STATE OF ILLINOIS

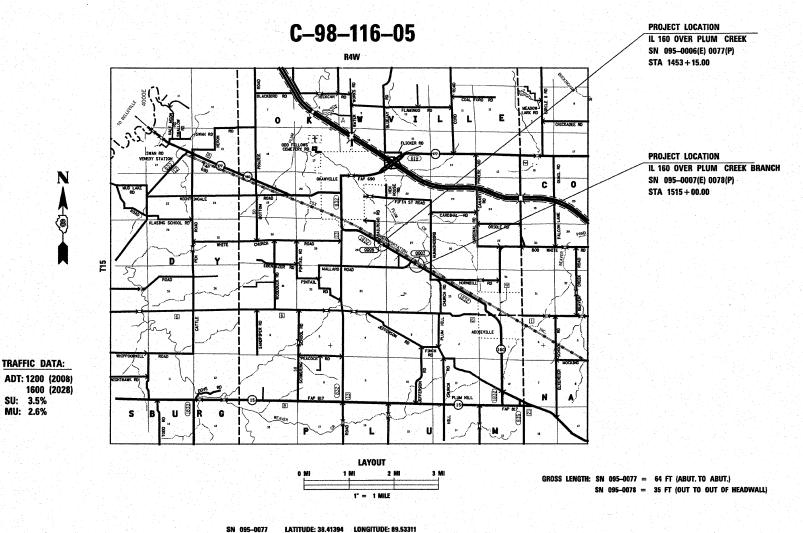
**DEPARTMENT OF TRANSPORTATION** 

**DIVISION OF HIGHWAYS** 

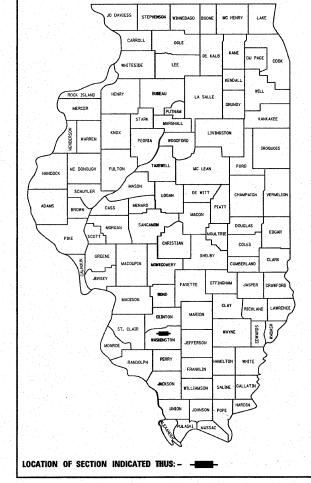
FOR INDEX OF SHEETS, SEE SHEET NO. 2

# **PROPOSED HIGHWAY PLANS**

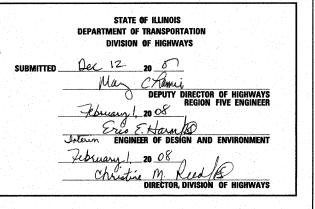
**FAS ROUTE 1832 (IL 160)** SECTION 5BR-2 PROJECT NO.RS-BRS1832 ( 101 ) STRUCTURE REPLACEMENT OVER PLUM CREEK & PLUM CREEK BRANCH **WASHINGTON COUNTY** 



D-98-091-05



SECTION



PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS

346-3209 (618) **CONTACT: ART MUEHLFELD** ENGINEER: PATTI LEBEAU SQUAD

**CONTRACT NO. 76949** 

MICROFILMED

**REEL NUMBER** AWARDED

J.U.L.I.E.

1-800-892-0123

RESIDENT ENGINEER

AS BUILT CHANGES WERE MADE ON THE FOLLOWING SHEETS

WASHINGTON COUNTY

FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES, REDUCED SIZED PLANS WILL NOT

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION

CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

SECTION: 5RR-2 FAS ROUTE 1832 (U. 160)

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WIDE LOAD SIGNING

STORM WATER POLLUTION PREVENTION PLAN

#### SN 095-0006(E) 0077(P)

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#### SN 095-0007(E) 0078(P)

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701901 704001-04

728001

780001-01

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# HIGHWAY STANDARDS

TRAFFIC CONTROL DEVICES

TEMPORARY CONCRETE BARRIER

TYPICAL PAVEMENT MARKINGS

TELESCOPING STEEL SIGN SUPPORT

DECIMAL OF AN INCH OF A FOOT 001006 280001-04 TEMPORARY EROSION CONTROL SYSTEMS 420401-06 BRIDGE APPROACH PAVEMENT 515001-02 NAME PLATE FOR BRIDGES STEEL PLATE BEAM GUARDRAIL SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS 630001-07 630301-04 631031-06 TRAFFIC CONTROL BARRIER TERMINAL, TYPE 6 631032-03 TRAFFIC CONTROL BARRIER TERMINAL, TYPE 6A 635006-02 REFLECTOR AND TERMINAL MARKER PLACEMENT 635011-01 REFLECTOR MARKER AND MOUNTING DETAILS RIGHT OF WAY MARKERS

OFF-ROAD OPERATIONS, 2L, 2W, 4.5m (15') TO 600m (24") FROM PAVEMENT EDGE

OFF-ROAD MOVING OPERATIONS, 2L, 2W, DAY ONLY

LANE CLOSURE, 2L, 2W, MOVING OPERATIONS - DAY ONLY 666001 701006-02 701011-01 701311-02 701321-09 LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER 701326-02 LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING, FOR SPEEDS >= 45 MPH

TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS

STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS AREAS OF REINFORCEMENT BARS

GENERAL NOTES

1. THE STANDARDS AND REVISION NUMBERS SHALL APPLY TO THIS PROJECT.

2. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO CONSTRUCTION AND ORDERING MATERIALS

THE THICKNESS OF THE HMA MIXTURES SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS MAY OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE HMA MIXTURE IS PLACED.

4. ILLINOIS STATE LAW REQUIRES A 48-HOUR NOTICE BE GIVEN TO UTILITIES WITHIN THE PROJECT AREA BEFORE DIGGING BY CALLING J.U.L.I.E. AND BY NOTIFYING NON-J.U.L.I.E. MEMBERS INDIVIDUALLY. AGENCIES KNOWN TO HAVE FACILITIES WITHIN THE PROJECT AREA ARE AS FOLLOWS:

\* AMEREN IP (ELECTRICAL)

\* AMEREN IP (GAS)

\* CHARTER COMMUNICATIONS, INC.

\* FRONTIER COMMUNICATIONS CO. (COMMUNICATIONS)

\* VERIZON NORTH, INC. (COMMUNICATIONS)

\* WASHINGTON COUNTY WATER CO.

MEMBERS OF J.U.L.I.E (800) 892-0123 ARE INDICATED BY \*. NON-MEMBERS MUST BE NOTIFIED INDIVIDUALLY.

5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SEEDING, FERTILIZING, AND MULCHING ANY AREAS DISTURBED OUTSIDE THE LIMITS OF CONSTRUCTION. THIS WORK WILL NOT BE MEASURED FOR PAYMENT. THE SEEDING SHALL BE CLASS 1 & 2. THE APPLICATION OF THE SEEDING, FERTILIZER, AND MULCH SHALL BE TO THE SATISFACTION OF THE ENGINEER. FINAL SEEDING SHALL BE PERFORMED AS SOON AS POSSIBLE.

6. IF THE CONTRACTOR REMOVES TREES WITHIN THE RIGHT-OF-WAY LIMITS FOR HIS CONSTRUCTION ACTIVITY, I.E. IN ORDER TO GAIN ACCESS TO THE PROJECT SITE, IT WILL BE HIS RESPONSIBILITY TO REPLACE THE TREES AT A 1:1 RATIO. THE TREES WILL BE REPLACED WITH A 1 GALLON NATIVE ILLINOIS TREE SPECIES AND SHALL BE APPROVED BY THE ENGINEER. THE TREE REMOVAL AND TREE REPLACEMENT WILL BE AT THE CONTRACTOR'S EXPENSE, AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.

 "ROAD CONSTRUCTION AHEAD" SIGNS SHALL BE PLACED AT THE BEGINNING AND ENDING OF THE PROJECT AND WILL BE INCLUDED IN THE TRAFFIC CONTROL PAY ITEMS. ALL CONSTRUCTION SIGNS SHALL BE FLOURESCENT ORANGE.

8. NO TRENCHES OR OPEN PITS WILL BE PERMITTED ADJACENT TO A TRAFFIC LANE DURING NON-WORKING HOURS. ALL WIDENING TRENCHES SHALL BE BACKFILLED DURING THE SAME WORKING DAY IT WAS EXCAVATED.

9. THE COST OF GRADING AND SHAPING ALONG THE PROPOSED BASE COURSE SHALL BE INCLUDED IN THE COST OF "EARTH EXCAVATION".

10. RIGHT OF WAY MARKERS SHALL BE SET SO THE BACK OF THE POST IS TWELVE (12")
INCHES INSIDE THE RIGHT OF WAY BOUNDARY. RIGHT OF WAY PROPERTY CORNERS ARE
MARKED BY A 5%" IRON ROD WITH IDOT ALUMINUM CAP AND SHALL NOT BE REMOVED OR DAMAGED WHEN SETTING THE RIGHT OF WAY MARKERS.

# COMMITMENTS

- PROPERTY OWNER BRAD BARKAU SHALL BE GIVEN A MINIMUM OF 48 HOURS NOTICE PRIOR TO WORK COMMERNEING ON HIS ENTRANCE LOCATED AT STATION 1455+97.90. MR BARKAU CAN BE REACHED AT (618) 407-6639.
- 2. NO VEHICLES ARE TO BE PARKED IN THE THREE (3) PRAIRIE MEMNANTS LOCATED WITHIN THE PROJECT LIMITS. THE CONTRACTOR AND RESIDENT ENGINEER SHALL DESIGNATE AN AREA FOR PARKING, SO THERE IS NO MISUNDERSTANDING AS TO PRAIRIE LOCATION.

ILLINOIS DEPARTMENT OF TRANSPORTATION DATE NDEX OF SHEETS/HIGHWAY SATNDARDS GENERAL NOTES/COMMITMENTS FAS ROUTE 1832 SECTION 5BR-2

WASHINGTON COUNTY SCALE: VERT. DRAWN BY

DATE

CHECKED BY

CONTRACT NO. 76949

ILLINOIS DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES

CONSTRUCTION TYPE CODE SUMMARY OF QUANTITIES SN 095-0078 SN 095-0077 X080-2A FED 80% STATE 20% X071-2A FED 80% I000-2A TOTAL FED 80% FED 80% QUANTITIES UNIT STATE 20% STATE 20% STATE 20% ITEM CODE NO UNIT 378 356 22 TREE REMOVAL (6 TO 15 UNITS DIAMETER) 20100110 TREE REMOVAL (OVER 15 UNITS DIAMETER) UNIT 540 580 20100210 545 CU YD 1145 600 20200100 EARTH EXCAVATION CU YD 120 65 55 20200500 EARTH EXCAVATION (WIDENING) 635 CU YD 635 CHANNEL EXCAVATION 20300100 CU YD 750 655 95 FURNISHED EXCAVATION 20400800 86 CU YD 86 POROUS GRANULAR EMBANKMENT, SPECIAL 20700400 ACRE 0.25 0.25 SEEDING, CLASS 1 25000100 ACRE 0.75 0.5 0.25 25000200 SEEDING, CLASS 2 POUND 66 43.5 22.5 NITROGEN FERTILIZER NUTRIENT 25000400 43.5 22.5 POUND 66 PHOSPHORUS FERTILIZER NUTRIENT 25000500 43.5 22.5 66 POUND POTASSIUM FERTILIZER NUTRIENT 25000600 ACRE 2.5 1.5 MULCH, METHOD 1 25100105 51.5 POUND 123 71.5 TEMPORARY EROSION CONTROL SEEDING 28000250 EACH 22 15 28000300 TEMPORARY DITCH CHECKS 280 FOOT 730 450 PERIMETER EROSION BARRIER 28000400 EACH INLET AND PIPE PROTECTION 28000500 525 SQ YD 525 STONE RIPRAP, CLASS A4 28100107 340 SQ YD 340 28100109 STONE RIPRAP, CLASS A5 340 865 525 SQ YD FILTER FABRIC 28200200 51 51 SQ YD SUB-BASE GRANULAR MATERIAL, TYPE A 12" 31100910 46 SQ YD 46 AGGREGATE BASE COURSE, TYPE B 6" 35101800 254 200 HOT-MIX ASPHALT BASE COURSE WIDENING, SQ YD 454 35600712 0.5 BITUMINOUS MATERIALS (PRIME COAT) TON 1.1 40600200 3 TON 3 AGGREGATE (PRIME COAT) 40600300 27 27 54 SQ YD TEMPORARY RAMP 40600990 642 HOT-MIX ASPHALT BINDER COURSE, IL-19.0, 652 TON 1294 40603085 133.5 TON 291 157.5 40603340 HOT-MIX ASPHALT SURFACE COURSE, MIX TON 6 INCIDENTAL HOT-MIX ASPHALT SURFACING 40800050 220 220 BRIDGE APPROACH PAVEMENT SQ YD 42001165 44 BRIDGE APPROACH PAVEMENT CONNECTOR SQ YD 44 42001430 (FLEXIBLE) 268 SQ YD 777 509 PAVEMENT REMOVAL 44000100 574 SQ YD 948 374 HOT-MIX ASPHALT SURFACE REMOVAL, 44000198 VARIABLE DEPTH 468 397 SQ YD 865 AGGREGATE SHOULDERS, TYPE B 6" 48101500

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1832	5BR-2	W.	ASHIN	GTON	97	
F.A.S. SECTI		С	SHEETS	N		

ILLINOIS DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES

	SOMMAN	OI	QU/	BR5 R5						
	SUMMARY OF QUANTITIES					ON TYPE CODE	0079			
	SUMMARY OF QUANTITIES	1	TOTAL	SN 095 I000-2A	-0077 X0 <b>7</b> ∜-2A	SN 095 I000-2A	X080-2A			
CODE NO	ITEM .	UNIT	QUANTITIES	FED 80% STATE 20%	FED 80% STATE 20%	FED 80% STATE 20%	FED 80% STATE 20%			
48203029	HOT-MIX ASPHALT SHOULDERS, 8"	sa yd	865	468		397				
50100300	REMOVAL OF EXISTING STRUCTURES NO. 1	EACH	1		1					
50100400	REMOVAL OF EXISTING STRUCTURES NO. 2	EACH	1 0 0				1			
50105220	PIPE CULVERT REMOVAL	FOOT	25.5	25.5						
50200100	STRUCTURE EXCAVATION	CU YD	505		140		365			
50300100	FLOOR DRAINS	EACH	8		8					
50300225	CONCRETE STRUCTURES	CU YD	173.8		28.8		145			
50300255	CONCRETE SUPERSTRUCTURE	CU YD	88.1		88.1					
50300260	BRIDGE DECK GROOVING	SQ YD	213		213					
50300280	CONCRETE ENCASEMENT	CU YD	4.2		4.2					
50300300	PROTECTIVE COAT	SQ YD	281		281					
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1		1					
50500505	STUD SHEAR CONNECTORS	EACH	1134		1134					
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	28550		21960		6540			
50800515	BAR SPLICERS	EACH	313		285		28			
50900 <i>200</i> 58700300 51201600	STEEL RAILING, TYPE 2399 CONCRETE SEALER FURNISHING STEEL PILES HP12X53	F00T 5@ F7 F00T	64 /4/4 245		245		64 144			
51202305	DRIVING PILES	FOOT	245		245					
51203600	TEST PILE STEEL HP12X53	EACH	1		1					
51500100	NAME PLATES	EACH	2		1		1			
52100520	ANCHOR BOLTS, 1"	EACH	24		24					
54213453	END SECTIONS 18"	EACH	2	2						
542D0223	PIPE CULVERTS, CLASS D, TYPE 1 18"	FOOT	25.5	25.5						
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	51		51					
60109580	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	138		138					
63000000	STEEL PLATE BEAM GUARD RAIL, TYPE A	FOOT	750	375		375				
63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4	4						
63100087	TRAFFIC BARRIER TERMINAL, TYPE 6A	EACH	4			4				
63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	8	4		4				
63200310	GUARDRAIL REMOVAL	FOOT	1152	559		593				
66600105	FURNISHING AND ERECTING RIGHT-OF-WAY MARKERS	EACH	25	12		13				
67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	9	4.5		4.5				
67100100	MOBILIZATION	L SUM	1	0.5		0.5				
70100500	TRAFFIC CONTROL AND PROTECTION, STANDARD 701326	L SUM	1	0.5		0.5				
					1		1			
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 F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
1832	5BR-2	WASHINGTON	97	4
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.OT DATE = 11/36
LE NAME = c1/p
.OT SCALE = 50.00
EFERENCE = \$REF

\* SPECIALTY ITEMS

SUMMARY OF QUANTITIES

		<b>○</b> I	40/	BRS		R.		
	CHAMADY OF CHAMITITIES				CONSTRUCTION	N TYPE CODE SN 095-0078		
	SUMMARY OF QUANTITIES		TOTAL	SN 095-			0078 X080-2A	
CODE NO	ITEM	UNIT	QUANTITIES	I000-2A FED 80% STATE 20%	X071-2A FED 80% STATE 20%	IOOO-2A FED 80% STATE 20%	FED 80% STATE 20%	
70101205	TRAFFIC CONTROL AND PROTECTION, STANDARD 701321 (SPECIAL)	EACH	2	1		<b>1</b>		
	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	2	1		1		
	TEMPORARY RUMBLE STRIP	EACH	12	6		6		
	CHANGEABLE MESSAGE SIGN	CAL MO	9	4.5		4.5		
70106800	TEMPORARY PAVEMENT MARKING - LINE 4"	FOOT	10286	5276		5010		
70300220	TEMPORARY PAVEMENT MARKING - LINE 6"	FOOT	3520	1820		1700		
70300240	TEMPORARY PAVEMENT MARKING - LINE 24"	FOOT	154	60		94		
70300280	WORK ZONE PAVEMENT MARKING REMOVAL	SQ FT	3737	1879		1858		
70301000		FOOT	1840		910		930	
70400100	TEMPORARY CONCRETE BARRIER	FOOT	1680		910		770	
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	SQ FT	20	10		10		
72000100	SIGN PANEL - TYPE 1		48	24		24		
72800100	TELESCOPING STEEL SIGN SUPPORT	F00T				3131		
78000200	THERMOPLASTIC PAVEMENT MARKING - LINE 4"	FOOT	5825	2694				
78000650	THERMOPLASTIC PAVEMENT MARKING - LINE 24"	FOOT	38			38		
78008210	POLYUREA PAVEMENT MARKING TYPE I - LINE 4"	FOOT	279		279			
78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	31	15		16		
78100105	RAISED REFLECTIVE PAVEMENT MARKER (BRIDGE)	EACH	2		2			
78200200	BIDIRECTIONAL PRISMATIC BARRIER REFLECTOR	EACH	4		2		2	
78200410	GUARDRAIL MARKERS, TYPE A	EACH	18	9		9		
78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	8	4		4		
78300100	PAVEMENT MARKING REMOVAL	SQ FT	1939	990		949		
X0321099	GEOTEXTILE RETAINING WALL	SQ YD	19.1				19.1	
X0323330	PRECAST CONCRETE SUBSTRUCTURE	L SUM	1				1	
X0323988	TEMPORARY SOIL RETENTION SYSTEM	SQ FT	808.4		364.3		444.1	
X7200200	WIDE LOAD SIGNING	L SUM	1	0.5		0.5		
Z0030250	IMPACT ATTENUATORS, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3	EACH	4		2		2	
Z0030260	IMPACT ATTENUATORS, TEMPORARY (FULLY REDIRECTIVE, NARROW), TEST LEVEL 3	EACH	3		2		1	
Z0030350	IMPACT ATTENUATORS, RELOCATE (NON- REDIRECTIVE), TEST LEVEL 3	EACH	3		2			
X0325916	THREE-SIDED PRECAST CONCRETE STRUCTURE 28' X 9'	FOOT	37.25				37.25	
€0007600		HOUR	500	500				
					1		<del></del>	

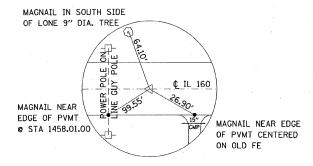
\* SPECIALTY ITEMS

#### TIE POINT

MAGNAIL 580' TO ¢ OF BRIDGE SN 095-006 OVER PLUM CREEK STA 1447+34.76



TIE POINT MAGNAIL @ BRIDGE C STA 1453+15.00



#### TIE POINT

MAGNAIL 585' FROM CENTER OF BRIDGE OVER PLUM CREEK @ STA 1459+00.00

#### **BENCHMARKS**

BM 1 - RR SPIKE IN POWER POLE, (MIDDLE POLE OF 3) E SE CORNER OF IL 160 & GOLDFINCH RD/RAILROAD RD STA 1445+90, 30' RT ELEVATION = 447.09

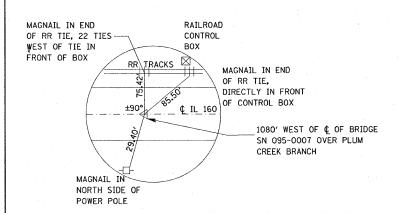
RR SPIKE IN POWER POLE @ THE SOUTH SIDE OF IL 160, ± 400' EAST OF THE CENTER OF SN 095-0006 STA 1458+02, 32' RT ELEVATION = 444.48

# SN 095-0007(E) 0078(P)

COUNTY TOTAL SHEET NO. SECTION WASHINGTON 97 1832 5BR-2 STA. TO STA.\_\_\_ FED. ROAD DIST. NO. \_ ILLINOIS FED. AID PROJECT

CONTRACT NO. 76949

NOTE: ALL TIES ARE DIRECT / SLOPE TAPED MEASUREMENTS



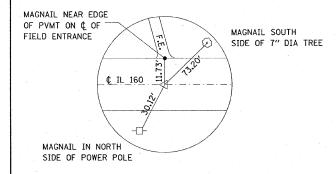
#### TIE POINT

MAGNAIL @ C STATION EQUATION 1500+07.63 BACK TO SN 095-0006 = 1500+00.00 FORWARD TO SN 095-0007



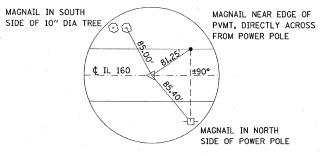
TIE POINT

MAGNAIL @ BRIDGE C STA 1510.80.00



#### TIE POINT

MAGNAIL 580' FROM CENTER OF BRIDGE SN 095-0007 OVER PLUM CREEK BRANCH @ STA 1505+00.00



#### TIE POINT

MAGNAIL 610.5' EAST OF CENTER OF BRIDGE SN 095-0007 OVER PLUM CREEK BRANCH @ STA 1516+90.54

#### **BENCHMARKS**

RR SPIKE IN POWER POLE, (NEAR SE CORNER OF CULTIVATED FIELD), ± 250' WEST OF NS 095-0007, ± 175' SOUTH OF IL 160 STA 1506+77.8, 30' RT ELEVATION = 451.13

BM 5 - CHISELED " " ON BRIDGE ABUTMENT @ NW CORNER OF SN 095-0007 STA 1510+84.7, 18.4' LT ELEVATION = 445.50

RR SPIKE IN POWER POLE, @ SW CORNER OF IL 160 & PLUM HILL CHURCH RD / HUMMINGBIRD RD STA 1520+20, 30' RT ELEVATION = 453.68

ILLI		REVISIONS NAME
ILLI	DATE	NAME
SCALE VI		
SCALE: N		

ILLINOIS DEPARTMENT OF TRANSPORTATION

TIES & BENCHMARKS

FAS ROUTE 1832 SECTION 5BR-2 WASHINGTON COUNTY

VERT. HORIZ. DATE

DRAWN BY CHECKED BY

CONTRACT NO. 76949 F.A.S. SECTION COUNTY TOTAL SHEET NO. 1832 5BR-2 WASHINGTON 97 7 STA. \_\_\_\_\_TO STA.\_\_\_\_ FED. ROAD DIST. NO. \_ ILLINOIS FED. AID PROJECT TO STA.\_\_\_\_

FAS ROUTE 1832 SECTION 5BR-2 WASHINGTON COUNTY

DRAWN BY

CHECKED BY

SCALE: VERT. HORIZ.

DATE

1. ALL SIGNS REQUIRED WILL BE SUPPLIED TO THE CONTRACTOR BY I.D.O.T.

2. THE CONTRACTOR SHALL FURNISH THE POSTS AND ERECT SIGNS AT THE LOCATIONS SHOWN ON THIS SHEET, AS DIRECTED BY THE RE/RT. THE POSTS SHALL REMAIN THE PROPERTY OF

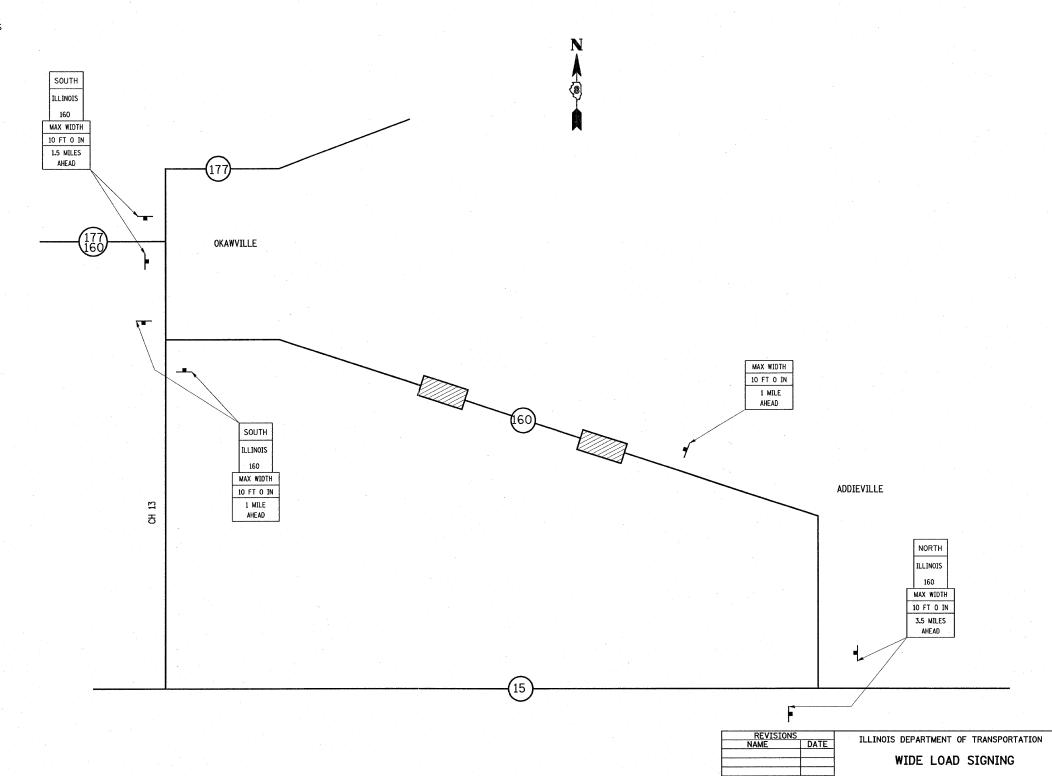
3. THE CONTRACTOR SHALL GIVE ILLINOIS DEPARTMENT OF TRANSPORTATION, BUREAU OF OPERATIONS TWO WEEKS NOTICE FOR SIGNS. THE CONTRACTOR SHALL PICK UP THE SIGNS AT THE T.M. BUILDING IN FAIRVIEW HEIGHTS, AND RETURN THEM UPON COMPLETION OF THE CONTRACT. CONTACT JEAN SLAPE @ (618) 346-3289.

4. THE ABOVE NOTED WORK SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE, LUMP SUM, FOR WIDE LOAD SIGNING AND NO OTHER COMPENSATION WILL BE ALLOWED.

5. SIGN SPACING WILL BE 400' OR TO FIT FIELD CONDITIONS.

6. THE HEIGHT TO THE BOTTOM OF THE LOWEST SIGN SHALL NOT BE LESS THAN 6'.

SIGNS REQUIRED MAX WIDTH 10 FT 0 IN NORTH (2) (3) 1 MILE AHEAD SOUTH MAX WIDTH 10 FT 0 IN ILLINOIS (2) 1.5 MILES 160 AHEAD MAX WIDTH 10 FT 0 IN (2) 3.5 MILES AHEAD



I CERTIFY UNDER PENALTY OF LAW THAT THIS DOCUMENT AND ALL ATTACHMENTS WERE PREPARED UNDER MY DIRECTION OR SUPERVISION IN ACCORDANCE WITH A SYSTEM DESIGNED TO ASSURE THAT QUALIFIED PERSONNEL. PROPERTY GATHERED AND EVALUATED THE INFORMATION SUBMITTED. BASED ON MY INQUIRY OF THE PERSON OR PERSONS WHO MANAGE THE SYSTEM, OR THOSE PERSONS DIRECTLY RESPONSIBLE FOR GATHERING THE INFORMATION, THE INFORMATION SUBMITTED IS, TO THE BEST OF MY KNOWLEDGE AND BELIEF, TRUE, ACCURATE AND COMPLETE. I AM AWARE THAT THERE ARE SIGNIFICANT PENALTIES FOR SUBMITTING FALSE INFORMATION, INCLUDING THE POSSIBILITY OF FINE AND IMPRISONMENT FOR KNOWING VIOLATIONS.

MARY C. LAMIE
PRINT NAME

DEPUTY DIRECTOR OF HIGHWAYS
REGION FIVE ENGINEER

TITLE

DATE

IL DEPT. OF TRANSPORTATION

I. SITE DESCRIPTION:

A. THE FOLLOWING IS A DESCRIPTION OF THE PROJECT LOCATION:

THE PROJECT IS LOCATED ON IL 160 OVER PLUM CREEK 2.7 MILES WEST OF ADDIEVILLE

B. THE FOLLOWING IS A DESCRIPTION OF THE CONSTRUCTION ACTIVITY WHICH IS THE SUBJECT OF THIS PLAN:

CONSTRUCTION WILL INCLUDE THE REMOVAL AND REPLACEMENT OF THE IL RTE 160 STRUCTURE OVER PLUM CREEK, SCOUR MITIGATION, PAVING, GRADING, CULVERT, LANDSCAPIND, AND ALL INCIDENTAL WORK NECESSARY TO COMPLETE THE PROPOSED STRUCTURE AND ROADWAY AS SHOWN IN THE PLANS.

C. THE FOLLOWING IS A DESCRIPTION OF THE INTENDED SEQUENCE OF MAJOR ACTIVITIES WHICH WILL DISTURB SOILS FOR MAJOR PORTIONS OF THE CONSTRUCTION SITE, SUCH AS GRUBBING, EXCAVATION AND GRADING:

DESCRIPTION OF INTENDED SEQUENCE FOR MAJOR CONSTRUCTION ACTIVITIES WHICH WILL DISTURB SOILS FOR MAJOR PORTIONS OF THE CONSTRUCTION SITE:

PLUM CREEK (SN 095-0077) AND PLUM CREEK BRANCH (SN 095-0078)

PRE-STAGE 1: CONSTRUCTION OF THE 3' TEMPORARY PAVEMENT WIDENING ON THE NORTHEAST AND NORTHWEST CORNERS OF THE STRUCTURE.

STAGE 1: BEGIN STAGE 1 REMOVAL OF THE EXISTING STRUCTURE, AND STAGE I CONSTRUCTION OF THE REPLACEMENT STRUCTURE, EROSION CONTROL, GRADING, PAVING THE RIGHT SIDE PAVEMENT EAST AND WEST OF THE STRUCTURE, CONSTRUCTION OF PROPOSED BITUMINOUS AND AGGREGATE SHOULDERS, GUARDRAIL, RIPRAP, ETC.

STAGE 2: BEGIN STAGE 2 REMOVAL OF THE EXISTING STRUCTURE AND STAGE 2 CONSTRUCTION OF THE REPLACEMENT STRUCTURE, REMOVE TEMPORARY WIDENING, GRADING, PAVING THE LEFT SIDE PAVEMENT EAST AND WEST OF THE STRUCTURE, CONSTRUCTION OF THE PROPOSED BITUMINOUS AND AGGREGATE SHOULDERS, GUARDRAIL, RIPRAP. FTC.

D. THE TOTAL AREA OF THE CONSTRUCTION SITE IS ESTIMATED TO BE 2.61 ACRES.

THE TOTAL AREA OF THE SITE THAT IS ESTIMATED WILL BE DISTURBED BY EXCAVATION, GRADING OR OTHER

E. THE FOLLOWING IS A WEIGHTED AVERAGE OF THE RUNOFF COEFFICIENT FOR THIS PROJECT AFTER CONSTRUCTION ACTIVITIES ARE COMPLETED: C= 0.50

F. THE FOLLOWING IS A DESCRIPTION OF THE SOIL TYPES FOUND AT THE PROJECT SITE FOLLOWED BY INFORMATION REGARDING THEIR EROSIVITY:

ORION SILT LOAM (3415) - A SOMEWHAT POORLY DRAINED SOIL WITH MODERATE PERMEABILITY. THIS SOIL IS FREQUENTLY FLOODED WITH 0 TO 2 PERCENT SLOPES, THIS SOIL HAS A SLIGHT SUSCEPTIBILITY TO WATER AND

G. THE FOLLOWING IS A DESCRIPTION OF POTENTIALLY EROSIVE AREAS ASSOCIATED WITH THIS PROJECT:

THERE IS ONLY ONE TYPE OF SOIL WITHIN THE PROJECT LIMITS, ORION SILT LOAM (3415).

H. THE FOLLOWING IS A DESCRIPTION OF SOIL DISTURBING ACTIVITIES, THEIR LOCATIONS, AND THEIR EROSIVE FACTORS (E.G. STEEPNESS OF SLOPES, LENGTH OF SLOPES, ETC):

THE NATURE AND PURPOSE OF LAND DISTURBING ACTIVITIES ON THIS PROJECT IS TO REMOVE AND REPLACE THE IL ROUTE 160 BRIDGE OVER PLUM CREEK (PROPOSED STRUCTURE NO. 095-0006, EXISTING STRUCTURE 095-0077). PROPOSED RIGHT-OF-WAY WILL BE REQUIRED TO ACCOMMODATE RECONSTRUCTION OF THE BRIDGE AND THE ROADWAY APPROACHES. THERE ARE NO SCHEDULED NEIGHBORING ACTIVITIES THAT WILL AFFECT THE SOIL EROSION AND SEDIMENT CONTROL PLANS AND NO OFF-SITE LAND DISTURBING ACTIVITIES.

ORION SILT LOAM (3415) HAS EROSIVE CHARACTERISTICS. GRADING ON BOTH SIDES OF THE STRUCTURE, SLOPE STEEPNESS AND LENGTH VARY FROM 1:2.5 TO 1:3 AND 4' TO 35' RESPECTIVELY.

I. SEE THE EROSION CONTROL PLANS AND/OR DRAINAGE PLANS FOR THIS CONTRACT FOR INFORMATION REGARDING DRAINAGE PATTERNS, APPROXIMATE SLOPES ANTICIPATED BEFORE AND AFTER MAJOR GRADING ACTIVITIES, LOCATIONS WHERE VEHICLES ENTER OR EXIT THE SITE AND CONTROLS TO PREVENT OFF SITE SEDIMENT TRACKING (TO BE ADDED AFTER CONTRACTOR IDENTIFIES LOCATIONS), AREAS OF SOLD DISTURBANCE, THE LOCATION OF MAJOR STRUCTURAL AND NON-STRUCTURAL CONTROLS IDENTIFIED IN THE PLAN, THE LOCATION OF AREAS WHERE STABILIZATION PRACTICES ARE EXPECTED TO OCCUR, SURFACE WATERS (INCLUDING WETLANDS) AND LOCATIONS WHERE STORM WATER IS DISCHARGED TO SURFACE WATER INCLUDING WETLANDS.

J. THE FOLLOWING IS A LIST OF RECEIVING WATER(S) AND THE ULTIMATE RECEIVING WATER(S), AND AREAL EXTENT OF WETLAND ACREAGE AT THE SITE. THE LOCATION OF THE RECEIVING WATERS CAN BE FOUND ON THE EROSION AND SEDIMENT CONTROL PLANS:

PLUM CREEK (SN 095-0077) AND PLUM CREEK BRANCH (SN 095-0078)

K. THE FOLLOWING POLLUTANTS OF CONCERN WILL BE ASSOCIATED WITH THIS CONSTRUCTION PROJECT: (CHECK ALL THAT APPLY)

M SOTI SEDIMENT ■ PETROLEUM (GAS, DIESEL, OIL, KEROSENE, HYDRAULIC OIL / FLUIDS) ■ ANTIFREEZE / COOLANTS ■ CONCRETE □ CONCRETE TRUCK WASTE ■ WASTE WATER FROM CLEANING CONSTRUCTION EQUIPMENT ■ CONCRETE CURING COMPOUNDS □ OTHER (SPECIFY)\_\_\_\_ ☑ SOLID WASTE DEBRIS OTHER (SPECIFY)\_\_\_\_\_ D PAINTS D OTHER (SPECIFY) OTHER (SPECIFY)\_\_ □ SOLVENTS □ FERTILIZERS / PESTICIDES OTHER (SPECIFY)\_.

#### II. CONTROLS

THIS SECTION OF THE PLAN ADDRESSES THE CONTROLS THAT WILL BE IMPLEMENTED FOR EACH OF THE MAJOR CONSTRUCTION ACTIVITIES DESCRIBED IN I.C. ABOVE AND FOR ALL USE AREAS, BORROW SITES, AND WASTE SITES. FOR EACH MEASURE DISCUSSED, THE CONTRACTOR WILL BE RESPONSIBLE FOR ITS IMPLEMENTATION AS INDICATED. THE CONTRACTOR SHALL PROVIDE TO THE RESIDENT ENGINEER A PLAN FOR THE IMPLEMENTATION OF THE MEASURES INDICATED. THE CONTRACTOR, AND SUBCONTRACTORS, WILL NOTIFY THE RESIDENT ENGINEER OF ANY PROPOSED CHANGES, MAINTEMANCE, OR MODIFICATIONS TO KEEP CONSTRUCTION ACTIVITIES COMPLIANT WITH THE PERMIT. EACH SUCH CONTRACTOR HAS SIGNED THE REQUIRED CERTIFICATION ON FORMS WHICH WILL BE PROVIDED AT THE PRE-CONSTRUCTION CONFERENCE, AND ARE A PART OF, THIS PLAN:

A. EROSION AND SEDIMENT CONTROL

- STABILIZED PRACTICES: PROVIDED BELOW IS A DESCRIPTION OF INTERIM AND PERMANENT STABILIZATION PRACTICES, INCLUDING SITE SPECIFIC SCHEDULING OF THE IMPLEMENTATION OF THE PRACTICES. SITE PLANS WILL ENSURE THAT EXISTING VEGETATION IS PRESERVED WHERE ATTAINABLE AND DISTURBED PORTIONS OF THE SITE WILL BE STABILIZED. STABILIZATION PRACTICES MAY INCLUDE BUT ARE NOT LIMITED TO: TEMPORARY SEEDING, PERMANENT SEEDING, MULCHING, GEOTEXTILES, SODDING, VEGETATIVE BUFFER STRIPS, PROTECTION OF TREES, PRESERVATION OF MATURE VEGETATION, AND OTHER APPROPRIATE MEASURES. EXCEPT AS PROVIDED BELOW IN II(A)(I)(A) AND II(A)(3), STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICABLE IN PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED, BUT IN NO CASE MORE THAN 14 DAYS AFTER THE CONSTRUCTION ACTIVITY IN THAT PORTION OF THE SITE HAS TEMPORARILY OR PERMANENTLY CEASED ON ALL DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION WILL NOT OCCUR FOR A PERIOD OF 21 OR MORE CALENDAR DAYS.
  - G. WHERE THE INITIATION OF STABILIZATION MEASURES BY THE 14TH DAY AFTER CONSTRUCTION ACTIVITY TEMPORARILY OR PERMANENTLY CEASES IS PRECLUDED BY SNOW COVER, STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICABLE THEREAFTER.

THE FOLLOWING STABILIZATION PRACTICES WILL BE USED FOR THIS PROJECT:

DESCRIBE HOW THE STABILIZATION PRACTICES LISTED ABOVE WILL BE UTILIZED

 TEMPORARY EROSION CONTROL SEEDING - THIS ITEM WILL BE APPLIED TO ALL BARE AREAS EVERY SEVEN DAYS TO MINIMIZE THE AMOUNT OF EXPOSED SURFACE AREAS.

EARTH STOCKPILES SHALL BE TEMPORARILY SEEDED IF THEY ARE TO REMAIN UNUSED FOR MORE THAN 14 DAYS.

WITHIN THE CONSTRUCTION LIMITS, AREAS WHICH MAY BE SUSCEPTIBLE TO EROSION AS DETERMINED BY THE ENGINEER SHALL REMAIN UNDISTURBED UNTIL FULL SCALE CONSTRUCTION IS UNDERWAY TO PREVENT UNNECESSARY SOIL EROSION.

BARE AND SPARSELY VEGETATED GROUND IN HIGHLY ERODIBLE AREAS AS DETERMINED BY THE ENGINEER SHALL BE TEMPORARILY SEEDED AT THE BEGINNING OF CONSTRUCTION WHERE NO CONSTRUCTION ACTIVITIES ARE EXPECTED WITHIN 7 DAYS.

2. PERMANENT SEEDING - SEEDING, CLASS 1 & 2 WILL BE INSTALLED PER IDOT SPECIFICATIONS.

3. EROSION CONTROL BLANKETS/MULCHING - EROSION CONTROL BLANKETS WILL BE INSTALLED OVER FILL SLOPES AND IN HIGH VELOCITY AREAS (LE. DITCHES) THAT HAVE BEEN BROUGHT TO FINAL GRADE AND SEEDED TO PROTECT SLOPES FROM EROSION AND ALLOW SEEDS TO GERMINATE. MULCH, METHOD 2 WILL BE APPLIED IN RELATIVELY FLAT AREAS TO PROTECT THE DISTURBED AREAS AND PREVENT FURTHER EROSION.

MULCH AS APPLIED TO TEMPORARY EROSION CONTROL SEEDING SHALL BE BY THE METHOD SPECIFIED IN THE CONTRACT AND AT THE DIRECTION OF THE ENGINEER. MULCH WILL BE PAID SEPARATELY AND SHALL CONFORM TO SECTION 251 OF THE STANDARD SPECIFICATIONS.

PERMANENT STABILIZATION - ALL AREAS DISTURBED BY CONSTRUCTION WILL BE STABILIZED WITH PERMANENT SEEDING IMMEDIATELY FOLLOWING THE FINISHED GRADING. EROSION CONTROL BLANKETS WILL BE INSTALLED OVER FILL SLOPES WHICH HAVE BEEN BROUGHT TO FINAL GRADE AND HAVE BEEN SEEDED TO PROTECT THE SLOPES FROM RILL AND GULLY EROSION AND ALLOW SEED TO GERMINATE PROPERLY. MULCH, METHOD 2 WILL BE USED ON RELATIVELY FLAT AREAS.

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2. STRUCTURAL PRACTICES: PROVIDED BELOW IS A DESCRIPTION OF STRUCTURAL PRACTICES THAT WILL BE IMPLEMENTED, TO THE DEGREE ATTAINABLE, TO DIVERT FLOWS FROM EXPOSED SOILS, STORE FLOWS OR OTHERWISE LIMIT RUNOFF AND THE DISCHARGE OF POLLUTANTS FROM EXPOSED AREAS OF THE SITE. SUCH PRACTICES MAY INCLUDE BUT ARE NOT LIMITED TO: PERIMETER EROSION BARRIER, EARTH DIKES, DRAINAGE SWALES, SEDIMENT TRAPS, DITCH CHECKS, SUBSURFACE DRAINS, PIPE SLOPE DRAINS, LEVEL SPREADERS, STORM DRAIN INLET PROTECTION, ROCK OUTLET PROTECTION, REINFORCED SOIL RETAINING SYSTEMS, GABIONS, AND TEMPORARY OR PERMANENT SEDIMENT BASINS. THE INSTALLATION OF THESE DEVICES MAY BE SUBJECT TO SECTION 404 OF THE CLEAN WATER ACT.

THE FOLLOWING STRUCTURAL PRACTICES WILL BE USED FOR THIS PROJECT (CHECK ALL THAT APPLY)

M PERIMETER EROSION BARRIER □ ROCK OUTLET PROTECTION ■ TEMPORARY DITCH CHECK ⊠ RIPRAP ■ STORM DRAIN INLET PROTECTION
■ SEDIMENT TRAP ☐ GABIONS ☐ SLOPE MATTRESS ☐ TEMPORARY PIPE SLOPE DRAIN □ RETAINING WALLS D TEMPORARY SEDIMENT BASIN □ SLOPE WALLS D CONCRETE REVETMENT MATS TEMPORARY STREAM CROSSING ☐ STABILIZED CONSTRUCTION EXITS D LEVEL SPREADERS OTHER (SPECIFY)\_. □ TURF REINFORCEMENT MATS □ PERMANENT CHECK DAMS D OTHER (SPECIFY)\_ D PERMANENT SEDIMENT BASIN D OTHER (SPECIFY)\_\_\_\_\_ □ AGGREGATE DITCH □ OTHER (SPECIFY)\_\_\_\_\_ IT PAVED DITCH D OTHER (SPECIFY)

DESCRIBE HOW THE STRUCTURAL PRACTICES LISTED ABOVE WILL BE UTILIZED:

BDE 2342 (REV. 06/07)

1. PERIMETER EROSION BARRIER - SILT FENCES WILL BE PLACED ALONG THE BANKS OF PLUM CREEK IN AN EFFORT TO CONTAIN SILT AND RUNOFF FROM LEAVING THE SITE.

CONSTRUCT AT BEGINNING OF CONSTRUCTION. REMOVE AT END OF CONSTRUCTION.

2. STORM DRAIN INLET PROTECTION - INLET AND PIPE PROTECTION WILL BE PROVIDED FOR CULVERTS AND WILL BE CLEANED ON A REGULAR BASIS.

3. TEMPORARY DITCH CHECKS - DITCH CHECKS WILL BE PLACED IN SWALES WHERE RUNOFF VELOCITY IS HIGH. ALL STRUCTURAL PRACTICES ARE SHOWN IN DETAIL ON THE EROSION CONTROL PLANS.

TEMPORARY DITCH CHECKS SHALL BE LOCATED AT EVERY 1.5 FT. FALL/RISE IN DITCH GRADE.

TEMPORARY DITCH CHECKS, AGGREGATE USES GRADING NO. 3- REMOVE AT END OF CONSTRUCTION.

