

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	D9 CM BRIDGE REPAIR FY 09-1	WHITE	16	1
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

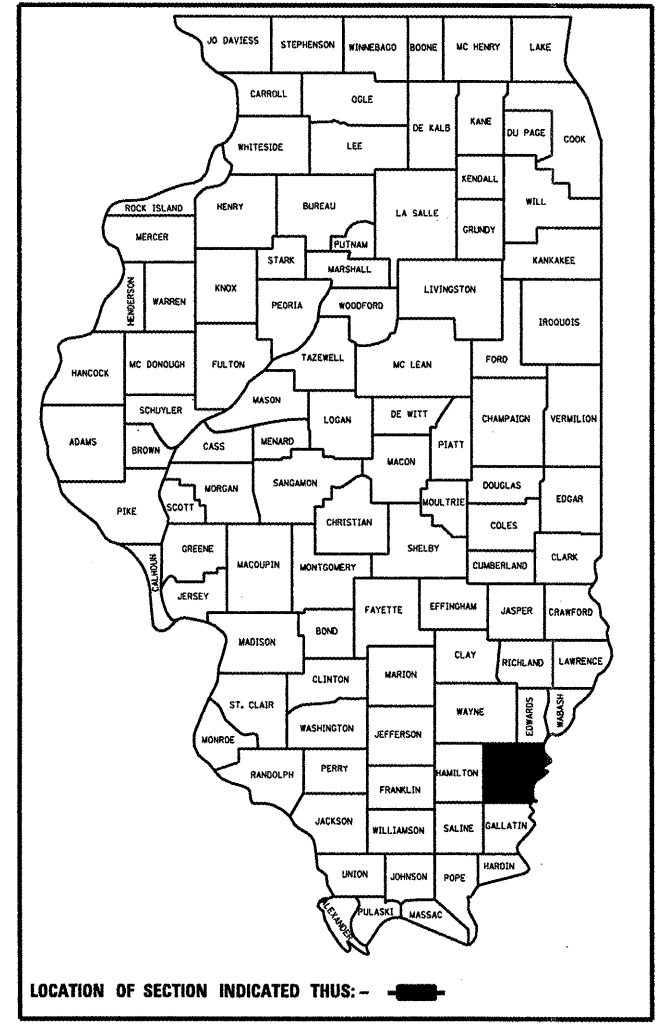
PROPOSED HIGHWAY PLANS

F.A.S. ROUTE 815 (CH 20) OVER I-64 SECTION D9 CM BRIDGE REPAIR FY 09-1 WHITE COUNTY

C-99-004-09

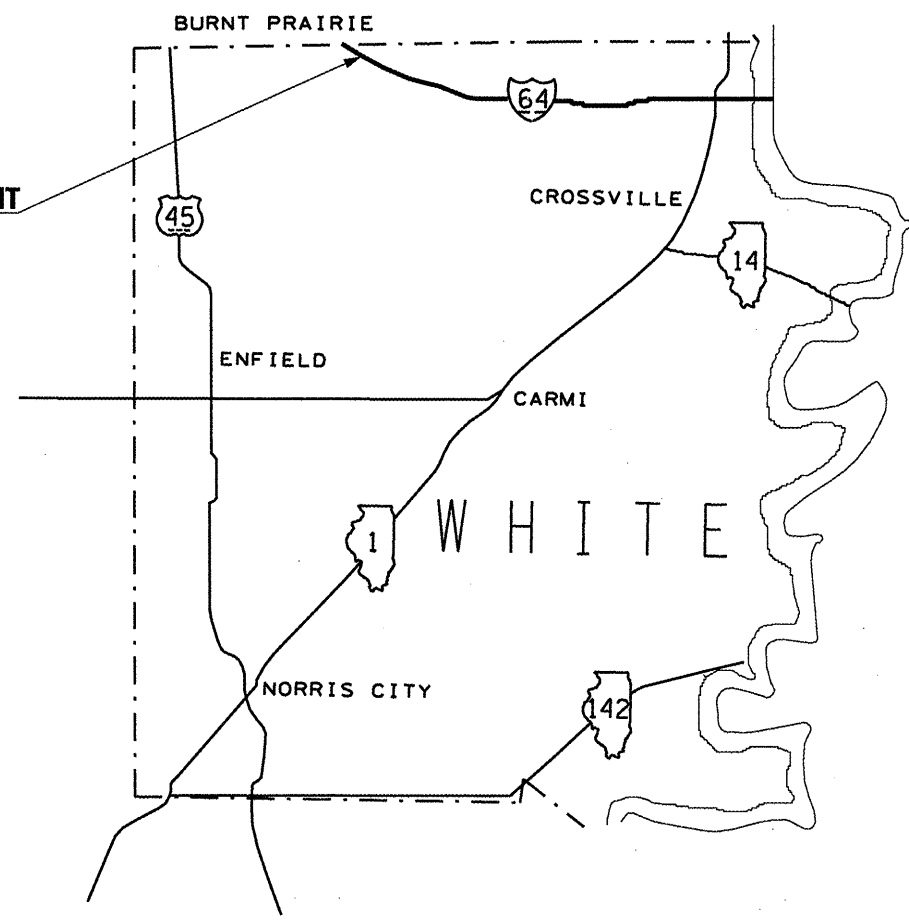
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ILLINOIS TOWNSHIP: BURNT PRAIRIE
 2005 ADT = 950
 TRUCKS = 11% ADT
 POSTED SPEED = 55 MPH
 INVENTORY RATING HS 16.7
 OPERATING RATING HS 31.1

PROPOSED IMPROVEMENT
 S.N. 097-0044
 I-64 MILEPOST 117
 EXPANSION JOINT
 BEARINGS
 HOT-MIX OVERLAY



STANDARDS

- 701201-03 LANE CLOSURE, 2L, 2W, DAY ONLY
- 701301-03 LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS
- 701316-04 LANE CLOSURE, 2L, 2W, BRIDGE REPAIR
- 701400-03 LANE CLOSURE, FREEWAY/EXPRESSWAY
- 701406-05 LANE CLOSURE, FREEWAY/EXPRESSWAY, DAY ONLY
- 701901-01 TRAFFIC CONTROL DEVICES

J.U.L.I.E.
 JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
 1-800-892-0123
 OR 811

MAP NOT TO SCALE

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS

SUBMITTED Dec 5 2008

Mary C. Lami
 DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER

January 30, 2009
Charles J. Ingersoll
 ENGINEER OF DESIGN AND ENVIRONMENT

January 30, 2009
Christine M. Reed
 DIRECTOR OF HIGHWAYS, CHIEF ENGINEER

**PRINTED BY THE AUTHORITY
OF THE STATE OF ILLINOIS**

CONTRACT NO. 78094

PROJECT ENGINEER: CASEY N. TECKENBROCK (618) 549-2171 CENTREX: 782-4654
SQUAD LEADER: RITA GAUTNEY

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	D9 CM BRIDGE REPAIR FY 09-1	WHITE	16	2
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		

GENERAL NOTES

WHILE SIGNAL HEADS ARE MOUNTED IN PLACE, BUT NOT YET IN OPERATION, THEY SHALL BE SECURELY COVERED IN WHITE PLASTIC.

THE ADVANCE DETECTOR LOOPS ARE TYPICALLY LOCATED 275 FEET IN ADVANCE OF THE STOP BAR. THE BUREAU OF OPERATIONS SHOULD APPROVE THE LOOP LOCATIONS PRIOR TO INSTALLATION.

ANY TIME THAT HOLES AT EXPANSION JOINTS ARE OPEN, THEY SHALL BE COVERED WITH METAL PLATES CAPABLE OF CARRYING THE FULL WEIGHT OF AN ERRANT VEHICLE. PLATES SHALL BE PLACED DIRECTLY ON THE CONCRETE DECK. PROJECTIONS ABOVE THE ROADWAY SURFACE GREATER THAN 1" ARE NOT PERMITTED EXCEPT FOR HEADS OF BOLTS.

FACTORS USED FOR ESTIMATING PLAN QUANTITIES ARE AS FOLLOWS AND SHALL NOT BE USED FOR THE BASIS OF FINAL QUANTITIES:

ALL HOT MIX ASPHALT	2.016 TONS/CU.YD.
ALL AGGREGATE	2.05 TONS/CU.YD.
BITUMINOUS MATERIALS:	
ON PAVEMENT	0.09 GAL./SQ.YD.
INTERMEDIATE LIFTS (FOG COAT)	0.04 GAL./SQ.YD.
ON AGGREGATE SURFACE	0.32 GAL./SQ.YD.
AGGREGATE (PRIME COAT)	0.0015 TONS/SQ.YD.

THE TRAFFIC BARRIER TERMINAL AT THE SOUTHEAST CORNER OF THE STRUCTURE SHALL BE REMOVED AND REPLACED.

IN ADDITION TO THE REQUIREMENTS OF ARTICLE 107.16 THE CONTRACTOR SHALL PROTECT THE SURFACE OF ALL BRIDGE DECKS AND BRIDGE APPROACH PAVEMENTS IN A MANNER SATISFACTORY TO THE ENGINEER BEFORE ANY EQUIPMENT IS ALLOWED TO CROSS THE STRUCTURE. PROTECTION SHALL BE PROVIDED FOR ALL EQUIPMENT AS DEFINED IN ARTICLE 101.16 REGARDLESS IF TRACK MOUNTED OR WHEELED.

PLACEMENT OF FINAL PAVEMENT MARKINGS IS NOT INCLUDED IN THIS CONTRACT.

AFTER A LIFT OF HOT-MIX ASPHALT HAS BEEN PLACED ON A LANE, THAT LANE SHALL REMAIN CLOSED TO TRAFFIC UNTIL THE NEW MAT HAS COOLED TO 150°.

AT ALL LOCATIONS WHERE THE PROPOSED HOT MIX ASPHALT OR CONCRETE PAVEMENT JOINS AN EXISTING HOT MIX ASPHALT OR CONCRETE PAVEMENT, A SAWED JOINT SHALL BE CONSTRUCTED. THE COST OF THIS JOINT WILL BE INCLUDED IN THE COST OF THE HOT MIX ASPHALT SURFACE ITEMS.

THE THICKNESS OF HOT-MIX ASPHALT MIXTURE SHOWN ON THE PLANS IS THE NOMINAL THICKNESS BASED ON VISUAL INSPECTION.

NO DECK PATCHING IS ANTICIPATED BASED ON A VISUAL INSPECTION PERFORMED IN NOVEMBER, 2008. THE ENGINEER WILL PERFORM A DECK SURVEY AFTER REMOVAL OF THE EXISTING HOT-MIX ASPHALT SURFACE IS COMPLETED, AND MAY DETERMINE PATCHING LOCATIONS AND QUANTITIES. PATCHING WILL BE PAID FOR ACCORDING TO ARTICLE 109.04. THE ENGINEER SHALL MARK PATCHES IN THE AS-BUILT PLANS.

COMMITMENTS: NONE

Prepared By: *Joe Zbruniewicz*
DISTRICT STUDIES & PLANS ENGINEER

Examined By: *James Kravits Emery*
DISTRICT LAND ACQUISITION ENGINEER

Examined By: *Carrie Nelson*
DISTRICT PROGRAM DEVELOPMENT ENGINEER

Examined By: *Kevin Hammy*
DISTRICT OPERATIONS ENGINEER

Examined By: *Jim Shuster*
DISTRICT CONSTRUCTION ENGINEER

Examined By: *Bruce Weckel*
DISTRICT MATERIALS ENGINEER

Examined By: *Jim Shuster*
DISTRICT PROJECT IMPLEMENTATION ENGINEER

Examined By: *David J. [Signature]*
ASSISTANT REGIONAL ENGINEER

Approved By: *Mary C. Lamson*
DEPUTY DIRECTOR OF HIGHWAYS, REGION 5 ENGINEER

Dec 5 2008
DATE

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
SUMMARY OF QUANTITIES

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	09 CM BRIDGE REPAIR FY 09-1	WHITE	16	3
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		

CODE NO.	CONSTRUCTION TYPE CODE: SFTY-2A ITEM DESCRIPTION	100% STATE	
		UNIT	TOTAL QUANTITY
40300100	BITUMINOUS MATERIALS (PRIME COAT)	GALLON	5
40600300	AGGREGATE (PRIME COAT)	TON	1
44001005	HOT-MIX ASPHALT SURFACE REMOVAL	SQ YD	835.3
40600990	TEMPORARY RAMP	SQ YD	57
40603320	HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N90	TON	93
50102400	CONCRETE REMOVAL	CU YD	8.1
50300255	CONCRETE SUPERSTRUCTURE	CU YD	7.9
50500405	FURNISHING AND ERECTING STRUCTURAL STEEL	POUND	1690
50800515	BAR SPLICERS	EACH	14
50500715	JACK AND REMOVE EXISTING BEARINGS	EACH	10
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	1030
52000110	PREFORMED JOINT STRIP SEAL	FOOT	65
52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	10
52100520	ANCHOR BOLTS, 1"	EACH	20
59000200	EPOXY CRACK INJECTION	FOOT	57.5
58100200	WATERPROOFING MEMBRANE SYSTEM	SQ YD	803
63304385	TRAFFIC BARRIER TERMINAL REMOVAL, TYPE 1	EACH	1
67100100	MOBILIZATION	L SUM	1
70100100	TRAFFIC CONTROL AND PROTECTION, STANDARD 701316	EACH	1
70100450	TRAFFIC CONTROL AND PROTECTION, STANDARD 701201	L SUM	1
70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	1
* 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	1
X0325305	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT	96.4

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	D9 CM BRIDGE REPAIR FY 09-1	WHITE	16	4
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		

HOT-MIX ASPHALT MIX DESIGN

LOCATION:	HOT MIX ASPHALT SURFACE COURSE
MIXTURE USE(S):	HOT-MIX ASPHALT SURFACE COURSE, MIX C, N90
AC/PG:	PG64-22
RAP % (MAX):	10
DESIGN AIR VOIDS:	4.0%, 90 GYRATION DESIGN
MIXTURE COMPOSITION: (GRADATION MIXTURE)	IL - 9.5 mm OR IL - 12.5
FRICTION AGGREGATE:	C SURFACE

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL NOTES

All structural steel shall conform to AASHTO Classification M-270 Gr. 36, unless otherwise noted.
Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.
Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

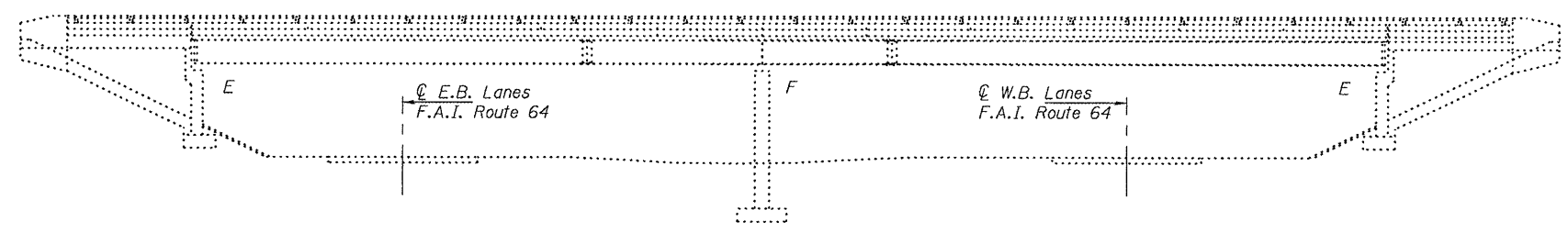
All structural steel shall be shop painted with the inorganic zinc rich primer per AASHTO M300, Type 1. Cost included with Furnishing and Erecting Structural Steel.

Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

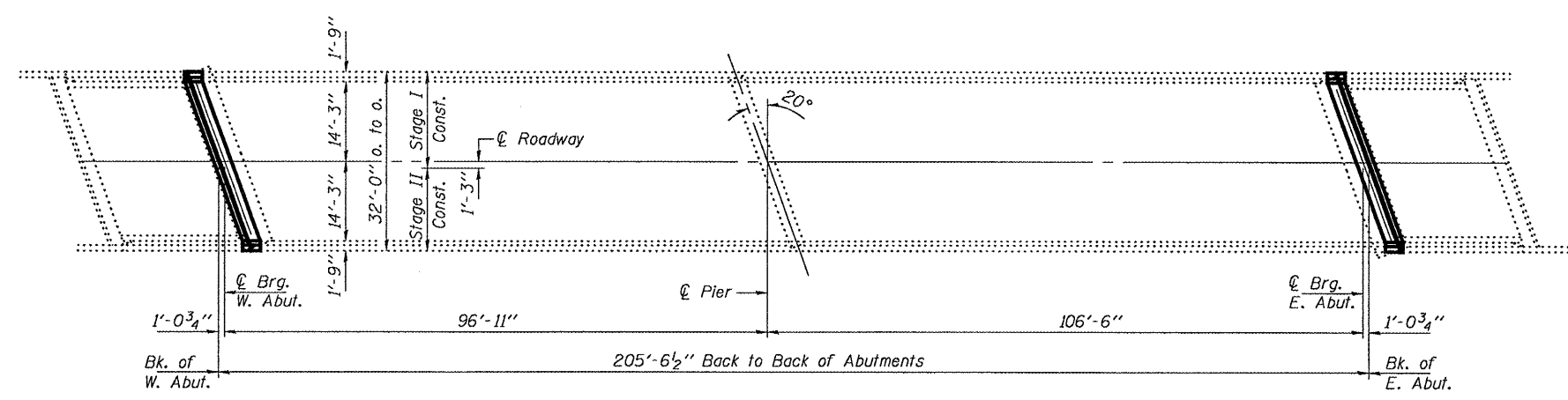
The deck surface shall have its final finish tined according to Article 420.09(e)(1) of the Standard Specifications. Cost included with Concrete Superstructure.

If the analysis submitted to the Contractor for the jacking/temporary support system to be used shows temporary stiffeners are required to prevent web crippling or buckling, the stiffeners shall be steel and bolted to the web. If stiffeners are not required, hardwood timbers shall be installed tightly between the top and bottom flange to prevent flange rotation.

Joint openings shall be adjusted according to Article 520.04 of the Std. Specs. when the deck is poured at an ambient temperature other than 50° F.



ELEVATION



PLAN

Remove and Replace Bearings at Abutments
Remove Preformed Joint Sealer and Install Preformed Joint Strip Seal

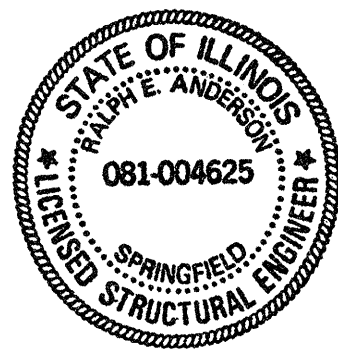
TOTAL BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Furnishing and Erecting Structural Steel	Pound	1690
Anchor Bolts, 1"Ø	Each	20
Elastomeric Bearing Assembly, Type I	Each	10
Jack and Remove Existing Bearings	Each	10
Concrete Removal	Cu. Yd.	8.1
Concrete Superstructure	Cu. Yd.	7.9
Preformed Joint Strip Seal	Foot	65
Reinforcement Bars, Epoxy Coated	Pound	1030
Bar Splicers	Each	14

PLAN & ELEVATION
F.A.S. 815 (CH-20)
OVER F.A.I. 64
SN 097-0044

DESIGNED	<i>[Signature]</i>
CHECKED	<i>[Signature]</i>
DRAWN	Kyle M. Steffen
CHECKED	ASB ATH

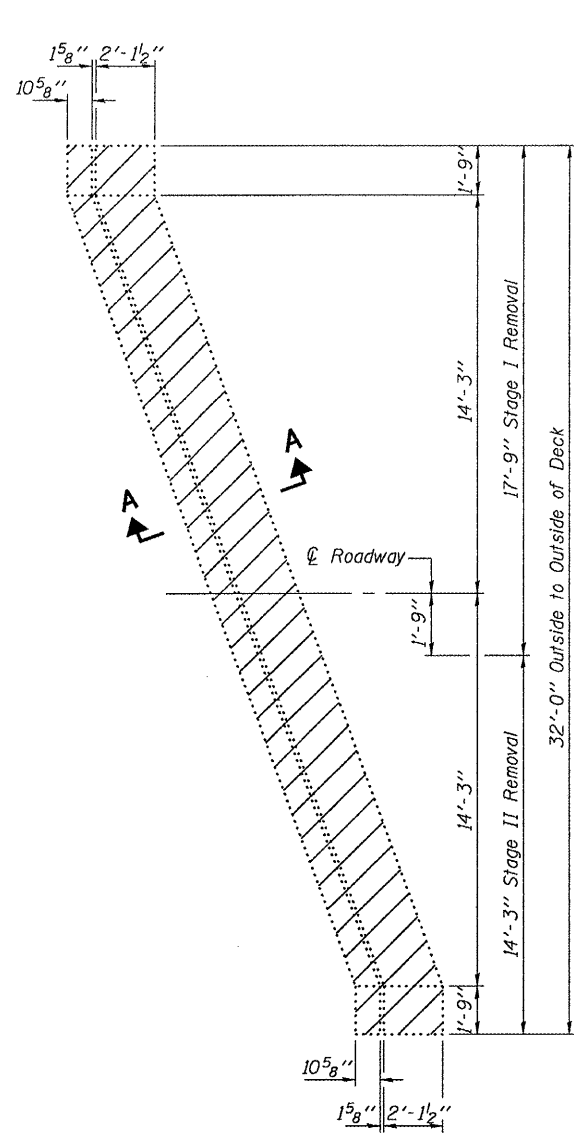
JANUARY 13, 2009
EXAMINED *[Signature]*
PASSED *[Signature]*
ENGINEER OF STRUCTURAL SERVICES
ENGINEER OF BRIDGES AND STRUCTURES



Expires: November 30, 2010

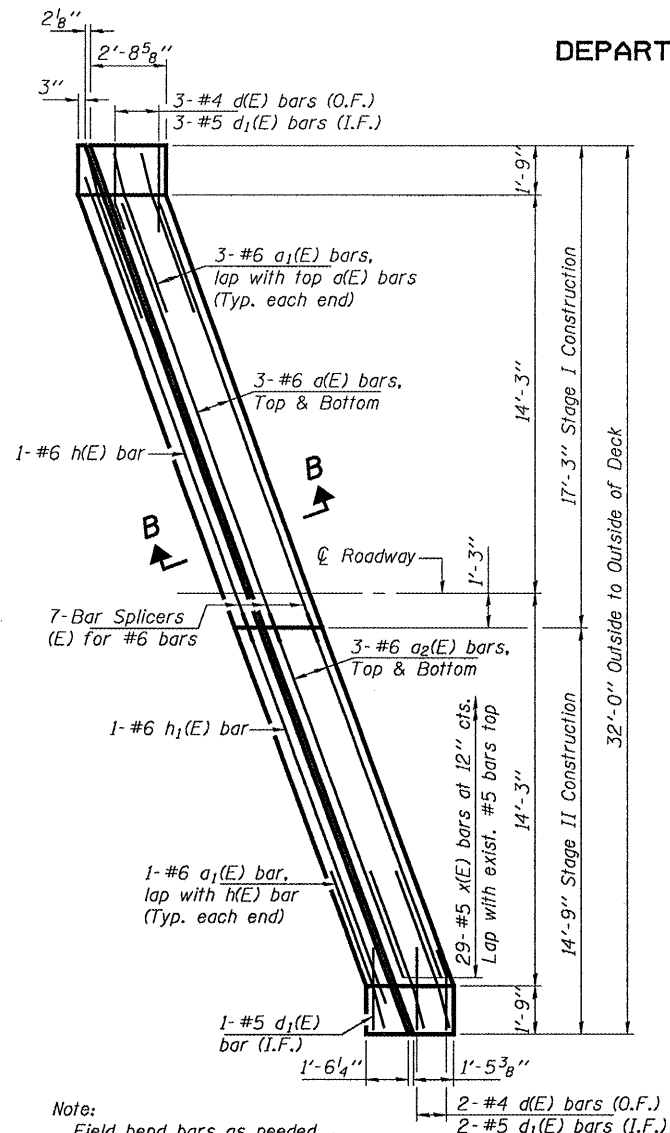
SHEET NO. 1 5 SHEETS	F.A.S. RTE. 815	SECTION D9 CM BRIDGE REPAIR FY09-1	COUNTY WHITE	TOTAL SHEETS 16	SHEET NO. 5
	CONTRACT NO. 78094				
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



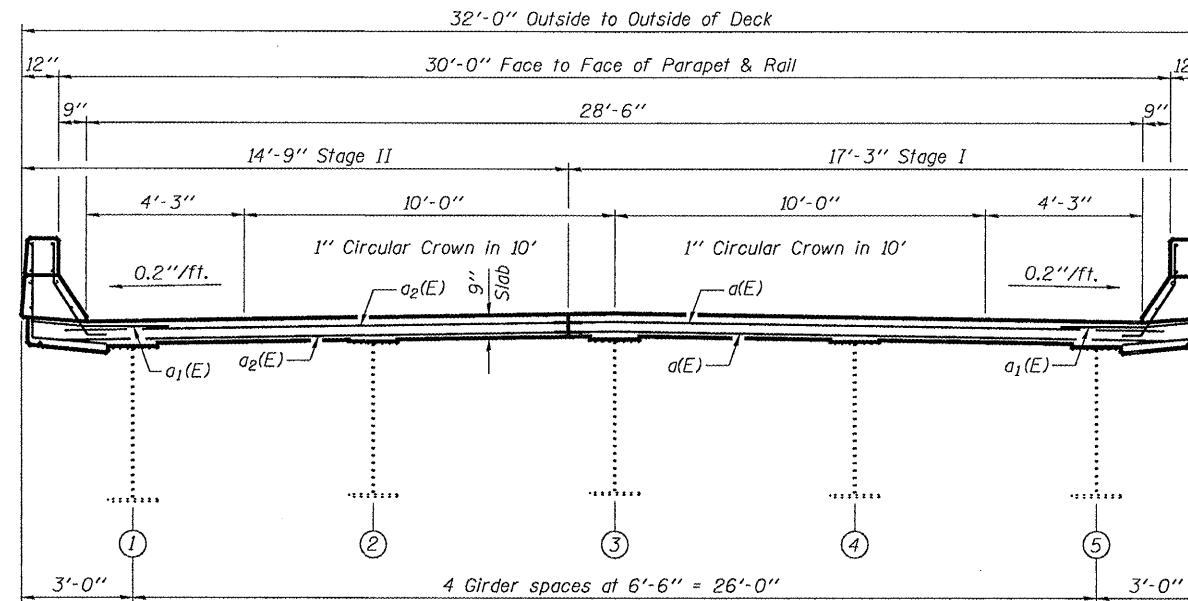
PARTIAL PLAN AT W. ABUT.

(Showing Concrete Removal)
E. Abut. similar by 180° rotation



PARTIAL PLAN AT W. ABUT.

(Showing Reinforcement)
E. Abut. similar by 180° rotation

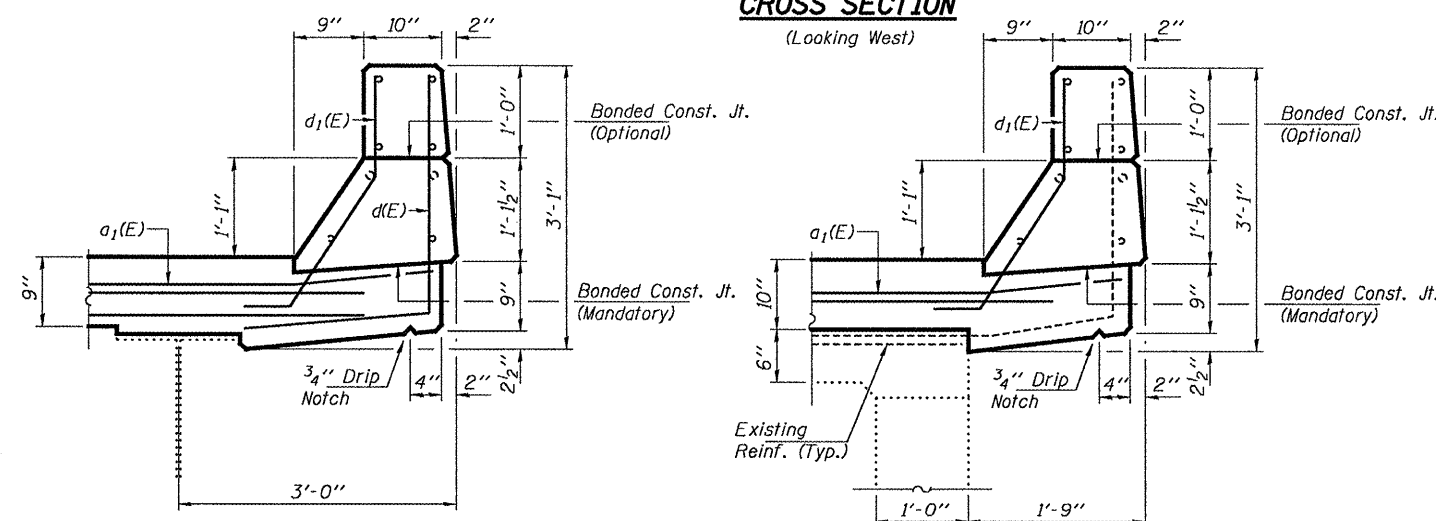


SECTION THRU PARAPET

(Bridge Side)

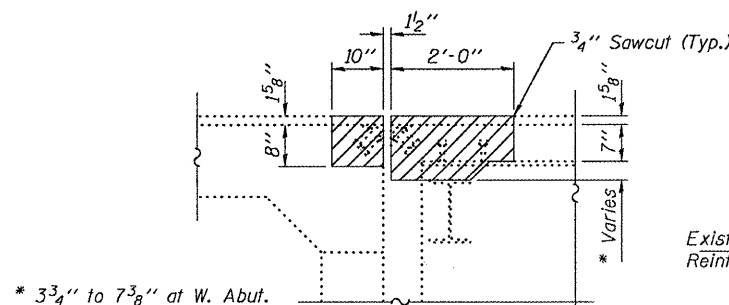
CROSS SECTION

(Looking West)



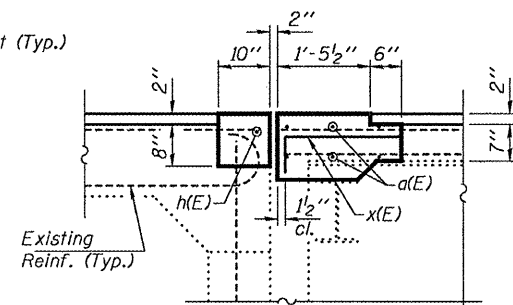
SECTION THRU PARAPET

(Approach Side)

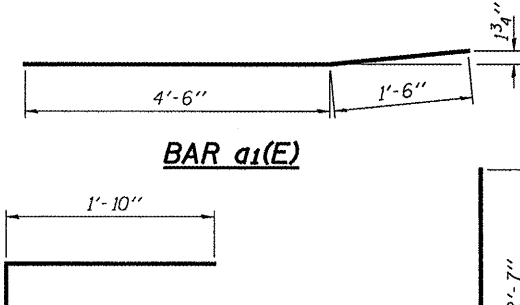


SECTION A-A

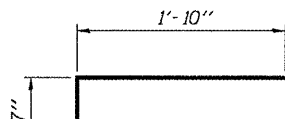
* 3 3/4" to 7 3/8" at W. Abut.
3 3/4" to 6 1/4" at E. Abut.



