

01-19-2024 LETTING ITEM 097

INDEX OF SHEETS

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- 2 TYPICAL SECTIONS
- 2 SUMMARY OF QUANTITIES
- 2 PLAN - PROFILE
- 3-4 CROSS SECTIONS
- 5-13 BRIDGE PLANS
- 14-15 BORINGS

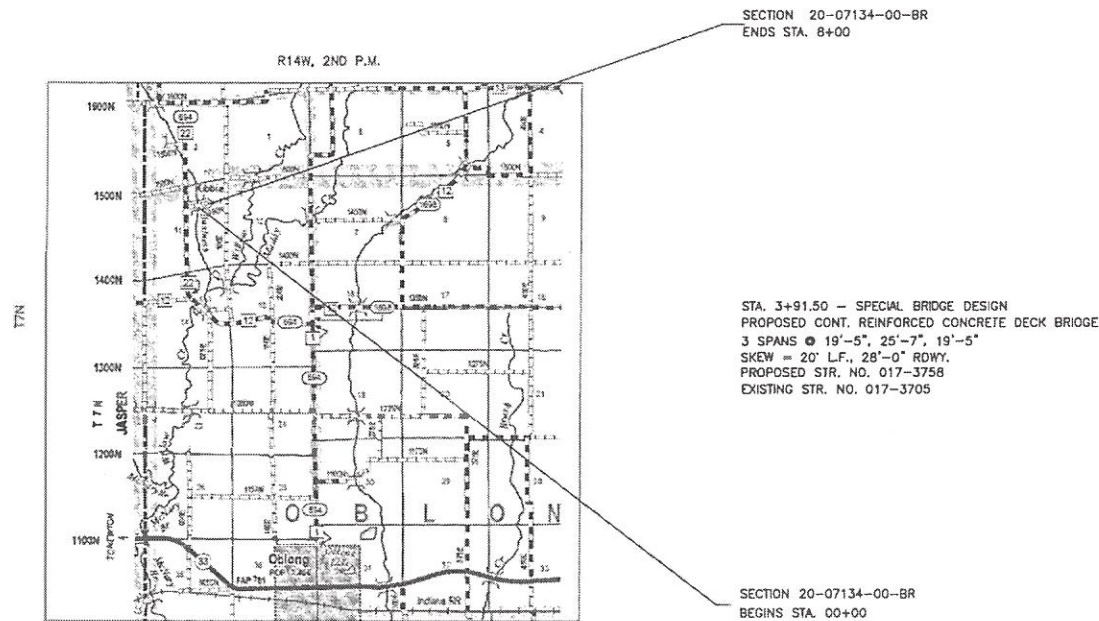
STANDARDS: 515001-04
701901-09
725001-01
BLR 21-9

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
PLANS FOR PROPOSED
SURFACE TRANSPORTATION PROGRAM-OFF SYSTEM BRIDGE
CRAWFORD COUNTY
SECTION 20-07134-00-BR
STRUCTURE NO. 017-3758
TR 93 OVER NO BUSINESS CREEK
PROJECT NO. TT08(650)
JOB NO. C-97-061-22
CONTRACT NO. 95933



SCALES

PLAN 1 INCH = 50 FEET
CROSS SECTIONS 1 INCH = 5 FEET
PROFILE HORZ. 1 INCH = 50 FEET
PROFILE VERT. 1 INCH = 10 FEET



LOCATION MAP

APPROXIMATE SCALE: 1 INCH = 1 MILE
NET LENGTH = 800.00 FT. = 0.15 MILES

CLASS - LOCAL ROAD
ADT = 50
DESIGN SPEED = 30 MPH

TOLL FREE JOINT UTILITY LOCATING
INFORMATION FOR EXCAVATORS (J.U.L.I.E.)
TELEPHONE NO. 1-800-892-0123



John A. Stone 02/10/2023
ILLINOIS REGISTERED PROFESSIONAL ENGINEER # 55012
LICENSE EXPIRES NOVEMBER 30, 2023
PROFESSIONAL DESIGN FIRM #184-000832

ILLINOIS DEPARTMENT OF TRANSPORTATION	
APPROVED: February 14, 2023	<i>Just R. Chiu</i> CRAWFORD COUNTY ENGINEER
PASSED: 11/01/23	<i>Scott Walker</i> DISTRICT SEVEN ENGINEER OF LOCAL ROADS & STREETS
RELEASING FOR BID BASED ON LIMITED REVIEW	11/01/23 <i>Jimmyson</i> REGION FOUR ENGINEER

Connor & Connor, Inc.
210 East Locust Street
P.O. Box 618
Robinson, Illinois 62454



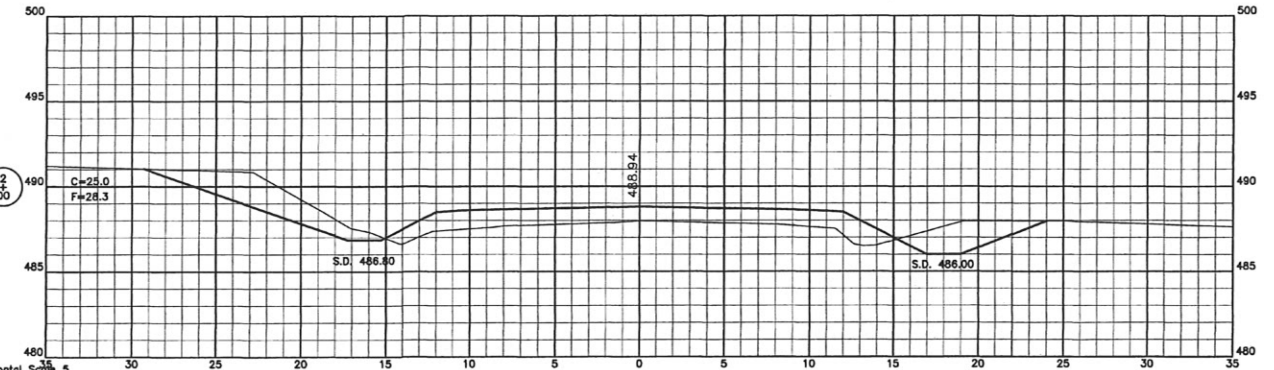
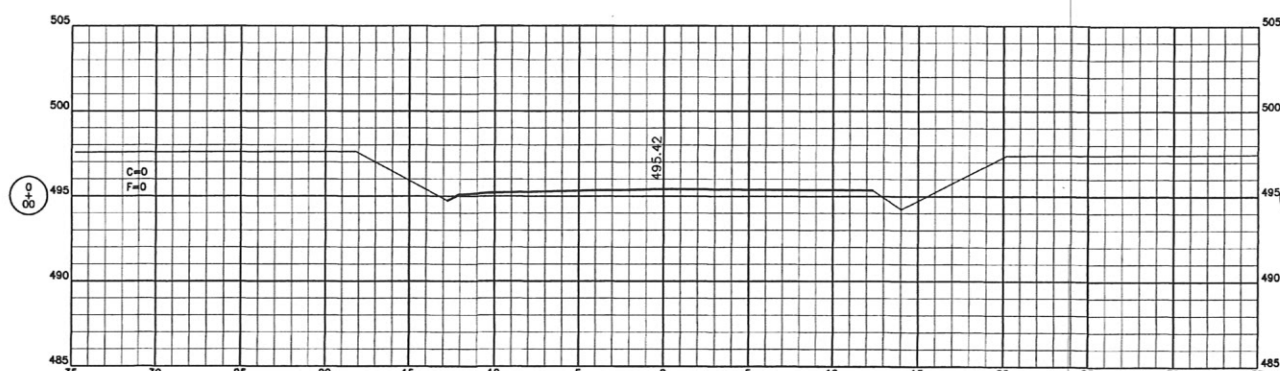
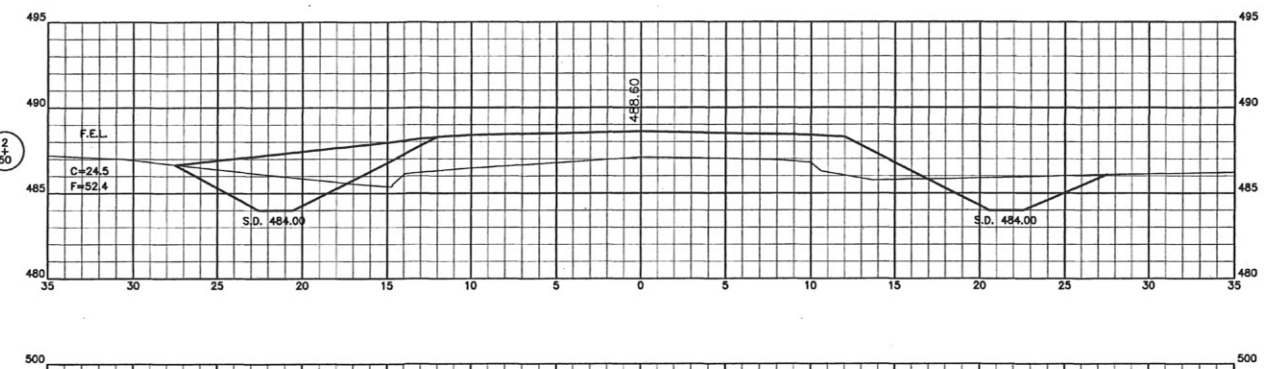
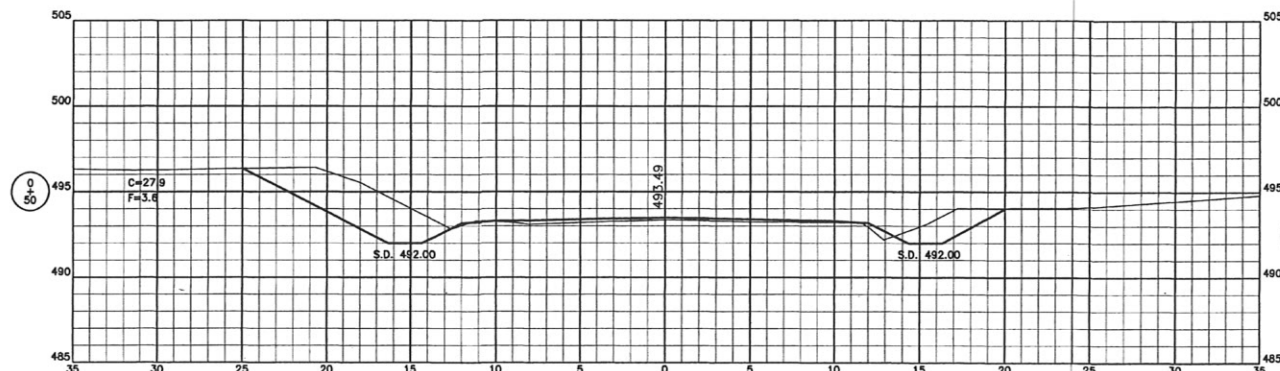
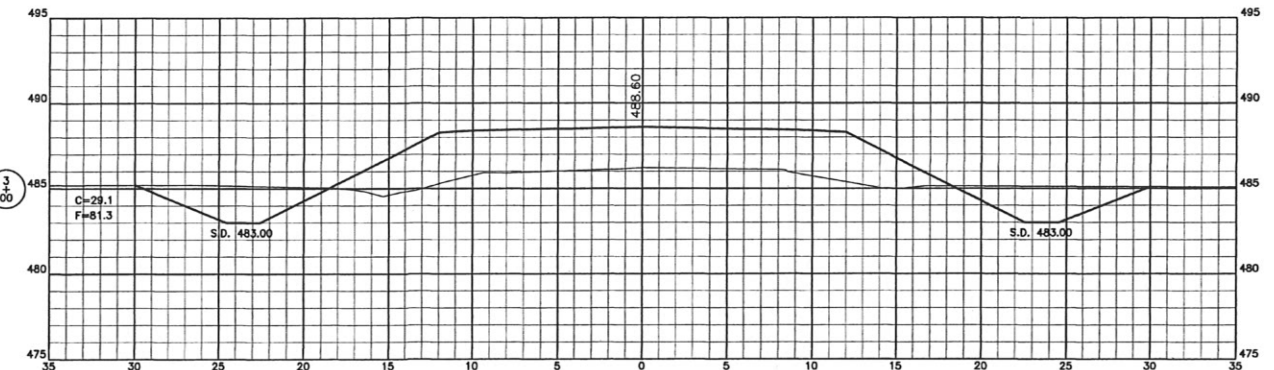
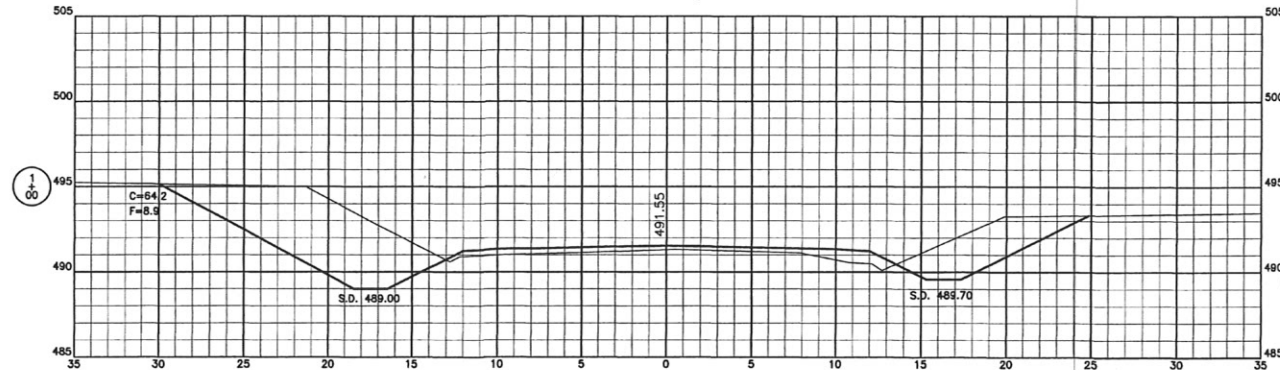
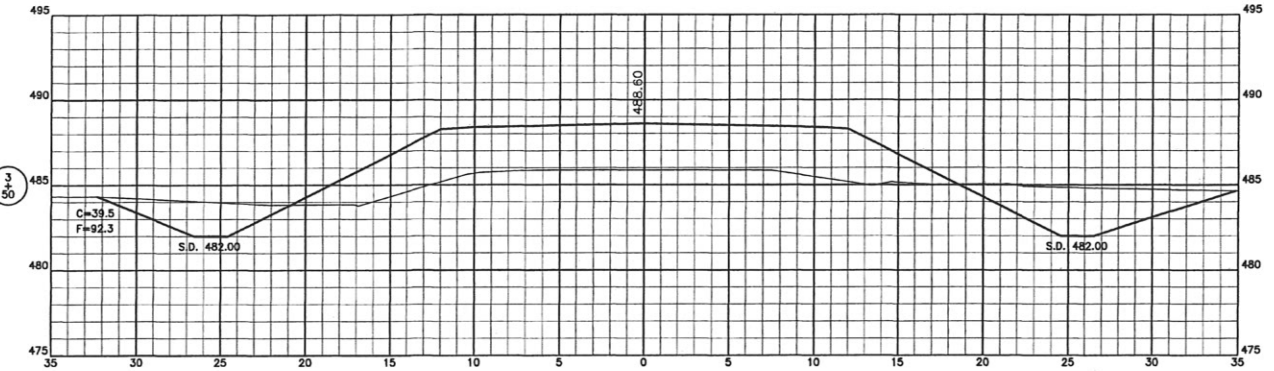
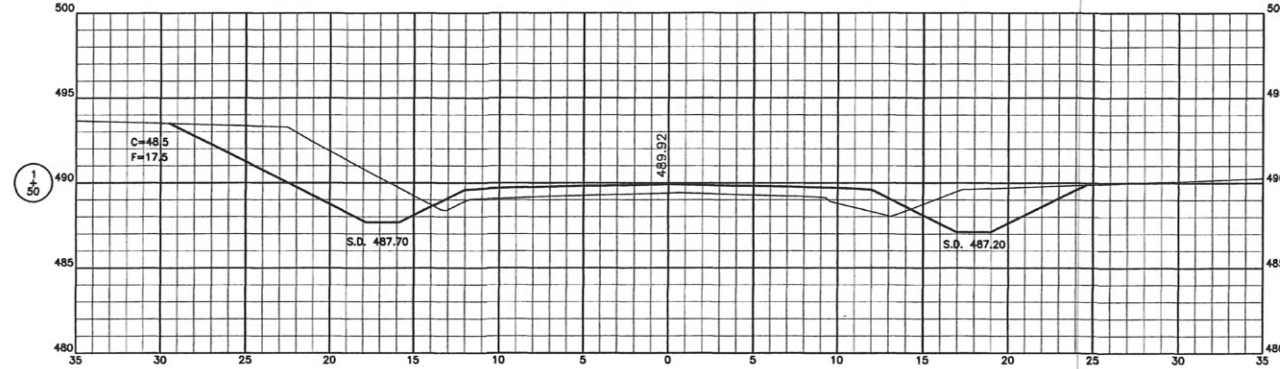
Phone: (618) 544-8623
Fax: (618) 544-3012
Design Firm #: 184-000832
www.connorengeers.com