STRAW BALES, HAY BALES, PERIMETER EROSION BARRIER AND SILT FENCE WILL NOT BE PERMITTED FOR TEMPORARY OR PERMANENT DITCH CHECKS. DITCH CHECKS SHALL BE COMPOSED OF AGGREGATE (IF SPECIFIED), ENVIROBERM, TRIANGULAR SILT DIKES, GEORIGGE AND ROLLED EXCELSIOR.

4. RIPRAP - THE BRIDGE OPENING WILL BE PROTECTED WITH RR-5 RIPRAP FROM ABUTMENT TO ABUTMENT TO PREVENT FROSTON AND SCOURING

AS SOON AS REASONABLE ACCESS IS AVAILABLE TO ALL LOCATIONS WHERE WATER DRAINS AWAY FROM THE PROJECT, TEMPORARY DITCH CHECKS, INLET AND PIPE PROTECTION, AND PERIMETER EROSION BARRIER SHALL BE INSTALLED AS CALLED OUT IN THIS PLAN AND DIRECTED BY THE ENGINEER.

ALL EROSION CONTROL PRODUCTS FURNISHED SHALL BE SPECIFICALLY RECOMMENDED BY THE MANUFACTURER FOR THE USE SPECIFIED IN THE EROSION CONTROL PLAN. PRIOR TO THE APPROVAL AND USE OF THE PRODUCT, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER A NOTARIZED CERTIFICATION BY THE PRODUCER STATING THE INTENDED USE OF THE PRODUCT AND THAT THE PHYSICAL PROPERTIES REQUIRED FOR THIS APPLICATION ARE MET OR EXCEEDED. THE CONTRACTOR SHALL PROVIDE MANUFACTURER INSTALLATION PROCEDURES TO FACILITATE THE ENGINEER IN CONSTRUCTION INSPECTION.

ILLINOIS DEPARTMENT OF TRANSPORTATION

STORM WATER POLLUTION

PREVENTION PLAN

FAS ROUTE 1832

SECTION 5BR-2

WASHINGTON COUNTY

PLOT DATE:\_12/7/2007

\$\$UA|E\$\$ 12/7/2007 c:\$projec†s&ed09105&plan&pln09105a.dgn .\*REF-

- 3. STORM WATER MANAGEMENT: PROVIDED BELOW IS A DESCRIPTION OF MEASURES THAT WILL BE INSTALLED DURING THE CONSTRUCTION PROCESS TO CONTROL POLLUTANTS IN STORM WATER DISCHARGES THAT WILL OCCUR AFTER CONSTRUCTION OPERATIONS HAVE BEEN COMPLETED. THE INSTALLATION OF THESE DEVICES MAY BE SUBJECT TO SECTION 404 OF THE CLEAN WATER ACT.
- G. SUCH PRACTICES MAY INCLUDE BUT ARE NOT LIMITED TO: STORM WATER DETENTION STRUCTURES (INCLUDING WET PONDS), STORM WATER RETENTION STRUCTURES, FLOW ATTENUATION BY USE OF OPEN VEGETATED SWALES AND NATURAL DEPRESSIONS, INFILTRATION OF RUNOFF ON SITE, AND SEQUENTIAL SYSTEMS (WHICH COMBINE SEVERAL DEACTICES)

THE PRACTICES SELECTED FOR IMPLEMENTATION WERE DETERMINED ON THE BASIS OF THE TECHNICAL GUIDANCE IN SECTION 59-8 (EROSION AND SEDIMENT CONTROL) IN CHAPTER 59 (LANDSCAPE DESIGN AND EROSION CONTROL) OF THE ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF DESIGN AND ENVIRONMENT MANUAL. IF PRACTICES OTHER THAN THOSE DISCUSSED IN SECTION 59-8 ARE SELECTED FOR IMPLEMENTATION OR IF PRACTICES ARE APPLIED TO SITUATIONS DIFFERENT FROM THOSE COVERED IN SECTION 59-8, THE TECHNICAL BASIS FOR SUCH DECISIONS WILL BE EXPLAINED BELOW.

D. VELOCITY DISSIPATION DEVICES WILL BE PLACED AT DISCHARGE LOCATIONS AND ALONG THE LENGTH OF ANY OUTFALL CHANNEL AS NECESSARY TO PROVIDE A NON-EROSIVE VELOCITY FLOW FROM THE STRUCTURE TO A WATER COURSE SO THAT THE NATURAL PHYSICAL AND BIOLOGICAL CHARACTERISTICS AND FUNCTIONS ARE MAINTAINED AND PROTECTED (E.G. MAINTENANCE OF HYDROOGIC CONDITIONS SUCH AS THE HYDROPERIOD AND HYDRODYNAMICS PRESENT PRIOR TO THE INITIATION OF CONSTRUCTION ACTIVITIES).

DESCRIPTION OF STORM WATER MANAGEMENT CONTROLS:

THE PHASE I LOCATION DRAINAGE STUDY, PERFORMED BY LOCATION STUDIES HAS DETERMINED THAT NO STORM WATER DETERMINO IS REQUIRED FOR THIS PROJECT.

- 4. OTHER CONTROLS:
- d. VEHICLE ENTRANCES AND EXITS STABILIZED CONSTRUCTION ENTRANCES AND EXITS MUST BE CONSTRUCTED TO PREVENT TRACKING OF SEDIMENTS ONTO ROADWAYS.

THE CONTRACTOR WILL PROVIDE THE RESIDENT ENGINEER WITH A WRITTEN PLAN IDENTIFYING THE LOCATION OF STABILIZED ENTRANCES AND EXITS AND THE PROCEDURES (SHE WILL USE TO CONSTRUCT AND MAINTAIN THEM.

- b. MATERIAL DELIVERY, STORAGE, AND USE THE FOLLOWING BMPS SHALL BE IMPLEMENTED TO HELP PREVENT DISCHARGES OF CONSTRUCTION MATERIALS DURING DELIVERY, STORAGE, AND USE:
  - ALL PRODUCTS DELIVERED TO THE PROJECT SITE MUST BE PROPERLY LABELED.
  - A STORAGE/CONTAINMENT FACILITY SHOULD BE CHOSEN FOR LARGER ITEMS SUCH AS DRUMS AND ITEMS SHIPPED OR STORED ON PALLETS. SUCH MATERIAL IS TO BE COVERED BY A TIN ROOF OR LARGE SHEETS OF PLASTIC TO PREVENT PRECIPITATION FROM COMING IN CONTACT WITH THE PRODUCTS BEING STORED.
  - WATER TIGHT SHIPPING CONTAINERS AND/OR SEMI TRAILERS SHALL BE USED TO STORE HAND TOOLS, SMALL
    PARTS, AND MOST CONSTRUCTION MATERIALS THAT CAN BE CARRIED BY HAND, SUCH AS PAINT CANS, SOLVENTS,
    AND GREASE.
  - LARGE ITEMS SUCH AS LIGHT STANDS, FRAMING MATERIALS AND LUMBER SHALL BE STORED IN THE OPEN IN A
    GENERAL STORAGE AREA. SUCH MATERIAL SHALL BE ELEVATED WITH WOOD BLOCKS TO MINIMIZE CONTACT WITH
    STORM WATER RUNOFF.
  - SPILL CLEAN-UP MATERIALS, MATERIAL SAFETY DATA SHEETS, AN INVENTORY OF MATERIALS, AND EMERGENCY
    CONTACT NUMBERS SHALL BE MAINTAINED AND STORED IN ONE DESIGNATED AREA AND EACH CONTRACTOR IS TO
    INFORM HIS/HER EMPLOYEES AND THE RESIDENT ENCINEER OF THIS LOCATION.
- c. STOCKPILE MANAGEMENT BMPS SHALL BE IMPLEMENTED TO REDUCE OR ELIMINATE POLLUTION OF STORM WATER FROM STOCKPILES OF SOIL AND PAVING MATERIALS SUCH AS BUT NOT LIMITED TO PORTLAND CEMENT CONCRETE RUBBLE, ASPHALT CONCRETE, ASPHALT CONCRETE RUBBLE, AGGREGATE BASE, AGGREGATE SUB BASE, AND PRE-MIXED AGGREGATE. THE FOLLOWING BMPS MAY BE CONSIDERED:
  - PERIMETER EROSION BARRIER
  - TEMPORARY SEEDING
  - TEMPORARY MULCH
  - PLASTIC COVERS
  - SOIL BINDERS
  - PERIMETER EROSION BARRIER

THE CONTRACTOR WILL PROVIDE THE RESIDENT ENGINEER WITH A WRITTEN PLAN OF THE PROCEDURES (S)HE WILL USE ON THE PROJECT AND HOW THEY WILL BE MAINTAINED.

- d. WASTE DISPOSAL. NO MATERIALS, INCLUDING BUILDING MATERIALS, SHALL BE DISCHARGED INTO WATERS OF THE STATE, EXCEPT AS AUTHORIZED BY A SECTION 404 PERMIT.
- e. THE PROVISIONS OF THIS PLAN SHALL ENSURE AND DEMONSTRATE COMPLIANCE WITH APPLICABLE STATE AND/OR LOCAL WASTE DISPOSAL, SANITARY SEWER OR SEPTIC SYSTEM REGULATIONS.
- f. THE CONTRACTOR SHALL PROVIDE A WRITTEN AND GRAPHIC PLAN TO THE RESIDENT ENGINEER IDENTIFYING WHERE EACH OF THE ABOVE AREAS WILL BE LOCATED AND HOW THEY ARE TO BE MANAGED.

#### 5. APPROVED STATE OR LOCAL LAWS

THE MANAGEMENT PRACTICES, CONTROLS AND PROVISIONS CONTAINED IN THIS PLAN WILL BE IN ACCORDANCE WITH IDOT SPECIFICATIONS, WHICH ARE AT LEAST AS PROTECTIVE AS THE REQUIREMENTS CONTAINED IN THE ILLINOIS ENVIRONMENTAL PROTECTION AGENCY'S ILLINOIS URBAN MANUAL, 1995. PROCEDURES AND REQUIREMENTS SPECIFIED IN APPLICABLE SEDIMENT AND EROSION SITE PLANS OR STORM WATER MANAGEMENT PLANS APPROVED BY LOCAL OFFICIALS SHALL BE DESCRIBED OR INCORPORATED BY REFERENCE IN THE SPACE PROVIDED BELOW. REQUIREMENTS SPECIFIED IN SEDIMENT AND EROSION SITE PLANS, SITE PERMITS. SPECIFIED IN SEDIMENT AND EROSION SITE PLANS, SITE PERMITS SPECIFIED IN SEDIMENT AND EROSION SITE PLANS OR SITE PERMITS APPROVED BY LOCAL OFFICIALS THAT ARE APPLICABLE TO PROTECTING SURFACE WATER RESOURCES ARE, UPON SUBMITTAL OF AN NOI, TO BE AUTHORIZED TO DISCHARGE UNDER PERMIT ILRIO INCORPORATED BY REFERENCE AND ARE ENFORCEABLE UNDER THIS PERMIT EVEN IF THEY ARE NOT SPECIFICALLY INCLUDED IN THE PLAN.

DESCRIPTION OF PROCEDURES AND REQUIREMENTS SPECIFIED IN APPLICABLE SEDIMENT AND EROSION SITE PLANS OR STORM WATER MANAGEMENT PLANS APPROVED BY LOCAL OFFICIALS.

ALL MANAGEMENT PRACTICES, CONTROLS, AND OTHER PROVISIONS PROVIDED IN THIS PLAN ARE IN ACCORDANCE WITH "IDOT STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION AND THE ILLINOIS URBAN MANUAL".

#### III. MAINTENANCE:

THE FOLLOWING IS A DESCRIPTION OF PROCEDURES THAT WILL BE USED TO MAINTAIN, IN GOOD AND EFFECTIVE OPERATING CONDITIONS, THE VECETATION, EROSION AND SEDIMENT CONTROL MEASURES AND OTHER PROTECTIVE MEASURES. IDENTIFIED IN THIS PLAN

1. SEEDING - ALL ERODIBLE BARE EARTH WILL BE TEMPORARILY SEEDED ON A WEEKLY BASIS TO MINIMIZE THE AMOUNT OF ERODIBLE SURFACE WITHIN THE CONTRACT LIMITS.

2. PERIMETER EROSION BARRIER - SEDIMENT WILL BE REMOVED IF THE INTEGRITY OF THE FENCING IS IN JEOPARDY AND ANY FENCING KNOCKED DOWN WILL BE REPAIRED IMMEDIATELY. THE COST OF THIS MAINTENANCE SHALL BE ACCORDING TO ARTICLE 109.04 OF THE STANDARD SPECIFICATIONS.

3. DITCH CHECKS - SEDIMENT WILL BE REMOVED IF THE INTEGRITY OF THE DITCH CHECK IS IN JEOPARDY. ANY DITCH CHECKS WHICH FAIL WILL BE REPAIRED OR REPLACED IMMEDIATELY. THE COST OF THIS MAINTENANCE SHALL BE ACCORDING TO ARTICLE 109.04 OF THE STANDARD SPECIFICATIONS.

THE RESIDENT ENGINEER WILL PROVIDE MAINTENANCE GUIDES TO THE CONTRACTOR FOR THESE PRACTICES. ALL MAINTENANCE OF EROSION CONTROL SYSTEMS WILL BE THE RESPONSIBILITY OF THE CONTRACTOR UNTIL CONSTRUCTION IS COMPLETE AND ACCEPTED BY IDOT AFTER FINAL INSPECTION. ALL LOCATIONS WHERE VEHICLES ENTER AND EXIT THE CONSTRUCTION SITE AND ALL OTHER AREAS SUBJECT TO EROSION SHOULD ALSO BE INSPECTED PERIODICALLY.

INSPECTION OF THESE AREAS SHALL BE MADE AT LEAST ONCE EVERY SEVEN DAYS AND WITHIN 24 HOURS OF THE END OF EACH 0.5 INCHES OR GREATER RAINFALL, OR AN EQUIVALENT SNOWFALL. THE PROJECT SHALL ADDITIONALLY BE INSPECTED BY THE CONSTRUCTION FIELD ENGINEER ON A BI-WEEKLY BASIS TO DETERMINE THAT EROSION CONTROL EFFORTS ARE IN PLACE AND EFFORTIVE AND IF OTHER EROSION CONTROL WORK IS NECESSARY.

THE TEMPORARY EROSION CONTROL SYSTEMS SHALL BE REMOVED AS DIRECTED BY THE ENGINEER AFTER USE IS NO LONGER NEEDED. THE COST OF THIS REMOVAL SHALL BE INCLUDED IN THE UNIT BID PRICE FOR THE TEMPORARY EROSION CONTROL SYSTEM.

#### IV. INSPECTIONS

QUALIFIED PERSONNEL SHALL INSPECT DISTURBED AREAS OF THE CONSTRUCTION SITE WHICH HAVE NOT YET BEEN FINALLY STABILIZED, STRUCTURAL CONTROL MEASURES, AND LOCATIONS WHERE VEHICLES AND EQUIPMENT ENTER AND EXIT THE SITE. SUCH INSPECTIONS SHALL BE CONDUCTED AT LEAST ONCE EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF THE END OF A STORM THAT IS 0.5 INCHES OR GREATER OR EQUIVALENT SNOWFALL.

- A. DISTURBED AREAS, USE AREAS (STORAGE OF MATERIALS, STOCKPILES, MACHINE MAINTENANCE, FUELING, ETC.), BORROW SITES, AND WASTE SITES SHALL BE INSPECTED FOR EVIDENCE OF, OR THE POTENTIAL FOR, POLLUTANTS ENTERING THE DRAINAGE SYSTEM. EROSION AND SEDIMENT CONTROL MEASURES IDENTIFIED IN THE PLAN SHALL BE OBSERVED TO ENSURE THAT THEY ARE OPERATING CORRECTLY. DISCHARGE LOCATIONS OR POINTS THAT ARE ACCESSIBLE, SHALL BE INSPECTED TO ASCERTAIN WHETHER EROSION CONTROL MEASURES ARE EFFECTIVE IN PREVENTING SIGNIFICANT IMPACTS TO RECEIVING WATERS. LOCATIONS WHERE VEHICLES ENTER OR EXIT THE SITE SHALL BE INSPECTED FOR EVIDENCE OF OFF SITE SEDIMENT TRACKING.
- B. BASED ON THE RESULTS OF THE INSPECTION, THE DESCRIPTION OF POTENTIAL POLLUTANT SOURCES IDENTIFIED IN SECTION I ABOVE AND POLLUTION PREVENTION MEASURES IDENTIFIED IN SECTION II ABOVE SHALL BE REVISED AS APPROPRIATE AS SOON AS PRACTICABLE AFTER SUCH INSPECTION. ANY CHANGES TO THIS PLAN RESULTING FROM THE REQUIRED INSPECTIONS SHALL BE IMPLEMENTED WITHIN 1/2 HOUR TO 1 WEEK BASED ON THE URGENCY OF THE SITUATION. THE RESIDENT ENGINEER WILL NOTIFY THE CONTRACTOR OF THE TIME REQUIRED TO IMPLEMENT SUCH ACTIONS THROUGH THE WEEKLY INSPECTION REPORT.
- C. A REPORT SUMMARIZING THE SCOPE OF THE INSPECTION, NAME(S) AND QUALIFICATIONS OF PERSONNEL MAKING
  THE INSPECTION, THE DATE(S) OF THE INSPECTION, MAJOR OBSERVATIONS RELATING TO THE IMPLEMENTATION OF
  THIS STORM WATER POLLUTION PREVENTION PLAN, AND ACTIONS TAKEN IN ACCORDANCE WITH SECTION IV(B)
  SHALL BE MADE AND RETAINED AS PART OF THE PLAN FOR AT LEAST THREE (3) YEARS AFTER THE DATE OF THE
  INSPECTION. THE REPORT SHALL BE SIGNED IN ACCORDANCE WITH PART VI. G OF THE GENERAL PERMIT.
- D. IF ANY VIOLATION OF THE PROVISIONS OF THIS PLAN IS IDENTIFIED DURING THE CONDUCT OF THE CONSTRUCTION WORK COVERED BY THIS PLAN, THE RESIDENT ENGINEER SHALL COMPLETE AND FILE AN "INCIDENCE OF NONCOMPLIANCE" (ION) REPORT FOR THE IDENTIFIED VIOLATION. THE RESIDENT ENGINEER SHALL USE FORMS PROVIDED BY THE ILLINOIS ENVIRONMENTAL PROTECTION AGENCY AND SHALL INCLUDE SPECIFIC INFORMATION ON THE CAUSE OF NONCOMPLIANCE, ACTIONS WHICH WERE TAKEN TO PREVENT ANY FURTHER CAUSES OF NONCOMPLIANCE, AND A STATEMENT DETAILING ANY ENVIRONMENTAL IMPACT WHICH MAY HAVE RESULTED FROM THE NONCOMPLIANCE. ALL REPORTS OF NONCOMPLIANCE SHALL BE SIGNED BY A RESPONSIBLE AUTHORITY IN ACCORDANCE WITH PART VI. G OF THE GENERAL PERMIT.

THE INCIDENCE OF NON-COMPLIANCE SHALL BE MAILED TO THE FOLLOWING ADDRESS:

ILLINOIS ENVIRONMENTAL PROTECTION AGENCY DIVISION OF WATER POLLUTION CONTROL ATTN: COMPLIANCE ASSURANCE SECTION 1021 NORTH GRAND EAST POST OFFICE BOX 19276 SPRINGFIELD, ILLINOIS 62794-9276

#### V. NON-STORM WATER DISCHARGES:

EXCEPT FOR FLOWS FROM FIRE FIGHTING ACTIVITIES, SOURCES OF NON-STORM WATER THAT IS COMBINED WITH STORM WATER DISCHARGES ASSOCIATED WITH THE INDUSTRIAL ACTIVITY ADDRESSED IN THIS PLAN MUST BE DESCRIBED BELOW. APPROPRIATE POLLUTION PREVENTION MEASURES, AS DESCRIBED BELOW, WILL BE IMPLEMENTED FOR THE NON-STORM WATER COMPONENTS) OF THE DISCHARGE.

- A. SPILL PREVENTION AND CONTROL BMPS SHALL BE IMPLEMENTED TO CONTAIN AND CLEAN-UP SPILLS AND PREVENT MATERIAL DISCHARGES TO THE STORM DRAIN SYSTEM. THE CONTRACTOR SHALL PRODUCE A WRITTEN PLAN STATING HOW HIS/HER COMPANY WILL PREVENT, REPORT, AND CLEAN UP SPILLS AND PROVIDE A COPY TO ALL OF HIS/HER EMPLOYEES AND THE RESIDENT ENGINEER. THE CONTRACTOR SHALL NOTIFY ALL OF HIS/HER EMPLOYEES ON THE PROPER PROTOCOL FOR REPORTING SPILLS. THE CONTRACTOR SHALL NOTIFY THE RESIDENT ENGINEER OF ANY SPILLS IMMEDIATELY.
- B. CONCRETE RESIDUALS AND WASHOUT WASTES THE FOLLOWING BMPS SHALL BE IMPLEMENTED TO CONTROL RESIDUAL CONCRETE, CONCRETE SEDIMENTS, AND RINSE WATER:
  - 1. TEMPORARY CONCRETE WASHOUT FACILITIES SHALL BE CONSTRUCTED FOR RINSING OUT CONCRETE TRUCKS. SIGNS SHALL BE INSTALLED DIRECTING CONCRETE TRUCK DRIVERS WHERE DESIGNATED WASHOUT FACILITIES ARE LOCATED.

CONTRACT NO. 76949

COUNTY TOTAL SHEET NO

WASHINGTON 97

RTE. SECTION 1832 5BR-2

EXISTING CONDITIONS:

- THE CONTRACTOR SHALL HAVE THE LOCATION OF TEMPORARY CONCRETE WASHOUT FACILITIES APPROVED BY THE RESIDENT ENGINEER.
- 3. ALL TEMPORARY CONCRETE WASHOUT FACILITIES ARE TO BE INSPECTED BY THE CONTRACTOR AFTER EACH USE AND ALL SPILLS MUST BE REPORTED TO THE RESIDENT ENGINEER AND CLEANED UP IMMEDIATELY.
- 4. CONCRETE WASTE SOLIDS/LIQUIDS SHALL BE DISPOSED OF PROPERLY.
- C. LITTER MANAGEMENT A PROPER NUMBER OF DUMPSTERS SHALL BE PROVIDED ON SITE TO HANDLE DEBRIS AND LITTER ASSOCIATED WITH THE PROJECT. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING HIS/HER EMPLOYEES PLACE ALL LITTER INCLUDING MARKING PAINT CANS, SODA CANS, FOOD WRAPPERS, WOOD LATHE, MARKING RIBBON, CONSTRUCTION STRING, AND ALL OTHER CONSTRUCTION RELATED LITTER IN THE PROPER DUMPSTERS.
- D. VEHICLE AND EQUIPMENT CLEANING VEHICLES AND EQUIPMENT ARE TO BE CLEANED IN DESIGNATED AREAS ONLY,
  PREFERABLY OFF SITE.
- E. VEHICLE AND EQUIPMENT FUELING A VARIETY OF BMPS CAN BE IMPLEMENTED DURING FUELING OF VEHICLES AND EQUIPMENT TO PREVENT POLLUTION. THE CONTRACTOR SHALL INFORM THE RESIDENT ENGINEER AS TO WHICH BMPS WILL BE USED ON THE PROJECT. THE CONTRACTOR SHALL INFORM THE RESIDENT ENGINEER HOW (SHE WILL BE INFORMING HIS/HER EMPLOYEES OF THESE BMPS (I.E. SIGNS, TRAINING, ETC.). BELOW ARE A FEW EXAMPLES OF THESE BMPS:
  - 1. CONTAINMENT
  - 2. SPILL PREVENTION AND CONTROL
  - 3. USE OF DRIP PANS AND ABSORBENTS
  - 4. AUTOMATIC SHUT-OFF NOZZLES
  - 5. TOPPING OFF RESTRICTIONS
  - 6. LEAK INSPECTION AND REPAIR
- F. VEHICLE AND EQUIPMENT MAINTENANCE ON SITE MAINTENANCE MUST BE PERFORMED IN ACCORDANCE WITH ALL ENVIRONMENTAL LAWS SUCH AS PROPER STORAGE AND NO DUMPING OF OLD ENGINE OIL OR OTHER FLUIDS ON SITE.

#### VI. FAILURE TO COMPLY:

FAILURE TO COMPLY WITH ANY PROVISIONS OF THIS STORM WATER POLLUTION PREVENTION PLAN WILL RESULT IN THE IMPLEMENTATION OF AN EROSION AND SEDIMENT CONTROL DEFICIENCY DEDUCTION AGAINST THE CONTRACTOR AND/OR PENALTIES UNDER THE NPDES PERMIT WHICH COULD BE PASSED ONTO THE CONTRACTOR.

#### LEGEND

TEMPORARY DITCH CHECK- ROLLED EXCELSIOR, SILT WEDGES/PANELS

TEMPORARY DITCH CHECK- AGGREGATE

EROSION CONTROL BLANKET

PERIMETER EROSION BARRIER- SILT FILTER
FENCE OR OTHER AS APPROVED BY THE ENGINEER

INLET AND PIPE PROTECTION- STRAW BALES,
FILTER FABRIC, AGGREGATES

REVISIONS
NAME
DATE
FAS ROUTE 1832
SECTION 5BR-2
WASHINGTON COUNTY

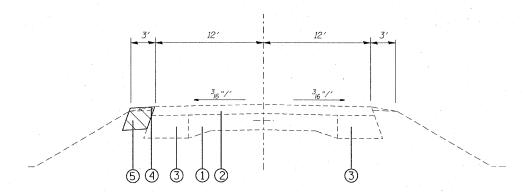
BDE 2342 (REV. 06/07)

REVISIONS
NAME
DATE
REVISIONS
PREVENTION PLAN
FAS ROUTE 1832
SECTION 5BR-2
WASHINGTON COUNTY

PLOT DATE: 12/7/2007

#### EXISTING TYPICAL SECTION

STA. 1449+25.00 TO STA. 1457+50.00

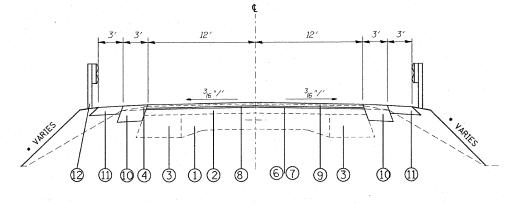


#### PROPOSED TYPICAL SECTION

STA. 1449+25.00 TO STA. 1457+50.00 - LT



REMOVAL



\* SEE CROSS SECTIONS

#### PROPOSED TYPICAL SECTION

STA. 1449+25.00 TO STA. 1457+50.00

## LEGEND

- EXISTING P.C.C. PAVEMENT 9-6-9
- EXISTING BITUMINOUS OVERLAY 6" (±)
- EXISTING BASE COURSE WIDENING 8"
- EXISTING AGGREGATE SHOULDERS
- (5) PROPOSED HOT-MIX ASPHALT BASE COURSE WIDENING, 9"
- 6 PROPOSED BITUMINOUS MATERIALS (PRIME COAT)
- 7 PROPOSED AGGREGATE (PRIME COAT)
- PROPOSED HOT-MIX ASPHALT BINDER COURSE (VARIES 3/4" TO 16")
- 9 PROPOSED HOT-MIX ASPHALT SURFACE COURSE, 11/2"
- PROPOSED HOT-MIX ASPHALT SHOULDER, 8 "
- (11) PROPOSED AGGREGATE SHOULDER, TYPE B 6"
- PROPOSED GUARDRAIL

MIXTURE REQUIREMENTS

A Company of the Comp					
MIXTURE USE	SURFACE	BINDER	WIDENING COURSE	INCIDENTAL SURF	SHOULDERS
AC/PG	PG 64-22	PG 64-22	PG 64-22	PG 64-22	PG 58-22
RAP % (MAX)	10%	15%	15%	10%	30%
DESIGN AIR VOIDS	4.0% @ Ndes= 70	4.0% @ Ndes= 70	4.0% @ Ndes= 70	4.0% @ Ndes=70	2.0% @ Ndes=30
MIX COMPOSITION		*			
(GRADATION MIXTURE)					
FRICTION AGG	MIXTURE "D"	MIXTURE "B"	MIXTURE "B"	MIXTURE "C"	ВАМ

PLAN QUANTITIES FOR HOT-MIX ASPHALT SURFACE COURSE ITEMS ARE CALCULATED USING A UNIT WEIGHT OF 112 LB/SQ YD/IN (59.8 KG/SQ M/25 MM THICKNESS).

S.N. 095-0077

ILLINOIS		REVISIONS
ILLINOIS	DATE	NAME
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MI		
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SCALE: VERT. HORIZ.		
DATE		

DEPARTMENT OF TRANSPORTATION TYPICAL SECTIONS MIXTURE REQUIREMENT

FAS ROUTE 1832 SECTION 5BR-2 WASHINGTON COUNTY

COUNTY TOTAL SHEET SHEETS NO.

WASHINGTON 97 10

SECTION 5BR-2

STA. \_\_\_\_\_\_
FED. ROAD DIST. NO. \_ ILLINOIS FED. AID PROJECT

1832

STA.

#### RESURFACING SCHEDULE

STATION	RT/LT	HOT-MIX ASPHALT BS WIDENING 9" (SQ YD)		BIT. MAT'L PRIME COAT (TON)	HMA CONC BINDER SUPER, MIX "B" (TON)	HMA CONC SURF CSE., SUPER., MIX "C", N70	INCIDENTAL HMA SURF. (TON)	HMA SHOULDERS 8" (SQ YD)	AGG. SHLD TYPE B 6" (SQ YD)	AGG. BASE COURSE TYPE B 6" (SQ YD)
1449+25.00 TO 1450+32.00			(1017)	(10.0)	11.98	(1014)	(1011)	130 107	130 157	150 157
1449+25.00 TO 1452+49.50	-				11100					
1449+25.00 TO 1452+83.00	1 T	119.33	-							
1449+25.00 TO 1452+49.50	RT/LT		1.30	0.27		72.69		216.33	216.33	
1450+32.00 TO 1452+49.50					292.32					
1453+47.00 TO 1457+50.00	LT	134.33								
1453+73.50 TO 1457+00.00	RT/LT				341.30					
1453+73.50 TO 1457+50.00	RT									
1453+73.50 TO 1457+50.00	RT/LT		1.51	0.31		84.34	***************************************	251.00	251.00	
1455+97.90 PE	RT						5.08			45.33
1457+00.00 TO 1457+50.00	RT/LT				5.60					
TOTAL		253.67	2.80	0.58	651.21	157.03	5.08	467.33	467.33	45.33

#### TEMPORARY PAVEMENT MARKING SCHEDULE

						PA	VEMENT MARKIN	NG .		
	STATION					LINE 4" LINE 6" PAVEMENT (TEMP. BAR (FT) (FT)		LINE 24" PAVEMENT (FT)	WORK ZONE PVMT REMOVAL (SQ FT)	PVMT MRKG REMOVAL (SQ FT)
1446+78.00			RT	STOP	BARS			24	48.0	
1446+78.00	TO	1459+97.00		STAGE	1 & 2	5276.00			1758.7	
1446+78.00	TO	1459+97.00	RT/LT							879.3
1446+78.00	TO	1459+97.00	CL					*		109.9
1448+48.00	TO	1458+27.00		STAGE	1 & 2		1820			
1455+97.90		ENTRANCE	LT	STOP	BARS			12.0	24.0	
1459+97.00			LT	STOP	BARS			24.0	48.0	-
SUB	-T0	TAL				1000 1000 1000 1000 1000				
T	OTA	L				5276.0	1820.0	60.0	1878.7	989.3

#### PAVEMENT MARKING SCHEDULE

				PAVEN	MENT - THERM	OPLATIC	BRID	OGE - POLYURE	EA T-1
S1	ΓΑΤΙ	ON		4" WHITE LINE (FT)	YELLOW SKIP DASH LINE 4" (FT)	DBL AMBER RSD REFL PMK (EA)	4" WHITE LINE (FT)	YELLOW SKIP DASH LINE 4" (FT)	DBL AMBER RSD REFL PMK (EA)
1446+77.00	TO	1452+49.50	CL		143.125	7			
1446+77.00	TO	1452+49.50	RT/LT	1145					
1452+49.50	TO	1453+73.50	CL					31	2
1452+49.50	TO	1453+73.50	RT/LT				248.00		
1453+73.50	TO	1459+98.00	RT/LT	1249					
1453+73.50	TO	1459+98.00	CL		156.125	8			
SUE	3~TO	TAL		2394.00	299.25	15	248.00	31.00	2
7	ATOTA	L			2693.25	15		279	2

#### GUARDRAIL SCHEDULE

ST	ATI	ON	RT/LT	SPBGR (FT)	TBT - T1 (SPECIAL) (EA)	TBT - T6	GUARDRAIL MRKS TY-A (EA)	BI-DIREC PRISM BARR REFLEC (EA)
1450+50.50	TO	1452+81.75	RT	137.5	1	1	3	
1451+38.00	TO	1452+81.75	LT	50	1	1	2	
1452+79.50	TO	1453+43.50	RT/LT					2
1453+42.50	TO	1455+73.75	LT	137.5	1	1	3	
1453+42.50	TO	1454+86.25	RT	50	1	1	2	,
		TOTAL		375	4	4	9	2

#### PIPE CULVERT SCHEDULE

		LOCATION		DSFL	USFL	CLASS D TYPE 1 18 INCH	SECT.	
FROM	OFFSET	ТО	OFFSET	RT/LT			(FT)	(FT)
1456+00.20	29	1456+25.70	29	RT	436.44	436.63	25.5	2
		TOTAL					25.5	2

#### REMOVAL SCHEDULE

					PVMT R	EMOVAL			
	LOCATION					WIDENING REMOVAL	HMA SURF REMOVAL VAR. DEPTH	SPBGR	PIPE CULVERT 18 INCH
FROM	OFFSET	TO	OFFSET	RT/LT	(SQ YD)	(SQ YD)	(SQ YD)	(FT)	(FT)
1449+25.00		1450+32.00		RT/LT			285.33		
1449+25.00		1452+83.00		LT		119.33			
1451+05.00		1452+82.00		RT				177.00	
1451+80.00		1452+82.00		LT				102.00	
1452+49.50		1453+00.50		RT/LT	136.00				
1453+29.00		1453+73.50		RT/LT	118.67				
1453+47.00		1454+49.00		RT				102.00	
1453+47.00		1455+25.00		LT				178.00	
1453+47.00		1457+50.00		LT		134.33			
1456+00.00	25.5	1456+25.40	24.3	RT					25.50
1457+17.00		1457+50.00		RT/LT			88.00		
		TOTAL			254.67	253.66	373.33	559.00	25.50
Sec. 1	,				508	3.33			

S.N. 095-0077

REVISIONS		ILLINOIS DEPARTMENT OF TRANSPORTATION						
NAME	DATE	ILLINOIS DEI AIN	TIMENT OF TRANSPORTATION					
		SCHEDUL	E OF QUANTITIES					
		FA FA	S ROUTE 1832					
		SECTION 5BR-2						
		WASI	HINGTON COUNTY					
		SCALE: VERT.	DRAWN BY					
		HORIZ.	DRAWN DI					
		DATE	CHECKED BY	-				

#### TREE REMOVAL SCHEDULE

CTATION	OFFERT	DT /I T		. TA	15 '	JNITS	,			OVED	1E 1	JNITS		
STATION	OFFSET	RT/LT	<u>'</u>	5 10	15 (		,			UVER	15 (	1 1 1 NIC		
1450+57.90	32.5	LT			<u> </u>	12								
1450+63.50	30.4	LT			10									
1450+63.50	30.3	LT				12								
1450+63.70	28.7	LT			10									-
1450+65.90	32.9	LT	6											
1450+82.69	31.2	LT							20					<u> </u>
1450+98.04	31.3	LT				12								
1451+05.00	32.5	LT			10									
1451+08.90	32.8	LT	6											
1451+12.00	28.1	LT					15							
1451+13.30	28.0	LT					15							
1451+29.70	30.1	LT			10									
1451+38.80	34.2	LT					15							
1451+39.67	42.4	LT										36		
1451+40.50	34.8	LT								24				
1451+51.60	46.3	LT	<u> </u>		<b></b>	l					30			
1451+53.20	31.5	LT			10									
1451+53.77	44.1	LT		8					<b></b>					
1451+54.67	31.9	LT	<b></b>	-						24				-
1451+62.24	41.5	LT		-	<del> </del>		-		-	-7	30			
1451+63.48	34.5	LT							<del> </del>	24	30			-
			-						-	24	ļ			
1451+68.40	33.1	LT	6	<u> </u>	10									-
1451+71.60	33.8	LT	-		10		-	10				-		
1451+73.03	42.3	LT						18						
1451+80.05	34.9	LT	6											
1451+80.90	33.0	LT	6	<u> </u>										_
1451+84.40	37.0	LT										36		
1451+85.70	28.6	LT						18						
1452+01.20	41.0	LT		8										
1452+03.85	45.4	LT		8										
1452+16.90	38.9	LT										36		
1452+18.40	30.1	LT						18						
1452+19.17	31.3	LT						18						
1452+19.86	30.8	LT								24				
1452+20.50	46.5	LT	6		<u> </u>									
1452+22.70	44.7	LT		8										
1452+27.89	46.4	LT		8										
1452+35.77	40.0	LT		۰			15				-			-
1452+36.50	42.2	LT					13			24	-			
1452+40.80	44.0	LT	6		-		-							-
							<del> </del>							
1452+48.92	39.5	LT	6		-	ļ								-
1452+51.23	42.0	LT	ļ .	8			-	<u> </u>		-		-		
1452+53.86	40.2	LT		8		ļ		ļ	ļ	0.1	ļ			
1452+55.64	36.1	LT	<u> </u>			ļ	ļ		ļ	24	<u> </u>			
1452+62.53	49.4	LT	6		ļ	ļ			<u> </u>	-				
1452+66.71	47.5	LT	6			<u> </u>								
1452+75.03	31.4	LT			10		L	ļ		<u> </u>	ļ			<u>.</u>
1452+75.60	32.5	LT	6											
1452+81.28	46.7	LT			10									
1452+98.54	26.3	LT		8										L
1452+99.00	26.2	LT	6		l									
1454+04.19	33.3	LT											44	
1454+04.64	30.8	LT					15			-				
1454+06.74	32.6	LT	l		<u> </u>	<u> </u>	İ			24				T
1454+13.04	31.0	LT	6			l .				<b></b>				
1454+19.94	33.3	LT	F	<b></b>	<u> </u>	<b></b>	<b> </b>	18		<b> </b>			-	
1456+85.80	33.0	LT	<b></b>		<del> </del>	<u> </u>	15	<u> </u>	<del> </del>	<del> </del>				<u> </u>
1457+23.60	30.3	LT	<del> </del>	-				-	-	<b></b>			-	50
1457+43.90	34.1	LT		8		<b></b>	-			-	-	-		33
11011700	J 101		-	٦	<u> </u>	-								-
ÇI IE	B-TOTAL		78	72	80	36	90	90	20	168	60	108	44	50

#### SEEDING SCHEDULE

			SEEDING	SEEDING	NITROGEN	PHOSPHORUS	POTASSIUM	MULCH
STA	STA	AREA (SQ FT)	CLASS 2 (ACRE)	CLASS 1 (ACRE)	FERT. NUTR (POUND)	FERT. NUTR (POUND)	FERT. NUTR (POUND)	METHOD :
1449+25.00 TO	1457+50.00 LT	6370	0.15		13.16	13.16	13.16	0.15
1449+25.00 TO	1452+79.50 RT	2952		0.1	6.10	6.10	6.10	0.07
1455+92.00 TO	1458+00.00 RT	2113		0.1	4.37	4.37	4.37	0.05
1449+25.00 TO	1457+50.00 RT	9568	0.22		19.77	19.77	19.77	0.22
·TO	OTAL		0.37	0.20	43.40	43.40	43.40	0.49

#### EARTHWORK SCHEDULE

LOCATION	EARTH EXCAVATION (CU YD)	EARTH EXCAVATION ADJTD FOR SHRINKAGE (CU YD)	EMBANKMENT (CU YD)	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-) (CU YD)
STA. 1449+25.00 TO STA. 1452+79.50	197.3	147.9	668.1	-520.2
STA. 1453+43.50 TO STA. 1457+50.00	401.3	301.0	435.0	-134.1
TOTAL	598.6	448.9	1103.1	-654.3

#### EARTHWORK SCHEDULE (WIDENING)

LOCATION	EARTH EXCAVATION (CU YD)	EARTH EXCAVATION ADJTD FOR SHRINKAGE (CU YD)	EMBANKMENT (CU YD)	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-) (CU YD)
STA. 1449+25.00 TO STA. 1452+83.00	30.5	22.9	0.0	22.9
STA. 1453+47.00 TO STA. 1457+50.00	34.3	25.7	0.0	25.7
TOTAL	64.8	48.6	0.0	48.6

#### ROW MARKERS SCHEDULE

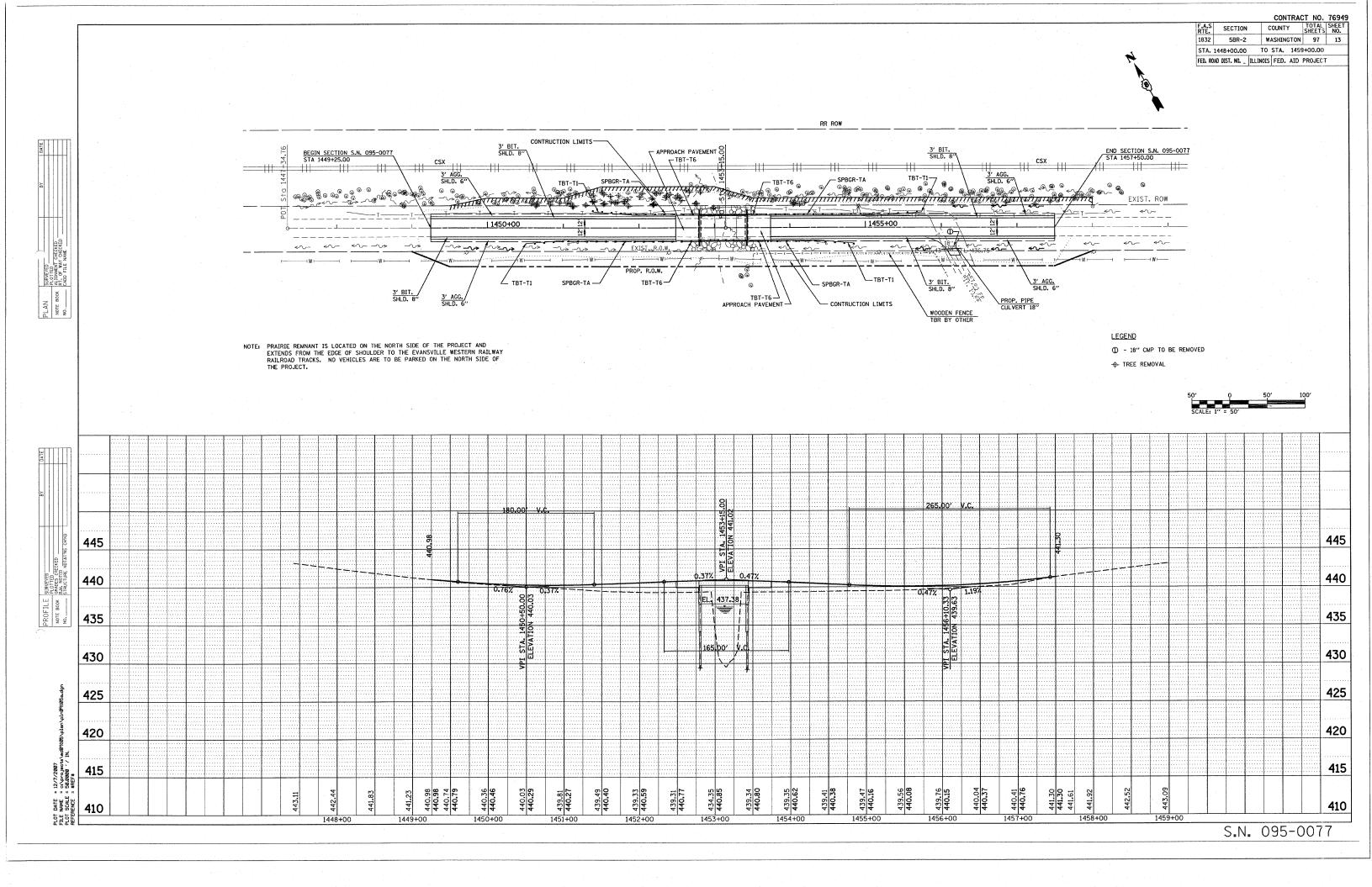
LC STATION	ROW MARKERS (EA)		
1449+00.00	29.11	L†	1
1449+00.00	30.89	Rt	1
1449+50.00	50.89	R†	1
1450+00.00	39.11	L†	1
1451+00.00	39.12	L†	1
1451+50.00	54.13	L†	1
1453+00.00	54.14	L†	1
1453+50.00	39.14	Lt	1
1457+50.00	50.83	R†	1
1458+00.00	30.83	R†	1
1458+00.00	39.17	L†	1
1458+00.00	29.17	L+	1
	TOTAL		12

