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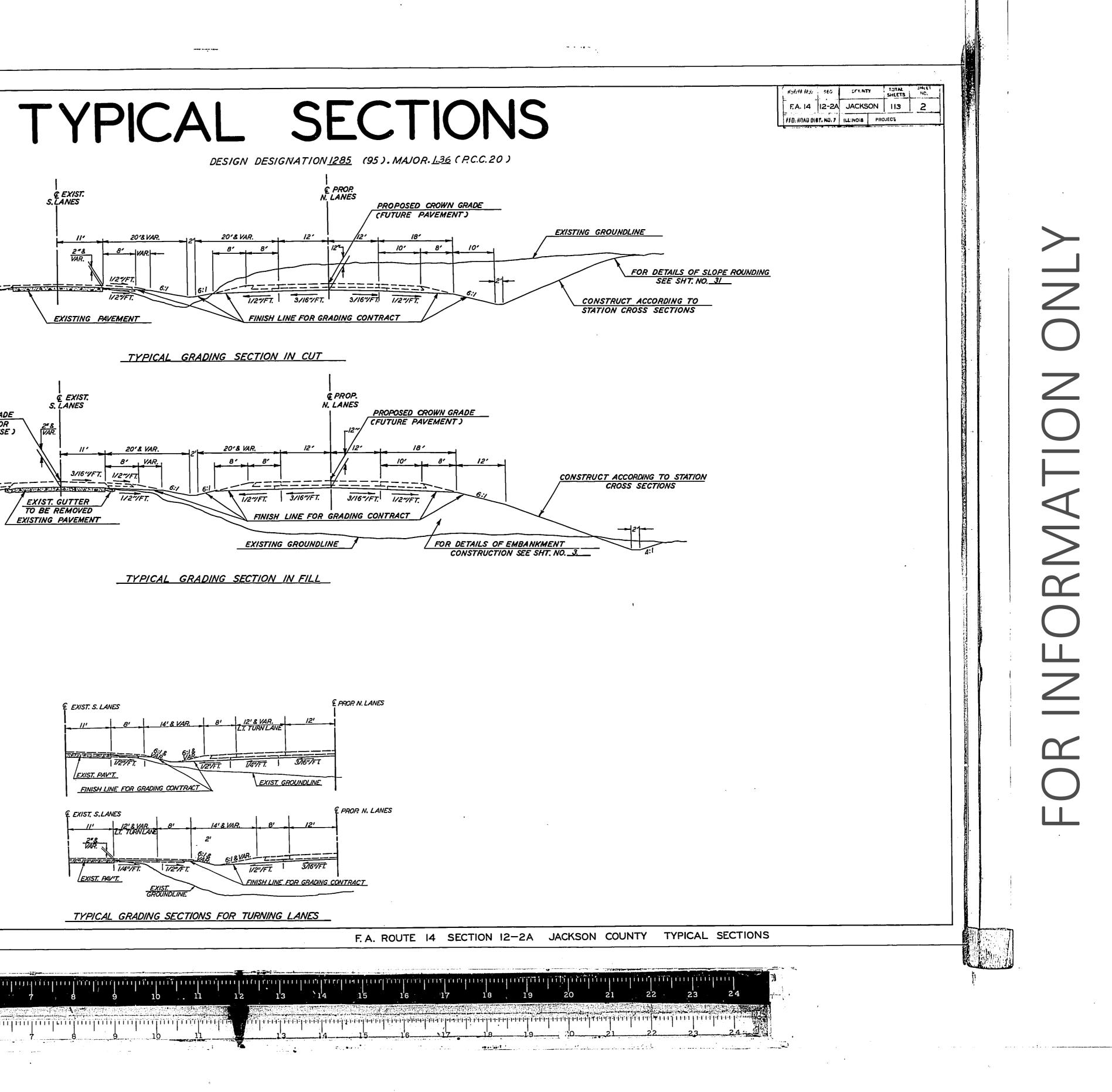
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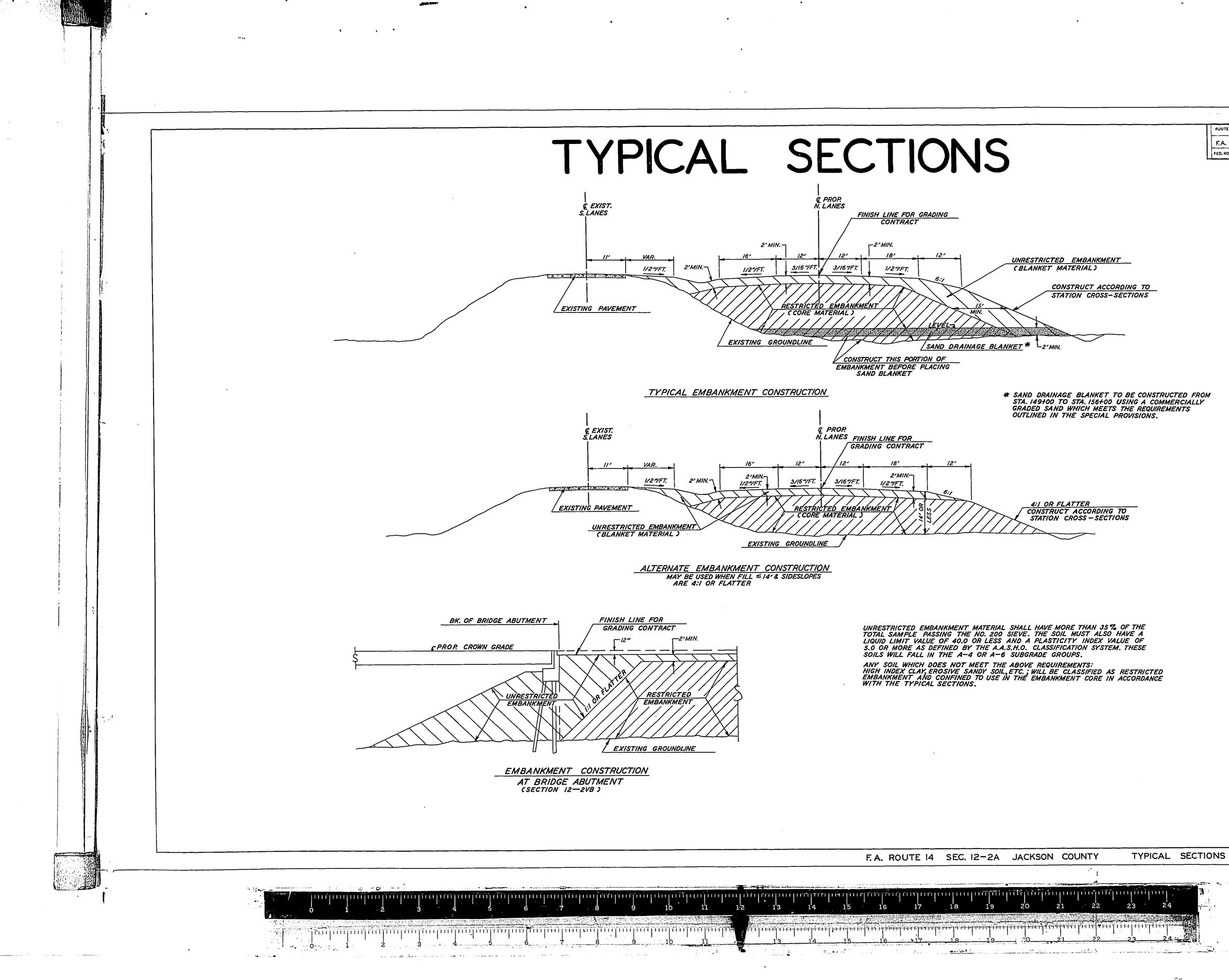
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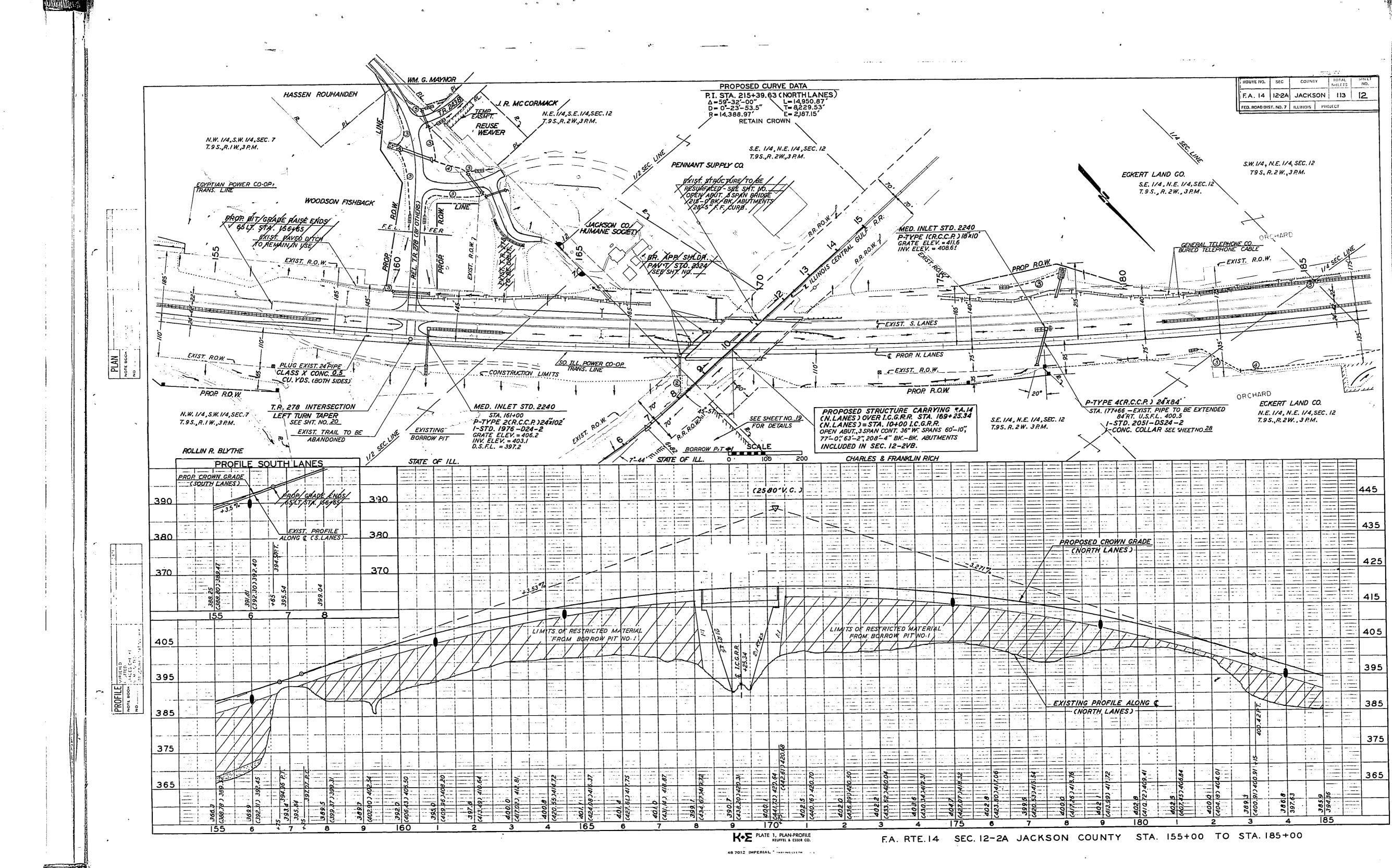
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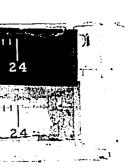
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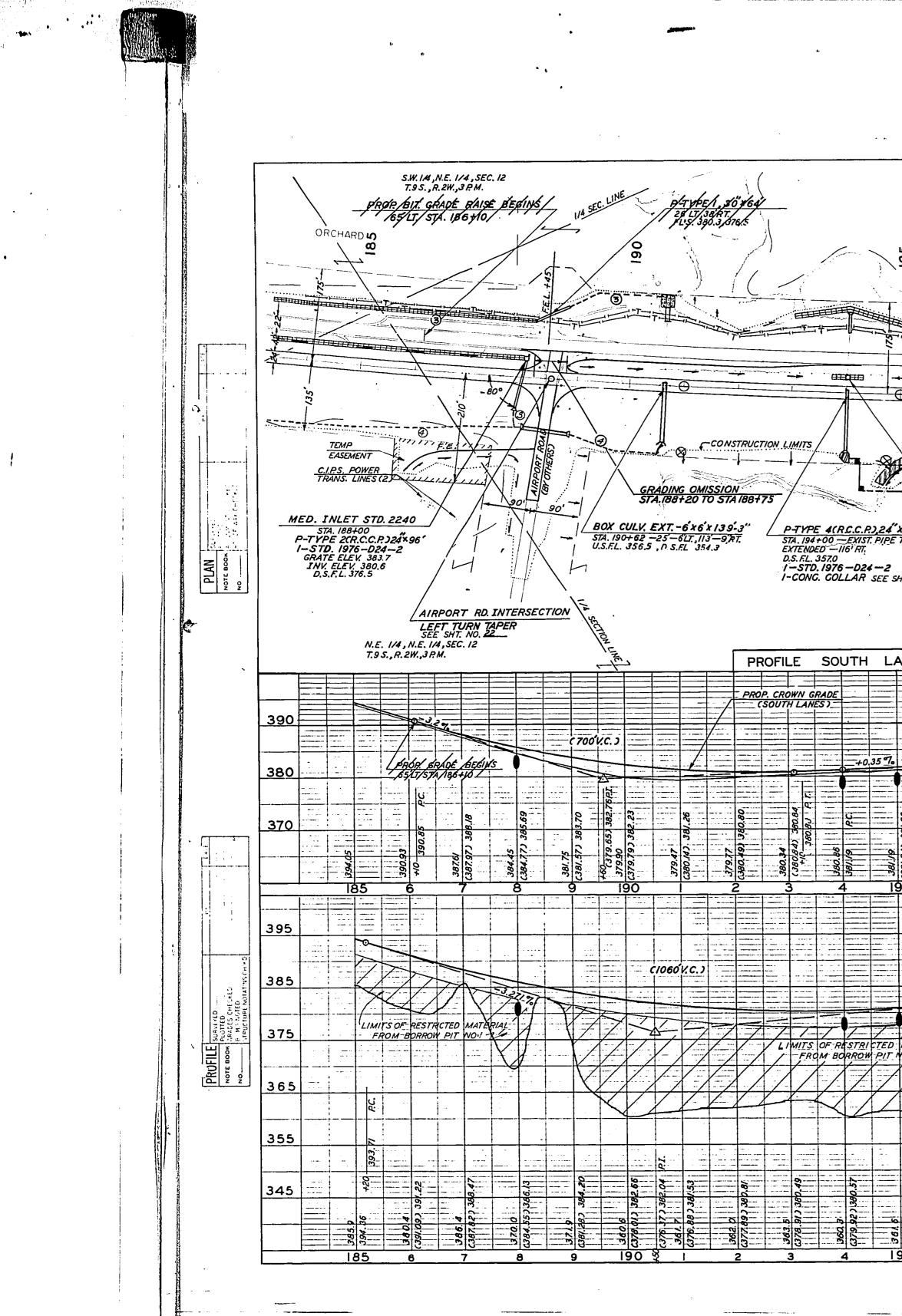


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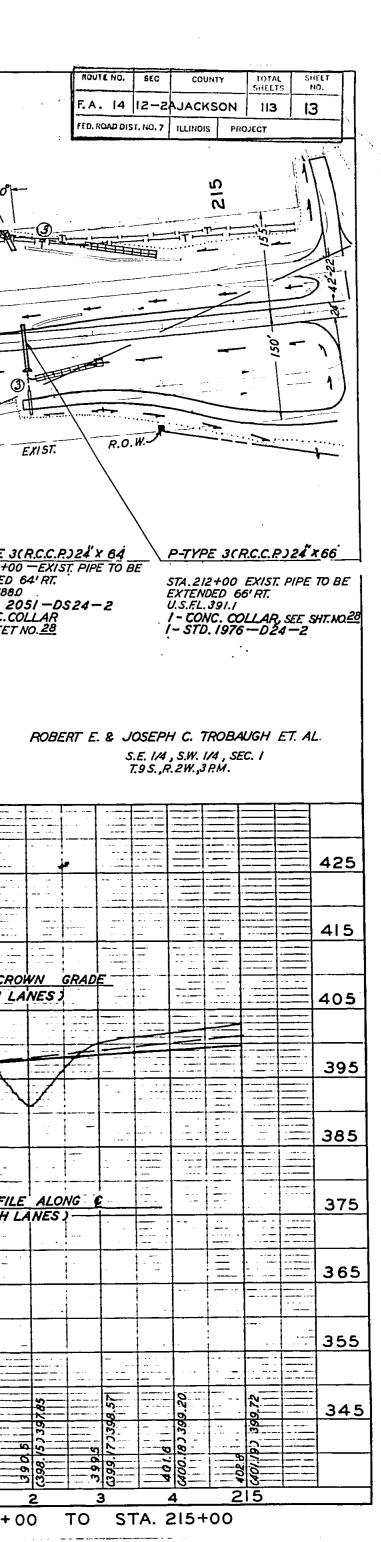
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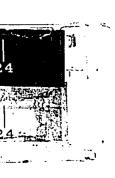
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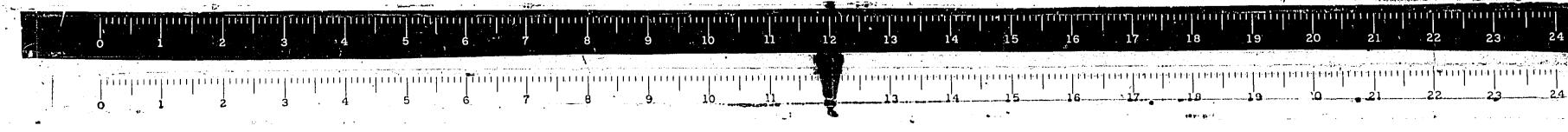
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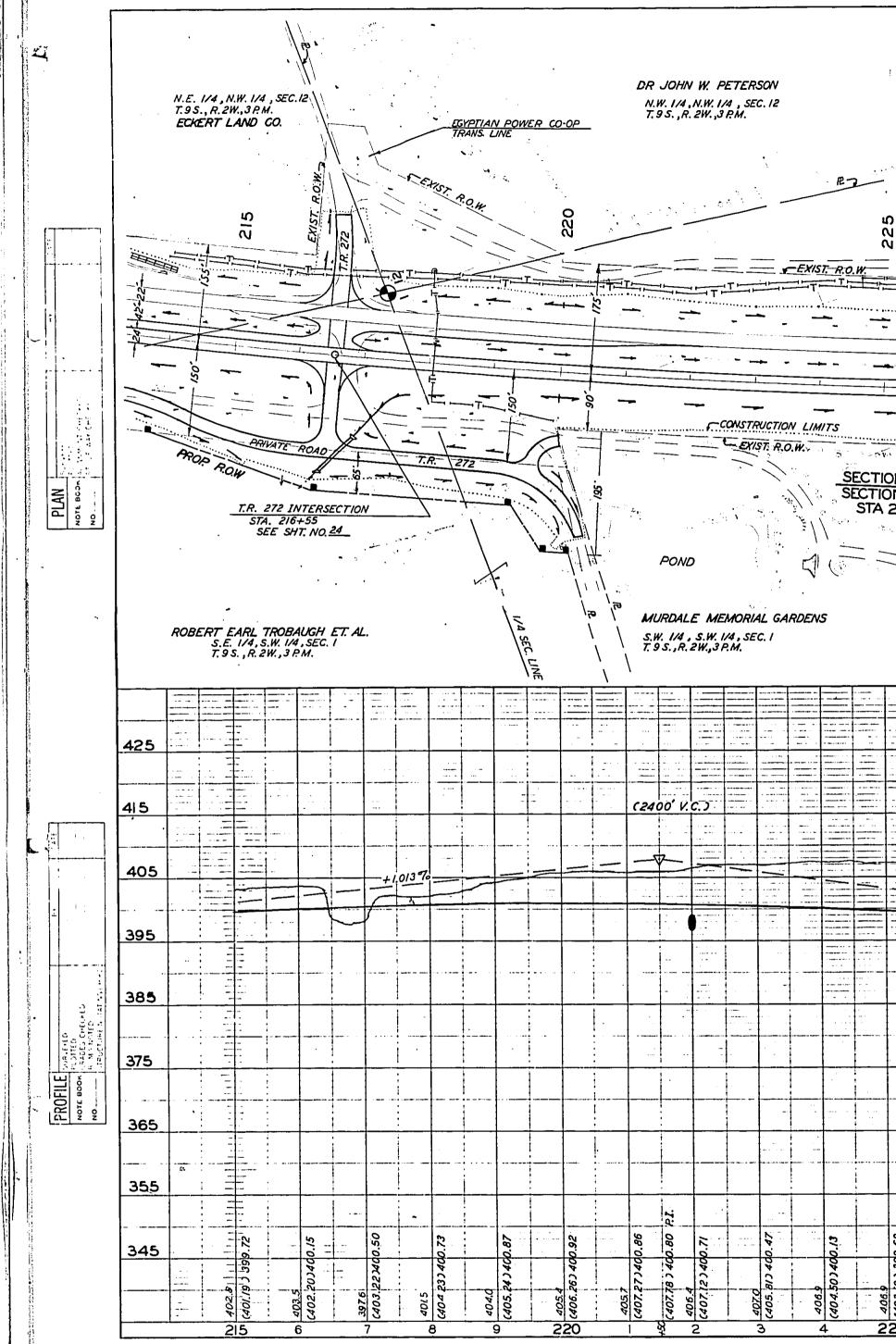