PROJECT NAME: CRAWFORD COUNTY
SECTION 20-07134-00-BR
OBLONG ROAD DISTRICT
TR 93

COVER SHEET

SHEET NUMBER:
1 OF 15
CONTRACT NUMBER:
95933

BRIDGE



Horizontal Scale 5
Vertical Scale 5

Connor & Connor, Inc.
210 East Locust Street
P.O. Box 618
Robinson, Illinois 62454

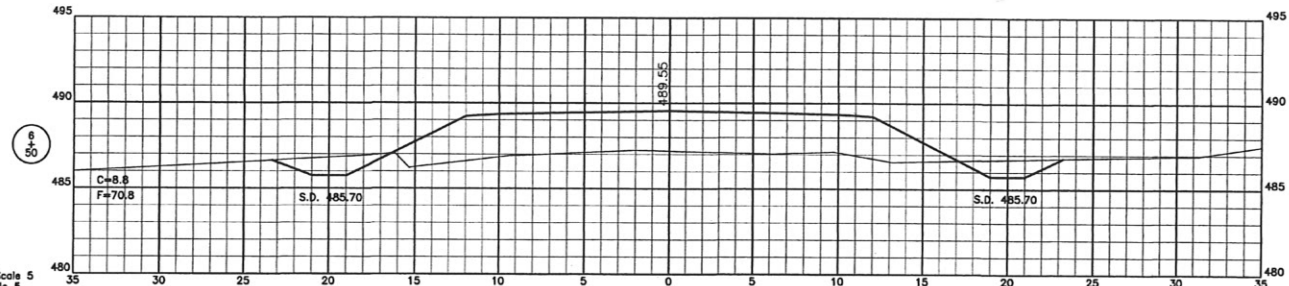
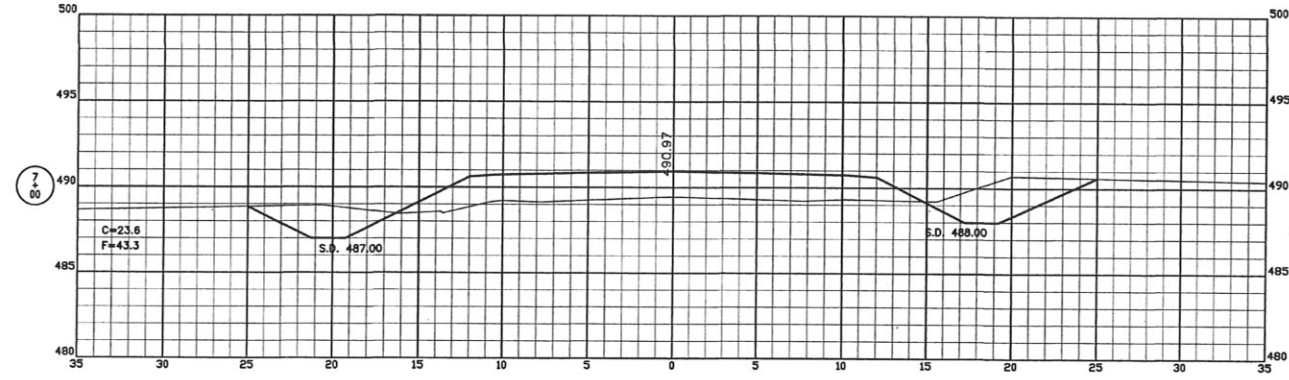
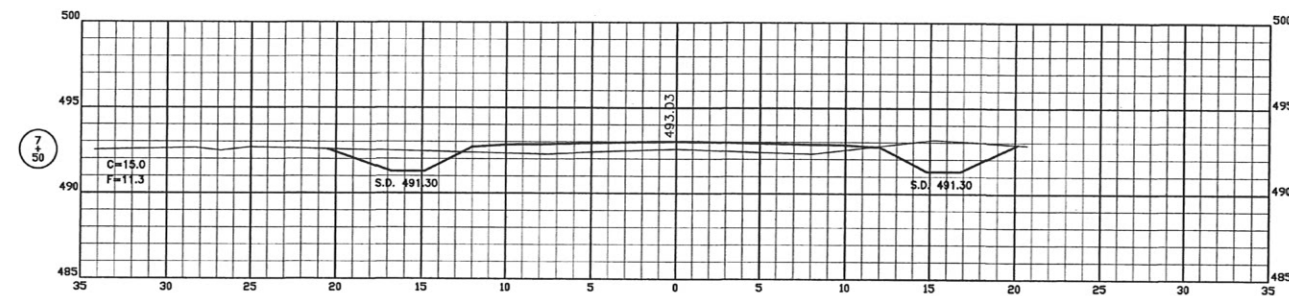
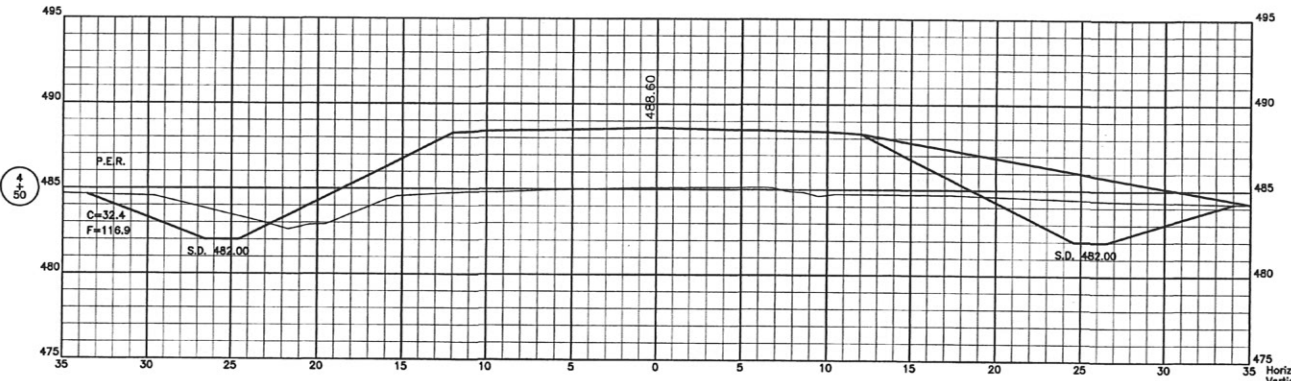
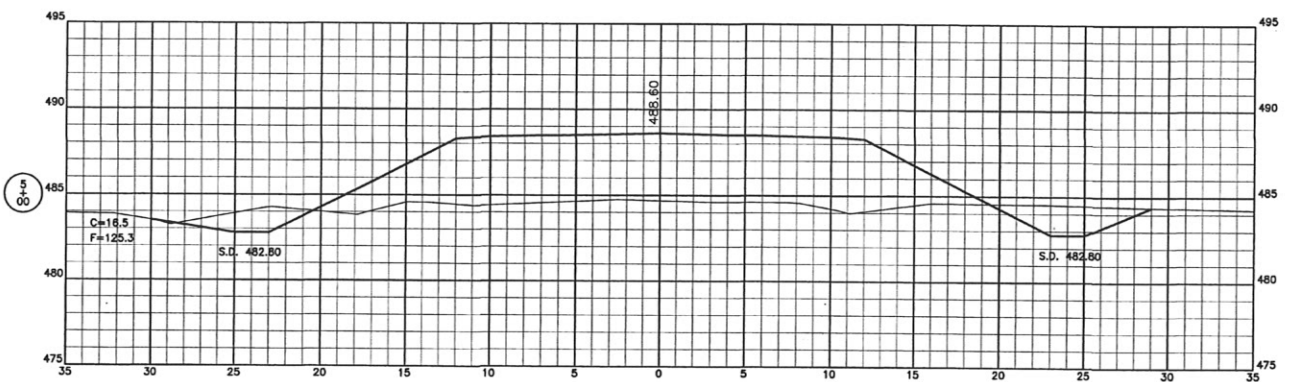
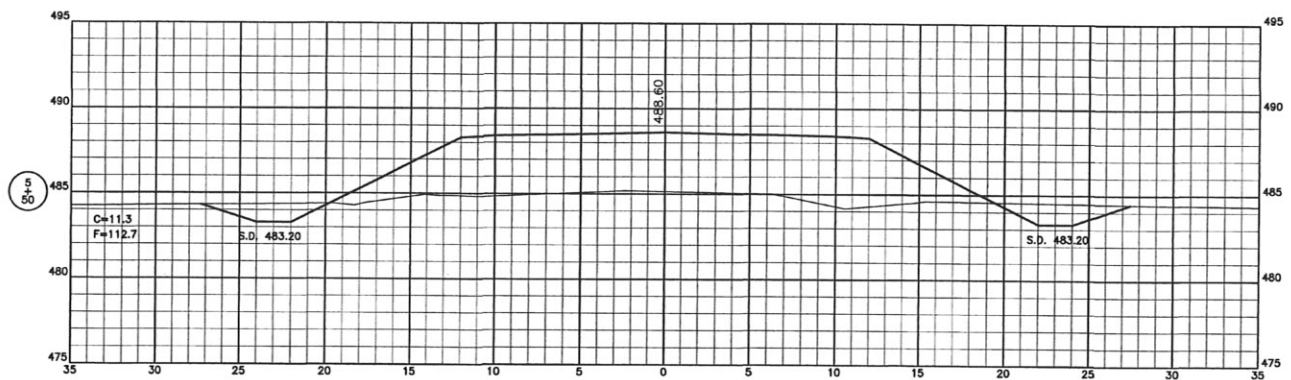
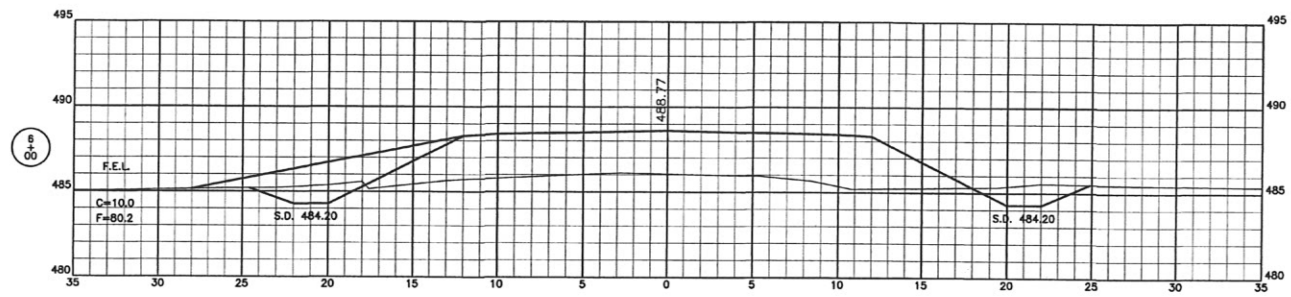
CONNOR & CONNOR
Consulting Engineers
Land Surveyors

