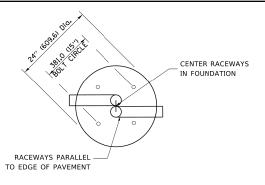


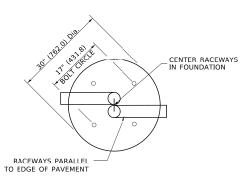
TOTAL SHEET SHEETS NO.

**542** 201

#### LIGHT POLE FOUNDATION DEPTH TABLE 40 FT. (12.192 m) TO 47.5 FT. (14.478 m) MOUNTING HEIGHT

	DESIGN DEPTH "D" OF FOUNDATION					
SOIL CONDITIONS		TWIN ARM POLE				
SOFT CLAY	13'-0"	15'-0"				
Qu = 0.375 TON/SQ. FT.	(3.96 m)	(4.57 m)				
MEDIUM CLAY	9'-6"	10'-9"				
Qu = 0.75 TON/SQ.FT	(2.09 m)	(3.23 m)				
STIFF CLAY	7'-0"	8'-0"				
Qu = 1.50 TON/SQ. FT.	(2.13 m)	(2.44 m)				
LOOSE SAND	9'-0"	10'-0"				
∅ = 34°	(2.74 m)	(3.05 m)				
MEDIUM SAND	8'-3"	9'-0"				
∅ = 37.5°	(2.52 m)	(2.74 m)				
DENSE SAND	7'-9"	9'-0"				
∅ − 40°	(2.36 m)	(2.74 m)				



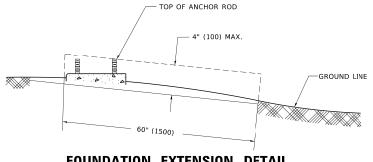


**TOP VIEW TOP VIEW** 

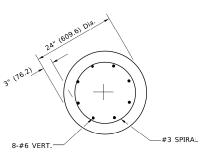
> ANCHOR ROD 4-1" Dia. X 5'-0" (4-25.4 Dia. X 1.524 m)

#### ¾" (19) CHAMFER 24" (609) #2/0 BARE COPPER EXOTHERMIC WELD CONNECTION TO GND ROD EXOTHERMIC WELD CONNECTION TO REINFORCING STEEL 3½" X 36" RADIUS (88.9 Dia. X 914.4) #2/0 BARE COPPER 6" (152.4) THREADED PVC RACEWAY (2 MIN.) GROUND CLAMP UL LISTED 8-#6 VERTICAL BARS #3 SPIRAL GROUND ROD (WHEN SPECIFIED) %" Dia. X 10¹ (15.875 Dia. X 3.048 m) %" T. X 4" Dia. (15.87 T. X 101.6 Dia.) WASHER, TACK WELDED RADIUS NOT LESS THAN 4 TIMES NOMINAL ROD DIA — 3 LOOPS MIN. AT TOP & BOTTOM **ANCHOR ROD DETAIL** 3" (76.2) 24" (609.6) Dia.

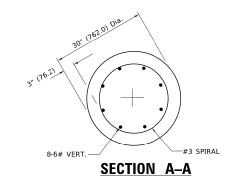
### **FOUNDATION DETAIL**



FOUNDATION EXTENSION DETAIL



SECTION A-A



#### **NOTES**

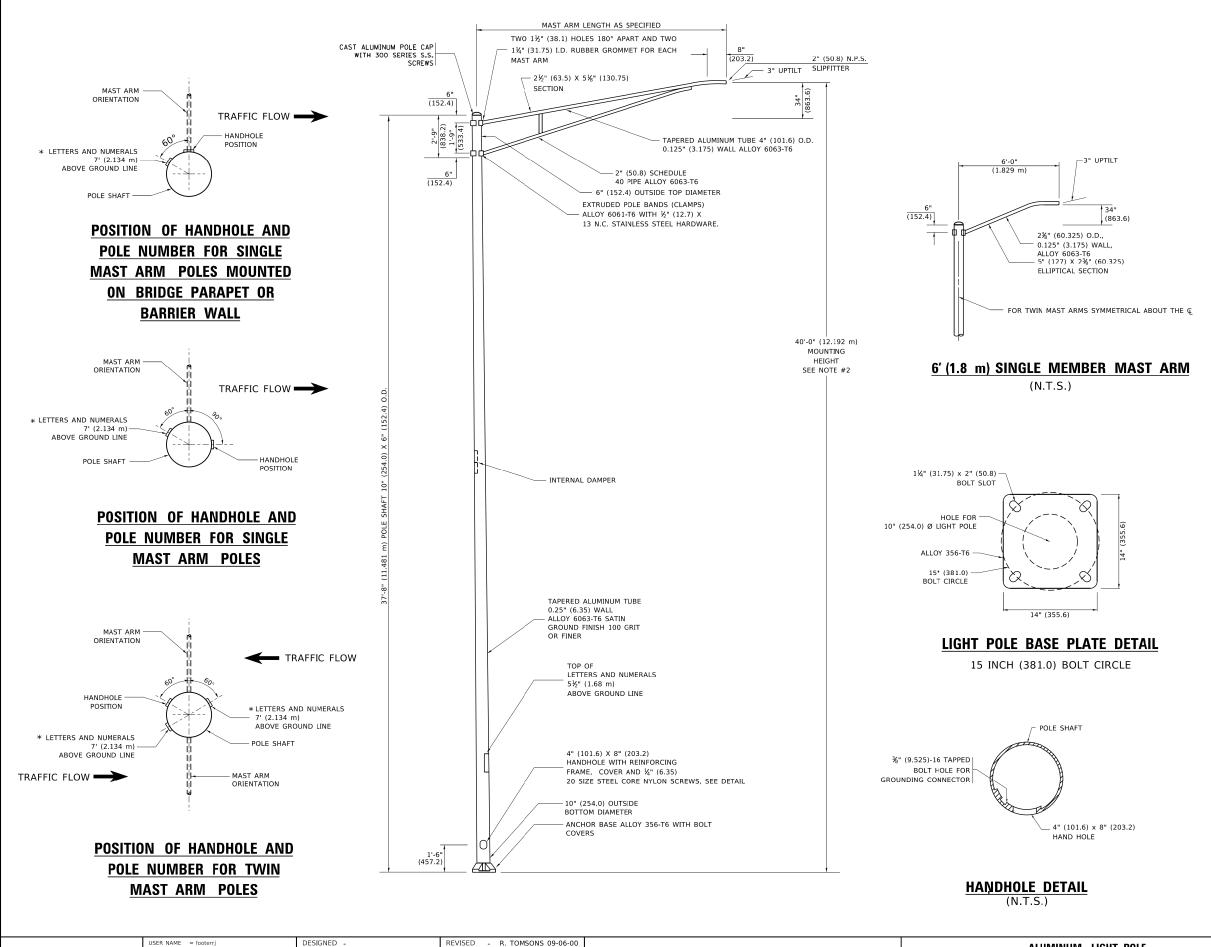
- 1. ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN
- 2. THE ANCHOR RODS AND RACEWAYS SHALL BE PROPERLY SECURED IN PLACE BEFORE THE CONCRETE IN PLACED.
- 3. THE FOUNDATION SHALL NOT PROTRUDE MORE THAN 100MM (4 IN.) ABOVE THE FINISHED GRADE WITHIN A 60 IN. (1.5 m) CHORD ACROSS THE FOUNDATION, WITH ANCHOR RODS INCLUDED, IN ACCORDANCE WITH AASHTO GUIDELINES. IF THE FOUNDATION HEIGHT, INCLUDING ANCHOR RODS, EXTENDS BEYOND THESE SPECIFIED LIMITS, THE FOUNDATION SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE. SEE FOUNDATION EXTENSION DETAIL.
- THE HOLE FOR THE FOUNDATION SHALL BE MADE BY DRILLING WITH AN AUGER, OF THE SAME DIAMETER AS THE FOUNDATION. IF SOIL CONDITIONS REQUIRE THE USE OF A LINER TO FORM THE HOLE, THE LINER SHALL BE WITHDRAWN AS THE CONCRETE IS DEPOSITED.
- 5. THE TOP OF THE FOUNDATION SHALL BE CONSTRUCTED LEVEL. A LINER OR FORM SHALL BE USED TO PRODUCE A UNIFORM SMOOTH SIDE TO THE TOP OF THE FOUNDATION. FOUNDATION TOP SHALL BE CHAMFERED 3#4-IN. (20 mm).
- 6. THE CONCRETE SHALL BE CLASS SI. CONCRETE SHALL CURE ACCORDING TO ARTICLE 1020.13 BEFORE LIGHT POLES ARE INSTALLED.
- 7. THE ANCHOR ROD SHALL BE A HOOK ROD TYPE. COLD BENDING OF THE ANCHOR ROD WILL NOT BE ALLOWED. THE RADIUS OF THE HOOK BEND SHALL NOT BE LESS THAN 4 TIMES THE NOMINAL DIAMETER OF THE ANCHOR ROD. A TACK WELDED ANCHOR ROD MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER.
- 8. THE ANCHOR RODS SHALL BE ACCORDING TO ASTM F1554 GRADE 725 (GRADE 105). NUTS SHALL BE HEXAGON NUTS ACCORDING TO ASTM A 194 2H OR ASTM A 563 DH, AND WASHERS SHALL BE ACCORDING TO ASTM F 436.
- 9 ANCHOR RODS, NUTS AND WASHERS SHALL BE COMPLETELY GALVANIZED BY EITHER THE HOT-DIPPED PROCESS CONFORMING WITH AASHTO M 232, THE MECHANICAL PLATING METHOD CONFORMING TO AASHTO M 298, CLASS 50 WITH A MAXIMUM COATING THICKNESS OF 150 UM(6 MILS) OR THE ELECTROLYTIC PROCESS ACCORDING TO ASTM F 1136.
- 10. THE ANCHOR RODS SHALL BE THREADED A MINIMUM OF 6 INCHES (150 mm) WITH A MINIMUM OF 3 INCHES (75 mm) OF THREADED ANCHOR ROD EMBEDDED IN THE FOUNDATION.
- 11. ANCHOR RODS SHALL PROJECT 23#4" (69.9 mm) ABOVE THE TOP OF THE FOUNDATION. IF BREAKAWAY COUPLINGS ARE SPECIFIED, THE CONTRACTOR SHALL CAREFULLY COORDINATE THE ANCHOR ROD PROJECTION WITH THE INSTALLATION REQUIREMENTS OF THE BREAKAWAY COUPLINGS.
- 12. THE CONTRACTOR SHALL USE A #3 SPIRAL AT 6" (152.4 mm) PITCH OR MAY SUBSTITUTE #3 TIES AT 12" (304.8 mm) O.C. WITH THE APPROVAL OF THE ENGINEER.
- 13. THE CABLE TRENCHES AND FOUNDATION SHALL BE BACK FILLED AND COMPACTED AS SPECIFIED BEFORE THE LIGHT POLE IS ERECTED.
- 14 THE RACEWAYS SHALL PROJECT 1" (25.4 mm) ABOVE THE TOP OF THE FOUNDATION.

USER NAME = footemj	DESIGNED -	REVISED - 04-22-02
	DRAWN -	REVISED -
PLOT SCALE = 50.0000 ' / in.	CHECKED -	REVISED -
PLOT DATE = 4/19/2019	DATE -	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

LIGHT POLE FOUNDATION							F.A RT	
40′ (12.192 n	n) <b>TO</b>	47 1	⁄2′ (14	.478	B m) M.	H. 15"	(381 mm) BOLT CIRCLE	110
LE: NONE	SHEET	4	OF	7	SHEETS	STA.	TO STA.	-

SECTION COUNTY 15-00277-01-BR KANE 542 202 BE-301 CONTRACT NO. 61H95



REVISED - R. TOMSONS 09-02-03

R. TOMSONS 01-18-13

REVISED

DRAWN

CHECKED

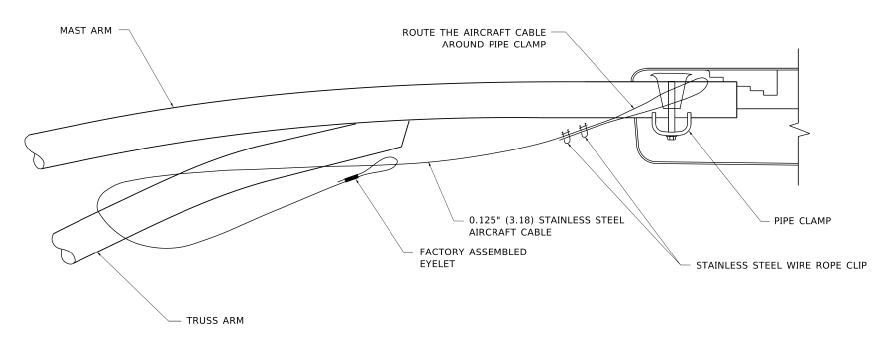
DATE

LOT DATE = 4/19/2019

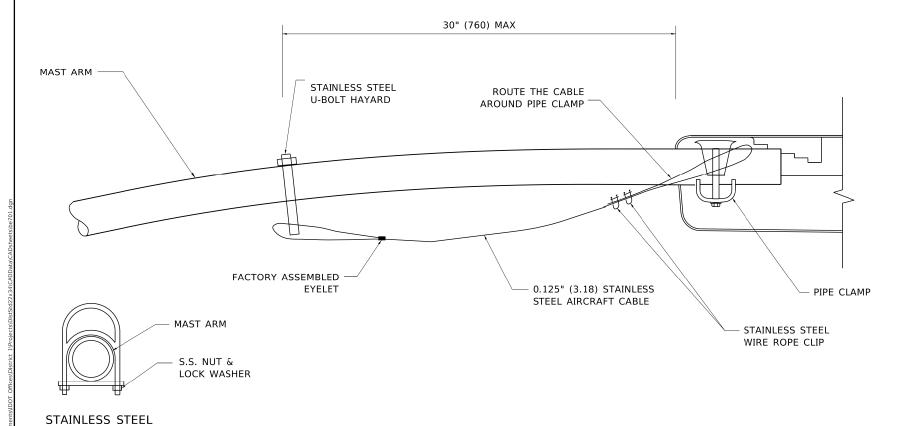
#### **NOTES**

- 1. ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.
- 2. MOUNTING HEIGHT IS DEFINED AS THE DISTANCE FROM THE CENTERLINE OF THE TENON TO THE BOTTOM OF THE ANCHOR BASE.
- 3. THE LIGHT POLE WILL MEET AASHTO DESIGN CRITERIA AS SPECIFIED.
- 4. THE INSTALLING CONTRACTOR WILL PROVIDE A UL LISTED GROUNDING CONNECTOR. BURNDY K2C23, T&B SP4DL OR APPROVED EQUAL.
- 5. LIGHT POLES WILL NOT BE INSTALLED WITHOUT MAST ARMS AND LUMINAIRES.
- 6. LIGHT POLES WILL BE SET PLUMB ON THE FOUNDATION WITHOUT THE USE OF LEVELING NUTS, WASHERS OR SHIMS.
- 7. LIGHTING UNIT IDENTIFICATION NUMBERS SHALL BE INSTALLED BEFORE THE LIGHTING UNIT IS ENERGIZED.

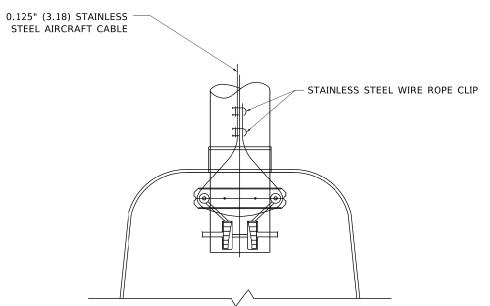
OTATE OF HIMOIO	ALUMINUM LIGHT POLE					F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.			
STATE OF ILLINOIS		40'-0" (12.192 m) MOUNTING HEIGHT					1107	15-00277-01-BR	KANE	542	203		
DEPARTMENT OF TRANSPORTATION		70-0	112.11	2 111	,				BE-401	CONTRAC	T NO. 6	1H95	
	SCALE: NONE	SHEET 5	OF	7	SHEETS	STA.	TO STA.		ILLINOIS FED.	AID PROJECT			



# SIDE VIEW (TRUSS ARM) N.T.S.



# $\frac{\text{SIDE VIEW}}{\text{N.T.S.}} \hspace{0.1cm} \text{(SINGLE MEMBER OR DAVIT ARM)}$



BOTTOM VIEW N.T.S.

#### NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE SHOWN.
- 2. CONTRACTOR SHALL ADJUST THE WIRE CLIP TO ELIMINATE ANY SLACK FROM THE WIRE ROPE.
- 3. THE 0.125" (3.18) STAINLESS STEEL AIRCRAFT CABLE SHALL REMAIN VISIBLE FROM THE GROUND LEVEL.
- 4. THE BREAKING STRENGTH OF THE CABLE SHALL BE 1700 LBS. MIN.

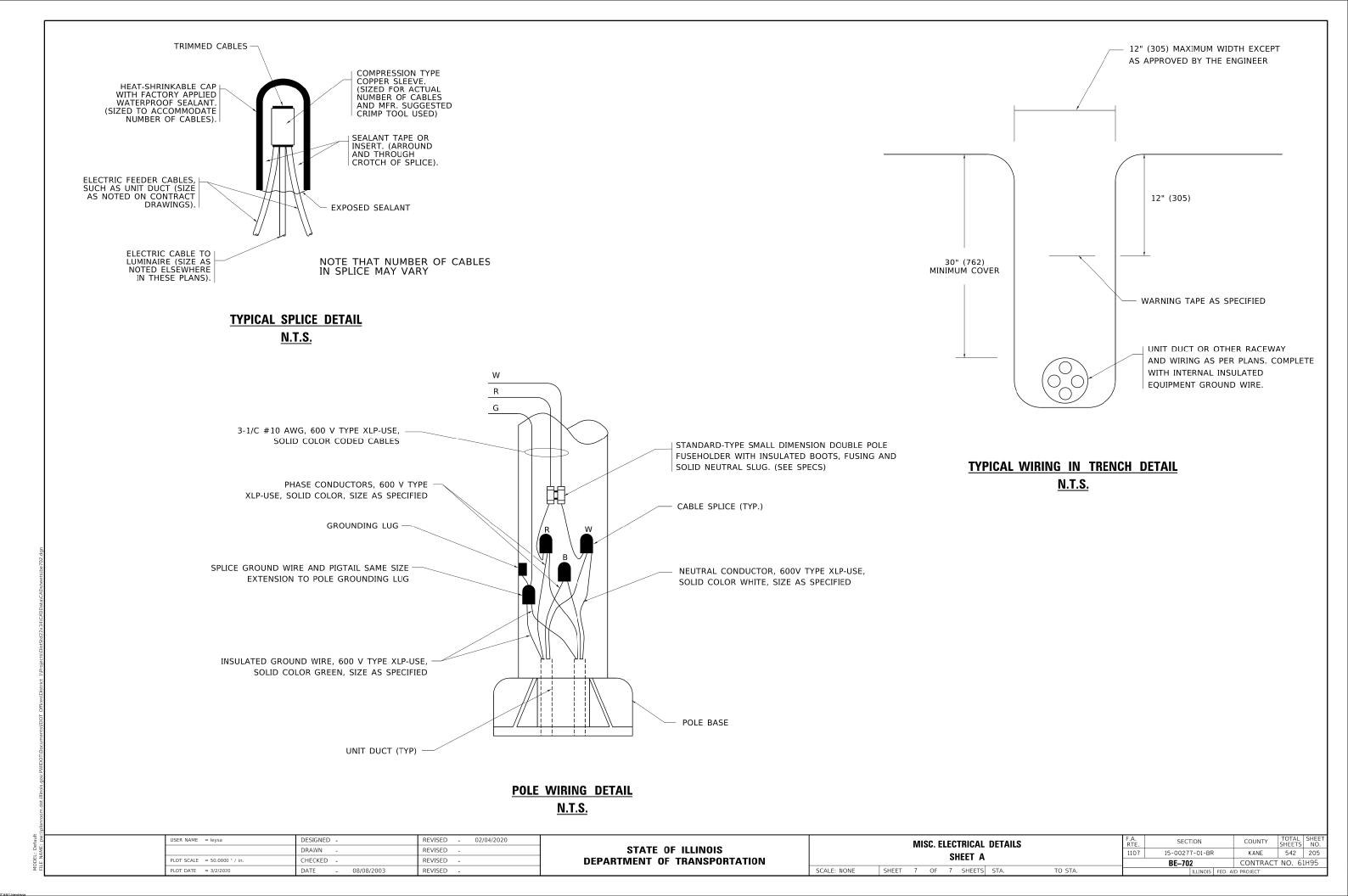
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	DRAWN -	REVISED -
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PLOT DATE = 4/19/2019	DATE -	REVISED -

U-BOLT HAYARD

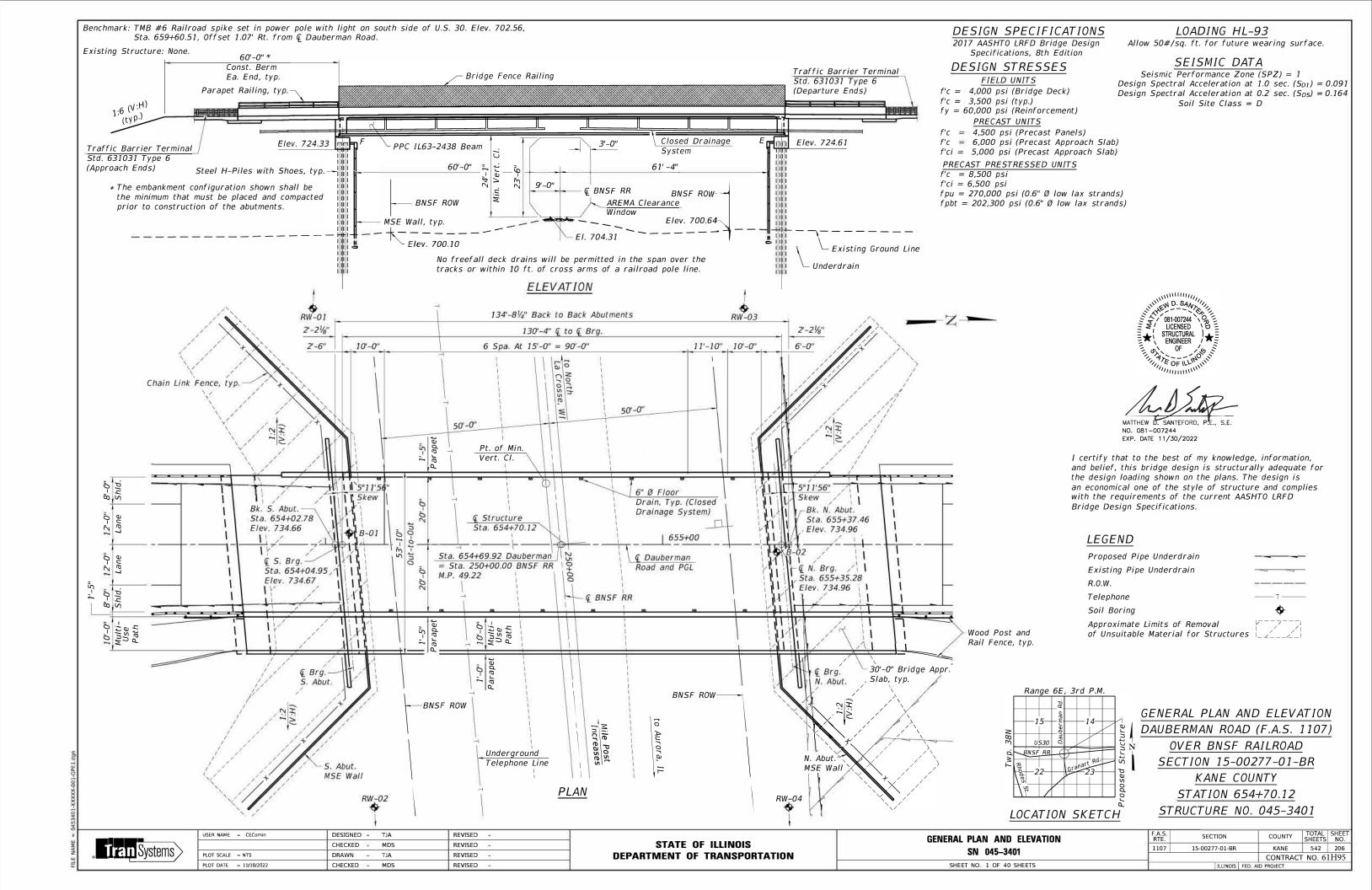
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SCALE: NONE

	F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
LUMINAIRE SAFETY CABLE ASSEMBLY		15-00277-01-BR	KANE	542	204
		BE-701	CONTRACT	NO. 6	LH95
SHEET 6 OF 7 SHEETS STA. TO STA.		TILINOIS EED A	ID DDOIECT		



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#### GENERAL NOTES:

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Protective coat shall be applied to the top of multi-use path, top of parapets, inside faces of exterior parapets, and both faces of interior parapet.
- 3. Slipforming of the parapets is not allowed.

#### SUGGESTED SEQUENCE OF CONSTRUCTION

- 1. Locate existing utilities that are to remain. Contractor to coordinate any required improvements to or removals of existing utilities with utility owner(s). See Utility Location Plans.
- 2. Complete the Removal and Disposal of Unsuitable Materials and replace with Aggregate Subgrade Improvement.
- 3. Install Piles
- 4. Construct the abutments and MSE walls.
- 5. Place the Precast Prestressed Concrete Beams on the abutments.
- 6. Construct the bridge deck, parapets, and railings.
- 7. All Lightweight Cellular Concrete Fill shall be Class IV. See Special Provisions.

#### TOTAL BILL OF MATERIAL

Item	Unit	Super	Sub	Total
Porous Granular Embankment	Cu. Yd.		703	703
Structure Excavation	Cu. Yd.		990	990
Removal And Disposal Of Unsuitable Material For Structures	Cu. Yd.		703	703
Floor Drains	Each	16		16
Concrete Structures	Cu. Yd.		107.2	107.2
Concrete Superstructure	Cu. Yd.	321.9		321.9
Protective Coat	Sq. Yd.	545		545
Furnishing And Erecting Precast Prestressed Concrete Beams, IL63N	Foot	1,054		1,054
Reinforcement Bars, Epoxy Coated	Pound	71,740	9,280	81,020
Bridge Fence Railing	Foot	266		266
Parapet Railing	Foot	253		253
Furnishing Steel Piles HP12X53	Foot		3,216	3,216
Driving Piles	Foot		3,216	3,216
Test Pile Steel HP12X53	Each		2	2
Pile Shoes	Each		26	26
Name Plates	Each	1		1
Preformed Joint Strip Seal	Foot	107		107
Elastomeric Bearing Assembly, Type I	Each	8		8
Anchor Bolts, 1 1/4"	Each	32		32
Mechanically Stabilized Earth Retaining Wall	Sq. Ft.		4,297	4,297
Drainage System For Structures	L. Sum	1.0		1.0
Granular Backfill For Structures	Cu. Yd.		184	184
Chain Link Fence, 4'	Foot	219		219
Lightweight Cellular Concrete Fill	Cu. Yd.		7,450	7,450
Anti-Graffiti Coating	Sq. Ft.		8,308	8,308
Concrete Wearing Surface, 5"	Sq. Yd.	356		356
Precast Bridge Approach Slab	Sq. Ft.	3,065		3,065
Bridge Deck Thin Polymer Overlay 3/8"	Sq. Yd.	857		857
Mechanically Stabilized Earth Retaining Wall, Special	Sq. Ft.		4,613	4,613

#### INDEX OF SHEETS

Boring Logs 4

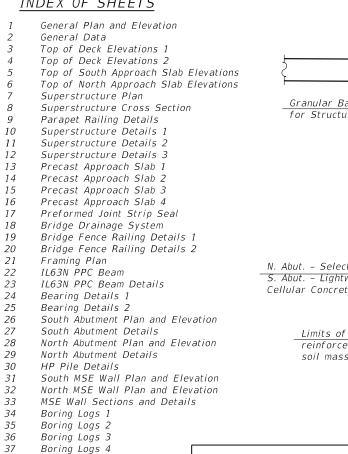
Boring Logs 5

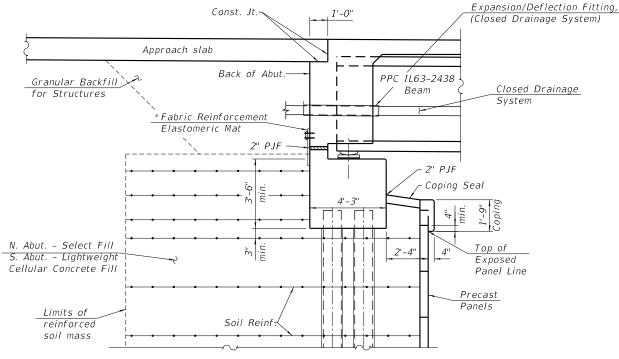
Boring Logs 6

Boring Logs 7

38

39



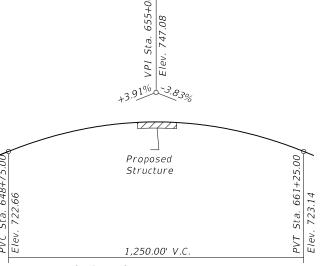


#### SECTION THRU ABUTMENT

(South Abutment shown, North Abutment similar) (See sheets 31 thru 33 of 40 for MSE Wall details) \* Cost included with Concrete Superstructure

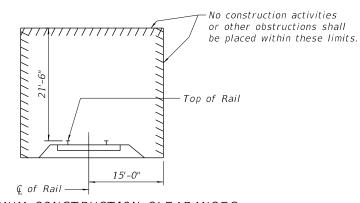
BNSF CROSSING NUMBER 977805V BUILT 20-- BY KANE COUNTY SEC. 15-00277-01-BR F.A.S. RT. 1107 STA. 654+70.12 STR. NO. 045-3401 LOADING HL-93

> NAME PLATE See Std. 515001



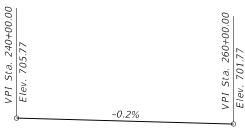
PROFILE GRADE DAUBERMAN

Along Q Dauberman Rd.



MINIMUM CONSTRUCTION CLEARANCES

(Normal to railroad) Not to scale



PROFILE GRADE BNSF Along & BNSF Railroad

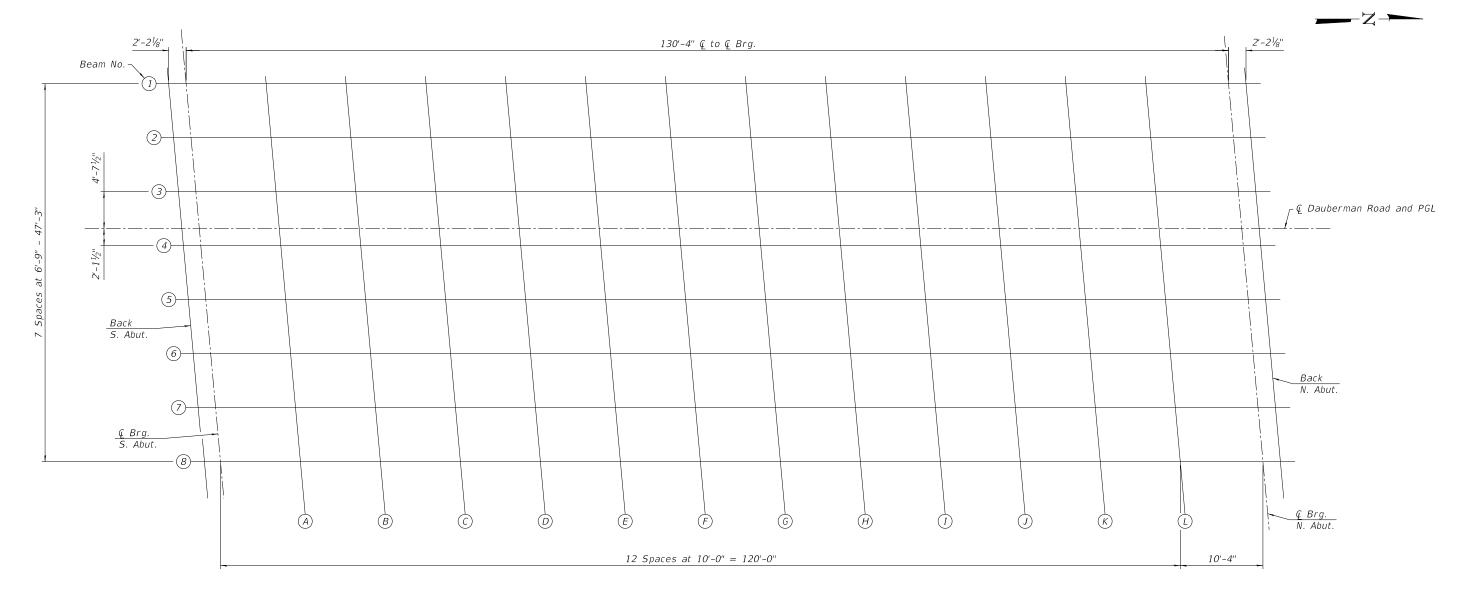


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	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 11/18/2022	CHECKED - MDS	REVISED -

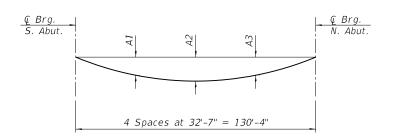
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

GENERAL DATA				
SN	045-3401			
SHEET NO.	2 OF 40 SHEETS			

SECTION COUNTY 15-00277-01-BR 1107 KANE 542 207 CONTRACT NO. 61H95



#### PLAN



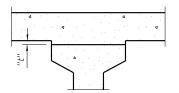
1			
Beam No.	A1	A2	A3
1	1 1/4"	17/8"	11/4"
2 thru 7	11/4"	17/8"	11/4"
8	11/4"	17/8"	11/4"

#### DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete deck, fillets, and parapets only.)

#### Note

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown on Sheet 4.



To determine "t": After all precast prestressed beams have been erected, elevations of the top flanges of the beams shall be taken at intervals shown above. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflections" shown on Sheet 4, minus slab thickness, equals the fillet heights "t" above top flanges of beams.

#### FILLET HEIGHTS

» Lie in Systems >
--------------------

USER NAME = bmsetzke	DESIGNED - TJA	REVISED -
	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF DECK ELEVATIONS 1		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
SN 045-3401	1107	15-00277-01-BF		KANE	542	208
014 043-3401				CONTRAC	T NO. 6	1H95
SHEET NO. 3 OF 40 SHEETS		ILLINOIS	FED. A	ID PROJECT		

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections
Bk. S. Abut. CL Brg. S. Abut. A B C D E F G H I J K CL Brg. N. Abut. Bk. N. Abut.	654+01.12 654+03.30 654+13.30 654+23.30 654+23.30 654+33.30 654+63.30 654+63.30 654+63.30 654+93.30 655+03.30 655+13.30 655+23.30 655+33.63 655+33.63	-18.13 -18.13 -18.13 -18.13 -18.13 -18.13 -18.13 -18.13 -18.13 -18.13 -18.13 -18.13 -18.13 -18.13	734.29 734.30 734.36 734.42 734.46 734.51 734.54 734.60 734.61 734.63 734.63 734.62 734.63 734.62 734.63	734.29 734.30 734.39 734.49 734.56 734.63 734.68 734.75 734.75 734.75 734.75 734.70 734.65 734.60

Location		Station	0ffset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections
Bk. S. Abu CL Brg. S. Abu CL Brg. N. Abu Bk. N. Abu	t. ABCDEFGHIJKL.	654+01.74 654+03.92 654+13.92 654+23.92 654+33.92 654+53.92 654+63.92 654+63.92 654+83.92 654+93.92 655+03.92 655+03.92 655+23.92 655+23.92 655+23.92	- 11 . 38 - 11 . 38	734.42 734.44 734.50 734.55 734.60 734.64 734.70 734.73 734.74 734.75 734.76 734.75 734.75 734.74	734.42 734.44 734.54 734.62 734.70 734.82 734.86 734.89 734.89 734.88 734.87 734.87

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections
Bk. S. Abut. CL Brg. S. Abut. A B C D E F G H I J K C CL Brg. N. Abut. Bk. N. Abut.	654+02.35 654+04.53 654+14.53 654+24.53 654+34.53 654+44.53 654+54.53 654+64.53 654+74.53 654+94.53 655+04.53 655+14.53 655+24.53 655+24.53 655+34.86	-4.63 -4.63 -4.63 -4.63 -4.63 -4.63 -4.63 -4.63 -4.63 -4.63 -4.63 -4.63	734.57 734.58 734.64 734.69 734.74 734.78 734.82 734.85 734.87 734.88 734.90 734.90 734.89 734.89	734.57 734.58 734.67 734.76 734.84 734.91 735.00 735.02 735.02 735.02 735.02 735.02 735.03 736.00 734.87

<u> PGL</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections
Bk. S. Abut.	654+02.77	0.00	734.66	734.66
CL Brg. S. Abut.	654+04.95	0.00	734.67	734.67
A	654+14.95	0.00	734.73	734.77
В	654+24.95	0.00	734.79	734.86
С	654+34.95	0.00	734.83	734.93
D	654+44.95	0.00	734.87	735.00
E	654+54.95	0.00	734.91	735.05
F	654+64.95	0.00	734.94	735.09
G	654+74.95	0.00	734.96	735.11
Н	654+84.95	0.00	734.98	735.12
I	654+94.95	0.00	734.99	735.11
J	655+04.95	0.00	734.99	735.09
K	655+14.95	0.00	734.99	735.06
L	655+24.95	0.00	734.98	734.98
CL Brg. N. Abut.	655+35.28	0.00	734.96	734.96
Bk. N. Abut.	655+37 . 46	0.00	734.96	734.96

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections
Bk. S. Abut. CL Brg. S. Abut. A B C D E F G H	654+02.97 654+05.15 654+15.15 654+25.15 654+35.15 654+45.15 654+65.15 654+65.15 654+75.15	2.12 2.12 2.12 2.12 2.12 2.12 2.12 2.12	734.62 734.63 734.69 734.75 734.79 734.83 734.87 734.90 734.92 734.94	734.62 734.63 734.72 734.82 734.89 734.96 735.01 735.05 735.07 735.08
I	654+95.15 655+05.15	2.12 2.12	734.95 734.95	735.08 735.05
K	655+15.15	2.12	734.95	735.02
CL Brg. N. Abut. Bk. N. Abut.	655+25 . 15 655+35 . 48 655+37 . 66	2.12 2.12 2.12	734.94 734.92 734.92	734.94 734.92 734.92

<u>BEAM 4</u>

Location	Station	0ffset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections
Bk. S. Abut. CL Brg. S. Abut.	654+03.58 654+05.76	8.87 8.87	734.48 734.50	734.48 734.50
A	654+15.76	8.87	734.56	734.60
В	654+25.76	8.87	734.61	734.68
С	654+35.76	8.87	734.66	734.76
D	654+45.76	8.87	734.70	734.83
E	654+55.76	8.87	734.73	734.88
F	654+65.76	8.87	734.76	734.92
G	654+75.76	8.87	734.78	734.94
Н	654+85.76	8.87	734.80	734.95
I	654+95.76	8.87	734.81	734.94
J	655+05.76	8.87	734.81	734.92
K	655+15.76	8.87	734.81	734.88
L	655+25.76	8.87	734.80	734.80
CL Brg. N. Abut.	655+36.09	8.87	734.78	734.78
Bk. N. Abut.	655+38.27	8.87	734.78	734.78

<u>BEAM 5</u>

<u>BEAM 6</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections
Bk. S. Abut.	654+04.19	15.62	734.36	734.36
CL Brg. S. Abut.	654+06.37	15.62	734.37	734.37
A	654+16.37	15.62	734.43	734.46
В	654+26.37	15.62	734.48	734.55
С	654+36.37	15.62	734.53	734.63
D	654+46.37	15.62	734.57	734.70
E	654+56.37	15.62	734.60	734.74
F	654+66.37	15.62	734.63	734.78
G	654+76.37	15.62	734.65	734.80
Н	654+86.37	15.62	734.67	734.81
I	654+96.37	15.62	734.68	734.81
J	655+06.37	15.62	734.68	734.78
K	655+16.37	15.62	734.68	734.75
L	655+26.37	15.62	734.67	734.67
CL Brg. N. Abut.	655+36.70	15.62	734.65	734.65
Bk. N. Abut.	655+38.88	15.62	734.65	734.65

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections
Bk. S. Abut.	654+04.81	22.37	734.31	734.31
CL Brg. S. Abut.	654+06.99	22.37	734.31	734.31
	654+16.99	22.37	734.38	734.32
A				
В	654+26.99	22.37	734.44	734.51
C	654+36.99	22.37	734.48	734.58
D	654+46.99	22.37	734.52	734.64
Ε	654+56.99	22.37	734.55	734.69
F	654+66.99	22.37	734.58	734.73
G	654+76.99	22.37	734.60	734.75
Н	654+86.99	22.37	734.62	734.76
I	654+96.99	22.37	734.63	734.75
J	655+06.99	22.37	734.63	734.73
K	655+16.99	22.37	734.63	734.70
l i	655+26.99	22.37	734.62	734.62
CL Brg. N. Abut.	655+37.32	22.37	734.60	734.60
Bk. N. Abut.	655+39.50	22.37	734.60	734.60
BR. N. Abut.	055,55.50	22.5/	7 5 7 . 00	, 57.00

<u>BEAM 7</u>

<u>BEAM 8</u>					
Location	Station	0ffset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflections	
Bk. S. Abut.	654+05.42	29.12	734.41	734.41	
CL Brg. S. Abut.	654+07.60	29.12	734.43	734.43	
Α	654+17.60	29.12	734.49	734.52	
В	654+27.60	29.12	734.54	734.61	
С	654+37.60	29.12	734.58	734.68	
D	654+47.60	29.12	734.62	734.74	
Ε	654+57.60	29.12	734.66	734.80	
F	654+67.60	29.12	734.68	734.83	
G	654+77.60	29.12	734.70	734.85	
Н	654+87.60	29.12	734.72	734.86	
I	654+97.60	29.12	734.73	734.85	
J	655+07.60	29.12	734.73	734.83	
K	655+17.60	29.12	734.73	734.80	
L	655+27.60	29.12	734.72	734.72	
CL Brg. N. Abut.	655+37.93	29.12	734.70	734.70	
Bk. N. Abut.	655+40.11	29.12	734.69	734.69	

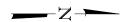
• **Tran** Systems

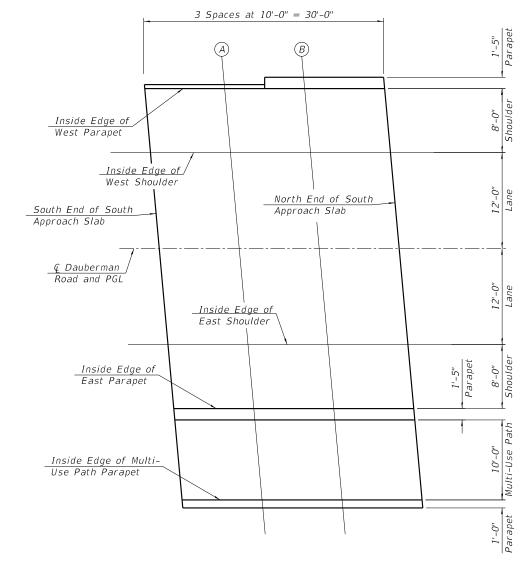
	USER NAME = bmsetzke	DESIGNED - TJA	REVISED -
		CHECKED - MDS	REVISED -
	PLOT SCALE = NTS	DRAWN - TJA	REVISED -
	PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -
_			

STAT	E OI	F ILLINOIS
DEPARTMENT	0F	TRANSPORTATION

TOP OF DECK ELEVATIONS 2	F.A.S. RTE	
SN 045-3401	1107	
OIN UTU-UTUI		
SHEET NO. 4 OF 40 SHEETS		

F.A.S. RTE	SECT	ION		COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-0027	7-01-BR		KANE	542	209
				CONTRACT	NO. 6	1H95
		ILLINOIS	FED. Al	ID PROJECT		





<u>PLAN</u>

# **Tran** Systems

USER NAME = bmsetzke	DESIGNED - TJA	REVISED -
	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

### INSIDE EDGE OF WEST PARAPET

Location	Station	0ffset	Theoretical Grade Elevations
S. End S. Approach	653+71.96	- 20 . 00	734.03
A	653+81.96	- 20 . 00	734.11
B	653+91.96	- 20 . 00	734.19
N. End S. Approach	654+01.96	- 20 . 00	734.25

#### INSIDE EDGE OF WEST SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
S. End S. Approach	653+72.69	- 12.00	734.20
A	653+82.69	- 12.00	734.28
B	653+92.69	- 12.00	734.35
N. End S. Approach	654+02.69	- 12.00	734.42

#### © DAUBERMAN ROAD AND PGL

Location	Station	Offset	Theoretical Grade Elevations	
S. End S. Approach	653+73.78	0.00	734.45	
A	653+83.78	0.00	734.53	
B	653+93.78	0.00	734.60	
N. End S. Approach	654+03.78	0.00	734.66	

#### INSIDE EDGE OF EAST SHOULDER

Location	Station	0ffset	Theoretical Grade Elevations	
S. End S. Approach	653+74.87	12.00	734.22	
A	653+84.87	12.00	734.29	
B	653+94.87	12.00	734.37	
N. End S. Approach	654+04.87	12.00	734.43	

#### INSIDE EDGE OF EAST PARAPET

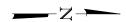
Location	Station	Offset	Theoretical Grade Elevations
S. End S. Approach	653+75.60	20.00	734.06
A	653+85.60	20.00	734.14
B	653+95.60	20.00	734.21
N. End S. Approach	654+05.60	20.00	734.28

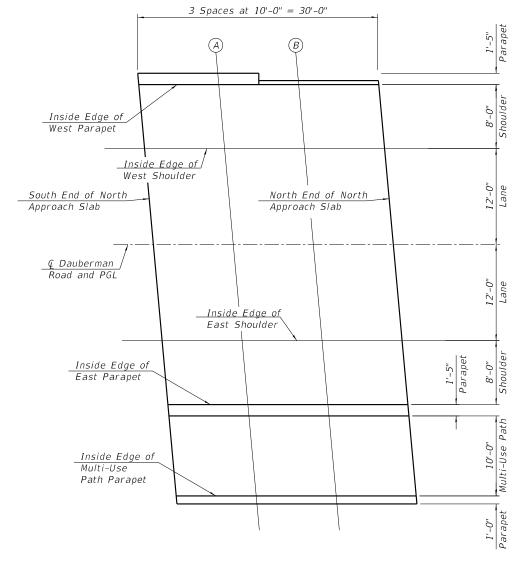
#### INSIDE EDGE OF MULTI-USE PATH PARAPET

Location	Station	0ffset	Theoretical Grade Elevations
S. End S. Approach	653+76.64	31.42	734.24
A	653+86.64	31.42	734.32
B	653+96.64	31.42	734.39
N. End S. Approach	654+06.64	31.42	734.45

ГОР	OF SOUTH			ELEVATIONS	
		SN 045-3	401		
	SHE	ET NO. 5 OF 4	0 SHEETS		

F.A.S. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-00277-01-BR		KANE	542	210
			CONTRACT	NO. 6	1H95
	ILLINOIS	FED. Al	ID PROJECT		





<u>PLAN</u>



USER NAME = bmsetzke	DESIGNED - TJA	REVISED -
	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

### INSIDE EDGE OF WEST PARAPET

Location	Station	0ffset	Theoretical Grade Elevations
S. End N. Approach	655+34.64	- 20 . 00	734.56
A	655+44.64	- 20 . 00	734.54
B	655+54.64	- 20 . 00	734.52
N. End N. Approach	655+64.64	- 20 . 00	734.48

#### INSIDE EDGE OF WEST SHOULDER

Location	Station	0ffset	Theoretical Grade Elevations
S. End N. Approach	655+35.37	-12.00	734.72
A	655+45.37	-12.00	734.70
B	655+55.37	-12.00	734.67
N. End N. Approach	655+65.37	-12.00	734.64

#### © DAUBERMAN ROAD AND PGL

Location	Station	0ffset	Theoretical Grade Elevations
S. End N. Approach	655+36 . 46	0.00	734.96
A	655+46 . 46	0.00	734.94
B	655+56 . 46	0.00	734.91
N. End N. Approach	655+66 . 46	0.00	734.88

#### INSIDE EDGE OF EAST SHOULDER

Location	Station	0ffset	Theoretical Grade Elevations
S. End N. Approach	656+68.32	12.00	733.94
A	656+78.32	12.00	733.83
B	656+88.32	12.00	733.72
N. End N. Approach	656+98.32	12.00	733.61

#### INSIDE EDGE OF EAST PARAPET

Location	Station	0ffset	Theoretical Grade Elevations
S. End N. Approach	655+38.28	20.00	734.56
A	655+48.28	20.00	734.53
B	655+58.28	20.00	734.51
N. End N. Approach	655+68.28	20.00	734.47

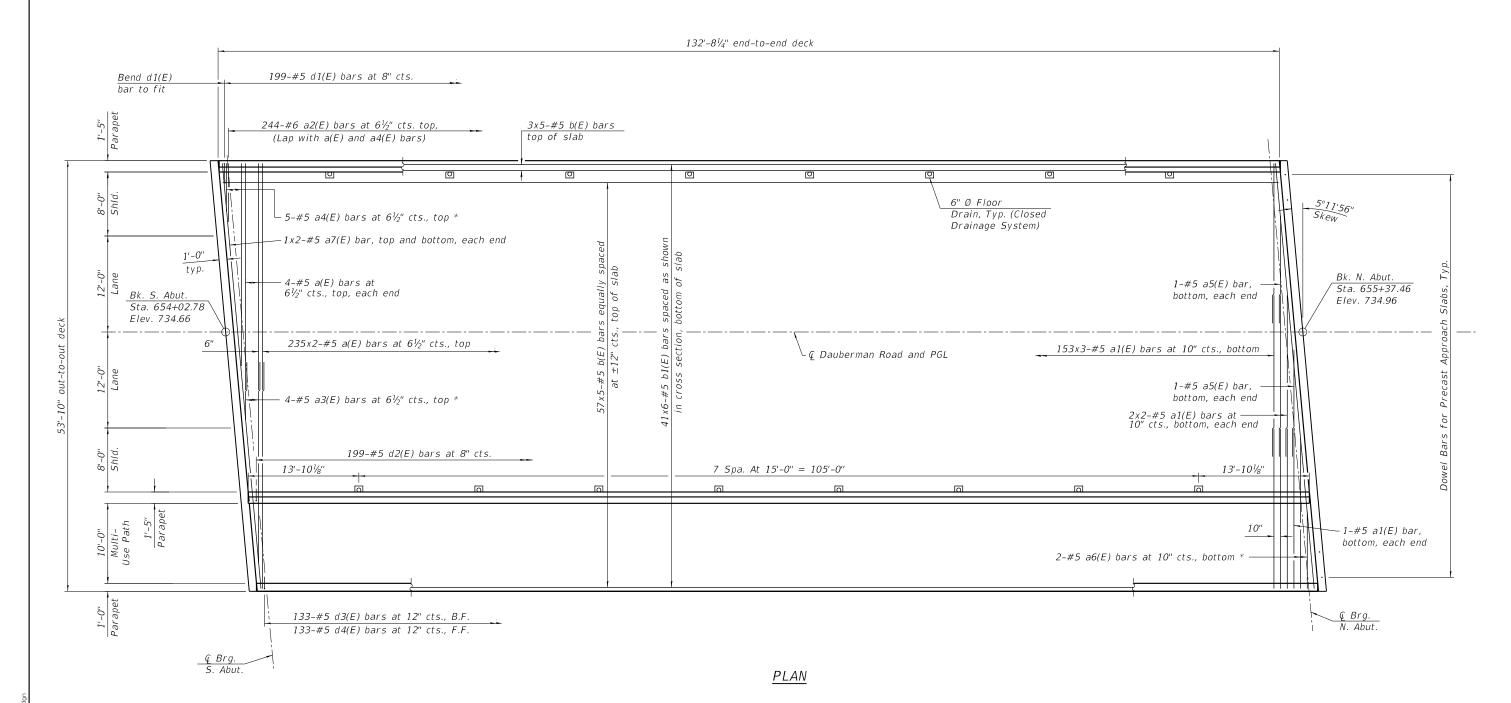
#### INSIDE EDGE OF MULTI-USE PATH PARAPET

Location	Station	0ffset	Theoretical Grade Elevations
S. End N. Approach	655+39.32	31.42	734.73
A	655+49.32	31.42	734.70
B	655+59.32	31.42	734.67
N. End N. Approach	655+69.32	31.42	734.64

OP	OF NORTH	APPROACH	SLAB	ELEVATIONS
		SN 045-3	401	
	SHEE	ET NO. 6 OF 4	0 SHEETS	

F.A.S. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-00277-01-BR		KANE	542	211
			CONTRACT	NO. 6	1H95
	ILLINOIS	FED. A	ID PROJECT		

**—**-Z-



 $\frac{MINIMUM BAR LAP}{\#5 BAR = 3'-6''}$ 

\* See Field Cutting Diagram on sheet 12 of 40 .

