

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	SHEETS	SHEET	SHEET NO. 14 22 SHEETS
F.A.U. Rte. 6120	#	LaSALLE	492	282	
FED. ROAD DIST. NO. 7			ILLINOIS	FED. AID PROJECT-	CONTRACT NO. 66542

	0.4 Sp. 1 or 0.6 Sp. 2	Pier
$I_s$	(in <sup>4</sup> ) 14054	21798
$I_c$ (n)	(in <sup>4</sup> ) 32693	
$I_c$ (3n)	(in <sup>4</sup> ) 24621	
$S_s$	(in <sup>3</sup> ) 646	980
$S_c$ (n)	(in <sup>3</sup> ) 868	
$S_c$ (3n)	(in <sup>3</sup> ) 798	
Z	(in <sup>3</sup> )	1086
$M_D$	(k/ft.) 0.98	1.57
$s_D$	(k/ft.) 0.52	
$M_{sD}$	(k)	
$M_L$	(k)	708
$M$ (Imp)	(k)	163
$S_3[M_L + M(Imp)]$	(k)	1852
$M_a$	(k)	3877
$M_u$	(k)	4456.4
$f_s$ (non-comp) (k.s.i.)	10.61	22.81
$f_s$ (comp) (k.s.i.)	5.09	
$f_s$ (L+Imp) (k.s.i.)	25.62	17.78
$f_s$ (Overload) (k.s.i.)	41.32	40.59
VR	(k)	70

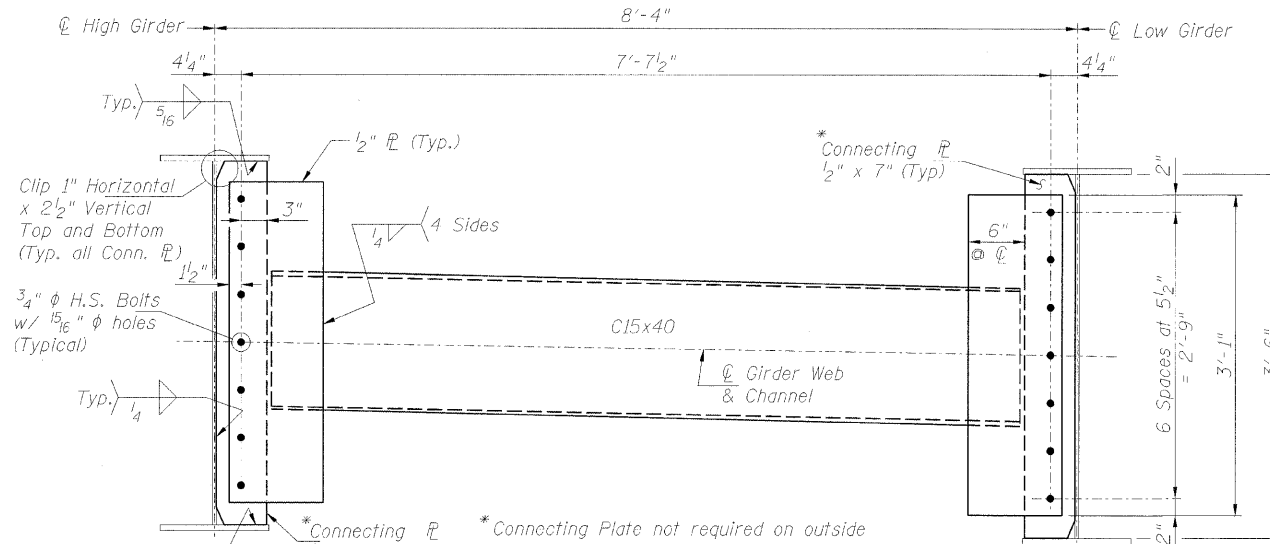
\*\*\*\* Compact Section

	Abut.	Pier
$R_D$	(k) 52.8	183.8
$R_L$	(k) 52.6	75.9
Imp.	(k) 12.1	17.5
$R$ (Total)	(k) 117.5	277.2