Phone: (618) 544-8623
Fax: (618) 544-3012
Design Firm #: 184-000832
www.connorengineers.com

PROJECT NAME: CRAWFORD COUNTY
SECTION 20-07134-00-BR
OBLONG ROAD DISTRICT
TR 93

CROSS SECTIONS

SHEET NUMBER:
3 OF 15
CONTRACT NO.
95933



Horizontal Scale 5
Vertical Scale 5

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210 East Locust Street
P.O. Box 618
Robinson, Illinois 62454

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PROJECT NAME: CRAWFORD COUNTY
SECTION 20-07134-00-BR
OBLONG ROAD DISTRICT
TR 93

CROSS SECTIONS

SHEET NUMBER:
4 OF 15
CONTRACT NO.
95933

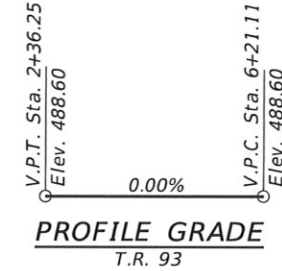
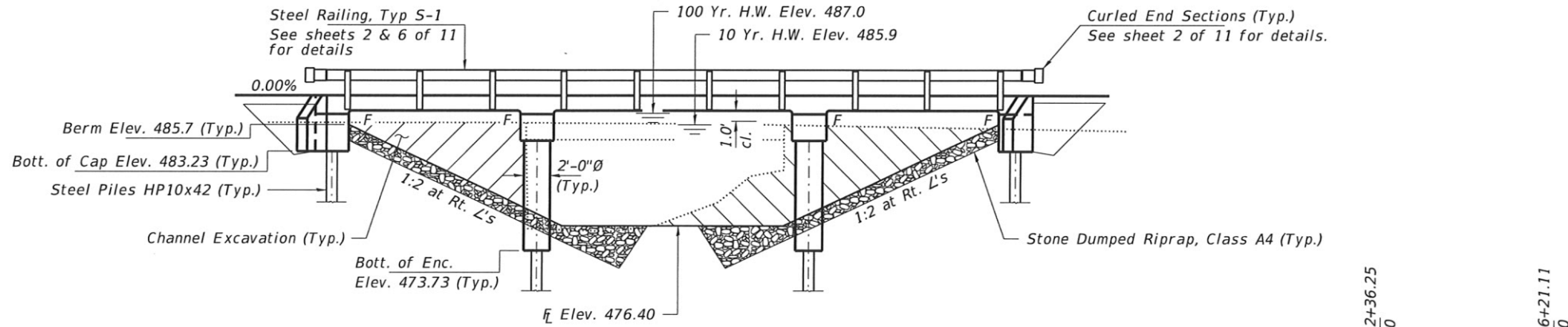
EXISTING STRUCTURE NO. 017-3705: Single span RC slab bridge with concrete deck on closed concrete abutments and wings. Single span at 26.0', 13.0' Rdwy., Skew = 0°

Road closed to traffic during construction.

No Salvage

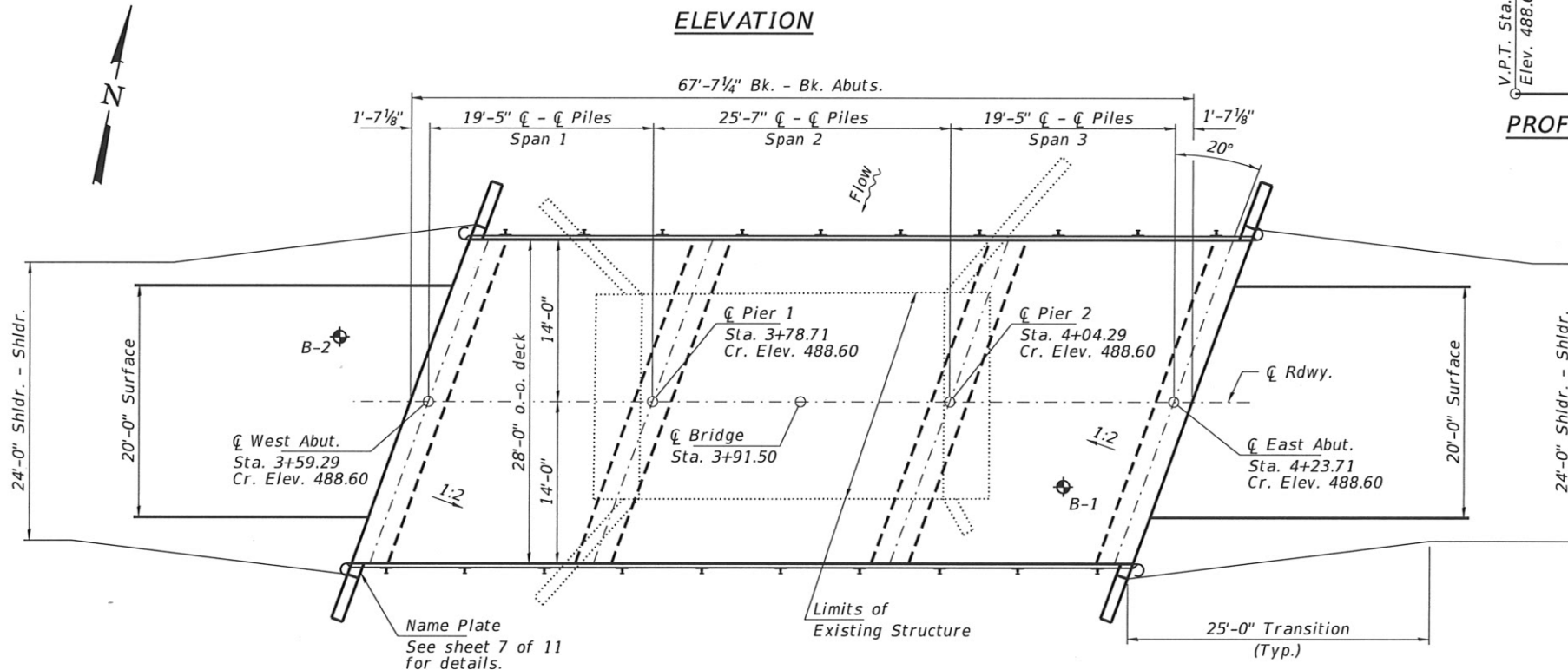
GENERAL NOTES

Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
 The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at East Abutment and Pier 1 or approved by the Engineer before ordering the remainder of piles.
 All proposed construction activities shall be in accordance with Nationwide Permit number 14 of the Department of the Army authorized under Section 404 of the Clean Water Act.
 The Contractor shall make allowance for the deflection of forms, shrinkage, and settlement of falsework, in addition to allowance for dead load deflection. Forms for deck slab shall be removed prior to placement of bridge approach slab.
 Protective Coat shall be applied to the top surface and the fascia of the concrete deck and exposed face of the wingwalls.
 Reinforcement bars designated (E) shall be epoxy coated.
 Excavation required to construct the Abutments and Piers shall be included in the cost of Concrete Structures. No additional compensation will be allowed for Structure Excavation or Cofferdam Excavation.



INDEX OF STRUCTURE SHEETS

1. General Plan & Elevation
2. General Details
3. Top of Slab Elevations
4. Superstructure
5. Superstructure Details
6. Steel Railing, Type S-1
7. Abutments
8. Piers
9. HP Pile Details
- 10-11. Borings



SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
 Design Spectral Acceleration at 1.0 sec. (SD1) = 0.146g
 Design Spectral Acceleration at 0.2 sec. (SDS) = 0.357g
 Soil Site Class = c

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications, 9th Edition with all interims.

LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

DESIGN STRESSES

FIELD UNITS

f'c = 5,000 psi (Superstructure)
 f'c = 3,500 psi (Substructure)
 fy = 60,000 psi (Reinf.)
 fy = 50,000 psi (Steel H-Pile) (M270 Gr. 50)

OVER-THE-ROAD FLOW AREA

Freq. Yr.	Exist.	Prop.
10	155	0
100	402	0

DESIGN SCOUR ELEVATION TABLE

Event/Limit State	Design Scour Elevations (ft.)				Item 113
	W. Abut.	Pier 1	Pier 2	E. Abut.	
Q100	483.23	473.73	473.73	483.23	5
Q200	483.23	473.73	473.73	483.23	
Design	483.23	473.73	473.73	483.23	
Check	483.23	473.73	473.73	483.23	

WATERWAY INFORMATION

Drainage Area = 5.61 Mi² Existing Low Grade Elev. 484.4 at Sta. 3+90 Proposed Low Grade Elev. 486.9 at Sta. 8+40.5

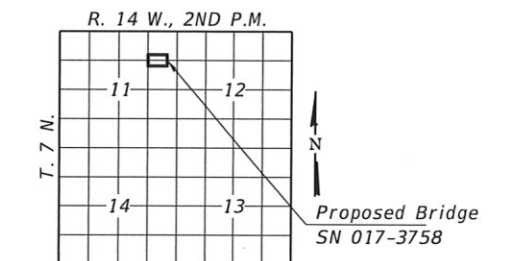
Flood	Freq. Yr.	Q C.F.S.	Opening Ft ²		Head - Ft.		Headwater El.		
			Exist.	Prop.	H.W.E. Exist.	H.W.E. Prop.	Exist.	Prop.	
Design	10	1210	165	331	485.9	-	-	485.9	485.9
Base	100	2270	165	390	487.0	0.0	0.8	487.0	487.0

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current "AASHTO LRFD Specifications."

Steven W. Megginson 02/09/2023
 ILLINOIS STRUCTURAL ENGINEER NO. 081-6064



Expires 11-30-2024

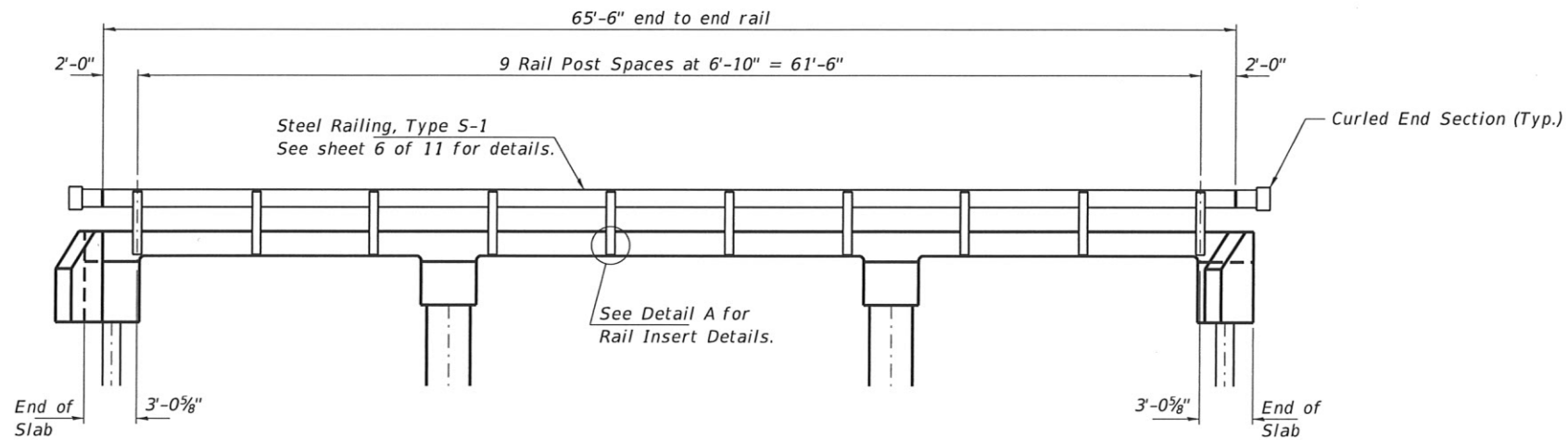


LOCATION SKETCH

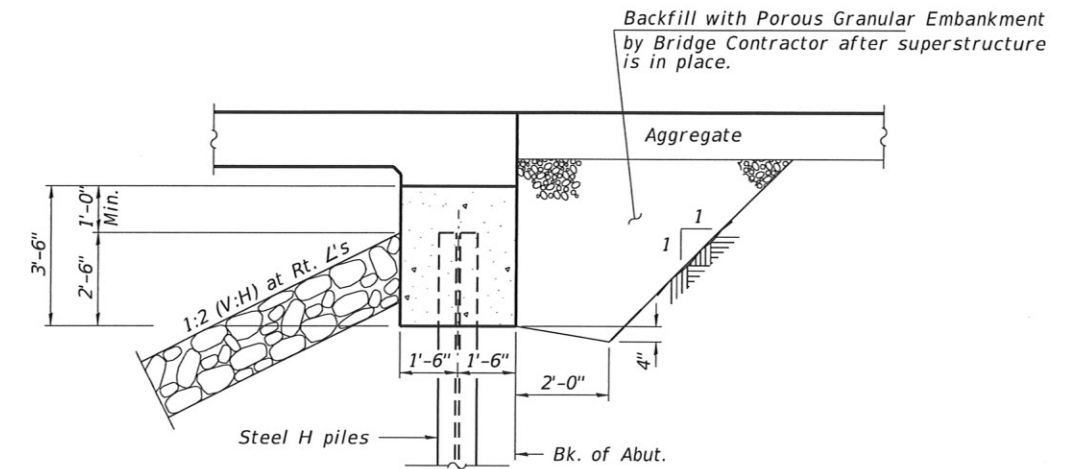
GENERAL PLAN & ELEVATION

T.R. 93
 OVER NO BUSINESS CREEK
 SECTION 20-07134-00-BR
 CRAWFORD COUNTY
 STATION 3+91.50
 STRUCTURE NO. 017-3758

FILE NAME = 210076-eh-bridge.dgn	USER NAME = rthoick	DESIGNED - J.R.B.	REVISED -	STATE OF ILLINOIS CRAWFORD COUNTY HIGHWAY DEPARTMENT	GENERAL PLAN AND ELEVATION STRUCTURE NO. 017-3758	T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 308 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	CHECKED - S.T.M./S.W.M.	REVISED -			93	20-07134-00-BR	CRAWFORD	15	5
ILLINOIS PROFESSIONAL DESIGN FIRM 18 / PE / SE CORP. 184.909899	PLOT DATE = 2/9/2023	DRAWN - R.D.H.	REVISED -			OBLONG ROAD DISTRICT				CONTRACT NO. 959933
		CHECKED - S.T.M./S.W.M.	REVISED -							