Notes:

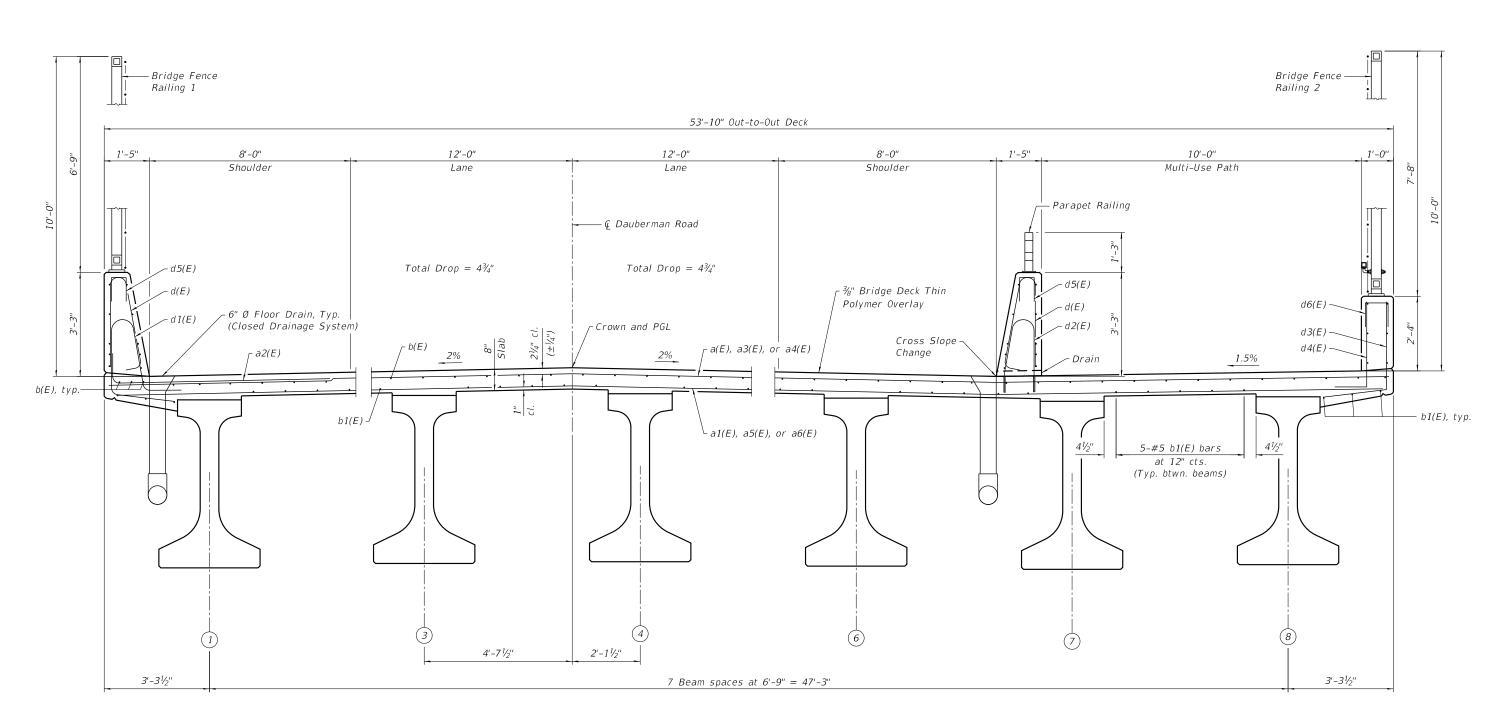
See sheet 13 of 40 for dowel bar details.



USER NAME = bmsetzke	DESIGNED - TJA	REVISED	=
	CHECKED - MDS	REVISED	=
PLOT SCALE = NTS	DRAWN - TJA	REVISED	=
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED	=

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERST	SUPERSTRUCTURE PLAN				
SN	045–3401				
SHEET NO	7 OF 40 SHEETS				



CROSS SECTION
(Looking North)

#### Notes:

See sheet 18 of 40 for drainage details.
See sheet 10 of 40 for additional parapet details.
See sheet 9 of 40 for formed drain details.

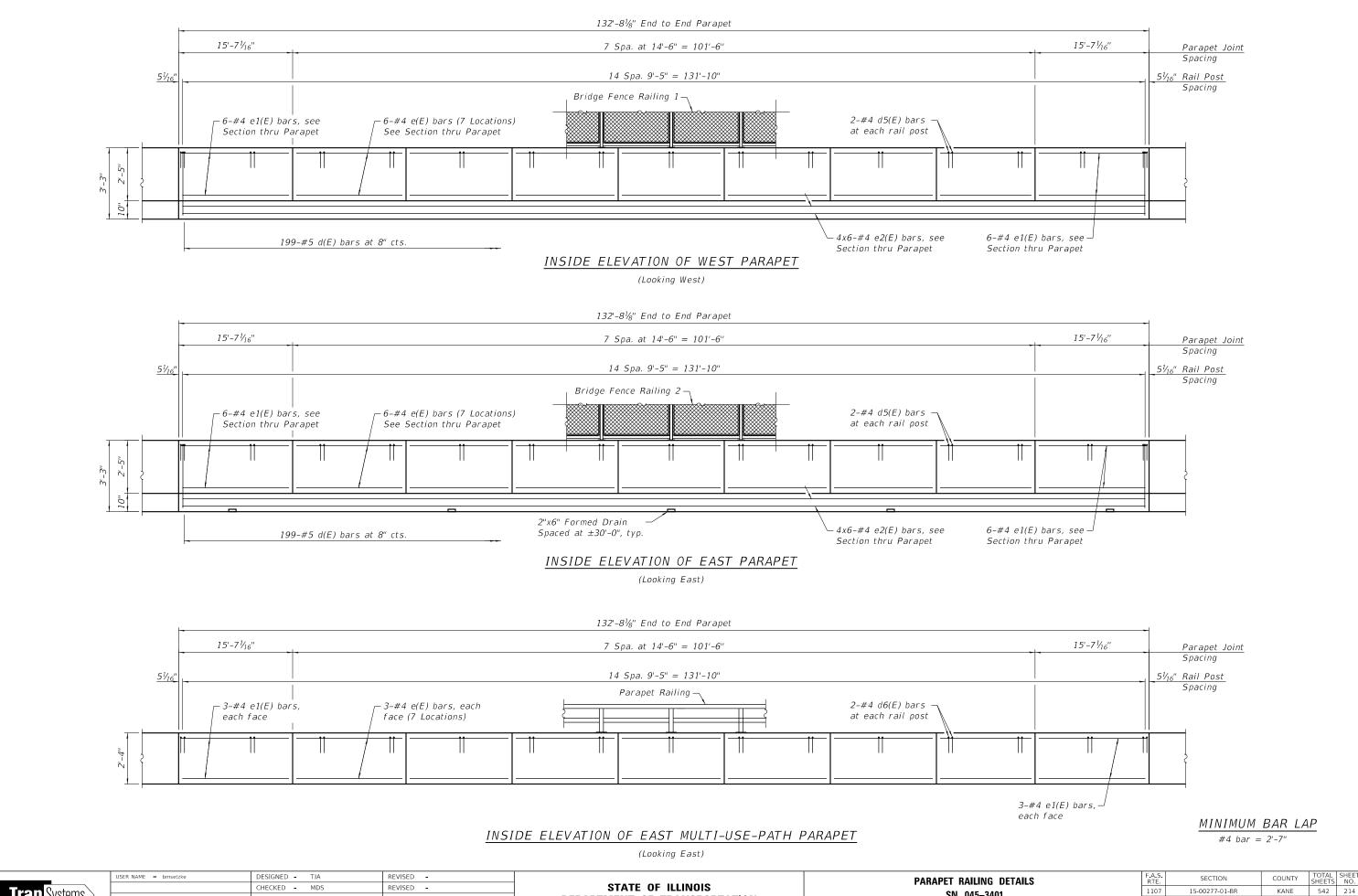
**Tran** Systems

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE CROSS SECTION
SN 045-3401
SHEET NO. 8 OF 40 SHEETS

F.A.S. RTE. SECTION COUNTY TOTAL SHEETS NO.
1107 15-00277-01-BR KANE 542 213

CONTRACT NO. 61H95



**DEPARTMENT OF TRANSPORTATION** 

SN 045-3401

SHEET NO. 9 OF 40 SHEETS

CONTRACT NO. 61H95

**Tran** Systems

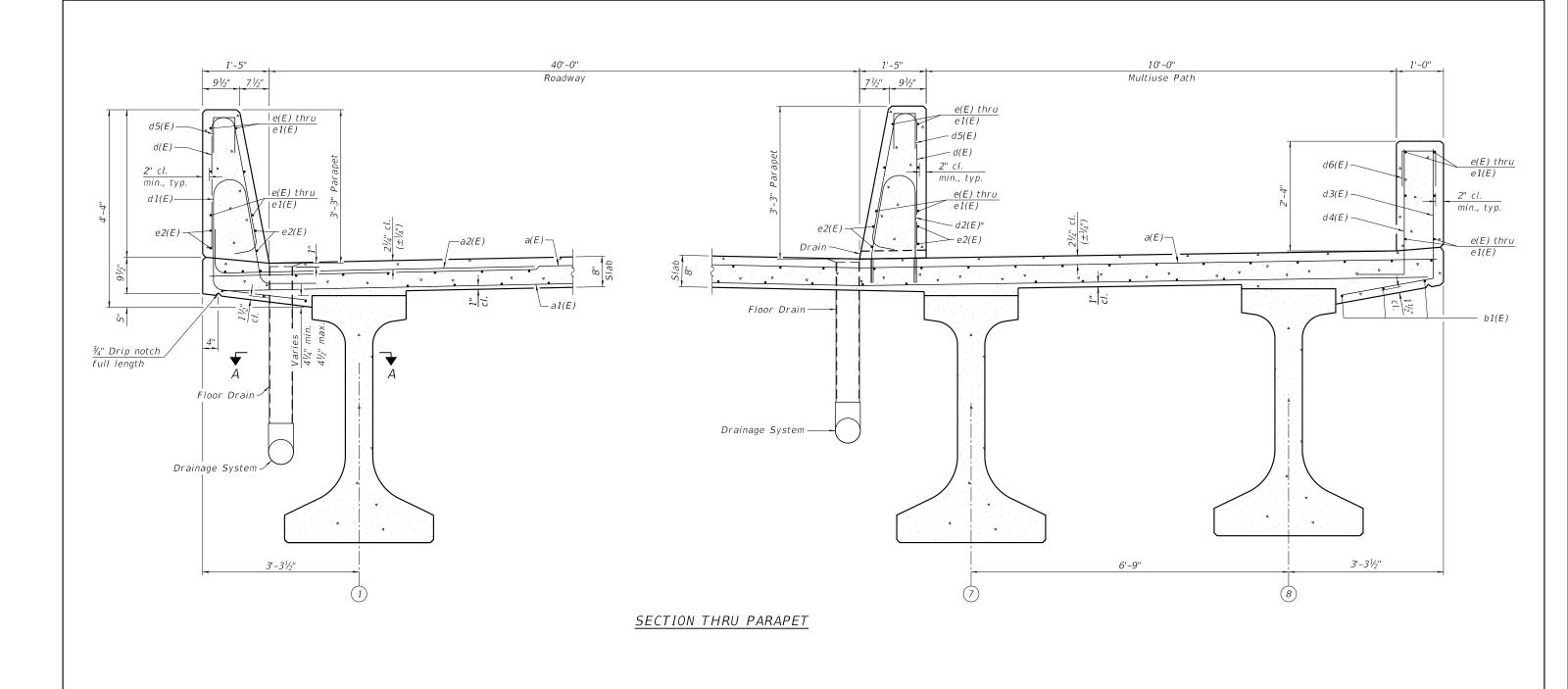
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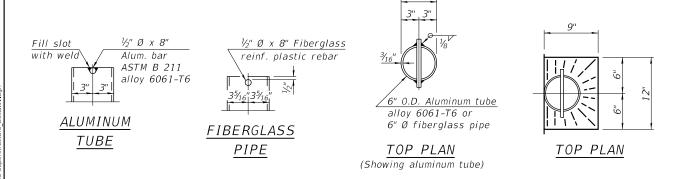
DRAWN - TJA

CHECKED - MDS

REVISED -

REVISED -





\* Core and set #5 d2(E) bar according to Article 509.06 of the Standard Specifications. Cored holes shall be roughened or scored per manufacturer's recommendations. Maximum depth of hole shall not exceed 6".

#### FLOOR DRAIN DETAILS

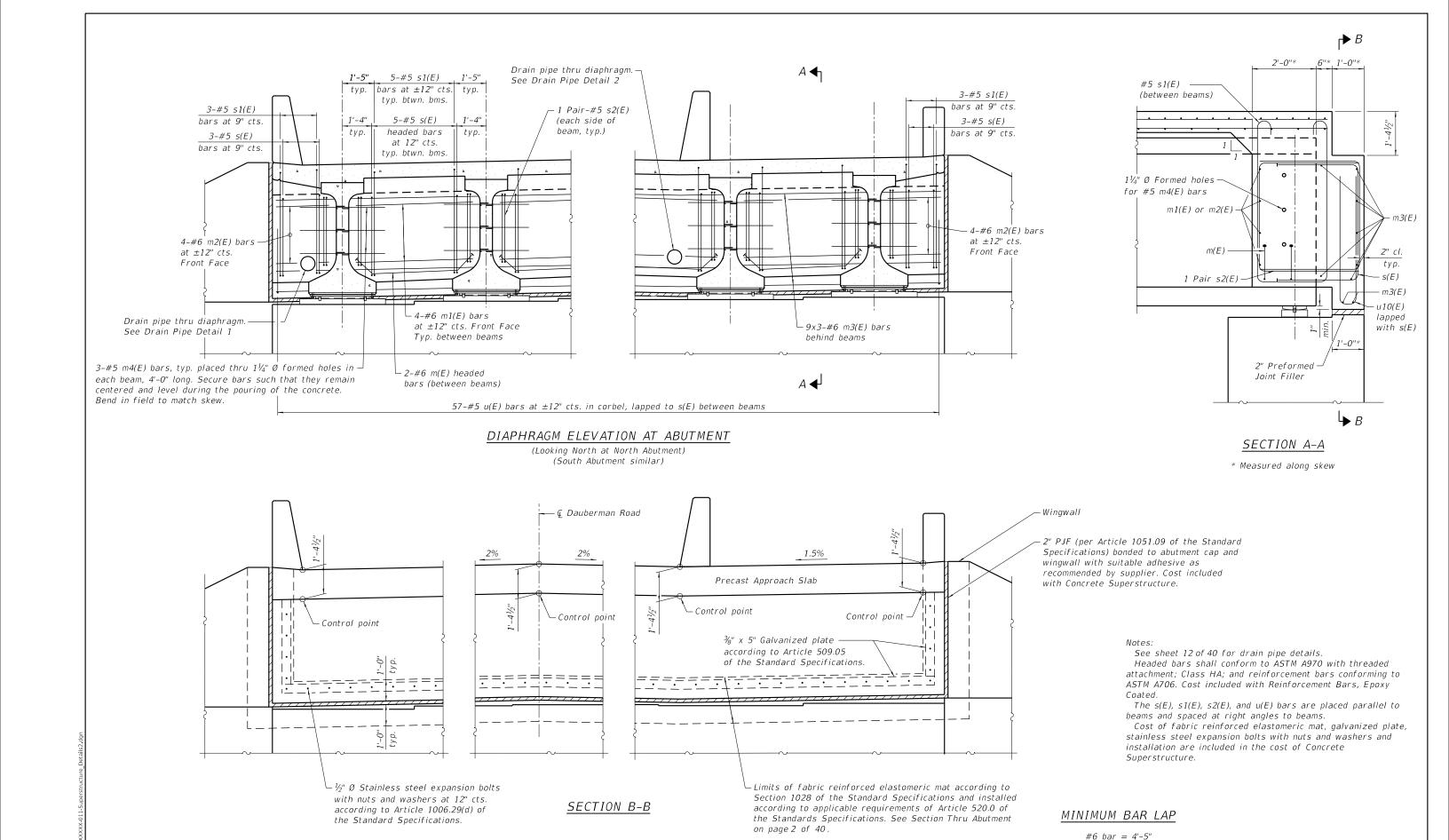


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	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS 1	F.A.S. RTE	
SN 045-3401	1107	Ī
011 073-3701		
SHEET NO. 10 OF 40 SHEETS		-

S. SECTION COUNTY TOTAL SHEETS NO.
17 15-00277-01-BR KANE 542 215
CONTRACT NO. 61H95



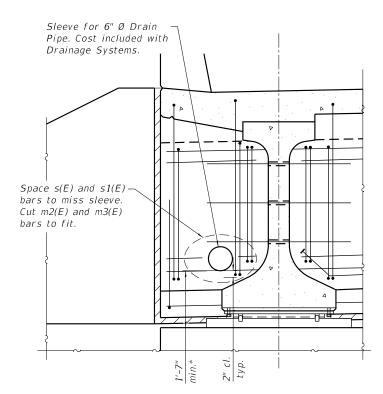
• Tran Systems

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS 2 SN 045-3401 SHEET NO. 11 OF 40 SHEETS  
 F.A.S. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

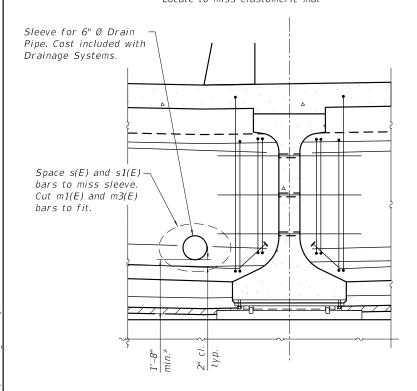
 1107
 15-00277-01-BR
 KANE
 542
 216

 CONTRACT NO. 61H95



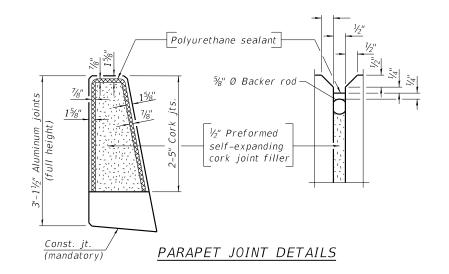
#### DRAIN PIPE DETAIL 1

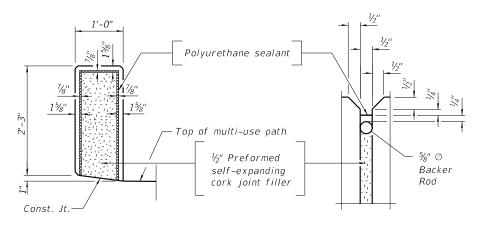
(Looking North at North Abutment) (South Abutment similar) \* Locate to miss elastomeric mat



#### DRAIN PIPE DETAIL 2

(Looking North at North Abutment) (South Abutment similar) \* Locate to miss elastomeric mat





#### MULTI-USE PATH PARAPET JOINT DETAILS

#### Notes:

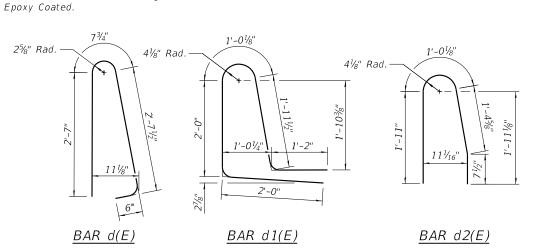
Fiberglass pipe shall conform to ASTM D2996, with short-time rupture strength hoop tensile stress of 30,000 p.s.i. minimum.

The exterior surfaces of the fiberglass floor drains shall be pigmented by the manufacturer with a color that matches the concrete.

The top portion of aluminum floor drains shall be coated to minimize reaction with wet concrete.

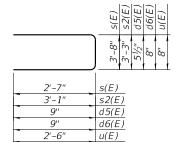
The  $\frac{1}{8}$ " Aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure. The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and

Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars,

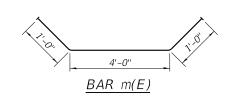


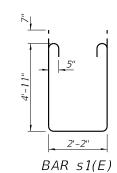


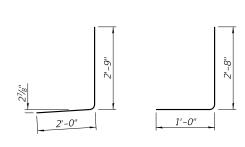
### $BAR \ a2(E)$



BARS s(E), s2(E), d5(E), d6(E) & u(E)(s(E) bars are headed)







BAR d4(E)

### FIELD CUTTING DIAGRAM

32'-3" 28'-3" 28'-10"

4-#5 a3(E) bars 5-#5 a4(E) bars 2-#5 a6(E) bars

Order a3(E) and a4(E) bars full length. Cut as shown and use remainder of bars in opposite end of deck.

#### *SUPERSTRUCTURE* BILL OF MATERIAL

<u>E</u>	SILL (	) F M A	ALEKIA	<u>L</u>
Bar	No.	Size	Length	Shape
a(E)	478	#5	28'-6"	
a1(E)	469	#5	20'-2"	
a2(E)	244	#6	8'-4"	<i>L</i>
a3(E)	4	#5	32'-3"	
a4(E)	5	#5	28'-3"	
a5(E)	4	#5	13'-4"	
a6(E)	2	#5	28'-10"	
a7(E)	8	#5	28'-10"	
b(E)	300	#5	29'-3"	
b1(E)	246	#5	25'-0"	
d(E)	398	#5	6'-5"	[]
d1(E)	199	#5	8'-2"	_
d2(E)	199	#5	5'-1"	U
d3(E)	133	#5	4'-9"	
d4(E)	133	#5	3'-8"	
d5(E)	60	#4	2'-0"	
d6(E)	30	#4	2'-2"	
e(E)	126	#4	14'-2"	
e1(E)	36	#4	15'-3"	
e2(E)	48	#4	24'-3"	
m(E)	14	#6	6'-0"	$\overline{}$
m1(E)	28	#6	5'-6"	
m2(E)	8	#6	2'-6"	
m3(E)	27	#6	20'-10"	
m4(E)	24	#5	4'-0"	
s(E)	82	#5	8'-10"	
s1(E)	82	#5	13'-2"	l l
52(E)	64	#5	9'-5"	
u(E)	114	#5	5'-8"	
Reinforc		Bars,	Lbs.	56,900
Ероху Со			200.	30,300
Concrete			Cu. Yd.	305.0
Superstr			54. 74.	303.0
Bridge D			Sq. Yd.	590
Polymer		1 3/8"	·	
Protectiv	re Coat		Sq. Yd.	383
Rars indi	cated to	hus 1 x	2-#4 eta	r indicate

Bars indicated thus 1 x 2-#4 etc. indicates 1 line of bars with 2 lengths per line.



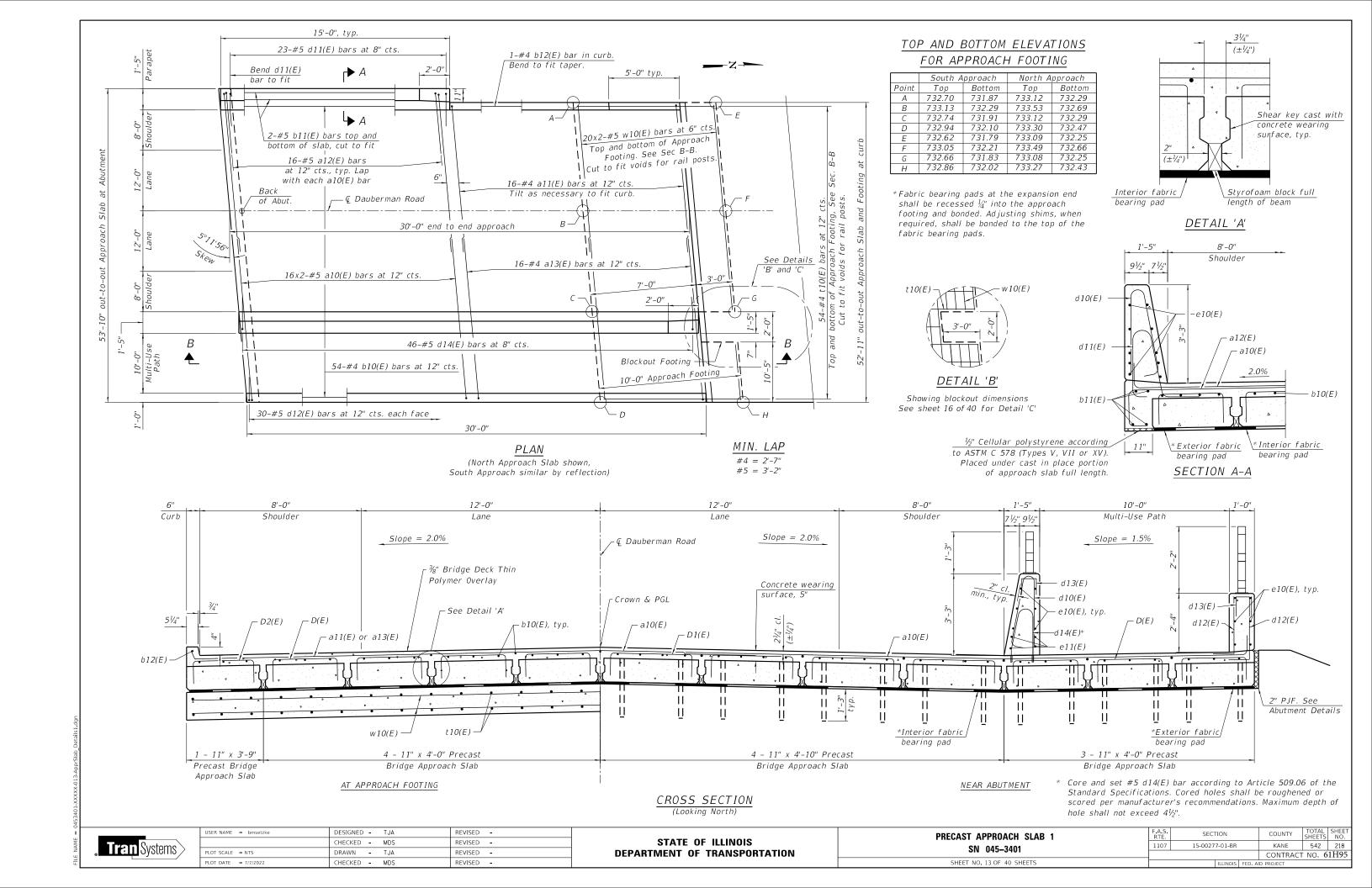
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PLOT SCALE = NTS	DRAWN -	TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED -	MDS	REVISED -

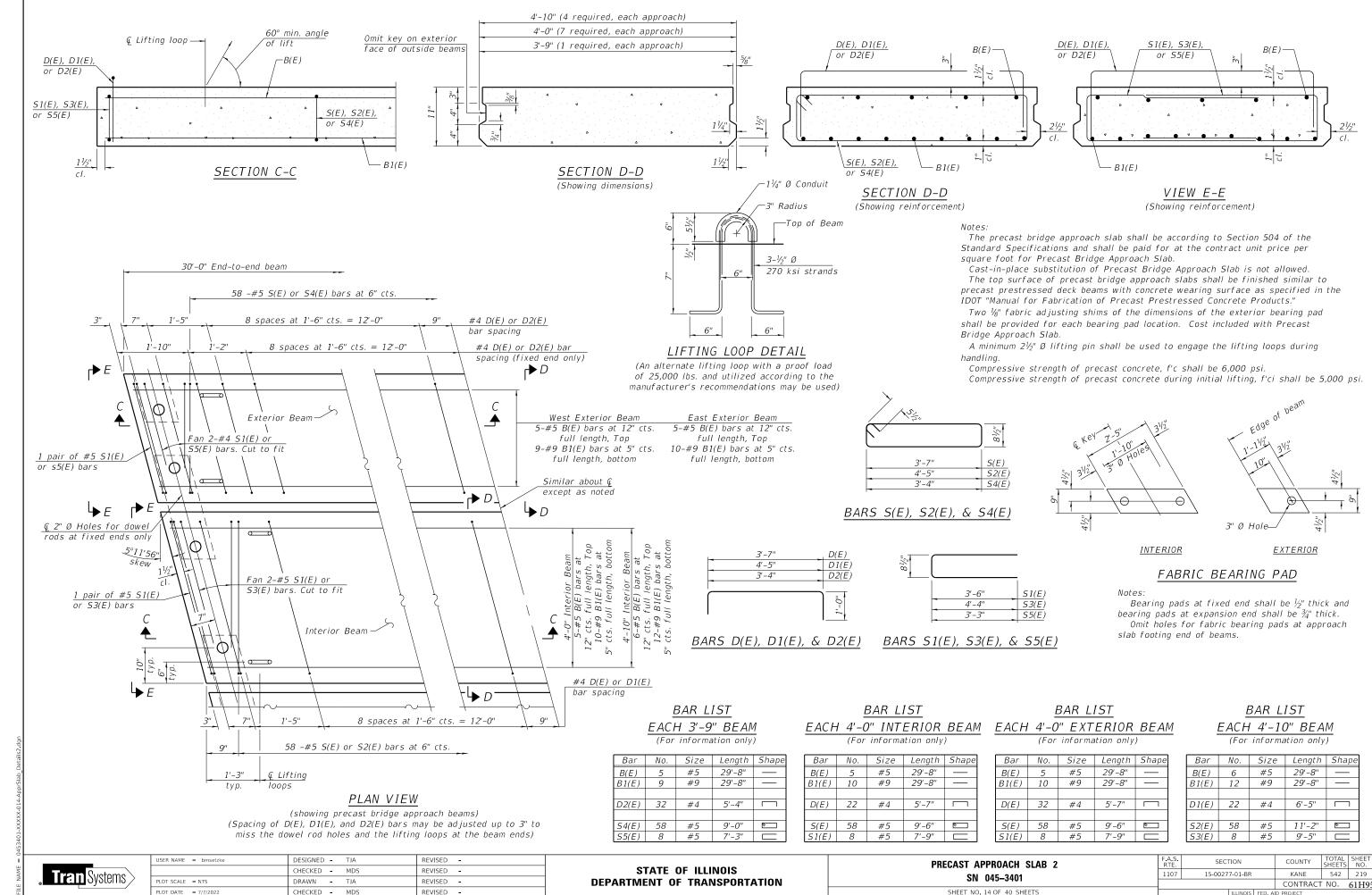
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

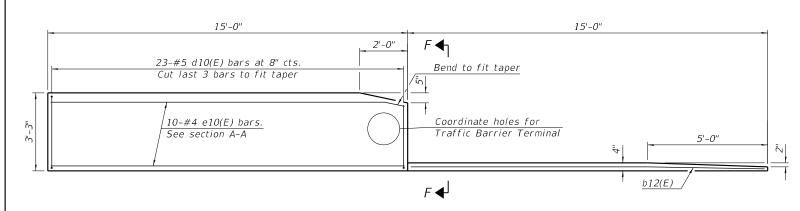
SUPERSTRUCTURE DETAILS 3	
SN 045-3401	
SHEET NO. 12 OF 40 SHEETS	

 $BAR \ d3(E)$ 

F.A.S. RTE	SEC	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHE
1107	15-0027	7-01-BR		KANE	542	217
				CONTRACT	NO. 6	51H9
		THE PROPERTY.	EED A	ID DDO IFOT		

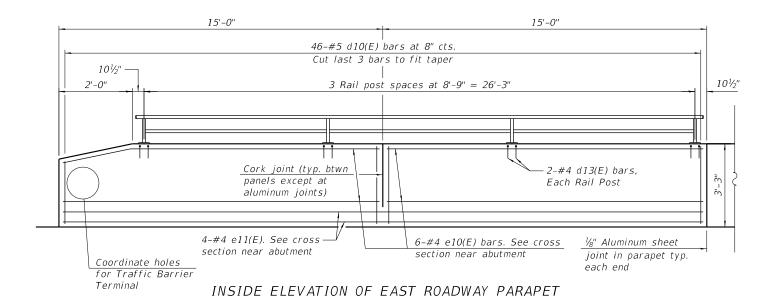




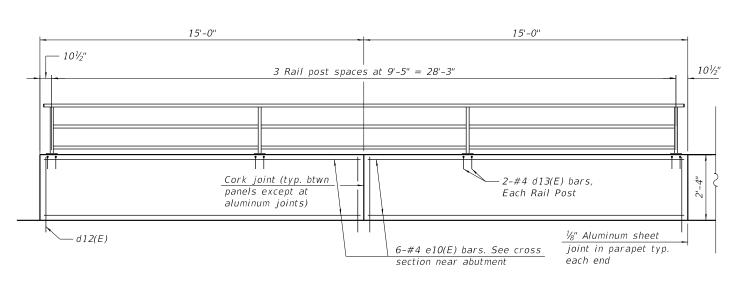


#### INSIDE ELEVATION OF WEST ROADWAY PARAPET

(North approach shown, South approach similar)



(North approach shown, South approach similar)



#### INSIDE ELEVATION OF EAST MULTI-USE PATH PARAPET

(North approach shown, South approach similar)



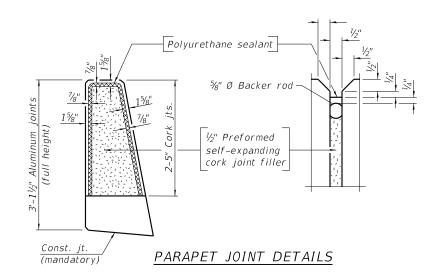
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BLOT DATE 7/7/2022	CHECKED N	4DC	DEVICED	

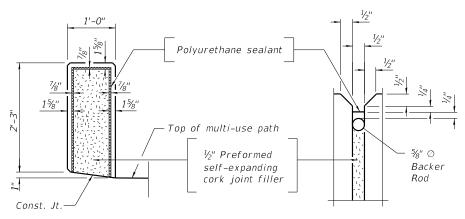
## STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PRECAST APPROACH SLAB 3
SN 045-3401
SHEET NO. 15 OF 40 SHEETS

34" 51/4" 11"

VIEW F-F



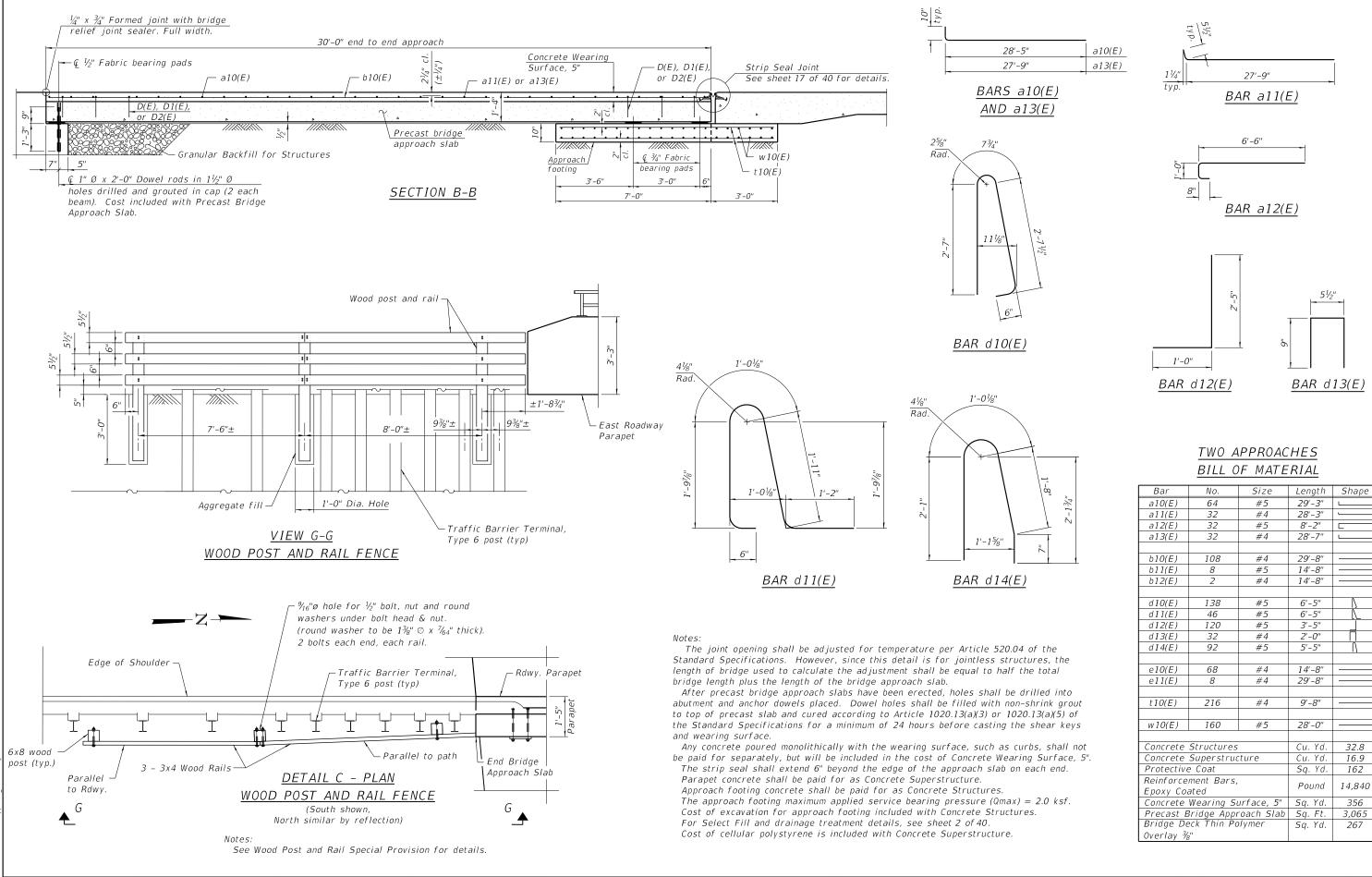


#### MULTI-USE PATH PARAPET JOINT DETAILS

#### Note

The %" Aluminum sheet shall be ASTM B 209 alloy 3003–H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure.

The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.



Tran Systems

USER NAME = CEComin DESIGNED - TJA REVISED -CHECKED - MDS REVISED -DRAWN - TJA REVISED -PLOT DATE = 11/18/2022 CHECKED - MDS REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  PRECAST APPROACH SLAB 4 SN 045-3401 SHEET NO. 16 OF 40 SHEETS

SECTION COUNTY 1107 15-00277-01-BR KANE 542 221 CONTRACT NO. 61H95

32.8

16.9

162

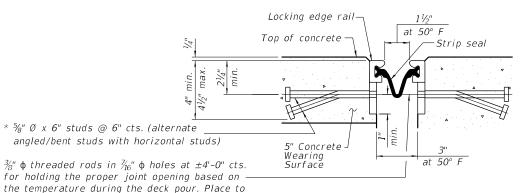
14,840

356

3,065

267

SHOWING ROLLED RAIL JOINT



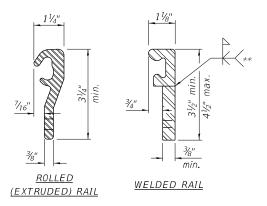
SHOWING WELDED RAIL JOINT

#### SECTION A-A

miss studs. All rods shall be burned, or sawed

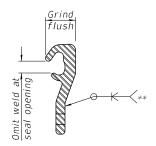
off flush with the plates after concrete is set.

\* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



#### LOCKING EDGE RAILS

\*\* Back gouge not required if complete joint penetration is verified by mock-up.



#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

## **Tran** Systems

USER NAME = bmsetzke	DESIGNED - TJA	REVISED -
	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

## STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

# PREFORMED JOINT STRIP SEAL SN 045-3401 SHEET NO. 17 OF 40 SHEETS

# F.A.S. SECTION COUNTY TOTAL SHEETS NO. 1107 15-00277-01-BR KANE 542 222 CONTRACT NO. 61H95

#### Notes:

The strip seal shall be made continuous and shall have a minimum thickness of  $V_4$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

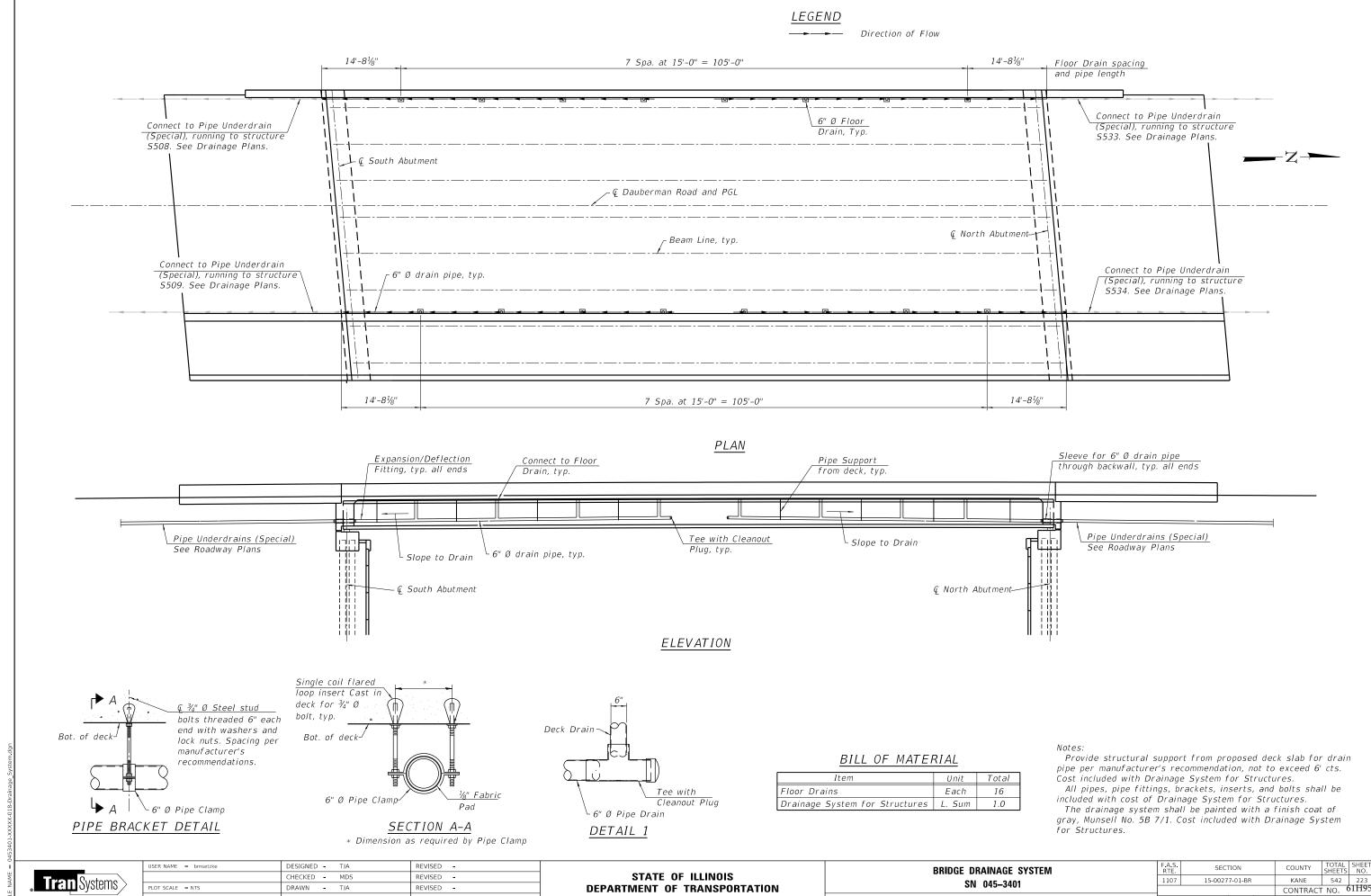
The maximum space between locking edge rail segments shall be  $\frac{3}{6}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of anchorage studs included with Preformed Joint Strip Seal.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

BILL OF MATERIAL

Item Unit Total
Preformed Joint Strip Seal Foot 107



**DEPARTMENT OF TRANSPORTATION** 

SHEET NO. 18 OF 40 SHEETS

**Tran** Systems

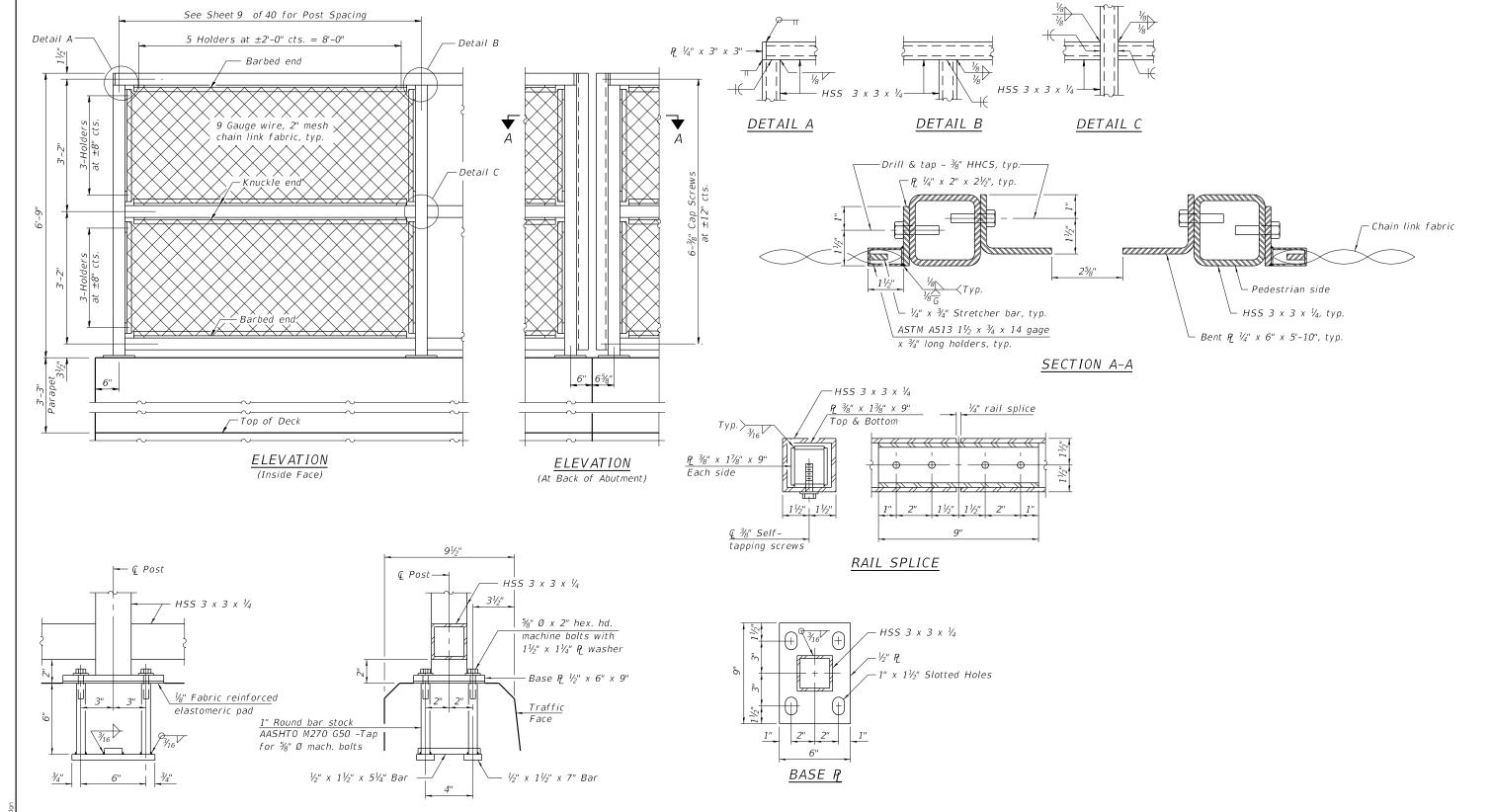
DRAWN - TJA

CHECKED - MDS

PLOT DATE = 7/7/2022

REVISED -

REVISED -



#### ANCHOR BOLT DETAILS

In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and setting  $\frac{5}{8}$ " Ø anchor rods according to Article 509.06 of the Standard Specifications. Embedment shall be according to the manufacturer's specifications.

