INTERIOR GIRDER MOMENT TABLE						
		0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.6 Sp. 3
I_s	(in^4)	7779	7779	7779	7779	7779
$I_c(n)$	(in^4)	19788	19788	19788	19788	19788
$I_c(3n)$	(in^4)	14890	14890	14890	14890	14890
$I_c(cr)$	(in^4)	-	10592	-	10592	-
S_s	(in^3)	414.9	414.9	414.9	414.9	414.9
$S_c(n)$	(in^3)	583.4	1708.9	583.4	1708.9	583.4
$S_c(3n)$	(in^3)	534.6	843.8	534.6	843.8	534.6
$S_c(cr)$	(in^3)	-	524.6	-	524.6	-
S_{xc}	(in^3)	567.0	507.2	568.5	506.9	566.7
DC1	(k/')	0.80	0.80	0.80	0.80	0.80
M_{DC1}	('k)	154	274	140	279	157
DC2	(k/')	0.08	0.08	0.08	0.08	0.08
M_{DC2}	('k)	12	21	10	21	12
DW	(k/')	0.28	0.28	0.28	0.28	0.28
M_{DW}	('k)	53	95	48	97	54
M_{LL+IM}	('k)	556	517	562	532	567
f_l (Strength I)	(ksi)	3.9	5.6	3.9	6.1	4.2
$M_u + \frac{1}{3} f_l S_{xc}$	('k)	1322	1495	1305	1366	1350
$\phi_f M_n$	('k)	2274	1911	2286	1908	2274
f_s DC1	(ksi)	4.5	7.9	4.0	8.1	4.5
f_s DC2	(ksi)	0.3	0.5	0.2	0.5	0.3
f_s DW	(ksi)	1.2	2.2	1.1	2.2	1.2
f_s (LL+IM)	(ksi)	11.4	11.8	11.6	12.2	11.7
f_l (Service II)	(ksi)	0.8	4.3	1.2	4.6	1.4
$f_s + f_l/2$ (Service II)	(ksi)	21.2	28.1	21.0	28.9	21.9
$0.95R_hF_{yf}$	(ksi)	47.5	47.5	47.5	47.5	47.5
$f_s + f_l/3$ (Total)(Strength I)	(ksi)	29.0	36.3	28.5	39.8	29.6
$\phi_f F_n$	(ksi)	50	50	50	50	50
V_f	(k)	9.9	13.7	10.4	14.9	10.1

I_s, S_s: Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads ($in.^4$ and $in.^3$).
$I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short term composite live loads ($in.^4$ and $in.^3$).
$I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to long-term composite (superimposed) dead loads ($in.^4$ and $in.^3$).
$I_c(cr)$, $S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite dead loads ($in.^4$ and $in.^3$).
S_{xc}: Section modulus about the major axis of section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange ($in.^3$).
DC1: Un-factored non-composite dead load (kips/ft.).
M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).
DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
M_{LL+IM}: Un-factored live load moment plus dynamic load allowance (impact)(kip-ft.).
M_u (Strength I): Factored design moment (kip-ft.).
$1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{LL+IM}$
f_l: Factored calculated normal stress at edge of flange for controlling steel flange plate due to lateral bending, Strength I or Service II as applicable (ksi).
$\phi_f M_n$: Factored resistance available according to A6.1.1 ('k).
f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
M_{DC1} / S_s
f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
$M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.
f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
$M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.
f_s (LL+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live plus impact loads as calculated below (ksi).
$M_{LL+IM} / S_c(3n)$ or $M_{LL+IM} / S_c(cr)$ as applicable.
$f_s + f_l/2$ (Service II): Sum of stresses as computed below (ksi).
$f_{s DC1} + f_{s DC2} + f_{s DW} + 1.3 f_{s LL+IM} + f_l/2$
$0.95R_hF_{yf}$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
$f_s + f_l/3$ (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
$1.25 (f_{sDC1} + f_{sDC2}) + 1.5 f_{sDW} + 1.75 f_{sLL+IM} + f_l/3$
$\phi_f F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7.2 (ksi).
V_f: Maximum factored shear range computed according to Article 6.10.10.
M_{LL} and R_{LL} include the effects of centrifugal force and superelevation.

DIAPHRAGM SPACING - LEFT (feet)

Beam	DL_1	DL_2	DL_3	DL_4	DL_5	DL_6	DL_7	DL_8	DL_9
1	--	--	--	--	--	--	--	--	--
2	8.35	15.31	13.19	2.13	15.94	14.26	1.68	15.77	3.50
3	8.41	15.38	13.22	2.15	15.99	14.29	1.71	15.80	3.50
4	8.48	15.44	13.27	2.18	16.04	14.32	1.72	15.84	3.50
5	8.55	15.51	13.31	2.20	16.09	14.35	1.74	15.88	3.50