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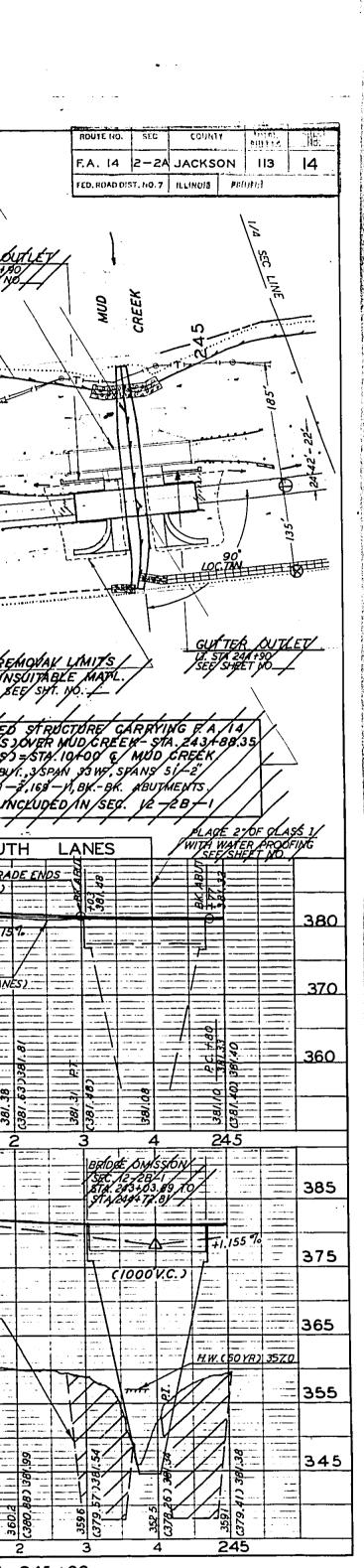
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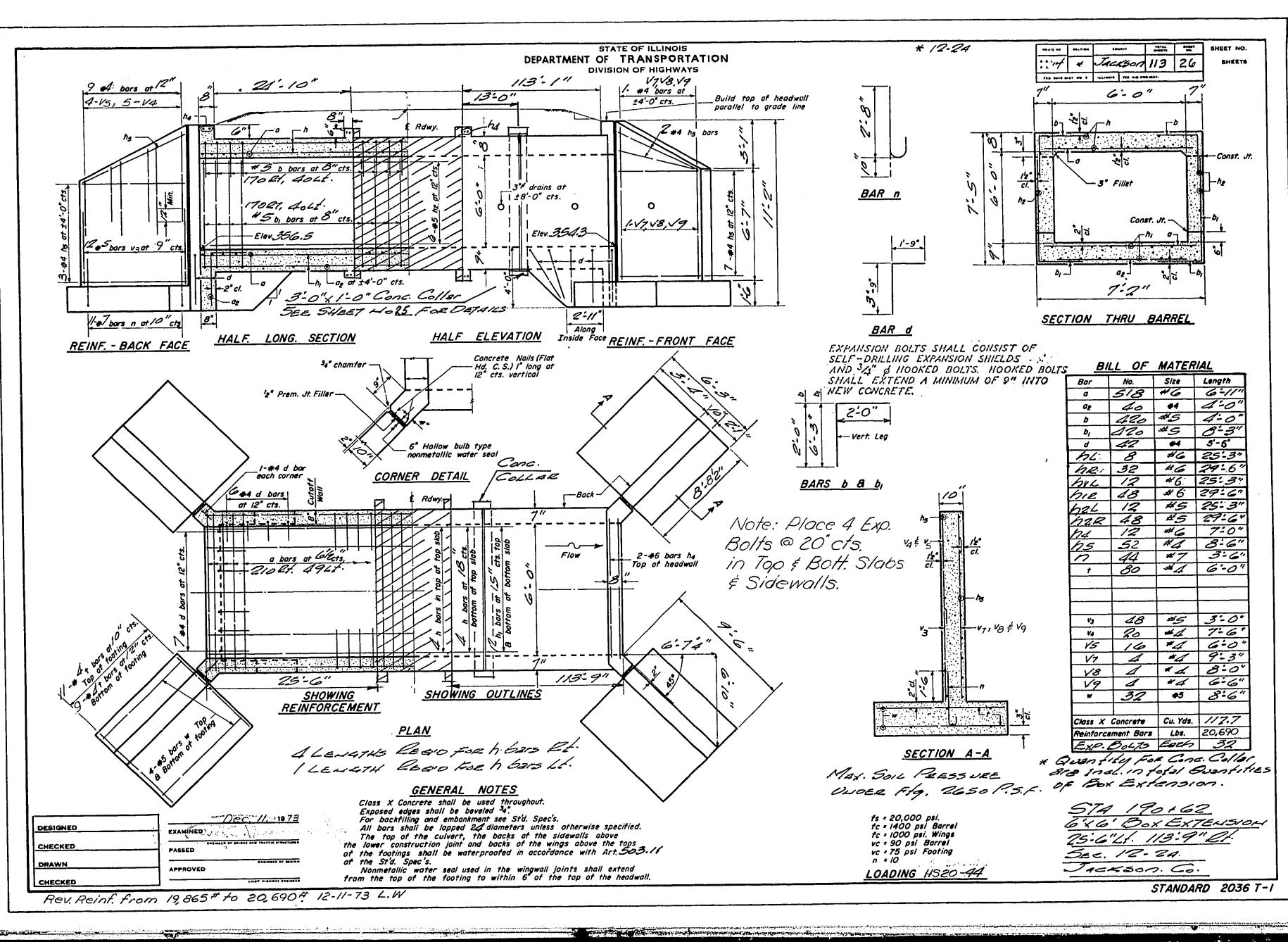
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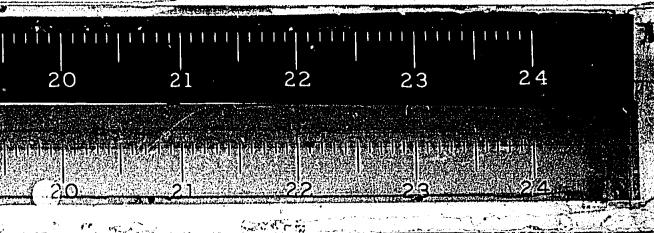


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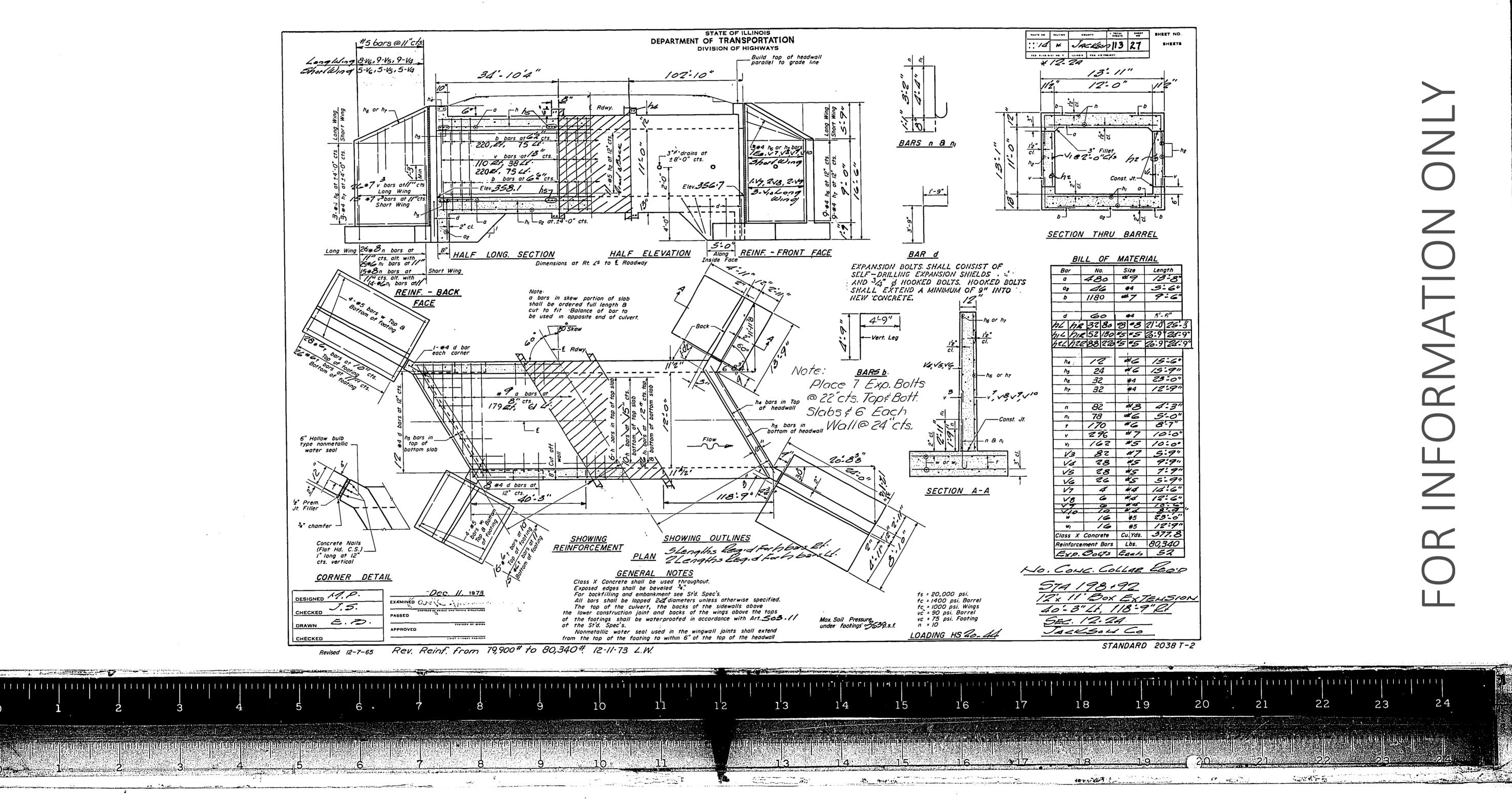
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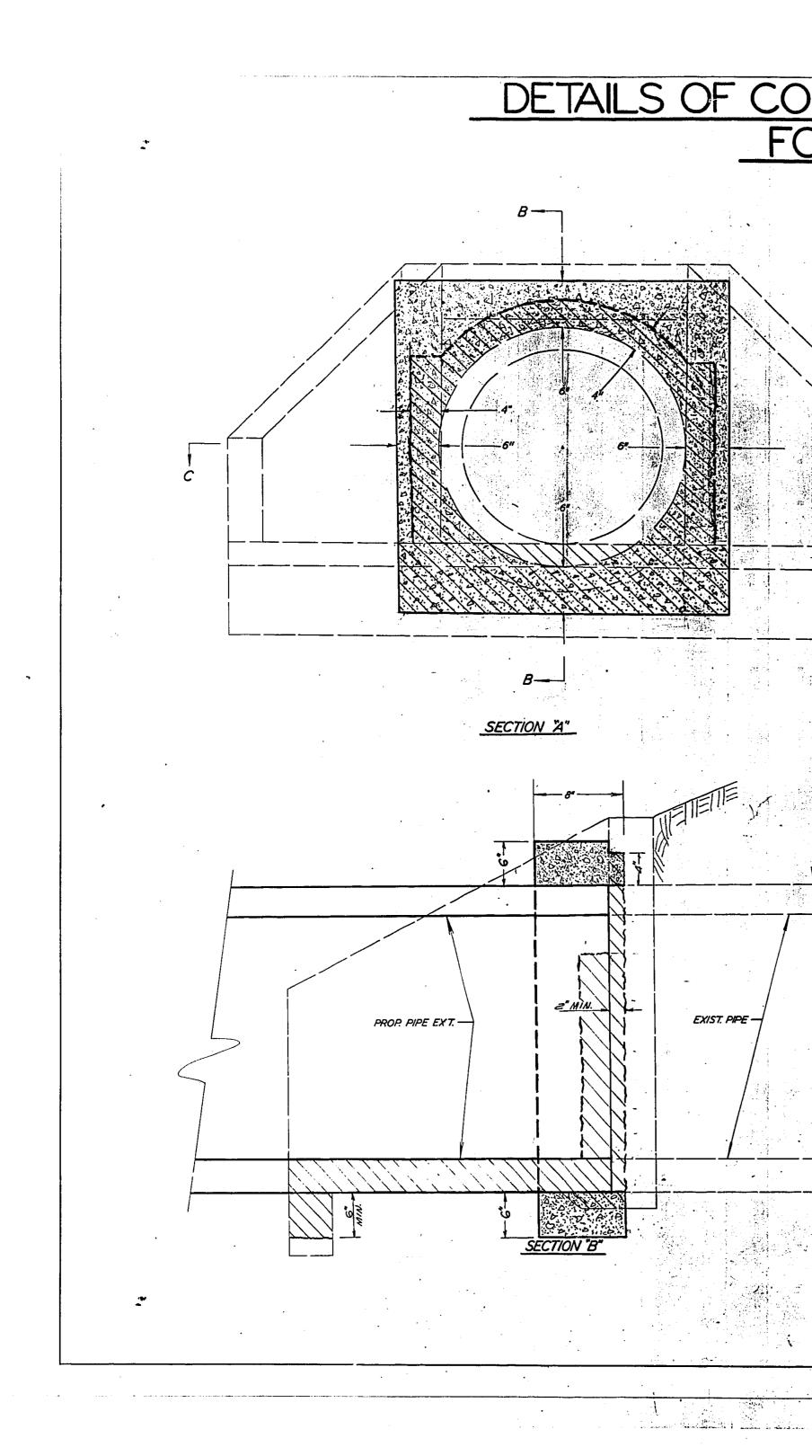
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出现了15次。2015年3月3月2日,19月1日,19月1日,19月2日,19月2日,19月2日,19月2日,19月11日,19月11日,19月11日,19月11日,19月11日,19月11日,19月11日,19月11日,19月11日,19月11111

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1'3 1'4 1'5

CONCRETE REMOVED TO EXTEND PIPE SHALL NOT BE PAID FOR SEPARATELY, BUT SHALL BE CONSIDERED INCIDENTAL TO THE PROPOSED PIPE EXTENSION. THE CONCRETE COLLAR SHALL BE PAID FOR AS "CLASS,

INDICATES PROP. CONC. COLLAR

F.A. RTE 14 SEC. 12-2A JACKSON COUNTY

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LEGEND INDICATES PORTION OF EXIST. HOWL. TO BE REMOVED

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PIPE SIZE PIPE @ RT. A CU. YD. 18" 0.2 24" 0.2 30" 0.2 0.3 36" 42" 0.3 0.4 48" 54" 0.4 60" 0.5 0.7 72" 12" 0.1

EXIST. PIPE

CONCRETE COLLAR

PROP. PIPE EXT. SECTION'C"

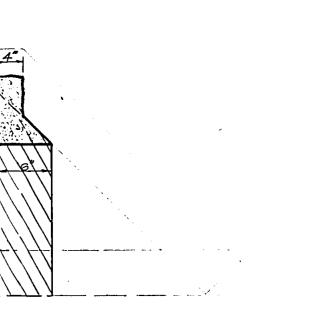
STA. 102+42 24" 20° SKEW STA. 122+74 24" RT. LS STA. 126+62 24" RT. LS STA. 136 +30 24" RT. LS 20° SKEN STA. 177+66 24" 5TA. 194+00 24" STA. 209+00 、 TA. 212+00

TO BE USED:

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DETAILS OF CONCRETE REMOVAL ON EXISTING HEADWALLS FOR PROPOSED PIPE EXTENSIONS

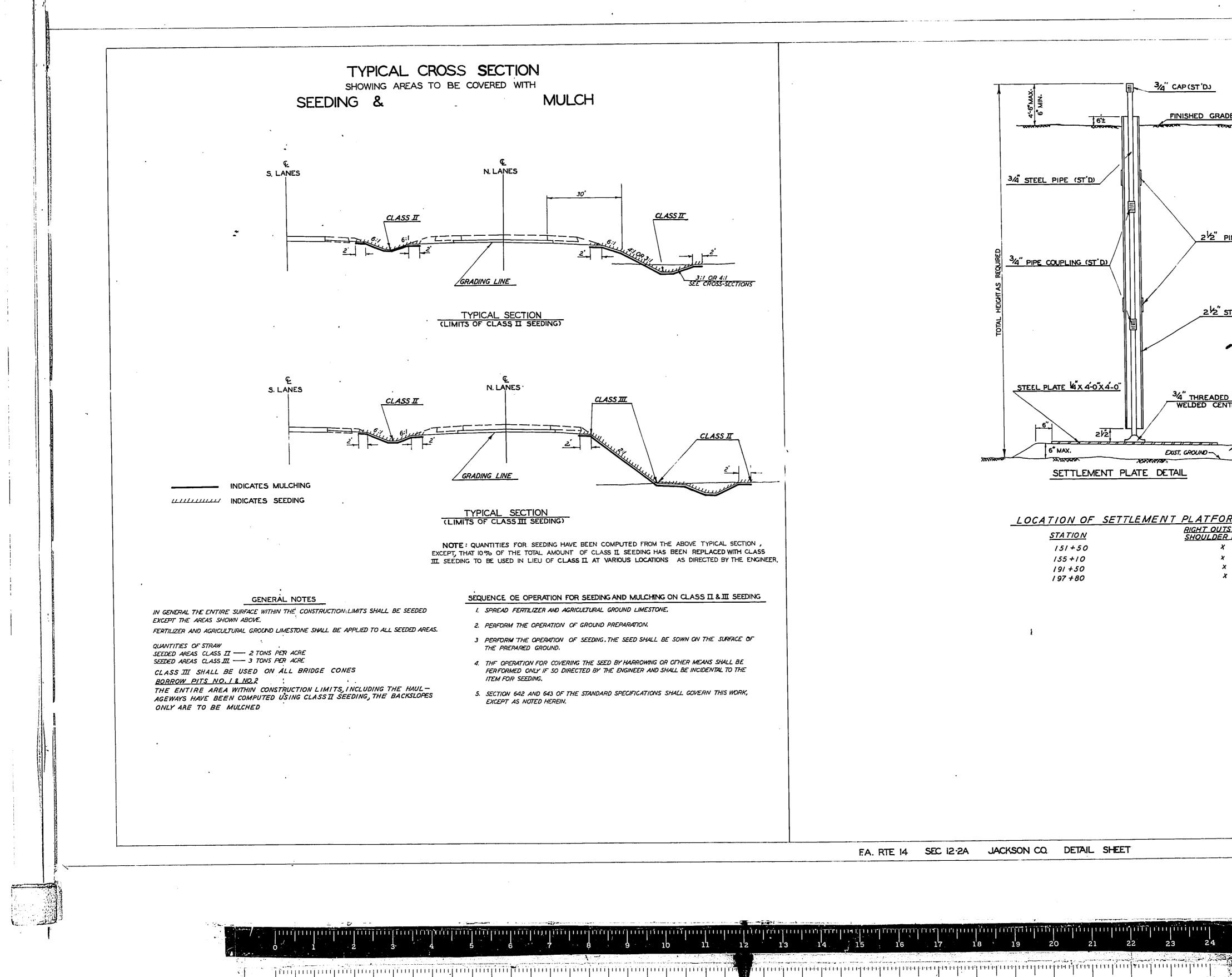
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DETAILS FOR CONCRETE REMOVAL & COLLAR (EXISTING HEADWALL)