#### A/ - 4 - .

CVN testing may be omitted for the railing.
All steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.

#### BILL OF MATERIAL

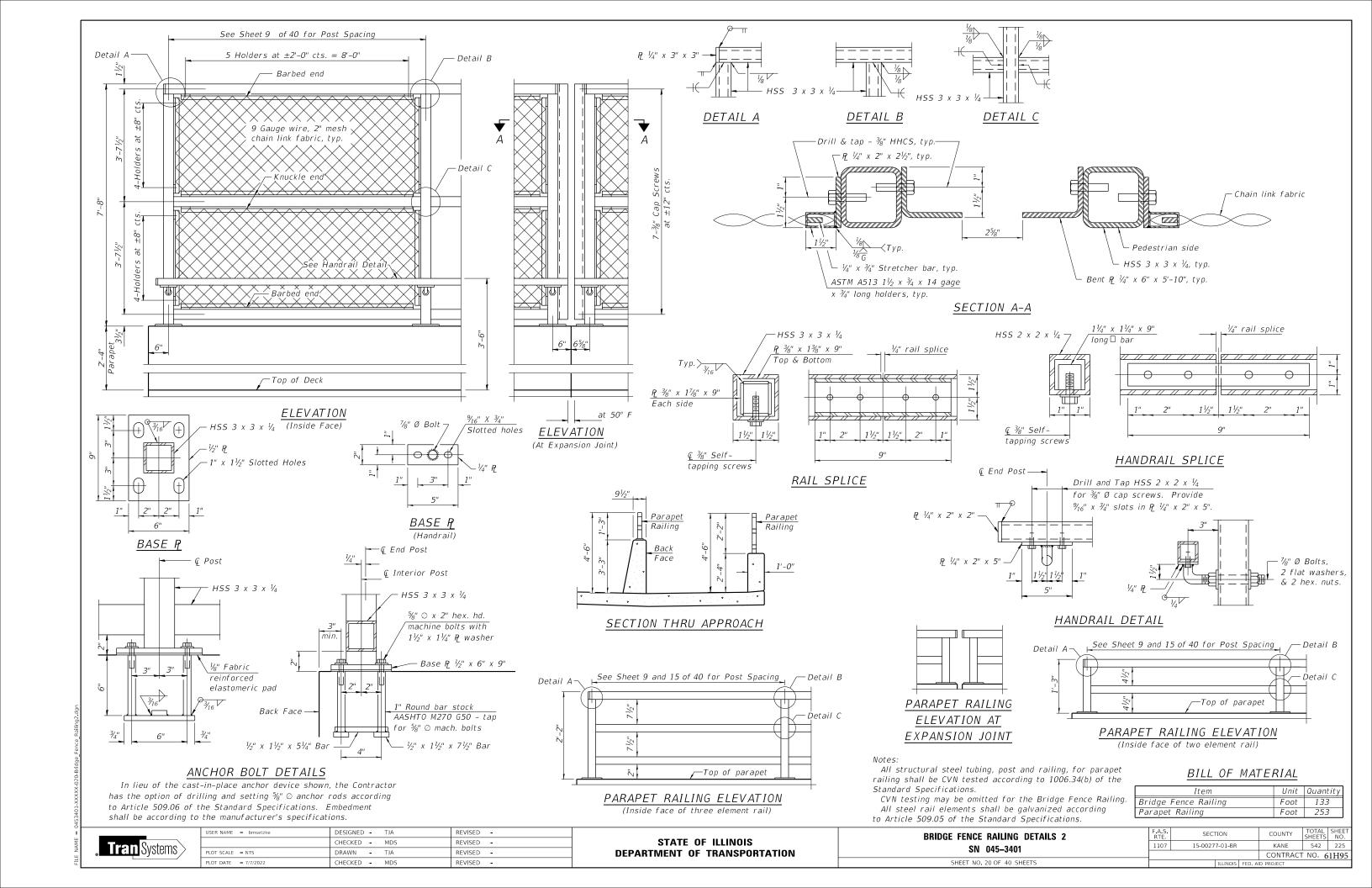
Item	Unit	Quantity
Bridge Fence Railing	Foot	133

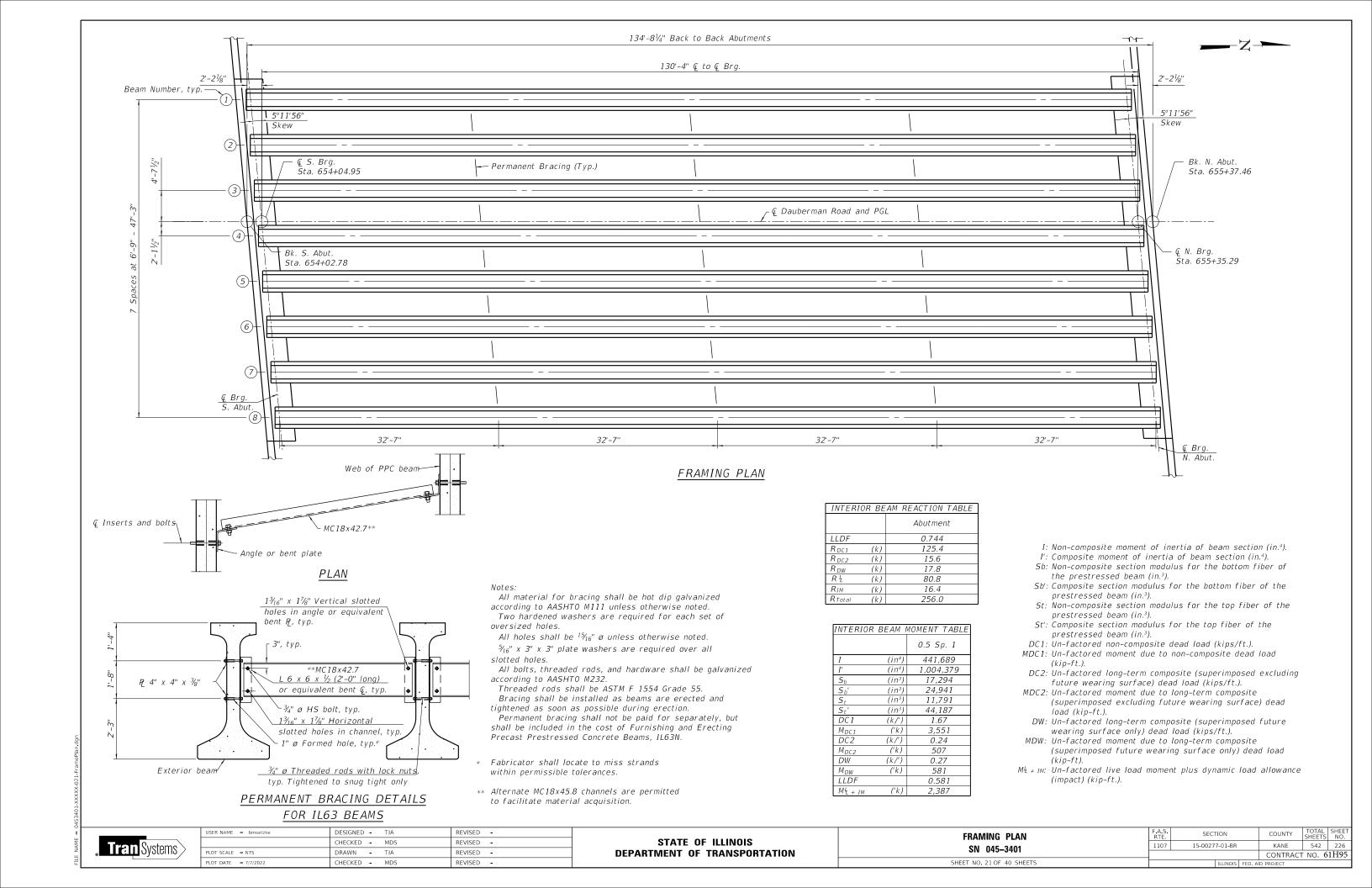


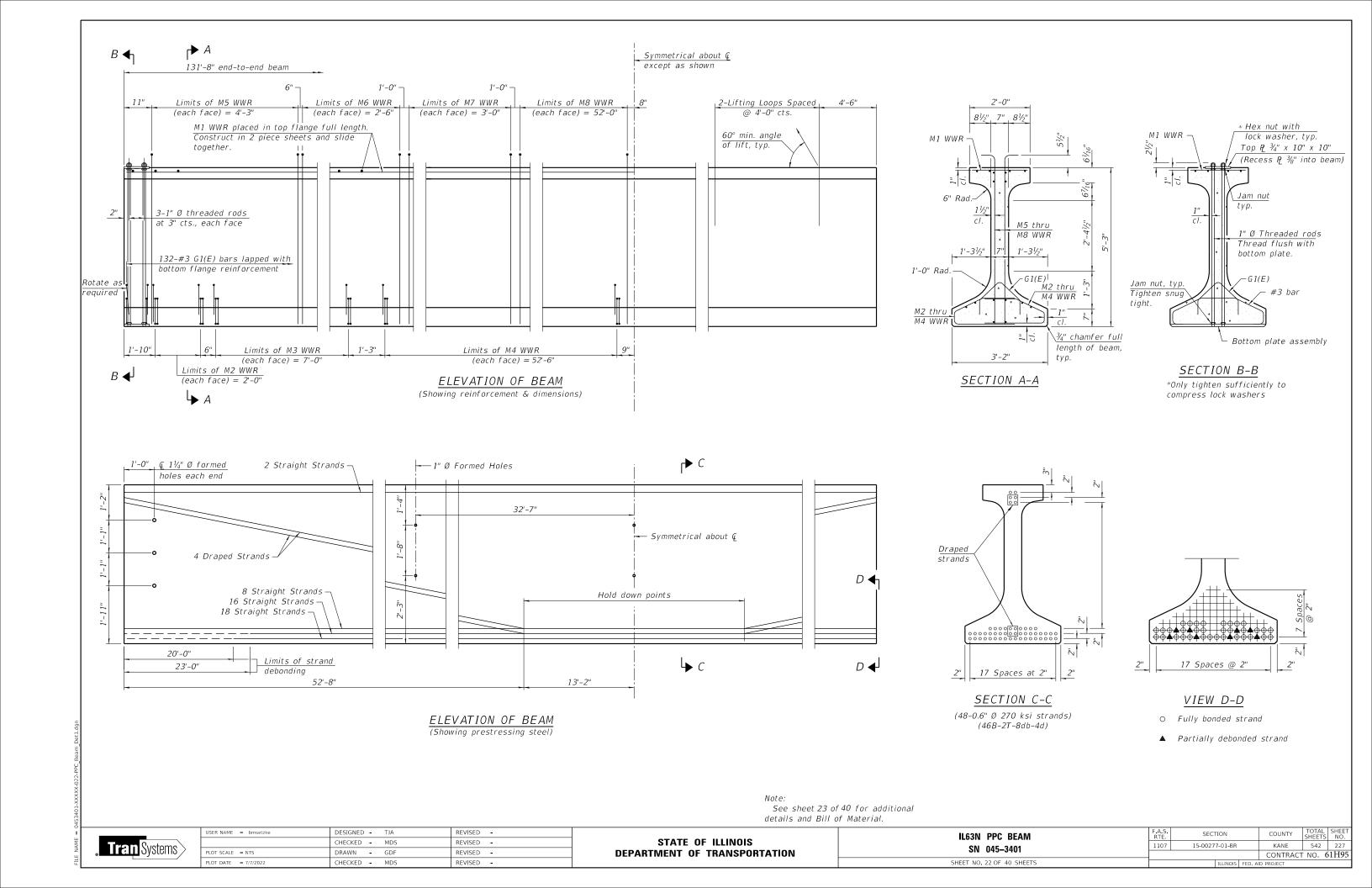
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	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

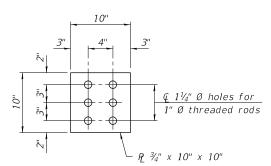
BRIDGE		RAILING 045–3401	DETAILS	1
5	HEET NO.	19 OF 40 SH	EETS	

F.A.S. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-00277-01-BR		KANE	542	224
			CONTRACT	NO. 6	1H95
	ILLINOIS F	FD. Al	D PROJECT		









111/4"

<u>¾" Cham</u>fer

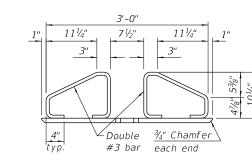
each end

PLAN - TOP PLATE

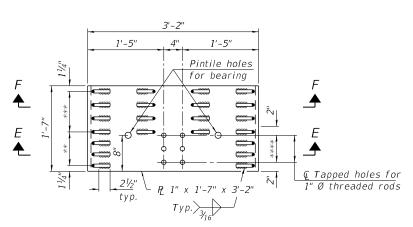
1'-11/2"

#3 bar

111/4"



SECTION E-E



#### PLAN - BOTTOM PLATE

\*\* 3 Spaces at  $2\frac{1}{2}$ " =  $7\frac{1}{2}$ "

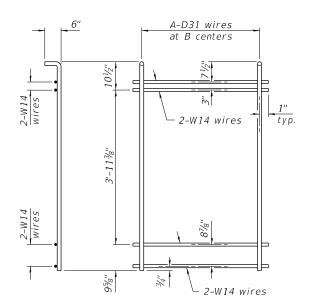
BAR G1(E)

\*\*\* 3 Spaces at 3" = 9"

\*\*\*\* 2 Spaces at 3" = 6"

See bearing details for pintile hole locations

SECTION F-F



D31 wires at 1'-6" cts.

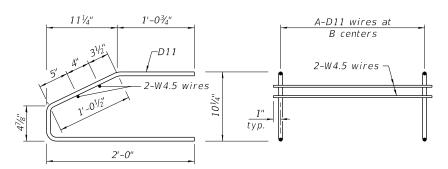
M<sub>1</sub> WWR DETAIL When multiple sheets of M1 WWR are required along the

the longitudinal D31 wires together (Min. Lap 2'-2").

beam length, #5(E) bars (5'-0" long) shall be used to splice

2-D31 wires

M5 THRU M8 WWR DETAIL (See Table of Dimensions)



#### M2 THRU M4 WWR DETAIL

(See Table of Dimensions)

#### NOTES

Inserts for  $\frac{3}{4}$ " Ø threaded dowel rods, when specified, are to be two strut, ferrule type for interior beams and single ferrule, flared loop type for exterior beams. Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter for beam strands shall be 0.6" and the nominal cross-sectional area shall be 0.217 sq. in. The nominal diameter for lifting loops shall be  $\frac{1}{2}$ " and the nominal cross sectional area shall be 0.153 sq. in.

The beams shall have a final concrete compressive strength, f'c, of 8500 psi and a release concrete compressive strength, f'ci, of 6500 psi.

A minimum  $2\frac{1}{2}$ " Ø lifting pin shall be used to engage the lifting loops during handling. The top and bottom plates shall be AASHTO M270 Grade 50.

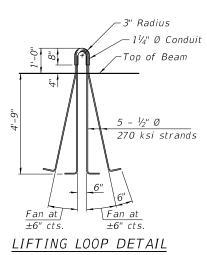
The top plates and bottom plate assemblies shall be galvanized according to AASHTO M111. The threaded rods, nuts and washers shall be galvanized according to AASHTO M232. Threaded rods shall be ASTM F 1554 Grade 55.

Welded Wire Reinforcement (WWR) shall conform to ASTM A884 with a Class A, Type 1 epoxy coating or ASTM A1060, Table 3 galvanized coating.

#### TABLE OF DIMENSIONS

(WWR tables are based on Grade 70)

WWR	Α	В
M2	9	3"
М3	15	6"
M4	37	1'-6"
M5	18	3"
М6	6	6"
M7	4	1'-0"
M8	27	2'-0"



BILL OF MATERIAL

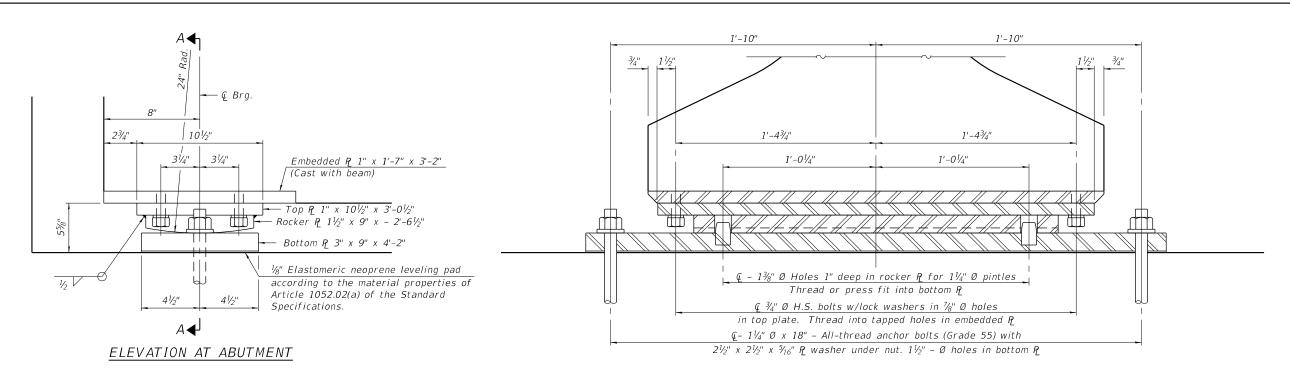
Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete Beams, IL63N	Ft.	1,054



USER NAME = CEComin	DESIGNED - TJA	REVISED -
	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - GDF	REVISED -
PLOT DATE = 11/18/2022	CHECKED - MDS	REVISED -

IL63N	PPC	BEAN	DETAILS	
	SN	045–3	401	
SH	EET NO.	23 OF 40	SHEETS	

F.A.S. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
1107	15-00277-01-BR		KANE	542	228
		CONTRACT	NO. 6	1H95	
	ILLINOIS	FED. Al	ID PROJECT		



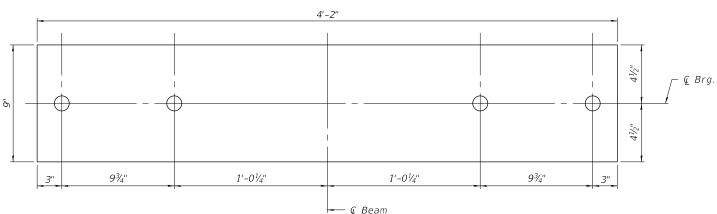
SECTION A-A

**PINTLE** 

## 3'-01/2" 2'-61/2" .01/2" 1'-01/4" 1'-01/4" 11/2" 1'-43/4" 1'-43/4" 11/2"

— ⊈ Beam

#### PLAN OF TOP PLATE & ROCKER PLATE (Looking from below at top plate and rocker plate only)



Notes:

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM

Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place and prior to pouring the deck.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Anchor bolts shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

See sheet 22 of 40 for additional details of embedded plate.

All plates, hardware, and leveling pads required for the bearing, except anchor bolts, shall be included in the cost of Furnishing and Erecting Precast Concrete Beams, IL63N.

All plate material for bearings shall be hot dip galvanized according to AASHTO M111.

All bolts and washers shall be galvanized according to AASHTO M232.

The structural steel plates and pintles of the bearing shall conform to the requirements of AASHTO M 270 Grade 50.

#### BILL OF MATERIAL

Item	Unit	Total
Anchor Bolts, 11/4	Each	16



USER NAME = bmsetzke	DESIGNED - TJA	REVISED -
	CHECKED - MDS	REVISED -
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PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

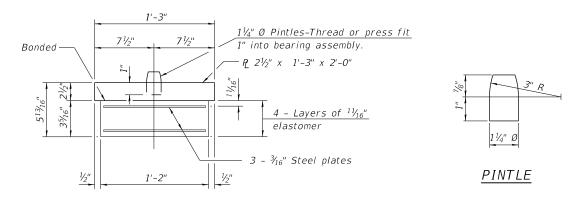
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

BEARIN	IG DETAILS 1
SN	045-3401
SHEET NO	24 OF 40 SHEETS

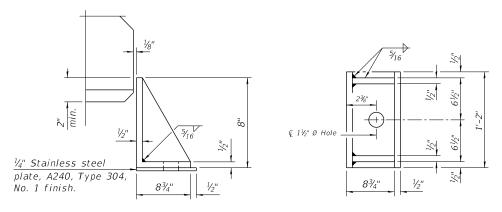
F.A.S. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
1107	1107 15-00277-01-BR		KANE	542	229
			CONTRACT	NO. 6	1H9

├-- Ç Beam PLAN OF BOTTOM PLATE

#### TYPE I ELASTOMERIC EXP. BRG.



#### BEARING ASSEMBLY



#### SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

#### lotes:

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554. Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.

See sheet 22 of 40 for additional details of embedded plate. Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

All (embedded and separate) bearing plates, side retainers, anchor bolts, nuts, washers and pintiles shall be galvanized according to AASHTO M111 or M232 as applicable.

The structural steel plates and pintiles of the Bearing Assembly and Fixed Bearing shall conform to the requirements of AASHTO M270 Grade 50.

#### BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	8
Anchor Bolts, 11/4"	Each	16

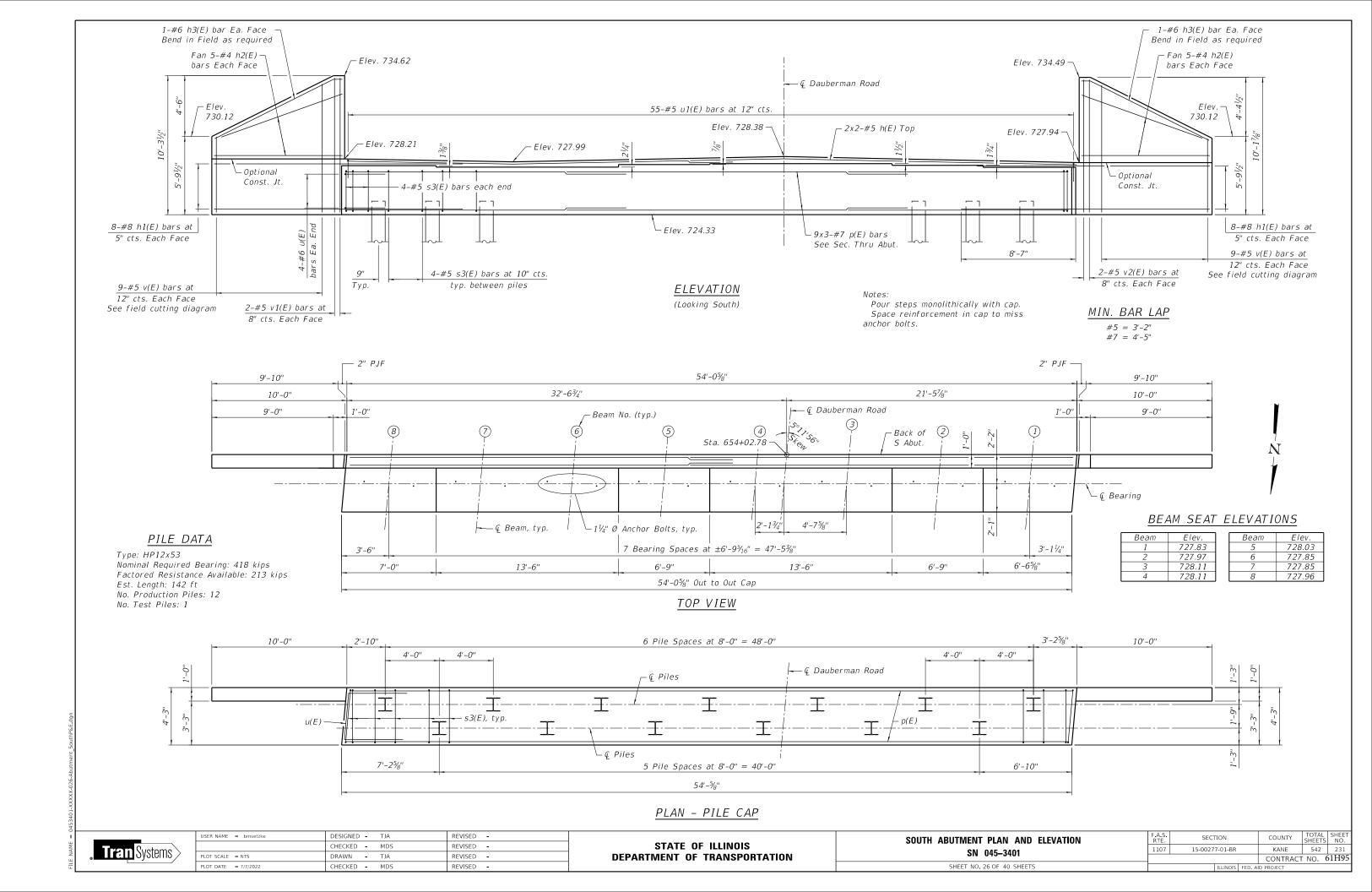


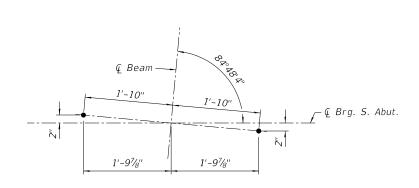
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	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - GDF	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

BEARING DETAILS 2 SN 045-3401	
SHEET NO. 25 OF 40 SHEETS	_

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107	15-0027	7-01-BR		KANE	542	23
			CONTRACT	NO. 6	1H9	
		ILLINOIS	FED. A	ID PROJECT		

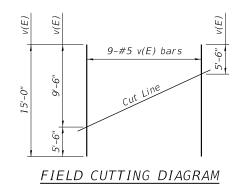
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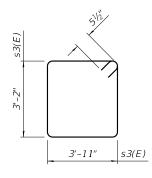


ANCHOR BOLT DETAILS

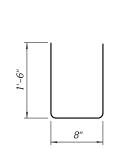
SECTION THRU ABUT.



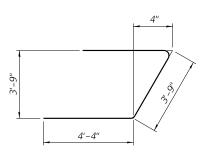
Order v(E) full length. Cut as shown and use remainder of bars in opposite face.



<u>BAR s3(E)</u>



*BAR u1(E)* 



BAR u(E)

#### BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	4	#5	28'-5"	
h1(E)	32	#8	18'-5"	
h2(E)	20	#4	9'-6"	
h3(E)	4	#6	10'-6"	
p(E)	27	#7	20'-10"	
s3(E)	56	#5	14'-11"	
u(E)	8	#6	13'-0"	
u1(E)	55	#5	3'-8"	
v(E)	18	#5	15'-0"	
v1(E)	4	#5	9'-11''	
v2(E)	4	#5	9'-9"	
Concre	te Stru	ctures	Cu. Yd.	37.2
	rcement Coated	Bars,	Pound	4,640
	hing St HP12x5.		Foot	1,704
Driving	Piles		Foot	1,704
Test P. HP12x.	ile Stee 53	e/	Each	1
Pile Sh	nes		Each	1.3

#### Notes:

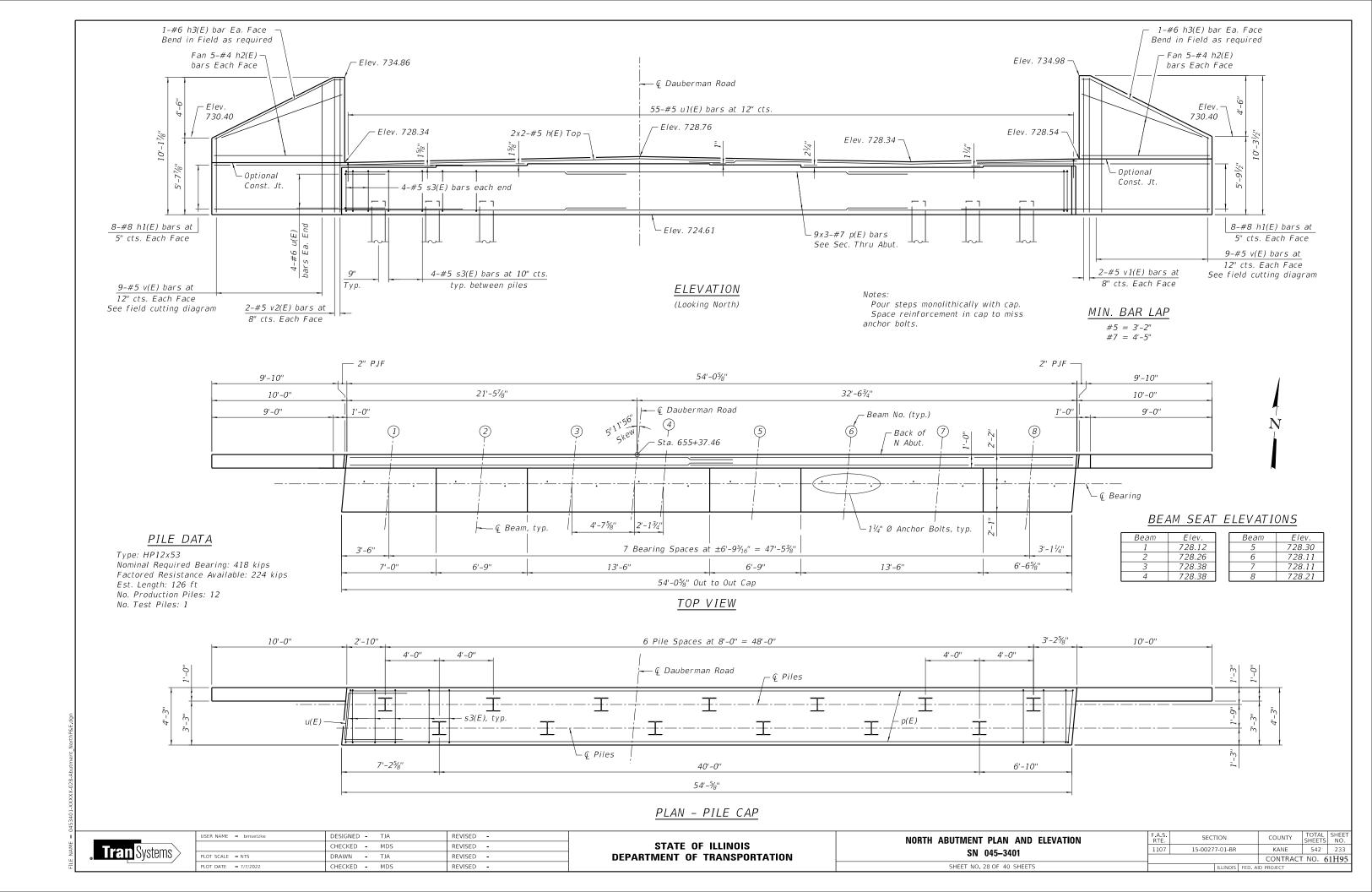
Pour steps monolithically with cap.
Headed bars shall conform to ASTM A970 with
threaded attachment; Class HA; and reinforcement
bars conforming to ASTM A706. Cost included with
Reinforcement Bars. Epoxy Coated.

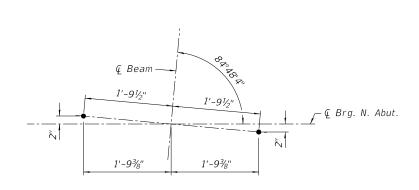
Reinforcement Bars, Epoxy Coated. For details of piles see Sheet 30 of 40.

**Tran** Systems

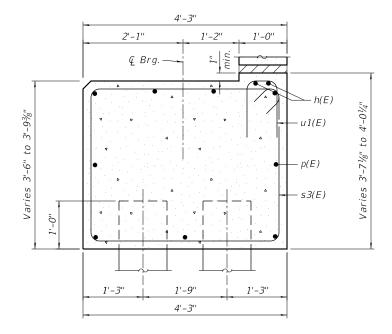
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PLOT SCALE = NTS	DRAWN - TJA	REVISED -	
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -	

SOUTH ABUTMENT DETAILS		SECTION		COUNTY	TOTAL SHEETS	SHE
SN 045-3401	1107	1107 15-00277-01-BR		KANE	542	23
				CONTRACT	NO. 6	1H9
CHEET NO 37 OF 40 CHEETS		71 1 1 1 1 1	VC 550 A	ID DROJECT		

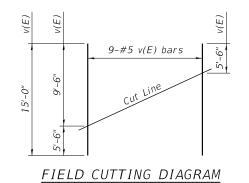




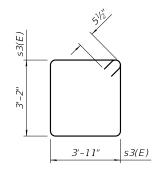
ANCHOR BOLT DETAILS



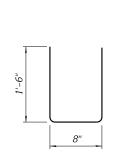
SECTION THRU ABUT.



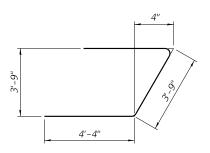
Order v(E) full length. Cut as shown and use remainder of bars in opposite face.



BAR s3(E)



BAR u1(E)



BAR u(E)

#### BILL OF MATERIAL

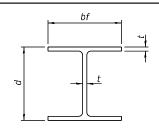
Bar	No.	Size	Length	Shape
h(E)	4	#5	28'-5"	
h1(E)	32	#8	18'-5"	
h2(E)	20	#4	9'-6"	
h3(E)	4	#6	10'-6"	
p(E)	27	#7	20'-10"	
s3(E)	56	#5	14'-11"	[]
u(E)	8	#6	13'-0"	
u1(E)	55	#5	3'-8"	
v(E)	18	#5	15'-0"	
v1(E)	4	#5	9'-11"	
v2(E)	4	#5	9'-9"	
Concre	te Stru	ctures	Cu. Yd.	37.2
	rcement Coated		Pound	4,640
	shing Steel HP12x53		Foot	1,512
Driving	g Piles		Foot	1,512
Test P HP12x	ile Stee 53	el	Each	1
Pile Si	hoes		Each	13

Pour steps monolithically with cap.
Headed bars shall conform to ASTM A970 with
threaded attachment; Class HA; and reinforcement
bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated. For details of piles see Sheet 30 of 40.



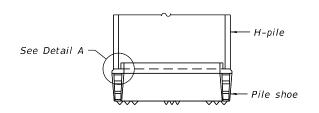
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	PLOT DATE = 7/7/2022	CHECKED -	MDS	REVISED	=
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NORTH ABUTMENT DETAILS		SEC	ΓΙΟΝ		COUNTY	TOTAL SHEETS	
SN 045-3401	1107	15-00277-01-BR			KANE	542	234
					CONTRACT	NO.	61H9
SHEET NO. 29 OF 40 SHEETS			ILLINOIS	FED. All	D PROJECT		

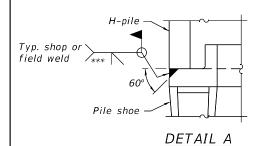


#### STEEL PILE TABLE

Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	141/4"	14 <sup>7</sup> /8"	<sup>13</sup> / <sub>16</sub> "	30"
x102	14"	14¾"	11/ <sub>16</sub> "	30"
x89	137/8"	1 43/4"	5/8"	30"
x73	13%"	14%"	1/2"	30"
HP 12x84	121/4"	121/4"	<sup>1</sup> 1/ <sub>16</sub> "	24"
x74	121/8"	121/4"	5/8"	24"
x63	12"	12½"	1/2"	24"
x53	113/4"	12"	<sup>7</sup> / <sub>16</sub> "	24"
HP 10x57	10"	101/4"	%16"	24"
x42	9¾"	101/8"	<sup>7</sup> /16"	24"
HP 8x36	8"	81/8"	<sup>7</sup> /16"	18"



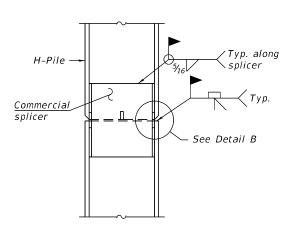
#### ELEVATION

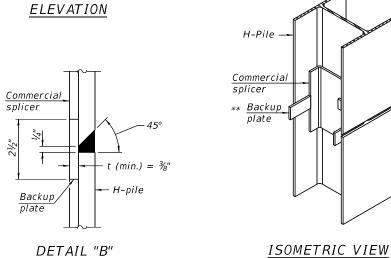


#### SHOE ATTACHMENT

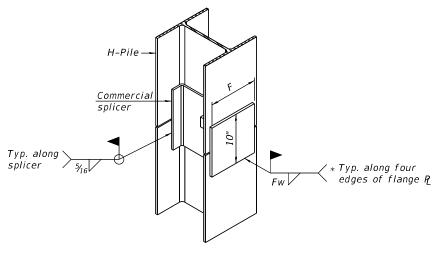
Note

The steel H-piles shall be according to AASHTO M270 Grade 50.





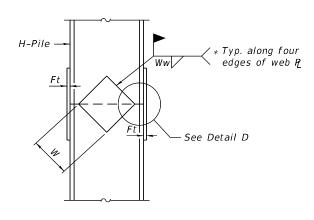
#### WELDED COMMERCIAL SPLICE

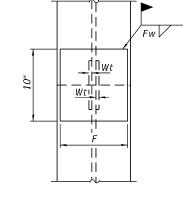


#### ISOMETRIC VIEW

#### WELDED COMMERCIAL SPLICE ALTERNATE

- $_*$  Interrupt welds  $^{1}\!\!\!/_{\!\!4}^{\!\!\!\!4}$  from end of web and/or each flange.
- \*\* Remove portions of backup plates that extend outside the flanges.
- \*\*\* Weld size per pile shoe manufacturer ( $\frac{5}{16}$ " min.).



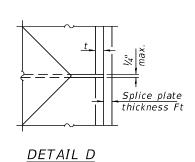


∕∗Typ. along four k edges of flange ₽

Wt

#### ELEVATION

END VIEW



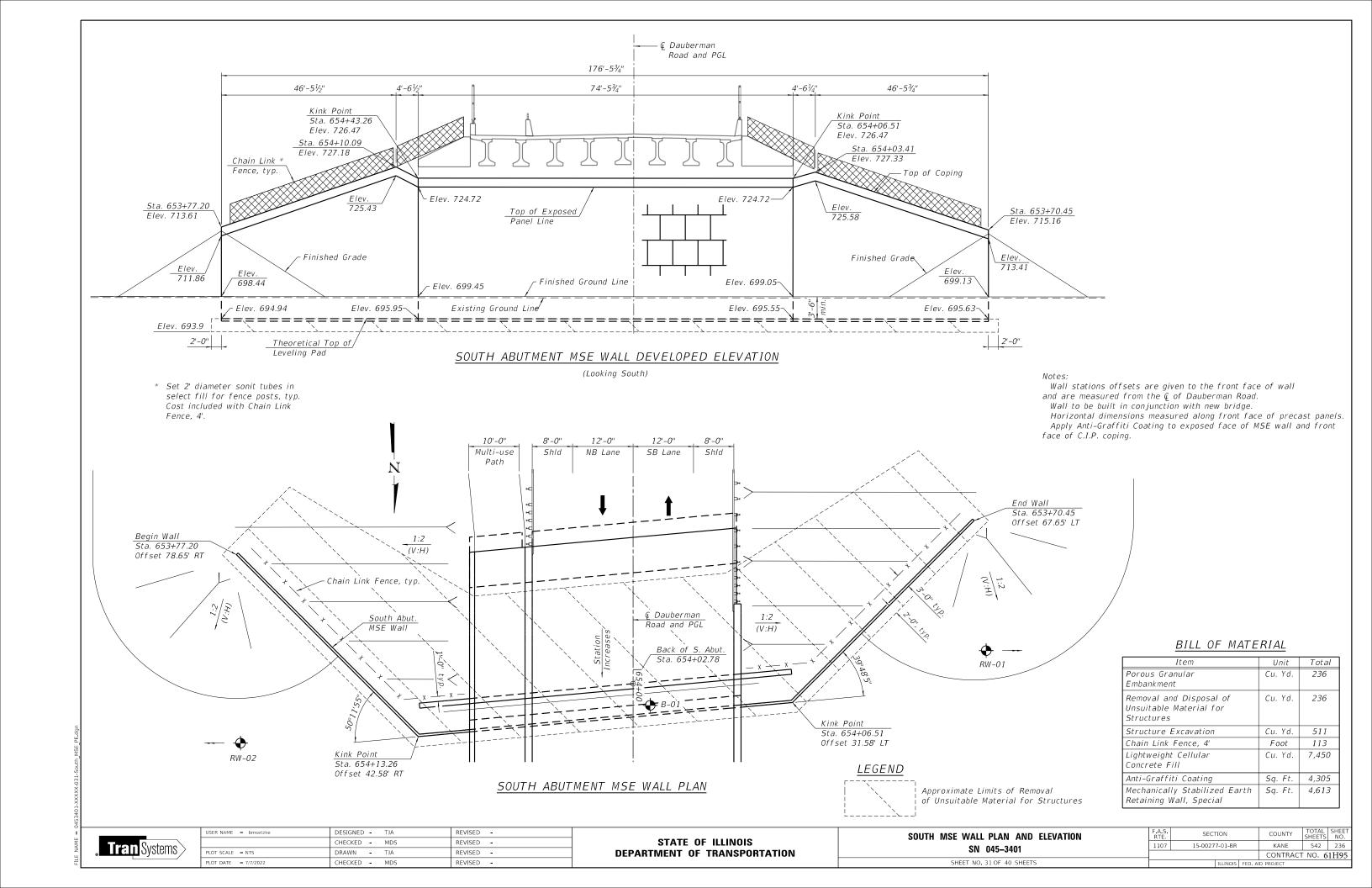
HP 14x117	12½"	1"	7/8"	7¾"	<i>5</i> ⁄8"	1/2"
x 102	12½"	7/8"	3/4"	73/4"	5/8"	1/2"
x89	12½"	3/4"	11/ <sub>16</sub> "	73/4"	5/8"	1/2"
x73	121/2"	5/8"	%16"	73/4"	5/8"	1/2"
HP 12x84	10"	<sup>7</sup> /8"	11/ <sub>16</sub> "	6½"	5/8"	1/2"
x74	10"	7/8"	11/16"	6½"	5/8"	1/2"
x63	10"	5/8"	1/2"	6½"	1/2"	3/8"
x53	10"	5/8"	1/2"	6½"	1/2"	3/8"
HP 10x57	8"	3/4"	%16"	51/4"	1/2"	3/8"
x42	8"	5/8"	%16"	51/4"	1/2"	3/8"
HP 8x36	7"	5/,"	7/16"	41/4"	1/5"	3/2"

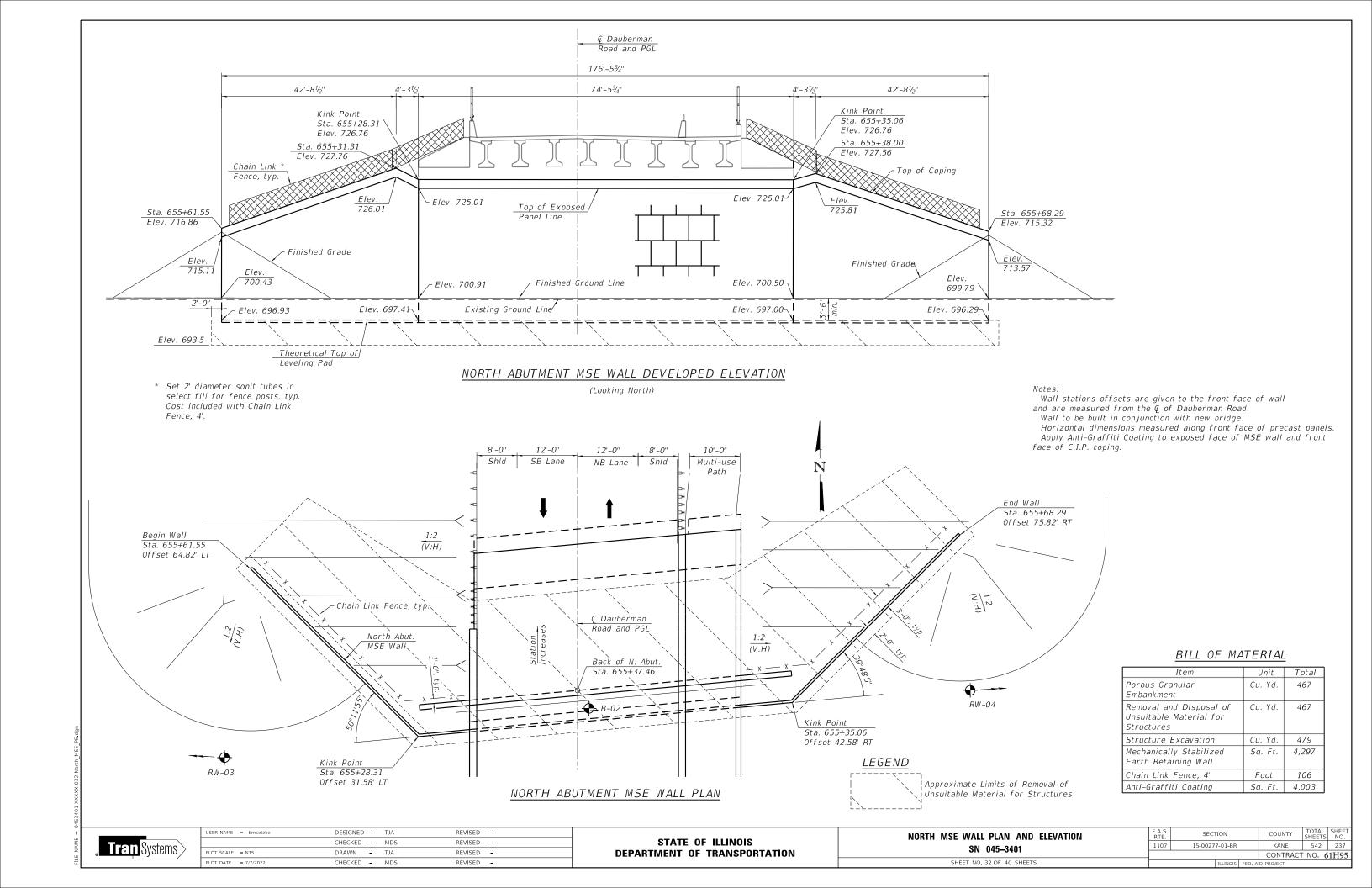
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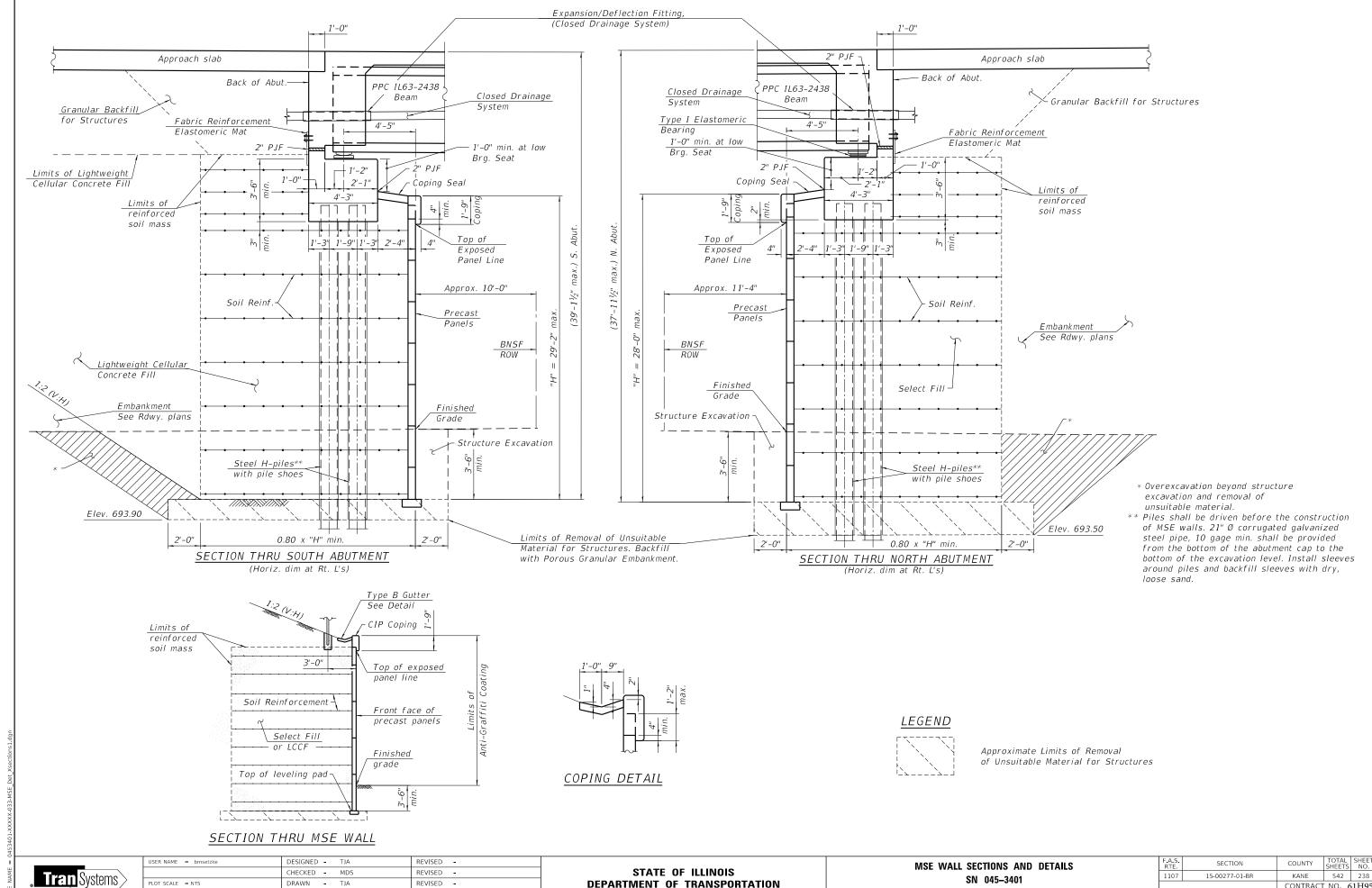