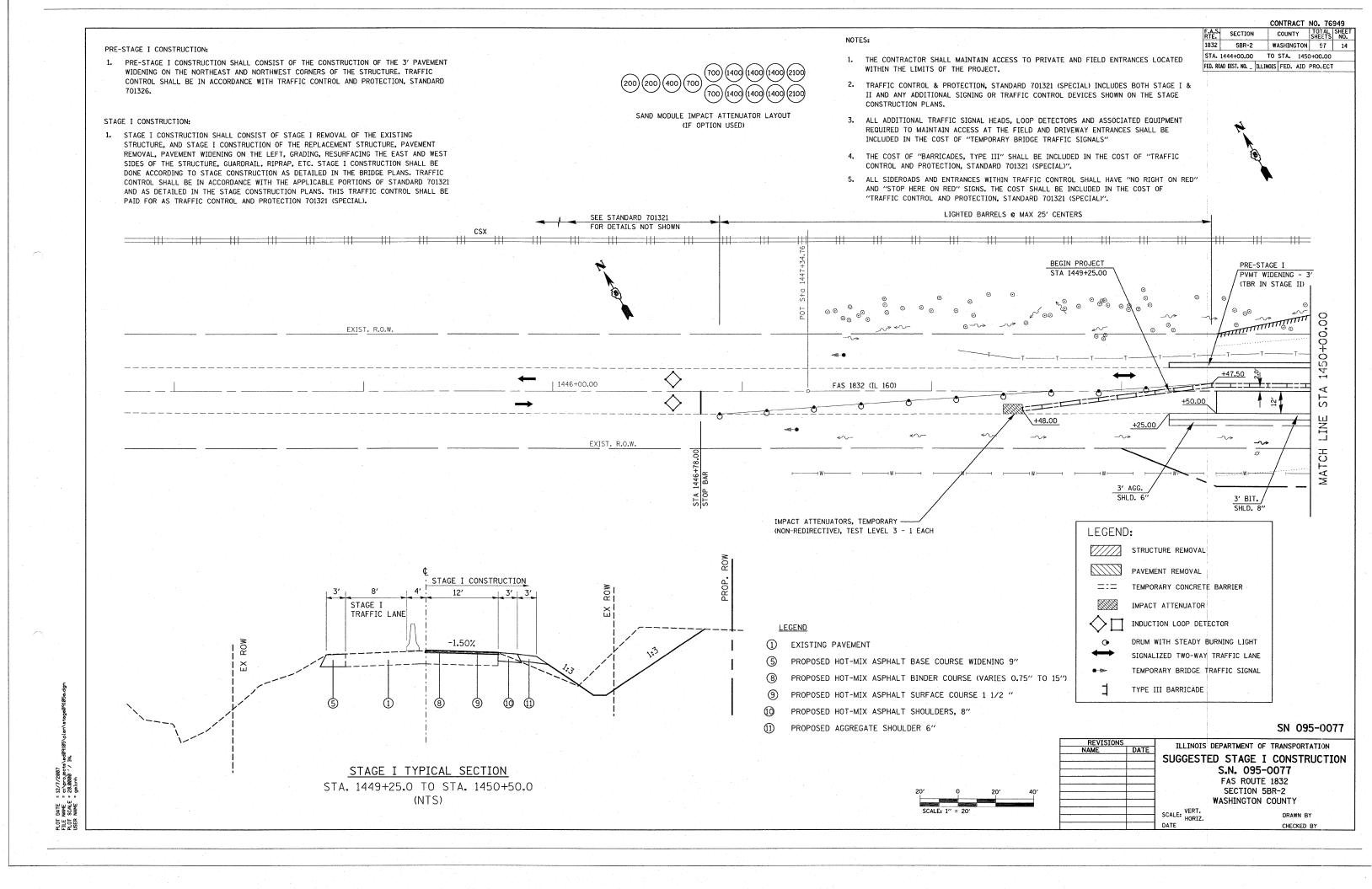
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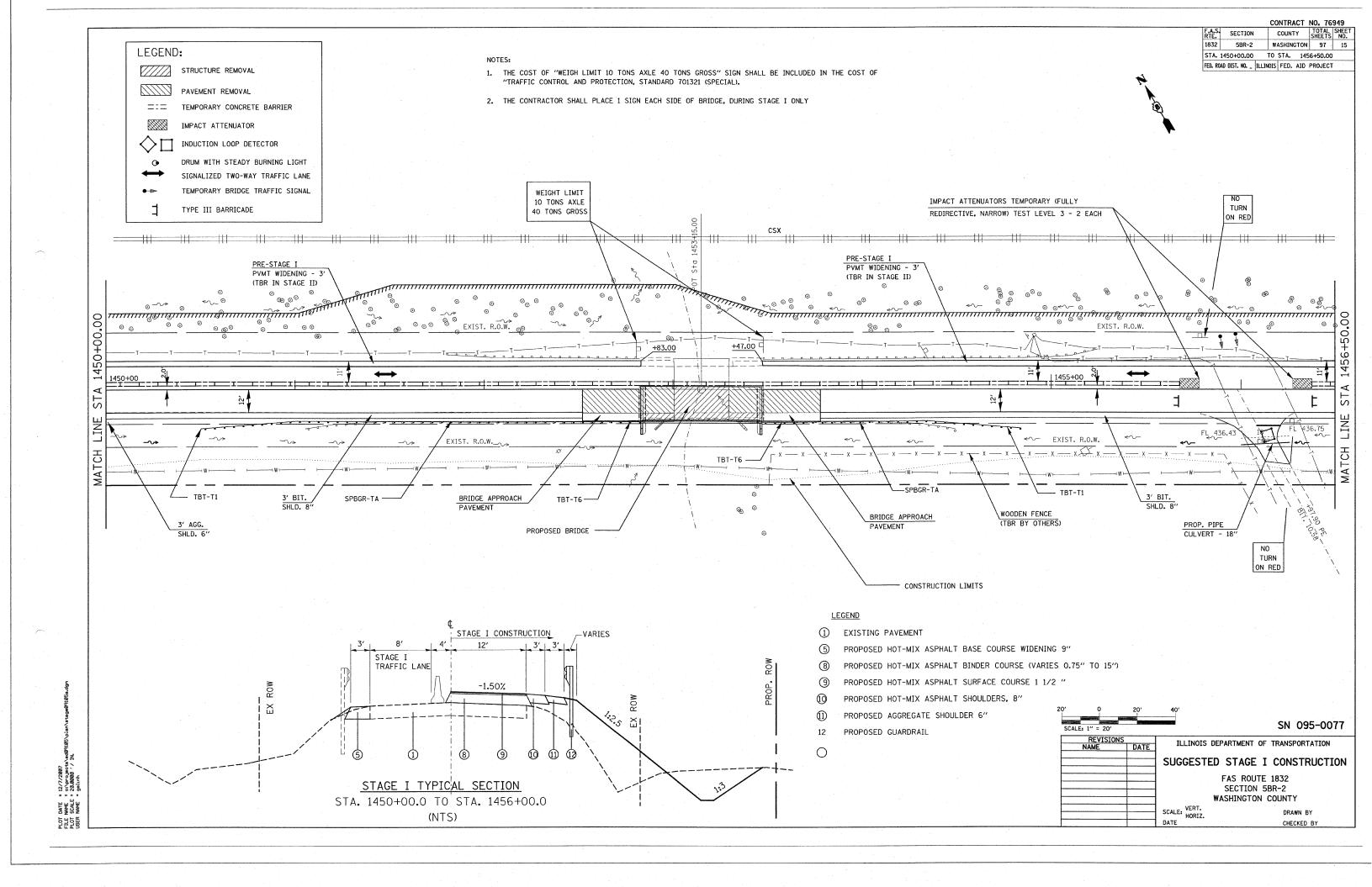
REVISIONS		ILLINOIS DEPARTMENT OF TRANSPORTATION					
NAME DATE		ILLINOIS DEPARTMENT OF TRANSPORTATION					
		SCHEDULE OF QUANTITIES					
		SCHEDULE OF GUARTITIES					
		FAS ROUTE 1832					
	1 1	TAS ROUTE 1032					
		SECTION 5BR-2					
		WASHINGTON COUNTY					

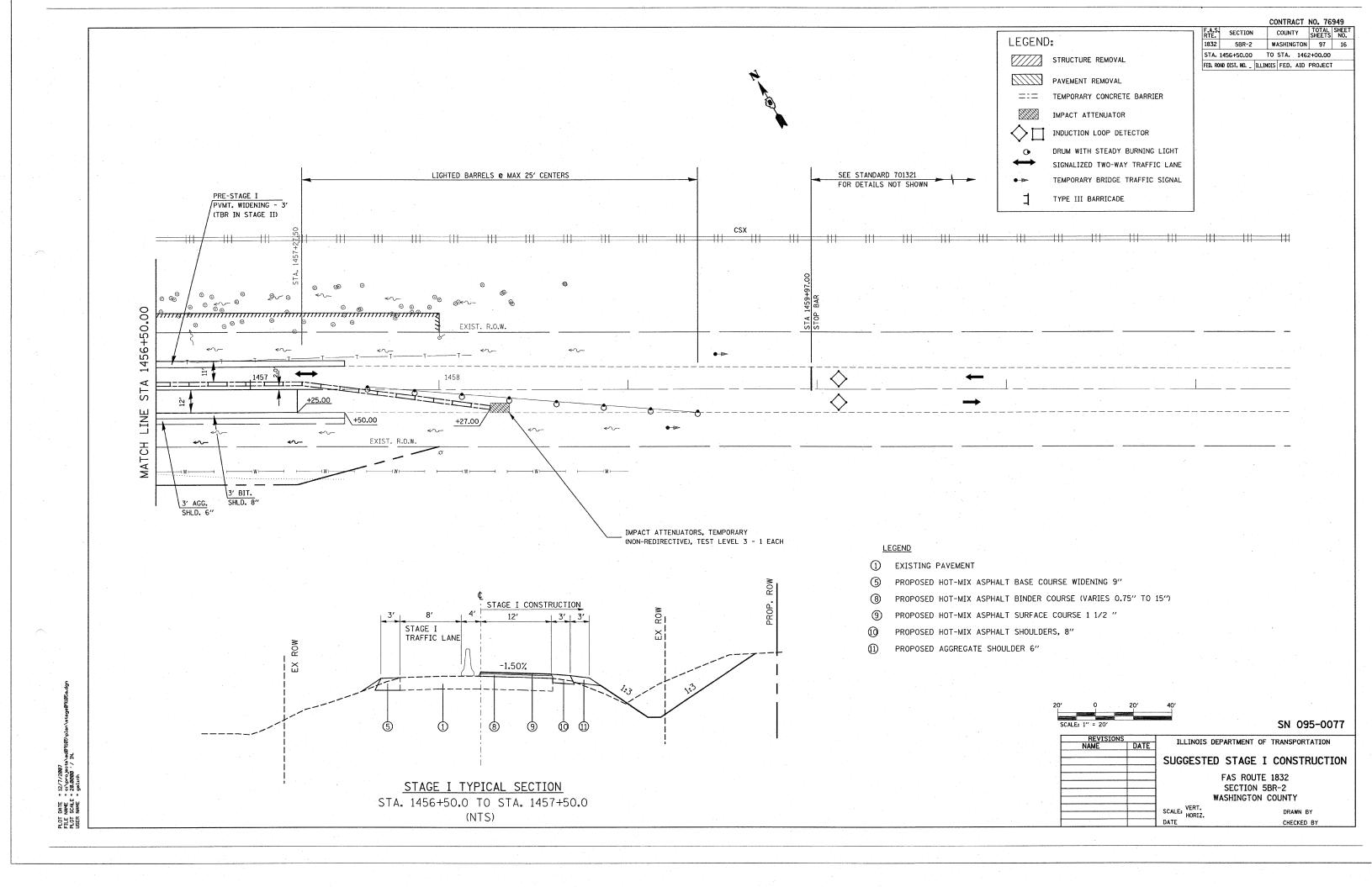
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SAND MODULE IMPACT ATTENUATOR LAYOUT (IF OPTION USED)

#### NOTES:

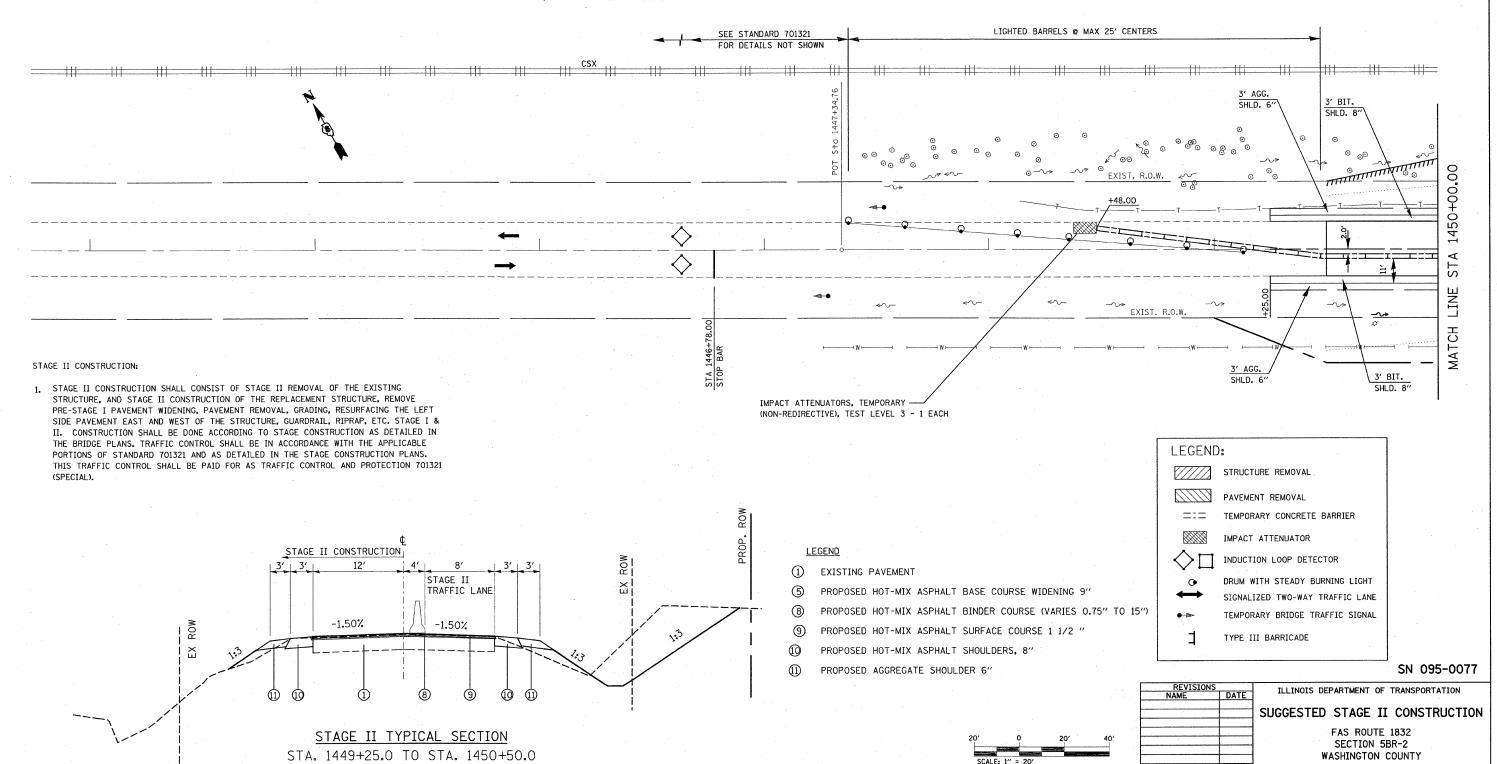
(NTS)

- 1. THE CONTRACTOR SHALL MAINTAIN ACCESS TO PRIVATE AND FIELD ENTRANCES LOCATED
- 2. TRAFFIC CONTROL & PROTECTION, STANDARD 701321 (SPECIAL) INCLUDES BOTH STAGE I & II AND ANY ADDITIONAL SIGNING OR TRAFFIC CONTROL DEVICES SHOWN ON THE STAGE
- 3. ALL ADDITIONAL TRAFFIC SIGNAL HEADS, LOOP DETECTORS AND ASSOCIATED EQUIPMENT REQUIRED TO MAINTAIN ACCESS AT THE FIELD AND DRIVEWAY ENTRANCES SHALL BE INCLUDED IN THE COST OF "TEMPORARY BRIDGE TRAFFIC SIGNALS"
- 4. THE COST OF "BARRICADES, TYPE III" SHALL BE INCLUDED IN THE COST OF "TRAFFIC CONTROL AND PROTECTION, STANDARD 701321 (SPECIAL)".

5. ALL SIDEROADS AND ENTRANCES WITHIN TRAFFIC CONTROL SHALL HAVE "NO TURN ON RED" AND "STOP HERE ON RED" SIGNS. THE COST SHALL BE INCLUDED IN THE COST OF "TRAFFIC CONTROL AND PROTECTION, STANDARD 701321 (SPECIAL)".

COUNTY TOTAL SHEET NO. F.A.S. SECTION 1832 5BR-2 WASHINGTON 97 STA. 1444+00.00 TO STA. 1450+00.00 FED. ROAD DIST. NO. \_ ILLINOIS FED. AID PROJECT

CONTRACT NO. 76949

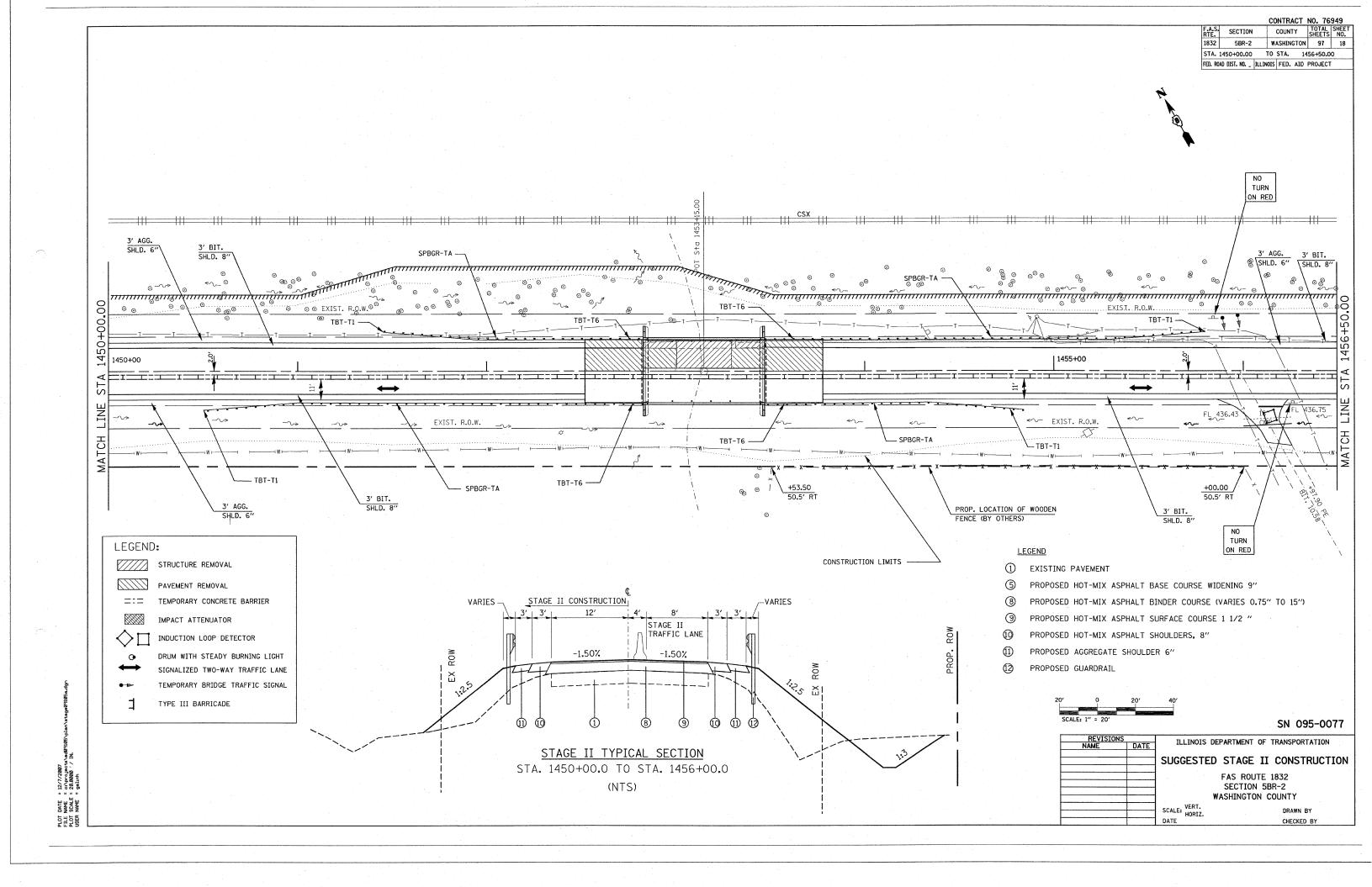


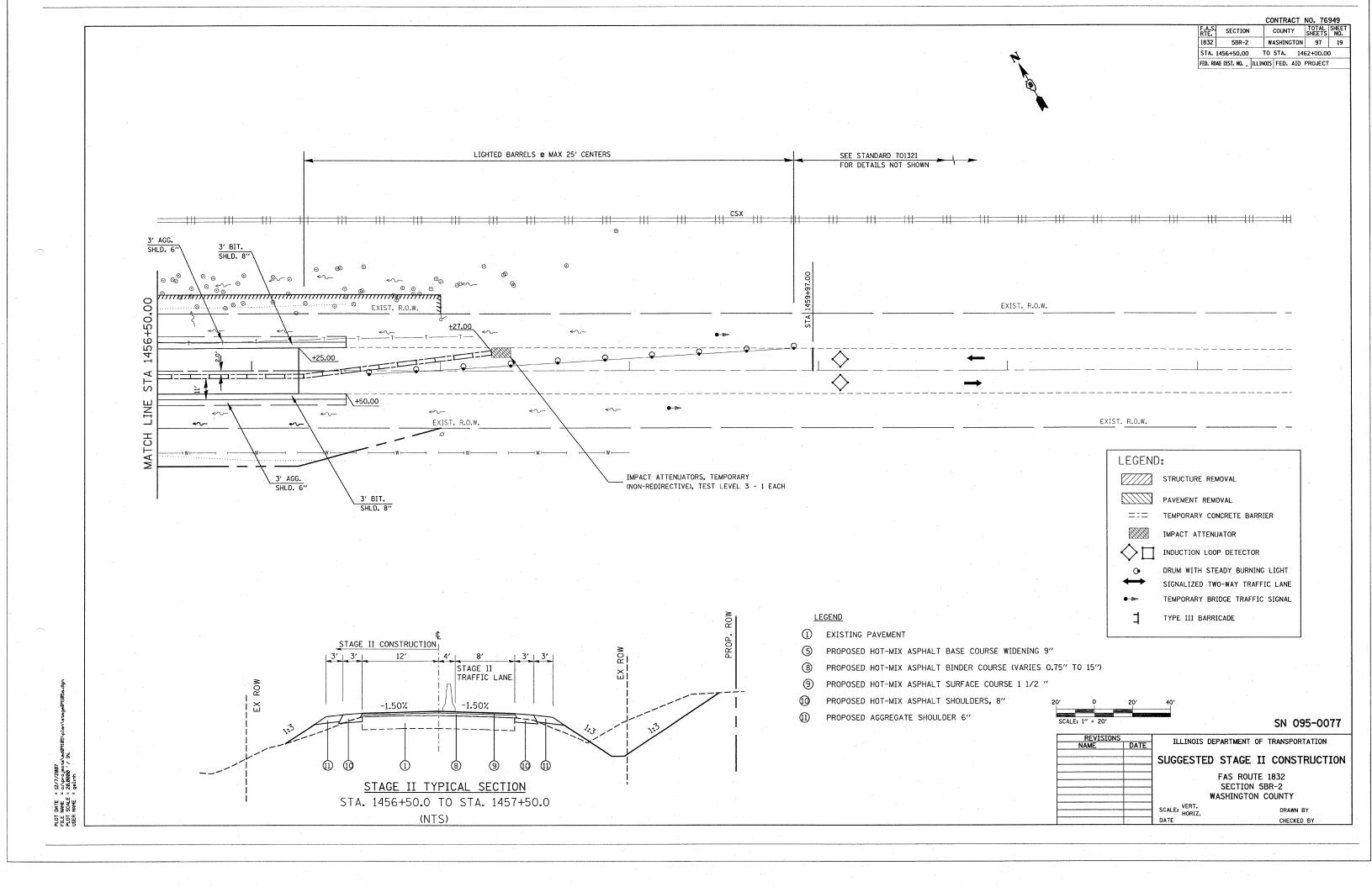
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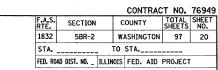
WASHINGTON COUNTY

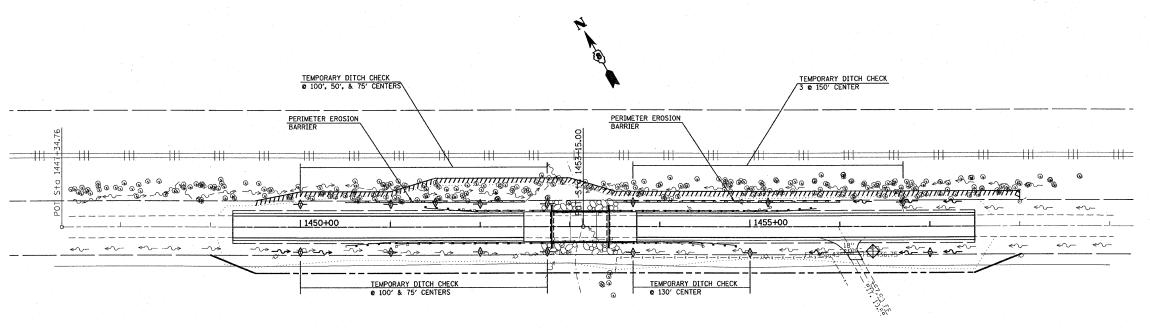
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DATE









#### LEGEND

INLET AND PIPE PROTECTION - STRAW BALES, FILTER FABRIC, AGGREGATES TREE REMOVAL

- → TEMPORARY DITCH CHECK - ROLLED EXCELSIOR, SILT WEDGES/PANELS

PERIMETER EROSION BARRIER- SILT FILTER
FENCE OR OTHER AS APPROVED BY THE ENGINEER

#### TEMPORARY EROSION CONTROL SCHEDULE

LO	CATI	ON	RT/LT	TEMP. EROSION CONTROL SEEDING (POUND)	MULCH METHOD 1 (ACRE)	PERIMETER EROSION BARRIER (FOOT)	TEMP. DITCH CHECK (EACH)	INLET AND PIPE PROTECTION (EACH)
1449+25.00	TO	1457+50.00	LT	31.50	0.35			
1449+25.00	TO	1457+50.00	RT	39.60	0.44			
1450+00.00	TO	1452+75.00	LT				5	
1450+00.00	TO	1452+75.00	RT				4	
1450+50.00	TO	1452+50.00	LT			200		
1453+50.00	TO	1456+00.00	LT			250		
1453+70.00	TO	1456+70.00	LT				3	
1453+70.00	TO	1457+00.00	RT				3	
1455+97.00		PE						1
	7	TOTAL		71.10	0.79	450	15	1



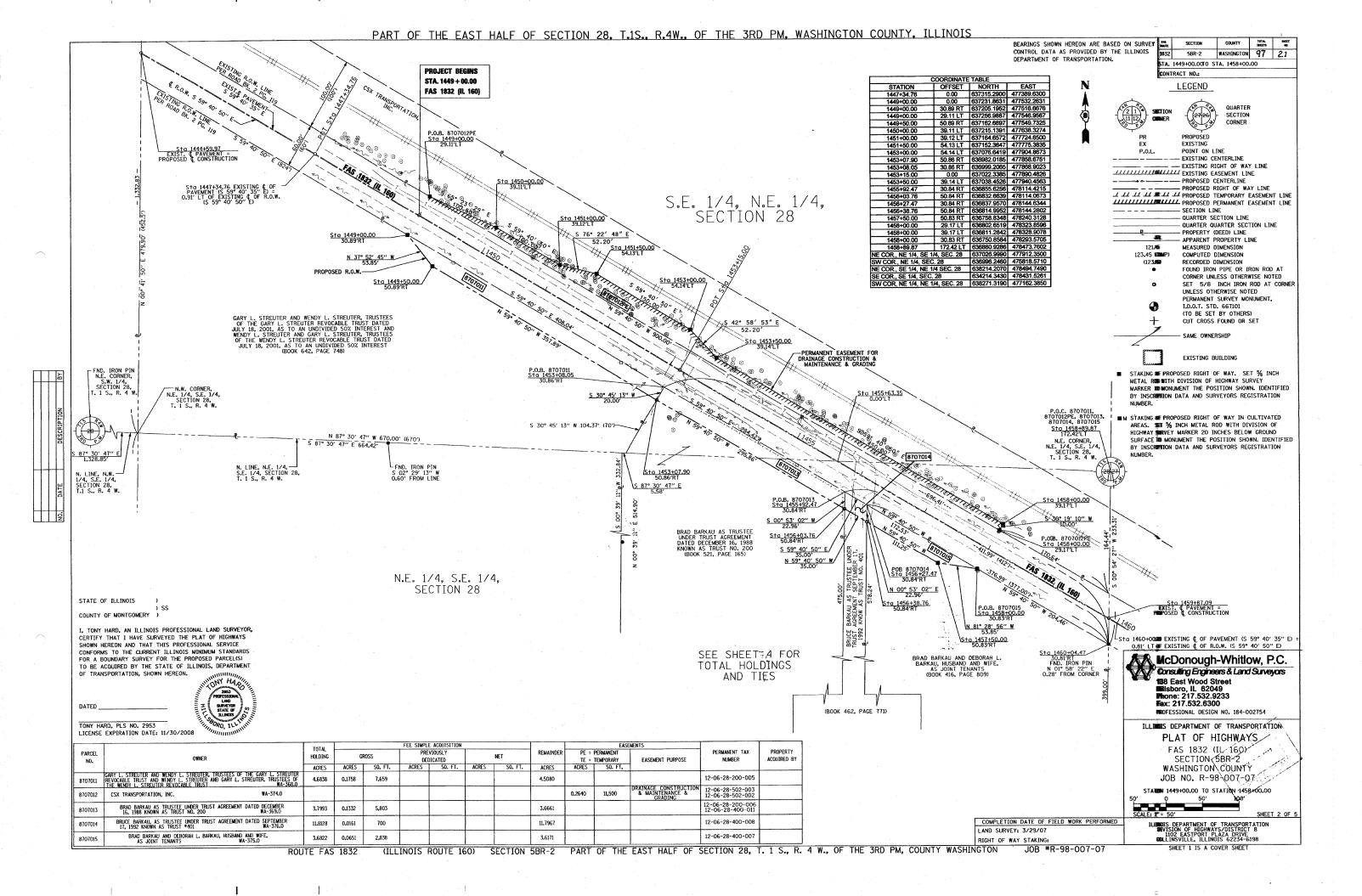
S.N. 095-0077

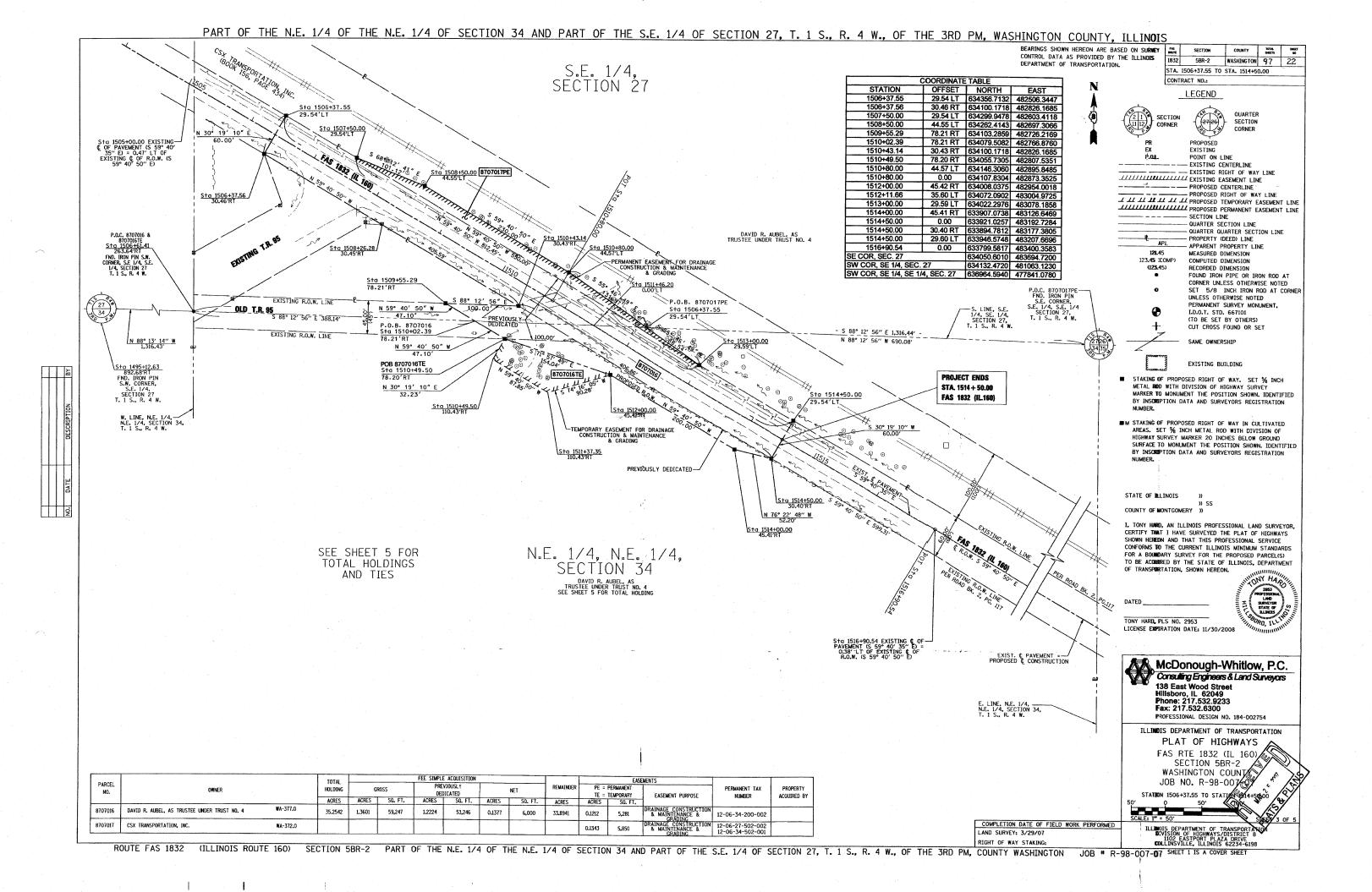
ILLINOIS DEPARTMENT OF TRANSPORTAT	DATE	NAME
EROSION CONTROL PLAN		
FAS ROUTE 1832		

SECTION 5BR-2
WASHINGTON COUNTY SCALE: VERT. HORIZ. DRAWN BY

DATE

CHECKED BY

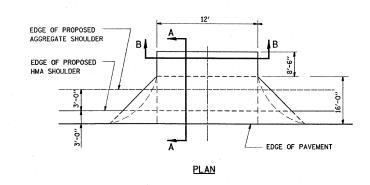


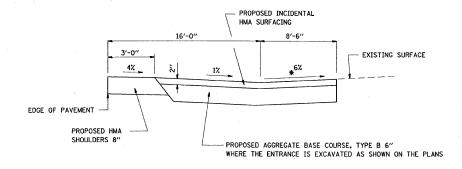


			CUN	1 KA	U	NO.	1034
F.A.S. RTE.	SECTION	С	OUNT	Y	TO SHE	TAL	SHEE NO.
1832	5BR-2	W	ASHIN	GTON	ı	97	23
STA.		_ T0	STA				
FED. ROA	D DIST. NO	ILLINOIS	FED.	AID	PRO	JECT	

#### DETAIL OF HMA ENTRANCES

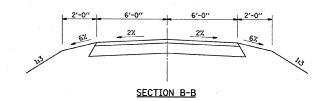
P.E. AT STA. 1455+97.90

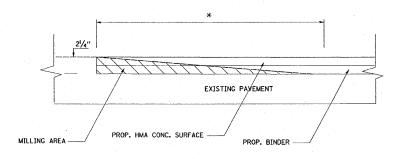




#### SECTION A-A WITH POSITIVE GRADE

\* SEE CROSS SECTIONS FOR SLOPES

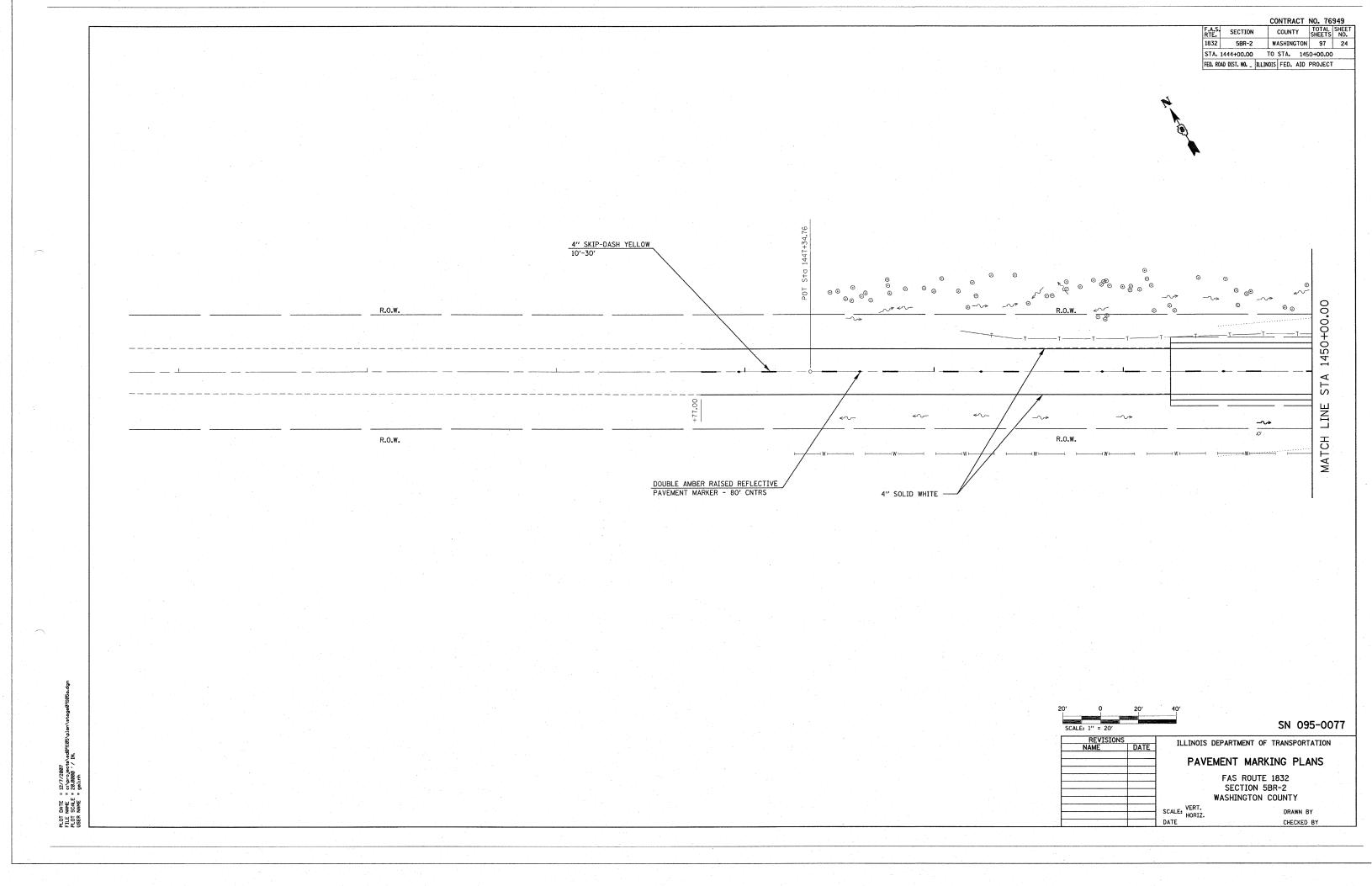


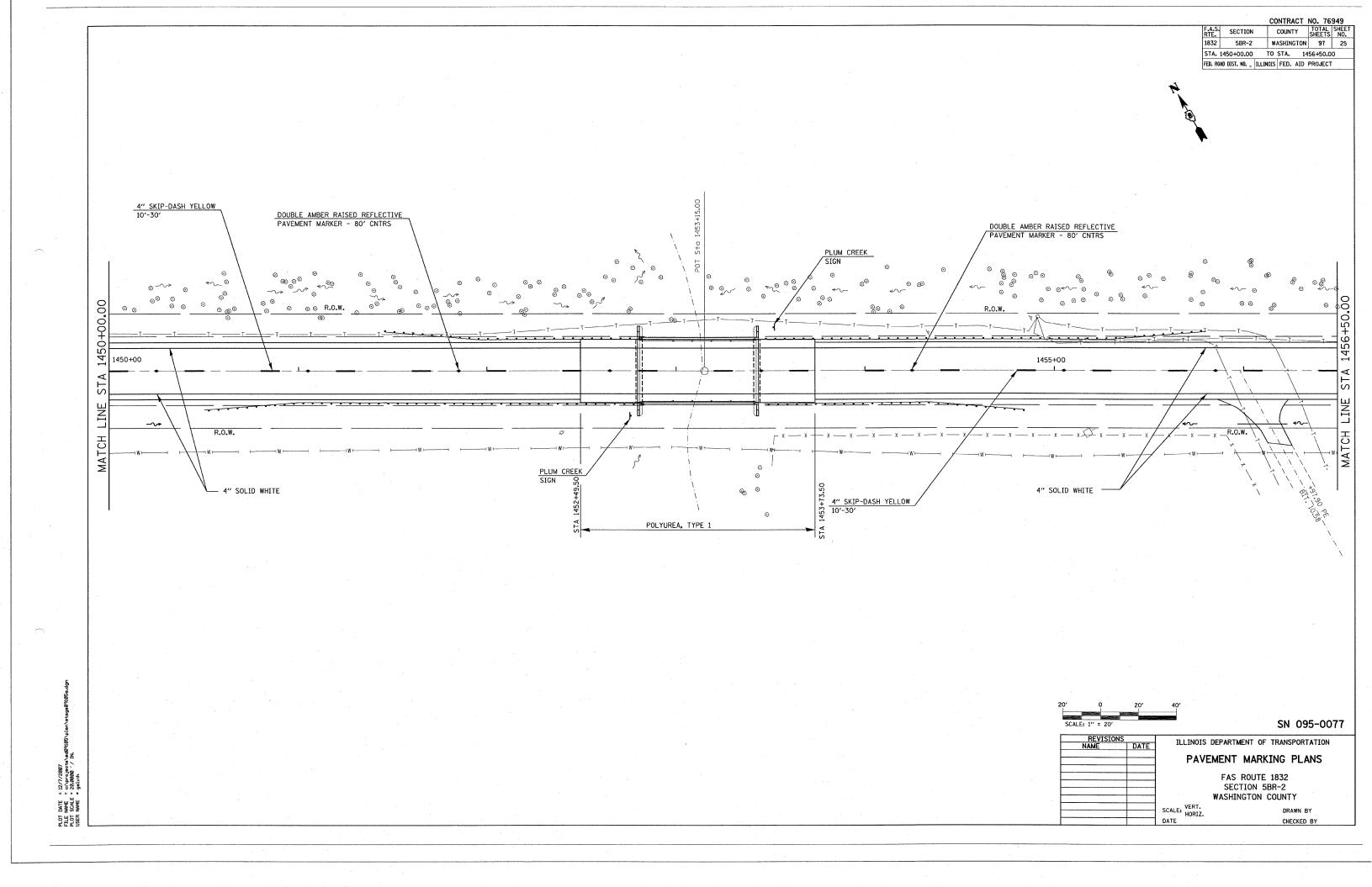


# HMA SURFACE REMOVAL DETAIL \* BEGINNING STA. 1449+25.0 TO STA. 1450+31.88 ENDING STA. 1457+19.35 TO STA. 1457+00.00 S.N. 095-0077

S.N. 095-0077

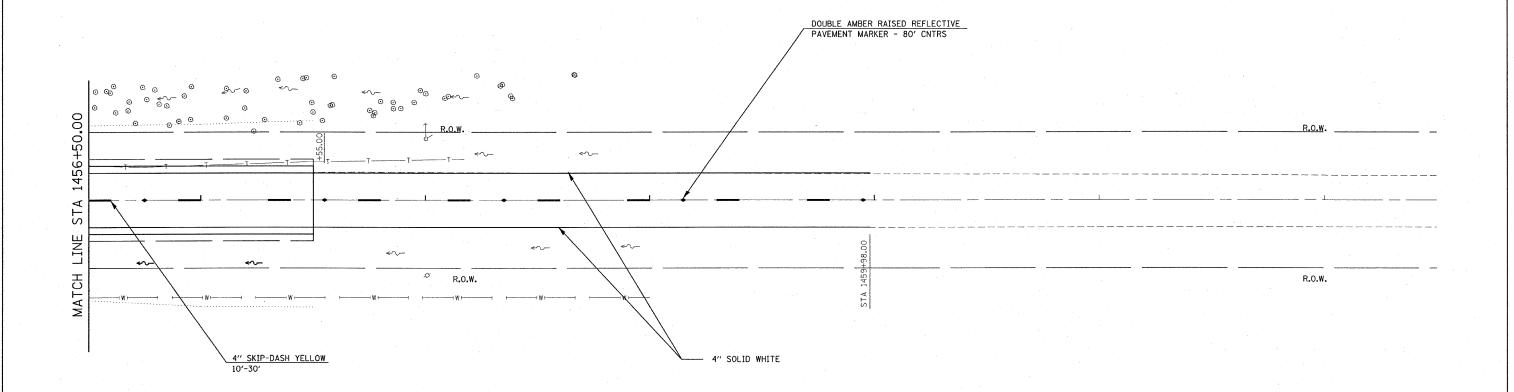
	REVISIONS DATE	ILLINOIS DEPARTMENT OF TRANSPORTATION
		ENTRANCE DETAILS
		FAS ROUTE 1832
		SECTION 5BR-2
		WASHINGTON COUNTY
DRAWING NOT TO SCALE		SCALE: VERT. DRAWN BY
DIAWING NOT TO SCALE		DATE CHECKED BY

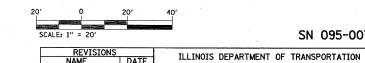




| CONTRACT NO. 76949 | F.A.S. SECTION | COUNTY | TOTAL SHEETS NO. 1832 | SBR-2 | WASHINGTON | 97 | 26 STA. 1456+50.00 TO STA. 1462+00.00 FED. ROAD DIST. NO. \_ ILLINOIS FED. AID PROJECT





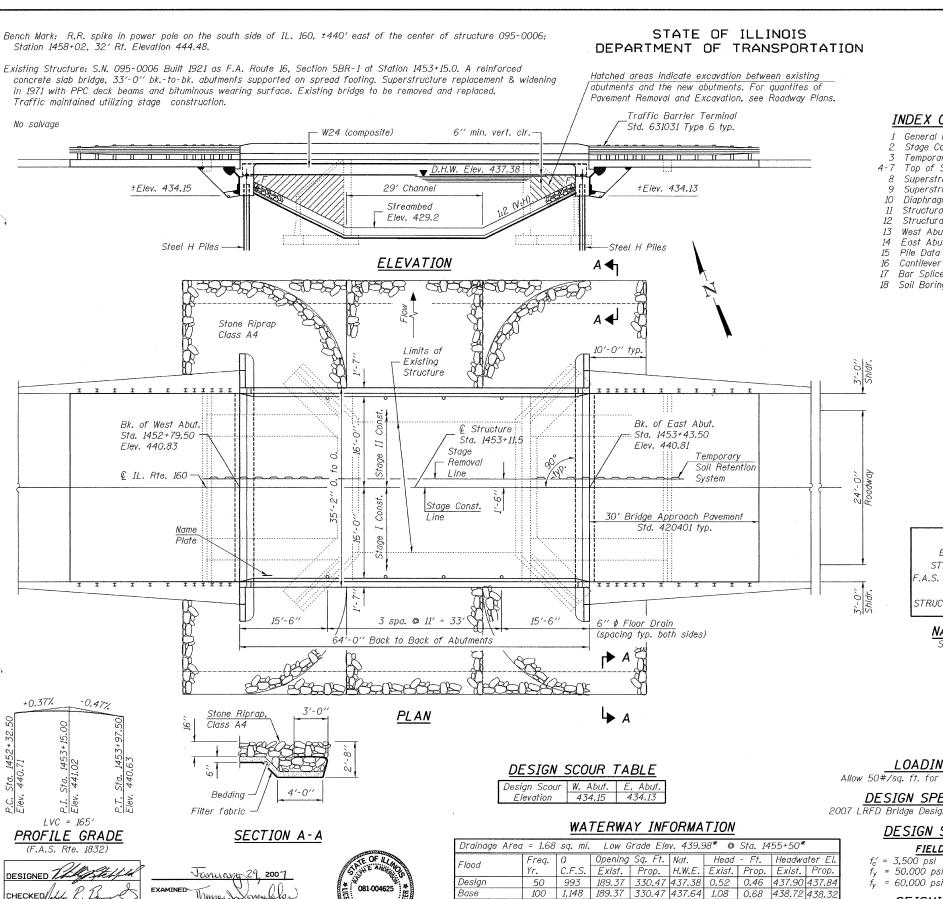


SN 095-0077

NAME	DATE	TELLINOTO DEI ANTMIENT OF TRANSFORTATIO
		PAVEMENT MARKING PLANS
		FAS ROUTE 1832
	:	SECTION 5BR-2 WASHINGTON COUNTY
		SCALE: VERT. DRAWN BY

DATE HORIZ.