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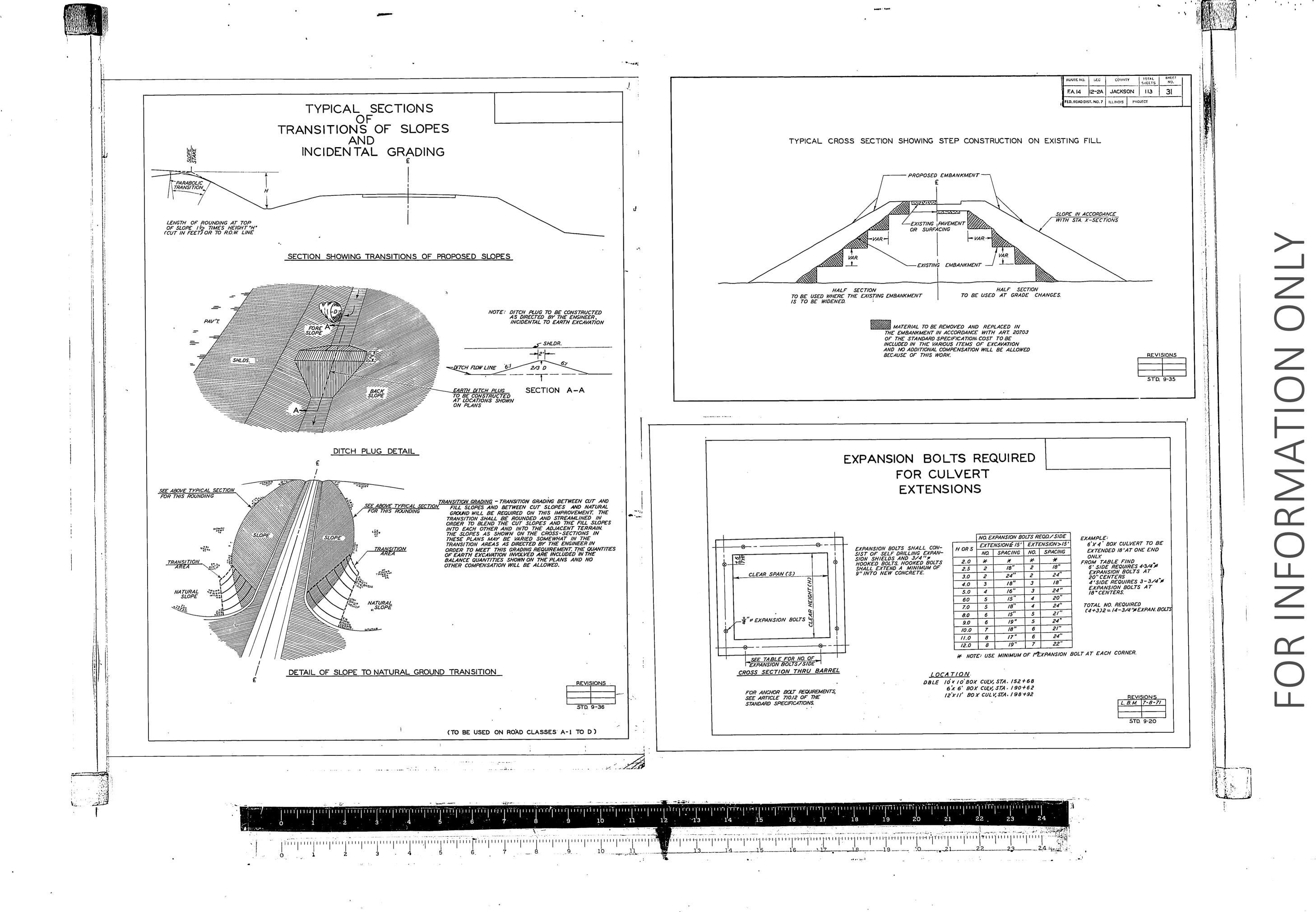
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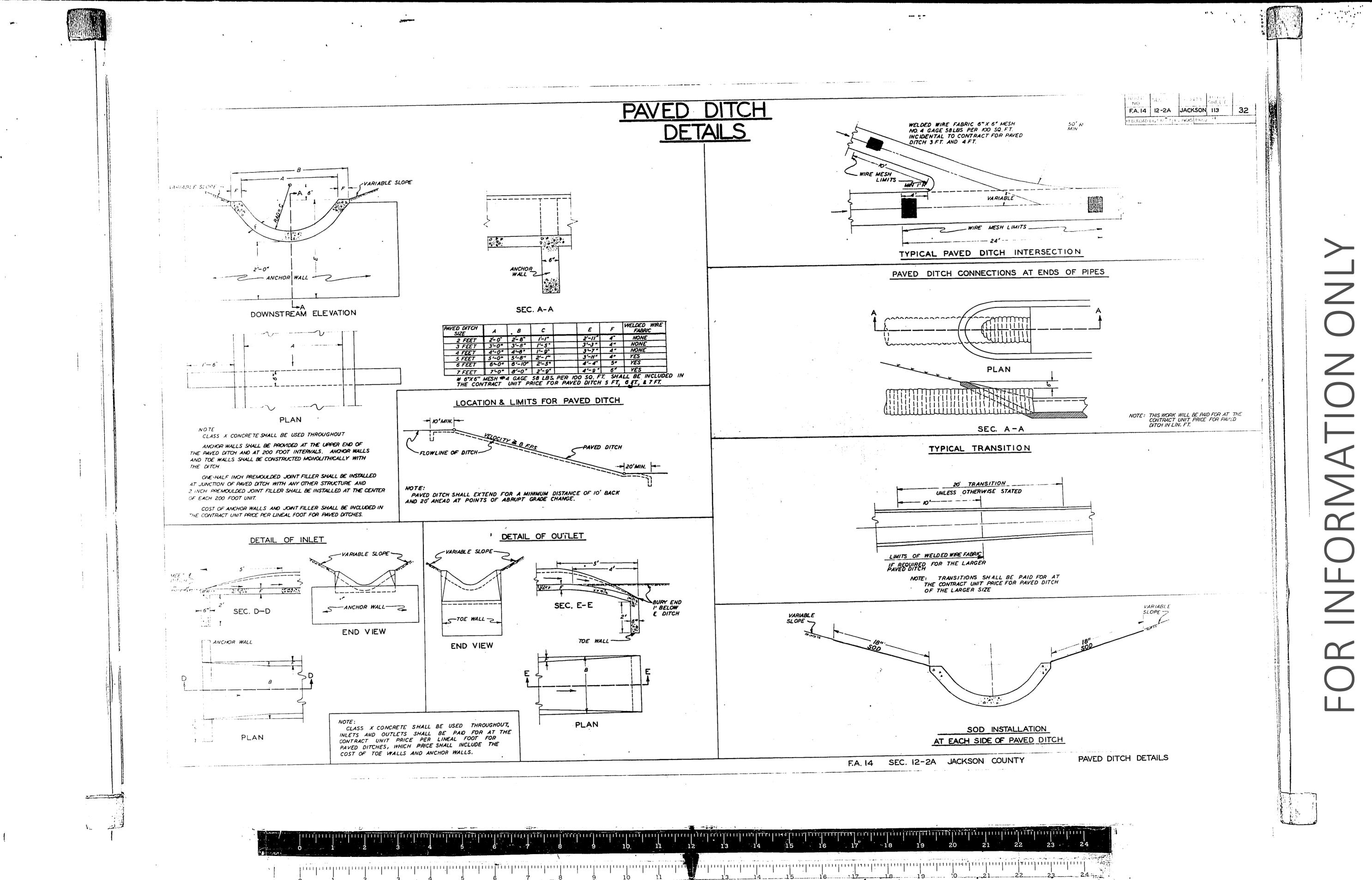
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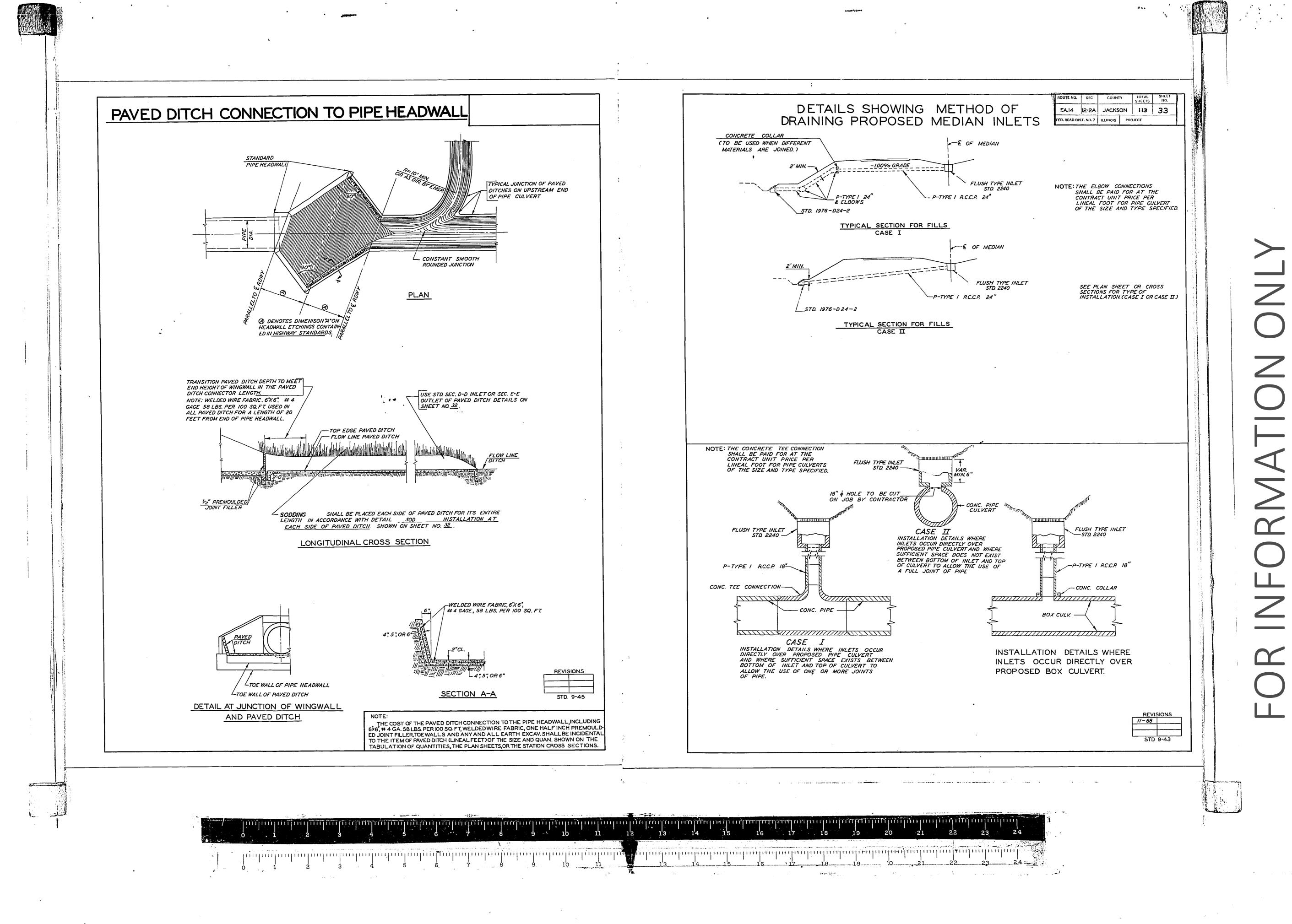
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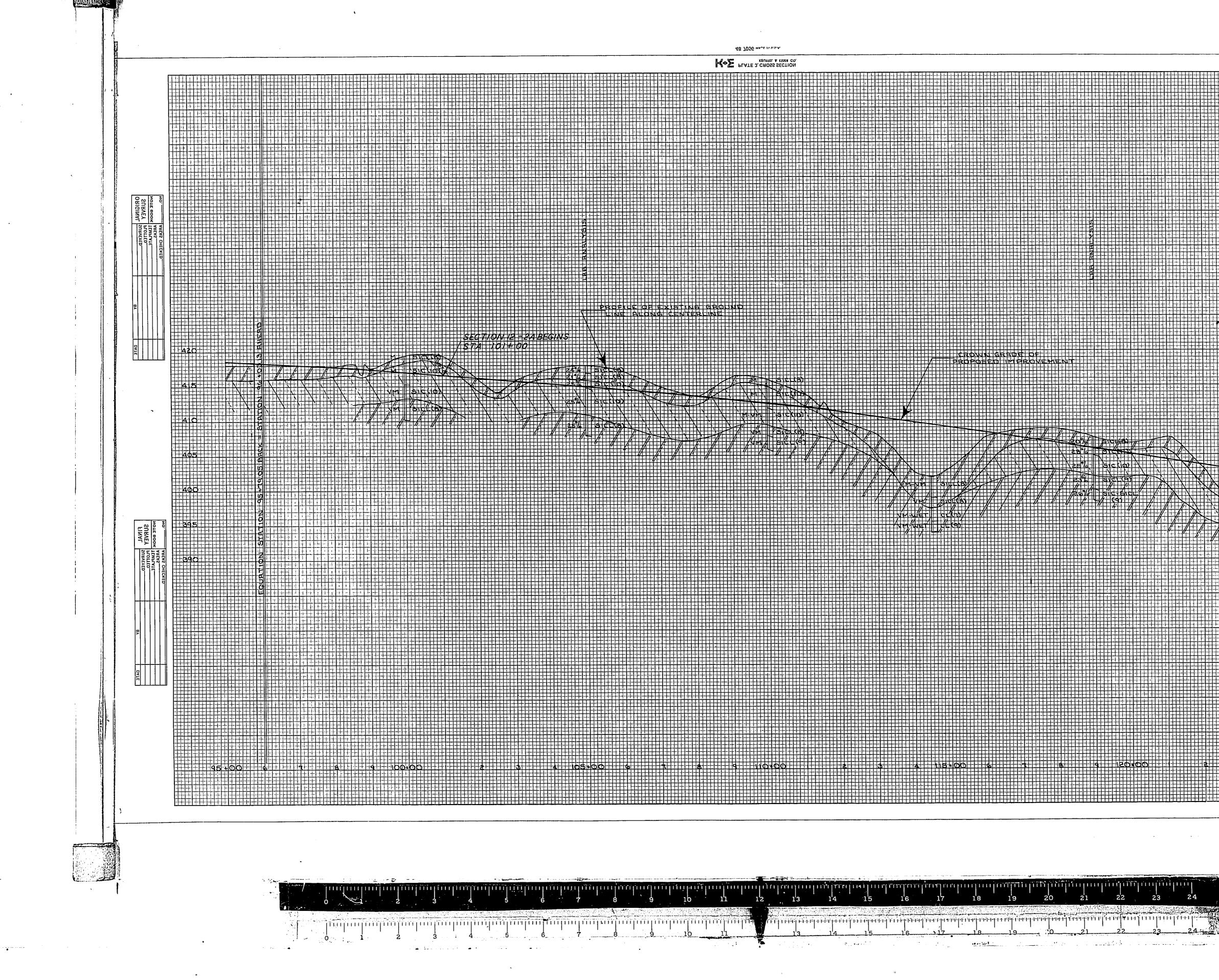


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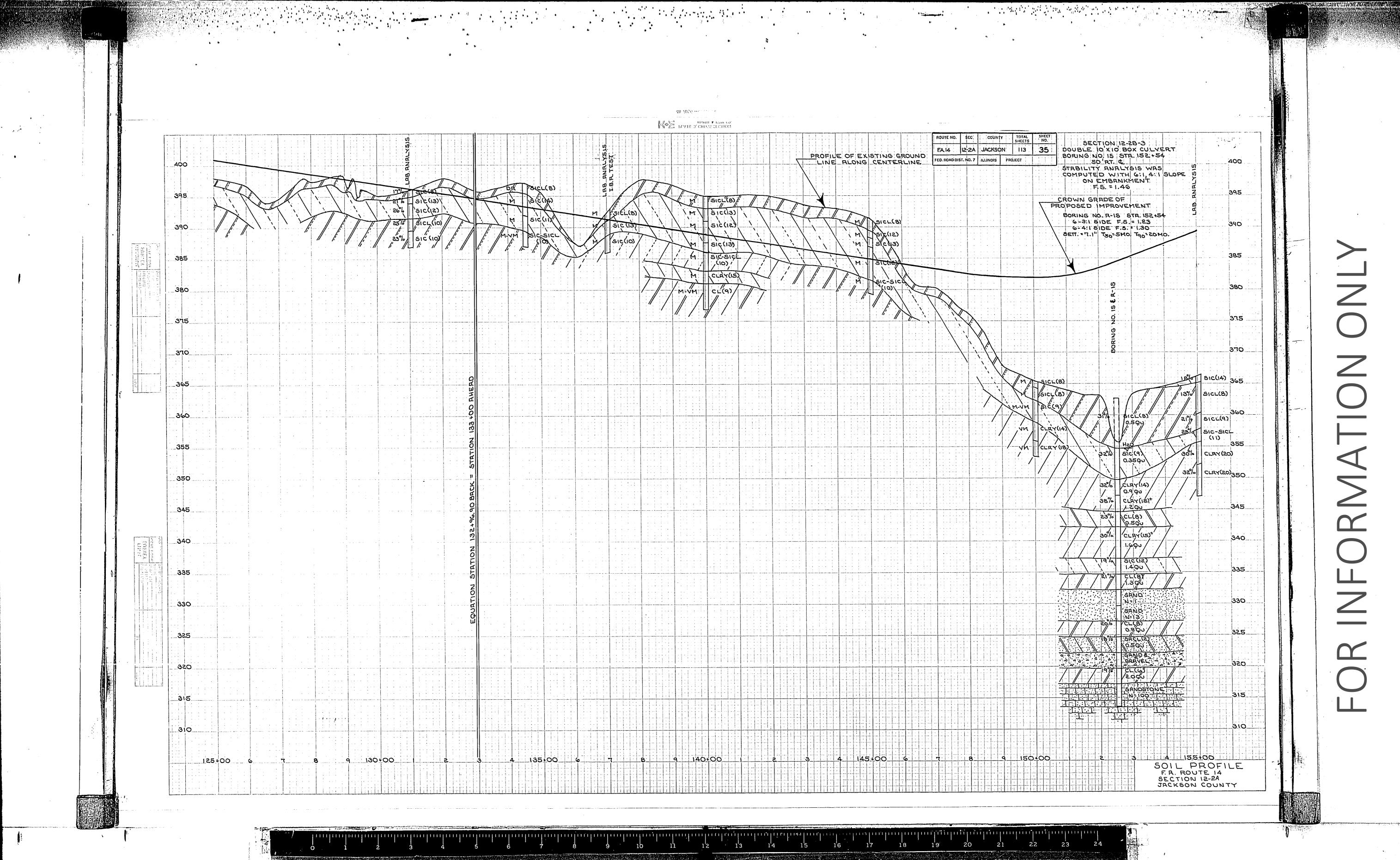




