

FOR INDEX OF SHEETS, SEE SHEET 3

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS

PROPOSED  
HIGHWAY PLANS

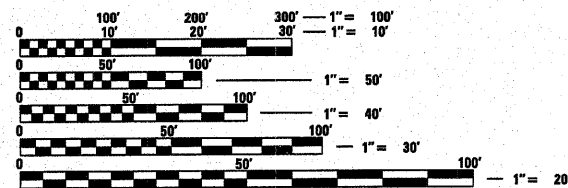
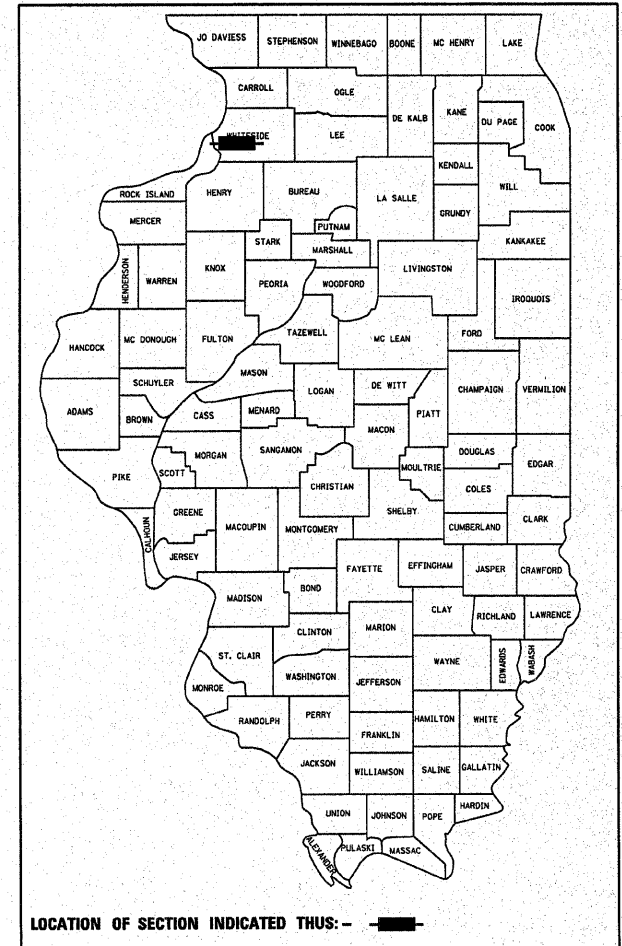
FAS 1197 (WEST LINCOLNWAY ROAD) & FAP 308 (IL 84)  
SECTION D2 BRIDGE PAINTING 2009-3  
BRIDGE PAINTING  
WHITESIDE COUNTY  
C-92-090-09

STANDARDS

701006-03  
701201-03  
701901-01

| F.A. RTE.           | SECTION  | COUNTY       | TOTAL SHEETS | SHEET NO. |
|---------------------|----------|--------------|--------------|-----------|
| ---                 | ---      | Whiteside    | 15           | 1         |
| FED. ROAD DIST. NO. | ILLINOIS | CONTRACT NO. | 64E93        |           |

\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*\*D2 Bridge Painting 2009-3  
D-92-054-09



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.  
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION  
1-800-892-0123  
OR 811

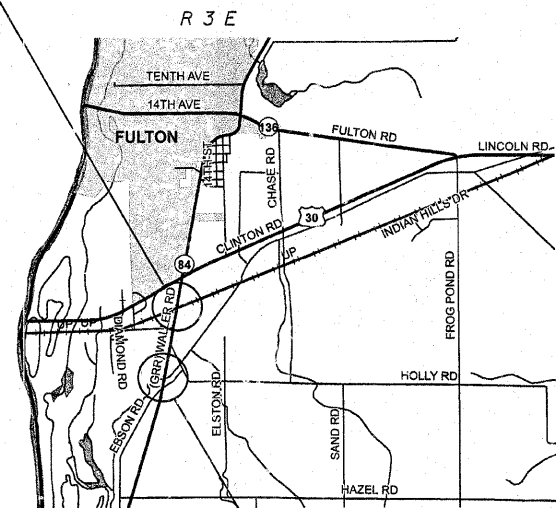
District 2 Bridge Paint Technician – Dan Link 815/284-5416  
District 2 Bridge Maintenance Engineer – Mahmoud Etemadi 815/284-5393

CONTRACT NO. 64E93

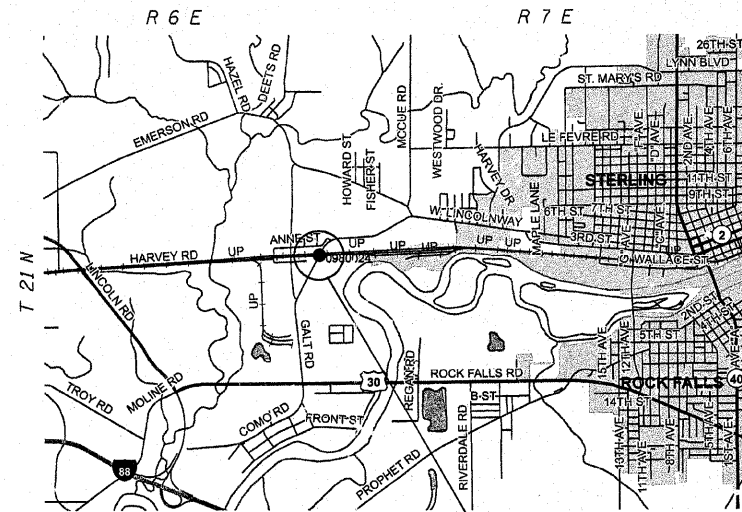
GROSS LENGTH = 1727.1 FT. = 0.327 MILE  
NET LENGTH = 1727.1 FT. = 0.327 MILE

Structure Number 098-0096

T 22 N



Structure Number 098-0095



Structure Number 098-0024

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS

SUBMITTED Jan. 29 2009  
George F. Ryan  
DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER

March 27 2009  
Charles J. Ingersoll  
ENGINEER OF DESIGN AND ENVIRONMENT

March 27 2009  
Christine M. Reed  
DIRECTOR OF HIGHWAYS, CHIEF ENGINEER

PRINTED BY THE AUTHORITY  
OF THE STATE OF ILLINOIS

# SUMMARY OF QUANTITIES

| Pay Item Number | Description   | Units | Total Quantity | 100% STATE          |                    |
|-----------------|---|-------|----------------|---------------------|--------------------|
|                 |   |       |                | SFTY-2A<br>FAS 1197 | SFTY-2A<br>FAP 308 |
| 50606401        | CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES NO.1 | L SUM | 1              | 1                   |                    |
| 50606402        | CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES NO.2 | L SUM | 1              |                     | 1                  |
| 50606403        | CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES NO.3 | L SUM | 1              |                     | 1                  |
| 50600600        | CLEANING AND PAINTING STEEL BRIDGE NO. 1                      | L SUM | 1              | 1                   |                    |
| 50600700        | CLEANING AND PAINTING STEEL BRIDGE NO. 2                      | L SUM | 1              |                     | 1                  |
| 50600800        | CLEANING AND PAINTING STEEL BRIDGE NO. 3                      | L SUM | 1              |                     | 1                  |
| 67100100        | MOBILIZATION  | L SUM | 1              | 0.3                 | 0.7                |
| 70100450        | TRAFFIC CONTROL AND PROTECTION, STANDARD 701201               | L SUM | 1              | 0.3                 | 0.7                |
| Z0048665        | RAILROAD PROTECTIVE LIABILITY INSURANCE                       | L SUM | 1              | 0.5                 | 0.5                |
|                 |   |       |                |                     |                    |
|                 |   |       |                |                     |                    |
|                 |   |       |                |                     |                    |
|                 |   |       |                |                     |                    |

\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*\*D2 Bridge Painting 2009-3

|   |                   |            |           |   |                              |   |         |           |              |           |  |
|---|-------------------|------------|-----------|---|------------------------------|---|---------|-----------|--------------|-----------|--|
| FILE NAME =   | USER NAME = lmkdj | DESIGNED - | REVISED - | <b>STATE OF ILLINOIS<br/>DEPARTMENT OF TRANSPORTATION</b> | <b>Summary of Quantities</b> | F.A. RTE.                                     | SECTION | COUNTY    | TOTAL SHEETS | SHEET NO. |  |
| Q:\BRV\Bridg Painting\Contract\PAINTING\64E93\PLANeng.dgn |                   | DRAWN -    | REVISED - |   |                              | *   | **      | Whiteside | 15           | 2         |  |
| PLOT SCALE = 50.0000' / IN.                               |                   | CHECKED -  | REVISED - |   |                              | CONTRACT NO. 64E93                            |         |           |              |           |  |
| PLOT DATE = Wed Mar 11 10:41:06 2009                      |                   | DATE -     | REVISED - |   |                              | FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT |         |           |              |           |  |

# GENERAL NOTES

A minimum of 2 air monitors will be required to monitor abrasive blasting operations at each location, see special provision for "Containment and Disposal of Lead Paint Cleaning Residues".

The Contractor shall seed all disturbed areas within the project limits. Seeding Class 4 or 2A shall be used, except in front of properties where the grass will be mowed, then use Seeding, Class 1. Class 2A shall be used on front slopes and ditch bottoms. Class 4 shall be used behind Type A gutter, on all backslopes and areas behind the backslope, and beyond the toe of front slope on fill sections without ditches. This work will be included in the contract unit price per LUMP SUM for CLEANING AND PAINTING STEEL BRIDGE NO. 1.

Fertilizer shall be applied to all disturbed areas and incorporated into the seedbed prior to seeding or placement of sod at the rate specified in Sections 250 and 252 of the Standard Specifications. This work shall be included in the cost of CLEANING AND PAINTING STEEL BRIDGE NO. 1.

Mulch Method II shall be applied over all seeded areas. This shall be included in the cost of the CLEANING AND PAINTING STEEL BRIDGE NO. 1.

The Contractor shall be responsible for protecting utility property during construction operations as outlined in Article 107.31 of the Standard Specifications. A minimum of 48 hours advance notice is required for non-emergency work. The JULIE number is 800-892-0123.

THE SSPC QP-1 AND QP-2 CONTRACTOR CERTIFICATIONS WILL NOT BE REQUIRED FOR THIS CONTRACT.

# INDEX OF SHEETS

1. Cover Sheet, Index of Sheets, Standards
2. Summary of Quantities
3. General Notes
4. -7. Existing Bridge Plans Locations SN 098-0024
8. -11. Existing Bridge Plans Locations SN 098-0095
12. -15. Existing Bridge Plans Locations SN 098-0096