SECTION B-B



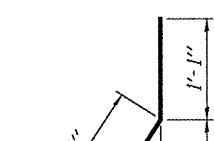
BAR a1(E)



BAR x(E)



BAR d(E)



BAR d1(E)

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	12	#6	17'-1"	—
a1(E)	16	#6	6'-0"	—
a2(E)	12	#6	14'-6"	—
d(E)	10	#4	4'-7"	J
d1(E)	12	#5	3'-3"	J
h(E)	2	#6	17'-1"	—
h1(E)	2	#6	14'-6"	—
x(E)	58	#5	2'-5"	┌
Concrete Removal		Cu. Yd.	8.1	
Concrete Superstructure		Cu. Yd.	7.9	
Reinforcement Bars, Epoxy Coated		Pound	1030	

REPAIR DETAILS
F.A.S. 815 (CH-20)
OVER F.A.I. 64
SN 097-0044

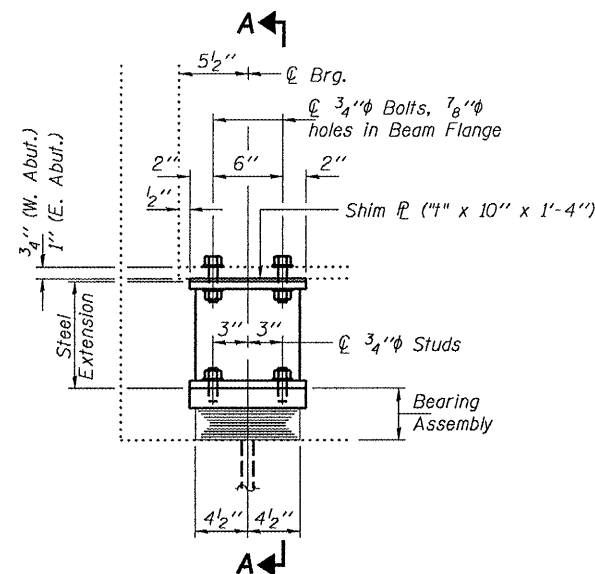
DESIGNED	AJB
CHECKED	ATH
DRAWN	Kyle M. Steffen
CHECKED	AJB ATH

JANUARY 13, 2009
EXAMINED *Carl Perry*
ENGINEER OF STRUCTURAL SERVICES
PASSED *Ralph E. Anderson*
ENGINEER OF BRIDGES AND STRUCTURES

Note:
Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Weld Rail Strip Seal Joint, deck dimensions may require adjustments to satisfy the details on sheet 4 of 5.

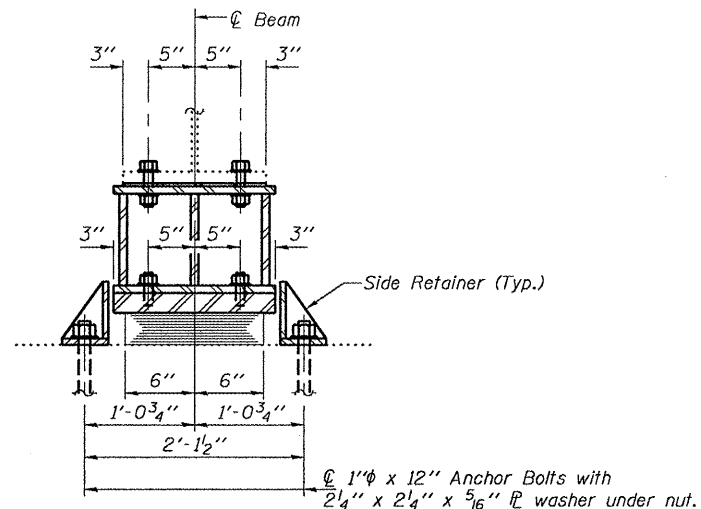
SHEET NO. 2 5 SHEETS	F.A.S. RTE. 815	SECTION D9 CM BRIDGE REPAIR FY09-1	COUNTY WHITE	TOTAL SHEETS 16	SHEET NO. 6
	FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT	CONTRACT NO. 78094	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



ELEVATION AT ABUTMENT

TYPE I ELASTOMERIC EXP. BRG.

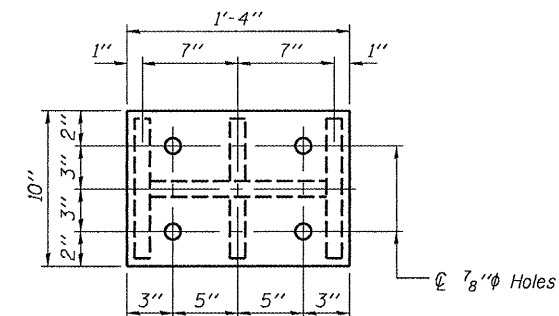


SECTION A-A

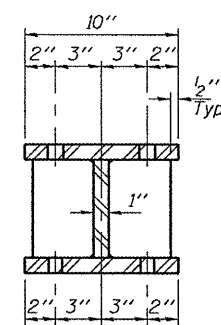
BEAM REACTIONS

	W. Abut.	E. Abut.
R _∅	(K) 37.15	43.83
R _∅	(K) 28.11	28.59
Imp.	(K) 6.35	6.18
R (Total)	(K) 71.61	78.60

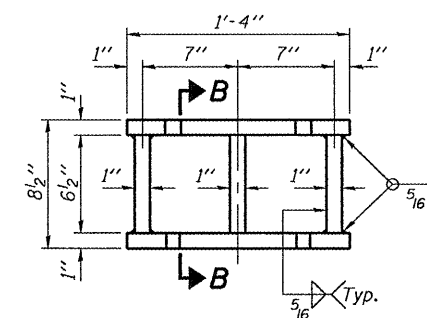
Notes:
Diaphragm removal and reinstallation may be required to facilitate drilling holes. Cost included with Furnishing and Erecting Structural Steel.
New steel extensions, shim plates and connection bolts are included with Furnishing and Erecting Structural Steel.
Prior to ordering any material, the Contractor shall verify in the field all bearing height and shim thickness dimensions. Min. Jack capacity = 50 Tons E. Abut., 45 Tons W. Abut.
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (F_y=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
Side retainers shall be included in the cost of Elastomeric Bearing Assembly, Type I.



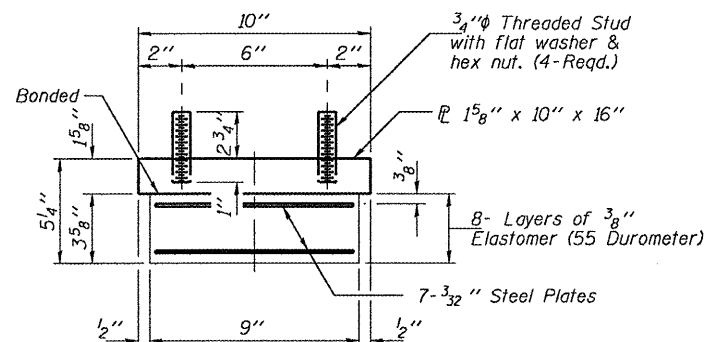
PLAN TOP AND BOTTOM PLATE



SECTION B-B

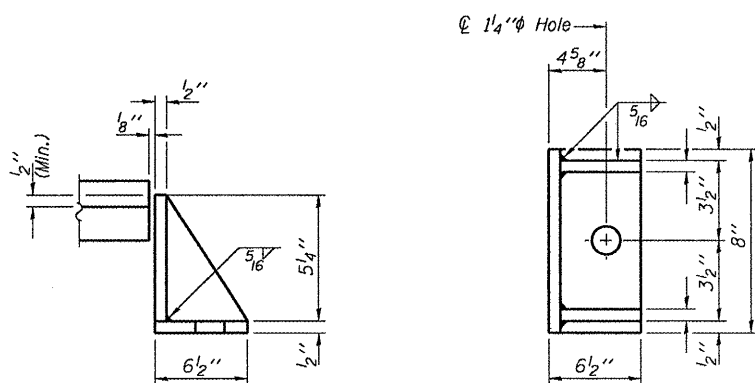


STEEL EXTENSION DETAIL



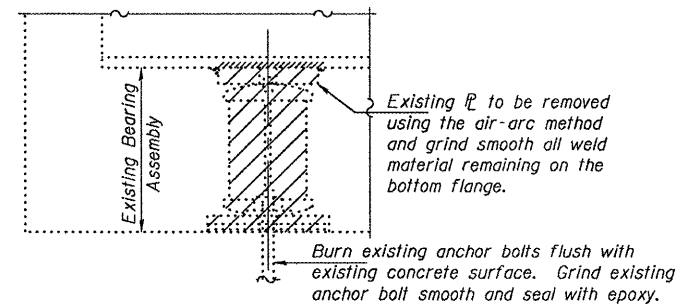
BEARING ASSEMBLY

Note:
Shim plates shall not be placed under Bearing Assembly.



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



EXISTING BEARING REMOVAL DETAIL

Cost included with Jack and Remove Existing Bearings.

SHIM PLATES

Girder	Abut.	Thickness
2	West	3/8"

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type I	Each	10
Jack and Remove Existing Bearings	Each	10
Furnishing and Erecting Structural Steel	Pound	1690
Anchor Bolts, 1" ∅	Each	20

BEARING REPLACEMENT DETAILS
F.A.S. 815 (CH-20) OVER F.A.I. 64
SN 097-0044

DESIGNED	AJB
CHECKED	ATH
DRAWN	Kyle M. Steffen
CHECKED	AJB ATH

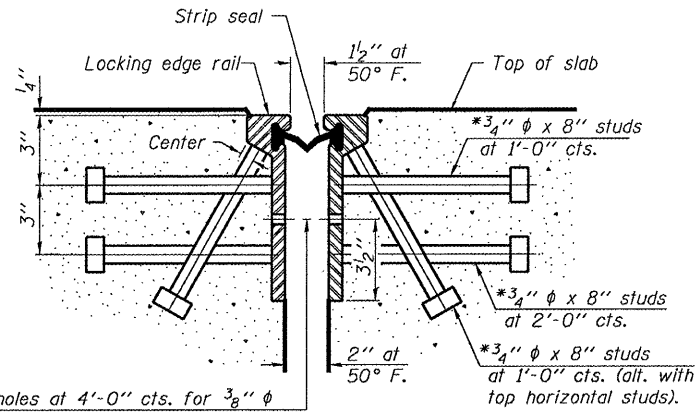
JANUARY 13, 2009
EXAMINED <i>Carl Kruse</i> ENGINEER OF STRUCTURAL SERVICES
PASSED <i>Ralph E. Anderson</i> ENGINEER OF BRIDGES AND STRUCTURES

TYI/REPS 11-01-2006

SHEET NO. 3	F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	815	D9 CM BRIDGE REPAIR FY09-1	WHITE	16	7
5 SHEETS	FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT	CONTRACT NO. 78094	

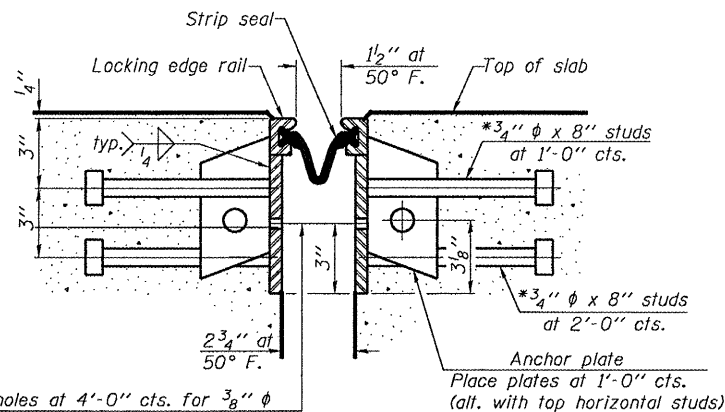
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

*Granular or solid Flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



7/16" ϕ holes at 4'-0" cts. for 3/8" ϕ bolts. All bolts shall be burned, sawed, or chipped off flush with the plates after forms are removed, typ.

SECTION THRU ROLLED RAIL JOINT



7/16" ϕ holes at 4'-0" cts. for 3/8" ϕ bolts. All bolts shall be burned, sawed, or chipped off flush with the plates after forms are removed, typ.