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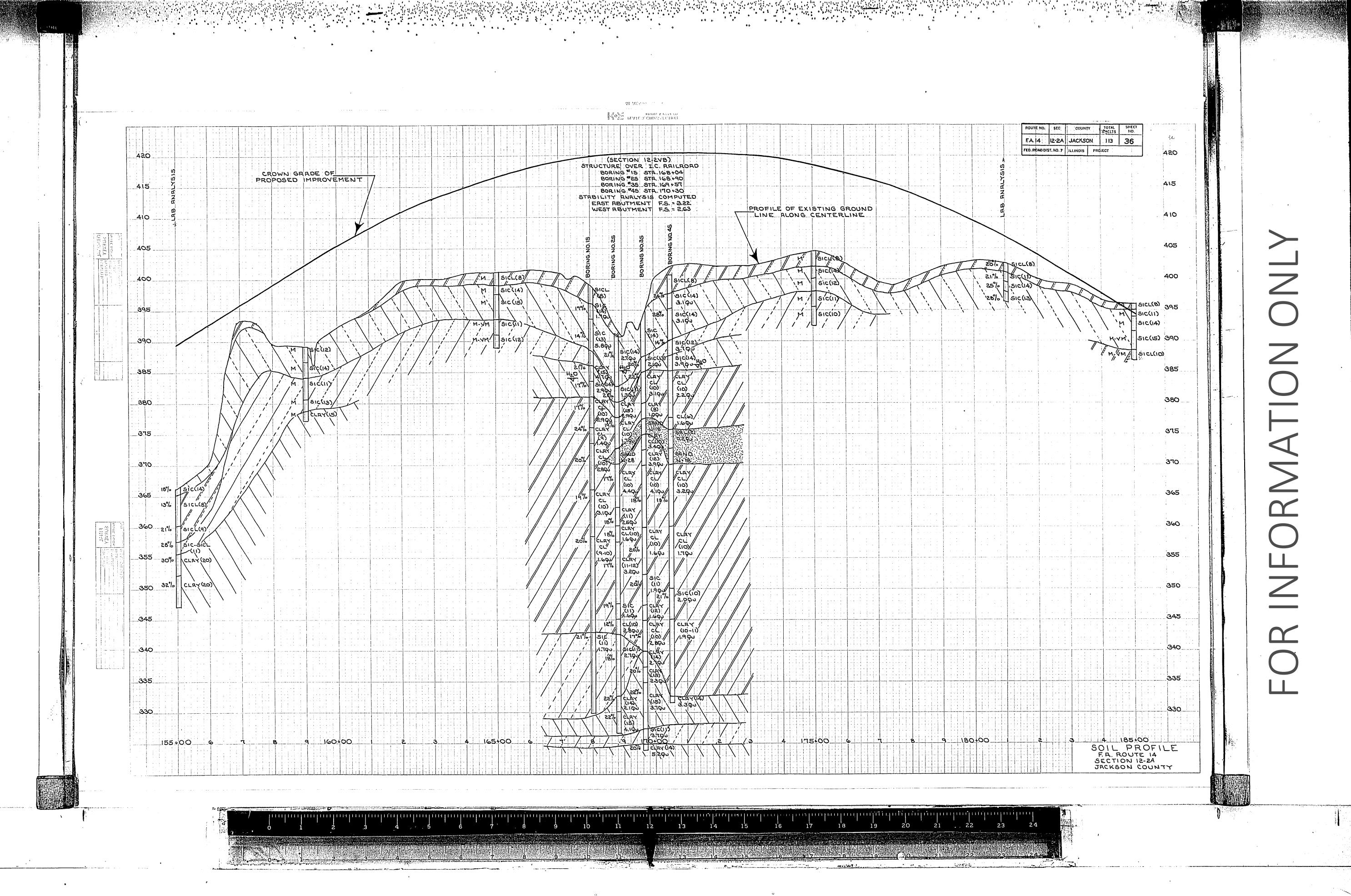
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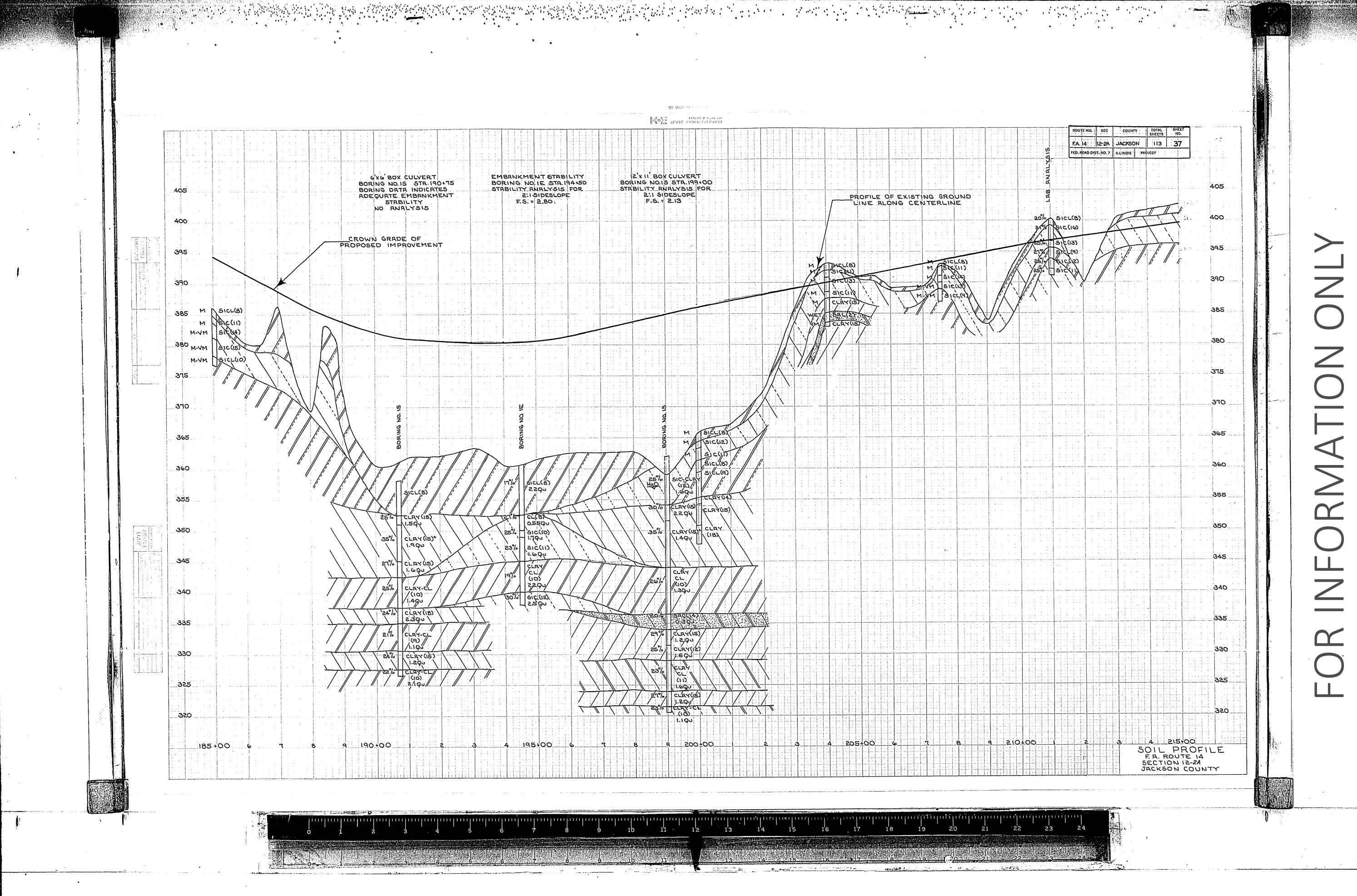
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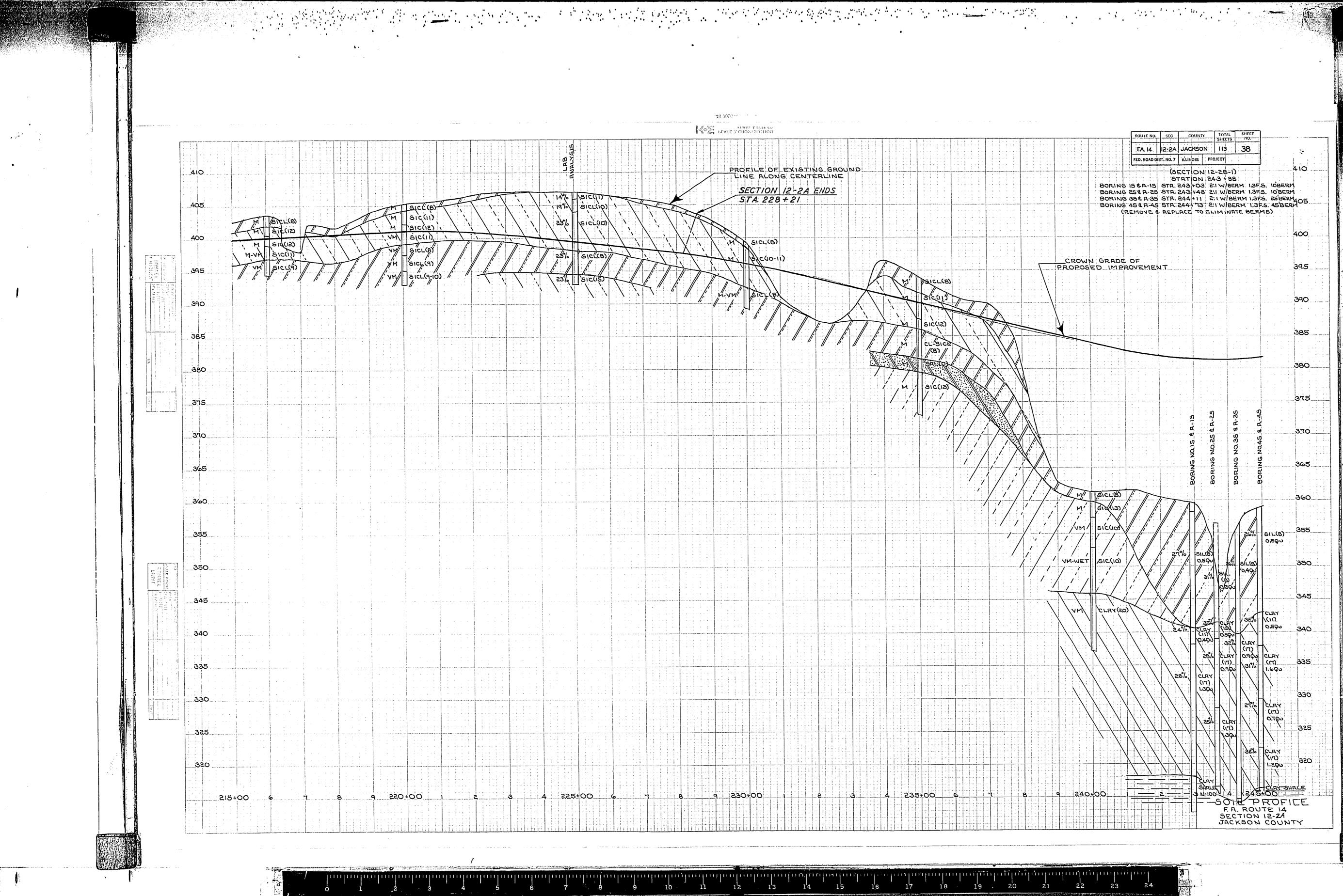
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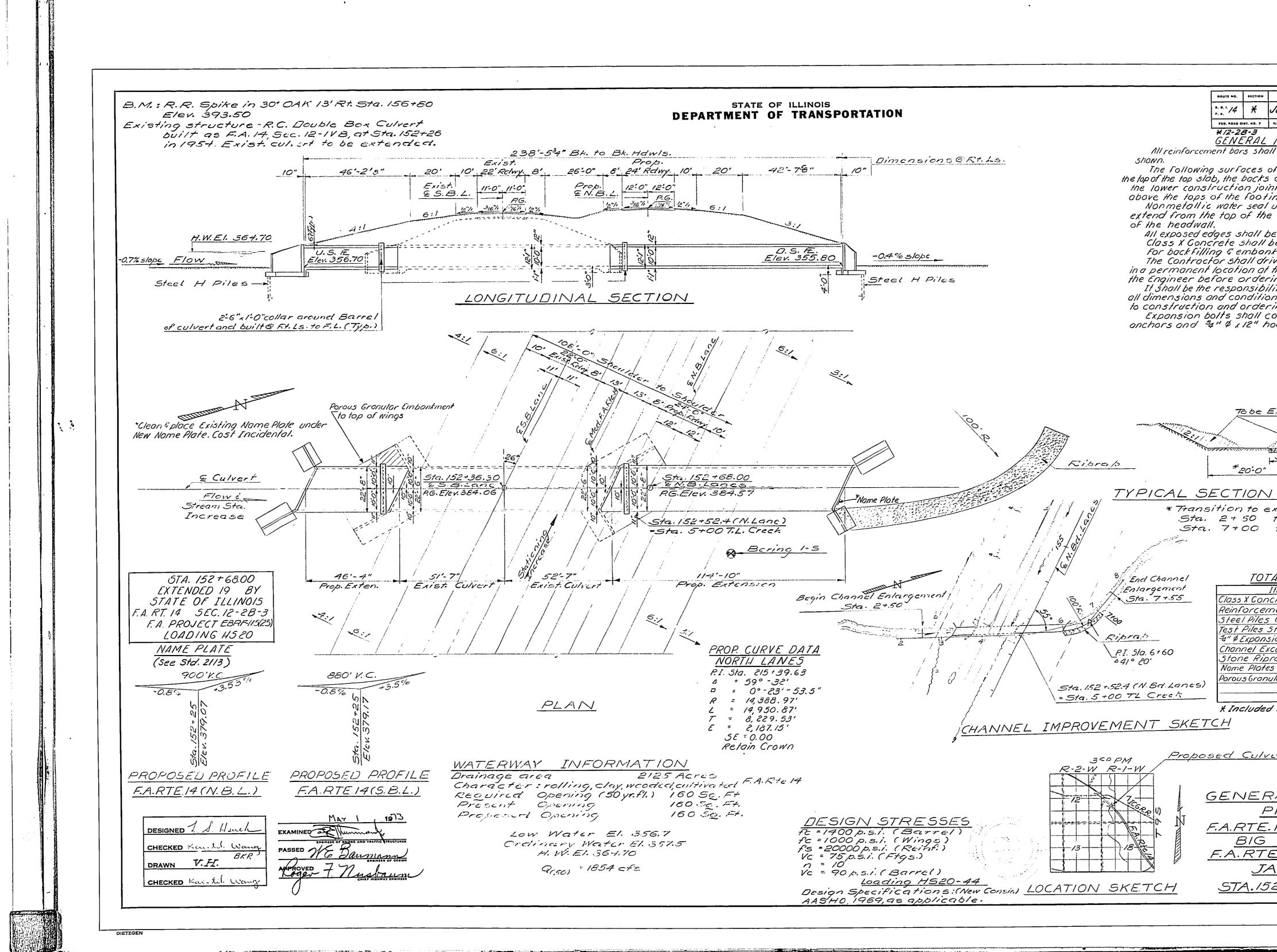
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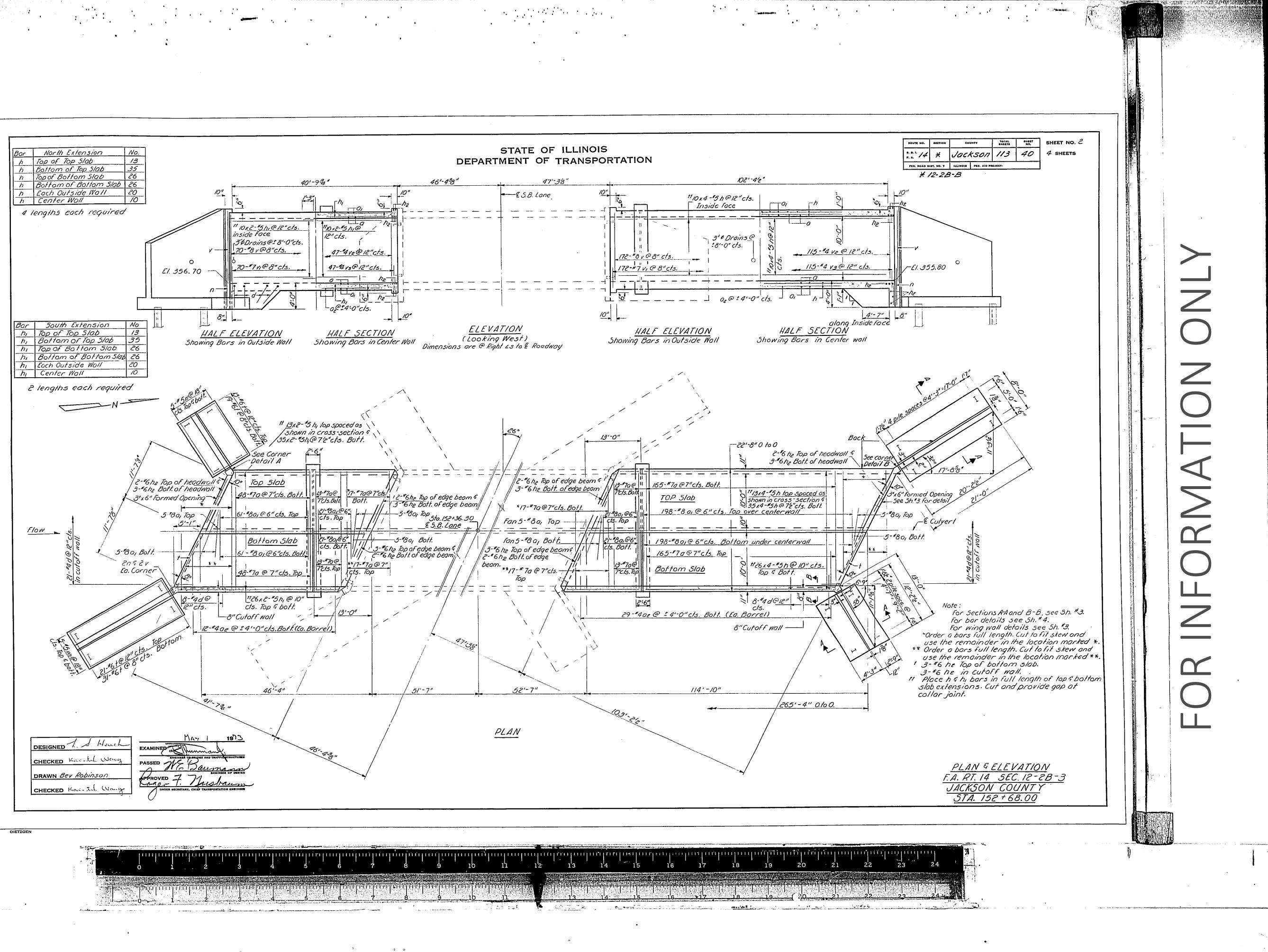
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COUNTY TOTAL SHEET NO. /		
Jackson 113 39 4 SHEETS		
ILLINOIS FED. ALD PROJECT		
NOTES all be lapped 24 Dia. unless otherwise .		
of the culvert shall be waterproofed;		
s of the exterior walls above int and the backs of the wings		
tings. I used in the wingwall joints shall be footing to within 6" of the top		
be beveled 34".		
be used throughout. nkment see Std. Specs.		
trive one Steel HP 8x 36 test pile		
t the Northwest wing as directed by ring the remainder of piles ility of the Contractor to verify ons existing in the field prior		
ring of materials. consist of self drilling expansion		
hooked bolts.		
Excavated		
2:10 Ripras - Rt. Sta. 6+00 to		
5+c. 7+00 12" Slone Riprop +6"		
bedding.		
N CHANNEL ENLARGEMENT		
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to Sta. 7+55		
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xcavation Cu.Yds. 480* orap Sq.Yds. 222 es Ea. 1		
es Ea. l nular Embankment Cu.Yds. 196		
d in Earthwork Balance		
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PAL PLAN ÉELEVATION PROJECT: EBRF-115(25)		
. 14 OVER TRIBLITARY OF		
MUDDY RIVER		
E.14 SECTION 12-2B-3 ACKSON CO.		
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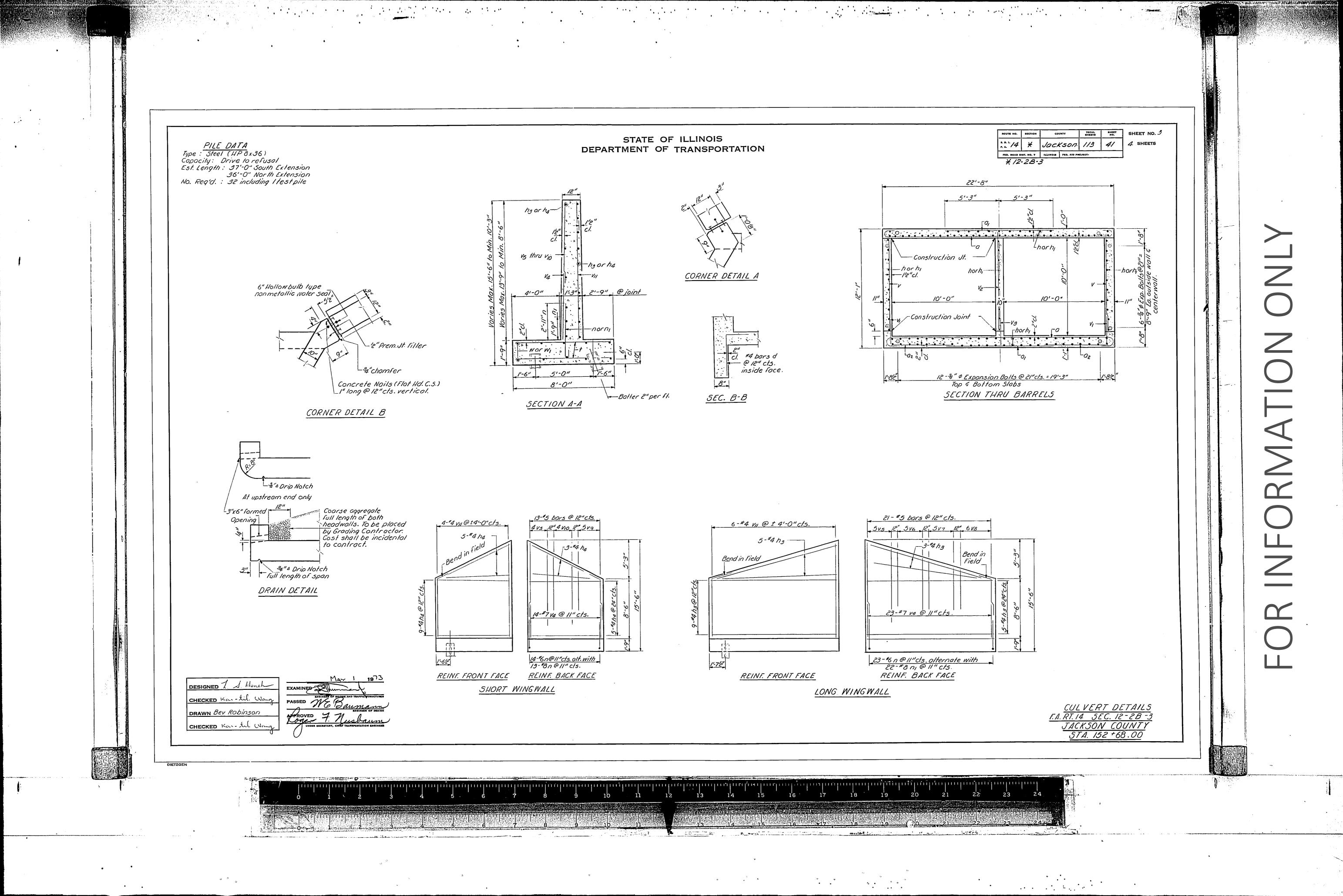
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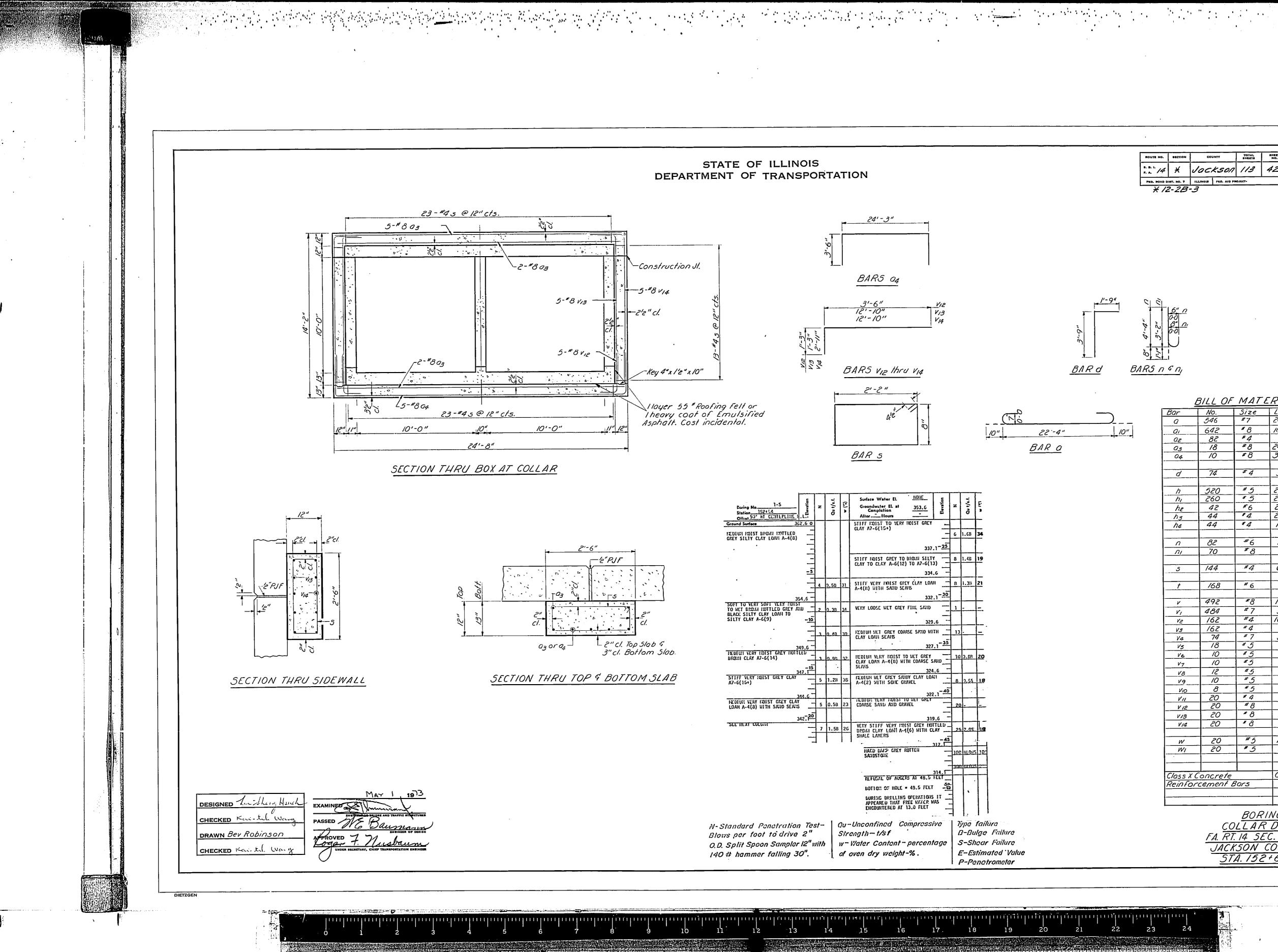
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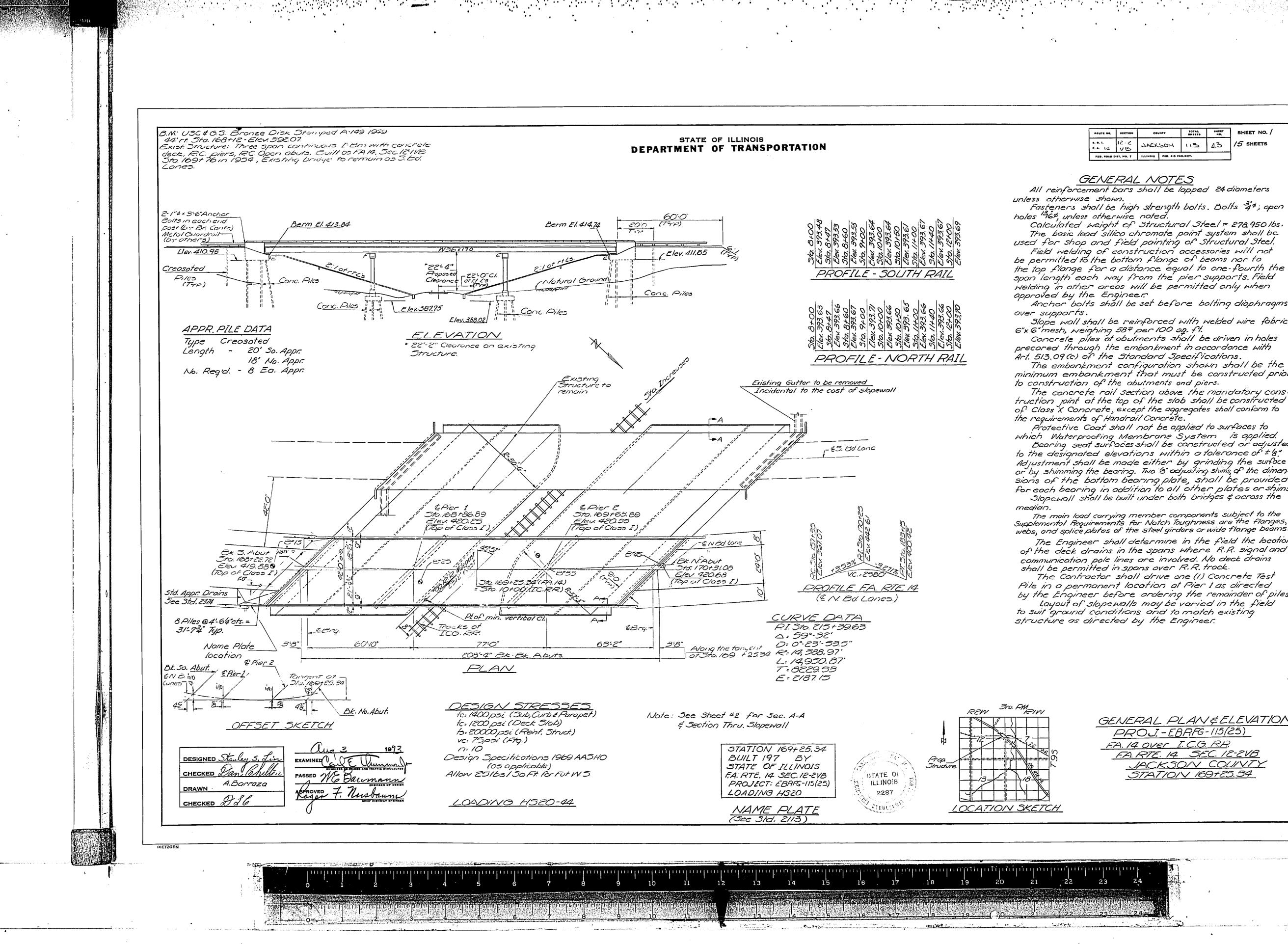
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COUNTY TOTAL HERTS NO. HERTS NO. HERTS HEET NO. 4 JOCKSON 113 42 ILLINOIS FED. AND PROJECT. 3		
BILL OF MATERIAL No. Size Length Shape 546 *7 $24' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 0''$ $24'' - 20'' - 0''$ $24'' - 20'' - 0''$ $260'' - 5'' - 23' - 6'''$ $260'' - 5'' - 23'' - 6''' - 24'' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26''' - 26'''' - 26''' - 26'''' - 26'''' - 26''' - 26'''' - 26''' - 26'''' - 26'''' - 26'''' - 26'''' - 26''''''''' - 26''''' - 26''''''''''$	FOR INFORMATION ONLY	



