



# Illinois Department of Transportation

2300 South Dirksen Parkway / Springfield, Illinois / 62764

April 7, 2016

SUBJECT: FAP Route 626 (IL 97)  
Project ACF-0626 (010)  
Section 42-(B,B-1)BR-1  
Knox County  
Contract No. 68754  
Item No. 35, April 22, 2016 Letting  
Addendum A

## NOTICE TO PROSPECTIVE BIDDERS:

Attached is an addendum to the plans or proposal. This addendum involves revised and/or added material.

1. Revised Table of Contents to the Special Provisions
2. Revised page 30 of the Special Provisions
3. Added pages 186-189 to the Special Provisions

Prime contractors must utilize the enclosed material when preparing their bid and must include any Schedule of Prices changes in their bidding proposal.

Bidders using computer-generated bids are cautioned to reflect any and all Schedule of Prices changes, if involved, into their computer programs.

Very truly yours,

Maureen M. Addis, P.E.  
Acting Engineer of Design and Environment

A handwritten signature in black ink, appearing to read "Ted B. Walschleger" followed by a small "P.E." to the right.

By: Ted B. Walschleger, P. E.  
Engineer of Project Management

cc: Kensil Garnett, Region 3, District 4; Tim Kell; D. Carl Puzey; Estimates

JW/ck

PIPE UNDERDRAINS FOR STRUCTURES .....23  
 SLIPFORM PARAPET .....24  
 GRANULAR BACKFILL FOR STRUCTURES.....29  
 COARSE AGGREGATE QUALITY (BDE).....31  
 CONCRETE GUTTER, CURB, MEDIAN, AND PAVED DITCH (BDE) .....32  
 CONCRETE MIX DESIGN – DEPARTMENT PROVIDED (BDE) .....33  
 CONTRACT CLAIMS (BDE).....33  
 DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION (BDE).....34  
 EQUAL EMPLOYMENT OPPORTUNITY (BDE).....46  
 FRICTION AGGREGATE (BDE) .....49  
 HOT-MIX ASPHALT - DENSITY TESTING OF LONGITUDINAL JOINTS (BDE).....52  
 HOT-MIX ASPHALT – MIXTURE DESIGN COMPOSITION AND VOLUMETRIC  
 REQUIREMENTS (BDE) .....54  
 HOT-MIX ASPHALT – MIXTURE DESIGN VERIFICATION AND PRODUCTION (BDE).....64  
 HOT MIX ASPHALT – PRIME COAT (BDE) .....67  
 LRFD PIPE CULVERT BURIAL TABLES (BDE) .....72  
 LRFD STORM SEWER BURIAL TABLES (BDE).....91  
 PAVED SHOULDER REMOVAL (BDE) .....100  
 PAVEMENT MARKING TAPE TYPE IV (BDE) .....101  
 PAVEMENT STRIPING - SYMBOLS (BDE).....103  
 PORTLAND CEMENT CONCRETE BRIDGE DECK CURING (BDE) .....104  
 PROGRESS PAYMENTS (BDE) .....106  
 RECLAIMED ASPHALT PAVEMENT AND RECLAIMED ASPHALT SHINGLES (BDE).....107  
 REINFORCEMENT BARS (BDE) .....117  
 SIDEWALK, CORNER, OR CROSSWALK CLOSURE (BDE) .....118  
 STEEL SLAG IN TRENCH BACKFILL (BDE).....118  
 TEMPORARY CONCRETE BARRIER (BDE) .....119  
 TRACKING THE USE OF PESTICIDES (BDE).....120  
 TRAFFIC BARRIER TERMINALS TYPE 6 OR 6B (BDE) .....120  
 WARM MIX ASPHALT (BDE) .....120  
 WEEKLY DBE TRUCKING REPORTS (BDE).....122  
 WORKING DAYS (BDE).....122  
 BITUMINOUS MATERIALS COST ADJUSTMENTS (BDE) (RETURN FORM WITH BID).....123

FUEL COST ADJUSTMENT (BDE) (RETURN FORM WITH BID).....	126
STEEL COST ADJUSTMENT (BDE) (RETURN FORM WITH BID).....	130
SWPPP .....	134
404 PERMIT .....	143
BRIDGE DECK CONSTRUCTION .....	186

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Revised 4-7-16

**BRIDGE DECK CONSTRUCTION**

Effective: October 22, 2013

Revised: April 18, 2014

**Revise the Second Paragraph of Article 503.06(b) to read as follows.**

“When the Contractor uses cantilever forming brackets on exterior beams or girders, additional requirements shall be as follows.”

