INTERIOR GIRDER MOMENT TABLE						
		0.4 Sp. 1	Pier 3	0.5 Sp. 2	Pier 4	0.6 Sp. 3
I_s	(in^4)	28062	26007	26007	26007	28062
$I_c(n)$	(in^4)	63940	-	-	-	63940
$I_c(3n)$	(in^4)	47113	-	-	-	47113
S_s	(in^3)	1095	972	972	972	1095
$S_c(n)$	(in^3)	1450	-	-	-	1450
$S_c(3n)$	(in^3)	1328	-	-	-	1328
$\mathcal{D}$	(k/')	1.03	1.72	1.72	1.72	1.03
$M_{\mathcal{D}}$	('k)	1086	1485	704	1232	1005
$s_{\mathcal{D}}$	(k/')	.71	-	-	-	.71
$M_{s\mathcal{D}}$	('k)	837	-	-	-	596
M_L	('k)	1050	546	350	496	974
M_{imp}	('k)	224	132	85	120	208
$\frac{5}{3}[M_L + M_{imp}]$	('k)	2126	1131	723	1025	1968
M_a	('k)	5264	3401	1856	2935	4640
f_s $\mathcal{D}$ non-comp	(ksi)	11.9	18.3	8.7	15.2	11.0
f_s $\mathcal{D}$ (comp)	(ksi)	7.6	-	-	-	5.4
f_s $\frac{5}{3}[M_L + M_{imp}]$	(ksi)	17.6	14.0	8.9	12.7	16.3
f_s (Overload)	(ksi)	37.1	32.3	17.6	27.9	32.7
** f_s (Total)	(ksi)	48.2	42.0	22.9	36.3	42.5
VR	(k)	59.5	-	-	-	59.4

** Braced non-compact and partially braced section

INTERIOR GIRDER REACTION TABLE			
		Abut.	Pier
$R_{\mathcal{D}}$	(k)	83.1	162.6
R_L	(k)	47.5	69.3
Imp.	(k)	10.1	16.7
R_{Total}	(k)	140.7	248.6

I_s, S_s: Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads (in.^{4} and in.^{3}).

$I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total and Overload) due to short-term composite live loads (in.^{4} and in.^{3}).

$I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads (in.^{4} and in.^{3}).

$\mathcal{D}$: Un-factored non-composite dead load (kips/ft.).

$M_{\mathcal{D}}$: Un-factored moment due to non-composite dead load (kip-ft.).

$s_{\mathcal{D}}$: Un-factored long-term composite (superimposed) dead load (kips/ft.)

$M_{s\mathcal{D}}$: Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).

M_L: Un-factored live load moment (kip-ft.).

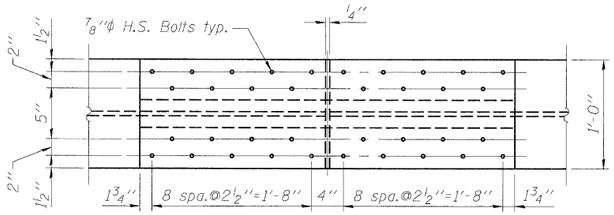
M_{imp}: Un-factored moment due to impact (kip-ft.).

M_a: Factored design moment (kip-ft.).
$1.3\ [\ M_{\mathcal{D}} + M_{s\mathcal{D}} + \frac{5}{3}(M_L + M_{Imp})]$

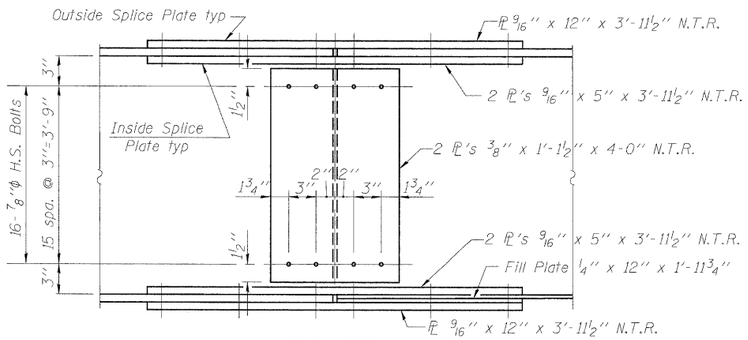
f_s (Overload): Sum of stresses as computed from the moments below (ksi).
$M_{\mathcal{D}} + M_{s\mathcal{D}} + \frac{5}{3}(M_L + M_{Imp})$

f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).
$1.3\ [M_{\mathcal{D}} + M_{s\mathcal{D}} + \frac{5}{3}(M_L + M_{Imp})]$

VR: Maximum L + impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).

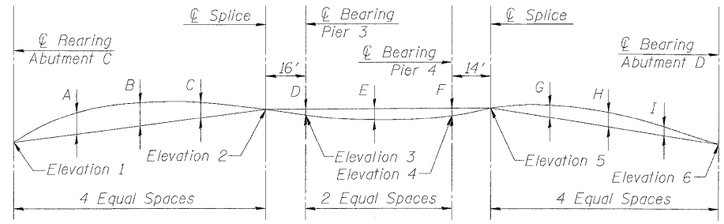


PLAN



ELEVATION

FIELD SPLICE #1 and #2 DETAIL



CAMBER DIAGRAM

Camber includes correction for Vertical Curve.

LOCATION	GIRDER G1	GIRDER G2	GIRDER G3	GIRDER G4
A	2 3/4"	2 1/2"	2 1/4"	2 1/2"
B	3 3/4"	3 1/2"	3 1/4"	3 1/4"
C	2 3/4"	2 1/2"	2 1/4"	2 1/4"
℄ Bearing Pier 3-D	1 3/4"	1 3/4"	1 3/4"	1 1/2"
E	1 3/4"	1 3/4"	1 3/4"	1 1/2"
℄ Bearing Pier 4-F	1 1/4"	1 1/2"	1 1/4"	1 1/2"
G	2"	2"	2"	2"
H	2 3/4"	2 1/2"	2 3/4"	2 3/4"
I	2"	2"	1 3/4"	1 3/4"

CAMBER VALUES TABLE

LOCATION	GIRDER G1	GIRDER G2	GIRDER G3	GIRDER G4
Elevation 1	592.31	592.27	592.23	592.18
Elevation 2	593.31	593.18	593.05	592.91
Elevation 3	593.31	593.16	593.02	592.85
Elevation 4	593.34	593.15	592.97	592.78
Elevation 5	593.34	593.15	592.95	592.76
Elevation 6	592.61	592.35	592.09	591.82

TOP OF GIRDER WEB ELEVATIONS

(FOR FABRICATION ONLY)

Elevations shown are theoretical elevations before concrete dead load deflection.