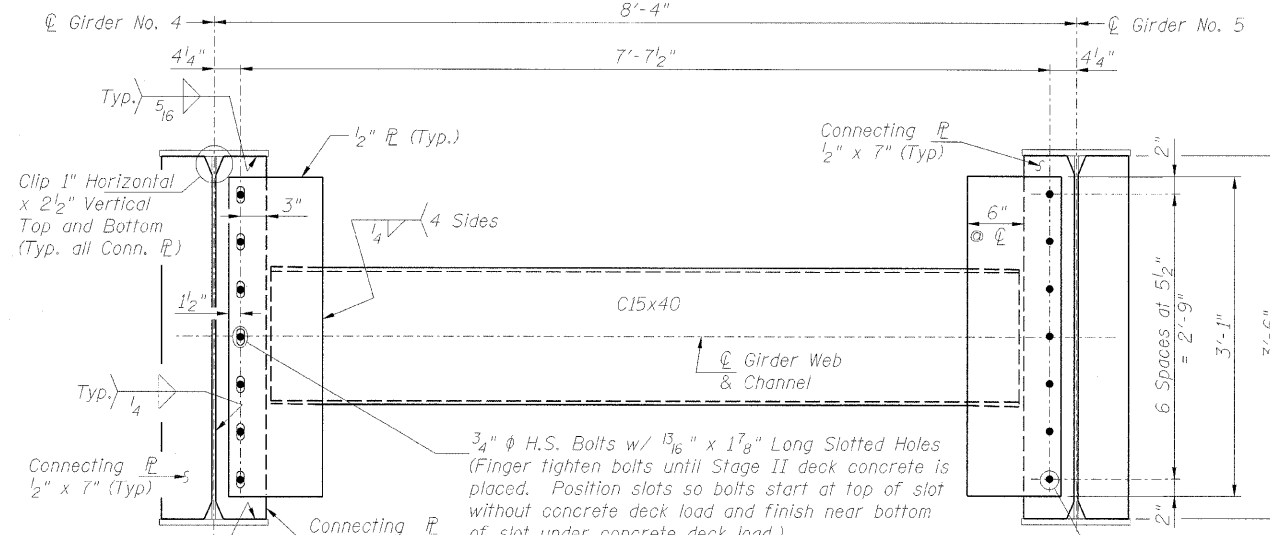
- $I_s, S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$  (Overload) due to non-composite dead loads (in<sup>4</sup> and in<sup>3</sup>).
- $I_c(n), S_c(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing  $f_s$  (Overload) due to short-term composite live loads (in<sup>4</sup> and in<sup>3</sup>).
- $I_c(3n), S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Overload) due to long-term composite (superimposed) dead loads (in<sup>4</sup> and in<sup>3</sup>).
- Z: Plastic Section Modulus of the steel section in non-composite areas (in<sup>3</sup>).
- $D$ : Un-factored non-composite dead load (kips/ft.).
- $M_D$ : Un-factored moment due to non-composite dead load (kip-ft.).
- $s_D$ : Un-factored long-term composite (superimposed) dead load (kips/ft.).
- $M_{sD}$ : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).
- $M_L$ : Un-factored live load moment (kip-ft.).
- $M_{Imp}$ : Un-factored moment due to impact (kip-ft.).
- $M_a$ : Factored design moment (kip-ft.).
- $1.3 [M_D + M_{sD} + \frac{2}{3} (M_L + M_{Imp})]$
- $M_u$ : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).
- $f_s$  (Overload): Sum of stresses as computed from the moments below (ksi).
- $M_D + M_{sD} + \frac{2}{3} (M_L + M_{Imp})$
- VR: Maximum  $L +$  impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).

The applied moment at the pier was 4307 Kft but has been reduced 430.0 Kft (10%) for moment redistribution.