DRAWN BY



EXPIRES 11-30-2008

### INDEX OF SHEETS

- General Plan and Elevation
- Stage Construction Details
- Temporary Concrete Barrier Top of Slab Elevations
- Superstructure
- Superstructure Details
- Diaphragm Details Structural Steel
- Structural Steel Details
- West Abutment
- East Abutment
- Cantilever Forming Brackets
- 17 Bar Splicer Details
- 18 Soil Boring Logs

GENERAL NOTES

TOTAL SHEET NO.

27

97

SHEET NO. 1

18 SHEETS

Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts. Bolts  $\frac{3}{4}$  in.  $\phi$ , holes  $\frac{15}{16}$  in.  $\phi$ , unless otherwise noted.

ROUTE NO. SECTION

Contract #76949

58R-2

F.A.P.

COUNTY

WASHINGTON

Calculated weight of Structural Steel = 59280 lb.

No field welding is permitted except as specified in the contract documents. Reinforcement bars shall conform to the requirements of ASTM A 706

Gr 60 (IL Modified). See Special Provisions

Reinforcement bars designated (E) shall be epoxy coated.

The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Gray, Munsell No. 5B 7/1. See Special Provision for "Cleaning and Painting New Metal Structures".

Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.

The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at substructures specified or approved by the Engineer before ordering the remainder of piles.

The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.

Slip forming of the parapets is not allowed.

Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure. The Contractor shall sawcut the upper portion of the existing abutment at the stage removal line before Stage I removal to ensure the remaining portion will not be prematurely damaged.

#### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment (Special)	Cu. Yd.		86	86
Stone Riprap, Class A4	Sq. Yd.			525
Filter Fabric	Sq. Yd.			525
Removal of Existing Structures No. 1	Each			1
Structure Excavation	Cu. Yd.		140	140
Concrete Structures	Cu. Yd.		28.8	28.8
Concrete Superstructure	Cu. Yd.	88.1		88.1
Bridge Deck Grooving	Sq. Yd.	213		213
Protective Coat	Sq. Yd.	281		281
Furnishing and Erecting	1. Sum	1		1
Structural Steel	L. 50///	_		
Stud Shear Connectors	Each	1134		1134
Reinforcement Bars, Epoxy Coated	Pound	18200	3760	21960
Bar Splicers	Each	267	18	285
Driving Piles	Foot		245	245
Test Pile Steel, HP12x53	Each		1	1
Furnishing Steel Piles, HP12x53	Foot		245	245
Name Plates	Each	1		1
Anchor Bolts, 1"	Each		24	24
Geocomposite Wall Drain	Sq. Yd.		51	51
Pipe Underdrains for Structures 4"	Foot		138	138
Floor Drains	Each	8		8
Temporary Soil Retention System	Sq. Ft.			364.3
Concrete Encasement	Cu. Yd.		4.2	4.2

LOADING HL-93

STATION BUILT 20 BY STATE OF ILLINOIS .A.S. RT. 1832 SEC.5BR-2 LOADING HL93 STRUCTURE NO. 095-0077

NAME PLATE

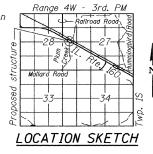
See Std. 515001

Allow 50#/sq. ft. for future wearing surface.

# DESIGN SPECIFICATIONS

FIELD UNITS

Seismic Performance Category (SPC) = 2 Bedrock Acceleration Coefficient (A) = .11a Site Coefficient (S) = 1.5



GENERAL PLAN IL. RTE. 160 OVER PLUM CREEK F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON CO. STATION 1453+11.50 STRUCTURE NO. 095-0077

D-98-091-05

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CHECKED

PRI /NRB

\*Proposed Condition (Existing Low Grade Elevation: 439,30 ft. © Sta. 1453+44,76)
\*\*The Proposed Condition Yielded No Overtopping through the 500-Year Frequency

10 632 168.35 289.84 436.60 0.03 -0.04 436.63 436.56

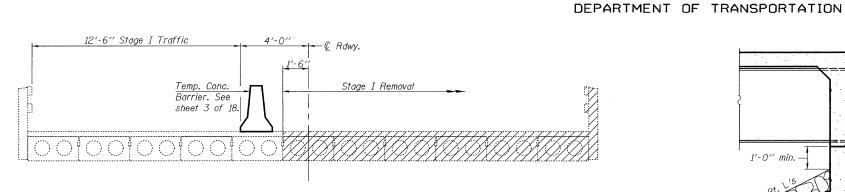
Exist. Overtop. 185\*\* 1,300 189.37 N/A 437.89 1.41 N/A 439.30 Max. Calc. 500 1,523 N/A 330.47 437.96 N/A 1.30

# DESIGN STRESSES

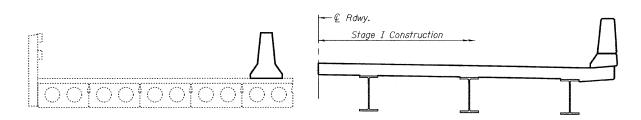
= 3,500 psi

= 50,000 psi (structural steel) = 60,000 psi (reinforcement)

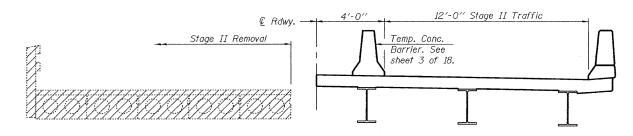
#### <u>SEISMIC DATA</u>



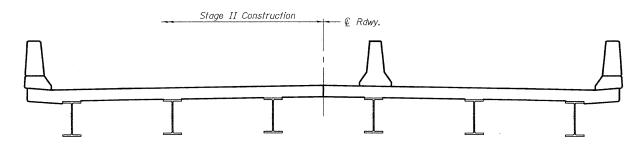
#### STAGE I REMOVAL



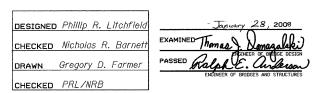
#### STAGE I CONSTRUCTION



#### STAGE II REMOVAL



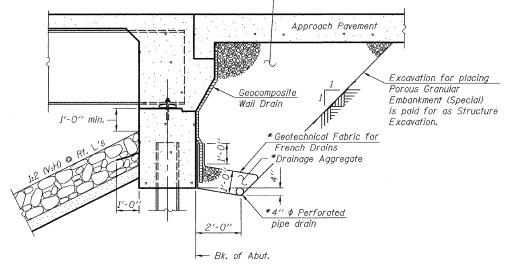
#### STAGE II CONSTRUCTION



Notes: All staging sections are looking East.
For quantity of Temporary Concrete
Barrier, see Roadway Plans.
Hatched areas indicate removal of
existing structure.

Backfill with uncompacted Porous Granular Embankment (Special) by Bridge Contractor after superstructure is in place.

Contract #76949



# SECTION THRU INTEGRAL ABUTMENT (Horiz. dim. © Rt. L's)

\* Included in the cost of Pipe Underdrains for Structures.

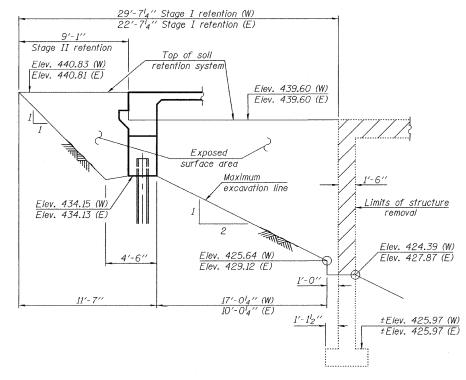
#### Note:

STATE OF ILLINOIS

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

#### Note.

A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.



TEMPORARY SOIL RETENTION
FOR STAGE CONSTRUCTION

STAGE CONSTRUCTION DETAILS

F.A.S. RTE. 1832 - SEC. 5BR-2

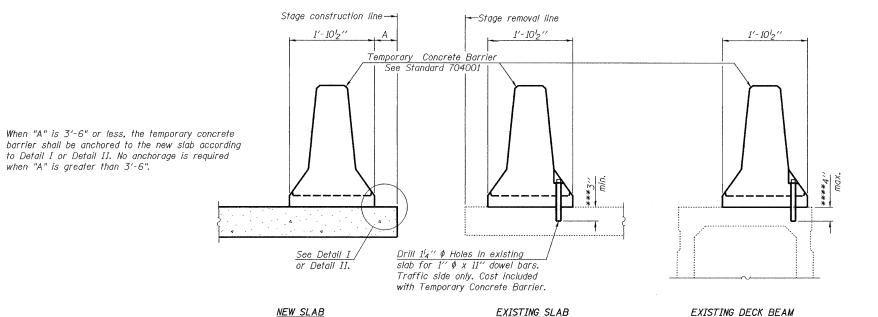
WASHINGTON COUNTY

STATION 1453+11.50

STRUCTURE NO. 095-0077

TOTAL SHEET SHEET NO. 3 ROUTE NO. F.A.S. 97 29 *18* SHEETS 5BR-2 WASHINGTON

Contract #76949



#### NOTES

Detail I - With Bar Splicer or Couplers: Connect one (1) 1"x7"x10" steel £ to the top layer of couplers with 2-5<sub>8</sub>" \$\phi\$ bolts screwed to coupler at approximate € of each barrier panel.

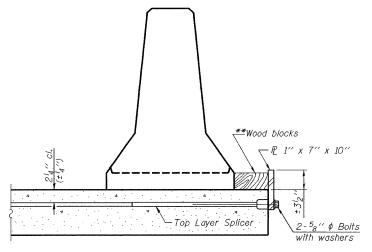
Detail II - With Extended Reinforcement Bars: Connect one (1) 1"x7"x10" steel P to the concrete slab or concrete wearing surface with 2-58"\$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate € of each barrier panel.

Cost of anchorage is included with Temporary Concrete Barrier. The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

#### SECTIONS THRU SLAB OR DECK BEAM

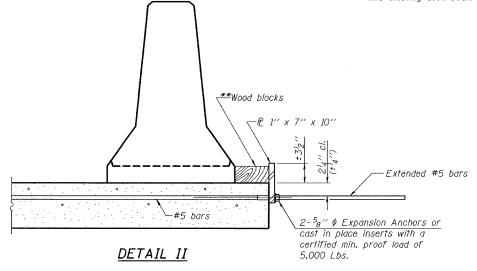
\*\*\*Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

\*\*\*\*If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.

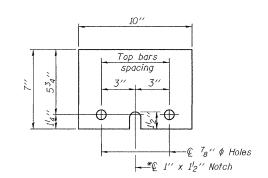


DETAIL I

when "A" is greater than 3'-6".



<sup>\*\*</sup>Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



#### STEEL RETAINER P 1" x 7" x 10"

\* Required only with Detail II

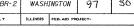
TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1453+11.50 STRUCTURE NO. 095-0077

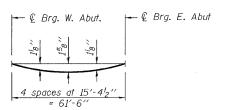
DESIGNED Phillip R. Litchfiel CHECKED Nicholas R. Barnet DRAWN Gregory D. Farmer

CHECKED PRL/NRB R-27

9-3-07



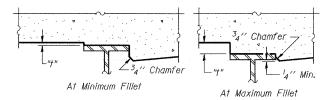




#### DEAD LOAD DEFLECTION DIAGRAM

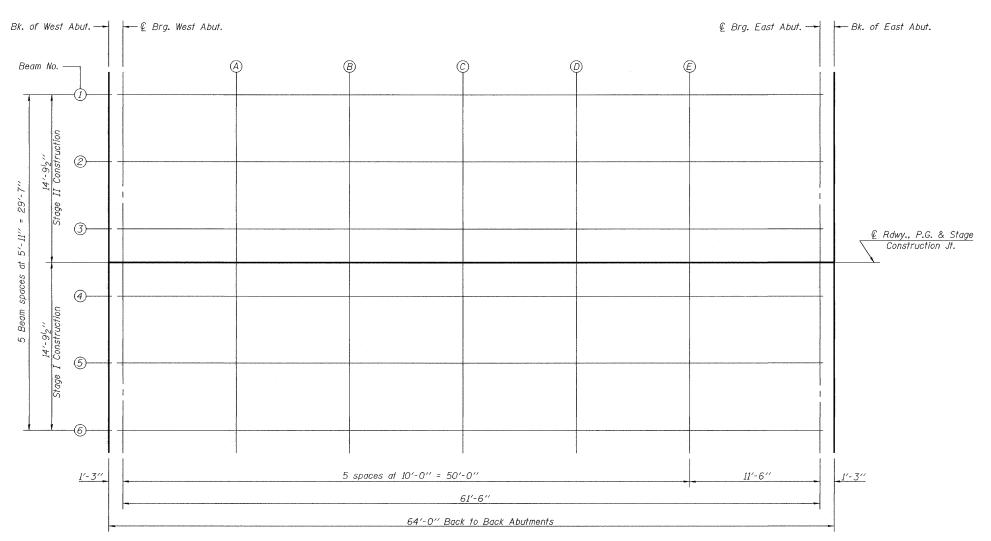
(Includes weight of concrete only.)

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown on page 5 of 18,



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below, minus slab thickness, equals the fillet heights "t" above top flange of

FILLET HEIGHTS



DESIGNED Phillip R. Litchfield CHECKED Nicholas R. Barnett

DRAWN Gregory D. Farmer CHECKED PRL/NRB

PLAN

TOP OF SLAB ELEVATIONS F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1453+11.50 STRUCTURE NO. 095-0077

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ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SH
F.A.S. 1832	5BR-2	WASHINGTON		97	31	18
FED. ROAD DIST	. NO. 7	ILLINOIS	FED. AID PRI	DJECT-		

SHEET NO. 5

Contract #76949

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
Bk. W. Abut.	145279.50	-14.79	440.59	440.59	
₡ Brg. W. Abut.	145280.75	-14.79	440.59	440.59	
A B C D E	145290.75 145300.75 145310.75 145320.75 145330.75	-14.79 -14.79 -14.79 -14.79 -14.79	440.60 440.60 440.60 440.60 440.59	440.66 440.71 440.73 440.71 440.66	
₡ Brg. E. Abut.	145342.25	-14.79	440.57	440.57	
Bk. E. Abut.	145343.50	-14.79	440.57	440.57	

#### <u>BEAM 2</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	145279.50	-8.87	440.69	440.69
© Brg. W. Abut.  A B C D E	145280.75	-8.87	440.70	440.70
	145290.75	-8.87	440.71	440.77
	145300.75	-8.87	440.71	440.82
	145310.75	-8.87	440.71	440.84
	145320.75	-8.87	440.70	440.81
	145330.75	-8.87	440.69	440.76
© Brg. E. Abut.	145342.25	-8.87	440.68	440.68
Bk. E. Abut.	145343.50	-8.87	440.67	440.67

#### BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	145279.50	-2.96	440.79	440.79
€ Brg. W. Abut.	145280.75	-2.96	440.79	440.79
A 8 C : D	145290.75 145300.75 145310.75 145320.75 145330.75	-2.96 -2.96 -2.96 -2.96 -2.96	440.80 440.80 440.80 440.80 440.79	440.86 440.91 440.93 440.91 440.86
⊈ Brg. E. Abut.	145342.25	-2.96	440.77	440.77
Bk. E. Abut.	145343.50	-2.96	440.77	440.77

#### 

Location :	Station	Offset -	Theoretical Grade - Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	145279.50	0.00	440.83	440.83
€ Brg. W. Abut.	145280.75	0.00	440.83	440.83
A B C D E	145290.75 145300.75 145310.75 145320.75 145330.75	0.00 0.00 0.00 0.00	440.84 440.85 440.85 440.84 440.83	440.91 440.95 440.98 440.95 440.90
€ Brg. E. Abut.	145342.25	0.00	440.81	440.81
Bk. E. Abut.	145343.50	0.00	440.81	440.81

## <u>BEAM 4</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	145279.50	2.96	440.79	440.79
& Brg. W. Abut.	145280.75	2.96	440.79	440.79
A B C D E	145290.75 145300.75 145310.75 145320.75 145330.75	2.96 2.96 2.96 2.96 2.96	440.80 440.80 440.80 440.80 440.79	440.86 440.91 440.93 440.91 440.86
€ Brg. E. Abut.	145342.25	2.96	440.77	440.77
Bk. E. Abut.	145343.50	2.96	440.77	440.77

#### BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	145279.50	8.88	440.69	440.69
© Brg. W. Abut.	145280.75	8.88	440.70	440.70
A B C D E	145290.75 145300.75 145310.75 145320.75 145330.75	8.88 8.88 8.88 8.88	440.71 440.71 440.71 440.70 440.69	440.77 440.82 440.84 440.81 440.76
⊈ Brg. E. Abut.	145342.25	8.88	440.68	440.68
Bk. E. Abut.	145343.50	8.88	440.67	440.67

## BEAM 6

The second secon	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
	Bk. W. Abut.	145279.50	14.79	440.59	440.59
	₽ Brg. W. Abut.	145280.75	14.79	440.59	440.59
	A B C D E	145290.75 145300.75 145310.75 145320.75 145330.75	14.79 14.79 14.79 14.79 14.79	440.60 440.60 440.60 440.60 440.59	440.66 440.71 440.73 440.71 440.66
	Brg. E. Abut.	145342.25	14.79	440.57	440.57
-	Bk. E. Abut.	145343.50	14.79	440.57	440.57

DESIGNED Phillip R. Litchfield

CHECKED Nicholas R. Barnett

DRAWN Gregory D. Farmer
CHECKED PRL/NRB

EXAMINED Thomas Lomas laki
PASSED Rolph E. Children
ENGINEER OF BRIDGES AND STRUCTURES

TOP OF SLAB ELEVATIONS

F.A.S. RTE. 1832 - SEC. 5BR-2

WASHINGTON CO.

STATION 1453+11.50

STRUCTURE NO. 095-0077

ROUTE NO.	SECTION	coi	UNITY	TOTAL SHEETS	SHEET NO.	SHE	ET	NO.	6
F.A.S. 1832	5BR-2	WASHINGTON		97	32	18	SHE	ETS	
FED. ROAD DIST	. NO. 7	ILLINOIS	FED, AID PR	OJECT-					

Offset

0.00

0.00

0.00

Theoretical

Grade Elevations

440.77

440.80

440.82

440.83

Contract #76949

@ ROADWAY, P.G. & STAGE CONST. JOINT

Location

End of W. Appr. Pavement

Back of W. Abutment

Station

145249.50

145259.50

145269.50

145279.50

#### NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
End of W. Appr. Pavement	145249.50	-16.00	440.50
A B	145259.50 145269.50	-16.00 -16.00	440.53 440.55
Back of W. Abutment	145279.50	-16.00	440.56

#### NORTH EDGE OF PAVEMENT

MONTH LDC	L OI INVENIL		
Location	Station	Offset	Theoretical Grade Elevations
End of W. Appr. Pavement	145249.50	-12.00	440.58
A B	145259.50 145269.50	-12.00 -12.00	440.61 440.63
Back of W. Abutment	145279.50	-12.00	440.64

SOUTH EDGE OF PAVEMENT					
Location	Station	Offset	Theoretical Grade Elevations		
End of W. Appr. Pavement	145249.50	12.00	440.58		
A B	145259.50 145269.50	12.00 12.00	440.61 440.63		
Back of W. Abutment	145279.50	12.00	440.64		

# SOUTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
End of W. Appr. Pavement	145249.50	16.00	440.50
A B	145259.50 145269.50	16.00 16.00	440.53 440.55
Back of W. Abutment	145279.50	16.00	440.56

End of West Back of West (A)  $\bigcirc$ approach pavement Abutment North curb North edge of pavement © Roadway, P.G., & Stage Const. Joint South edge of pavement South curb line 3 Spaces at 10'-0" = 30'-0"

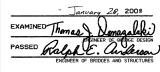
PLAN

DESIGNED Phillip R. Litchfield

CHECKED Nicholas R. Barnett DRAWN Gregory D. Farmer

CHECKED PRL/NRB

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TOP OF WEST APPROACH SLAB ELEVATIONS F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1453+11.50 STRUCTURE NO. 095-0077

ROUTE NO.	SECTION	cou	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 7
F.A.S. 1832	5BR-2	WASH.	INGTON	97	33	18 ѕнеетѕ
FEO. ROAD DIST	. NO. 7	ILLINOIS FED. AID PRO		FED. AID PROJECT-		

Contract #76949

#### NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
Back of E. Abutment	145343.50	-16.00	440.54
A B	145353.50 145363.50	-16.00 -16.00	440.52 440.49
End of E. Appr. Pavement	145373.50	-16.00	440.46

#### NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
Back of E. Abutment	145343.50	-12.00	440.62
A B	145353.50 145363.50	-12.00 -12.00	440.60 440.58
End of E. Appr. Pavement	145373.50	-12.00	440.54

# @ ROADWAY, P.G., & STAGE CONST. JOINT

Location	Station	Offset	Theoretical Grade Elevations
Back of E. Abutment	145343.50	0.00	440.81
A B	145353.50 145363.50	0.00 0.00	440.79 440.76
End of E. Appr. Pavement	145373.50	0.00	440.73

#### SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
Back of E. Abutment	145343.50	12.00	440.62
A B	145353.50 145363.50	12.00 12.00	440.60 440.58
End of E. Appr. Pavement	145373.50	12.00	440.54

# Back of East Abutment Abu

PLAN

#### SOUTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
Back of E. Abutment	145343.50	16.00	440.54
A B	145353.50 145363.50	16.00 16.00	440.52 440.49
End of E. Appr. Pavement	145373.50	16.00	440.46

DESIGNED Phillip R. Litchfield

CHECKED Nicholas R. Barnett

DRAWN Gregory D. Farmer

CHECKED PRL/NRB

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TOP OF EAST APPROACH

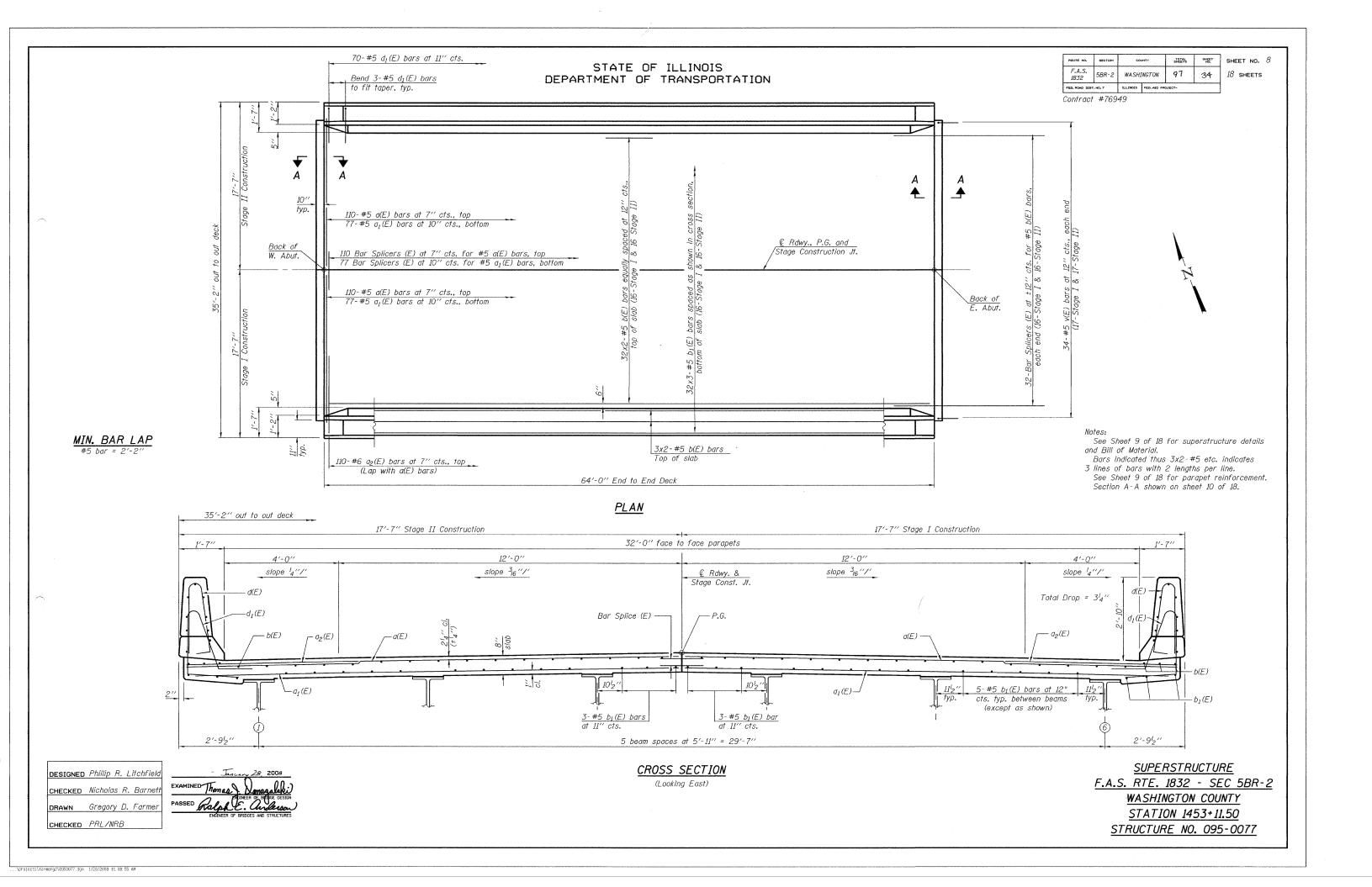
SLAB ELEVATIONS

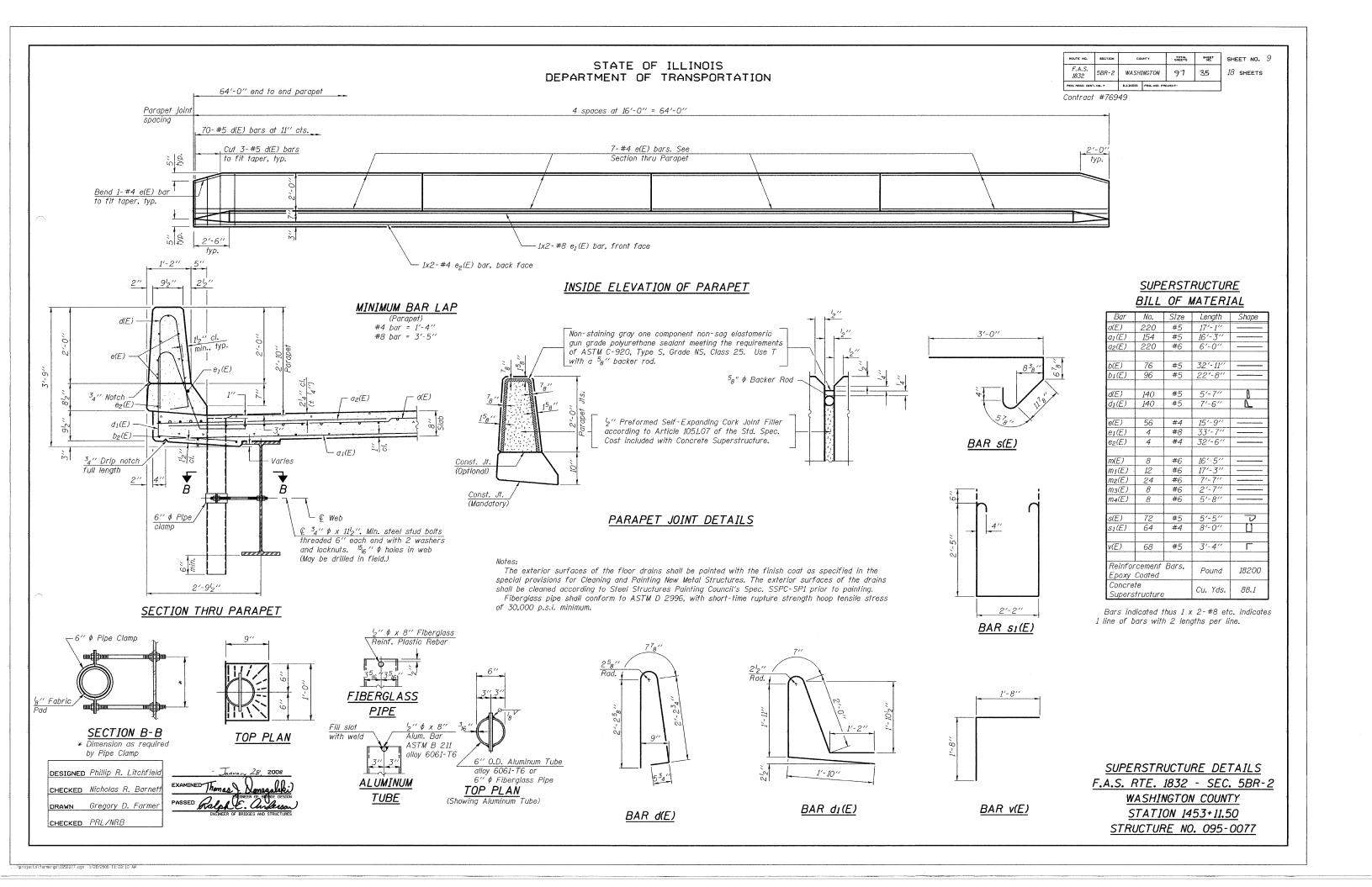
F.A.S. RTE. 1832 - SEC. 5BR-2

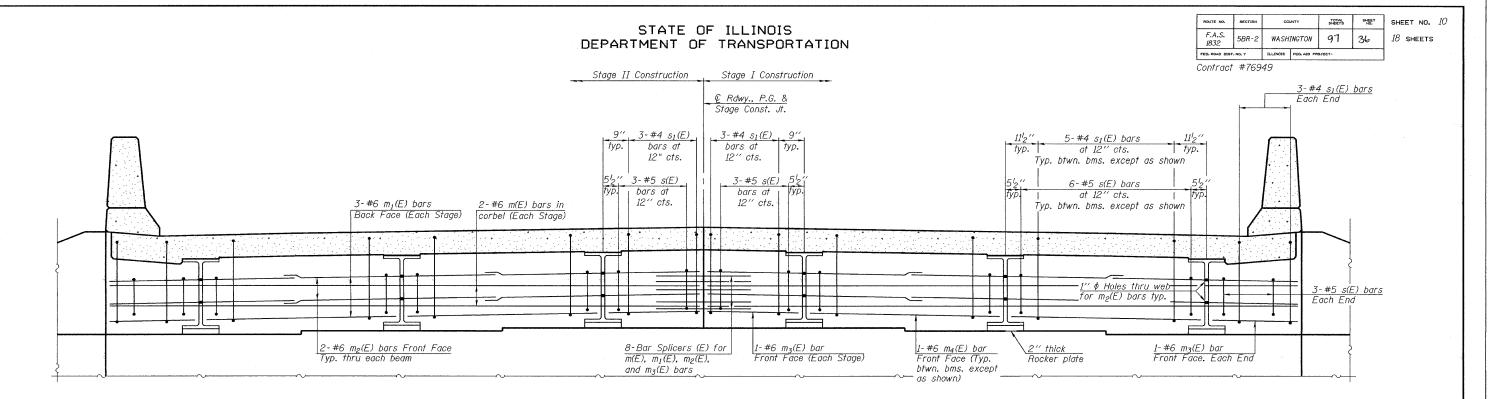
WASHINGTON COUNTY

STATION 1453+11.50

STRUCTURE NO. 095-0077







MIN. BAR LAP

DIAPHRAGM ELEVATION AT ABUTMENT

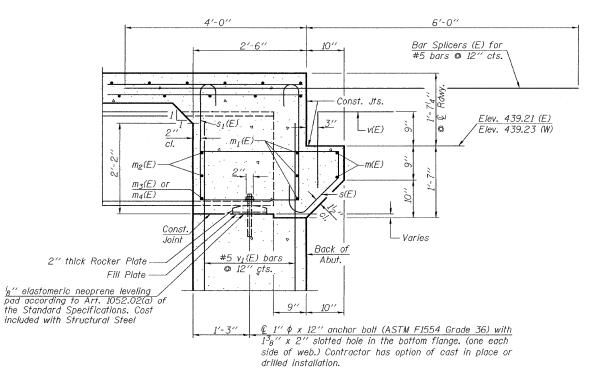
(East diaphragm shown, looking East. West diaphragm similar)

Notes: Reinforcement bars in diaphragm are billed with Superstructure on sheet 9 of 18.

Concrete in diaphragm is included with Concrete Superstructure on sheet 9 of 18.

For detail of s(E) and s<sub>1</sub>(E) bars see sheet 9 of 18.

The s(E) and s<sub>1</sub>(E) bars shall be placed parallel to the beam.



DESIGNED Phillip R. Litchfield

CHECKED Nicholas R. Barnett

DRAWN Gregory D. Farmer

CHECKED PRL/NRB

EXAMINED Thomas Summarship

PASSED Ralph E. Children

ENGINEER OF BRIDGES AND STRUCTURES

SECTION A-A

DIAPHRAGM DETAILS

F.A.S. RTE. 1832 - SEC. 5BR-2

WASHINGTON COUNTY

STATION 1453+11.50

STRUCTURE NO. 095-0077

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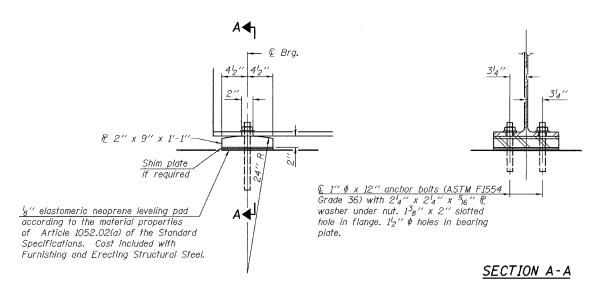
#### SHEET NO. SHEET NO. 11 STATE OF ILLINOIS F.A.S. 1832 97 37 18 SHEETS 5BR-2 WASHINGTON DEPARTMENT OF TRANSPORTATION Contract #76949 Beam No. — ├─ £ Brg. W. Abut. Brg. E. Abut. → 60 spaces at 12" = 60'-0" One space at 9" Stud shear connector spacing D $r \rightarrow A$ € C12x25 14'-9'<sub>2''</sub> II Consti D $_{\sim}B$ $\frac{3}{4}$ " $\phi$ H.S. bolts $\frac{15}{16}$ " $\phi$ holes at 5'-11" \<u>©</u> Beam or girder web and © C12x25 © Rdwy, P.G. & Stage Const. Jt. at end of channel SECTION B-B and have spaces - L 6" x 4" x ½" D DIAPHRAGM D 2 (18 Required) D Note: Two hardened washers required for each set of oversized holes. \*\* 34" \$\phi\$ HS bolts, \(^{15}\_{16}\)'' \$\phi\$ holes 19'-0" 19'-0" 19'-0" 61'-6" FRAMING PLAN (All beams are W24x146, NTR, and AASHTO M270 Gr. 50) Beam 4 → ├**- £** Beam 3 $\frac{Q}{Typ}$ both ends. See Note. 2" typ. Diaphragm D <sup>3</sup>₄'' ∮ Granular or solid flux L 6"x4"x12 filled headed studs, automatically typ. end welded to flange. (1134 Required) © W24x146 & © C12x25 34" \$ H.S. bolts 4 sides <sup>15</sup>16 '' Ø holes Fillet 14" typ. Varies C12x25 $\underbrace{\ell}_{for} \ m_2(E) \ bars. \ See$ Diaphragm D1 ← & Brg. Abut. SECTION A-A sheet 10 of 17. W 24x146 1'-0" 6' DIAPHRAGM D1 (2 Required) Note: Install only the center bolt at each end of Diaphragm D1. The bolts W24X146 shall be finger tightened prior to deck pour to permit rotation of 1<sup>3</sup>8'' x 2'' slotted holes Diaphragm D1. Install the remaining bolts and fully tighten after STRUCTURAL STEEL stage two deck pour is complete. F.A.S. RTE. 1832 - SEC. 5BR-2 Load carrying components designated "NTR" shall conform to the DESIGNED Phillip R. Litchfield Supplemental Requirements for Notch Toughness, Zone 2. WASHINGTON COUNTY CHECKED Nicholas R. Barnet STATION 1453+11.50 DRAWN Gregory D. Farme TYP. END OF BEAM ELEVATION STRUCTURE NO. 095-0077 CHECKED PRL/NRB

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#### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	co	WTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 12
F.A.S. 1832	5BR-2	WASH.	INGTON	97	38	<i>18</i> sheets
FED. ROAD DIST	. NO. 7	ILLINOIS	FED. ALD PRI	DJECT-		

Contract #76949



#### ELEVATION AT ABUTMENT

#### FIXED BEARING

lotes:

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (Fy=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications. Two  $^{l}_{8}$  in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and

placed as shown on bearing details,

INTERIOR BEAM	MOMEN	T TABLE
		0.5 Sp.
$I_{\mathcal{S}}$	(in <sup>4</sup> )	4580
$I_c(n)$	(in <sup>4</sup> )	12183
$I_o(3n)$	(in4)	8715
Ss	(in <sup>3</sup> )	371
So(n)	(in <sup>3</sup> )	544
Sc(3n)	(in³)	487
DC1	(k/')	0.773
M DC1	('k)	<i>365.</i> 5
DC2	(k/')	0.150
M DC2	('k)	70.9
DW	(k/')	0.296
Mow	('k)	139.8
M4 + Imp	('k)	708.3
Mu(Strength I)	('k)	1994.7
$\phi_f M_D$	('k)	2567.3
fs DC1	(ksi)	11.82
f <sub>s</sub> DC2	(ksi)	1.75
fs DW	(ksi)	3.44
fs 1.3(4+I)	(ksi)	20.31
fs (Service II)	(ksi)	37.32
fs (Total)(Strength I)	(ksi)	-
$V_f$	(k)	21.7

INTERIO		R REACTI Loading	ON TABLE
	11123	Loading	Abut.
0	(k)		23.8
Roci	(k)		2000
R <sub>DC2</sub>	7117		4.6
Row	(k)		9.1
R4 + Imp	(k)		67.0
RTotal	(k)		104.5

Is, Ss:	Non-composite moment of inertia and section modulus of the steel section used for computing $f_{\mathfrak{s}}$ (Total-Strength I, and
	Service II) due to non-composite dead loads (in.4 and in.3).
$I_o(n)$ , $S_o(n)$ :	Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing

and deck based upon the modular ratio, "n", used for computing  $f_s$  (Total-Strength I, and Service II) due to short-term composite live loads (in.4 and in.3).

 $I_c(3n)$ ,  $S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in.4 and in.3).

DC1: Un-factored non-composite dead load (kips/ft.).
MDC1: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

Mow: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

MŁ + Imp: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

Mu (Strength I): Factored design moment (kip-ft.).

 $1.25 \; (M_{DC1} + M_{DC2}) + 1.5 \; M_{DW} + 1.75 \; M_{\odot} + 1.75$ 

Φ<sub>f</sub>M<sub>n</sub>: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

fs (Service II): Sum of stresses as computed from the moments below (ksi).

MDC1 + MDC2 + MDW + 1.3 M + Imp

fs (Total)(Strength I): Sum of stresses as computed from the moments below on

non-compact section (ksi).

1.25 (MDC1 + MDC2) + 1.5 MDW + 1.75 M4 + Imp

V<sub>f</sub>: Factored shear range in span computed according to Article 6.10.10.

