

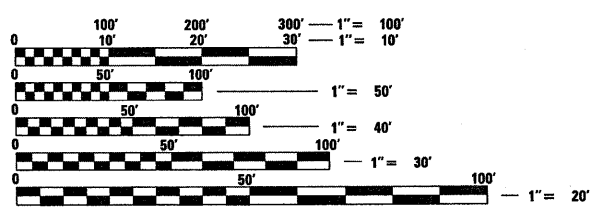
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
41	15 BR-2	LIVINGSTON	64	1
		ILLINOIS	CONTRACT NO. 66691	

INDEX OF SHEETS

- 1 COVER SHEET
- 2 GENERAL NOTES
- 3-4 SUMMARY OF QUANTITIES
- 5 TYPICAL SECTIONS
- 6-8 SCHEDULES
- 9 TIES
- 10-13 PLAN AND PROFILES SHEETS
- 14-16 TRAFFIC CONTROL SHEETS
- 17-50 STRUCTURAL SHEETS
- 51-53 EXISTING BRIDGE PLANS
- 54-58 DETAILS
- 59-64 CROSS SECTIONS

STANDARDS

- 000001-05 STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
- 280001-05 TEMPORARY EROSION CONTROL SYSTEMS
- 420401-08 BRIDGE APPROACH PAVEMENT CONNECTOR
- 515001-03 NAME PLATE FOR BRIDGES
- 542401-01 METAL END SECTION FOR PIPE CULVERTS
- 601101-01 CONCRETE HEADWALL FOR PIPE DRAINS
- 602301-02 INLET, TYPE A
- 602401-02 MANHOLE, TYPE A
- 602601-02 PRECAST REINFORCED CONCRETE FLAT SLAB TOP
- 602701-02 MANHOLE STEPS
- 604001-03 FRAME AND LIDS, TYPE 1
- 604036-02 GRATE, TYPE 8
- 630001-08 STEEL PLATE BEAM GUARDRAIL
- 630201-06 PCC/HMA STABILIZATION AT STEEL PLATE BEAM GUARDRAIL
- 630301-05 SHOULDER WIDENING FOR TYPE 1(SPECIAL) GUARDRAIL TERMINALS
- 631031-08 TRAFFIC BARRIER TERMINAL, TYPE 6
- 635006-03 REFLECTOR AND TERMINAL MARKER PLACEMENT
- 635011-02 REFLECTOR MARKER AND MOUNTING DETAILS
- 701001-02 OFF-ROAD OPERATIONS 2L, 2W, MORE THAN 15' (4.5 m) AWAY
- 701006-03 OFF-ROAD OPERATIONS 2L, 2W, 15' (4.5 m) TO 24" (600 mm) FROM PAVEMENT EDGE
- 701301-03 LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS
- 701306-02 LANE CLOSURE, 2L, 2W, SLOW MOVING OPERATIONS DAY ONLY, FOR SPEEDS ≥ 45 MPH
- 701311-03 LANE CLOSURE, 2L, 2W, MOVING OPERATIONS - DAY ONLY
- 701321-10 LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER
- 701326-03 LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING, FOR SPEEDS ≥ 45 MPH
- 701901-01 TRAFFIC CONTROL DEVICES
- 704001-06 TEMPORARY CONCRETE BARRIER
- 781001-03 TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123
OR 811

DISTRICT 3 NO. (815) 434-6131
PROJECT ENGINEER: CRAIG REED
UNIT CHIEF: R SCHWANKE
TOWNSHIP: DWIGHT
CONTRACT NO. 66691

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

**PROPOSED
HIGHWAY PLANS**

FAP ROUTE 41 (IL 17)
SECTION 15 BR-2
PROJECT ACBHF-0041(133)
SUPERSTRUCTURE REPLACEMENT
LIVINGSTON COUNTY

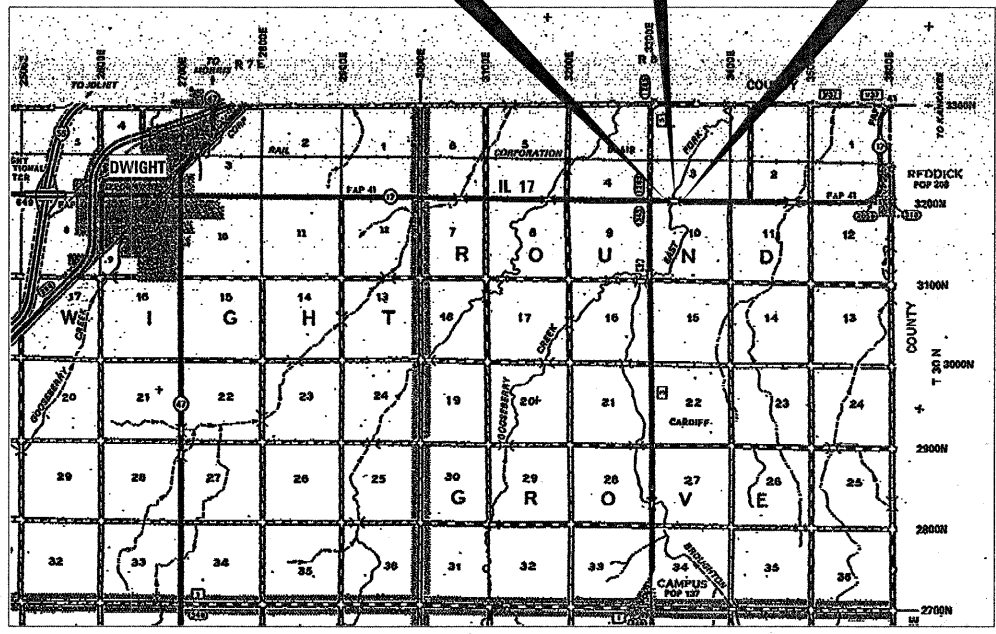
C-93-012-07

BRIDGE SUPERSTRUCTURE
REPLACEMENT
STA. 323 + 21.04
S.N. 053-0150 EX. & PR.

EXISTING S.N. 053-0150 CROSSES THE EAST FORK OF THE MAZON RIVER AT A LENGTH OF 181'-7". THE EXISTING SUPERSTRUCTURE WILL BE REMOVED AND REPLACED, AND EXISTING ABUTMENTS WILL BE CONVERTED TO SEMI-INTEGRAL ABUTMENTS.

BEGIN IMPROVEMENT
STA. 317 + 05

END IMPROVEMENT
STA. 329 + 23



GROSS & NET LENGTH = 1218 FT. = .231 MILE



FUNCTIONAL CLASSIFICATION
RURAL MINOR ARTERIAL
2009 ADT = 2550
P.V. = 69.6% S.U. = 6.9% M.U. = 23.5%

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

SUBMITTED _____ 20____

George Ryan
DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER
December 4, 2009

Charles J. Ingersoll
ENGINEER OF DESIGN AND ENVIRONMENT
December 4, 2009

Christine M. Reed
DIRECTOR OF HIGHWAYS, CHIEF ENGINEER

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OF THE STATE OF ILLINOIS**

GENERAL NOTES

(Revised May 13, 2009)

THE THICKNESS OF HMA SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE HMA IS PLACED.

THE HMA SURFACE OF ALL MAILBOX TURNOUTS, PRIVATE ENTRANCES, COMMERCIAL ENTRANCES, AND SIDE ROADS SHALL BE MADE NEATLY, IN A WORKMANLIKE MANNER, AND SHALL ACCURATELY CONFORM TO THE SHAPES AND DIMENSIONS SHOWN ON THE PLAN DETAILS. IF REQUIRED BY THE ENGINEER, THE CONTRACTOR SHALL BE REQUIRED TO SAW CUT THE HMA SURFACE TO CONFORM TO THE SHAPES AND DIMENSIONS SHOWN ON THE PLAN DETAILS. THIS WORK SHALL BE INCLUDED IN THE COST OF THE HMA SURFACE.

EXCEPT AS NOTED ON THE PLANS, PAVEMENT GRADES SHOWN ARE AT THE TOP OF PAVEMENT SURFACES.

BEFORE ORDERING PIPE CULVERTS OR PIPE DRAINS, THE CONTRACTOR SHALL CONSULT THE ENGINEER FOR EXACT LENGTHS.

THE ENGINEER WILL BE THE SOLE JUDGE CONCERNING CURING TIME FOR THE VARIOUS HMA LIFTS.

FOR STABILIZATION, ALL TYPE III BARRICADES SHALL REQUIRE A MINIMUM OF FOUR SAND BAGS PER BARRICADE.

SEEDING SHALL NOT BE PERMITTED AT ANY TIME WHEN THE GROUND IS FROZEN, WET, OR IN AN UNTILLABLE CONDITION. LOCATIONS TO BE SEEDED WILL BE DETERMINED BY THE ENGINEER.

THE FINISHED EARTHWORK SHALL HAVE A VEGETATION SUSTAINING SOIL COVERING THE TOP FOUR INCHES IN AREAS TO BE SEEDED OR SODDED. THE VEGETATION SUSTAINING SOIL REQUIRED WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION.

ALL ELEVATIONS REFERRING TO U.S.G.S. MEAN SEA LEVEL DATUM.

ABANDONED UNDERGROUND UTILITIES THAT CONFLICT WITH CONSTRUCTION SHALL BE DISPOSED OF OUTSIDE THE LIMITS OF THE RIGHT OF WAY ACCORDING TO ARTICLE 202.03 OF THE STANDARD SPECIFICATIONS AND AS DIRECTED BY THE ENGINEER. THIS WORK WILL NOT BE PAID FOR SEPARATELY, BUT SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION.

ANY REFERENCE TO A STANDARD IN THESE PLANS SHALL BE INTERPRETED TO MEAN THE EDITION AS INDICATED BY THE SUBNUMBER SHOWN IN THE LIST OF STANDARDS OR THE COPY INCLUDED IN THESE PLANS.

THE FOLLOWING RATES OF APPLICATION HAVE BEEN USED IN CALCULATING PLAN QUANTITIES:

GRANULAR MATERIALS	2.05	TONS / CU YD
BITUMINOUS MATERIALS (PRIME COAT)	0.08	GAL / SQ YD
FOR ADDITIONAL HMA LIFTS "FOG COAT"	0.05	GAL / SQ YD
AGGREGATE PRIME COAT	0.002	TONS / SQ YD
HMA RESURFACING	112	LBS / SQ YD / IN
SHORT TERM PAVEMENT MARKING	10	FT /100 FT OF APPLICATION
MIX FOR CRACKS, JTS & FLGWYS	0.0003	TONS / SQ YD
LEVEL BINDER (HAND METHOD)	0.0005	TONS / SQ YD
CALCIUM CHLORIDE	2	LB / SQ YD / APPLICATION
TEMPORARY DITCH CHECKS	5	TONS AGGREGATE

MEMBERS OF JULIE KNOWN TO BE WITHIN THE LIMITS OF THE IMPROVEMENT ARE:

COMMONWEALTH EDISON
VERIZON

NON-MEMBERS OF JULIE KNOWN TO BE WITHIN THE LIMITS OF THE IMPROVEMENT ARE:

THE CONTRACTOR SHALL CONTACT JULIE AT LEAST 48 HOURS PRIOR TO EXCAVATION TO DETERMINE WHICH UTILITIES ARE IN THE AREA.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DISTRICT THREE

PREPARED BY: Rick Powell
DISTRICT STUDIES & PLANS ENGINEER

DATE: 10/19/2009

EXAMINED BY: [Signature]
DISTRICT CONSTRUCTION ENGINEER

[Signature]
DISTRICT MATERIALS ENGINEER

[Signature]
DISTRICT OPERATIONS ENGINEER

FILE NAME =	USER NAME = schwanke	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL NOTES	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
ct:\pw\work\pwidot\schwankerg\dms73146\d	12345-sht-cover.dgn	DRAWN -	REVISED -			41	15 BR-2	LIVINGSTON	64	2	
	PLOT SCALE = 50.0000' / IN.	CHECKED -	REVISED -			CONTRACT NO. 66691					
	PLOT DATE = Oct 16, 2009 - 01:49:49 PM	DATE -	REVISED -			SCALE:	SHEET NO.	OF	SHEETS	STA.	TO STA.

SUMMARY OF QUANTITIES				80% FED. 20% STATE X070-2A
CODE NO.	CONSTRUCTION CODE TYPE: ITEM	UNIT	TOTAL QUANTITY	SN 050-150
20101000	TEMPORARY FENCE	FOOT	398	398
20200100	EARTH EXCAVATION	CU YD	677	677
20400800	FURNISHED EXCAVATION	CU YD	615	615
20700400	POROUS GRANULAR EMBANKMENT, SPECIAL	CU YD	144	144
21101625	TOPSOIL FURNISH AND PLACE, 6"	SQ YD	187	187
* 25000110	SEEDING, CLASS 1A	ACRE	1.0	1.0
* 25000400	NITROGEN FERTILIZER NUTRIENT	POUND	90	90
* 25000500	PHOSPHORUS FERTILIZER NUTRIENT	POUND	90	90
* 25000600	POTASSIUM FERTILIZER NUTRIENT	POUND	90	90
25100630	EROSION CONTROL BLANKET	SQ YD	4840	4840
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	100	100
28000305	TEMPORARY DITCH CHECKS	FOOT	50	50
28000400	PERIMETER EROSION BARRIER	FOOT	751	751
28000500	INLET AND PIPE PROTECTION	EACH	3	3
28100107	STONE RIPRAP, CLASS A4	SQ YD	1347	1347
28200200	FILTER FABRIC	SQ YD	1347	1347
35501316	HOT-MIX ASPHALT BASE COURSE, 8"	SQ YD	1631	1631
35501400	HOT-MIX ASPHALT BASE COURSE, VARIABLE DEPTH	TON	728	728
40200800	AGGREGATE SURFACE COURSE, TYPE B	TON	135	135
40600100	BITUMINOUS MATERIALS (PRIME COAT)	GALLON	346	346
40600300	AGGREGATE (PRIME COAT)	TON	9	9
40600400	MIXTURE FOR CRACKS, JOINTS, AND FLANGWAYS	TON	0.9	0.9
40600525	LEVELING BINDER (HAND METHOD), N50	TON	1.4	1.4
40600625	LEVELING BINDER (MACHINE METHOD), N50	TON	120	120
40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SQ YD	587	587
40600990	TEMPORARY RAMP	SQ YD	19	19
40603310	HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N50	TON	354	354
40800050	INCIDENTAL HOT-MIX ASPHALT SURFACING	TON	14	14
44000155	HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2"	SQ YD	779	779
50101500	REMOVAL OF EXISTING SUPERSTRUCTURES	EACH	1	1

SUMMARY OF QUANTITIES				80% FED. 20% STATE X070-2A
CODE NO.	CONSTRUCTION CODE TYPE: ITEM	UNIT	TOTAL QUANTITY	SN 050-150
50102400	CONCRETE REMOVAL	CU YD	11.4	11.4
50105200	REMOVE EXISTING CULVERTS	EACH	2	2
50200100	STRUCTURE EXCAVATION	CU YD	144	144
50300225	CONCRETE STRUCTURES	CU YD	46.2	46.2
50300255	CONCRETE SUPERSTRUCTURE	CU YD	359.0	359.0
50300260	BRIDGE DECK GROOVING	SQ YD	906	906
50300300	PROTECTIVE COAT	SQ YD	1144	1144
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1	1
50500505	STUD SHEAR CONNECTORS	EACH	4284	4284
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	94,670	94,670
50800515	BAR SPLICERS	EACH	915	915
51500100	NAME PLATES	EACH	1	1
52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	12	12
52100520	ANCHOR BOLTS, 1"	EACH	36	36
52100530	ANCHOR BOLTS, 1 1/4"	EACH	24	24
54200217	PIPE CULVERTS, CLASS D, TYPE 1 12"	FOOT	4	4
54200241	PIPE CULVERTS, CLASS D, TYPE 1 36"	FOOT	300	300
54215571	METAL END SECTIONS 36"	EACH	2	2
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	84	84
60109580	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	140	140
60221100	MANHOLES, TYPE A, 5'-DIAMETER, TYPE 1 FRAME, CLOSED LID	EACH	1	1
60236200	INLETS, TYPE A, TYPE 8 GRATE	EACH	1	1
61100605	MISCELLANEOUS CONCRETE	CU YD	7.0	7.0
* 63000001	STEEL PLATE BEAM GUARD RAIL, TYPE A, 6 ^{FOOT} POSTS	FOOT	163	163
* 63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4	4
* 63100169	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) FLARED	EACH	4	4
63200310	GUARDRAIL REMOVAL	FOOT	475	475
63301000	REMOVE AND RE ERECT STEEL PLATE BEAM GUARD RAIL	FOOT	250	250
67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	8	8
67100100	MOBILIZATION	L SUM	1	1

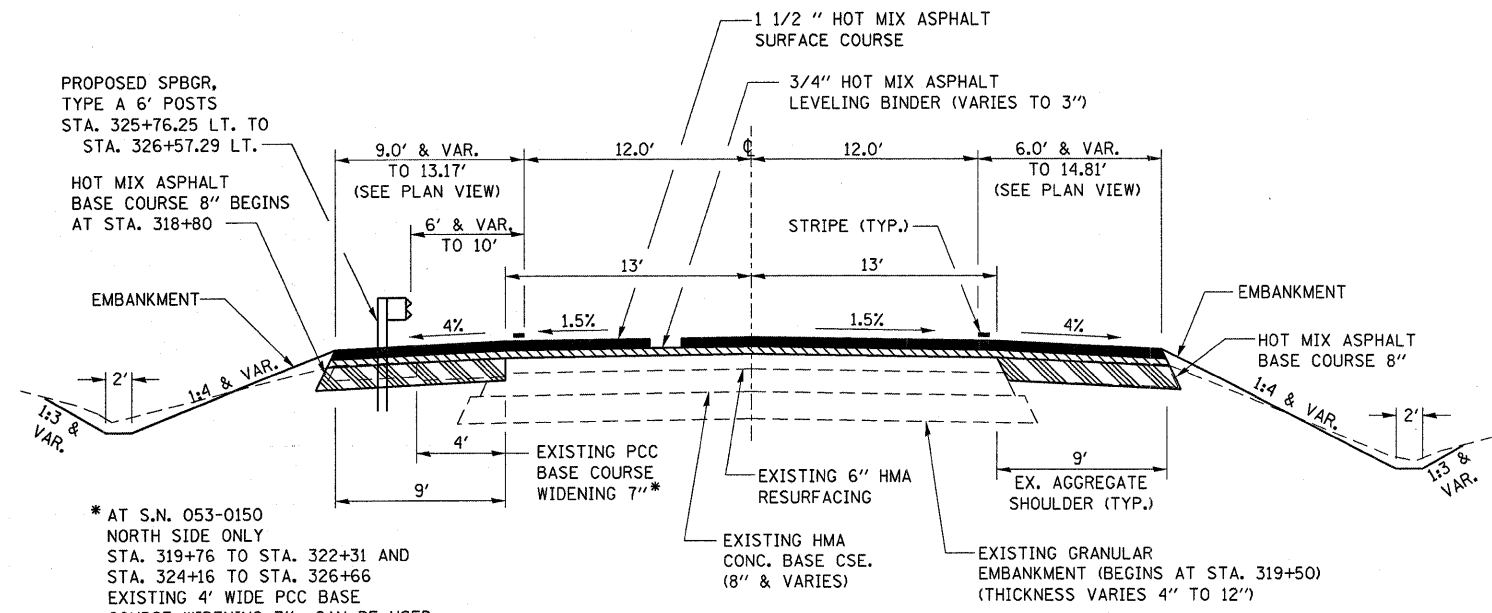
*SPECIALTY ITEM

SUMMARY OF QUANTITIES

80% FED.
20% STATE
X070-2A

CODE NO.	CONSTRUCTION CODE TYPE: ITEM	UNIT	TOTAL QUANTITY	SN 050-150
70100405	TRAFFIC CONTROL AND PROTECTION, STANDARD 701321	EACH	1	1
70100460	TRAFFIC CONTROL AND PROTECTION, STANDARD 701306	L SUM	1	1
70100500	TRAFFIC CONTROL AND PROTECTION, STANDARD 701326	L SUM	1	1
70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	5	5
70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	1	1
70300100	SHORT-TERM PAVEMENT MARKING	FOOT	678	678
70300220	TEMPORARY PAVEMENT MARKING - LINE 4"	FOOT	3813	3813
70300280	TEMPORARY PAVEMENT MARKING - LINE 24"	FOOT	48	48
70301000	WORK ZONE PAVEMENT MARKING REMOVAL	SQ FT	1099	1099
70400100	TEMPORARY CONCRETE BARRIER	FOOT	713	713
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	662	662
* 78008210	POLYUREA PAVEMENT MARKING TYPE I - LINE 4"	FOOT	2436	2436
* 78008230	POLYUREA PAVEMENT MARKING TYPE I - LINE 6"	FOOT	305	305
* 78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	12	12
* 78100105	RAISED REFLECTIVE PAVEMENT MARKER (BRIDGE)	EACH	3	3
* 78200410	GUARDRAIL MARKERS, TYPE A	EACH	7	7
* 78200520	BARRIER WALL MARKERS, TYPE B	EACH	6	6
* 78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	4	4
78300100	PAVEMENT MARKING REMOVAL	SQ FT	481	481
78300200	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL	EACH	15	15
X0323988	TEMPORARY SOIL RETENTION SYSTEM	SQ FT	98	98
X0325305	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT	49	49
Z0001900	ASBESTOS BEARING PAD REMOVAL	EACH	128	128
Z0030250	IMPACT ATTENUATORS, TEMPORARY (NON- REDIRECTIVE), TEST LEVEL 3	EACH	2	2
Z0030350	IMPACT ATTENUATORS, RELOCATE (NON- REDIRECTIVE), TEST LEVEL 3	EACH	2	2

* SPECIALTY ITEMS

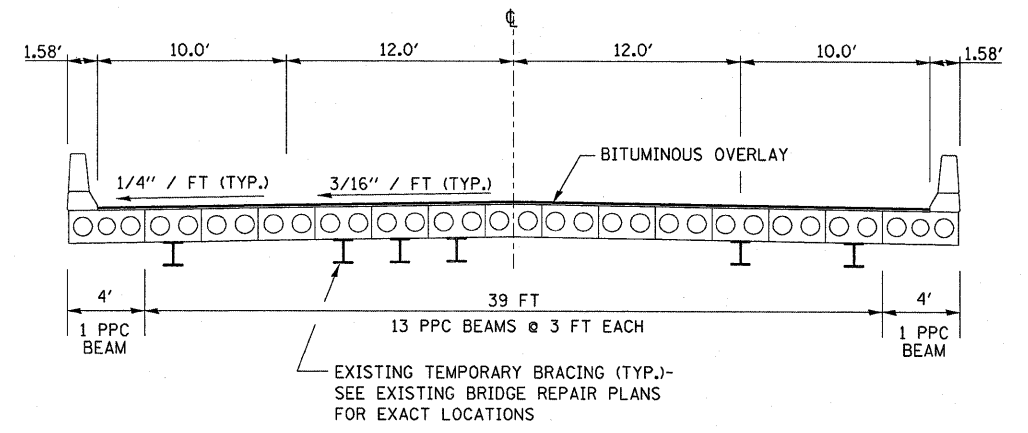


PROPOSED SPBGR, TYPE A 6' POSTS STA. 325+76.25 LT. TO STA. 326+57.29 LT.
HOT MIX ASPHALT BASE COURSE 8" BEGINS AT STA. 318+80

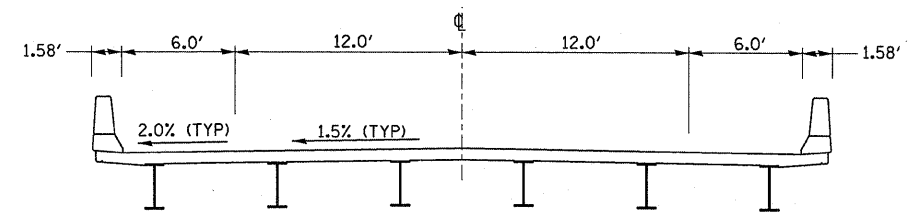
* AT S.N. 053-0150 NORTH SIDE ONLY STA. 319+76 TO STA. 322+31 AND STA. 324+16 TO STA. 326+66 EXISTING 4' WIDE PCC BASE COURSE WIDENING 7" CAN BE USED AT THE ENGINEER'S DISCRETION (SEE STAGING PLANS)

ROADWAY TYPICAL SECTION

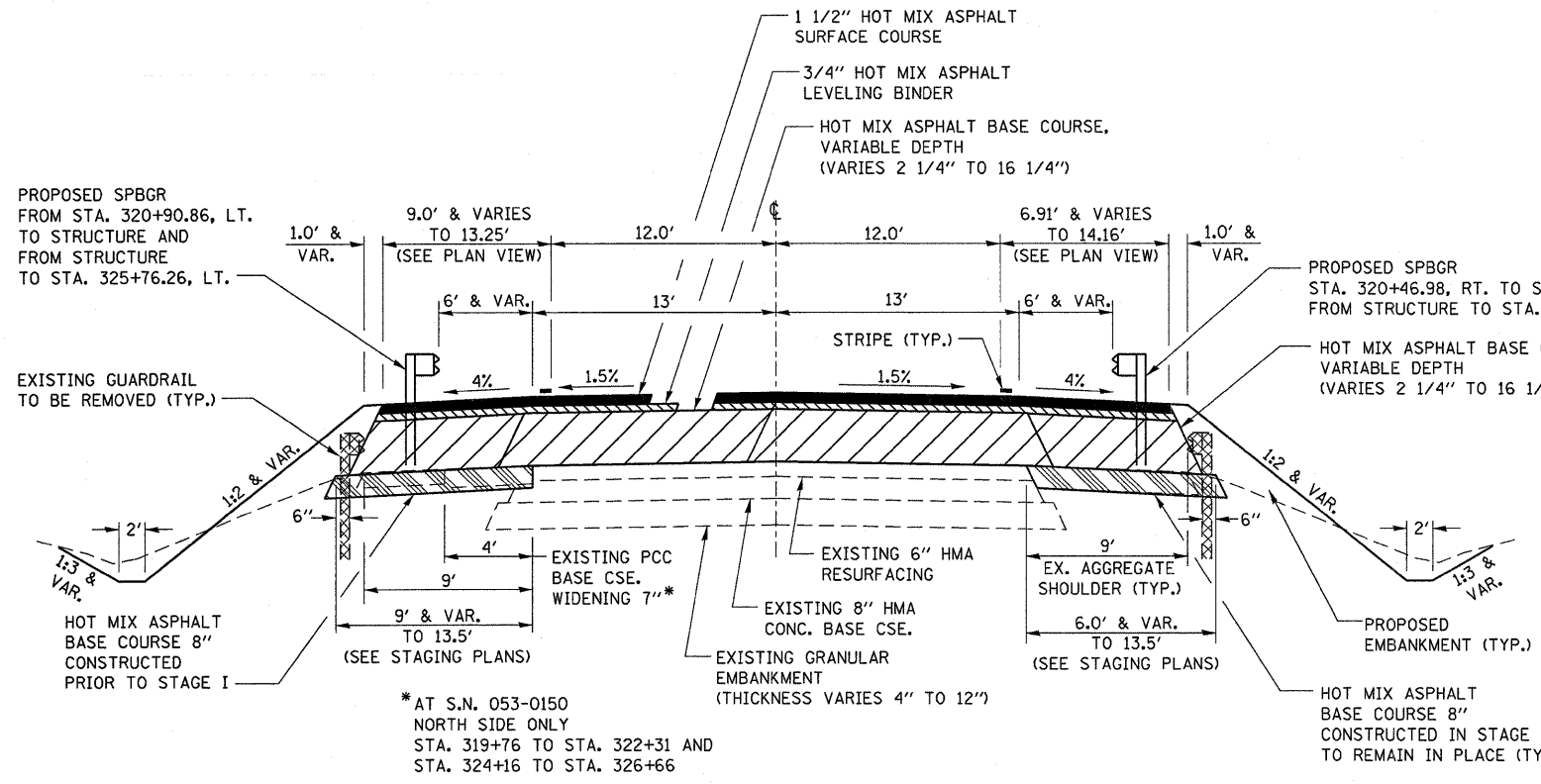
STA. 319+00 TO STA. 320+60.86
STA. 325+76.25 TO STA. 328+48.20



EXISTING BRIDGE TYPICAL SECTION



PROPOSED BRIDGE TYPICAL SECTION



PROPOSED SPBGR FROM STA. 320+90.86, LT. TO STRUCTURE AND FROM STRUCTURE TO STA. 325+76.26, LT.

PROPOSED SPBGR STA. 320+46.98, RT. TO STRUCTURE AND FROM STRUCTURE TO STA. 325+01.09, RT.

* AT S.N. 053-0150 NORTH SIDE ONLY STA. 319+76 TO STA. 322+31 AND STA. 324+16 TO STA. 326+66

ROADWAY TYPICAL SECTION

PROPOSED GUARDRAIL LEFT SIDE
STA. 320+90.86 TO STA. 322+21.78
STA. 324+26.98 TO STA. 326+57.29

STA. 320+60.86 TO STA. 322+01.10
STA. 324+40.98 TO STA. 325+76.25

PROPOSED GUARDRAIL RIGHT SIDE
STA. 320+46.98 TO STA. 322+15.16
STA. 324+20.30 TO STA. 325+01.14

APPROACH SLABS AND BRIDGE OMISSION STA. 322+01.10 TO STA. 324+40.98
APPROACH SLABS AND BRIDGE OMISSION STA. 322+01.10 TO STA. 324+40.98

	HMA BASE COURSE 8"	HMA LEVELING BINDER	HMA SURFACE
PG GRADE	PG64-22	PG64-22	PG64-22
DESIGN AIR VOIDS	4.0% @ N70	4.0% @ N50	4.0% @ N70
MIXTURE COMPOSITION	IL 19.0	IL 9.5	IL 12.5 OR IL 9.5
FRICTION AGGREGATE			MIXTURE C
DENSITY TEST METHOD	CORES*	SATISFACTION OF ENGINEER	CORES*

* MATERIAL SHALL BE COMPACTED TO 93.0-97.4 PERCENT OF THE MAXIMUM THEORETICAL DENSITY, EXCEPT THAT WHEN PLACED AS FIRST LIFT ON AN IMPROVED SUBGRADE THE MINIMUM PERCENT COMPACTION SHALL BE 92.0 PERCENT. THE MAXIMUM THEORETICAL DENSITY SHALL BE DETERMINED FROM THE MOVING AVERAGE AS SPECIFIED IN THE QC/QA SPECIFICATION.

ENTRANCES AND SIDE ROAD SCHEDULE									
STATION	ENTRANCE TYPE	SIDE	AREA	EXISTING SURFACE	INCIDENTAL HOT-MIX ASPHALT SURFACING	BITUMINOUS MATERIALS (PRIME COAT)	AGGREGATE (PRIME COAT)	TEMP RAMP	AGGREGATE SURFACE COURSE TYPE B
			SQ YD		TON	GALLON	TON	SQ YD	TONS
319+49.5	FE	LT	109	AGG					47.7
319+53.9	PE	RT	58	HMA/AGG	6.5	4.7	0.1	5.6	
320+45.9	PE	LT	65	HMA	7.9	5.2	0.1		
325+23.3	FE	RT	81	AGG					37.3
319+60 TO 320+40	TEMPORARY PE	LT	146						49.8
TOTALS					14.4	9.9	0.2	5.6	134.8

SEEDING					
LOCATION	SEEDING, CLASS 2	NITROGEN FERTILIZER NUTRIENT	PHOSPHORUS FERTILIZER NUTRIENT	POTASSIUM FERTILIZER NUTRIENT	EROSION CONTROL BLANKET
	ACRE	LBS	LBS	LBS	SQ YD
RIGHT SIDE STA. 319+00.00 TO STA. 329+00.00	0.33	29.7	29.7	29.7	1597.2
LEFT SIDE STA. 319+00.00 TO STA. 329+00.00	0.67	60.3	60.3	60.3	3242.8
TOTALS	1.0	90	90	90	4840

EARTHWORK SCHEDULE					
LOCATION STA. TO STA.	SIDE	EARTH EXCAVATION	EARTH EXCAVATION ADJUSTED FOR SHRINKAGE (25%)	EMBANKMENT	EARTHWORK BALANCE WASTE (+) OR SHORTAGE(-)
		CU YD	CU YD		CU YD
PRE-STAGE I 318+80.00 TO 322+01.10	LEFT	34.4	25.8	821.1	-795.3*
BRIDGE OMISSION 324+40.98 TO 328+48.20	LEFT	58.0	43.5	4.3	39.2
SUB-TOTAL PRE-STAGE I		92.4	69.3	825.4	-756.1
STAGE I 319+00.00 TO 322+01.10	RIGHT	119.3	89.5	103.6	-14.1
BRIDGE OMISSION 324+40.98 TO 328+48.20	RIGHT	99.7	74.8	87.2	-12.5
SUB-TOTAL STAGE I		219.0	164.3	190.8	-26.6
STAGE II 324+40.98 TO 328+48.20	LEFT	266.4	199.8	103.4	96.4
SUB-TOTAL STAGE II		266.4	199.8	103.4	96.4
STAGE III 319+62.00 TO 320+38.00	LEFT	99.2	74.4	0.0	74.4
SUB-TOTAL STAGE III		99.2	74.4	0.0	74.4
TOTAL		677	507.8	1119.6	-611.9

*330.2 CU YDS FOR TEMPORARY ENTRANCE STA. 319+62 TO STA. 320+38 TO REMAIN.
REMOVE TEMPORARY AGGREGATE AND SHAPE TO DRAIN TO INLET AT STA. 319+72, 53' LT.

GUARDRAIL							
LOCATION	STEEL PLATE BEAM GUARDRAIL TYPE A	TRAFFIC BARRIER TERMINAL TYPE 6	TRAFFIC BARRIER TERMINAL TYPE 1, SPECIAL (FLARED)	REMOVE AND RE-ERECT STEEL PLATE BEAM GUARDRAIL	GUARDRAIL REMOVAL	GUARDRAIL MARKERS, TYPE A (NOTE 3)	BARRIER WALL MARKERS, TYPE B (NOTE 4)
	FEET	EACH	EACH	FEET	FEET	EACH	EACH
AT S.N. 053-0150							
NW QUAD	25.0	1.0	1.0	125.0	125.0	2.0	
NORTH SIDE PARAPET							3.0
NE QUAD	75.0	1.0	1.0	125.0	125.0	2.0	
Note 1 & 2 SE QUAD	0	1.0	1.0		100.0	1.0	
SOUTH SIDE PARAPET							3.0
SW QUAD	63	1.0	1.0		125.0	2.0	
TOTALS	163	4	4	250	475	7	6

Note 1 -- In the SE quad, construct both the TBT Ty 1 (SP) FL and the TBT Ty 6 at a 1:24 rate away from the edge of the shoulder
Note 2 -- In the SE quad, furnish and install only enough of the TBT Ty 1 to provide a 25' length of need
Notes 3 & 4 -- Markers are to be Bi-Directional Crystal

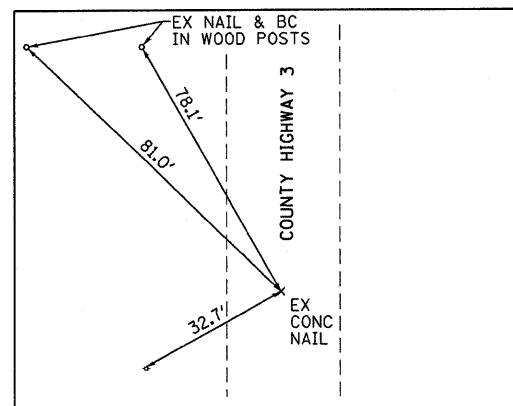
FILE NAME =	USER NAME = schwankeg	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SCHEDULES			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
ct:\pw\work\p\dot\schwankeg\dms73146\d	12345-sh-t-SCHEDULE.DGN	DRAWN -	REVISED -					41	15 BR-2	LIVINGSTON	64	7
PLOT SCALE = 50.0000' / IN.		CHECKED -	REVISED -		CONTRACT NO. 66691			ILLINOIS FED. AID PROJECT				
PLOT DATE = Oct 19, 2009 - 11:10:42 AM		DATE -	REVISED -		SCALE:	SHEET NO. OF SHEETS	STA. TO STA.					

TEMPORARY PAINT PAVEMENT MARKING				
U.S. 17 STATION LOCATIONS			4" LINE	24" LINE
			WHITE FOOT	WHITE FOOT
STAGE I				
317+05				12
329+23				12
INSIDE LINE				
317+05	TO	328+63.20	1159	
OUTSIDE LINE				
318+80	TO	327+63	883	
STAGE II				
317+05				12
329+23				12
INSIDE LINE				
318+25	TO	328+63.20	1039	
OUTSIDE LINE				
319+30	TO	326+62	732	
TOTALS			3,813	48

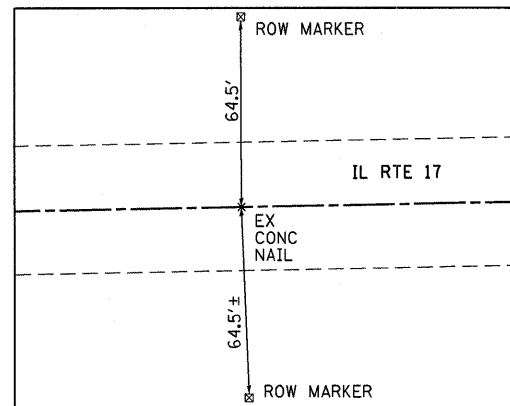
PAVEMENT MARKING					
U.S. 17 STATION LOCATIONS			LIN FT	4" LINE	6" LINE
				WHITE FOOT	YELLOW FOOT
317+05	TO	329+23	1218	2436	304.5
TOTALS			1218	2,436	304.5

RAISED REFLECTIVE PAVEMENT MARKERS				
STA.	TO	STA.	RAISED REFLECTIVE PAVEMENT MARKER	RAISED REFLECTIVE PAVEMENT MARKER (BRIDGE)
			EACH	EACH
317+05.00	TO	322+01.10	6	
322+01.10	TO	324+40.98		3
324+40.98	TO	329+23.00	6	
TOTALS			12	3

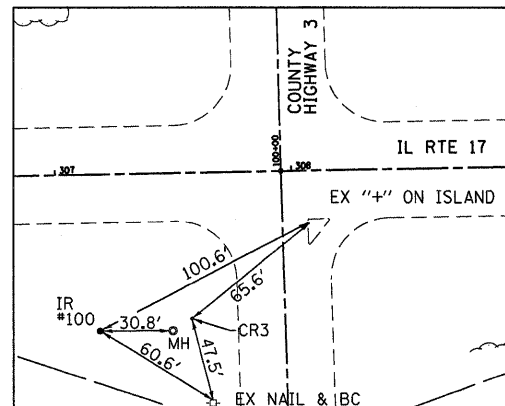
SHORT-TERM PAVEMENT MARKING & WORK ZONE PAVEMENT MARKING REMOVAL						
U.S. 17 STATION LOCATIONS		SHORT-TERM		PAVEMENT MARKING REMOVAL	WORK ZONE PAVEMENT MARKING REMOVAL	
		CENTER LINE YELLOW FOOT	EDGE DASH WHITE FOOT		4" LINE WHITE SQ FT	24" LINE WHITE SQ FT
STAGE I						
317+05				481		
329+23						24 24
INSIDE LINE						
317+05	TO	328+63.20			480	
OUTSIDE LINE						
318+80	TO	327+63				
STAGE II						
317+05						
329+23						
INSIDE LINE						
318+25	TO	328+63.20			345	
OUTSIDE LINE						
319+30	TO	326+62				
STAGE III						
317+05	TO	329+23	366	312		226
(3 APPLICATIONS)						
TOTALS			366	312	481	1051 48



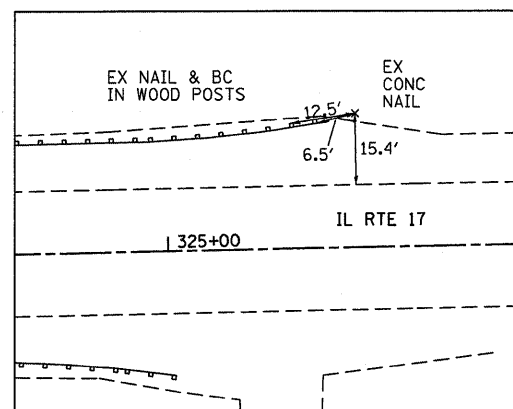
CONTROL TIE STATE #22 (CONC NAIL)
STA 117+20.82



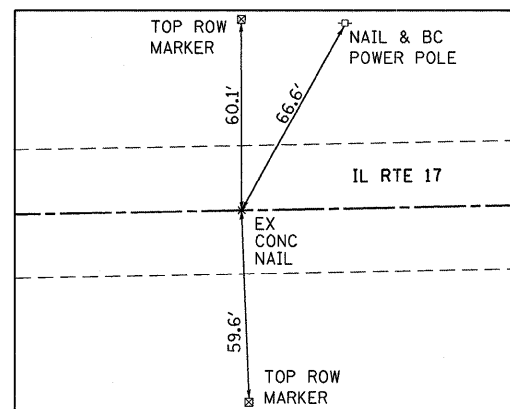
CONTROL TIE STATE #10 (CONC NAIL)
STA 295+00.67



CONTROL TIE STATE (CR3)
STA 307+57/ 61'± RT
CONTROL TIE #100 (IRON ROD)



CONTROL TIE #500 (PK NAIL)
STA 325+40/ 28'± RT



CONTROL TIE STATE #14 (NAIL)
STA 334+58.41

POINT #	NORTHING	EASTING
22	1,614,830.805	991,002.774
10	1,613,087.330	989,754.831
100	1,613,039.675	990,977.467
500	1,613,169.743	992,792.971
14	1,613,157.914	993,711.938

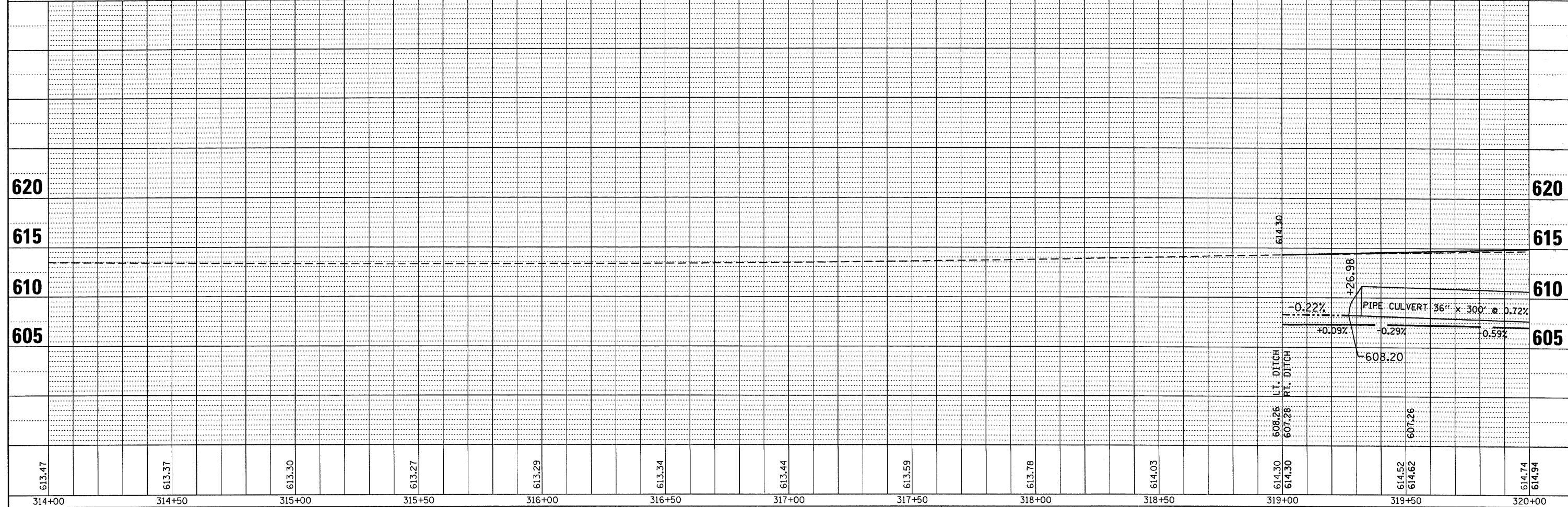
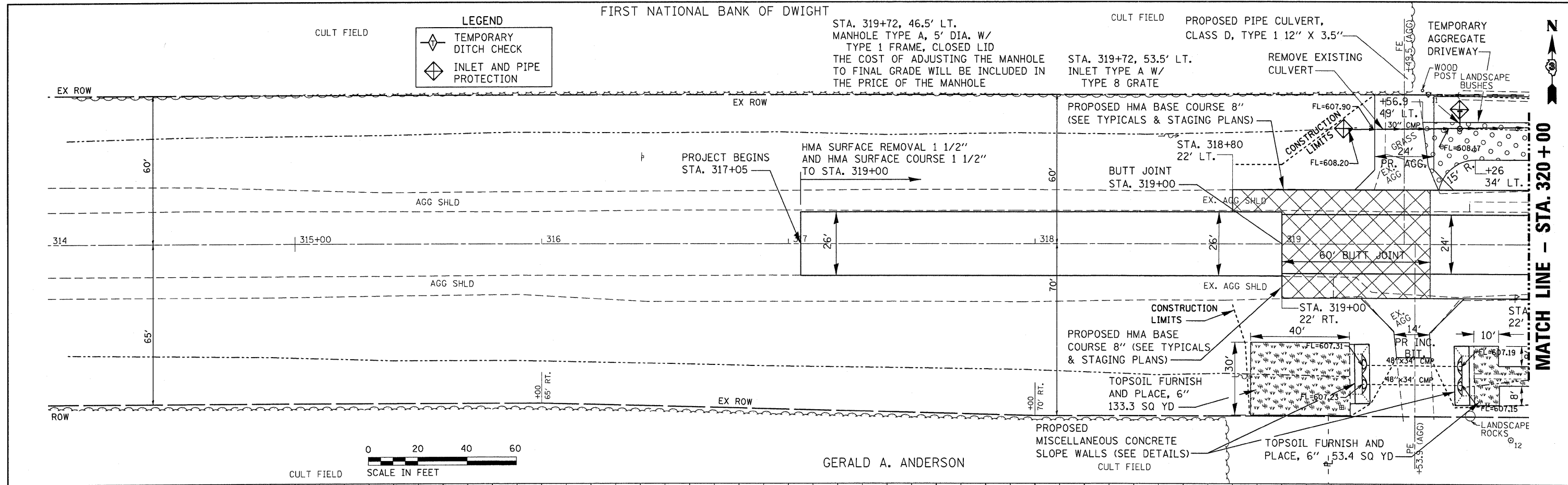
SURVEY CONTROL TIES



FILE NAME =	USER NAME = schwanerg	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TIE POINTS				F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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PLOT SCALE = 28.0000' / IN.	CHECKED -	REVISED -	REVISED -		CONTRACT NO. 66691								
PLOT DATE = Oct 19, 2009 - 01:48:29 PM	DATE -	REVISED -	REVISED -		SCALE:	SHEET NO. OF SHEETS	STA. TO STA.	FED. ROAD DIST. NO.	ILLINOIS FED. AID PROJECT				