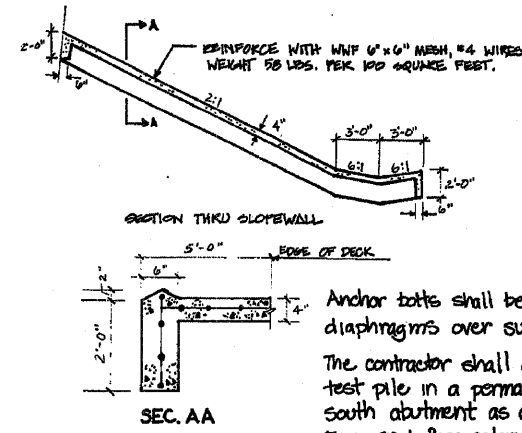
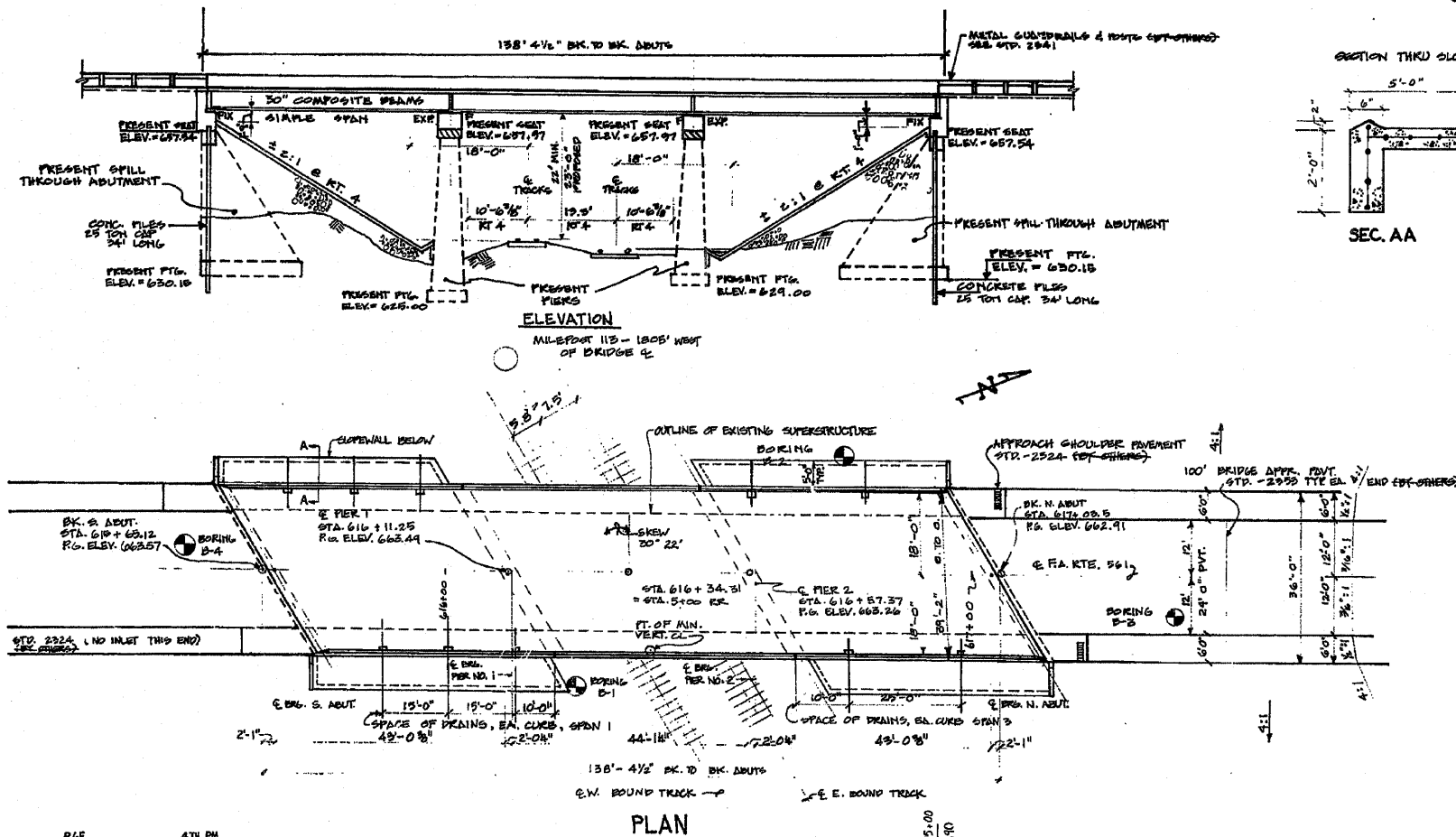
\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
 \*\*D2 Bridge Painting 2009-3

|   |                                      |                |   |   |  |                             |         |                          |              |           |
|---|--------------------------------------|----------------|---|---|--|-----------------------------|---------|--------------------------|--------------|-----------|
| FILE NAME =<br>P:\PAINTING\64E93\PLA\Neng.dgn | USER NAME = linkdj                   | DESIGNED - --- | REVISED - ---                                   | <b>STATE OF ILLINOIS<br/>DEPARTMENT OF TRANSPORTATION</b> | <b>General Notes<br/>Index of Sheets</b> | F.A. RTE.:                  | SECTION | COUNTY                   | TOTAL SHEETS | SHEET NO. |
|   | PLOT SCALE = 50.0000' / IN.          | DRAWN - ---    | REVISED - ---                                   |   |  | .                           | ..      | Whiteside                | 15           | 3         |
|   | PLOT DATE = Thu Jan 29 08:03:17 2009 | CHECKED - ---  | REVISED - ---                                   |   |  | CONTRACT NO. 64E93          |         |                          |              |           |
|   | DATE - ---                           | REVISED - ---  | FED. ROAD DIST. NO. [ILLINOIS] FED. AID PROJECT |   |  |                             |         |                          |              |           |
|   |                                      |                |   |   | SCALE: -----                             | SHEET NO. ___ OF ___ SHEETS |         | STA. ----- TO STA. ----- |              |           |

Existing Structure: See Note on Sheet 1 of 34  
Structure Number 098-0024

| ROUTE NO. | SECTION | COUNTY    | SHEET NO. | TOTAL SHEETS | SHEET NO. | SHEETS |
|-----------|---------|-----------|-----------|--------------|-----------|--------|
| 561       | 105V-BR | WHITESIDE | 34        | 11           | 2         | 13     |

# FOR INFORMATION ONLY



### GENERAL NOTES

See proposal for boring data.  
Fasteners shall be high strength bolts, AASHTO M164, TYPE 3. BOLTS 3/4"  $\phi$ , OPEN HOLES 13/16"  $\phi$ , UNLESS OTHERWISE NOTED.  
Calculated weight of structural steel = 83,100 #  
All structural steel shall be AASHTO M 222 unpainted Type

Anchor bolts shall be set before bolting diaphragms over supports.

The contractor shall drive one concrete test pile in a permanent location at the south abutment as directed by the Engineer before ordering the remainder of piles.

Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work, however the contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Reinforcement bars shall conform to the requirements of AASHTO M-31 or M-53 grade 60.

The structural steel bearing plates of the Elastomeric Bearing Assembly shall conform to the requirements of AASHTO M222.

The concrete rail section above the mandatory construction joint at the top of the slab shall be constructed of Class X concrete, except the approaches shall conform to the requirements of Normal Concrete.

Expansion bolts shall consist of approved expansion anchors, providing certified minimum proof load = 4080 LBS, and 3/4"  $\phi$  x 12" hooked bolts.

The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Notch Toughness Zone 2. These components are the wide flange beams.

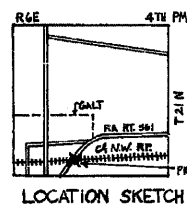
All structural steel for a distance of three times the depth of the beams, but not exceeding 10 feet each way from deck joints, shall be cleaned and given one coat of the basic lead silico chromate primer and maroon field coat. Both coats to be applied in the shop with spot painting only in the field.

### TOTAL BILL OF MATERIAL

| Item                               | Unit     | Super | Sub   | Total    |
|------------------------------------|----------|-------|-------|----------|
| Structure Excavation               | Cu. Yd.  | —     | 45    | 45       |
| Class X Concrete                   | Cu. Yd.  | —     | 108.6 | 268.6    |
| Reinforcement Bars                 | Pound    | 18670 | 12390 | 31,060   |
| Reinforcement Bars (Epoxy)         | Pound    | 27020 | —     | 27,020   |
| Structural Steel                   | LUMP SUM | —     | —     | LUMP SUM |
| Slope Wall (4")                    | Sa. Yd.  | —     | 528   | 528      |
| Neoprene Exp. Jt. (2")             | Lin. Ft. | 92    | —     | 92       |
| Name Plates                        | Each     | 1     | —     | 1        |
| Concrete Piles                     | Lin. Ft. | —     | 136   | 136      |
| Test Pile Concrete                 | Each     | —     | 1     | 1        |
| Protective Coat                    | Sa. Yd.  | 602   | —     | 602      |
| Stud Shear Connectors              | Each     | —     | 3270  | 3270     |
| Concrete Removal                   | Cu. Yd.  | —     | 24.8  | 24.8     |
| Removal of Existing Superstructure | EACH     | 1     | —     | 1        |
| Expansion Bolts 3/4" $\phi$        | EACH     | —     | 168   | 168      |
| Elastomeric Brg. Assy. Type I      | Each     | —     | 10    | 10       |

DESIGN STRESSES  
(LOAD FACTOR DESIGN)  
 $f'_c = 3500$  PSI  
 $f_y = 60,000$  PSI (REINFC)  
 $f_y = 50,000$  PSI (STRUCTURAL STEEL)

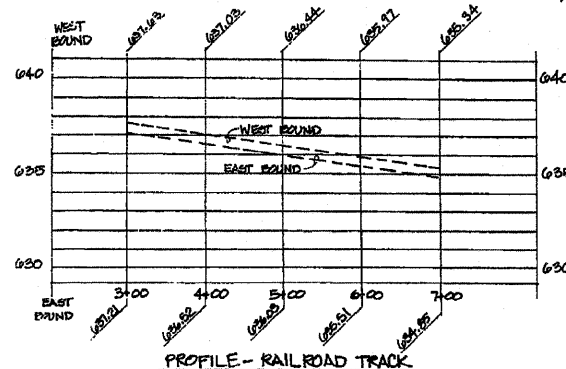
LOADING HS 20-44  
Allowance of twenty-five (25) pounds per square foot for tube wearing surface. Designed in accordance with the "Standard Specifications for Highway Bridges" adopted by The American Association of State, Highway and Transportation Officials (AASHTO), 1977 edition and Interims 1978, 1979 and 1980.



STATION 616+34.31  
BUILT 198 BY  
STATE OF ILLINOIS  
FA. ROUTE 561 SECTION 105 V-BR  
F.A. PROJECT BR-F-56(N7)  
LOADING HS20  
STR NO 098 0024

NAME PLATE  
(SEE STANDARD 2113)

PROPOSED PROFILE FA. RTE. 561  
AT E. RDWY



APPROVED  
FOR SUBMITTAL TO THE STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION



Frank E. Stewart

GENERAL PLAN & ELEVATION  
FA. ROUTE 561 (ILLINOIS RT. 2)  
OVER C&NW RR AT GALT  
SECTION 105 VBR  
WHITESIDE COUNTY  
STA. 616+34.31

