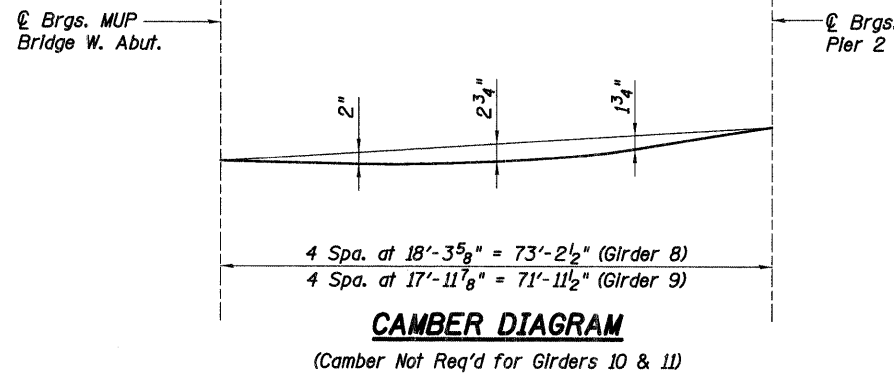


F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
361	06-00214-20-BR	KANE	320	170
STA. 511+80.00		TO STA. 609+14.92		
FED. ROAD DIST. NO. 7		ILLINOIS FED. AID PROJECT		
SHEET NO. S65 OF S108				

0.5 Sp. 2		
$I_s$	(in <sup>4</sup> )	5,582
$I_o(n)$	(in <sup>4</sup> )	13,846
$I_o(3n)$	(in <sup>4</sup> )	10,293
$S_s$	(in <sup>3</sup> )	388
$S_o(n)$	(in <sup>3</sup> )	534
$S_o(3n)$	(in <sup>3</sup> )	489
Z	(in <sup>3</sup> )	--
DC1	(k/')	1
M <sub>DC1</sub>	(k)	47
DC2	(k/')	0
M <sub>DC2</sub>	(k)	11
DW	(k/')	0
M <sub>DW</sub>	(k)	0
M <sub>ℓ + IM</sub>	(k)	29
M <sub>u</sub> (Strength I)	(k)	123
* $\phi_r M_n, \phi_r M_{nc}$	(k)	2,618
$f_s$ DC1	(ksi)	2
$f_s$ DC2	(ksi)	0
$f_s$ DW	(ksi)	0
$f_s$ 1.3(ℓ+IM)	(ksi)	1
$f_s$ (Service II)	(ksi)	3
** $f_s$ (Total)(Strength I)	(ksi)	--
V <sub>r</sub>	(k)	0

\* Compact sections  
 \*\* Non-Compact and slender sections

	MUP Bridge W. Abut.	Brgs. Pier 2
R <sub>DC1</sub>	(k) 28	(k) 25
R <sub>DC2</sub>	(k) 8	(k) 8
R <sub>DW</sub>	(k) 0	(k) 0
R <sub>ℓ + IM</sub>	(k) 23	(k) 20
R <sub>Total</sub>	(k) 59	(k) 53



**TOP OF WEB ELEVATIONS**

Girder Number	℄ Brg. MUP Bridge W. Abut.	End of Girder 9	℄ Pier 2
8	700.67	--	702.77
9	700.54	--	702.64
10	700.44	700.56	--
11	700.33	700.63	--

(For Fabrication Only)

**Notes:**

- For additional Framing Plan and Girder Elevation Information, see sheets S64 and S66 of S108.
- All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
- Load carrying components designated "NTR" shall conform to the Supplemental Requirements Notch Toughness, Zone 2.
- For Diaphragm details, see sheet S66 of S108.
- For Connection details for the connection between Girder 10 & 11 to Girder 9, see sheet S66 of S108.
- The structural steel for girders, bearing stiffeners, and splice plate material except fill plates shall conform to the requirements of AASHTO M270, Gr. 50.

$I_s, S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$  (Total-Strength I, and Service II) due to non-composite dead loads (in<sup>4</sup> and in<sup>3</sup>).  
 $I_o(n), S_o(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing  $f_s$  (Total-Strength I, and Service II) due to short-term composite live loads (in<sup>4</sup> and in<sup>3</sup>).  
 $I_o(3n), S_o(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in<sup>4</sup> and in<sup>3</sup>).  
 Z: Plastic Section Modulus of the steel section in non-composite areas. Omit line in Moment Table if not used in design calculations (in<sup>3</sup>).  
 DC1: Un-factored non-composite dead load (kips/ft.).  
 M<sub>DC1</sub>: Un-factored moment due to non-composite dead load (kip-ft.).  
 DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).  
 M<sub>DC2</sub>: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).  
 DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).  
 M<sub>DW</sub>: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).  
 M<sub>ℓ + IM</sub>: Un-factored live load moment plus dynamic load allowance (Impact) (kip-ft.).  
 M<sub>u</sub> (Strength I): Factored design moment (kip-ft.). 1.25 (M<sub>DC1</sub> + M<sub>DC2</sub>) + 1.5 M<sub>DW</sub> + 1.75 M<sub>ℓ + IM</sub>  
 $\phi_r M_n$ : Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).  
 $\phi_r M_{nc}$ : Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).  
 $f_s$  (Service II): Sum of stresses as computed from the moments below (ksi). M<sub>DC1</sub> + M<sub>DC2</sub> + M<sub>DW</sub> + 1.3 M<sub>ℓ + IM</sub>  
 $f_s$  (Total)(Strength I): 1.25 (M<sub>DC1</sub> + M<sub>DC2</sub>) + 1.5 M<sub>DW</sub> + 1.75 M<sub>ℓ + IM</sub>  
 V<sub>r</sub>: Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

h:\3005\3.0 phase 11 deliverables\3.3 structure\drawings\final\045-3164 MUP Bridge SteelDetails 1.dgn 1/16/2009

**Baker**

Baker Engineering, Inc.

REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION  
**MUP BRIDGE -**  
**STEEL DETAILS I**  
 MULTI-USE PATH BRIDGE OVER THE FOX RIVER PUBLIC WATERS  
 STRUCTURE NUMBER 045-3164  
 KANE COUNTY FAP 361 SECTION 06-00214-20-BR  
 STATION 572+37.71 DESIGNED: GWG DRAWN: GWG  
 DATE: JANUARY 16, 2009 CHECKED: KPZ CHECKED: KPZ