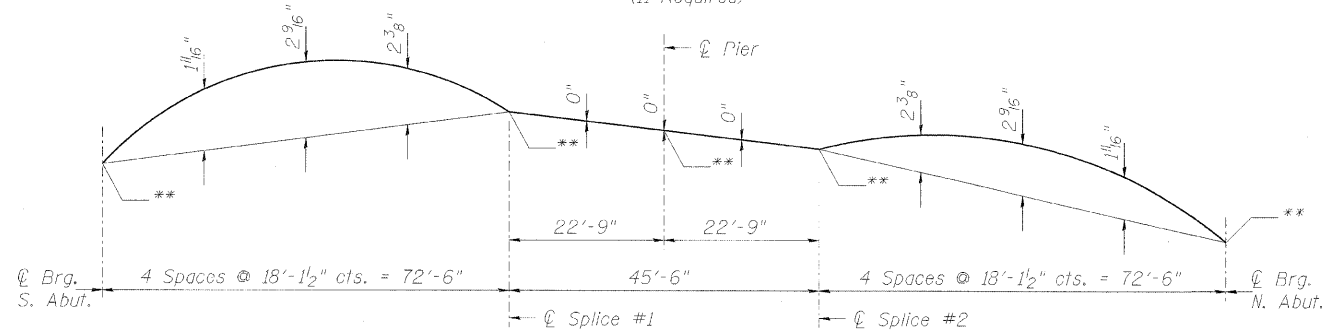
The applied moment at 0.4 span 1 or 0.6 span 2 was 3590 Kft but has been increased 172.0 Kft (4.8%) for moment redistribution.



INTERIOR DIAPHRAGM D1  
(66 Required)



INTERIOR DIAPHRAGM D2  
(11 Required)



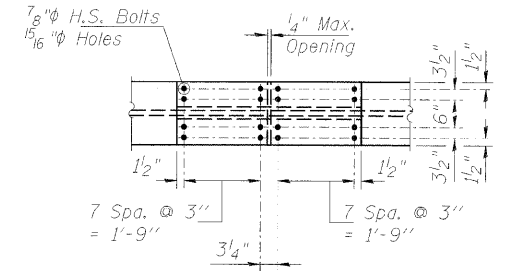
CAMBER DIAGRAM

\*\* See Table for Top of Web Elevations

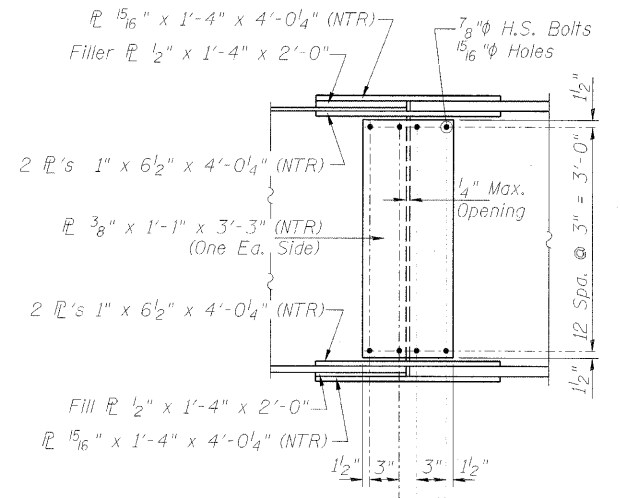
\*\*\* TOP OF WEB ELEVATIONS

	Girder 1	Girder 2	Girder 3	Girder 4	Girder 5	Girder 6	Girder 7	Girder 8
Brig. S. Abut.	645.892	646.048	646.173	646.298	646.298	646.173	646.048	645.892
Splice No. 1	646.094	646.244	646.367	646.495	646.495	646.367	646.244	646.094
Brig. Pier	646.094	646.244	646.367	646.495	646.495	646.367	646.244	646.094
Splice No. 2	646.094	646.244	646.367	646.495	646.495	646.367	646.244	646.094
Brig. N. Abut.	645.892	646.048	646.173	646.298	646.298	646.173	646.048	645.892

\*\*\* For Fabrication Only



TOP & BOTTOM FLANGE SPLICES



WEB SPLICE  
FIELD SPLICE DETAIL  
(16 Required)

Notes:  
Load carrying components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.

Two hardened washers shall be required over all oversized and slotted holes.

All flanges, webs, bearing stiffeners, splice plates, and structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M270, Grade 50.

Fill plates, Diaphragms, and diaphragm connecting plates shall conform to the requirements of AASHTO M270, Grade 35.

STRUCTURAL STEEL DETAILS  
UTICA ROAD (IL. RTE. 178) over F.A.I. ROUTE 80  
SECTION 50-3HBK  
LaSALLE COUNTY  
STATION 115+00.00  
STRUCTURE NO. 050-0248

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DATE  
10/12/09

1/23/2009 C:\PW\_L\exp\AS-01F-P-001.dgn  
 LAYOUT: MMW 11/16/05  
 DRAWN: Rcd 1/15/07  
 REVIEWED: JUT 1/15/07