

"CONTROL LIMITS						
Parameter	IL-19.0, IL-9.5, IL-9.5FG, IL-19.0L, IL-9.5L		SMA-12.5, SMA-9.5		IL-4.75	
	Individual Test	Moving Avg. of 4	Individual Test	Moving Avg. of 4	Individual Test	Moving Avg. of 4
% Passing: ^{1/}						
1/2 in. (12.5 mm)	±6%	±4%	±6%	±4%		
3/8 in. (9.5mm)			±4%	±3%		
# 4 (4.75 mm)	±5%	±4%	±5%	±4%		
# 8 (2.36 mm)	±5%	±3%	±4%	±2%		
# 16 (1.18 mm)			±4%	±2%	±4%	±3%
# 30 (600 µm)	±4%	±2.5%	±4%	±2.5%		
Total Dust Content # 200 (75 µm)	±1.5%	±1.0%			±1.5%	±1.0%
Asphalt Binder Content	±0.3%	±0.2%	±0.2%	± 0.1 %	±0.3%	±0.2%
Air Voids ^{2/}	±1.2%	±1.0%	±1.2%	± 1.0 %	±1.2%	±1.0%
Field VMA ^{3/}	-0.7%	-0.5%	-0.7%	-0.5 %	-0.7%	-0.5%

1/ Based on washed ignition oven or solvent extraction gradation.

2/ The air voids target value shall be 3.2 to 4.8 percent.

3/ Allowable limit below minimum design VMA requirement."

Revise Article 1030.09(g)(2) of the Standard Specifications to read:

"(2) The Contractor shall complete split verification sample tests listed in the Limits of Precision table in Article 1030.09(h)(1)."

In the Supplemental Specifications, replace the revision for the end of the third paragraph of Article 1030.09(h)(2) with the following:

"When establishing the target density, the HMA maximum theoretical specific gravity (G_{mm}) will be the Department mix design verification test result."

Replace the last sentence of the fourth paragraph of Article 1030.10 of the Standard Specifications with the following:

"The mixture test results shall meet the requirements of Article 1030.05(d), except tensile strength and TSR testing will only be conducted on the first use of a mix design for the year and Hamburg wheel tests will only be conducted on High ESAL mixtures. To be considered acceptable to remain in place, the Department's mixture test results shall meet the acceptable limits stated in Article 1030.09(i)(1). In addition, no visible pavement distress such as, but not limited to, segregation, excessive coarse aggregate fracturing outside of growth curves, excessive dust balls, or flushing shall be present as determined by the Engineer."

Revise the tenth paragraph of Article 1030.10 of the Standard Specifications to read:

"Production is not required to stop after a test strip has been constructed."

Replace the eleventh paragraph of Article 1030.10 of the Standard Specifications with the following:

"If an initial Hamburg wheel or I-FIT test fails to meet the requirements of Article 1030.05(d), the Department will verify the results by testing the retained gyratory cylinders. Upon notification by the Engineer of a Hamburg wheel or I-FIT test failure on the retained gyratory cylinders, the Contractor shall substitute an approved mix design, submit a new mix design for mix verification testing according to Article 1030.05(d), or pave 250 tons with or without an adjustment and resample for Department Hamburg wheel and I-FIT testing as directed by the Engineer. Paving may continue as long as all other mixture criteria is being met. If Hamburg wheel or I-FIT tests on the resampled HMA fail, production of the affected mixture shall cease, and the Contractor shall substitute an approved mix design or submit a new mix design for mix verification testing according to Article 1030.05(d)."

District Special Provisions

STATE OF ILLINOIS

SPECIAL PROVISIONS

The following Special Provisions supplement the "Standard Specifications for Road and Bridge Construction, adopted January 1, 2022, (revised January 1, 2026)", the latest edition of the "Manual on Uniform Traffic Control Devices for Streets and Highways", and the "Manual of Test Procedures for Materials" in effect on the date of invitation for bids, and the "Supplemental Specifications and Recurring Special Provisions" indicated on the Check Sheet included herein, and the "Recommended Standards for Water Works", (Ten State Standards), latest edition, which apply to and govern the construction of

and in case of conflict with any part, or parts, of said Specifications, the said Special Provisions shall take precedence and shall govern.

LOCATION OF PROJECT

DESCRIPTION OF PROJECT