

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	1
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT BRS-1365(111)	
*05-00626-00-BR		CONTRACT #87368		

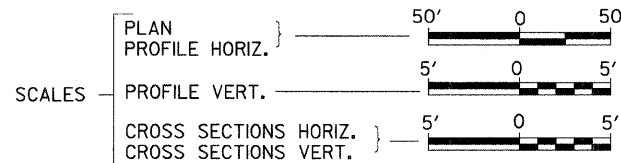
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
PLANS FOR PROPOSED
MAJOR BRIDGE FUNDS
LASALLE COUNTY
SECTION 05-00626-00-BR
F.A.S. 1365 (CH 33) OVER
LITTLE VERMILION RIVER
PROJECT NO. BRS-1365(111)
JOB NUMBER C-93-116-08

INDEX OF SHEETS

SHEET NO.	DESCRIPTION
1.	COVER SHEET
2.	GENERAL NOTES, DETAILS, AND TYPICAL SECTIONS
3.	SUMMARY OF QUANTITIES AND DETAILS
4.	SCHEDULES OF QUANTITIES
5.	TRAFFIC CONTROL PLAN
6.-7.	STORM WATER POLLUTION PREVENTION PLAN
8.-9.	PLAN AND PROFILE
10.-31.	STRUCTURE PLANS
32.-53.	CROSS SECTIONS

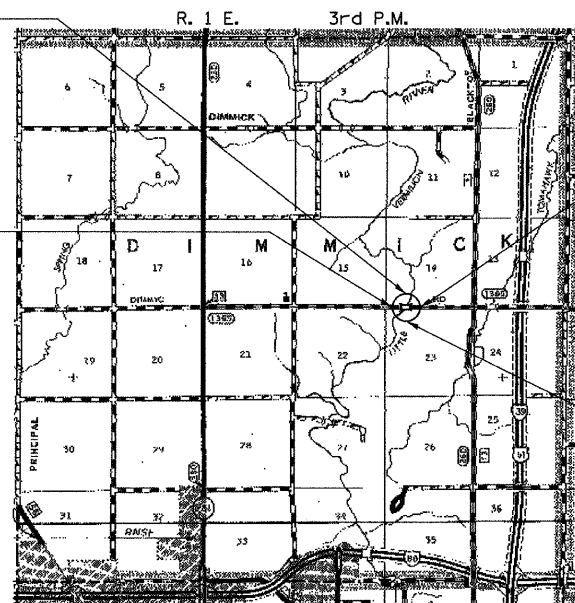
REQUIRED HIGHWAY STANDARDS

000001-05	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
280001-04	TEMPORARY EROSION CONTROL SYSTEMS
515001-03	NAME PLATE FOR BRIDGES
542101-02	REINFORCED CONCRETE END SECTIONS FOR PIPE CULVERTS 15" (375 MM) THRU 36" (900 MM) DIA. AT RIGHT ANGLES WITH ROADWAY
542401-01	METAL END SECTION FOR PIPE CULVERTS
601101-01	CONCRETE HEADWALL FOR PIPE DRAIN
606001-04	CONCRETE CURB TYPE B AND COMBINATION CONCRETE CURB AND GUTTER
606401-01	PAVED DITCH
609006-04	BRIDGE APPROACH PAVEMENT (DRAIN DETAIL)
630001-08	STEEL PLATE BEAM GUARDRAIL
630301-05	SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS
631031-07	TRAFFIC BARRIER TERMINAL, TYPE 6
635006-03	REFLECTOR AND TERMINAL MARKER PLACEMENT
635011-02	REFLECTOR MARKER AND MOUNTING DETAILS
701901-01	TRAFFIC CONTROL DEVICES
780001-02	TYPICAL PAVEMENT MARKINGS
BLR 21-8	TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES FOR CONSTRUCTION ON RURAL LOCAL HIGHWAYS
BLR 24-2	MAILBOX TURNOUT FOR LOCAL ROADS



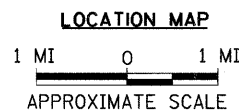
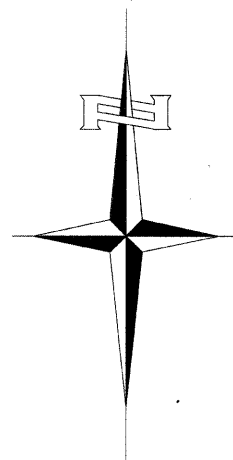
PROPOSED STRUCTURE NO. 050-3587
 THREE SPAN 36" P.P.C. I-DECK BEAM
 WITH CONCRETE DECK STRUCTURE
 ON CONC. INTEGRAL ABUTMENTS
 AND CONC. ENCASED PIERS,
 169'-4" BK. TO BK. AND 35'-2" O. TO O.,
 30° LT. AH. SKEW

SECTION 05-00626-00-BR
 BEGINS
 STATION 12+25.00



SECTION 05-00626-00-BR
 ENDS
 STATION 31+25.00

EXISTING STRUCTURE 050-3055
 SINGLE SPAN CONCRETE T-BEAM
 SUPERSTRUCTURE ON CONCRETE CLOSED
 ABUTMENTS ON TIMBER PILE SUPPORTED
 CONCRETE FOOTINGS,
 ±53'-11" BK. TO BK., AND ±25'-4" O. TO O.,
 30° LT. AH. SKEW (TO BE REMOVED)