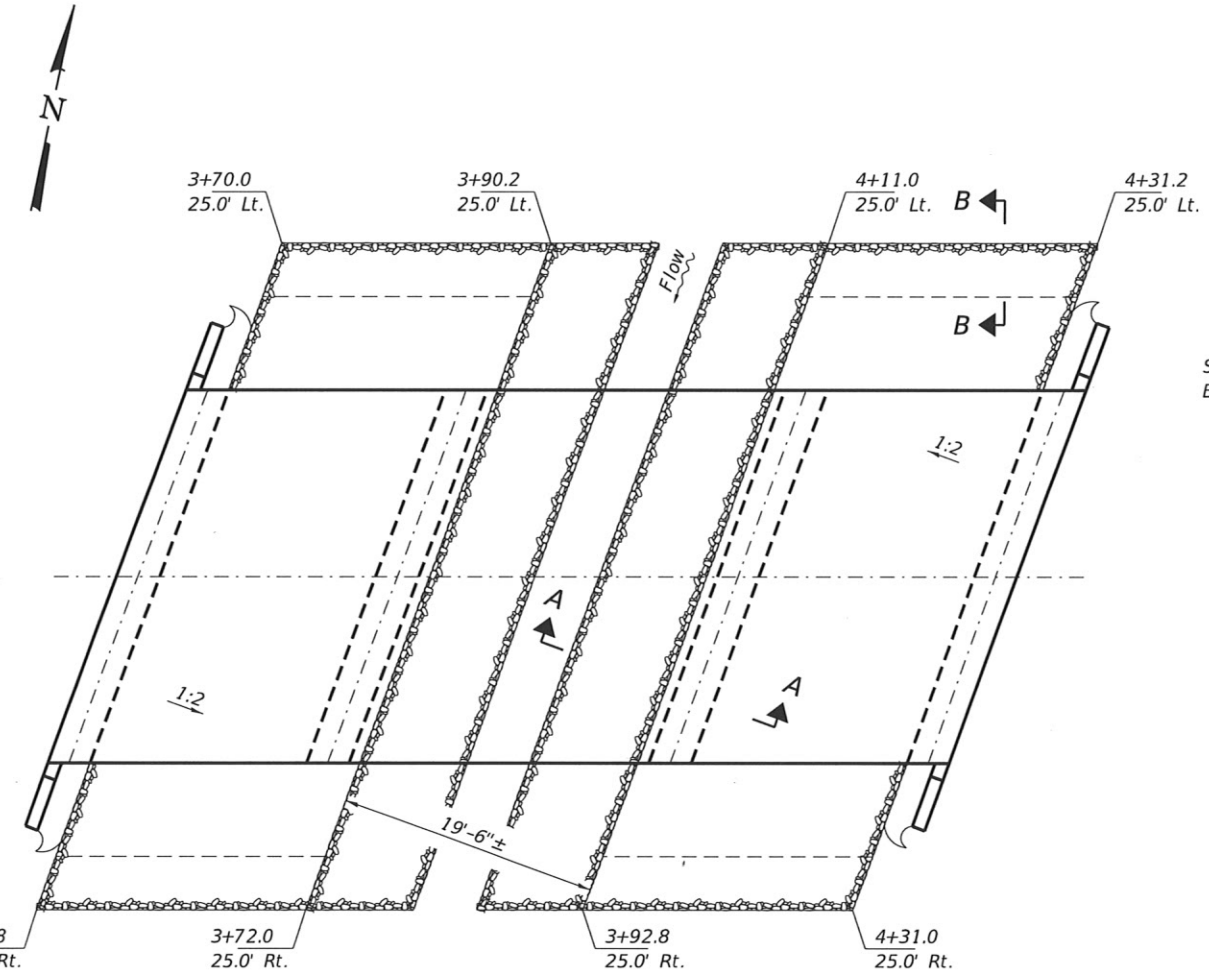


RAILING ELEVATION

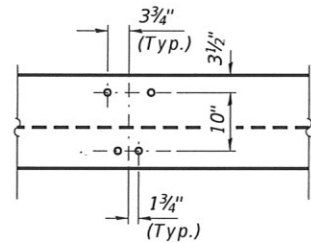


SECTION THRU INTEGRAL ABUTMENT

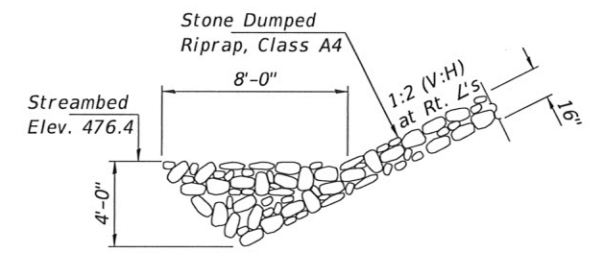
(Horiz. dim. at Rt. L's)
Porous Granular Embankment shall extend the width of the abutment to 2'-0" from the end of wingwalls.



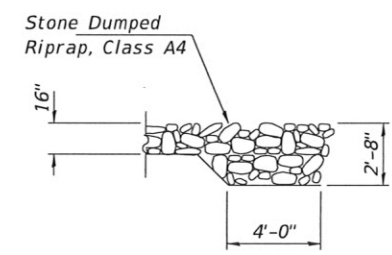
RIPRAP PLAN



DETAIL A

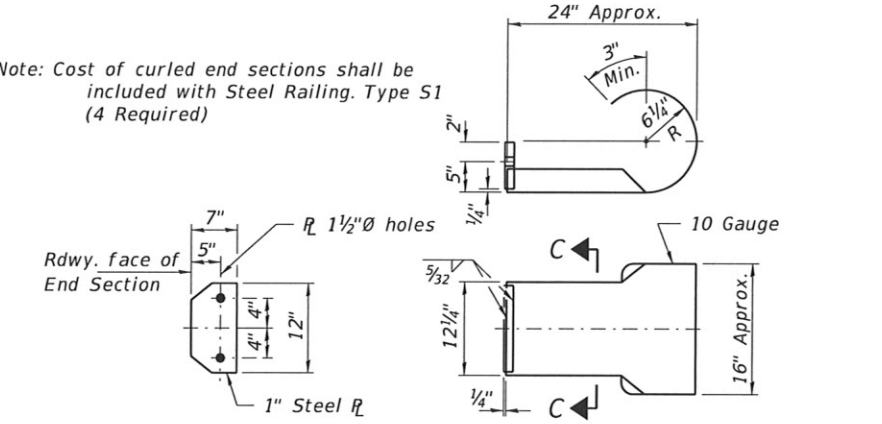


SECTION A-A



SECTION B-B

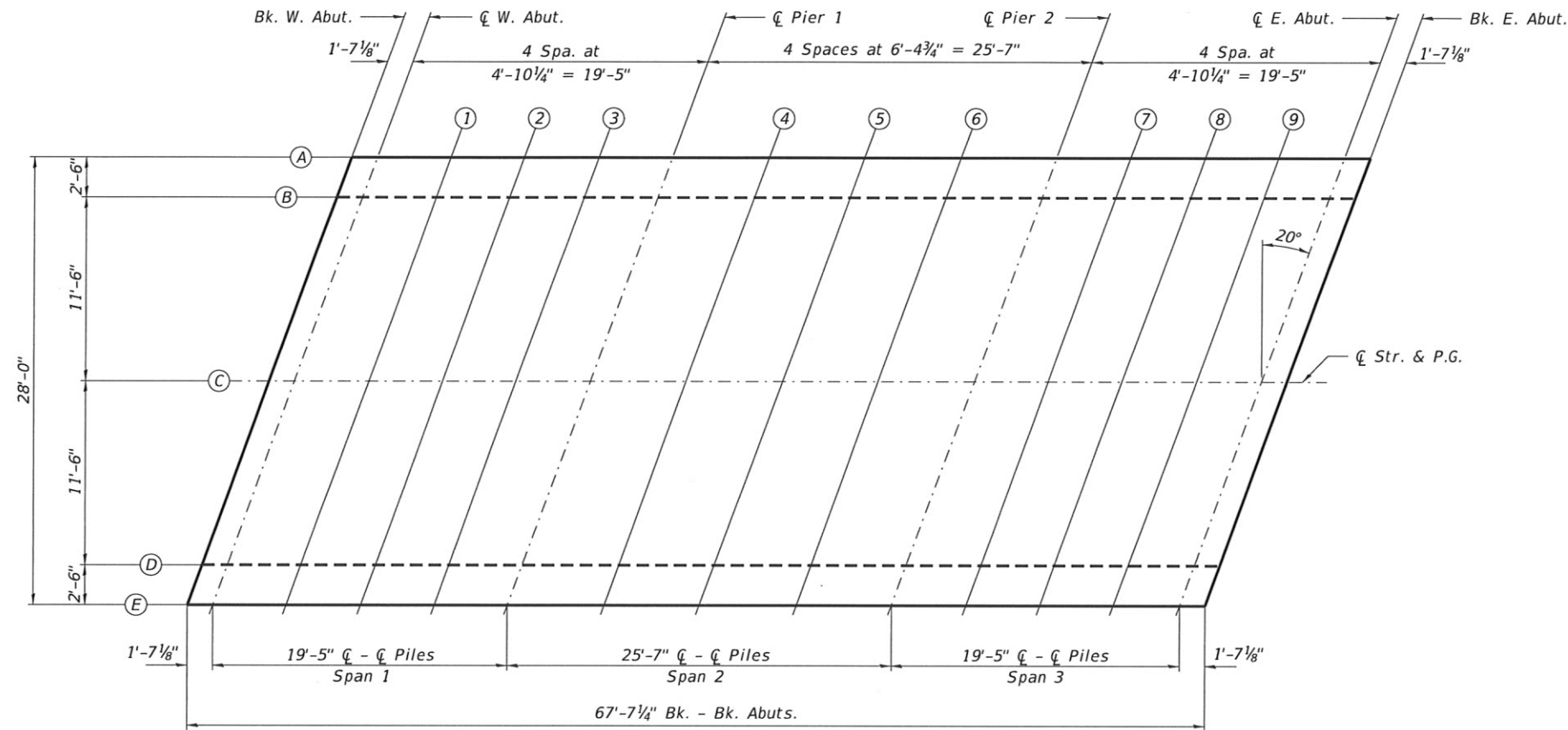
Note: Cost of curled end sections shall be included with Steel Railing, Type S1 (4 Required)



SECTION C-C CURLED END SECTION DETAILS

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Channel Excavation	Cu. Yd.			240
Porous Granular Embankment	Ton			115
Stone Dumped Riprap, Class A4	Ton			330
Protective Coat	Sq. Yd.	232	15	247
Removal of Existing Structures	Each			1
Concrete Structures	Cu. Yd.		43.4	43.4
Concrete Superstructure	Cu. Yd.	84.5		84.5
Bridge Deck Grooving	Sq. Yd.	210		210
Concrete Encasement	Cu. Yd.		12.2	12.2
Reinforcement Bars, Epoxy Coated	Pound	43,800	5,720	49,520
Steel Railing, Type S-1	Foot	131		131
Furnishing Steel Piles HP10x42	Foot		820	820
Driving Piles	Foot		820	820
Test Pile Steel HP10x42	Each		2	2
Pile Shoes	Each		18	18
Name Plates	Each		1	1
Terminal Marker - Direct Applied	Each	4		4



PLAN

LOCATION		BK. W.	CL W.	SPAN 1			CL	SPAN 2			CL	SPAN 3			CL E.	BK. E.
LINE	T.	ABUT.	ABUT.	1	2	3	PIER 1	4	5	6	PIER 2	7	8	9	ABUT.	ABUT.
A	ADJ.	488.308	488.308	488.329	488.329	488.319	488.308	488.340	488.360	488.340	488.308	488.319	488.329	488.329	488.308	488.308
Bott. of Slab		486.892	486.892	486.913	486.913	486.902	486.892	486.923	486.944	486.923	486.892	486.902	486.913	486.913	486.892	486.892

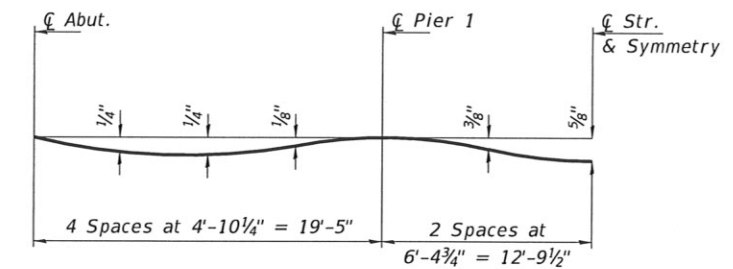
LOCATION		BK. W.	CL W.	SPAN 1			CL	SPAN 2			CL	SPAN 3			CL E.	BK. E.
LINE	T.	ABUT.	ABUT.	1	2	3	PIER 1	4	5	6	PIER 2	7	8	9	ABUT.	ABUT.
B	ADJ.	488.360	488.360	488.381	488.381	488.371	488.360	488.392	488.413	488.392	488.360	488.371	488.381	488.381	488.360	488.360
Bott. of Slab		487.360	487.360	487.381	487.381	487.371	487.360	487.392	487.413	487.392	487.360	487.371	487.381	487.381	487.360	487.360