SECTION THRU WELDED RAIL JOINT

Notes:

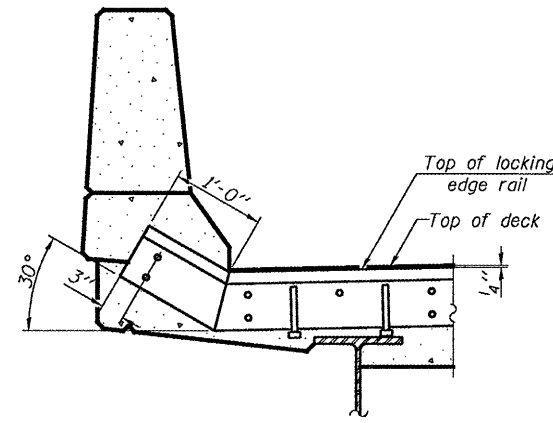
The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

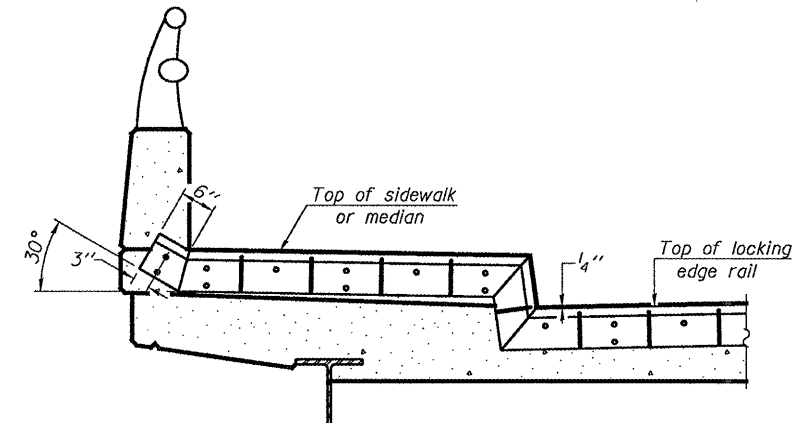
The manufacturer's recommended installation methods shall be followed.

The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

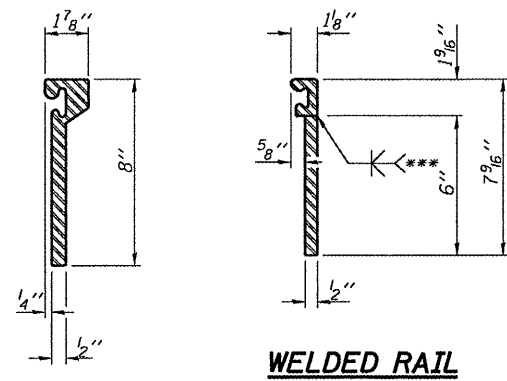


AT PARAPET

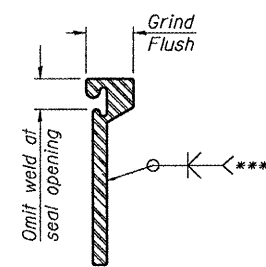


AT SIDEWALK OR MEDIAN

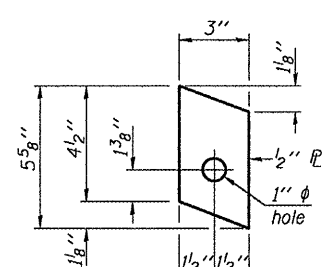
Shorter plates with a single row of studs at 12" cts. may be necessary on medians which are shallower than 9". See manufacturer's recommendation.



WELDED RAIL



***Back gouge not required if complete joint penetration is verified by mock-up.



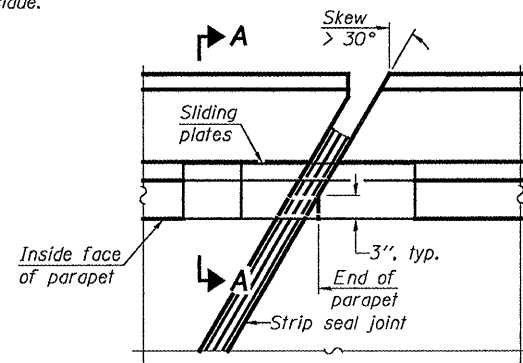
ANCHOR PLATE
(for welded rail)

ROLLED EXTRUDED RAIL

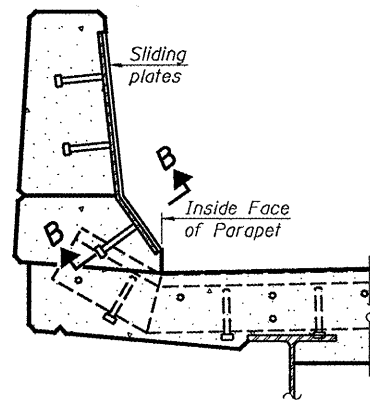
LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.

LOCKING EDGE RAILS



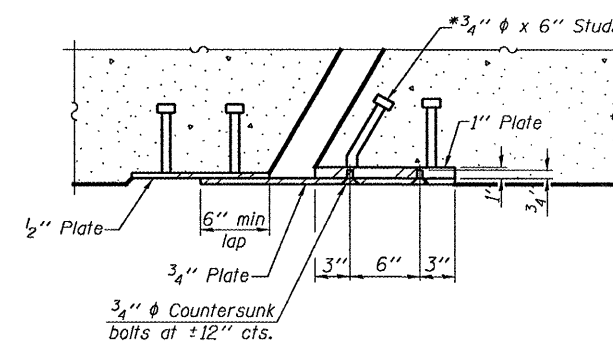
PLAN



SECTION A-A

POINT BLOCK DETAILS
(for skews > 30°)

TYPICAL END TREATMENTS



SECTION B-B

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	65

PREFORMED JOINT STRIP SEAL
F.A.S. 815 (CH-20) OVER F.A.I. 64
SN 097-0044

DESIGNED	AJB
CHECKED	ATH
DRAWN	Kyle M. Steffen
CHECKED	AJB ATH

JANUARY 13, 2009
EXAMINED *Carl Perry*
PASSED *Ralph E. Anderson*
ENGINEER OF STRUCTURAL SERVICES
ENGINEER OF BRIDGES AND STRUCTURES

EJ-SSJ 5-16-08

SHEET NO. 4 5 SHEETS	F.A.S. RTE. 815	SECTION D9 CM BRIDGE REPAIR FY09-1	COUNTY WHITE	TOTAL SHEETS 16	SHEET NO. 8
	FED. ROAD DIST. NO. ILLINOIS		FED. AID PROJECT CONTRACT NO. 78094		

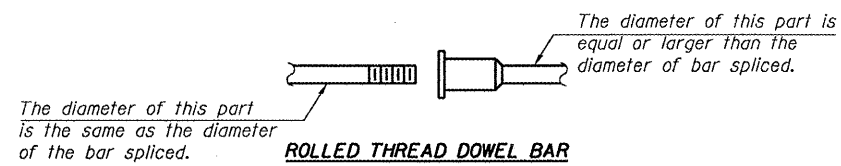
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NOTES

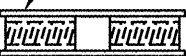
Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.
Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length.
All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.
Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.
Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

- ① Minimum Capacity = $1.25 \times f_y \times A_f$
(Tension in kips)
 - ② Minimum *Pull-out Strength = $0.66 \times f_y \times A_f$
(Tension in kips)
- Where f_y = Yield strength of lapped reinforcement bars in ksi.
 A_f = Tensile stress area of lapped reinforcement bars.
* = 28 day concrete

BAR SPLICER ASSEMBLIES			
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Strength Requirements	
		Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension
#4	1'-8"	14.7	7.9
#5	2'-0"	23.0	12.3
#6	2'-7"	33.1	17.4
#7	3'-5"	45.1	23.8
#8	4'-6"	58.9	31.3
#9	5'-9"	75.0	39.6
#10	7'-3"	95.0	50.3
#11	9'-0"	117.4	61.8

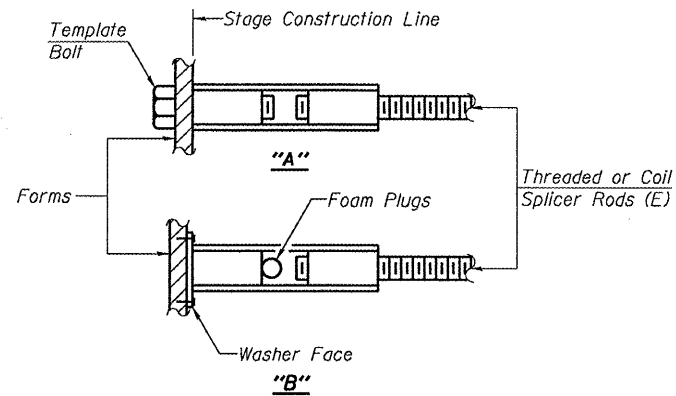


Wire Connector



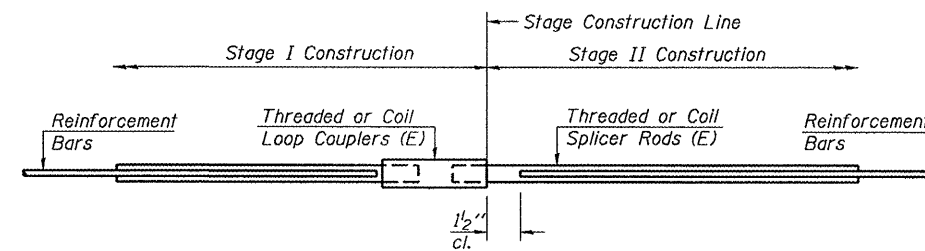
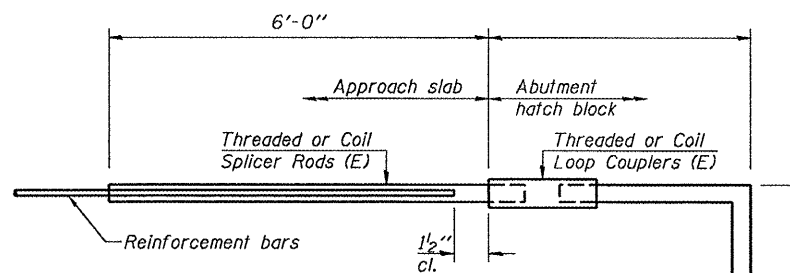
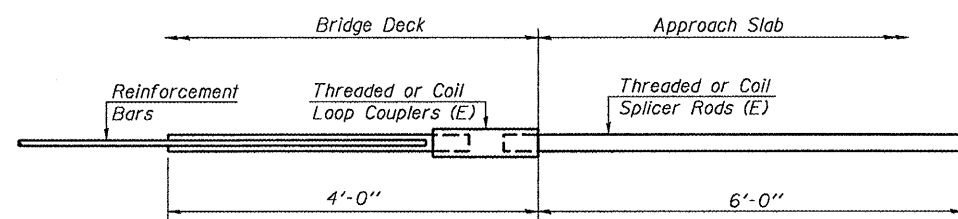
BAR SPLICER ASSEMBLY ALTERNATIVES

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.
"B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
(E) : Indicates epoxy coating.



Bar Size	No. Assemblies Required	Location
#6	14	Abutments

Bar Splicer for #5 bar	
Min. Capacity =	23.0 kips - tension
Min. Pull-out Strength =	12.3 kips - tension
No. Required =	

Bar Splicer for #5 bar	
Min. Capacity =	23.0 kips - tension
Min. Pull-out Strength =	12.3 kips - tension
No. Required =	

BAR SPLICER ASSEMBLY DETAILS
F.A.S. 815 (CH-20) OVER F.A.I. 64
SN 097-0044

DESIGNED	AJB
CHECKED	ATH
DRAWN	Kyle M. Steffen
CHECKED	AJB ATH

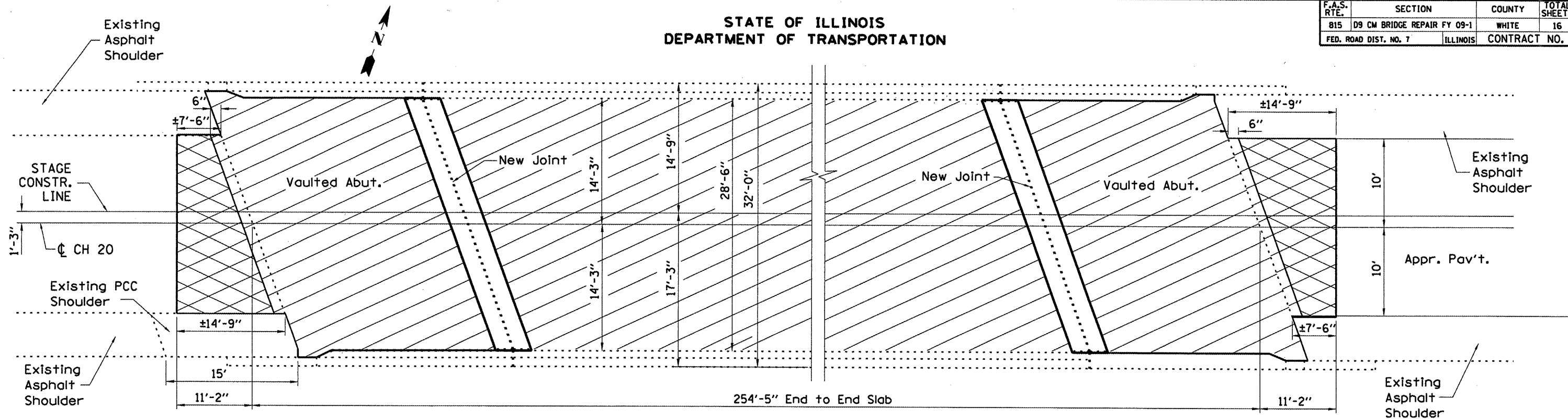
JANUARY 13, 2009
EXAMINED *Carl Perry*
ENGINEER OF STRUCTURAL SERVICES
PASSED *Ralph E. Anderson*
ENGINEER OF BRIDGES AND STRUCTURES

BSD-1 5-16-08

SHEET NO. 5 5 SHEETS	F.A.S. RTE. 815	SECTION D9 CM BRIDGE REPAIR FY09-1	COUNTY WHITE	TOTAL SHEETS 16	SHEET NO. 9
	FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT	CONTRACT NO. 78094	

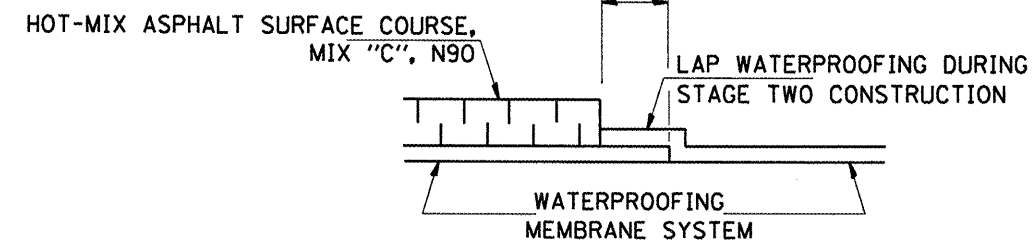
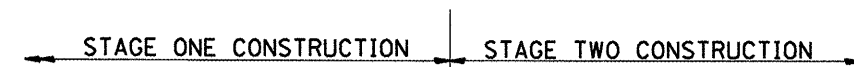
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	D9 CM BRIDGE REPAIR FY 09-1	WHITE	16	10
FED. ROAD DIST. NO. 7		ILLINOIS	CONTRACT NO. 78094	