PLAN
 SURVEYED _____
 PLOTTED _____
 NOTE BOOK _____
 RT. OF WAY CHECKED _____
 CAD FILE NAME _____

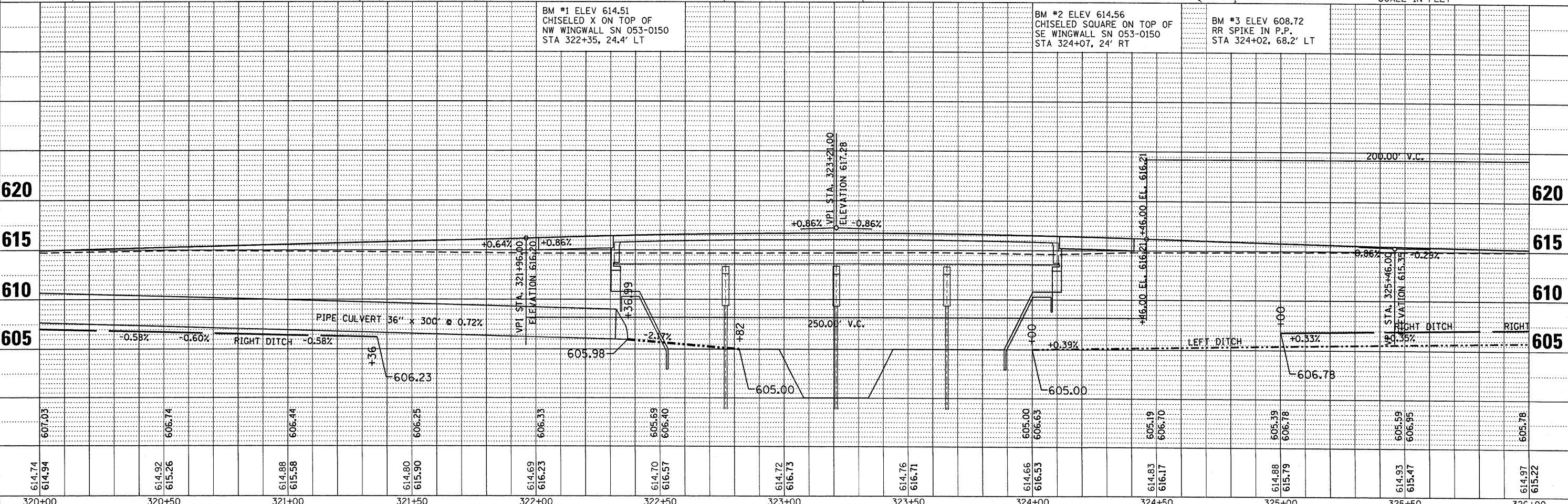
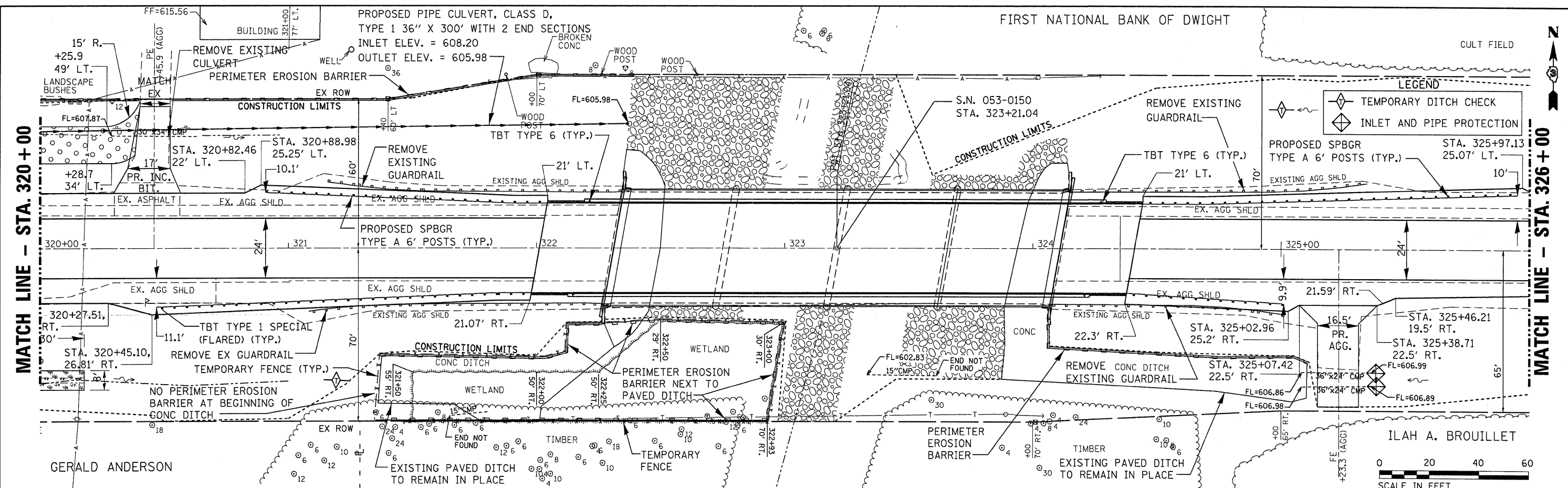
PROFILE
 SURVEYED _____
 PLOTTED _____
 NOTE BOOK _____
 GRADES CHECKED _____
 STRUCTURE NOTATIONS CHKD _____



FILE NAME =	USER NAME = schwankerg	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PLAN AND PROFILE	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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PLOT SCALE = 28.0000' / IN.	CHECKED -	REVISED -	CONTRACT NO. 66691							
PLOT DATE = Oct 19, 2009 - 01:45:19 PM	DATE -	REVISED -	FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT							

PLAN SURVEYED PLOTTED REVISIONS MADE BY DATE
 NOTE BOOK NO. DATE
 STRUCTURE NOTATIONS CHRD

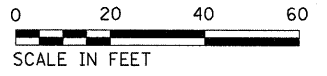
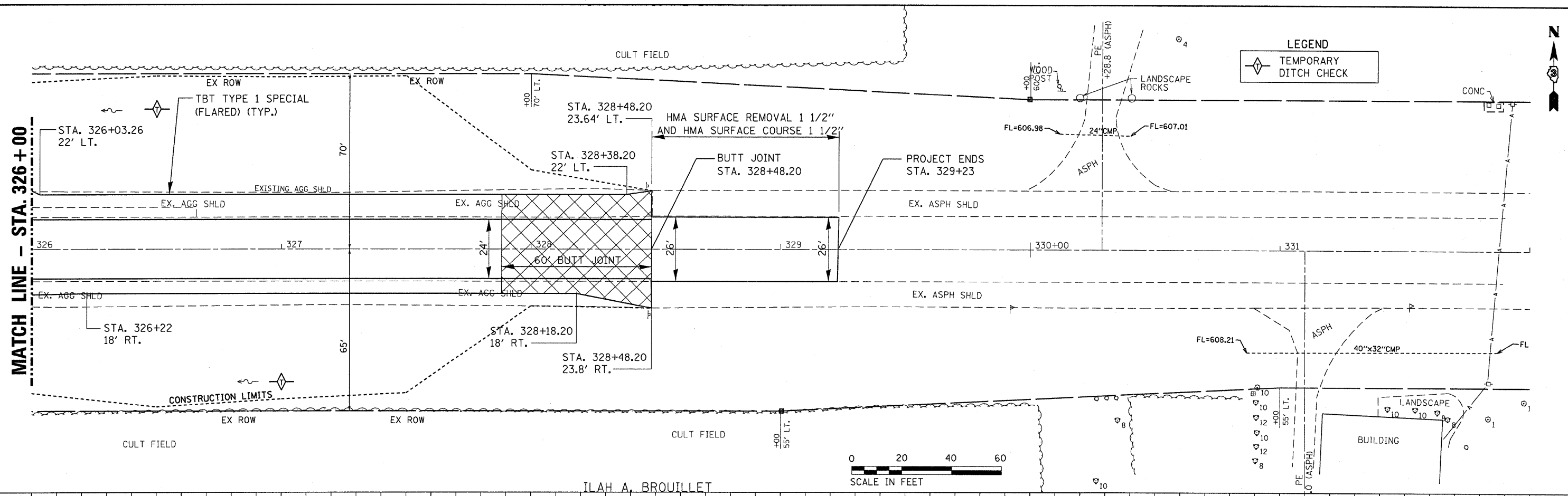
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 NOTE BOOK NO. DATE
 STRUCTURE NOTATIONS CHRD



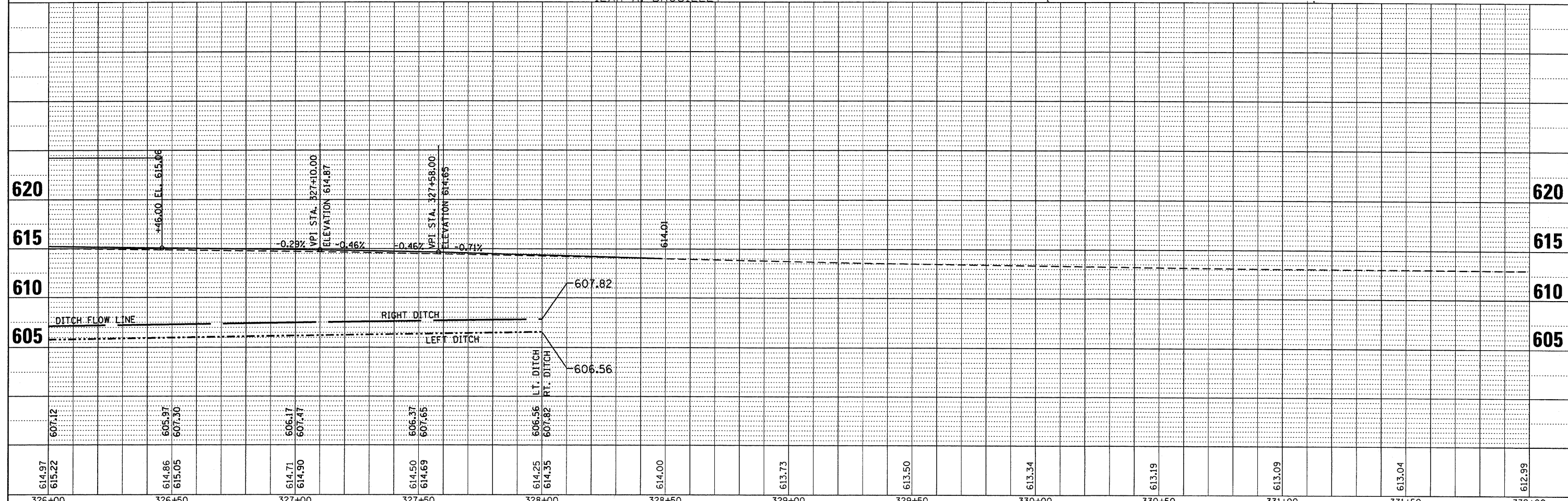
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PLAN	SURVEYED	DATE
	PLOTTED	BY
	GRADES CHECKED	
	STRUCTURE NOTATIONS CHKD	
	NO.	

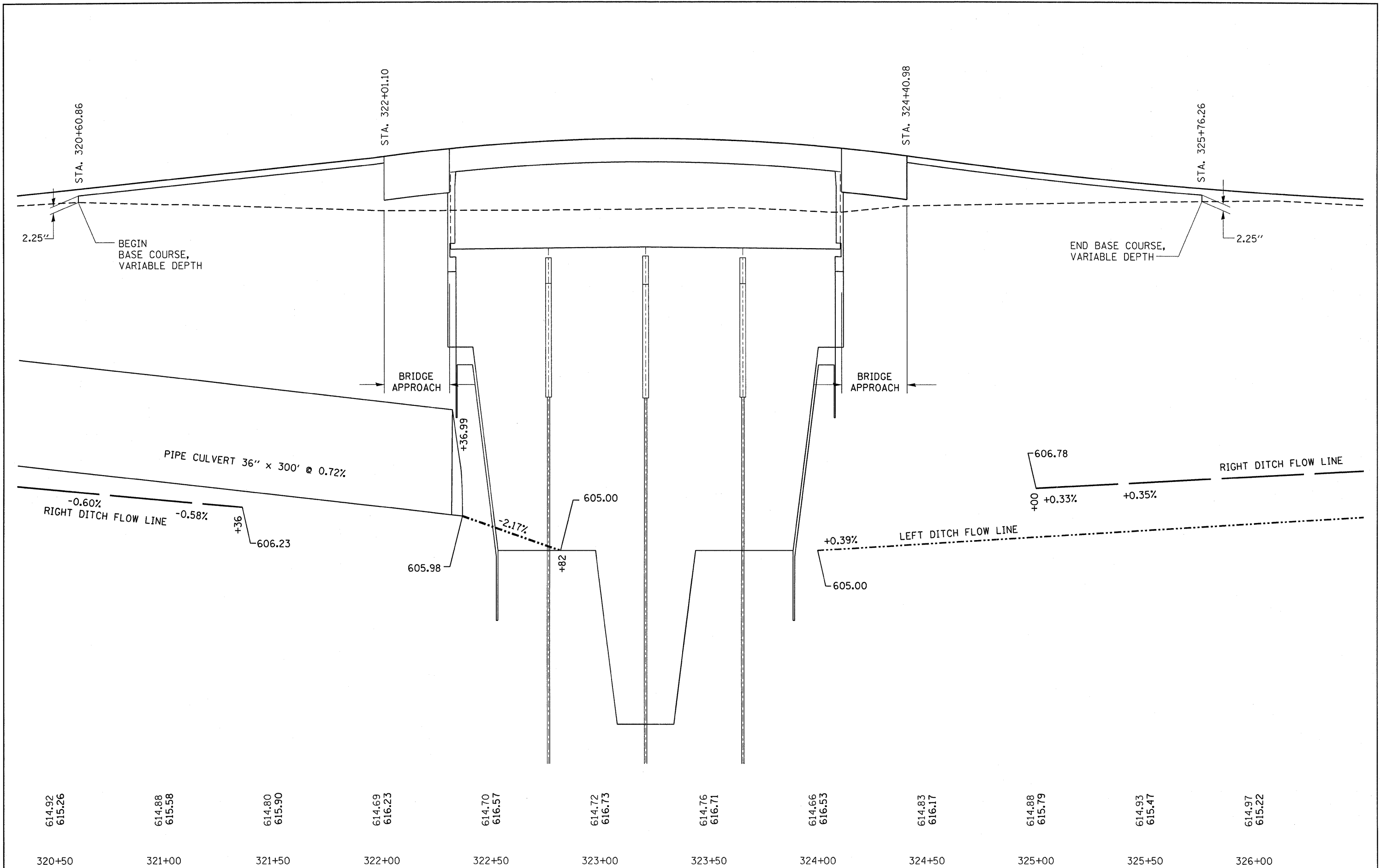
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	PLOTTED	BY
	GRADES CHECKED	
	STRUCTURE NOTATIONS CHKD	
	NO.	



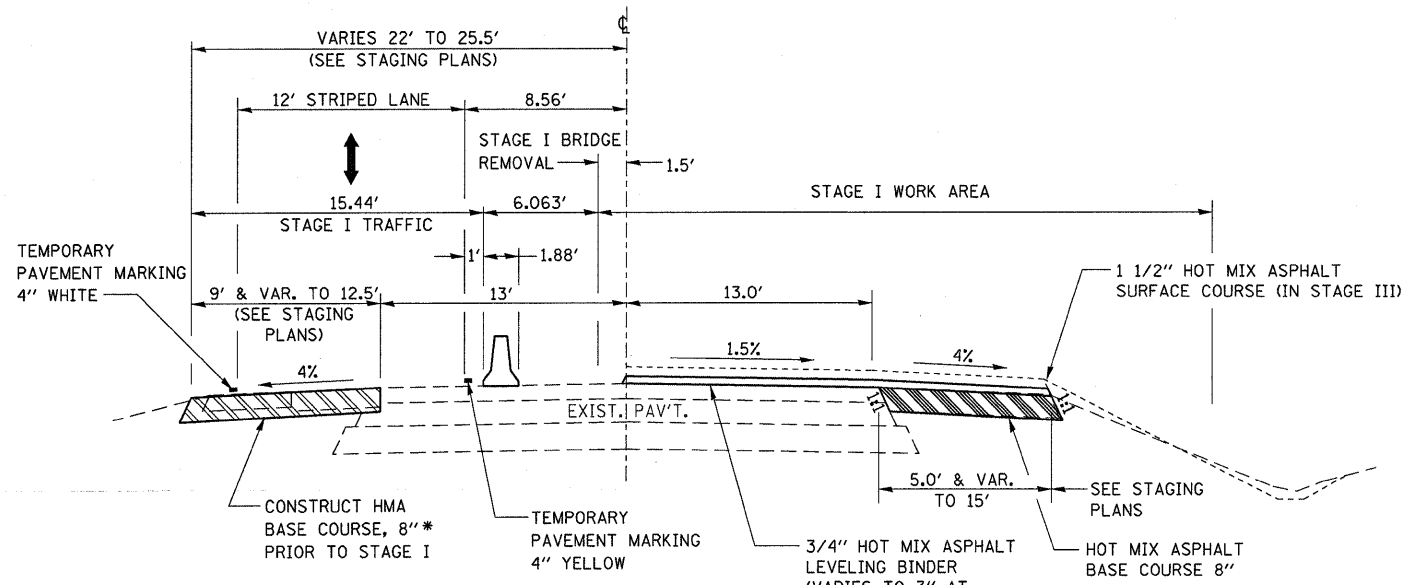
ILAH A. BROUILLET



FILE NAME =	USER NAME = schwanberg	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PLAN AND PROFILE	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
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PLT SCALE = 20.0000' / IN.	CHECKED -	REVISED -	SCALE: 20			SHEET NO. OF SHEETS	STA. 326+00 TO STA. 332+00	CONTRACT NO. 66691			
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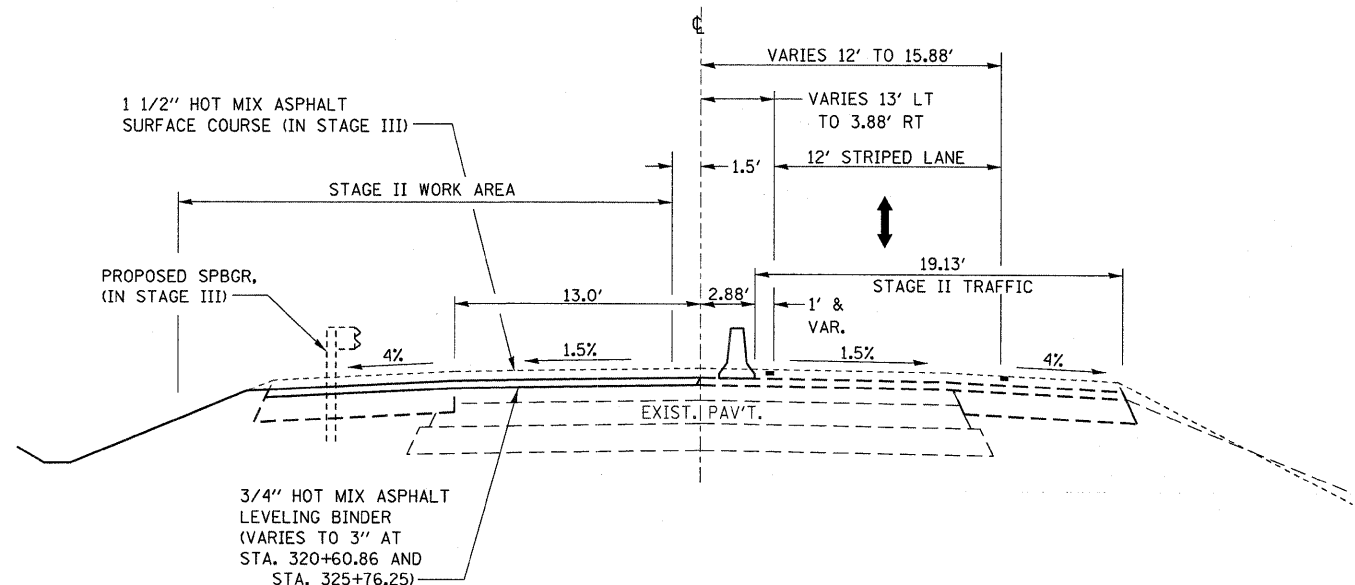


FILE NAME =	USER NAME = schwankeg	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	EXPANDED PROFILE ALONG CENTERLINE				F.A.P. RTE. 41	SECTION 15BR-2	COUNTY LIVINGSTON	TOTAL SHEETS 64	SHEET NO. 13
ci\pw_work\pwwidat\schwankeg\dms73146\d	ta:ls.dgn	DRAWN -	REVISED -		SCALE:	SHEET NO.	OF	SHEETS	STA.	TO STA.	CONTRACT NO. 66691		
		CHECKED -	REVISED -								ILLINOIS FED. AID PROJECT		
		DATE -	REVISED -										

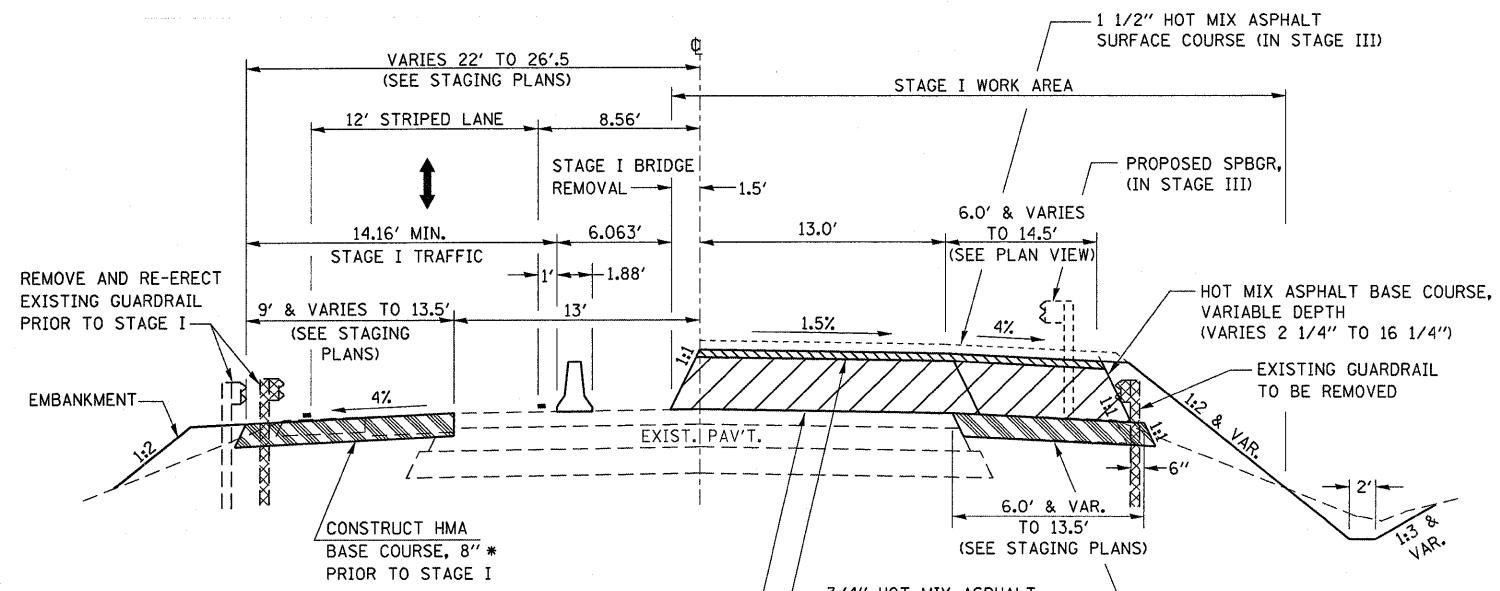


* EXISTING 4' WIDE PCC BASE COURSE WIDENING 7" ON NORTH SIDE CAN BE LEFT IN PLACE AS TEMPORARY PAVEMENT (STA. 319+76 TO STA. 322+31 AND STA. 324+16 TO STA. 326+66)

STAGE I TYPICAL SECTION
 STA. 319+00 TO STA. 320+60.86
 STA. 325+76.25 TO STA. 328+48.20

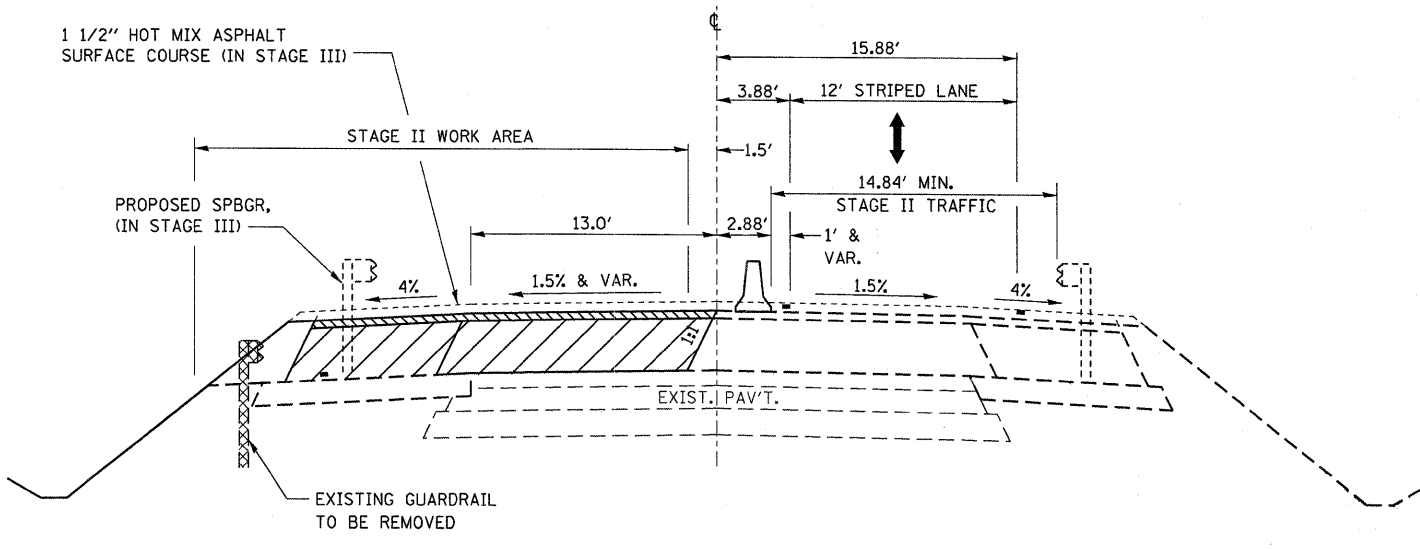


STAGE II TYPICAL SECTION
 STA. 319+00 TO STA. 320+60.86
 STA. 325+76.25 TO STA. 328+48.20



* EXISTING 4' WIDE PCC BASE COURSE WIDENING 7" ON NORTH SIDE CAN BE LEFT IN PLACE AS TEMPORARY PAVEMENT (STA. 319+76 TO STA. 322+31 AND STA. 324+16 TO STA. 326+66)

STAGE I TYPICAL SECTION
 STA. 320+60.86 TO STA. 322+00.25
 BRIDGE OMISSION STA. 322+00.25 TO STA. 324+41.83
 STA. 324+41.83 TO STA. 325+76.26



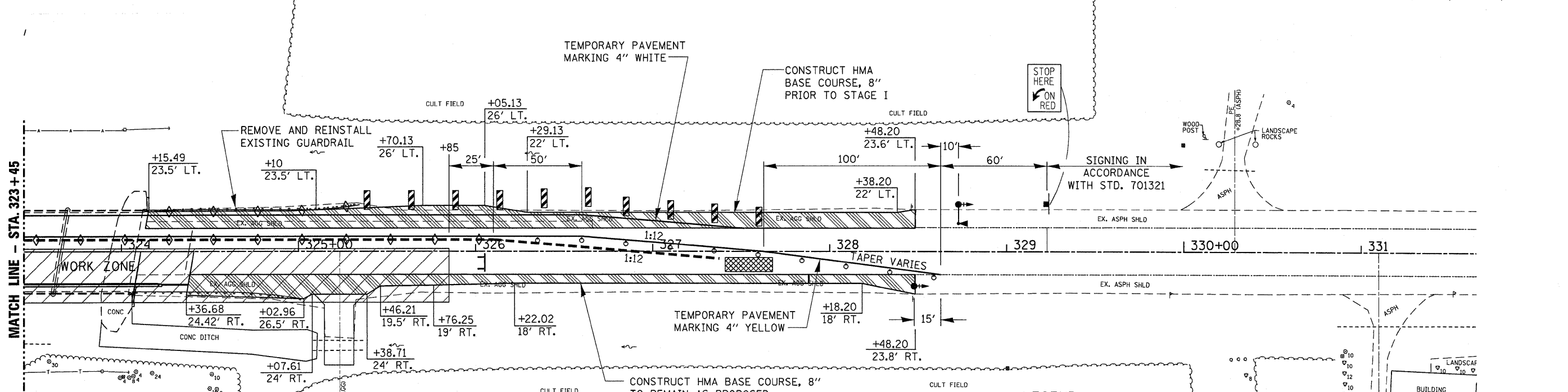
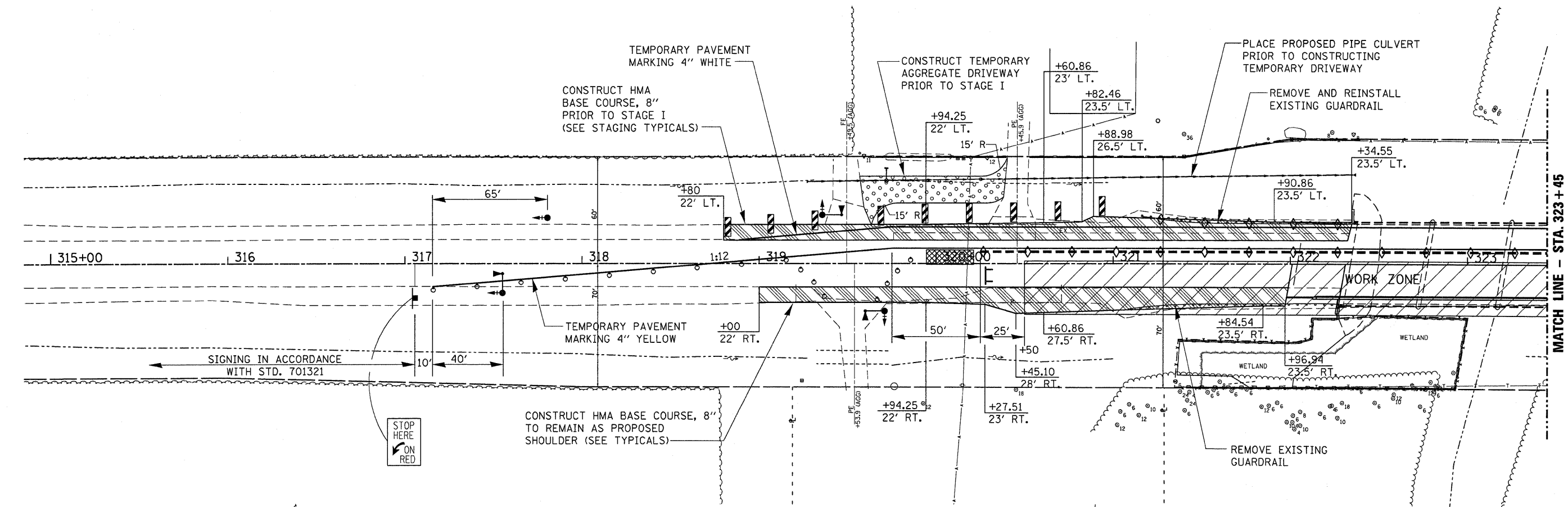
STAGE II TYPICAL SECTION
 STA. 320+60.86 TO STA. 322+00.25
 BRIDGE OMISSION STA. 322+00.25 TO STA. 324+41.83
 STA. 324+41.83 TO STA. 325+76.26

FILE NAME =	USER NAME = schwenkerg	DESIGNED -	REVISED -
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	PLOT DATE = Oct 19, 2009 - 01:40:12 PM	DATE -	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

STAGING TYPICALS			
SCALE:	SHEET NO.	OF SHEETS	STA. TO STA.

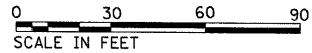
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
41	15BR-2	LIVINGSTON	64	14
CONTRACT NO. 66691				
ILLINOIS FED. AID PROJECT				



NOTES:
 THIS WORK SHALL BE PAID FOR AS TRAFFIC CONTROL AND PROTECTION, STANDARD 701321.
 SEE STANDARD 701321 FOR DETAILS NOT SHOWN.
 SEE SPECIAL PROVISIONS.

LEGEND

	SIGN		IMPACT ATTENUATOR
	TYPE III BARRICADE		TEMPORARY TRAFFIC SIGNAL WITH BACKPLATE
	TC 26 MICROWAVE		TEMPORARY CONCRETE BARRIER
	DOUBLE VERTICAL PANEL		DRUM WITH STEADY BURNING LIGHT
			TYPE C BIDIRECTIONAL REFLECTOR

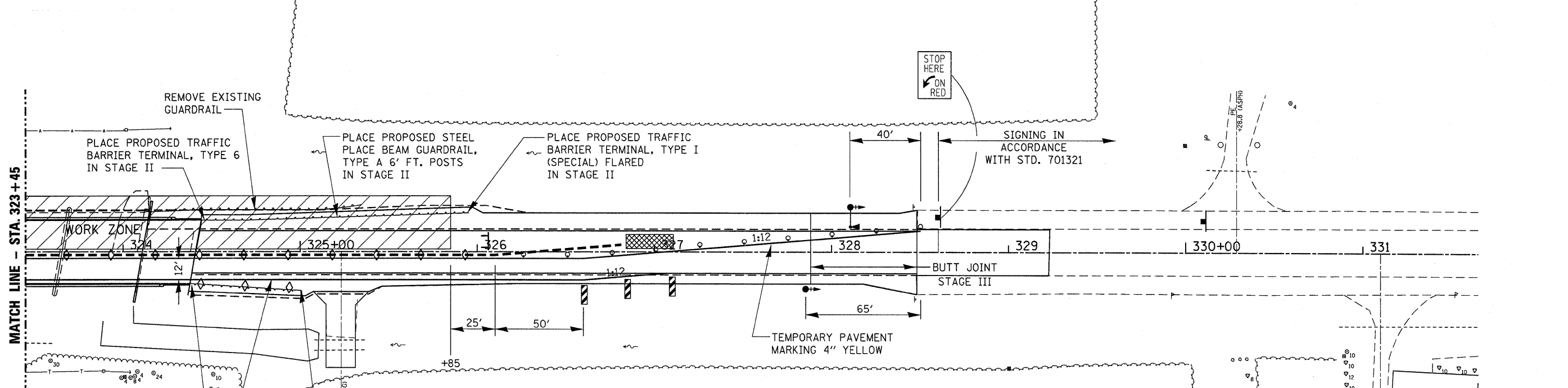
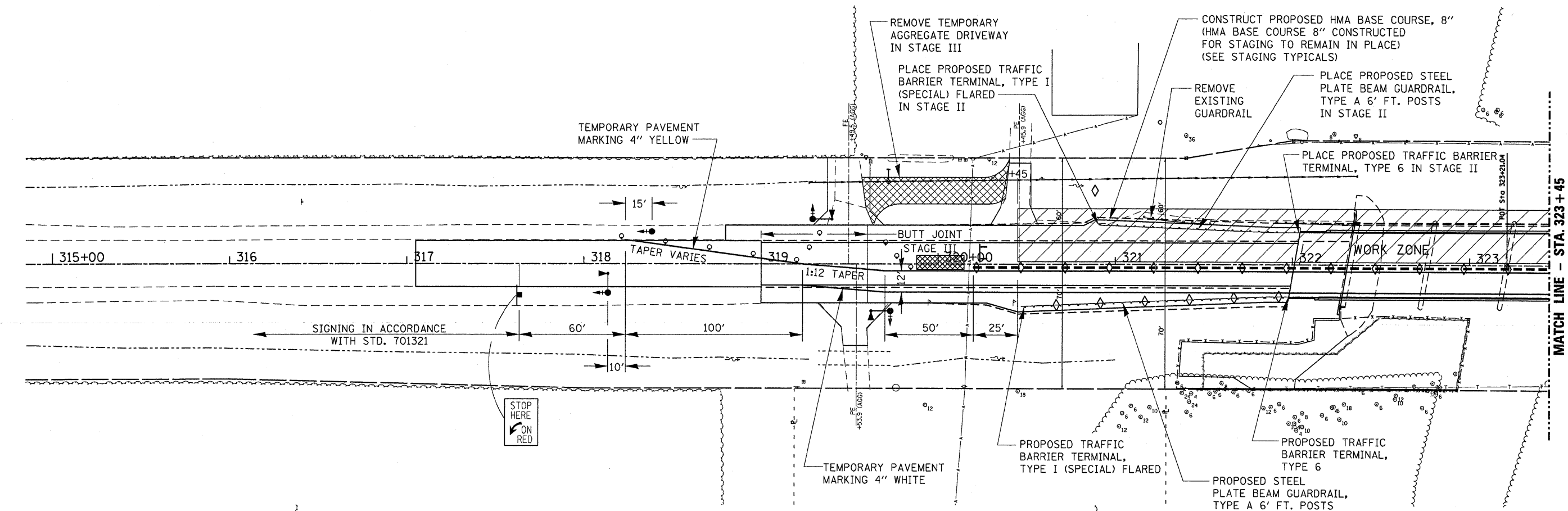


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		CHECKED -	REVISED -
		DATE -	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

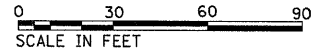
TRAFFIC CONTROL CONSTRUCTION STAGE I			
SCALE:	SHEET NO.	OF SHEETS	STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
41	15 BR-2	LIVINGSTON	64	15
			CONTRACT NO. 66691	
ILLINOIS FED. AID PROJECT				



LEGEND

	SIGN		IMPACT ATTENUATOR
	TYPE III BARRICADE		TEMPORARY TRAFFIC SIGNAL WITH BACKPLATE
	TC 26 MICROWAVE		TEMPORARY CONCRETE BARRIER
	DOUBLE VERTICAL PANEL		DRUM WITH STEADY BURNING LIGHT
			TYPE C BIDIRECTIONAL REFLECTOR



FILE NAME =	USER NAME = schwenkerg	DESIGNED -	REVISED -
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		CHECKED -	REVISED -
		DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL CONSTRUCTION STAGE II			
SCALE:	SHEET NO.	OF SHEETS	STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
41	15 BR-2	LIVINGSTON	64	16
				CONTRACT NO. 66691
ILLINOIS FED. AID PROJECT				

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SCOPE OF WORK

- 1.) Remove and replace the existing bridge superstructure.
- 2.) Remove and replace the existing concrete approach slab.
- 3.) Remove the existing abutment backwall and wingwalls as shown.
- 4.) Convert the existing abutments to semi-integral abutments.
- 5.) Place additional concrete on the existing pier caps in order to meet the proposed grade change.
- 6.) Repair the substructure elements as required.

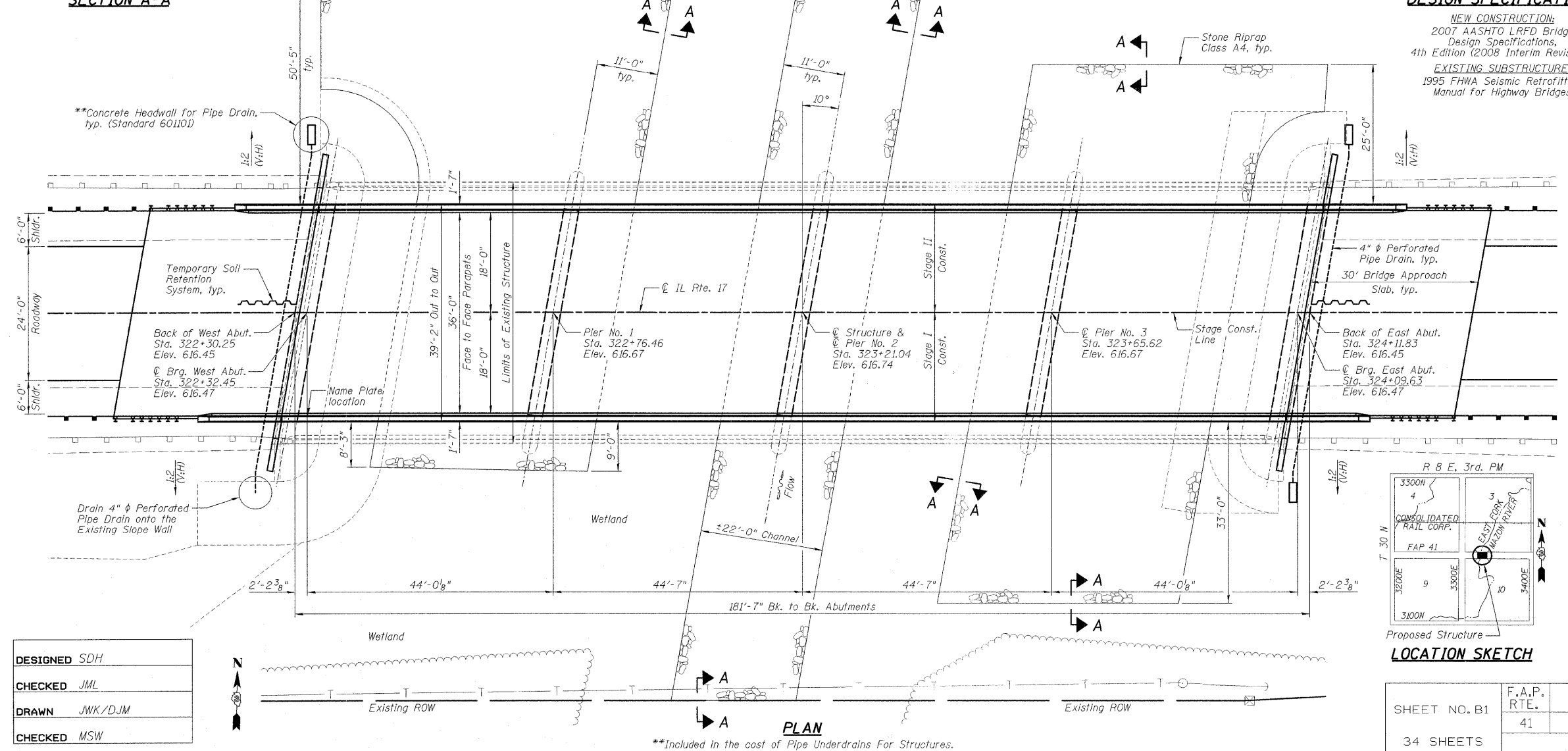
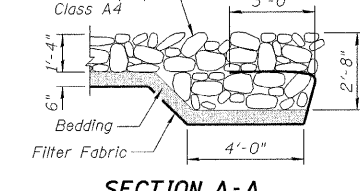
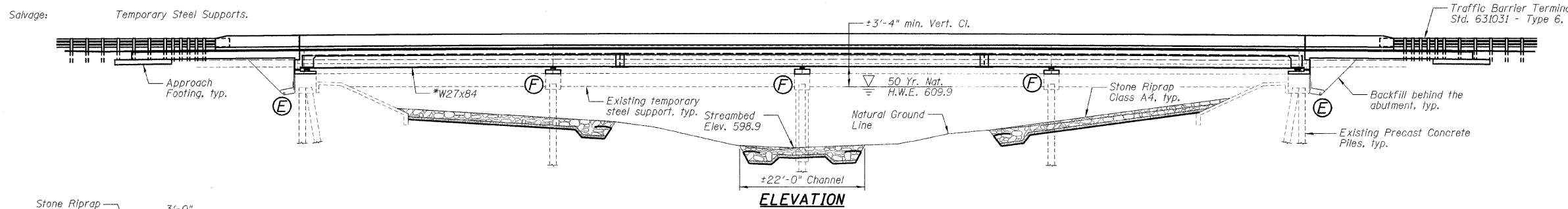
INDEX TO SHEETS

SHEET NO.	TITLE
B1	GENERAL PLAN AND ELEVATION
B2	GENERAL DATA
B3	STAGE CONSTRUCTION
B4	TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION
B5	TOP OF SLAB ELEVATION LOCATIONS
B6-B7	TOP OF SLAB ELEVATIONS
B8	TOP OF WEST APPROACH SLAB ELEVATIONS
B9	TOP OF EAST APPROACH SLAB ELEVATIONS
B10	SUPERSTRUCTURE DECK
B11	SUPERSTRUCTURE CROSS SECTION
B12	SUPERSTRUCTURE DETAILS
B13	WEST ABUTMENT DIAPHRAGM DETAILS
B14	EAST ABUTMENT DIAPHRAGM DETAILS
B15	WEST BRIDGE APPROACH SLAB DETAILS
B16	EAST BRIDGE APPROACH SLAB DETAILS
B17-B18	STRUCTURAL STEEL
B19	TYPE I ELASTOMERIC BEARING DETAILS
B20	FIXED BEARING DETAILS
B21	WEST ABUTMENT REMOVAL
B22-B23	WEST ABUTMENT
B24	EAST ABUTMENT REMOVAL
B25-B26	EAST ABUTMENT
B27	PIER NO. 1 REPAIR
B28	PIER NO. 1
B29	PIER NO. 2 REPAIR
B30	PIER NO. 2
B31	PIER NO. 3 REPAIR
B32	PIER NO. 3
B33	BAR SPLICER ASSEMBLY DETAILS
B34	CANTILEVER FORMING BRACKETS FOR SUPERSTRUCTURES WITH W27 BEAMS AND SMALLER

- Bench Marks:
- 1.) BM #1 Chiseled "□" on top of Northwest wingwall of bridge S.N. 053-0150, Station 322+35/24.4' LT., Elevation = 614.51.
 - 2.) BM #2 Chiseled "□" on top of Southeast wingwall of bridge S.N. 053-0150, Station 324+07/24.0' RT., Elevation = 614.56.
 - 3.) BM #3 Railroad spike in power pole, Station 324+02/68.0' LT., Elevation = 608.72.

Existing Structure: Structure No. 053-0150, built in 1979 as Section 15 BR-2. The superstructure consists of precast prestressed concrete deck beams with a concrete parapet attached to the exterior beams and bituminous wearing surface. The substructure consists of concrete pile bent abutments supported by precast concrete piles, two concrete pile bent piers (Pier No. 1 and Pier No. 3) supported by precast concrete piles and one concrete solid shaft pile bent pier (Pier No. 2) supported by precast concrete piles. The back-to-back of abutments dimension measures 181'-7" and the out-to-out dimension measures 47'-2". The span lengths are 43'-11 3/4", 44'-7" and 43'-11 3/4" (∅ bearing to ∅ bearing) with a 10° left forward skew. The existing superstructure shall be removed and replaced, existing abutments to be converted to semi-integral abutments and the existing piers will be repaired. One lane of traffic will be maintained utilizing stage construction.

Salvage: Temporary Steel Supports.



LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

NEW CONSTRUCTION:
2007 AASHTO LRFD Bridge Design Specifications, 4th Edition (2008 Interim Revisions)

EXISTING SUBSTRUCTURE:
1995 FHWA Seismic Retrofitting Manual for Highway Bridges

DESIGN STRESSES

FIELD UNITS: NEW CONSTRUCTION

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinforcement)
 $f_y = 50,000$ psi (AASHTO M270 Grade 50W)

FIELD UNITS: EXISTING CONSTRUCTION

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinforcement)

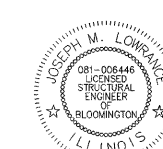
SEISMIC DATA

NEW CONSTRUCTION:

Seismic Performance Zone (SPZ) = 1
Design Spectral Acceleration at 1.0 sec. (SD1) = 0.074 g
Design Spectral Acceleration at 0.2 sec. (SD5) = 0.130 g
Soil Site Class = C

EXISTING CONSTRUCTION:

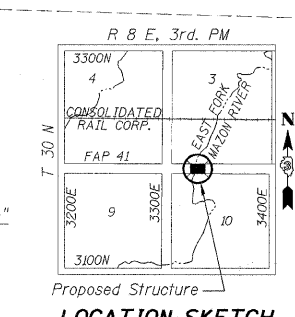
Seismic Performance Category (SPC) = A
Bedrock Acceleration Coefficient (A) = 0.041 g
Site Coefficient (S) = 1.0



Joseph M. Lowrance
ILLINOIS STRUCTURAL ENGINEER
NO. 081-006446
Exp. Date 11/30/10

APPROVED
FOR STRUCTURAL ADEQUACY ONLY

Ralph E. Anderson (TSP)
ENGINEER OF BRIDGES AND STRUCTURES



GENERAL PLAN AND ELEVATION
IL. ROUTE 17 OVER EAST
FORK MAZON RIVER
F.A.P. ROUTE 41 - SECTION 15BR-2
LIVINGSTON COUNTY
STATION 323+21.04
STRUCTURE NO. 053-0150

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

SHEET NO. B1	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	64	17
CONTRACT NO. 66691					
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			

24-8168

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment, Special	Cu. Yd.		144	144
Stone Riprap, Class A4	Sq. Yd.		1,347	1,347
Filter Fabric	Sq. Yd.		1,347	1,347
Removal of Existing Superstructures	Each	1		1
Concrete Removal	Cu. Yd.		11.4	11.4
Structure Excavation	Cu. Yd.		144	144
Concrete Structures	Cu. Yd.		46.2	46.2
Concrete Superstructure	Cu. Yd.	359.0		359.0
Bridge Deck Grooving	Sq. Yd.		906	906
Protective Coat	Sq. Yd.		1,144	1,144
Furnishing and Erecting Structural Steel	L. Sum	1		1
Stud Shear Connectors	Each	4,284		4,284
Reinforcement Bars, Epoxy Coated	Pound	92,230	2,440	94,670
Bar Splicers	Each	898	17	915
Name Plates	Each	1		1
Elastomeric Bearing Assembly, Type I	Each	12		12
Anchor Bolts, 1"	Each	36		36
Anchor Bolts, 1/2"	Each	24		24
Geocomposite Wall Drain	Sq. Yd.		84	84
Pipe Underdrains for Structures 4"	Foot		140	140
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.		49	49
Temporary Soil Retention System	Sq. Ft.		98	98
Asbestos Bearing Pad Removal	Each	128		128

GENERAL NOTES:

- Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts in painted areas and M164 Type 3 in unpainted areas. Bolts 1/8 in. ϕ , holes 1/8 in. ϕ , unless otherwise noted.
- Calculated weight of Structural Steel = 105,440 lbs Grade 50W.
- All structural steel shall be AASHTO M 270 Grade 50W.
- No field welding is permitted except as specified in the contract documents.
- Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60. See Special Provisions.
- Reinforcement bars designated (E) shall be epoxy coated.
- If the Contractor elects to use cantilever forming brackets on the exterior beams or girders, the brackets shall be placed at the same locations as required for the hardwood blocks in Article 503.06(b) of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and first interior beam at each of these additional bracket locations.
- Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- Bearing seat surfaces shall be constructed or adjusted to their designated elevations within a tolerance of 1/8 inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete cap plus 3 in. Those areas shall be primed in the shop with a Department approved zinc rich primer. No field painting shall be required. All structural steel shall be cleaned as specified in the Special Provision for "Surface Preparation and Painting Requirements for Weathering Steel".
- All exposed structural steel of the bearings shall be cleaned and shop painted as specified in the Special Provision for "Surface Preparation and Painting Requirements for Weathering Steel".
- Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- Existing Name Plate shall be cleaned and relocated next to new Name Plate. Cost included with Name Plates.
- Slipforming of parapets is not allowed.
- The SSPC QP-1 Painting Contractor Certification will be required for the contract.
- The pay item Removal of Existing Superstructures shall include the removal of the Temporary Support Connections at the Abutments & Piers, the Temporary Supports under each Superstructure Span and the Approach Pavement at each end of the bridge (beneath the proposed Bridge Approach Slab). All structural steel from the existing temporary supports shall become the property of IDOT and delivered to the Pontiac Maintenance Yard.
- The Contractor is advised that the existing PPC Deck beams are in a deteriorated condition with reduced load-carrying capacity. It is the Contractor's responsibility to account for the condition of the beams when developing construction procedures for removal and replacement of the superstructure.

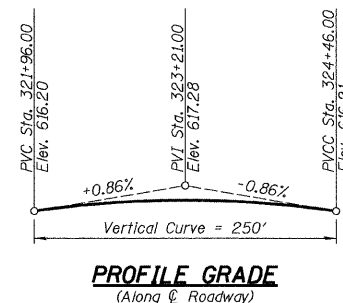
WATERWAY INFORMATION

Flood		Opening		Nat. Head - Ft.		Headwater EL.	
Freq. Yr.	C.F.S.	Exist.	Prop.	H.W.E., Exist.	Prop.	Exist.	Prop.
10	1760	906	906	609.2	0.1	609.3	609.3
Design	50	2540	1011	1011	609.9	0.1	610.0
Base	100	2850	1041	1041	610.1	0.1	610.2
Overtopping	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Max. Calc.	500	3570	1107	1107	610.5	0.1	610.6

10 Yr. Velocity = 1.94 ft./sec. (Proposed)
10 Yr. Velocity = 1.94 ft./sec. (Existing)

STATION 323+21.04
REBUILT 20__ BY
STATE OF ILLINOIS
F.A.P. ROUTE 41 - SECTION 15BR-2
LOADING HL-93
STRUCTURE NO. 053-0150

NAME PLATE
See Std. 515001

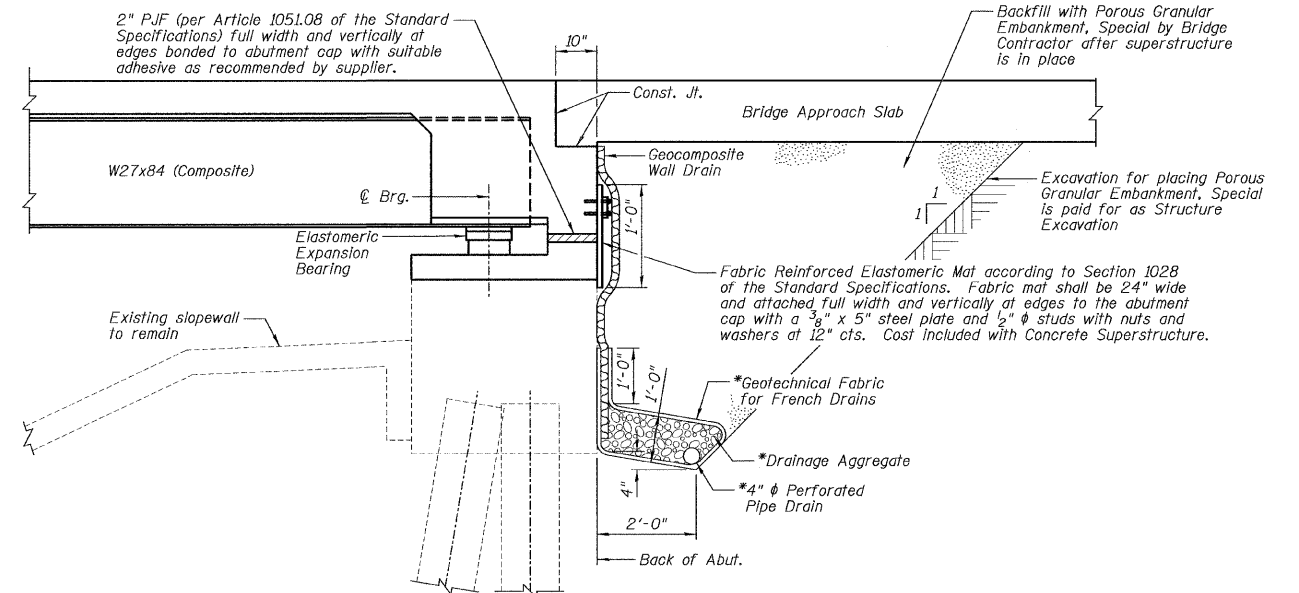


DESIGNED SDH
CHECKED JML
DRAWN JWK/DJM
CHECKED MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

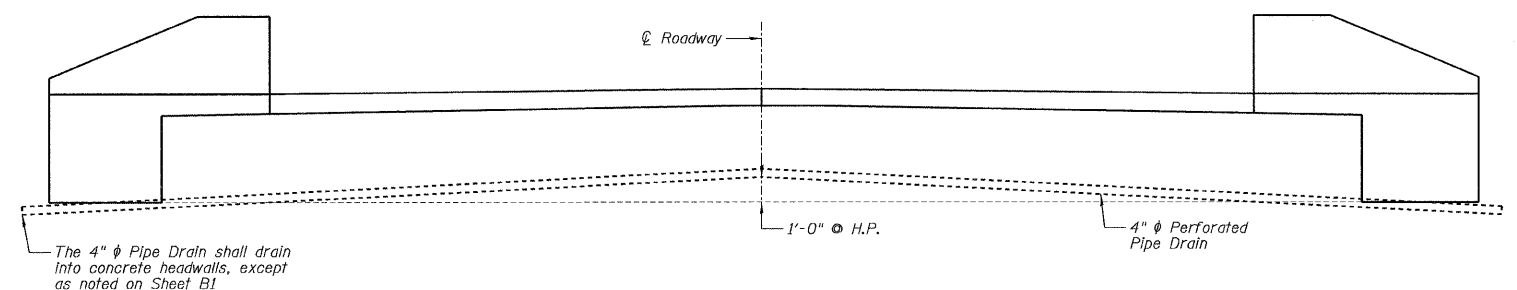
CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX



SECTION THRU WEST ABUTMENT
(Similar for East Abutment)

NOTES:

- Horizontal dimensions ϕ Rt. L's to Abutment.
- Included in the cost of Pipe Underdrains for Structures.
- All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101)

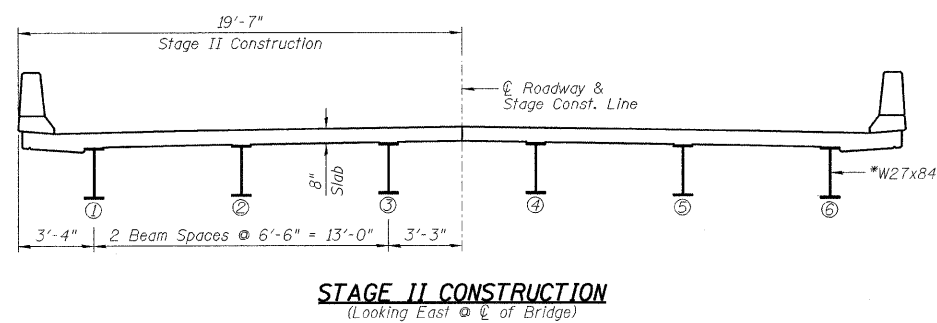
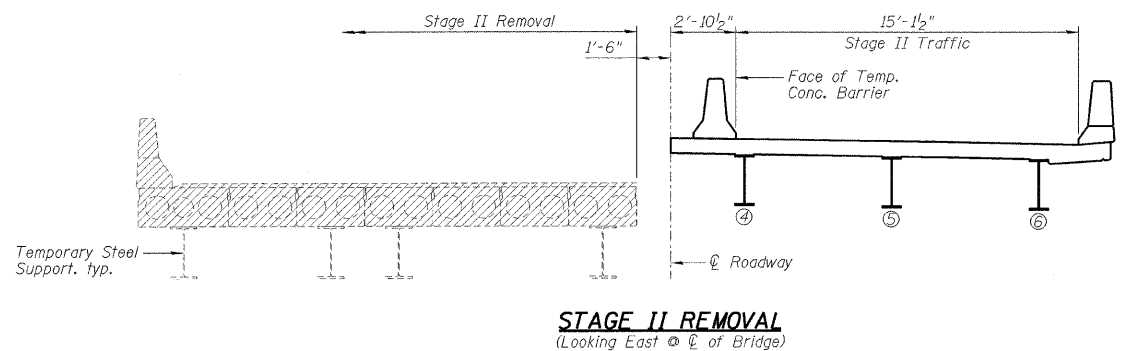
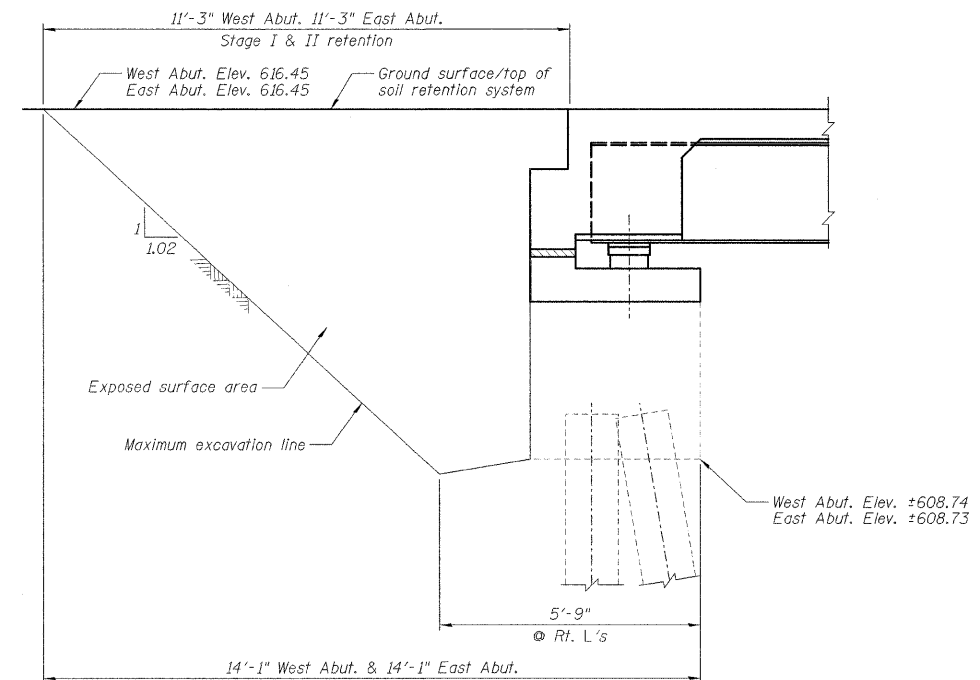
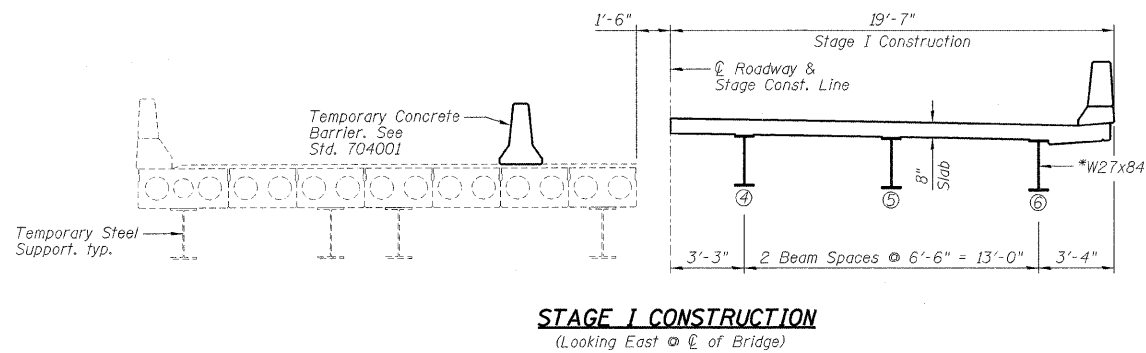
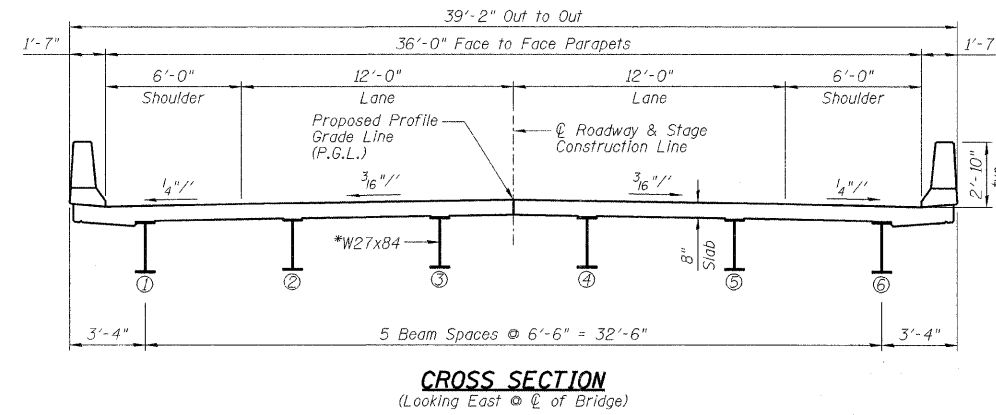
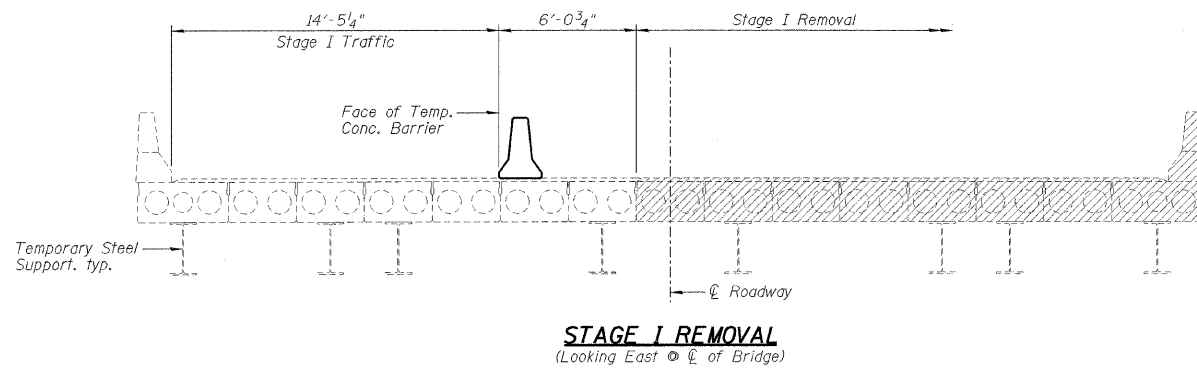


ABUTMENT ELEVATION, TYPICAL DRAIN DETAIL

GENERAL DATA
STRUCTURE NO. 053-0150

SHEET NO. B2 34 SHEETS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	18
FED. ROAD DIST. NO.			ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 66691					

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



TEMPORARY SOIL RETENTION SYSTEM

NOTES:

- 1.) A cantilever sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
- 2.) All dimensions are along roadway unless otherwise noted.

BILL OF MATERIAL

Item	Unit	Total
Temporary Soil Retention System	Sq. Ft.	98

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW
DATE	10/07/09

NOTES:

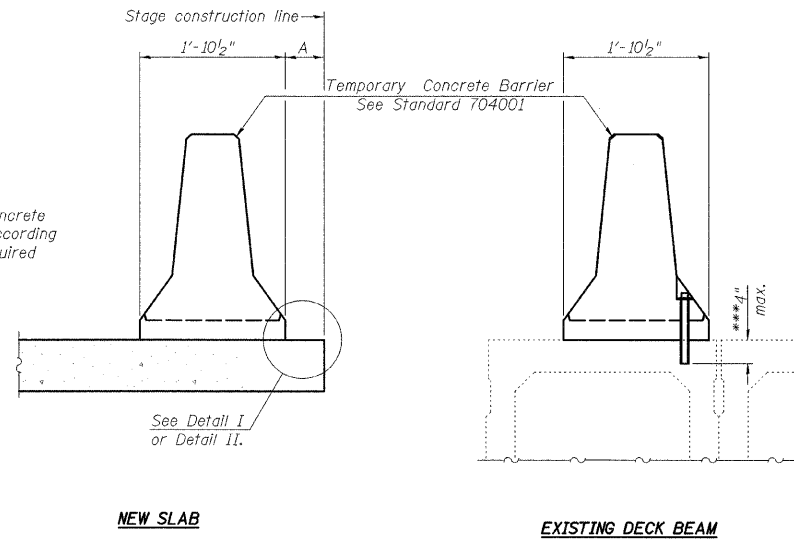
- 1.) *Composite in positive moment region only.
- 2.) Hatched area indicates Removal of Existing PPC Deck Beam Superstructure. Removal of the existing bituminous wearing surface shall be included with Removal of Existing Structures.
- 3.) See Sheet B4 for Temporary Concrete Barrier (Standard 704001). See roadway plans for quantity.

**STAGE CONSTRUCTION
STRUCTURE NO. 053-0150**

SHEET NO. B3	F.A.P. RTE. 41	SECTION 15BR-2	COUNTY LIVINGSTON	TOTAL SHEETS 04	SHEET NO. 19
34 SHEETS	CONTRACT NO. 66691		ILLINOIS FED. AID PROJECT		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

When "A" is 3'-6" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required when "A" is greater than 3'-6".



SECTIONS THRU SLAB OR DECK BEAM

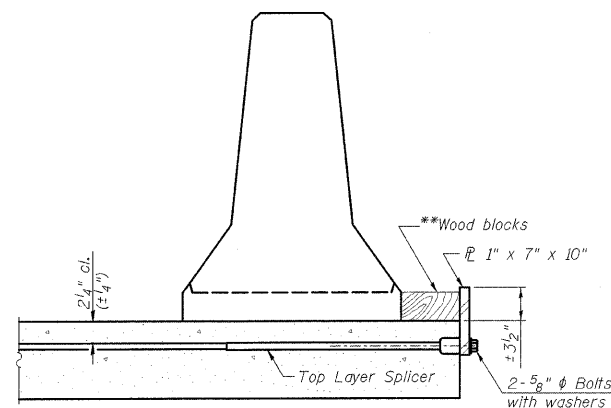
NOTES

Detail I - With Bar Splicer or Couplers:
Connect one (1) 1"x7"x10" steel \bar{L} to the top layer of couplers with 2- $\frac{5}{8}$ " ϕ bolts screwed to coupler at approximate \bar{C} of each barrier panel.

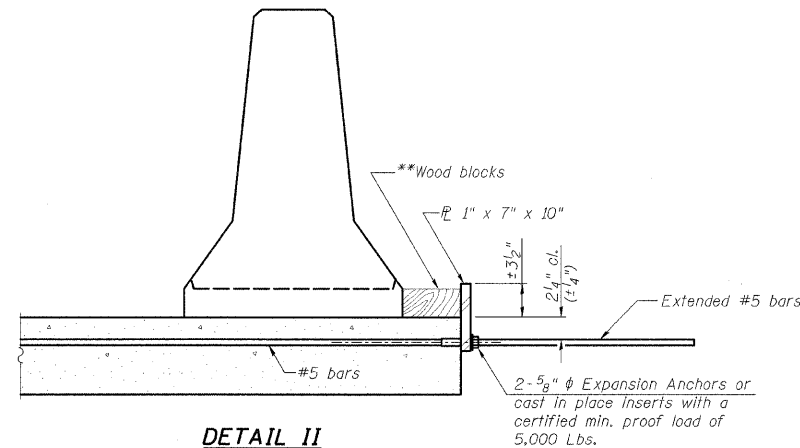
Detail II - With Extended Reinforcement Bars:
Connect one (1) 1"x7"x10" steel \bar{L} to the concrete slab or concrete wearing surface with 2- $\frac{5}{8}$ " ϕ Expansion Anchors or cast in place Inserts spaced between the top layer of reinforcement at approximate \bar{C} of each barrier panel.

Cost of anchorage is included with Temporary Concrete Barrier. The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

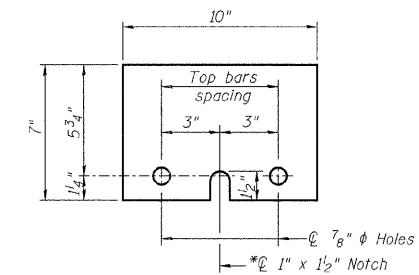
***If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



DETAIL I



DETAIL II



STEEL RETAINER \bar{L} 1" x 7" x 10"

* Required only with Detail II

**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

R-27

10-1-08

FARNSWORTH GROUP, INC.

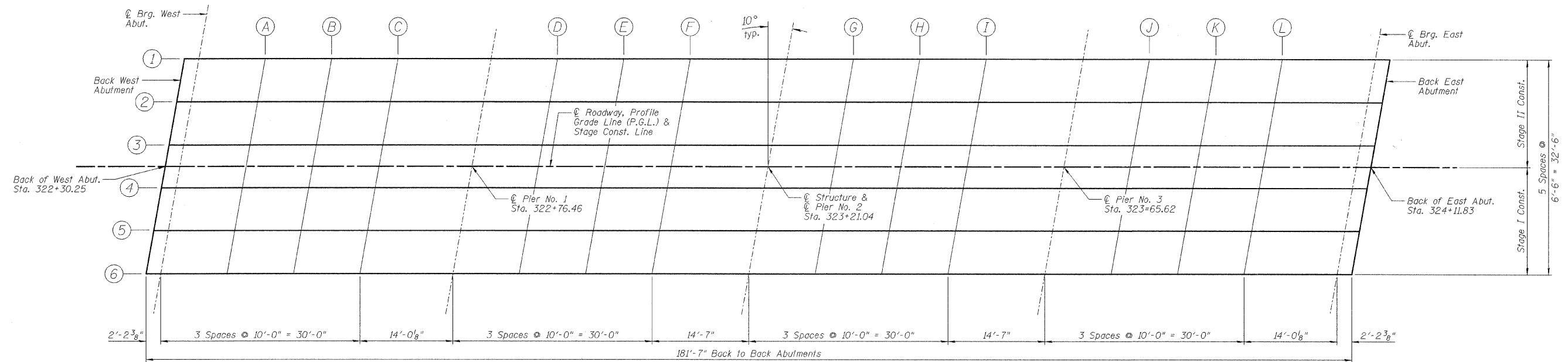
CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

TEMPORARY CONCRETE BARRIER
FOR STAGE CONSTRUCTION
STRUCTURE NO. 053-0150

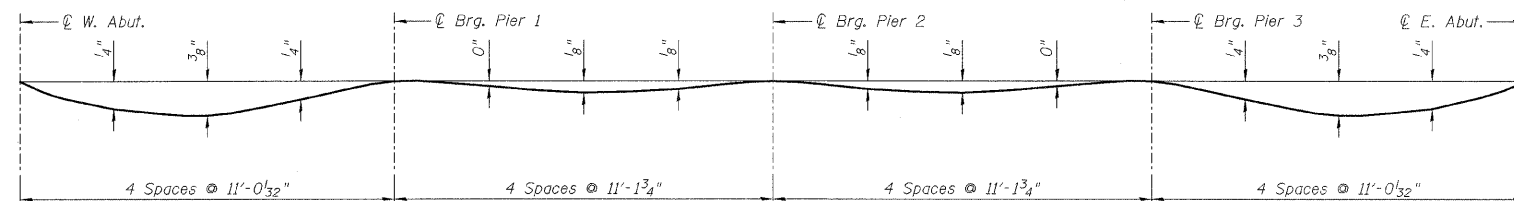
SHEET NO. B4	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	64	20
FED. ROAD DIST. NO.			ILLINOIS FED. AID PROJECT		
CONTRACT NO. 66691					

24-8168

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



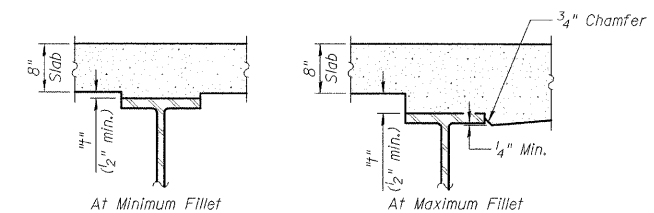
PLAN



DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

Note:
The above deflections are not for use in the field if the Engineer is working from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection".



To determine "h": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown on the plans. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on the plans, minus slab thickness, equals the fillet heights "h" above top flange of beams.

FILLET HEIGHTS

DESIGNED	SDH
CHECKED	JML
DRAWN	JMK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

TOP OF SLAB
ELEVATION LOCATIONS
STRUCTURE NO. 053-0150

SHEET NO. B5	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	64	21
FED. ROAD DIST. NO.			ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 66691					

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevation Adjusted for Dead Load Deflection
Bk. of West Abut.	322+33.12	-16.25	616.20	616.20
☉ Brg. West Abut.	322+35.31	-16.25	616.21	616.21
A	322+45.31	-16.25	616.26	616.29
B	322+55.31	-16.25	616.31	616.34
C	322+65.31	-16.25	616.35	616.38
☉ Brg. Pier 1	322+79.32	-16.25	616.40	616.40
D	322+89.32	-16.25	616.43	616.43
E	322+99.32	-16.25	616.45	616.46
F	323+09.32	-16.25	616.46	616.46
☉ Brg. Pier 2	323+23.91	-16.25	616.46	616.46
G	323+33.91	-16.25	616.46	616.46
H	323+43.91	-16.25	616.44	616.45
I	323+53.91	-16.25	616.42	616.43
☉ Brg. Pier 3	323+68.49	-16.25	616.38	616.38
J	323+78.49	-16.25	616.35	616.36
K	323+88.49	-16.25	616.30	616.33
L	323+98.49	-16.25	616.25	616.28
☉ Brg. East Abut.	324+12.50	-16.25	616.17	616.17
Bk. of East Abut.	324+14.70	-16.25	616.16	616.16

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevation Adjusted for Dead Load Deflection
Bk. of West Abut.	322+31.97	-9.75	616.31	616.31
☉ Brg. West Abut.	322+34.17	-9.75	616.33	616.33
A	322+44.17	-9.75	616.38	616.40
B	322+54.17	-9.75	616.43	616.46
C	322+64.17	-9.75	616.47	616.50
☉ Brg. Pier 1	322+78.18	-9.75	616.52	616.52
D	322+88.18	-9.75	616.55	616.55
E	322+98.18	-9.75	616.57	616.58
F	323+08.18	-9.75	616.58	616.59
☉ Brg. Pier 2	323+22.76	-9.75	616.59	616.59
G	323+32.76	-9.75	616.58	616.59
H	323+42.76	-9.75	616.57	616.58
I	323+52.76	-9.75	616.55	616.56
☉ Brg. Pier 3	323+67.34	-9.75	616.51	616.51
J	323+77.34	-9.75	616.48	616.49
K	323+87.34	-9.75	616.43	616.46
L	323+97.34	-9.75	616.38	616.41
☉ Brg. East Abut.	324+11.35	-9.75	616.30	616.30
Bk. of East Abut.	324+13.55	-9.75	616.29	616.29

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevation Adjusted for Dead Load Deflection
Bk. of West Abut.	322+30.82	-3.25	616.41	616.41
☉ Brg. West Abut.	322+33.02	-3.25	616.42	616.42
A	322+43.02	-3.25	616.48	616.50
B	322+53.02	-3.25	616.53	616.56
C	322+63.02	-3.25	616.57	616.59
☉ Brg. Pier 1	322+77.03	-3.25	616.62	616.62
D	322+87.03	-3.25	616.65	616.65
E	322+97.03	-3.25	616.67	616.68
F	323+07.03	-3.25	616.68	616.69
☉ Brg. Pier 2	323+21.61	-3.25	616.69	616.69
G	323+31.61	-3.25	616.68	616.69
H	323+41.61	-3.25	616.67	616.68
I	323+51.61	-3.25	616.65	616.66
☉ Brg. Pier 3	323+66.20	-3.25	616.62	616.62
J	323+76.20	-3.25	616.58	616.60
K	323+86.20	-3.25	616.54	616.57
L	323+96.20	-3.25	616.49	616.52
☉ Brg. East Abut.	324+10.21	-3.25	616.41	616.41
Bk. of East Abut.	324+12.41	-3.25	616.40	616.40

☉ ROADWAY, PROFILE GRADE LINE (P.G.L.) & STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevation Adjusted for Dead Load Deflection
Bk. of West Abut.	322+30.25	0.00	616.45	616.45
☉ Brg. West Abut.	322+32.45	0.00	616.47	616.47
A	322+42.45	0.00	616.53	616.55
B	322+52.45	0.00	616.58	616.61
C	322+62.45	0.00	616.62	616.64
☉ Brg. Pier 1	322+76.46	0.00	616.67	616.67
D	322+86.46	0.00	616.70	616.70
E	322+96.46	0.00	616.72	616.73
F	323+06.46	0.00	616.73	616.74
☉ Brg. Pier 2	323+21.04	0.00	616.74	616.74
G	323+31.04	0.00	616.73	616.74
H	323+41.04	0.00	616.72	616.73
I	323+51.04	0.00	616.71	616.71
☉ Brg. Pier 3	323+65.62	0.00	616.67	616.67
J	323+75.62	0.00	616.63	616.65
K	323+85.62	0.00	616.59	616.62
L	323+95.62	0.00	616.55	616.57
☉ Brg. East Abut.	324+09.63	0.00	616.47	616.47
Bk. of East Abut.	324+11.83	0.00	616.45	616.45

DESIGNED	SDH
CHECKED	JML
DRAWN	JMK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

**TOP OF SLAB ELEVATIONS
STRUCTURE NO. 053-0150**

SHEET NO. B6 34 SHEETS	F.A.P. RTE. 41	SECTION 15BR-2	COUNTY LIVINGSTON	TOTAL SHEETS 64	SHEET NO. 22
	CONTRACT NO. 66691			FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevation Adjusted for Dead Load Deflection
Bk. of West Abut.	322+29.68	3.25	616.40	616.40
☉ Brg. West Abut.	322+31.88	3.25	616.41	616.41
A	322+41.88	3.25	616.47	616.49
B	322+51.88	3.25	616.52	616.55
C	322+61.88	3.25	616.57	616.59
☉ Brg. Pier 1	322+75.89	3.25	616.62	616.62
D	322+85.89	3.25	616.64	616.65
E	322+95.89	3.25	616.67	616.68
F	322+05.89	3.25	616.68	616.69
☉ Brg. Pier 2	323+20.47	3.25	616.69	616.69
G	323+30.47	3.25	616.68	616.69
H	323+40.47	3.25	616.67	616.68
I	323+50.47	3.25	616.66	616.66
☉ Brg. Pier 3	323+65.05	3.25	616.62	616.62
J	323+75.05	3.25	616.59	616.60
K	323+85.05	3.25	616.55	616.57
L	323+95.05	3.25	616.50	616.53
☉ Brg. East Abut.	324+09.06	3.25	616.42	616.42
Bk. of East Abut.	324+11.26	3.25	616.41	616.41

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevation Adjusted for Dead Load Deflection
Bk. of West Abut.	322+28.53	9.75	616.29	616.29
☉ Brg. West Abut.	322+30.73	9.75	616.30	616.30
A	322+40.73	9.75	616.36	616.39
B	322+50.73	9.75	616.42	616.45
C	322+60.73	9.75	616.46	616.48
☉ Brg. Pier 1	322+74.74	9.75	616.51	616.51
D	322+84.74	9.75	616.54	616.54
E	322+94.74	9.75	616.56	616.57
F	323+04.74	9.75	616.58	616.58
☉ Brg. Pier 2	323+19.32	9.75	616.59	616.59
G	323+29.32	9.75	616.58	616.59
H	323+39.32	9.75	616.57	616.58
I	323+49.32	9.75	616.56	616.56
☉ Brg. Pier 3	323+63.91	9.75	616.52	616.52
J	323+73.91	9.75	616.49	616.50
K	323+83.91	9.75	616.45	616.48
L	323+93.91	9.75	616.40	616.43
☉ Brg. East Abut.	324+07.92	9.75	616.33	616.33
Bk. of East Abut.	324+10.11	9.75	616.31	616.31

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevation Adjusted for Dead Load Deflection
Bk. of West Abut.	322+27.39	16.25	616.16	616.16
☉ Brg. West Abut.	322+29.58	16.25	616.17	616.17
A	322+39.58	16.25	616.23	616.26
B	322+49.58	16.25	616.29	616.32
C	322+59.58	16.25	616.33	616.35
☉ Brg. Pier 1	322+73.59	16.25	616.38	616.38
D	322+83.59	16.25	616.41	616.42
E	322+93.59	16.25	616.44	616.45
F	323+03.59	16.25	616.45	616.46
☉ Brg. Pier 2	323+18.18	16.25	616.46	616.46
G	323+28.18	16.25	616.46	616.47
H	323+38.18	16.25	616.45	616.46
I	323+48.18	16.25	616.44	616.44
☉ Brg. Pier 3	323+62.76	16.25	616.40	616.40
J	323+72.76	16.25	616.37	616.38
K	323+82.76	16.25	616.33	616.36
L	323+92.76	16.25	616.28	616.31
☉ Brg. East Abut.	324+06.77	16.25	616.21	616.21
Bk. of East Abut.	324+08.97	16.25	616.20	616.20

DESIGNED SDH
CHECKED JML
DRAWN JWK/DJM
CHECKED MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

**TOP OF SLAB ELEVATIONS
STRUCTURE NO. 053-0150**

SHEET NO. B7	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	04	23
			CONTRACT NO. 66691		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH CURB LINE/NORTH FACE OF PARAPET

Location	Station	Offset	Theoretical Grade Elevations
End West Appr. Slab	322+04.35	-18.42	615.95
A	322+14.35	-18.42	616.03
B	322+24.27	-18.00	616.10
West End of Deck	322+34.27	-18.00	616.17

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
End West Appr. Slab	322+03.22	-12.00	616.07
A	322+13.22	-12.00	616.15
B	322+23.22	-12.00	616.22
West End of Deck	322+33.22	-12.00	616.28

☉ ROADWAY, STAGE CONSTRUCTION LINE & PROFILE GRADE LINE

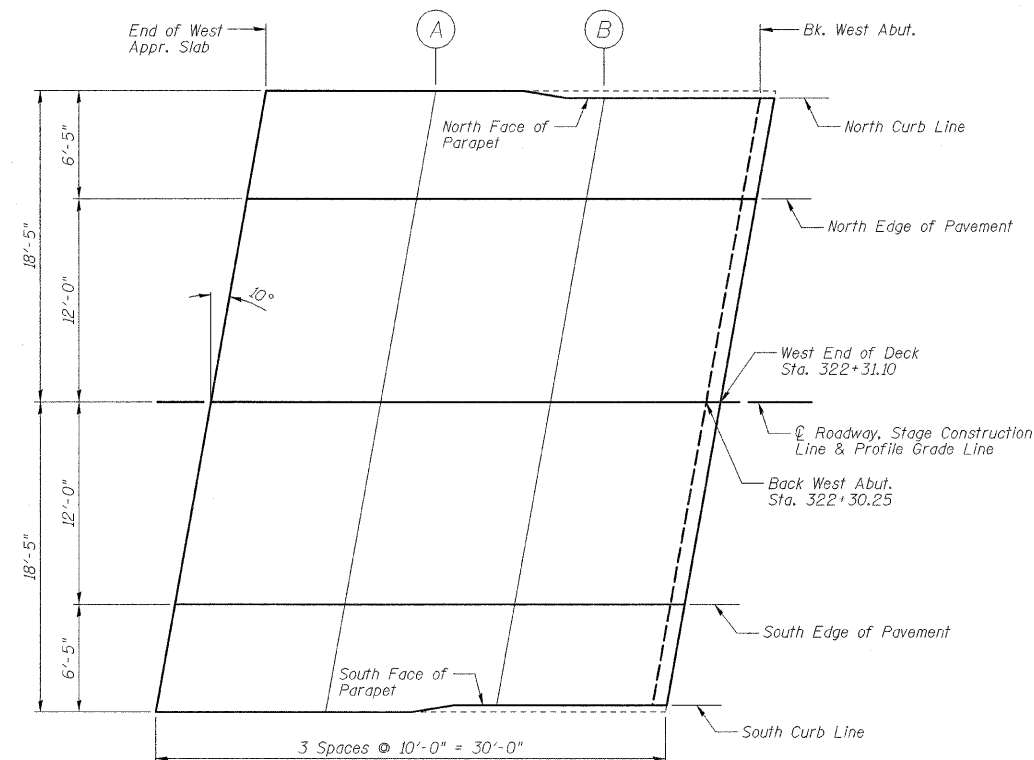
Location	Station	Offset	Theoretical Grade Elevations
End West Appr. Slab	322+01.10	0.00	616.24
A	322+11.10	0.00	616.32
B	322+21.10	0.00	616.39
West End of Deck	322+31.10	0.00	616.46

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
End West Appr. Slab	321+98.98	12.00	616.04
A	322+08.98	12.00	616.12
B	322+18.98	12.00	616.19
West End of Deck	322+28.98	12.00	616.26

SOUTH CURB LINE/SOUTH FACE OF PARAPET

Location	Station	Offset	Theoretical Grade Elevations
End West Appr. Slab	321+97.85	18.42	615.89
A	322+07.85	18.42	615.98
B	322+17.93	18.00	616.06
West End of Deck	322+27.93	18.00	616.13



WEST APPROACH SLAB PLAN

TOP OF WEST APPROACH
SLAB ELEVATIONS
STRUCTURE NO. 053-0150

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

SHEET NO. B8	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	04	24
FED. ROAD DIST. NO.			ILLINOIS FED. AID PROJECT		
CONTRACT NO. 66691					

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH CURB LINE/NORTH FACE OF PARAPET

Location	Station	Offset	Theoretical Grade Elevations
East End of Deck	324+14.15	-18.00	616.13
A	324+24.15	-18.00	616.06
B	324+34.23	-18.42	615.98
End East Appr. Slab	324+44.23	-18.42	615.89

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
East End of Deck	324+13.10	-12.00	616.26
A	324+23.10	-12.00	616.19
B	324+33.10	-12.00	616.12
End East Appr. Slab	324+43.10	-12.00	616.04

☉ ROADWAY, STAGE CONSTRUCTION
LINE & PROFILE GRADE LINE

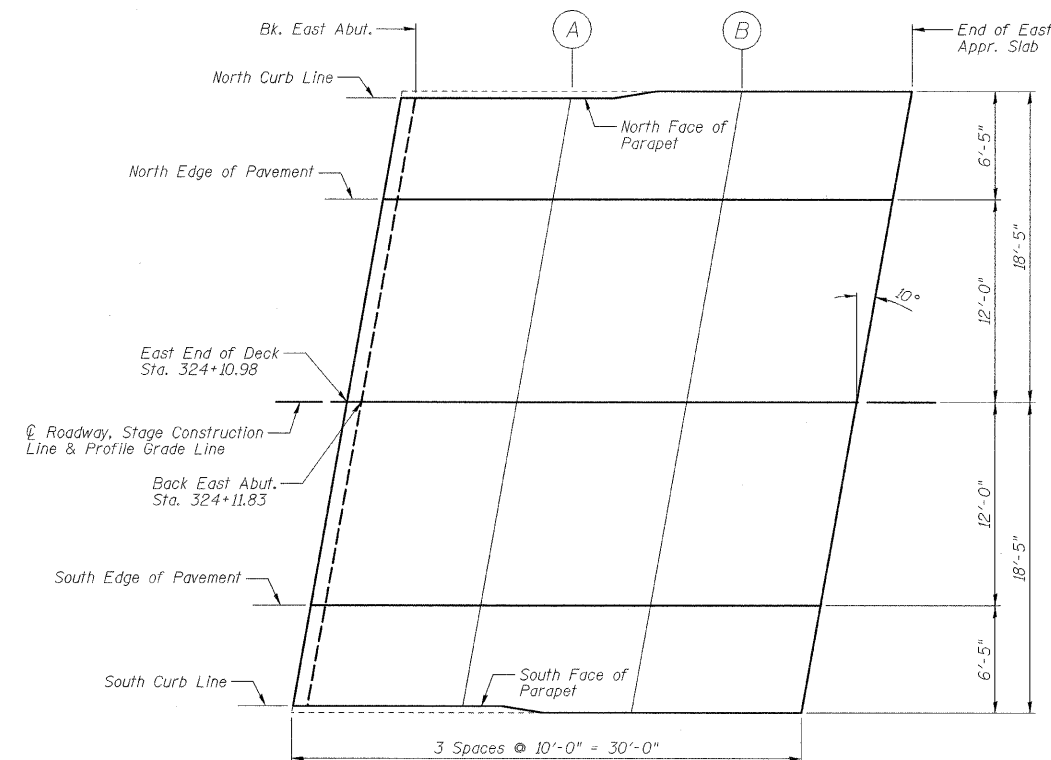
Location	Station	Offset	Theoretical Grade Elevations
East End of Deck	324+10.98	0.00	616.46
A	324+20.98	0.00	616.39
B	324+30.98	0.00	616.32
End East Appr. Slab	324+40.98	0.00	616.24

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
East End of Deck	324+08.86	12.00	616.28
A	324+18.86	12.00	616.22
B	324+28.86	12.00	616.15
End East Appr. Slab	324+38.86	12.00	616.07

SOUTH CURB LINE/SOUTH FACE OF PARAPET

Location	Station	Offset	Theoretical Grade Elevations
East End of Deck	324+07.81	18.00	616.17
A	324+17.81	18.00	616.10
B	324+27.73	18.42	616.02
End East Appr. Slab	324+37.73	18.42	615.95



EAST APPROACH SLAB PLAN



**TOP OF EAST APPROACH
SLAB ELEVATIONS
STRUCTURE NO. 053-0150**

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

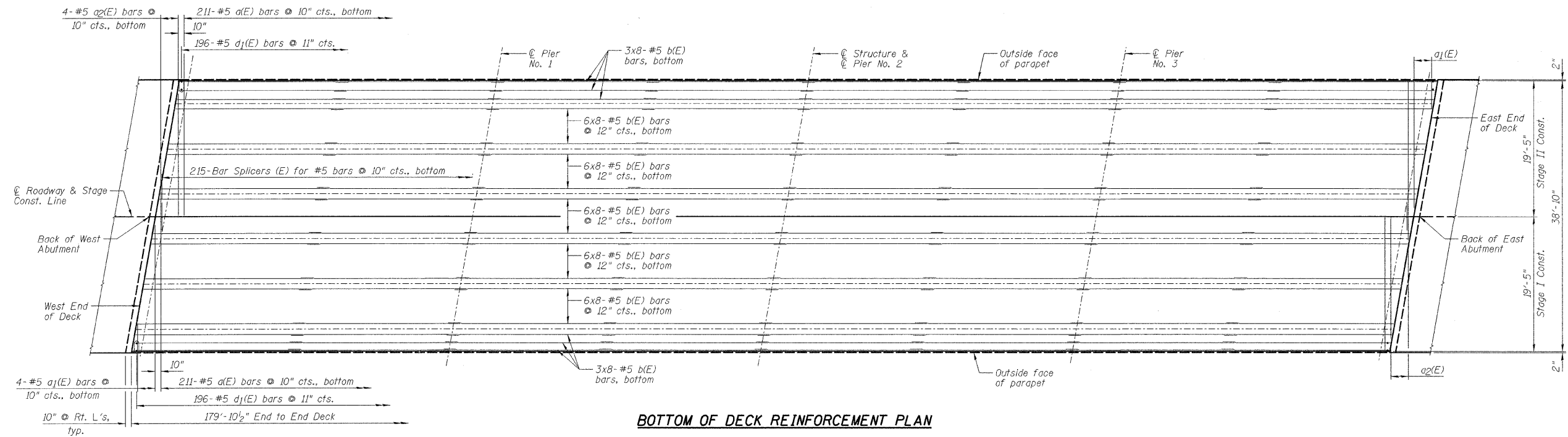
DATE 10/07/09

FARNSWORTH GROUP, INC.