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GENERAL NOTES

All reinforcement bars shall be lopped 24 diameters

used for shop and field painting of Structural Steel. Field welding of construction accessories will not be permitted to the bottom flange of beams nor to the top flange for a distance equal to one-fourth the span length each way from the pier supports. Field

welding in other areas will be permitted only when Anchor bolts shall be set before bolting diophragms

Slope wall shall be reinforced with welded wire fabric 6"x 6" mesh, weighing 58# per 100 sg. ft.

Concrete piles at abutments shall be driven in holes precored through the embankment in accordance with Art. 513.09 (c) of the Standard Specifications.

The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments and piers.

The concrete rail section above the mandatory construction joint at the top of the slab shall be constructed of Class X Concrete, except the aggregates shall conform to the requirements of Handrail Concrete. Protective Coat shall not be applied to surfaces to

Slopewall shall be built under both bridges & across the

which Waterproofing Membrane System is applied. Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of + 8." Adjustment shall be made either by grinding the surface or by shimming the bearing. Two 's' adjusting shims, of the dimensions of the bottom bearing plate, shall be provided for each bearing in addition to all other plates or shims.

The main load corrying member components subject to the Supplemental Requirements for Notch Taughness are the flanges, webs, and splice plates of the steel girders or wide flange beams. The Engineer shall determine in the field the location of the deck drains in the spans where R.R. signal and communication pole lines are involved. No deck drains

shall be permitted in spons over R.R. trock.

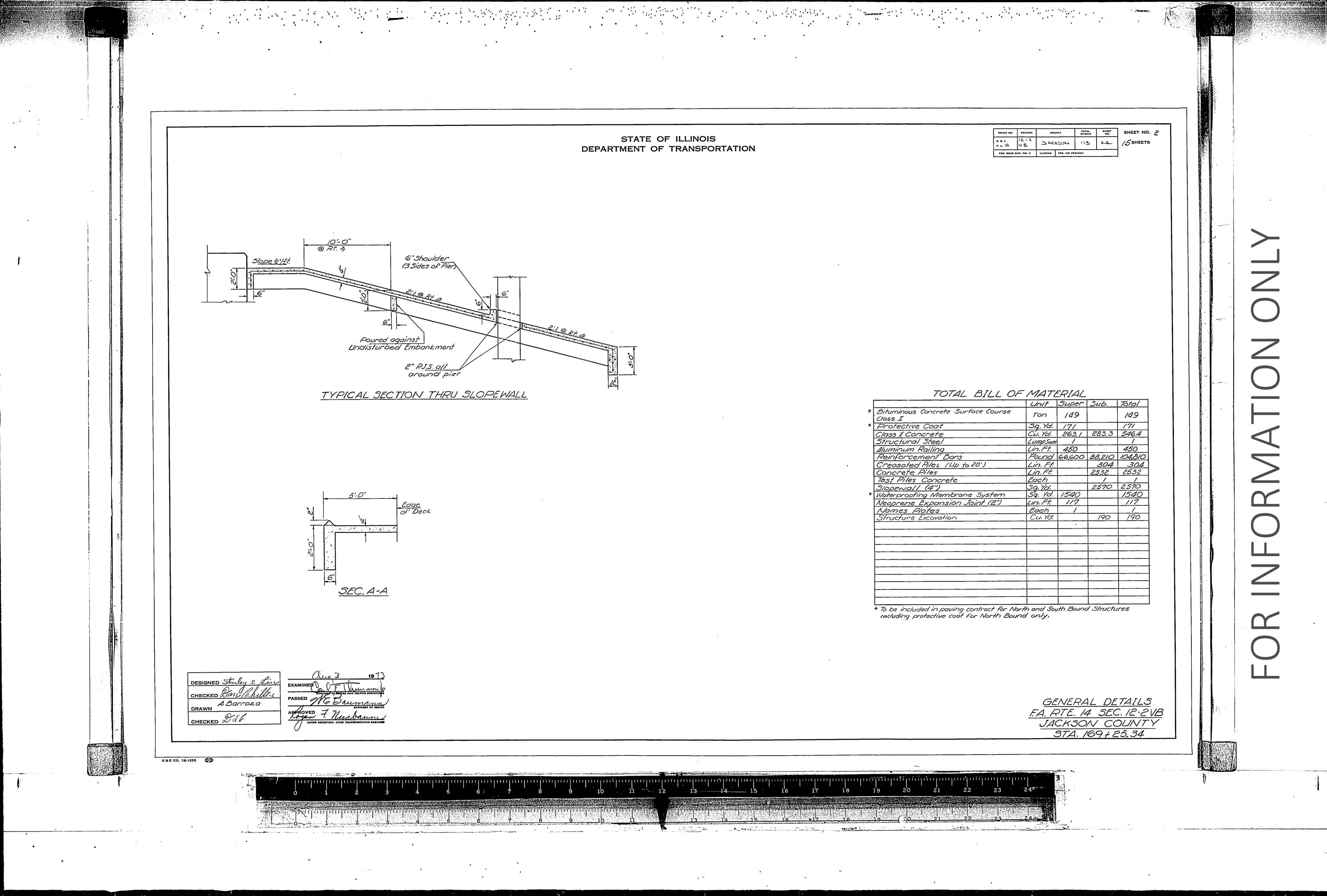
The Contractor shall drive one (1) Concrete Test Pile in a permanent location at Pier 1 as directed by the Engineer before ordering the remainder of piles. Layout of slopewalls may be varied in the field to suit ground conditions and to motch existing structure as directed by the Engineer.

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TOTAL DHEETS	BHEET NO,	SHEET NO. /
113	43	5 SHEETS
PROJECT-]
		113 43

GENERAL PLAN & ELEVATION PROJ.-EBRFG-115(25) FA. 14 OVER I.C.G. R.R. FA. RTE. 14 SEC. 12-2VB JACKSON COUNTY STATION 169+25.34