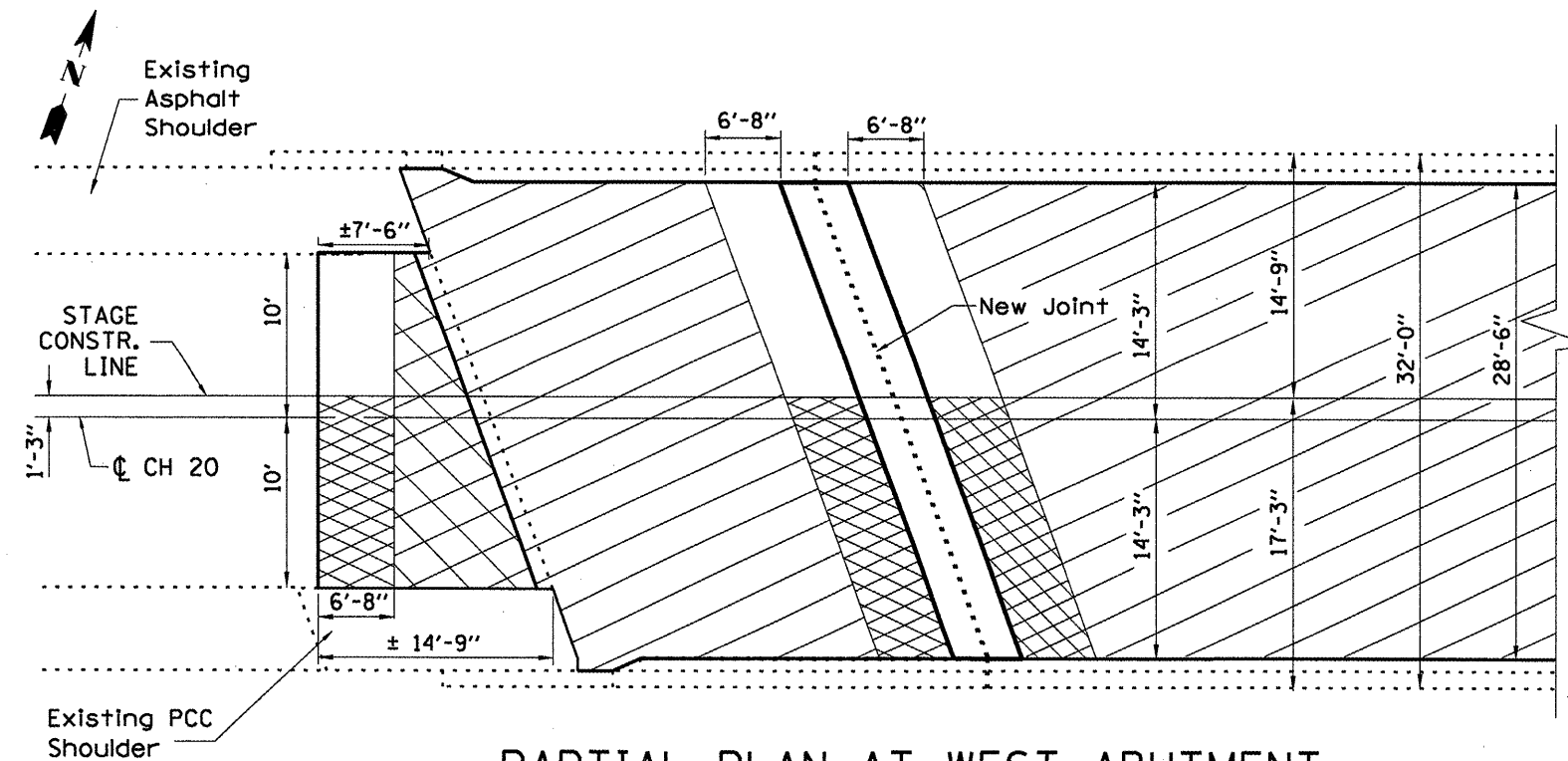


PLAN SHOWING WEARING SURFACE

- HOT-MIX ASPHALT SURFACE REMOVAL
HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N90
BITUMINOUS MATERIALS, PRIME COAT
AGGREGATE (PRIME COAT)
- HOT-MIX ASPHALT SURFACE REMOVAL
HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N90
WATERPROOFING MEMBRANE SYSTEM

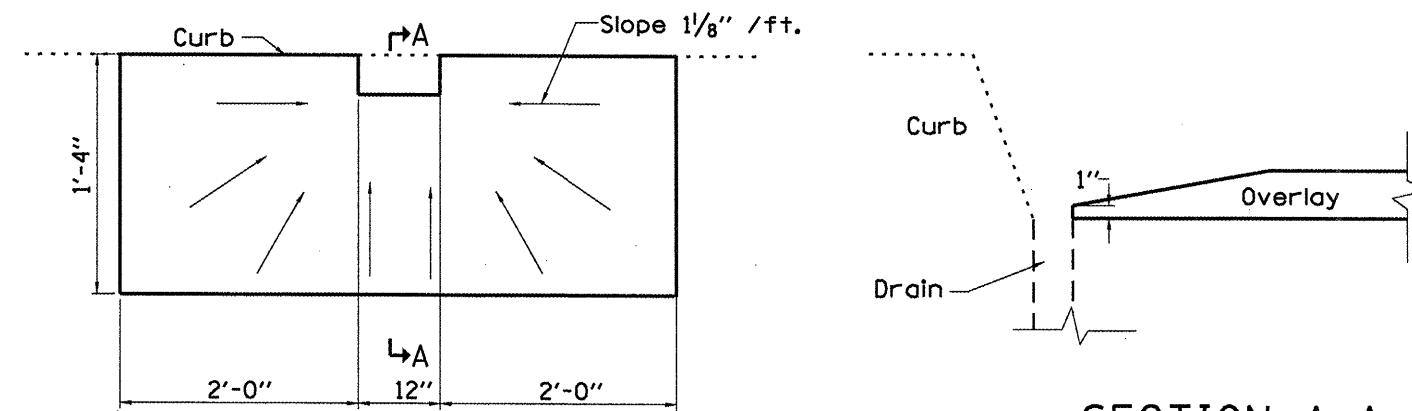


WATERPROOFING TREATMENT
AT STAGE CONSTRUCTION



PARTIAL PLAN AT WEST ABUTMENT
(SHOWING TEMPORARY RAMPS)
EAST ABUTMENT SIMILAR BY 180° ROTATION

- TEMPORARY RAMP

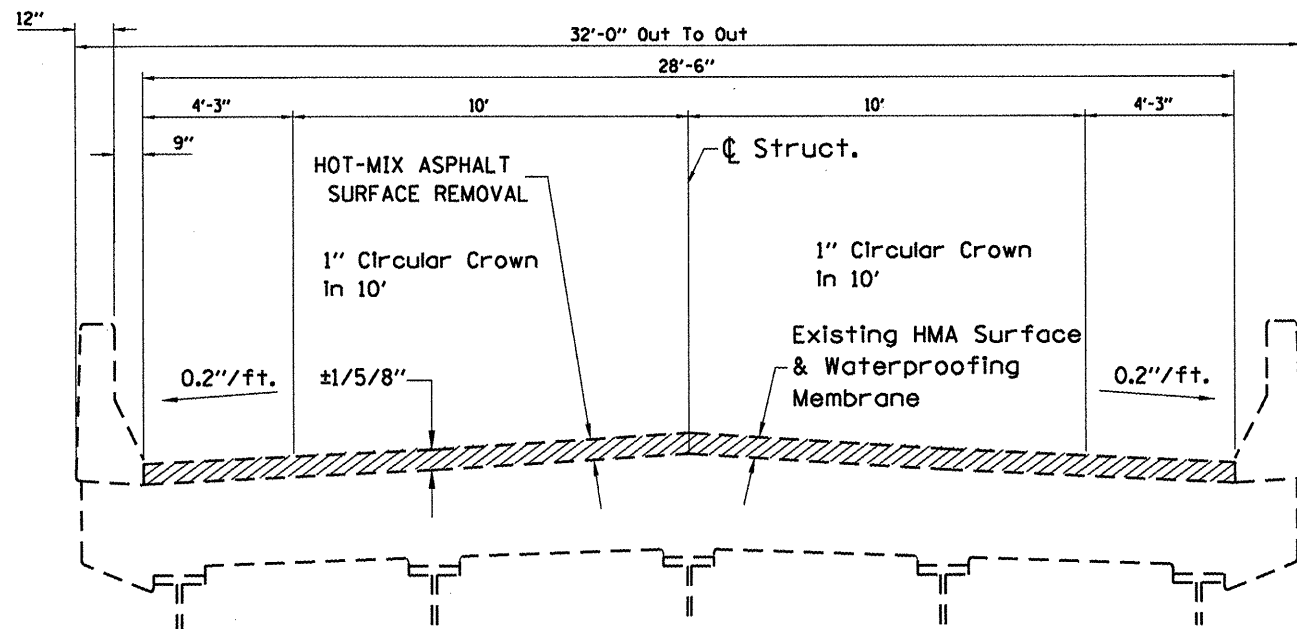


OVERLAY TREATMENT AT
DECK DRAIN LOCATION

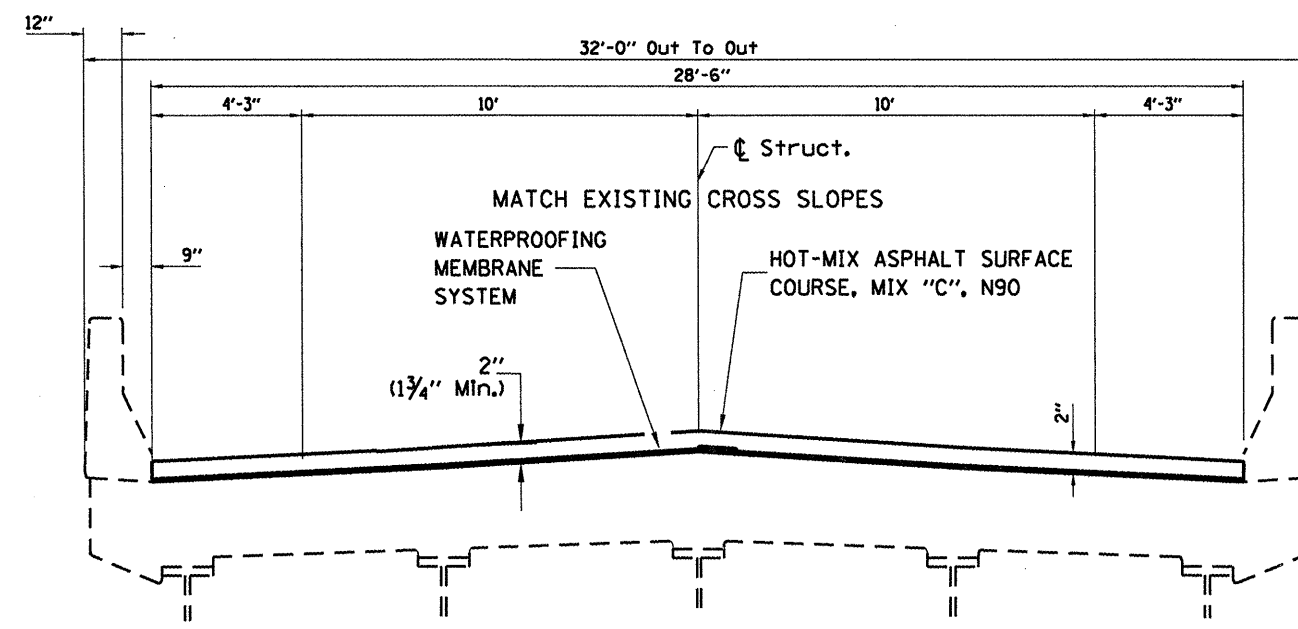
SECTION A-A

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

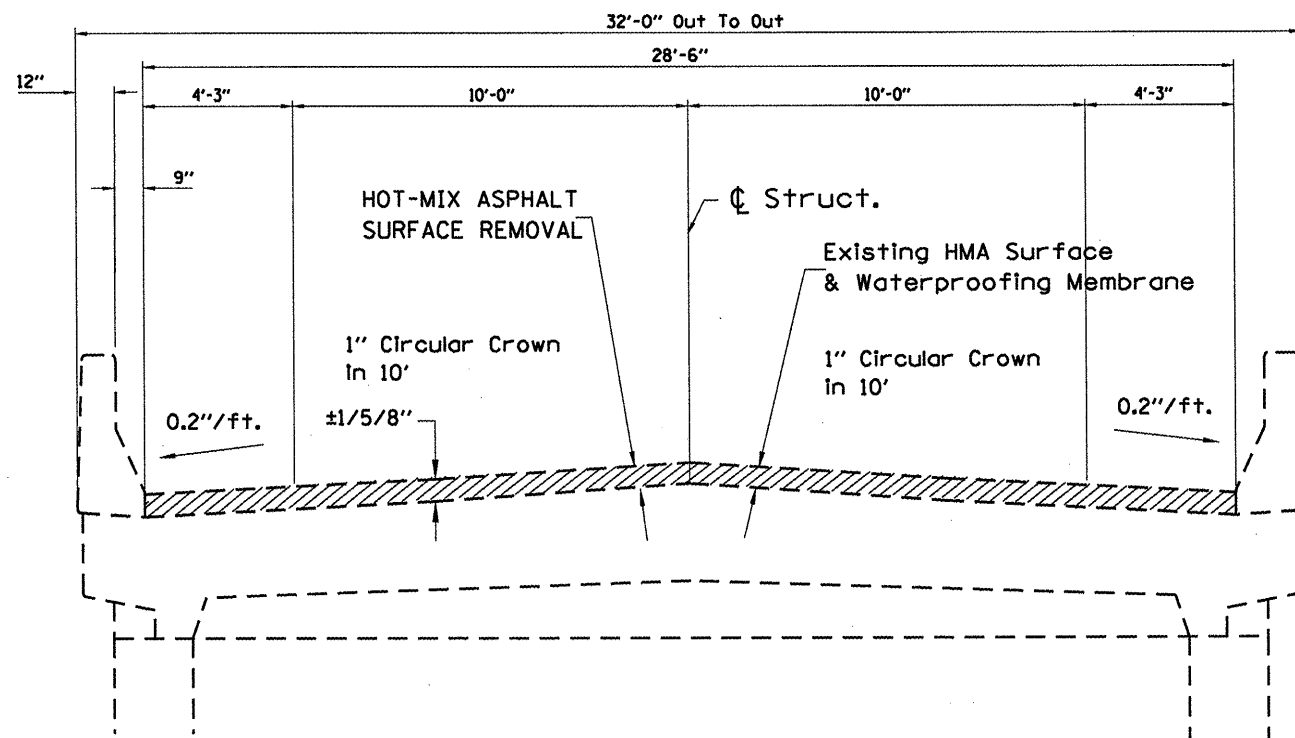
F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
B15	D9 CM BRIDGE REPAIR FY 09-1	WHITE	16	11
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		