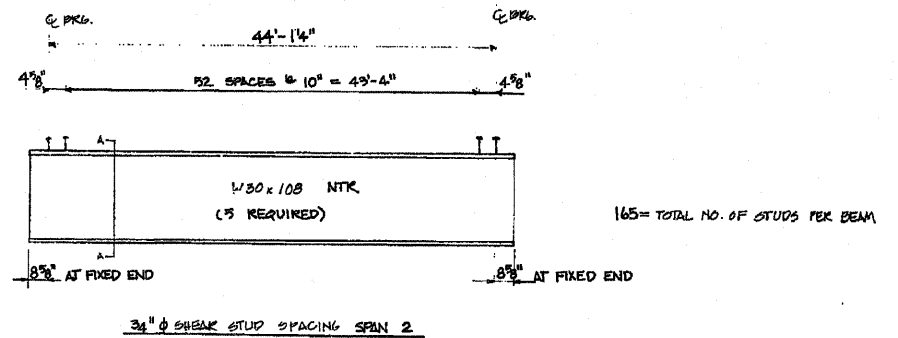
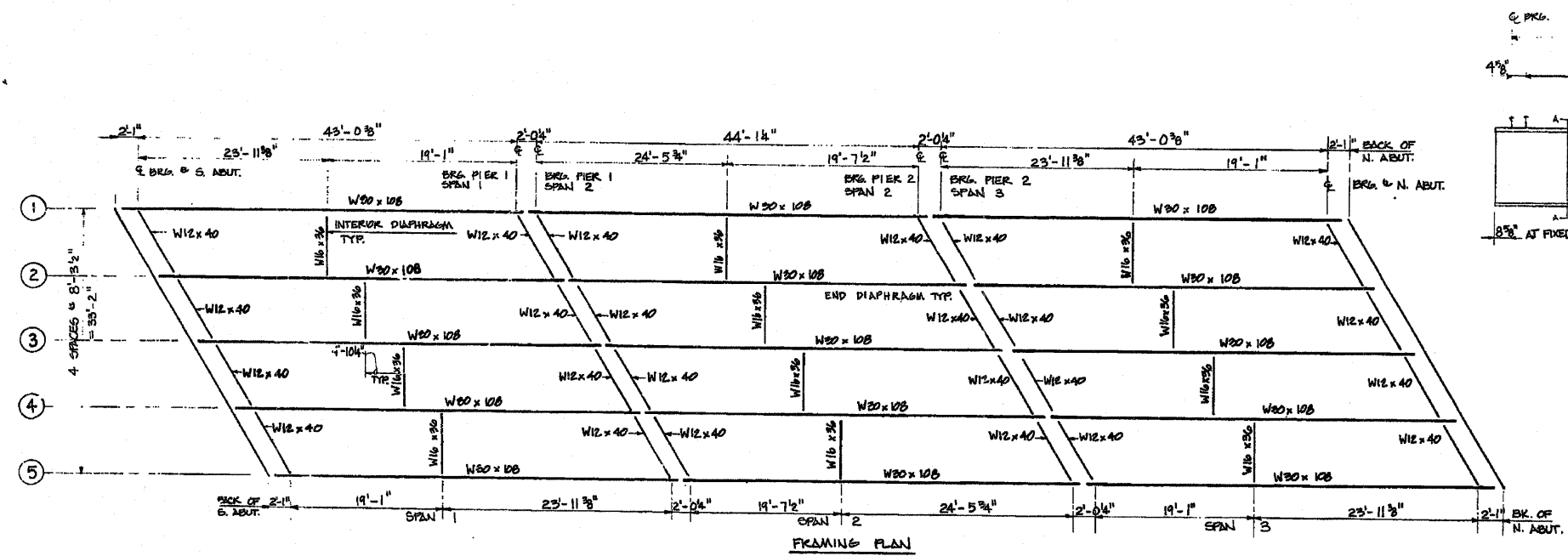
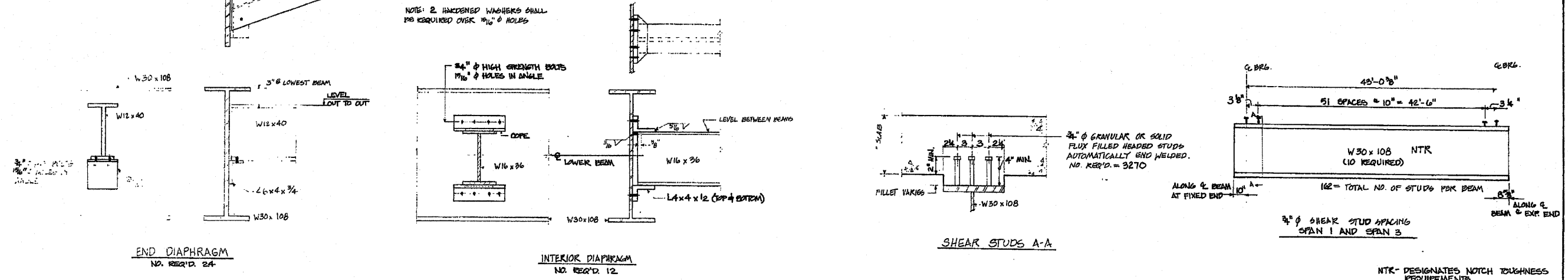
\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*\*D2 Bridge Painting 2009-3

|   |                   |            |                           |   |                            |           |         |        |              |           |
|---|-------------------|------------|---------------------------|---|----------------------------|-----------|---------|--------|--------------|-----------|
| FILE NAME = P:\PAINTING\64E93\PLN\eng.dgn | USER NAME = lmkdj | DESIGNED - | REVISED -                 | STATE OF ILLINOIS<br>DEPARTMENT OF TRANSPORTATION | Existing Plans SN 098-0024 | F.A. RTE. | SECTION | COUNTY | TOTAL SHEETS | SHEET NO. |
| PLOT SCALE = 50.0000' / IN.               | DRAWN -           | REVISED -  | Whiteside                 |   |                            | 15        | 4       |        |              |           |
| PLOT DATE = Thu Jan 29 10:18:09 2009      | CHECKED -         | REVISED -  | CONTRACT NO. 64E93        |   |                            |           |         |        |              |           |
| DATE                                      | DATE              | REVISED    | ILLINOIS FED. AID PROJECT |   |                            |           |         |        |              |           |



# FOR INFORMATION ONLY

| ROUTE NO. | SECTION | COUNTY    | SHEET NO. | TOTAL SHEETS | SHEET NO. 7 |
|-----------|---------|-----------|-----------|--------------|-------------|
| 561       | 105V-BR | WHITESIDE | 34        | 16           | 13          |



SPAN 2

|                                   |         |
|-----------------------------------|---------|
| I <sub>s</sub> (in <sup>4</sup> ) | 4470    |
| I <sub>c</sub> (in <sup>4</sup> ) | 12602   |
| S <sub>s</sub> (in <sup>3</sup> ) | 299     |
| S <sub>c</sub> (in <sup>3</sup> ) | 449     |
| DL (K/FT)                         | 1.151   |
| MDL (F-T)                         | 219.86  |
| f <sub>s</sub> NON COMP(S)        | 11.23   |
| S DL (K/FT)                       | 0.599   |
| M SCL                             | 145.65  |
| M LL                              | 853.19  |
| M IMP                             | 252.47  |
| TOTAL (F-T)                       | 1251.91 |
| f <sub>s</sub> COMP(S)            | 33.46   |
| f <sub>s</sub> TOTAL (KSI)        | 44.69   |
| VR (K)                            | 55.41   |

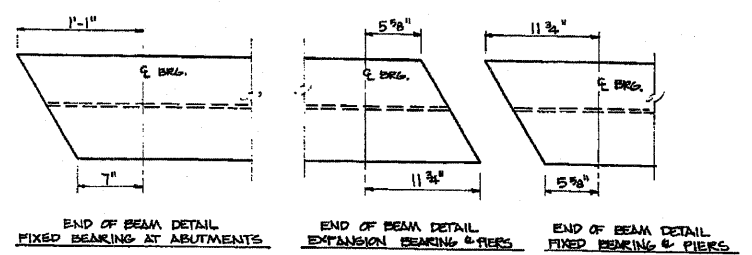
|             |       |
|-------------|-------|
| R DL (K)    | 38.59 |
| R LL (K)    | 45.64 |
| R IMP (K)   | 13.64 |
| R TOTAL (K) | 97.72 |

I<sub>s</sub> AND S<sub>s</sub> ARE THE MOMENT OF INERTIA AND SECTION MODULUS OF THE STEEL SECTION USED IN COMPUTING f<sub>s</sub> TOTAL.

I<sub>c</sub> AND S<sub>c</sub> ARE THE MOMENT OF INERTIA AND SECTION MODULUS OF THE COMPOSITE SECTION USED IN COMPUTING f<sub>s</sub> TOTAL.

VR IS THE MAXIMUM LL + IMPACT SHEAR RANGE IN SPAN.

THE LOAD FACTOR 1.3 [DL + 3/8 (LL + IMP)] IS USED IN COMPUTING MOMENTS AND STRESSES.



| LOCATION             | BEAM NO. 1 | NO. 2  | NO. 3  | NO. 4  | NO. 5  |
|----------------------|------------|--------|--------|--------|--------|
| ¢ BRG. S. ABUT.      | 662.66     | 662.82 | 662.95 | 662.81 | 662.65 |
| ¢ BRG. PIER 1 SPAN 1 | 662.61     | 662.79 | 662.86 | 662.72 | 662.54 |
| ¢ BRG. PIER 2 SPAN 2 | 662.60     | 662.74 | 662.86 | 662.71 | 662.54 |
| ¢ BRG. PIER 2 SPAN 3 | 662.42     | 662.55 | 662.65 | 662.44 | 662.30 |
| ¢ BRG. PIER 2 SPAN 3 | 662.41     | 662.53 | 662.63 | 662.47 | 662.29 |
| ¢ BRG. N. ABUT.      | 662.10     | 662.22 | 662.30 | 662.15 | 662.05 |

GIRDER & FRAMING DETAILS  
FA. ROUTE 561 (ILLINOIS RT. 2)  
OVER C&NW RR AT GALT  
SECTION 105 VBR  
WHITESIDE COUNTY  
STA. 616 + 34.31

\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*\*D2 Bridge Painting 2009-3

|   |                    |            |                    |   |                            |                           |         |        |              |           |
|---|--------------------|------------|--------------------|---|----------------------------|---------------------------|---------|--------|--------------|-----------|
| FILE NAME = P:\PAINTING\64E93VPLA\eng.dgn | USER NAME = linkdj | DESIGNED - | REVISED -          | STATE OF ILLINOIS<br>DEPARTMENT OF TRANSPORTATION | Existing Plans SN 098-0024 | F.A. RTE. -               | SECTION | COUNTY | TOTAL SHEETS | SHEET NO. |
| PLOT SCALE = 5/8" = 1' / IN.              | CHECKED -          | REVISED -  | **                 |   |                            | Whiteside                 | 15      | 6      |              |           |
| PLOT DATE = Thu Jan 29 10:19:28 2009      | DATE -             | REVISED -  | CONTRACT NO. 64E93 |   |                            |                           |         |        |              |           |
|   |                    |            |                    |   |                            | ILLINOIS FED. AID PROJECT |         |        |              |           |



# FOR INFORMATION ONLY

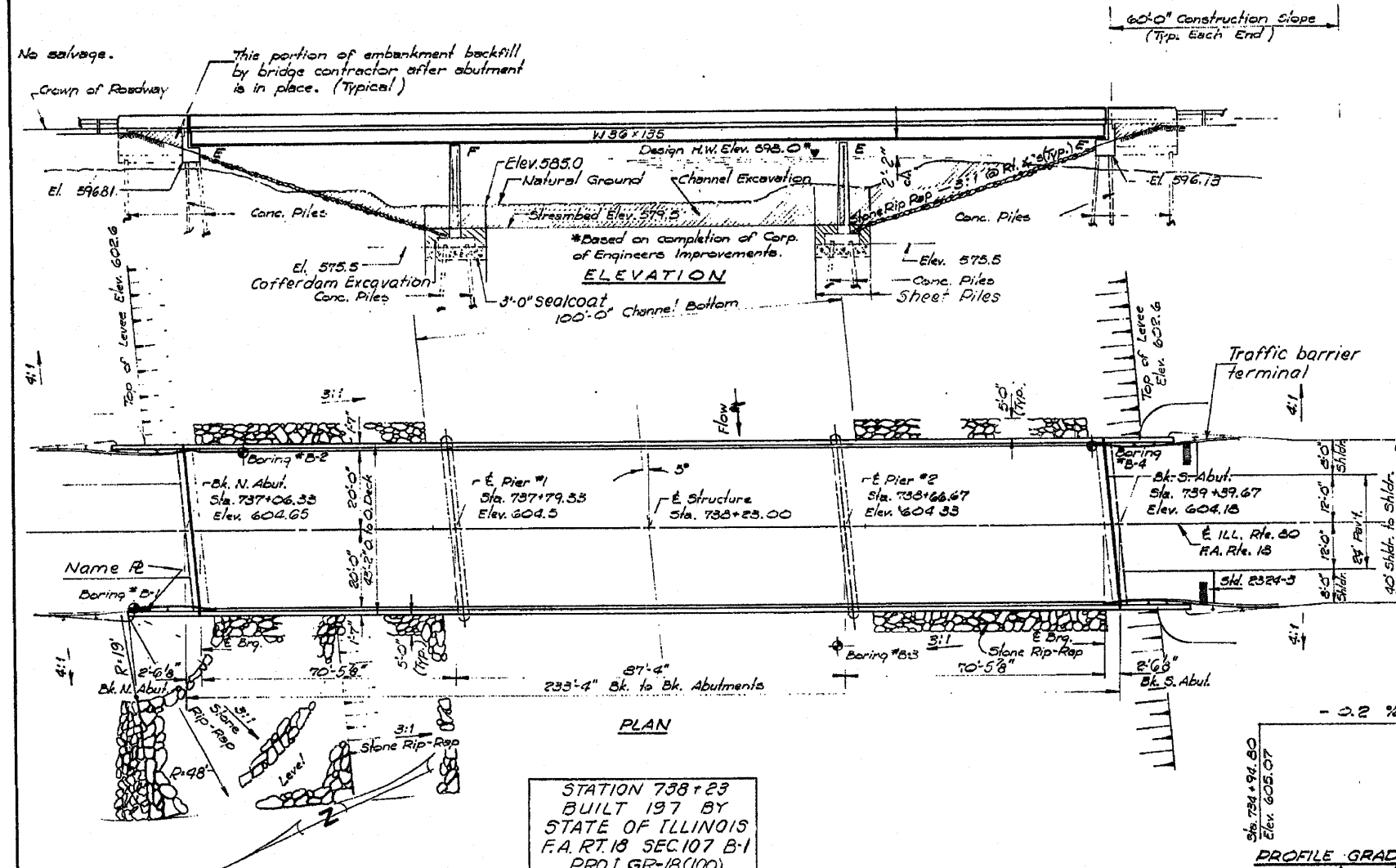
098-0093

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

|                 |              |       |             |
|-----------------|--------------|-------|-------------|
| DATE            | QUANTITY     | TOTAL | SHEET NO. / |
| 107-1<br>1072-1 | WHITESIDE 61 | 49    | 13 SHEETS   |

BM. USGS BM #121 Chis "D" NE W of Bridge over Johnson Creek. Elev 594.77

Existing Structure: @ Sta. 738+23, Built in 1931 as SBI Rte. 80, Sec. 107-B, Existing Steel Truss W/40' Rwy, Superstructure and P.C. closed Abut. to be removed after construction of new structure



**GENERAL NOTES**

See Proposal for Boring Data.