**Revise Article 503.06(b)(1) to read as follows.**

“(1) Bracket Placement. The spacing of brackets shall be per the manufacturer’s published design specifications for the size of the overhang and the construction loads anticipated. The resulting force of the leg brace of the cantilever bracket shall bear on the web within 6 inches (150 mm) of the bottom flange of the beam or girder.”

**Revise Article 503.06(b)(2) to read as follows.**

“(2) Beam Ties. The top flange of exterior steel beams or girders supporting the cantilever forming brackets shall be tied to the bottom flange of the next interior beam. The top flange of exterior concrete beams supporting the cantilever forming brackets shall be tied to the top flange of the next interior beam. The ties shall be spaced at 4 ft (1.2 m) centers. Permanent cross frames on steel girders may be considered a tie. Ties shall be a minimum of 1/2 inch (13 mm) diameter threaded rod with an adjusting mechanism for drawing the tie taut. The ties shall utilize hanger brackets or clips which hook onto the flange of steel beams. No welding will be permitted to the structural steel or stud shear connectors, or to reinforcement bars of concrete beams, for the installation of the tie bar system. After installation of the ties and blocking, the tie shall be drawn taut until the tie does not vary from a straight line from beam to beam. The tie system shall be approved by the Engineer.”

**Revise Article 503.06(b)(3) to read as follows.**

“(3) Beam Blocks. Suitable beam blocks of 4 in x 4 in (100 x 100 mm) timbers or metal structural shapes of equivalent strength or better, acceptable to the Engineer, shall be wedged between the webs of the two beams tied together, within 6 inches (150 mm) of the bottom flange at each location where they are tied. When it is not feasible to have the resulting force from the leg brace of the cantilever brackets transmitted to the web within 6 inches (150 mm) of the bottom flange, then additional blocking shall be placed at each bracket to transmit the resulting force to within 6 inches (150 mm) of the bottom flange of the next interior beam or girder.”

**Delete the last paragraph of Article 503.06(b).**

Added 4-7-16

**Revise the third paragraph of Article 503.16 to read as follows.**

“Fogging equipment shall be in operation unless the evaporation rate is less than 0.1 lb/sq ft/hour (0.5kg/sq m/hour) and the Engineer gives permission to stop. The evaporation rate shall be determined according to the following formula.

$$E = (T_c^{2.5} - rT_a^{2.5})(1 + 0.4V)x10^{-6} \text{ (English)}$$

$$E = 5[(T_c + 18)^{2.5} - r(T_a + 18)^{2.5}](V + 4)x10^{-6} \text{ (Metric)}$$

Where:

$E$  = Evaporation Rate, lb/ft<sup>2</sup>/h (kg/sq m/h)

$T_c$  = Concrete Temperature, °F (°C)

$T_a$  = Air Temperature, °F (°C)

$r$  = Relative Humidity in percent/100

$V$  = Wind Velocity, mph (km/h)

The Contractor shall provide temperature, relative humidity, and wind speed measuring equipment. Fogging equipment shall be adequate to reach or cover the entire pour from behind the finishing machine or vibrating screed to the point of curing covering application, and shall be operated in a manner which shall not accumulate water on the deck until the curing covering has been placed.”

**Revise the third paragraph of Article 503.16(a)(1) to read as follows.**

“At the Contractor’s option, a vibrating screed may be used in lieu of a finishing machine for superstructures with a pour width less than or equal to 24 ft (7.3 m). After the concrete is placed and consolidated, it shall be struck off with a vibrating screed allowing for camber, if required. The vibrating screed shall be of a type approved by the Engineer. A slight excess of concrete shall be kept in front of the cutting edge at all times during the striking off operation. After screeding, the entire surface shall be finished with hand-operated longitudinal floats having blades not less than 10 ft (3 m) in length and 6 in. (150 mm) in width. Decks so finished need not be straightedge tested as specified in 503.16(a)(2).”