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	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - GDF	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

HP PILE DETAILS SN 045-3401	F.A.S. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
	1107 15-00277-01-BR		KANE	542	235		
					CONTRAC	T NO. 6	1H95
SHEET NO 30 OF 40 SHEETS			THIMOIS	EED A	ID DDOIECT		







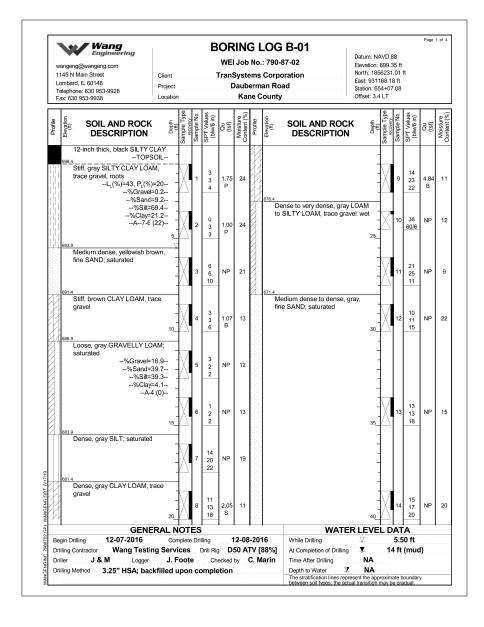
CONTRACT NO. 61H95

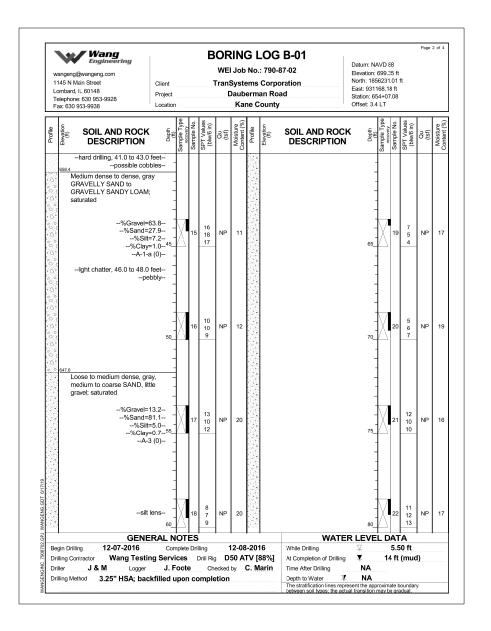
SHEET NO. 33 OF 40 SHEETS

CHECKED - MDS

REVISED -

PLOT DATE = 7/7/2022



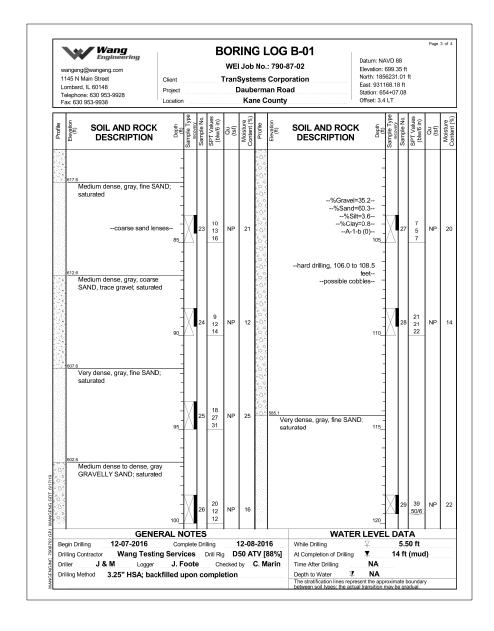


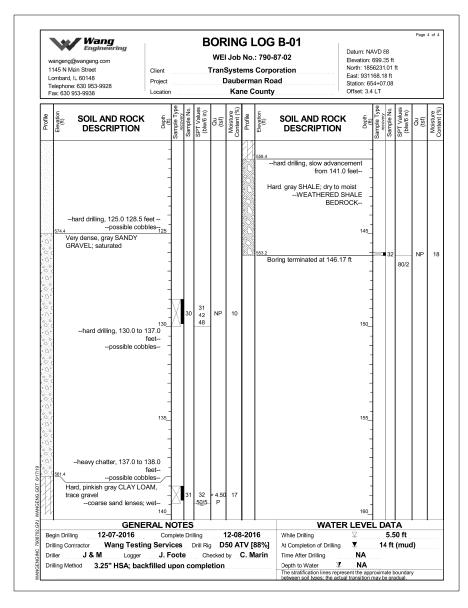


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PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

STATE OF ILLINOIS					
DEPARTMENT	0F	TRANSPORTATION			

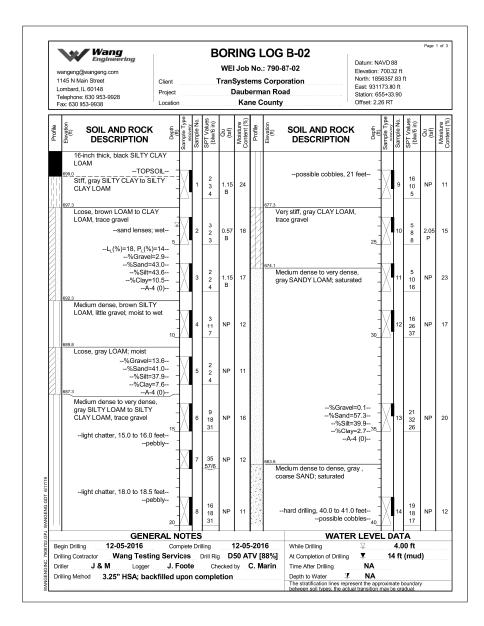
BORING LOGS 1	F.A.S. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.	
SN 045-3401	1107	15-0027	7-01-BR		KANE	542	239
311 043-3401					CONTRAC	T NO. 6	1H9
SHEET NO. 34 OF 40 SHEETS			ILLINOIS	FED. Al	ID PROJECT		

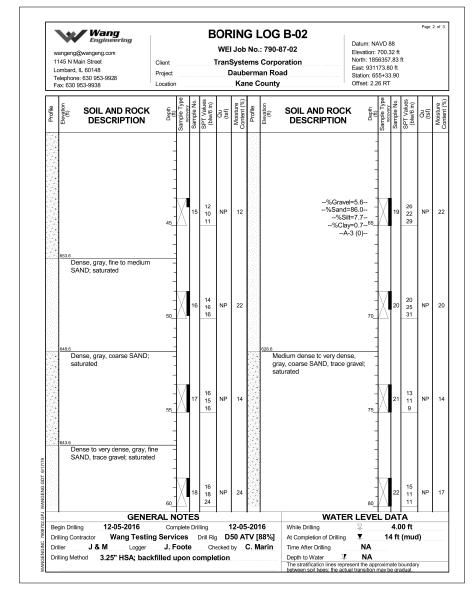


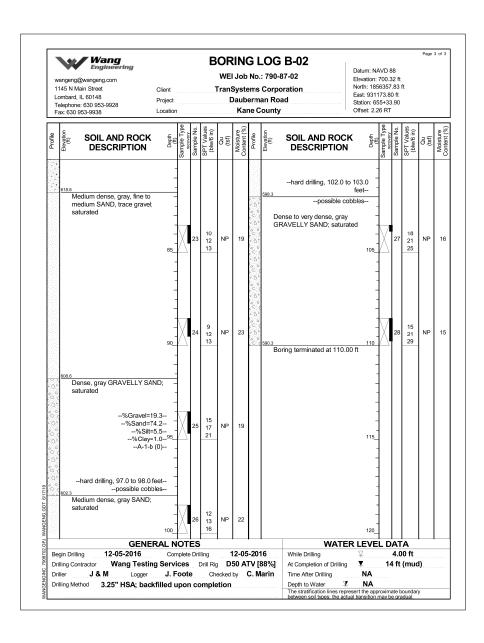




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PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -





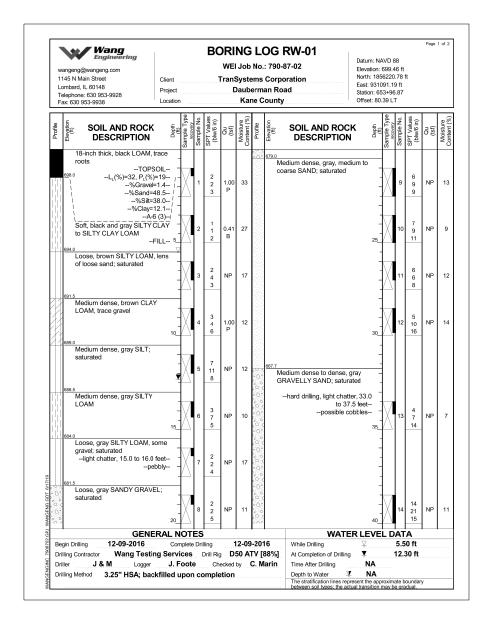


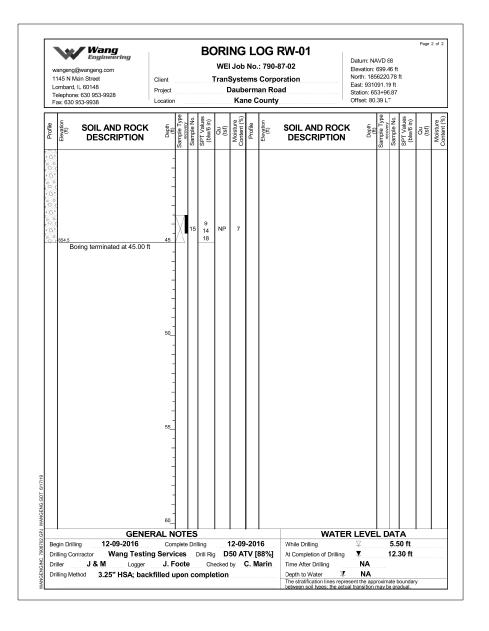


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	CHECKED - MDS	REVISED -
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PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

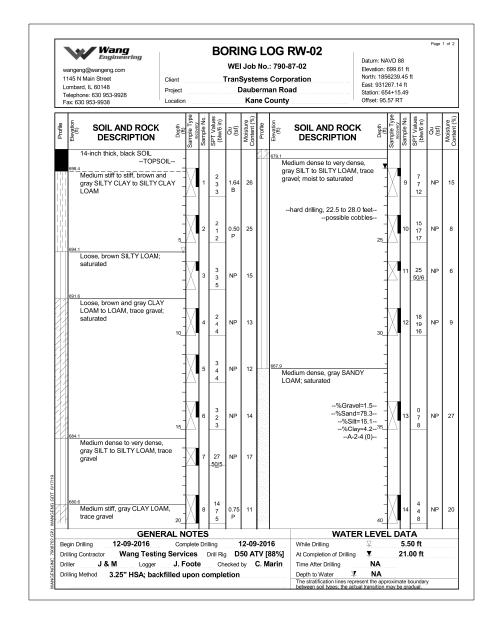
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314 043-3401					CONTRACT	NO. 6	1H95
HEET NO. 36 OF 40 SHEETS		ILL	LINOIS F	ED AI	D PROJECT		

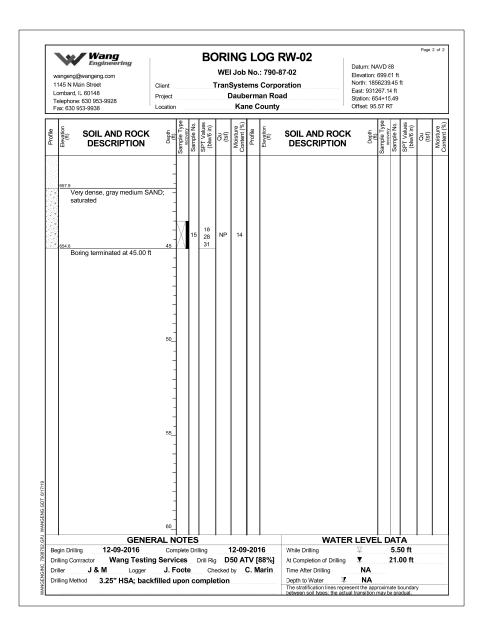






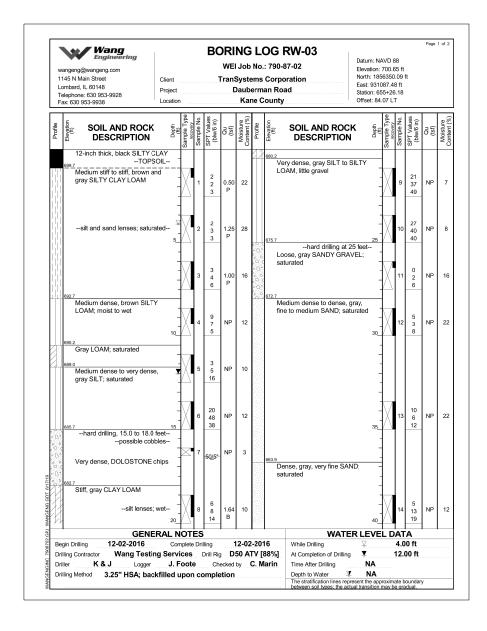
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	CHECKED - MDS	REVISED -
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PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

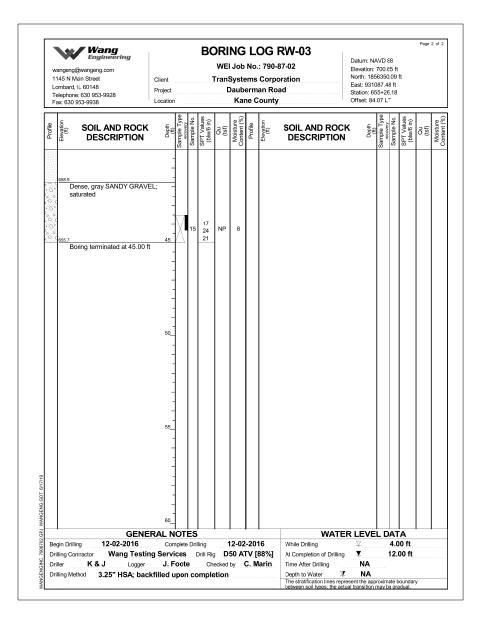






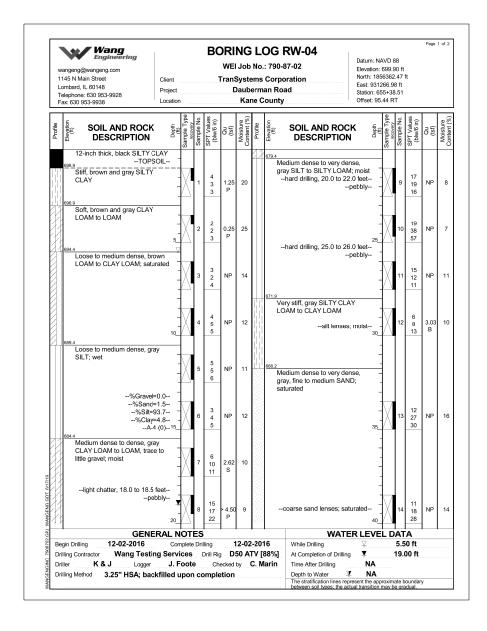
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PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -

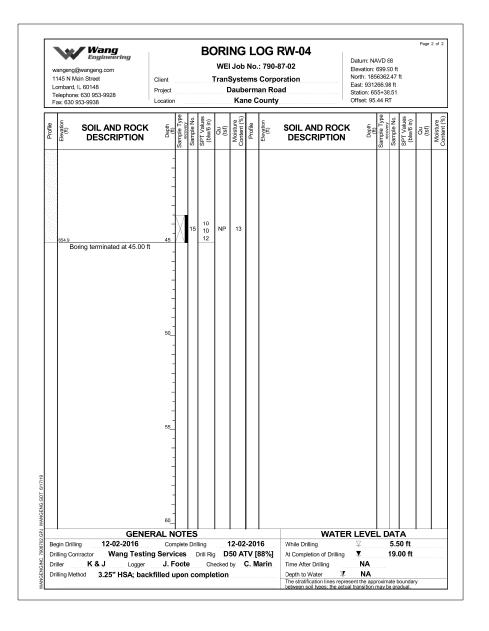






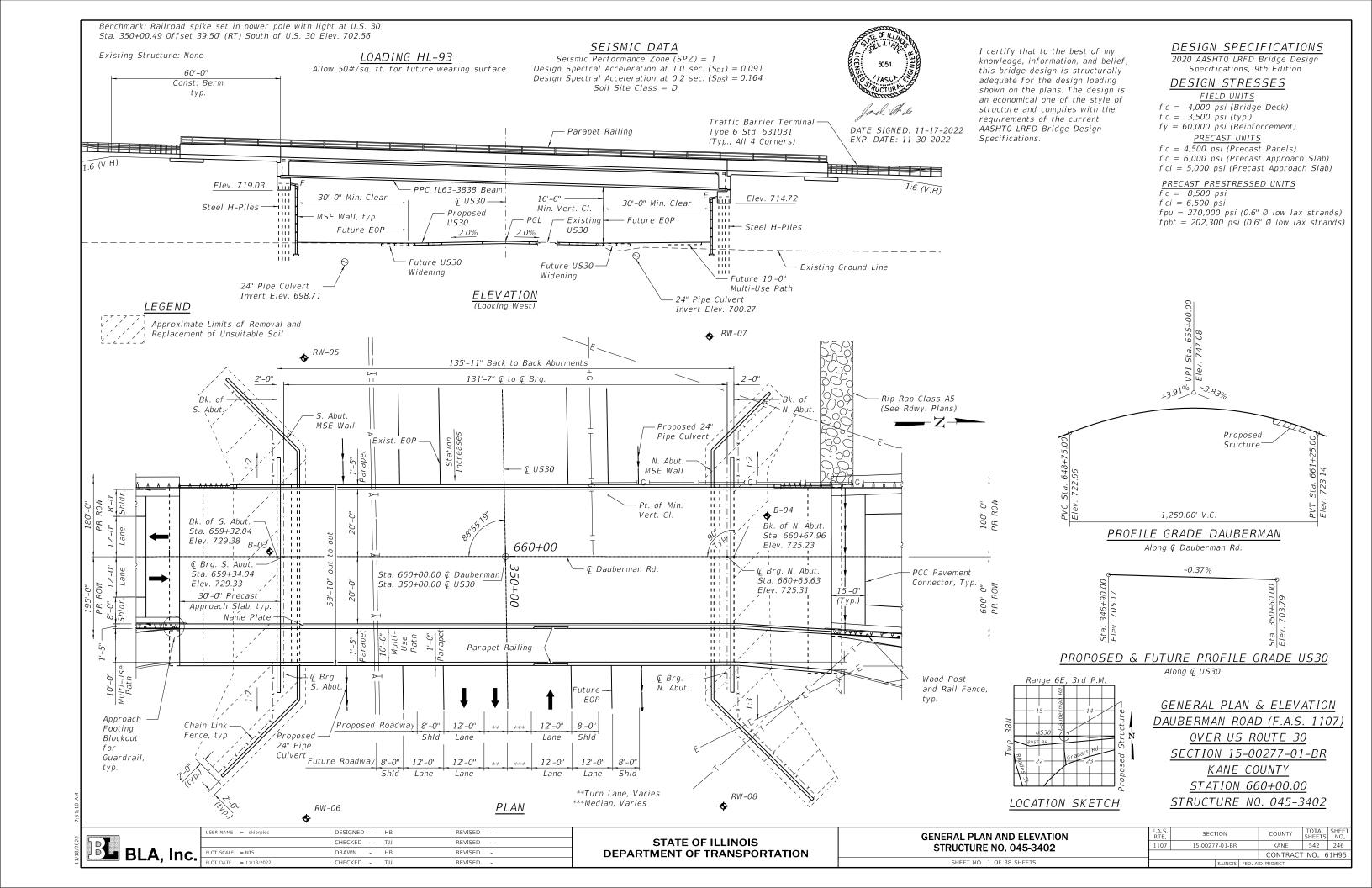
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PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -







USER NAME = bmsetzke	DESIGNED - TJA	REVISED -
	CHECKED - MDS	REVISED -
PLOT SCALE = NTS	DRAWN - TJA	REVISED -
PLOT DATE = 7/7/2022	CHECKED - MDS	REVISED -



#### GENERAL NOTES

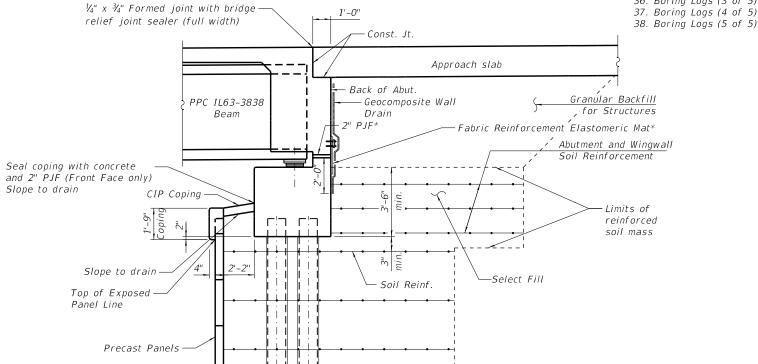
Reinforcement bars designated (E) shall be epoxy coated.

Slipforming of the parapets is not allowed.

The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.

- 1. Locate existing utilities that are to remain. Contractor to coordinate any required improvements to or removals of existing utilities with utility owner(s). See Civil Plans.
- 2. Complete the Removal and Disposal of Unsuitable Materials and replace with Porous Granular Embankment.
- 3. Drive the Piles.
- 4. Construct the abutments and MSE walls.
- 5. Place the Precast Prestressed Concrete Beams on the abutments.
- 6. Construct the bridge deck, parapets, and railings.

\*See Diaphragm Details sht. 11 of 38.



#### SECTION THRU ABUTMENT

(North Abutment shown, South Abutment similar) (See sheets 30 thru 32 of 38 for MSE Wall details)

#### INDEX OF SHEETS

- 1. General Plan & Elevation
- 2. General Data
- 3. Top of Slab Elevations I
- 4. Top of Slab Elevations II
- 5. Top of South Approach Slab Elevations
- 6. Top of North Approach Slab Elevations
- 7. Superstructure Plan
- 8. Superstructure Cross Section
- 9. Parapet and Path Cross Sections
- 10. Parapet Elevations
- 11. Diaphragm Details
- 12. Superstructure Details
- 13. South Precast Bridge Approach Slab
- 14. North Precast Bridge Approach Slab
- 15. Precast Bridge Approach Slab Details I
- 16. Precast Bridge Approach Slab Details II 17. Precast Bridge Approach Slab Details III
- 18. Bridge Approach Slab Parapet Elevations
- 19. Preformed Joint Strip Seal and Wood Railing Details
- 20. Parapet Railing Details
- 21. Framing Plan
  22. IL63 PPC Beam Details I
- 23. IL63 PPC Beam Details II
- 24. Fixed Bearing Details
- 25. Elastomeric Bearing Details
- 26. South Abutment Plan and Elevation
- 27. South Abutment Details
- 28. North Abutment Plan and Elevation
- 29. North Abutment Details
- 30. South Abutment MSE Wall Plan
- 31. North Abutment MSE Wall Plan
- 32. MSE Wall Sections and Details
- 33. HP Pile Details
- 34. Boring Logs (1 of 5)
- 35. Boring Logs (2 of 5)
- 36. Boring Logs (3 of 5)
- 37. Boring Logs (4 of 5)

#### TOTAL BILL OF MATERIAL

7AL 087 38 087 6.7 7.0 24 32 140 84 080
38 087 6.7 7.0 24 32 140 84
087 6.7 7.0 24 32 140 84
6.7 7.0 24 32 140 84
7.0 24 32 140 84 080
24 32 140 84 080
32 140 84 080
140 84 080
84 180
080
180
2
30
1
0.5
7
28
05
38
32
09
06
71
251
67
1 (7)

BUILT 202 BY KANE COUNTY SEC. 15-00277-01-BR F.A.S. RT. 1107 STA. 660+00.00 STR. NO. 045-3402 LOADING HL-93

> NAME PLATE See Std. 515001



	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
•	PLOT SCALE = NTS	DRAWN - HB	REVISED -
-	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

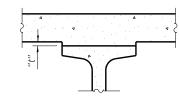
F.A.S. RTE.	SECTION			COUNTY	TOTAL SHEETS	SHEE
1107	15-00277-01-BR			KANE	542	24
				CONTRACT	NO. 6	1H9!
	TILINOIS	EED	ΔΙ	ID PROJECT		

#### BEAM 1 Theoretical Grade heoreticai Elevations Off set Location Station Grade Adjusted For Dead Elevations Load Deflection Back of South Abut. 659+32.04 -18.50 729.01 729.01 © Brg. South Abut. 659+34.04 728.96 728.96 728.73 659+44.04 -18.50 728.69 659+54.04 -18.50 728.42 728.49 659+64.04 -18.50 728.14 728.25 659+74.04 -18.50 727.85 727.99 727.71 659+84.04 -18.50 7*2*7.56 659+94.04 -18.50 727.26 727.42 660+04.04 -18.50 726.95 727.12 726.80 660+14.04 -18.50 726.64 660+24.04 -18.50 726.32 726.47 660+34.04 -18.50 726.00 726.12 660+44.04 -18.50 725.67 725.75 660+54.04 -18.50 725.33 725.38 ⊕ Brg. North Abut. 660+65.63 724.94 724.94 -18.50 Back of North Abut. 660+67.96 -18.50 724.86 724.86

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Deac Load Deflection
Back of South Abut.	659+32.04	-10.50	729.17	729.17
û Brg. South Abut.	659+34.04	-10.50	729.12	729.12
A	659+44.04	-10.50	728.85	728.89
В	659+54.04	-10.50	728.58	728.66
С	659+64.04	-10.50	728.30	728.42
D	659+74.04	-10.50	728.01	728.16
E	659+84.04	-10.50	727.72	727.89
F	659+94.04	-10.50	727.42	727.60
G	660+04.04	-10.50	727.11	727.30
Н	660+14.04	-10.50	726.80	726.98
I	660+24.04	-10.50	726.48	726.64
J	660+34.04	-10.50	726.16	726.29
K	660+44.04	-10.50	725.83	725.92
L	660+54.04	-10.50	725.49	725.55
₡ Brg. North Abut.	660+65.63	-10.50	725.10	725.10
Back of North Abut.	660+67.96	-10.50	725.02	725.02

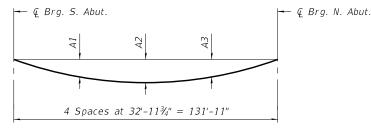
(I)

BEAM 2



To determine "t": After all precast prestressed beams have been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflections" shown below, minus slab thickness, equals the fillet heights "t" above top flanges of beams.

#### FILLET HEIGHTS

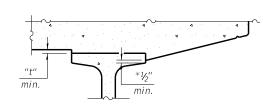


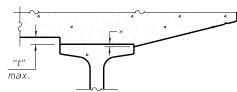
#### DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown in these plans.

Beam No.	A1	A2	A3
1 & 7	13/8"	2"	13/8"
2 thru 6	15/8"	21/4"	15/8"





\* Variable (not less than 1/3"

At Minimum Fillet

€ @ Brg. North Abut.

- Back of North Abut.

← Dauberman Road

At Maximum Fillet

## <u>PLAN</u>

131'-7" € Brg. to € Brg.

135'-11" Back to Back Abutments

12 sp. @ 10'-0" = 120'-0"

# NOTE:

Top of deck elevations shown on this sheet do not include adjustments for the  $\frac{3}{6}$ " polymer overlay.



← Brg. South Abut.

= 48'-0"

Beam Spaces at 8'-0"

Тур.

**4**)—

(5)-

2'-0"

Back of South Abut. --

USER NAME = dkierpiec	DESIGNED - HB	REVISED -
	CHECKED - TJJ	REVISED -
PLOT SCALE = NTS	DRAWN - HB	REVISED -
PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -
		CHECKED - TJJ

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  TOP OF SLAB ELEVATIONS I STRUCTURE NO. 045-3402 SHEET NO. 3 OF 38 SHEETS

A.S. TE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
107	15-00277-01-BR	KANE	542	248	
		CONTRACT	NO. 6	1H95	
	·				

11'-7"

2'-4"

# BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Abut.	659+32.04	-2.50	729.33	729.33
€ Brg. South Abut.	659+34.04	-2.50	729.28	729.28
A	659+44.04	-2.50	729.01	729.05
В	659+54.04	-2.50	728.74	728.82
С	659+64.04	-2.50	728.46	728.58
D	659+74.04	-2.50	728.17	728.32
Ε	659+84.04	-2.50	727.88	728.05
F	659+94.04	-2.50	727.58	727.76
G	660+04.04	-2.50	727.27	727.46
Н	660+14.04	-2.50	726.96	727.14
I	660+24.04	-2.50	726.64	726.80
J	660+34.04	-2.50	726.32	726.45
K	660+44.04	-2.50	725.99	726.08
L	660+54.04	-2.50	725.65	725.71
€ Brg. North Abut.	660+65.63	-2.50	725.26	725.26
Back of North Abut.	660+67.96	-2.50	725.18	725.18

# © DAUBERMAN ROAD & PGL

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Abut.	659+32.04	0.00	729.38	729.38
₢ Brg. South Abut.	659+34.04	0.00	729.33	729.33
Α	659+44.04	0.00	729.06	729.10
В	659+54.04	0.00	728.79	728.87
С	659+64.04	0.00	728.51	728.63
D	659+74.04	0.00	728.22	728.37
Ε	659+84.04	0.00	727.93	728.10
F	659+94.04	0.00	727.63	727.81
G	660+04.04	0.00	727.32	727.51
Н	660+14.04	0.00	727.01	727.19
I	660+24.04	0.00	726.69	726.85
J	660+34.04	0.00	726.37	726.50
K	660+44.04	0.00	726.04	726.13
L	660+54.04	0.00	725.70	725.76
₡ Brg. North Abut.	660+65.63	0.00	725.31	725.31
Back of North Abut.	660+67.96	0.00	725.23	725.23

# <u>BEAM 4</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Abut.	659+32.04	5.50	729.27	729.27
€ Brg. South Abut.	659+34.04	5.50	729.22	729.22
A	659+44.04	5.50	728.95	728.99
В	659+54.04	5.50	728.68	728.76
С	659+64.04	5.50	728.40	728.52
D	659+74.04	5.50	728.11	728.26
Ε	659+84.04	5.50	727.82	727.99
F	659+94.04	5.50	727.52	727.70
G	660+04.04	5.50	727.21	727.40
Н	660+14.04	5.50	726.90	727.08
I	660+24.04	5.50	726.58	726.74
J	660+34.04	5.50	726.26	726.39
K	660+44.04	5.50	725.93	726.02
L	660+54.04	5.50	725.59	725.65
€ Brg. North Abut.	660+65.63	5.50	725.20	725.20
Back of North Abut.	660+67.96	5.50	725.12	725.12

# BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Abut.	659+32.04	13.50	729.11	729.11
₡ Brg. South Abut.	659+34.04	13.50	729.06	729.06
A B C D E F G H I J K	659+44.04 659+54.04 659+64.04 659+74.04 659+84.04 659+94.04 660+04.04 660+24.04 660+34.04	13.50 13.50 13.50 13.50 13.50 13.50 13.50 13.50 13.50	728.79 728.52 728.24 727.95 727.66 727.36 727.05 726.74 726.42 726.10 725.77	728.83 728.60 728.36 728.10 727.83 727.54 727.24 726.92 726.58 726.23 725.86
Ĺ	660+54.04	13.50	725.43	725.49
Ç Brg. North Abut.	660+65.63	13.50	725.04	725.04
Back of North Abut.	660+67.96	13.50	724.96	724.96

# BEAM 6

<u> </u>					
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
Back of South Abut.	659+32.04	21.50	729.00	729.00	
Q Brg. South Abut.	659+34.04	21.50	728.95	728.95	
A B C D E F G	659+44.04 659+54.04 659+64.04 659+74.04 659+84.04 659+94.04	21.50 21.50 21.50 21.50 21.50 21.50 21.50	728.68 728.41 728.13 727.84 727.55 727.25 726.94	728.73 728.49 728.25 728.00 727.73 727.44 727.13	
H I J K L	660+14.04 660+24.04 660+34.04 660+44.04 660+54.04	21.50 21.50 21.50 21.50 21.50	726.63 726.32 725.99 725.66 725.33	726.81 726.48 726.12 725.76 725.38	
© Brg. North Abut. Back of North Abut.	660+65.63 660+67.96	21.50 21.50	724.93 724.85	724.93 724.85	

# BEAM 7

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
Back of South Abut.	659+32.04	29.50	729.12	729.12	
∉ Brg. South Abut.	659+34.04	29.50	729.07	729.07	
A B C D E F G H I J K L	659+44.04 659+54.04 659+64.04 659+74.04 659+84.04 659+94.04 660+04.04 660+14.04 660+24.04 660+34.04 660+44.04	29.50 29.50 29.50 29.50 29.50 29.50 29.50 29.50 29.50 29.50 29.50	728.80 728.53 728.25 727.96 727.67 727.37 727.06 726.75 726.44 726.11 725.78 725.45	728.84 728.60 728.36 728.10 727.83 727.54 727.23 726.91 726.58 726.23 725.87 725.49	
& Brg. North Abut. Back of North Abut.	660+65.63 660+67.96	29.50 29.50	725.05 724.97	725.05 724.97	

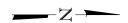
# NOTE:

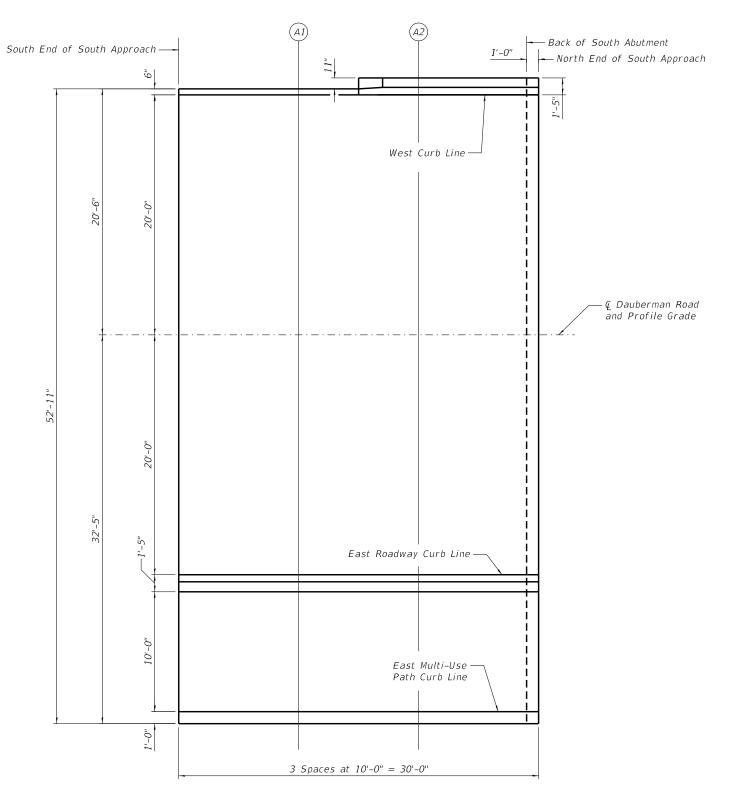
Top of deck elevations shown on this sheet do not include adjustments for the  $\frac{3}{6}$ " polymer overlay.



	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

A.S. ITE	SECT	TON		COUNTY	TOTAL SHEETS	SHEET NO.
107	15-0027	7-01-BR		KANE	542	249
CONTRACT NO. 61H9					1H95	
	[					





#### PLAN

#### NOTE:

Approach slab elevations shown on this sheet do not include adjustments for the  $\frac{3}{6}$ " polymer overlay.

# WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South End of South Appr.	659+03.04	-20.00	729.72
A1 A2	659+13.04 659+23.04	-20.00 -20.00	729.47 729.21
North End of South Appr.	659+33.04	-20.00	728.95

# @ DAUBERMAN ROAD & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations
South End of South Appr.	659+03.04	0.00	730.12
A1 A2	659+13.04 659+23.04	0.00 0.00	729.87 729.61
North End of South Appr.	659+33.04	0.00	729.35

# EAST ROADWAY CURB LINE

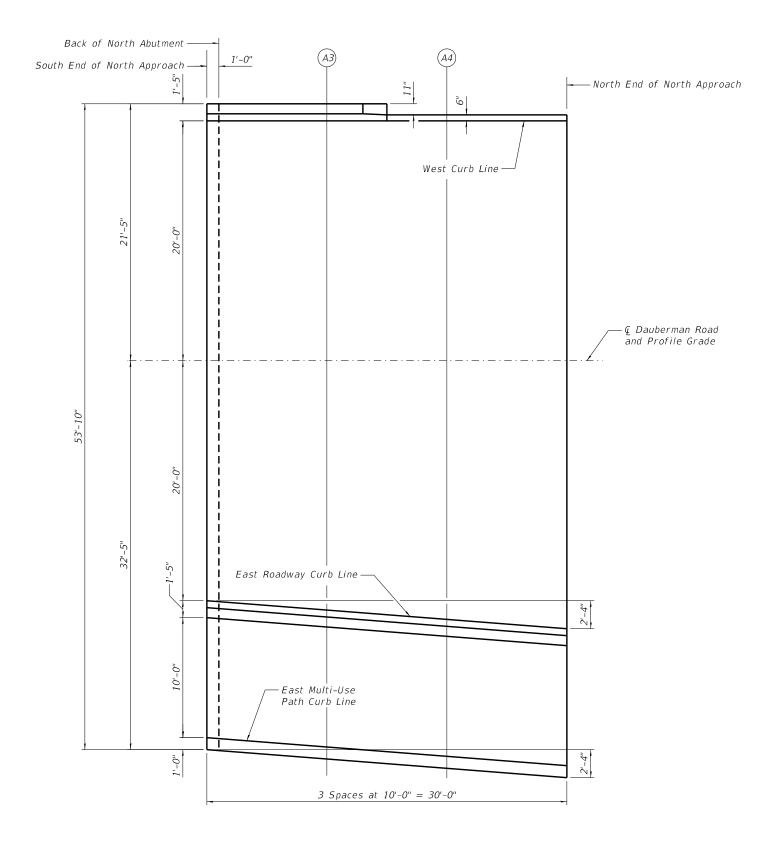
Location	Station	Offset	Theoretical Grade Elevations
South End of South Appr.	659+03.04	20.00	729.72
A1 A2	659+13.04 659+23.04	20.00 20.00	729.47 729.21
North End of South Appr.	659+33.04	20.00	728.95

# EAST MULTI-USE PATH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South End of South Appr.	659+03.04	31.42	729.89
A1 A2	659+13.04 659+23.04	31.42 31.42	729.64 729.39
North End of South Appr.	659+33.04	31.42	729.12

	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
٥.	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

A.S. TE.	SECT	ION			COUNTY	TOTAL SHEETS	SHEET NO.
107 15-00277-01-BR		KANE	542	250			
					CONTRACT	NO. 6	1H95
		TELIMOIS	EED	ΛΙ	ID PROJECT		



# <u>PLAN</u>

# NOTE:

Approach slab elevations shown on this sheet do not include adjustments for the  $\frac{3}{6}$ " polymer overlay.

# WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South End of North Appr.	660+66.96	-20.00	724.86
A3 A4	660+76.96 660+86.96	-20.00 -20.00	724.51 724.15
North End of North Appr.	660+96.96	-20.00	723.79

# 

Location	Station	Offset	Theoretical Grade Elevations
South End of North Appr.	660+66.96	0.00	725.26
A3 A4	660+76.96 660+86.96	0.00 0.00	724.91 724.55
North End of North Appr.	660+96.96	0.00	724.19

## EAST ROADWAY CURB LINE

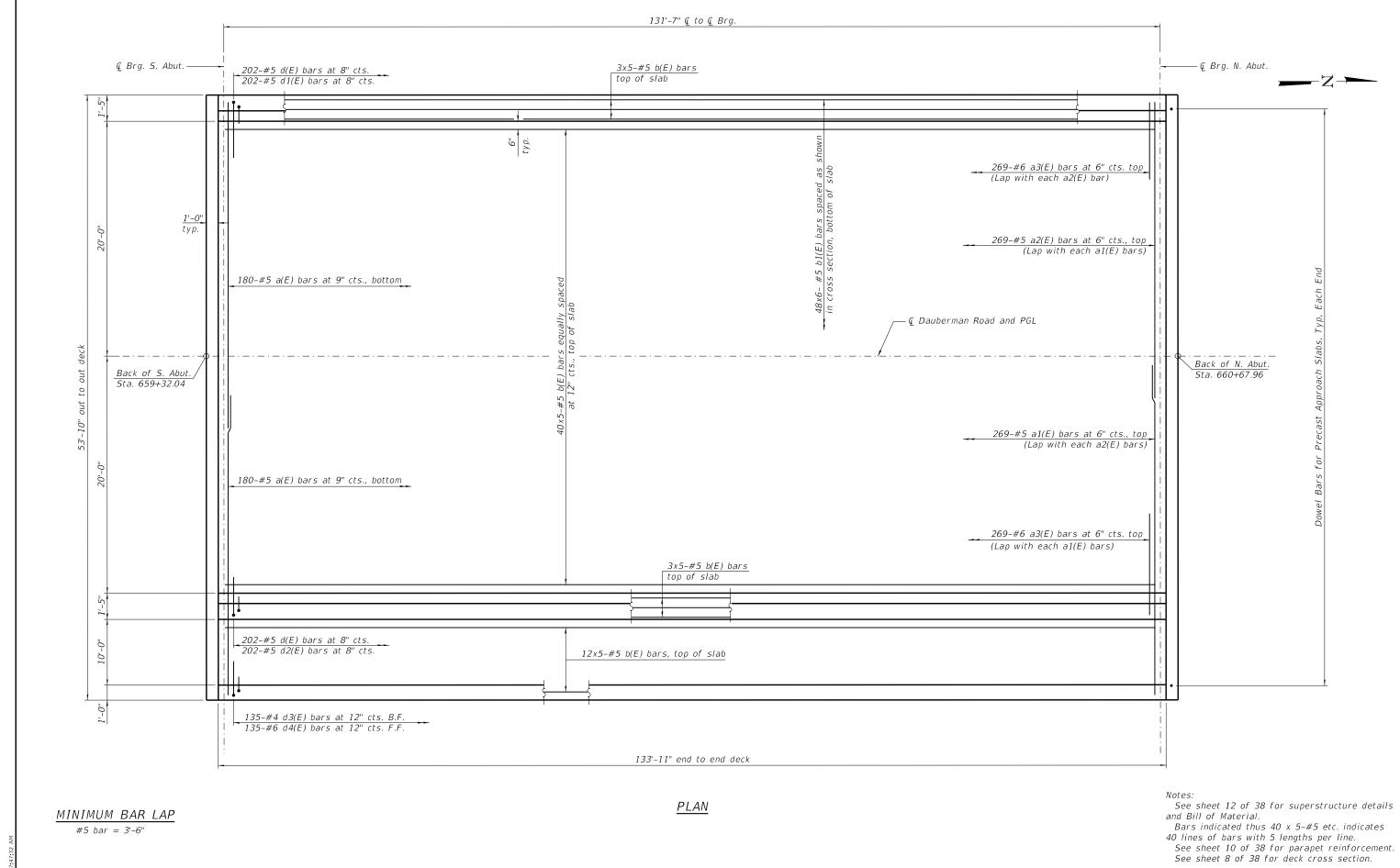
Location	Station	Offset	Theoretical Grade Elevations
South End of North Appr.	660+66.96	20.00	724.86
A3 A4	660+76.96 660+86.96	20.78 21.56	724.50 724.12
North End of North Appr.	660+96.96	22.33	723.75

# EAST MULTI-USE PATH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South End of North Appr.	660+66.96	31.42	725.03
A3 A4	660+76.96 660+86.96	32.19 32.97	724.67 724.29
North End of North Appr.	660+96.96	33.75	723.92

	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
•	PLOT SCALE = NTS	DRAWN - HB	REVISED -
<b>J</b> .	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

F.A.S. RTE.	SECT	ION			COUNTY	TOTAL SHEETS	SHEE NO.
1107	15-00277-01-BR			KANE	542	251	
					CONTRACT	NO. 6	1H95
		BUDIOIC	CCD	Α.	D DDOLECT		



**BL** BLA, Inc.

 USER NAME
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 REVISED 

 PLOT SCALE = NTS
 DRAWN - HB
 REVISED 

 PLOT DATE = 11/18/2022
 CHECKED - TJJ
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

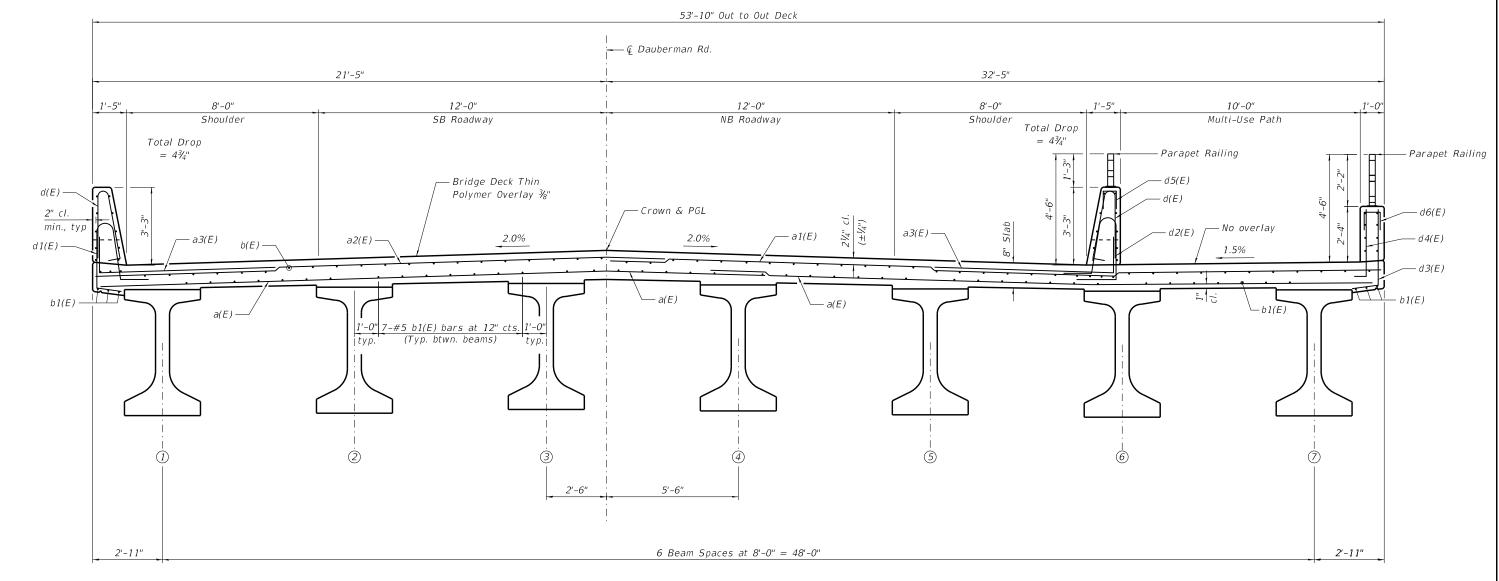
SUPERSTRUCTURE PLAN
STRUCTURE NO. 045-3402

SHEET NO. 7 OF 38 SHEETS

 
 F.A.S. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 1107
 15-00277-01-BR
 KANE
 542
 252

 CONTRACT
 NO.
 61H95



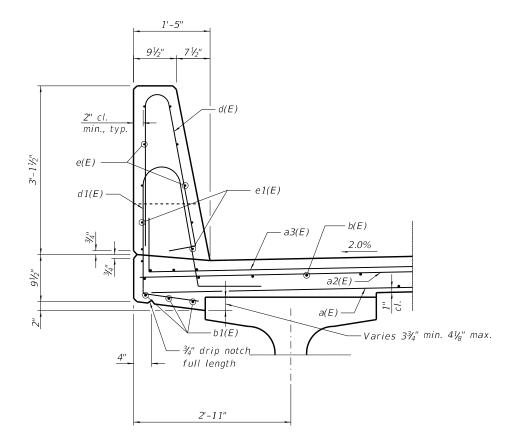
 $\frac{CROSS\ SECTION}{(Looking\ North)}$ 

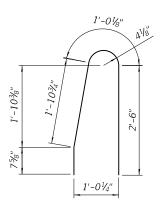
BLA, Inc.