Fasteners shall be high strength bolts. Bolts 3/4" dia, open holes 1 1/8", unless otherwise noted.

Calculated weight of Structural Steel = 224,040 lbs. M222

All structural steel shall be AASHTO: M222 unpainted except expansion joint angles and attached bars which shall be AASHTO: M183 and shop painted with two coats of basic lead silico chromate paint.

Field welding of construction accessories will not be permitted to the bottom flange of beams or girders for a distance equal to one-fourth the span length each way from the pier supports. Field welding in other areas will be permitted only when approved by the Engineer.

Anchor bolts shall be set before setting diaphragms over supports.

Lapout of stone riprap may be varied in the field to suit ground conditions as directed by the Engineer.

The Contractor shall drive on (1) test pile in a permanent location of Pier #1 South Abut. as directed by the Engineer before ordering the remainder of piles.

The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments.

The concrete rail section above the mandatory construction joint at the top of the slab shall be constructed of Class X Concrete, except the aggregates shall conform to the requirements of Standard Concrete.

The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Notch Toughness Zone 2. These components are the tension flanges, webs and all splice plate material of the steel girders or wide flange beams.

Bearing seat surfaces shall be constructed or adjusted to the designated dimensions within a tolerance of +/- 1/8". Adjustment shall be made either by grinding the surface or by shimming the bearing. 1/2" adjusting shims, the dimensions of the bottom bearing plate, shall be provided for each bearing in addition to all other plates or shims.

No shims shall be allowed under Type I Elastomeric Bearing Pads at Pier 2 & North Abutment.

Protective coat shall be applied to surfaces of the deck & face of parapet and curb in accordance with Art. 503.12 of the Standard Specifications.

Reinforcement bars shall conform to the requirements of AASHTO M-31 Grade 60

**TOTAL BILL OF MATERIAL**

| Item                          | Unit     | Super  | Sub    | Total  |
|-------------------------------|----------|--------|--------|--------|
| Removal of Existing Structure | Each     |        |        | 1      |
| Protective Coat               | Sq. Yd.  | 1,228  |        | 1,228  |
| Class X Concrete              | Cu. Yd.  | 289.7  | 79.0   | 368.7  |
| Class A Concrete              | Cu. Yd.  |        | 244.9  | 244.9  |
| Stud Shear Connectors         | Each     | 4140   |        | 4140   |
| Reinforcement Bars            | Pound    | 32,380 | 30,150 | 62,530 |
| Reinf. Bars (Epoxy Coated)    | Pound    | 46,850 |        | 46,850 |
| Concrete Piles                | Lin. Ft. |        | 4258   | 4258   |
| Test Pile Concrete            | Each     |        | 2      | 2      |
| Name Plates                   | Each     | 1      |        | 1      |
| Stone Riprap                  | Sq. Yd.  |        |        | 1,228  |
| Neoprene Expansion Joint 2"   | Lin. Ft. | 43     |        | 43     |
| Preformed Joint Sealer 2 1/2" | Lin. Ft. | 44     |        | 44     |
| Channel Excavation            | Cu. Yd.  |        |        | 886.6  |
| Seal Coat Concrete            | Cu. Yd.  | 131.6  |        | 131.6  |
| Cofferdam Pier 1              | Each     |        | 1      | 1      |
| Cofferdam Pier 2              | Each     |        | 1      | 1      |
| Cofferdam Excavation          | Cu. Yd.  |        | 307    | 307    |
| Structural Steel              | L. Sum.  |        |        | 1      |

\*\* Quantity based on 100' upstream & 150' down stream

STATION 738+23  
BUILT 197 BY  
STATE OF ILLINOIS  
F.A. RT. 18 SEC. 107 B-1  
PROJ. GR-18(100)  
LOADING HS20  
\*STR. NO.  
NAME PLATE  
See 5th. 2113

**DESIGN STRESSES**

$f_c = 3,500$  psi  
 $f_y = 60,000$  psi (Reinforcement)  
 $f_y = 50,000$  psi (Structural Steel)

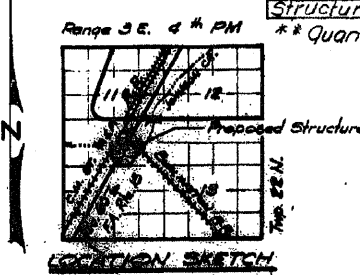
**WATERWAY INFORMATION**

Drainage Area = 65.5 sq. mi.  
Design Discharge (below 50yr HWE) = 17,000 cfs.  
Existing Opening = 1,200 sq. ft.  
Required Opening (below 50yr HWE) = 2,800 sq. ft.  
Proposed Opening = 3,140 sq. ft.  
Created Head for Design Flood = 0  
200 yr. Discharge = 22,400 cfs.  
Created Head 100 yr. Flood = 0  
50 yr. H.W.E. = 598.0 Elev.  
200 yr. H.W.E. = 599.4 Elev.

DESIGNED: J. A. Hensch  
CHECKED: [Signature]  
DRAWN: Mercado  
APPROVED: [Signature]

**LOADING HS 20-44**

Allow 25' / sq. ft. for future wearing surface. Design Specifications: 1977 AASHTO, 1973 Interim Specifications.



**GENERAL PLAN & ELEVATION**  
PROJECT GR-18(100)  
F.A. RT. 18 SEC. 107 B-1  
WHITESIDE COUNTY

DATE 0  
11 13  
2003

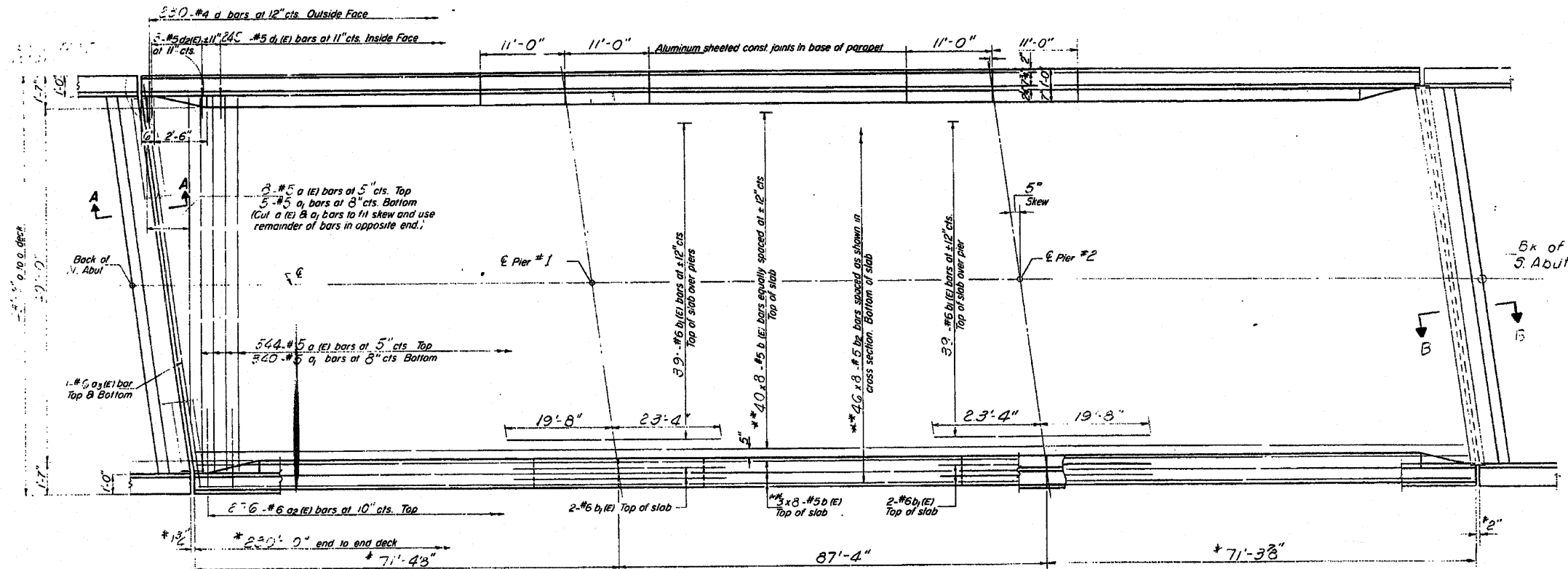
FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*D2 Bridge Painting 2009-3



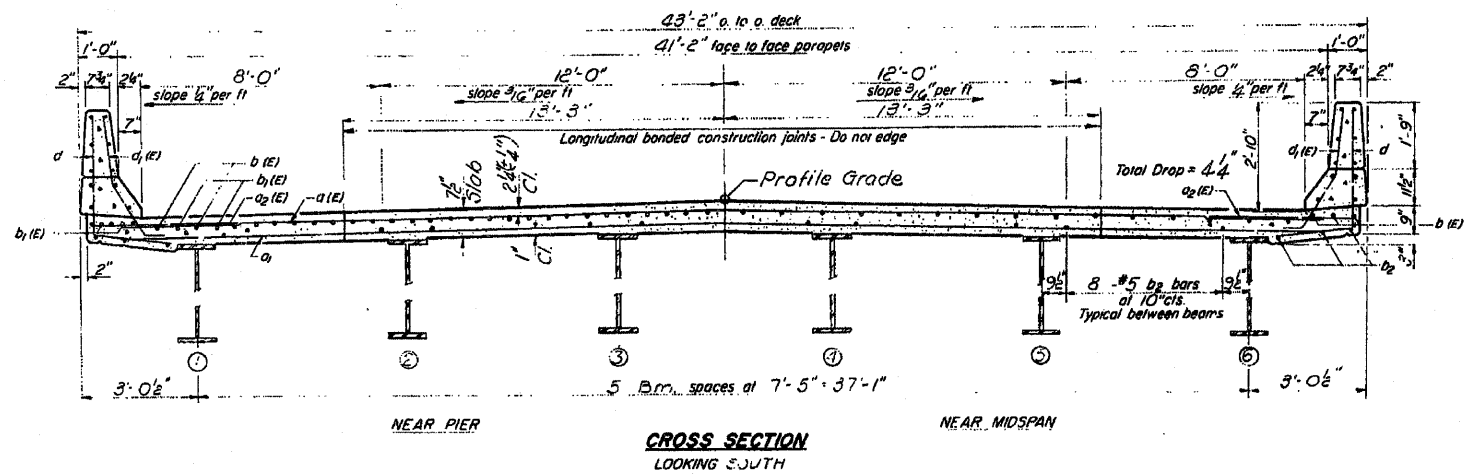
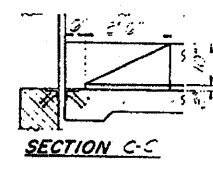
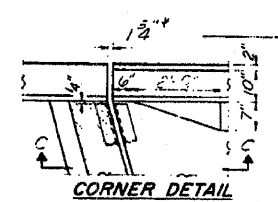
# FOR INFORMATION ONLY

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

|           |                      |           |              |           |
|-----------|----------------------|-----------|--------------|-----------|
| ROUTE NO. | SECTION              | COUNTY    | TOTAL SHEETS | SHEET NO. |
| FA 118    | 107B<br>107B<br>107B | WHITESIDE | 61           | 52        |



**PLAN**  
\* At 50° F  
\*\* Min. lap 1'-8"



**NOTES:**  
See sheet # 5 for superstructure details and Bill of Material  
Reinforcement bars designated (E) shall be epoxy coated. See Special Provisions  
Bars indicated thus 20 x 3 #5 etc indicates 20 lines of bars with 3 lengths per line  
For sections A-A and B-B see sheet # 5

|          |          |
|----------|----------|
| DESIGNED | EXAMINED |
| CHECKED  | PASSED   |
| DRAWN    | APPROVED |
| CHECKED  |          |