#### \*TOP OF BEAM ELEVATIONS

Ì	Location	Beam 1	Beam 2	Beam 3	Beam 4	Beam 5	Beam 6
	West Abut.	439.88	439.99	440.08	440.08	439.99	439.88
	East Abut.	439.86	439.97	440.06	440.06	439.97	439.86

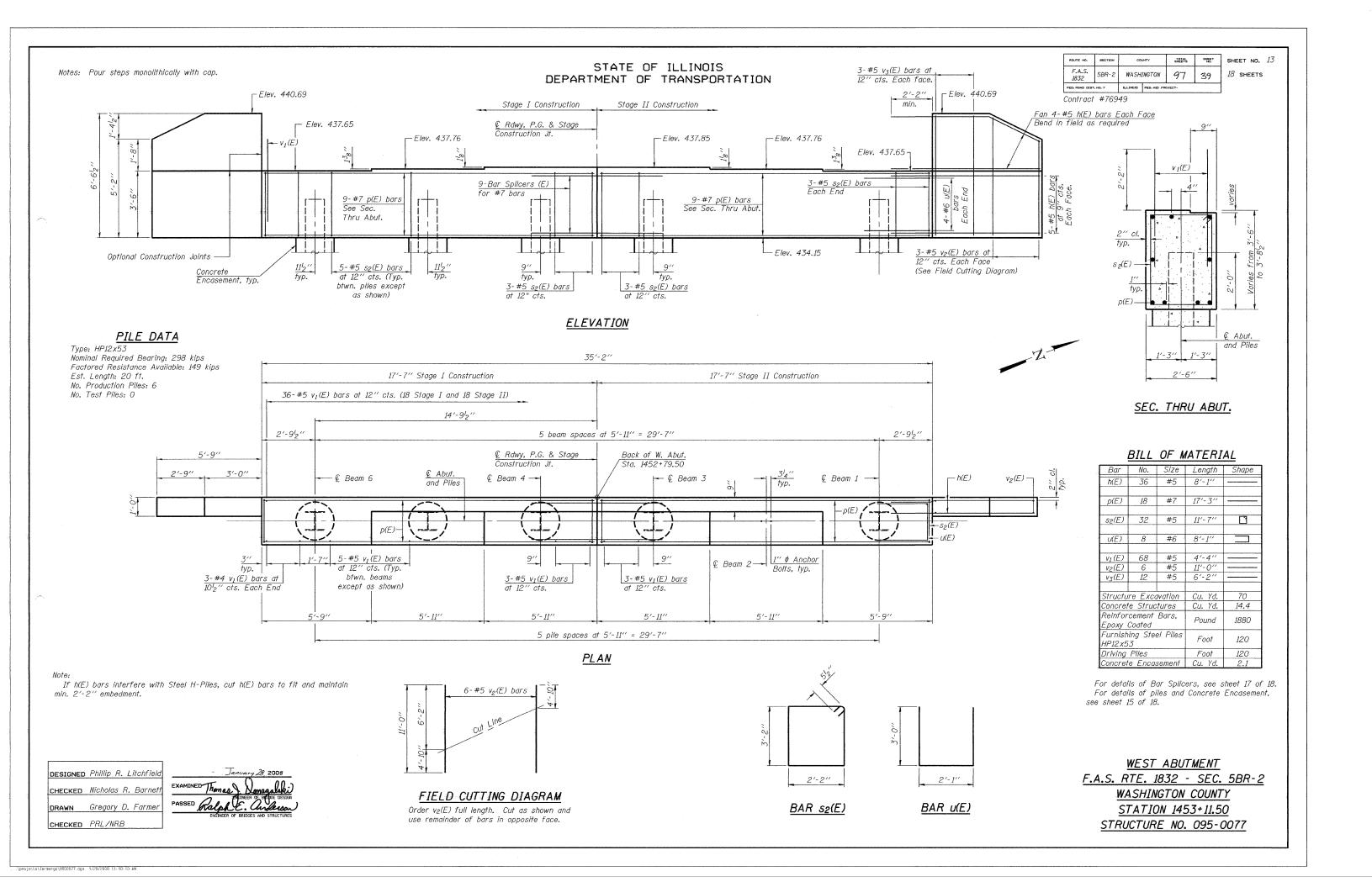
\*For Fabrication only

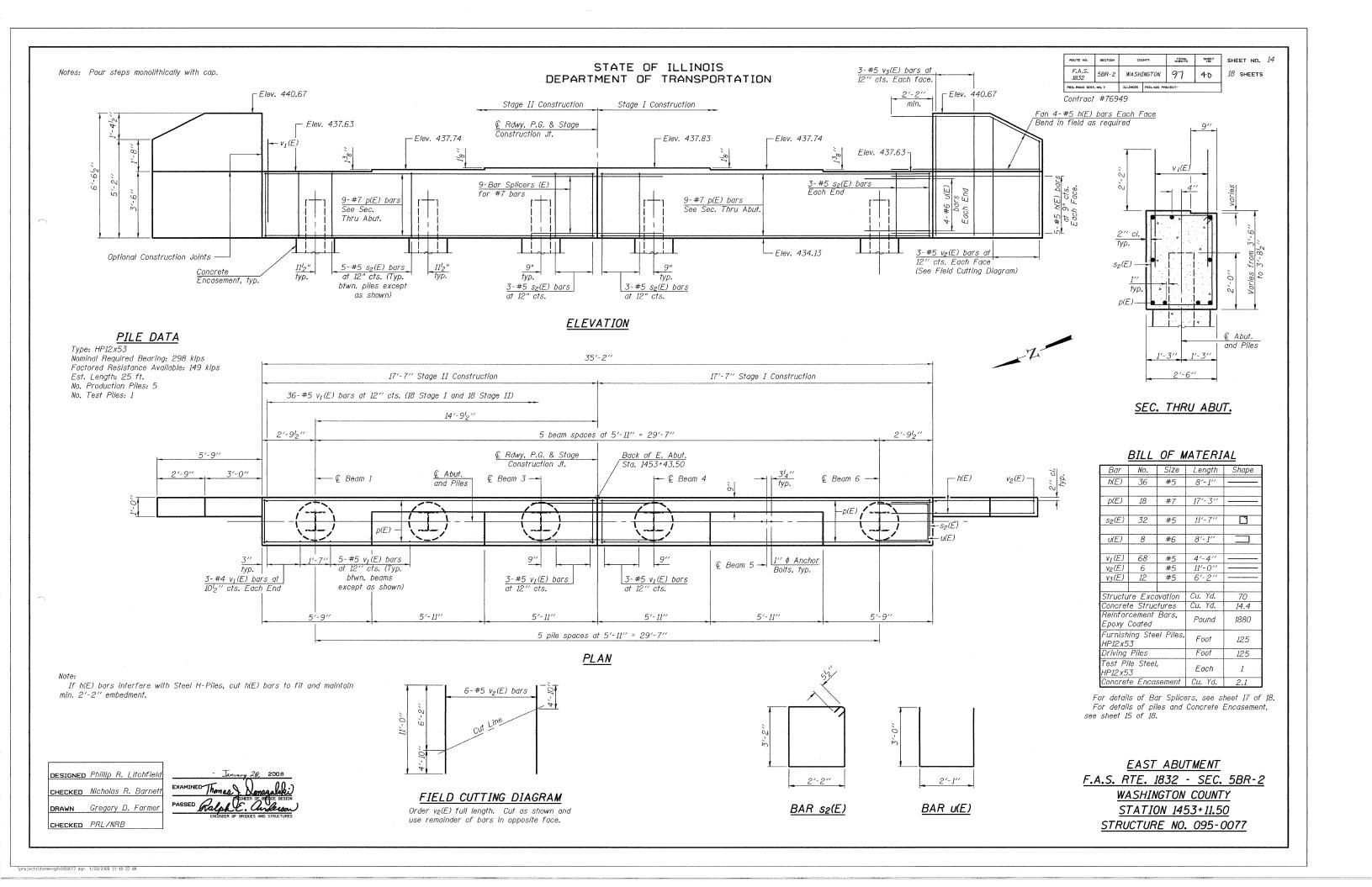
DESIGNED Phillip R. Litchfield
CHECKED Nicholas R. Barnett
DRAWN Gregory D. Farmer
CHECKED PRL/NRB

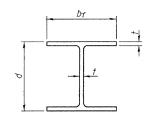
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STRUCTURAL STEEL DETAILS
F.A.S. RTE. 1832 - SEC. 5BR-2
WASHINGTON COUNTY
STATION 1453+11.50
STRUCTURE NO. 095-0077

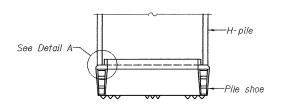




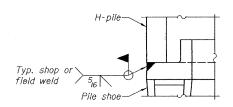


#### STEEL PILE TABLE

Designation	Depth d	Flange width b <sub>f</sub>	Web and Flange thickness t	Encasement diameter A
HP 14x117	1414''	14 <sup>7</sup> 8′′	13 <sub>16</sub> ′′	30′′
x102	14''	1434''	16'	30′′
x89	13 <sup>7</sup> 8′′	1434''	58′′	30′′
x73	13 <sup>5</sup> 8′′	14 <sup>5</sup> 8′′	2"	30′′
HP 12x84	12 <sup>1</sup> 4 ′′	1214''	116''	24''
x74	1218''	1214''	5 <sub>8</sub> ′′	24''
x63	12''	1218''	2"	24''
x53	11 <sup>3</sup> 4′′	12''	<sup>7</sup> 16 ′′	24''
HP 10x57	10''	1014''	916"	24''
x42	934''	1018''	7 <sub>16</sub> ′′	24''
HP 8x36	8''	81 <sub>8</sub> ''	<sup>7</sup> 16 ′′	18′′

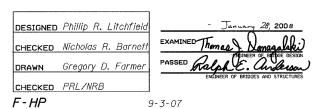


#### **ELEVATION**

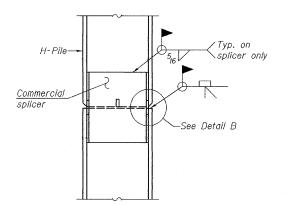


DETAIL A

#### H-PILE SHOE ATTACHMENT



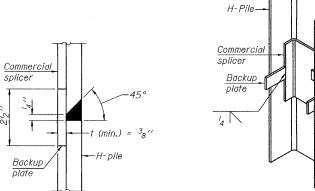
#### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION



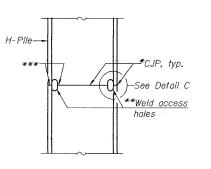


DETAIL "B"

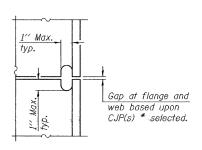
**ELEVATION** 



WELDED COMMERCIAL SPLICE







ISOMETRIC VIEW

DETAIL C

# Bottom of abutment or pier

#### ELEVATION

SHEET'S SHEET NO. 15

*18* SHEETS

97

Welded wire fabric 6 x 6-W4.0 x W4.0 weighing 58#/100 sq. ft. Bend as

required to fit into wall.

WASHINGTON

Forms for encasement may be omitted when soil conditions permit.

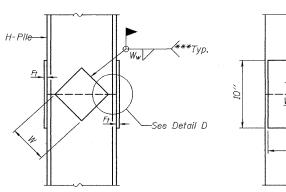
#### SECTION A-A

F.A.S. 1832

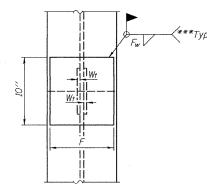
5BR-2

Contract #76949

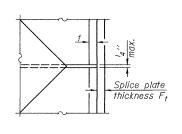
#### PILE ENCASEMENT



ELEVATION



**END VIEW** 



)	Ε	7	Α	1	L	D	

Designation	F	F <sub>t</sub>	F <sub>w</sub>	W	₩ <sub>f</sub>	Ww
HP 14×117	122"	1''	78"	734"	58"	12"
x102	1212''	78′′	34''	734''	58"	2"
x89	1212"	34''	16'	734"	58′′	2"
x73	1212"	58′′	916 ''	734''	58′′	2"
HP 12x84	10′′	7 <sub>8</sub> ′′	"16"	6 <sup>l</sup> 2′′	5 <sub>8</sub> ′′	2"
x74	10′′	78′′	1/6′′	612"	58"	2"
x63	10''	58′′	2"	62"	2"	38''
x53	10′′	58′′	2"	612"	2"	38'1
HP 10x57	8''	34''	916 ''	514"	2"	38''
x42	8′′	58′′	9 <sub>16</sub> ′′	5 <sup>1</sup> 4''	2"	38''
HP 8x36	7''	5811	· · · <sup>7</sup> /6 ′′	414''	2"	38"

#### COMPLETE PENETRATION WELD SPLICE

- \*Use joint conforming to Figure 3.4 in AWS D1.1, Structure Welding Code Steel.
- \*\*Preparation per Fig. 5.2 in AWS D1.1, Structure Welding Code Steel.
- \*\*\*Interrupt welds  $l_4$ " from end of each pile.

#### WELDED PLATE FIELD SPLICE

The steel H-piles shall be according to AASHTO M270 Grade 50.

PILE DATA F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1453+11.50 STRUCTURE NO. 095-0077

### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

| FOUTE NO. | SECTION | COLATY | TOTAL | SHEET NO. | F.A.S. | 5BR-2 | WASHINGTON | 97 | 42 | 18 SHEETS | FED. NO. | OLD | 18 SHEETS | OLD | OLD | 18 SHEETS | O

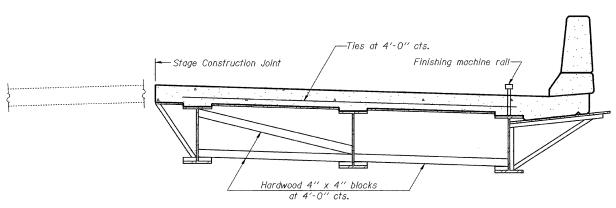
Contract #76949

When cantilever forming brackets are used, the work shall be done according to Article 503.06(b) of the Standard Specifications, except as modified below and in the details shown on this sheet.

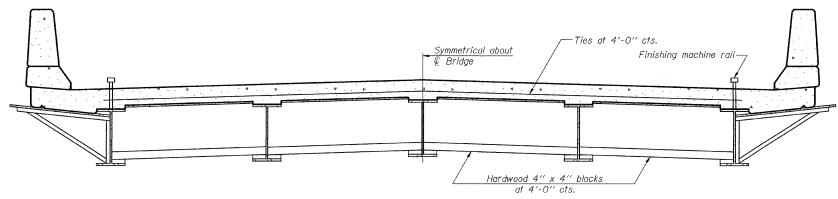
The finishing machine rails shall be placed on the top flange of the exterior beams.

The beams or girders, supporting cantilever forming brackets, shall be tied together at 4 foot intervals.

For Standard construction, or Stage Construction the Hardwood bracing materials shall be placed as shown between webs of beams in each bay.



FORM BRACES FOR STAGE CONSTRUCTION



FORM BRACES FOR STANDARD CONSTRUCTION

CANTILEVER FORMING BRACKETS
F.A.S. RTE. 1832 - SEC. 5BR-2
WASHINGTON COUNTY
STATION 1453+11.50
STRUCTURE NO. 095-0077

DESIGNED Phillip R. Litchfield

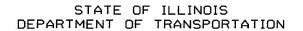
CHECKED Nicholas R. Barnett

DRAWN Gregory D. Farmer

CHECKED PRL/NRB

**SB-1** 11-1-06

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<u>NOTES</u>

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

reinforcement bars.

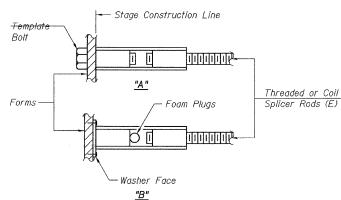
Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

- Minimum Capacity (Tension in kips) =  $1.25 \times fy \times A_t$
- Minimum \*Pull-out Strength = 0.66 x fy x A<sub>f</sub> (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

 $A_t$  = Tensile stress area of lapped reinforcement bars. \* = 28 day concrete

BAR SPLICER ASSEMBLIES											
0 0: 1		Strengt	h Requirements								
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension								
#4	1′-8′′	14.7	7.9								
#5	2'-0''	23.0	12.3								
#6	2'-7"	33.1	17.4								
#7	3′-5′′	45.1	23.8								
#8	4'-6''	58.9	31.3								
#9	5′-9″	75.0	39.6								
#10	7'-3''	95.0	50.3								
#11	9′-0′′	117.4	61.8								



#### BAR SPLICER ASSEMBLY ALTERNATIVES

WELDED SECTIONS

ROLLED THREAD DOWEL BAR

Wire Connector

\*\* ONE PIECE

The diameter of this part is

equal or larger than the

diameter of bar spliced.

\*\* Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

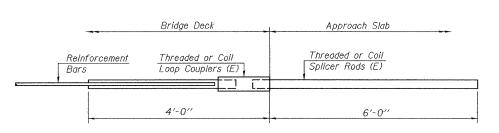
The diameter of this part

of the bar spliced.

is the same as the diameter

### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E) : Indicates epoxy coating.



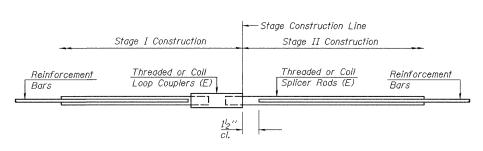
#### FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required = 64

DESIGNED Phillip R. Litchfield CHECKED Nicholas R. Barne DRAWN Gregory D. Farmer CHECKED PRL/NRB BSD-1 11-1-06

6'-0" Approach slab Abutment Threaded or Coil Threaded or Coil Loop Couplers (E) Splicer Rods (F) Reinforcement bars FOR STUB **ABUTMENTS** Bar Splicer for #5 bar

Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required =



#### STANDARD

	Bar Size	No. Assemblies Required	Location
	#6	16	Diaphragm
	#5	187	Deck
	#7	18	Abutment
ı			

BAR SPLICER ASSEMBLY DETAILS F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1453+11.50 STRUCTURE NO. 095-0077

### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHEET NO. 18
F.A.S. 1832	5BR-2	WASH	INGTON	97	43a.	18 SHEETS
FED. ROAD DIST	. NO. 7	ILLINOIS	FED. AID PRI	OJECT-		

Contract #76949

Illinois Depa of Transport	ertme ation	ent		S	OIL BORING LO	OG	Page	1	of .
Division of Highways District 8 Materials	uuoi			_	OIL DOMMO L		Date	10	29/70
ROUTE FAS 1832 DESCRIP	TION .			IL 160	over Plum Creek	_ LOGGED E	BY	. Hoffr	nan
SECTION 5BR-2	LOCA	TION _	SE 14	, NE 1	4, SEC. 28, TWP. 15, RNG. 4W, 3	PM			
COUNTY Washington DRI	LLING 1	METHOE		Holle	ow Stem Auger HAMMI	ER TYPE	140#	Automa	atic
STRUCT. NO. 095-0006 (E) / 095-0077 (P)		р В	U	м	Surface Water Fley	ft D	В	u	м
Station 1453+15	_   !		C	0	Surface Water Elev	51 ft E		C	0
DODING NO. 45 Above	- 13	PO	s	S	Groundwater Elev.:	P	O W	S	l S
BORING NO. 1 E. Abut Station 1453 + 57		H S	Qu	Ť	First Encounter 43	<u>.9 ft ▼</u> H		Qu	Ť
Offset 10,00ft Left	_		1	l	Upon Completion	ft	1		
Ground Surface Elev. 441.31	_ft (	(t) (6")	(tsf)	(%)	After Hrs Gray Weathered SHALE	ft (ft)	(6")	(tsf)	(%
Brown CLAY		-	1		(continued)	-	50	1.65	15
	_			-			1	S	L
			1	1			4		
		-	l			-	┨		
	-	12	0.76	29			100+	1.55	16
	–	]_	S	<u> </u>			1-	S	<u> </u>
	436.9	_	1	1		_	1		
Dark Gray Clayey SILT		-9				-2!	7		
		8	0.52	29		_	100+		14
	***	_	В	-		_	-	NC	├
	434.4	-					-		
Dark Gray SILT				<u> </u>		-	<u> </u>		_
		4	0.16 B	35			100+	NC	12
	431.9 ▼	-	1 5	<del>                                     </del>		412.4	$\vdash$	NC	-
		-10	1		END OF BORING	-3	5		
Dark Gray Silty Sandy CLAY		7	0.62	25	NOTE: Value in Blows'6" Colum	on -	]		
	_	-   '	В	1	is the N-Value of the Sample		1		
	429.4	_				7	1		
Gray and Brown Coarse SAND	_	]				_	1		
and Fine GRAVEL		22	+	$\vdash$			-		
		L	NC			-	1		
	426.9								
Gray and Brown Coarse SAND	_=	-15	1			_3:	5		
and Coarse GRAVEL		12	+	<del> </del>		-	4	1	
	_	-L	NC			-	1		
	424.4					_			l
Brown Highly Weathered SHALE		4	1	1		_	1		
Diene inginy resultation of Oracle		38	0.81	20			-		
		L_	s			-	1		
a water and a	421.9		1				]		
Gray Weathered SHALE		20				-41	J		

of Transport  Division of Highways District 8 Materials	auo	**				OIL BORING LOG			Date	10/	30/70
_	PTION				IL 160	over Plum Creek LC	OGGEI	D B	YC	. Hoffn	nan
SECTION 58R-2	_ LOC	ATIO	ON _	SE 1/4	, NE 1	4, SEC. 28, TWP. 1S, RNG. 4W, 3 PM					
COUNTY Washington DR	LLING	ME	THOD		Holle	ow Stem Auger HAMMER T	YPE _	1	40# A	utoma	tic
STRUCT. NO. 095–0006 (E) / 095–0077 (P) Station 1453+15	_ [	D E P	B L O	U C S	M O	Surface Water Elev. Stream Bed Elev. 429.51	ft ft	D E P	B L O	U C S	M 0
BORING NO. <u>2 W. Abut</u> Station <u>1452+71</u> Offset 10.00ft Right	_	T H	w	Qu	S T	Groundwater Elev.: First Encounter 428.0		T	w	On	s T
Ground Surface Elev. 439.91		(ft)	(/6")	(tsf)	(%)	Upon Completion	ft ft	(ft)	(6")	(tsf)	(%)
Brown Silty CLAY	-	4				Gray Weathered SHALE (continued)		_	100+	NC	11
	-							_			
	435.5		10	0.97 S	26			_	100+	NC	14
Brown and Gray Clayey SILT	435.5	-5						-25			
	433.0		11	0.62 B	26		413.5		100+	NC	14
Dark Gray Silty CLAY	433.0					END OF BORING					
	430.5	_	12	1.96 B	24	NOTE: Value in Blows6" Column is the N-Value of the Sample		_			
Gray Clay TILL		-10						-30			
			12	1,46 B	22						
	3										
	426.0		10	1.07 B	20			_			
Brown Clay Til.i.	-	-15						-35			
	423.0	-	86	3.42 B	14			_			
Gray Clay TILL			80	4.33	12			_			
	420.5			8							
Gray Weathered SHALE		-20			[			-40	1 1		

DESIGNED Phillip R. Litchfield

CHECKED Nicholas R. Barnett

DRAWN Gregory D. Farmer

CHECKED PRL/NRB

EXAMINED Thomas Demonstration

PASSED Ralph E. Children

ENGINEER OF BRIDGES AND STRUCTURES

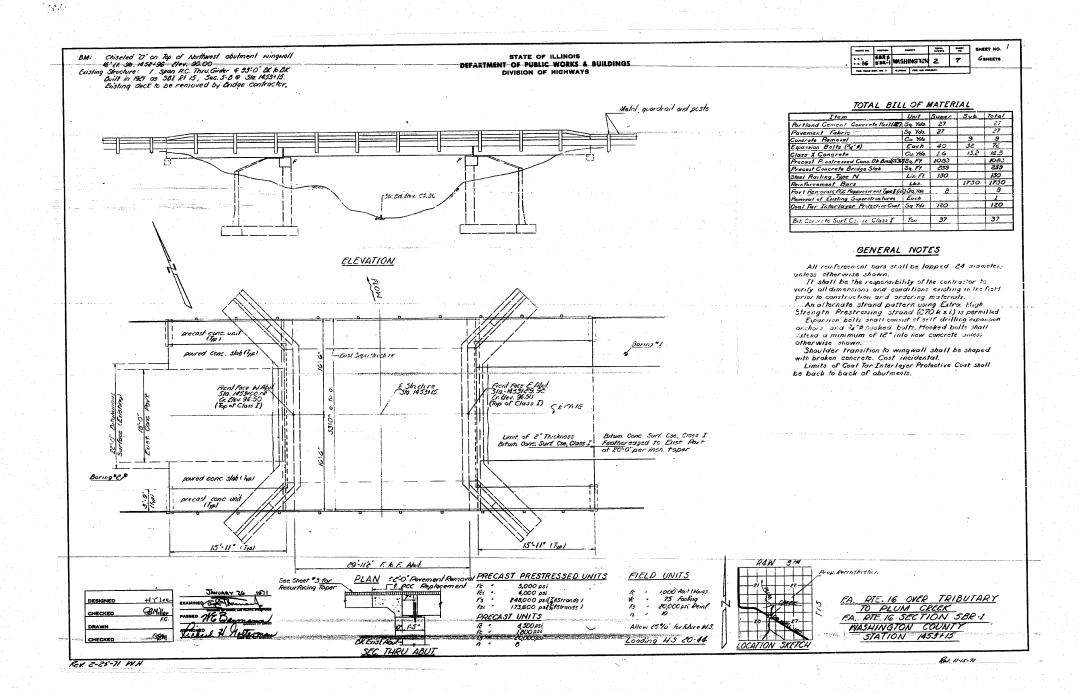
SOIL BORING LOGS

F.A.S. RTE. 1832 - SEC. 5BR-2

WASHINGTON CO.

STATION 1453+11.50

STRUCTURE NO. 095-0077



S.N. 095-0077

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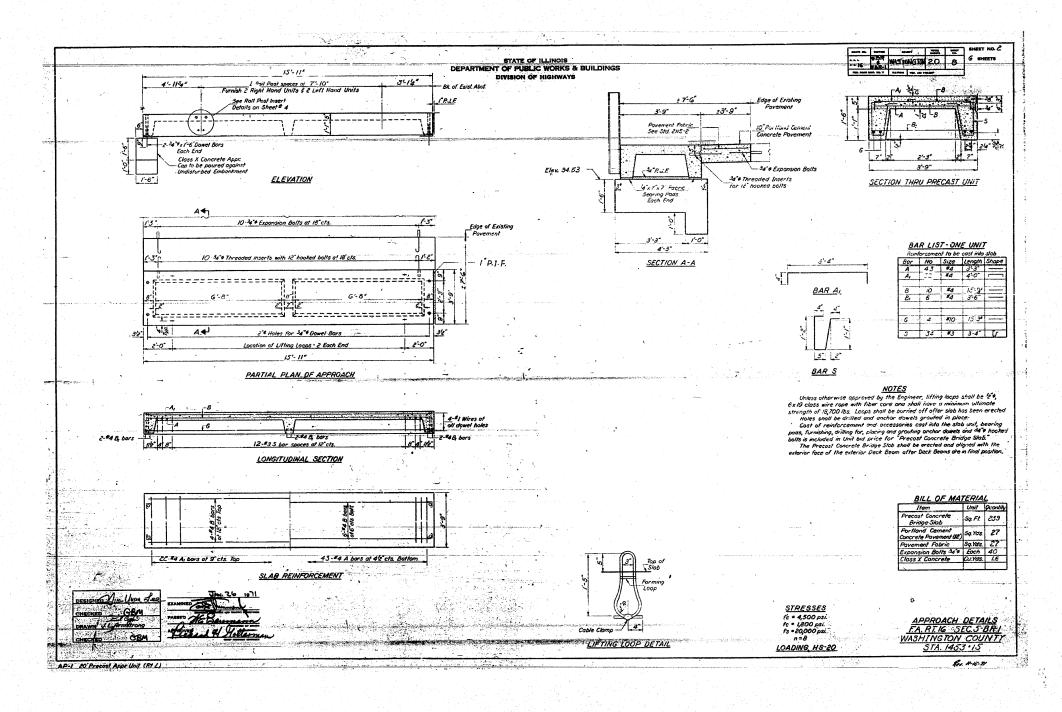
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EXISTING STRUCTURE PLANS

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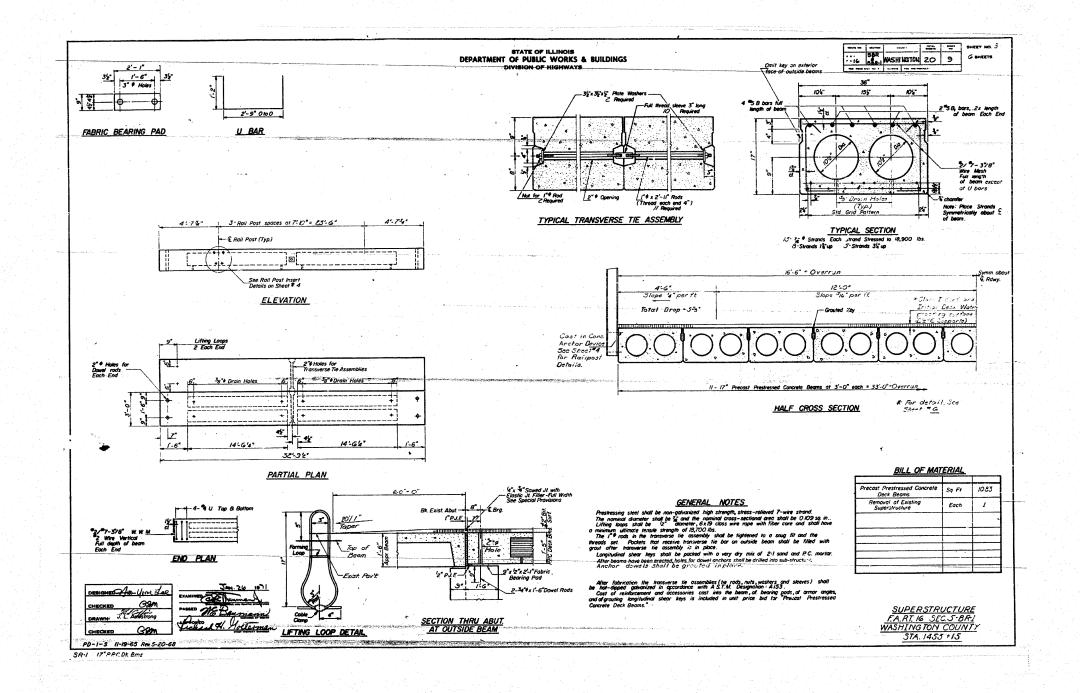
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ILLINOIS DEPARTMENT OF TRANSPORTATION

EXISTING STRUCTURE PLANS

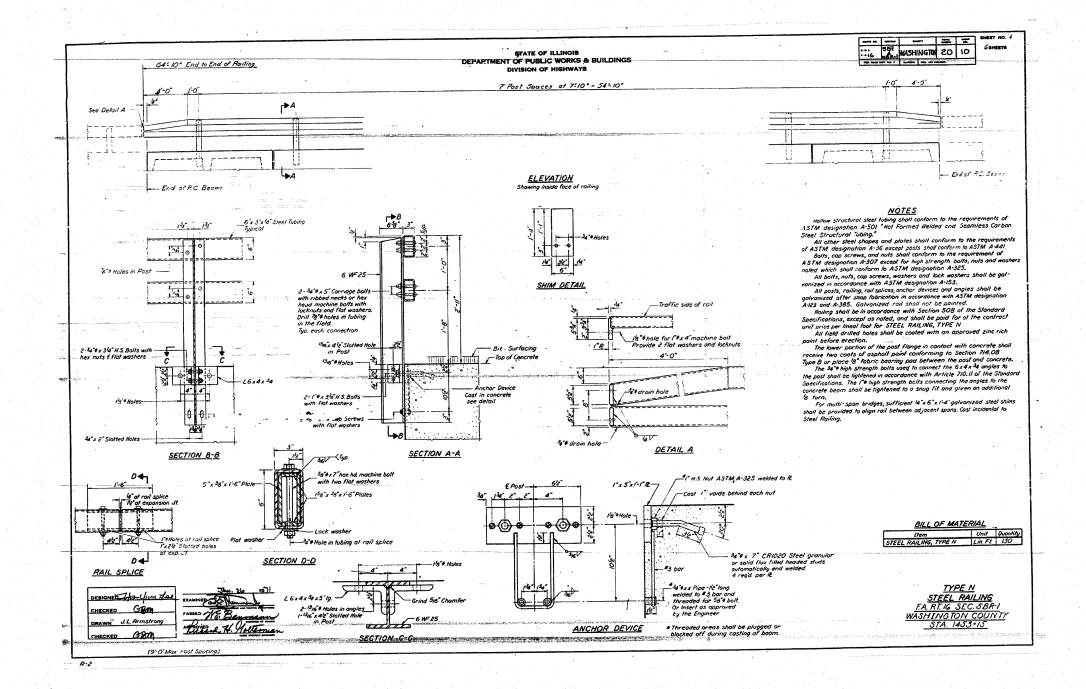
EXISTING STRUCTURE PLANS

FAS ROUTE 1832 SECTION 5BR-2 WASHINGTON COUNTY

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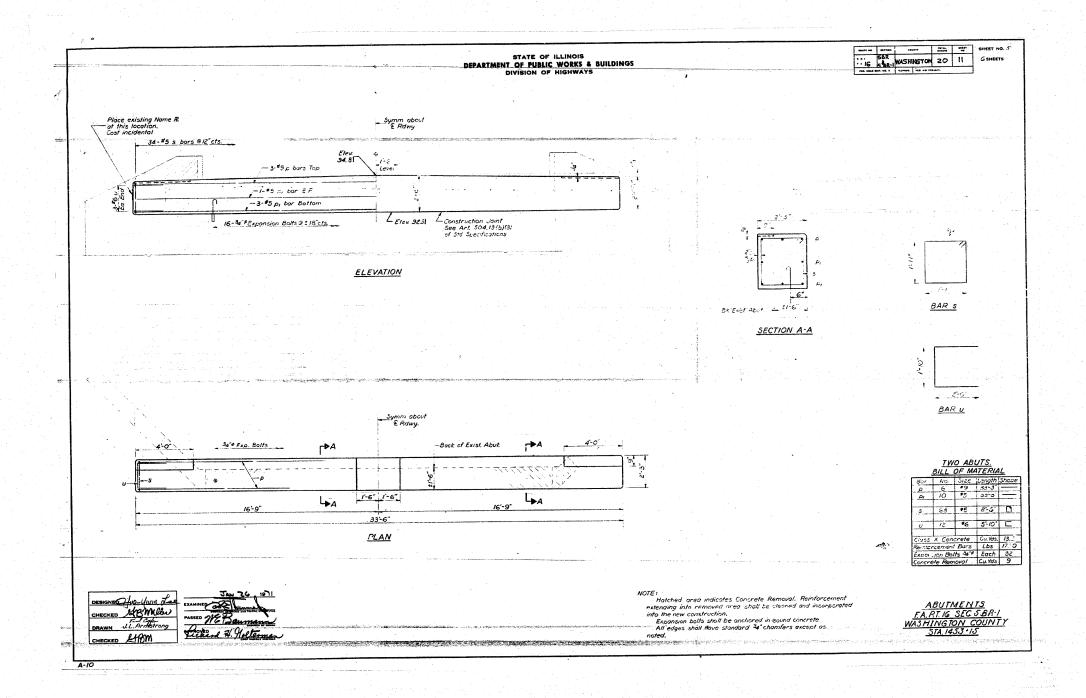
ILLINOIS DEPARTMENT OF TRANSPORTATION

EXISTING STRUCTURE PLANS

FAS ROUTE 1832 SECTION 5BR-2 WASHINGTON COUNTY

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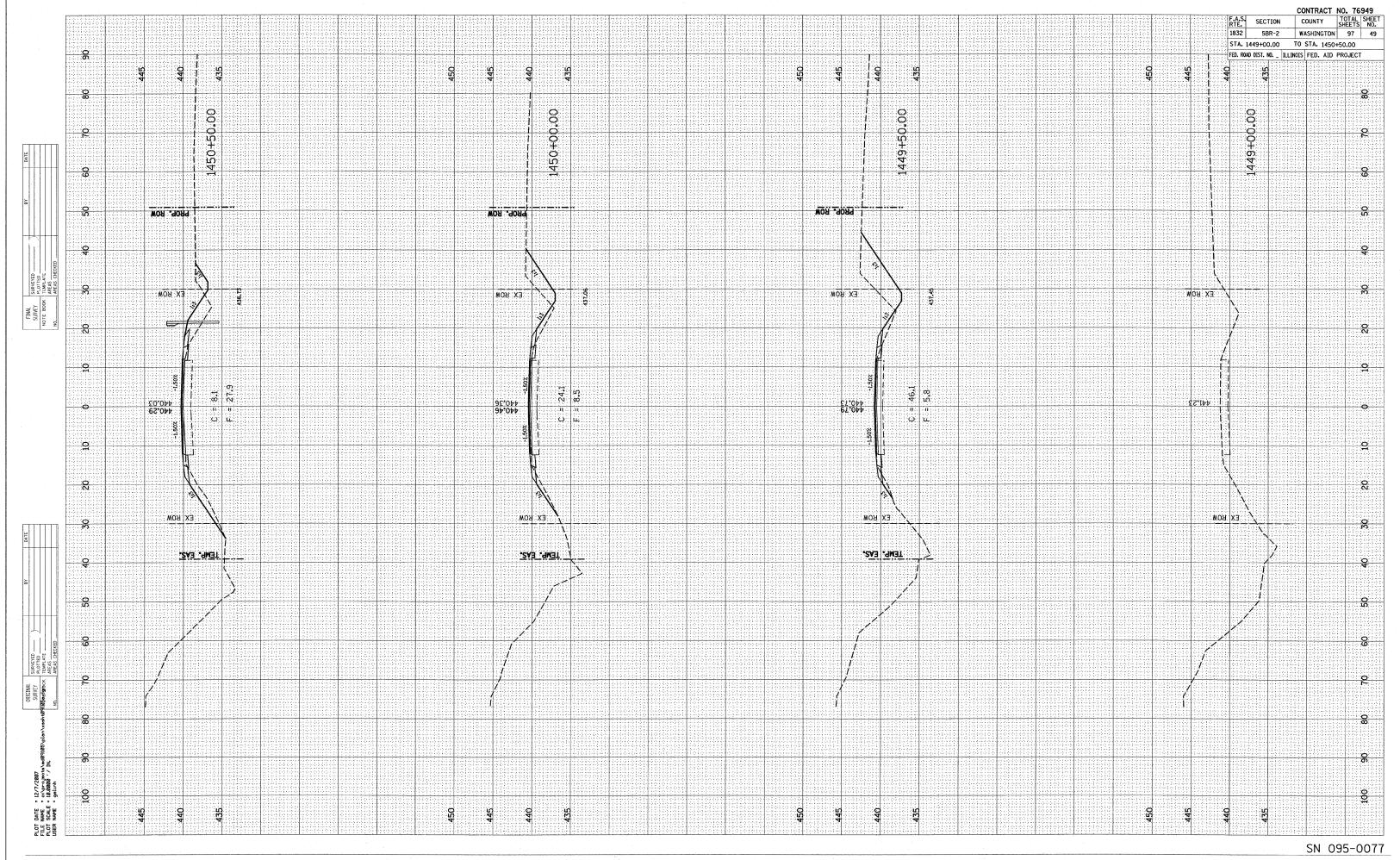
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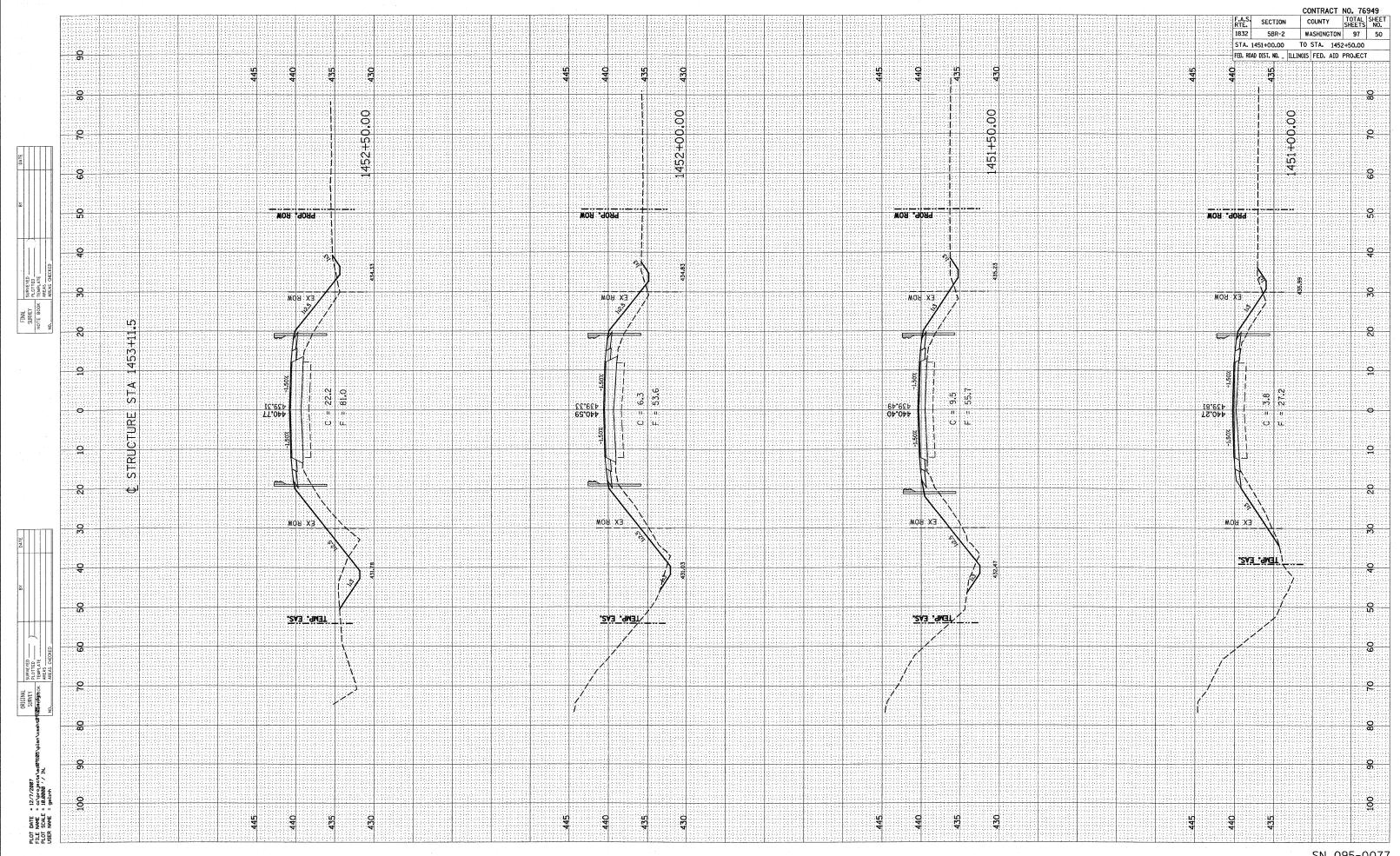


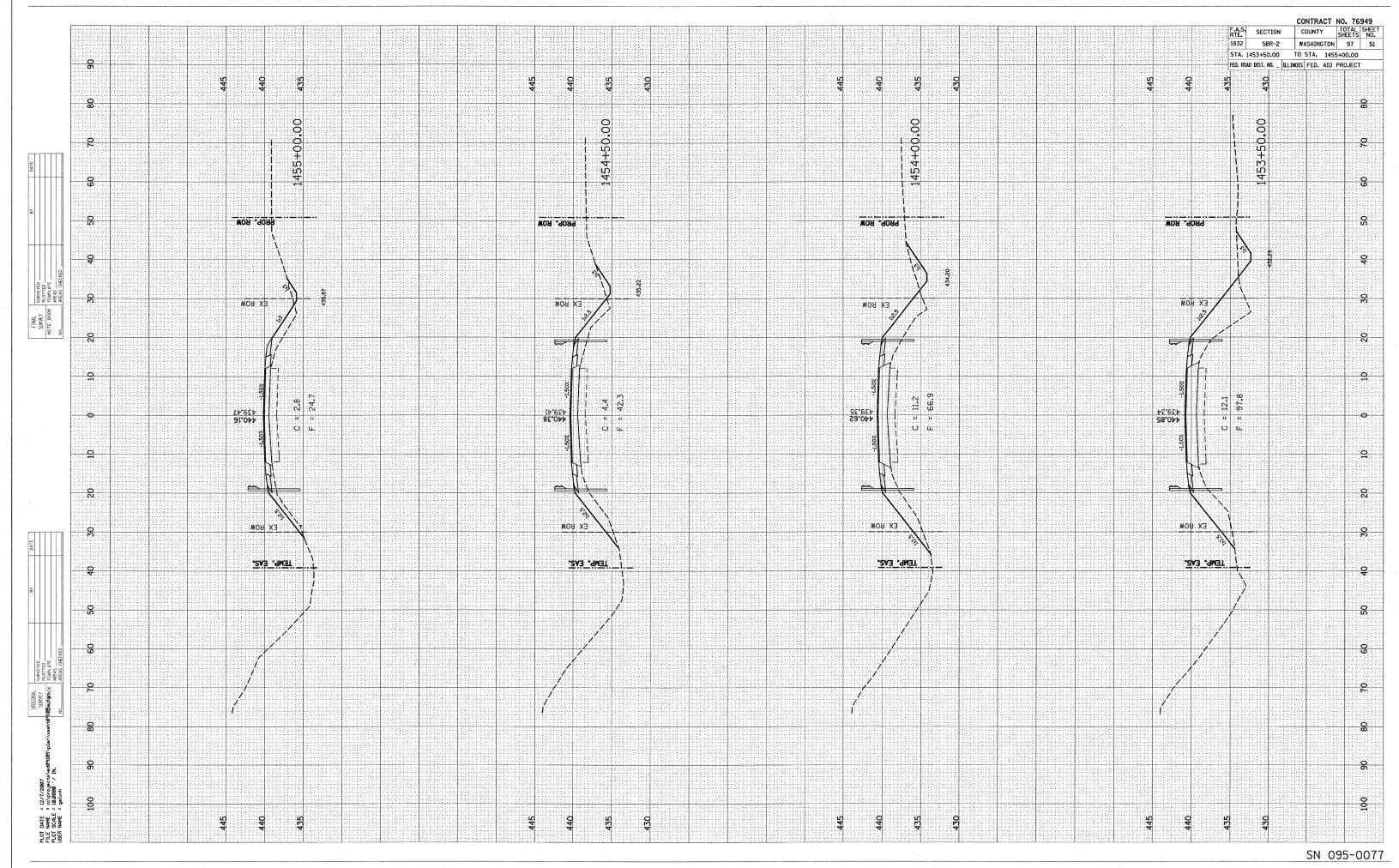
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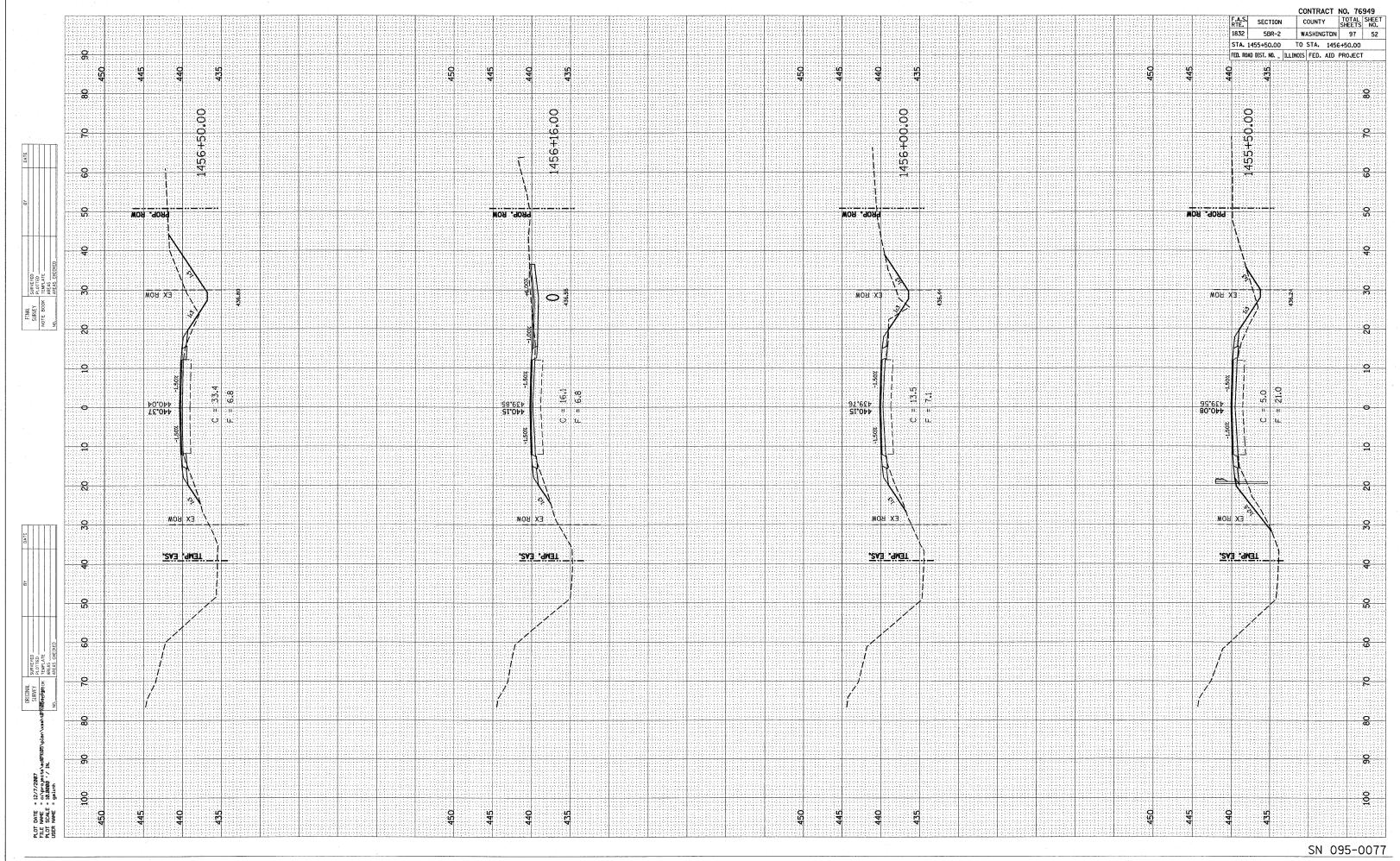
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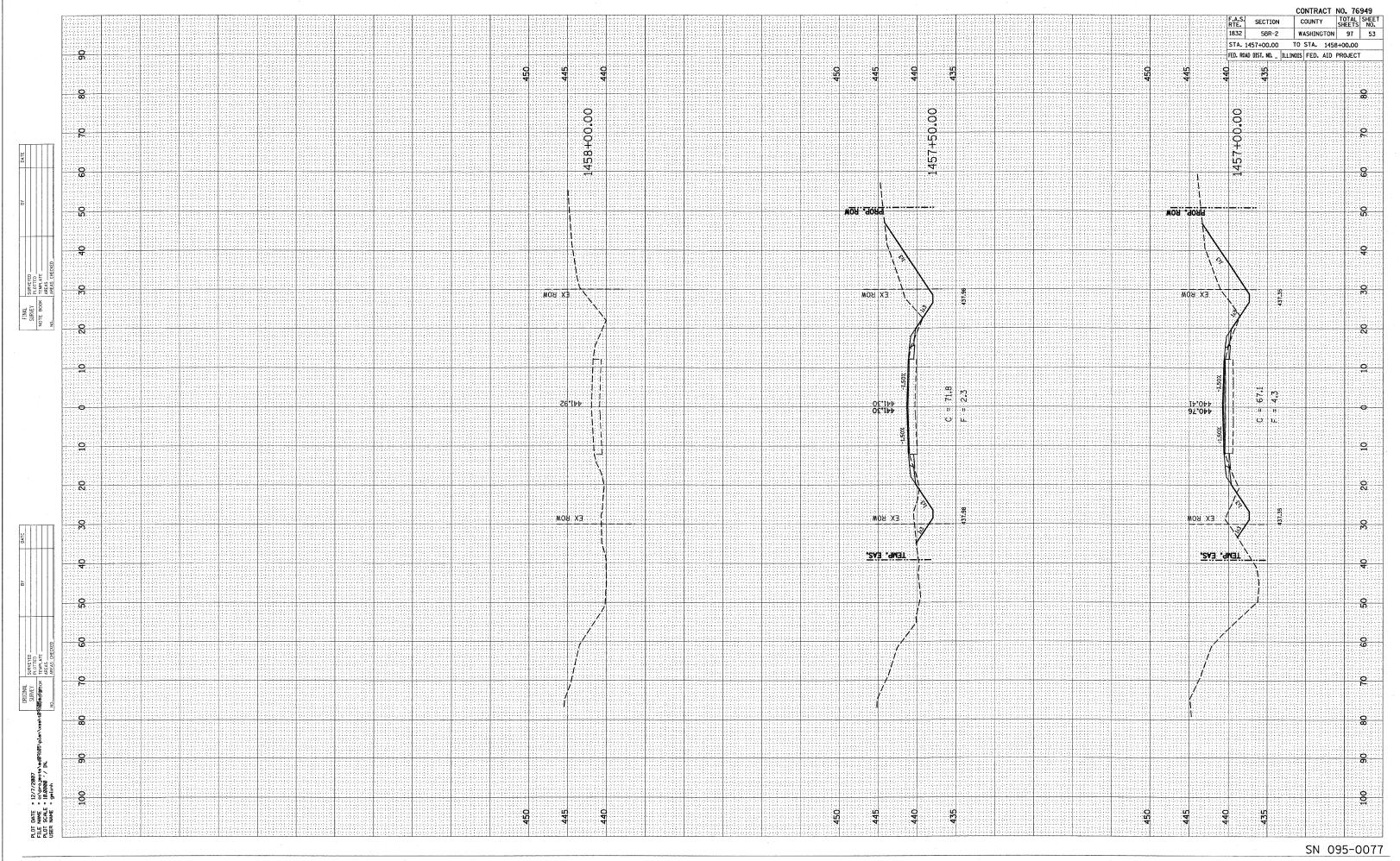
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		WASHINGTON COUNTY
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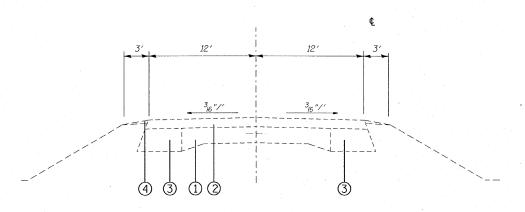