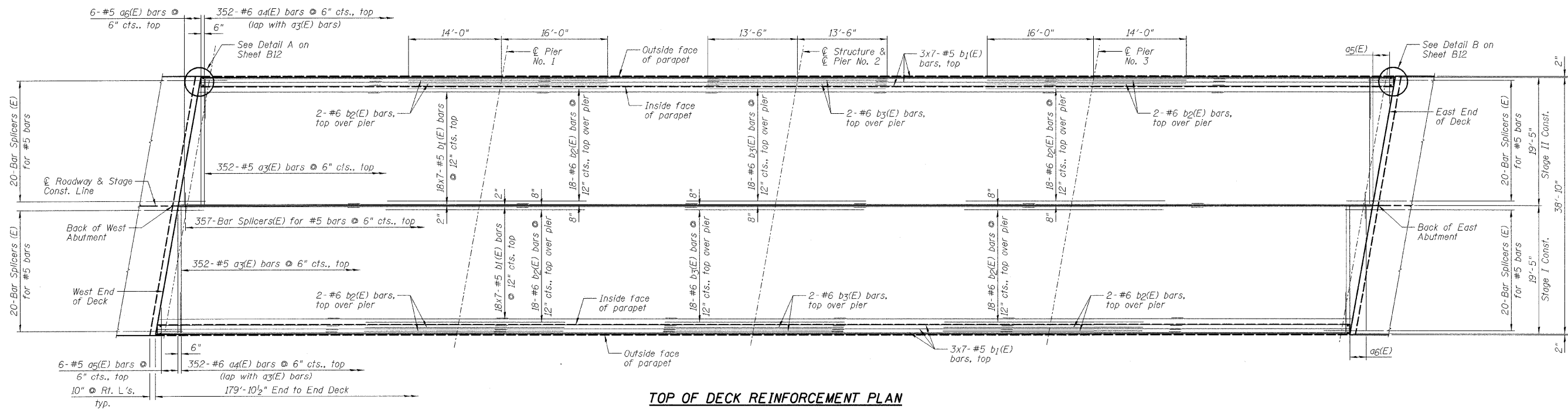
CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

SHEET NO. B9 34 SHEETS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	04	25
FED. ROAD DIST. NO.			ILLINOIS FED. AID PROJECT		
CONTRACT NO. 66691					

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



BOTTOM OF DECK REINFORCEMENT PLAN



TOP OF DECK REINFORCEMENT PLAN

NOTES:

- 1.) See Sheet B11 for Deck Cross Section.
- 2.) See Sheet B12 for Superstructure Details and Bill of Material.
- 3.) Order a1(E), a2(E), a3(E) and a4(E) bars full length. Cut according to Bar Cutting Diagram on Sheet B12. Use remainder of bars on opposite corner of deck.
- 4.) Bars indicated thus 6x8-#5 etc. indicates 6 lines of bars with 8 lengths per line.
- 5.) See Sheet B33 for Bar Splicer Details.

BAR LAP
#5 - 1'-8"

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

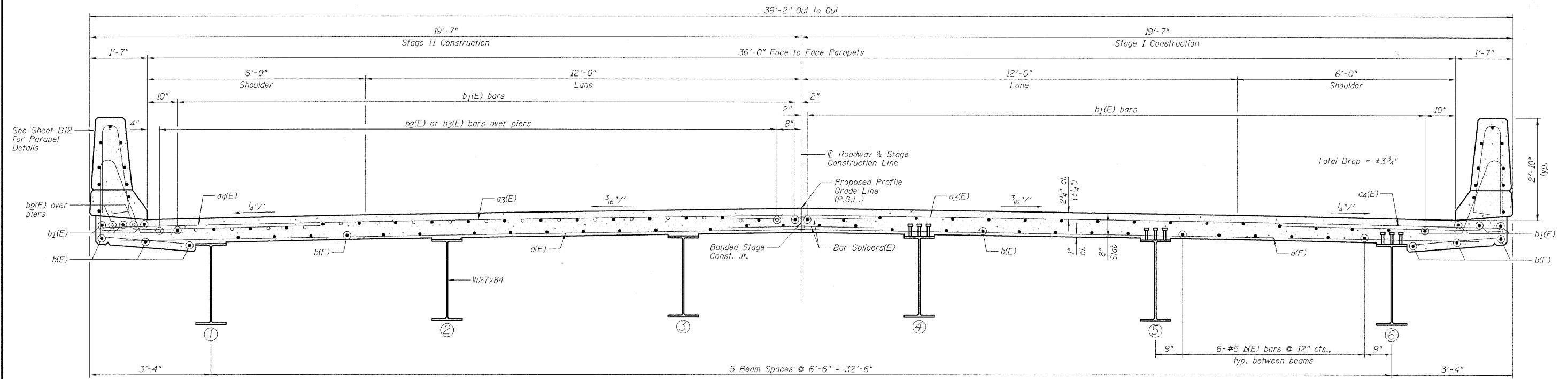
FARNSWORTH GROUP, INC.

CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

**SUPERSTRUCTURE DECK
STRUCTURE NO. 053-0150**

SHEET NO. B10	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	26
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



NEAR PIER

NEAR MIDSPAN

CROSS SECTION
(Looking East @ ϕ of Bridge)

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

- NOTES:**
- 1.) See Sheet B12 for Superstructure Details and Bill of Material.
 - 2.) See Sheet B33 for Bar Splicer Details.

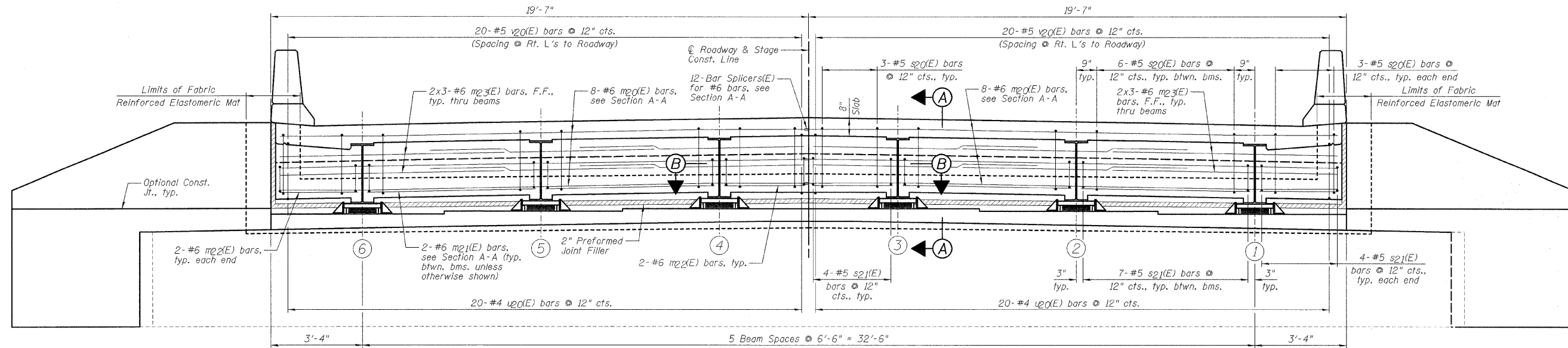
SUPERSTRUCTURE CROSS SECTION
STRUCTURE NO. 053-0150

SHEET NO. B11 34 SHEETS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	27
FED. ROAD DIST. NO.			ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 66691					

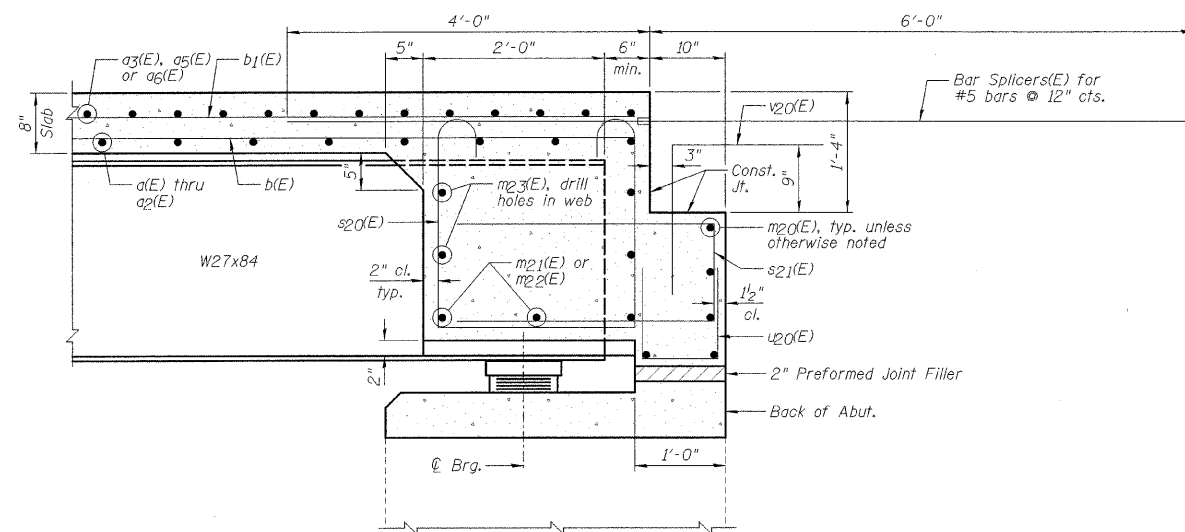
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STAGE I

STAGE II

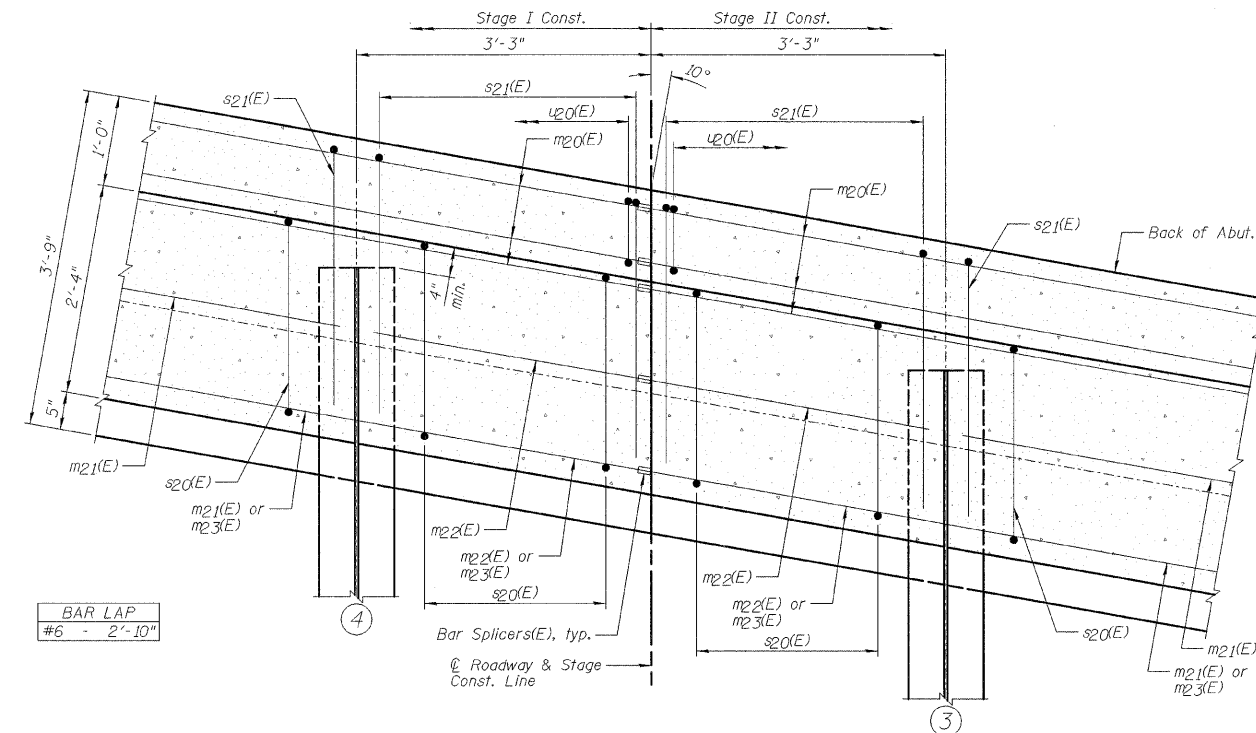


ELEVATION OF DIAPHRAGM AT WEST ABUTMENT
(Looking West)



SECTION A-A

Dimensions @ Rt. L's except as noted.



SECTION B-B

NOTES:

- 1.) See Sheet B12 for Superstructure Details and Bill of Material.
- 2.) See Sheet B19 for abutment bearing details.
- 3.) F.F. denotes Front Face.
- 4.) See Sheet B33 for Bar Splicer Details.
- 5.) Bars indicated thus 2x3-#6 etc. indicates 2 lines of bars with 3 lengths per line.

WEST ABUTMENT
DIAPHRAGM DETAILS
STRUCTURE NO. 053-0150

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

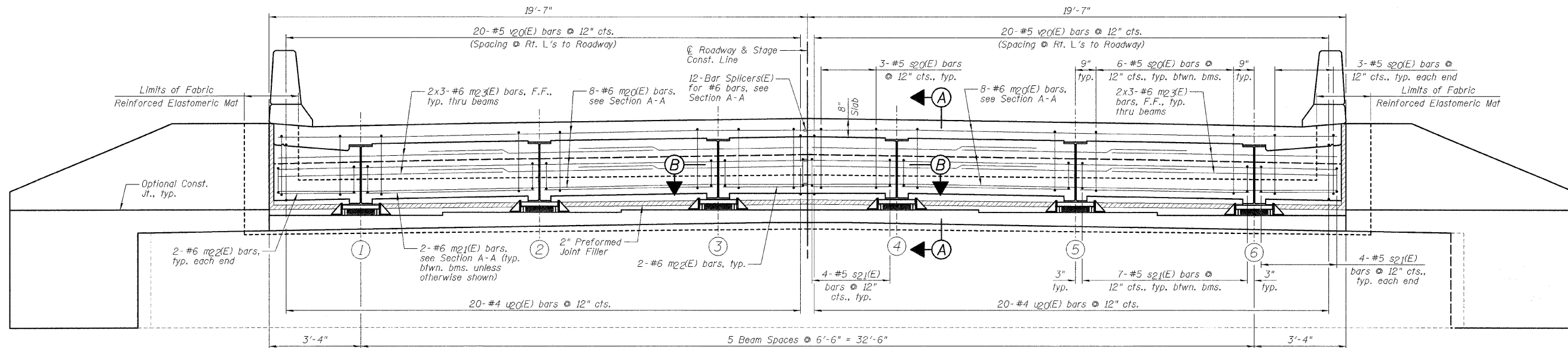
CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

SHEET NO. B13	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	29
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			

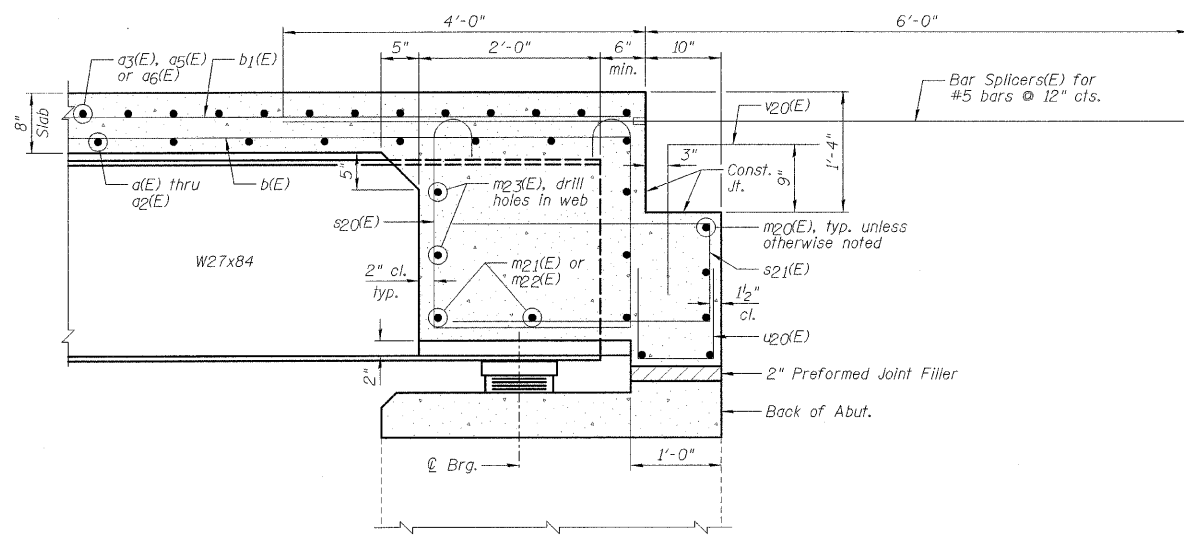
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STAGE II

STAGE I

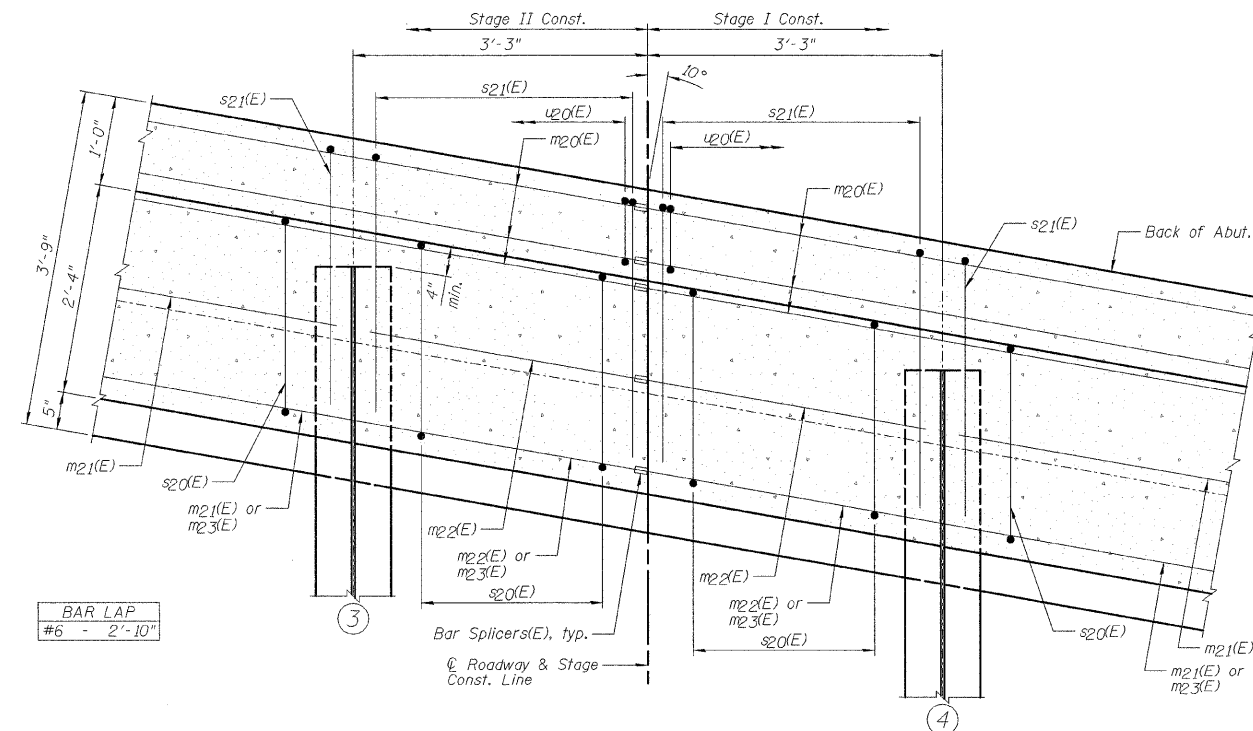


ELEVATION OF DIAPHRAGM AT EAST ABUTMENT
(Looking East)



SECTION A-A

Dimensions © Rt. L's except as noted.



SECTION B-B

NOTES:

- 1.) See Sheet B12 for Superstructure Details and Bill of Material.
- 2.) See Sheet B19 for abutment bearing details.
- 3.) F.F. denotes Front Face.
- 4.) See Sheet B33 for Bar Splicer Details.
- 5.) Bars Indicated thus 2x3-#6 etc. indicates 2 lines of bars with 3 lengths per line.

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

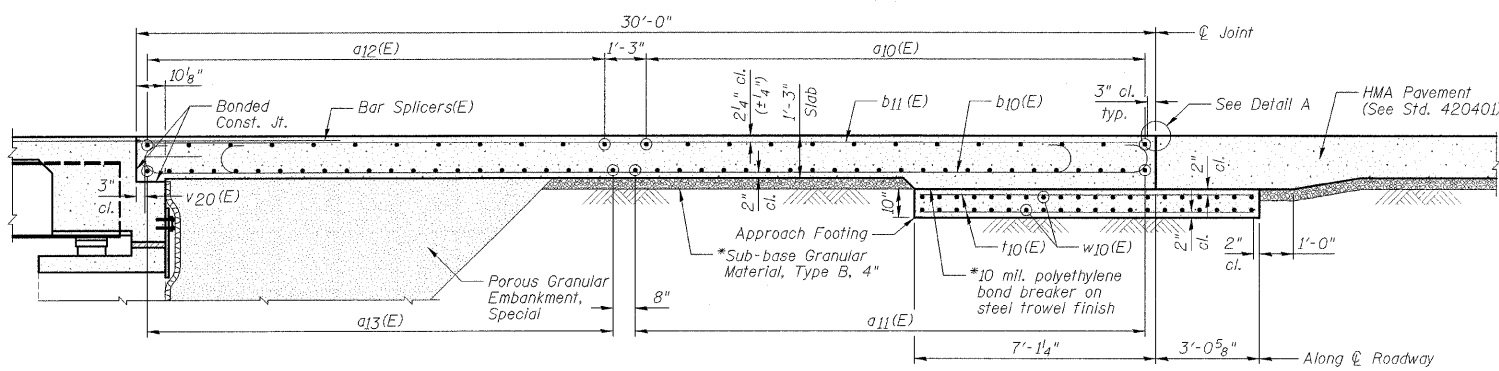
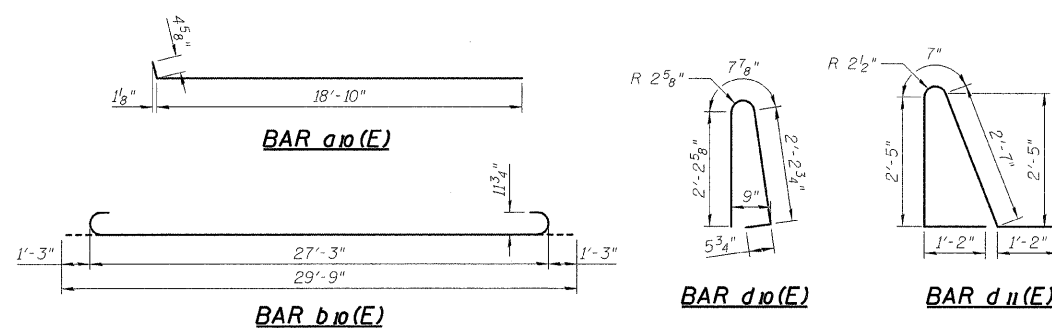
EAST ABUTMENT
DIAPHRAGM DETAILS
STRUCTURE NO. 053-0150

SHEET NO. B14 34 SHEETS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	30
FED. ROAD DIST. NO.			ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 66691					

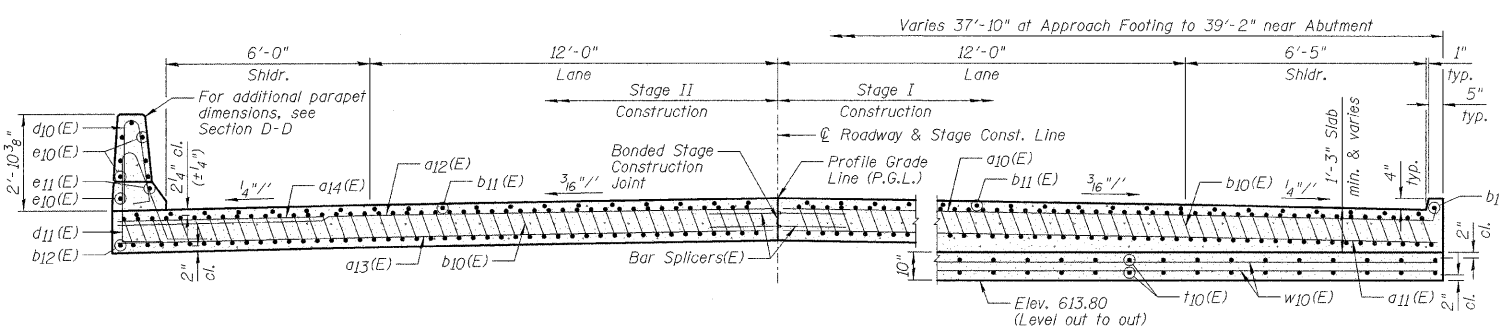
**WEST APPROACH SLAB
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a10(E)	26	#4	19'-3"	
a11(E)	48	#5	18'-10"	
a12(E)	24	#4	19'-6"	
a13(E)	44	#5	19'-6"	
a14(E)	24	#6	6'-0"	
b10(E)	90	#9	29'-9"	
b11(E)	38	#4	29'-8"	
b12(E)	2	#4	14'-8"	
b13(E)	2	#4	14'-9"	
d10(E)	34	#5	5'-7"	
d11(E)	34	#5	7'-11"	
e10(E)	16	#4	14'-8"	
e11(E)	2	#8	14'-8"	
i10(E)	76	#4	9'-10"	
w10(E)	80	#5	18'-10"	
Item	Unit	Quantity		
Concrete Superstructure	Cu. Yd.	58.5		
Bridge Deck Grooving	Sq. Yd.	114		
Protective Coat	Sq. Yd.	137		
Concrete Structures	Cu. Yd.	12.1		
Reinforcement Bars, Epoxy Coated	Pound	15,380		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



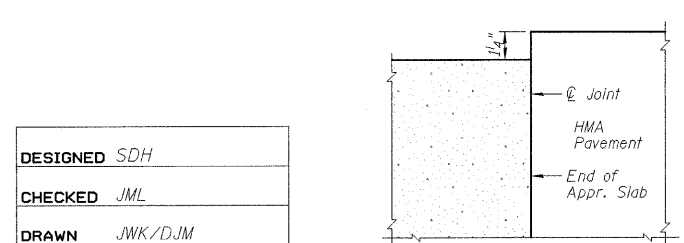
SECTION A-A



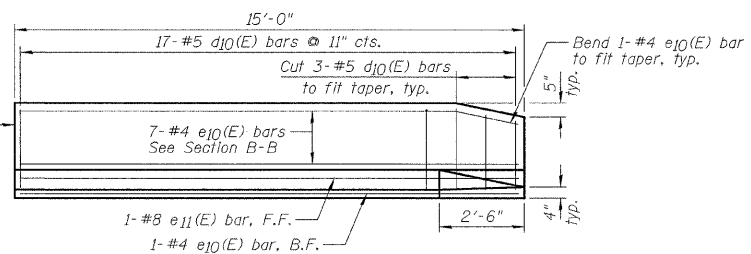
NEAR ABUTMENT

AT APPROACH FOOTING

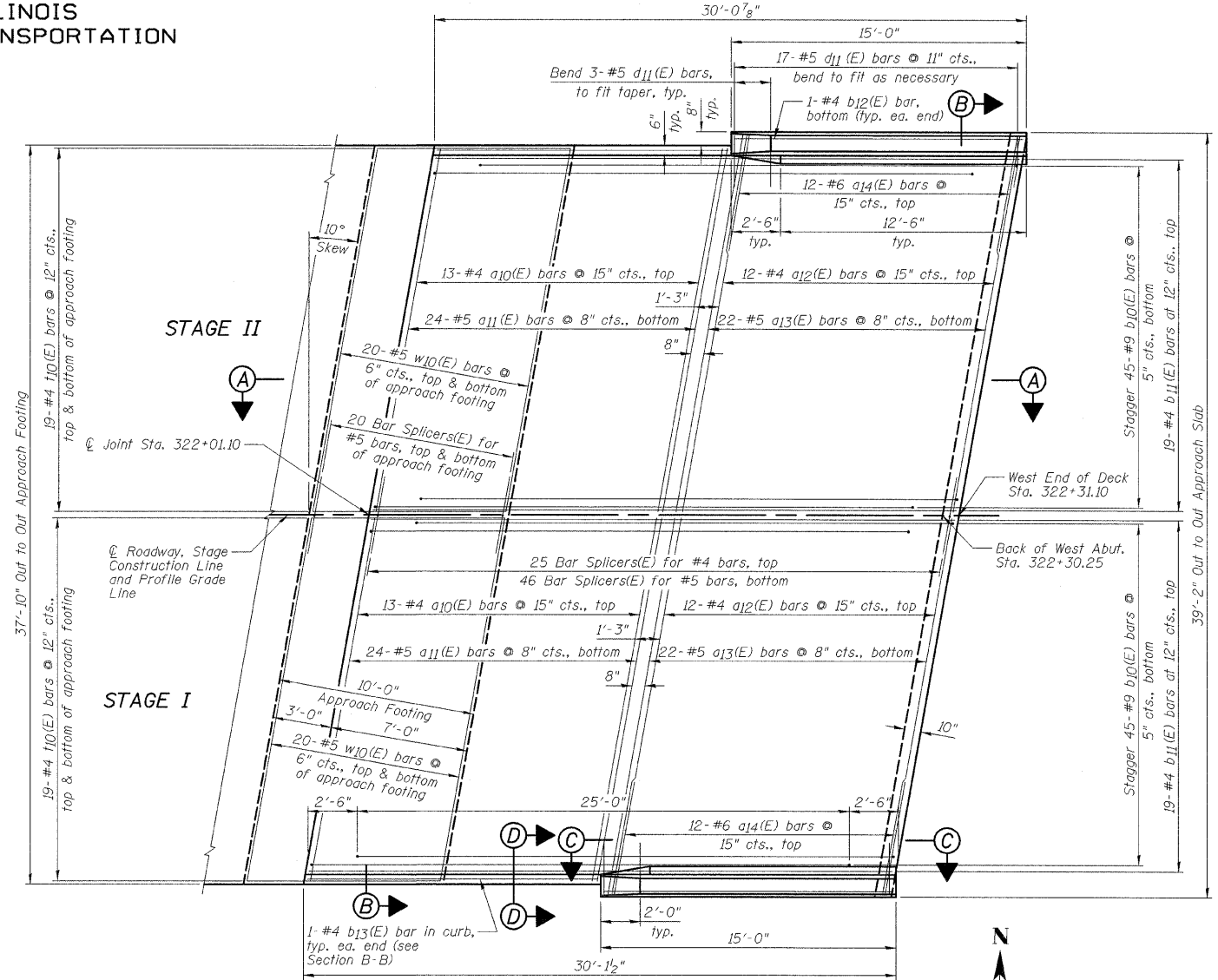
SECTION B-B



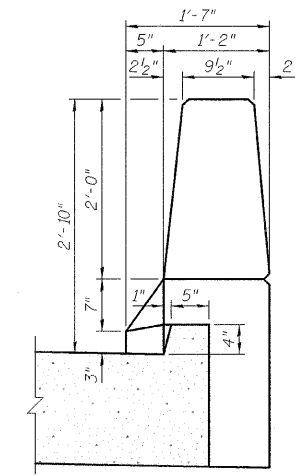
DETAIL A - FLEXIBLE PAVEMENT



SECTION C-C



WEST APPROACH PLAN



SECTION D-D

NOTES:

- a10(E) thru a14(E) bar spacings measured perpendicular to \perp Roadway.
- Tilt #9 b10(E) bars as required to maintain clearance.
- Approach Slab and parapet concrete shall be paid for as Concrete Superstructure.
- Approach Footing concrete shall be paid for as Concrete Structures.
- Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
- For v20(E) bar details, see Sheet B13.
- The Approach Footing maximum applied service bearing pressure (Q_{max}) = 2.0 ksf.
- See Sheet B33 for Bar Splicer Details.
- Cost of excavation for Approach Footing included with Concrete Structures.
- For Porous Granular Embankment, Special and drainage treatment details, see Sheet B2.
- *Cost included with Concrete Superstructure.

**WEST BRIDGE
APPROACH SLAB DETAILS
STRUCTURE NO. 053-0150**

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

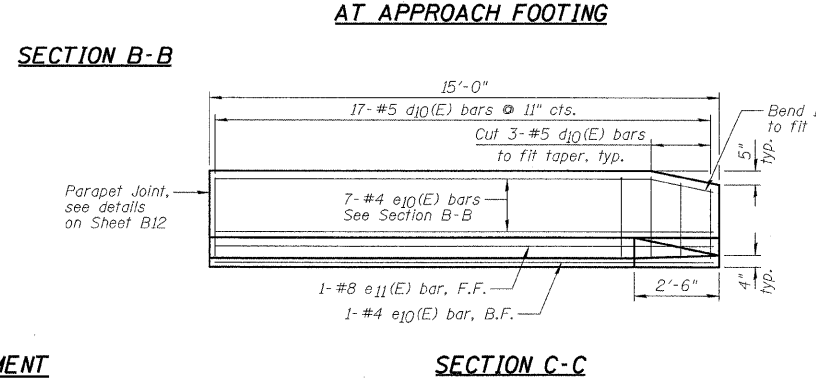
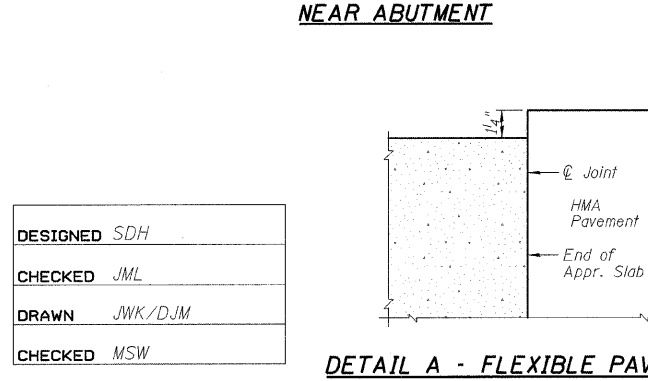
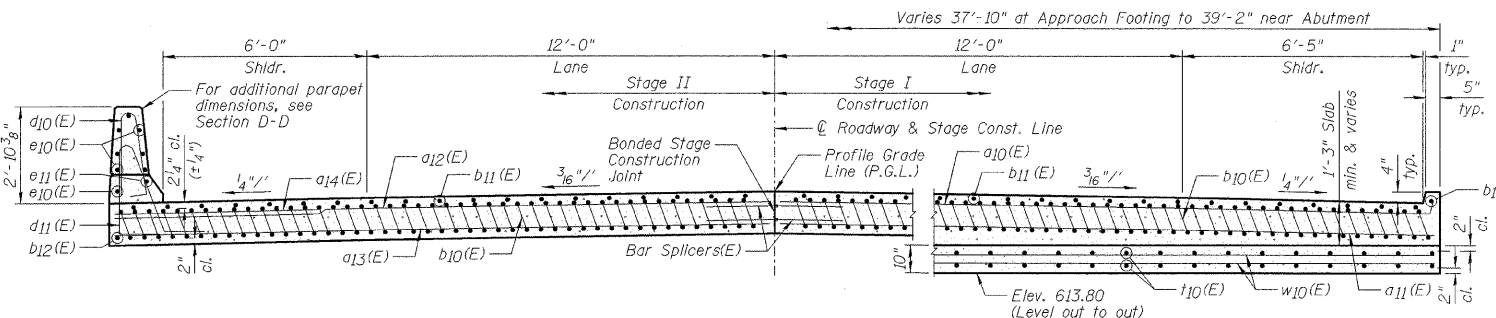
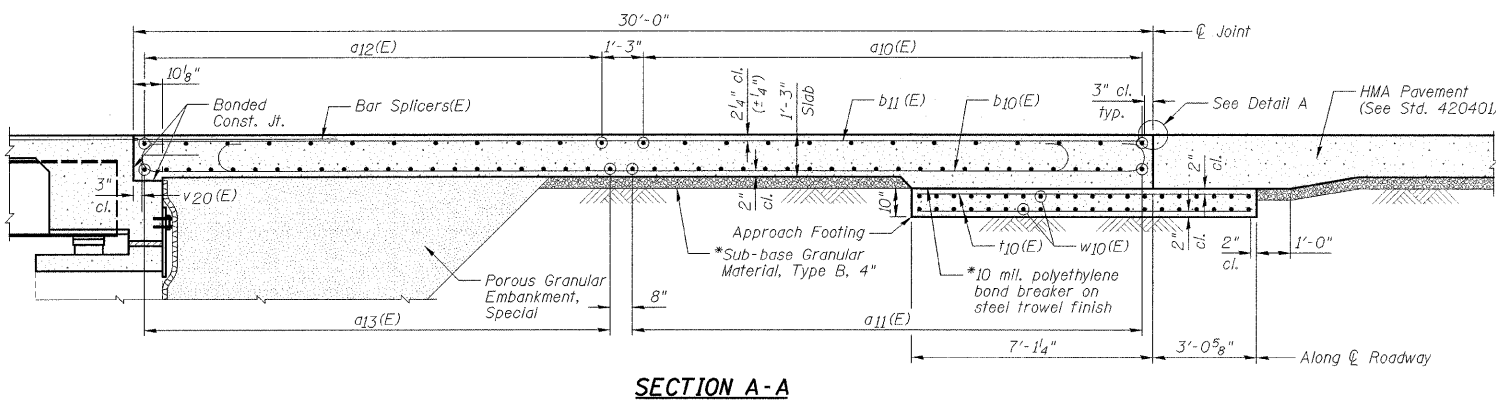
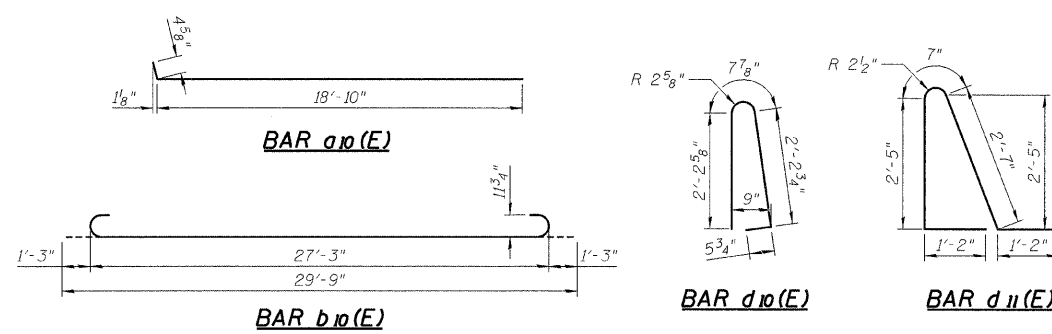
DATE 10/07/09

SHEET NO. B15	F.A.P. RTE. 41	SECTION 15BR-2	COUNTY LIVINGSTON	TOTAL SHEETS 04	SHEET NO. 31
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			

**EAST APPROACH SLAB
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a10(E)	26	#4	19'-3"	
a11(E)	48	#5	18'-10"	
a12(E)	24	#4	19'-6"	
a13(E)	44	#5	19'-6"	
a14(E)	24	#6	6'-0"	
b10(E)	90	#9	29'-9"	
b11(E)	38	#4	29'-8"	
b12(E)	2	#4	14'-8"	
b13(E)	2	#4	14'-9"	
d10(E)	34	#5	5'-7"	
d11(E)	34	#5	7'-11"	
e10(E)	16	#4	14'-8"	
e11(E)	2	#8	14'-8"	
f10(E)	76	#4	9'-10"	
w10(E)	80	#5	18'-10"	
Item	Unit	Quantity		
Concrete Superstructure	Cu. Yd.	58.5		
Bridge Deck Grooving	Sq. Yd.	114		
Protective Coat	Sq. Yd.	137		
Concrete Structures	Cu. Yd.	12.1		
Reinforcement Bars, Epoxy Coated	Pound	15,380		

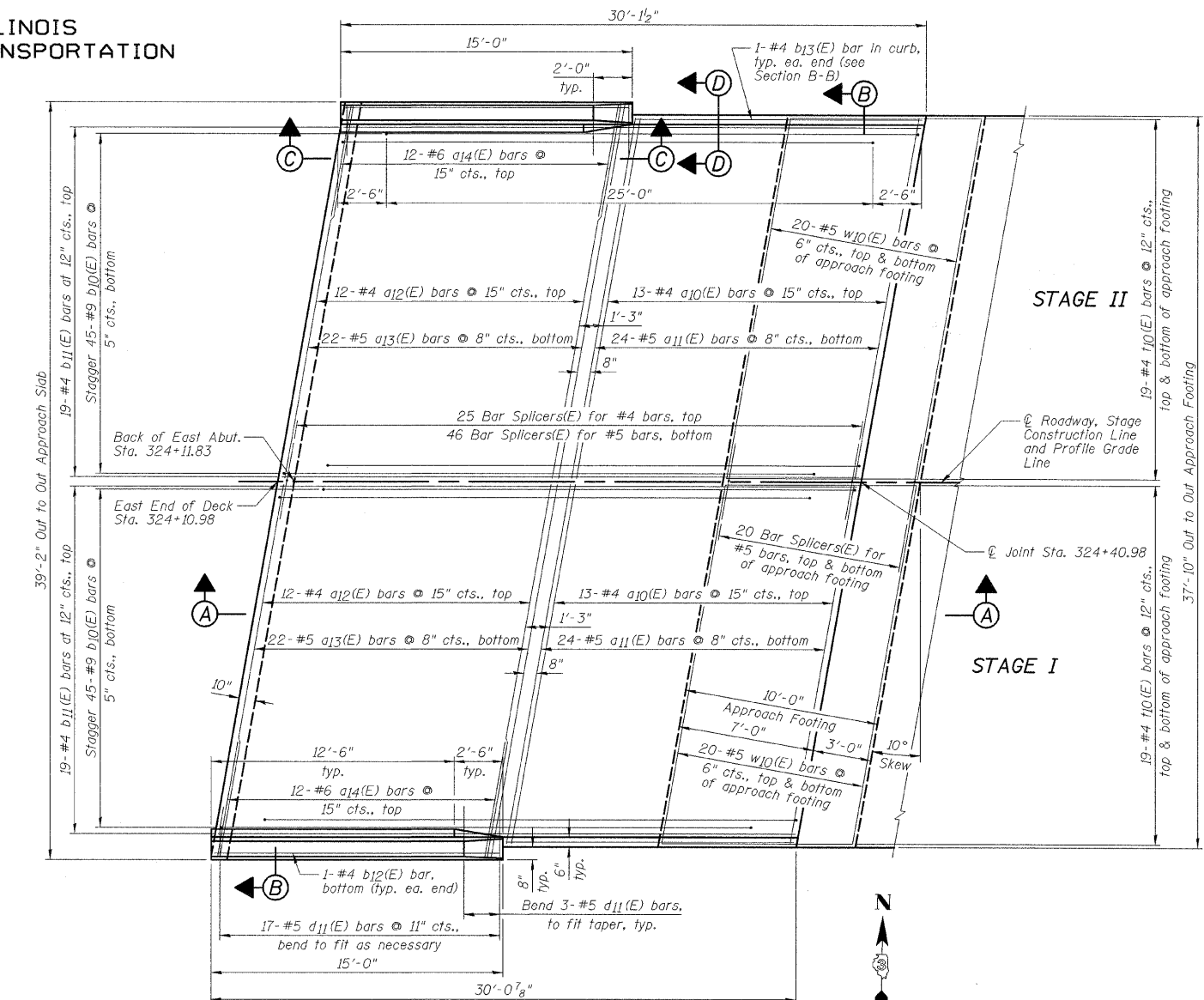
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



DESIGNED	SDH
CHECKED	JML
DRAWN	JMK/DJM
CHECKED	MSW
DATE	10/07/09

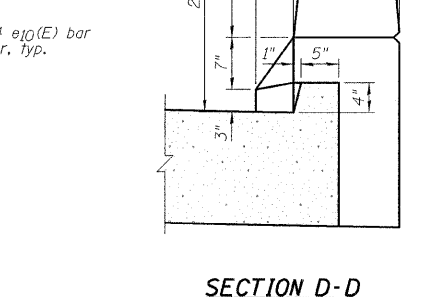
FARNSWORTH GROUP, INC.

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EAST APPROACH PLAN

- NOTES:**
- a10(E) thru a14(E) bar spacings measured perpendicular to $\text{\textcircled{C}}$ Roadway.
 - Tilt #9 b10(E) bars as required to maintain clearance.
 - Approach Slab and parapet concrete shall be paid for as Concrete Superstructure.
 - Approach Footing concrete shall be paid for as Concrete Structures.
 - Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
 - For v20(E) bar details, see Sheet B14.
 - The Approach Footing maximum applied service bearing pressure ($\text{\textcircled{Q}}$ max) = 2.0 ksf.
 - See Sheet B33 for Bar Splicer Details.
 - Cost of excavation for Approach Footing included with Concrete Structures.
 - For Porous Granular Embankment, Special and drainage treatment details, see Sheet B2.
 - *Cost included with Concrete Superstructure.

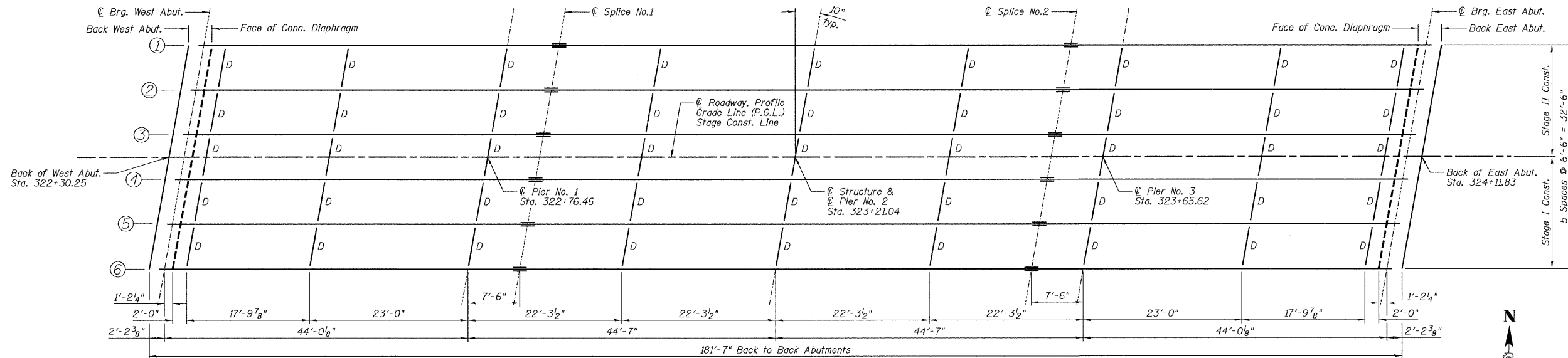


SECTION D-D

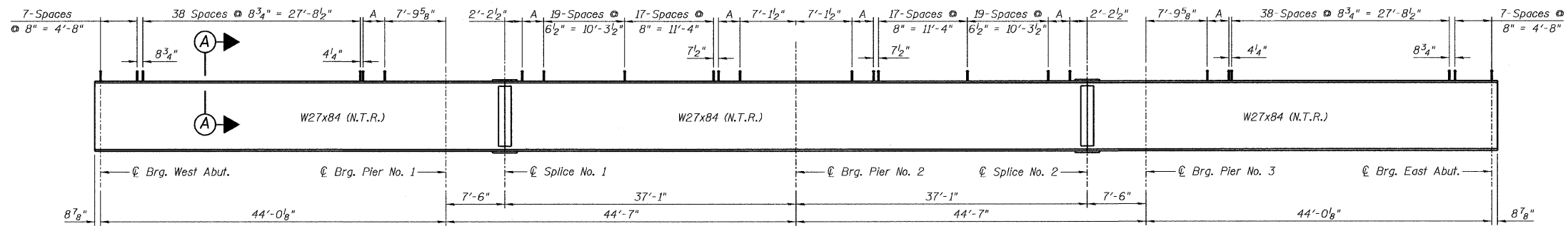
**EAST BRIDGE
APPROACH SLAB DETAILS
STRUCTURE NO. 053-0150**

SHEET NO. B16	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	64	32
FED. ROAD DIST. NO.			ILLINOIS FED. AID PROJECT		
CONTRACT NO. 66691					

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

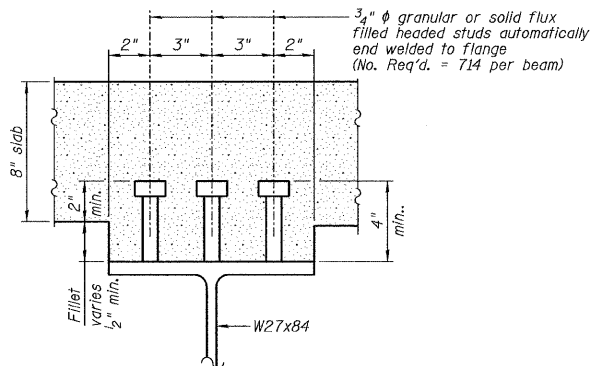


PLAN



ELEVATION

A = 11 spaces @ 3" = 2'-9"



SECTION A-A

FABRICATED TOP OF BEAM ELEVATION TABLE

Location	Beam No. 1	Beam No. 2	Beam No. 3	Beam No. 4	Beam No. 5	Beam No. 6
℄ Brg. W. Abut.	615.50	615.62	615.71	615.70	615.59	615.46
℄ Pier No. 1	615.64	615.76	615.86	615.85	615.74	615.63
℄ Splice No. 1	615.66	615.78	615.88	615.88	615.77	615.66
℄ Pier No. 2	615.66	615.78	615.88	615.88	615.77	615.66
℄ Splice No. 2	615.65	615.77	615.88	615.88	615.78	615.66
℄ Pier No. 3	615.62	615.74	615.85	615.86	615.76	615.64
℄ Brg. E. Abut.	615.46	615.59	615.70	615.71	615.62	615.50

For fabrication use only.