**SUPERSTRUCTURE**  
F.A. RT. 118 SEC. 107B  
WHITESIDE COUNTY  
STA. 738+50

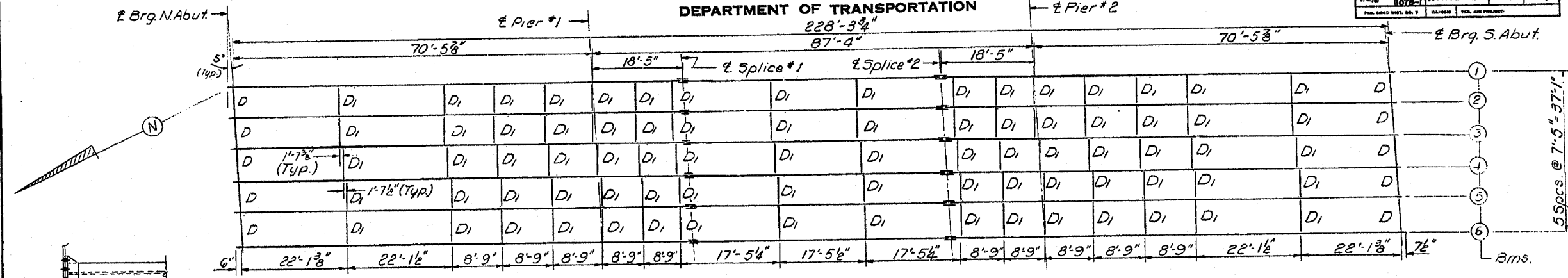
\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*\* D2 Bridge Painting 2009-3

|  |                    |            |           |   |                                   |           |           |        |              |                           |
|--|--------------------|------------|-----------|---|-----------------------------------|-----------|-----------|--------|--------------|---------------------------|
| FILE NAME = P:\PAINTING\64E93\PLAN\ang.dgn | USER NAME = linkdj | DESIGNED - | REVISED - | <b>STATE OF ILLINOIS<br/>DEPARTMENT OF TRANSPORTATION</b> | <b>Existing Plans SN 098-0095</b> | F.A. RTE. | SECTION   | COUNTY | TOTAL SHEETS | SHEET NO.                 |
| PLOT SCALE = 50.0000' / IN.                | CHECKED -          | REVISED -  |           |   |                                   |           | Whiteside | 15     | 9            |                           |
| PLOT DATE = Thu Jan 29 10:20:19 2009       | DATE -             | REVISED -  |           |   |                                   |           |           |        |              | CONTRACT NO. 64E93        |
|  |                    |            |           |   |                                   |           |           |        |              | ILLINOIS FED. AID PROJECT |

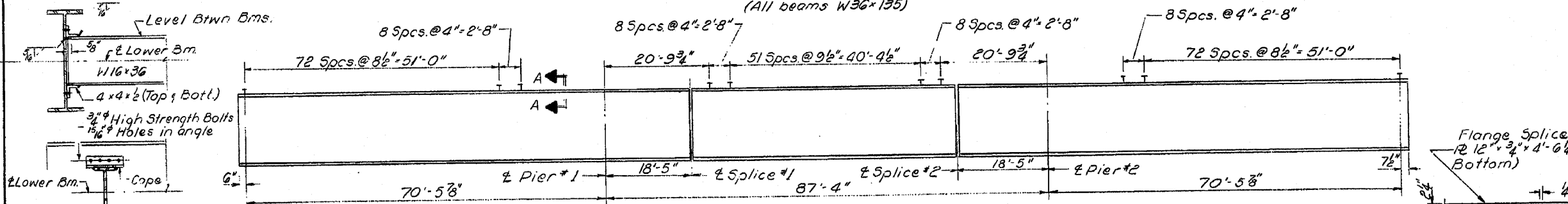
# FOR INFORMATION ONLY

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

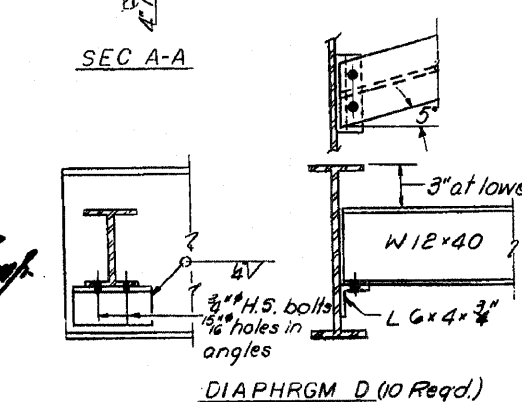
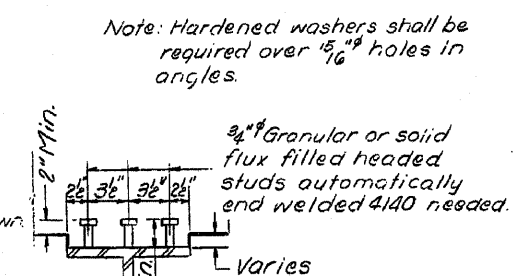
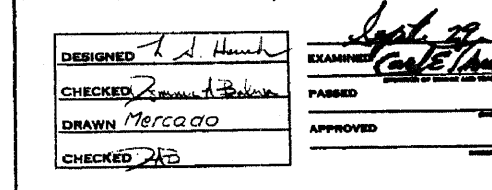
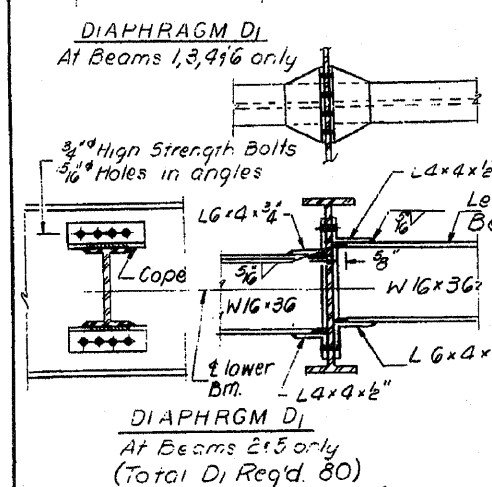
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|-----------|---------|-----------|-----------|--------------|
| ROUTE NO. | SECTION | COUNTY    | SHEET NO. | TOTAL SHEETS |
| 107       | 107B-1  | WHITESIDE | 61        | 54           |
|           |         |           |           | 13 SHEETS    |



FRAMING PLAN  
(All beams W36x135)



ELEVATION



INTERIOR BEAM MOMENT TABLE

|                          | 4Sp. for 3 | Piers | 5 Span 2 |
|--------------------------|------------|-------|----------|
| $I_s$ (in <sup>4</sup> ) | 7800       | 7800  | 7800     |
| $I_c$ (in <sup>4</sup> ) | 20632      |       | 20632    |
| $S_s$ (in <sup>3</sup> ) | 439        | 439   | 439      |
| $S_c$ (in <sup>3</sup> ) | 644        |       | 644      |
| $Z_s$ (in <sup>2</sup> ) | 509        | 509   | 509      |
| $Z_c$ (in <sup>2</sup> ) | 0.872      | 1.262 | 0.872    |
| $S^2$ (Kil)              | 0.390      |       | 0.390    |
| $M^2$ (IK)               | 300        | 742   | 282      |
| $M_s^2$ (IK)             | 156        |       | 180      |
| $M^2$ (IK)               | 556        | 342   | 605      |
| $M$ Imp. (IK)            | 142        | 84    | 143      |
| $M$ (k + Imp)            | 698        | 426   | 748      |
| $M$ max. (IK)            | 2105       | 1888  | 2221     |
| $M_u$ (IK)               | 3722       | 2121  | 3722     |
| $VR$ (K)                 | 54.21      |       | 45.86    |

INTERIOR BEAM REACTION TABLE

|               | Abuts | Piers  |
|---------------|-------|--------|
| $R^2$ (K)     | 33.95 | 110.11 |
| $R^4$ (K)     | 39.54 | 53.88  |
| $Imp$ (K)     | 10.11 | 18.21  |
| $R$ Total (K) | 83.6  | 177.2  |

$M_{max}$  is the maximum moment induced by the maximum design load  $\cdot 1.3(M_{12} + M_{s2} + \dots)$   
 $M_u$  is the maximum moment capacity.

TOP OF BEAM ELEVATIONS (12 Req'd)

|                 | Bm #1  | Bm #2  | Bm #3  | Bm #4  | Bm #5  | Bm #6  |
|-----------------|--------|--------|--------|--------|--------|--------|
| ± Brg. N. Abut. | 603.59 | 603.74 | 603.86 | 603.85 | 603.74 | 603.59 |
| ± Pier #1       | 603.46 | 603.60 | 603.72 | 603.71 | 603.60 | 603.45 |
| ± Splice #1     | 603.42 | 603.56 | 603.68 | 603.68 | 603.56 | 603.41 |
| ± Splice #2     | 603.31 | 603.46 | 603.58 | 603.58 | 603.46 | 603.31 |
| ± Pier #2       | 603.28 | 603.43 | 603.54 | 603.54 | 603.43 | 603.27 |
| ± Brg. S. Abut. | 603.14 | 603.28 | 603.40 | 603.40 | 603.28 | 603.13 |

STRUCTURAL STEEL  
F.A. RT. 18 SEC. 107 B-1  
WHITESIDE COUNTY  
STA. 738+23.00

DESIGNED: T. J. Hensch  
CHECKED: [Signature]  
DRAWN: Mercado  
APPROVED: [Signature]

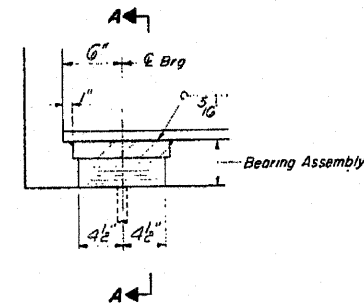
$I_s$  and  $S_s$  are the moment of inertia and Section Modulus of the steel section.  $Z_s$  is the plastic section modulus of the steel section.  $I_c$  and  $S_c$  are the Moment of Inertia and Section Modulus of the Composite Section.  $VR$  is the Maximum  $\pm$  Imp. Shear Range in span used to determine Shear Connector Spacing.

\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*\* D2 Bridge Painting 2009-3

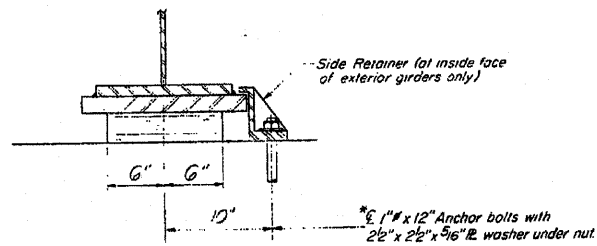
# FOR INFORMATION ONLY

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

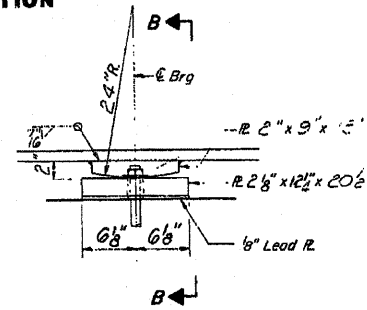
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|-------------|-----------|--------|--------------|-----------|
| PROJECT NO. | SECTION   | COUNTY | TOTAL SHEETS | SHEET NO. |
| 107B-1      | WHITESIDE | 61     | 55           | 13        |



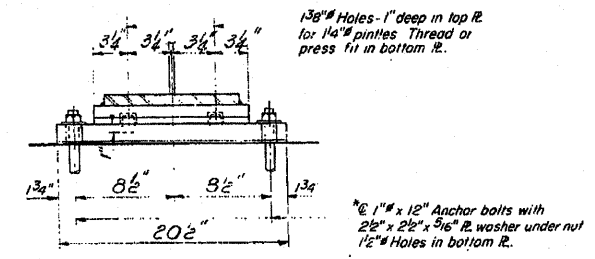
SECTION AT ABUT.