	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
_	PLOT SCALE = NTS	DRAWN - HB	REVISED -
٠.	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

SUPERSTRUCTURE CROSS SECTION STRUCTURE NO. 045-3402
SHEET NO. 8 OF 38 SHEETS

F.A.S. RTE.	SECTION			COUNTY	TOTAL SHEETS	SHE
1107	15-00277-01-BR			KANE	542	25
				CONTRACT	NO. 6	1H9
		ILLINOIS	EED, A	ID PROJECT		





#### ALTERNATE BAR d2(E)

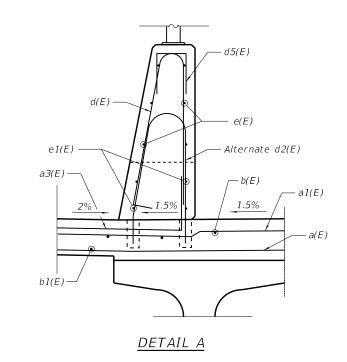
At East Roadway Barrier

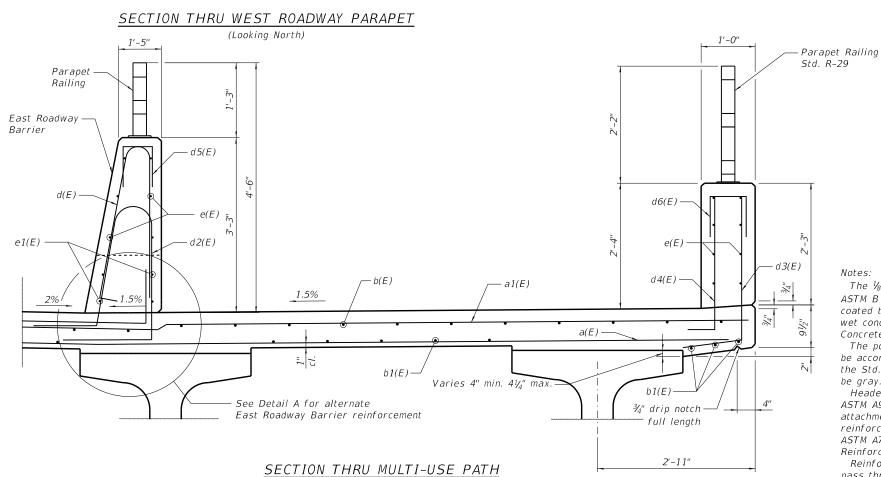
Drill and set Alternate #5 d2(E) bar according to Article 509.06 of the Standard Specifications. Drilled holes shall be roughened or scored per manufacturer's recommendations. Maximum depth of hole shall not exceed 6".

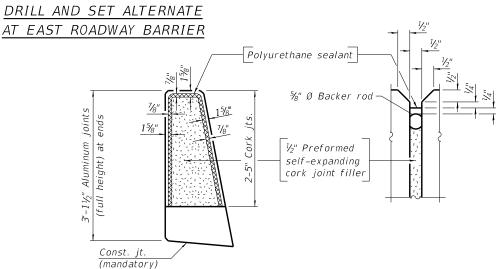
The Contractor shall take all necessary precautions to prevent drilled hole interference with deck reinforcement bars. Locate longitudinal bars to miss drilled locations. Locate drilled holes to miss transverse bars in deck.

If alternate parapet reinforcement is chosen, cost of alternate d2(E) bars, drilling, and setting is included with the cost of Reinforcement Bars, Epoxy Coated. No additional payment will be made.

Drill and set alternate shall only be allowed at the East Roadway Barrier.







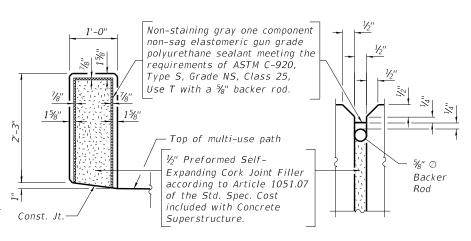
#### ROADWAY PARAPET/BARRIER JOINT DETAILS

The 1/8" Aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure.

The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.

Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated. Reinforcement bars shall not

pass thru aluminum sheets and cork joint filler.



#### MULTI-USE PATH PARAPET JOINT DETAILS

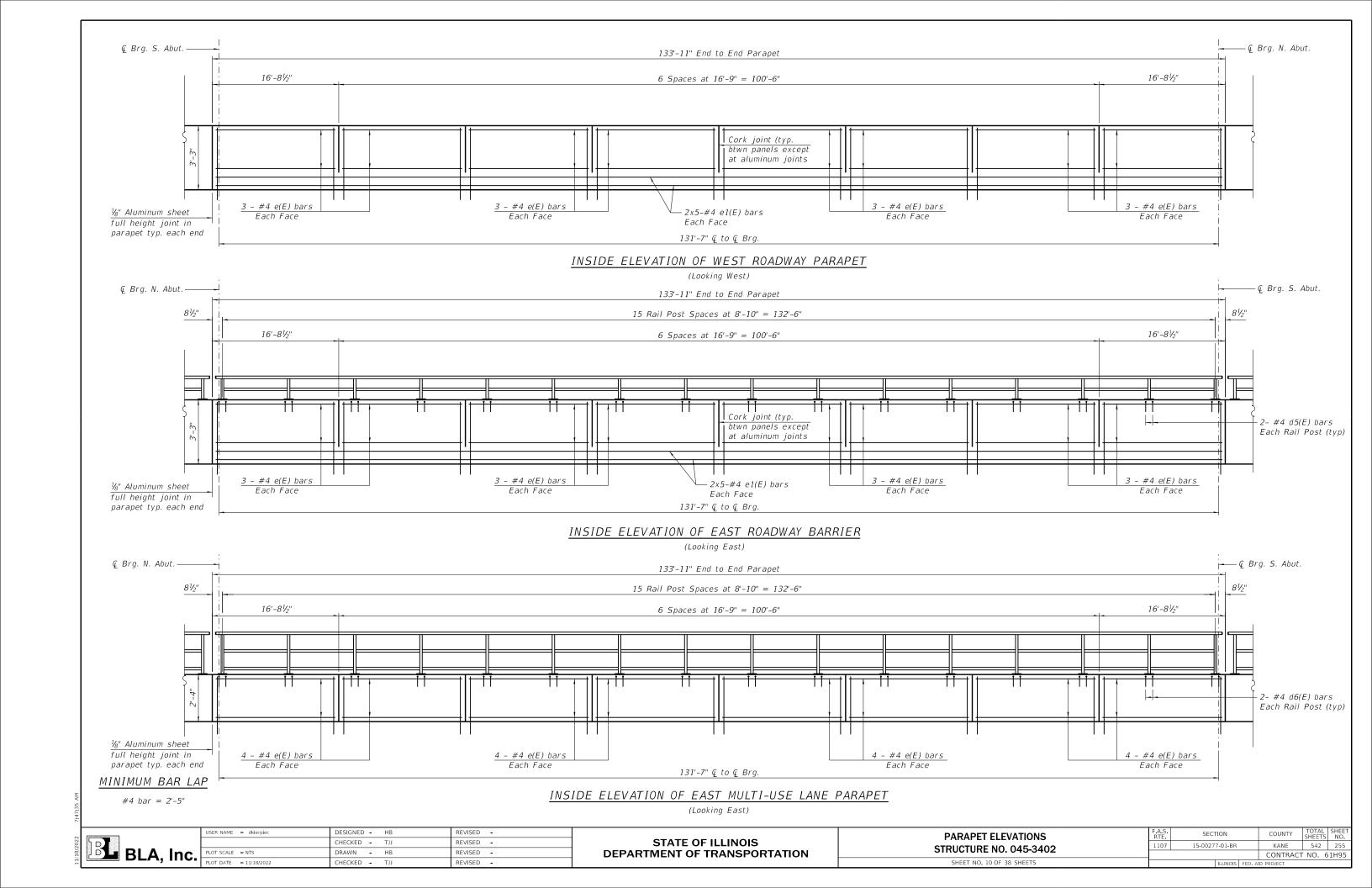


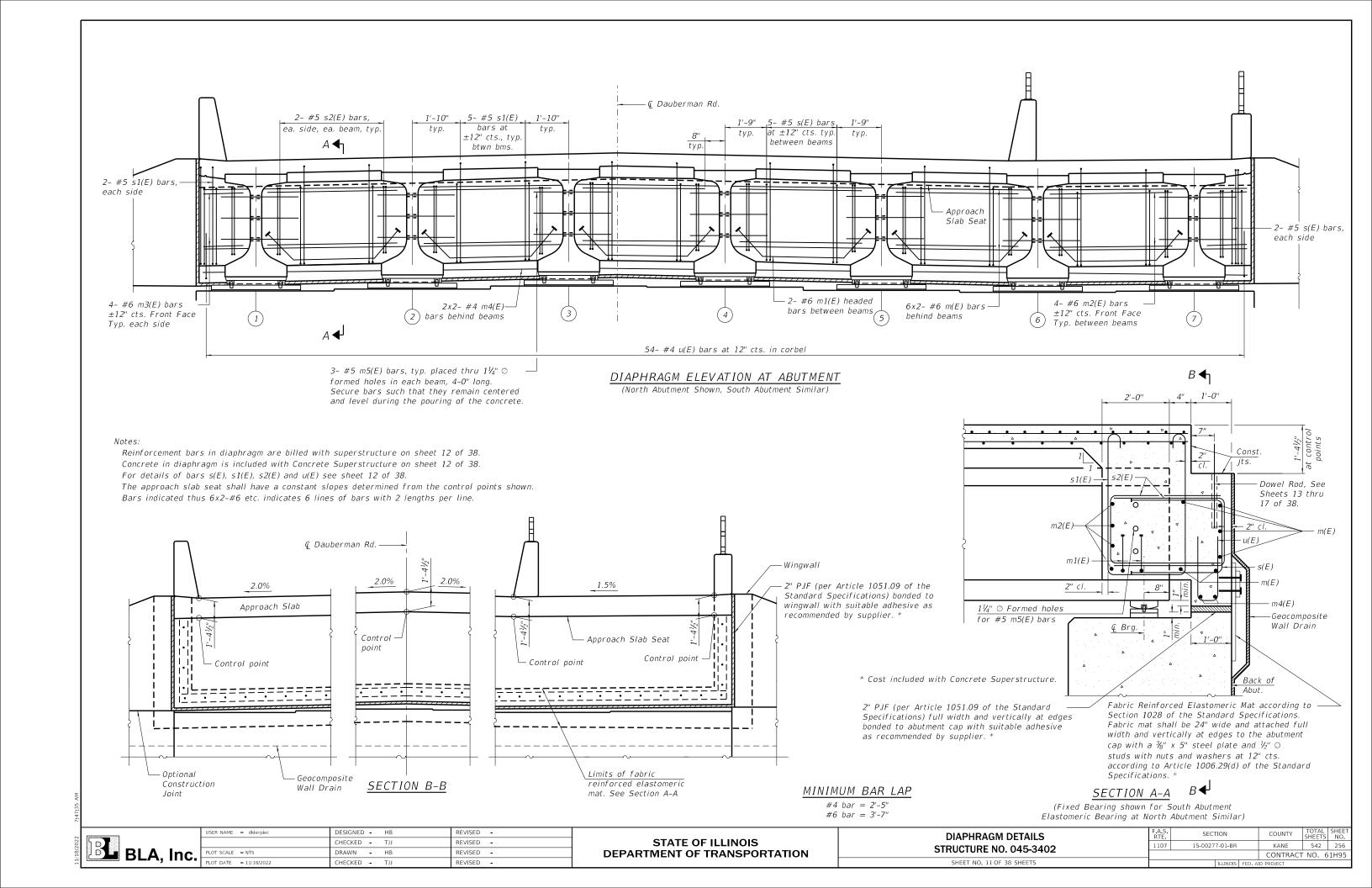
	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
٠.	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

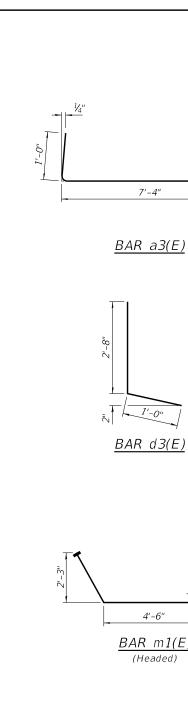
(Looking North)

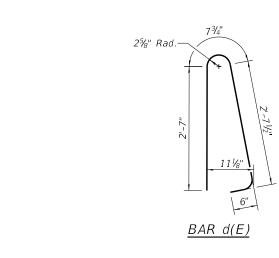
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  PARAPET AND PATH CROSS SECTIONS **STRUCTURE NO. 045-3402** SHEET NO. 9 OF 38 SHEETS

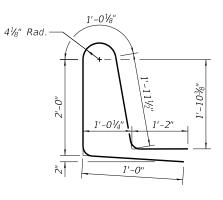
F.A.S. RTE.	SECTION	ON		COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-00277-01-BR			KANE	542	254
				CONTRACT	NO. 6	1H95
	TI TI	LLINOIS	EED Δ	ID PROJECT		



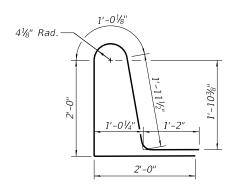




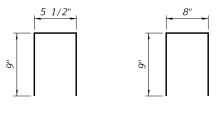




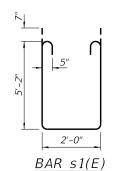
*BAR d1(E)* 



BAR d2(E)



BAR d6(E)



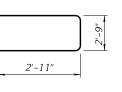
*BAR d5(E)* 

<u>SUPERSTRUCTURE</u> BILL OF MATERIAL

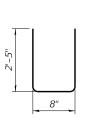
Bar         No.         Size         Length         Shape           a(E)         360         #5         28'-7"         —           a1(E)         269         #5         32'-6"         —           a2(E)         269         #5         24'-8"         —           a3(E)         538         #6         8'-4"         —           b(E)         290         #5         29'-9"         —           d(E)         404         #5         6'-5"         L           d1(E)         202         #5         7'-2"         L           d2(E)         202         #5         8'-2"         L           d3(E)         135         #4         3'-8"         L           d4(E)         135         #6         3'-7"         L           d5(E)         32         #4         1'-11½"         —           m(E)					
a1(E) 269 #5 32'-6"  a2(E) 269 #5 24'-8"  a3(E) 538 #6 8'-4"  b(E) 290 #5 29'-9"  b1(E) 288 #5 25'-5"  d(E) 404 #5 6'-5" d1(E) 202 #5 7'-2" d2(E) 202 #5 8'-2" d3(E) 135 #4 3'-8" d4(E) 135 #6 3'-7" L d5(E) 32 #4 1'-11½"  d6(E) 32 #4 2'-2"  m(E) 40 #4 28'-10"  m(E) 24 #6 9'-8" m3(E) 48 #6 6'-8" m3(E) 48 #6 6'-8" m3(E) 48 #6 6'-8" m3(E) 48 #6 6'-8" m3(E) 42 #5 4'-0"  s(E) 8 #4 28'-1" m4(E) 8 #4 28'-1" m5(E) 42 #5 4'-0"  s(E) 68 #5 13'-6" □ s(E) 68 #5 9'-7" □  Reinforcement Bars, Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sa vd 506	Bar	No.	Size	Length	Shape
a1(E)       269       #5       32'-6"       —         a2(E)       269       #5       24'-8"       —         a3(E)       538       #6       8'-4"       —         b(E)       290       #5       29'-9"       —         b(E)       290       #5       25'-5"       —         d(E)       404       #5       6'-5"       L         d1(E)       202       #5       7'-2"       L         d3(E)       135       #4       3'-8"       L         d4(E)       135       #6       3'-7"       L         d5(E)       32       #4       1'-11½"       I         d6(E)       32       #4       1'-11½"       I         e(E)       160       #4       16'-4"       —         e(E)       40       #4       28'-10"       —         m3(E)       48       #6       6'-8"       —         m3(E)       48       #6       6'-8"       —         m3(E)       42       #5       4'-0"       —         s(E)       68       #5       8'-11"       —         s(E)       68       #5       9'-7"	a(E)	360	#5	28'-7"	
Box   Box		269	#5	32'-6"	
B(E)   290   #5   29'-9"		269	#5	24'-8"	
b(E) 290 #5 29'-9" b1(E) 288 #5 25'-5"  d(E) 404 #5 6'-5" d1(E) 202 #5 7'-2" d2(E) 202 #5 8'-2" d3(E) 135 #4 3'-8" d4(E) 135 #6 3'-7" d5(E) 32 #4 1'-11½"  d6(E) 32 #4 2'-2"  m(E) 160 #4 16'-4" e1(E) 40 #4 28'-10"  m(E) 24 #6 9'-8" m1(E) 24 #6 9'-8" m2(E) 48 #6 6'-8" m3(E) 16 #6 2'-1" m4(E) 8 #4 28'-1" m5(E) 42 #5 4'-0"  s(E) 68 #5 13'-6" g1(E) 56 #5 9'-7"  u(E) 108 #4 5'-5" U  Reinforcement Bars, Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sa vd 506					ш
D1(E)   288   #5   25'-5"					
D1(E)   288   #5   25'-5"					
D1(E)   288   #5   25'-5"	b(E)	290	#5	29'-9"	
d(E)					
d1(E)     202     #5     7'-2"     □       d2(E)     202     #5     8'-2"     □       d3(E)     135     #4     3'-8"     □       d4(E)     135     #6     3'-7"     □       d5(E)     32     #4     1'-11½"     □       d6(E)     32     #4     2'-2"     □       e(E)     160     #4     16'-4"     —       e1(E)     40     #4     28'-10"     —       m1(E)     24     #6     9'-8"     —       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated     Concrete     Cu. Yd.     289.9       Superstructure     Protective Coat     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     182					
d1(E)     202     #5     7'-2"     □       d2(E)     202     #5     8'-2"     □       d3(E)     135     #4     3'-8"     □       d4(E)     135     #6     3'-7"     □       d5(E)     32     #4     1'-11½"     □       d6(E)     32     #4     2'-2"     □       e(E)     160     #4     16'-4"     —       e1(E)     40     #4     28'-10"     —       m1(E)     24     #6     9'-8"     —       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated     Concrete     Cu. Yd.     289.9       Superstructure     Protective Coat     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     182					
d1(E)     202     #5     7'-2"     □       d2(E)     202     #5     8'-2"     □       d3(E)     135     #4     3'-8"     □       d4(E)     135     #6     3'-7"     □       d5(E)     32     #4     1'-11½"     □       d6(E)     32     #4     2'-2"     □       e(E)     160     #4     16'-4"     —       e1(E)     40     #4     28'-10"     —       m1(E)     24     #6     9'-8"     —       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated     Concrete     Cu. Yd.     289.9       Superstructure     Protective Coat     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     182	d(E)	404	#5	6'-5"	Λ
d2(E)     202     #5     8'-2"     L       d3(E)     135     #4     3'-8"     L       d4(E)     135     #6     3'-7"     L       d5(E)     32     #4     1'-11½"     □       d6(E)     32     #4     2'-2"     □       e(E)     160     #4     16'-4"     —       e1(E)     40     #4     28'-10"     —       m1(E)     24     #6     9'-8"     —       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182		202	#5	7'-2"	
d3(E)     135     #4     3'-8"     ↓       d4(E)     135     #6     3'-7"     ↓       d5(E)     32     #4     1'-11½"     □       d6(E)     32     #4     2'-2"     □       e(E)     160     #4     16'-4"     —       e1(E)     40     #4     28'-10"     —       m(E)     24     #6     28'-8"     —       m1(E)     24     #6     9'-8"     ✓       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated     Concrete     Cu. Yd.     289.9       Superstructure     Protective Coat     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     182					
d4(E)     135     #6     3'-7"     L       d5(E)     32     #4     1'-11½"     □       d6(E)     32     #4     2'-2"     □       e(E)     160     #4     16'-4"     —       e1(E)     40     #4     28'-10"     —       m(E)     24     #6     9-8"     ✓       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated Concrete     Cu. Yd.     289.9       Superstructure     Cu. Yd.     289.9       Bridge Deck Thin     Sa. Yd.     182				3'-8"	ī
d5(E)   32					T
d6(E)     32     #4     2'-2"     □       e(E)     160     #4     16'-4"     —       e1(E)     40     #4     28'-10"     —       m(E)     24     #6     28'-8"     —       m1(E)     24     #6     9'-8"     ✓       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       s(E)     68     #5     4'-0"     —       s1(E)     68     #5     13'-6"     U       s2(E)     56     #5     9'-7"     □       u(E)     108     #4     5'-5"     U       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182				1'-111/2"	一一
e(E)       160       #4       16'-4"       —         e1(E)       40       #4       28'-10"       —         m(E)       24       #6       28'-8"       —         m1(E)       24       #6       9'-8"       _         m2(E)       48       #6       6'-8"       —         m3(E)       16       #6       2'-1"       —         m4(E)       8       #4       28'-1"       —         m5(E)       42       #5       4'-0"       —         s1(E)       68       #5       13'-6"       ①         s2(E)       56       #5       9'-7"       —         u(E)       108       #4       5'-5"       U         Reinforcement Bars, Epoxy Coated       Lbs.       64,520         Concrete Superstructure       Cu. Yd.       289.9         Protective Coat Bridge Deck Thin       Sq. Yd.       182				2'-2"	i i
m(E)     24     #6     28'-10"       m1(E)     24     #6     9'-8"       m2(E)     48     #6     6'-8"       m3(E)     16     #6     2'-1"       m4(E)     8     #4     28'-1"       m5(E)     42     #5     4'-0"       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     182	33(2)		· · ·		
m(E)     24     #6     28'-10"       m1(E)     24     #6     9'-8"       m2(E)     48     #6     6'-8"       m3(E)     16     #6     2'-1"       m4(E)     8     #4     28'-1"       m5(E)     42     #5     4'-0"       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     182					
m(E)     24     #6     28'-10"       m1(E)     24     #6     9'-8"       m2(E)     48     #6     6'-8"       m3(E)     16     #6     2'-1"       m4(E)     8     #4     28'-1"       m5(E)     42     #5     4'-0"       s(E)     68     #5     13'-6"     □       s1(E)     68     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     182	e(E)	160	#4	16'-4"	
m(E) 24 #6 28'-8"   m1(E) 24 #6 9'-8"   ✓/ m2(E) 48 #6 6'-8"   —/ m3(E) 16 #6 2'-1"   —/ m4(E) 8 #4 28'-1"   —/ m5(E) 42 #5 4'-0"    s(E) 68 #5 13'-6"   S2(E) 56 #5 9'-7"   ✓/ u(E) 108 #4 5'-5"   U  Reinforcement Bars, Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sa vd 506					
m1(E)     24     #6     9'-8"     ✓       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     8'-11"     —       s1(E)     68     #5     13'-6"     []       s2(E)     56     #5     9'-7"     —       u(E)     108     #4     5'-5"     U       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     50.6	01(1)	,,,	· · ·		
m1(E)     24     #6     9'-8"     ✓       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     8'-11"     —       s1(E)     68     #5     13'-6"     []       s2(E)     56     #5     9'-7"     —       u(E)     108     #4     5'-5"     U       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     50.6					
m1(E)     24     #6     9'-8"     ✓       m2(E)     48     #6     6'-8"     —       m3(E)     16     #6     2'-1"     —       m4(E)     8     #4     28'-1"     —       m5(E)     42     #5     4'-0"     —       s(E)     68     #5     8'-11"     —       s1(E)     68     #5     13'-6"     []       s2(E)     56     #5     9'-7"     —       u(E)     108     #4     5'-5"     U       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     50.6	m(E)	24	#6	28'-8"	
m2(E)     48     #6     6'-8"       m3(E)     16     #6     2'-1"       m4(E)     8     #4     28'-1"       m5(E)     42     #5     4'-0"       s(E)     68     #5     8'-11"       s1(E)     68     #5     13'-6"     1       s2(E)     56     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     50.6					\_/
m3(E)     16     #6     2'-1"       m4(E)     8     #4     28'-1"       m5(E)     42     #5     4'-0"       s(E)     68     #5     8'-11"       s1(E)     68     #5     13'-6"     1       s2(E)     56     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     50.6					
m4(E)     8     #4     28'-1"       m5(E)     42     #5     4'-0"       s(E)     68     #5     8'-11"       s1(E)     68     #5     13'-6"     □       s2(E)     56     #5     9'-7"     □       u(E)     108     #4     5'-5"     □       Reinforcement Bars, Epoxy Coated Concrete Superstructure     Cu. Yd.     289.9       Protective Coat Bridge Deck Thin     Sq. Yd.     182       Bridge Deck Thin     Sq. Yd.     50.6		16	#6	2'-1"	
S(E)   68   #5   8'-11"   S1(E)   68   #5   13'-6"   U   S2(E)   56   #5   9'-7"   S2(E)   108   #4   5'-5"   U				28'-1"	
S(E)   68   #5   8'-11"					
S1(E)   68   #5   13'-6"   1	(2)				
S1(E)   68   #5   13'-6"   1					
S1(E)   68   #5   13'-6"   1	s(E)	68	#5	8'-11"	
s2(E)       56       #5       9'-7"       □         u(E)       108       #4       5'-5"       □         Reinforcement Bars, Epoxy Coated Concrete Superstructure       Cu. Yd.       289.9         Protective Coat Bridge Deck Thin       Sq. Yd.       182         Bridge Deck Thin       Sq. Yd.       506					11
Reinforcement Bars, Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sq. Vd. 50-5" U				9'-7"	
Reinforcement Bars, Lbs. 64,520 Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sa. Vd. 506	32(2)	- 50	" "	,	
Reinforcement Bars, Lbs. 64,520 Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sa. Vd. 506	u(E)	108	#4	5'-5"	11
Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sa Vd 506	=(=)				
Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sa Vd 506					
Epoxy Coated Concrete Superstructure Protective Coat Bridge Deck Thin Sa Vd 506	Reinforc	ement I	Bars.		
Concrete Superstructure Protective Coat Bridge Deck Thin Sa Vd 506			Lbs.	64,520	
Superstructure Protective Coat Bridge Deck Thin Sa vd 506					
Protective Coat Sq. Yd. 182 Bridge Deck Thin Sq. Yd. 506			Cu. Yd.	289.9	
Bridge Deck Thin			Sq. Yd.	182	
Polymer Overlay 3/8"   Sq. Yd.   596			·		
, , , , , ,			5q. Ya.	596	



7'-4"



BARS s2(E)



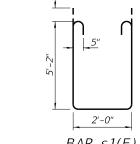
2'-7"

 $\frac{BARS\ s(E)}{(Headed)}$ 

1'-0"

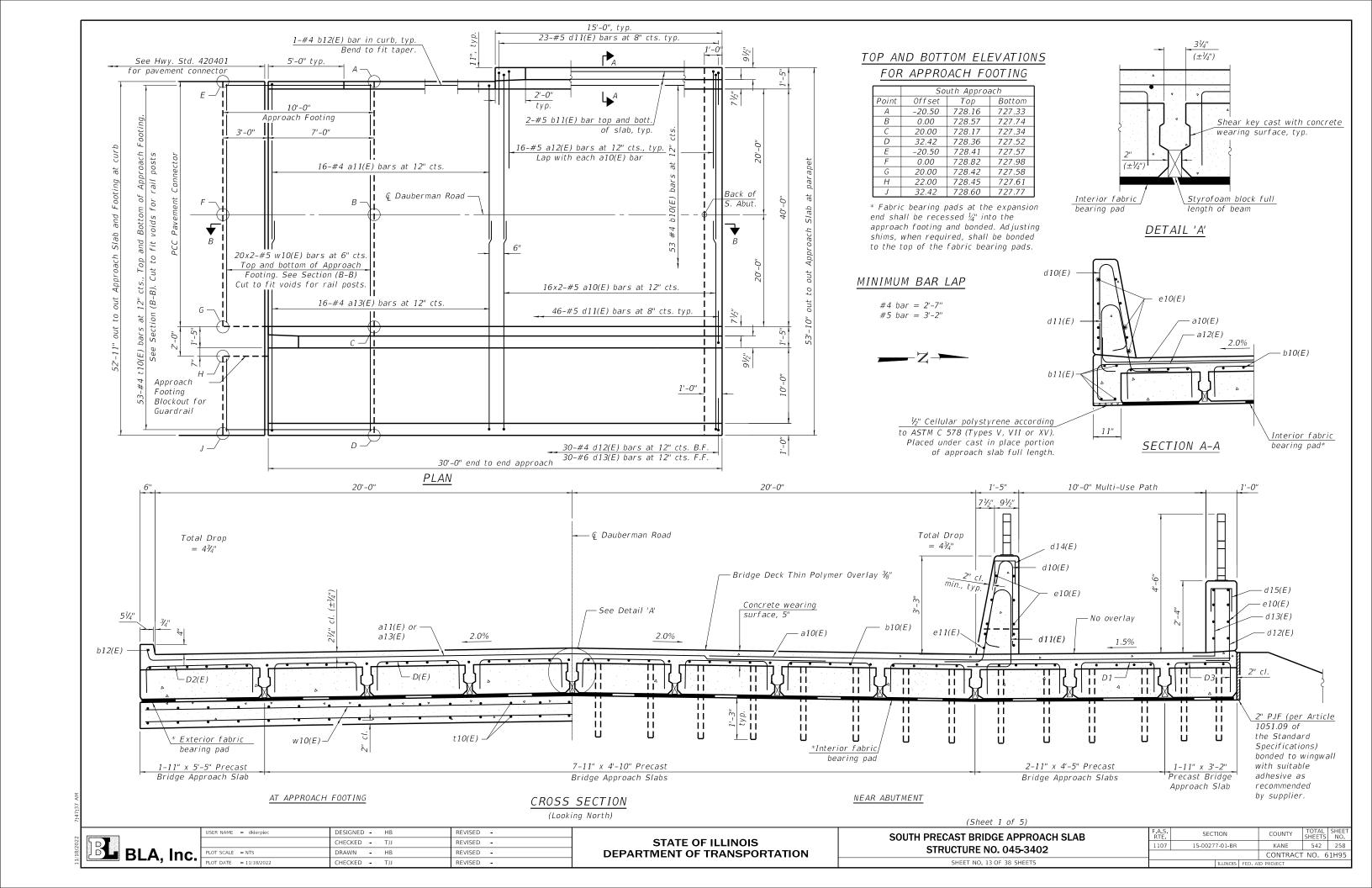
*BAR d4(E)* 

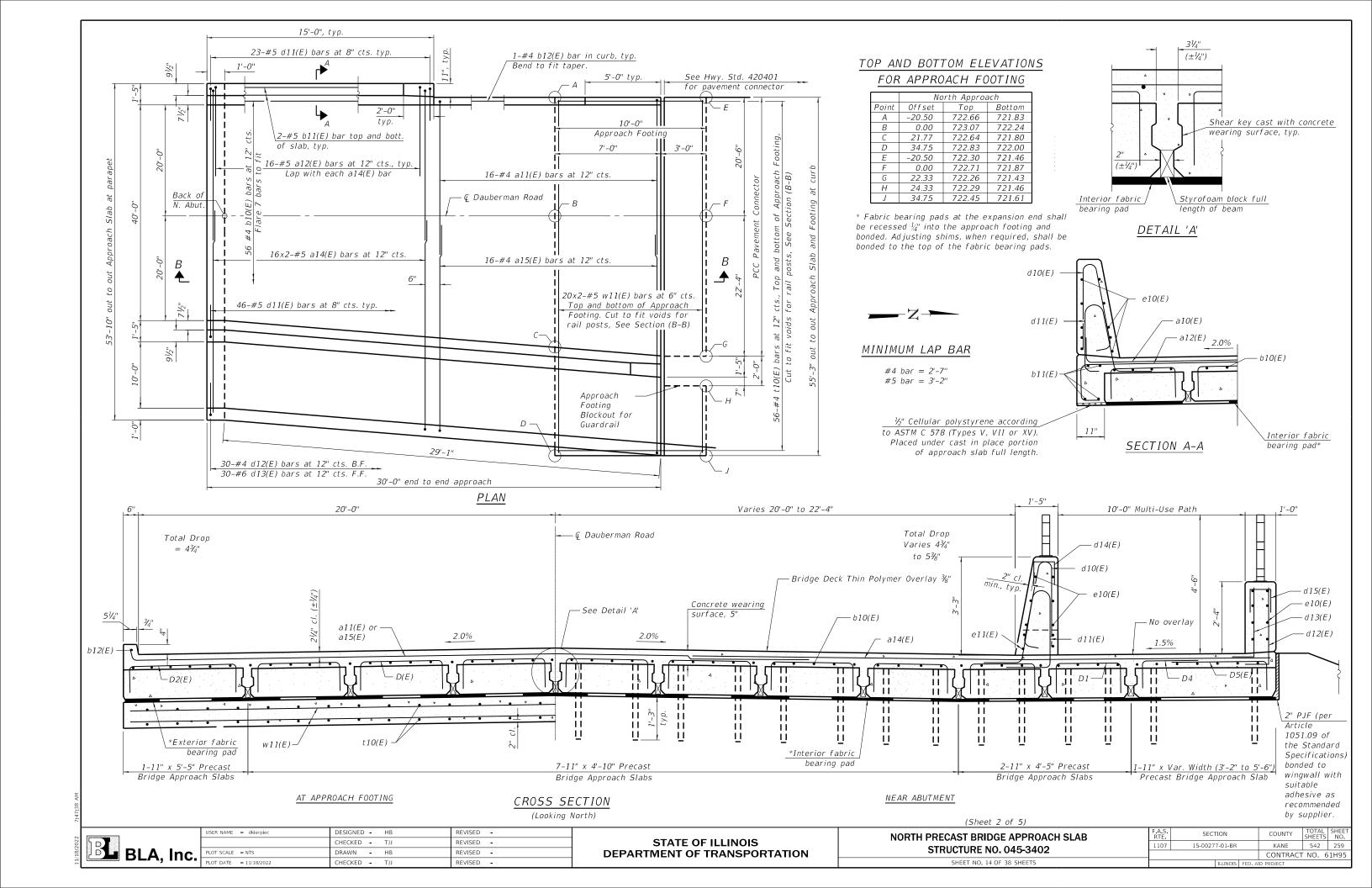
BAR u(E)

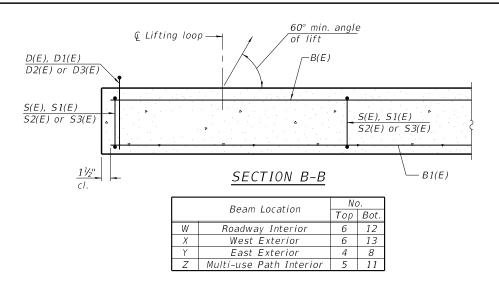




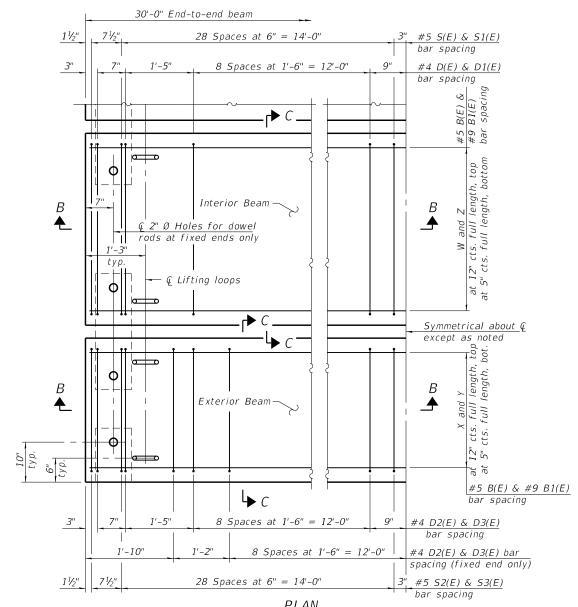
	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
•	PLOT SCALE = NTS	DRAWN - HB	REVISED -
-	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -





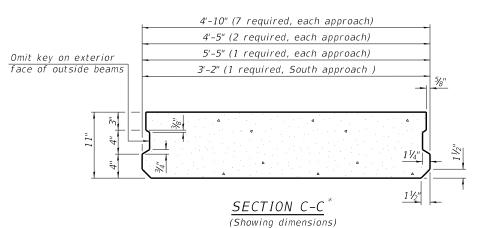


#### BARS B(E) and B1(E)



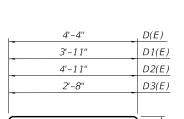
#### <u>r lan</u>

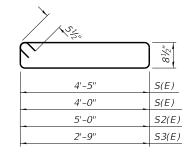
(Northwest Corner, South Approach Shown, Others Similar)



# $\begin{array}{c|c} D(E), D1(E), \\ \hline D2(E) \ or \ D3(E) \end{array}$ $\begin{array}{c|c} B(E) \\ \hline \tilde{S} \\ \hline \end{array}$ $S(E), S1(E), \\ S2(E) \ or \ S3(E) \end{array}$ B(E) $SECTION \ C-C$

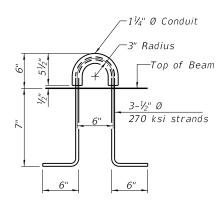
(Showing reinforcement)





#### BARS D(E) Thru D3(E)

BARS S(E) Thru S3(E)



#### LIFTING LOOP DETAIL

(An alternate lifting loop with a proof load of 25,000 lbs. and utilized according to the manufacturer's recommendations may be used)

#### Notes

The precast bridge approach slab shall be according to Section 504 of the Standard Specifications and shall be paid for at the contract unit price per square foot for Precast Bridge Approach Slab.

square foot for Precast Bridge Approach Slab.

Cast-in-place substitution of Precast Bridge Approach Slab is not allowed.

The top surface of precast bridge approach slabs shall be finished similar to

precast prestressed deck beams with concrete wearing surface as specified in the IDOT "Manual for Fabrication of Precast Prestressed Concrete Products." Two  $\frac{1}{2}$ " fabric adjusting shims of the dimensions of the exterior bearing pad

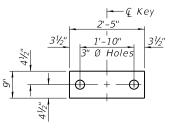
shall be provided for each bearing pad location. Cost included with Precast Bridge Approach Slab.

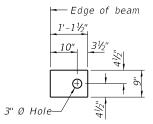
A minimum 2  $\frac{1}{2}$  0 lifting pins shall be used to engage the lifting loops during handling.

Compressive strength of precast concrete, f'c shall be 6,000 psi.

Compressive strength of precast concrete during initial lifting, f'ci shall be 5,000 psi.

\* 1 Flared beam is required for North Approach. See Sheet 16 of 38 for details.





**EXTERIOR** 

<u>INTERIOR</u>

# FABRIC BEARING PAD

#### Notes:

Bearing pads at fixed end shall be ½" thick and bearing pads at expansion end shall be ¾" thick. Omit holes for fabric bearing pads at approach slab footing end of beams.

#### BAR LIST

# EACH ROADWAY INTERIOR BEAM EACH WE

(For information only)

Bar	No.	Size	Length	Shape
B(E)	6	#5	29'-8"	_
B1(E)	12	#9	29'-8"	_
D(E)	22	#4	6'-4"	
S(E)	58	#5	11'-2"	<b></b>

	<u>B</u> A	R	LIST			
$^{\circ}H$	WFST	F	XTFR	IOR	RF A	ιM

(For information only)

Bar	No.	Size	Length	Shape
B(E)	6	#5	29'-8"	
B1(E)	13	#9	29'-8"	
D2(E)	32	#4	6'-11"	
S2(E)	58	#5	12'-4"	

# BAR LIST EACH MULTI-USE PATH INTERIOR BEAM

(For information only)

Bar	No.	Size	Length	Shape
B(E)	5	#5	29'-8"	_
B1(E)	11	#9	29'-8"	
D1(E)	22	#4	5'-11"	
S1(E)	58	#5	10'-4"	

# <u>BAR LIST</u> <u>EACH EAST EXTERIOR BEAM</u>

(For information only)

	A / -	C	1	Chara
Bar	No.	Size	Length	Shape
B(E)	4	#5	29'-8"	
B1(E)	8	#9	29'-8"	
D3(E)	32	#4	4'-8"	]
S3(E)	58	#5	7'-10"	
	·			

(Sheet 3 of 5)

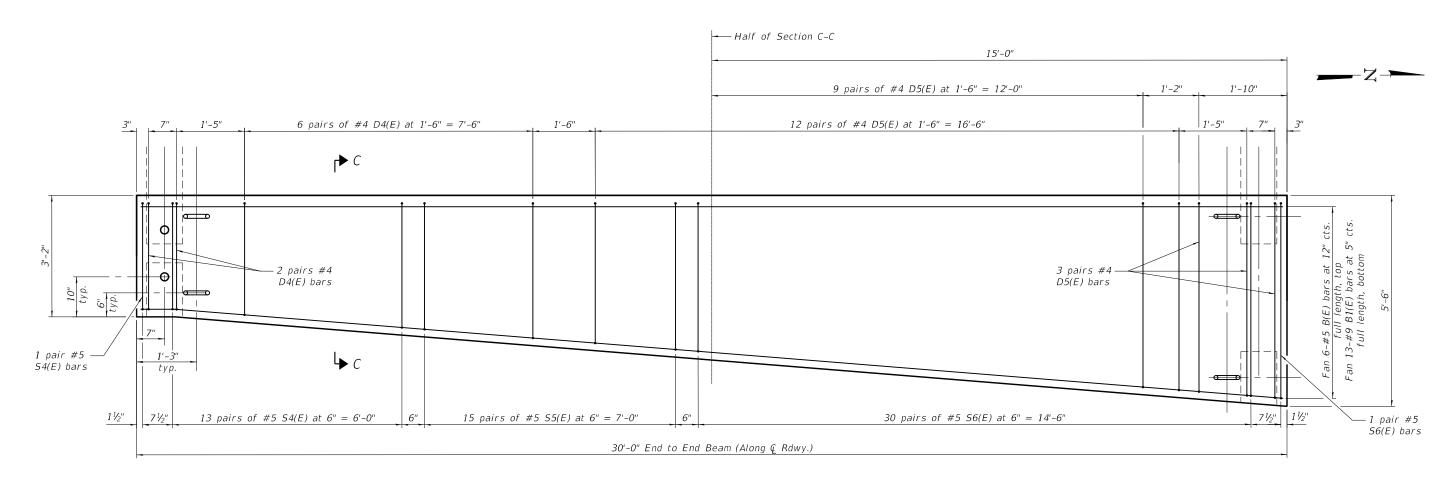
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		CHECKED - TJJ	REVISED -
BLA, Inc.	PLOT SCALE = NTS	DRAWN - HB	REVISED -
	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

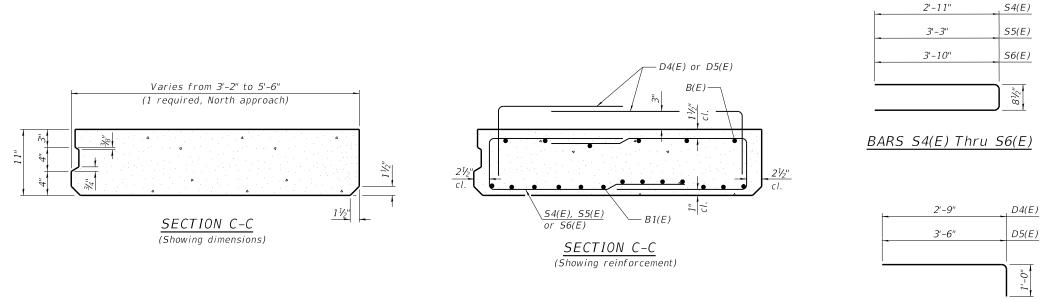
PRECAST BRIDGE APPROACH SLAB DETAILS I STRUCTURE NO. 045-3402

SHEET NO. 15 OF 38 SHEETS

F.A.S. RTE	RTE. SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-00277-01-BR			KANE	542	260
			CONTRACT	NO. 6	1H95	
ILLINOIS FED. AI			ID PROJECT			



<u>PLAN</u> (North approach exterior flared beam)



<u>BAR LIST</u> <u>FLARED EXTERIOR BEAM</u>

(For information only)

Bar	No.	Size	Length	Shape
B(E)	6	#5	29'-8"	
B1(E)	13	#9	29'-8"	
D4(E)	16	#4	3'-9"	
D5(E)	48	#4	4'-6"	
S4(E)	28	#5	6'-6 <sup>1</sup> / <sub>2</sub> "	
S5(E)	30	#5	7'-21/2"	
S6(E)	62	#5	8'-4 <sup>1</sup> / <sub>2</sub> "	

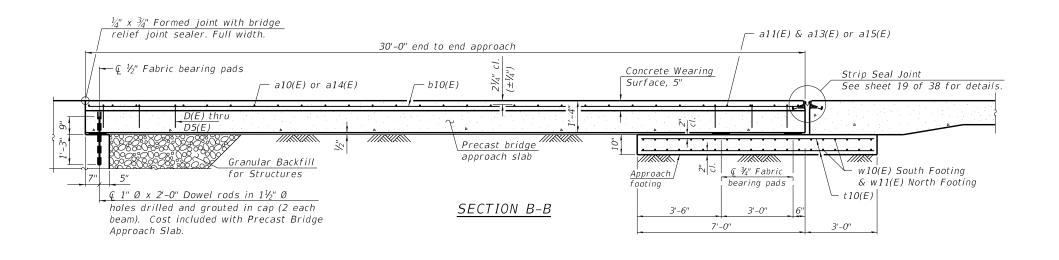
#### MINIMUM BAR LAP

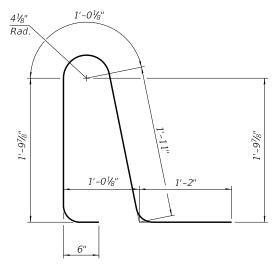
#4 bar = 2'-7" #5 bar = 3'-2" BARS D4(E) & D5(E)

(Sheet 4 of 5)

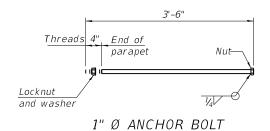


	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
٠.	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

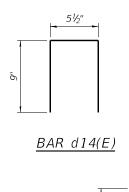


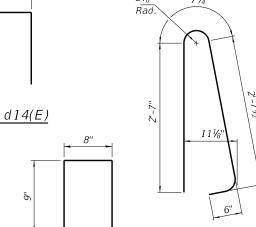


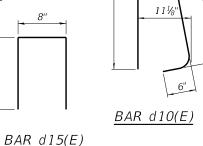
BAR d11(E)

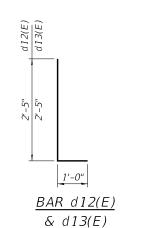


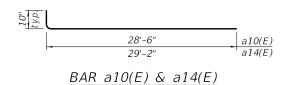
(Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications. Cost of anchor bolt assemblies included with Concrete Superstructure)





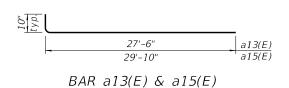








# 6'-6" BAR a12(E)



## TWO APPROACHES BILL OF MATERIAL

Bar	No.	Size	Length	Shape				
a10(E)	32	#5	29'-4"					
a11(E)	32	#4	28'-6"					
a12(E)	32	#5	8'-2"					
a13(E)	16	#4	28'-5"					
a14(E)	32	#5	30'-0"					
a15(E)	16	#4	30'-8"					
b10(E)	109	#4	29'-8"	-				
b11(E)	8	#5	14'-8"	-				
b12(E)	2	#4	14'-8"	-				
d10(E)	138	#5	6'-5"	Ŋ				
d11(E)	138	#5	6'-5"	1				
d12(E)	60	#4	4'-3"	Ĺ				
d13(E)	60	#6	4'-3"	L				
d14(E)	16	#4	1'-111/2"	П				
d15(E)	16	#4	2'-2"	П				
e10(E)	76	#4	14'-8"					
e11(E)	8	#4	29'-8"					
, ,								
t10(E)	218	#4	9'-8"					
w10(E)	80	#5	28'-0"					
w11(E)	80	#5	29'-1"					
Concrete	Superstruc	ture	Cu Yd	17.1				
Concrete	Structures	Cu Yd	33.4					
Reinforcement Bars,			Pound	15,320				
	Epoxy Coated Precast Bridge Approach Slab			3,251				
	Wearing Su		Sq Ft Sq Yd	271				
Protectiv		iii ace, J	Sq Yd	42				
	eck Thin Po	olvmer	34 TU	42				
Overlay 3		,	Sq Yd	271				
OVERTAL 7	ŏ .	Overlay 1/8" , ,						

The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.

After precast bridge approach slabs have been erected, holes shall be drilled into abutment and anchor dowels placed. Dowel holes shall be filled with non-shrink grout to top of precast slab and cured according to Article 1020.13(a)(3) or 1020.13(a)(5) of the Standard Specifications for a minimum of 24 hours before casting the shear keys and wearing surface.

Any concrete poured monolithically with the wearing surface, such as curbs, shall not be paid for separately, but will be included in the cost of Concrete Wearing Surface, 5". The strip seal shall extend 6" beyond the edge of the approach slab on each end.