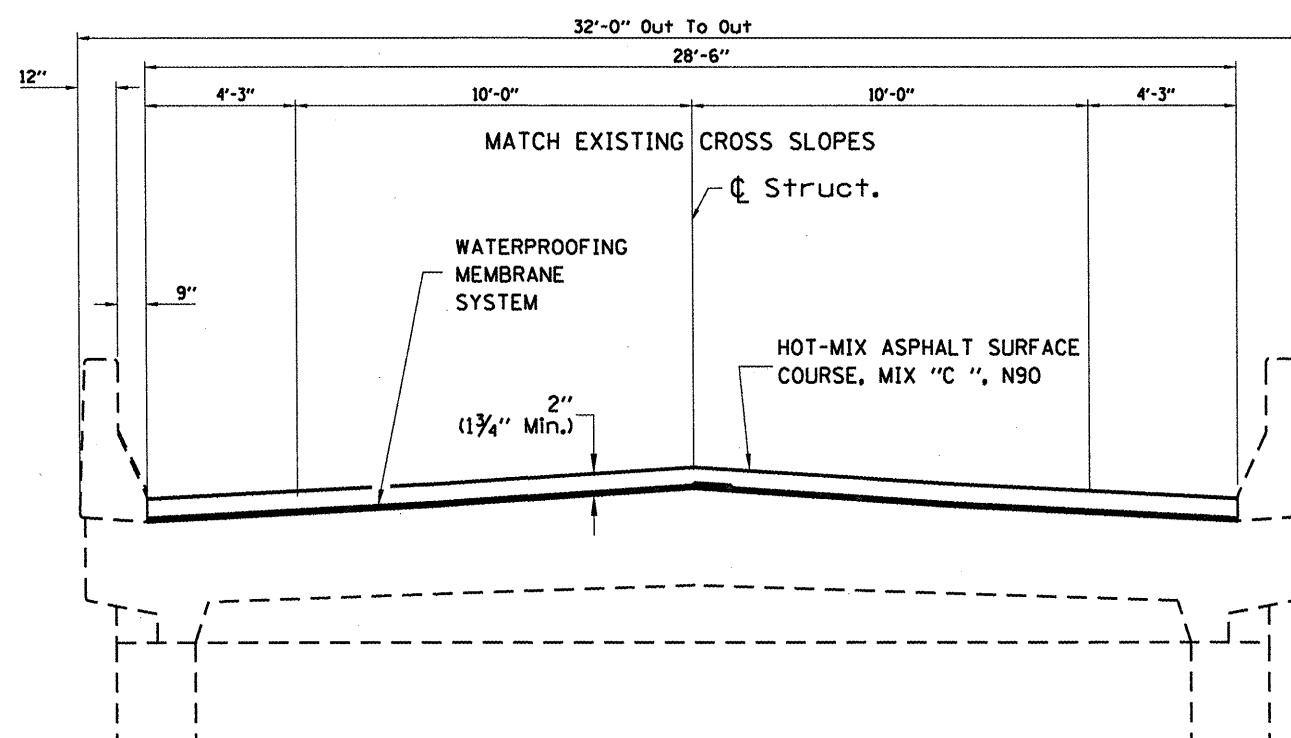
TYPICAL SECTION MID-SPAN
EXISTING BRIDGE DECK SURFACE



TYPICAL SECTION
PROPOSED BRIDGE DECK SURFACE



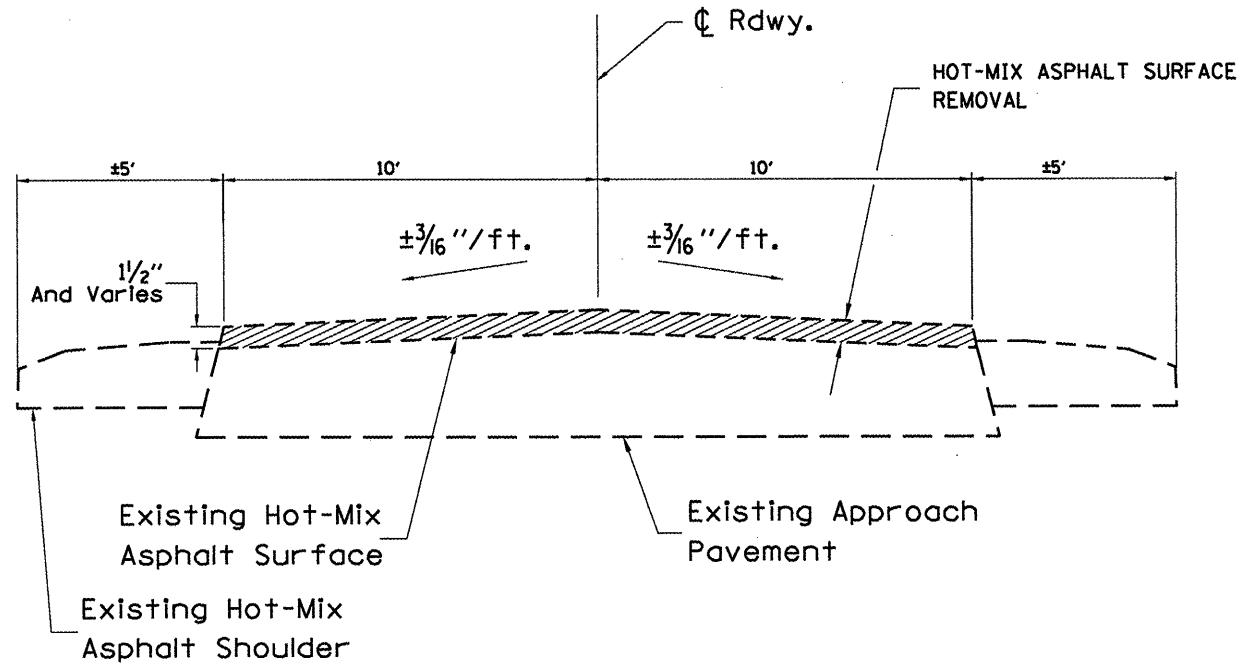
TYPICAL SECTION
EXISTING VAULTED ABUTMENT SURFACE



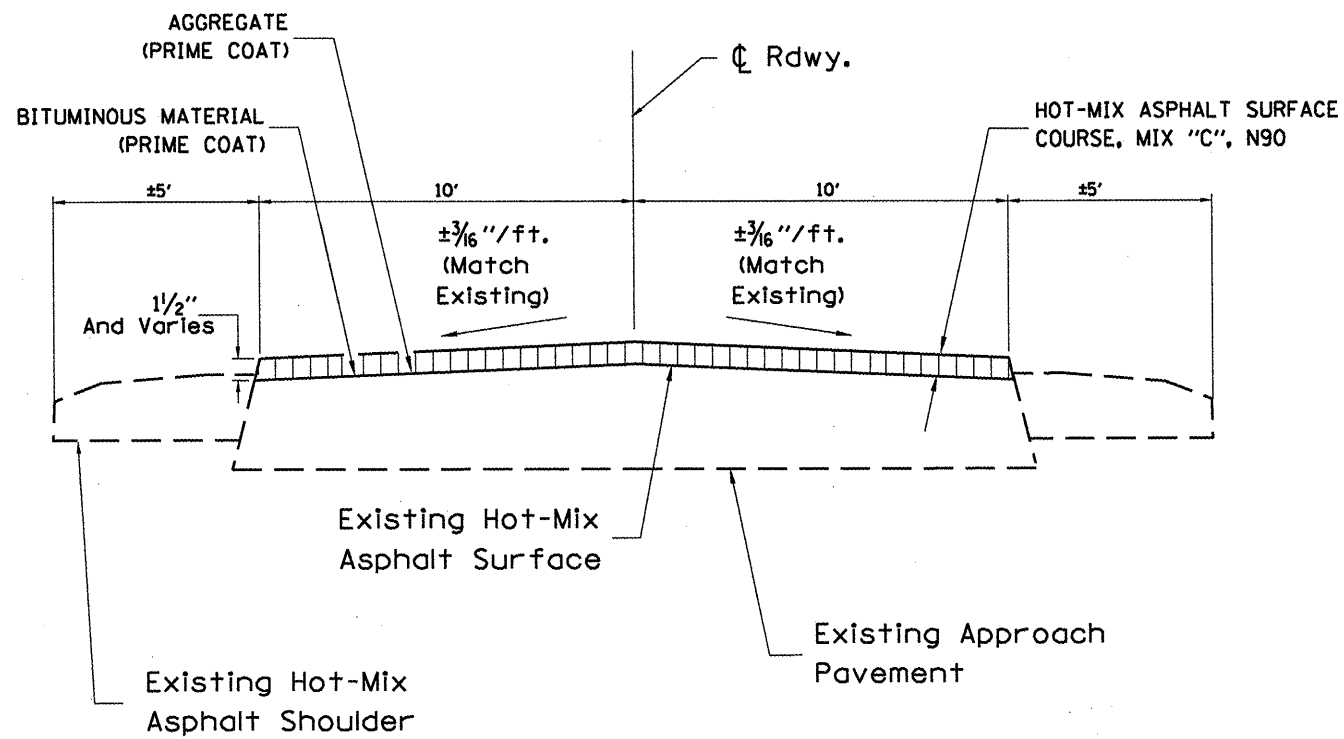
TYPICAL SECTION
PROPOSED VAULTED ABUTMENT SURFACE

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

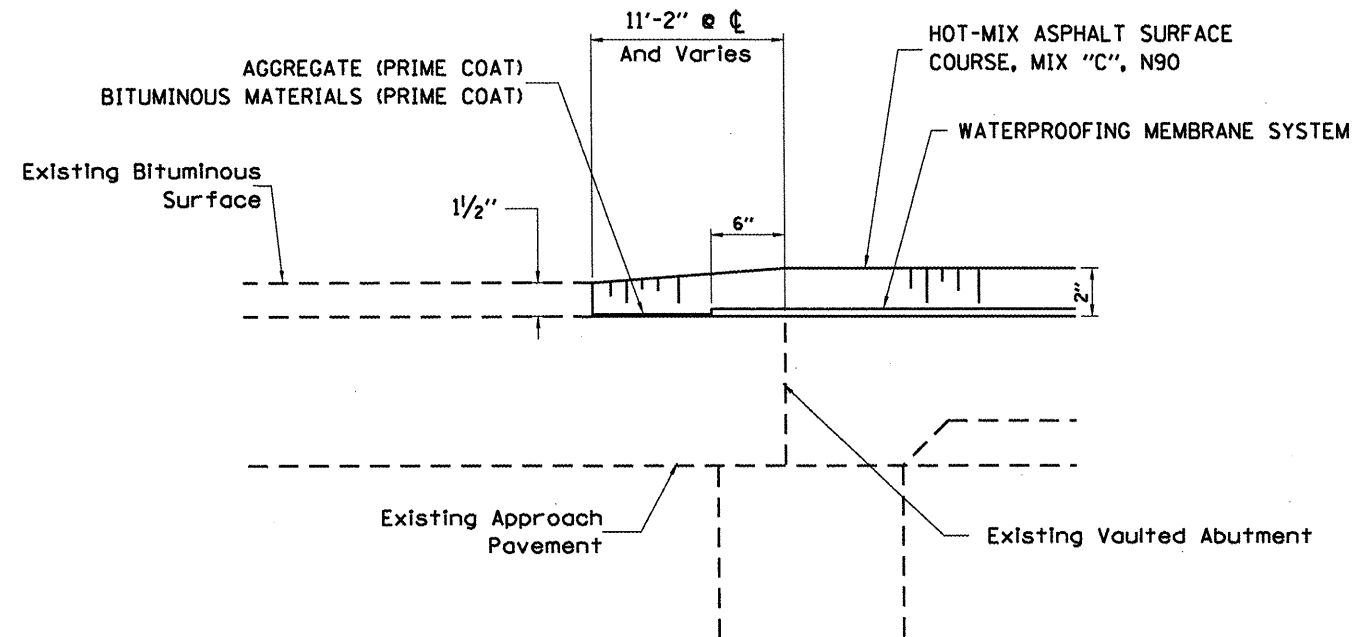
F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	D9 CM BRIDGE REPAIR FY 09-1	WHITE	16	12
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		