**Delete the fifth paragraph of 503.16(a)(1).**

**Revise Article 503.16(a)(2) to read as follows.**

“(2) Straightedge Testing and Surface Correction. After the finishing has been completed and while the concrete is still plastic, the surface shall be tested for trueness with a 10 ft (3 m) straightedge, or a hand-operated longitudinal float having blades not less than 10 ft (3 m) in length and 6 in. (150 mm) in width. The Contractor shall furnish and use an accurate 10 ft (3 m) straightedge or float which has a handle not less than 3 ft (1 m) longer than 1/2 the pour width. The straightedge or float shall be held in contact with the surface and passed gradually from one side of the superstructure to the other. Advance along the surface shall be in successive stages of not more than 1/2 the length of the straightedge or float. Any depressions found shall be immediately filled with freshly mixed concrete, struck off, consolidated, and refinished. High areas shall be cut down and refinished.”

**Replace the second sentence of the first paragraph of Article 1020.13(a)(5) with the following sentences.**

“Cotton mats in poor condition will not be allowed. The cotton mats shall be placed in a manner which will not create indentations greater than 1/4 inch (6 mm) in the concrete surface. Minor marring of the surface is tolerable and is secondary to the importance of timely curing.”

**Revise Article 1020.14(b) to read as follows.**

“(b) Concrete in Structures. Concrete may be placed when the air temperature is above 40 °F (4 °C) and rising, and concrete placement shall stop when the falling temperature reaches 45 °F (7 °C) or below, unless otherwise approved by the Engineer.

(1) Bridge Deck Concrete. For concrete in bridge decks, slabs, and bridge approach slabs the Contractor shall schedule placing and finishing of the concrete during hours in which the ambient air temperature is forecast to be lower than 85 °F (30 °C). It shall be understood this may require scheduling the deck pour at night in order to utilize the temperature window available. The temperature of the concrete immediately before placement shall be a minimum of 50 °F (10 °C) and a maximum of 85 °F (30 °C).

(2) Non-Bridge Deck Concrete. Except as noted above, the temperature of the concrete immediately before placement shall be a minimum of 50 °F (10 °C) and a maximum of 90 °F (32 °C).

If concrete is pumped, the temperature restrictions above shall be considered at point of placement. When insulated forms are used according to Article 1020.13(d)(1), the maximum temperature of the concrete mixture immediately before placement shall be 80 °F (25 °C). When concrete is placed in contact with previously placed concrete, the temperature of the freshly mixed concrete may be increased by the Contractor to offset anticipated heat loss, but in no case shall the maximum concrete temperature be permitted to exceed the limits stated in this Article.”

Added 4-7-16

**Revise Article 1103.13(a) to read as follows.**

“(a) Bridge Deck. The finishing machine shall be equipped with: (1) a mechanical strike off device; (2) either a rotating cylinder(s) or a longitudinal oscillating screed which transversely finishes the surface of the concrete. The Contractor may attach other equipment to the finishing machine to enhance the final finish when approved by the Engineer. The finishing machine shall produce a deck surface of uniform texture, free from porous areas, and with the required surface smoothness.

The finishing machine shall be operated on rails or other supports that will not deflect under the applied loads. The maximum length of rail segments supported on top of beams and within the pour shall be 10 ft (3 m). The supports shall be adjustable for elevation and shall be completely in place to allow the finishing machine to be used for the full length of the area to be finished. The supports shall be approved by the Engineer before placing of the concrete is started.”

**Revise Article 1103.17(k) to read as follows.**

“(k) Fogging Equipment. Fogging equipment shall be hand held fogging equipment for humidity control. The equipment shall be capable of atomizing water to produce a fog blanket by the use of pressure 2500 psi minimum (17.24 MPa) and an industrial fire hose fogging nozzle or equivalent. Fogging equipment attached to the finishing machine will not be permitted.”