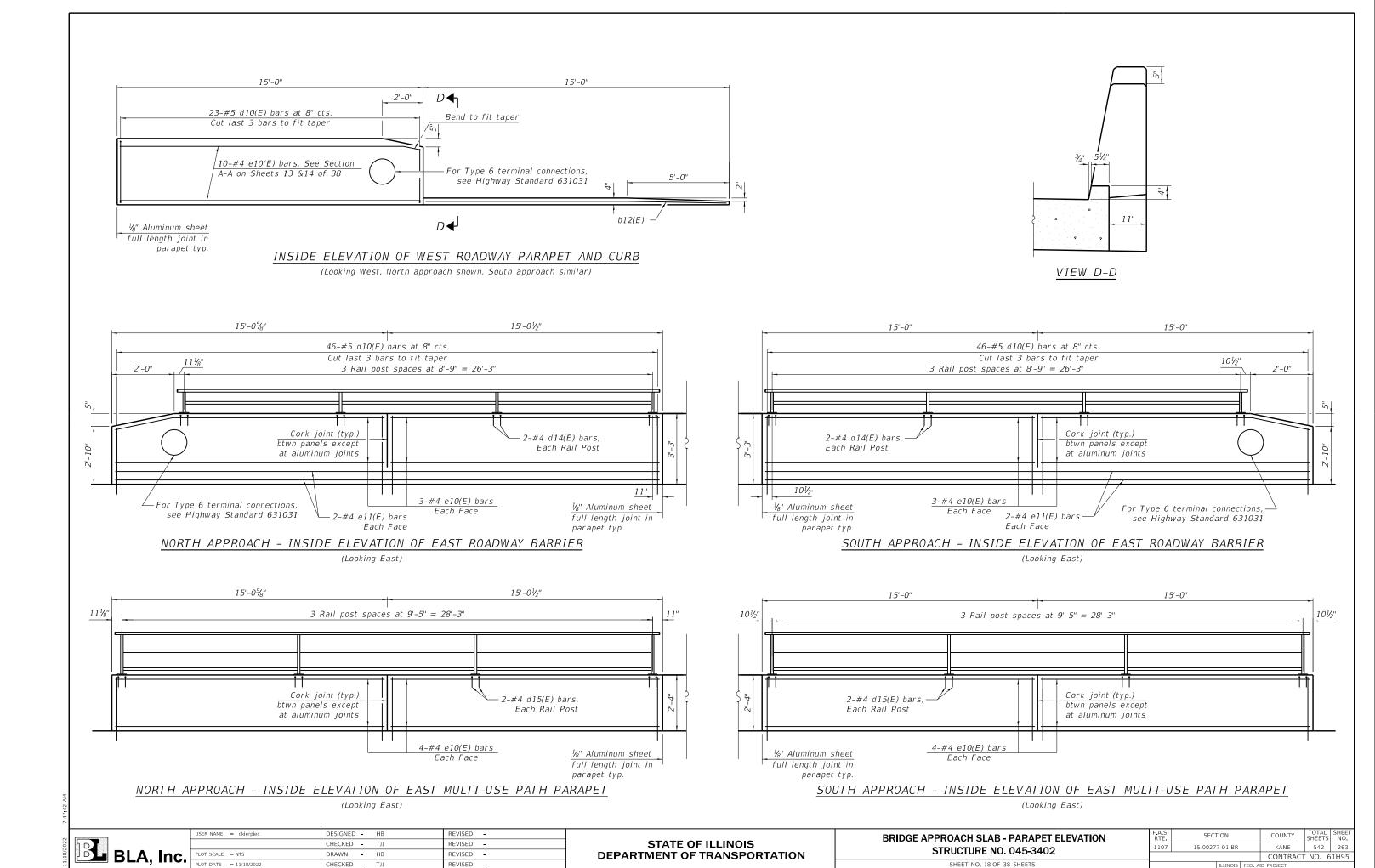
Parapet concrete shall be paid for as Concrete Superstructure. Approach footing concrete shall be paid for as Concrete Structures. The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf. Cost of excavation for approach footing included with Concrete Structures. For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 38. Cost of cellular polystyrene is included with Concrete Superstructure.

(Sheet 5 of 5)



	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
•	PLOT SCALE = NTS	DRAWN - HB	REVISED -
-	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

F.A.S. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-00277-01-BR			KANE	542	262
				CONTRACT	NO. 6	1H95
	ILLINOIS FED. AI			ID PROJECT		



Locking edge rail-Top of concrete -

Roadway Barrier — SHOWING ROLLED RAIL JOINT

at 50° F

at 50° F

-Strip seal

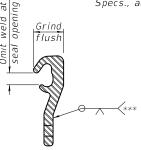
Locking edge railat 50° F Top of concrete ∕—Strip seal \* ¾" Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs) at 50° F

 $rac{3}{8}$ "  $\phi$  threaded rods in  $rac{7}{16}$ "  $\phi$  holes at ±4'-0" cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

SHOWING WELDED RAIL JOINT

#### SECTION A-A

\* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



# LOCKING EDGE RAIL SPLICE

Rolled rail shown, welded rail similar.

Notes:

The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4

The locking edge rails depicted configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the  $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

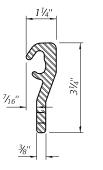
The manufacturer's recommended installation methods shall be followed. All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $rac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

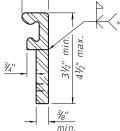
Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

39" constant slope barrier shown, 44" constant slope barrier similar as noted.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.



ROLLED(EXTRUDED) RAIL



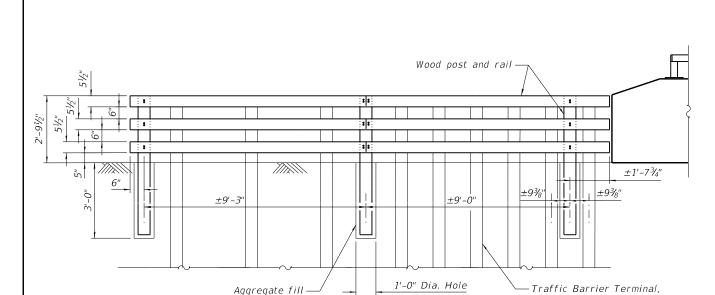
WELDED RAIL

## LOCKING EDGE RAIL

\*\*\* Back gouge not required if complete joint penetration is verified by mock-up.

#### BILL OF MATERIAL

Preformed Joint Strip Seal	Foot	110.5



WOOD POST AND RAIL FENCE PLAN (South shown, North similar)

Traffic Barrier

Terminal, Type 6

End Bridge —

Type 6 post (typ)

Approach Slab

#### WOOD POST AND RAIL FENCE ELEVATION

(South shown, North similar)

See Wood Post and Rail Special Provision for details.

 $\mathscr{Y}_{16}$ "  $\oslash$  hole for  $\mathscr{V}_2$ "  $\oslash$  bolt, nut and round -

3-2 x 6 Wood Rails, Typ. —

washers under bolt head & nut. (round

washer to be  $^{13}\!/_{8}" \oslash x ^{7}\!/_{64}"$  thick).

2 bolts each end, each rail.

6x6 wood

post (typ.)

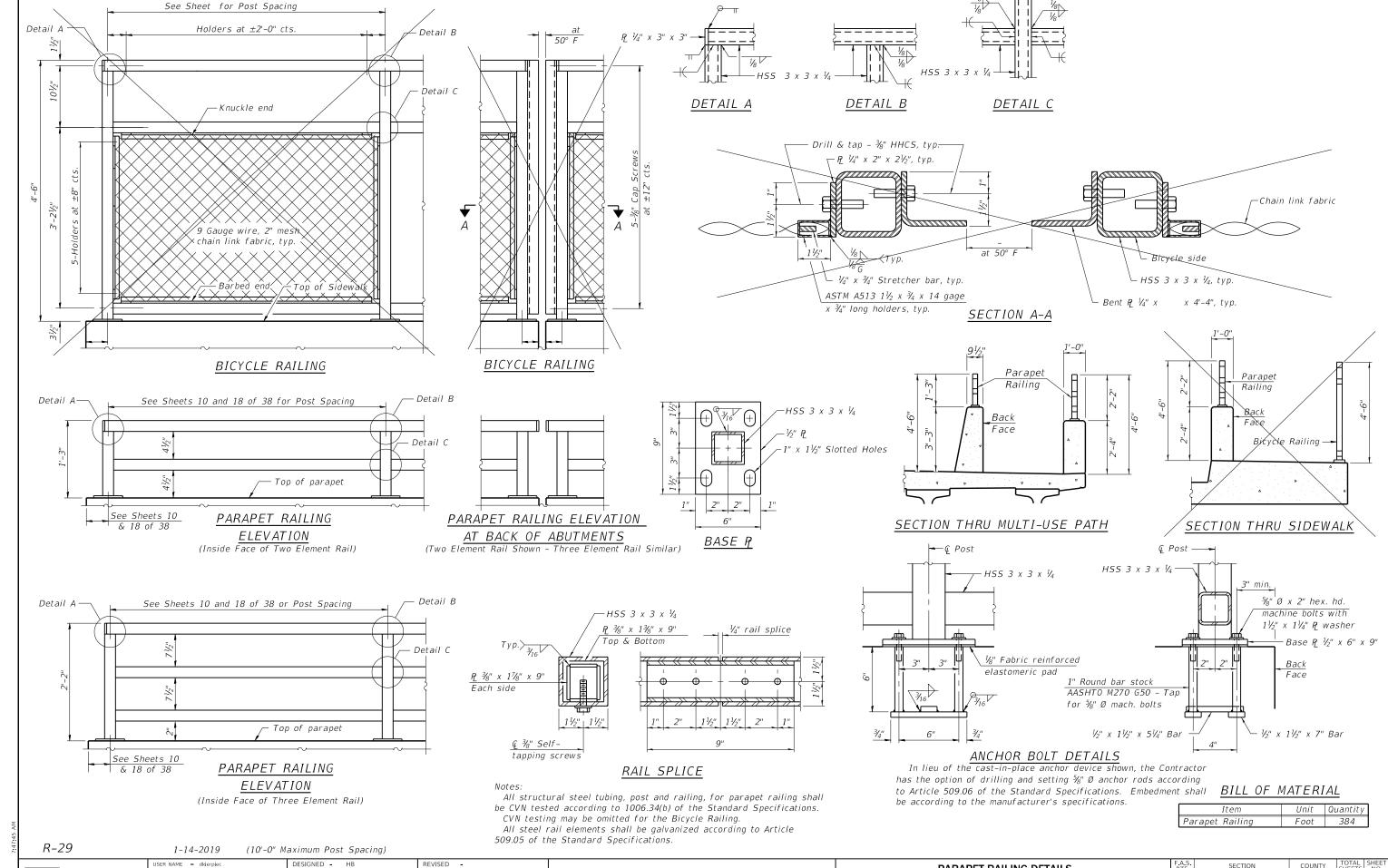
West Edge of Multi-Use Path —

BLA, Inc

	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
٠.	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  PREFORMED JOINT STRIP SEAL & WOOD RAILING DETAILS **STRUCTURE NO. 045-3402** SHEET NO. 19 OF 38 SHEETS

F.A.S. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-00277-01-BR			KANE	542	264
			CONTRACT	NO. 6	1H95	
	ILLINOIS FED. AL			ID PROJECT		



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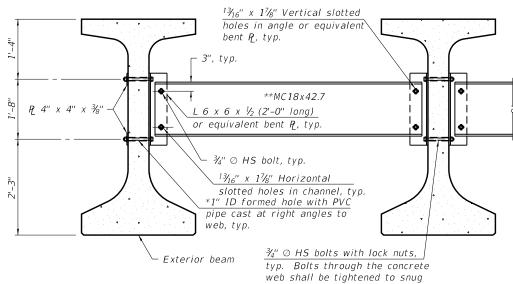
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PARAPET RAILING DETAILS STRUCTURE NO. 045-3402 SHEET NO. 20 OF 38 SHEETS

INTERIOR	BEAM I	MOMENT TABLE
		0.5 Span
I	(in⁴)	527,741
I'	(in⁴)	1,157,207
Sb	(in³)	18,688
S <sub>b</sub> '	(in³)	26,396
$S_t$	(in³)	15,182
$S_t'$	(in³)	60,397
DC 1	(k/')	1.915
$M_{DC1}$	('k)	4,145
DC2	(k/')	0.251
$M_{DC2}$	('k)	543
DW	(k/')	0.286
$M_{DW}$	('k)	619
LLDF		0.653
ML + IM	('k)	2,728

INTERIOR BEAM REACTION TABLE		
		Abutment
LLDF		0.814
R <sub>DC1</sub>	(k)	126.0
$R_{DC2}$	(k)	16.5
$R_{DW}$	(k)	18.8
R Ł	(k)	88.9
RIM	(k)	18.0
RTotal	(k)	268.2

- I: Non-composite moment of inertia of beam section (in.4).
- I': Composite moment of inertia of beam section (in.4). Sb: Non-composite section modulus for the bottom fiber of
- the prestressed beam (in.3).
- Sb': Composite section modulus for the bottom fiber of the prestressed beam (in.3).
- St: Non-composite section modulus for the top fiber of the prestressed beam (in.³).
- St': Composite section modulus for the top fiber of the prestressed beam (in.3).
- DC1: Un-factored non-composite dead load (kips/ft.).
- MDC1: Un-factored moment due to non-composite dead load (kip-ft.).
- DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
- MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
- DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
- MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft).
- M½ + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).



tight only. PERMANENT BRACING DETAILS

\*Fabricator shall locate to miss strands within permissible tolerances.

\*\*Alternate MC18x45.8 channels are permitted to facilitate material acquisition.

 $133'-\frac{1}{2}''$  End to End Beam – @ Brg. South Abutment @ Brg. North Abutment -IL63-3838 Beam Bk. S. Abut. Abut. (3)-4)-(5)-(6)Beam @ Permanent Bracing -© Permanent Bracing -@ Permanent Bracing -No. typ. 32'-11" 32'-101/5" 32'-101/5" 32'-11" 2'-0" 131'-7" € Brg. to € Brg. 2'-4" 135'-11" Back to Back Abutment

#### FRAMING PLAN

All material for bracing shall be hot dip galvanized according to AASHTO M111 unless otherwise noted.

Two hardened washers are required for each set of oversized holes.

All holes shall be  $^{15}\!\!/_{16}$ "  $\oslash$  unless otherwise noted.

 $\frac{5}{16}$ " x 3" x 3" plate washers are required over all slotted holes.

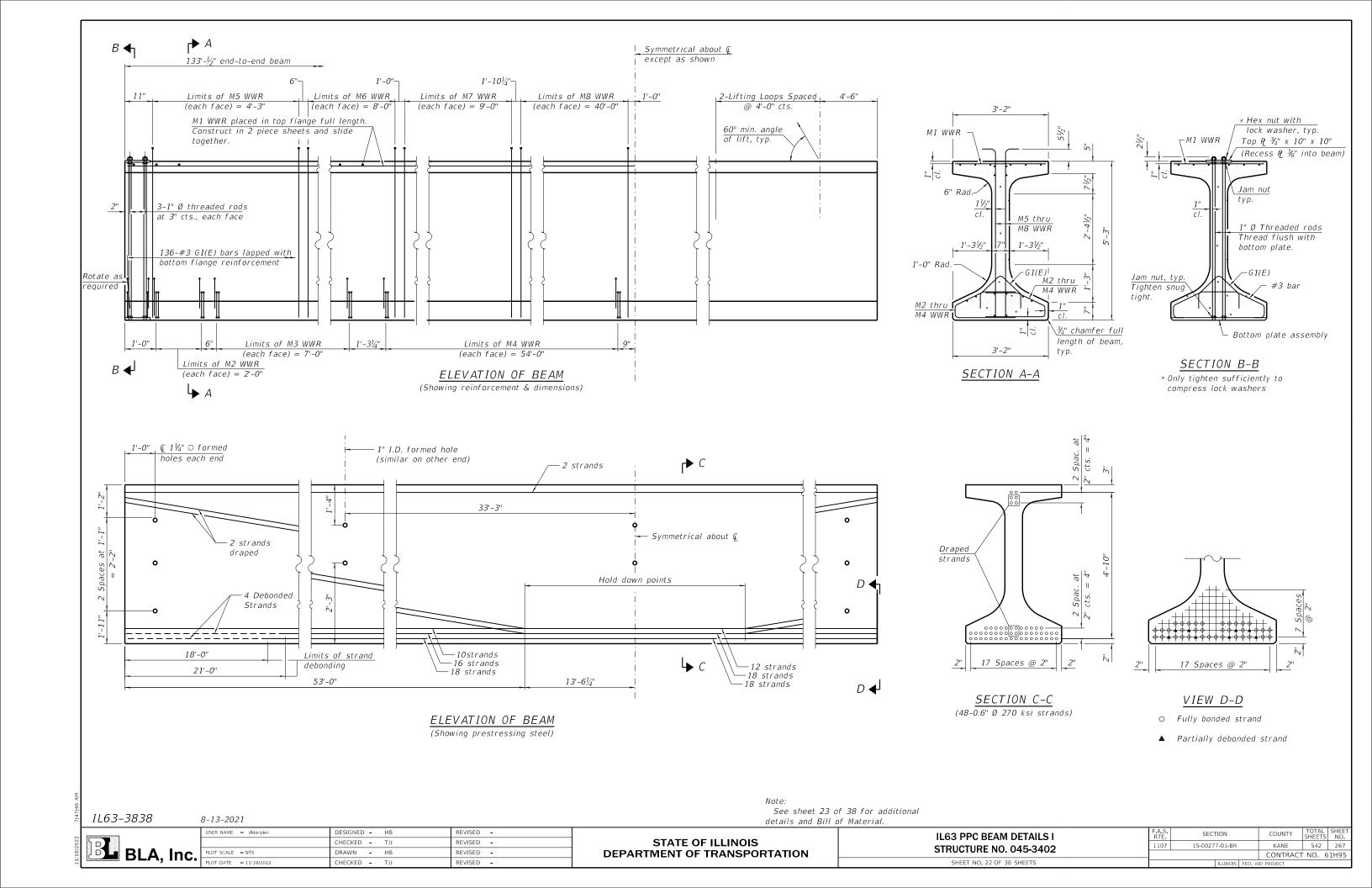
All bolts shall be galvanized according to AASHTO M232. Bracing shall be installed as beams are erected and tightened as soon as possible during erection.

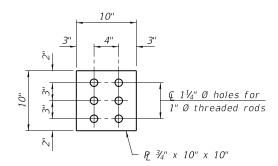
Permanent bracing shall not be paid for separately, but shall be included in the cost of Furnishing and Erecting Precast Prestressed Concrete Beams, IL63.

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		CHECKED - TJJ	REVISED -
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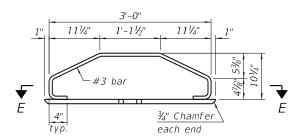
FRAMING PLAN STRUCTURE NO. 045-3402
SHEET NO. 21 OF 38 SHEETS

F.A.S. RTE	SECT	ΓΙΟΝ	COUNTY	TOTAL SHEETS	SHEE NO.
1107	15-0027	7-01-BR	KANE	542	266
			CONTRACT	NO. 6	1H95

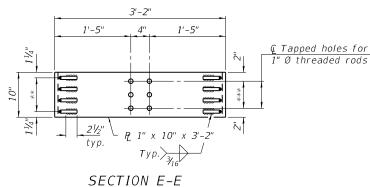




#### PLAN - TOP PLATE

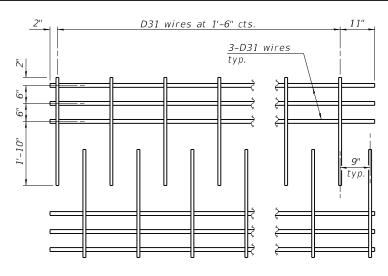


ELEVATION - BOTTOM PLATE ASSEMBLY



\*\* 3 Spaces at  $2\frac{1}{2}$ " =  $7\frac{1}{2}$ "

\*\*\* 2 Spaces at 3" = 6"



#### M1 WWR DETAIL

When multiple sheets of M1 WWR are required along the beam length, #5(E) bars (5'-0" long) shall be used to splice the longitudinal D31 wires together (Min. Lap 2'-2").

A-D31 wires

at B centers

2-W14 wires

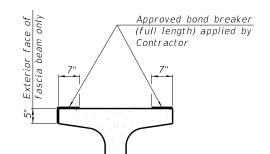
2-W14 wires

## TABLE OF DIMENSIONS

(The WWR designs assume grade 60. If necessary, this permits the fabricator to directly substitute grade 60 rebar as detailed in the Manual for Fabrication of Precast Prestressed Concrete Products.)

#### SPAN 1

WWR	Α	В
M2	9	3"
М3	15	6"
M4	37	1'-6"
M5	18	3"
М6	17	6"
M7	10	1'-0"
M8	21	2'-0"



SECTION THRU TOP FLANGE (Showing limits of bond breaker)

NOTES

Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand,

The beams shall have a final concrete compressive strength, f'c, of 8500 psi and

Bend the extended strands inward on the fascia beams to maintain  $1\frac{1}{2}$ " clearance

A minimum  $2\frac{1}{2}$ " Ø lifting pin shall be used to engage the lifting loops during handling.

Welded Wire Reinforcement (WWR) shall conform to ASTM A884 with a Class A, Type 1

The top plates and bottom plate assemblies shall be galvanized according to AASHTO M111. The threaded rods, nuts and washers shall be galvanized according to AASHTO M232.

Grade 270. The nominal diameter for beam strands shall be 0.6" and the nominal cross-sectional area shall be 0.217 sq. in. The nominal diameter for lifting loops shall

be  $\frac{1}{2}$ " and the nominal cross sectional area shall be 0.153 sq. in.

The top and bottom plates shall be AASHTO M270 Grade 50.

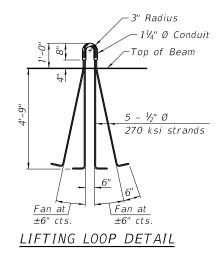
a release concrete compressive strength, f'ci, of 6500 psi.

Threaded rods shall be ASTM F 1554 Grade 55.

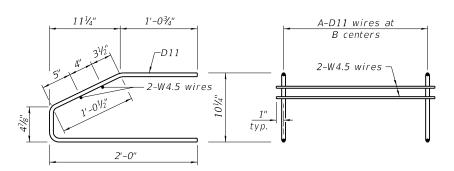
epoxy coating or ASTM A1060, Table 3 galvanized coating.

inside the pier diaphragm.

Inserts for ¾" Ø threaded dowel rods, when specified, are to be two strut, ferrule type for interior beams and single ferrule, flared loop type for exterior beams.



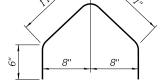
#### M5 THRU M8 WWR DETAIL (See Table of Dimensions)



#### M2 THRU M4 WWR DETAIL (See Table of Dimensions)

# BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete Beams, IL63	Ft.	932



 $BAR\ G1(E)$ 

8-13-2021 DESIGNED - HB REVISED -CHECKED - TJJ REVISED -DRAWN - HB REVISED -CHECKED - TJJ REVISED -

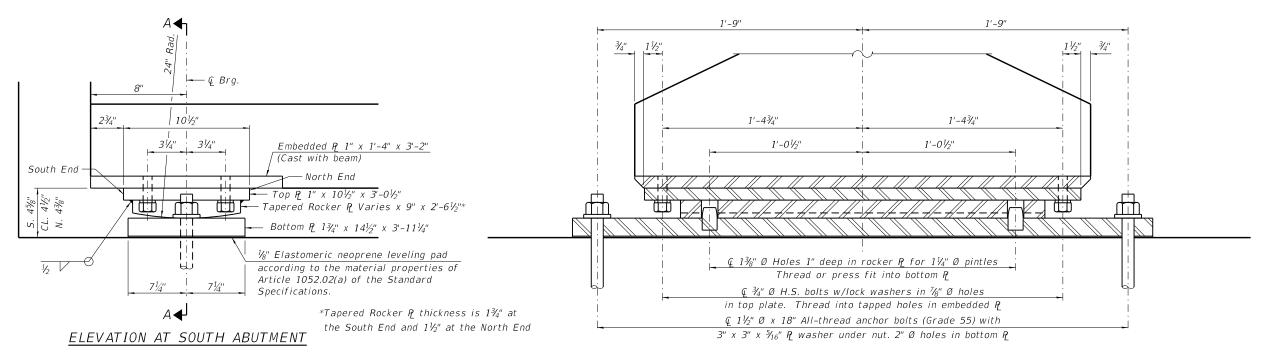
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  **IL63 PPC BEAM DETAILS II STRUCTURE NO. 045-3402** SHEET NO. 23 OF 38 SHEETS

SECTION COUNTY 1107 15-00277-01-BR KANE 542 268 CONTRACT NO. 61H95

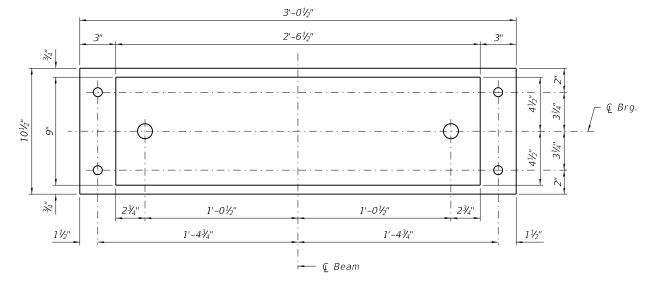
IL63-3838D

BLA, Inc.

2-W14 wires

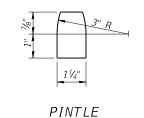


#### SECTION A-A





PLAN OF BOTTOM PLATE



# 3'-111/4" 81/2" 81/2" 25/8" 25/8" 1'-01/2" 1'-01/2" - Q Beam

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts as fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place and prior to poring the deck.

Drilled and set achor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Anchor bolts shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

See sheet 23 of 38 for additional details of embedded plate. All plates, hardware, and leveling pads required for the bearing, except anchor bolts, shall be included in the cost of Furnishing and Erecting Precast Prestressed Concrete IL63 Beams.

The structural steel plates and pintles of the Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50.

All plate material for bearings shall be hot dip galvanized according to AASHTO M111.

All bolts and washers shall be galvanized according to AASHTO M232.

#### BILL OF MATERIAL

Item	Unit	Total
Anchor Bolts, $1\frac{1}{2}$ "	Each	14

PI-2FB

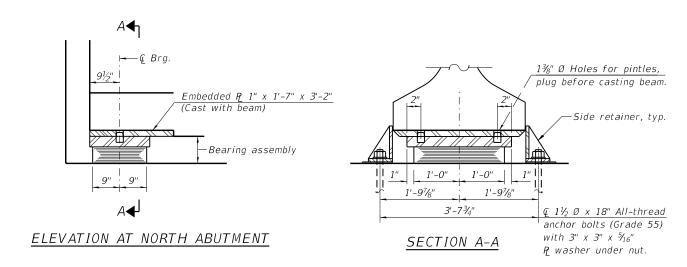
6-15-2019

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		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

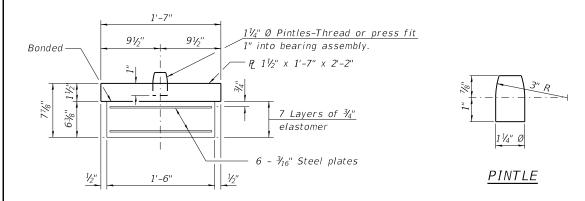
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

**FIXED BEARING DETAILS STRUCTURE NO. 045-3402** SHEET NO. 24 OF 38 SHEETS

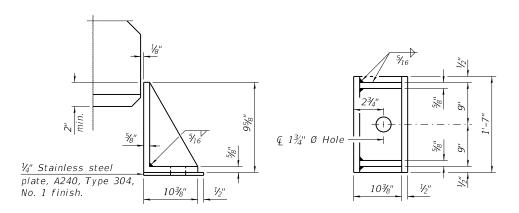
SECTION 15-00277-01-BR KANE 542 269 CONTRACT NO. 61H95



#### TYPE I ELASTOMERIC EXP. BRG.



#### BEARING ASSEMBLY



#### SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

#### Notes:

Side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.

See sheet 23 of 38 for additional details of embedded plate.

Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

All exposed bearing plates and side retainers shall be hot dip galvanized according to AASHTO M111.

#### BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	7
Anchor Bolts, 1½"	Each	14

PI-2E-1

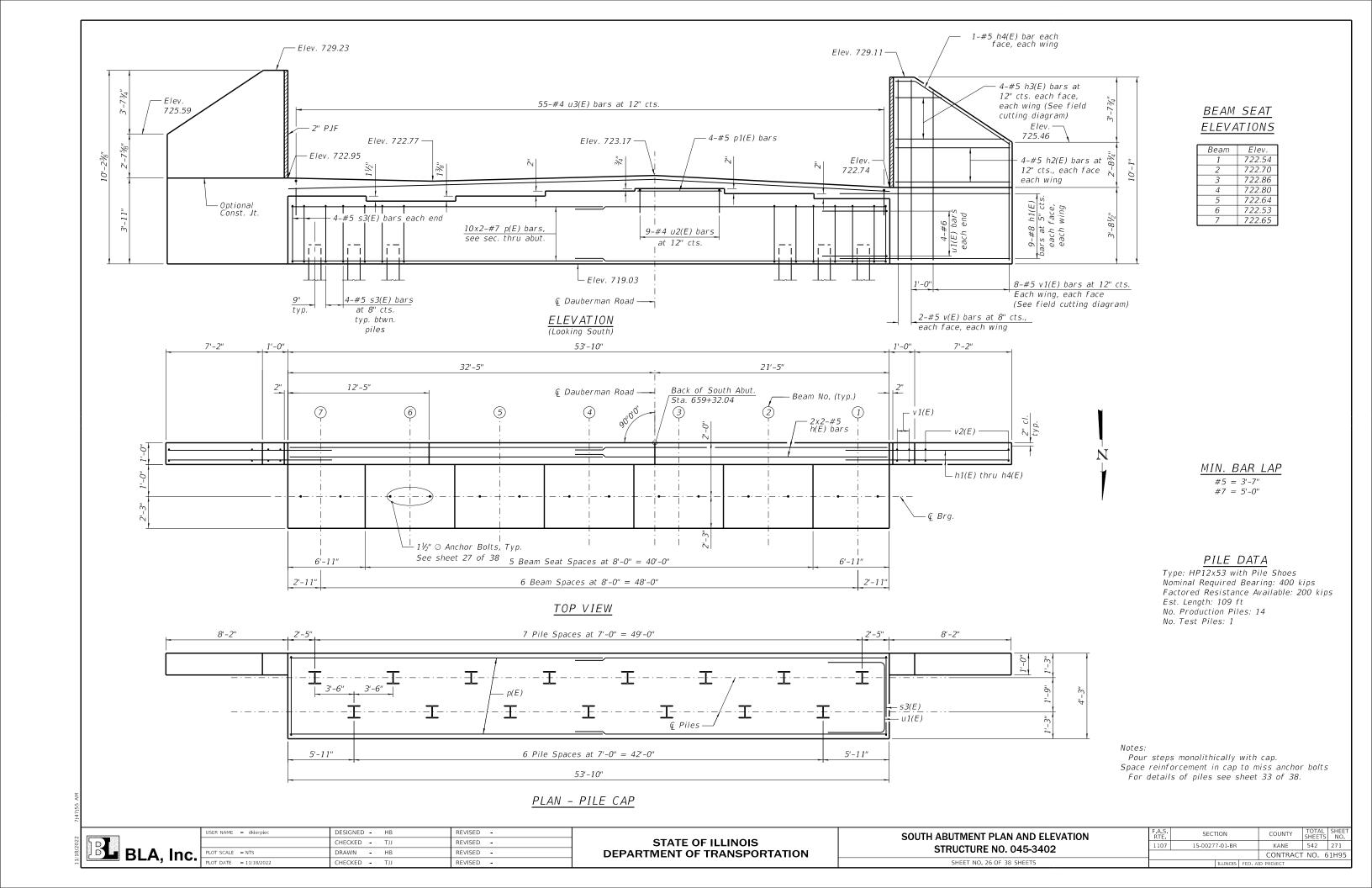
6-15-2019

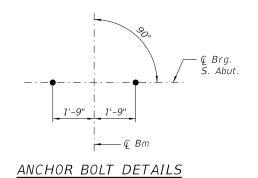
40/4044			
4474074	BLA,	Inc.	

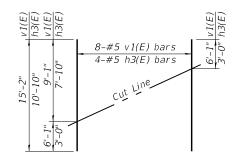
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		CHECKED - TJJ	REVISED -
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•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

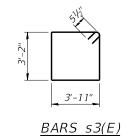
ELASTOMERIC BEARING DETAILS STRUCTURE NO. 045-3402	
SHEET NO. 25 OF 38 SHEETS	

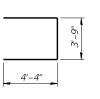
F.A.S. RTE.	SECTION			COUNTY	TOTAL SHEETS	SHEE NO.
1107	15-00277-01-BR			KANE	542	270
				CONTRACT	NO. 6	51H95
	TILINOIS	EED	Δ	ID PROJECT		



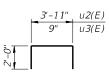








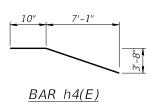
*BAR u1(E)* 

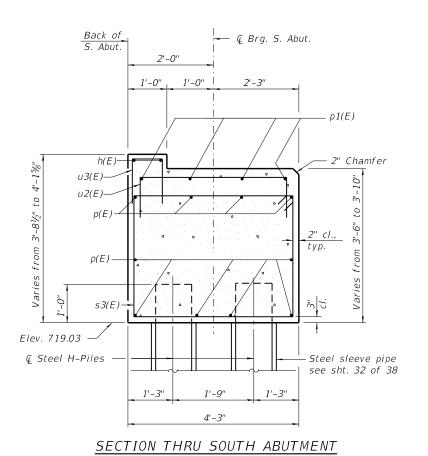


BAR u2(E) & u3(E)



Order v1(E) and h3(E) bars full length. Cut as shown and use remainder of bars in opposite face.





# SOUTH ABUTMENT BILL OF MATERIAL

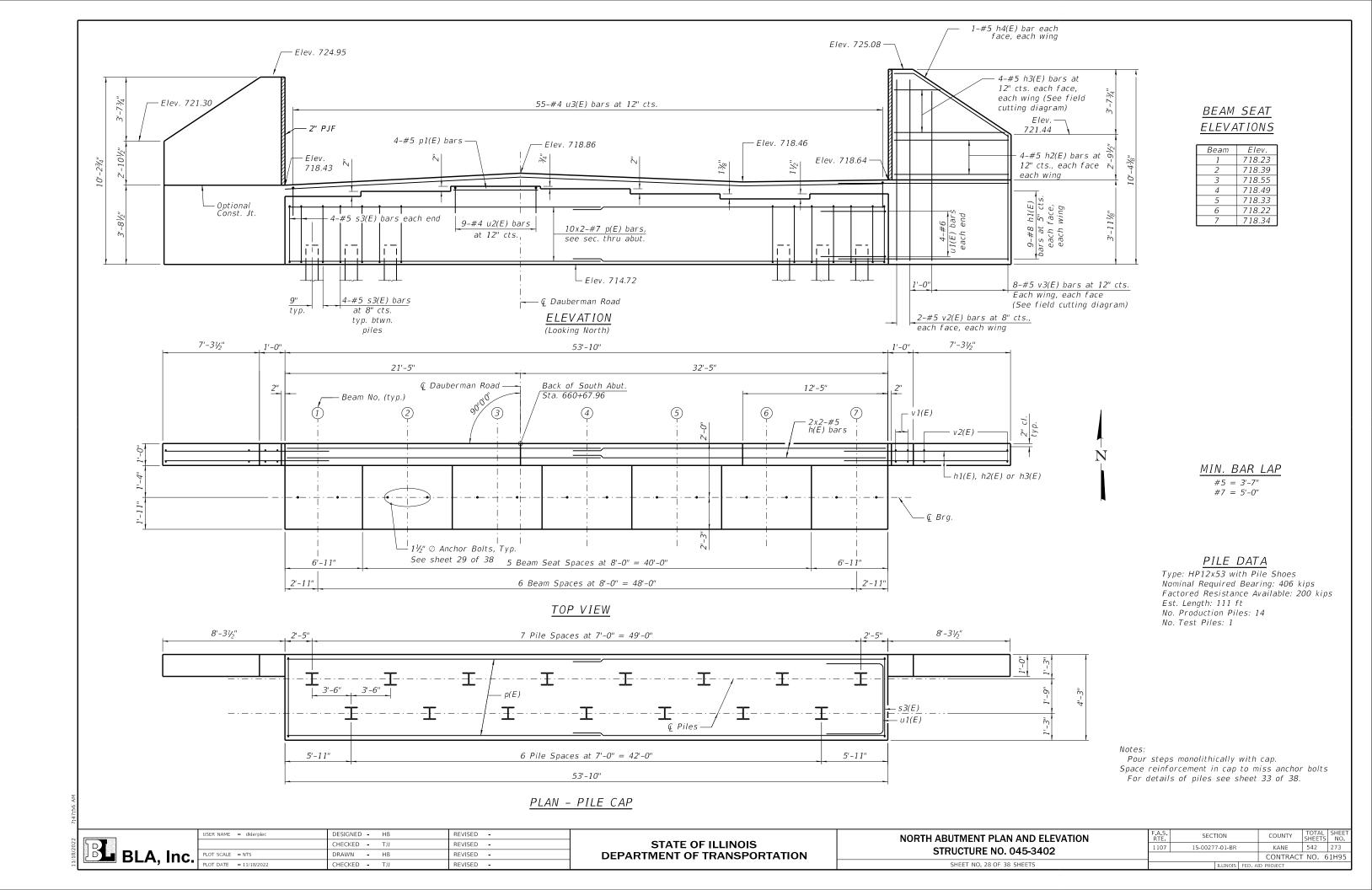
BILL OF MATERIAL				
Bar	No.	Size	Length	Shape
h(E)	4	#5	28'-7"	
h1(E)	36	#8	13'-7"	
h2(E)	16	#5	8'-0"	
h3(E)	8	#5	10'-10"	
h4(E)	4	#5	8'-10"	
p(E)	20	#7	29'-3"	
p1(E)	4	#5	7'-9"	
s3(E)	64	#5	15'-1"	<u></u>
u1(E)	8	#6	9'-7"	
u2(E)	9	#4	7'-11"	
u3(E)	55	#4	4'-9"	
ν(E)	8	#5	9'-9"	
v 1(E)	16	#5	15'-2"	
Concrete			Cu. Yd.	36.6
Reinforce		ars,	Pound	4,600
Ероху Сс			7 04774	4,000
Furnishing Steel Piles HP 12x53			Foot	1,526
			Foot	1 5 2 6
Driving F		) v E 2	Each	1,526
est Pile			Eacn	1
Geocompo Drain	isite Wa	311	Sq. Yd.	66
71 GIII				

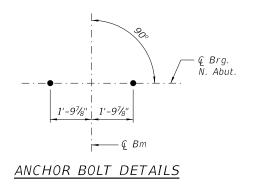
For details of piles see sheet 33 of 38.

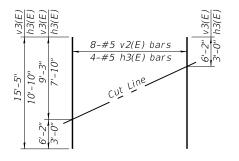
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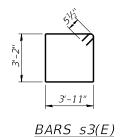
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		CHECKED - TJJ	REVISED -	
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'•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -	

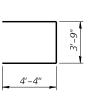
F.A.S. RTE.	SECT	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEET NO.
1107	15-0027	7-01-BR		KANE	542	272
				CONTRACT	NO. 6	1H95
		ILLINOIS	FED. A	ID PROJECT		



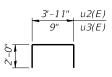




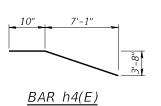




*BAR u1(E)* 

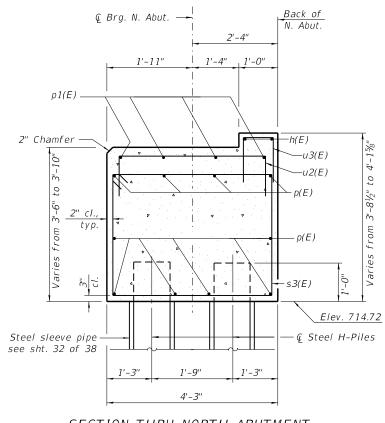


BAR u2(E) & u3(E)



## FIELD CUTTING DIAGRAM

Order v3(E) and h3(E) bars full length. Cut as shown and use remainder of bars in opposite face.



SECTION THRU NORTH ABUTMENT

## NORTH ABUTMENT BILL OF MATERIAL

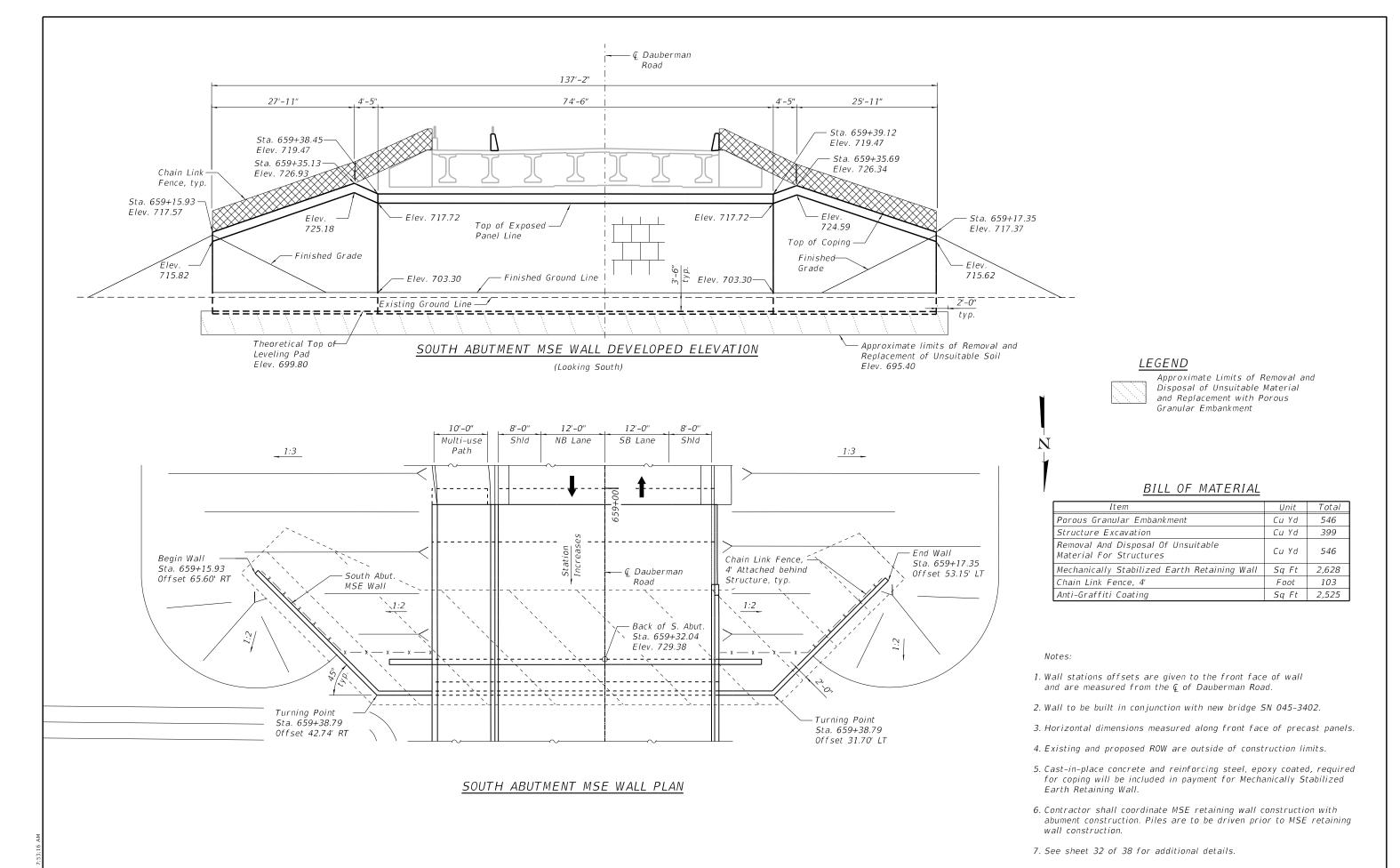
<u> </u>	JILL (	)	11 L 111/1	<u> </u>
Bar	No.	Size	Length	Shape
h(E)	4	#5	28'-7"	
h1(E)	36	#8	13'-7"	
h2(E)	16	#5	8'-0"	
h3(E)	8	#5	10'-10"	
h4(E)	4	#5	8'-10"	
p(E)	20	#7	29'-3"	
p1(E)	4	#5	7'-9"	
s3(E)	64	#5	15'-1"	
u1(E)	9	#6	9'-7"	
u2(E)	24	#4	7'-11"	
u3(E)	55	#4	4'-9"	
v2(E)	8	#5	9'-10"	
v3(E)	16	#5	15'-5"	
Concrete			Cu. Yd.	36.7
Reinforce		ars,	Pound	4,700
Ероху Со	pated		1 ound	4,700
Furnishii		Foot	1,554	
HP 12x5.	3			
Driving F		Foot	1,554	
est Pile	, HP 12	Each	1	
Geocompo	osite Wa	Sq. Yd.	66	
Drain			Jq. ru.	

For details of piles see sheet 33 of 38.

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		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

F.A.S. RTE.	SECT	ΓΙΟΝ	COUNTY	TOTAL SHEETS	SHEET NO.	
1107	15-0027	7-01-BR		KANE	542	274
			CONTRACT	NO. 6	1H95	
		ILLINOIS	ID PROJECT			



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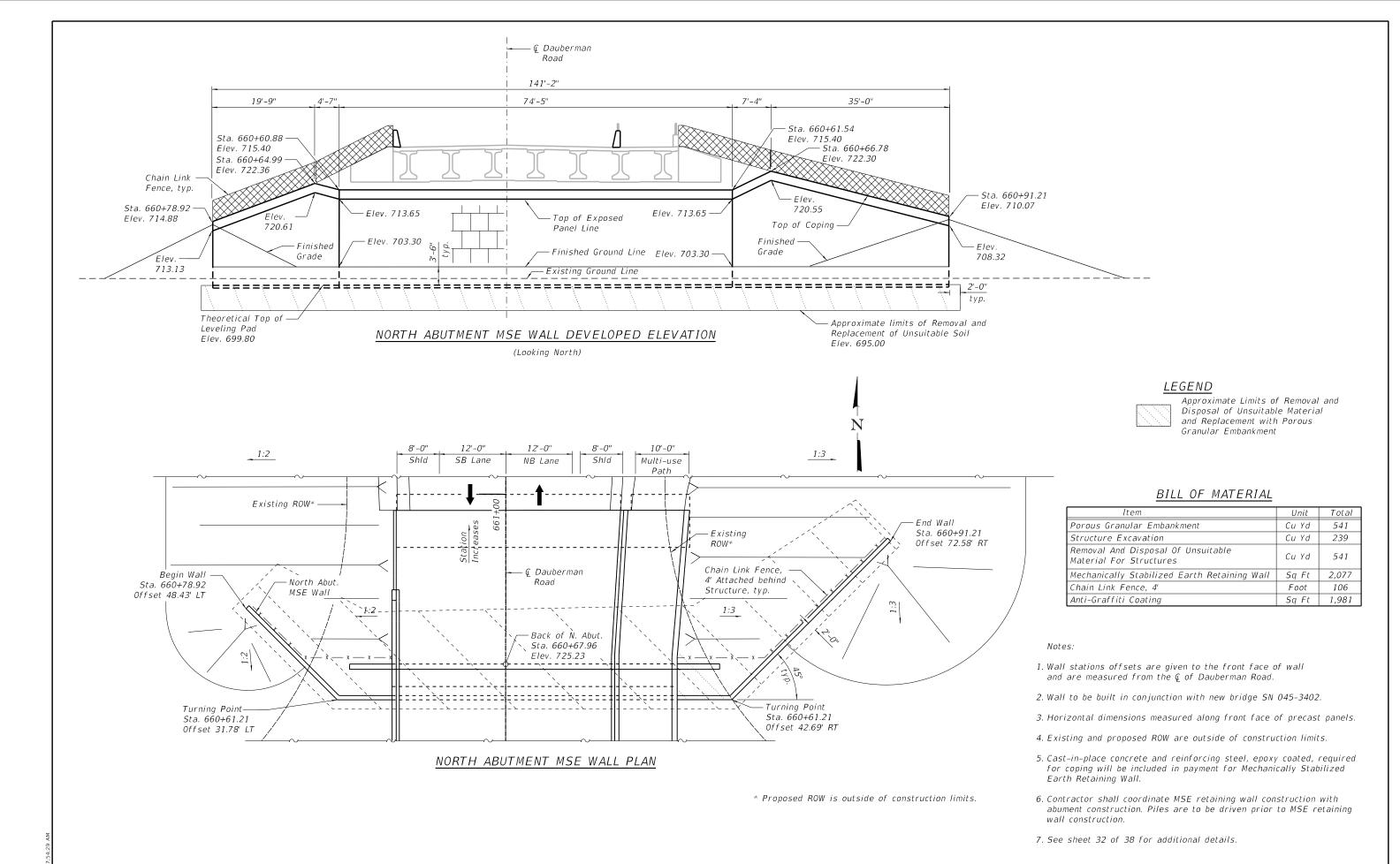
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOUTH ABUTMENT MSE WALL PLAN STRUCTURE NO. 045-3402 SHEET NO. 30 OF 38 SHEETS  
 F.A.S. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEE'S NO.