NOTES:

- See Sheet B18 for Splice Details and Diaphragm Details.
- Load carrying components designated N.T.R. shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.
- All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor rods.

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

STRUCTURAL STEEL
STRUCTURE NO. 053-0150

SHEET NO. B17 34 SHEETS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	33
CONTRACT NO. 66691					
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

INTERIOR BEAM MOMENT TABLE					
		0.4 Sp. 1 or 0.6 Sp. 4	Pier 1 or Pier 3	0.5 Sp. 2 or 0.5 Sp. 3	Pier 2
I_s	(in ⁴)	2850	2850	2850	2850
$I_c(n)$	(in ⁴)	9051		9051	
$I_c(3n)$	(in ⁴)	6798		6798	
S_s	(in ³)	213	213	213	213
$S_c(n)$	(in ³)	341		341	
$S_c(3n)$	(in ³)	309		309	
Z	(in ³)		244		244
DC1	(k/ft)	0.752	0.752	0.752	0.752
M _{DC1}	(k)	112	157	54	108
DC2	(k/ft)	0.150	0.150	0.150	0.150
M _{DC2}	(k)	25	16	19	
DW	(k/ft)	0.300	0.300	0.300	0.300
M _{DW}	(k)	50	49	31	39
M _{ℓ + IM}	(k)	458	216	394	204
M _u (Strength I)	(k)	1048	678	824	574
φ _r M _{nc} , φ _r M _{nc}	(k)	1592	997	1634	1021
f _s DC1	(ksi)	6.3	8.8	3.0	6.1
f _s DC2	(ksi)	1.0	1.4	0.6	1.1
f _s DW	(ksi)	1.9	2.8	1.2	2.2
f _s 1.3ℓ+IM	(ksi)	21.0	15.8	18.0	14.9
f _s (Service II)	(ksi)	30.2	28.8	22.9	24.3
f _s (Total)(Strength I)	(ksi)				
V _r	(k)	19.0		17.2	

* Compact Sections
** Non-Compact and Slender Sections

INTERIOR BEAM REACTION TABLE				
		Abut.	Pier 1 or Pier 3	Pier 2
R _{DC1}	(k)	19.6	38.0	31.3
R _{DC2}	(k)	22.6	7.3	6.5
R _{DW}	(k)	5.5	14.6	12.9
R _{ℓ + IM}	(k)	63.0	84.6	82.7
R _{Total}	(k)	110.7	144.5	133.4

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in⁴ and in³).

Z: Plastic Section Modulus of the steel section in non-composite areas. Omit line in Moment Table if not used in design calculations (in³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{ℓ + IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).
1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{ℓ + IM}

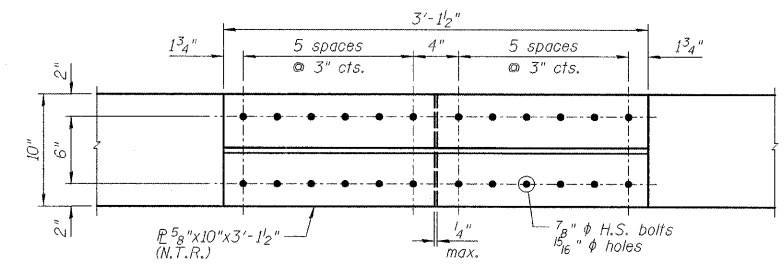
φ_rM_{nc}: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

φ_rM_{nc}: Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).

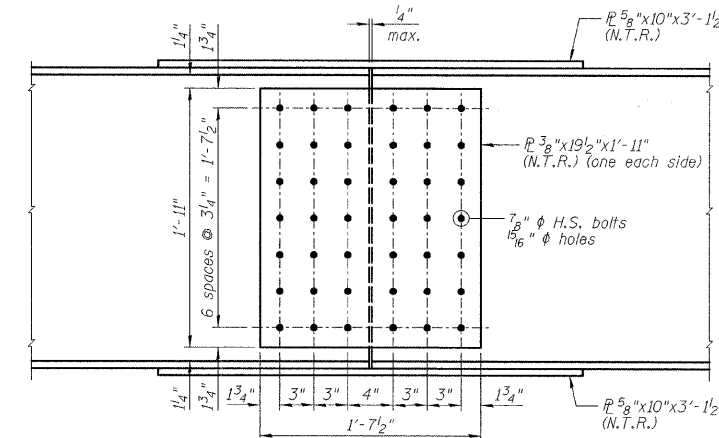
f_s (Service II): Sum of stresses as computed from the moments below (ksi).
M_{DC1} + M_{DC2} + M_{DW} + 1.3 M_{ℓ + IM}

f_s (Total)(Strength I): Sum of stresses as computed from the moments below on non-compact section (ksi).
1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{ℓ + IM}

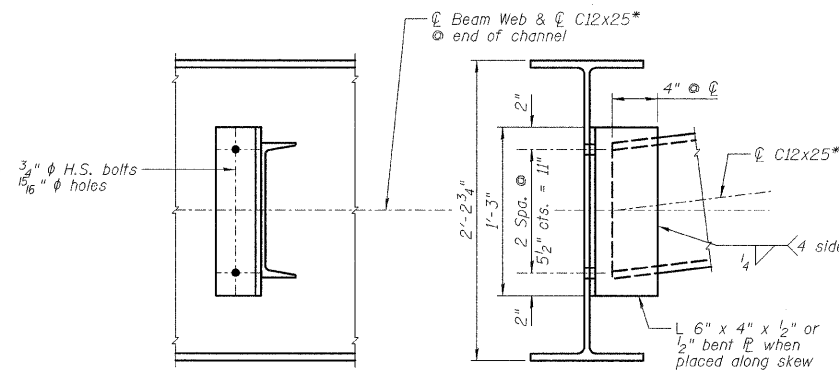
V_r: Maximum factored shear range in composite portion of span computed according to Article 6.10.10.



TOP AND BOTTOM FLANGE



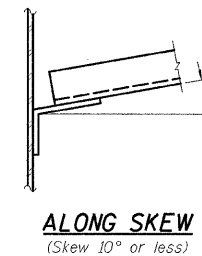
WEB
SPLICE DETAILS
(12 - Required)



DIAPHRAGM D
(45 - Required)

Note:
Two hardened washers required for each set of oversized holes.

*Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on C12x25 section. The C12x30, if utilized, shall be provided at no extra cost to the department.



ALONG SKEW
(Skew 10° or less)

NOTES:

- See Sheet B17 for Splice and Diaphragm Locations.
- Load carrying components designated N.T.R. shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.
- All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor rods.

STRUCTURAL STEEL
STRUCTURE NO. 053-0150

DESIGNED SDH
CHECKED JML
DRAWN JWK/DJM
CHECKED MSW

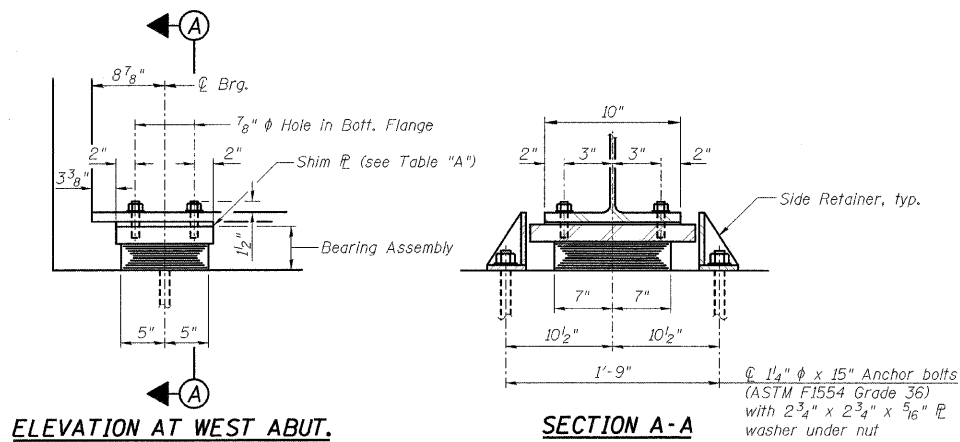
DATE 10/07/09

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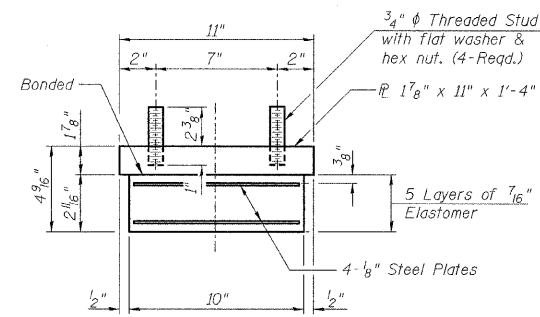
SHEET NO. B18	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	64	34
FED. ROAD DIST. NO.			ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 66691					

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

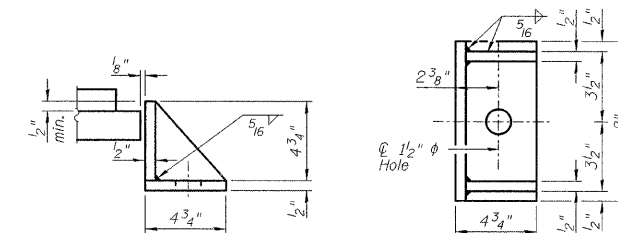


TYPE I ELASTOMERIC EXP. BRG.

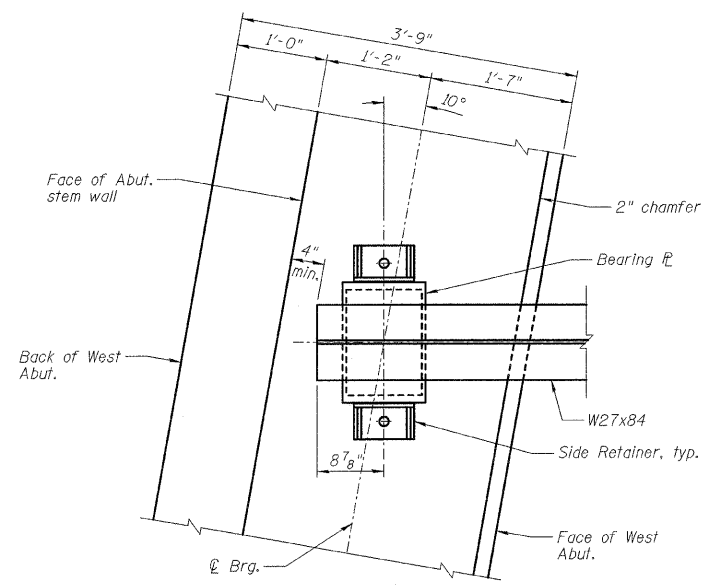
(At West Abutment - 6 Required)
(At East Abutment - 6 Required)



Note: Shim plates shall not be placed under Bearing Assembly.

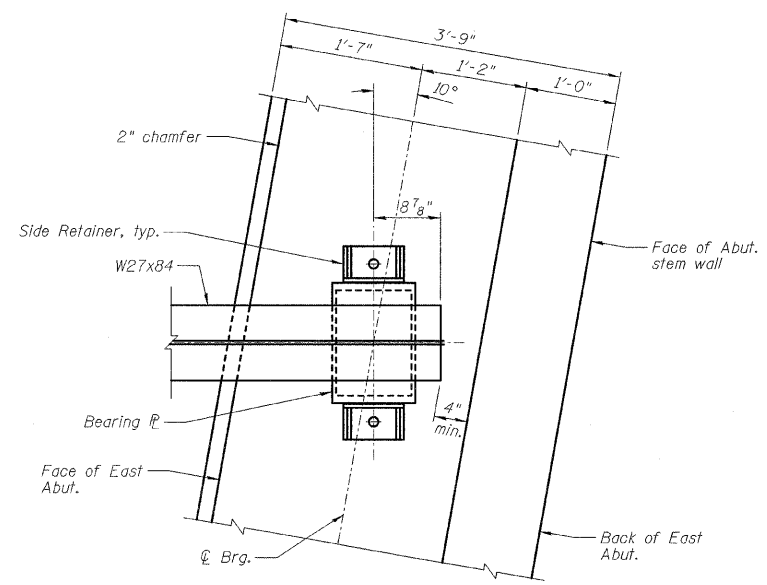


Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



BEARING PLAN AT WEST ABUTMENT

Notes:
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (Fy=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Anchor bolts for side retainers may be cast in place or installed in holes drilled before or after members are in place.
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
Side retainers and other steel members required for the bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.



BEARING PLAN AT EAST ABUTMENT

TABLE "A"

Beam No.	Shim Thickness
West Abut. - 3	1/8"
East Abut. - 4	1/8"

BILL OF MATERIAL

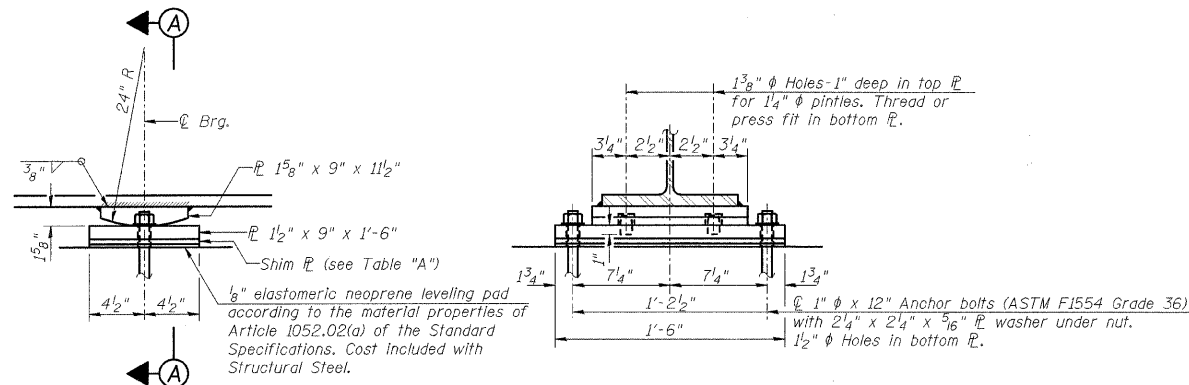
Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	12
Anchor Bolts, 1 1/4"	Each	24

- NOTES:**
- Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.
 - The structural steel plates of the bearing assembly shall conform to the requirements of AASHTO M 270 Grade 50W.

DESIGNED SDH
CHECKED JML
DRAWN JWK/DJM
CHECKED MSW
DATE 10/07/09

SHEET NO. B19	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	35
34 SHEETS		CONTRACT NO. 66691			
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

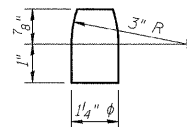


ELEVATION AT PIERS NO. 1 AND NO. 3

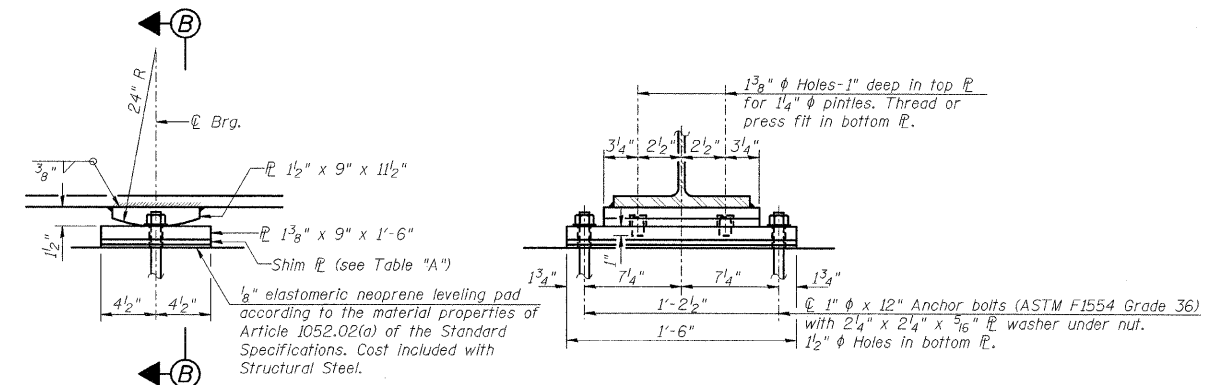
SECTION A-A

FIXED BEARING

(At Pier No. 1 - 6 Required)
(At Pier No. 3 - 6 Required)



PINTLE

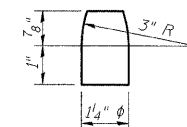


ELEVATION AT PIER NO. 2

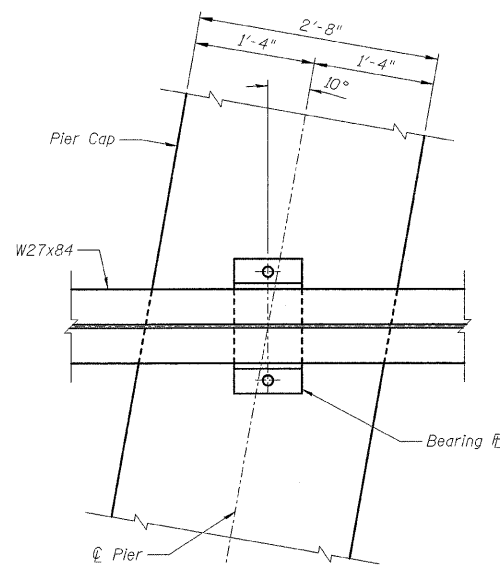
SECTION B-B

FIXED BEARING

(At Pier No. 2 - 6 Required)



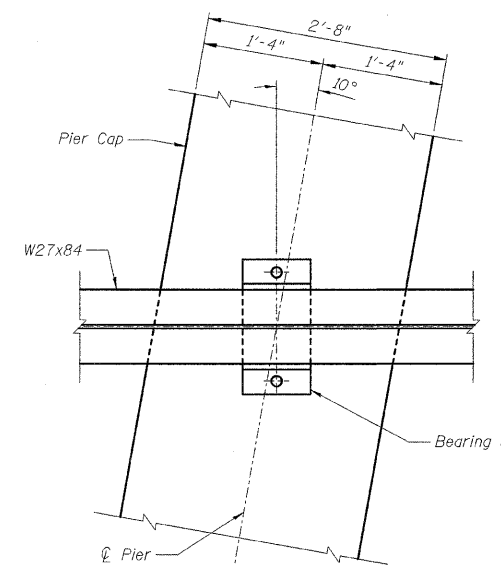
PINTLE



BEARING PLAN AT PIERS NO. 1 AND NO. 3

Notes:

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (Fy=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.



BEARING PLAN AT PIER NO. 2

TABLE "A"

Beam No.	Shim Thickness
Pier No. 1 - 3	1/8"
Pier No. 3 - 4	1/8"

BILL OF MATERIAL

Item	Unit	Total
Anchor Bolts, 1"	Each	36

NOTES:

- Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.
- The structural steel plates of the bearing assembly shall conform to the requirements of AASHTO M 270 Grade 50W.

DESIGNED SDH
CHECKED JML
DRAWN JWK/DJM
CHECKED MSW

DATE 10/07/09

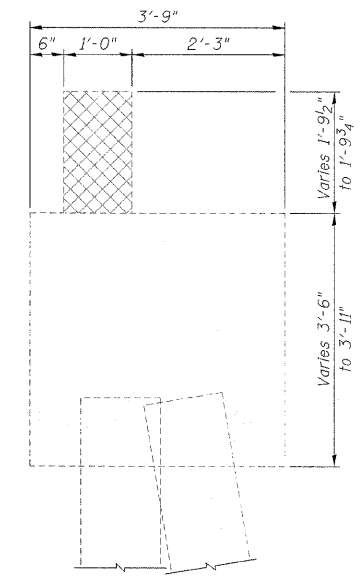
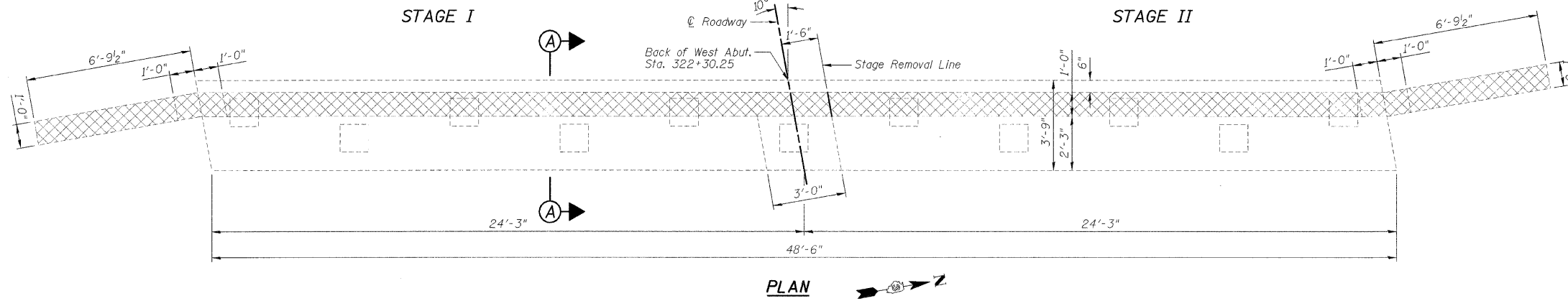
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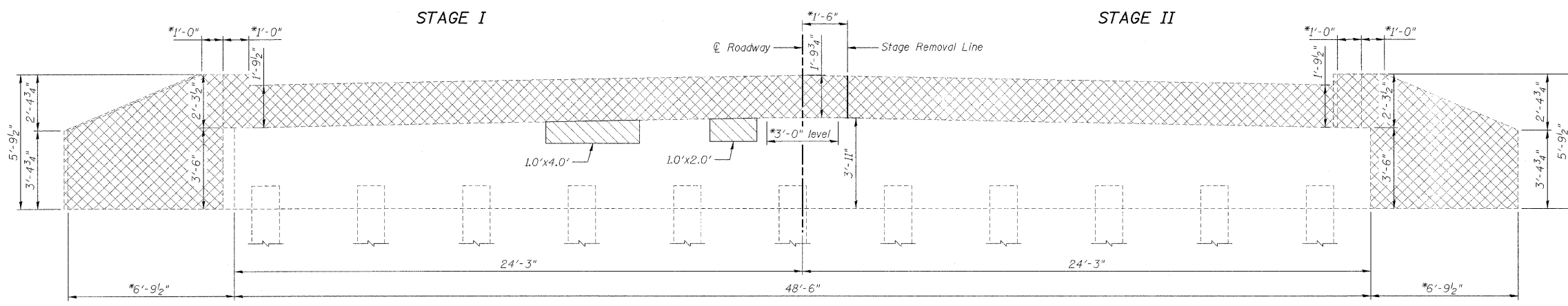
**FIXED BEARING DETAILS
STRUCTURE NO. 053-0150**

SHEET NO. B20	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	64	36
FED. ROAD DIST. NO.			ILLINOIS FED. AID PROJECT		
CONTRACT NO. 66691					

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



SECTION A-A
(@ Rf. L's)



ELEVATION
(Looking West)

*Dimensions @ Rf. L's to Roadway.

BILL OF MATERIAL

Item	Unit	Total
Concrete Removal	Cu. Yd.	5.7
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	6

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

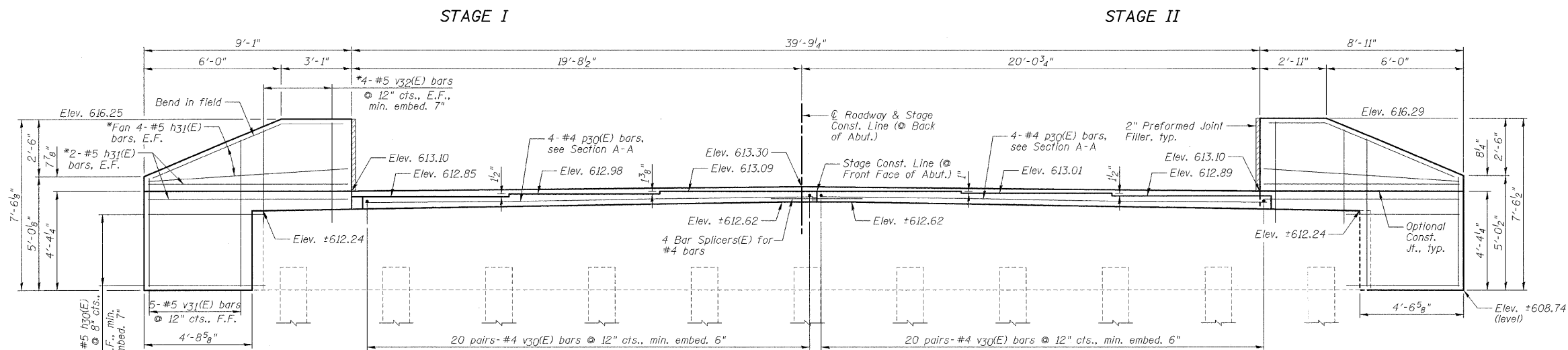
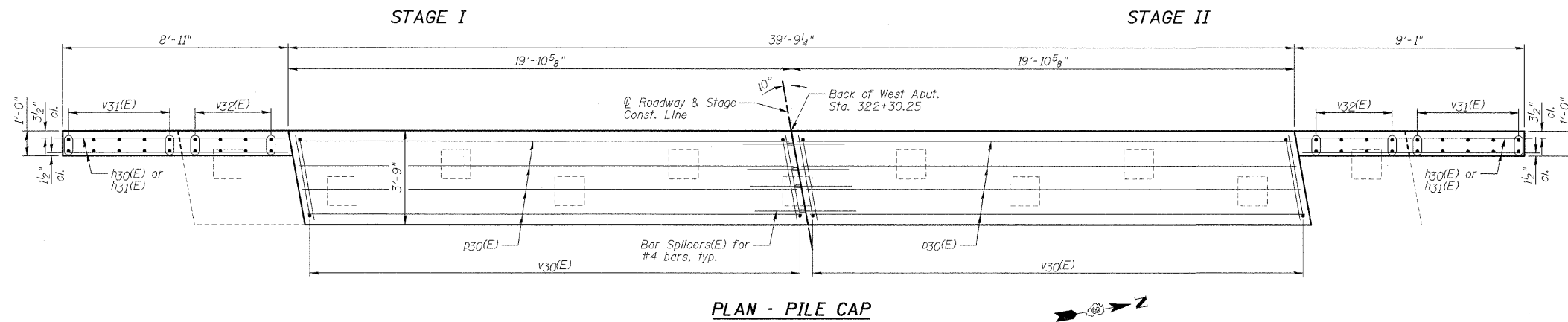
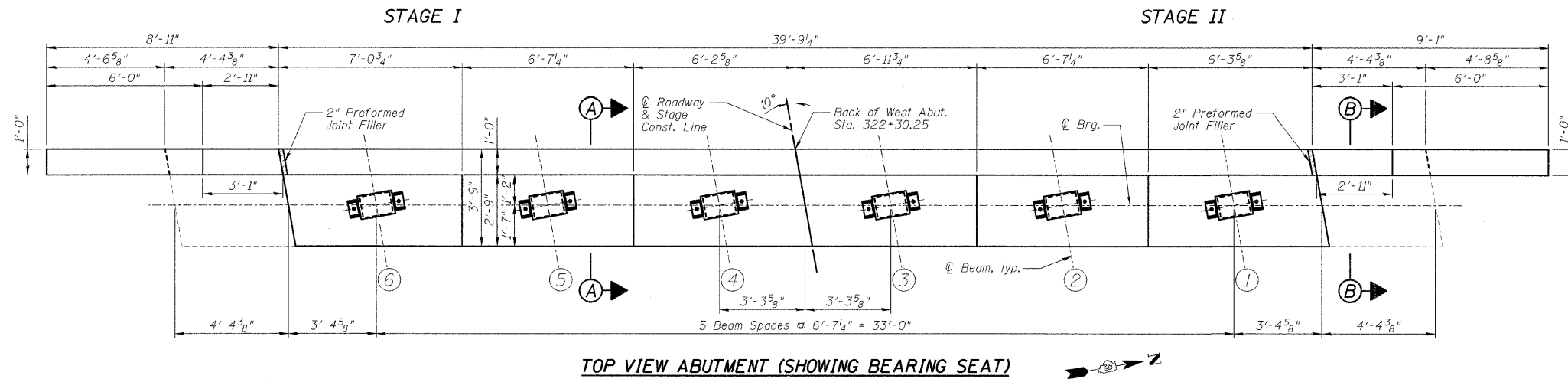
LEGEND

- Concrete Removal
- Structural Repair of Concrete
(Depth Equal to or Less Than 5 Inches)

**WEST ABUTMENT REMOVAL
STRUCTURE NO. 053-0150**

SHEET NO. B21	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	04	37
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



ELEVATION
(Looking West)

NOTES:

- 1) See Sheet B23 for Section A-A and Section B-B.
- 2) See Sheet B23 for Bill of Material and Bar Cutting Diagram. Order v31(E) bars full length. Cut according to Bar Cutting Diagram on Sheet B23. Use remainder of bars in back face.
- 3) Drill & epoxy grout h30(E), v30(E) & v32(E) bars in appropriate drilled holes according to Section 584 of the Standard Specifications. The type of epoxy grout shall be approved by the Engineer.
- 4) Space reinforcement in cap to miss anchor bolts.
- 5) E.F. denotes Each Face, F.F. denotes Front Face.
- 6) See Sheet B33 for Bar Splicer Details.

*Cut reinforcement as required.
Reinforcement is typical for both wingwalls.

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

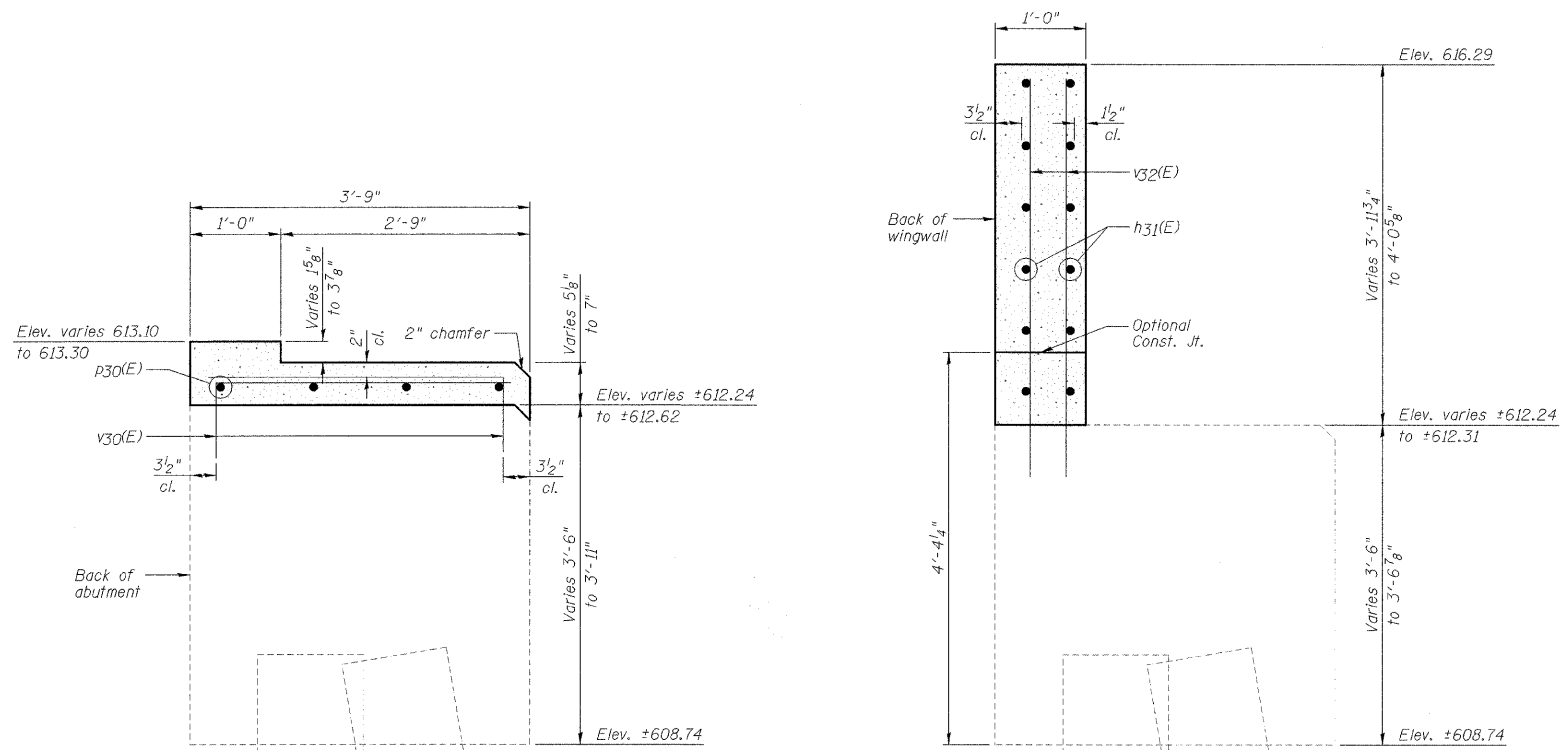
FARNSWORTH GROUP, INC.

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WEST ABUTMENT
STRUCTURE NO. 053-0150

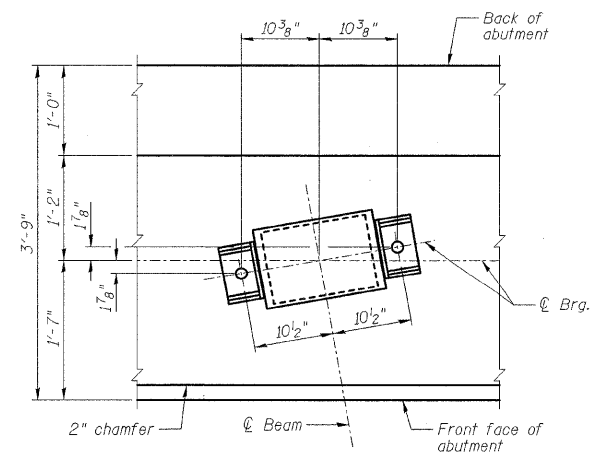
SHEET NO. B22	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	38
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



SECTION A-A

SECTION B-B



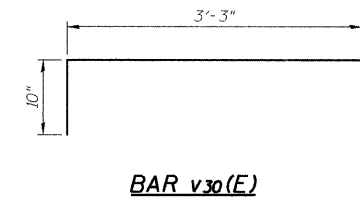
TYPICAL ANCHOR BOLT
PLACEMENT DETAIL

NOTES:

- 1.) See Sheet B22 for location of Section A-A and Section B-B.
- 2.) Drill & epoxy grout v30(E) & v32(E) bars in appropriate drilled holes according to Section 584 of the Standard Specifications. The type of epoxy grout shall be approved by the Engineer.
- 3.) Space reinforcement in cap to miss anchor bolts.

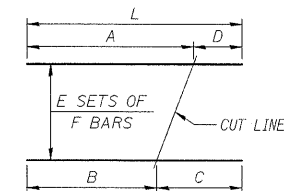
WEST ABUTMENT
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h30(E)	24	#5	5'-2"	—
h31(E)	24	#5	8'-8"	—
p30(E)	8	#4	19'-6"	—
v30(E)	80	#4	4'-1"	—
v31(E)	10	#5	11'-2"	—
v32(E)	16	#5	4'-5"	—
Item	Unit	Quantity		
Porous Granular Embankment, Special	Cu. Yd.	72		
Structure Excavation	Cu. Yd.	72		
Concrete Structures	Cu. Yd.	6.5		
Reinforcement Bars, Epoxy Coated	Pound	860		
Bar Splicers	Each	4		
Geocomposite Wall Drain	Sq. Yd.	42		
Pipe Underdrains For Structures 4"	Foot	70		



BAR v30(E)

BAR CUTTING DIAGRAM



BAR	A	B	C	D	E	F	L
v31(E)	6'-5"	4'-9"	6'-5"	4'-9"	2	5	11'-2"

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

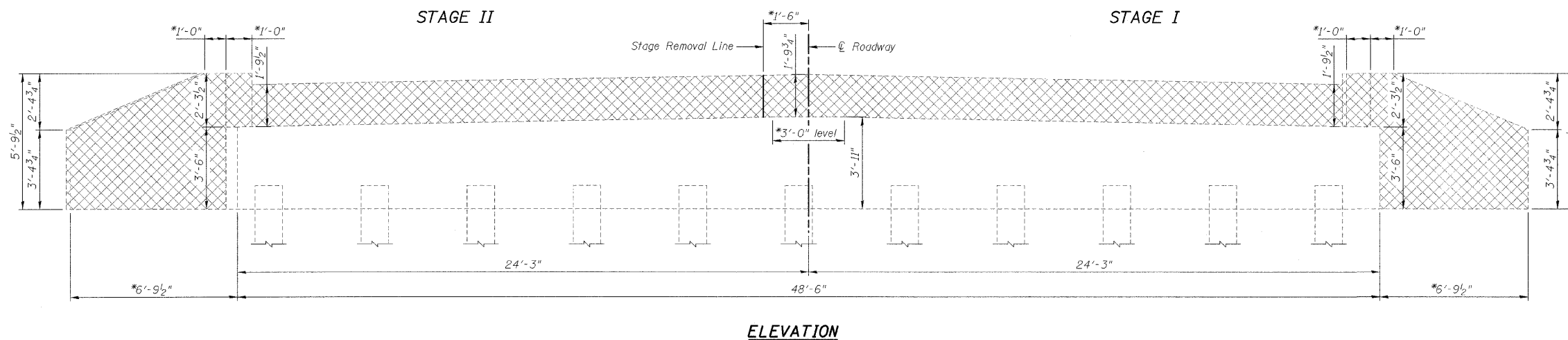
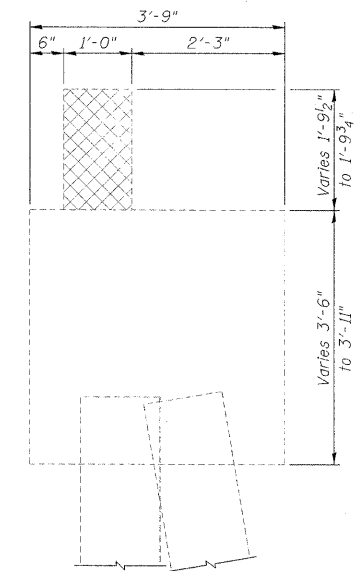
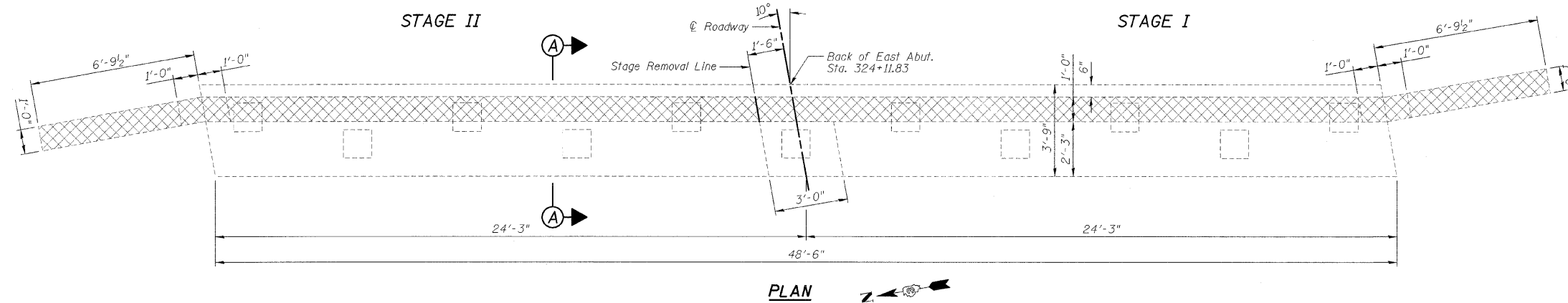
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WEST ABUTMENT
STRUCTURE NO. 053-0150

SHEET NO. B23	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	44	39
			CONTRACT NO. 66691		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



*Dimensions @ Rt. L's to Roadway.

BILL OF MATERIAL

Item	Unit	Total
Concrete Removal	Cu. Yd.	5.7

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

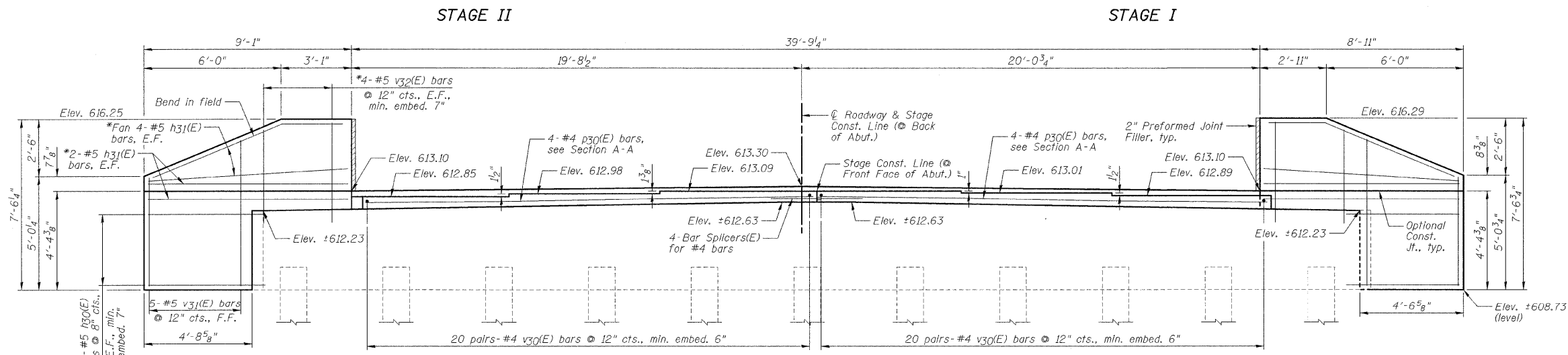
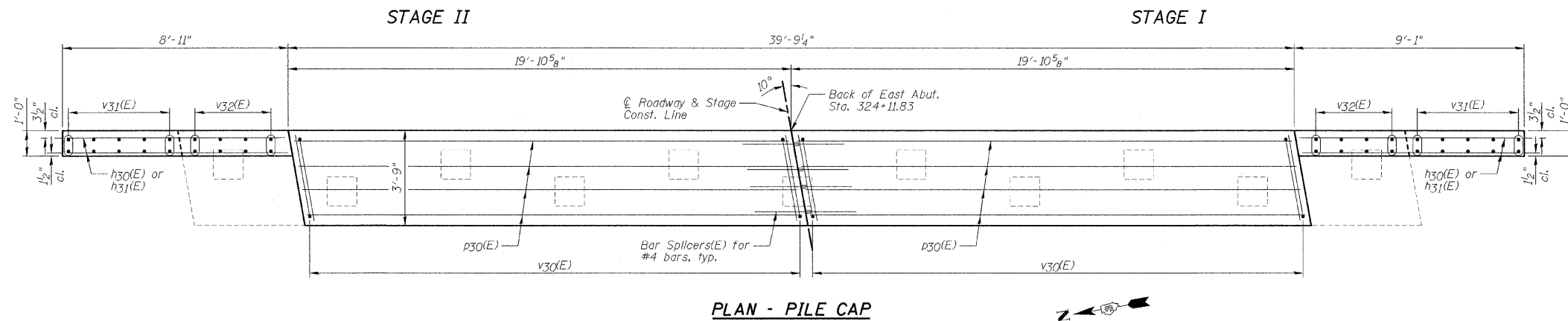
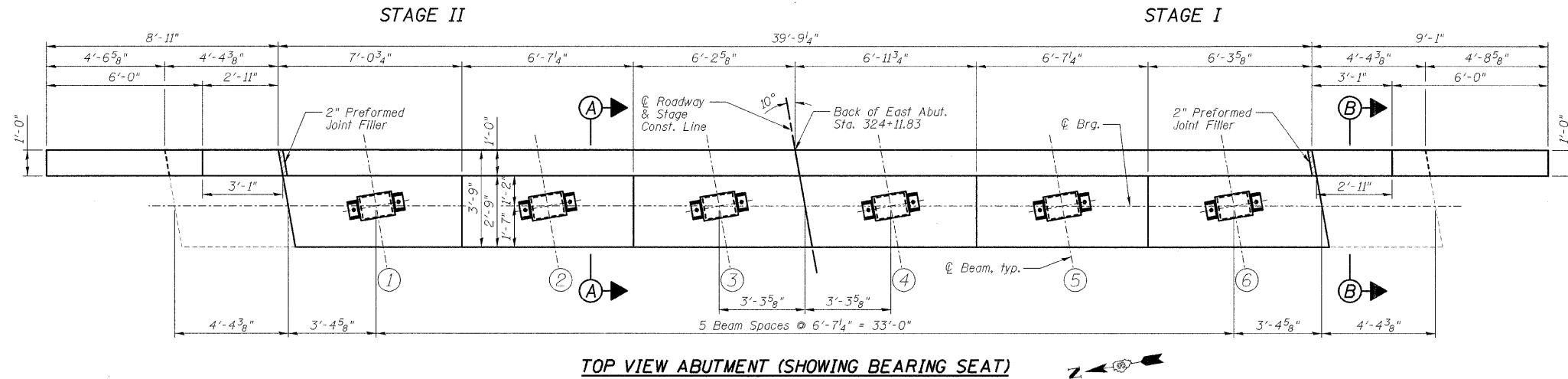
LEGEND

Concrete Removal

**EAST ABUTMENT REMOVAL
STRUCTURE NO. 053-0150**

SHEET NO. B24	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	40
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



ELEVATION
(Looking East)

NOTES:

- 1.) See Sheet B26 for Section A-A and Section B-B.
- 2.) See Sheet B26 for Bill of Material and Bar Cutting Diagram. Order v31(E) bars full length. Cut according to Bar Cutting Diagram on Sheet B26. Use remainder of bars in back face.
- 3.) Drill & epoxy grout h30(E), v30(E) & v32(E) bars in appropriate drilled holes according to Section 584 of the Standard Specifications. The type of epoxy grout shall be approved by the Engineer.
- 4.) Space reinforcement in cap to miss anchor bolts.
- 5.) E.F. denotes Each Face, F.F. denotes Front Face.
- 6.) See Sheet B33 for Bar Splicer Details.