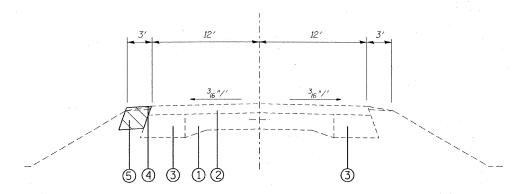






#### TYPICAL SECTION

STA. 1505+00.00 TO STA. 1517+00.00



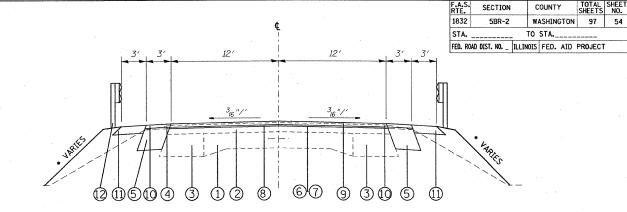
#### TYPICAL SECTION

STA. 1507+75.00 TO STA. 1514+25.00 - LT



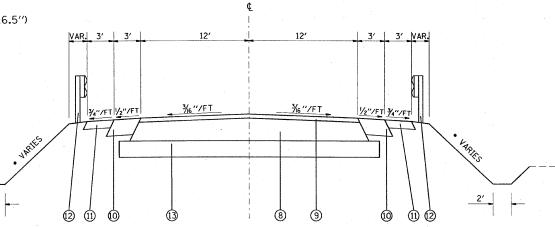
#### LEGEND

- EXISTING P.C.C. PAVEMENT 9-6-9
- EXISTING BITUMINOUS OVERLAY 6" (±)
- 3 EXISTING BASE COURSE WIDENING 8"
- EXISTING AGGREGATE SHOULDERS
- PROPOSED HOT-MIX ASPHALT BASE COURSE WIDENING, 9"
- PROPOSED BITUMINOUS MATERIALS (PRIME COAT)
- 7 PROPOSED AGGREGATE (PRIME COAT)
- PROPOSED HOT-MIX ASPHALT BINDER COURSE (VARIES 3/4" TO 16.5")
- PROPOSED HOT-MIX ASPHALT SURFACE COURSE, 11/2"
- PROPOSED HOT-MIX ASPHALT SHOULDER, 8"
- PROPOSED AGGREGATE SHOULDER, TYPE B 6"
- PROPOSED GUARDRAIL
- PROPOSED SUB-BASE GRANULAR MATERIAL, TYPE A 12"



#### TYPICAL SECTION

STA. 1507+75.00 TO STA. 1510+84.00 STA. 1511+16.16 TO STA. 1514+25.00



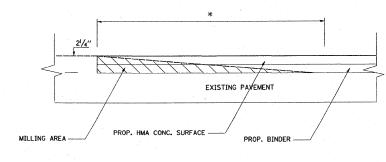
#### PROPOSED TYPICAL SECTION

STA. 1510+67.40 TO STA. 1510+84.90

#### MIXTURE REQUIREMENTS

MIXTURE USE	SURFACE	BINDER	WIDENING COURSE	SHOULDERS
AC/PG	PG 64-22	PG 64-22	PG 64-22	PG 58-22
RAP % (MAX)	10%	15%	15%	30%
DESIGN AIR VOIDS	4.0% @ Ndes= 70	4.0% @ Ndes= 70	4.0% @ Ndes= 70	2.0% @ Ndes=30
MIX COMPOSITION	·		-	
(GRADATION MIXTURE)				
FRICTION AGG	MIXTURE "D"	MIXTURE "B"	MIXTURE "B"	BAM

PLAN QUANTITIES FOR HOT-MIX ASPHALT SURFACE COURSE ITEMS ARE CALCULATED USING A UNIT WEIGHT DF 112 LB/SQ YD/IN (59.8 KG/SQ M/25 MM THICKNESS).



HMA SURFACE REMOVAL DETAIL BEGINNING STA. 1508+00.00 TO STA. 1508+89.34 ENDING STA. 1315+00.69 TO STA. 1514+25.00 S.N. 095-0078

DRAWING NOT TO SCALE

S.N. 095-0078

CONTRACT NO. 76949

ILLINOIS DEPARTMENT OF TRANSPORTATION TYPICAL SECTIONS MIXTURE REQUIREMENT & HMA REM. DETAIL FAS ROUTE 1832 SECTION 5BR-2

WASHINGTON COUNTY SCALE: VERT.

DRAWN BY

#### RESURFACING SCHEDULE

		HOT-MIX ASPHALT BS WIDENING 9"	AGGREGATE PRIME COAT	BIT. MAT'L PRIME COAT	HMA CONC BINDER SUPER, MIX "B"	HMA CONC SURF CSE, SUPER, MIX "C", N70	SUB-BASE GRANULAR MATERIAL TY-A 12"	HMA SHOULDERS 8 "	AGG. SHLE
STATION	RT/LT	(SQ YD)	(TON)	(TON)	(TON)	(TON)	(SQ YD)	(SQ YD)	(SQ YD)
1508+00.00 TO 1508+90.00	RT/LT		1 .		10.08				
1507+75.00 TO 1510+52.50	RT								
1507+75.00 TO 1510+68.30	LT	97.77							
1508+00.00 TO 1510+86.00	RT/LT		1.14	0.24		64.06		190.67	190.67
1508+90.00 TO 1510+86.00	RT/LT				307.33		1		
1510+67.40 TO 1510+84.00	RT/LT						50.56		
1511+18.90 TO 1514+25.00	LT	102.03							
1511+16.00 TO 1513+15.00	RT/LT				312.03	-			
1511+09.00 TO 1514+25.00	RT								
1511+16.00 TO 1514+25.00	RT/LT		1.24	0.26		69.22		206.00	206.00
1513+15.00 TO 1514+25.00	RT/LT			-	12.32				
TOTAL		199.80	2.38	0.50	641.76	133.28	50.56	396.67	396.67

#### TEMPORARY PAVEMENT MARKING SCHEDULE

					PA	VEMENT MARKIN	NG.		
	ST	ATION			LINE 4" PAVEMENT (FT)	LINE 6" (TEMP. BARR.) (FT)	LINE 24" PAVEMENT (FT)	WORK ZONE PVMT REMOVAL (SQ FT)	PVMT MRKG REMOVAL (SQ FT)
1504+95.00			RT	STOP BAR			12	24.0	
1504+70.00			LT	STOP BAR			12	24.0	
1504+70.00		1517+35.00		STAGE 2	2530			843.3	
1504+95.00	TO	1517+35.00		STAGE 1	2480			826.7	
1504+70.00	ТО	1517+35.00	RT/LT						843.3
1504+70.00	TO	1517+35.00	CL						105.4
1507+95.00	TO	1515+64.50		STAGE 1		770			
1506+40.50	TO	1515+69.50		STAGE 2		930			
1507+51.24		MALLARD	RT				46	92.0	
1517+35.00			LT	STOP BARS			24	48.0	
SUB	-TO	TAL							1
. Т	OTA	L			5010	1700	94	1858.0	948.7

#### GUARDRAIL SCHEDULE

ST	ATI	ON	RT/LT	SPBGR (FT)	TBT - T1 (SPECIAL) (EA)		GUARDRAIL MRKS TY-A (EA)	BI-DIREC PRISM BARR REFLEC (EA)
1508+45.25	TO	1510+76.50	RT	137.5	1	1	3 .	
1509+45.25	TO	1510+89.00	LT	50	1	1	2	
1510+86.00	TO	1511+16.00	RT/LT					2
1511+22.25	TO	1513+53.50	LT	137.5	1	1	3	
1511+11.00	TO	1512+54.75	RT	50	1	1	2	
		TOTAL		375	4	4	9	2

#### PAVEMENT MARKING SCHEDULE

					PAVEMENT	- THERMOPLAS	ric	
S <sup>-</sup>	TATI	ON		4" WHITE LINE (FT)	4" YELLOW LINE (FT)	YELLOW SKIP DASH LINE 4" (FT)	DBL AMBER RSD REFL PMK (EA)	24" WHITE LINE (FT)
1504+69.00	TO	1517+36.00	CL			316.75	16	
1504+69.00	TO	1517+36.00	RT/LT	2414				
1507+51.24		STOP BAR	RT					38
1509+00.00	TO	1513+00.00	CL		400.0			
SUE	3-T0	TAL		2414	400.00	316.75	. 16	
	ГОТА	L			3130.75		16	38

S.N. 095-0078

REVISIONS		TILINOIS DEPARTM	ENT OF TRANSPORTATION
NAME	DATE	TEETIOIO DEI FILLIN	
		SCHEDULE	OF QUANTITIES
		FAS	ROUTE 1832
	+	SEC <sup>-</sup>	TION 5BR-2
		WASHIN	IGTON COUNTY
		SCALE: VERT.	DRAWN BY
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#### TREE REMOVAL SCHEDULE

STATION	OFFSET	RT/LT		0 15 ITS		R 15 ITS
1510+86.70	40.5	LT			16	
1510+97.80	61.2	RT		12		
1510+98.40	59.5	RT	10			
1511+27.20	33.5	LT				24
SL	B-TOTAL		10	12	16	24
	TOTAL		2	2	4	0

#### REMOVAL SCHEDULE

					PVMT R	EMOVAL			
FROM	OFFSET	OCATION TO	OFFSET	RT/LT	MAINLINE REMOVAL (SQ YD)	WIDENING REMOVAL (SQ YD)	HMA SURF REMOVAL VAR. DEPTH (SQ YD)	SPBGR (FT)	
1507+75.00		1510+68.30		LT		97.77			
1508+00.00		1508+90.00		RT/LT			240.00		
1508+59.00		1510+36.00		RT				177.00	
1509+54.00		1510+83.00		LT				129.00	
1510+86.50		1511+87.50		RT				101.00	
1510+93.25		1511+17.90		RT/LT	68.14				
1511+09.50		1512+95.50		LT				186.00	
1511+18.90		1514+25.00		LT		102.03			
1512+27.00	20.6	1512+44.50	26.6	RT					
1513+00.00		1514+25.00		RT/LT			333.33		
		TOTAL			68.14	199.80	573.33	593.00	
			267.94						

#### ROW MARKERS SCHEDULE

LC STATION	ROW MARKERS (FA)		
1506+37.55	SIDE 29.54	OFFSET L†	1
1506+37.56	30.46	R†	1
1507+50.00	29.54	L†	1
1508+50.00	44.55	L†	1
1509+55.29	78.21	R†	1
1510+43.14	30.43	R†	1
1510+49.50	78.2	R†	1
1510+80.00	44.57	L†	1
1512+00.00	45.42	R†	1
1513+00.00	29.59	L†	1
1514+00.00	45.41	R†	1
1514+50.00	30.4	R†	1
1514+50.00	29.54	L†	1
-	TOTAL		- 13

#### SEEDING SCHEDULE

STA		STA		AREA SQ FT)	SEEDING CLASS 2 (ACRE)	NITROGEN FERT. NUTR (POUND)	PHOSPHORUS FERT. NUTR (POUND)		
1507+75.00	TO 1	513+50.00	LT	5005	0.12	10.34	10.34	10.34	0.12
1507+75.00	TO 1	513+50.00	RT	5824	0.13	12.03	12.03	12.03	0.13
	TOT	AL			0.25	22.37	22.37	22.37	0.25

#### EARTHWORK SCHEDULE

LOCATION	EARTH EXCAVATION (CU YD)	EARTH EXCAVATION ADJTD FOR SHRINKAGE (CU YD)	EMBANKMENT (CU YD)	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-) (CU YD)
STA. 1507+75.00 TO STA. 1510+86.00	352.5	264.4	262.5	1.9
STA. 1511+16.00 TO STA. 1514+25.00	188.5	141.4	236.7	-95.3
TOTAL	541.0	405.8	499.2	-93.4

#### EARTHWORK SCHEDULE (WIDENING)

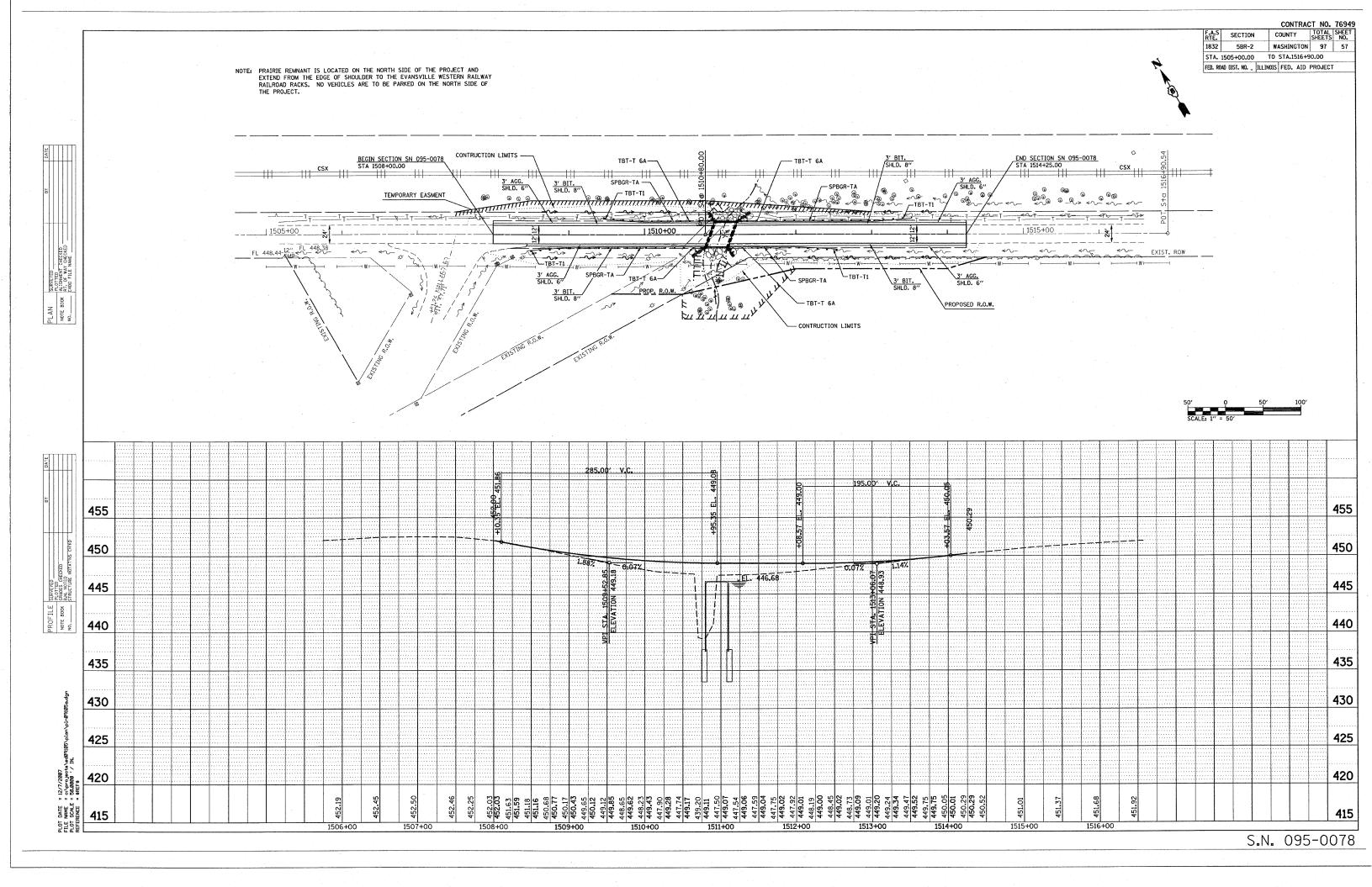
LOCATION	EARTH EXCAVATION (CU YD)	EARTH EXCAVATION ADJTD FOR SHRINKAGE (CU YD)	EMBANKMENT (CU YD)	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-) (CU YD)
STA. 1507+75.00 TO STA. 1510+68.30	25.0	18.7	0.0	18.7
STA. 1511+18.90 TO STA. 1514+25.00	26.1	19.6	0.0	19.6
TOTAL	51.1	38.3	0.0	38.3

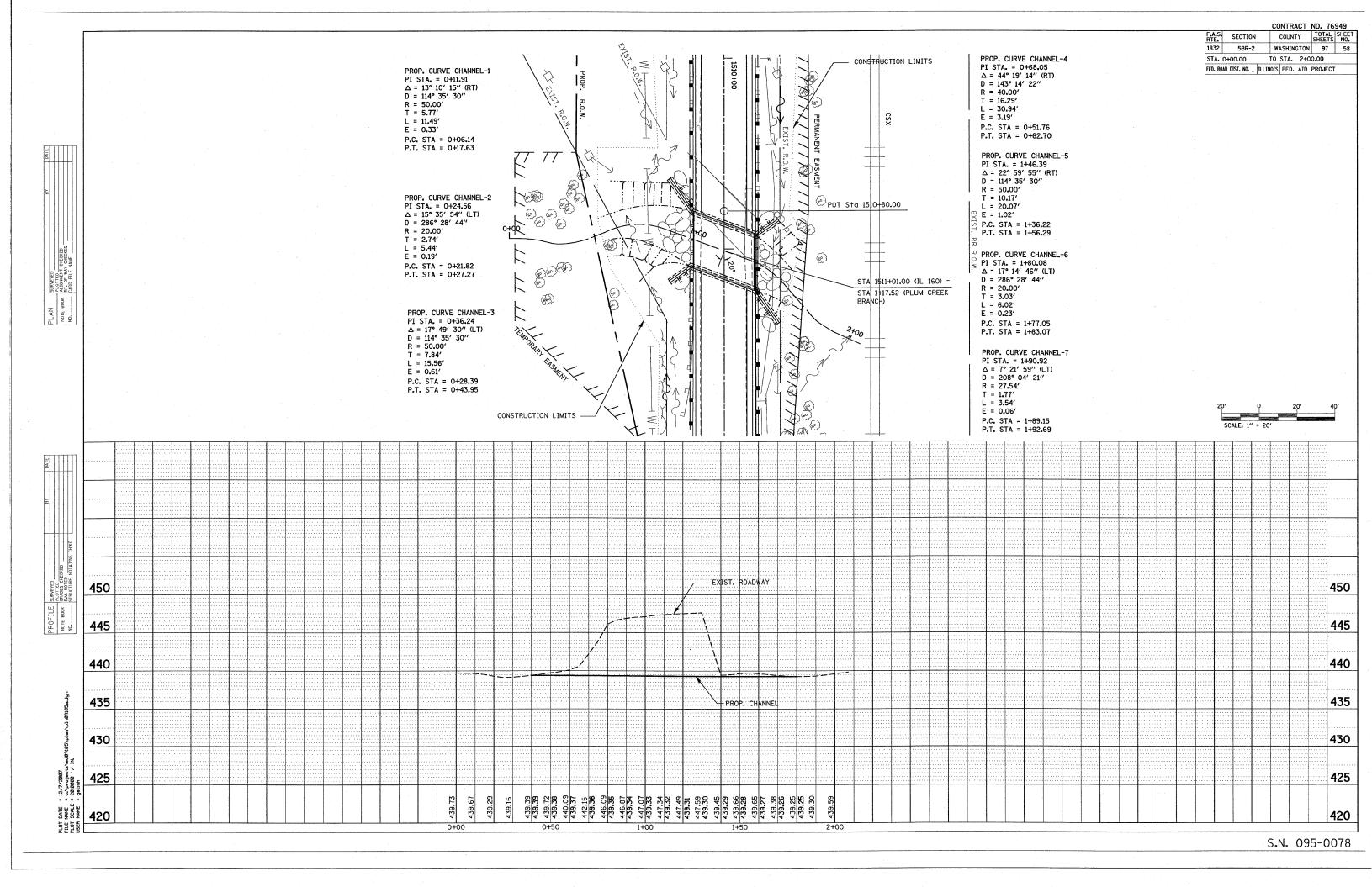
#### EARTHWORK SCHEDULE (CHANNEL)

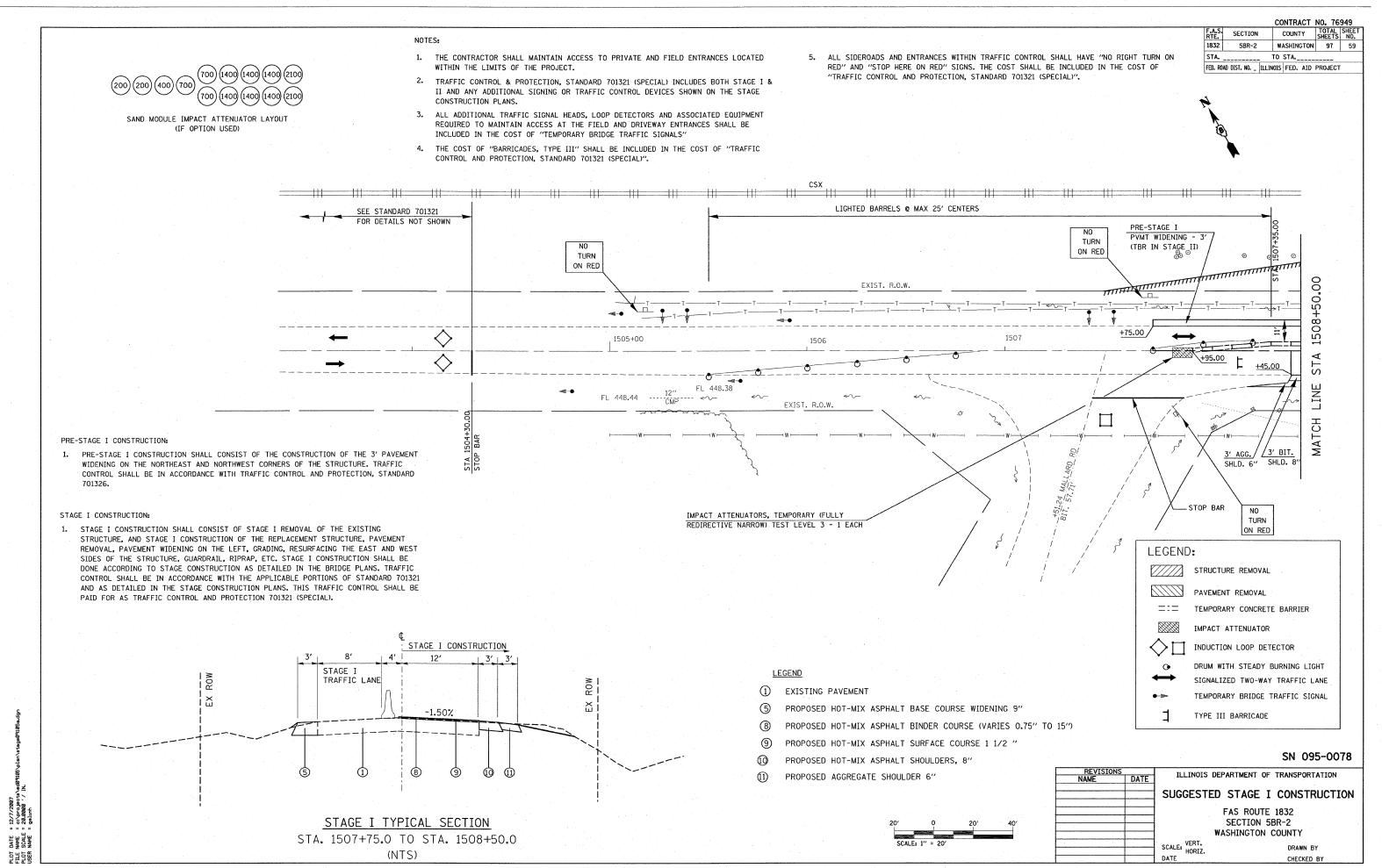
LOCATION	EARTH EXCAVATION (CU YD)	EARTH EXCAVATION ADJTD FOR SHRINKAGE (CU YD)	EMBANKMENT (CU YD)	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-) (CU YD)
STA. 0+40.00 TO STA. 1+80.00	632.4	474.3	241.5	232.8
TOTAL	632.4	474.3	241.5	232.8

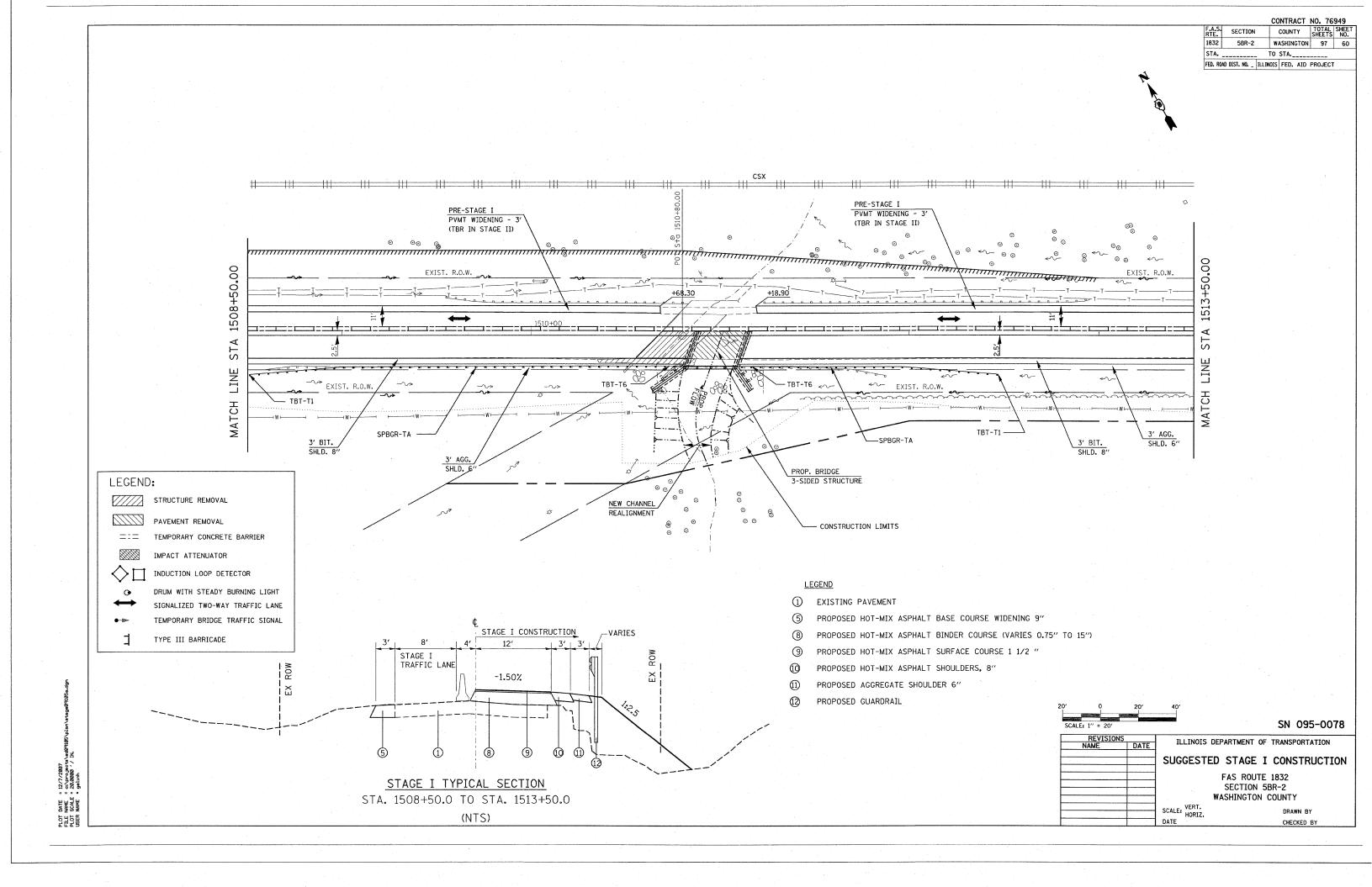
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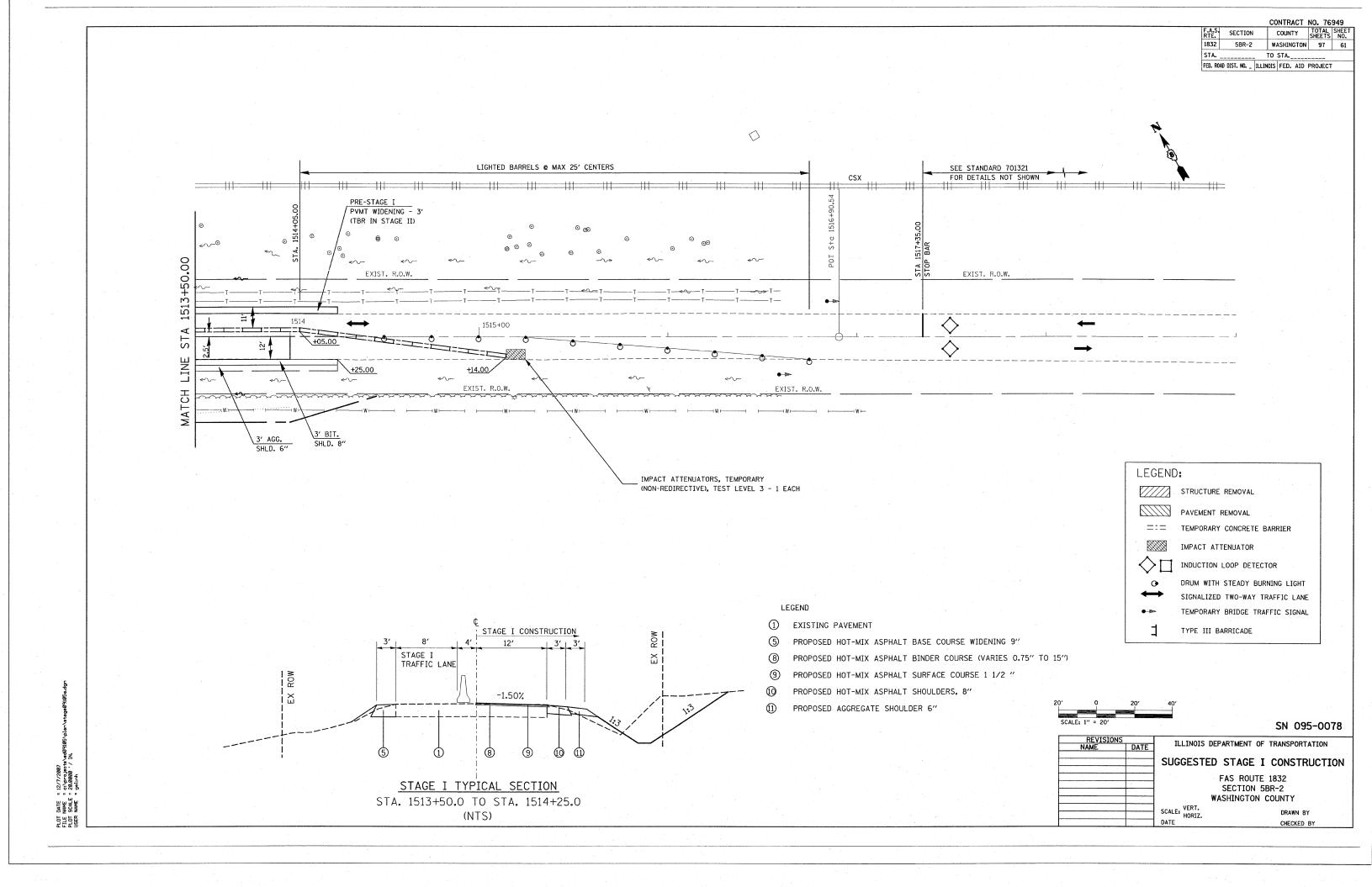
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			ON 5BR-2				
		WASHINGTON COUNTY					
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		HURIZ.					
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SAND MODULE IMPACT ATTENUATOR LAYOUT (IF OPTION USED)

NOTES:

- THE CONTRACTOR SHALL MAINTAIN ACCESS TO PRIVATE AND FIELD ENTRANCES LOCATED WITHIN THE LIMITS OF THE PROJECT.
- 2. TRAFFIC CONTROL & PROTECTION, STANDARD 701321 (SPECIAL) INCLUDES BOTH STAGE I & II AND ANY ADDITIONAL SIGNING OR TRAFFIC CONTROL DEVICES SHOWN ON THE STAGE CONSTRUCTION PLANS.
- 3. ALL ADDITIONAL TRAFFIC SIGNAL HEADS, LOOP DETECTORS AND ASSOCIATED EQUIPMENT REQUIRED TO MAINTAIN ACCESS AT THE FIELD AND DRIVEWAY ENTRANCES SHALL BE INCLUDED IN THE COST OF "TEMPORARY BRIDGE TRAFFIC SIGNALS"
- 4. THE COST OF "BARRICADES, TYPE III" SHALL BE INCLUDED IN THE COST OF "TRAFFIC CONTROL AND PROTECTION, STANDARD 701321 (SPECIAL)".

5. ALL SIDEROADS AND ENTRANCES WITHIN TRAFFIC CONTROL SHALL HAVE "NO RIGHT TURN ON RED" AND "STOP HERE ON RED" SIGNS. THE COST SHALL BE INCLUDED IN THE COST OF "TRAFFIC CONTROL AND PROTECTION, STANDARD 701321 (SPECIAL)".

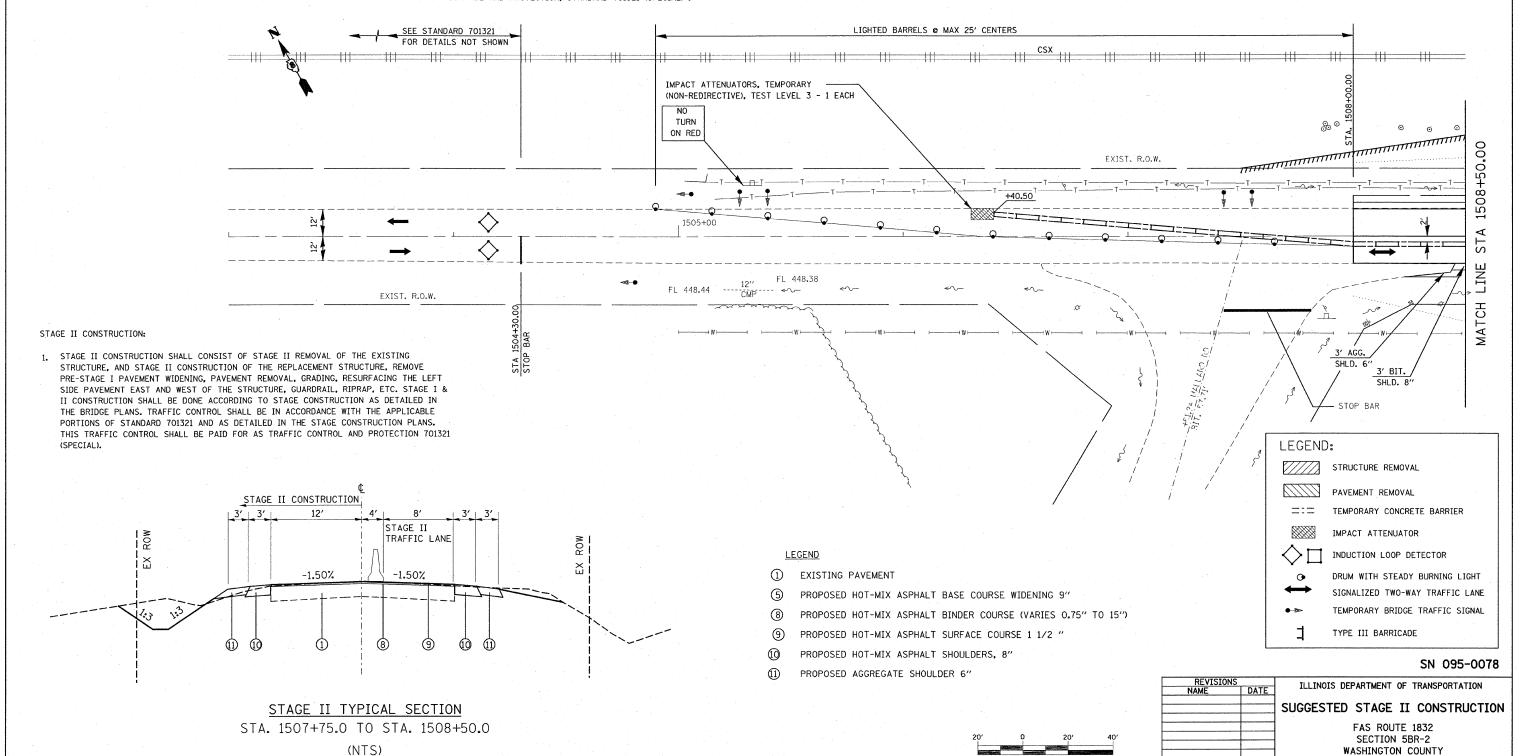
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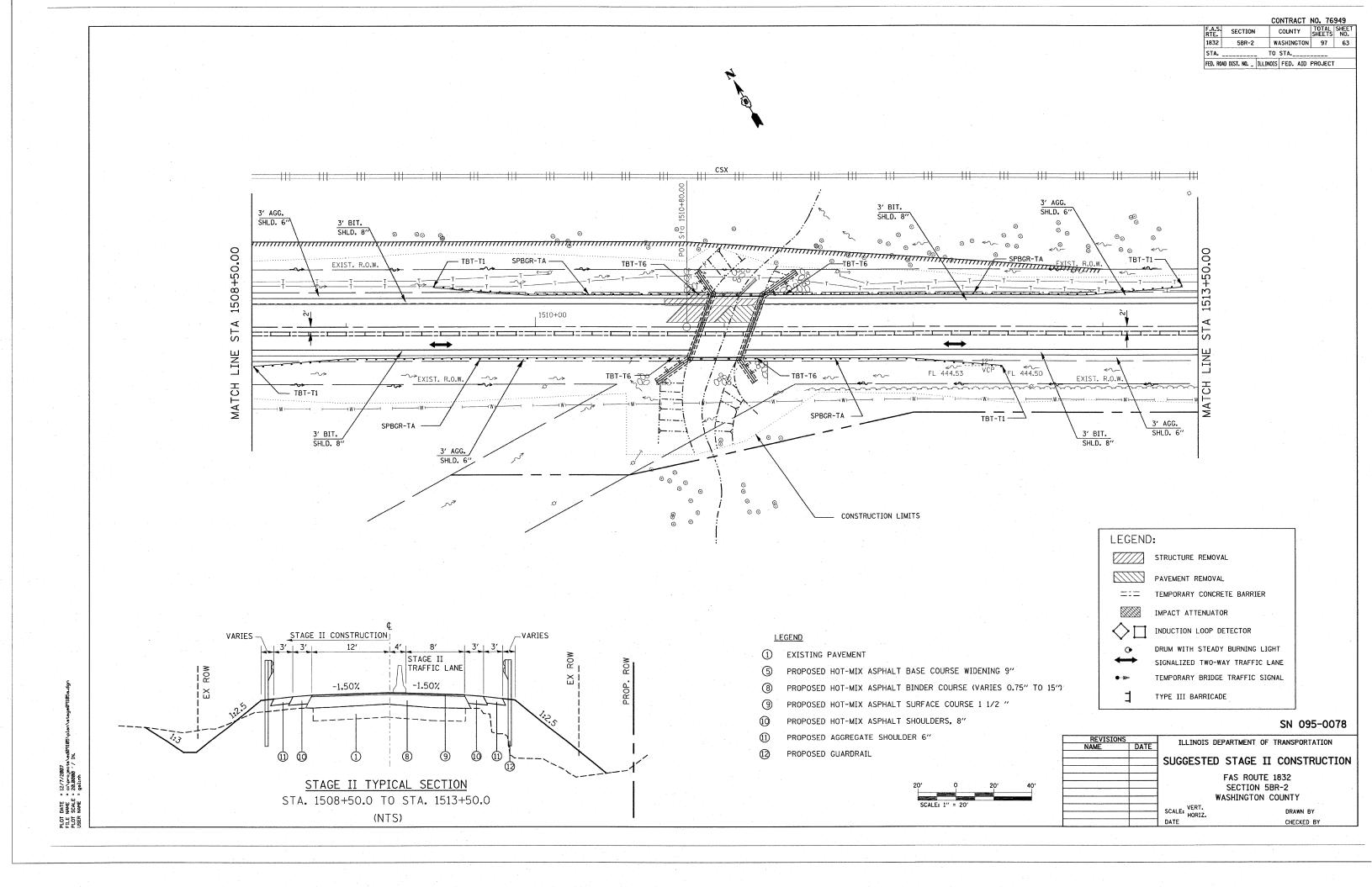
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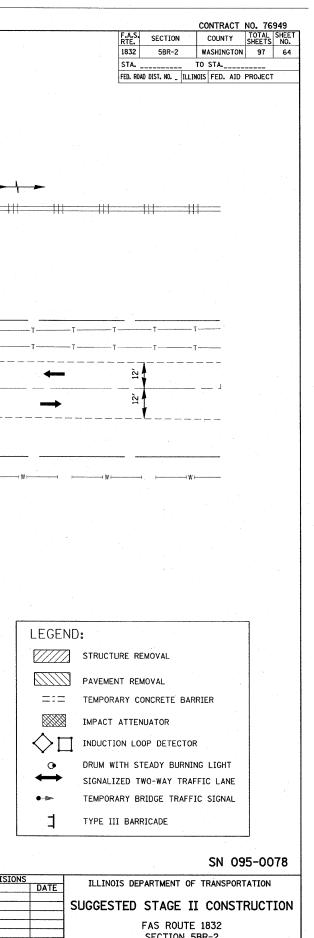
CONTRACT NO. 76949

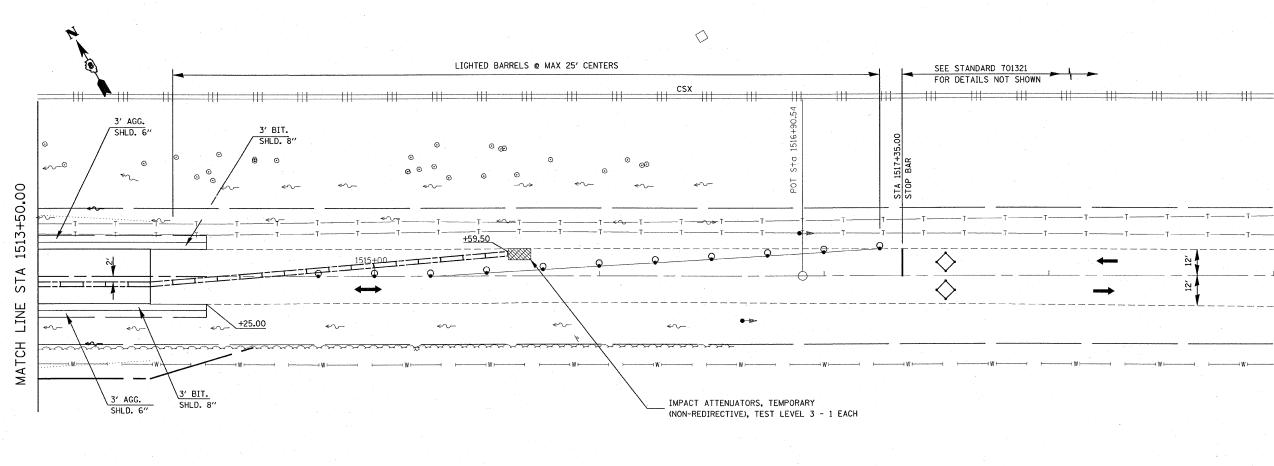


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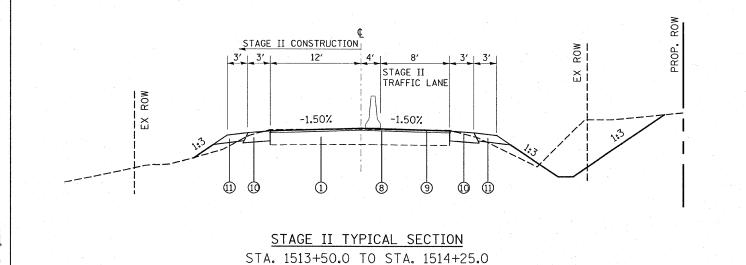
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<u>LEGEND</u>



① EXISTING PAVEMENT
⑤ PROPOSED HOT-MIX ASPHALT BASE COURSE WIDENING 9"
⑧ PROPOSED HOT-MIX ASPHALT BINDER COURSE (VARIES 0.75" TO 15")
⑨ PROPOSED HOT-MIX ASPHALT SURFACE COURSE 1 1/2 "

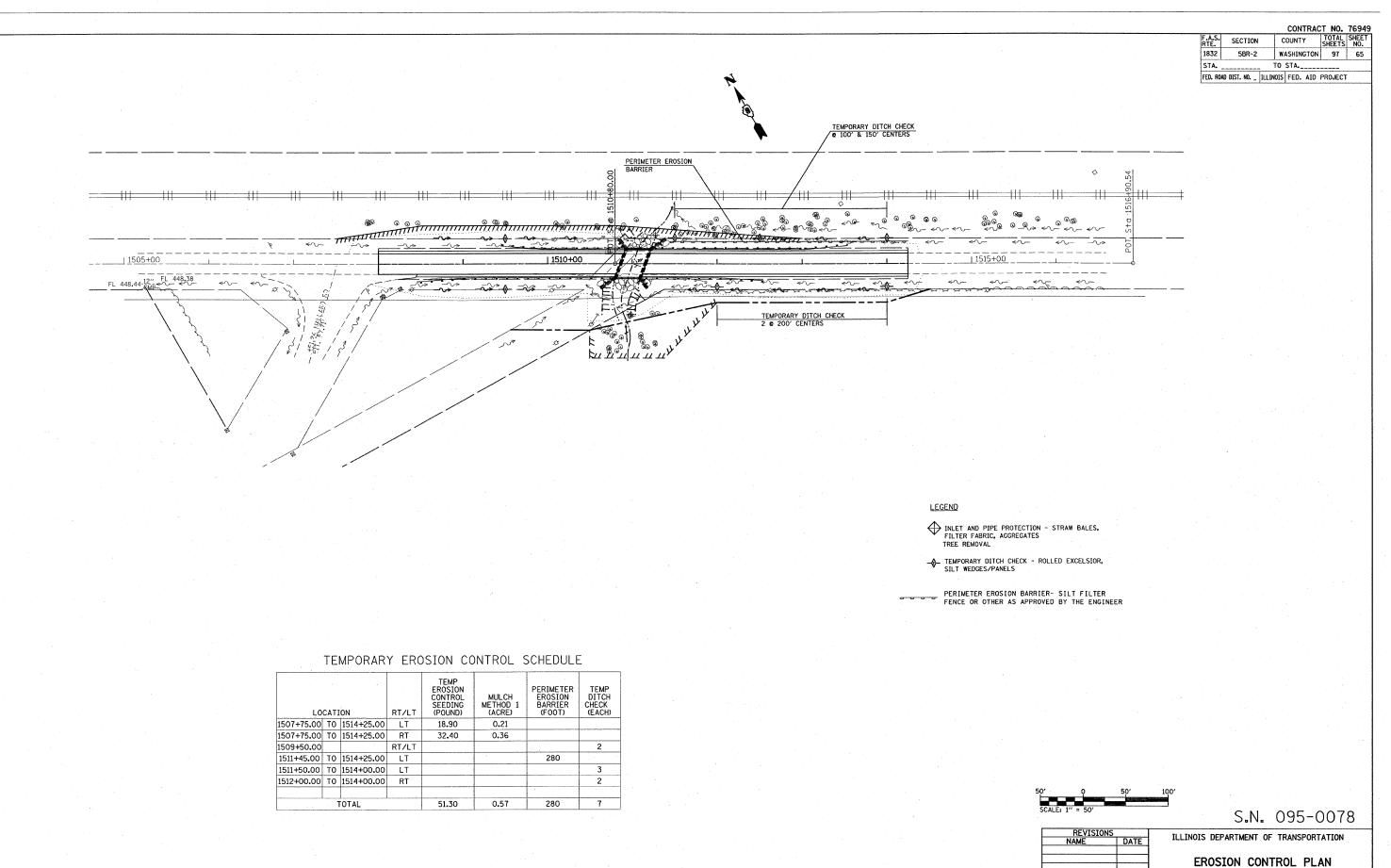
⑩ PROPOSED HOT-MIX ASPHALT SHOULDERS, 8"
① PROPOSED AGGREGATE SHOULDER 6"

REVISIONS ILLINOIS
NAME DATE
SUGGEST

SCALE: VERT. HORIZ.