SECTION A-A



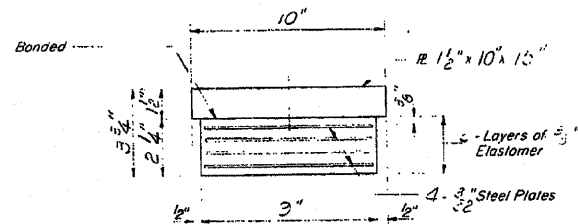
ELEVATION AT PIER #1



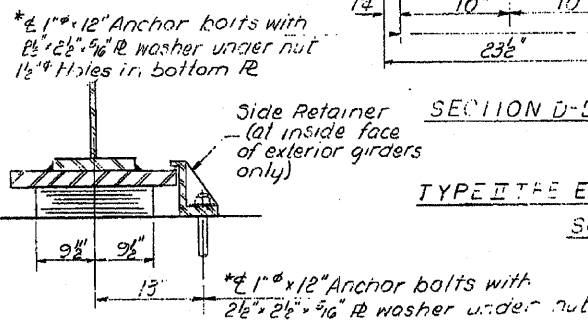
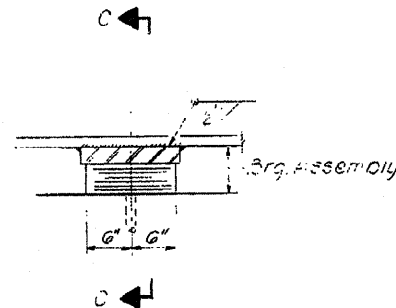
SECTION B-B

**TYPE I ELASTOMERIC EXP. BRG.**  
NORTH ABUTMENT

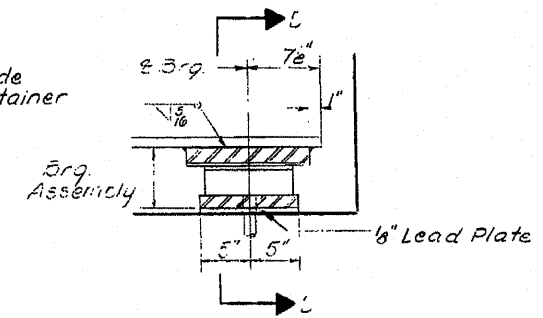
\*Note: After beams have been erected holes at expansion bearings shall be drilled and anchor bolts grouted in place. Anchor bolts of fixed bearings may be built into the masonry.



BEARING ASSEMBLY



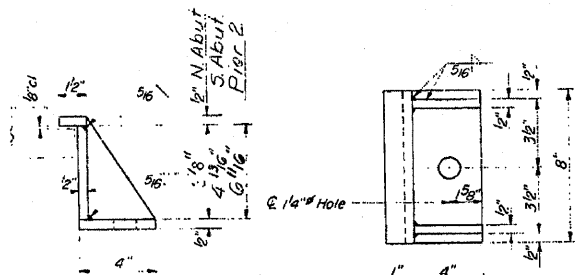
SECTION D-D



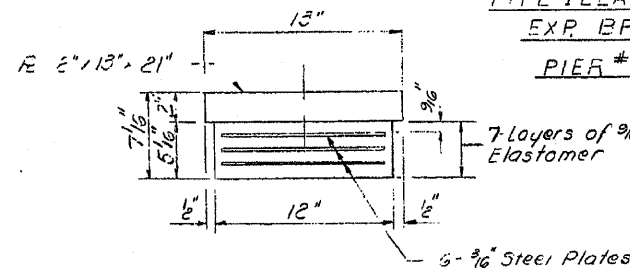
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**TYPE I ELASTOMERIC EXP. BRG.**  
PIER #2

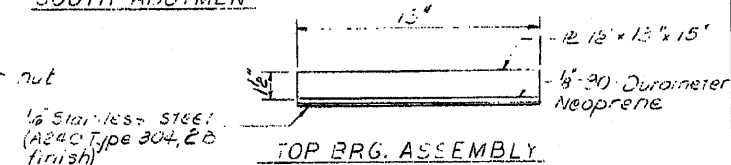
Note: The 1/16 TFE sheet shall be bonded directly to the top steel laminate sheet with a two component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces.



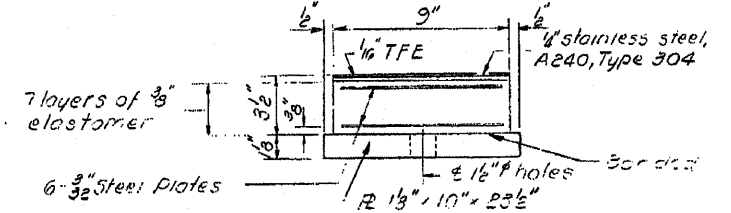
SIDE RETAINER



BEARING ASSEMBLY



TOP BRG. ASSEMBLY



BOTTOM BRG. ASSEMBLY

**BEARING DETAILS**  
F.A.R.T.18 SEC.107B-1  
WHITESIDE COUNTY  
STA. 738+23.00

|          |          |
|----------|----------|
| DESIGNED | EXAMINED |
| CHECKED  | PASSED   |
| DRAWN    | APPROVED |
| CHECKED  |          |

I-2-E1 2-1-76

\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*\*D2 Bridge Painting 2009-3

|   |                                      |            |           |   |   |           |         |           |              |                           |
|---|--------------------------------------|------------|-----------|---|---|-----------|---------|-----------|--------------|---------------------------|
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| PLOT SCALE = 50.0000 / IN.                | PLOT DATE = Thu Jan 29 10:20:35 2009 | DRAWN -    | REVISED - |   | SCALE: SHEET NO. OF SHEETS STA. TO STA. | ..        | ..      | Whiteside | 15           | 11                        |
|   |                                      | CHECKED -  | REVISED - |   |   |           |         |           |              |                           |
|   |                                      | DATE -     | REVISED - |   |   |           |         |           |              |                           |
|   |                                      |            |           |   |   |           |         |           |              | ILLINOIS FED. AID PROJECT |

# FOR INFORMATION ONLY

098-0096

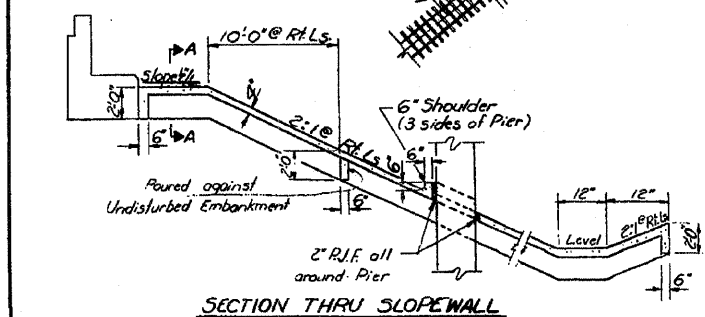
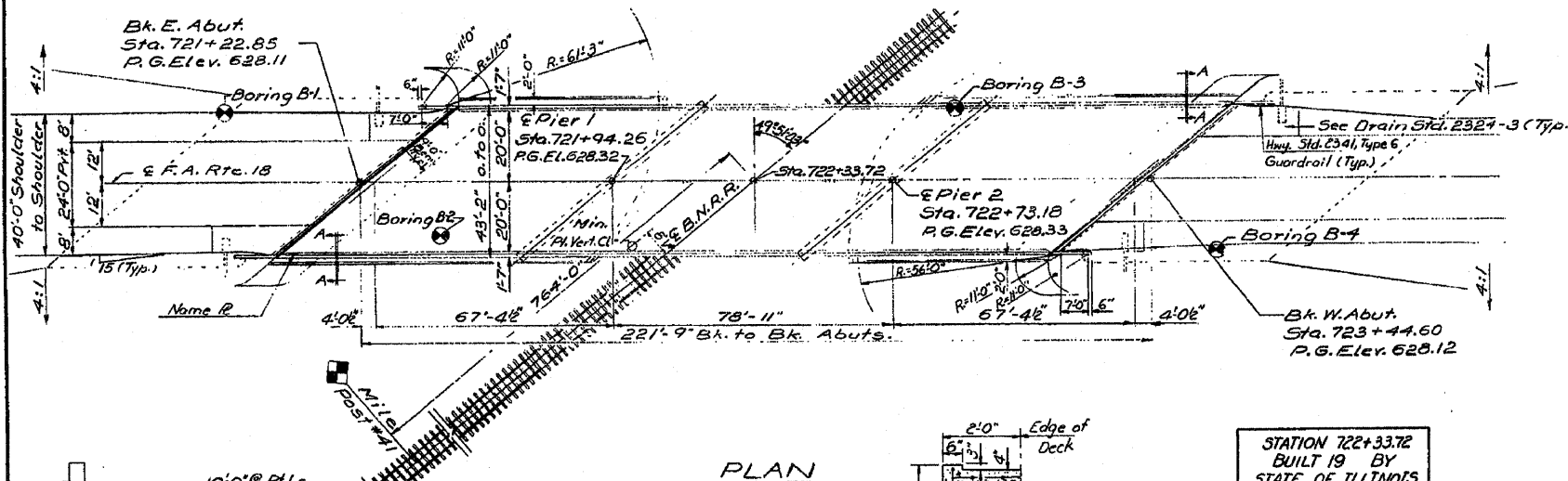
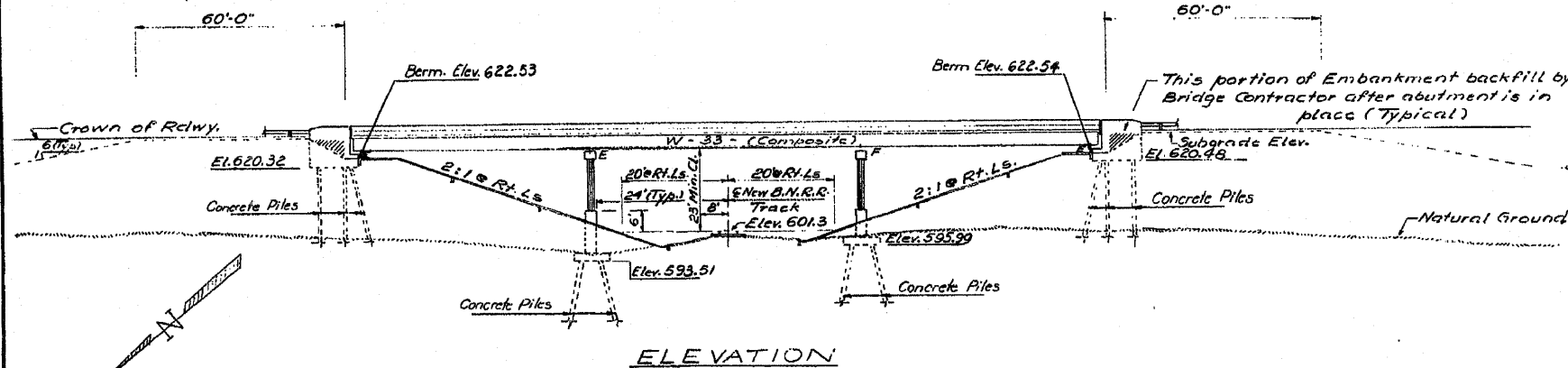
B.M.: Chis "A" on S. End E. Hubguard of CB & Q RR Bridge 722+03 Elev. 624.695.  
 Existing Structure: N 098-0019 built as S.B.I. 80, Sec. 107 VB in 1933 @ Sta. 720+92.3  
 Concrete slab on steel Bms., Timber substructure (Five Span, 209'-10" Back to Back and 25'-0" Out to Out) to be removed by contractor at time of construction.