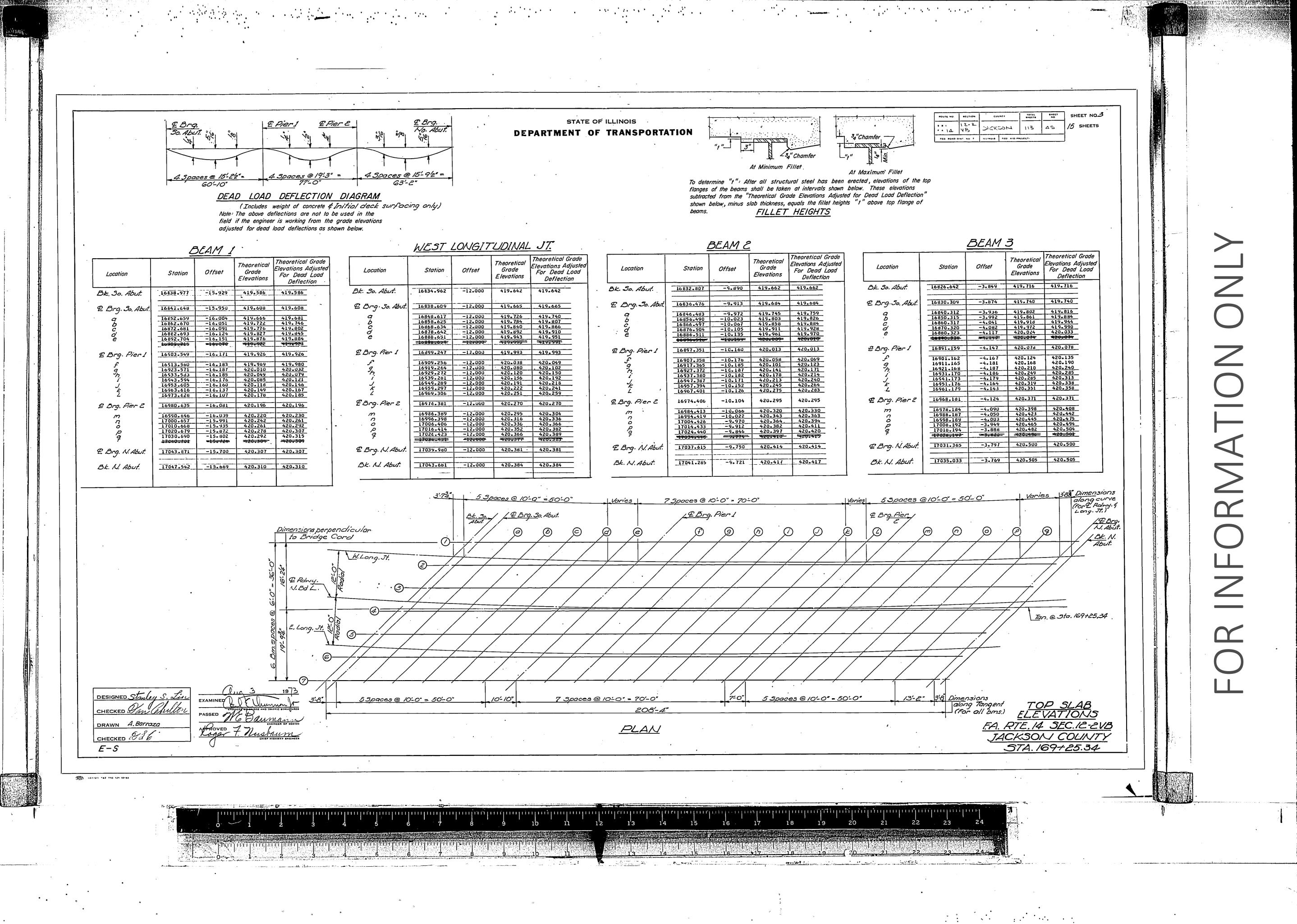


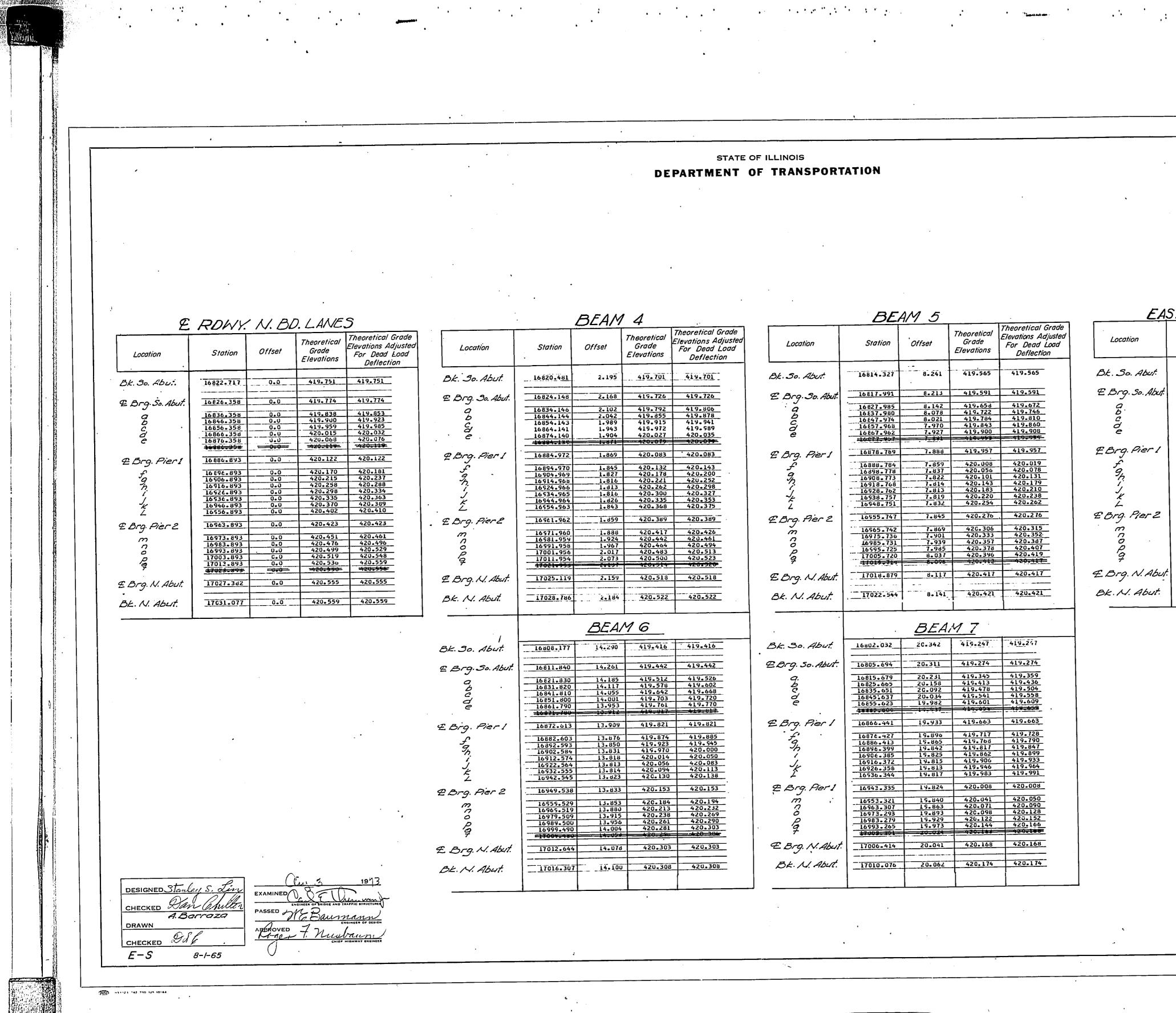
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Vituminous Concrete Surface Course Vass I Protective Coat Vass X Concrete tructural Steel Vuminum Railing Peinforcement Bars Preosoted Piles (Up to 20') oncrete Piles Preosoted Piles	Linit Ton Sq.Yd. Cu.Yd. LumpSum Lin.Ft. Pound Lin.Ft. Each Sq.Yd. Sq.Yd. Lin.Ft.	<i>Super</i> 149 171 263.1 1 450 66,600	5ub. 283.3 38,210 304 2532 1	149 171 546.4 1 450 104,810 304 2532 1 2570			\triangleleft
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Vituminous Concrete Surface Course Vass I Protective Coat Vass X Concrete tructural Steel Vuminum Railing Peinforcement Bars Preosoted Riles (Up to 20') Poncrete Piles Preosoted Riles (Up to 20') Preosoted Riles (Up to 20') Preosoted Riles Preosoted Riles Protective Course Protective Course	Unit Ton Sq.Yd. Cu.Yd. LumpSum Lin.Ft. Pound Lin.Ft. Each Sq.Yd. Sq.Yd. Sq.Yd. Lin.Ft. Each	Super 149 171 263.1 1 450 66,600 1540 117 1 1	5ub. 283.3 38,210 304 2532 1 2570	149 171 546.4 1 450 104,810 304 2532 1 2570 1540 117			ORMA
Vituminous Concrete Surface Course Vass I Protective Coat Vass X Concrete tructural Steel Vuminum Railing Peinforcement Bars Preosoted Riles (Up to 20') Poncrete Piles Preosoted Riles (Up to 20') Preosoted Riles (Up to 20') Preosoted Riles Preosoted Riles Protective Course Protective Course	Unit Ton Sq.Yd. Cu.Yd. LumpSum Lin.Ft. Pound Lin.Ft. Each Sq.Yd. Sq.Yd. Sq.Yd. Lin.Ft. Each	Super 149 171 263.1 1 450 66,600 1540 117 1 1	5ub. 283.3 38,210 304 2532 1 2570	149 171 546.4 1 450 104,810 304 2532 1 2570 1540 117			FORMA
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4.148	2.168	419.726	419.726	E Brg. So. Abut.	16 E1 7 . 991	8.213	419.591	419.591	E Brg. So. Abut.	16814.139 16824.130	
4.146 4.144 4.143	2.102 2.042 1.989	419.792 419.855 419.915	419.806 419.878 419.941	960	16827.985 16837.980 16847.974	8.142 8.078 8.021	419.658 419.722 419.784	419.672 419.746 419.810	06 0	16834.122 16844.114 16854.105	
4.141 4.140	1.943 <u>1.904</u>	419.972 420.027 420.079	419.589 420.035 420.075	୦୯୫	16857.968 16867.962 -16877.957	7.970 7.927 7.891	419.843 419.900	419.860 419.908 	e de	16864.097 10874.089	
4.972	1.869	420.083	420.083	£ Brg. Pier 1	16878.769	7.888	419.957	419.957	EBrg. Pier 1	16874.570	
4.970 4.969 4.968 4.966 4.965	1.845 1.827 1.816 1.813 1.816	420.132 420.178 420.221 420.262 420.300	420.143 420.200 420.252 420.298 420.327	fghij	16888.784 16898.778 16908.773 16918.768 16928.762 16938.757	7.859 7.837 7.822 7.814 7.813 7.819	420.008 420.056 420.101 420.143 420.183 420.220	420.019 420.078 420.131 420.179 420.210 420.238	f.9h:1	16894.554 16904.545 16914.537 16524.529 16934.520	
4.964	1.826	420.335 420.368	420.353 420.375 420.389		16948.751	7.832	420.254	420.262	k L E Brg. Pier 2	16944.512 16951.438	
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1.753 25.119	2.159	420.518	420.518	E Brg. N. Abut.	17018.879	8.098	420.412	420.417	£ Brg. N. Abut. Bk. N. Abut.	17014.816	
28.786	2.184	420.522	420.522	Bk. N. Abut.	17022.544	8.141	420.421	420.421	BK. N. Abut.	17018.504	
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1.840	14.261	419.442	419.442	Bk. 30. Abut. E.Drg. So. Abut.	16805.694	20.311	419.274	419.274			
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12.613	13.909	419.821	419,821	E Brg. Pier I	16866.441	19.933	419.663	419.663		Note. Shee	: K 1 #3
82.603 92.593 02.584 12.574 22.564 32.555	13.876 13.850 13.831 13.818 13.818 13.813 13.814	419.970 420.014 420.056 420.094	419.885 419.945 420.000 420.050 420.083 420.113 420.138	£ Brg. Pier 1 m	16876.427 16886.413 16896.399 16906.385 16916.372 16926.358 16536.344	19.896 19.865 19.842 19.825 19.815 19.813 19.817	419.717 419.768 419.817 419.862 419.906 419.946 419.983	419.728 419.790 419.847 419.899 419.933 419.964 419.991			
42.545 49.538	13.823	420.130	420.153	E Brg. Pier 1	16943.335	19.824	420.008	420.008			
59.529 65.519 79.509 89.500 99.490	13.853 13.880 13.915 13.956 14.004 14.054	420.213 420.238 420.261 420.281	420.232 420.269 420.290 420.303	TH P P P P P P P P P P P P P P P P P P P	16953.321 16963.307 16973.293 16983.279 16993.265 17003.251	15.840 15.863 19.893 19.929 15.973 	420.041 420.071 420.098 420.122 420.144 	420.050 420.090 420.128 420.152 420.166 420.166			
12.644	14.078			E. Brg. N. Abut.	17006-414	20.041	420.168	420.168			
16.307	14.100	420.308	420.308	BK. N. Abut.	17010.076	20.062	420.174	420.174			
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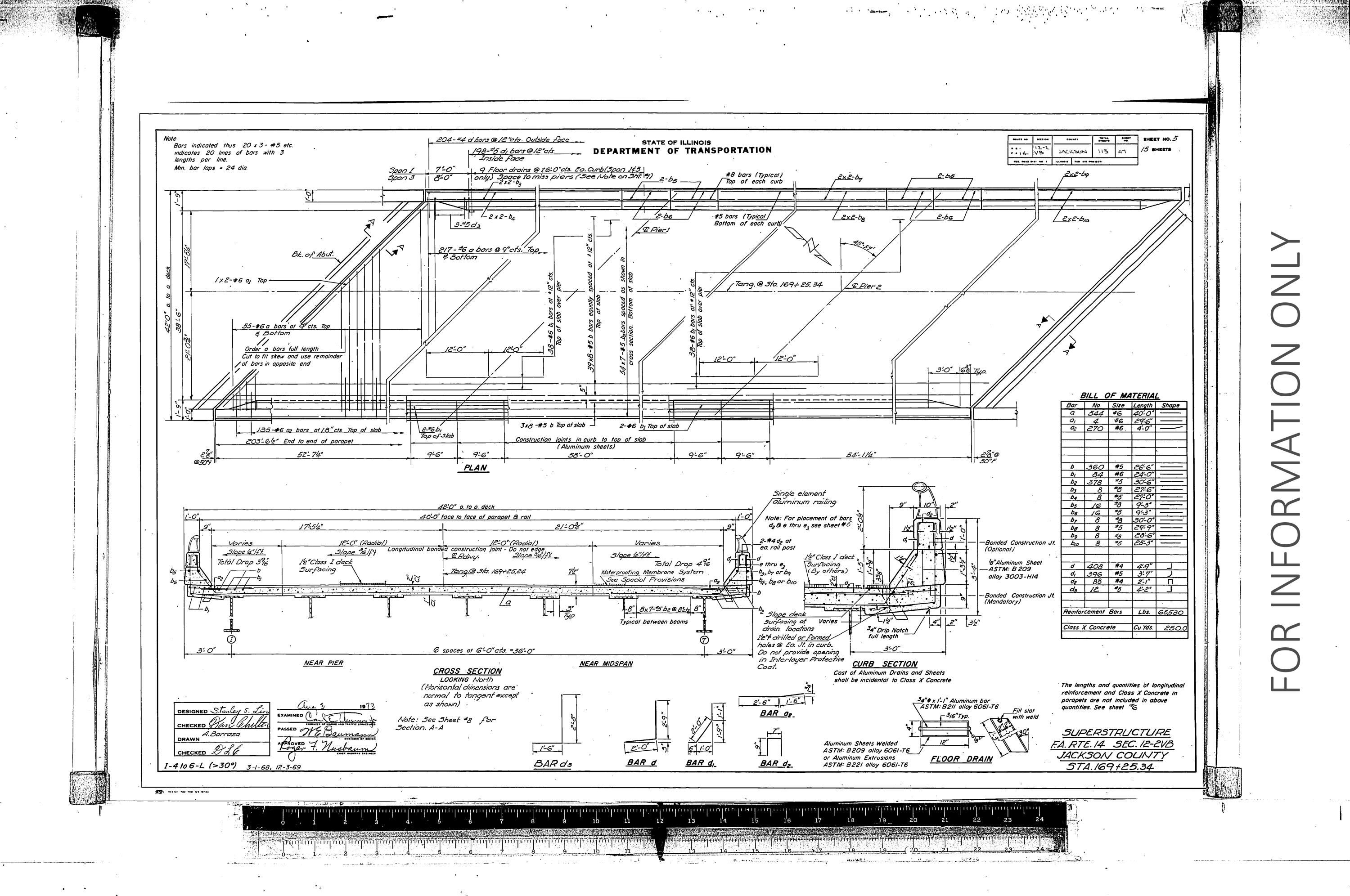
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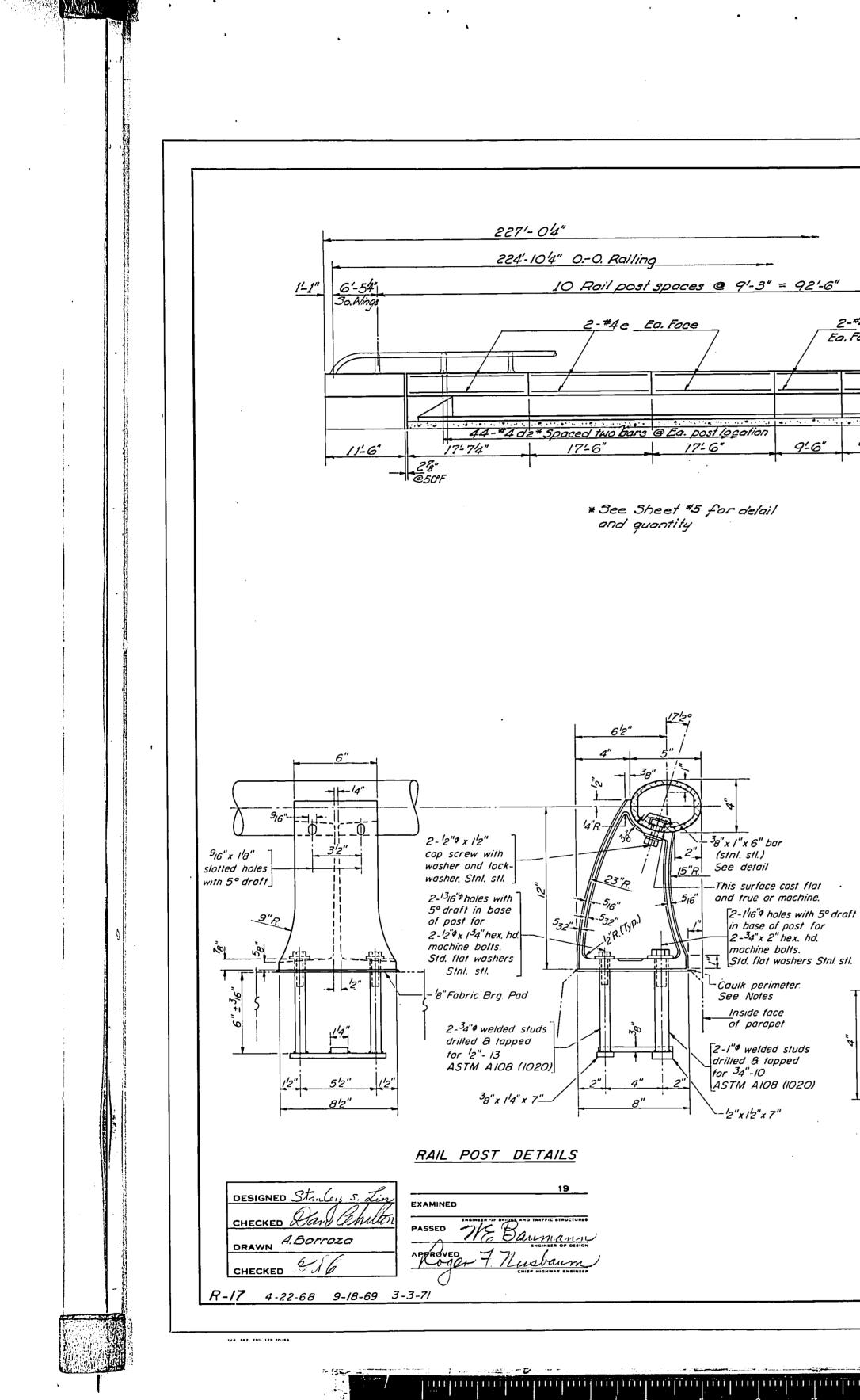
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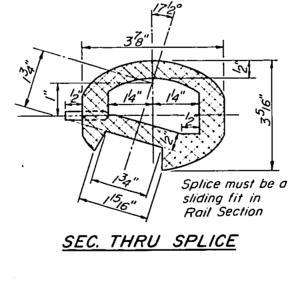


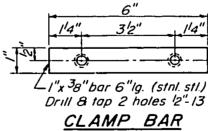
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POULE NU BECTION STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION • • \ 10 Rail post spaces @ 9'-3" = 92'-6" <u> 3 Rail post spaces@</u> 9'-0" = 27-0" 2-#4 e3 Ea. Face <u>2</u>-#4 e<u>,</u> [Ea.Face <u>2-</u>#4 e₂ <u>Ea. Face</u> <u>2-#4 ei</u> Ea, Face <u> 3 Spaces @ 19'-4" = 58'-0"</u> 9'-6" 9:6" 9-6 <u>ELEVATION</u> Showing inside face of West parapet 4'-3'4" ⁻¹8" Fabric Pad - Caulk perimeter - 2-1^{"\$}x6" Welded studs drilled & tapped for ³4"\$ x l¹2" hex. hd. 1'-2" 7" I stnl. stl. machine bolts with flat --- I'2" at expansion joint @ Abuts. stnl. stl. washers. _12" x 1/2" x 5" Bar --- ³8" at rail splice ₩€1"¢Holes ----¹2" Drain Notch ______ 516" Stnl. Stl. drive pin 14" long Two component non-staining RAIL SPLICE gray sealing compound with

SEC. THRU ELLIPTICAL RAIL SECTION

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NOTES:

2" Preformed Cork

Asphalt Joint Filler.

Cost incidental.

(meets qualifications for

ASTM: Designation D 1751)

polysulfide liquid polymers-

gun grade with primer.

All Aluminum Alloy Extruded Rail shall be supplied in modular lengths of 30 feet, except at the end of bridge or over open joints in bridge deck where the rail shall be attached to a minimum of 2 posts. If the rail is on a horizontal curve of 2300 foot radius or less, the modular lengths may be reduced but shall be attached to a minimum of 2 posts. All joints in rail shall be spliced per detail.

PARAPET JOINT DETAIL

12" 12" 12"

Provide $I = \frac{1}{8}$ and $2 = \frac{1}{16}$ Aluminum Shims for 25% of the Posts. Rail element shall be parallel to Grade – high spots shall be ground and low spots shimmed.

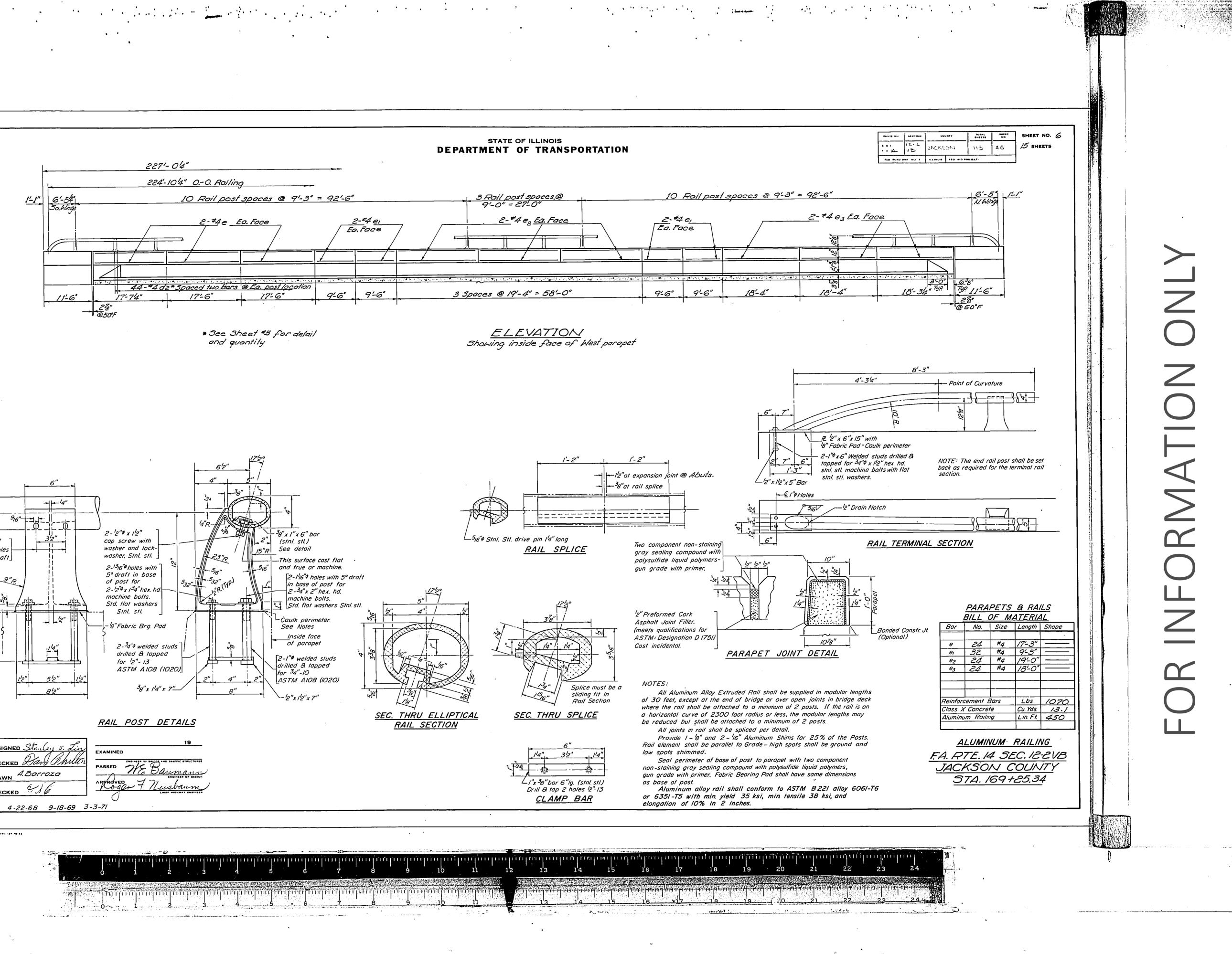
Seal perimeter of base of post to parapet with two component non-staining gray sealing compound with polysulfide liquid polymers, gun grade with primer. Fabric Bearing Pad shall have same dimensions as base of post.