INTERIOR GIRDER REACTION TABLE					
		W. Abut.	Pier 1	Pier 2	E. Abut.
R_{DC1}	(k)	16.6	51.9	52.4	16.5
R_{DC2}	(k)	1.1	3.9	4.0	1.1
R_{DW}	(k)	5.5	18.1	18.2	5.5
R_{LL+IM}	(k)	70.5	106.4	105.4	67.0
R_{Total}	(k)	93.7	180.3	180.0	90.1

DIAPHRAGM SPACING - RIGHT (feet)

Beam	DR_1	DR_2	DR_3	DR_4	DR_5	DR_6	DR_7	DR_8	DR_9
1	3.50	15.44	2.12	13.95	16.07	1.84	14.06	15.90	6.53
2	3.50	15.50	2.16	13.97	16.13	1.86	14.07	15.94	6.56
3	3.50	15.57	2.17	14.00	16.18	1.88	14.09	15.98	6.58
4	3.50	15.64	2.20	14.02	16.23	1.91	14.11	16.00	6.63
5	--	--	--	--	--	--	--	--	--

TOP OF WEB ELEVATIONS*						
Location	℄ Brg. W. Abut.	℄ Brg. Pier 1	℄ Splice 1	℄ Splice 2	℄ Brg. Pier 2	℄ Brg. E. Abut.
Beam 1	474.81	475.72	475.94	476.65	476.80	477.49
Beam 2	474.21	475.14	475.36	476.08	476.24	476.95
Beam 3	473.61	474.55	474.77	475.51	475.68	476.42
Beam 4	473.01	473.97	474.19	474.94	475.11	475.88
Beam 5	472.41	473.38	473.60	474.37	474.54	475.34

* For fabrication only

LAYOUT DIMENSIONS (feet)

Beam	W. Abut.-℄ Brg.		Inflection point		Pier 1-℄ Brg.		Splice 1		Splice 2		Pier 2-℄ Brg.		Inflection point		E. Abut.-℄ Brg.	
	X	Y	X	Y	X	Y	X	Y	X	Y	X	Y	X	Y	X	Y
1	-88.48	13.62	-50.13	17.05	-36.72	17.80	-24.89	18.27	15.07	18.52	27.26	18.19	40.15	17.63	79.52	14.60
2	-92.60	6.76	-53.91	10.45	-40.65	11.26	-28.79	11.80	11.38	12.25	23.53	11.97	36.70	11.46	75.91	8.59
3	-96.75	-0.13	-57.89	3.81	-44.59	4.70	-32.71	5.30	7.60	5.96	19.79	5.74	32.97	5.29	72.30	2.58
4	-100.91	-7.07	-62.04	-2.87	-48.55	-1.89	-36.65	-1.22	3.82	-0.34	16.03	-0.50	29.15	-0.89	68.68	-3.44
5	-105.10	-14.03	-65.71	-9.54	-52.52	-8.50	-40.72	-7.77	0.02	-6.67	12.27	-6.77	25.54	-7.10	65.05	-9.48

X dimensions are parallel to the local tangent at Sta. 9+96.56.
Y dimensions are at right angles to the local tangent at Sta. 9+96.56.

Note:.
See sheet 14 of 33 for beam plan and elevation dimensions depicted in tables.
Spacings given in tables are along respective beam centerlines.
See sheets 19 and 22 of 33 for abutment bearing orientations and anchor bolt layout dimensions.
See sheets 25 and 27 of 33 for pier bearing orientations and anchor bolt layout dimensions.

SHEAR STUD SPACING

Beam	SA_1	SA_2	SA_3	SA_4	SB_1	SB_2	SB_3	SB_4	SB_5	SB_6	SC_1	SC_2	SC_3
1	27 spa. @ 9"	7"	46 spa. @ 8"	8 1/8"	15 spa. @ 8"	27 spa. @ 8"	1'-1"	27 spa. @ 8"	14 spa. @ 9"	8"	46 spa. @ 8"	11 3/8"	27 spa. @ 9"
2	27 spa. @ 9"	9"	46 spa. @ 8"	10 1/8"	17 spa. @ 7"	27 spa. @ 8"	1'-3 5/8"	27 spa. @ 8"	15 spa. @ 8"	10"	47 spa. @ 8"	8 1/8"	27 spa. @ 9"
3	27 spa. @ 9"	11"	46 spa. @ 8"	11 1/4"	17 spa. @ 7"	28 spa. @ 8"	9 1/2"	27 spa. @ 8"	15 spa. @ 8"	10 1/4"	47 spa. @ 8"	9 1/2"	27 spa. @ 9"
4	27 spa. @ 9"	1'-2 7/8"	46 spa. @ 8"	10 1/2"	17 spa. @ 7"	28 spa. @ 8"	11 1/4"	27 spa. @ 8"	15 spa. @ 8"	10"	47 spa. @ 8"	11 3/4"	27 spa. @ 9"
5	27 spa. @ 9"	1'-3 3/4"	46 spa. @ 8"	10 1/2"	15 spa. @ 8"	28 spa. @ 8"	1'-2 5/8"	27 spa. @ 8"	14 spa. @ 9"	10 1/2"	46 spa. @ 8"	1'-3"	27 spa. @ 9"

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