STATE OF ILLINOIS  
 DEPARTMENT OF TRANSPORTATION

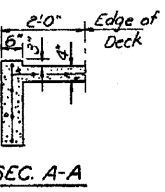
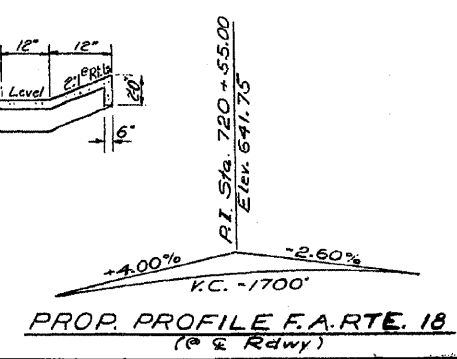
|             |         |           |              |           |
|-------------|---------|-----------|--------------|-----------|
| PROJECT NO. | SECTION | COUNTY    | TOTAL SHEETS | SHEET NO. |
| 107VB-1     | 61      | WHITESIDE | 36           | 13        |

### GENERAL NOTES

- Reinforcement bars shall conform to the requirements of AASHTO M31 or M53, Grade 60.
- See Proposal for Boring Data.
- Fasteners shall be high strength bolts. Bolts  $\frac{3}{4}$ " open holes  $\frac{1}{8}$ " unless otherwise noted.
- Calculated weight of Structural Steel = 199,710 Pounds.
- The basic lead silico chromate point system shall be used for shop and field painting of Structural Steel.
- Field welding of construction accessories will not be permitted to the bottom flange of beams near to the top flange for a distance equal to one-fourth the span length each way from the pier supports. Field welding in other areas will be permitted only when approved by the Engineer.
- Anchor bolts shall be set before bolting diaphragms over supports.
- Slope wall shall be reinforced with welded wire fabric 6"x6" mesh, weighing 50 lbs. per 100 sq. ft.
- Concrete piles at abutments shall be driven in holes precored through the embankment in accordance with Article 513.09(c) of the Standard Specifications.
- The concrete rail section above the mandatory construction joint at the top of the slab shall be constructed of Class X Concrete, except the aggregates shall conform to the requirements of Handrail Concrete.
- Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of  $\frac{1}{8}$ ". Adjustment shall be made either by grinding the surface or by shimming the bearing. Two 6" adjusting shims, of the dimensions of the bottom bearing plate, shall be provided for each bearing in addition to all other plates or shims.
- The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Notch Toughness Zone 2. These components are the tension flanges, webs and all splice plate material of the wide flange beams.
- The Contractor shall drive one concrete test pile each in a permanent location at Pier #1 and West Abutment as directed by the Engineer before ordering the remainder of piles.
- The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments.



|          |              |          |                   |
|----------|--------------|----------|-------------------|
| DESIGNED | Suresh Desai | EXAMINED | November 21, 2008 |
| CHECKED  | DAU KRULL    | PASSED   |                   |
| DRAWN    | R. Doly      | APPROVED |                   |
| CHECKED  | D.K.         |          |                   |



STATION 722+33.72  
 BUILT 19 BY  
 STATE OF ILLINOIS  
 F.A. RTE. 18 SEC. 107VB-1  
 PROJECT: GR-18(100)  
 LOADING HS20  
 \*STR. NO.  
 NAME PLATE  
 (See Std. 2113)

\*Structure Number to be supplied by District.

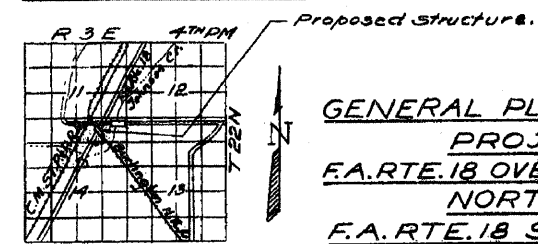
### DESIGN STRESSES

$f_c = 3,500 \text{ psi}$   
 $f_y = 60,000 \text{ psi (Reinf.)}$   
 $f_y = 50,000 \text{ psi (Struct. Steel M-223, Grade 50)}$

Loading HS20-44  
 Allow 25% for Future W.S.  
 Design Specifications: 1977 AASHTO  
 and 1978 Interim Specifications.  
 Epoxy Coated Reinf. Bars shall be used  
 in the top layer of the slab.

### TOTAL BILL OF MATERIAL

| Item                              | Unit     | Super  | Sub    | Total  |
|-----------------------------------|----------|--------|--------|--------|
| Removal of Existing Structures    | Each     |        |        | 2      |
| Structure Excavation              | Cu. Yd.  |        | 187    | 187    |
| Protective Coat                   | Sq. Yd.  | 1140   | 18     | 1158   |
| Class X Concrete                  | Cu. Yd.  | 274.7  | 329.4  | 604.1  |
| Structural Steel                  | Lump Sum |        |        | 1      |
| Stud Shear Connectors             | Each     | 3186   |        | 3186   |
| Reinforcement Bars                | Pound    | 29,660 | 29,460 | 59,120 |
| Reinforcement Bars (Epoxy Coated) | Pound    | 44,270 |        | 44,270 |
| Concrete Piles                    | Lin. Ft. |        | 4344   | 4344   |
| Test Piles Concrete               | Each     |        | 2      | 2      |
| Name Plates                       | Each     | 1      |        | 1      |
| Slope Wall (4')                   | Sq. Yd.  |        | 865    | 865    |
| Neoprene Expansion Joint (2')     | Lin. Ft. | 132    |        | 132    |



GENERAL PLAN & ELEVATION  
 PROJECT: GR-18(100)  
 F.A. RTE. 18 OVER BURLINGTON  
 NORTHERN R.R.  
 F.A. RTE. 18 SECTION 107VB-1  
 WHITESIDE COUNTY  
 STA. 722+33.72

### LOCATION SKETCH

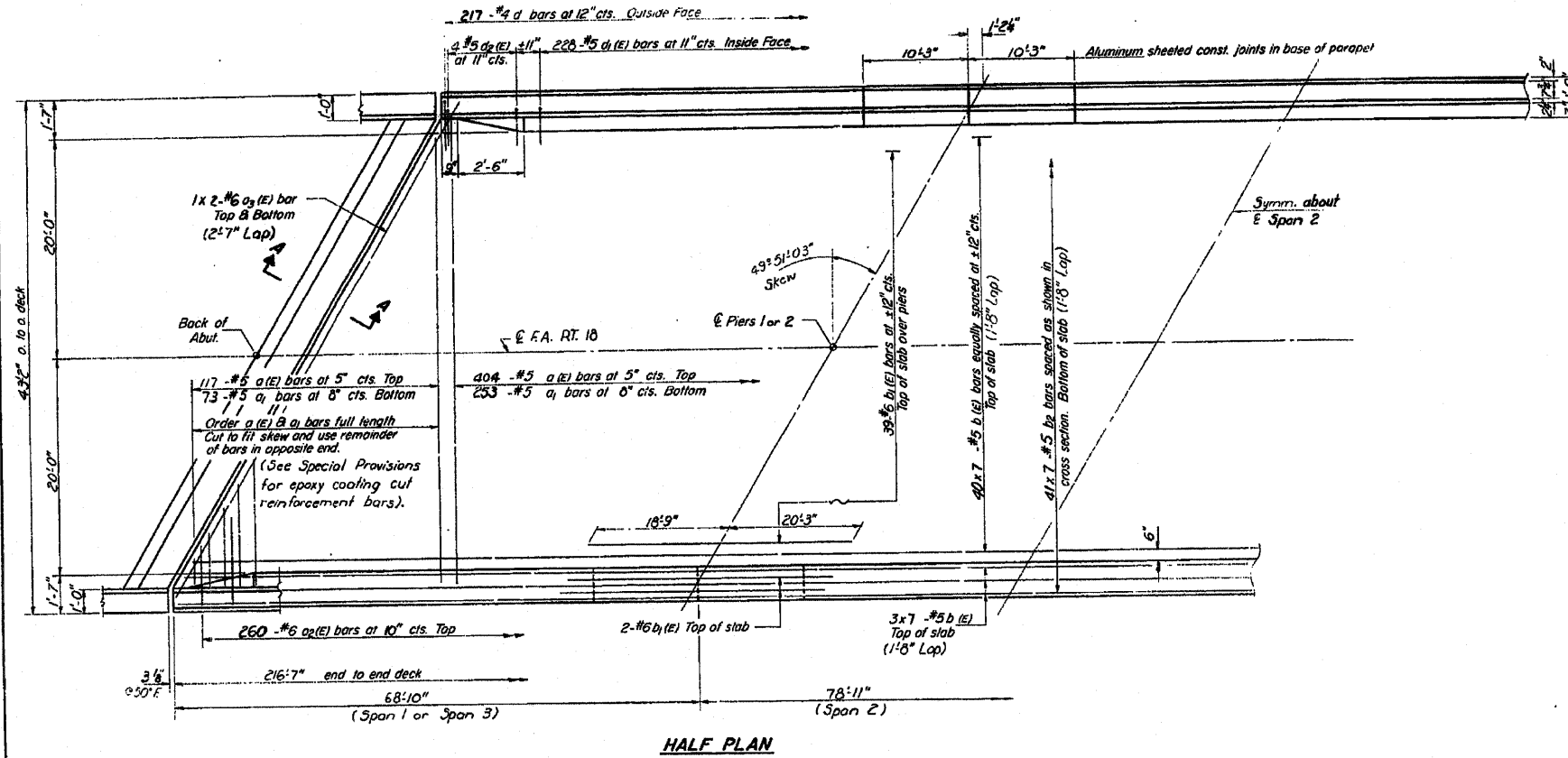
\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
 \*\* D2 Bridge Painting 2009-3

|   |                   |            |             |   |   |   |
|---|-------------------|------------|-------------|---|---|---|
| FILE NAME = P:\PAINTING\64E93\PLA\eng.dgn | USER NAME = jinkd | DESIGNED - | REVISIONS - | STATE OF ILLINOIS<br>DEPARTMENT OF TRANSPORTATION | Existing Plans SN 098-0096              | F.A. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO. |
| PLOT SCALE = 50,0000' / IN.               |                   | DRAWN -    | REVISIONS - |   | SCALE: SHEET NO. OF SHEETS STA. TO STA. | Whiteside 15 12                                 |
| PLOT DATE = Thu Jan 29 10:20:45 2009      |                   | CHECKED -  | REVISIONS - |   |   | CONTRACT NO. 64E93                              |
|   |                   | DATE -     | REVISIONS - |   |   | ILLINOIS FED. AID PROJECT                       |

# FOR INFORMATION ONLY

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

|           |         |           |              |           |
|-----------|---------|-----------|--------------|-----------|
| ROUTE NO. | SECTION | COUNTY    | TOTAL SHEETS | SHEET NO. |
| 1197-1    | 107VB-1 | WHITESIDE | 61           | 39        |
| 1197-1    | 107VB-1 |           |              | 13 SHEETS |



**TOP OF FLANGE ELEVATIONS**  
\*(Before any deflection)

|                 | Beam 1 | Beam 2 | Beam 3 | Beam 4 | Beam 5 | Beam 6 |
|-----------------|--------|--------|--------|--------|--------|--------|
| E Brg. E. Abut. | 627.22 | 627.34 | 627.42 | 627.34 | 627.23 | 627.04 |
| E Pier #1       | 627.30 | 627.44 | 627.55 | 627.49 | 627.40 | 627.23 |
| E Splice #1     | 627.32 | 627.47 | 627.58 | 627.53 | 627.45 | 627.28 |
| E Splice #2     | 627.29 | 627.45 | 627.53 | 627.58 | 627.47 | 627.32 |
| E Pier #2       | 627.24 | 627.41 | 627.49 | 627.55 | 627.45 | 627.30 |
| E Brg. W. Abut. | 627.05 | 627.24 | 627.35 | 627.43 | 627.35 | 627.23 |

