

	0.4 Sp. 1 or 0.6 Sp. 3	Pier #1 or Pier #2	0.5 Span 2
$I_s$	(in <sup>4</sup> ) 9807	20646	9807
$I_c(n)$	(in <sup>4</sup> ) 24491	-	24491
$I_c(3n)$	(in <sup>4</sup> ) 18436	-	18436
$S_s$	(in <sup>3</sup> ) 473	949	473
$S_c(n)$	(in <sup>3</sup> ) 661	-	661
$S_c(3n)$	(in <sup>3</sup> ) 606	-	606
DC1	(k/ft) 0.800	0.899	0.800
M <sub>DC1</sub>	(k) 390	969	300
DC2	(k/ft) 0.025	0.025	0.025
M <sub>DC2</sub>	(k) 13	26	12
DW	(k/ft) 0.300	0.300	0.300
M <sub>DW</sub>	(k) 160	309	145
M <sub>ℓ + Imp</sub>	(k) 1086	1115	1121
M <sub>u</sub> (Strength I)	(k) 2644	3659	2569
$\phi_r M_n, \phi_r M_{nc}$	(k) 3297	-	3359
$f_s$ DC1	(ksi) 9.9	12.3	7.6
$f_s$ DC2	(ksi) 0.3	0.3	0.2
$f_s$ DW	(ksi) 3.2	3.9	2.9
$f_s$ 1.3(ℓ+I)	(ksi) 25.6	18.3	26.5
$f_s$ (Service II)	(ksi) 39.0	34.8	37.2
$f_s$ (Total)(Strength I)	(ksi) -	46.2	-
V <sub>r</sub>	(k) 32	-	35

	Abuts.	Piers
R <sub>DC1</sub>	(k) 25.6	93.6
R <sub>DC2</sub>	(k) 0.8	2.8
R <sub>DW</sub>	(k) 9.8	33.3
R <sub>ℓ + Imp</sub>	(k) 76.8	137.9
R <sub>Total</sub>	(k) 113	267.6

	Girder #1	Girder #2	Girder #3	Girder #4	Girder #5	Girder #6
℄ Brg. S. Abut.	590.754	590.850	590.946	590.946	590.850	590.754
℄ Splice #1	590.968	591.065	591.161	591.161	591.065	590.968
℄ Brg. Pier #1	591.067	591.164	591.260	591.260	591.164	591.067
℄ Splice #2	591.171	591.268	591.364	591.364	591.268	591.171
℄ Splice #3	591.501	591.598	591.694	591.694	591.598	591.501
℄ Brg. Pier #2	591.617	591.714	591.810	591.810	591.714	591.617
℄ Splice #4	591.728	591.825	591.921	591.921	591.825	591.728
℄ Brg. N. Abut.	592.191	592.287	592.384	592.384	592.287	592.191

\*\* For Fabrication only

$I_s, S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$  (Total-Strength I, and Service II) due to non-composite dead loads (in.<sup>4</sup> and in.<sup>3</sup>).

$I_c(n), S_c(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing  $f_s$  (Total-Strength I, and Service II) due to short-term composite live loads (in.<sup>4</sup> and in.<sup>3</sup>).

$I_c(3n), S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in.<sup>4</sup> and in.<sup>3</sup>).

DC1: Un-factored non-composite dead load (kips/ft.).

M<sub>DC1</sub>: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M<sub>DC2</sub>: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M<sub>DW</sub>: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M<sub>ℓ + Imp</sub>: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M<sub>u</sub> (Strength I): Factored design moment (kip-ft.).  
1.25 (M<sub>DC1</sub> + M<sub>DC2</sub>) + 1.5 M<sub>DW</sub> + 1.75 M<sub>ℓ + Imp</sub>

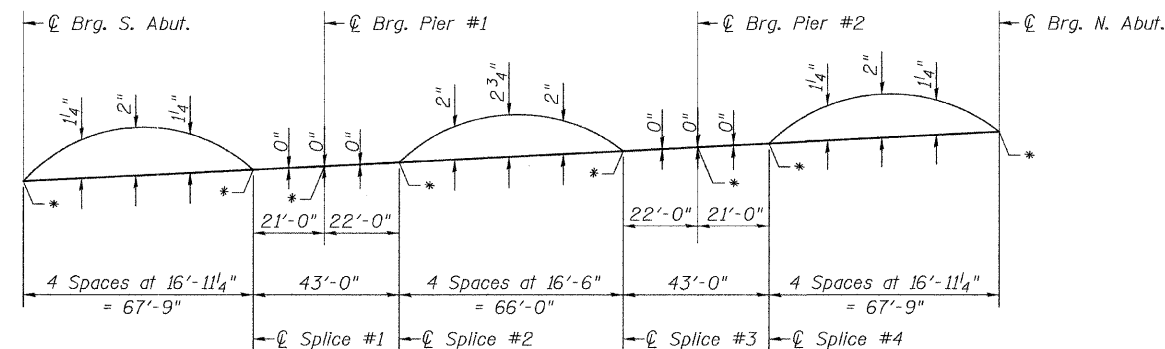
$\phi_r M_n$ : Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

$\phi_r M_{nc}$ : Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).

$f_s$  (Service II): Sum of stresses as computed from the moments below (ksi).  
M<sub>DC1</sub> + M<sub>DC2</sub> + M<sub>DW</sub> + 1.3 M<sub>ℓ + Imp</sub>

$f_s$  (Total)(Strength I): Sum of stresses as computed from the moments below on non-compact section (ksi).  
1.25 (M<sub>DC1</sub> + M<sub>DC2</sub>) + 1.5 M<sub>DW</sub> + 1.75 M<sub>ℓ + Imp</sub>

V<sub>r</sub>: Factored shear range computed according to Article 6.10.10.



CAMBER DIAGRAM

\* See Table for Top of Web Elevations.