APPROACH PAVEMENT
EXISTING SURFACE



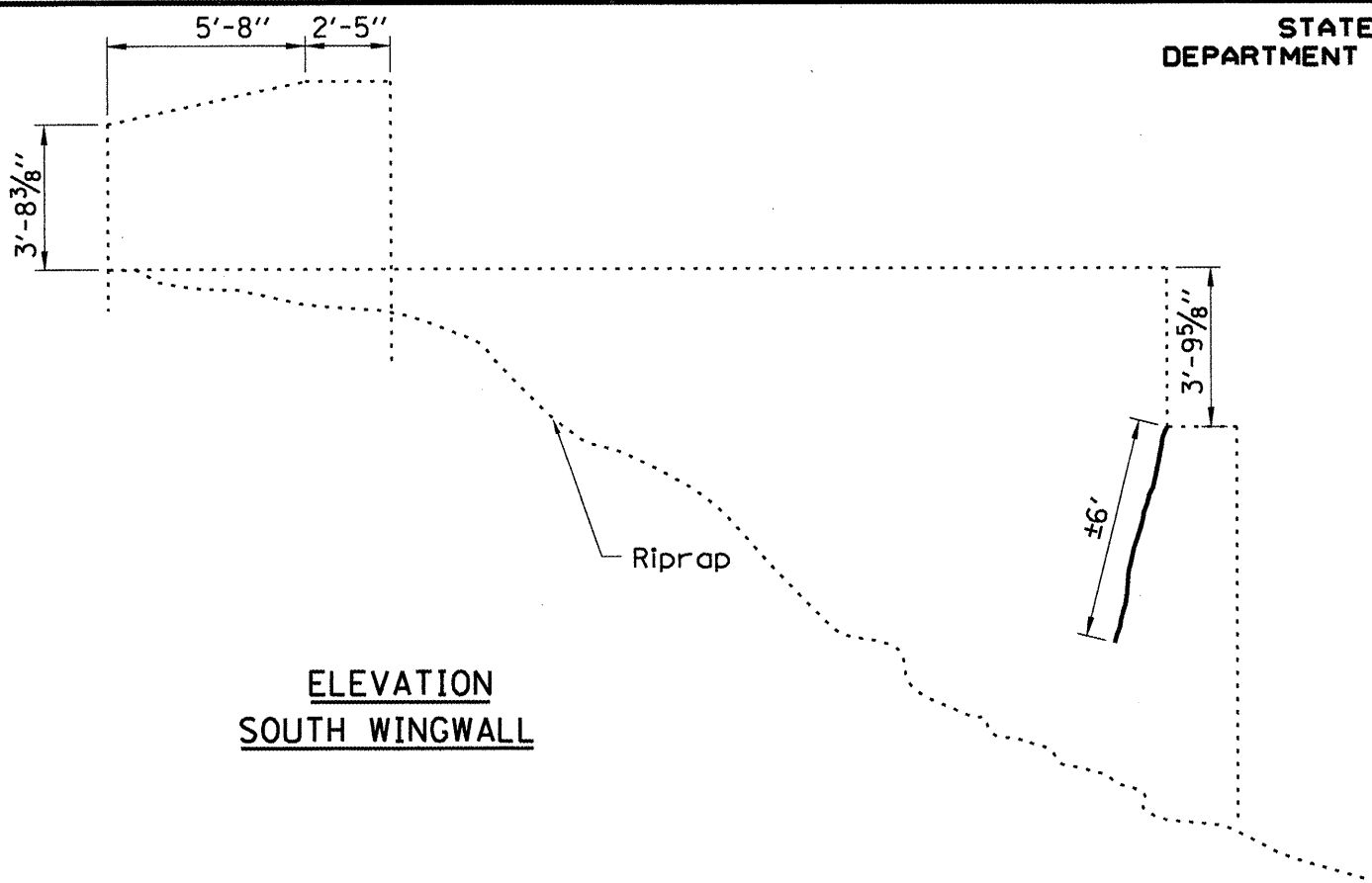
APPROACH PAVEMENT
PROPOSED SURFACE



SECTION THRU ABUTMENT

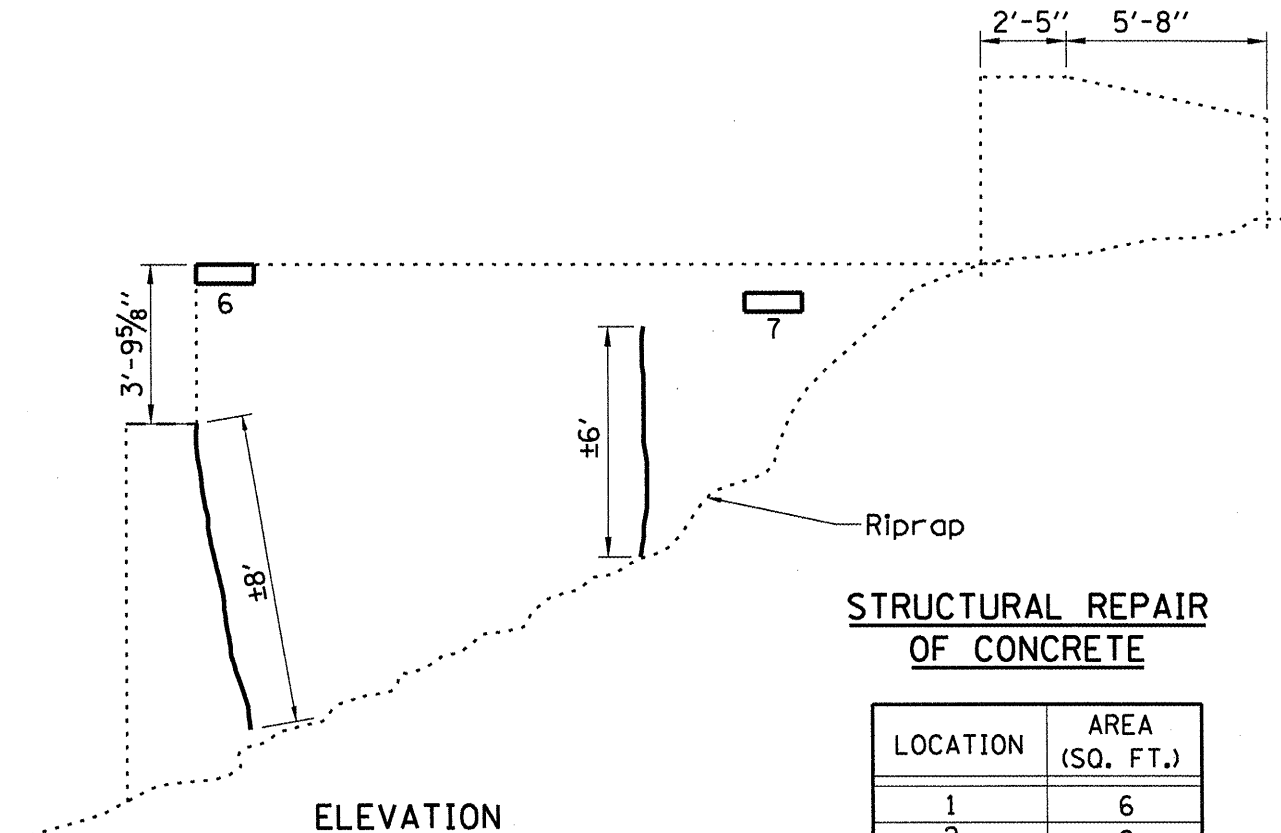
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	09 CM BRIDGE REPAIR FY 09-1	WHITE	16	13
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		



ELEVATION SOUTH WINGWALL

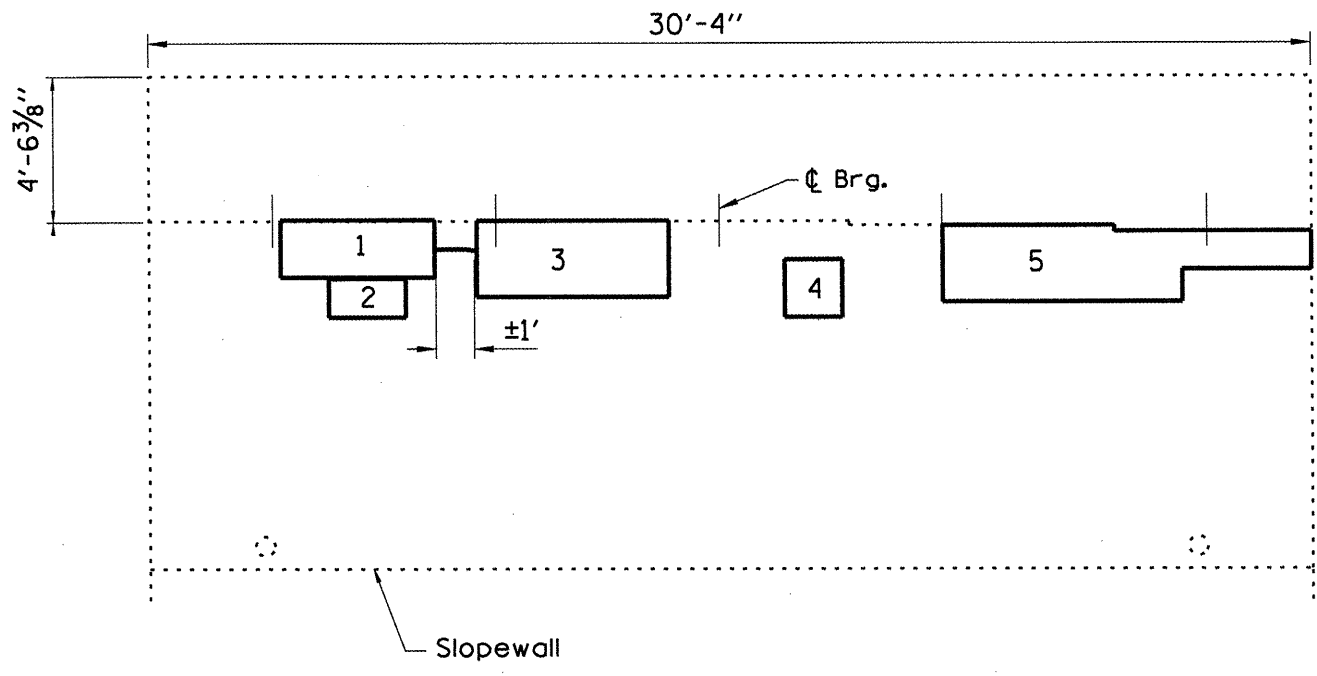
STRUCTURAL REPAIR OF CONCRETE, DEPTH LESS THAN OR EQUAL TO 5"
 EPOXY CRACK INJECTION



ELEVATION NORTH WINGWALL

STRUCTURAL REPAIR OF CONCRETE

LOCATION	AREA (SQ. FT.)
1	6
2	2
3	10
4	2.3
5	12.5
6	0.8
7	1
TOTAL	34.6



WEST ABUTMENT ELEVATION

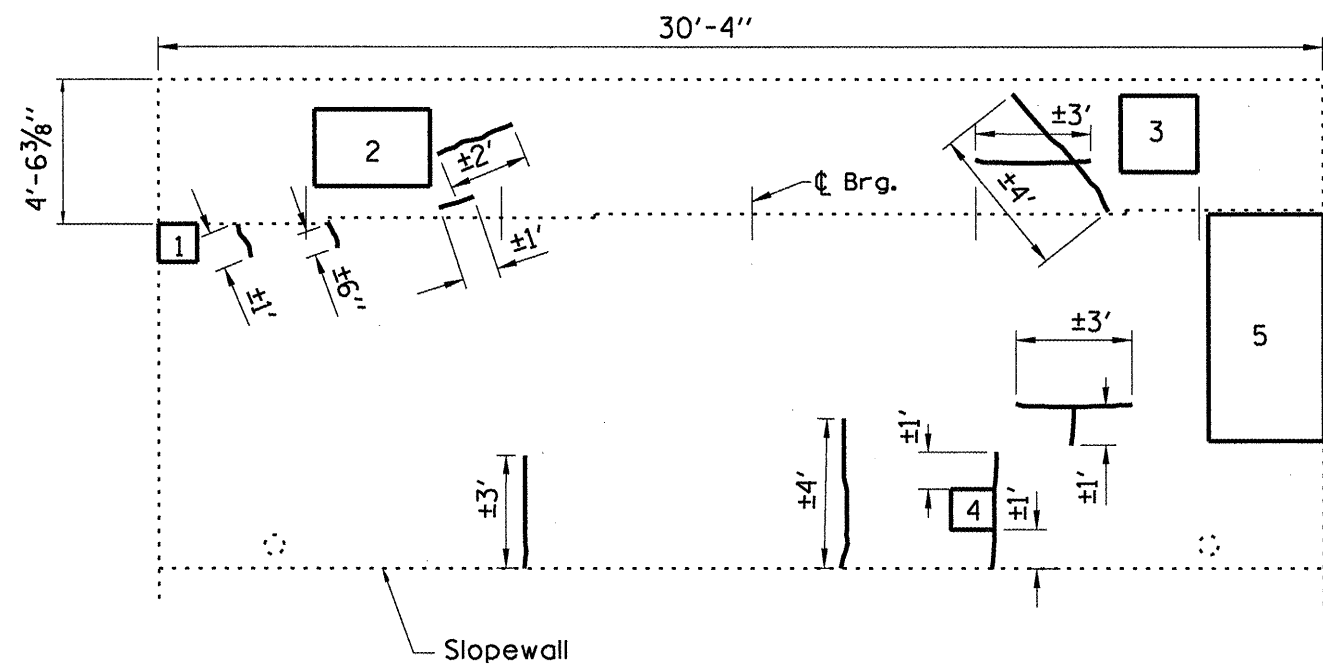
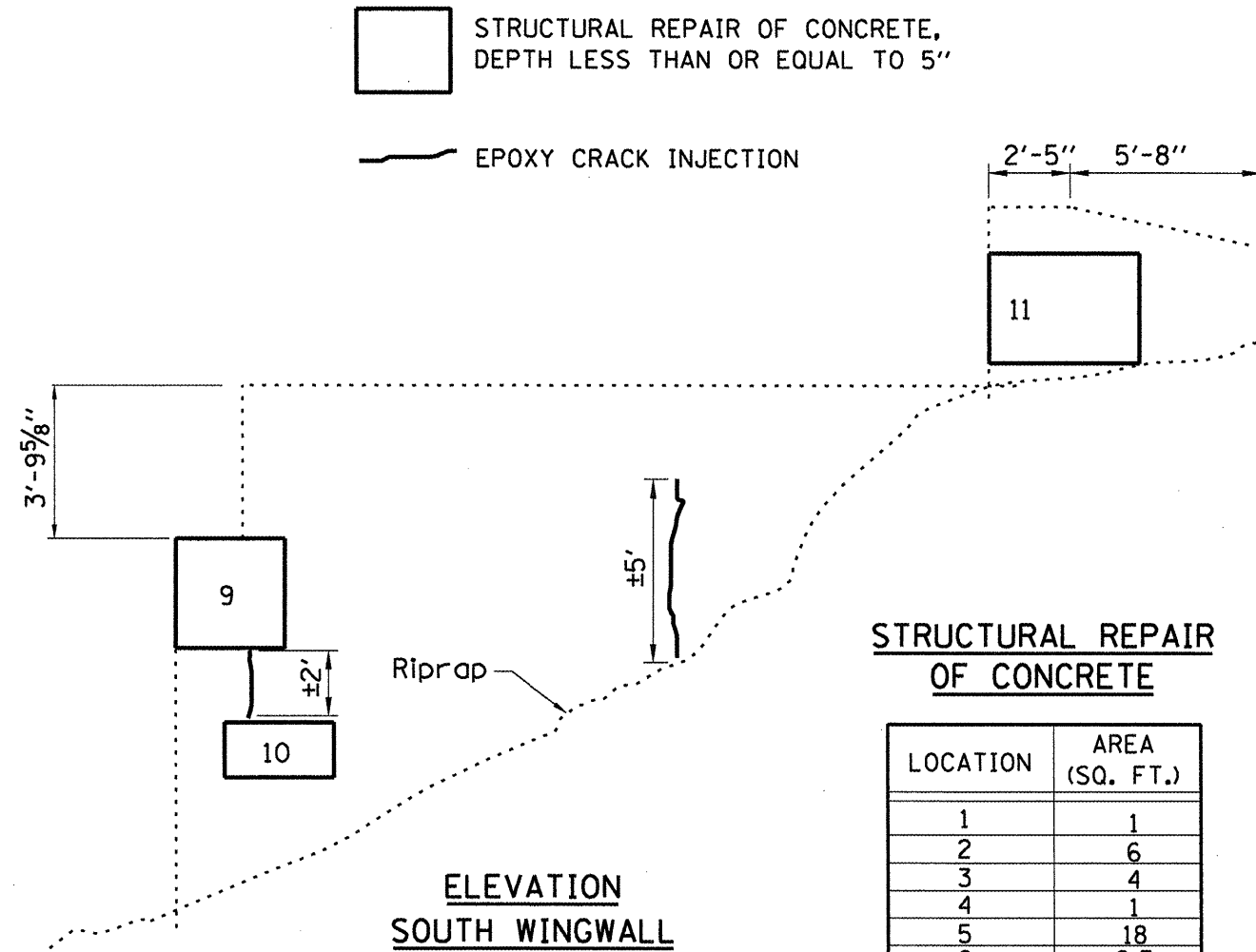
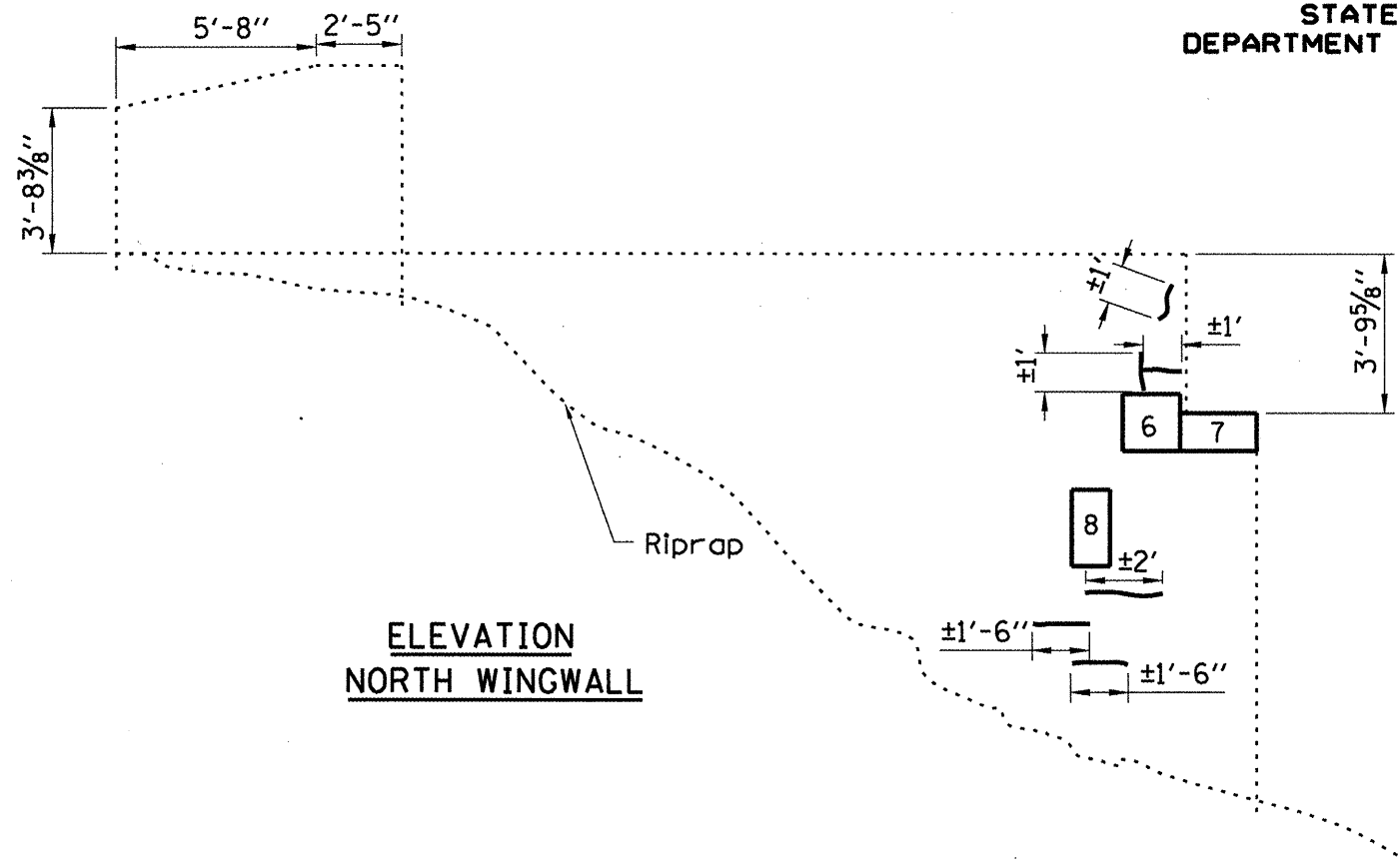
EPOXY CRACK INJECTION