\* For fabrication only

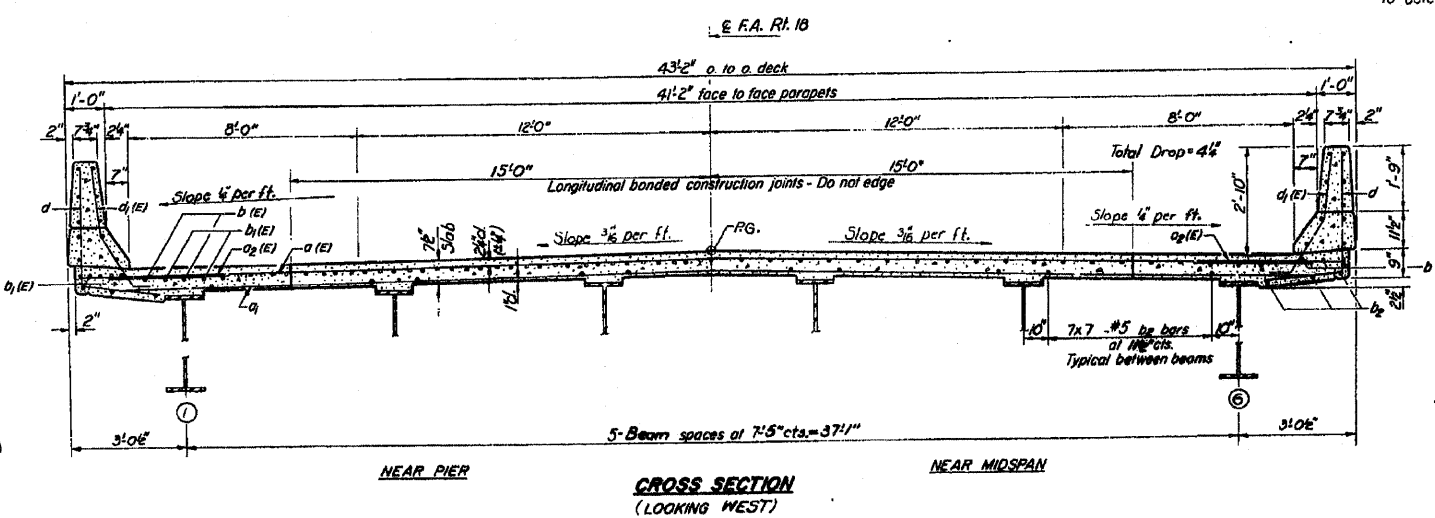
**INTERIOR BEAM MOMENT TABLE**

|                          | 4 Splice #3 Pier 1 or 2 | 5 Span 2 |
|--------------------------|-------------------------|----------|
| $I_s$ (in <sup>4</sup> ) | 6710                    | 6710     |
| $I_c$ (in <sup>4</sup> ) | 17173                   | 17173    |
| $S_s$ (in <sup>3</sup> ) | 405                     | 405      |
| $S_c$ (in <sup>3</sup> ) | 582                     | 582      |
| $R$ (in)                 | .862                    | .862     |
| $M_R$ (in)               | 206                     | 463      |
| $M_P$ (in)               | .323                    | .323     |
| $M_S$ (in)               | 122                     | 137      |
| $M_L$ (in)               | 544                     | 320      |
| $M_{max}$ (in)           | 142                     | 80       |
|                          | 2020                    | 1648     |
|                          | 3308                    | 1688     |
| $VR$ (k)                 | 53.9                    | 47.8     |

**INTERIOR BEAM REACTION TABLE**

|                 | E. Abut. | Pier 1 or 2 | W. Abut. |
|-----------------|----------|-------------|----------|
| $R_R$ (k)       | 31.1     | 95.7        | 31.1     |
| $R_L$ (k)       | 40.8     | 53.4        | 40.8     |
| $I_{mp}$ (k)    | 10.6     | 13.3        | 10.6     |
| $R_{TOTAL}$ (k) | 82.5     | 162.4       | 82.5     |

$I_s$  and  $S_s$  are the moment of inertia and section modulus of the steel section used in computing  $I_s$  TOTAL.  
 $I_c$  and  $S_c$  are the moment of inertia and section modulus of the composite section used in computing  $I_s$  TOTAL.  
 $VR$  is the maximum  $k+I$  impact shear range in span used to determine shear connector spacing.



**NOTES:**  
 See sheet #5 for superstructure details and Bill of Material.  
 Reinforcement bars designated (E) shall be epoxy coated. See Special Provisions.  
 Bars indicated thus 20 x 3-#5 etc. indicate 20 lines of bars with 3 lengths per line.

|          |              |
|----------|--------------|
| DESIGNED | Suresh Desai |
| CHECKED  | DAW KRULL    |
| DRAWN    | P. Joty      |
| CHECKED  | D.N.         |

|          |             |
|----------|-------------|
| EXAMINED | [Signature] |
| PASSED   | [Signature] |
| APPROVED | [Signature] |

S-I-L 1159 9-27-76

**SUPERSTRUCTURE**  
 F.A. RT. 10 SEC. 107VB-1  
 WHITESIDE COUNTY  
 STA. 722+33.72

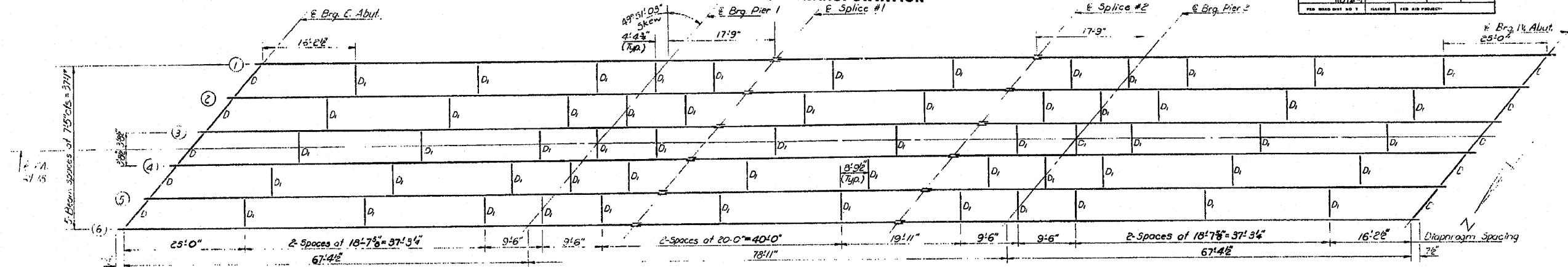
\* FAS 1197 (West Lincolnway Road) & FAP 308 (IL 84)  
 \*\*D2 Bridge Painting 2009-3

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| PLOT SCALE = 50.0000' / IN.               |                   | DRAWN -    | REVISED - |   |  |           |         | Whiteside | 15                        |
| PLOT DATE = Thu Jan 29 10:22:53 2009      |                   | CHECKED -  | REVISED - |   | SCALE: _____ SHEET NO. ____ OF ____ SHEETS |           |         |           | 13                        |
|   |                   | DATE -     | REVISED - |   | STA. _____ TO STA. _____                   |           |         |           | CONTRACT NO. 64E93        |
|   |                   |            |           |   |  |           |         |           | ILLINOIS FED. AID PROJECT |

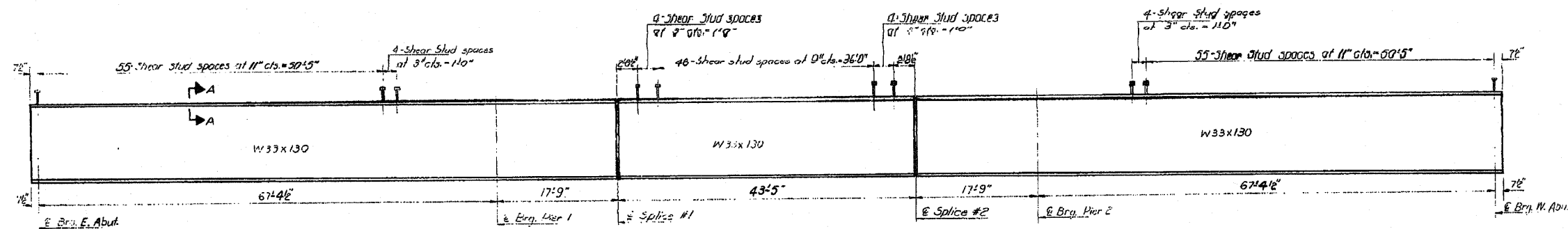
# FOR INFORMATION ONLY

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

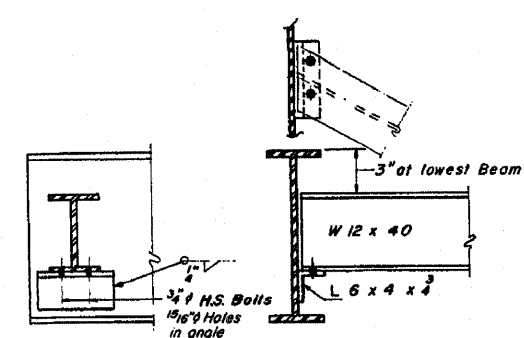
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|----------------------|----------|-----------------|--------------|-----------|-------------|
| PROJECT NO.          | SECTION  | COUNTY          | TOTAL SHEETS | SHEET NO. | SHEET NO. 7 |
| 197-1                | 107WB-1  | WHITESIDE       | 61           | 42        | 13 SHEETS   |
| 107B-1               |          |                 |              |           |             |
| FOR ROAD DIST. NO. 7 | ILLINOIS | FOR AID PROJECT |              |           |             |



FRAMING PLAN

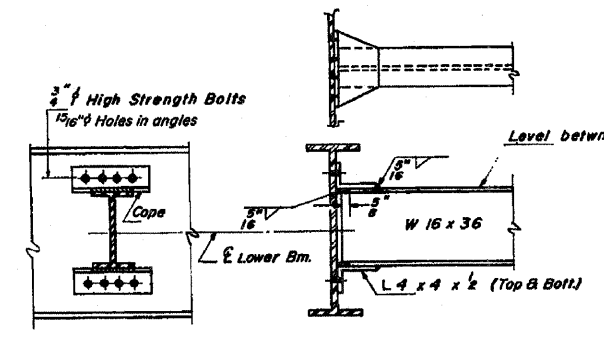


BEAM ELEVATION



DIAPHRAGM D

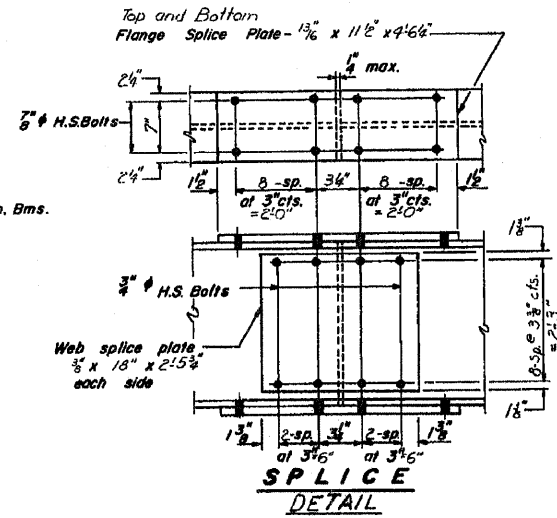
10 - Required



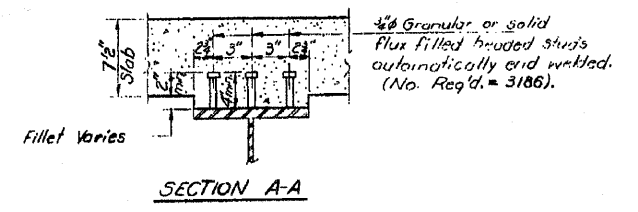
DIAPHRAGM D1

60 - Required

Note: Hardened washers shall be required over 1 1/2" holes in angles.



SPlice DETAIL



SECTION A-A

Note: For Top of Flange Elevations Table, Interior Beam Moment Table and Interior Beam Reaction Table see sheet #4.

|                       |                     |
|-----------------------|---------------------|
| DESIGNED Suresh Desai | EXAMINED            |
| CHECKED               | PASSED              |
| DRAWN R. Dury         | APPROVED            |
| CHECKED               | INCHARGE OF PROJECT |

I-2-D 4-15-73

STRUCTURAL STEEL  
F.A. RT. 10 SEC. 107WB-1  
WHITESIDE COUNTY  
STA. 722+33.72

\* FAS 197 (West Lincolnway Road) & FAP 308 (IL 84)  
\*\*D2 Bridge Painting 2009-3

|   |                   |            |           |   |                            |                           |         |        |              |           |
|---|-------------------|------------|-----------|---|----------------------------|---------------------------|---------|--------|--------------|-----------|
| FILE NAME = P:\PAINTING\64E93\PLA\Ang.dgn | USER NAME = Iinkd | DESIGNED - | REVISED - | STATE OF ILLINOIS<br>DEPARTMENT OF TRANSPORTATION | Existing Plans SN 098-0096 | F.A. RTE.                 | SECTION | COUNTY | TOTAL SHEETS | SHEET NO. |
| PLOT SCALE = 5/8" = 1' IN.                | DRAWN -           | REVISED -  | REVISED - |   |                            | Whiteside                 | 15      | 14     |              |           |
| PLOT DATE = Thu Jan 29 10:21:04 2009      | CHECKED -         | REVISED -  | REVISED - |   |                            | CONTRACT NO. 64E93        |         |        |              |           |
|   | DATE -            | REVISED -  | REVISED - |   |                            | ILLINOIS FED. AID PROJECT |         |        |              |           |