LOCATION		BK. W.	CL W.	SPAN 1			CL	SPAN 2			CL	SPAN 3			CL E.	BK. E.
LINE	T.	ABUT.	ABUT.	1	2	3	PIER 1	4	5	6	PIER 2	7	8	9	ABUT.	ABUT.
C	ADJ.	488.600	488.600	488.621	488.621	488.610	488.600	488.631	488.652	488.631	488.600	488.610	488.621	488.621	488.600	488.600
Bott. of Slab		487.600	487.600	487.621	487.621	487.610	487.600	487.631	487.652	487.631	487.600	487.610	487.621	487.621	487.600	487.600

LOCATION		BK. W.	CL W.	SPAN 1			CL	SPAN 2			CL	SPAN 3			CL E.	BK. E.
LINE	T.	ABUT.	ABUT.	1	2	3	PIER 1	4	5	6	PIER 2	7	8	9	ABUT.	ABUT.
D	ADJ.	488.360	488.360	488.381	488.381	488.371	488.360	488.392	488.413	488.392	488.360	488.371	488.381	488.381	488.360	488.360
Bott. of Slab		487.360	487.360	487.381	487.381	487.371	487.360	487.392	487.413	487.392	487.360	487.371	487.381	487.381	487.360	487.360

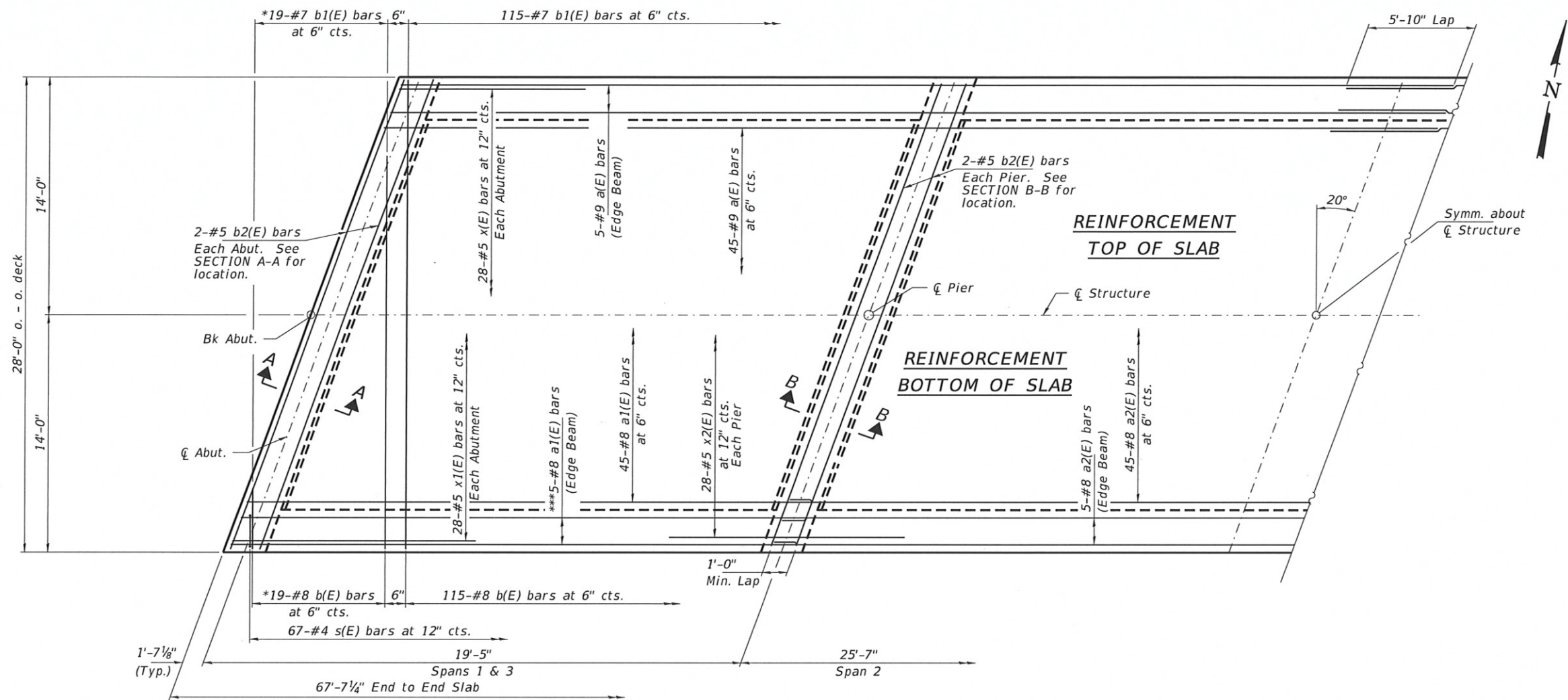
LOCATION		BK. W.	CL W.	SPAN 1			CL	SPAN 2			CL	SPAN 3			CL E.	BK. E.
LINE	T.	ABUT.	ABUT.	1	2	3	PIER 1	4	5	6	PIER 2	7	8	9	ABUT.	ABUT.
E	ADJ.	488.308	488.308	488.329	488.329	488.319	488.308	488.340	488.360	488.340	488.308	488.319	488.329	488.329	488.308	488.308
Bott. of Slab		486.892	486.892	486.913	486.913	486.902	486.892	486.923	486.944	486.923	486.892	486.902	486.913	486.913	486.892	486.892

T. - Theoretical elevation at top of slab
 Adj. - T adjusted for dead load deflection
 * Bottom of slab elevation equals bottom of edge beam

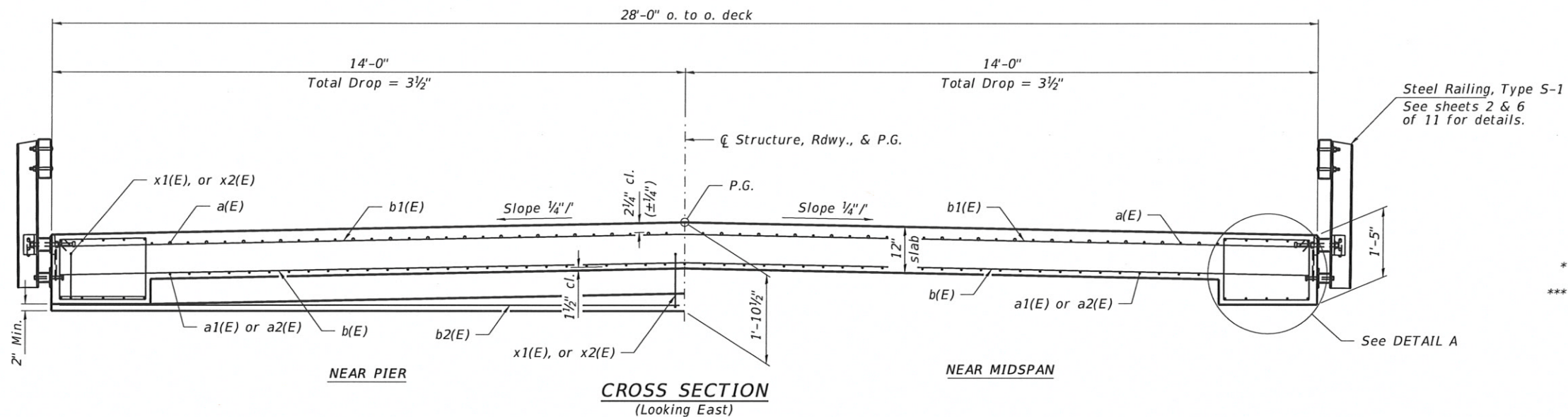


DEAD LOAD DEFLECTION DIAGRAM
 (Includes weight of concrete only.)

Notes:
 The deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown.
 The Contractor shall make allowance for the deflection of forms, shrinkage and settlement of falsework in addition to allowance for dead load deflection.



HALF PLAN

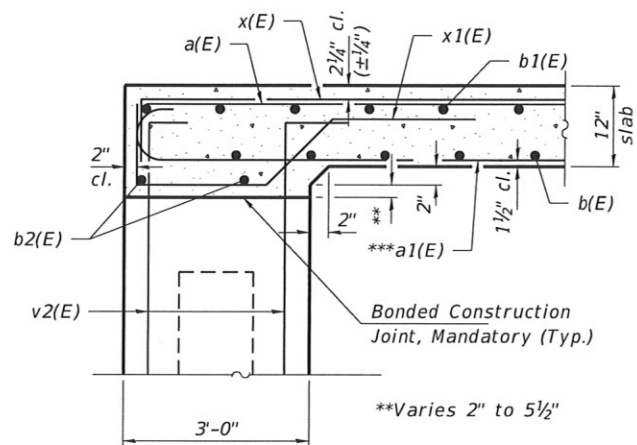


CROSS SECTION
(Looking East)

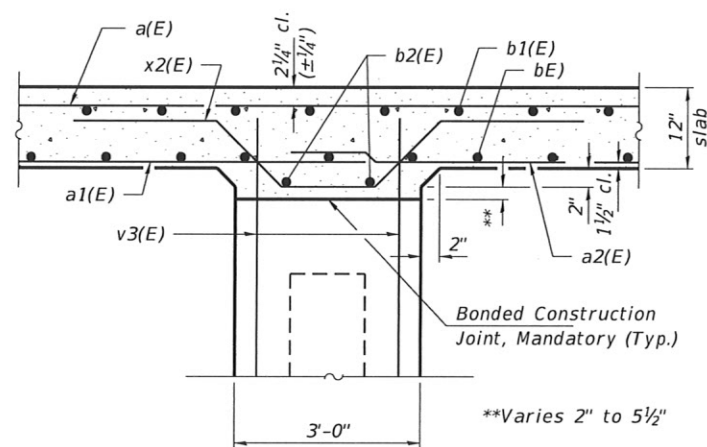
Notes:
 See sheets 5 of 11 for Superstructure Details and Bill of Material.
 See sheet 5 of 11 for SECTION A-A, SECTION B-B and DETAIL A.
 * Order b(E) bars full length. Cut to fit skew and use remainder of bars in opposite end.
 *** a1(E) bars may be rotated to provide clearance for hooked ends.

MIN. BAR LAP
 #9 = 5'-10"
 #8 = 1'-0"

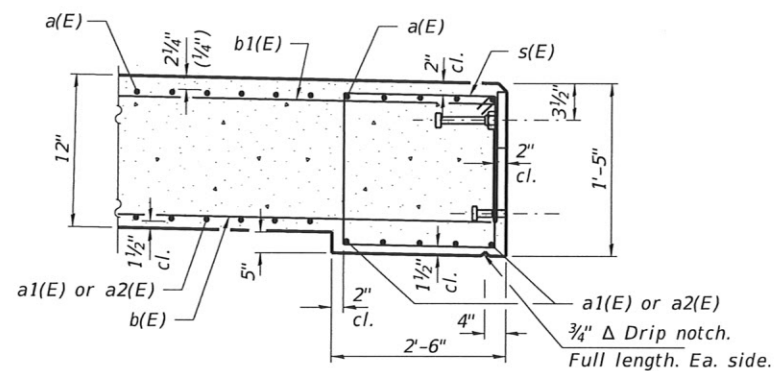
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HAMPTON, LENZINI AND RENWICK, INC. 3081 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703	PLOT SCALE =	CHECKED - S.T.M./S.W.M.	REVISED -			93	20-07134-00-BR	CRAWFORD	15	8
ILLINOIS PROFESSIONAL DESIGN FIRM 18 / PE / SE CORP. 184-000989	PLOT DATE = 2/9/2023	DRAWN - R.D.H.	REVISED -			OBLONG ROAD DISTRICT		CONTRACT NO. 95933		
		CHECKED - S.T.M./S.W.M.	REVISED -			C-97-061-22		ILLINOIS FED. AID PROJECT T108(650)		



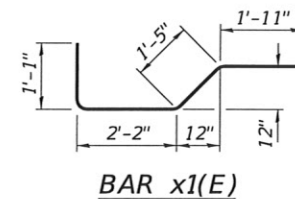
SECTION A-A
Dimensions at right L's to Abutments.



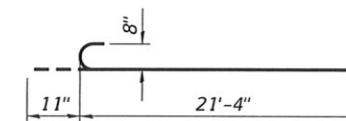
SECTION B-B
Dimensions at right L's to Piers.