FAS ROUTE 1832 SECTION 5BR-2 WASHINGTON COUNTY

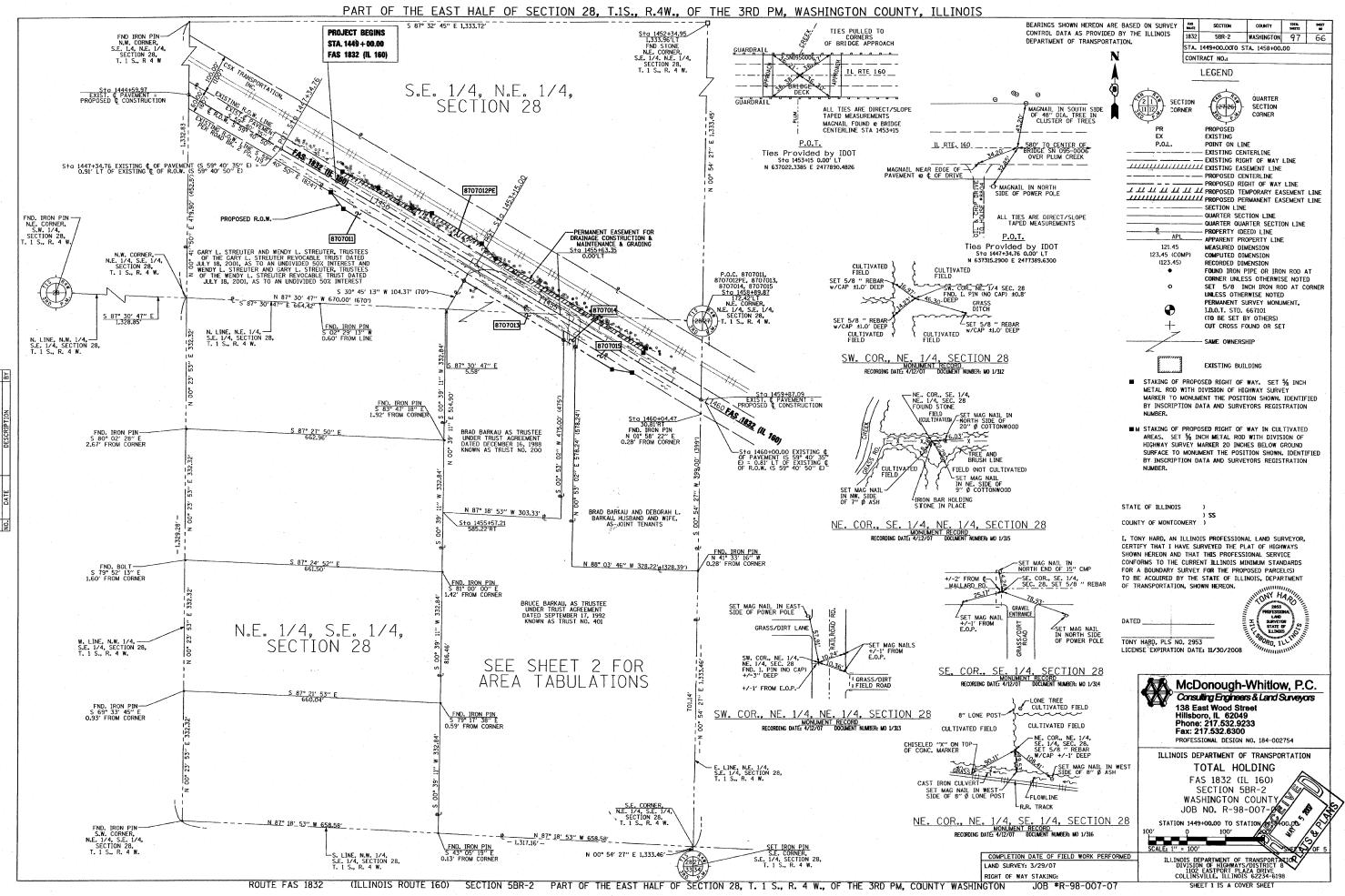
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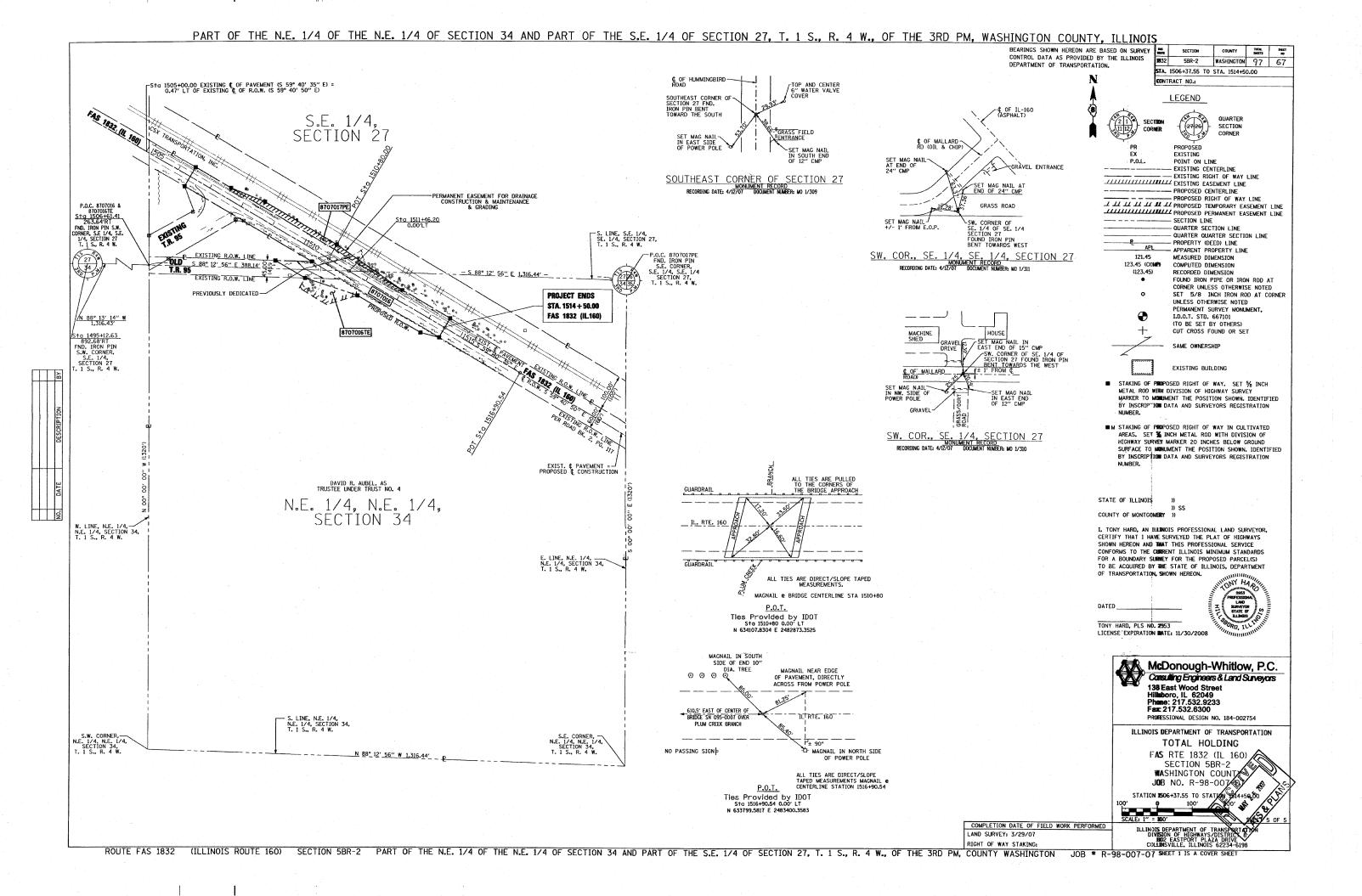


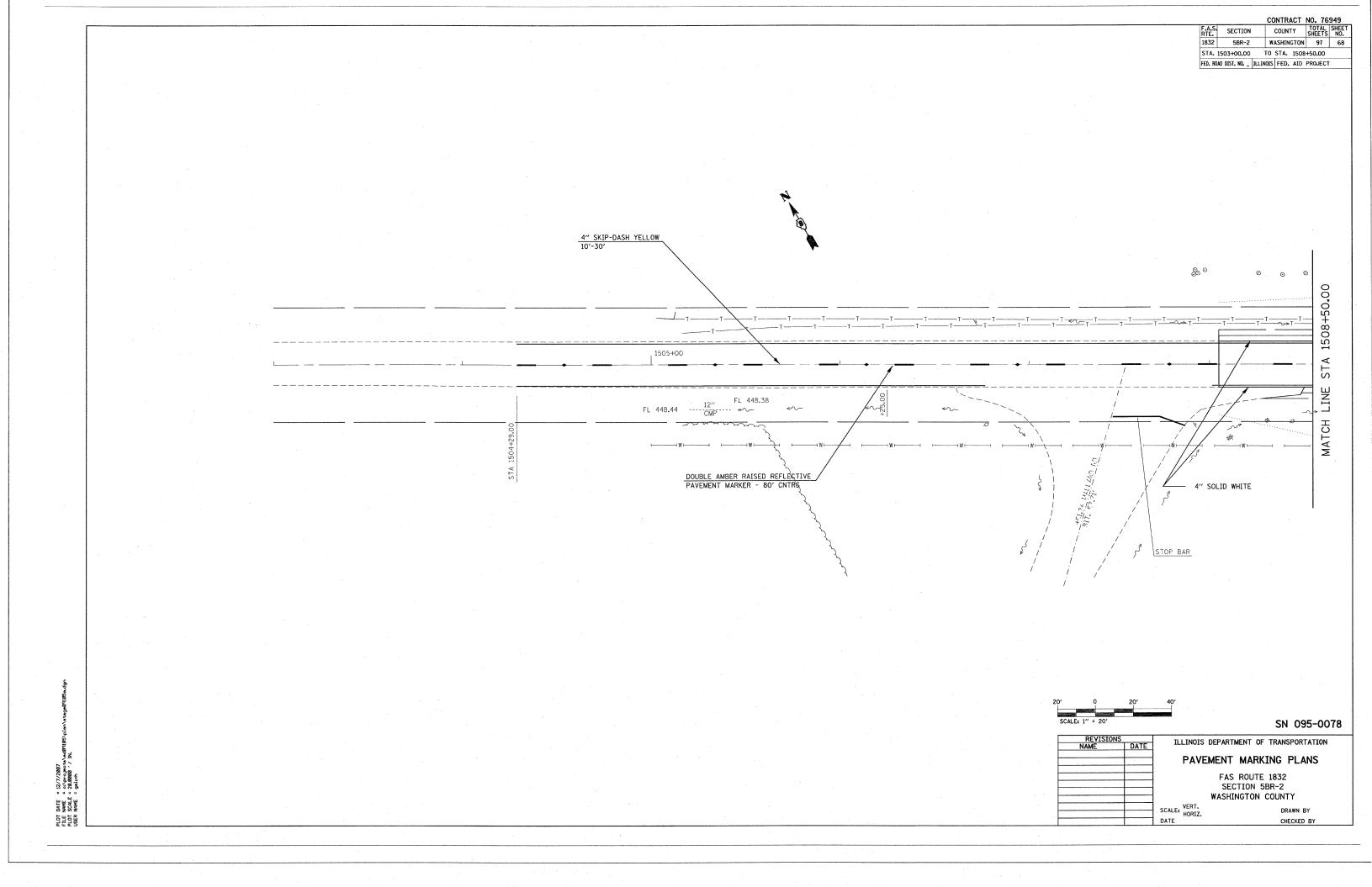
SCALE: VERT. DATE

FAS ROUTE 1832 SECTION 5BR-2 WASHINGTON COUNTY DRAWN BY

CHECKED BY







STA. 1508+50.00 TO STA. 1513+50.00 FED. ROAD DIST. NO. \_ ILLINOIS FED. AID PROJECT 4" SKIP-DASH YELLOW 10'-30' DOUBLE AMBER RAISED REFLECTIVE
PAVEMENT MARKER - 80' CNTRS DOUBLE AMBER RAISED REFLECTIVE
PAVEMENT MARKER - 80' CNTRS PLUM CREEK BRANCH 1508+50.00 1510+00 STA MATCH LINE PLUM CREEK BRANCH 4" SOLID WHITE 4" SOLID WHITE 4" SKIP-DASH YELLOW 10'-30'

SCALE: 1" = 20'

SN 095-0078

CHECKED BY

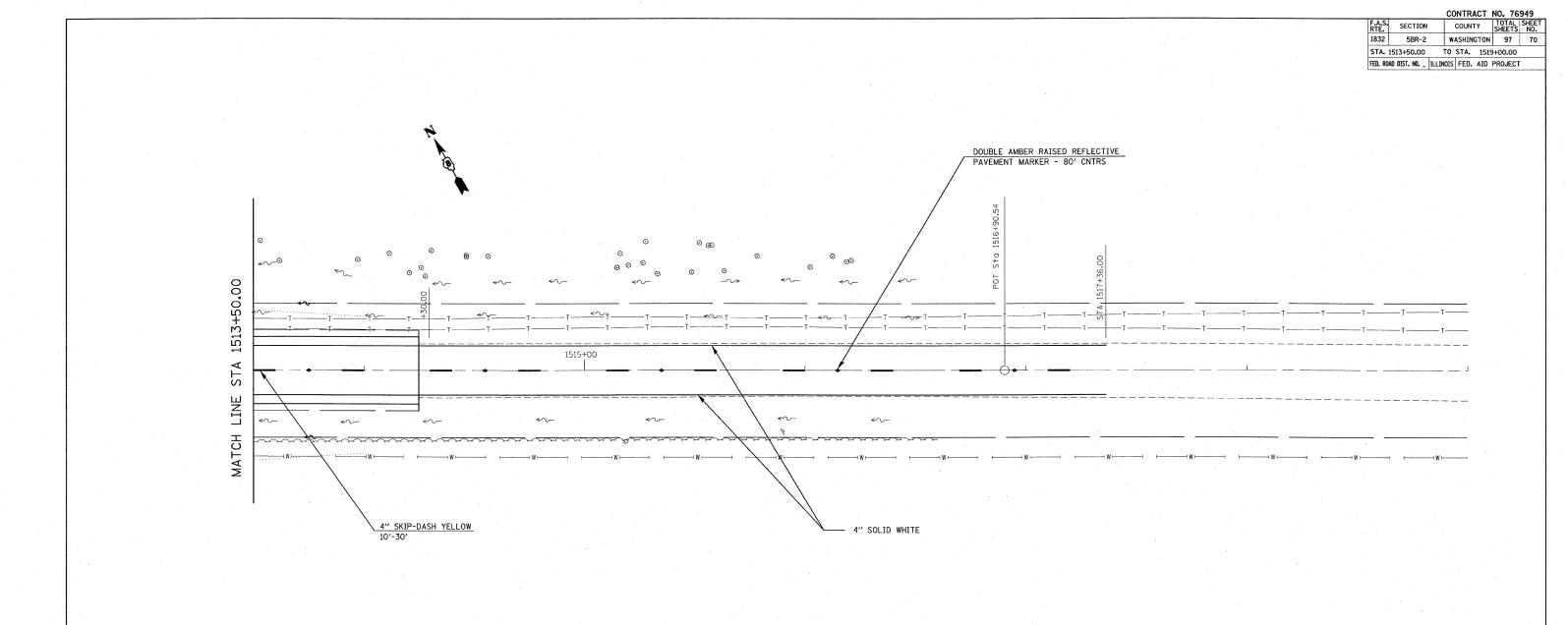
CONTRACT NO. 76949

COUNTY TOTAL SHEET NO.

REVISIONS		ILLINOIS DEPARTMENT OF TRANSPORTATION
NAME	DATE	TEETHOIS DELANTMENT OF TRANSFORTATION
		PAVEMENT MARKING PLANS
		FAS ROUTE 1832
		SECTION 5BR-2
		WASHINGTON COUNTY
		VERT.

HINGTON COUNTY SCALE: VERI. HORIZ. DATE

PLOT DATE = 12/7/2007 FILE NAME = c:\pro|eots\edgy PLOT SCALE = 20.0000 '/ IN. USER NAME = gelinh





SN 095-0078

	PAVEMENT	MARKING	PLANS
	a. S		
	FAS	ROUTE 1832	
		TION SRD-2	

SECTION 5BR-2 WASHINGTON COUNTY

ILLINOIS DEPARTMENT OF TRANSPORTATION

SCALE: VERT. DATE

DRAWN BY CHECKED BY

PLOT DATE = 12/7/2007 FILE NAME = orkprojects\cd091 PLOT SCALE = 20,0000 '/ IN, USER NAME = geluch

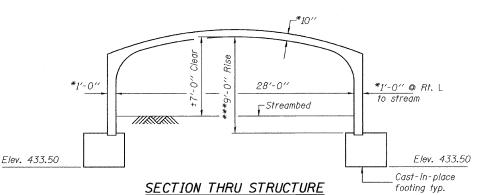
Bench Mark: Chiseled square on bridge abutment @ NW corner of structure 095-0007 Sta. 1510+84.7, 18.4 ft. Left, Elev. 445.40 Existing Structure: S.N. 095-0007 Built in 1921 as S.B.I. Route 15, Section 5B at Station 1510+80 as a 1 span RC slab bridge. Closed abutments on spread footings. 1971 superstructure replacement, and widening, with PPC deck beams. Existing bridge to be removed and replaced. Traffic maintained using stage construction. ⊈ IL. Rte. 160 Steel Bridge Rail Curb Mounted (2399) No salvaae 32'-0" Face To Face D/S Streambed 12'-0" 12'-0" 4'-0" U/S Streambed Elev. 439.72 Elev. 439.72 \_P.G. <u>³<sub>16\_''</sub>'</u>/' Precast wing wall D.H.W. Elev. 446.68 (typical) Elev. 433.50 Elev. 433.50 Cast-in-place footing LONGITUDINAL SECTION Stone Riprap Class A5 typ. Temporary Soil Flow New Channel (proposed) Realignment -Sta. 1511+01.00 Existing footing & (see Roadway Plans) Elev. 449.08 💠 Wingwall removal. Cost included with Removal Boring Bof Existing Structures Limits of . Geotextile - Existing Retaining Wall Structure Traffic Barrier Terminal Type 6A--1.88% -0.07% Std. 631032 typ. Boring B-2 Existing Telephone 35'-0" Out to Out of Headwa. Line Stage II Const. Note: Channel diversion shall be approved by Field Engineer. Temporary Soil LVC = 285' PROFILE GRADE (F.A.S. Rte. 1832) PLAN

EXPIRES 11-30-2008

#### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

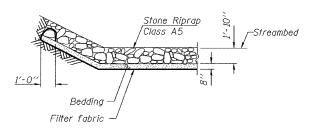
ROUTE NO.	SECTION	cor	JMTY	TOTAL SHEETS	SHEET NO.	SHEET NO. $1$
F.A.S. 1832	5BR-2	WASHINGTON		97	71	12 SHEETS
FED. ROAD DIST. NO. 7 ILLINOIS FED. AID PR			OJECT-			

Contract #76949



#### INDEX OF SHEETS

- 1 General Plan and Elevation
- Stage Construction Details
- Temporary Soil Retention System
- Geotextile Retaining Wall Temporary Concrete Barrier
- 6-7 Footing Details
- Winawall Details
- 9 Headwall Details
- 10 Bar Splicer Details
- 11 Steel Bridge Rail Details
- 12 Soil Boring Logs



#### DESIGN SCOUR **ELEVATION TABLE**

\*Slab and wall thickness and shape may vary as per manufacturer's design.

\*\*Stage I west footing to be constructed ±2' into Stage II Construction \*\*\*Based on Con Span sections that have a minimum rise of 9'-0''. Note: The selected structure by the contractor shall provide a hydraulically equivalent waterway opening specified in the waterway information table.

Design Scour	D.S.	U.S.
Elev. (ft.)	437.72	437.72

#### SEISMIC DATA

Seismic Performance Category (SPC) = B Bedrock Acceleration Coefficient (A) = .11g Site Coefficient (S) = 1.5

#### LOADING HS20

Allow 50#/sq. ft. for future wearing surface.

#### DESIGN SPECIFICATIONS 2002 AASHTO

#### DESIGN STRESSES

FIELD UNITS

 $f_{c}' = 3,500 psi$ = 60,000 psi (reinforcement)

#### PRECAST UNITS

 $f_{s}' = 5.000 \text{ psi}$ 

 $f_{\rm V} = 60,000$  psi (reinforcement) = 65,000 psi (welded wire fabric)

#### WATERWAY INFORMATION

Drainage Area	= 0.87	sq. mi.	. Low G	Grade Ele	v. 447.4	19 ft.	<b>⊘</b> Sta.	1510+95	5
Flood	Freq.	Q	Opening	Sq. Ft.	Nat.	Head	- Ft.	Headwo	iter El.
	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
Design	50	741	79.84	139.00	446.68	1.54	1.16	448.22	447.84
Base	100	860	79.84	139.00	446.85	1.58	1.52	448.43	448.37
Exist. Overtop.	<i>1</i> 5	<i>51</i> 6	79.84	N/A	446.17	1.32	N/A	447.49	N/A
Prop. Overtop.	35	670	N/A	139.00	446.54	N/A	0.95	N/A	447.49
Scour	10	466	79.84	139.00	446.03	1.08	0.48	447.11	446.51

#### SECTION A-A

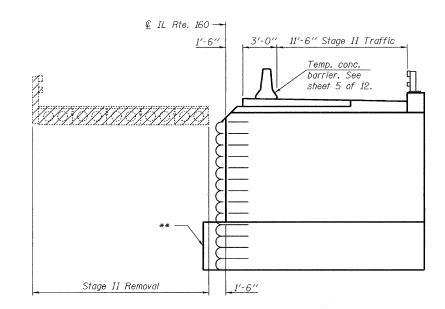
STATION 1511+01.00 BUILT 20 STATE OF ILLINOIS F.A.S. RTE 1832 - SEC. 5BR-2 LOADING HS20 STRUCTURE NO. 095-0078

> NAME PLATE See Std. 515001



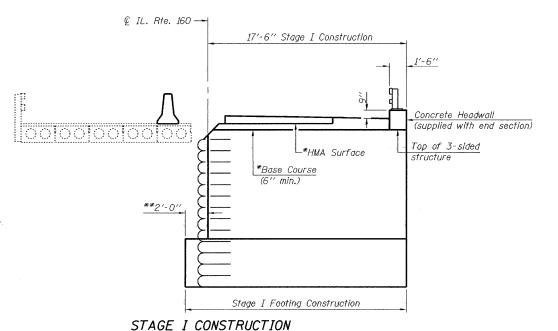
GENERAL PLAN AND ELEVATION IL. RTE. 160 OVER PLUM CREEK BRANCH F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON CO. STATION 1511+01.00 STRUCTURE NO. 095-0078

CHECKED NRB/GRA



#### STAGE II REMOVAL

(Looking East at New West footing at Rt. L to Footing)



\*For quantities of HMA Surface and Base Course, see Roadway Plans.
\*\*Stage I west footing to be constructed

Stage II Construction.

2'-0'' Rt. L to & IL Rte. 160 into

Stage I Removal

← € IL Rte. 160

1'-6"

STAGE I REMOVAL

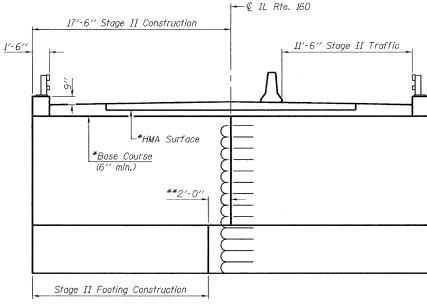
12'-0" Stage I Traffic

Temp. conc.

barrier, See

sheet 5 of 12

(Looking East at New West footing at Rt. L to Footing)



STAGE II CONSTRUCTION

(Looking East at New West footing at Rt. L to Footing)

Note: For quantity of Temporary Concrete Barrier, see Roadway Plans.

TOTAL SHEETS SHEET NO. SHEET NO. 2F.A.S. 1832 12 SHEETS 5BR-2 WASHINGTON 97 72 ILLINOIS FEO. AID PE

Contract #76949

#### GENERAL NOTES

Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60 (IL Modified), See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

The option of using a precast footing is not allowed.

Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.

After the keyways have been grouted and cured, the joints on the three sides of the structure shall be externally sealed using 13" wide external sealing bands conforming to Article 1057.01. Cost included with Three-Sided Precast Concrete Structures.

The footing design is based on the following maximum reactions applied at the top of the footing:

Vertical 15.4 k/ft

Horizontal 6.3 k/ft

The contractor shall verify that the selected structure meets these design parameters. If the design parameters are exceeded, a complete footing design with calculations, details, signed and sealed by an Illinois Licensed Structural Engineer shall be submitted for review and approval.

All construction joints shall be bonded.

Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure. The Contractor shall sawcut the upper portion of the existing abutment at the stage removal line before Stage I removal to ensure the remaining portion will not be prematurely damaged.

Dimensions for the Three-Sided Precast are for a Con Span section. Hy-Span, REDI-SPAN Bridge System and BEBO-Arch System are also acceptable, but dimensions may vary.

It shall be the responsibility of the Contractor to divert the stream flow during construction in order to keep the construction areas free of water. The method of water diversion shall be subject to the approval of the Engineer and the cost shall be included with the cost of "Concrete Structures",

Structural Seal does not include design of Precast elements.

For backfilling and embankment, see Standard Specifications.

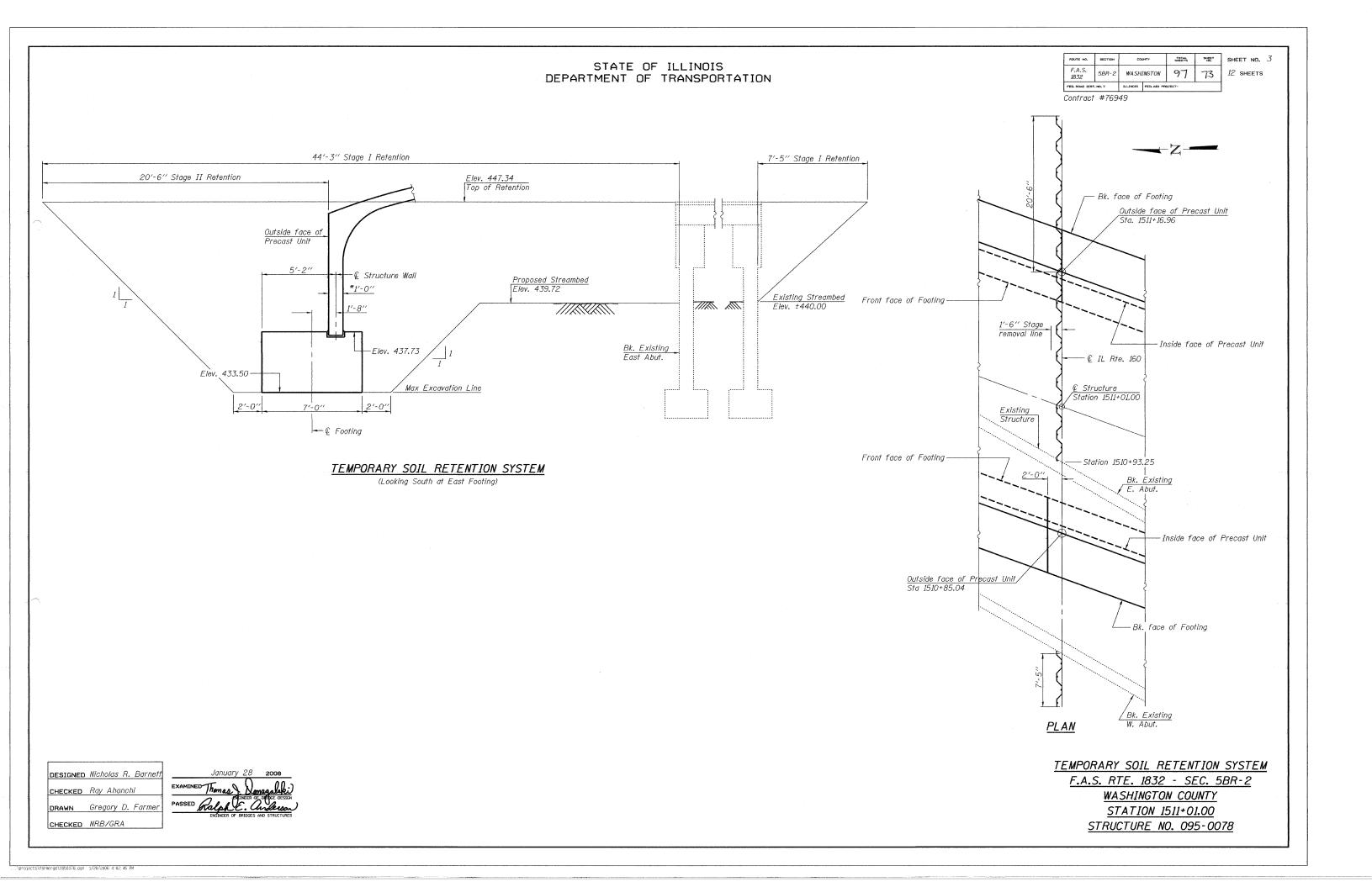
All exposed edges shall be chamfered  $\frac{3}{4}$ ". Allowable Bearing Pressure for Footing = 4 ksf.

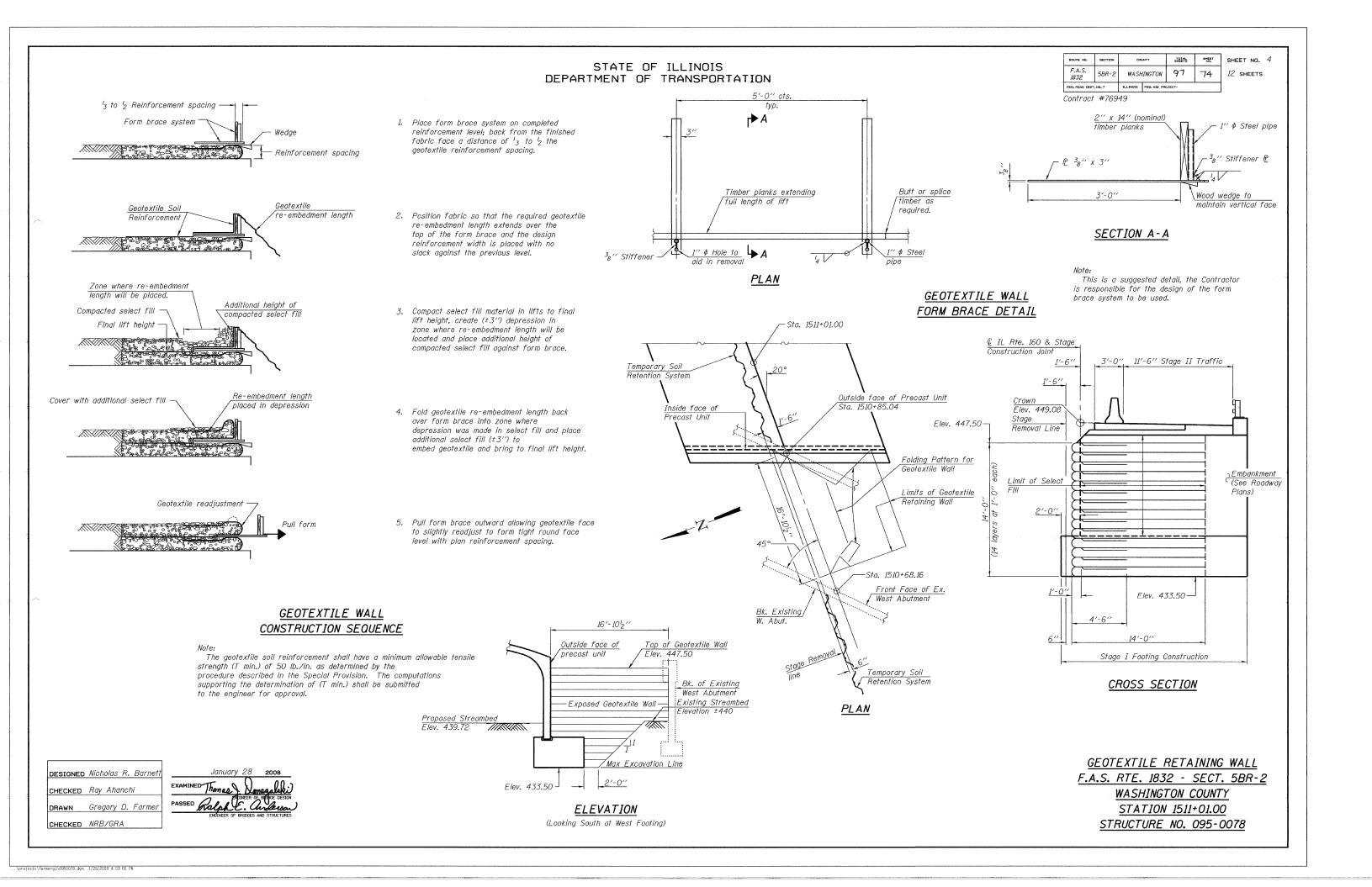
#### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Removal of Existing Structures No. 2	Each			1
Name Plates	Each	1		1
Stone Riprap, Class A5	Sq. Yd.			340
Filter Fabric	Sq. Yd.			340
Concrete Structures	Cu. Yd.		145.0	145.0
Reinforcement Bars, Epoxy Coated	Pound		6590	6590
Bar Splicers	Each		28	28
Structure Excavation	Cu. Yd.		<i>3</i> 65	365
Three Sided Precast Concrete Structure, 28' x 9'	Foot	37.25		37.25
Temporary Soil Retention System	Sq. Ft.			444.1
Steel Rail, Type 2399	Foot	64		64
Geotextile Retaining Wall	Sq. Yd.			19.1
Precast Concrete Substructure	L. Sum		1	1
Concrete Sealer	Sq. Ft.	144		144

STAGE CONSTRUCTION DETAILS F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1511+01.00 STRUCTURE NO. 095-0078

DESIGNED Nicholas R. Barnet CHECKED Ray Ahanchi DRAWN Gregory D. Farmer CHECKED NRB/GRA

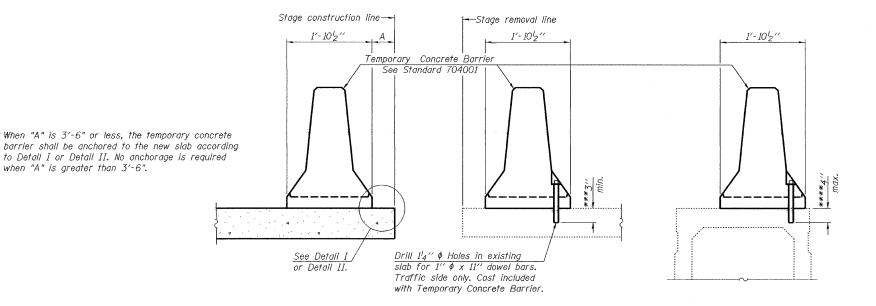




EXISTING DECK BEAM

TOTAL SHEETS SHEET NO. SHEET NO. 5 F.A.S. 1832 97 75 12 SHEETS 5BR-2 WASHINGTON

Contract #76949



#### NOTES

Detail I - With Bar Splicer or Couplers: Connect one (1) 1"x7"x10" steel P to the top layer of couplers with 2-58" \$\phi\$ bolts screwed to coupler at approximate & of each barrier panel.

Detail II - With Extended Reinforcement Bars:

Connect one (1) 1"x7"x10" steel £ to the concrete slab or concrete wearing surface with 2- $^58''\phi$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate € of each barrier panel.

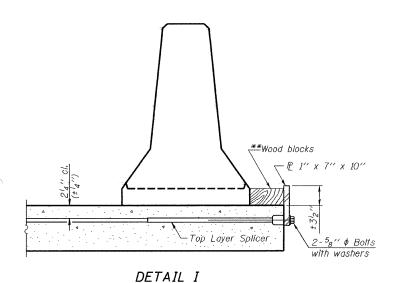
Cost of anchorage is included with Temporary Concrete Barrier. The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

#### SECTIONS THRU SLAB OR DECK BEAM

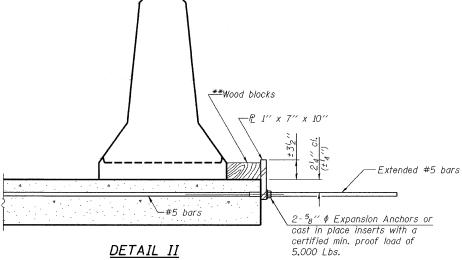
EXISTING SLAB

\*\*\*Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth. \*\*\*\*If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not

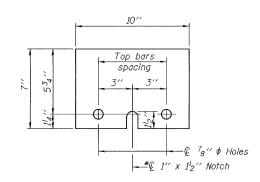
into existing deck beam concrete.



NEW SLAB



<sup>\*\*</sup>Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



### STEEL RETAINER P 1" x 7" x 10"

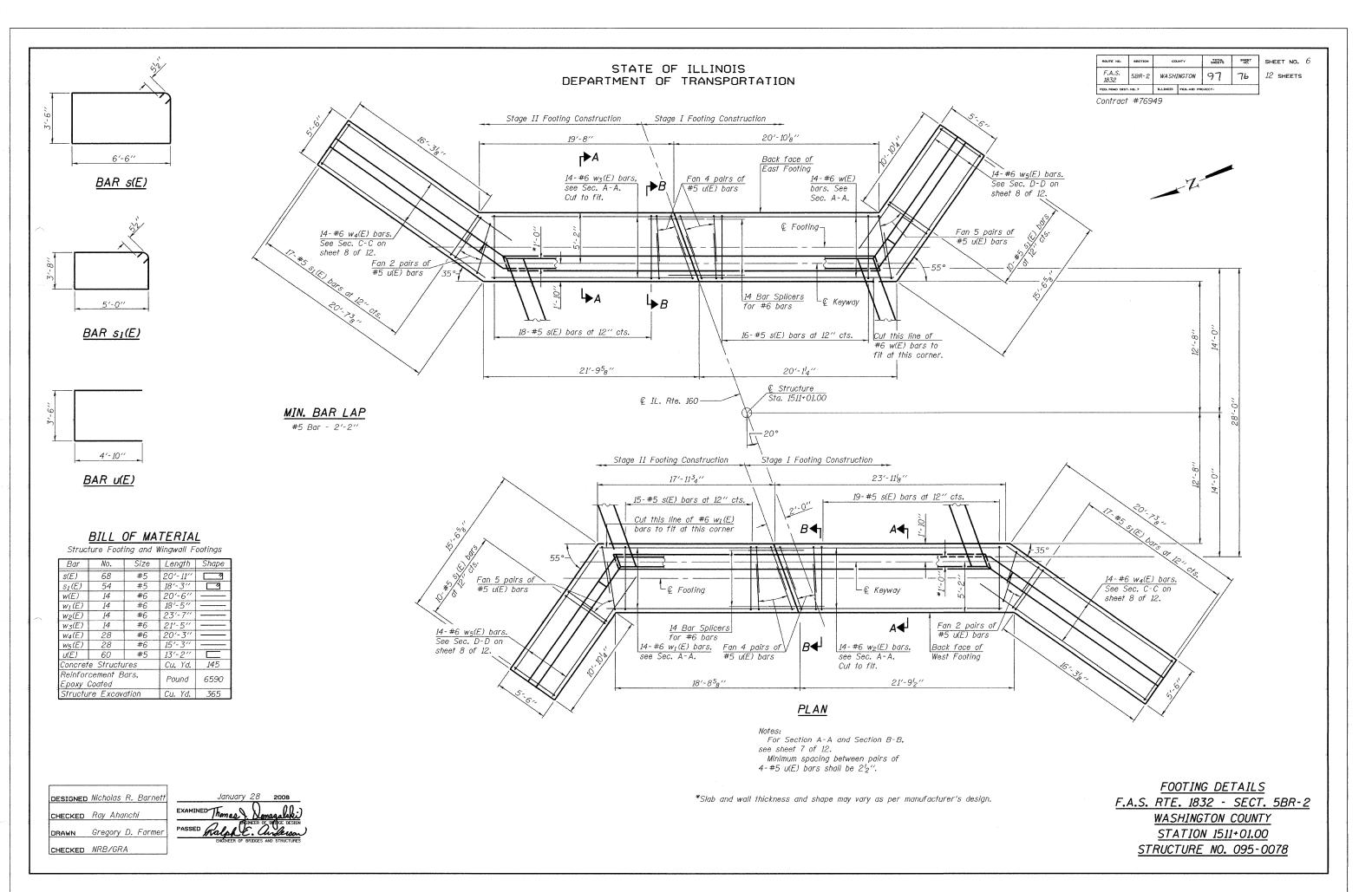
\* Required only with Detail II

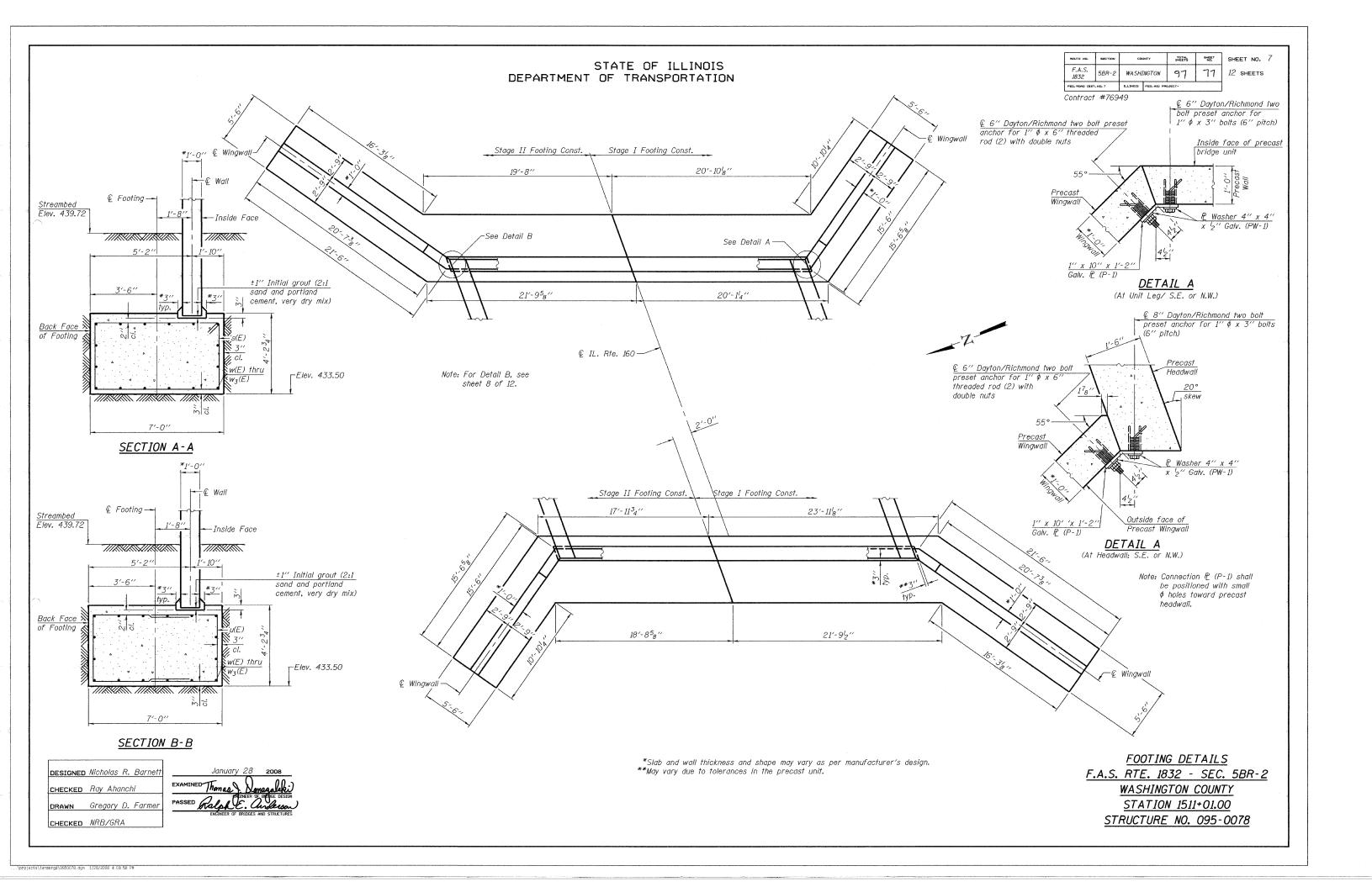
TEMPORARY CONCRETE BARRIER F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1511+01.00 STRUCTURE NO. 095-0078

DESIGNED Nicholas R. Barnett CHECKED Ray Ahanchi DRAWN Gregory D. Farme

CHECKED NRB/GRA R-27 9-3-07

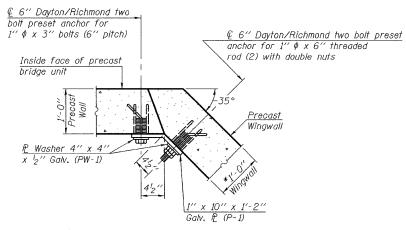
when "A" is greater than 3'-6".