**EAST ABUTMENT
STRUCTURE NO. 053-0150**

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

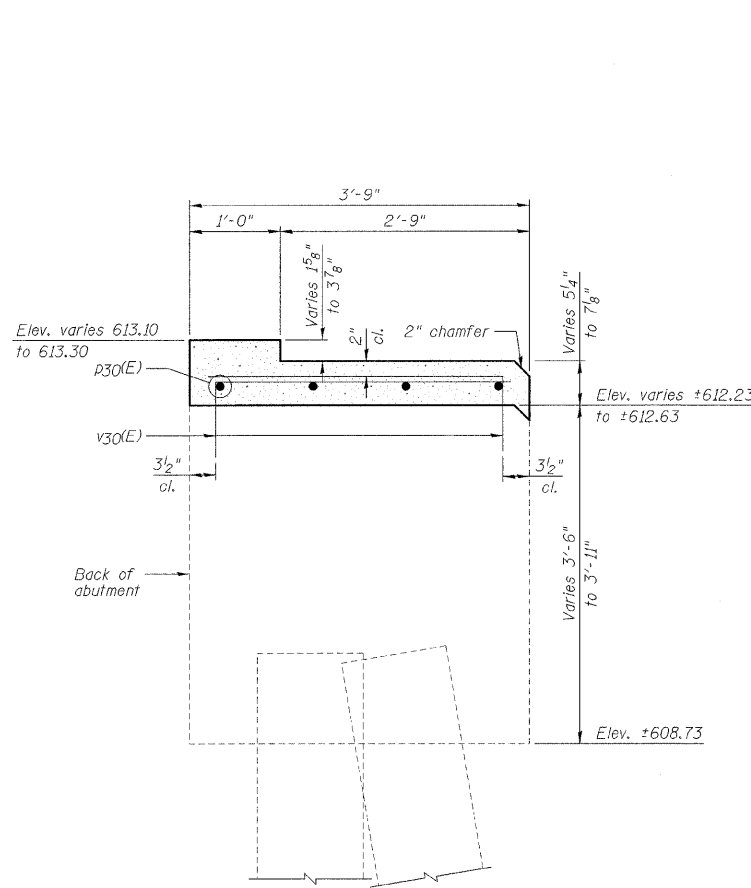
DATE 10/07/09

FARNSWORTH GROUP, INC.

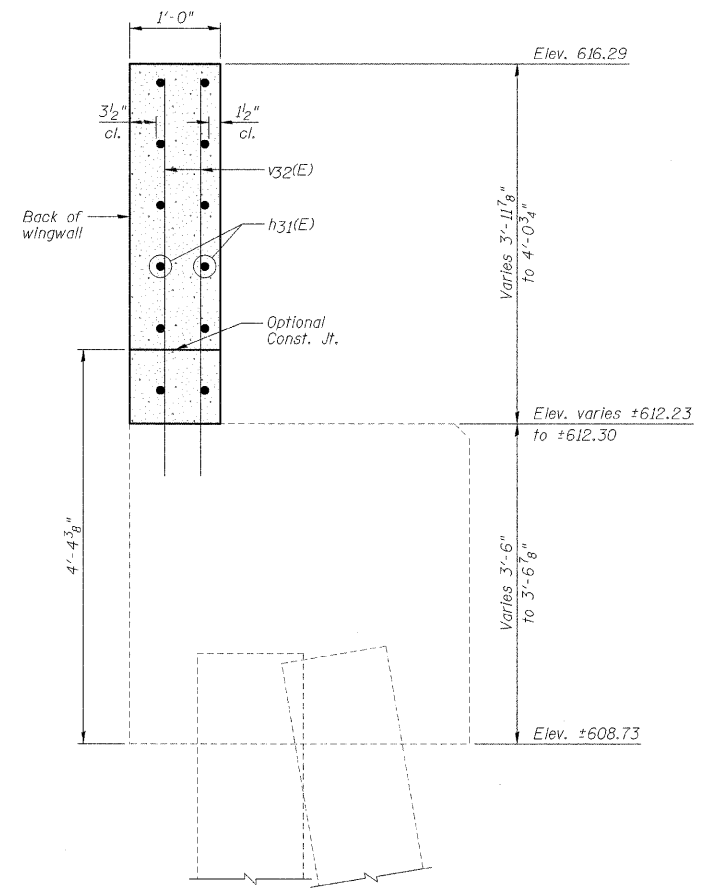
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SHEET NO. B25	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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34 SHEETS	CONTRACT NO. 66691		ILLINOIS FED. AID PROJECT		

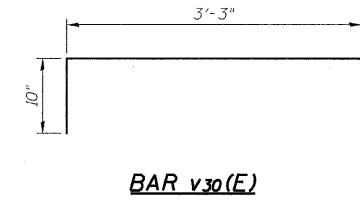
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



SECTION A-A



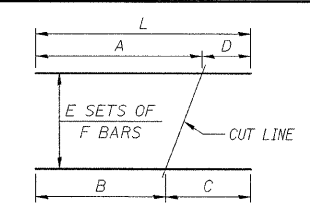
SECTION B-B



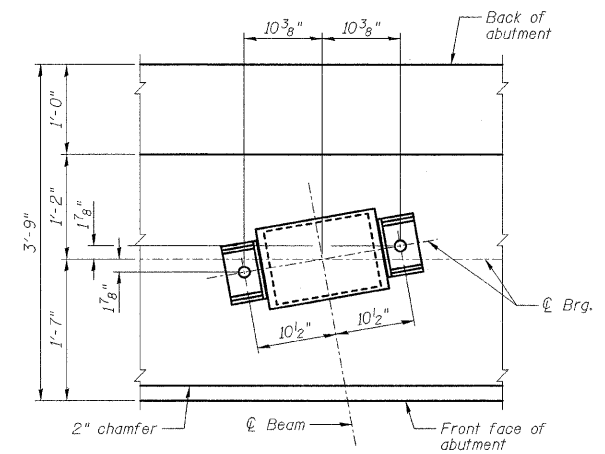
EAST ABUTMENT
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h30(E)	24	#5	5'-2"	—
h31(E)	24	#5	8'-8"	—
p30(E)	8	#4	19'-6"	—
v30(E)	80	#4	4'-1"	┌
v31(E)	10	#5	11'-2"	—
v32(E)	16	#5	4'-5"	—
Item	Unit	Quantity		
Porous Granular Embankment, Special	Cu. Yd.	72		
Structure Excavation	Cu. Yd.	72		
Concrete Structures	Cu. Yd.	6.5		
Reinforcement Bars, Epoxy Coated	Pound	860		
Bar Splicers	Each	4		
Geocomposite Wall Drain	Sq. Yd.	42		
Pipe Underdrains For Structures 4"	Foot	70		

BAR CUTTING DIAGRAM



BAR	A	B	C	D	E	F	L
v31(E)	6'-5"	4'-9"	6'-5"	4'-9"	2	5	11'-2"



TYPICAL ANCHOR BOLT
PLACEMENT DETAIL

NOTES:

- See Sheet B25 for location of Section A-A and Section B-B.
- Drill & epoxy grout v30(E) & v32(E) bars in appropriate drilled holes according to Section 584 of the Standard Specifications. The type of epoxy grout shall be approved by the Engineer.
- Space reinforcement in cap to miss anchor bolts.

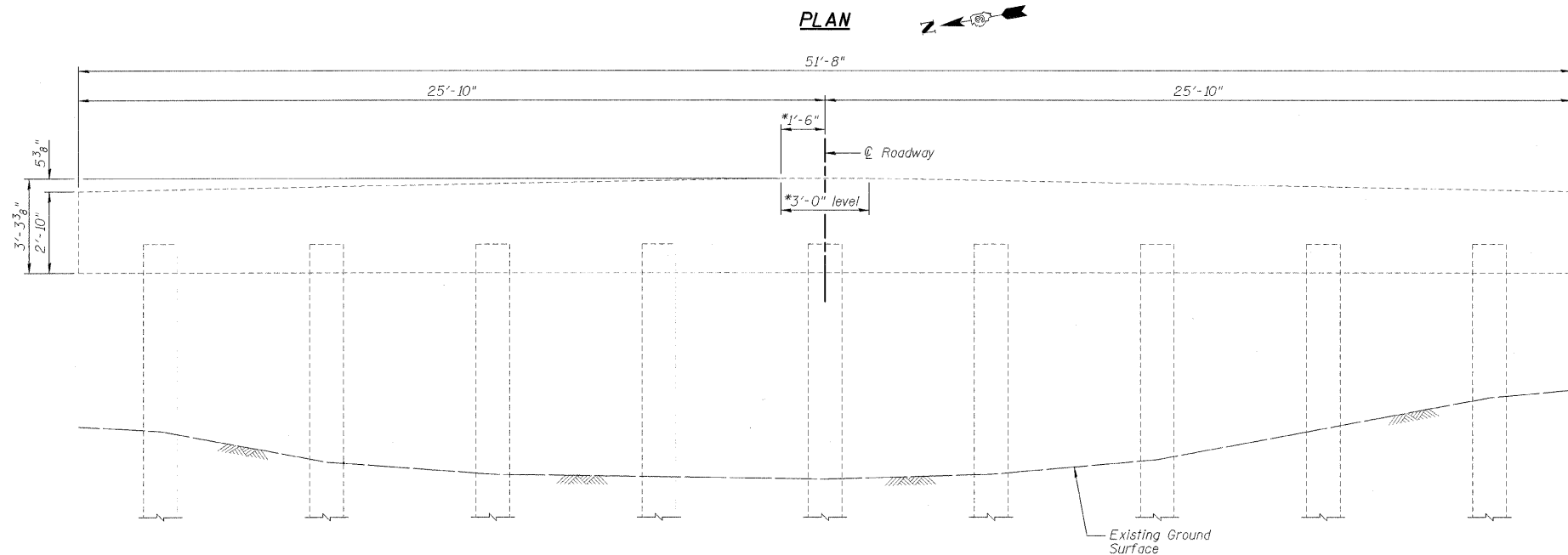
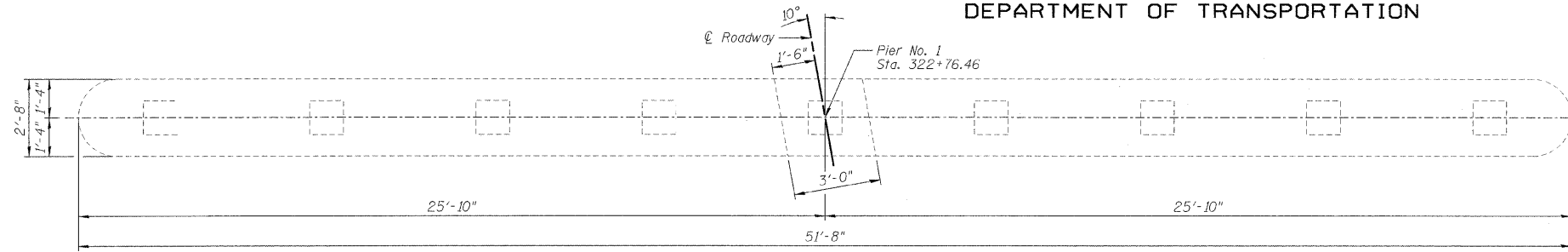
DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

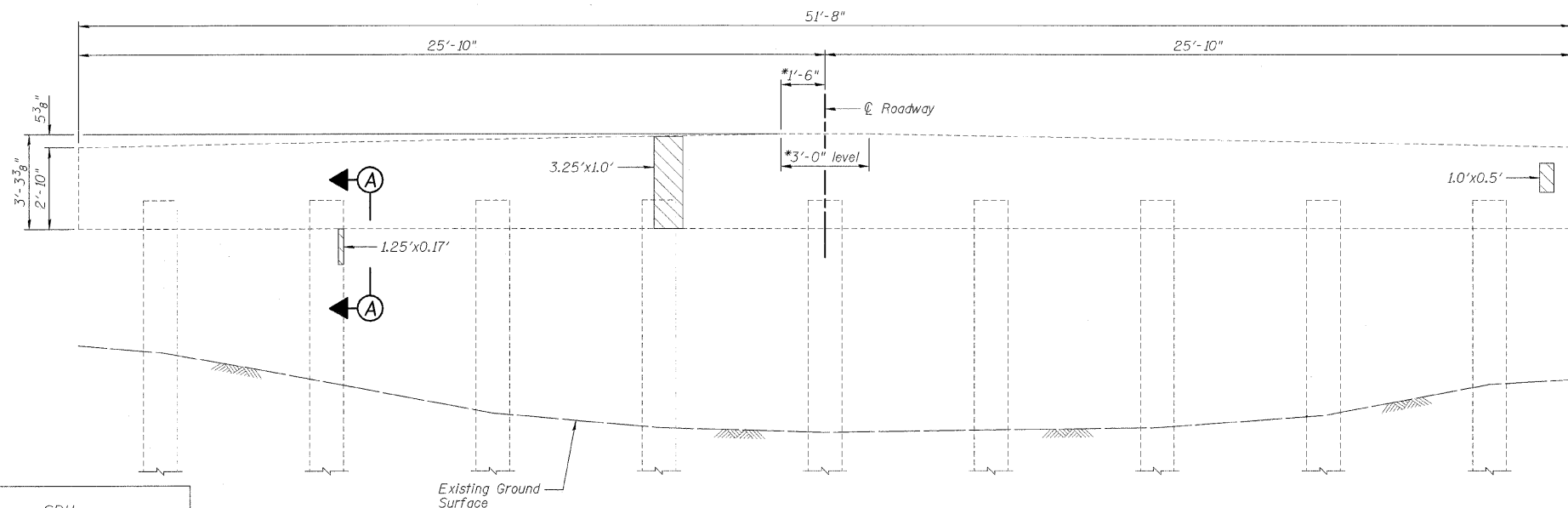
EAST ABUTMENT
STRUCTURE NO. 053-0150

SHEET NO. B26	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	42
34 SHEETS			CONTRACT NO. 66691		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT		

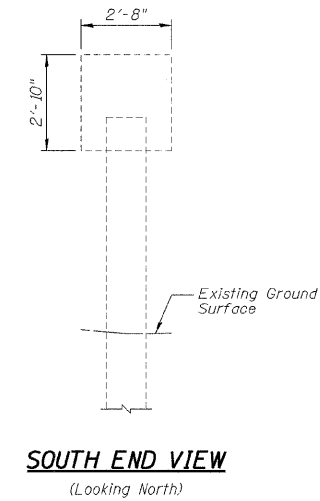
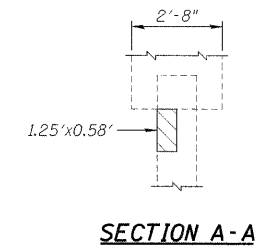
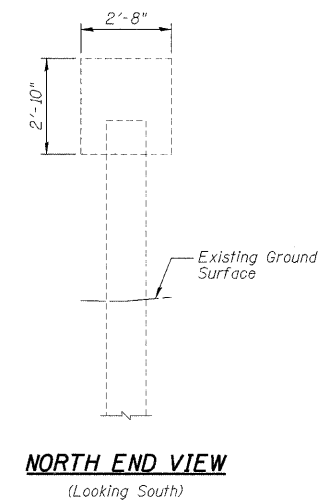
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



*Dimensions @ Rt. L's to Roadway.



*Dimensions @ Rt. L's to Roadway.



BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	5

LEGEND

Structural Repair of Concrete
(Depth Equal to or Less Than 5 Inches)

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

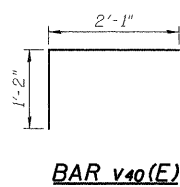
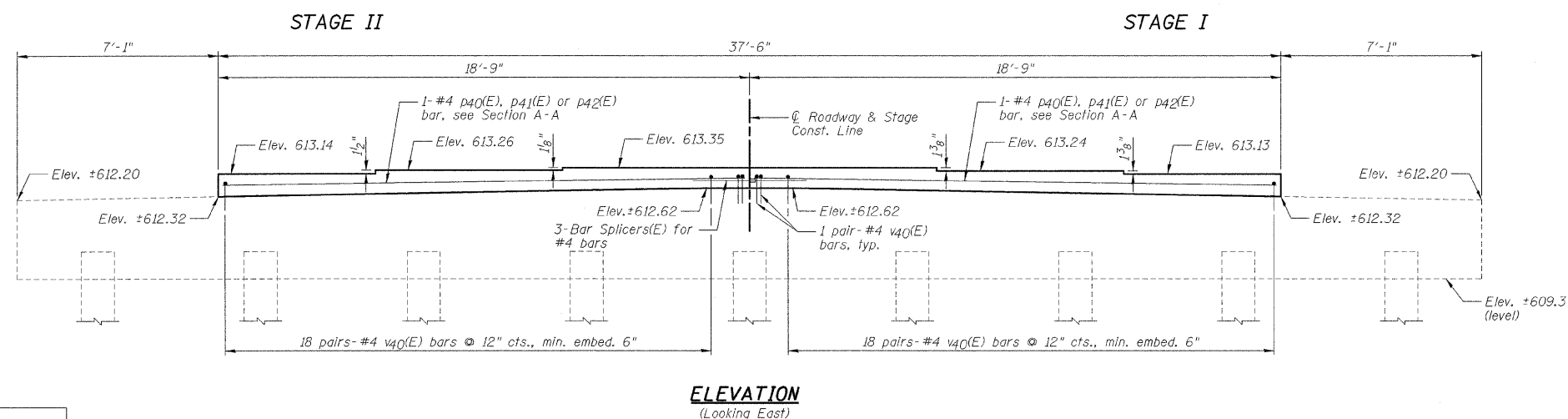
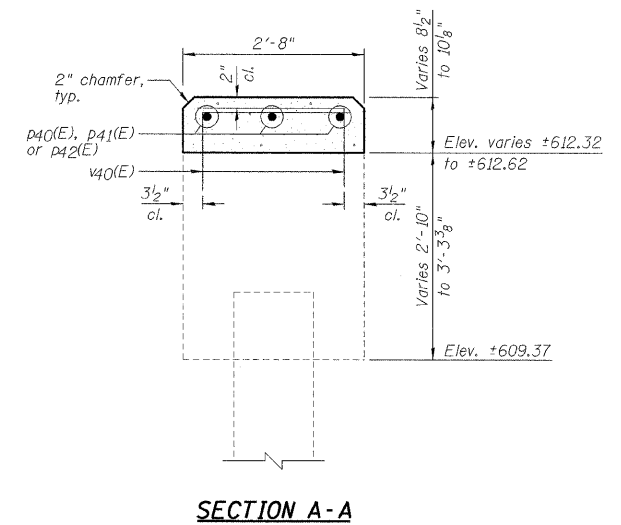
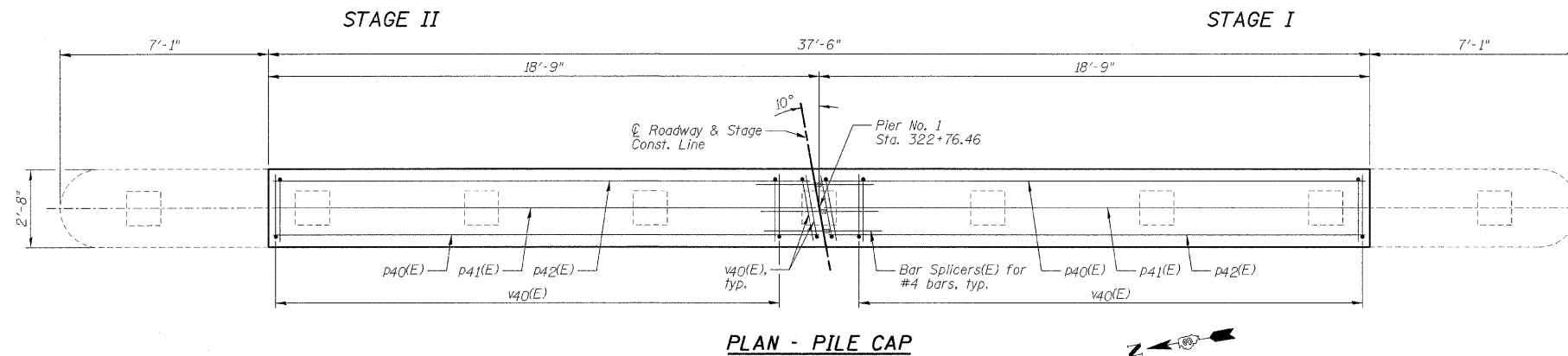
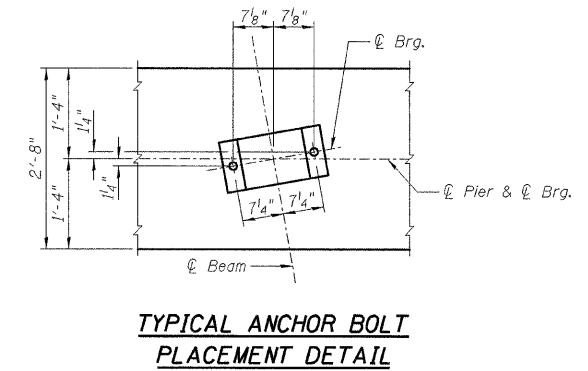
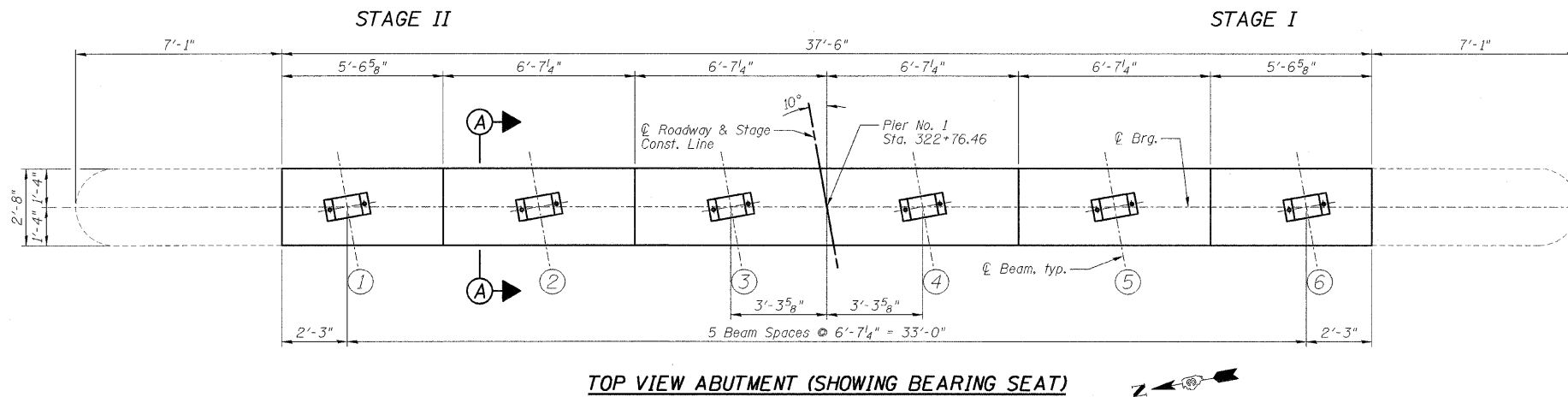
DATE 10/07/09

FARNSWORTH GROUP, INC.

PIER NO. 1 REPAIR
STRUCTURE NO. 053-0150

SHEET NO. B27	F.A.P. RTE. 41	SECTION 15BR-2	COUNTY LIVINGSTON	TOTAL SHEETS 44	SHEET NO. 43
34 SHEETS	CONTRACT NO. 66691		FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



PIER NO. 1
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
P40(E)	2	#4	18'-7"	—
P41(E)	2	#4	18'-5"	—
P42(E)	2	#4	18'-3"	—
V40(E)	76	#4	3'-3"	□
Item		Unit	Quantity	
Concrete Structures		Cu. Yd.	2.9	
Reinforcement Bars, Epoxy Coated		Pound	240	
Bar Splicers		Each	3	

PIER NO. 1
STRUCTURE NO. 053-0150

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

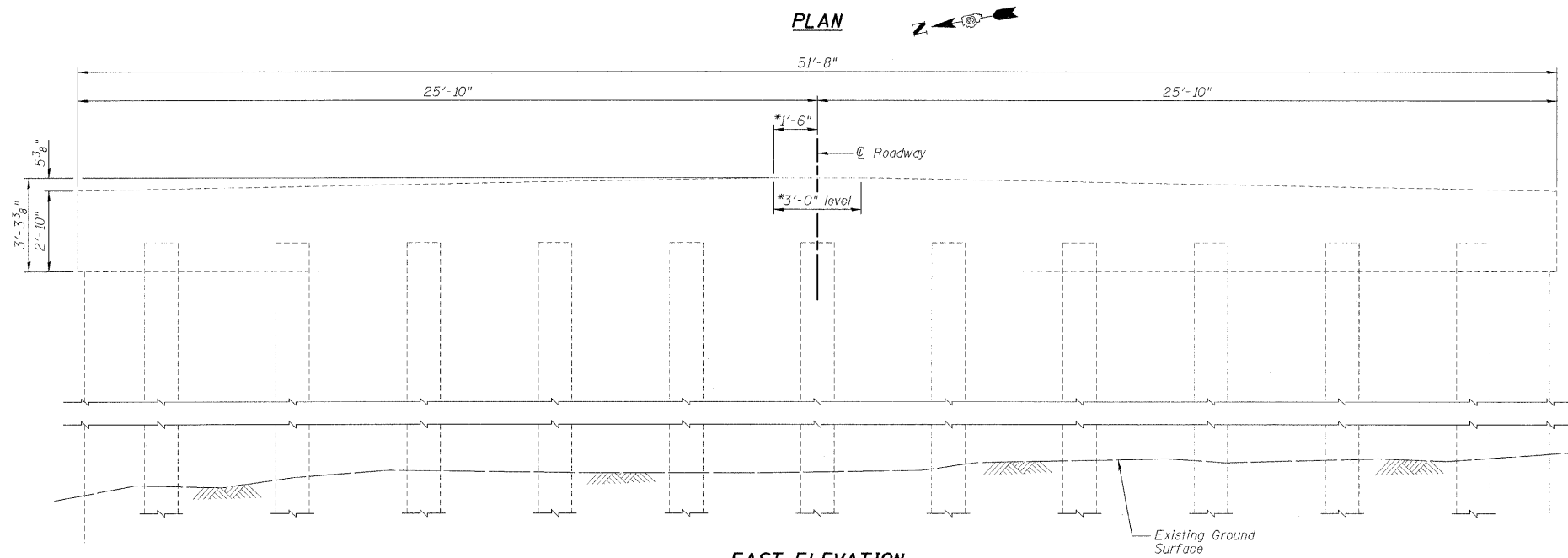
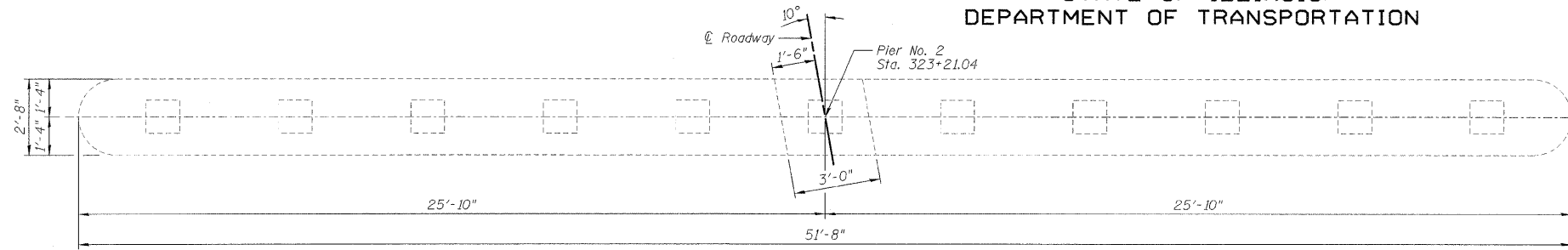
FARNSWORTH GROUP, INC.

NOTES:

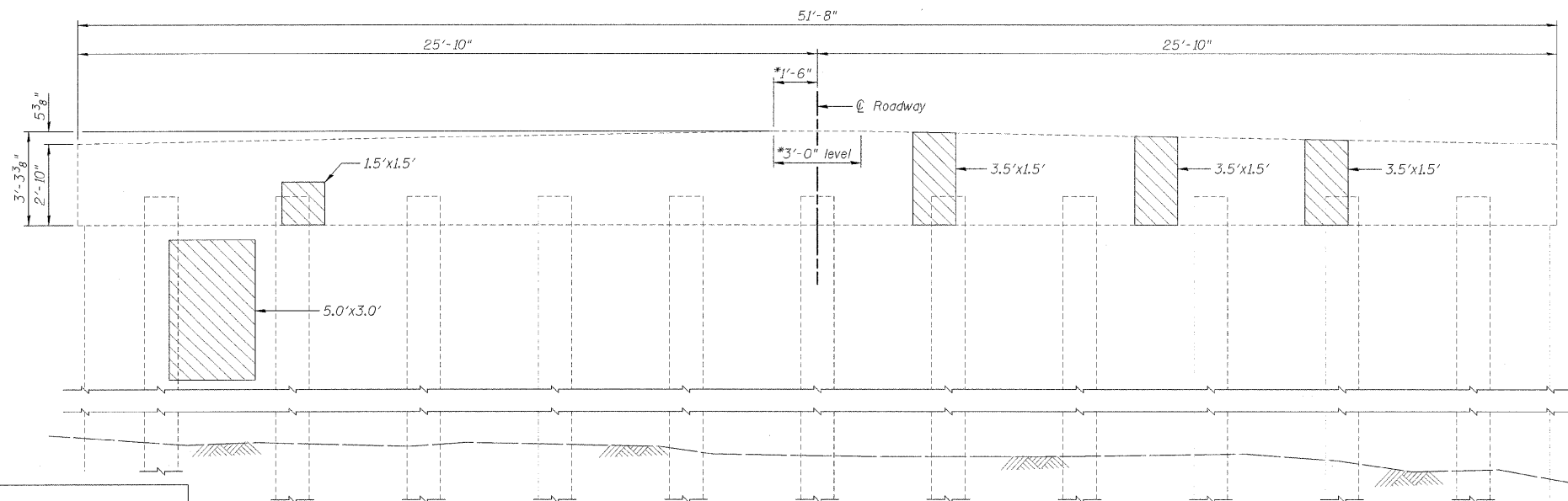
- 1.) Drill & epoxy grout V40(E) bars in appropriate drilled holes according to Section 584 of the Standard Specifications. The type of epoxy grout shall be approved by the Engineer.
- 2.) Space reinforcement in cap to miss anchor bolts.
- 3.) See Sheet B33 for Bar Splicer Details.

SHEET NO. B28	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	44	44
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				CONTRACT NO. 66691	

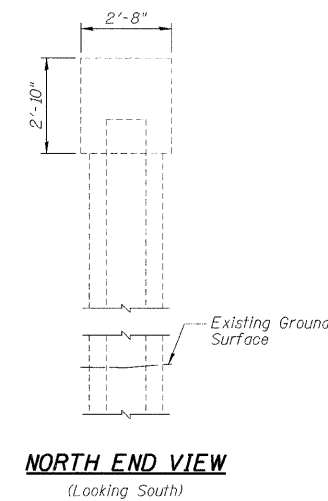
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



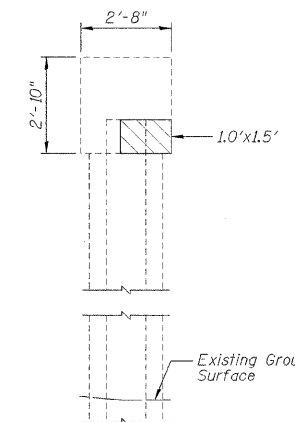
EAST ELEVATION
(Looking West)
*Dimensions @ Rt. L's to Roadway.



WEST ELEVATION
(Looking East)
*Dimensions @ Rt. L's to Roadway.



NORTH END VIEW
(Looking South)



SOUTH END VIEW
(Looking North)

BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	35

LEGEND

Structural Repair of Concrete
(Depth Equal to or Less Than 5 Inches)

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

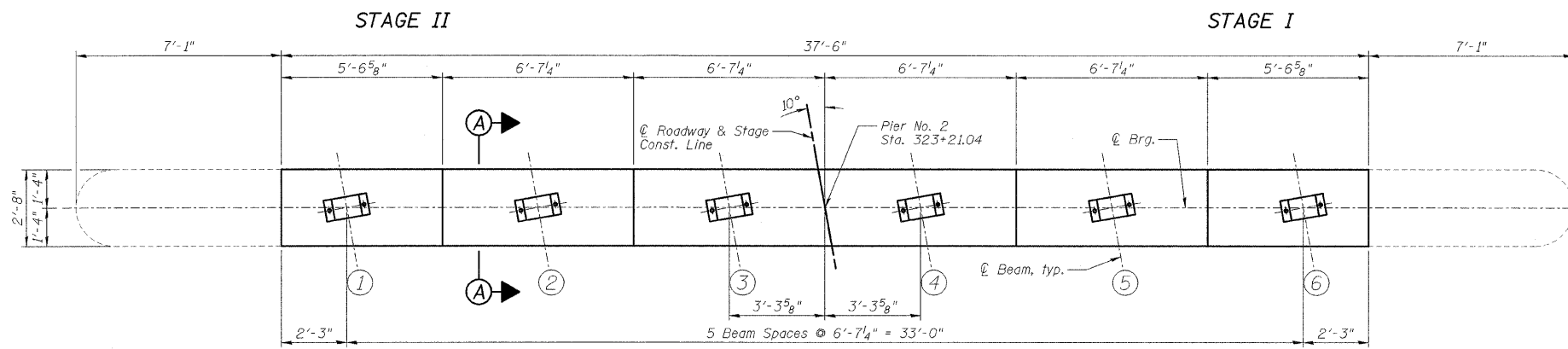
FARNSWORTH GROUP, INC.

CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

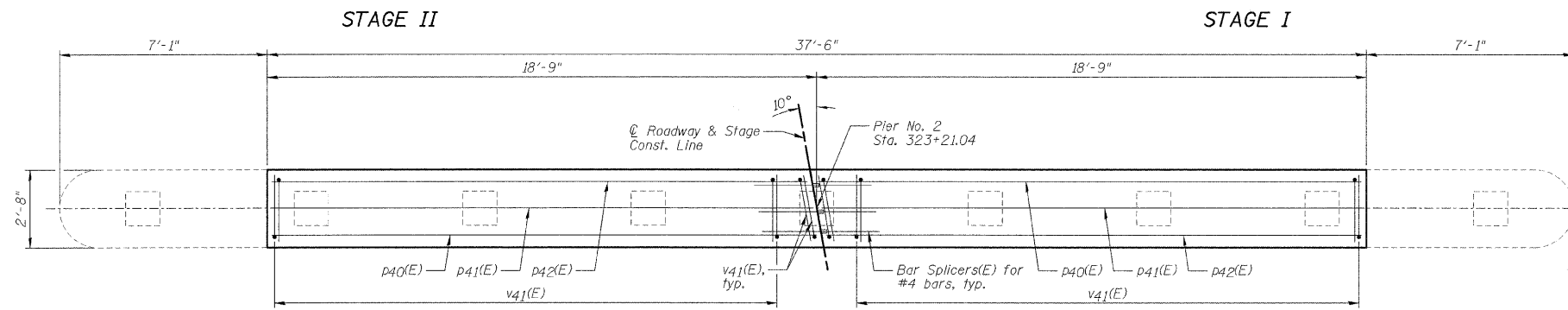
SHEET NO. B29	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	44	45
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT		

**PIER NO. 2 REPAIR
STRUCTURE NO. 053-0150**

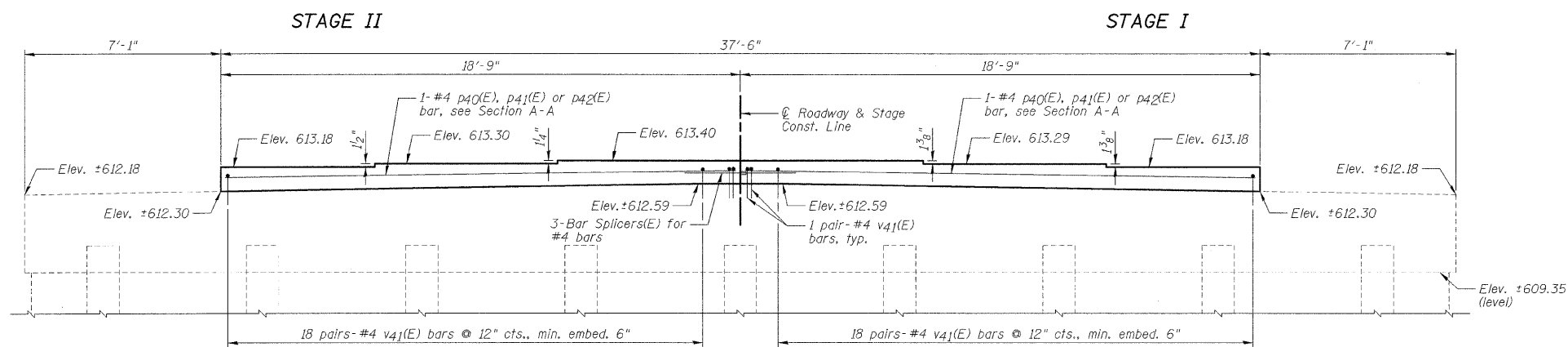
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



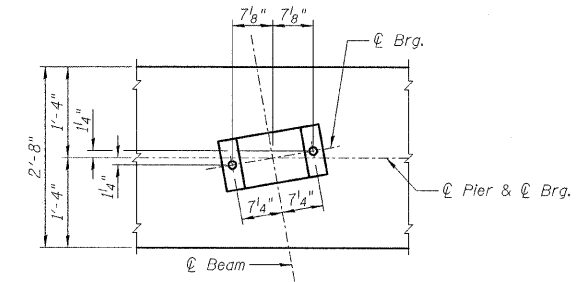
TOP VIEW ABUTMENT (SHOWING BEARING SEAT)



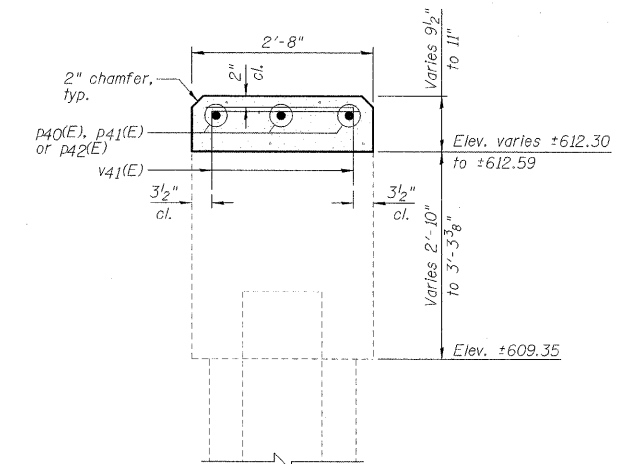
PLAN - PILE CAP



ELEVATION
(Looking East)



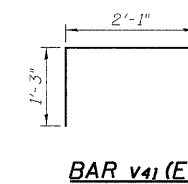
TYPICAL ANCHOR BOLT
PLACEMENT DETAIL



SECTION A-A

PIER NO. 2
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
p40(E)	2	#4	18'-7"	—
p41(E)	2	#4	18'-5"	—
p42(E)	2	#4	18'-3"	—
v41(E)	76	#4	3'-4"	—
Item	Unit	Quantity		
Concrete Structures	Cu. Yd.	3.2		
Reinforcement Bars, Epoxy Coated	Pound	240		
Bar Splicers	Each	3		



BAR v41 (E)

PIER NO. 2
STRUCTURE NO. 053-0150

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

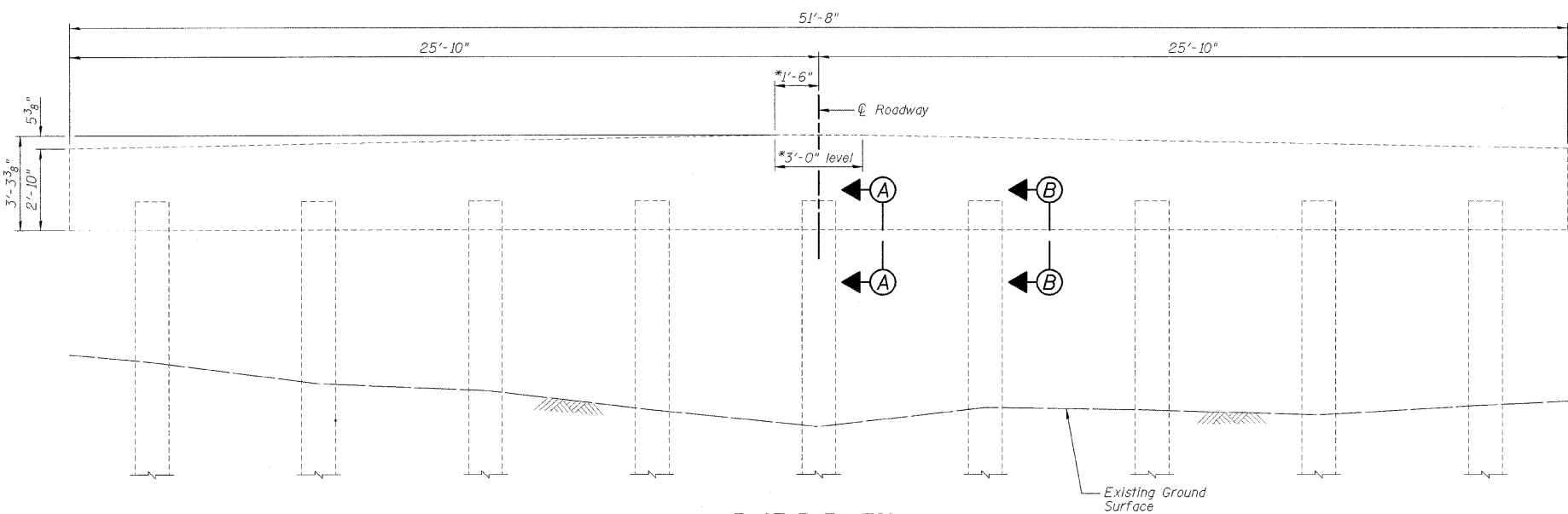
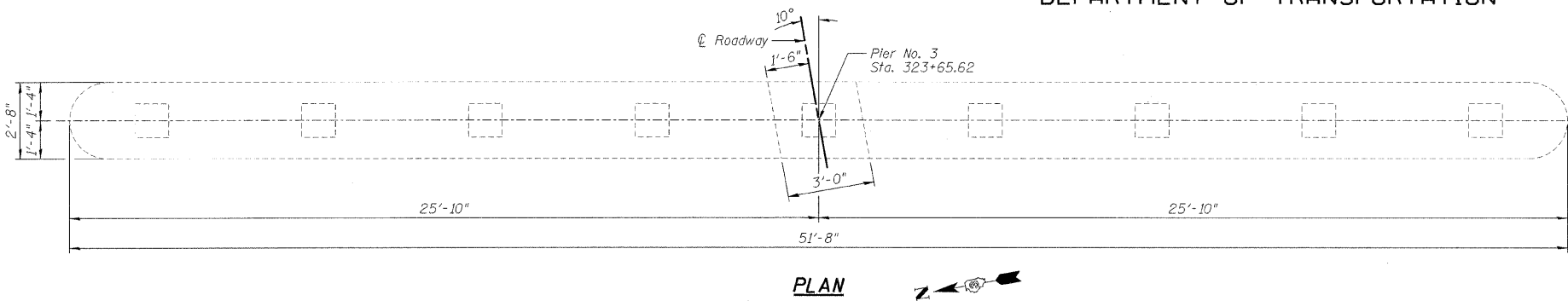
FARNSWORTH GROUP, INC.

NOTES:

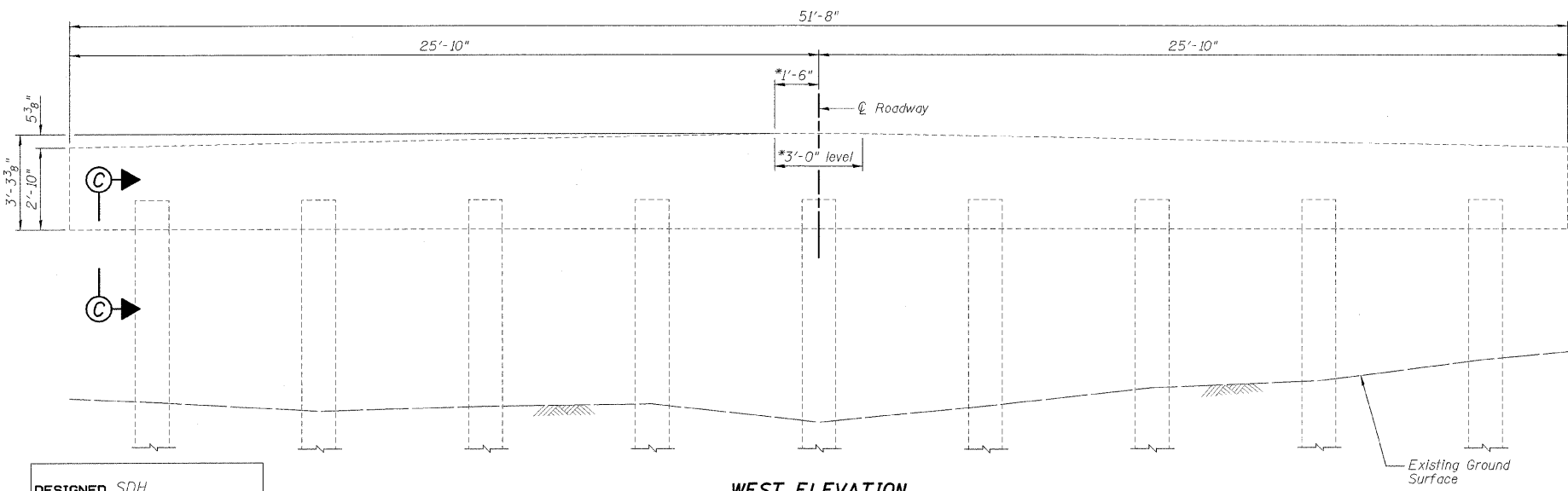
- 1.) Drill & epoxy grout v41(E) bars in appropriate drilled holes according to Section 584 of the Standard Specifications. The type of epoxy grout shall be approved by the Engineer.
- 2.) Space reinforcement in oop to miss anchor bolts.
- 3.) See Sheet B33 for Bar Splicer Details.

SHEET NO. B30	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	04	46
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



*Dimensions @ Rt. L's to Roadway.

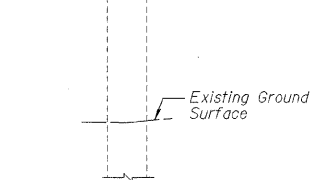
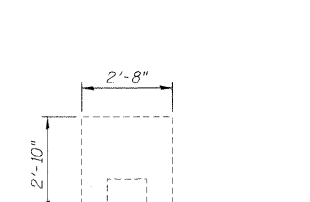


DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

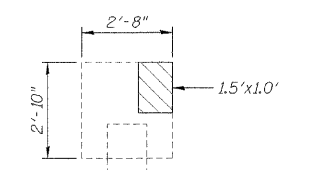
DATE 10/07/09

FARNSWORTH GROUP, INC.

CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX



NORTH END VIEW
(Looking South)



BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	3

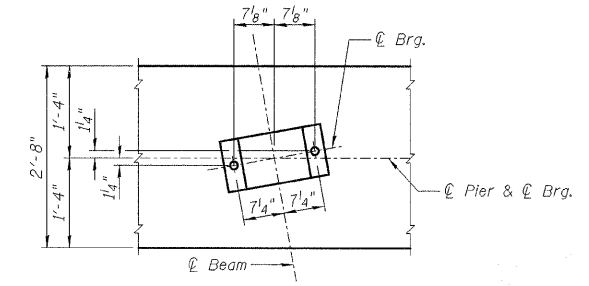
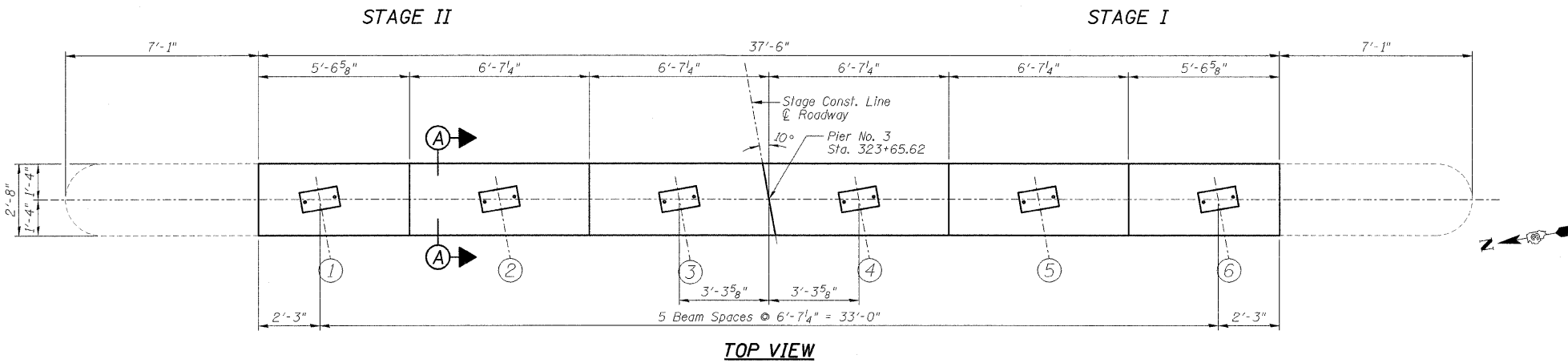
LEGEND

Structural Repair of Concrete
(Depth Equal to or Less Than 5 Inches)

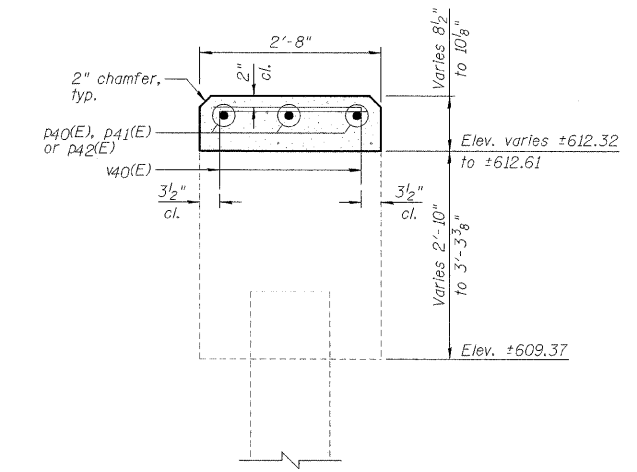
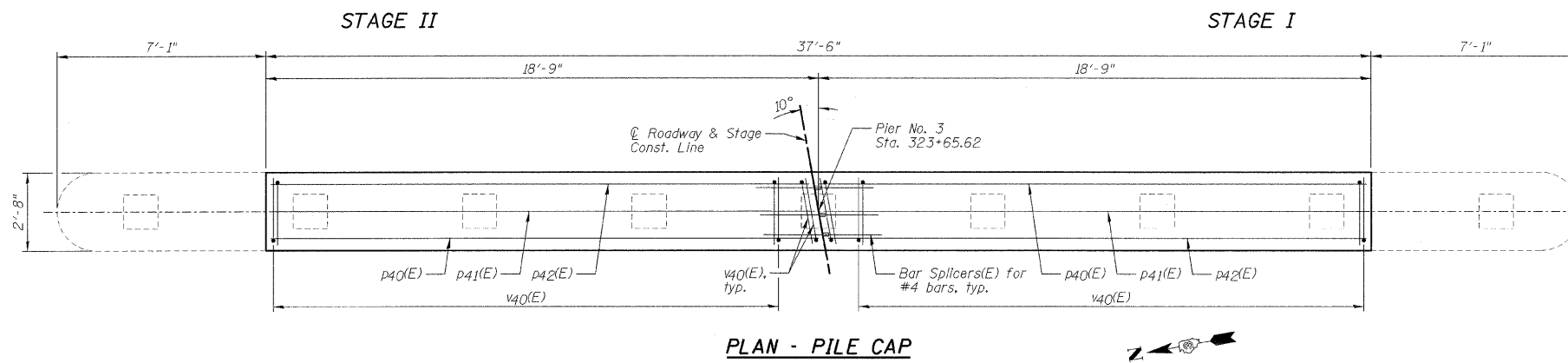
PIER NO. 3 REPAIR
STRUCTURE NO. 053-0150

SHEET NO. B31	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	64	47
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT		CONTRACT NO. 66691	

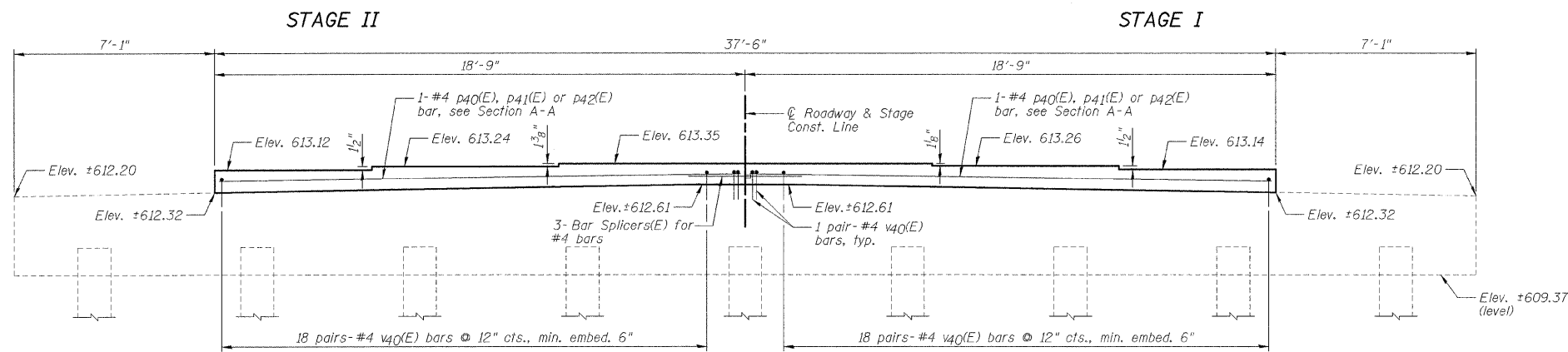
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



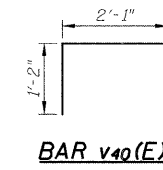
TYPICAL ANCHOR BOLT
PLACEMENT DETAIL



SECTION A-A



ELEVATION
(Looking East)



BAR v40(E)

PIER NO. 3
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
p40(E)	2	#4	18'-7"	—
p41(E)	2	#4	18'-5"	—
p42(E)	2	#4	18'-3"	—
v40(E)	76	#4	3'-3"	—
Item	Unit	Quantity		
Concrete Structures	Cu. Yd.	2.9		
Reinforcement Bars, Epoxy Coated	Pound	240		
Bar Splicers	Each	3		

PIER NO. 3
STRUCTURE NO. 053-0150

NOTES:

- 1.) Drill & epoxy grout v40(E) bars in appropriate drilled holes according to Section 584 of the Standard Specifications. The type of epoxy grout shall be approved by the Engineer.
- 2.) Space reinforcement in cap to miss anchor bolts.
- 3.) See Sheet B33 for Bar Splicer Details.

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

DATE 10/07/09

FARNSWORTH GROUP, INC.

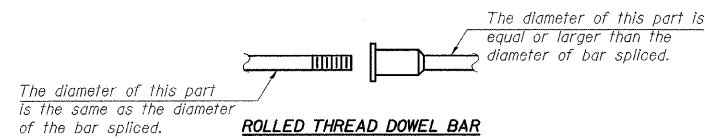
SHEET NO. B32	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	64	48
			CONTRACT NO. 66691		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.
Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length.
All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.
Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.
Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

- ① Minimum Capacity (Tension in kips) = $1.25 \times f_y \times A_1$
 - ② Minimum *Pull-out Strength (Tension in kips) = $0.66 \times f_y \times A_1$
- Where f_y = Yield strength of lapped reinforcement bars in ksi.
 A_1 = Tensile stress area of lapped reinforcement bars.
* = 28 day concrete

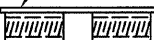


ROLLED THREAD DOWEL BAR



** ONE PIECE

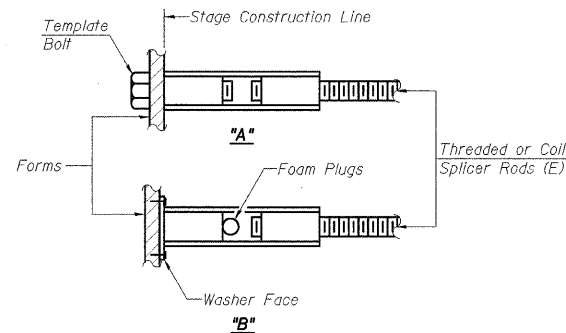
Wire Connector



WELDED SECTIONS

BAR SPLICER ASSEMBLY ALTERNATIVES

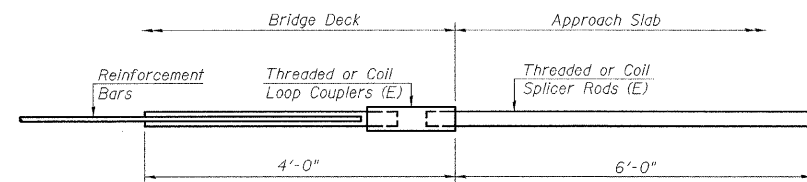
**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



INSTALLATION AND SETTING METHODS

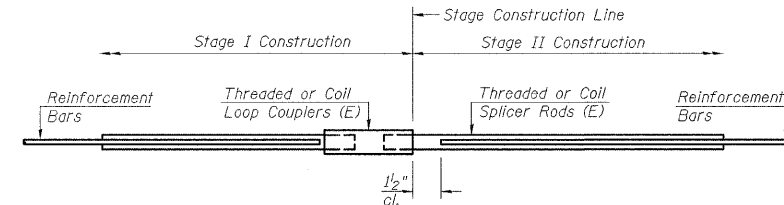
"A" : Set bar splicer assembly by means of a template bolt.
"B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
(E) : Indicates epoxy coating.

Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Strength Requirements	
		Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension
#4	1'-8"	14.7	7.9
#5	2'-2"	23.0	12.3
#6	2'-7"	33.1	17.4
#7	3'-5"	45.1	23.8
#8	4'-6"	58.9	31.3
#9	5'-9"	75.0	39.6
#10	7'-3"	95.0	50.3
#11	9'-0"	117.4	61.8



FOR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required = 80



STANDARD

Bar Size	No. Assemblies Required	Location
#5	357	Top of Deck
#5	215	Bottom of Deck
#4	25	West Approach
#5	86	West Approach
#4	25	East Approach
#5	86	East Approach
#6	24	Diaphragms
#4	4	West Abutment
#4	4	East Abutment
#4	3	Pier 1
#4	3	Pier 2
#4	3	Pier 3

DESIGNED SDH
CHECKED JML
DRAWN JWK/DJM
CHECKED MSW

DATE 10/07/09

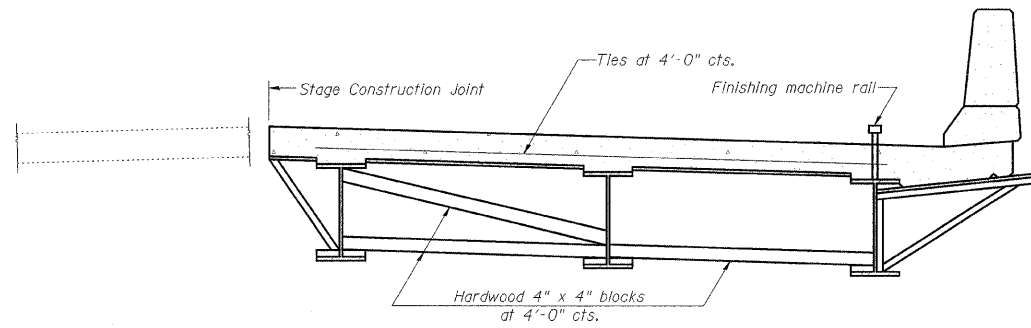
FARNSWORTH GROUP, INC.

CONSULTING ENGINEERS - 2709 MCGRAW DRIVE BLOOMINGTON, ILLINOIS 61704 (309) 663-8435 / (309) 663-1571 FAX

BAR SPLICER ASSEMBLY DETAILS
STRUCTURE NO. 053-0150

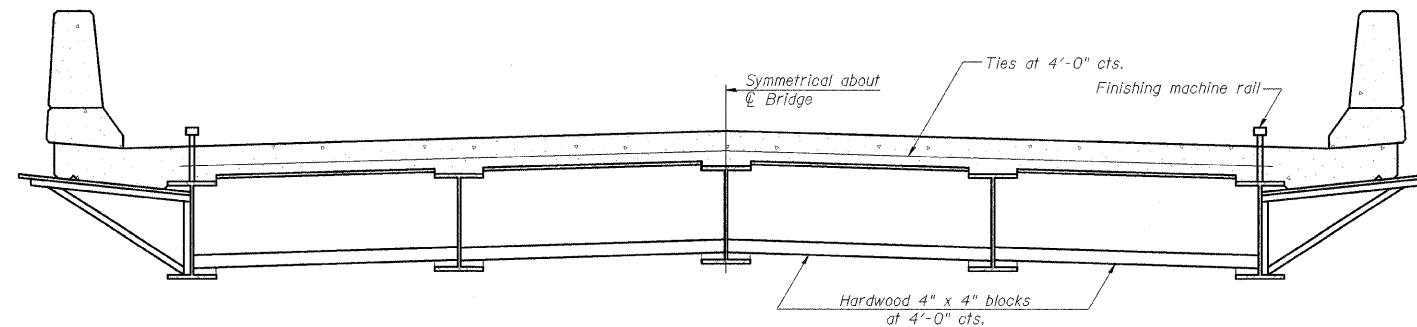
SHEET NO. B33	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
34 SHEETS	41	15BR-2	LIVINGSTON	44	49
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT		CONTRACT NO. 66691	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



**FORM BRACES FOR
STAGE CONSTRUCTION**

When cantilever forming brackets are used, the work shall be done according to Article 503.06(b) of the Standard Specifications, except as modified below and in the details shown on this sheet.
The finishing machine rails shall be placed on the top flange of the exterior beams.
The beams or girders, supporting cantilever forming brackets, shall be tied together at 4 foot intervals.
For Standard construction, or Stage Construction the Hardwood bracing materials shall be placed as shown between webs of beams in each bay.



**FORM BRACES FOR
STANDARD CONSTRUCTION**

DESIGNED	SDH
CHECKED	JML
DRAWN	JWK/DJM
CHECKED	MSW

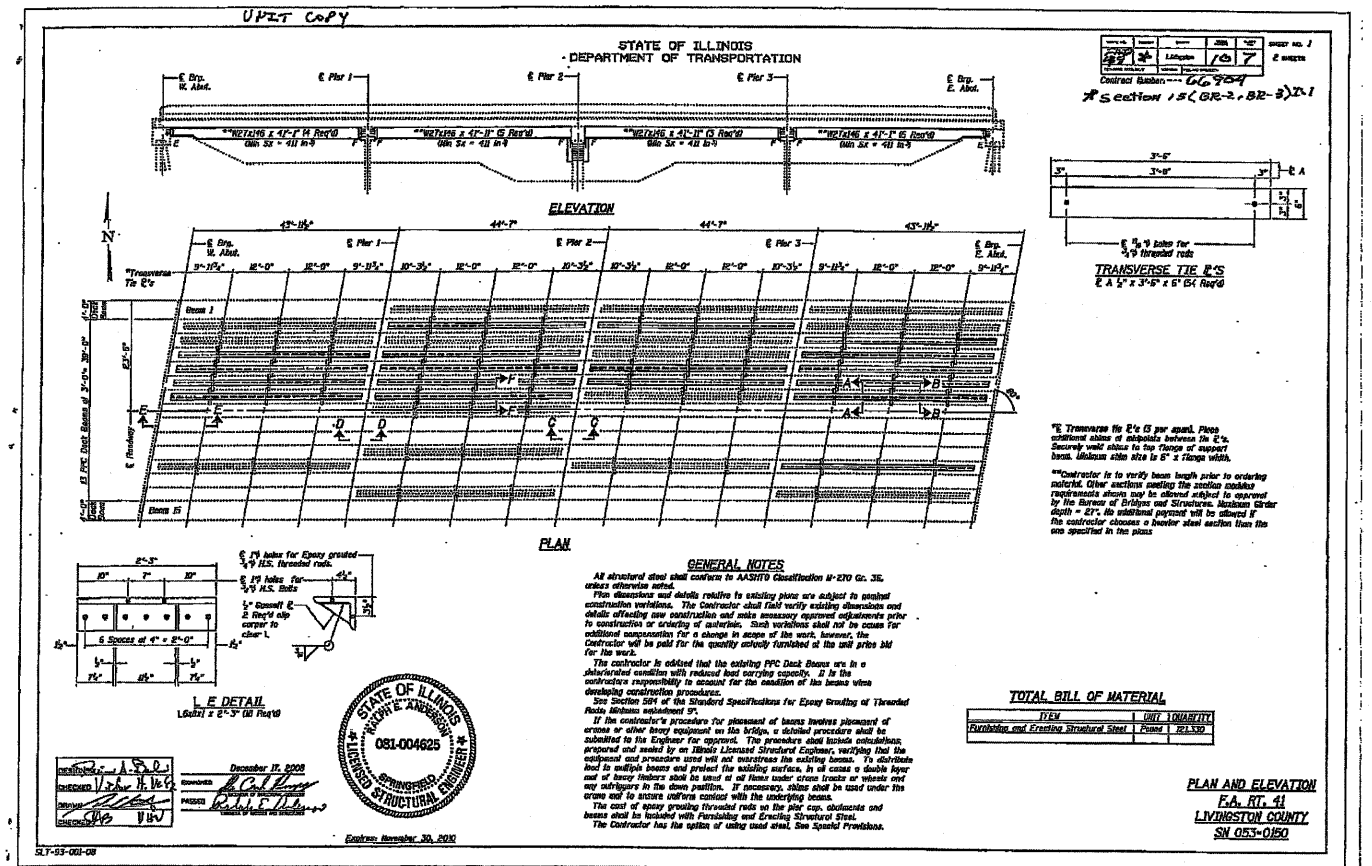
SB-1

10-1-08

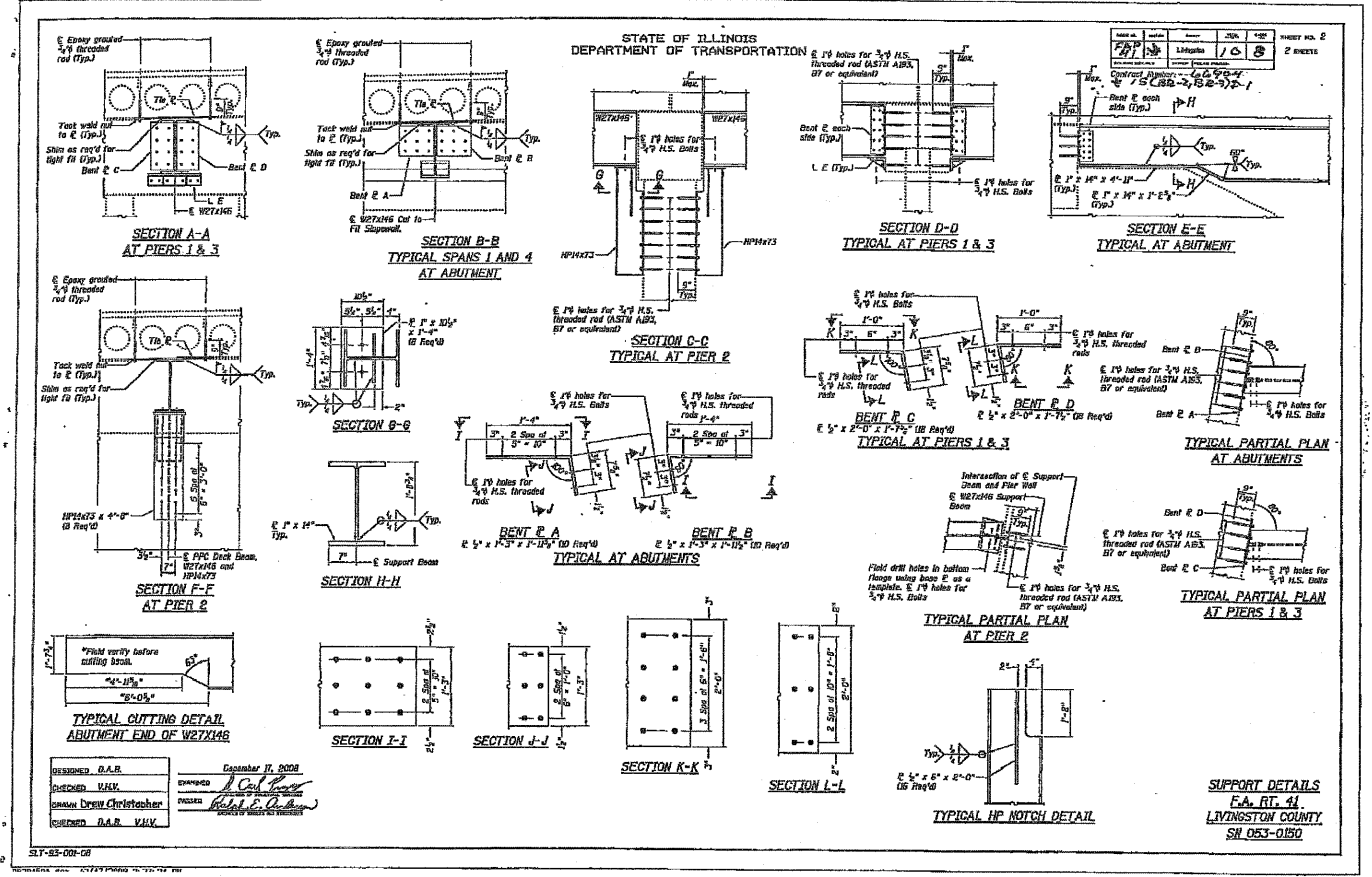
DATE 10/07/09

**CANTILEVER FORMING BRACKETS
FOR SUPERSTRUCTURES WITH
W27 BEAMS AND SMALLER
STRUCTURE NO. 053-0150**

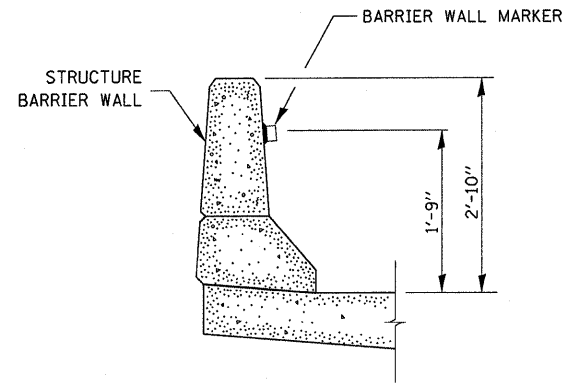
SHEET NO. B34	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	41	15BR-2	LIVINGSTON	64	50
34 SHEETS	CONTRACT NO. 66691				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT			



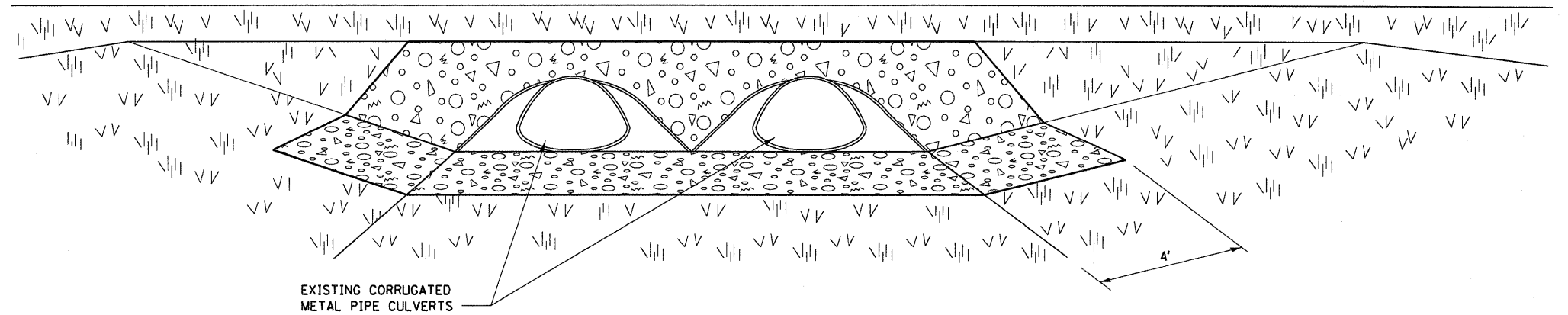
FOR INFORMATION ONLY



FILE NAME =	USER NAME = schwankerg	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	EXISTING BRIDGE REPAIR PLANS SUPPORT DETAILS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
ci:\pw\work\p\dot\schwankerg\dms73146	at Bridge Plans-tf.dgn	DRAWN -	REVISED -			41	15 BR-2	LIVINGSTON	64	53	
PLOT SCALE = 50.0000' / IN.		CHECKED -	REVISED -			CONTRACT NO. 66691					
PLOT DATE = Oct 16, 2009 - 03:13:14 PM		DATE -	REVISED -			ILLINOIS FED. AID PROJECT					



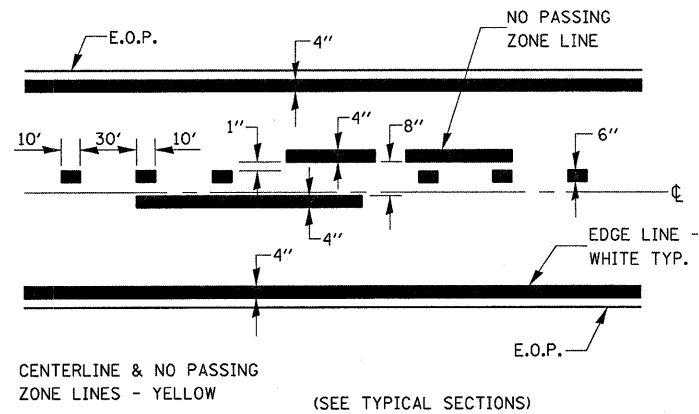
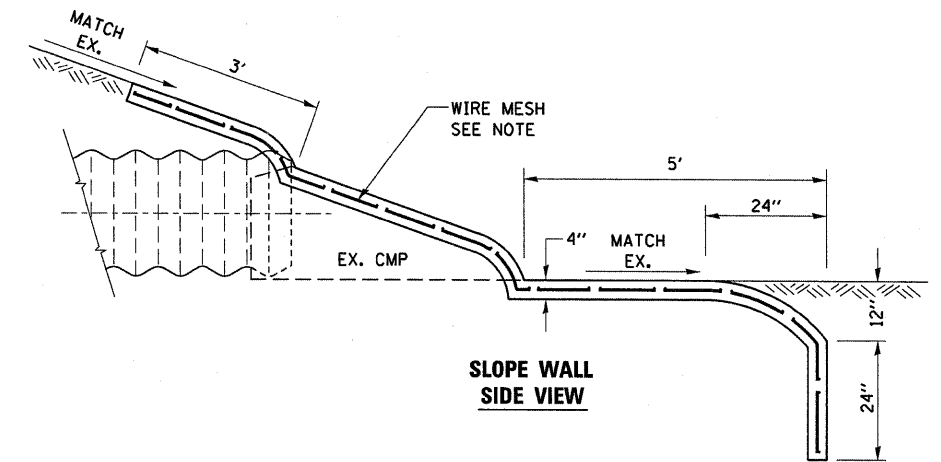
BARRIER WALL MARKER



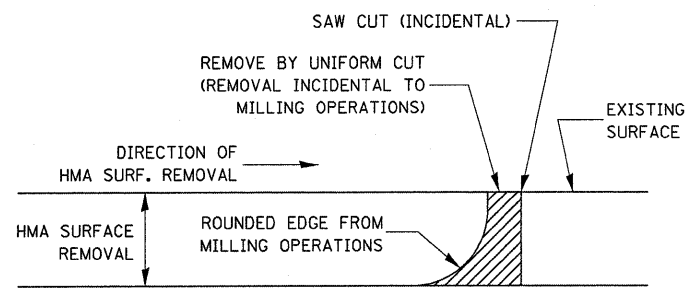
SLOPE WALL DETAIL

PLACED BOTH SIDE OF PRIVATE ENTRANCE
STA. 319+53.90

1. CONCRETE FOR SLOPE WALLS SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC YARD FOR MISCELLANEOUS CONCRETE.
2. COST OF FURNISHING AND INSTALLING WIRE MESH SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE PER CUBIC YARD FOR MISCELLANEOUS CONCRETE. WIRE MESH TO WEIGH NOT LESS THAN 58# PER 100 SQ. FT.
3. EXCAVATION FOR SLOPE WALLS WILL BE INCLUDED IN THE CONTRACT UNIT PRICE PER CUBIC YARD FOR MISCELLANEOUS CONCRETE.

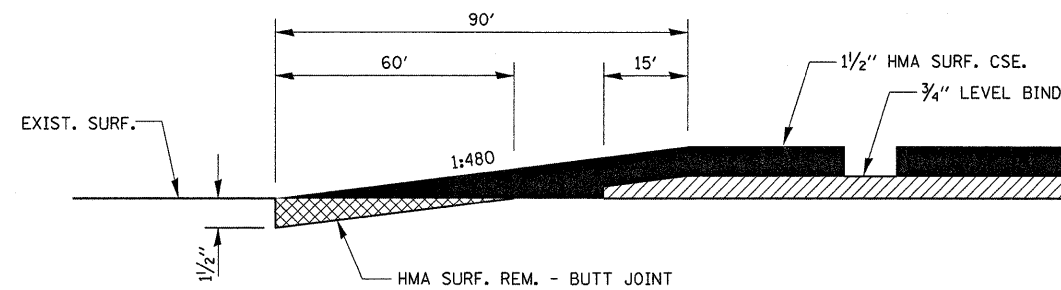


PAVEMENT MARKING

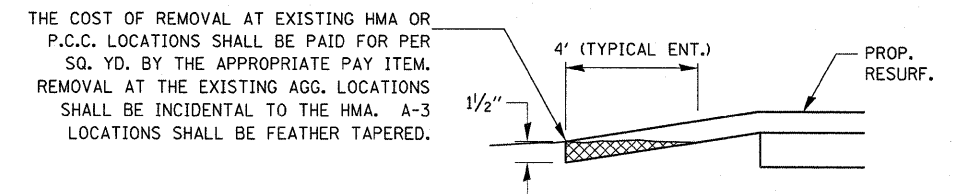


NOTE:
WHEN MILLING OPERATIONS PRODUCE A ROUNDED EDGE, THEN A SAW CUT SHALL BE USED TO MANUFACTURE A PERPENDICULAR EDGE AS SHOWN IN THE DETAIL. THE ENGINEER SHALL BE THE SOLE JUDGE CONCERNING THE USE OF THIS DETAIL

HMA DETAIL AT BUTT JOINTS



DETAIL AT BUTT JOINT



DETAIL AT ENTRANCES

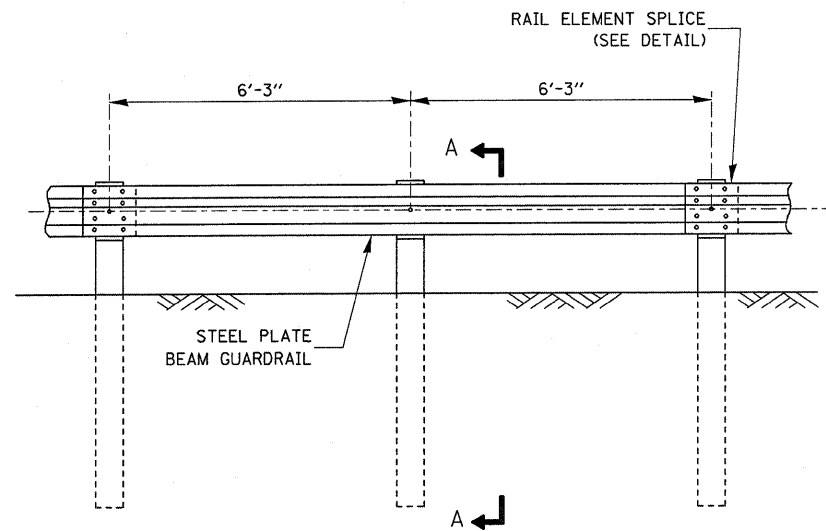
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PLOT DATE = Oct 16, 2009 - 03:00:15 PM		DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILS

SCALE: SHEET NO. OF SHEETS STA. TO STA.

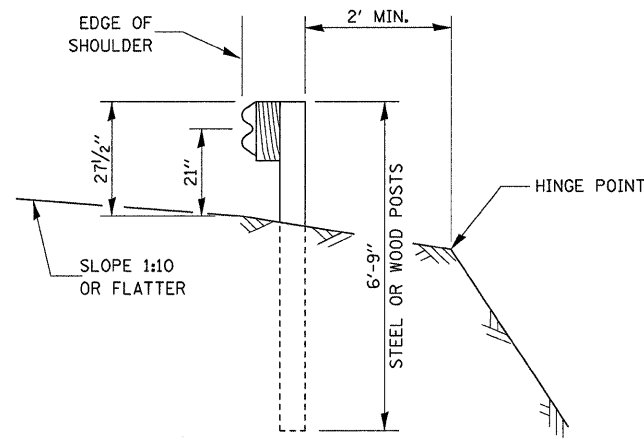
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41	15BR-2	LIVINGSTON	64	54
CONTRACT NO. 66691			ILLINOIS FED. AID PROJECT	



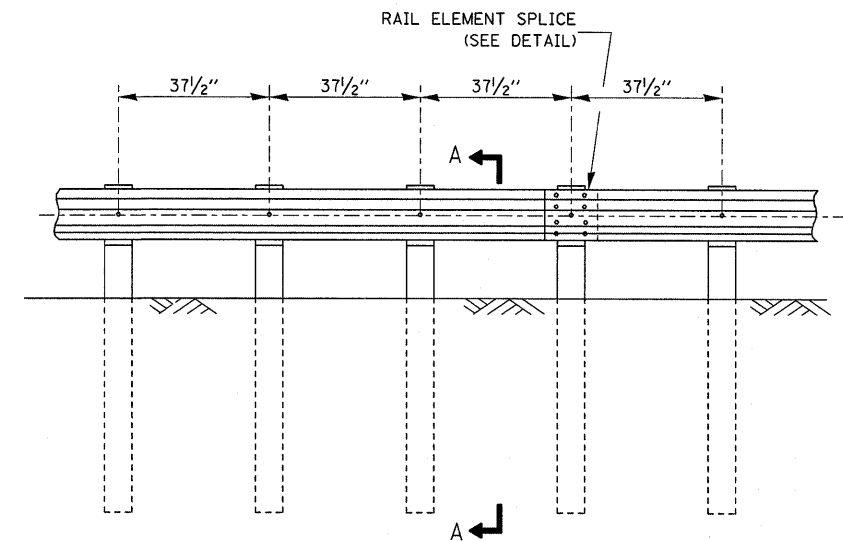
ELEVATION

TYPE A

6'-3" TYPICAL POST SPACING



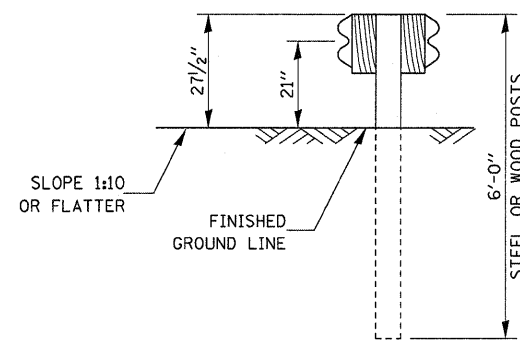
SECTION A-A



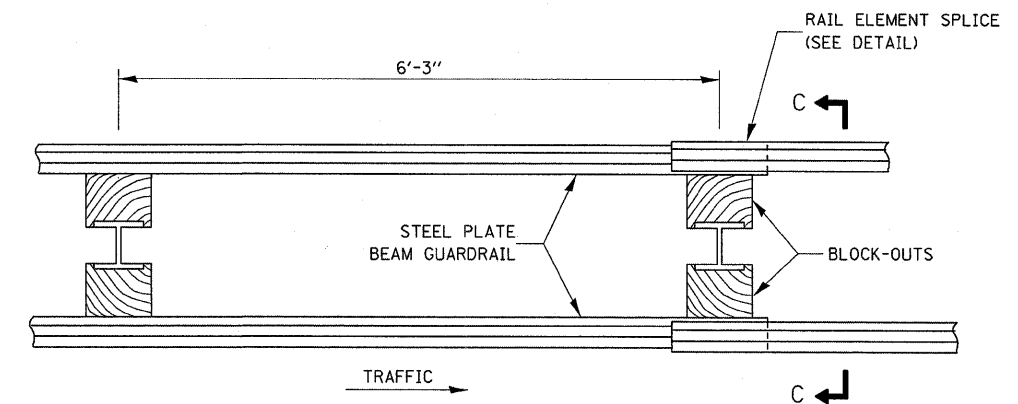
ELEVATION

TYPE B

37 1/2" CLOSED POST SPACING



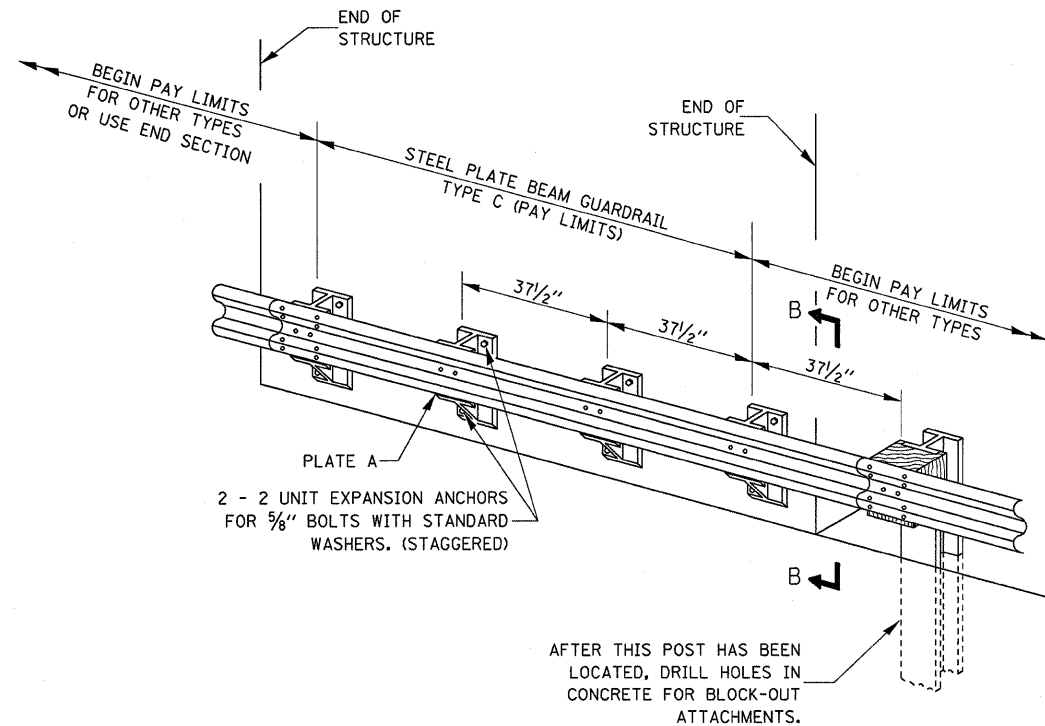
SECTION C-C



PLAN

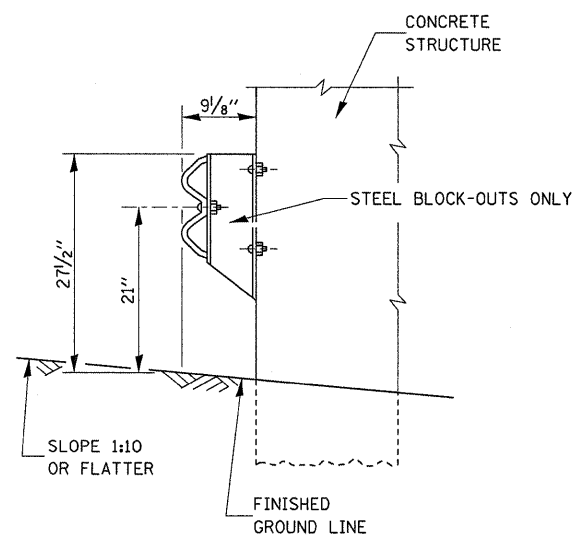
TYPE D

DOUBLE STEEL PLARE BEAM GUARDRAIL
6'-3" TYPICAL POST SPACING



TYPE C

37 1/2" BLOCK-OUT SPACING

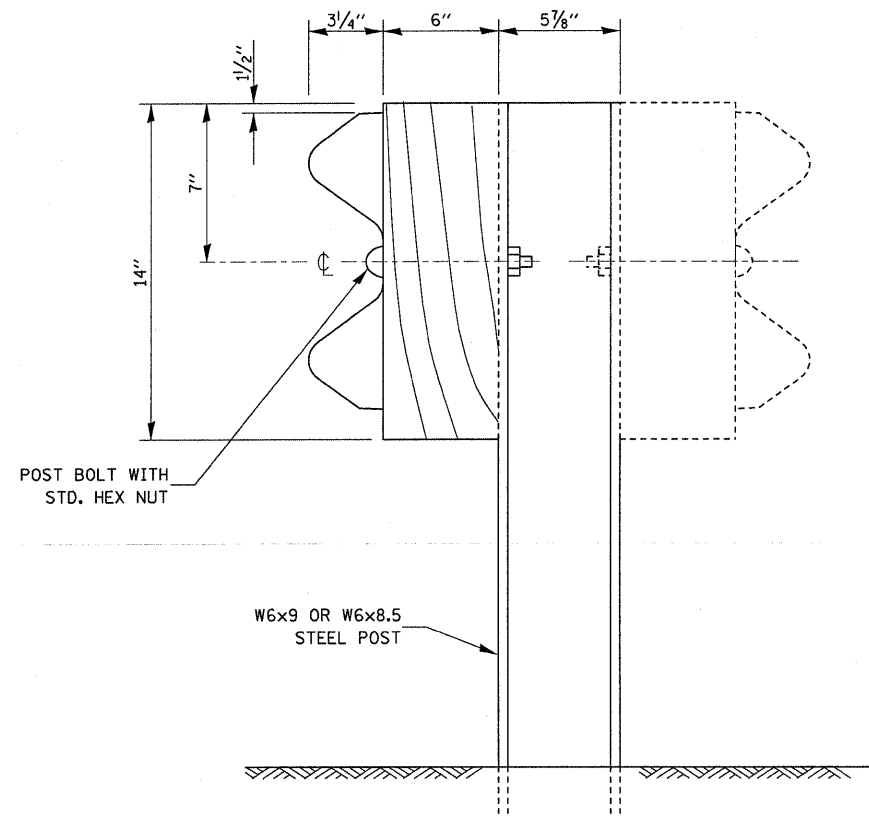


SECTION B-B

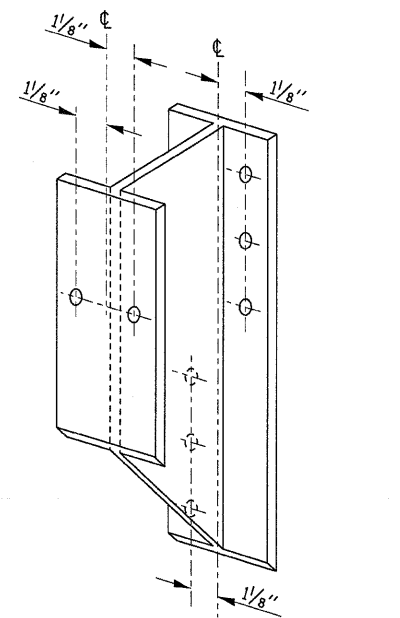
GENERAL NOTES

- ALL SLOPE RATIOS ARE EXPRESSED AS UNITS OF VERTICAL DISPLACEMENT TO UNITS OF HORIZONTAL DISPLACEMENT (V:H).
- THE EXISTING STEEL POSTS MAY BE DRILLED TO MATCH THE BOLT PATTERN SHOWN HEREIN FOR THE WOOD BLOCK-OUTS, OR A NEW STEEL POST SHALL BE PROVIDED.
- THIS DETAIL IS APPLICABLE TO THE GUARDRAIL SYSTEM USED PRIOR TO JANUARY 1, 2007. FOR DETAILS ON THE MIDWEST GUARDRAIL SYSTEM SEE STANDARD 630001.

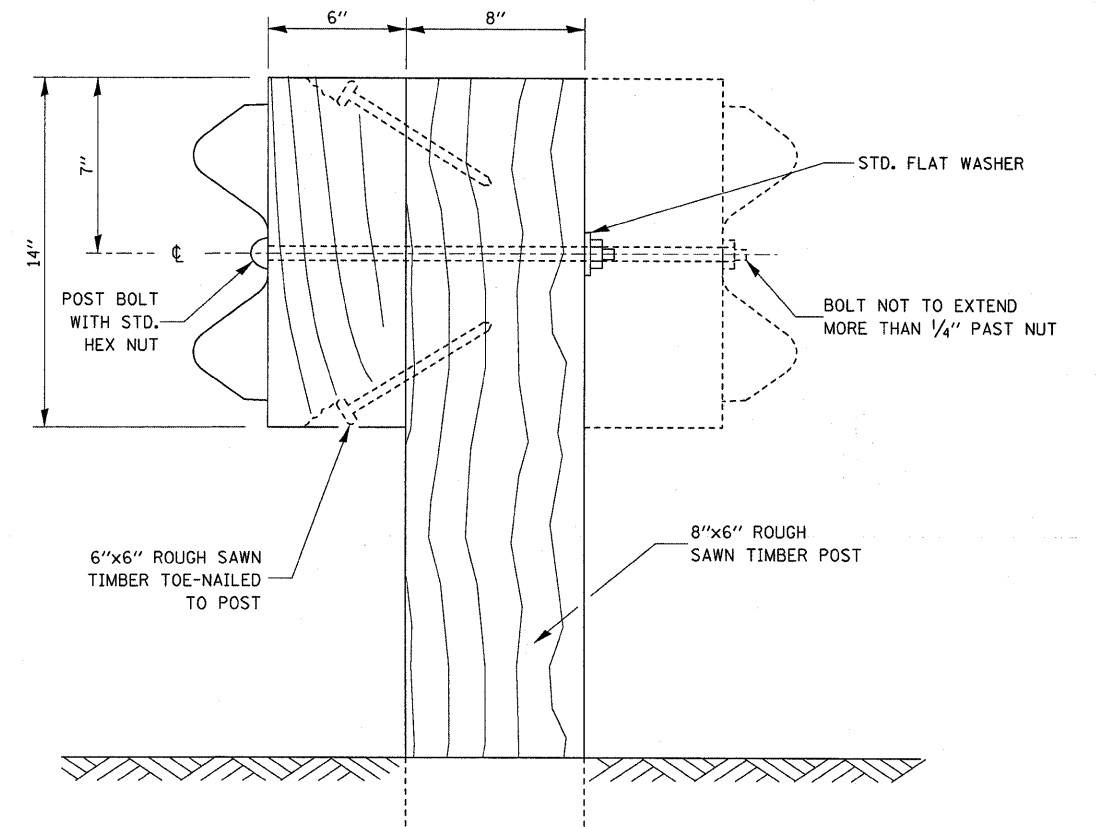
FILE NAME =	USER NAME = schwanke	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	REMOVE AND REERECT STEEL PLATE BEAM GUARDRAIL	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
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PLOT DATE = Oct 16, 2009 - 03:05:43 PM		DATE -	REVISED -			FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT					
				SCALE:		SHEET NO. 1 OF 4 SHEETS		STA.		TO STA.	