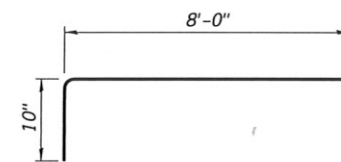
DETAIL A



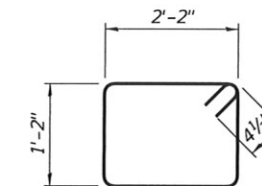
BAR x1(E)



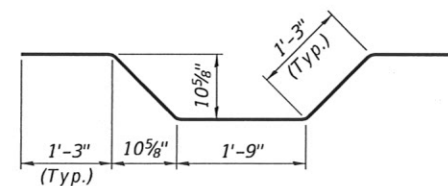
BAR a1(E)



BAR x(E)



BAR s(E)

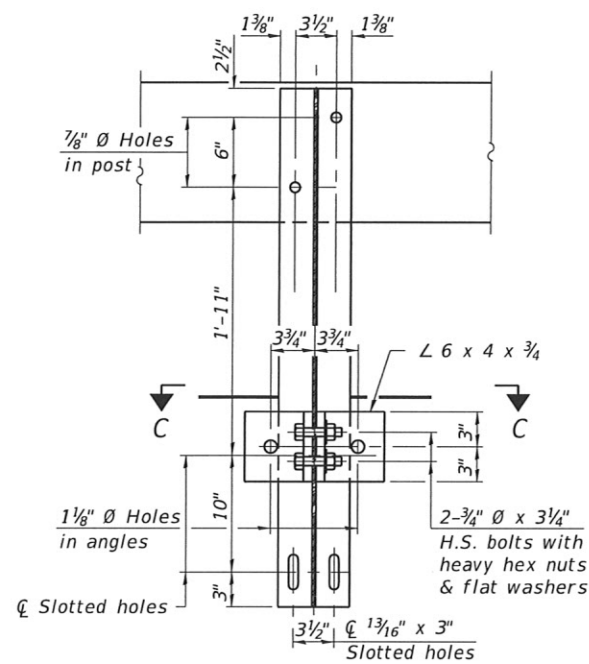


BAR x2(E)

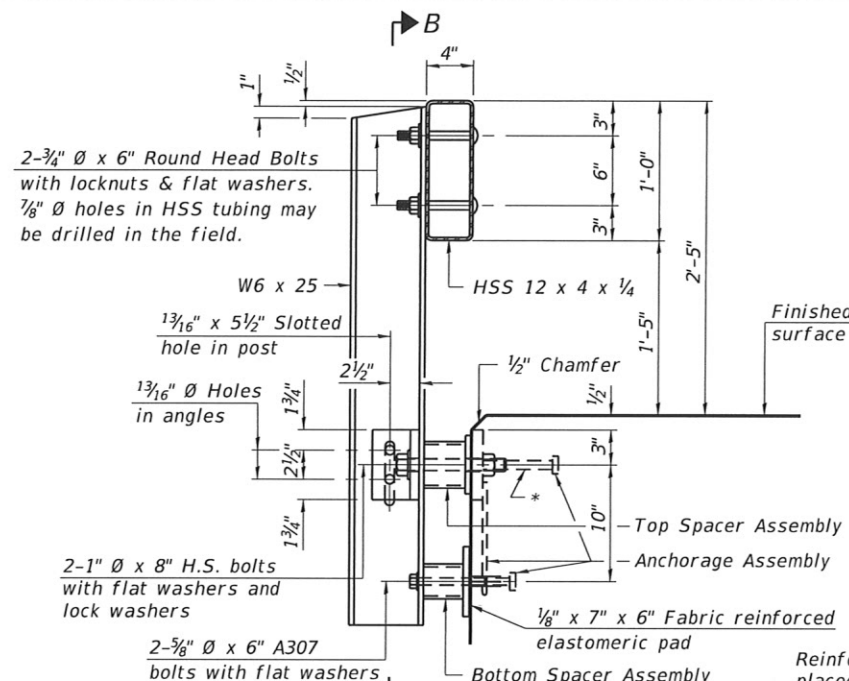
**SUPERSTRUCTURE
BILL OF MATERIAL**

BAR	NO.	SIZE	LENGTH	SHAPE
a(E)	110	#9	36'-7"	—
a1(E)	110	#8	22'-3"	C
a2(E)	55	#8	26'-7"	—
b(E)	134	#8	27'-8"	—
b1(E)	134	#7	27'-8"	—
b2(E)	8	#5	29'-6"	—
s(E)	134	#4	7'-5"	□
x(E)	56	#5	8'-10"	L
x1(E)	56	#5	6'-7"	L
x2(E)	56	#5	6'-9"	L
Protective Coat			Sq. Yd.	232
Concrete Superstructure			Cu. Yd.	84.5
Bridge Deck Grooving			Sq. Yd.	210
Reinforcement Bars, Epoxy Coated			Pound	43,800

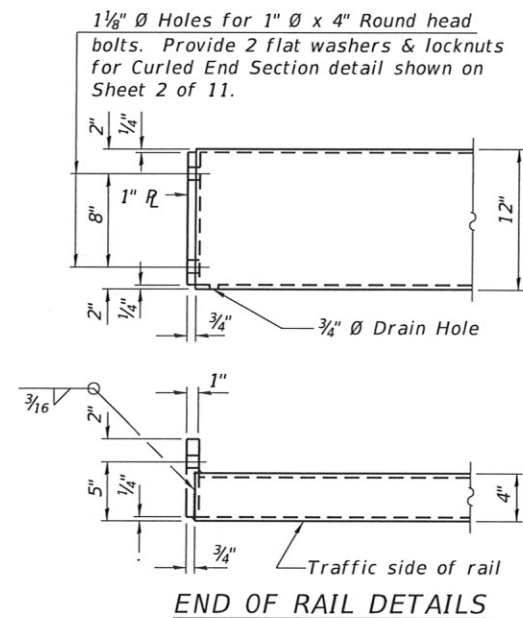
Notes:
See Sheet 7 of 11 for v2(E) placement.
See Sheet 8 of 11 for v3(E) placement.
*** a1(E) bars may be rotated to provide clearance for hooked ends.



SECTION B-B

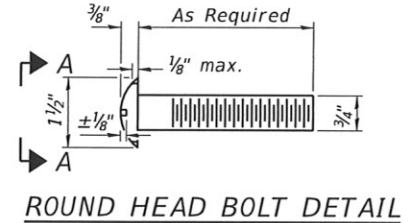


SECTION AT RAILING POST

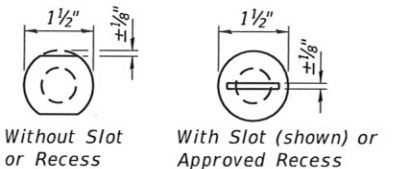


END OF RAIL DETAILS

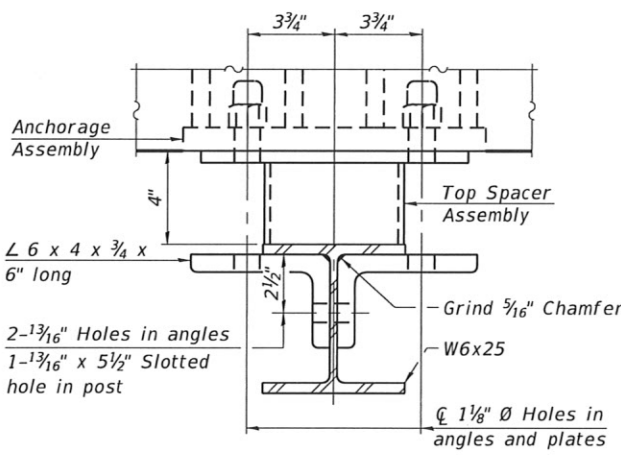
Notes:
 A sufficient number of shims of various thicknesses, sized to fit behind the top spacer assembly, 5" x 11 1/2", and bottom spacer assembly, 6" x 7", shall be provided to adjust posts for proper alignment. If the summation of shims is greater than 1/4" (top) or 1/2" (bottom), longer bolts are required. Cost included with Steel Railing, Type S-1.
 All steel rail elements including shims shall be galvanized according to Article 509.05 of the Standard Specifications.
 All HSS tubing serving as railing shall be CVN tested according to Article 1006.34(b) of the Standard Specifications.
 Rail splice inserts may be built out of 2 - 3/8" bent plates in lieu of the 4 plate rail splice inserts shown, provided the outside dimensions are matched.
 All round head bolts shall be ASTM A307 with locknuts according to ASTM A563 grade A.



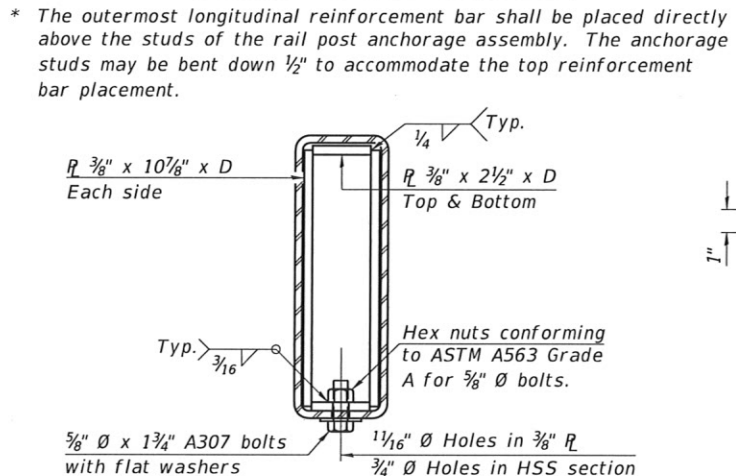
ROUND HEAD BOLT DETAIL



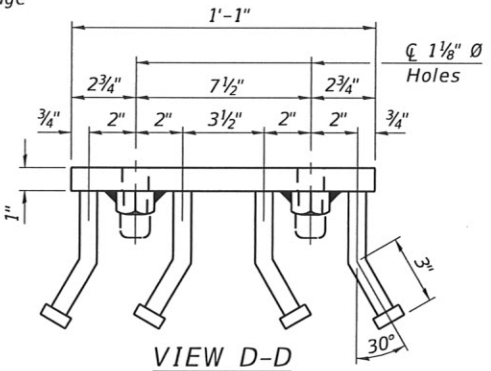
VIEW A-A



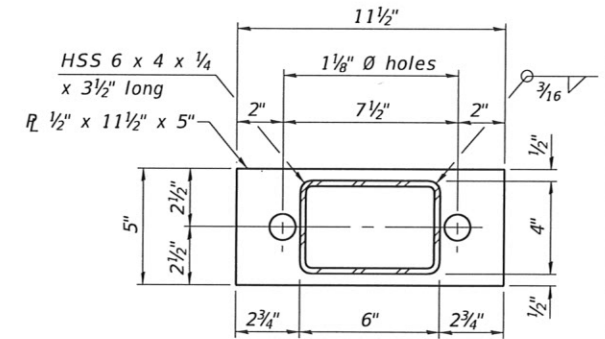
SECTION C-C



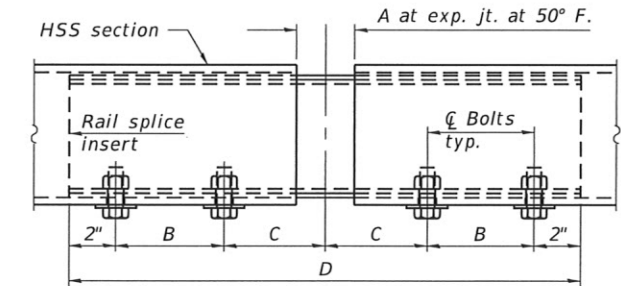
SECTION AT RAIL SPLICE



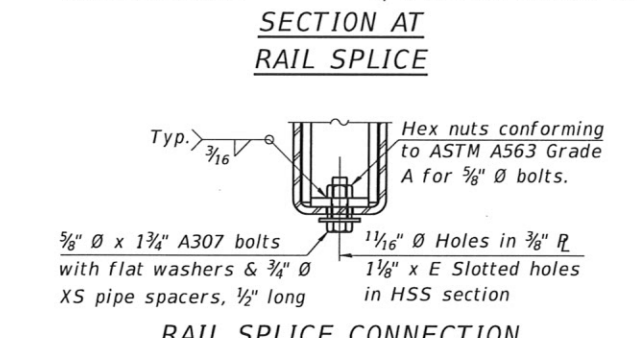
VIEW D-D



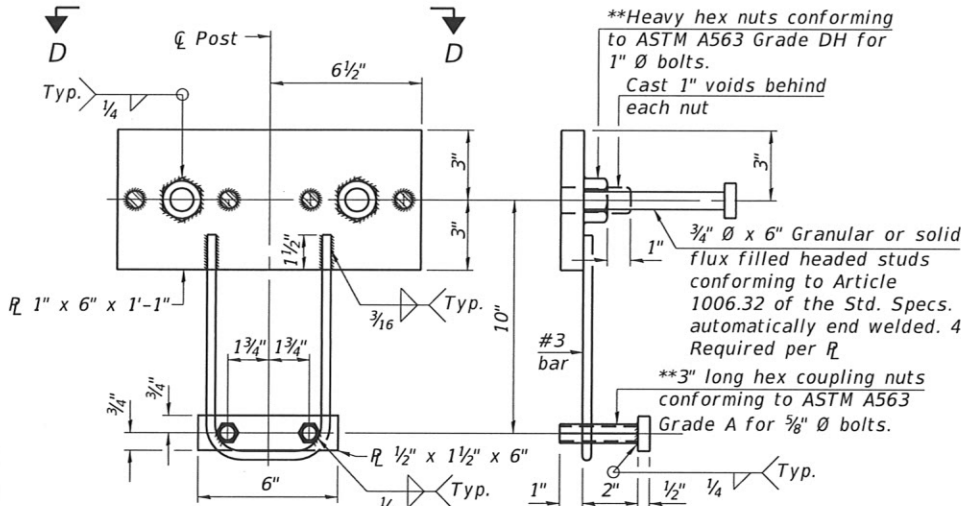
TOP SPACER ASSEMBLY



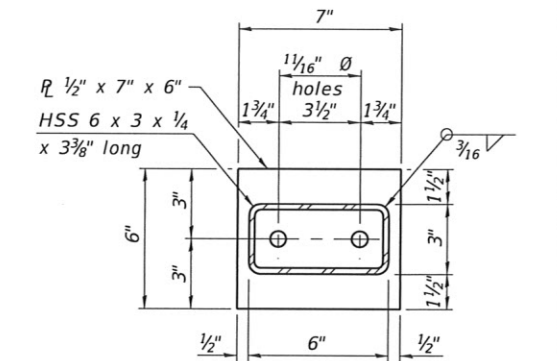
RAIL SPLICE ELEVATION



RAIL SPLICE CONNECTION AT EXPANSION JT.