 1107
 15-00277-01-BR
 KANE
 542
 275

 CONTRACT NO. 61H95

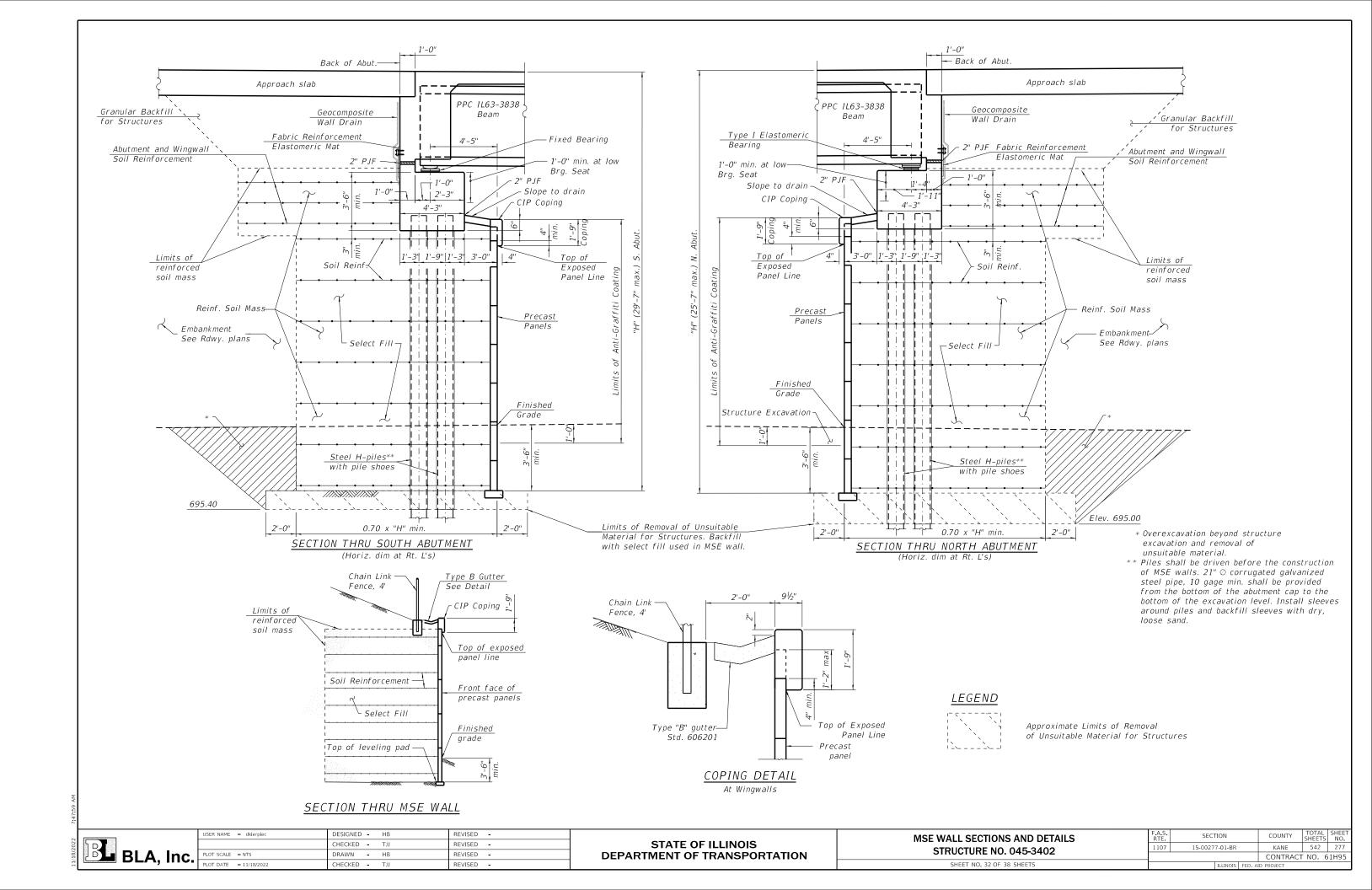
 ILLINOIS FED. AID PROJECT

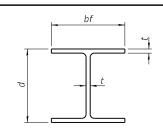


BLA, Inc.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

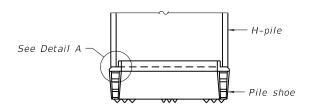
NORTH ABUTMENT MSE WALL PLAN STRUCTURE NO. 045-3402 SHEET NO. 31 OF 38 SHEETS F.A.S. SECTION COUNTY TOTAL SHEETS NO. 1107 15-00277-01-BR KANE 542 276 CONTRACT NO. 61H95



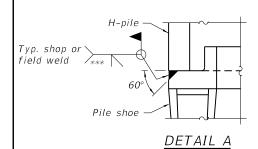


#### STEEL PILE TABLE

Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14½"	14 <sup>7</sup> / <sub>8</sub> "	13/ <sub>16</sub> "	30"
x102	1 4"	14¾"	11/ <sub>16</sub> "	30"
x89	13½"	1 4 3/4"	5/8"	30"
x73	13%"	145/8"	1/2"	30"
HP 12x84	121/4"	121/4"	11/ <sub>16</sub> "	24"
x74	12½"	121/4"	5/8"	24"
x63	12"	121/8"	1/2"	24"
x53	1 1 3/4"	12"	7/ <sub>16</sub> "	24"
HP 10x57	10"	101/4"	%16"	24"
x42	9¾"	101/8"	7/ <sub>16</sub> "	24"
HP 8x36	8"	8½"	7/ <sub>16</sub> "	18"



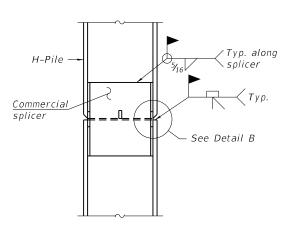
#### ELEVATION

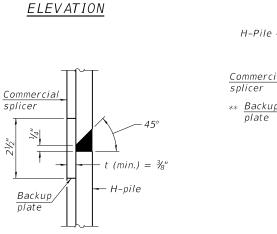


#### SHOE ATTACHMENT

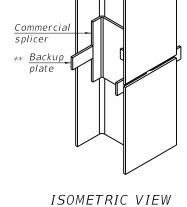
Note:

The steel H-piles shall be according to AASHTO M270 Grade 50.

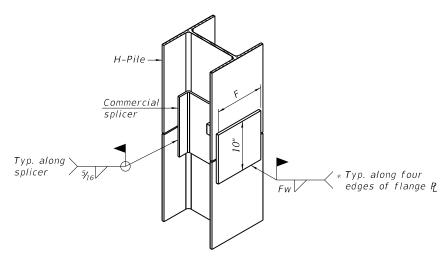




DETAIL "B'



#### WELDED COMMERCIAL SPLICE

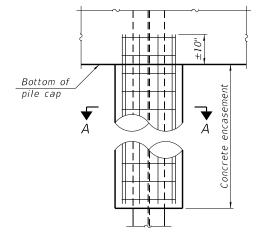


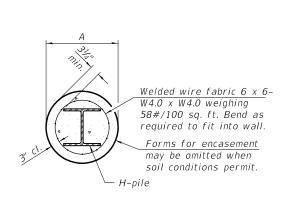
#### ISOMETRIC VIEW

#### WELDED COMMERCIAL SPLICE ALTERNATE

- $_{*}$  Interrupt welds  $\mathcal{V}_{\!\!4}$ " from end of web and/or each flange.
- \*\* Remove portions of backup plates that extend outside the flanges.

\*\*\* Weld size per pile shoe manufacturer (5/16" min.).



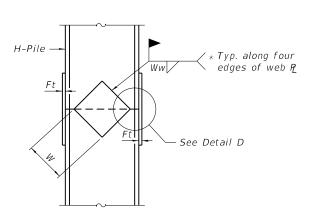


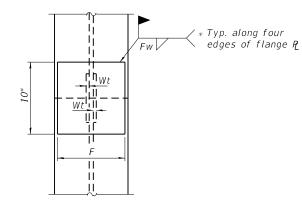
ELEVATION

SECTION A-A

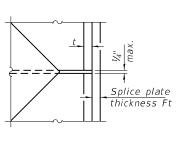
## <u>INDIVIDUAL PILE</u> <u>CONCRETE ENCASEMENT</u>

(when specified)





<u>ELEVATION</u>



DETAIL D

END	VIEW

Designation	F	Ft	Fw	W	Wt	Ww
HP 14x117	12½"	1"	7/8"	7¾"	5/8"	1/2"
x102	12½"	7/8"	3/4"	73/4"	5/8"	1/2"
x89	12½"	3/4"	11/ <sub>16</sub> "	73/4"	5/8"	1/2"
x73	12½"	5/8"	%16"	73/4"	5/8"	1/2"
HP 12x84	10"	7/8"	11/ <sub>16</sub> "	6½"	5/8"	1/2"
x74	10"	7/8"	11/ <sub>16</sub> "	6½"	5/8"	1/2"
x63	10"	5/8"	1/2"	6½"	1/2"	3/8"
x53	10"	5/8"	1/2"	6½"	1/2"	3/8"
HP 10x57	8"	3/4"	%16"	5½"	1/2"	3/8"
x42	8"	5/8"	%16"	5½"	1/2"	3/8"
HP 8x36	7"	5/8"	<sup>7</sup> / <sub>16</sub> "	41/4"	1/2"	3/8"

#### WELDED PLATE FIELD SPLICE

F-HP

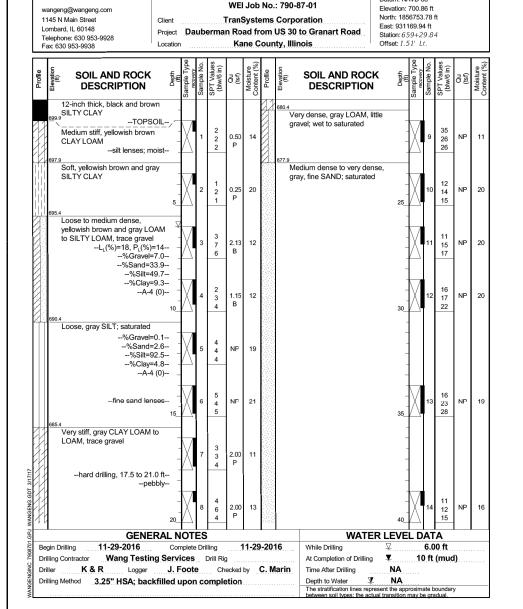
1-1-2020

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HP PILE DETAILS STRUCTURE NO. 045-3402 SHEET NO. 33 OF 38 SHEETS 
 F.A.S. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS NO.

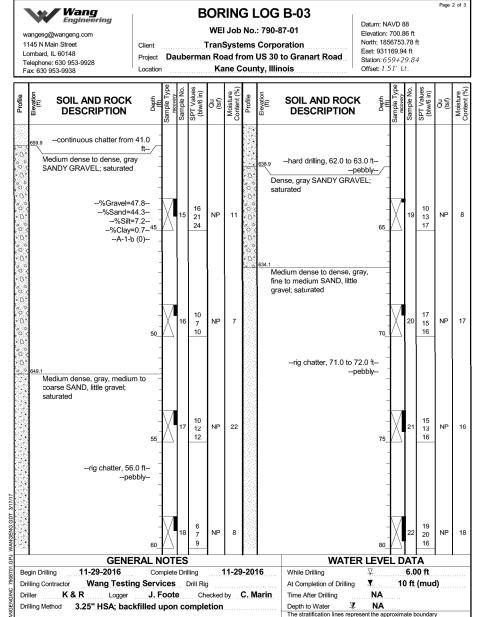
 1107
 15-00277-01-BR
 KANE
 542
 278

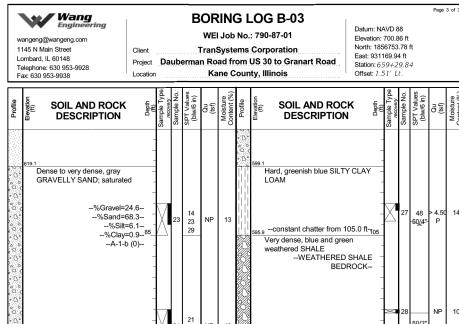
 CONTRACT NO. 61H95



**BORING LOG B-03** 

Datum: NAVD 88





Drilling Contractor Wang Testing Services Drill Rig						At Completion of Drilling	10 f	t (muc	(k	
Begin Drilling 11-29-20	SENERAL NOTES 16 Complete Di	_	1	1-29	-2016	WATER LEV		1 A .00 ft		
3.9	100_	igsqcup				WATER LEV				
0. 0. 0.	26	50/5"	NP	15			1			
GRAVEL; saturated							1			
604.1 Very dense, gray SAN	IDY -						1			
	95	37				115	]			
	- 25	25 28	NP	19			1			
Very dense, gray med to SANDY LOAM, tra saturated							1			
0.° - 0.09.1	-						1			
3 c C-	90	24			590.9 Bo	oring terminated at 110.00 ft	]			
2 ( C*	-	21	NP	13			2	50/2"	NP	1
0; 0;	]									
0* - {	-					WEATHERED SHALE BEDROCK	-			
3 0	A-1-b (0)					ery dense, blue and green eathered SHALE	4 1			ı

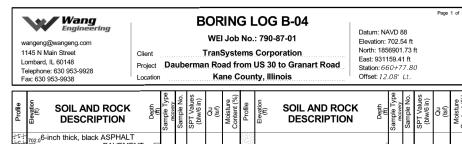
Drilling Method 3.25" HSA; backfilled upon completion

BLA,	Inc.
DLA,	1116

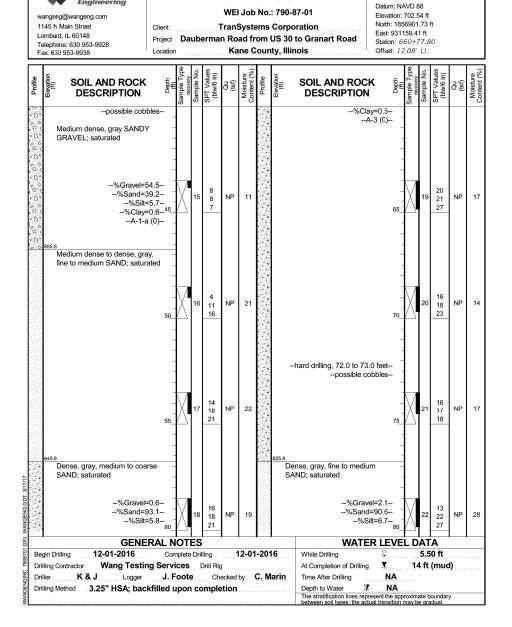
Wang Engineering

	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

Depth to Water The stratification lines represe



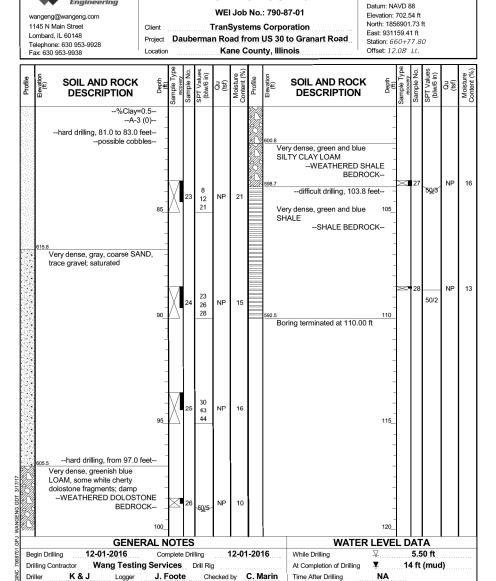
Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile		SOIL AND R DESCRIPT		Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
##. #. O.	701.5	8-inch thick, black ASPHALTPAVEMENT 6-inch thick, SANDY GRAVELBASE COURSE Dense, brown SANDY GRAVEIFILL		X	1	6 9 21	NP	4						X	9	11 17 17	NP	12
		Stiff, brown and gray SILTY CLAY LOAM to CLAY LOAM	5_	X	2	4 3 4	1.15 B	13					25_	X	10	8 18 13	NP	20
		oose, brown SILTY LOAM to OAM; saturated		X	3	3 3 4	NP	15		674.5			-	X	11	15 17 20	NP	19
		oose to medium dense, gray CLAY LOAM to LOAM, trace gravel	10	X	4	8 7 7	1.00 P	11			um dense, gray ated sand lenses		30	X	12	12 15 14	NP	20
	689.5		-	X	5	2 2 3	0.75 B	12			e, gray, fine to n ); saturated	nedium	-					
	8	.cose to medium dense, gray SILTY to SILTY LOAM; wet to saturated	15	X	6	5 5 4	NΡ	22					35_	X	13	7 5 3	NP	20
17/17	1684.5			X	7	6 4 7	NP	22			um dense, gray, D; saturated	coarse	-					
WANGENGINC 7908701.GPJ WANGENG.GDT 3/17/17	f	Medium dense to dense, gray ine SAND to SANDY LOAM; saturated	- - 20_	X	8	5 10 9	NP	12		662.5	-hard drilling fro	m 40.0 feet-	- - - 40	X	14	13 12 10	NP	15
1.GP,		GENERA										ATER LE	VE					
02 B	egin Dr	•		plete		-		2-01	-20		While Drilling					0 ft		
D 78	-	Contractor Wang Testing				-					At Completion of D			14	ft	(mud	1)	
D G	riller	K&J Logger							C. N		Fime After Drilling		IA					
ANGE	rilling M	Method 3.25" HSA; backfil	lled u	por	ı cc	mple	tion			Т	Depth to Water The stratification line	es represent the	IA e appr	roxima	ite b	oundar	у	
≥		***************************************								b	etween soil types: 1	ne actual trans	ition n	nay be	gra	dual.		



**BORING LOG B-04** 

Wang Engineering Page 2 of 3

Wang Engineering



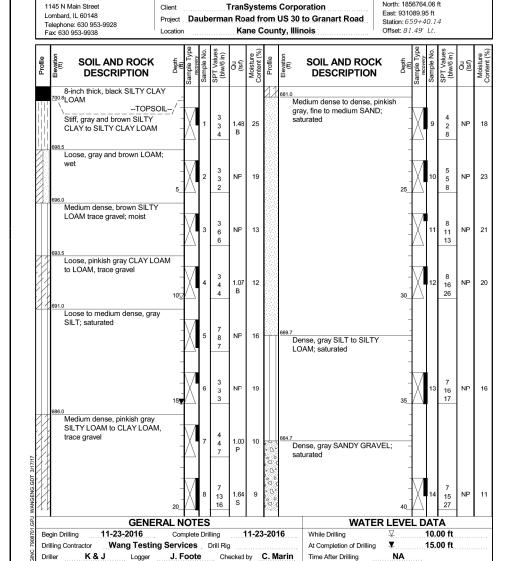
**BORING LOG B-04** 



	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

Drilling Method 3.25" HSA; backfilled upon completion

Depth to Water The stratification lines represe

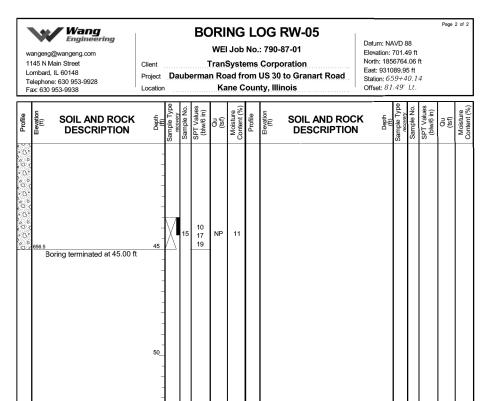


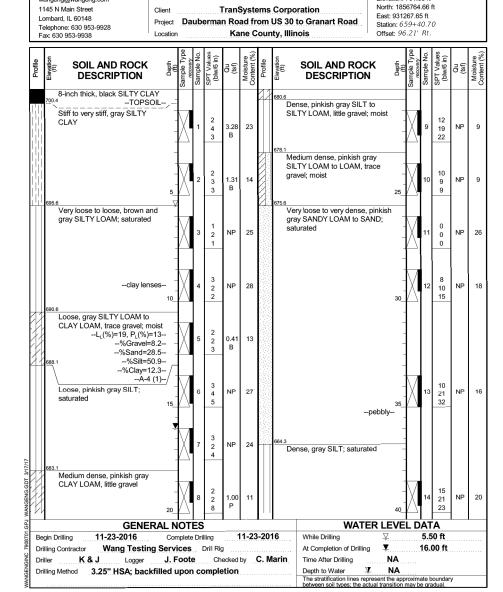
**BORING LOG RW-05** 

WEI Job No.: 790-87-01

Datum: NAVD 88

Flevation: 701 49 ft





**BORING LOG RW-06** 

WEI Job No.: 790-87-01

Datum: NAVD 88

Elevation: 701.06 ft

Wang Engineering

Drilling Method 3.25" HSA; backfilled upon completion

Wang Engineering

	USER NAME = dkierpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

11-23-2016

While Drilling

At Completion of Drilling

Time After Drilling

Depth to Water The stratification lines rep

WATER LEVEL DATA

10.00 ft

15.00 ft

**GENERAL NOTES** 

Complete Drilling

K & J Logger J. Foote Checked by C. Marin

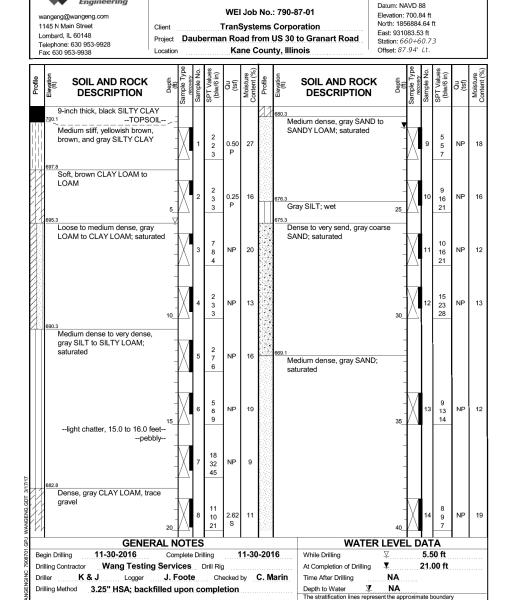
11-23-2016

Drilling Contractor Wang Testing Services Drill Rig

Drilling Method 3.25" HSA; backfilled upon completion

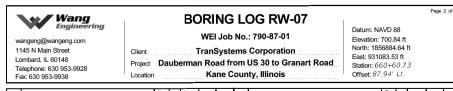


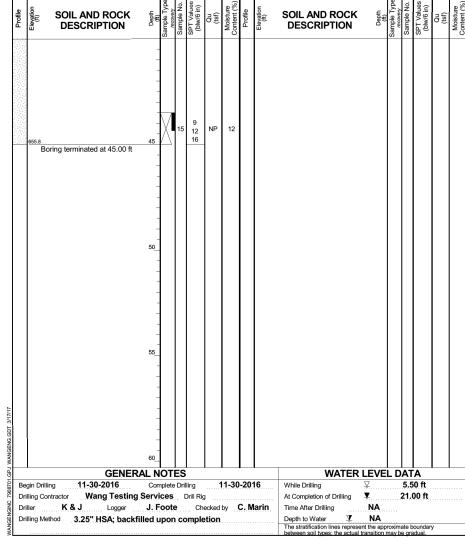
Elevation	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	SPT Values (blw/6 in)	Qu (tsf)	Moisture
659.	.3 Dense, gray GRAVEL; saturated	-												
o C	,	1												
656.	i.1	45	15	12 12 21	NP	10								
	Boring terminated at 45.00 ft													
		1												
		50												
		-												
		55												
		-												
		60_												
	GENERA	T NO.	TES	;	-		-		WATER I				-	_
-	Drilling 11-23-2016 G Contractor Wang Testing S	Comple Service	ete Dri	lling Drill Rig	9				While Drilling	<u>▼</u>	5	.50 ft 6.00 ft		
Driller Drilling	K & J Logger  Method 3.25" HSA; backfill						C. M	larin	Time After Drilling  Depth to Water  The stratification lines represent between soil types; the actual tr	NA NA				



**BORING LOG RW-07** 

Wang Engineering





ВΙΛ	Ino
BLA,	inc.

	USER NAME = dklerpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
	PLOT SCALE = NTS	DRAWN - HB	REVISED -
•	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -



1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928 Fax: 630 953-9938

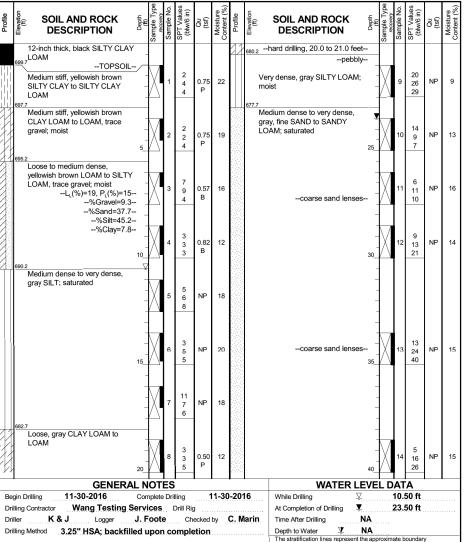
#### **BORING LOG RW-08**

WEI Job No.: 790-87-01

TranSystems Corporation Kane County, Illinois

Datum: NAVD 88 Elevation: 700.72 ft North: 1856888.77 ft East: 931264.05 ft Station: 660+64.81 Offset: 92.58' Rt.

Page 1 of 2





wangeng@wangeng.com 1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928 Fax: 630 953-9938

#### **BORING LOG RW-08**

WEI Job No.: 790-87-01

TranSystems Corporation Client Kane County, Illinois

Datum: NAVD 88 Elevation: 700.72 ft North: 1856888.77 ft East: 931264.05 ft Station: 660+64.81 Offset: 92.58' Rt.

	Profile		ND ROCK RIPTION	Depth (ft)	Sample Type recovery Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
				-												
		655.7		45	15	7 10 11	NP	15								
		Boring termina	ated at 45.00 ft	-												
				50_												
				-												
				- 55												
				-												
.GDT 3/17/17				-												
WANGENGINC 7908701.GPJ WANGENG.GDT 3/17/17			GENER <i>A</i>	60_ AL NC	DTES						WATER	LEVE	L DA	ГА		
.10280	Beg		-30-2016	Comp	lete Dr	lling		1-30	-201	16	While Drilling	₽	10	50 ft		
IC 790			Wang Testing S								At Completion of Drilling	¥	23	50 ft		
NGIN		ller K&J	Logger						C. M	arin	Time After Drilling	NA				
ANGE	Dri	Iling Method 3.2	5" HSA; backfil	led up	on c	omple	etion				Depth to Water  The stratification lines repres	NA ent the app	roximate	boundar	у	
>											perween soil types; the actua	uansition r	nav be di	aduai.		

BLA,	Inc.
,	

	USER NAME = dklerpiec	DESIGNED - HB	REVISED -
		CHECKED - TJJ	REVISED -
•	PLOT SCALE = NTS	DRAWN - HB	REVISED -
<b>,</b> .	PLOT DATE = 11/18/2022	CHECKED - TJJ	REVISED -

	F.A.S. RTE.	SEC <sup>-</sup>	COUNTY	TOTAL SHEETS	SHE		
	1107	15-0027	KANE	542	283		
_			CONTRACT	NO. 6	1H9		
			ILLINOIS	FED. A	D PROJECT		

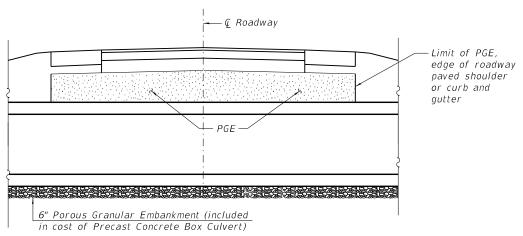
## BOX CULVERT DATA & POROUS GRANULAR EMBANKMENT SCHEDULE

Culvert Number	Station	Culvert Size (Span x Rise)	Cu. Yd.
1	Garnart Rd Sta. 628+16.69	3' x 2'	211
2	Dauberman Rd Sta. 636+51.94	4' x 2'	222
3	Bike Path Sta. 636+51.95	4' x 2'	26
4	Dauberman Rd Sta. 636+66.94	5' x 3'	208
5	Bike Path Sta. 636+66.94	5' x 3'	37
6	Dauberman Rd Sta. 649+50.00	4' x 2'	128
7	Dauberman Rd Sta. 651+50.00	4' x 2'	149
8	Dauberman Rd Sta. 655+92.00	4' x 2'	147
9	Dauberman Rd Sta. 666+21.23	5' x 2'	210
10	Dauberman Rd Sta. 666+38.13	4' x 3'	475
11	US 30 Sta. 354+70.27	7' x 3'	375

#### INDEX OF SHEETS

- 1. General Data Culvert No. 1 Thru Culvert No. 11
- 2. General Plan and Elevation Culvert No. 11
- 3. Stage Construction Details Culvert No. 11
- 4. Box Culvert End Section Details Culvert No. 11 Upstream
- 5. Box Culvert End Section Details Culvert No. 11 Upstream
- 6. Box Culvert End Section Details Culvert No. 11 Downstream
- 7. Box Culvert End Section Details Culvert No. 11 Downstream 8. Single Cell Precast Box Culvert Tapered End Sections Culvert
- 9. Single Cell Precast Box Culvert Tapered End Sections Culvert No. 1 Thru 8
- 10. Box Culvert End Section Details Culvert No. 9 & 10 Downstream
- 11. Box Culvert End Section Details Culvert No. 9 Upstream 12. Box Culvert End Section Details Culvert No. 10 Upstream
- 13. Box Culvert End Section Details Culvert No. 9 & 10
- 14. Box Culvert End Section Details Culvert No. 9 & 10
- 15. Box Culvert Boring Location Plan
- 16. Boring Logs
- 17. Boring Logs
- 18. Boring Logs
- PGE above top of box culvert where specified 19. Boring Logs 20. Boring Logs Roadway subgrade -Top of Pavement —

Top of Precast Concret Box Culvert Porous Granular Embankment 2'-0" 2'-0" CULVERT SECTION



#### CULVERT LONGITUDINAL SECTION

Work shown in the detail shall be performed in accordance with applicable portions of Section 207 and Section 540 of the Standard Specifications.

Precast concrete box culverts and box culvert end sections shall be backfilled with Porous Granular Embankment to the top of the box culvert extending to a vertical plane 2 ft from the exterior sides of the culvert, 2 ft from the back face of the end sections, and not closer than 2 ft from the face of embankment. Include additional backfill with Porous Granular Embankment with a 1:1 slant slope extending to above the top of box culverts as shown in Culvert Section.

Porous Granular Embankment shall be placed above the top of box culverts to the bottom of the subgrade, for the lateral limit shown at Culverts No. 1, 2, 4, 9, 10 and 11.

#### GENERAL NOTES

See Drainage plans for additional culvert details.

Any information concerning type or location of underground and other utilities is not guaranteed to be accurate or all inclusive. The Contractor is responsible for making his own determinations as to the type and location of the utilities as may be necessary to avoid damage thereto. Contractor shall call J.U.L.I.E. prior to excavation.

Structural seal does not include design of precast elements.

Exposed edges shall have a 3/4" chamfer.

Cover from the face of cast-in-place concrete to face of reinforcement bars shall be 3" for surfaces cast against earth and 2" for all other surfaces, unless otherwise noted.

The size specified shall conform to the applicable requirements of Section 540 of the Standard Specification and ASTM C 1577, and the

Precast box culverts shall be cast without bells and spigots on the ends at cast-in-place end sections.

The 6 in. thick layer of porous granular material required for precast concrete box culverts per Article 540.06 of the Standard Specifications shall also apply to the end sections. Cost of the porous granular material will not be paid for separately but shall be included in the unit price of the work for which it is required.

Nonwoven geotextile fabric shall conform to the requirements of Article 1080.01 of the Standard Specifications. The minimum weight of the fabric shall be 6 ounces per square yard.

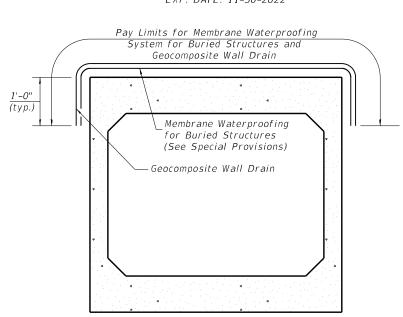
Cost of culvert wingwalls are included in the Box Culvert End Sections for the respective culvert.

Geocomposite Wall Drain shall be according to Section 591 of the Standard Specifications, except that concrete nails shall not be used in areas where it overlaps Membrane Waterproofing System for Buried



I certify that to the best of my knowledge, information and belief, this design is structurally adequate for the design loading shown on the plans. The design is an economical one of the style of structure and complies with the requirements of the current AASHTO LRFD Bridge Design Specifications.

DATE SIGNED: 11-17-2022 EXP. DATE: 11-30-2022



PRECAST BOX CULVERT MEMBRANE WATERPROOFING SYSTEM

For Box Culvert Nos. 3, 4 & 5

#### TOTAL BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Porous Granular Embankment	Cu. Yd.	2,188
Name Plates	Each	1
Temporary Soil Retention System	Sq. Ft.	432
Box Culvert End Sections, Culvert No. 1	Each	2
Box Culvert End Sections, Culvert No. 2	Each	2
Box Culvert End Sections, Culvert No. 3	Each	2
Box Culvert End Sections, Culvert No. 4	Each	2
Box Culvert End Sections, Culvert No. 5	Each	2
Box Culvert End Sections, Culvert No. 6	Each	2
Box Culvert End Sections, Culvert No. 7	Each	2
Box Culvert End Sections, Culvert No. 8	Each	2
Box Culvert End Sections, Culvert No. 9	Each	2
Box Culvert End Sections, Culvert No. 10	Each	2
Box Culvert End Sections, Culvert No. 11	Each	2
Precast Concrete Box Culverts 3'x2'	Foot	114
Precast Concrete Box Culverts 4'x2'	Foot	675
Precast Concrete Box Culverts 4'x3'	Foot	149
Precast Concrete Box Culverts 5'x2'	Foot	137
Precast Concrete Box Culverts 5'x3'	Foot	106
Precast Concrete Box Culverts 7'x3'	Foot	135
Geocomposite Wall Drain	Sq. Yd.	411
Membrane Waterproofing for Buried Structures	Sq. Yd.	411
Scructures		

#### DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications, 9th Edition

#### DESIGN STRESSES

FIELD UNITS f'c = 3,500 psi

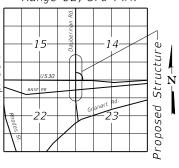
fy = 60,000 psi (Reinforcement)

#### PRECAST UNITS

f'c = 5,000 psi

fy = 60,000 psi (Reinforcement) fy = 65,000 psi (Welded Wire Fabric)

Range 6E, 3rd P.M.



LOCATION SKETCH

GENERAL DATA CULVERT NO. 1 THRU CULVERT NO. 11 DAUBERMAN ROAD

> F.A.S. RTE 1107 SECTION 15-00277-01-BR KANE COUNTY

STA. 628+00 TO STA. 667+00

#### POROUS GRANULAR EMBANKMENT DETAILS



USER NAME = dkierpiec	DESIGNED - HB	REVISED -
	CHECKED - JJI	REVISED -
PLOT SCALE = NTS	DRAWN -	REVISED -
PLOT DATE = 11/18/2022	CHECKED -	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

**GENERAL DATA** CULVERT NO. 1 THRU CULVERT NO. 11 SHEET NO. 1 OF 20 SHEETS

A.S. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
1107	15-00277-01-BR	KANE	542	284	
		CONTRACT	NO. 6	1H95	
	ILLINOIS	EED A	ID PROJECT		

Existing Structure: None

#### GENERAL NOTES

2020 AASHTO LRFD Bridge Design Specifications 9th Edition

LOADING HL-93

#### DESIGN STRESSES

DESIGN SPECIFICATIONS

#### PRECAST UNITS

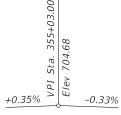
 $f'c = 5,000 \ psi$ 

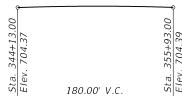
fy = 60,000 psi (Reinforcement) fy = 65,000 psi (Welded Wire Reinforcement)

> STATION 354+70.56 BUILT 202 BY STATE OF ILLINOIS F.A.S. RTE. 1107 SEC. 15-00277-01-BR LOADING HL-93

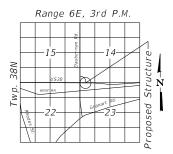
#### NAME PLATE See Std. 515001

STRUCTURE NO. 045-3803





#### PROPOSED PROFILE GRADE US 30



LOCATION SKETCH

GENERAL PLAN AND ELEVATION U.S RTE. 30 CULVERT NO. 11 OVER UNNAMED DITCH F.A.S. RTE. 1107 SEC. 15-00277-01-BR KANE COUNTY STATION 354+70.56 S.N. 045-8303

#### (Sheet 1 of 6)

#### DESIGNED - HB REVISED -CHECKED - JJI REVISED -BLA, Inc. REVISED -CHECKED -REVISED -

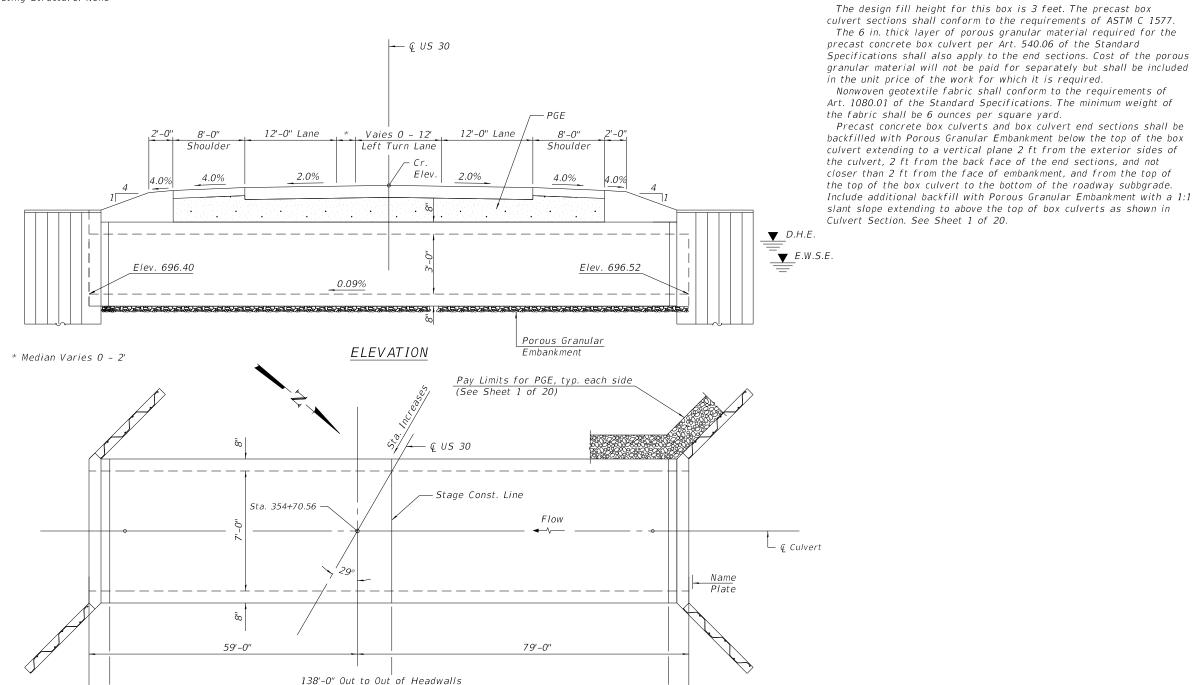
1'-6"

Pay Limits for

Box Culvert

End Sections

F.A.S. RTE	SECT	ION			COUNTY	TOTAL SHEETS	SHEE NO.
1107	15-00277-01-BR			KANE	542	285	
					CONTRACT	NO. 6	1H95
		TELEMOTS	EED	Λ	ID PROJECT		



#### PLAN

77'-0" Stage 1 Construction

Pay Limits for Precast Concrete Box Culverts

#### WATERWAY INFORMATION

61'-0" Prestage 1 Construction

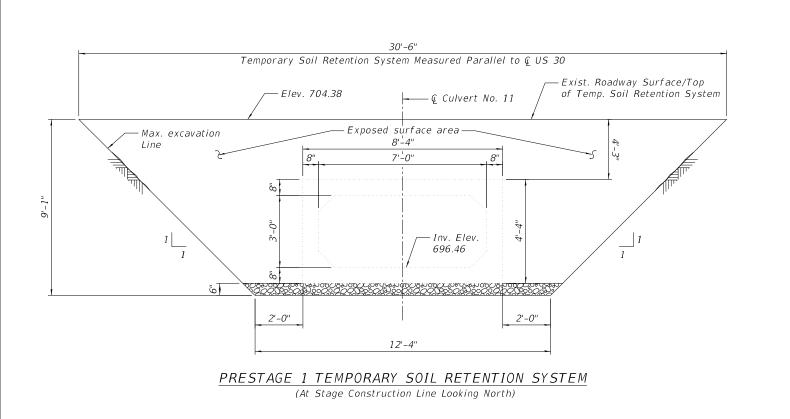
Pay Limits for Precast Concrete Box Culverts

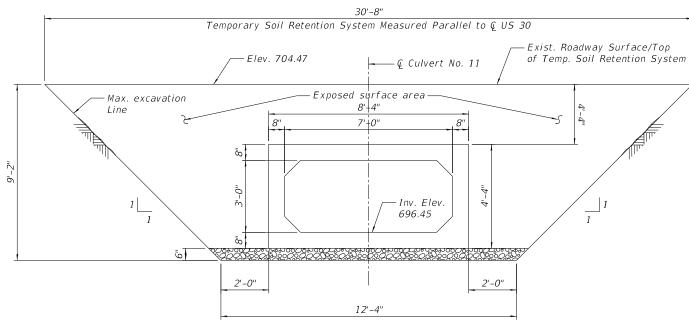
Drainage Area = sq. mi.				Low G	Grade Ele	ev. = (	⊚ Sta.			
	Flood	Freq.	Q	Opening	g Sq. Ft.	Nat.	Head	- Ft.	Headwa	ater El.
	F1000	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
		10	70.3	14.7	19.5	699.3	0.1	0.0	699.4	699.3
	Design	50	97.8	15.5	20.2	699.4	0.6	0.4	700.0	699.8
	Base	100	113.1	15.8	20.4	699.4	0.8	0.6	700.2	700.0
	Overtopping									
	Max. Calc.	500	213.9	18.3	21.0	700.0	3.4	2.7	703.4	702.7

Pay Limits for

Box Culvert

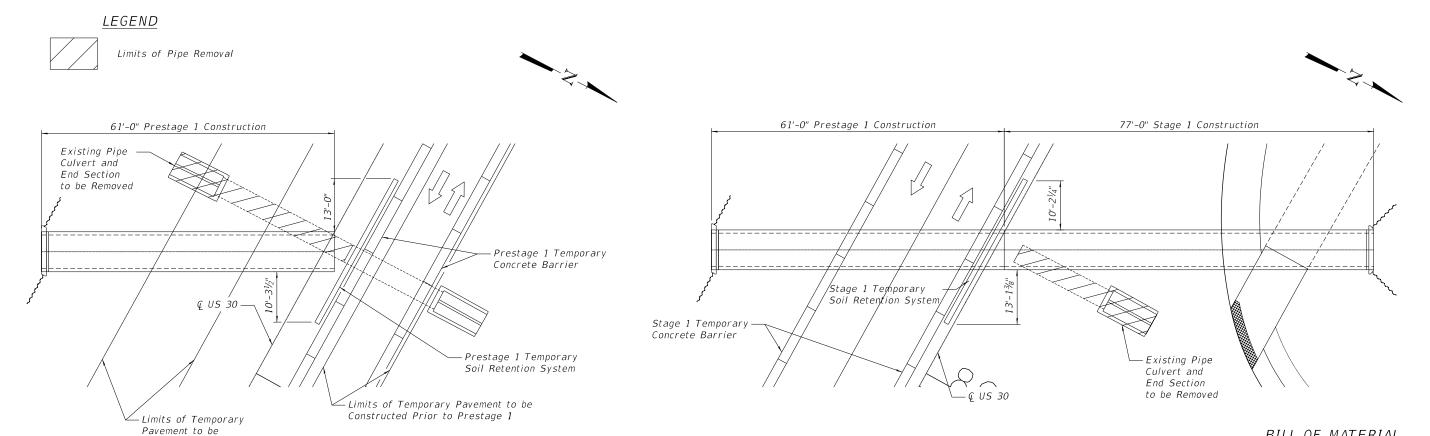
End Sections





#### STAGE 1 TEMPORARY SOIL RETENTION SYSTEM

(At Stage Construction Line Looking South)



Note: See Maintenance of Traffic plans for construction staging.

## BILL OF MATERIAL

Item	Unit	Total
Temporary Soil Retention System	Sq. Ft.	432

USER NAME = bmsetzke DESIGNED - HB REVISED -CHECKED - JJI REVISED -REVISED -

PRESTAGE 1

REVISED -

CHECKED -

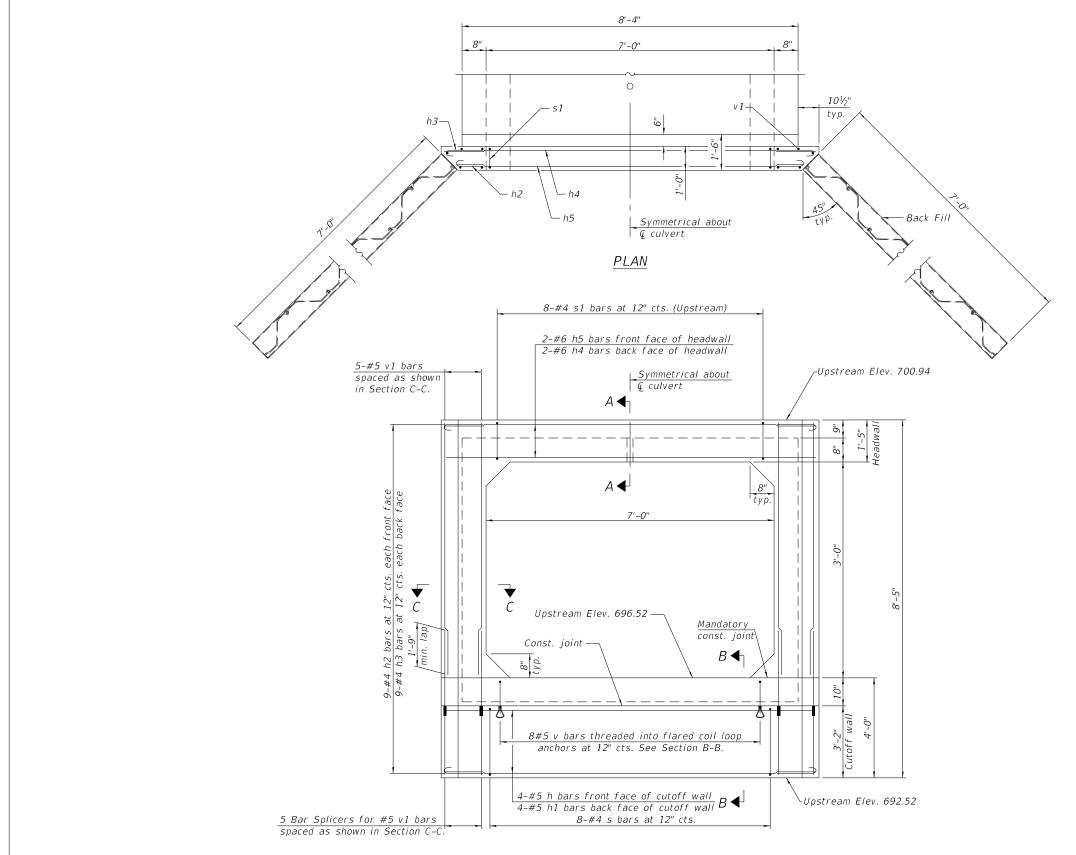
Constructed in Prestage 1

> STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

(Sheet 2 of 6) STAGE CONSTRUCTION DETAILS STRUCTURE NO. 045-8303 SHEET NO. 3 OF 20 SHEETS

STAGE 1

SECTION COUNTY 1107 15-00277-01-BR KANE 542 286 CONTRACT NO. 61H95



#### END ELEVATION

(Wingwalls omitted in this view for clarity.)

#### iote:

The design fill height for this structure is 3 feet. The precast concrete box culvert sections shall conform to the standard designs of ASTM C 1577.

The box culvert end section shall be built in the field and a precast option is not allowed except the cutoff wall may be precast. If the Contractor elects to use a precast cutoff wall, shop drawings and a proposed construction sequence shall be submitted to the Engineer for approval.

Areas of the precast box culvert in contact with cast-in-place concrete shall be sandblasted, cleaned, and wetted prior to placing concrete in the field according to Article 503.09(b).

The ends of the precast box sections adjacent to the end section shall be formed without male and female shapes.

The joints between precast box sections shall be sealed, all voids filled with a mastic joint sealer. In addition, the joints shall be externally sealed on all four sides with a 13 inch wide external sealing band. The seal shall be centered over the joint, secured in place and protected during the backfilling process.

Tilt h2 and h3 bars as required to maintain clearance. Extend precast concrete box culvert welded wire reinforcement into end section. Bend as necessary to provide 1½" clear cover.

See sheet 1 of 6 for culvert construction sequence. See sheet 4 of 6 for Section A-A, B-B and C-C. See sheet 4 of 6 for additional wing wall details.