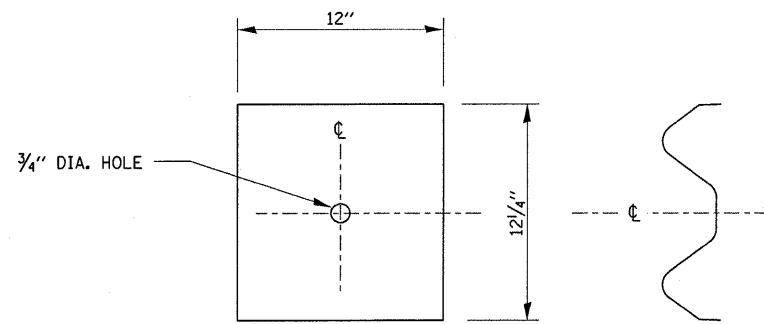
STEEL POST CONSTRUCTION



STEEL BLOCK-OUT DETAIL

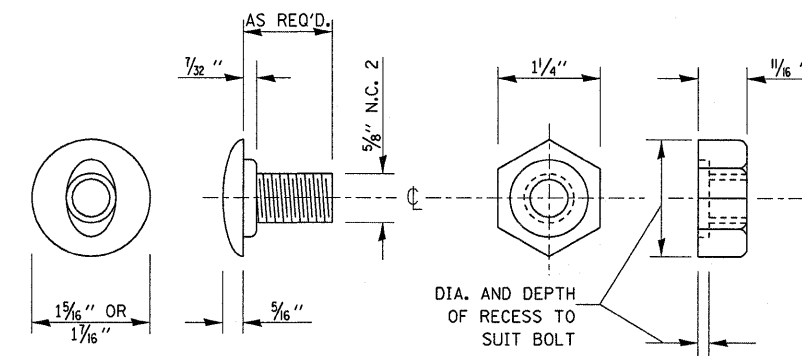


WOOD POST CONSTRUCTION



NOTE:
 PLATE A SHALL BE PLACED BETWEEN RAIL
 ELEMENT AND BLOCK-OUT AT NON-SPLICE
 MOUNTING POINTS ONLY WHEN STEEL
 BLOCK-OUTS ARE USED.

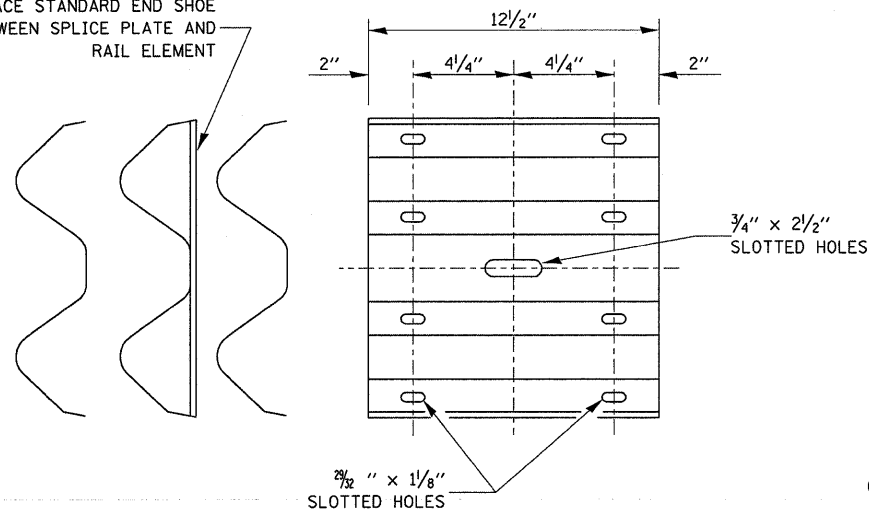
PLATE A



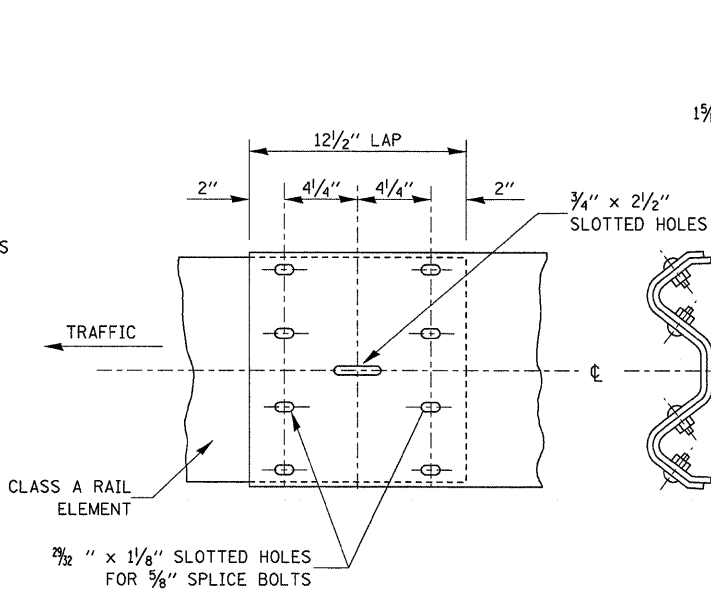
POST OR SPLICE BOLT & NUT

FILE NAME =	USER NAME = schworkerg	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	REMOVE AND REERECT STEEL PLATE BEAM GUARDRAIL	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
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		DATE -	REVISED -			SCALE:	SHEET NO. 2 OF 4 SHEETS	STA.	TO STA.	FED. ROAD DIST. NO.	ILLINOIS FED. AID PROJECT

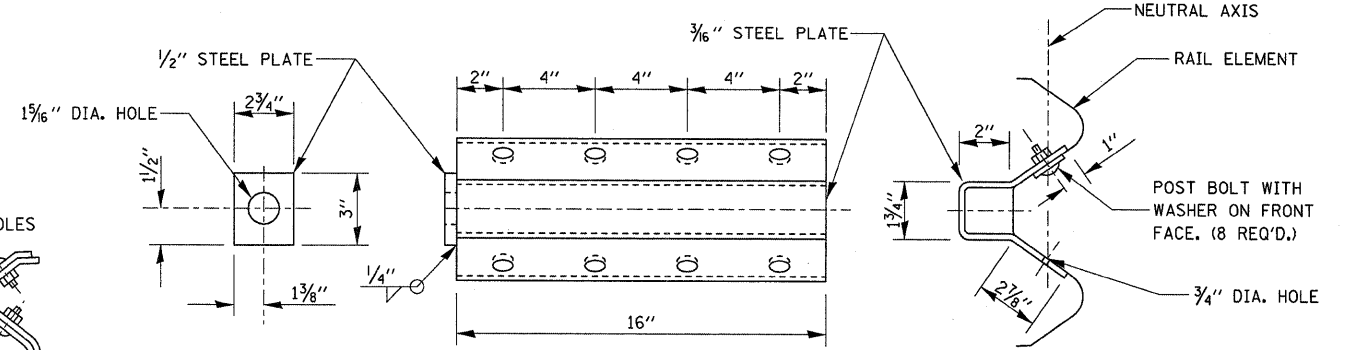
PLACE STANDARD END SHOE BETWEEN SPLICE PLATE AND RAIL ELEMENT



SPLICE PLATE

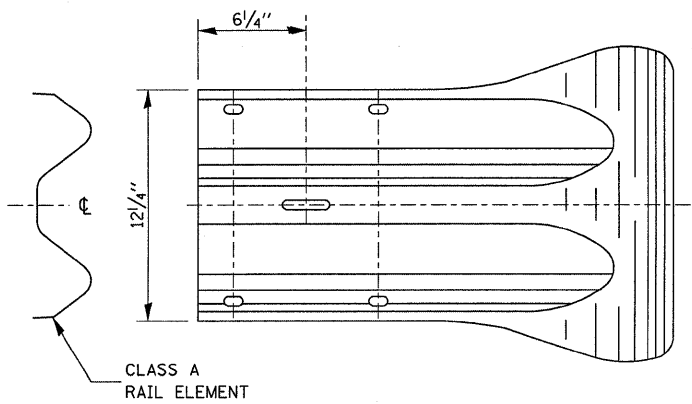
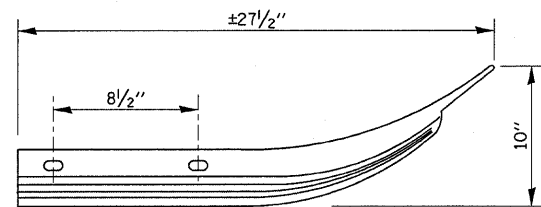


RAIL ELEMENT SPLICE

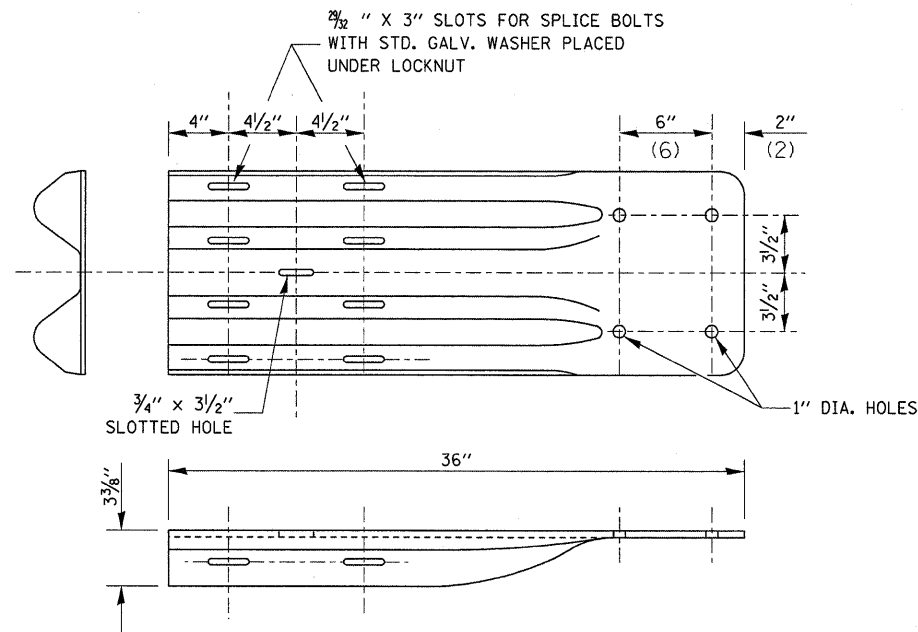


NOTE:
ANCHOR PLATE T SHALL BE USED TO ATTACH CABLE ASSEMBLY TO GUARDRAIL WHEN REQUIRED ON TRAFFIC BARRIER TERMINALS.

ANCHOR PLATE T DETAILS



END SECTION

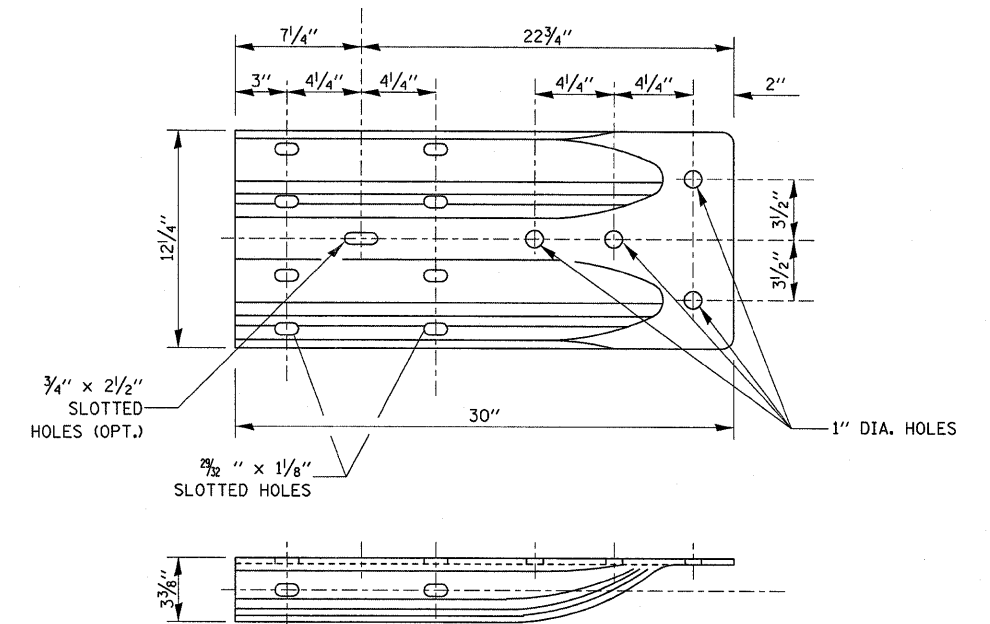


NOTE:
WHEN END SHOE IS ATTACHED TO A BRIDGE PARAPET WHICH HAS AN EXPANSION JOINT, THE BOLTS SHALL BE PROVIDED WITH A LOCKNUT OR DOUBLE NUT AND SHALL BE TIGHTENED ONLY TO A POINT THAT WILL ALLOW GUARDRAIL MOVEMENT.

THE STANDARD END SHOE SHALL BE ATTACHED TO THE CONCRETE WITH PRE-DRILLED OR SELF-DRILLING ANCHOR BOLTS. THE ANCHOR CONE SHALL BE SET FLUSH WITH THE SURFACE OF THE CONCRETE.

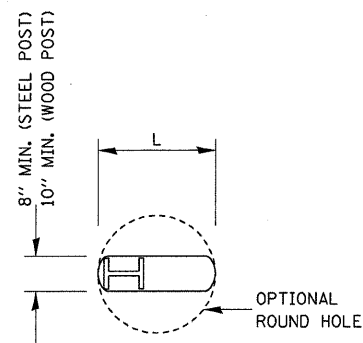
EXTERNALLY THREADED STUDS PROTRUDING FROM THE SURFACE OF THE CONCRETE WILL NOT BE PERMITTED.

END SHOE

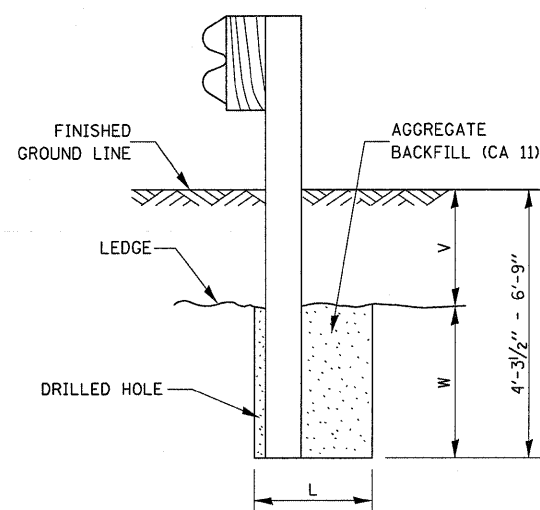


ALTERNATE END SHOE

FILE NAME =	USER NAME = schwankerg	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	REMOVE AND REERECT STEEL PLATE BEAM GUARDRAIL	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
ci:\pw\work\p1dot1\schwankerg\dms73146\d	tei1a.dgn	DRAWN -	REVISED -			41	15BR-2	LIVINGSTON	64	57	
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	PLOT DATE = Oct 16, 2009 - 03:00:36 PM	DATE -	REVISED -			FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT					



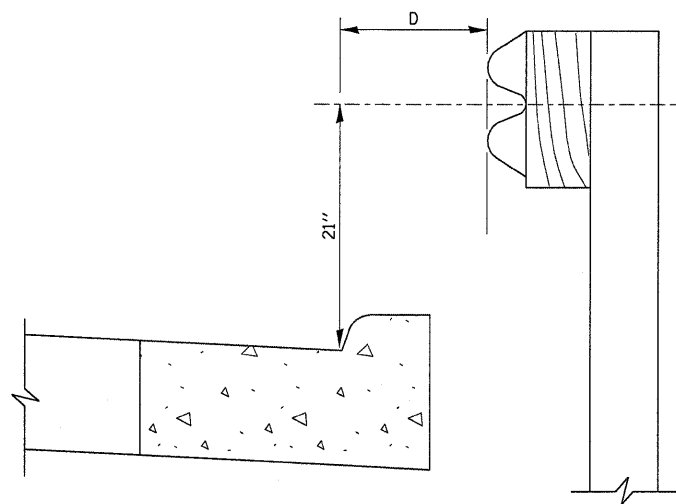
PLAN



NOTE:
LEDGE LINE IS TOP OF ROCK LEDGE OR HARD SLAG FILL.

ELEVATION

FOOTING FOR POST WHEN IMPERVIOUS MATERIAL IS ENCOUNTERED

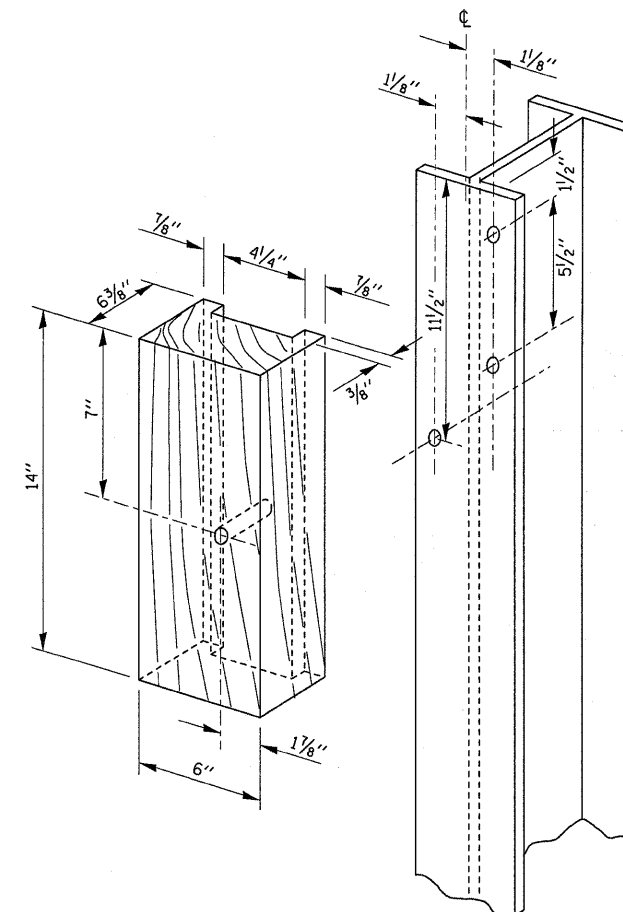


NOTE:
IF IT IS NECESSARY FOR D TO BE MORE THAN 12" AND LESS THAN 10'-0" TYPE M-2 CURB AND GUTTER (STD. 606001) SHALL BE USED IN FRONT OF AND IN ADVANCE OF THE GUARDRAIL.

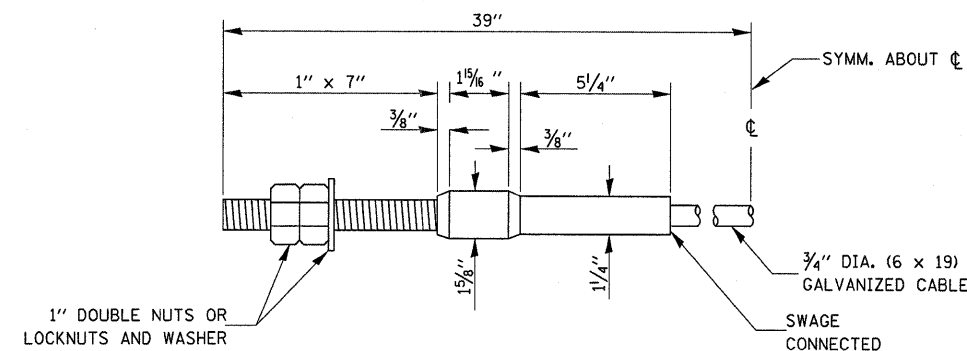
GUARDRAIL PLACED BEHIND CURB

(D = 0 DESIRABLE TO 12" MAXIMUM)

V	W	L	
		STEEL POST	WOOD POST
0" - 18"	24"	21"	23"
>18" - 41 1/2"	12"	8"	10"
>41 1/2" - 53 1/2"	12" - 0"	8"	10"



WOOD BLOCK-OUT AND STEEL POST DETAILS



CABLE ASSEMBLY

40,000 LBS. MIN. BREAKING STRENGTH
TIGHTEN TO TAUT TENSION.

FILE NAME =	USER NAME = schwankeg	DESIGNED -	REVISED -
ci:\pw_work\pwidot\schwankeg\dms73146\deto1a.dgn		DRAWN -	REVISED -
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

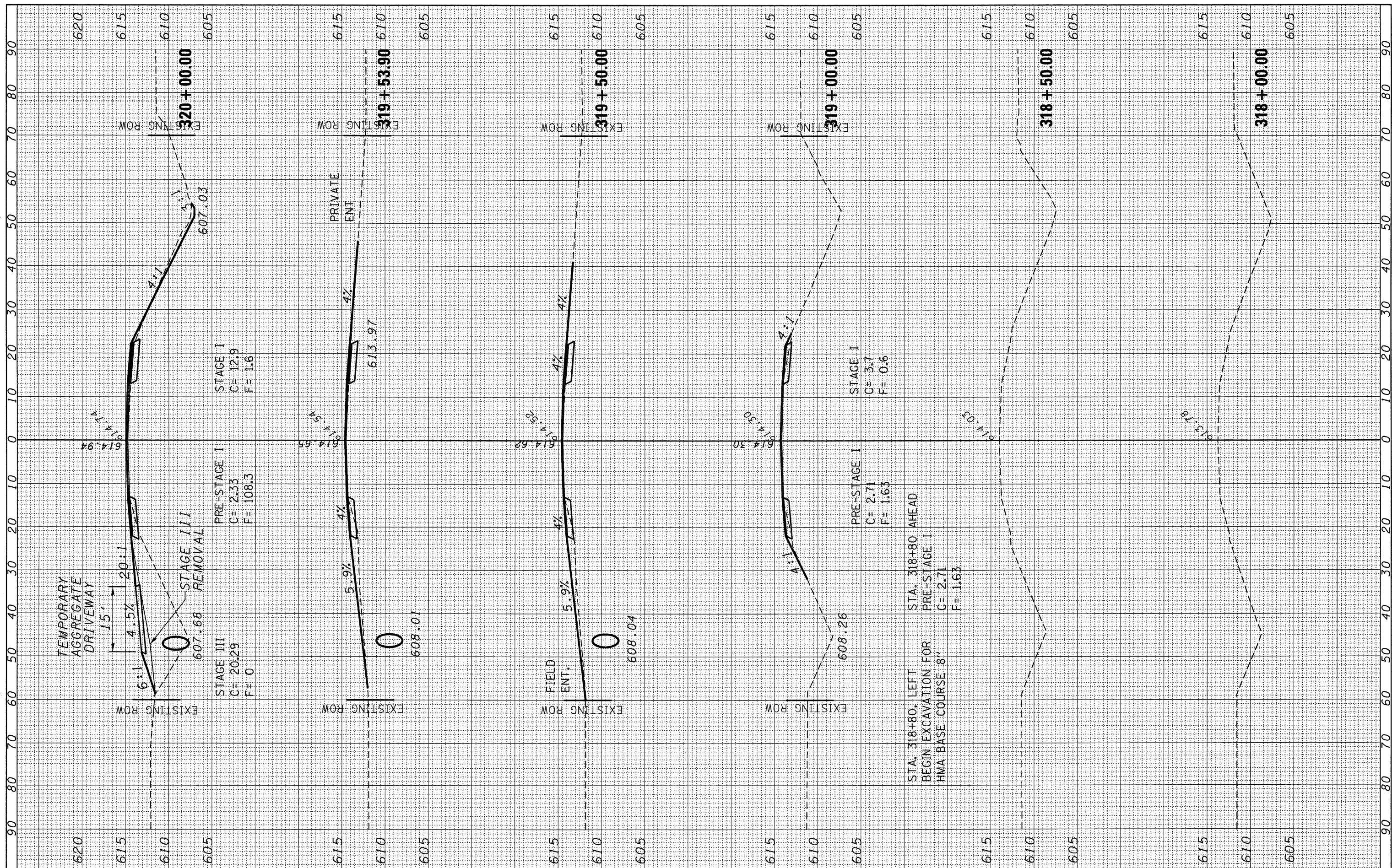
REMOVE AND REERECT STEEL PLATE BEAM GUARDRAIL

SCALE: SHEET NO. 4 OF 4 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
41	15BR-2	LIVINGSTON	64	58
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 66691	

FINAL SURVEY	SURVEYED	BY	DATE
NOTE BOOK NO.	PLOTTED		
AREAS CHECKED	REVISIONS		
	AREAS CHECKED		

ORIGINAL SURVEY	SURVEYED	BY	DATE
NOTE BOOK NO.	PLOTTED		
AREAS CHECKED	REVISIONS		
	AREAS CHECKED		



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 DESIGNED -
 DRAWN -
 CHECKED -
 DATE -

REVISIONS
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PLOT SCALE = 10.0000' / IN.
 PLOT DATE = Oct 16, 2009 - 03:23:03 PM

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

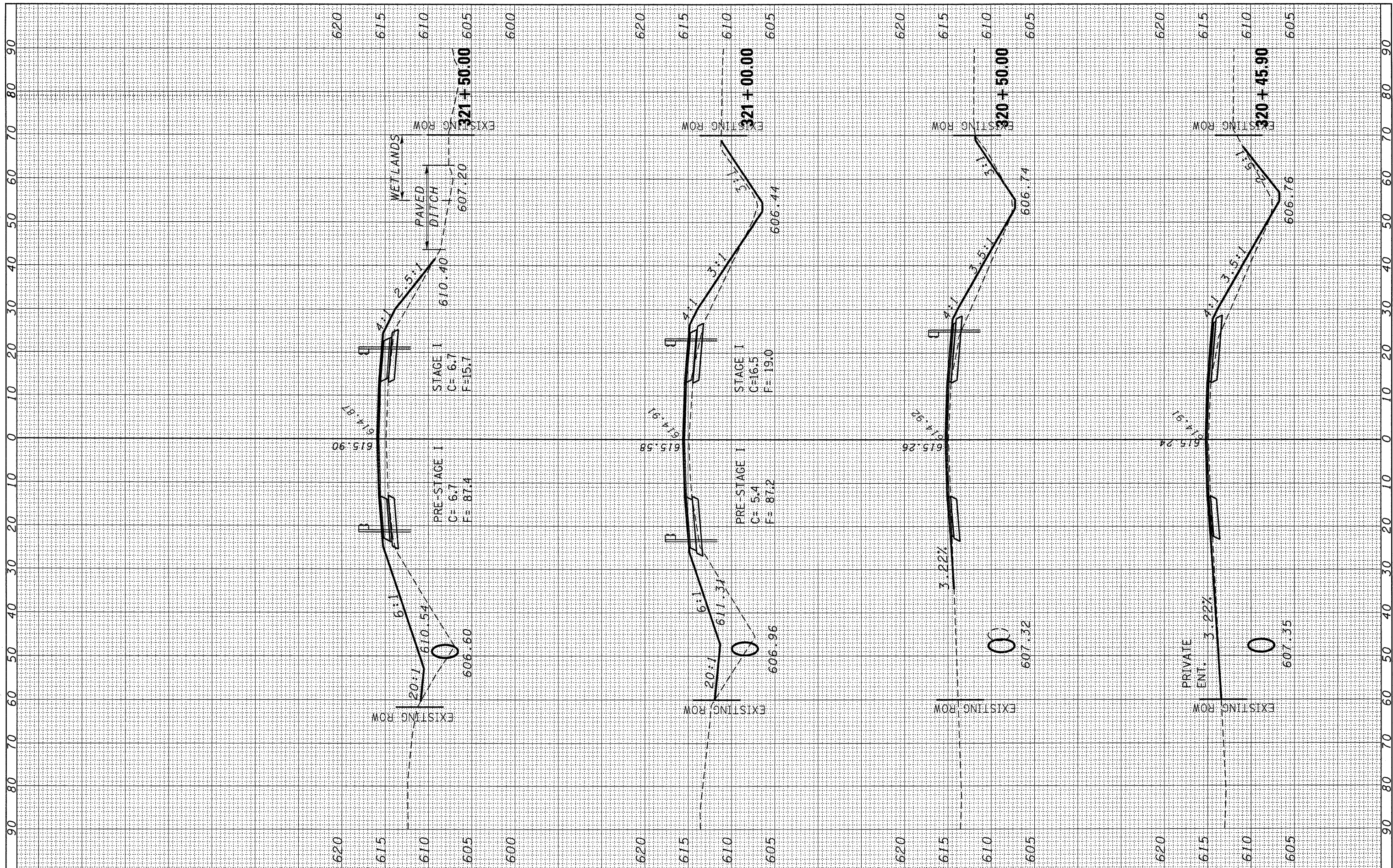
CROSS SECTIONS

SCALE: SHEET NO. OF SHEETS STA. 318+00.00 TO STA. 320+00.00

F.A.P. RTE. 41	SECTION 15 BR-2	COUNTY LIVINGSTON	TOTAL SHEETS 64	SHEET NO. 59
CONTRACT NO. 66691				
FED. ROAD DIST. NO. [ILLINOIS] FED. AID PROJECT				

FINAL SURVEY	BY	DATE
SURVEYED		
PLOTTED		
NOTE BOOK		
AREAS CHECKED		
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ORIGINAL SURVEY	BY	DATE
SURVEYED		
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NOTE BOOK		
AREAS CHECKED		
NO.		



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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

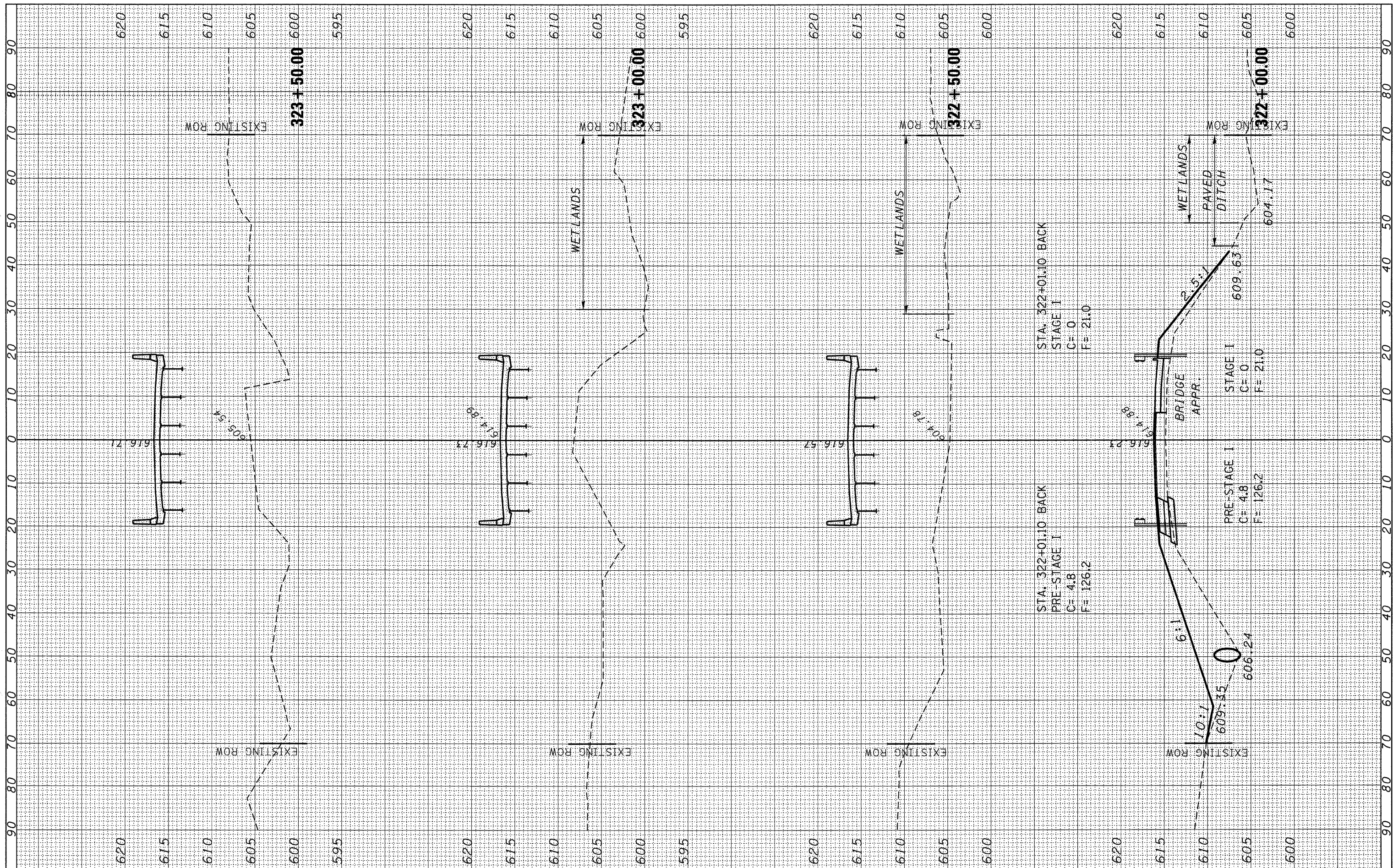
CROSS SECTIONS

SCALE: SHEET NO. OF SHEETS STA. 320+45.90 TO STA. 321+50.00

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
41	15 BR-2	LIVINGSTON	64	60
FED. ROAD DIST. NO. [ILLINOIS] FED. AID PROJECT			CONTRACT NO. 66691	

FINAL SURVEY	SURVEYED	BY	DATE
NOTE BOOK NO.	PLOTTED		
AREAS CHECKED	DATE		

ORIGINAL SURVEY	SURVEYED	BY	DATE
NOTE BOOK NO.	PLOTTED		
AREAS CHECKED	DATE		



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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

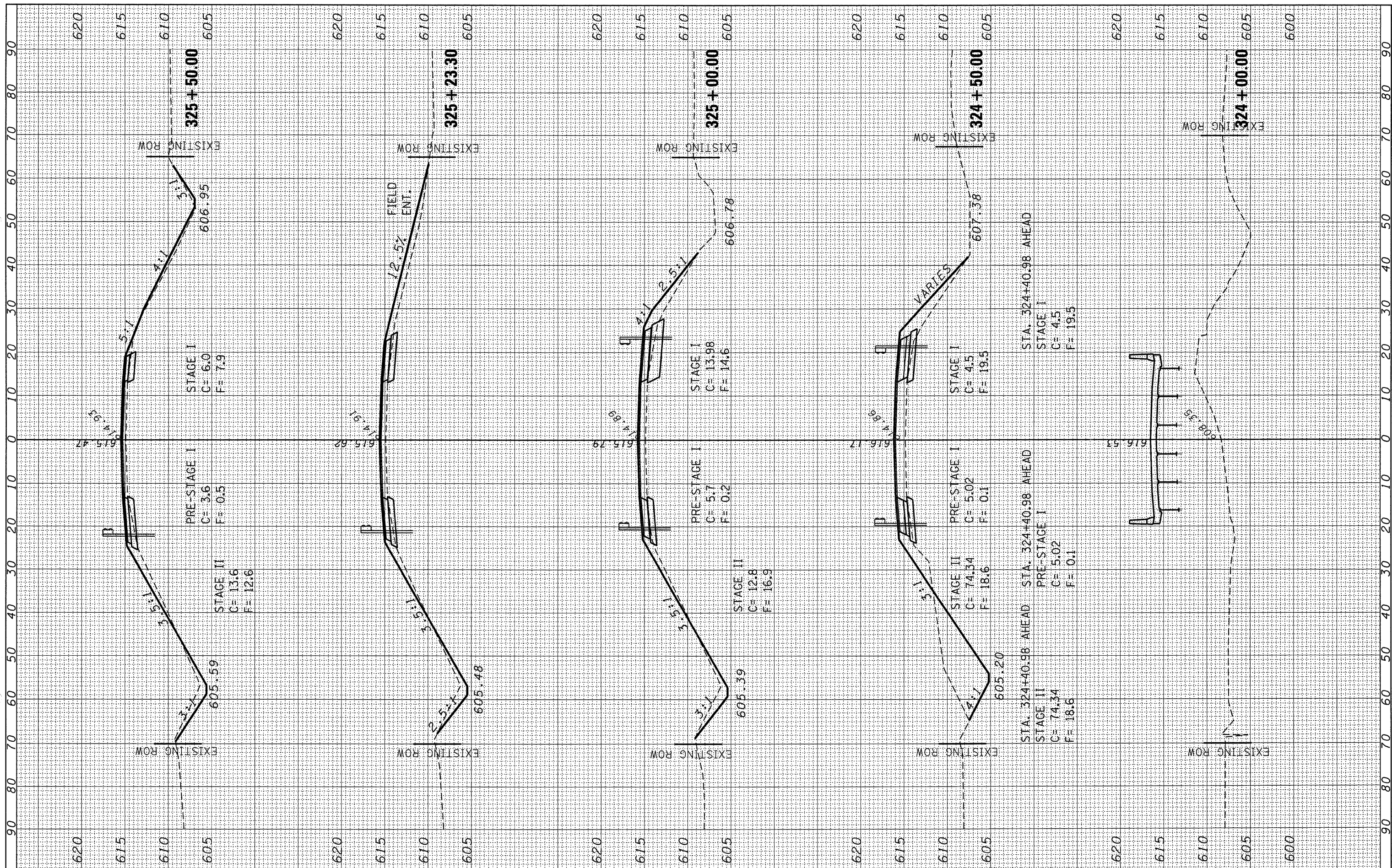
CROSS SECTIONS

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FED. ROAD DIST. NO. [ILLINOIS] FED. AID PROJECT			CONTRACT NO. 66691	

FINAL SURVEY	BY	DATE
SURVEYED		
PLOTTED		
DATE		
NO. OF SHEETS		
AREAS CHECKED		
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ORIGINAL SURVEY	BY	DATE
SURVEYED		
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

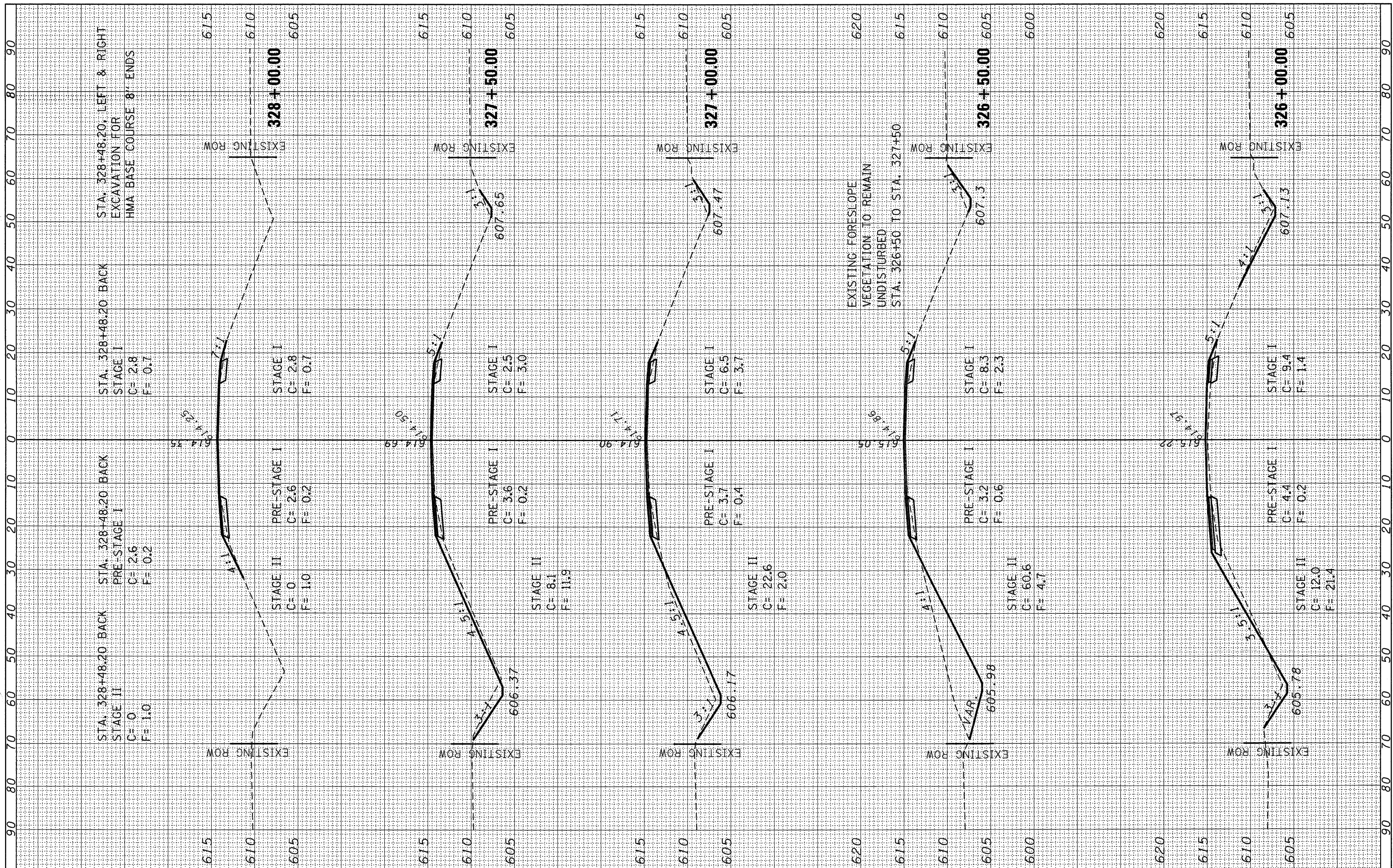
CROSS SECTIONS

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F.A.P. RTE. 41	SECTION 15 BR-2	COUNTY LIVINGSTON	TOTAL SHEETS 64	SHEET NO. 62
CONTRACT NO. 66691				
FED. ROAD DIST. NO. [ILLINOIS] FED. AID PROJECT				

FINAL SURVEY	BY	DATE
SURVEYED		
PLOTTED		
NOTE BOOK		
AREAS CHECKED		
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ORIGINAL SURVEY	BY	DATE
SURVEYED		
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NOTE BOOK		
AREAS CHECKED		
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

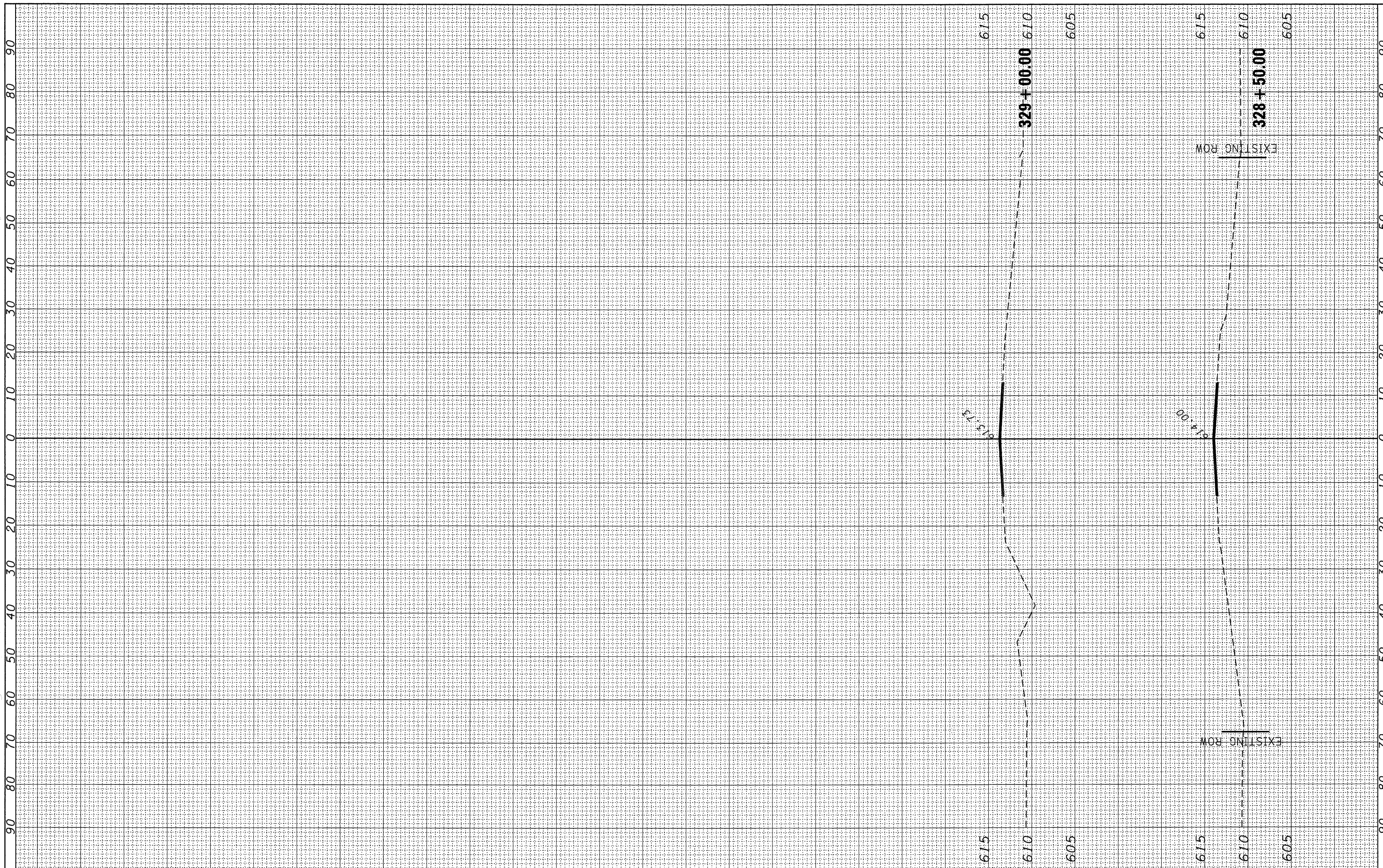
CROSS SECTIONS

SCALE: SHEET NO. OF SHEETS STA. 326+00.00 TO STA. 328+00.00

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
41	15 BR-2	LIVINGSTON	64	63
				CONTRACT NO. 66691
FED. ROAD DIST. NO. [ILLINOIS] FED. AID PROJECT				

FINAL SURVEY	BY	DATE
SURVEYED		
PLOTTED		
DATE		
AREAS		
AREAS CHECKED		
NO.		

ORIGINAL SURVEY	BY	DATE
SURVEYED		
PLOTTED		
DATE		
AREAS		
AREAS CHECKED		
NO.		



FILE NAME =	USER NAME = schwankerg	DESIGNED -	REVISED -
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		DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

CROSS SECTIONS			
SCALE:	SHEET NO.	OF SHEETS	STA. 328+50.00 TO STA. 329+00.00

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
41	15 BR-2	LIVINGSTON	64	64
CONTRACT NO. 66691				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				