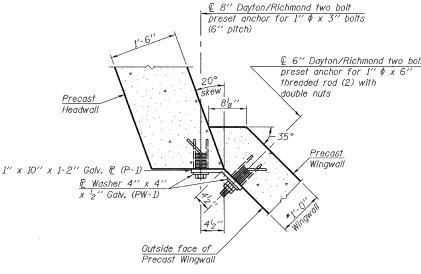


Contract #76949

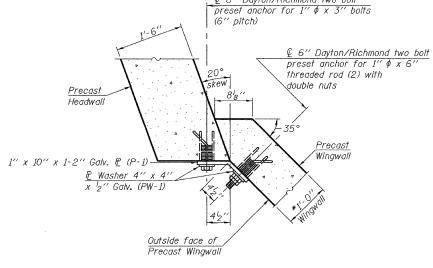


### DETAIL B

(At Unit Leg; S.W. or N.E.)



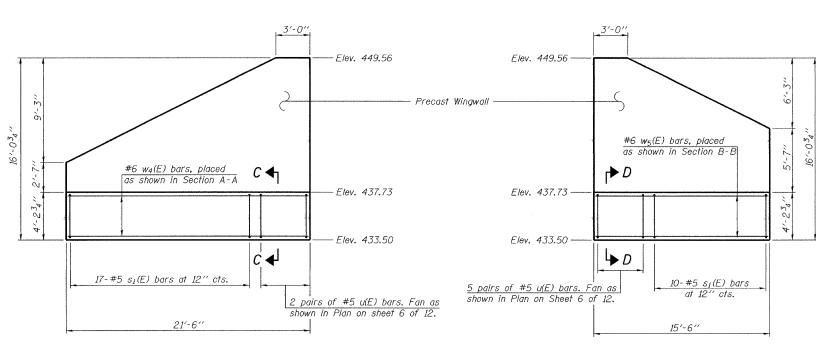
Note: Connection & (P-1) shall be positioned with small φ holes toward precast



DETAIL B

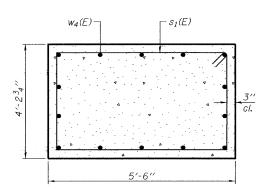
headwall.

PRECAST WINGWALL DETAILS F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1511+01.00 STRUCTURE NO. 095-0078



#### NORTHEAST AND SOUTHWEST WINGWALLS

(Looking Inside Face)



SECTION C-C

Note: Reinforcement for wingwalls is to be

determined by the fabricator. Precast Wingwall & associated hardware is billed as Precast Concrete Substructure, L. Sum.

SECTION D-D

5′-6″

NORTHWEST AND SOUTHEAST WINGWALLS

(Looking Inside Face)

 $-s_1(E)$ 

 $w_5(E)$ -

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DESIGNED Nicholas R. Barnet

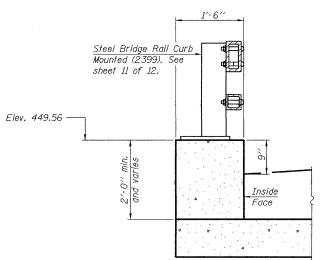
DRAWN Gregory D. Farmer

CHECKED Ray Ahanchi

CHECKED NRB/GRA

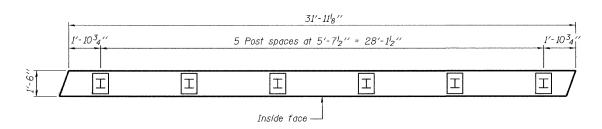
| ROUTE NO. | SECTION | COLATY | STOTAL SHEET NO. | F.A.S. | SBR-2 | WASHINGTON | 97 | 79 | 12 SHEETS | FED. RONG CIST. NO. 7 | BLERGS | FED. ADD PROJECT-

Contract #76949

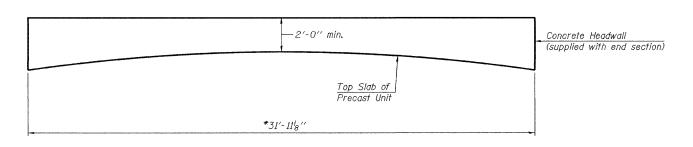


Note: Headwall to be designed by precast provider.
Concrete Sealer shall be applied to top and
inside exposed face of Precast Headwall.
Headwall shall be designed per AASHTO
Art. 2.7 of Standard Specifications.

SECTION THRU HEADWALL



PLAN OF HEADWALL POST LAYOUT



PRECAST HEADWALL ELEVATION

\*Dimension measrured along inside face of headwall

DESIGNED Nicholas R. Barnett
CHECKED Ray Ahanchi
DRAWN Gregory D. Farmer
CHECKED NRB/GRA

EXAMINED Thomas Longsalki
PASSED Ralph E. Children
ENTINEER OF BRIDGES AND STRUCTURES

HEADWALL DETAILS

F.A.S. 1832 - SEC. 5BR-2

WASHINGTON COUNTY

STATION 1511+01.00

STRUCTURE NO. 095-0078



Contract #76949

**NOTES** 

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum Capacity = 1.25 x fy x A<sub>t</sub>

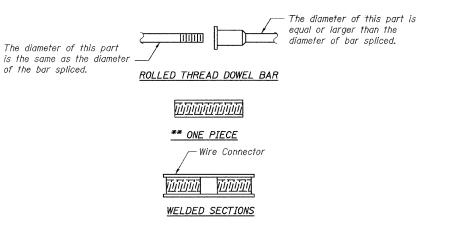
(Tension in kips) = 1.25 x fy x A<sub>t</sub>

Minimum \*Pull-out Strength = 0.66 x fy x A<sub>t</sub>

Where fy = Yield strength of lapped reinforcement bars in ksi.

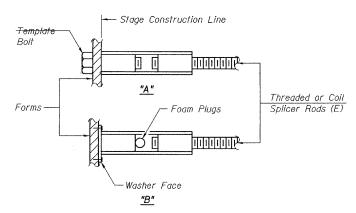
 $A_t$  = Tensile stress area of lapped reinforcement bars. \* = 28 day concrete

BAR SPLICER ASSEMBLIES									
		Strengt	h Requirements						
be Spliced	ar Size to Splicer Rod or Dowel Bar Length		Min. Pull-Out Strength kips - tension						
#4	1'-8''	14.7	7.9						
#5	2'-0''	23.0	12.3						
#6	2'-7''	33.1	17.4						
#7	3′-5″	45.1	23.8						
#8	4'-6''	58.9	<i>31.3</i>						
#9	5′-9″	75.0	39.6						
#10	7′-3′′	95.0	50.3						
# <u>11</u>	9′-0′′	117.4	61.8						



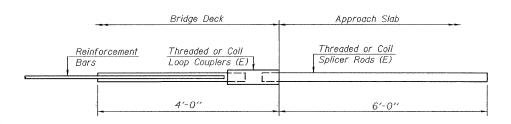
#### BAR SPLICER ASSEMBLY ALTERNATIVES

\*\* Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



#### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.



#### FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

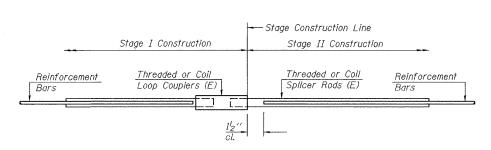
Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required =

DESIGNED Nicholas R. Barnett CHECKED Ray Ahanchi DRAWN Gregory D. Farme. CHECKED NRB/GRA

11-1-06

6'-0" Approach slab Abutment hatch block Threaded or Coil Threaded or Coil Splicer Rods (E) Loop Couplers (E) Reinforcement bars FOR STUB **ABUTMENTS** Bar Splicer for #5 bar

Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required =



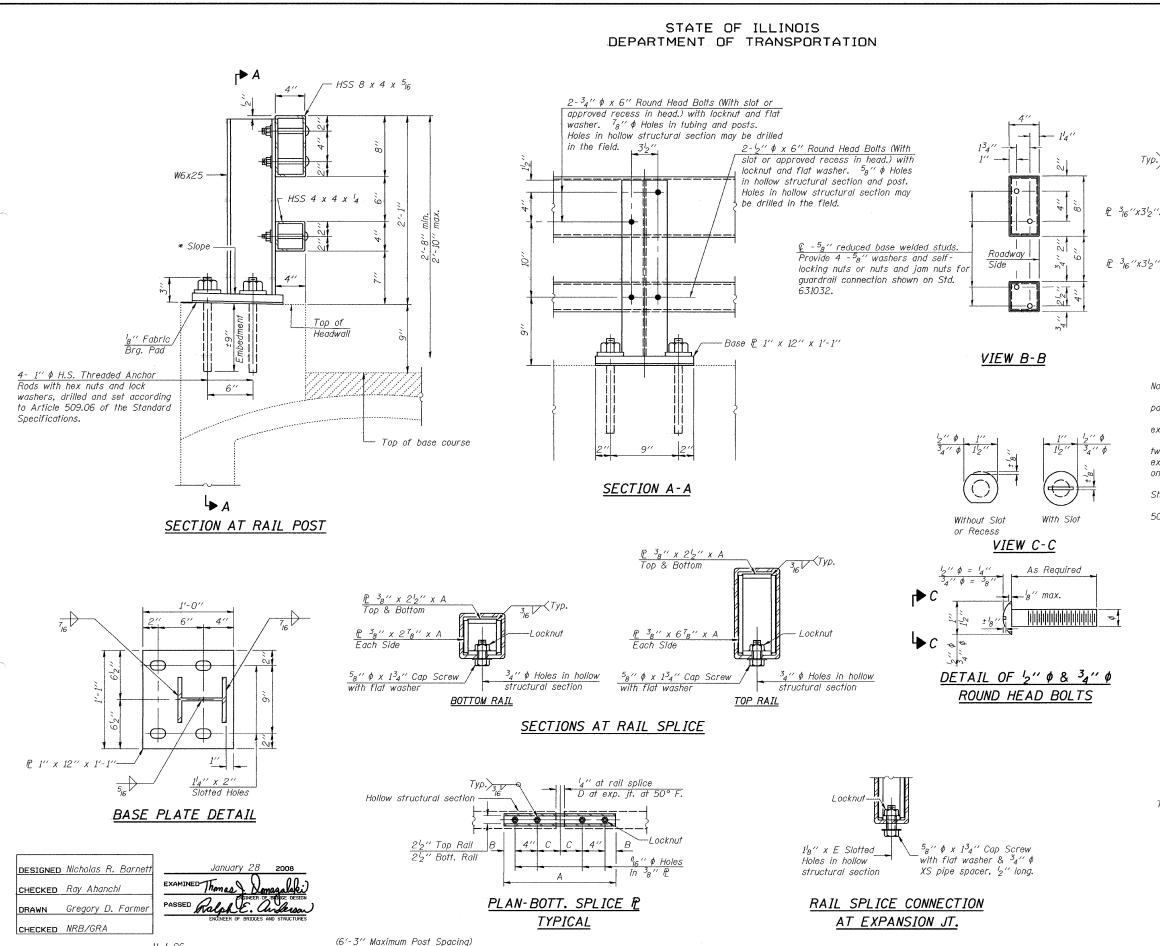
#### STANDARD

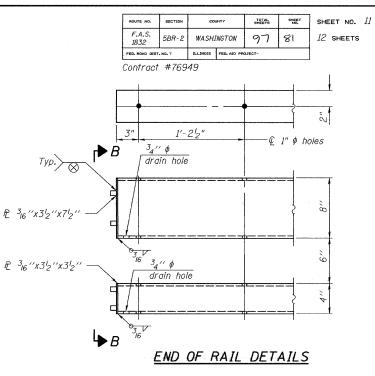
Bar Size	No. Assemblies Required	Location
#6	28	Footing

BAR SPLICER ASSEMBLY DETAILS F.A.S. 1832 - SEC. 5BR-2 WASHINGTON COUNTY STATION 1511+01.00 STRUCTURE NO. 095-0078

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BSD-1





Notes:

All field drilled holes shall be coated with an approved zinc rich paint before erection.

Posts shall not be located closer than  $1^\prime\text{-}3^{\prime\prime}$  to an existing bridge expansion joint or end of bridge.

Steel Bridge Rail expansion joint shall be provided between any two (2) posts which span a bridge expansion joint. Bolts located at expansion joint shall be provided with locknuts and shall be tightened only to a point that will allow railing movement.

Provide one  $l_{\theta}^{\prime\prime\prime}$  and two  $l_{\theta}^{\prime\prime\prime}$  steel shims for 25% of the posts. Shims shall be similar to base plates in size and holes.

All steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.

#### BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type 2399	Foot	64'-0"

#### SPLICE DIMENSIONS

T	D	Α	В	С	Ε
<i>≤4′′</i>	21/2"	1'-8''	2"	4′′	22"
>4''≤6 <sup>1</sup> 2''	334"	2'-0''	212"	5½"	312"
>6½″≤9″	5′′	2'-4"	312"	6 <sup>l</sup> 2"	9′′
>9′′≤13′′	7''	2'-10''	412"	812"	11''
Rail Splice	4"	1'-8''	2"	4′′	

T = Total movement at expansion joint as shown on the design plans.

STEEL RAILING, TYPE 2399

F.A.S. RTE 1832 - SEC. 5BR-2

WASHINGTON COUNTY

STATION 1511+01.00

STRUCTURE NO. 095-0078

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11-1-06

ROUTE NO.	SECTION	cou	UNTY	TOTAL	SHEET NO.	SHEET NO. 12
F.A.S. 1832	5BR-2	WASHINGTON		97	82	12 SHEETS
FED. ROAD DIST	. NO. 7	ILLINOIS	FED. AID PR	OJECT-		

Contract #76949

Illinois Departn of Transportatio	n			2	OIL BORING LOG	Date	2/	25/7
ROUTE FAS 1832 DESCRIPTION			11 160	) aver	Branch of Plum Creek LOGGED BY		J. Kin	
SECTIONSBR-2 LOC					4. SEC. 27, TWP. 15, RNG. 4W, 3 PN			
			<u> </u>					
COUNTY Washington DRILLING N	_				low Stem Auger HAMMER TYPE		utomatic	_
STRUCT. NO. 095-0078 (P) Station 1510+80	D E	B	U	0	Surface Water Elevft Bream Bed Elevft E	B	C	0
	P	0	5	I S	Show Bed Elex. Table !! P	0	S	I S
BORTING NO. 1 E. Abut Station 1510+90	Н	s	Qu	T	Groundwater Elev.: First Encounter 442.0 ft		Qυ	7
Offset15.00ft Right	(ft)	(/6")	(tsf)	(2)	Upon Completion ft After Hrs. ft (ft)	(/6")	(tsf)	00
	<u> </u>		<del> </del>		Gray SHALE (continued)	_	-	-
Brown SILT						100	<u> </u>	
•	_	3	0.65	23		100+	-	-
	_		5		-	L		
	_					1		
	-5	7	0.58	26	423.5	100+	-	÷
	_	Ľ.	S S	20	Gray LTMESTONE 422.5	100+	┝╌	-
442.0	7				END OF BORING			
Brown and Gray Clay TILL	-	6	107		HOTE Value is Class to Color	-		
		,	1.63 B	20	NOTE: Value in Blows/6" Column is the N-Value of the Sample			
					<del></del>	+		
	10					80	ļ	
		5	0.65 B	25				
436.7	-				-	-		
Gray Clay TILL	_				-	1		
,		3	0.20 B	34		1		
434.2						-		
Brown and Gray Clay TILL	-15					25		
Elown and Groy City 1122		64	<i>3.30</i>	11				
	_					-		
,		-10	1			1		
	-	100	2.57 S	12		+		
	_	-	-		<u> </u>	7		
Gray SHALE 429.2	-20					0		

Illinois Departm of Transportation Division of Highways District - Materials	M			3	OIL BORING LO	Ju		Date	2/2	25/71
ROUTE FAS 1832 DESCRIPTION			IL.	160 ove	Branch of Plum Creek	_ LOGGEE	BY		J. Kin	
SECTIONLOC	A710N		SE 1/4.	SE 1/	1, SEC. 27, TWP. IS, RNG. 4W, 3 PM					
COUNTY Washington DRILLING &	WETHO	D		Ho	Now Stem Auger HAMMER	TYPE		140# A	utomatic	
095-0007 (E) /   STRUCT. NO.	D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev. 439.	ft 72 ft	D E P	B L O	U C S	<b>M</b> 0
BORING NO. 2 W. Abul Station 15,00+70 Offset 15,00f1 Left	T H	S	Qu	S	Groundwater Elev.: First Encounter 435 Upon Completion	.8 ft \		S S	Qu	S T
Ground Surface Elev. 448.52 ft	(11)	(/6")	(tsf)	(%)	After Hrs	ff	(II)	(/67	(tst)	(2)
Brown Silty CLAY	-	-			(continued)		_	100+	-	_
	_	3	0.52	29				100+	-	
444.0			В				_			
Brown and Gray Clay TILL	-5					423.	<u></u>	100+	-	·
Brown and dray clay TILE	_	IJ	1.30 B	22	Gray LIMESTONE	422.		100+	-	-
	_				END OF BORING					
		10	LII B	22	NOTE: Value in Blows/6" Column is the N-Value of the Sample					
	-10			22			-30			
		5	1.10 S	-22			_			
	<b>T</b> -	li	0.39	20			_			
434.0			В	-						
Gray TILL	-15	40	5.54 S	Ш			-35			
429.8		31	3.12 S	11						
Gray Laminated SHALE	-20						-40			

DESIGNED Nicholas R. Barnett

CHECKED Ray Ahanchi

DRAWN Gregory D. Farmer

CHECKED NRB/GRA

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EXAMINED Thomas Lomanuki PASSED Rulph E. Curdenson
ENGINEER OF BRIDGES AND STRUCTURES

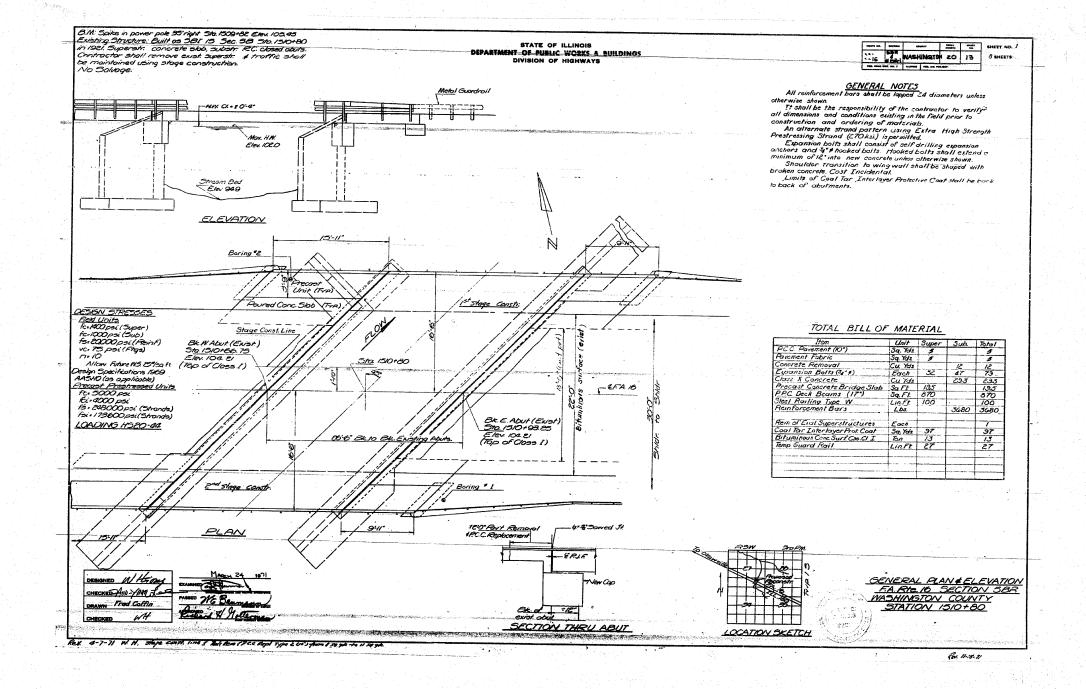
SOIL BORING LOGS

F.A.S. RTE. 1832 - SEC. 5BR-2

WASHINGTON COUNTY

STATION 1511+01.00

STRUCTURE NO. 095-0078



FOR INFORMATION ONLY

S.N. 095-0078

ILLINOIS DEPARTMENT OF TRANSPORTATION

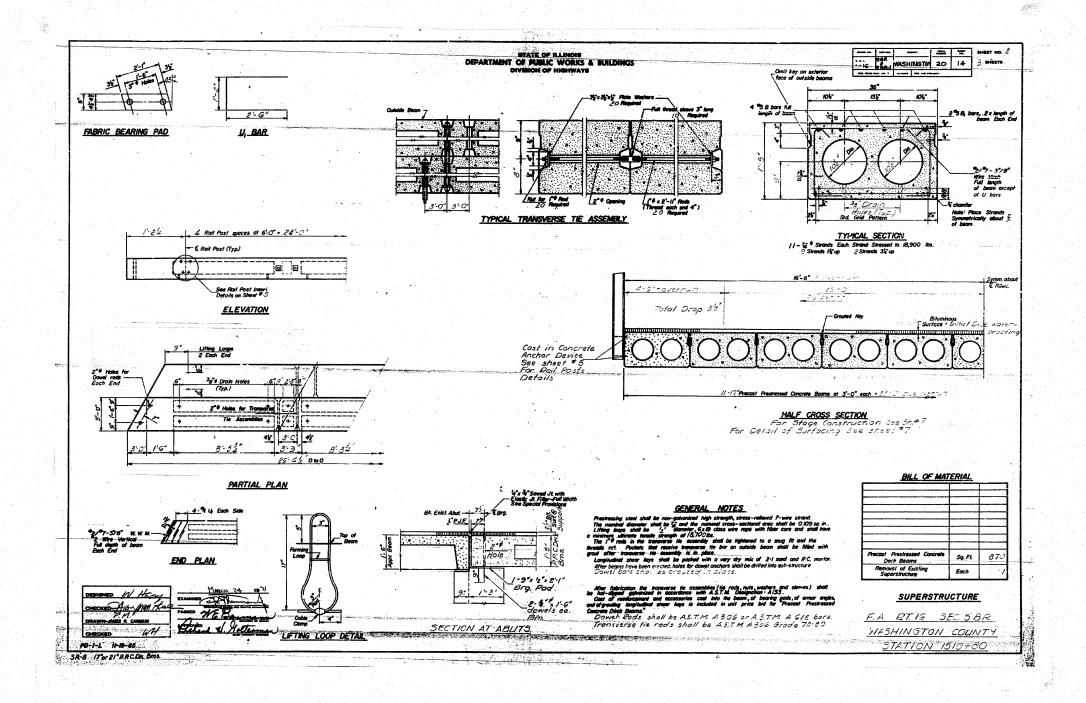
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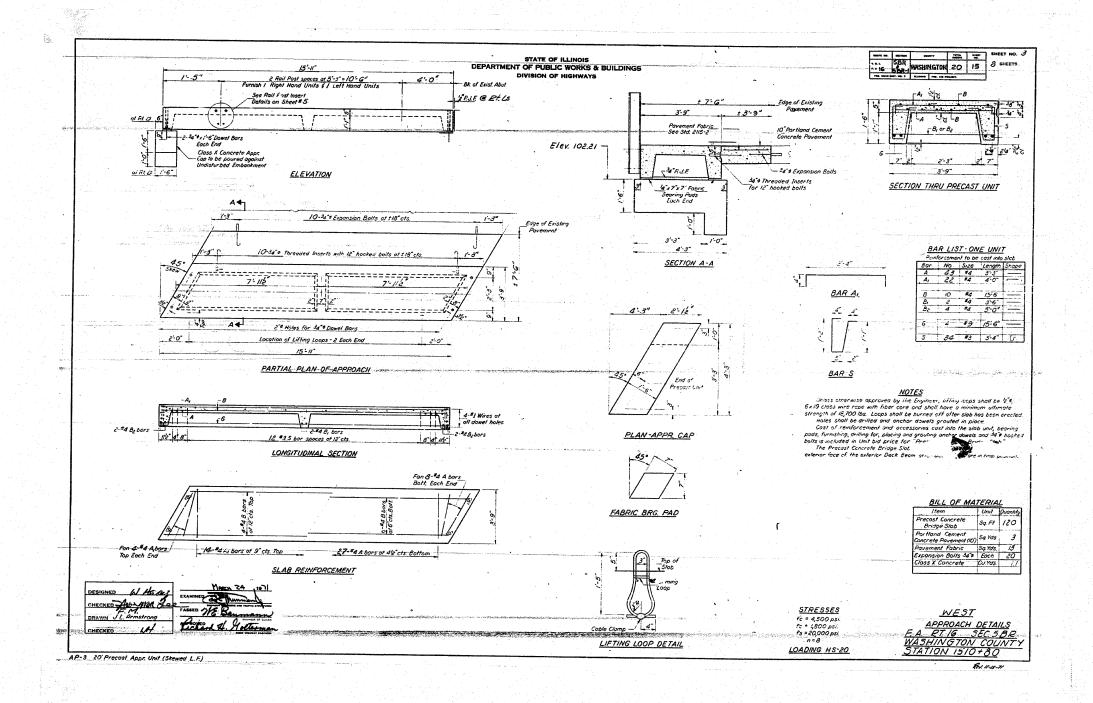
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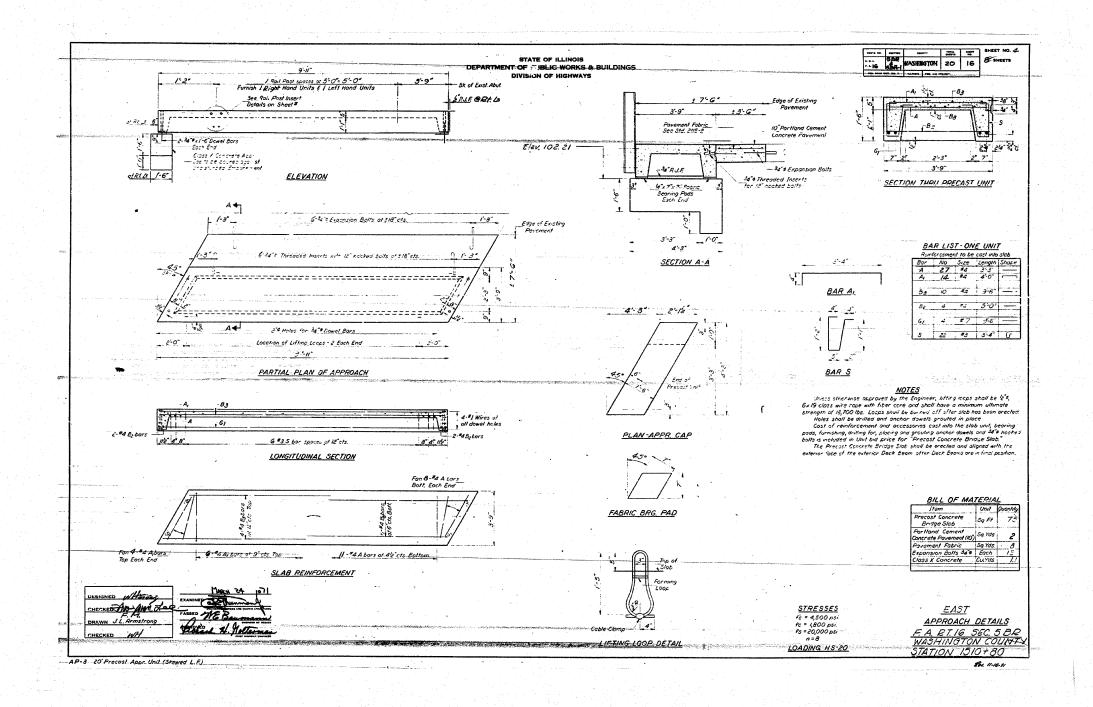
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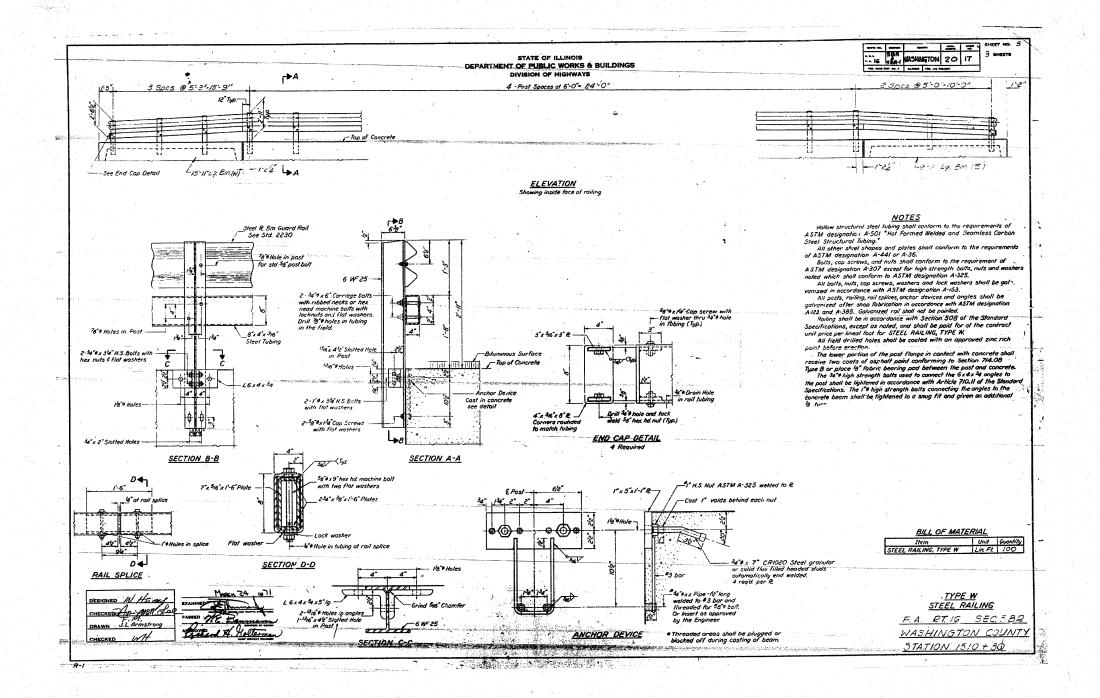
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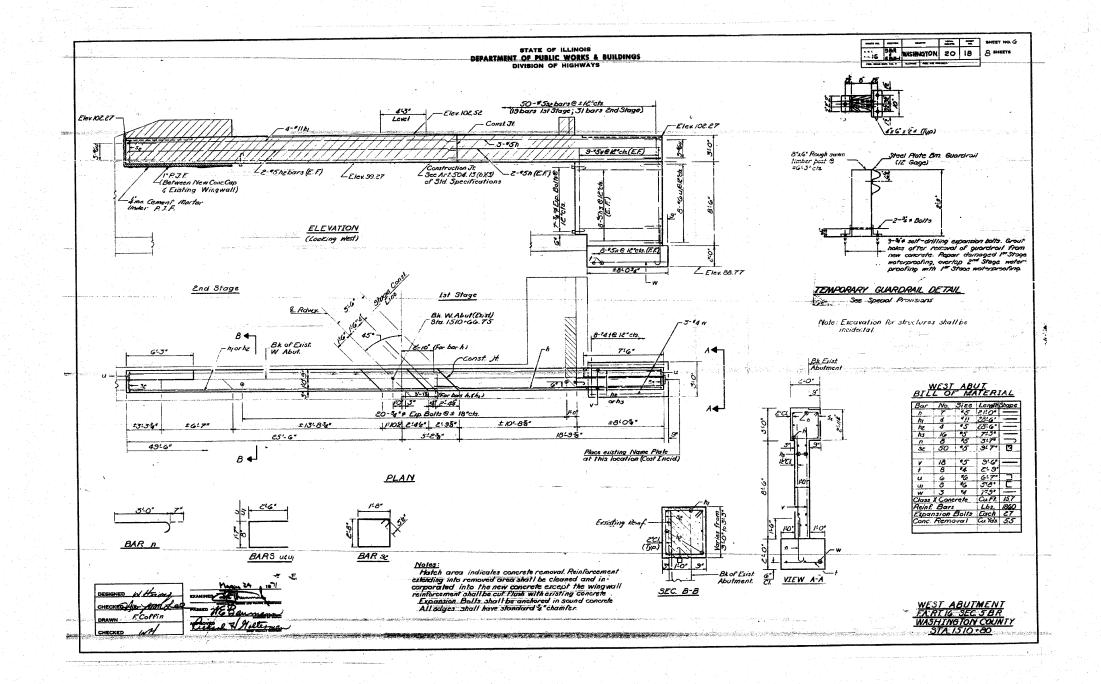
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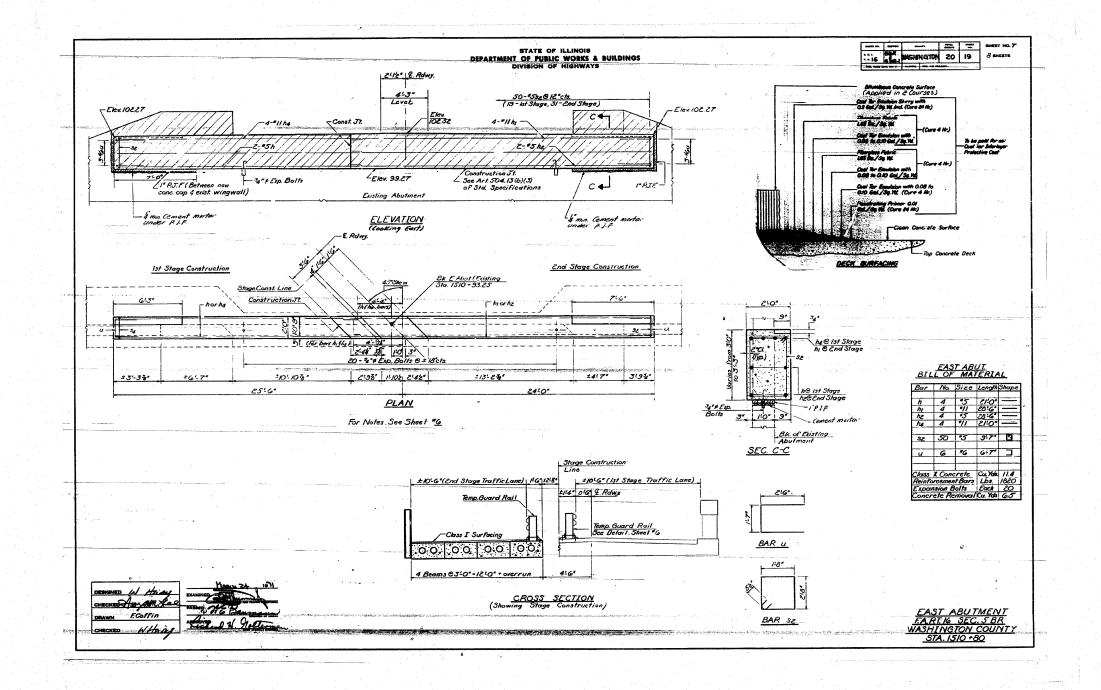
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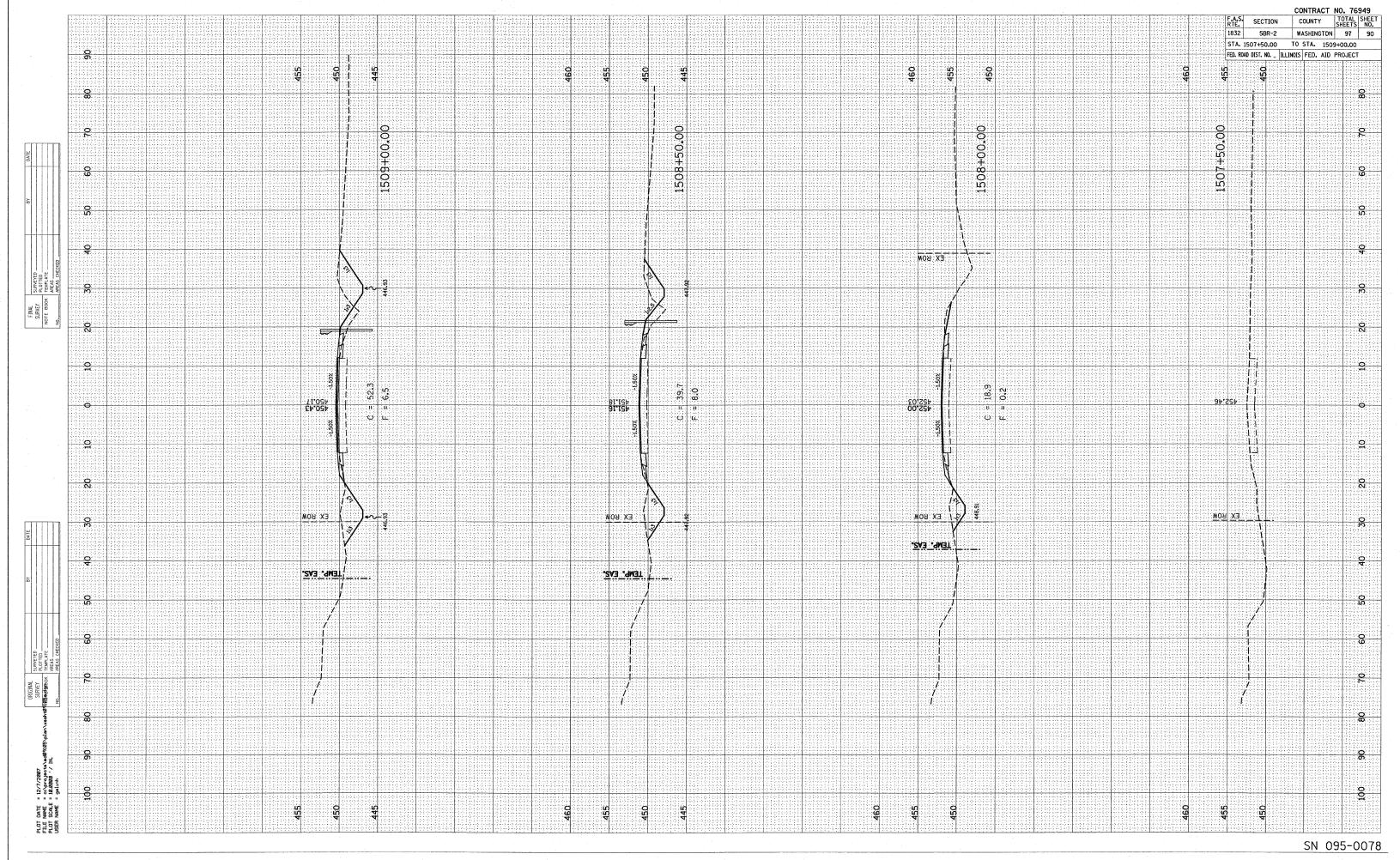
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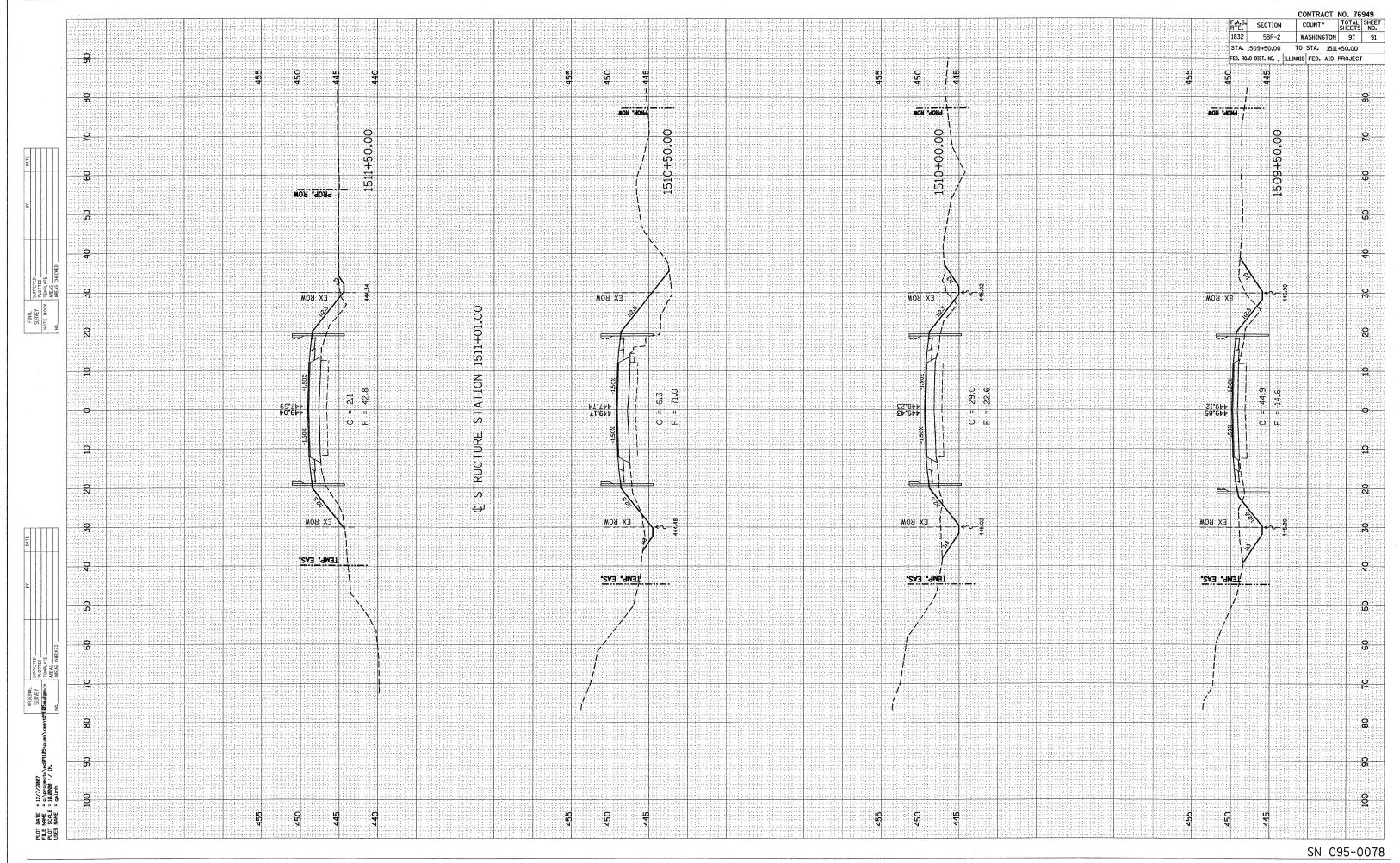
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