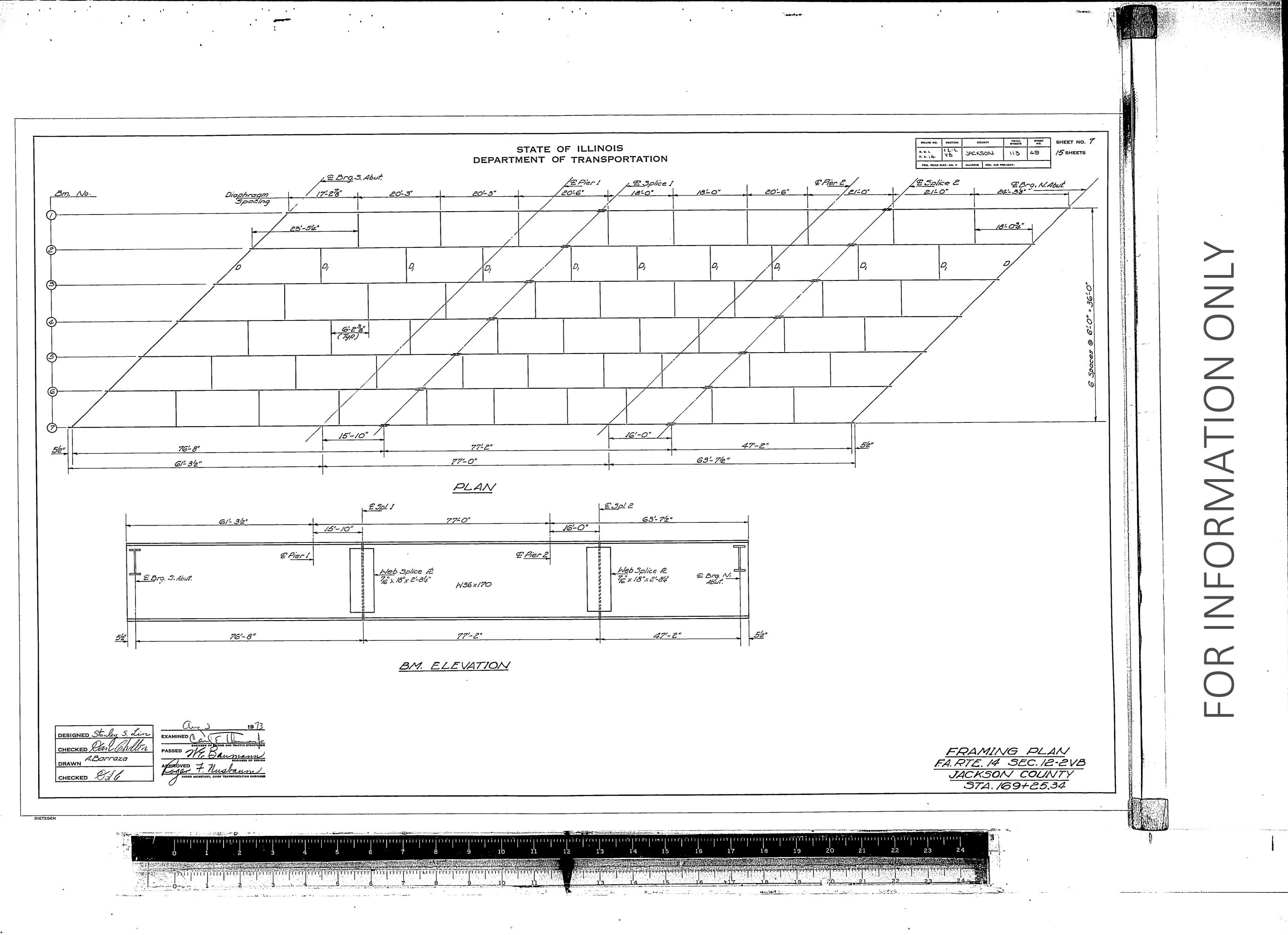
Aluminum alloy rail shall conform to ASTM B221 alloy 6061-T6 or 6351-T5 with min. yield 35 ksi, min. tensile 38 ksi, and elongation of 10% in 2 inches.

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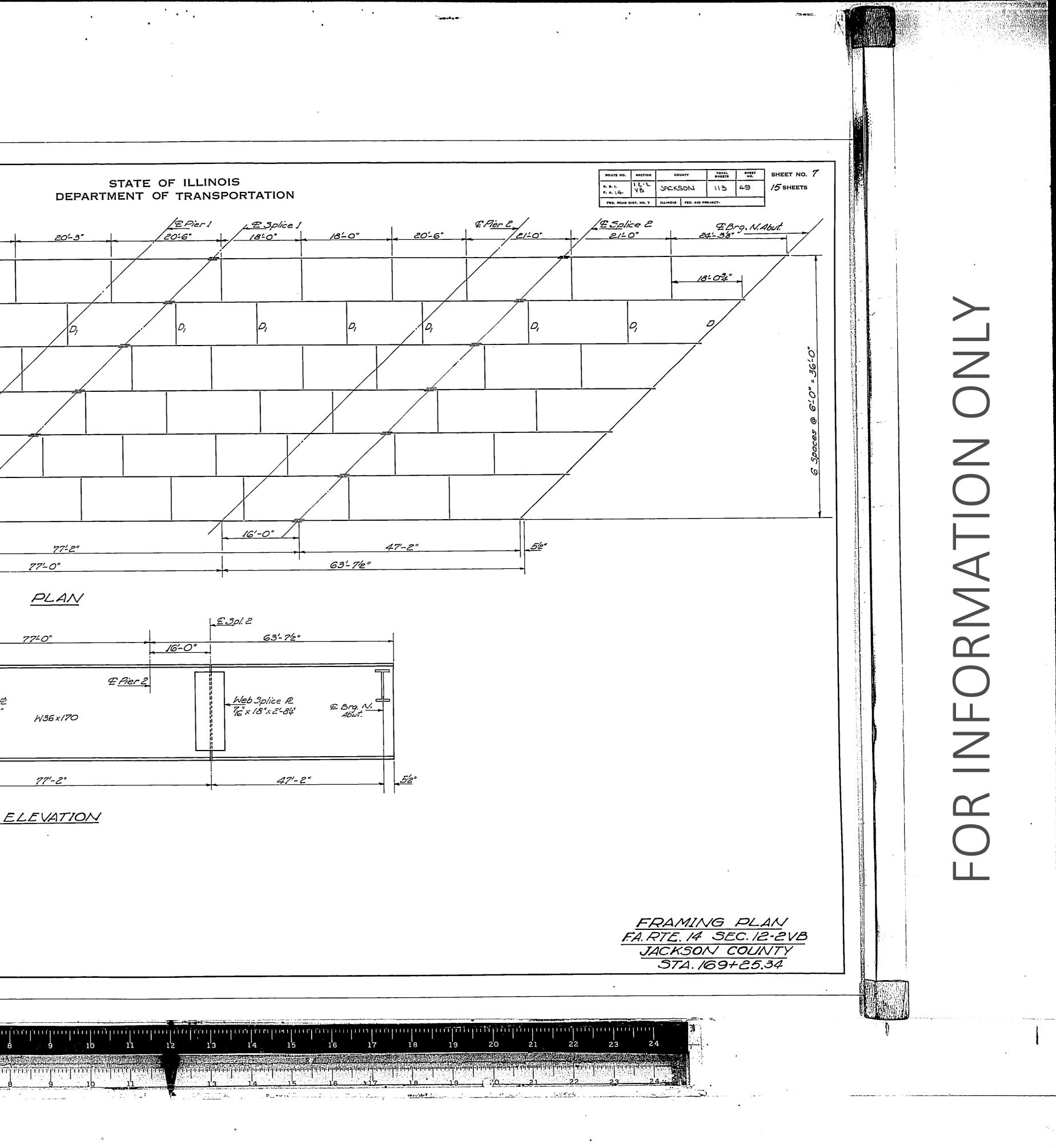
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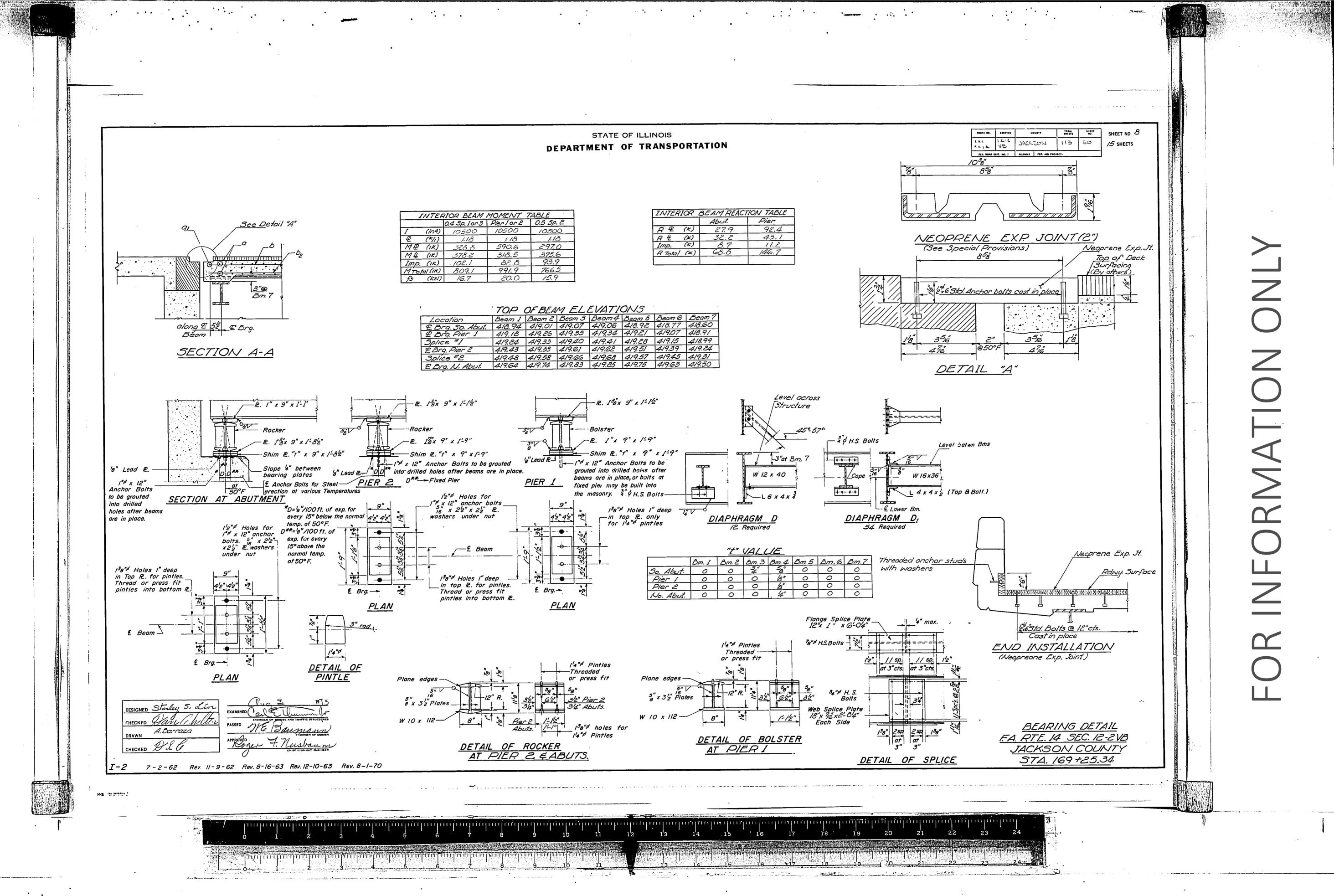