WEST ABUTMENT	21 FOOT
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WEST ABUTMENT CONCRETE REPAIR

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	D9 CM BRIDGE REPAIR FY 09-1	WHITE	16	14
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		



□ STRUCTURAL REPAIR OF CONCRETE, DEPTH LESS THAN OR EQUAL TO 5"

— EPOXY CRACK INJECTION

STRUCTURAL REPAIR OF CONCRETE

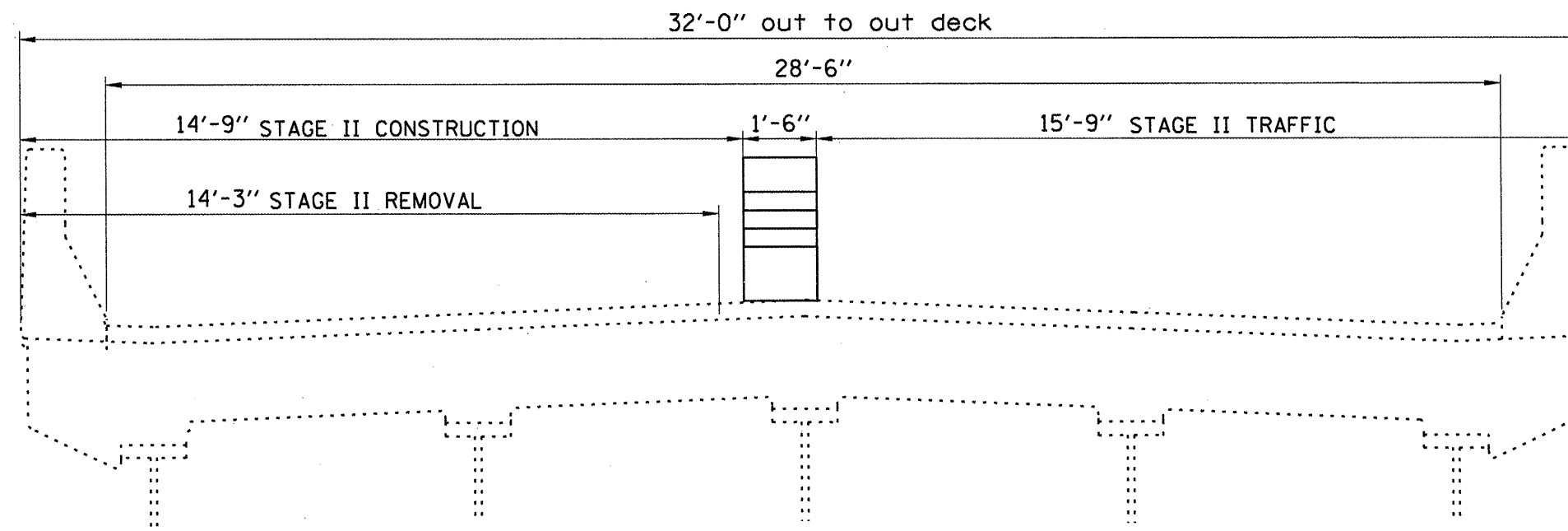
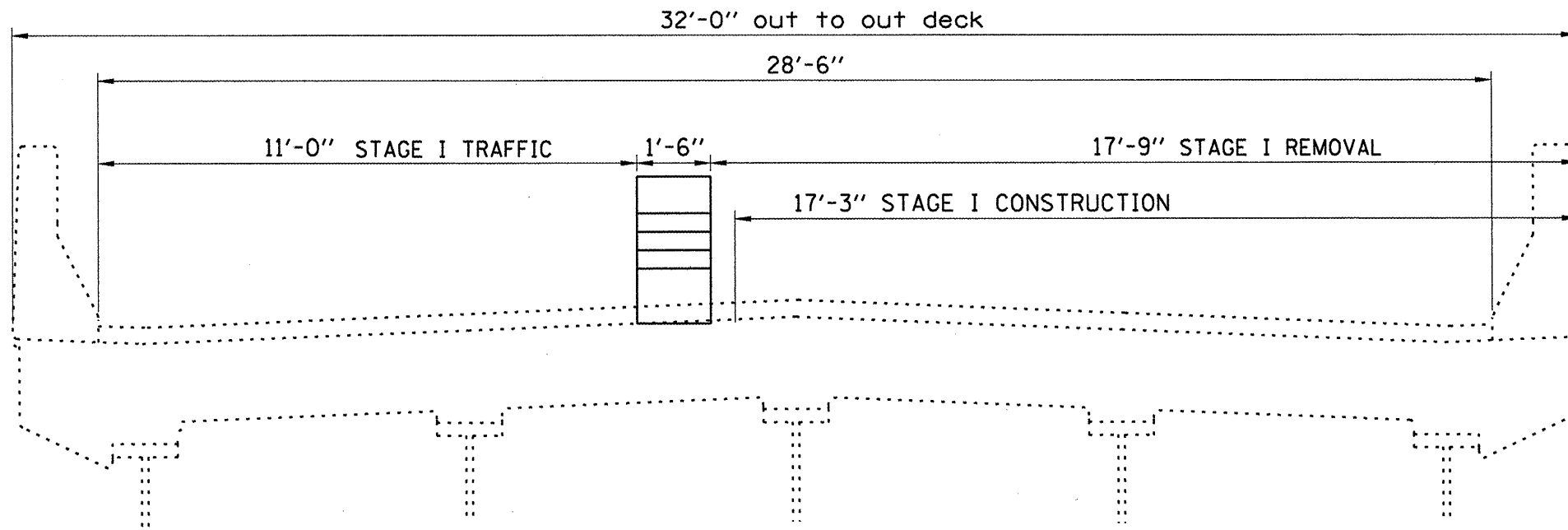
LOCATION	AREA (SQ. FT.)
1	1
2	6
3	4
4	1
5	18
6	2.3
7	2
8	2
9	3
10	4.5
11	18
TOTAL	61.8

EPOXY CRACK INJECTION

EAST ABUTMENT	36.5 FOOT
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EAST ABUTMENT CONCRETE REPAIR

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
815	D9 CM BRIDGE REPAIR FY 09-1	WHITE	16	15
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 78094		

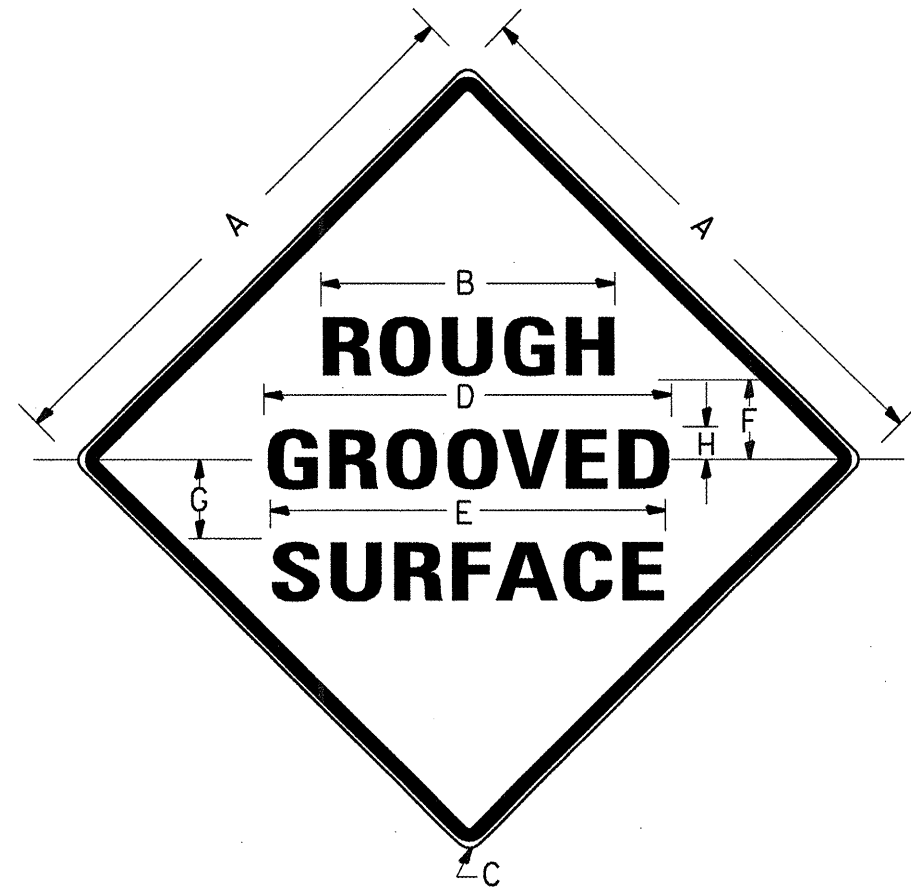


STAGES OF CONSTRUCTION

LOOKING WEST

ILLINOIS STANDARD

W8-I106



COLORS:

LEGEND AND BORDER- BLACK NON-REFLECTORIZED
BACKGROUND- ORANGE REFLECTORIZED

SIGN SIZE	DIMENSIONS							
	A	B	C	D	E	F	G	H
48X48	48.0	24.1	3.0	34.0	33.0	6.0	13.0	3.5

SIGN SIZE	SERIES LINES			MAR- GIN	BOR- DER	BLANK STD.
	1	2	3			
48X48	7C	7C	7C	0.8	1.2	B4-48D

ALL DIMENSIONS IN INCHES

NOTES:

PRIOR TO ALLOWING TRAFFIC ON ANY PORTION OF THE ROADWAY THAT HAS BEEN COLDMILLED, THE CONTRACTOR SHALL HAVE ERECTED "ROUGH GROOVED SURFACE" SIGNS THAT CONFORM TO THE ABOVE DETAILS. A MINIMUM OF ONE SIGN AT EACH END OF THE IMPROVEMENT WILL BE REQUIRED. THE CONTRACTOR SHALL MAINTAIN THE "ROUGH GROOVED SURFACE" SIGNS UNTIL THE COLDMILLED SURFACE IS COVERED WITH LEVELING BINDER OR SURFACE COURSE.

IF AT ANY TIME THE SIGNS ARE IN PLACE BUT NOT APPLICABLE, THEY SHALL BE TURNED FROM THE VIEW OF MOTORISTS OR COVERED AS DIRECTED BY THE ENGINEER.

THE COST OF FURNISHING, ERECTING, MAINTAINING, AND REMOVING THE REQUIRED SIGNS SHALL BE INCLUDED IN THE CONTRACT.

REVISIONS

REDRAWN	2-15-89
REVISED	4-6-93
REVISED	3-27-08
REVISED	-----

STD. 9-39