ANCHORAGE ASSEMBLY



BOTTOM SPACER ASSEMBLY

BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type S-1	Foot	131
Terminal Marker, Direct Applied	Each	4

SPLICE DIMENSIONS

Location	T	A	B	C	D	E
All locs. not over exp. jts.	0	1/4"	4"	4"	1'-8"	-
Over Strip Seal Jt.	≤4"	2 1/2"	4 3/8"	4 3/8"	1'-10"	3 1/16"
Over Finger or Modular Jt.	≤9 1/2"	5 1/2"	7 3/8"	7 1/4"	2'-9 1/4"	5 1 3/16"
Over Finger or Modular Jt.	≤15"	8 1/4"	10 1/8"	10"	3'-8 1/4"	8 3/16"

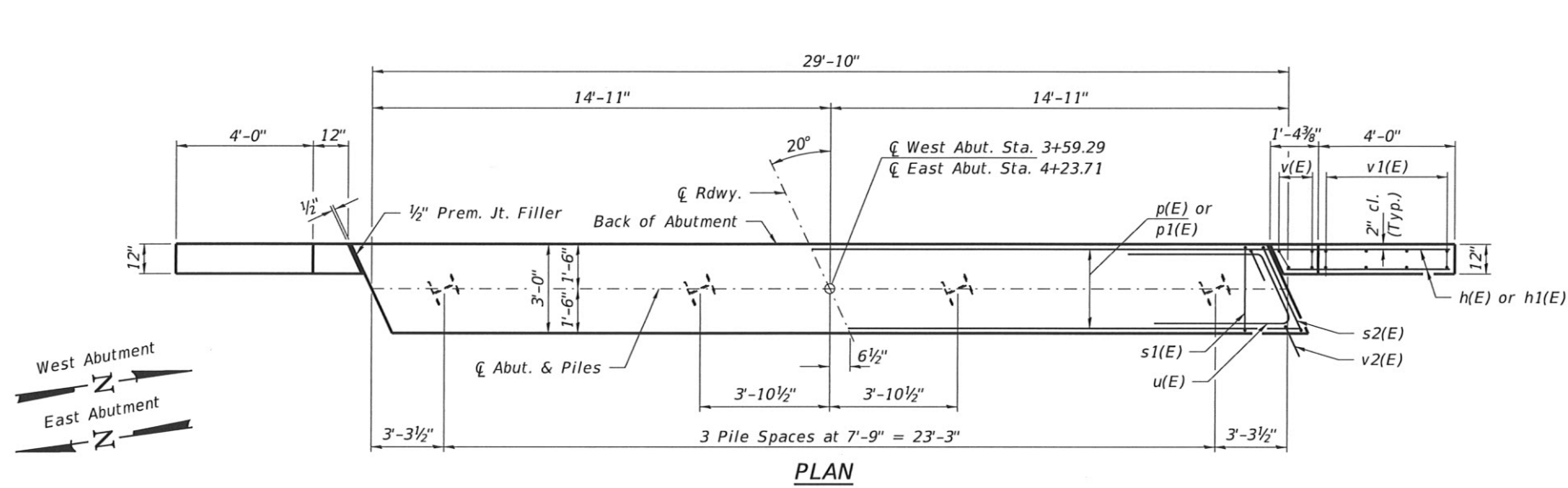
T = ; total movement along centerline of roadway at expansion joint.

RAILING CRITERIA

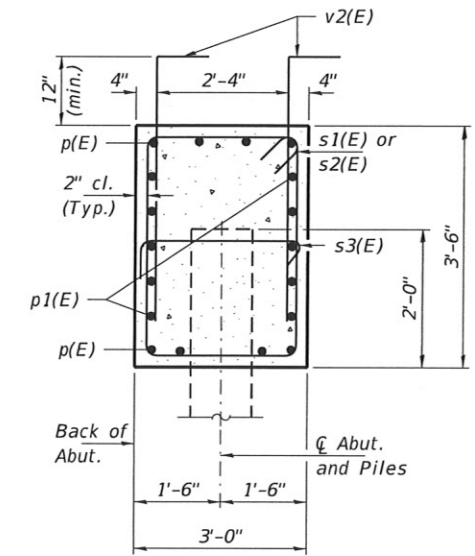
NCHRP 350 Test Level	2
Railing Weight (plf)	50
Max Post Spacing	10'-9"
HMA thickness range (in)	1 1/4 - 3 3/8

R-23A

10-12-2021

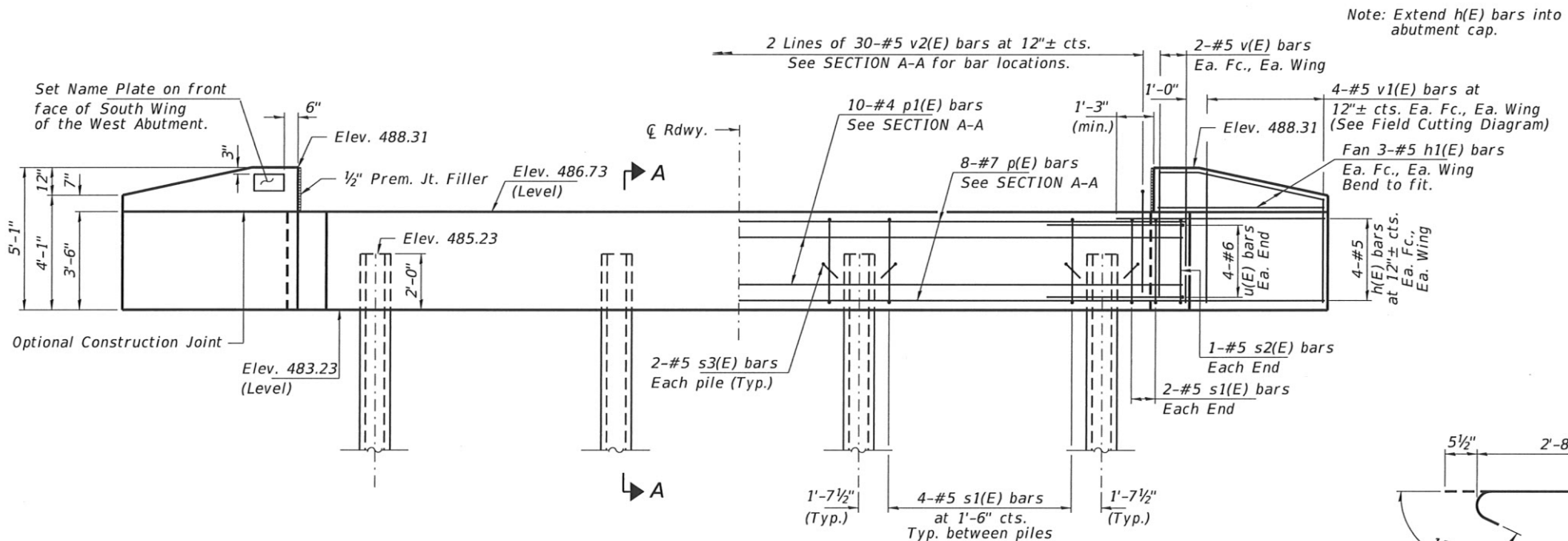


PLAN

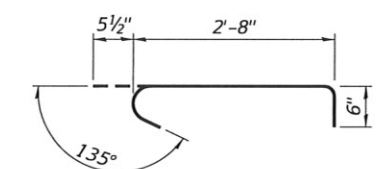


SECTION A-A

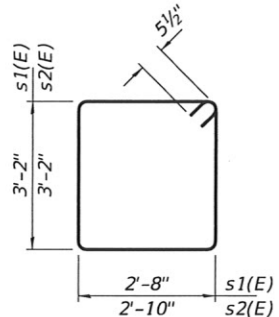
Dimensions at right L's to Abutment.



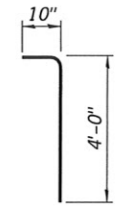
ELEVATION



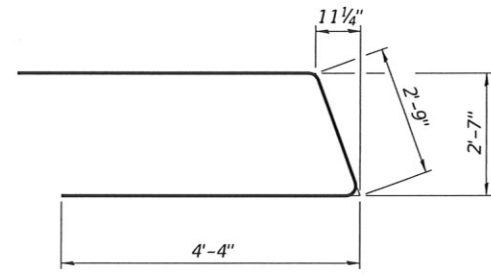
BAR s3(E)



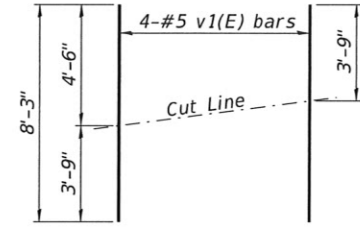
BAR s1(E) & s2(E)



BAR v2(E)



BAR u(E)



FIELD CUTTING DIAGRAM

Order v1(E) bars full length. Cut as shown and use remainder of bars in opposite face.

BILL OF MATERIAL - 2 ABUTS.

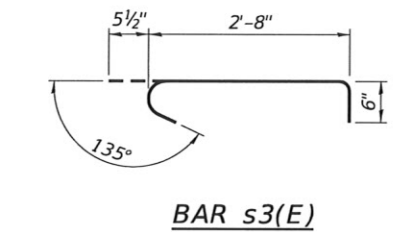
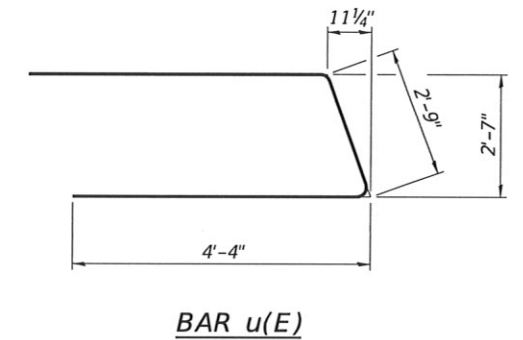
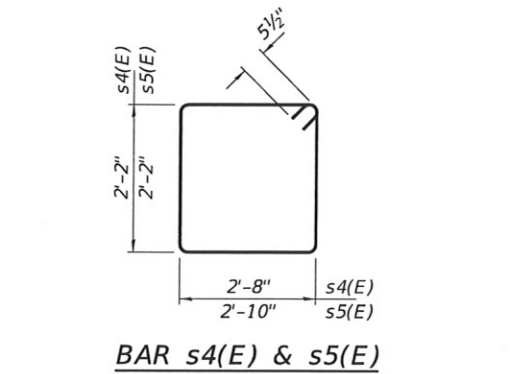
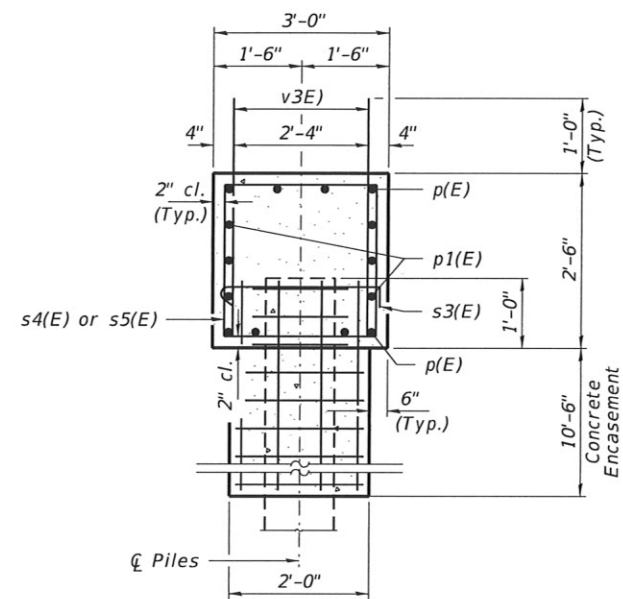
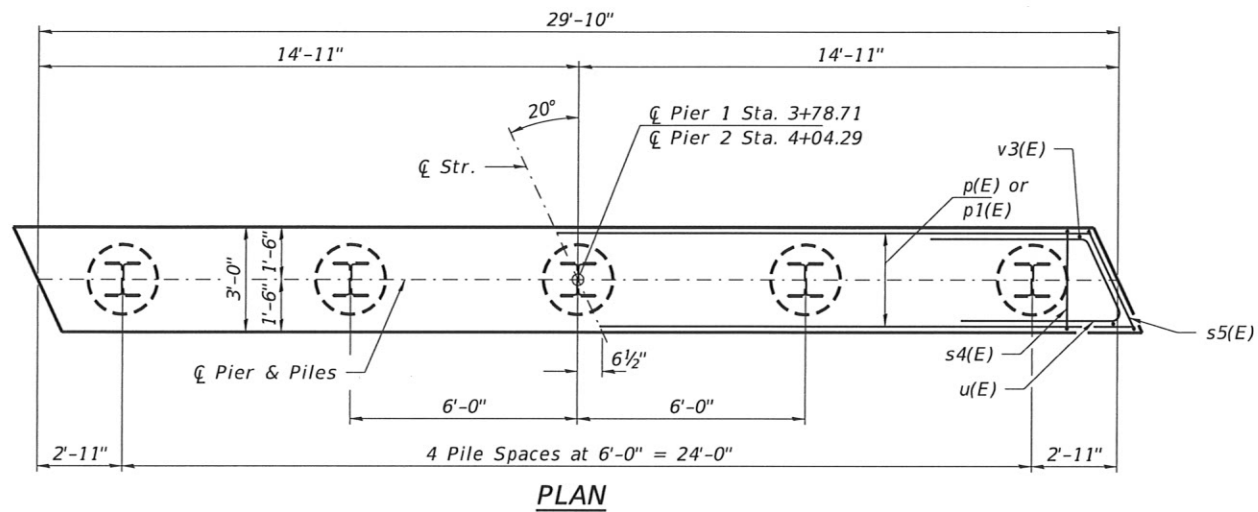
BAR	NO.	SIZE	LENGTH	SHAPE
h(E)	32	#5	6'-6"	—
h1(E)	24	#5	4'-11"	—
p(E)	16	#7	29'-6"	—
p1(E)	20	#4	29'-6"	—
s1(E)	32	#5	12'-7"	□
s2(E)	4	#5	12'-11"	□
s3(E)	16	#5	3'-8"	┌
u(E)	16	#6	11'-5"	┌
v(E)	16	#5	4'-10"	—
v1(E)	16	#5	8'-3"	—
v2(E)	120	#5	4'-10"	—
Protective Coat			Sq. Yd.	15
Concrete Structures			Cu. Yd.	26.8
Reinf. Bars, Epoxy Coated			Pound	3,330
Furnishing Steel Piles HP10x42			Foot	280
Driving Piles			Foot	280
Test Pile Steel HP10x42			Each	1
Pile Shoes			Each	8
Name Plates			Each	1

Notes: For details of piles, see Sheet 9 of 11. All edges shall have standard 3/4" chamfer.