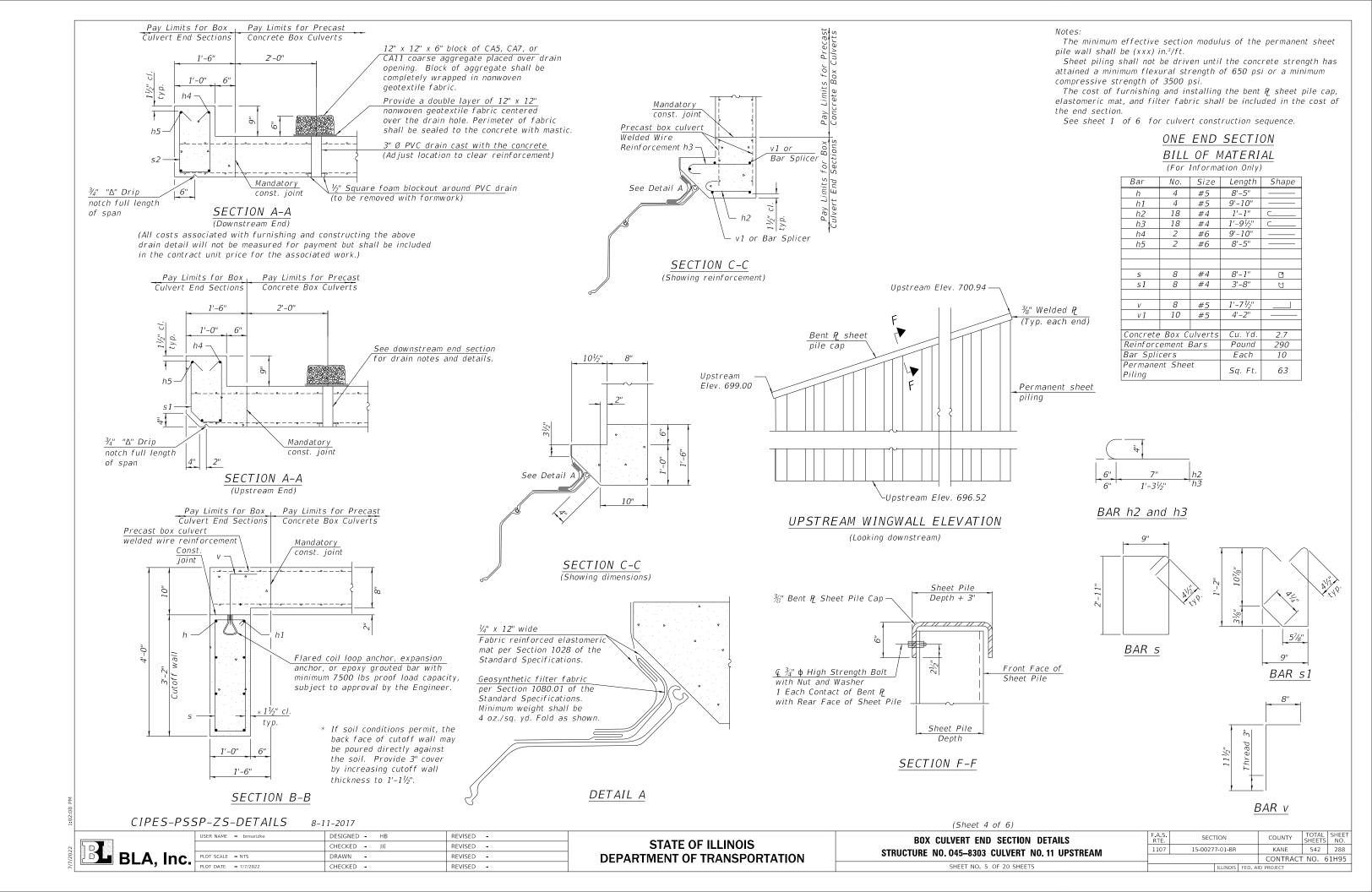
#### BILL OF MATERIAL

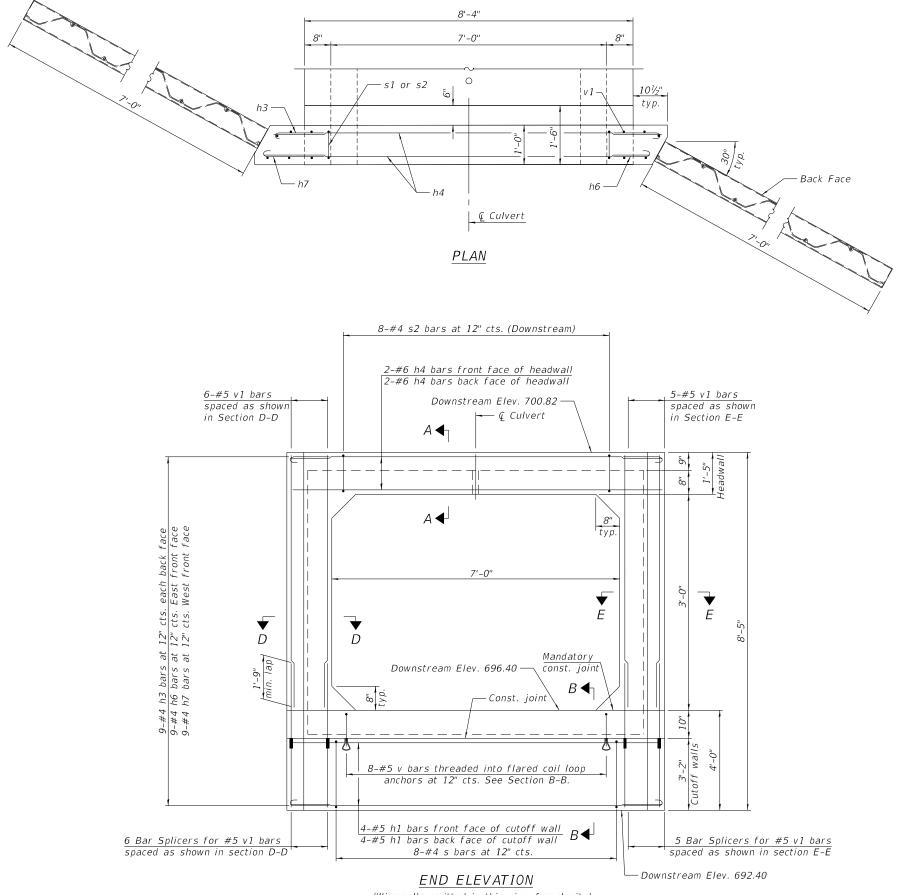
Item	Unit	Total
Box Culvert End Sectons, Culvert No. 11	Each	1

(Sheet 3 of 6)

BLA, Inc.

	USER NAME = bmsetzke	DESIGNED - HB	REVISED -
		CHECKED - JJI	REVISED -
	PLOT SCALE = NTS	DRAWN -	REVISED -
•	PLOT DATE = 7/7/2022	CHECKED -	REVISED -





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Tilt h2 and h3 bars as required to maintain clearance. Extend precast concrete box culvert welded wire reinforcement into end section. Bend as necessary to provide 1½" clear cover.

See sheet 1 of 6 for culvert construction sequence. See sheet 4 of 6 for Section A-A and B-B. See sheet 6 of 6 for additional wing wall details.

See sheet 6 of 6 for section D-D and E-E.

#### BILL OF MATERIAL

Item	Unit	Total
Box Culvert End Sections, Culvert No. 11	Each	1

(Wingwalls omitted in this view for clarity)

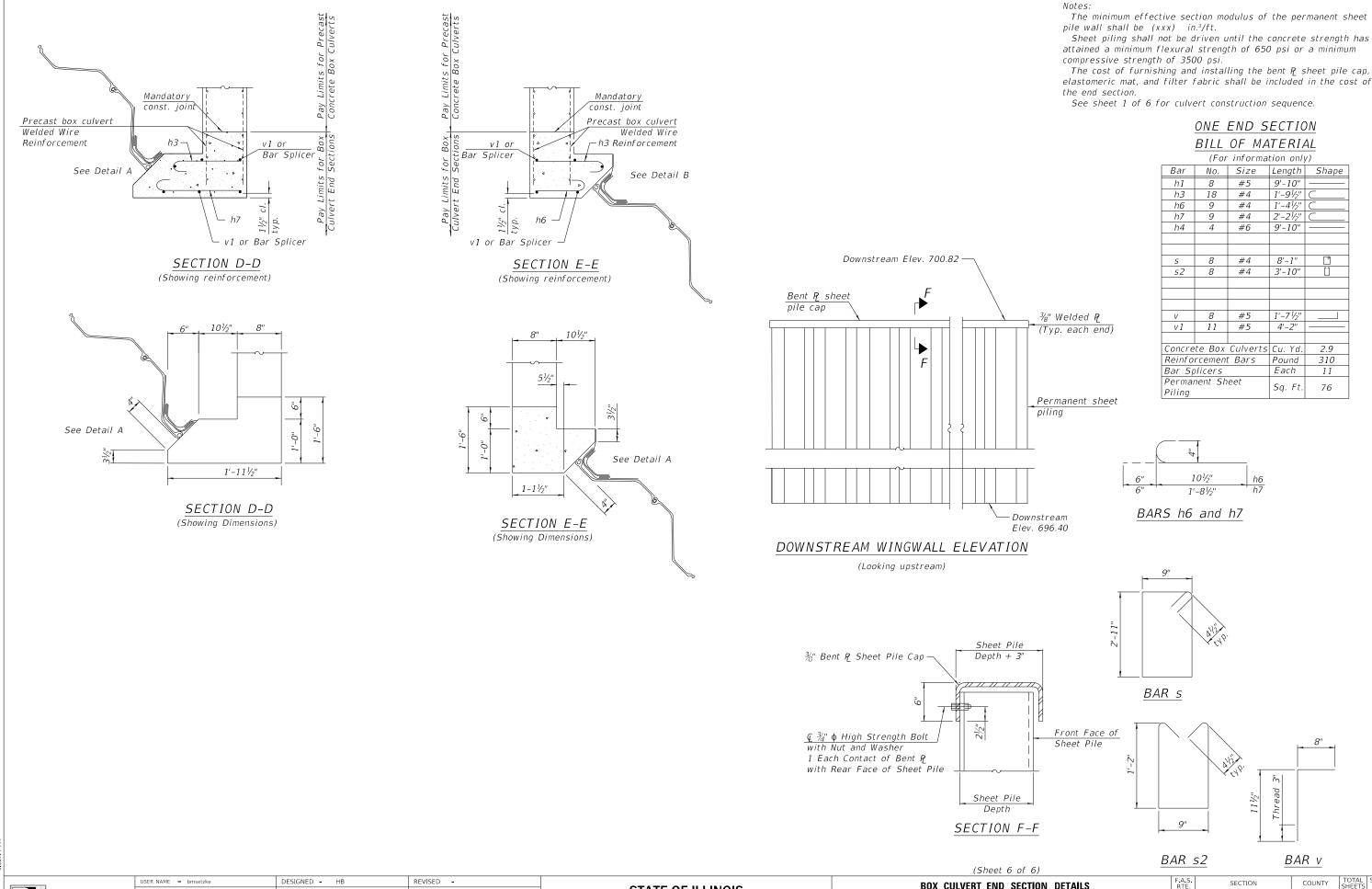
	USER NAME = bmsetzke	DESIGNED - HB	REVISED -
		CHECKED - JJI	REVISED -
	PLOT SCALE = NTS	DRAWN -	REVISED -
•	PLOT DATE = 7/7/2022	CHECKED -	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

BOX CULVERT END SECTION DETAILS						
STRUCTURE NO. 045-8303 CULVERT 11 DOWNSTREAM						
SHEET NO. 6 OF 20 SHEETS						

(Sheet 5 of 6)

SECTION COUNTY 1107 15-00277-01-BR KANE 542 289 CONTRACT NO. 61H95



BLA, Inc.

 STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BOX CULVERT END SECTION DETAILS
STRUCTURE NO. 045–8303 CULVERT 11 DOWNSTREAM

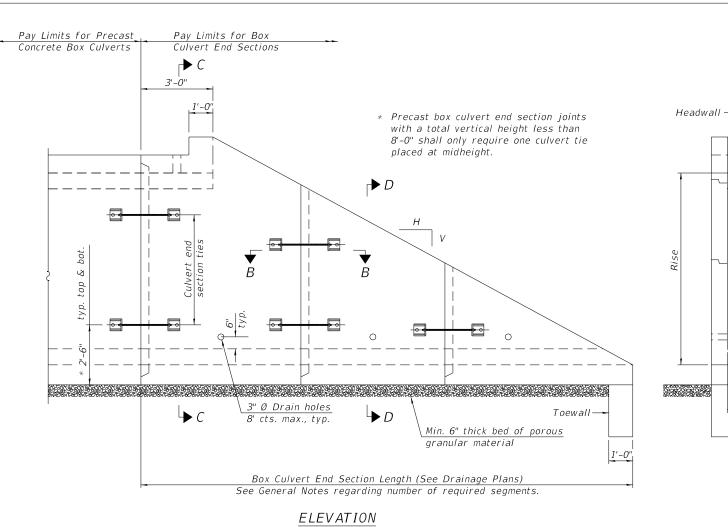
SHEET NO. 7 OF 20 SHEETS

 
 F.A.S. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL NO.
 SHEETS NO.

 1107
 15-00277-01-BR
 KANE
 542
 290

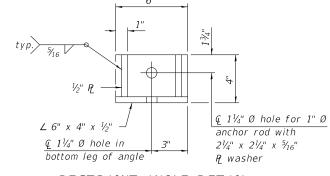
 CONTRACT NO. 61H95

 ILLINOIS FED. AID PROJECT



#### END VIEW

Span

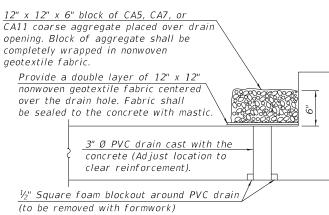


2'-0"

typ.

∖Porous granular

#### RESTRAINT ANGLE DETAIL



drain detail will not be measured for payment but shall be included (Sheet 1 of 2)

#### GENERAL NOTES

See sheet 1 of 20 for culvert spand and rise, box Culvert End Sections shall be constructed according to the requirements of Section 540 of the Standard Specifications except as modified herein. This work will be measured for payment as each, with each end of each culvert being one each. End sections will be paid for at the contract unit price per each for Box Culvert End Sections of the culvert number specified.

Typical box section dimensions, materials, and reinforcement details for Box Culvert End Sections shall be according to the requirements of ASTM C 1577 as required for

the design of the portion of the culvert within the limits of Precast Concrete Box Culverts except as modified herein.

Number of segments shown in Elevation is for example only. Length and number of precast box sections required to construct Box Culvert End Sections shall be determined by the Contractor.

See roadway plans for embankment slope (V:H).

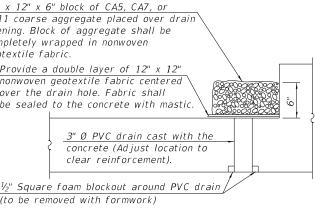
1" Ø anchor rods for the culvert ties shall conform to the requirements of ASTM F1554, Grade 105. Structural steel for tie plate and restraint angle shall conform to the requirements of Article 1006.04 of the Standard Specifications. All components of the culvert tie detail shall be galvanized according to the requirements of AASHTO M 111 or M 232 as applicable.  $2\frac{1}{4}$ " x  $2\frac{1}{4}$ " x  $\frac{5}{16}$ " plate washers shall be provided under each nut required for the anchor rods. Anchor rods connecting precast sections shall be brought to a snug tight condition followed by an additional ½ turn on one of the nuts for anchor rods installed in the walls. Match marks shall be provided on the bolt and nut to verify relative rotation between the bolt and the nut. Holes in the walls for the culvert tie assembly may be drilled using core bits in lieu of using formed holes.

All costs associated with furnishing and installing or constructing the toewall and culvert ties will not be measured for payment but shall be included in the contract unit price for Box Culvert End Sections of the culvert number specified.

Drain holes shall conform to the requirements of Article 503.11 of the Standard Specifications unless noted otherwise.

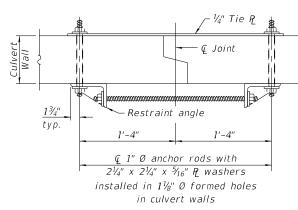
Nonwoven geotextile fabric shall conform to the requirements of Article 1080.01. The minimum weight of the fabric shall be 6 oz. / sq. yd..

For end sections with traversable pipe grate systems, see grate detail sheet for required modifications.

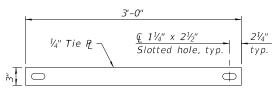


#### SECTION A-A

(All costs associated with furnishing and constructing the above in the contract unit price for the associated work.)



SECTION B-B (Showing end section tie details)



TIE PLATE DETAIL

SCB-TES

2-17-2017 USER NAME = bmsetzke

¢ 3" Ø Drain hole→

DESIGNED - HB REVISED -CHECKED - JJI REVISED -DRAWN REVISED -CHECKED -REVISED -

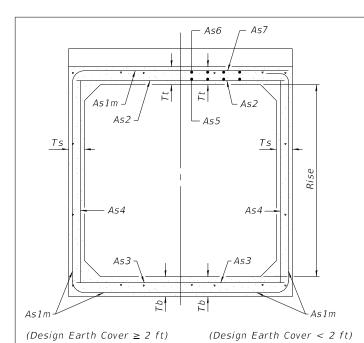
PLAN

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

E ♠

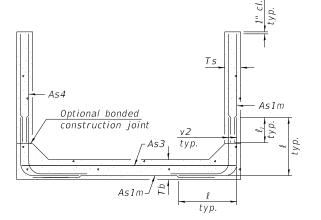
SINGLE CELL PRECAST BOX CULVERT TAPERED END SECTIONS **CULVERT NO.1 THRU CULVERT NO.8** SHEET NO. 8 OF 20 SHEETS

COUNTY 1107 15-00277-01-BR KANE 542 291 CONTRACT NO. 61H95



SECTION C-C

SECTION D-D



ALTERNATE SECTION D-D

As1m REINFORCEMENT (in.²/ ft)											
Rise (ft)	_	_		•		_	_				
Ts (in.)	2	3	4	5	6	7	8	9	10	11	12
4	0.19	0.17									
5	0.26	0.21	0.18								
6	0.22	0.26	0.23	0.22							
7	0.25	0.33	0.59	0.27	0.28						
8	0.40	0.35	0.43	0.39	0.36	0.34	0.40				
9	0.44	0.39	0.35	0.43	0.40	0.37	0.36	0.48			
10	0.48	0.42	0.38	0.47	0.44	0.41	0.38	0.42	0.56		
11	0.52	0.45	0.54	0.50	0.46	0.44	0.41	0.46	0.50	0.65	
12	0.55	0.49	0.58	0.54	0.50	0.48	0.45	0.46	0.46	0.61	0.75
reinforcement based war worlded wire reinforcement conforming to AACUTO M.FF or M. 221											

(As1m reinforcement based upon welded wire reinforcement conforming to AASHTO M 55 or M 221).

#### l, DIMENSION

#3 bar = 2'-0" #4 bar = 2'-8"  $#5 \ bar = 3'-4''$ 

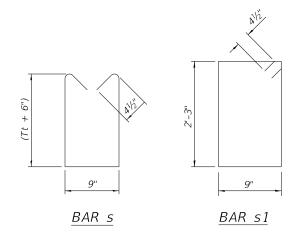
 $#6 \ bar = 3'-11''$ 

option of casting the bottom slab of the end section first followed by construction of the sidewalls using conventional forming methods. Shop drawings that detail slab thickness and reinforcement layout shall be submitted to the Engineer for review and approval when using Alternate Section D-D.

The size and spacing of the v2 bars shall provide a minimum reinforcement area along each face of the walls (in.2/ft.) equal to 1.10\*(As1m). v2 bars may consist of #3 thru #6 size reinforcement bars and the longitudinal spacing shall not exceed the lesser of the wall thickness or 8 inches.

Alternate Section D-D is provided to allow the Contractor the

Bonded construction joints shall be prepared according to Article 503.09 of the Standard Specifications.



# #4 s bars at spacing = Tt (Spacing need not be less than 8") $\vdash$ F4-h bars (See Section F-F)

HEADWALL ELEVATION (Allow sidewall reinforcement to extend into end of headwall.)

#### TOEWALL CONSTRUCTION SEQUENCE

- 1. Perform excavation and construct toewall.
- 2. Backfill according to the applicable paragraphs of Article 502.10 of the Standard Specifications and place bedding for precast box culvert end sections.
- 3. Set precast box culvert end section.
- 4. Drill and epoxy grout reinforcement in toewall in accordance with Section 584 of the Standard Specifications.
- 5. Pressure grout voids using non-shrink grout conforming to Section 1024 of the Standard Specifications.
- The Contractor may furnish a precast or cast-in-place toewall. The Contractor shall be responsible for the strength and stability of the precast toewall during handling. Additional lifting points may be required depending upon the length of the toewall or the Contractor may need to modify the design of the toewall for the proposed handling the method.
- \*\* If soil conditions permit, the sides of the toewall may be poured directly against the soil. The clear cover on the sides of the toewall shall be increased to 3" by increasing the thickness of the toewall.

#### 3" Ø corrugated PE pipe per Article 1040.04 of the Standard Specifications. Fill with non-shrink grout 6-#5 h1 bars placed as shown #4 v1 bars drilled and grouted into toewall in 9" min. deep holes at 1'-6" cts., max. #4 s1 bars at \* 1½" cI. 1'-0" cts., max. typ. 1'-0"

#### SECTION E-E

# $2-\#7\ h\ bars\ (S<8'-0'')$ $2-\#8 \text{ h bars } (S \ge 8'-0")$ Top and bottom of headwall C. ¾" "Δ" Drip notch full length of span

SECTION F-F

2-17-2017

	USER NAME = bmsetzke	DESIGNED - HB	REVISED -
		CHECKED - JJI	REVISED -
	PLOT SCALE = NTS	DRAWN -	REVISED -
"	PLOT DATE = 7/7/2022	CHECKED -	REVISED -

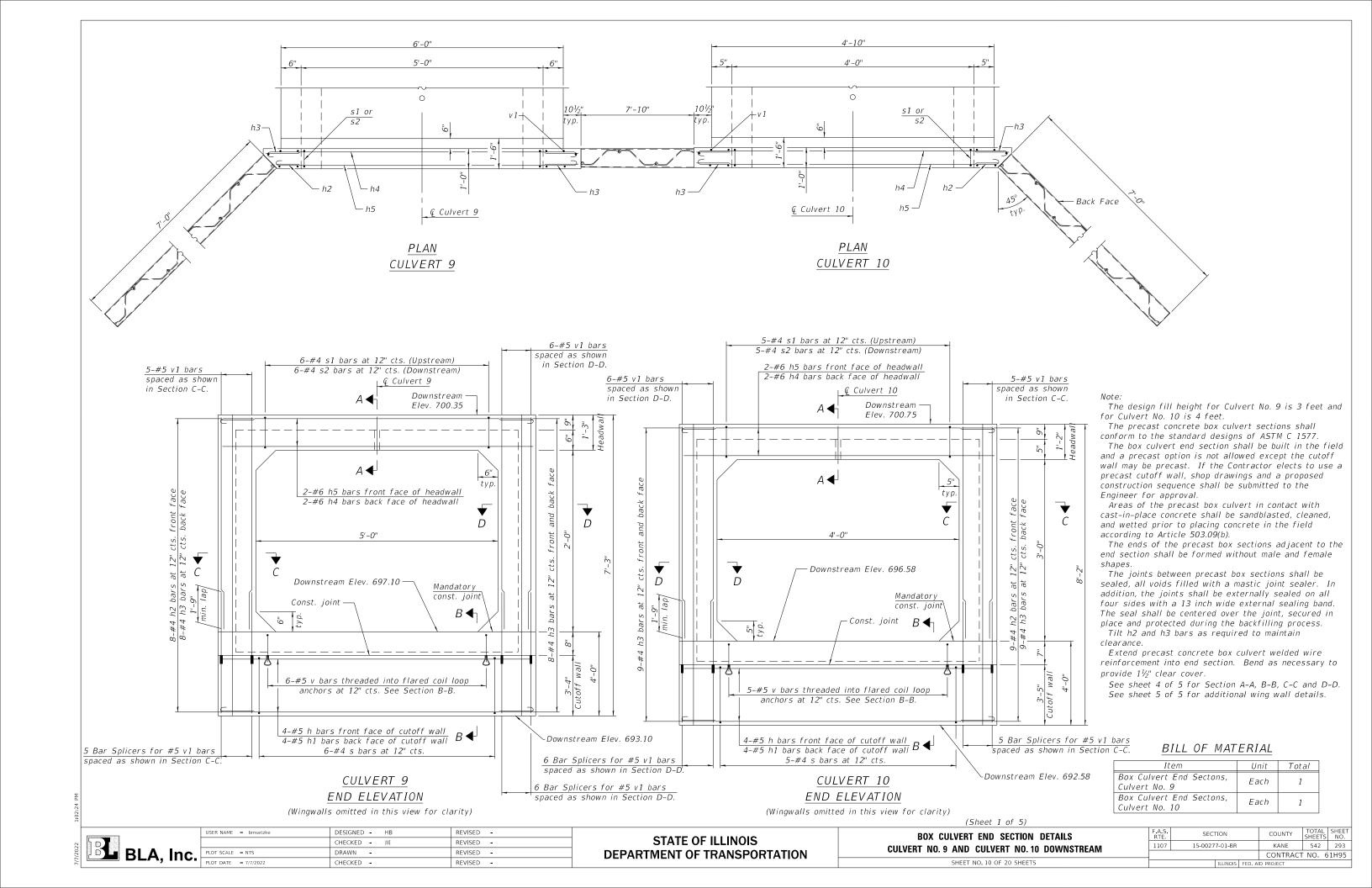
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

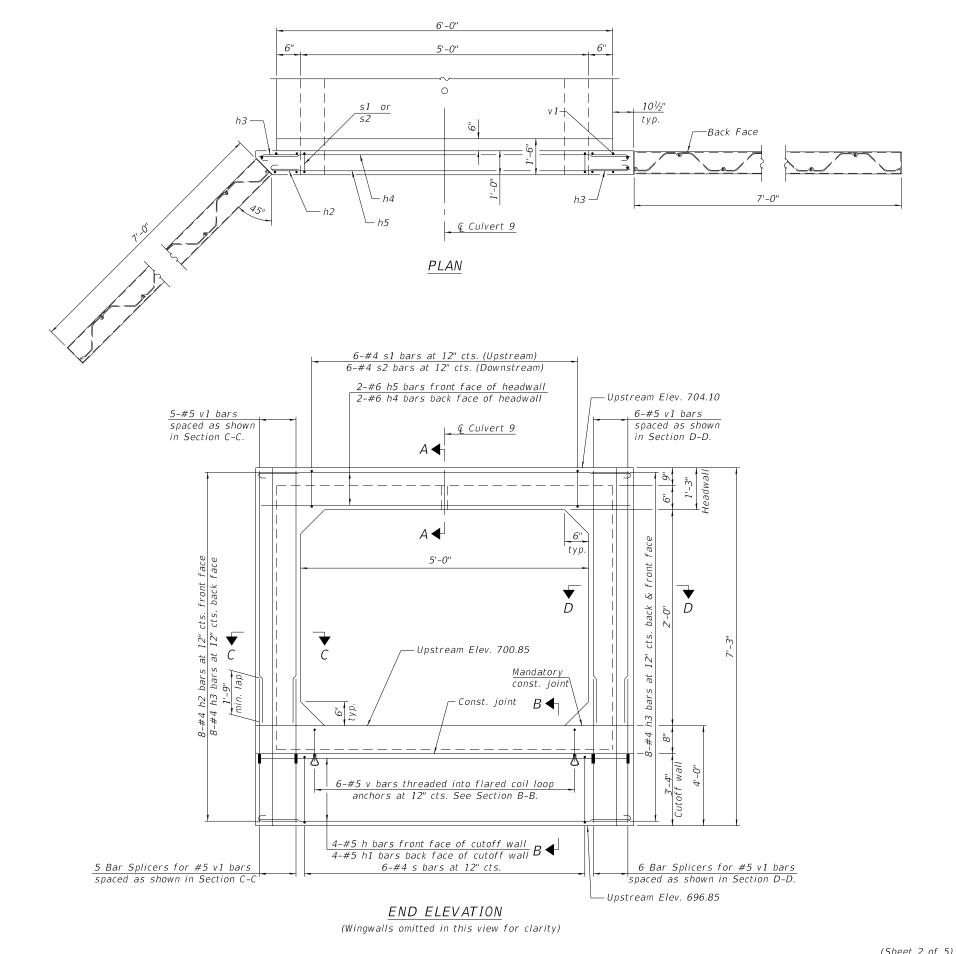
SINGLE CELL	PRECAST	BOX CULVER	RT TAPERED END
SECTIONS	CULVERT	NO.1 THRU	CULVERT NO. 8
	SHEET NO	. 9 OF 20 SHEET	S

(Sheet 2 of 2)

F.A.S. RTE	SECT	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEE NO
1107	15-0027	7-01-BR		KANE	542	292
				CONTRACT	NO. 6	1H95
		ILLINOIS	FED. Al	ID PROJECT		

SCB-TES





The design fill height for this structure is 3 feet. The precast concrete box culvert sections shall conform to the standard designs of ASTM C 1577.

The box culvert end section shall be built in the field and a precast option is not allowed except the cutoff wall may be precast. If the Contractor elects to use a precast cutoff wall, shop drawings and a proposed construction sequence shall be submitted to the Engineer for approval.

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The joints between precast box sections shall be sealed, all voids filled with a mastic joint sealer. In addition, the joints shall be externally sealed on all four sides with a 13 inch wide external sealing band. The seal shall be centered over the joint, secured in place and protected during the backfilling process.

Tilt h2 and h3 bars as required to maintain clearance. Extend precast concrete box culvert welded wire reinforcement into end section. Bend as necessary to provide  $1\frac{1}{2}$ " clear cover.

See sheet 4 of 5 for Section A-A, B-B, C-C and D-D. See sheet 5 of 5 for additional wing wall details.

#### BILL OF MATERIAL

Item	Unit	Total
Box Culvert End Sectons, Culvert No. 9	Each	1

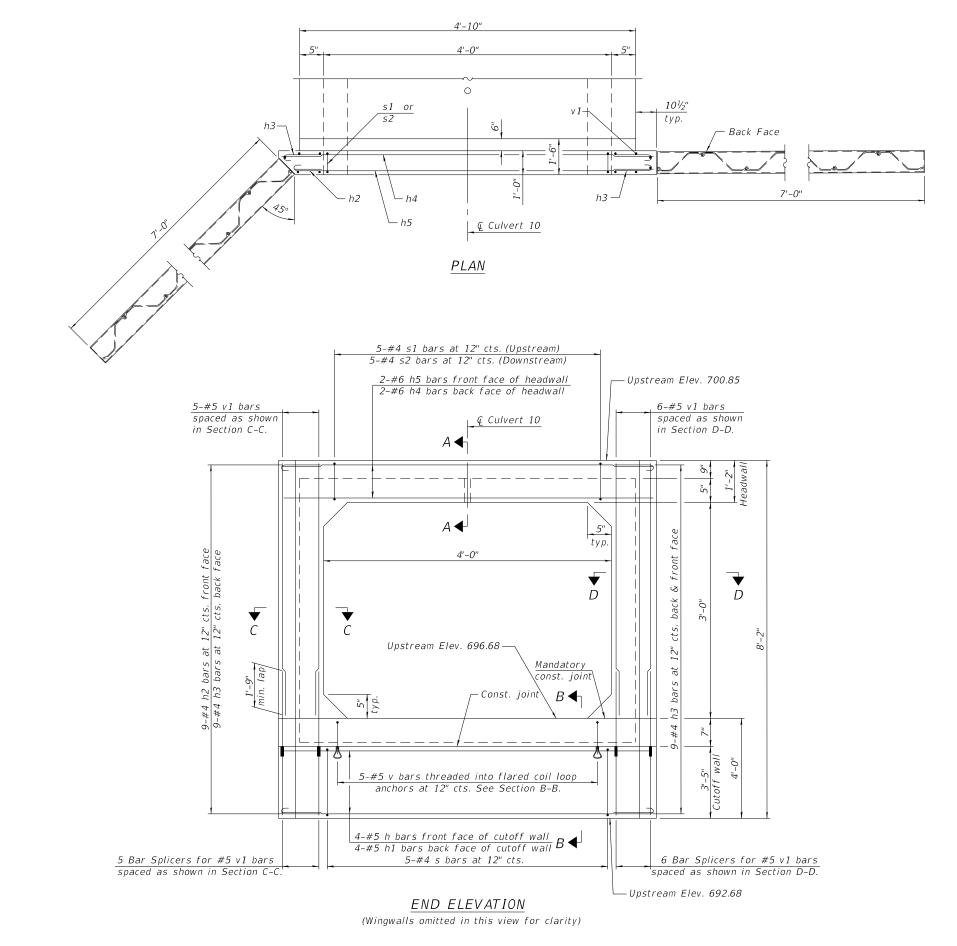
BLA, Inc.

USER NAME = bmsetzke	DESIGNED - HB	REVISED -
	CHECKED - JJI	REVISED -
PLOT SCALE = NTS	DRAWN -	REVISED -
PLOT DATE = 7/7/2022	CHECKED -	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

(3//666 2 6/ 3)
BOX CULVERT END SECTION DETAILS CULVERT NO. 9 UPSTREAM
SHEET NO. 11 OF 20 SHEETS

F.A.S. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEE NO.
1107	15-00277-01-BF			KANE	542	294
				CONTRACT	NO. 6	1H95
	THINOIS	EED	Λ	ID PROJECT		



Note:

The design fill height for this structure is 4 feet. The precast concrete box culvert sections shall conform to the standard designs of ASTM C 1577.

The box culvert end section shall be built in the field and a precast option is not allowed except the cutoff wall may be precast. If the Contractor elects to use a precast cutoff wall, shop drawings and a proposed construction sequence shall be submitted to the Engineer for approval.

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See sheet 4 of 5 for Section A-A, B-B, C-C and D-D. See sheet 5 of 5 for additional wing wall details.

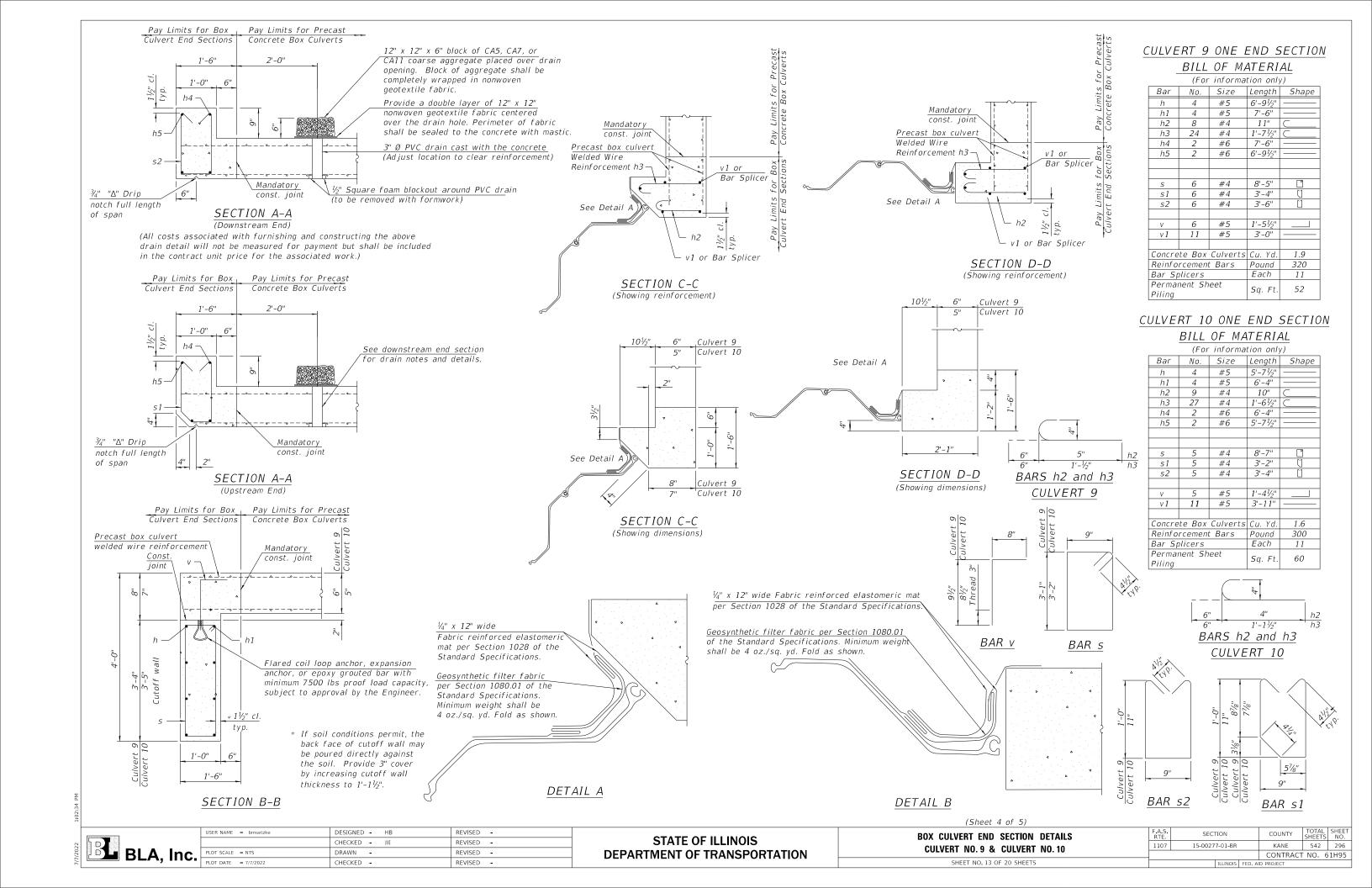
#### BILL OF MATERIAL

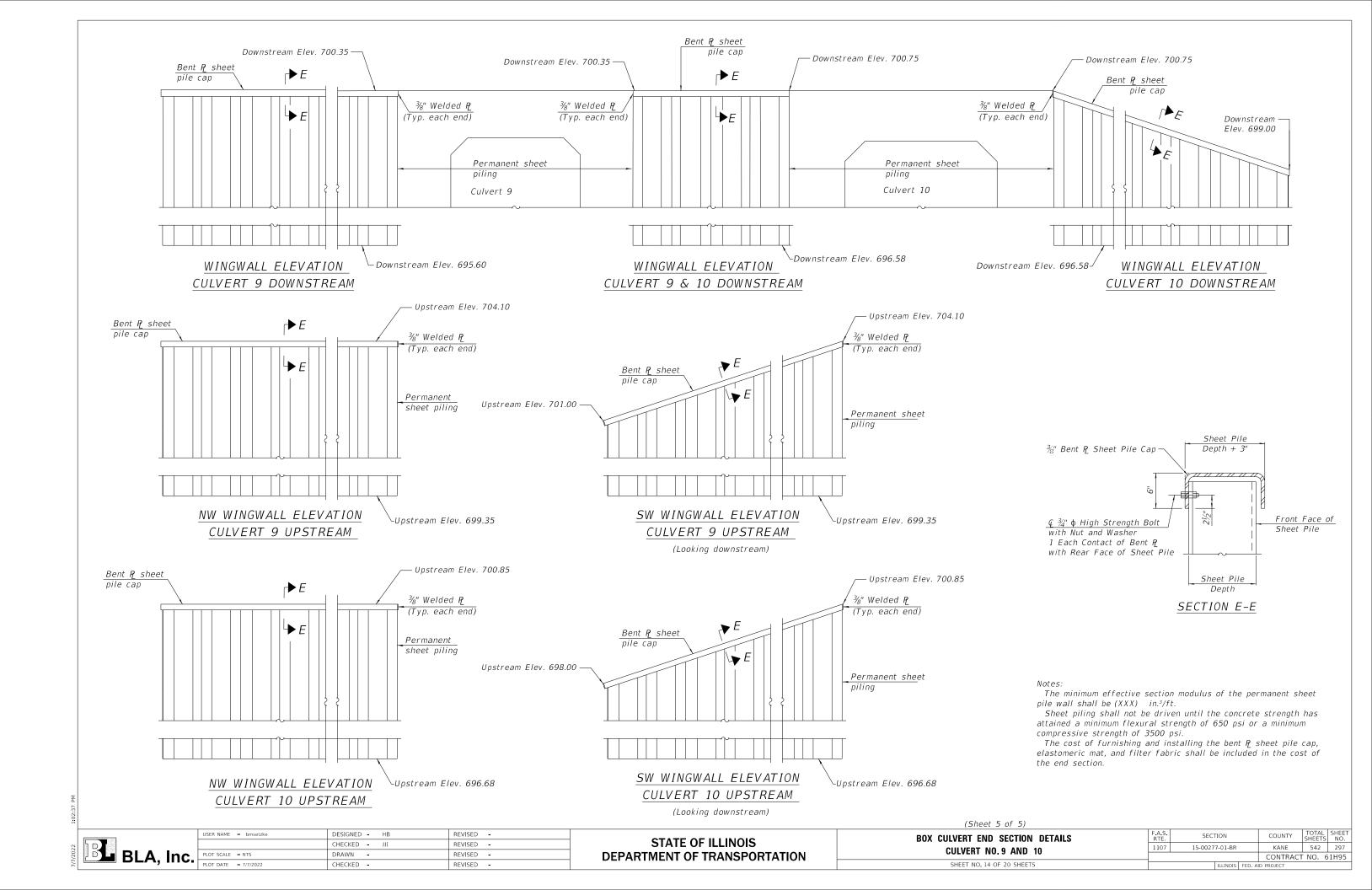
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Box Culvert End Sectons, Culvert No. 10	Each	1

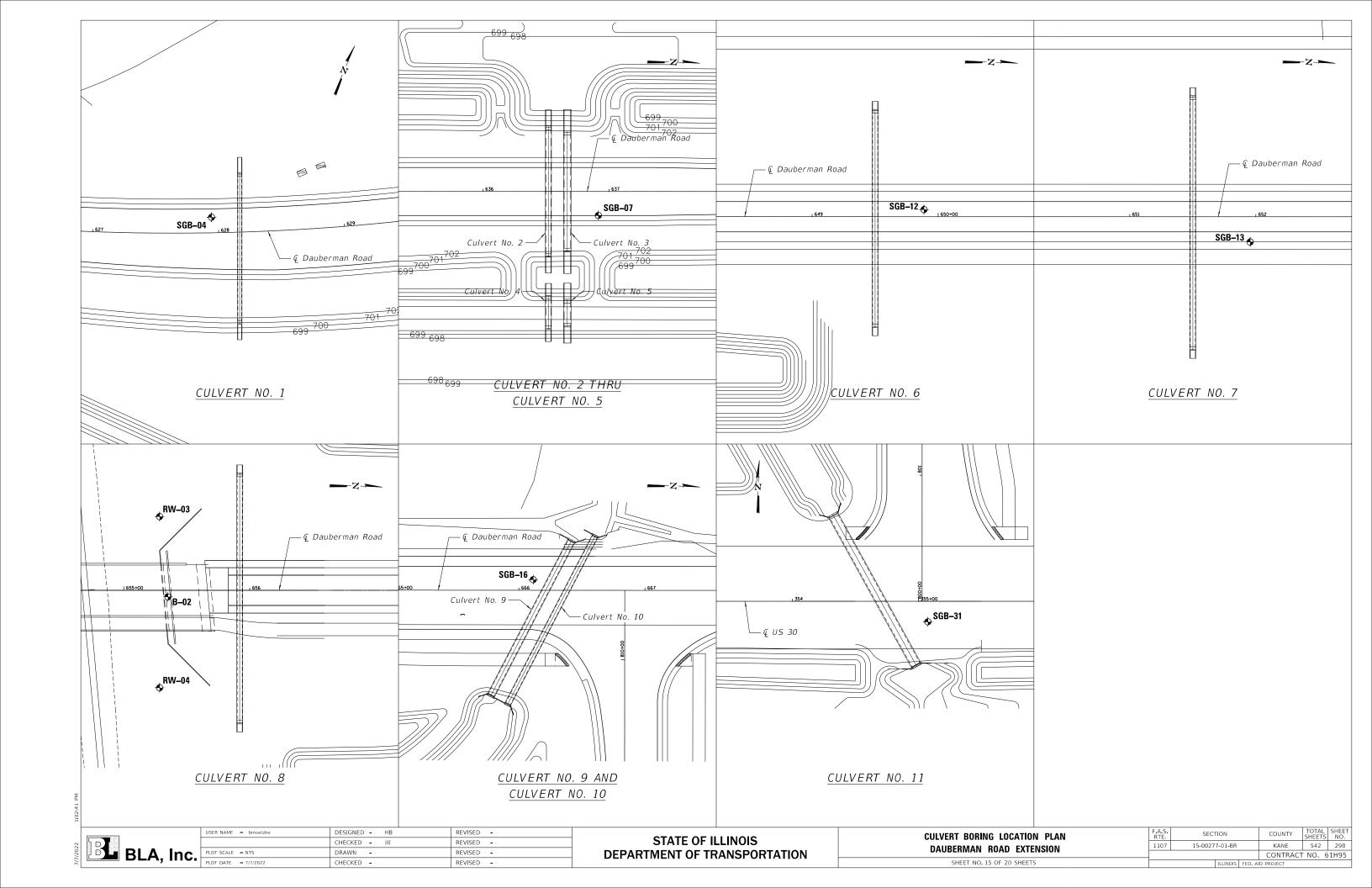
(Sheet 3 of 5)

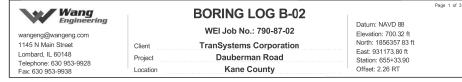
BLA, Inc.

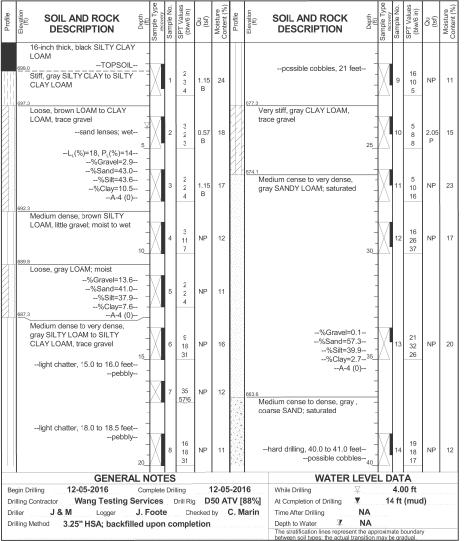
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•	PLOT DATE = 7/7/2022	CHECKED -	REVISED -



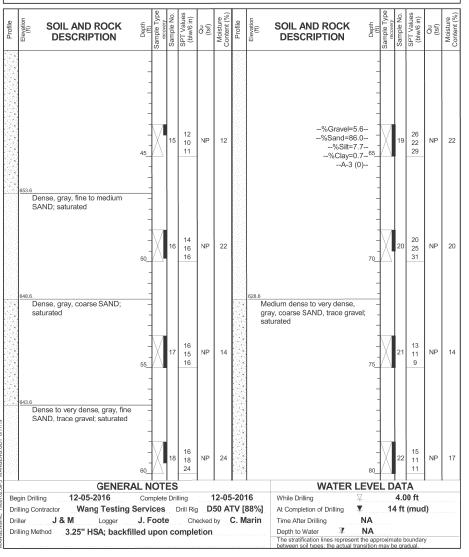


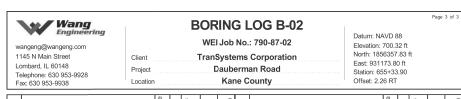


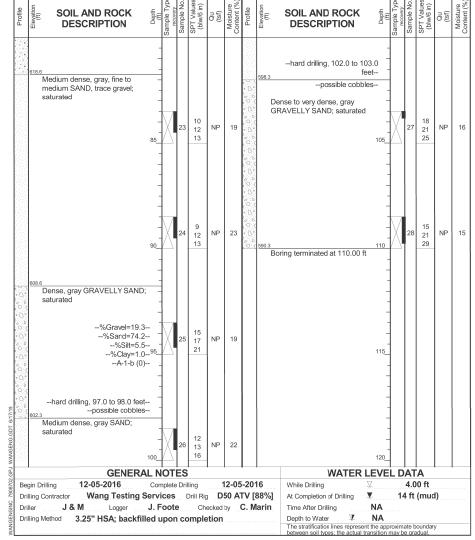




1	1	<b>Wang</b> Engineering				В	OR	IN	G L	.00	G B-02	I				Page	2 of
11 Lo	145 N Ma ombard, l elephone	Engineering  Dwangeng.com  ain Street  IL 60148  : 630 953-9928  153-9938	Client Project Location				rans	Syste Daub	ems erm	Cor		Datum: N Elevation: North: 18 East: 931 Station: 6 Offset: 2.	700.3 56357 173.8 55+33	32 ft 7.83 f 0 ft 3.90	ft		
Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ff)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROO DESCRIPTION		Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture
			-														

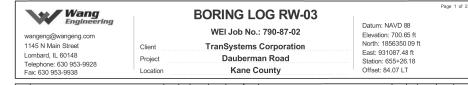


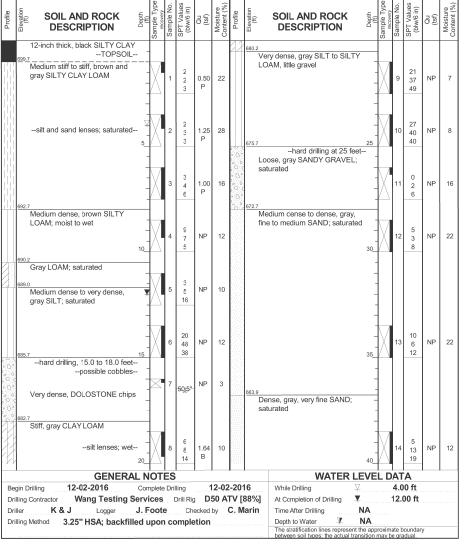




1112022	BLA,	Inc
2	BLA,	Inc

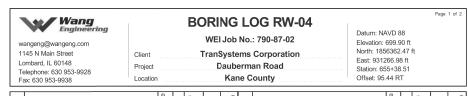
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		CHECKED - JJI	REVISED -
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•	PLOT DATE = 7/7/2022	CHECKED -	REVISED -

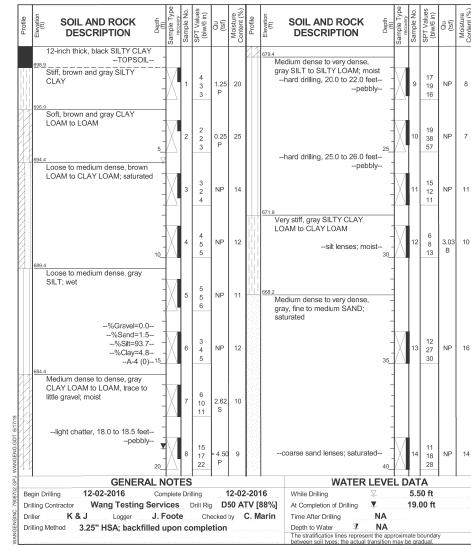




Wang		BORING LOG RW-03	Page 2
wangeng@wangeng.com		WEI Job No.: 790-87-02	Datum: NAVD 88 Elevation: 700.65 ft
1145 N Main Street	Client	TranSystems Corporation	North: 1856350.09 ft East: 931087.48 ft
Lombard, IL 60148 Telephone: 630 953-9928	Project	Dauberman Road	Station: 655+26.18
Fax: 630 953-9938	Location	Kane County	Offset: 84.07 LT

Profile	SOIL AND ROCK DESCRIPTION	Depth (ft)	sample Type	SPT Values	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No. SPT Values (blw/6 in)	Qu (tsf)	Moisture
051 D°	Dense, gray SANDY GRAVEL; saturated	- - - -												
0 ( 0°	55.7 Boring terminated at 45.00 ft	45	1	5 24 21		8								
		50_												
		-												
		55												
		60_												
	GENERA	ו אכ	TF	S			_		WATER	LEVE	L D	ATA	_	_
Drilling Driller	Drilling 12-02-2016 ng Contractor Wang Testing \$	Comp Servic J. Fo	es ote	Drilling Drill I	Rig <b>D</b> Checked	by .	TV [	88%]	While Drilling At Completion of Drilling Time After Drilling Depth to Water	¥ NA NA		4.00 ft 12.00 ft		





	USER NAME = bmsetzke	DESIGNED - HB	REVISED -
		CHECKED - JJI	REVISED -
	PLOT SCALE = NTS	DRAWN -	REVISED -
•	PLOT DATE = 7/7/2022	CHECKED -	REVISED -