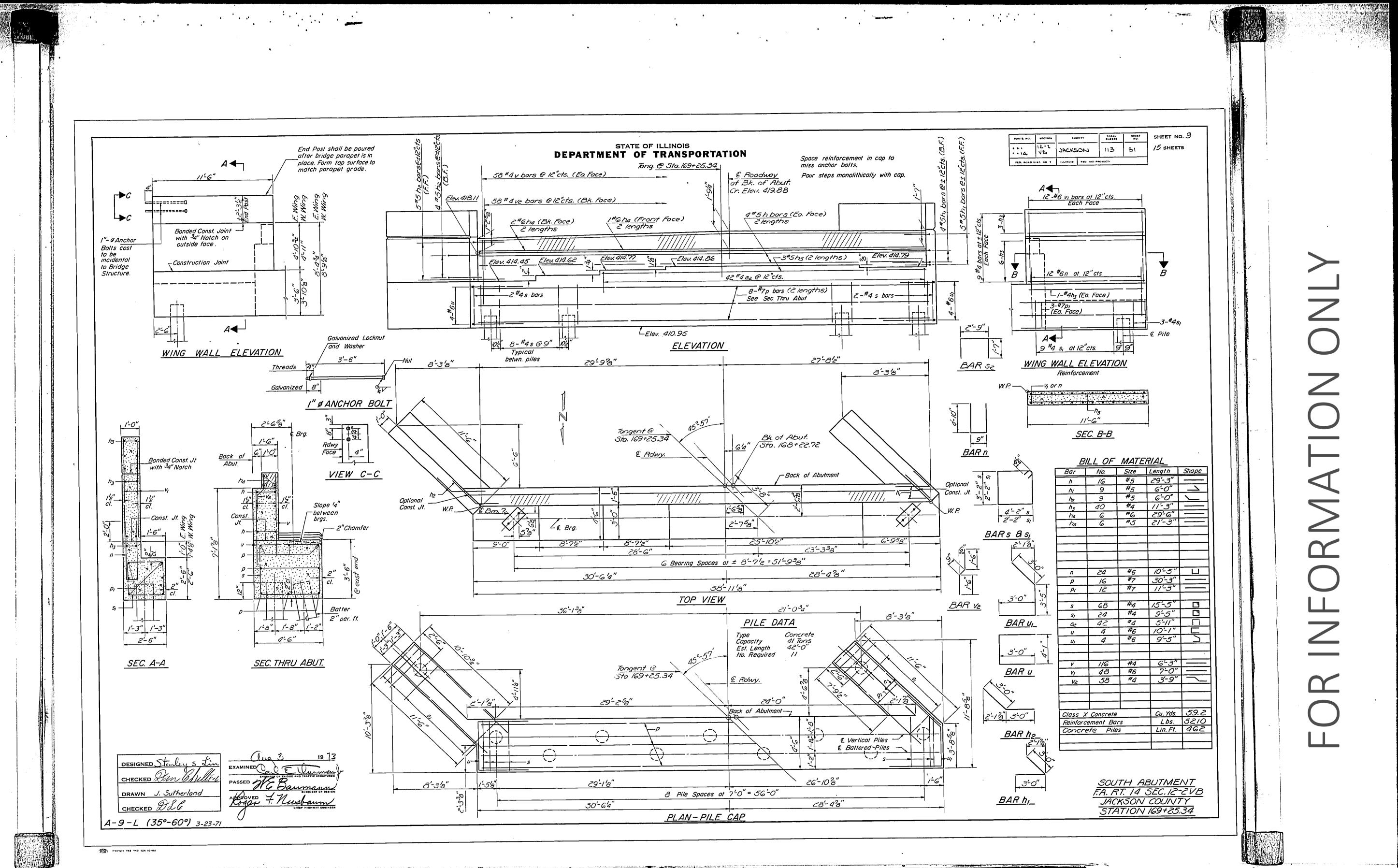


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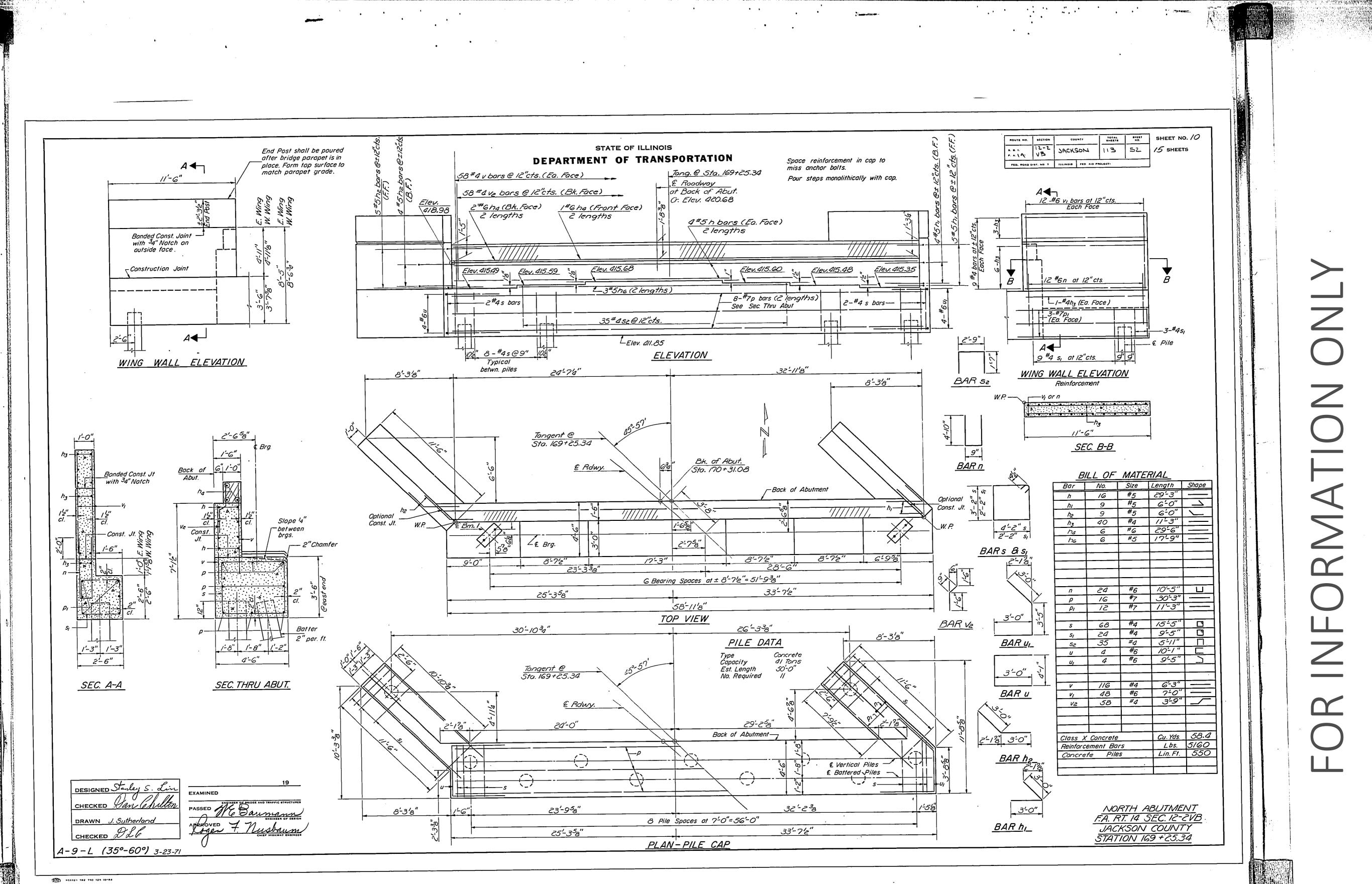
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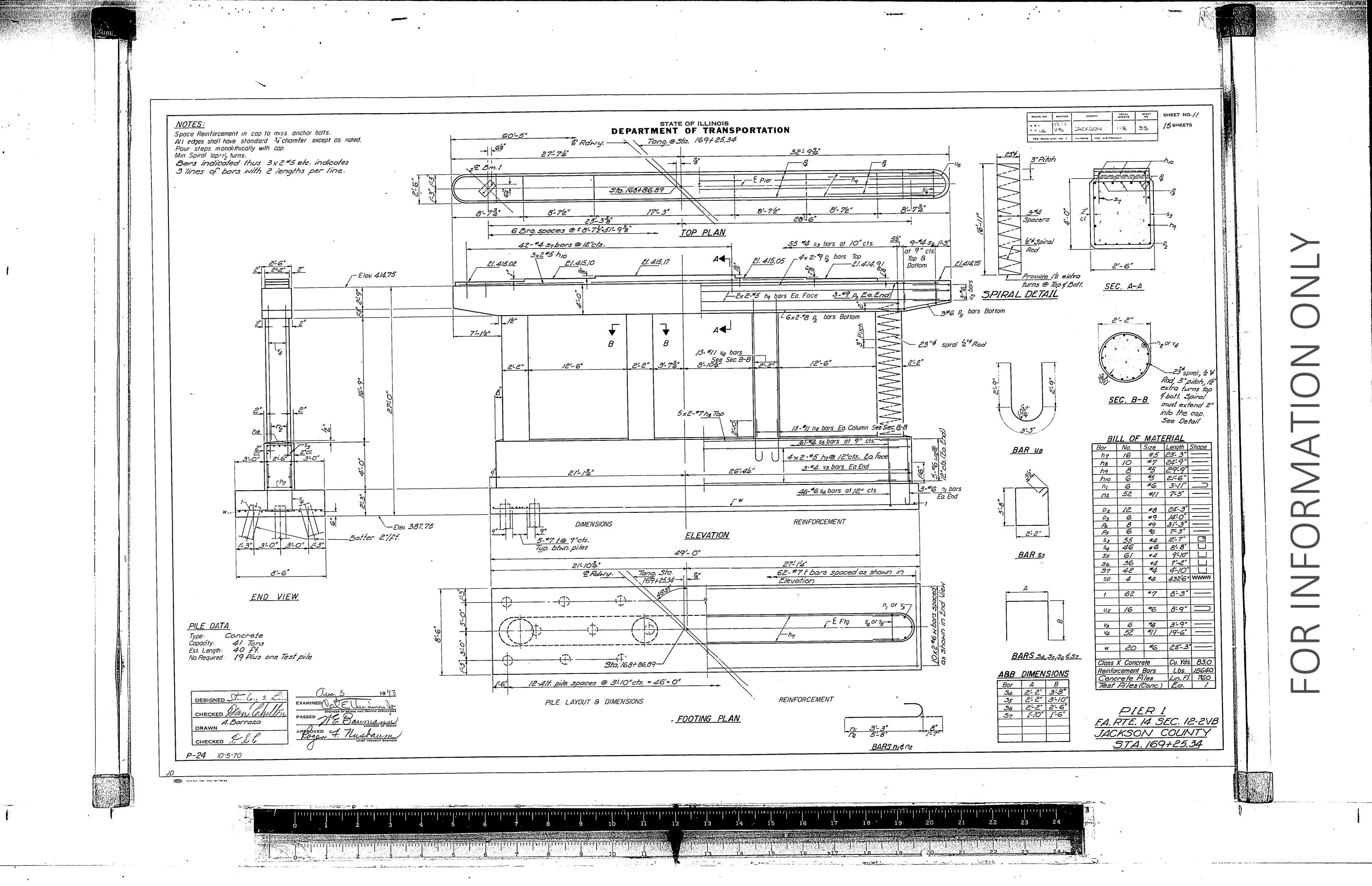
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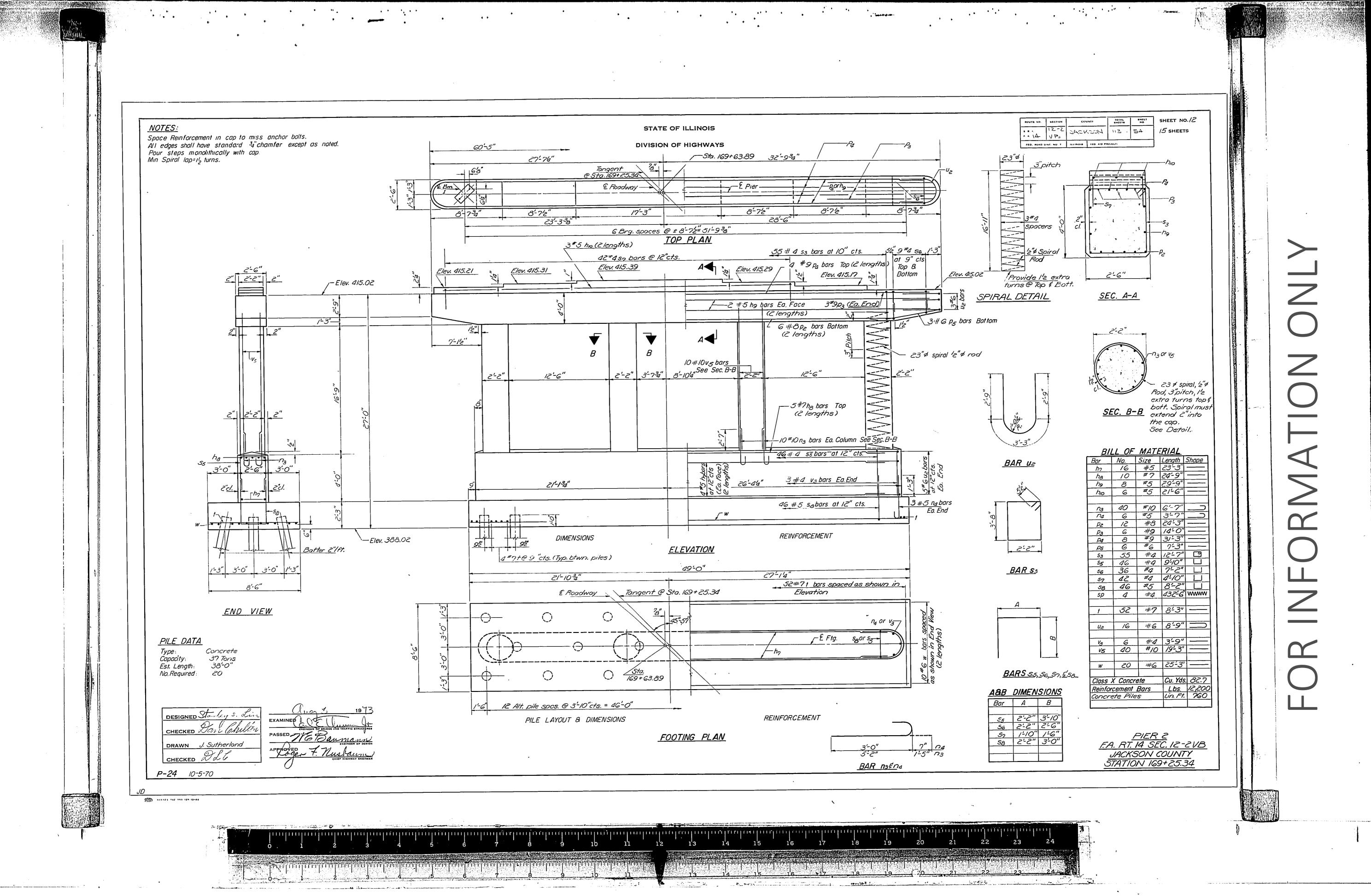


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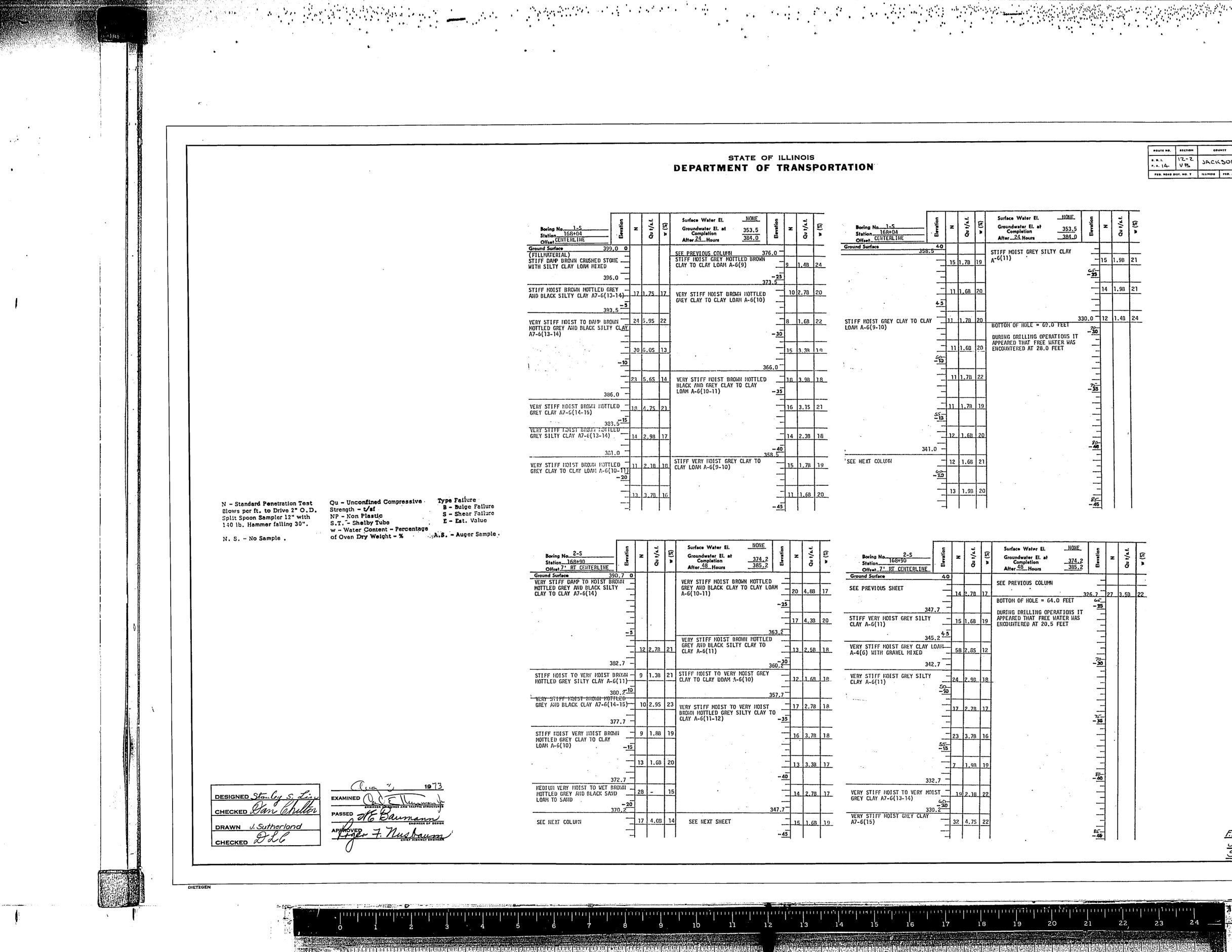
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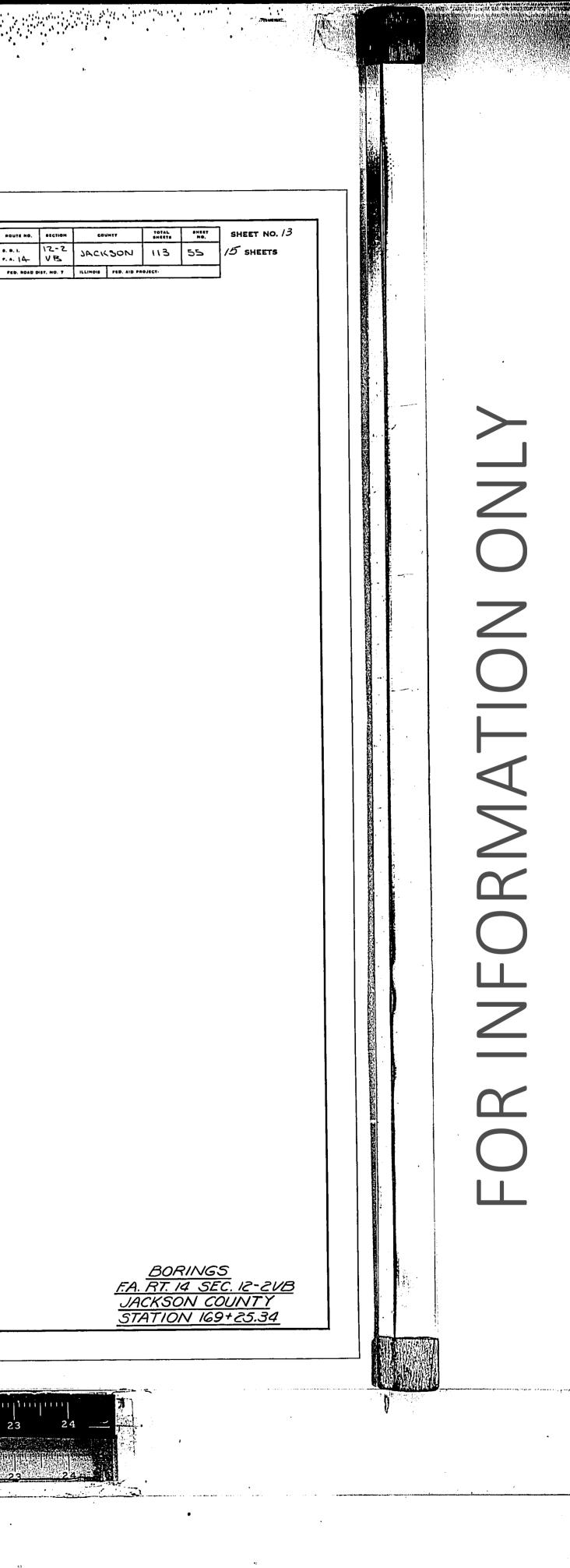
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ROUTE NO.

P.



Offet 7' LT CENTER Ground Surface 392.8 C VERY STIFF MOIST BROWN MOTTLED -GREY AND BLACK SILTY CLAY A7-6(13-14) 387.3 11 2.18 A7-6(13) VERY STIFF MOIST BROWN MOTTLED 11 2.5R GREY AND BLACK CLAY TO CLAY LOAM A-6(10) -10 379.8 MEDIUM VERY HOIST BROWN MOTTLED 9 GREY SILTY CLAY LOAM TO CLAY LOAM A-4(8) 377.3 377.3-15 MEDIUM WET BROWN COARSE SAND 374.8 VERY STIFF MOIST TO VERY MOIST 17 3.4B BROWN MOTTLED GREY AND BLACK CLAY TO CLAY LOAM A-6(10) -20 372.3 VERY STIFF MOIST BROWN MOTTLED 18 3.98. GREY AND BLACK SILTY CLAY TO CLAY A7-6(14) WITH GRAVEL Qu - Unconfined Compressive Strength - t/sf Type Failure N - Standard Penetration Test B - Bulge Failure S - Shear Failure Elows per ft. to Drive 2" O.D. Split Spoon Sampler 12" with NP - Non Plastic E - Est. Value S.T. - Shelby Tube 140 lb. Hammer falling 30". w - Water Content - Percentage of Oven Dry Weight - % A.S. - Auger Sample. N. S. - No Sample Boring No. 4-5 Station 170+30 Offset_CENTERLINE Ground Surface 40 MEDIUM MOIST BROWN MOTTLED GREY SILTY CLAY LOAM A-4(8) TO A-6(9) VERY STIFF HOIST BROWN MOTTLED ______ GREY SILTY CLAY A7-6(14) _____ 15 3.18 VERY STIFF MOIST TO DAMP BROWN MOTTLED GREY SILTY CLAY A7-6(13-14) - 10 2.1S VERY STIFF HOIST BROWN MOTTLED GREY SILTY CLAY A-6(11) VERY STIFF MOIST BROWN MOTTLED GREY SILTY CLAY TO CLAY A7-6(14) STIFF TO VERY STIFF MOIST BROWN MOTTLED GREY AND BLACK CLAY TO CLAY LOAM A-6(10) DESIGNED Stanley S. Lin HECKED Dan Chill DRAWN J. Sutherland CHECKED BLG K&E CO. 19-1255

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Invite.

ə 5

		STIFF MOIST BROWN MOTTLED GREY CLAY LOAM A-4(6)	-	11	1,6B	19
		37	5-825			
18	24	VERY SOFT WET BROWN SANDY CLAY LOAM A-4(4)		3	0,2B	21
		•				<u> </u>
			72.8 -			· ·
15	28	MEDIUM WET BROWN COARSE SAND	_	18		
		33	70.30			
2S	11			14	2,38	18
				{ ·		1
 7S	14	VERY STIFF MOIST BROWN MOTTLED GREY AND BLACK CLAY TO CLAY				
15	14	LOAM A-6(10-11)		- <u>16</u> .	<u>3.9B</u>	18
			- <u>35</u>	 		
95	22		-	14	2.95	18
			. —	1		
8B	17			15	3.1B	17
			- 40	-		
		3(50,3	1		<u> </u>
65	21	SEE NEXT SHEET		14	<u>2.78</u>	18
					ľ	
.05	21			10	1.6B	20
		• • • • •		1	1.00.	<u></u>
		•	- 45			

NONE

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386.2

377.8 -

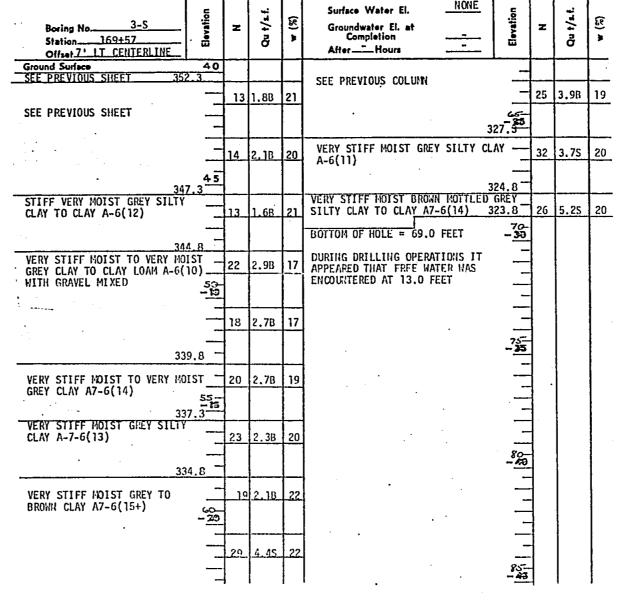
Surface Water E

Groundwater El. at Completion After <u>24</u> Hours

SEE PREVIOUS COLUMN

Boring No. 4-S Station 170+30 Offset CENTERLINE	Qu t/a.f.	(%) *	Surface Water El. NUNE. Groundwater El. at 385.3 Completion 386.2	Elevation	Z	Qu t/s.f.	(%) A
Ground Surface 40 SEE PICE VIOUS SHEET360.3			STIFF MOIST GREY SILTY CLAY TO CLAY A-6(10-11)	-			
	2.7B	18	CLAI A-0(10-11)	· _	14	2.1B	20
				- دى:			
STIFF MOIST GREY CLAY TO CLAY				-12			
LOAN A-6(10) - 10	1.6B	20			14	1.8B	18
45							
			VERY STIFF MOIST GREY CLAY	332.8			┣
	1.4B	20	A7-6(14) BOTTOM OF HOLE = 69.0 FEET	331.8	25	3.3B_	24
······································				- 30			
			DURING DRILLING OPERATIONS IT APPEARED THAT FREE WATER WAS				
	1.5B	20	ENCOUNTERED AT 25.5 FEET	•			
350.3				-			
VERY STIFF MOIST GREY SILTY 15	1.9B	19		_		ļ	
CLAY A-G(10)				73			
-	<u> </u>			-32			
- 12	2.1B	22			,	ļ	
		<u> </u>					
SEE NEXT COLUMN 14	<u>1.9B</u>	16					
				- 40	ĺ		
	1 50	1	•				
- 12	1.5B	18		· •			1
- <u>20</u>					}		
	1.7B	19		· –	ł		
	1	<u> </u>		85- - 45	{		
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(%) *	Surface Water El. NONE Groundwater El. at	Elevation	Z	Qu t/s.f.	~ (½) ~
	SEE PREVIOUS COLUMN 36	59.8			1
	VERY STIFF MOIST TO VERY MOIST BROWN MOTTLED GREY AND BLACK CI TO CLAY LOAM A-6(10)	LAY	21	<u>6.OB</u>	<u>17 '</u>
			17	3.5B	18
		_	<u> </u>	ļ	
20			15_	2.5B	19
. 		- <u>30</u>		·	
23			_18	4.6S	17
	3	59.8	ļ	ļ	
21	STIFF VERY MOIST GREY CLAY TO CLAY LOAM A-6(9-10)		lu	1.6B	20
		- <u>35</u>			
22		-	12	1.68	19_
		_			
-			14	1.8B	21
	3	352 <u>-</u> 40			
18	STIES TO VERY STIES VERY WAIST		13	1.88	21
_		_	-	·	
12		-	14	2.18	20
ļ		- 45	1		l



STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ROUTE NO. SECTION