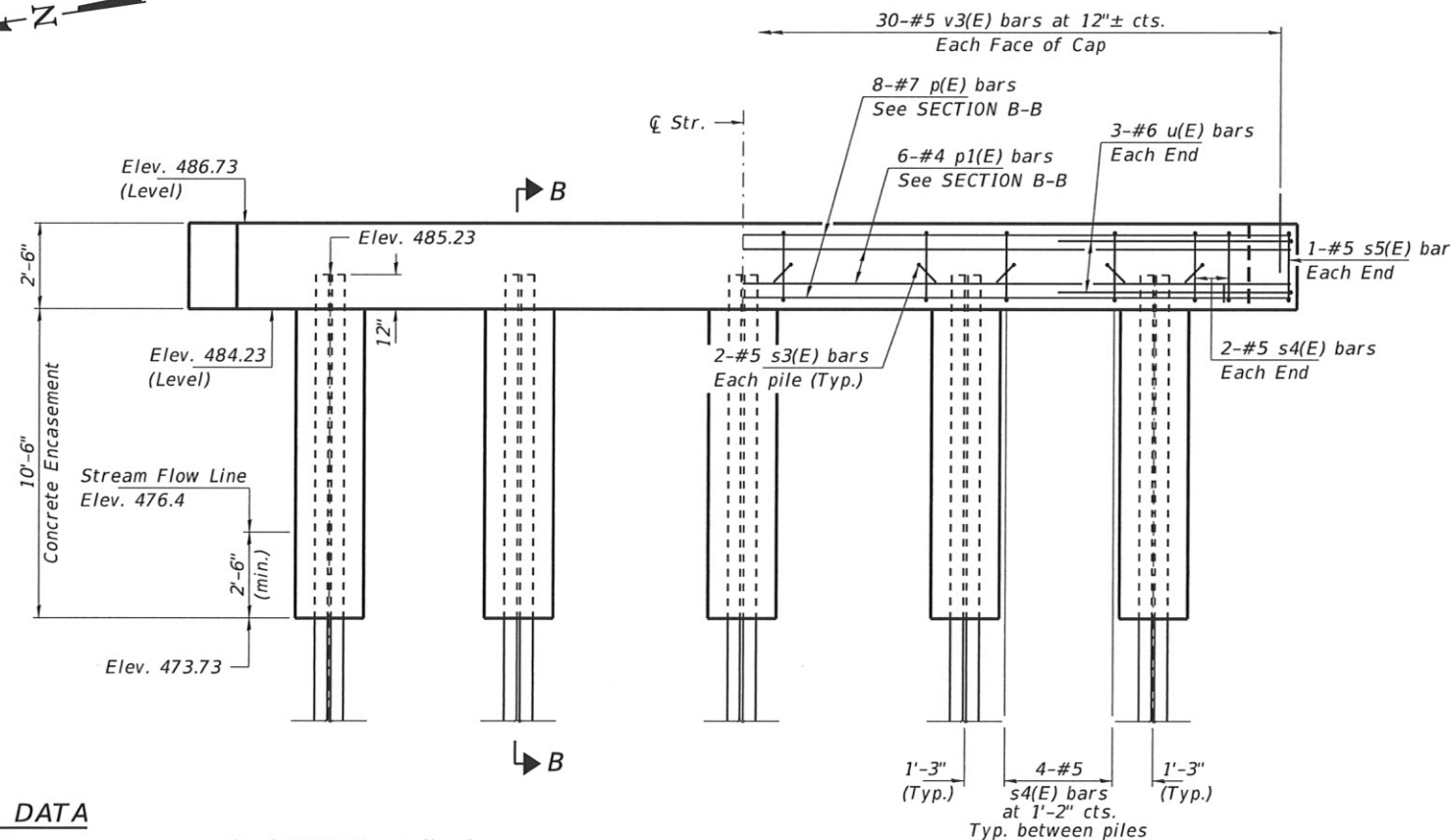
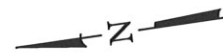
PILE DATA

Type: Steel HP10x42 w/ pile shoes
 Nominal Required Bearing: 335 Kips/Pile
 Factored Resistance Available: 184 Kips/Pile
 Est. Length: 40 Ft/Pile
 No. Production Piles: 7
 No. Test Piles: 1

Notes: Includes one test pile to be driven in a permanent location at the East Abutment.



SECTION B-B
Dimensions at right Z's to Pier.



PILE DATA

Type: Steel HP10x42 w/ pile shoes.
 Nominal Required Bearing: 335 Kips/Pile
 Factored Resistance Available: 184 Kips/Pile
 Est. Length: 60 Ft/Pile
 No. Production Piles: 9
 No. Test Piles: 1

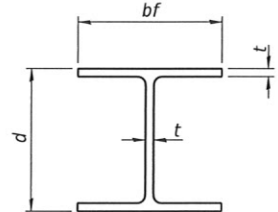
Notes: Includes one test pile to be driven in a permanent location at Pier 1.

ELEVATION
(Looking East)

BILL OF MATERIAL - 2 PIERS

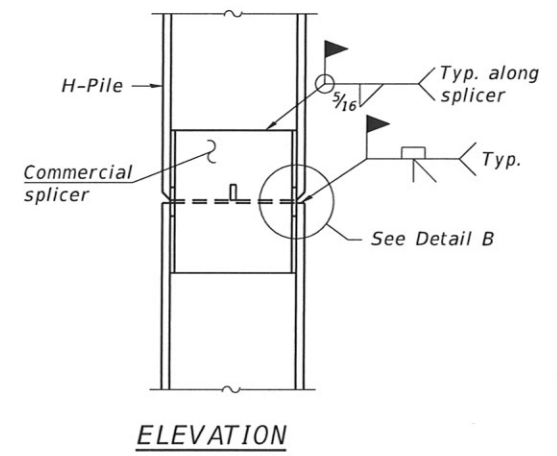
BAR	NO.	SIZE	LENGTH	SHAPE
p(E)	16	#7	29'-6"	—
p1(E)	12	#4	29'-6"	—
s3(E)	20	#5	3'-8"	U
s4(E)	40	#5	10'-7"	□
s5(E)	4	#5	10'-11"	□
u(E)	12	#6	11'-5"	U
v3(E)	120	#5	3'-4"	—
Concrete Structures			Cu. Yd.	16.6
Concrete Encasement			Cu. Yd.	12.2
Reinf. Bars, Epoxy Coated			Pound	2,390
Furnishing Steel Piles HP10x42			Foot	540
Driving Piles			Foot	540
Test Pile Steel HP10x42			Each	1
Pile Shoes			Each	10

Notes:
 For details of piles, see Sheet 9 of 11.
 For details of concrete encasement reinforcement, see Sheet 9 of 11.
 All edges shall have standard 3/4" chamfer.
 Pile spacing can be varied up to 1'-0" to avoid existing footings.

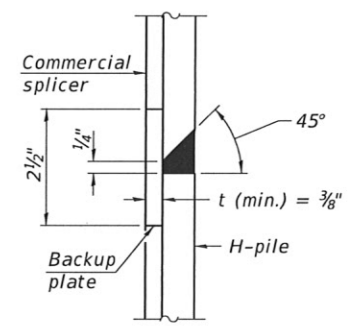


STEEL PILE TABLE

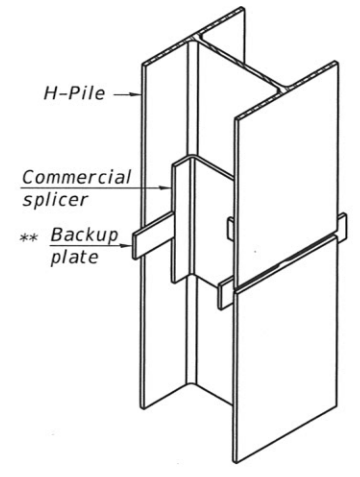
Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	1 3/16"	30"
x102	14"	14 3/4"	1 1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 3/8"	14 3/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1 1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



ELEVATION

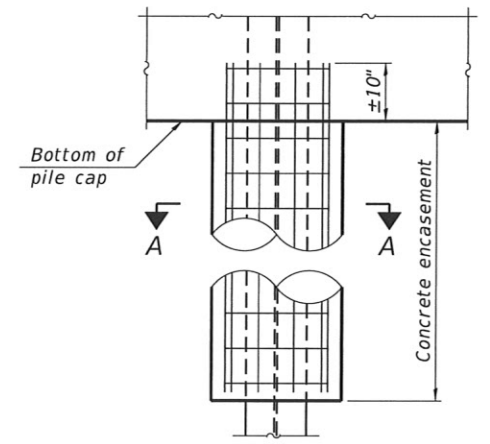


DETAIL "B"

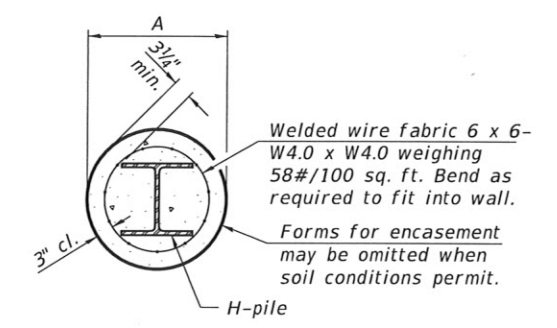


ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE

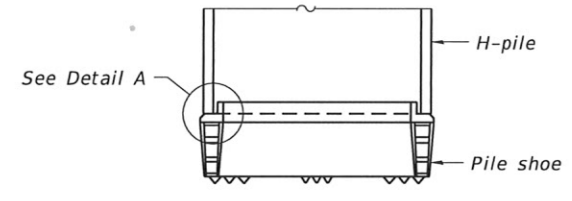


ELEVATION

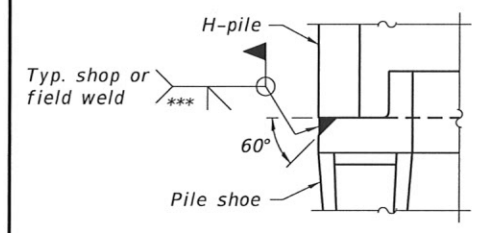


SECTION A-A

INDIVIDUAL PILE CONCRETE ENCASEMENT
(when specified)



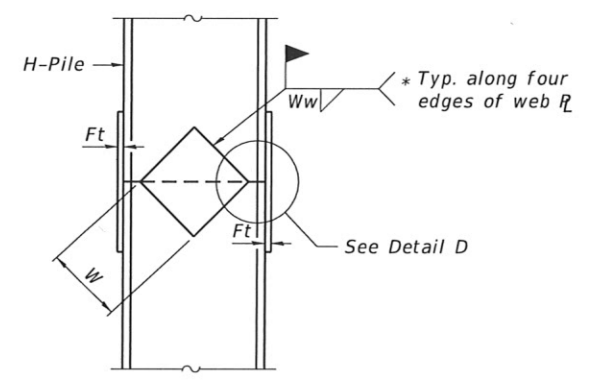
ELEVATION



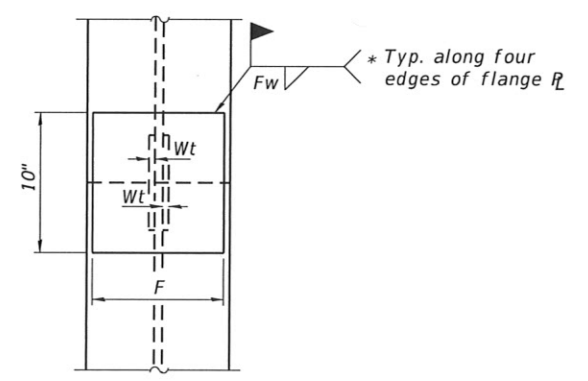
DETAIL A

SHOE ATTACHMENT

Note:
The steel H-piles shall be according to AASHTO M270 Grade 50.

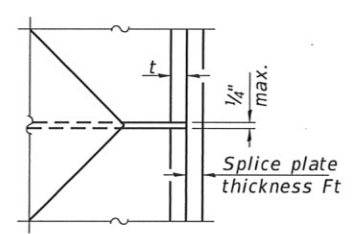


ELEVATION

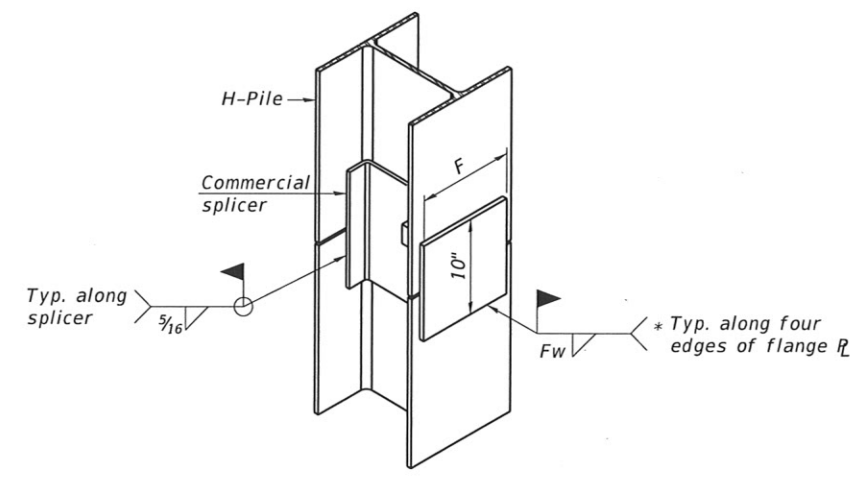


END VIEW

Designation	F	Ft	Fw	W	Wt	Ww
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5/8"	1/2"
x89	12 1/2"	3/4"	1 1/16"	7 3/4"	5/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5/8"	1/2"
HP 12x84	10"	7/8"	1 1/16"	6 1/2"	5/8"	1/2"
x74	10"	7/8"	1 1/16"	6 1/2"	5/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"



DETAIL D



ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds 1/4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (3/16" min.).

WELDED PLATE FIELD SPLICE