UTILITY COMPANIES

CORN BELT ENERGY CORP.
 PRINCETON, ILLINOIS

NICOR GAS
 OTTAWA, ILLINOIS

AT&T
 JOLIET, ILLINOIS

CALL J.U.L.I.E.
 BEFORE YOU DIG
 1-800-892-0123 OR 811

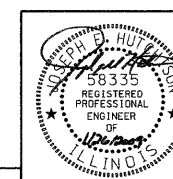
CONTRACT NO. 87368

NET LENGTH OF PROJECT = 1,900.00 FEET = 0.36 MILES
 DESIGN CLASSIFICATION: MAJOR-COLLECTOR (NON-URBAN)
 DESIGN ADT = 1000 (2029)
 DESIGN SPEED = 50 MPH

Hutchison Engineering, Inc.
 JACKSONVILLE ILLINOIS
 SHOREWOOD ILLINOIS

2008

JOB#2498



George E. Ryan
 SIGNATURE
 ENGINEER'S SEAL

APPROVED 1/29 2009

James G. King
 LASALLE COUNTY ENGINEER

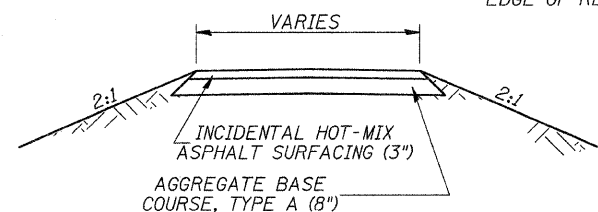
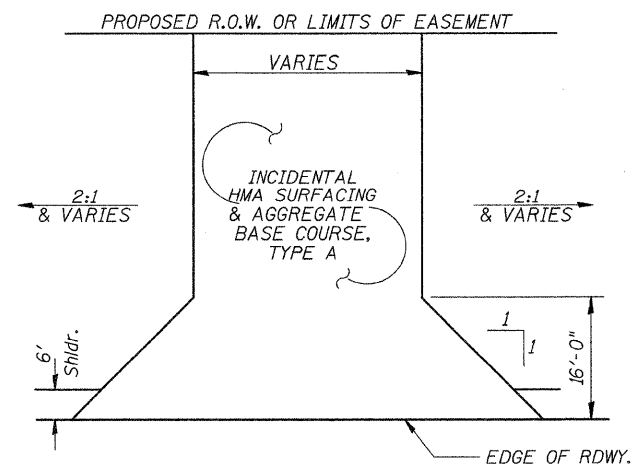
PASSED 01-30-09 2009

Keneth R. Lopez
 DISTRICT THREE ENGINEER OF
 LOCAL ROADS & STREETS

Released For
 Bld Based on
 Limited Review 01-30-09 2009

George E. Ryan
 DEPUTY DIRECTOR OF HIGHWAYS,
 REGION TWO ENGINEER

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION



PROPOSED PRIVATE ENTRANCES

- * STA 24+67 RT (16' WIDE)
- STA 26+40 LT (42' WIDE)
- STA 28+54 RT (24' WIDE)
- STA 28+92 LT (24' WIDE)

* SEE SHT. #3 OF 53 FOR ADDITIONAL DETAILS.

HOT-MIX ASPHALT MIXTURE REQUIREMENTS

	HMA BINDER	HMA SURFACE	HMA SHOULDERS
PG GRADE	PG 64-22	PG 64-22	PG 58-22
MAX % RAP ALLOWABLE**	25%	15%	50%
DESIGN AIR VOIDS	4% @ N50	4% @ N50	2% @ N30
MIXTURE COMPOSITION	IL-19.0	IL-12.5 OR IL-9.5	IL-19.0
FRICTION AGGREGATE		MIXTURE C	
DENSITY TEST METHOD	CORES*	CORES	*

* MATERIAL SHALL BE COMPACTED TO 93.0-97.4 PERCENT OF THE MAXIMUM THEORETICAL DENSITY, EXCEPT THAT WHEN PLACED AS FIRST LIFT ON AN UNIMPROVED SUBGRADE THE MINIMUM PERCENT COMPACTION SHALL BE 92.0 PERCENT. THE MAXIMUM THEORETICAL DENSITY SHALL BE DETERMINED FROM THE MOVING AVERAGE AS SPECIFIED IN THE QC/QA SPECIFICATION.

** WHEN MORE THAN 20% RAP IS USED, A SOFTER ASPHALT BINDER (PG 58-22) MAY BE REQUIRED AS DETERMINED BY THE ENGINEER.

**STRUCTURAL DESIGN INFORMATION
COUNTY HIGHWAY 33**

ROAD CLASSIFICATION: CLASS III 80,000 lb./20 YEAR DESIGN
STRUCTURAL DESIGN TRAFFIC:
PV = 805 SU = 64 MU = 46
PERCENT OF STRUCTURAL DESIGN TRAFFIC IN DESIGN LANE:
P = 88% S = 7% M = 5%
MINIMUM SUBGRADE SUPPORT RATING: GRANULAR
FLEXIBLE PAVEMENT DESIGN: MINIMUM TF = 0.25
ASPHALT PAVEMENT THICKNESS: 8"
AGGREGATE BASE COURSE, TYPE A: 12"

GENERAL NOTES

PLAN QUANTITIES FOR TREE REMOVAL HAVE BEEN BASED ON ALL TREES WITHIN THE EXISTING RIGHT OF WAY. THIS QUANTITY MAY BE REVISED DURING CONSTRUCTION, AT THE DIRECTION OF THE ENGINEER, BY DELETING FROM THE TREE REMOVAL QUANTITIES, SUCH TREES THAT DO NOT INTERFERE WITH THE PROPOSED CONSTRUCTION.

THE REMOVAL OF EXISTING ASPHALT SURFACE AND GRAVEL OR CRUSHED STONE BASE COURSE WHICH MAY BE NECESSARY FOR THE CONSTRUCTION OF THE NEW BRIDGE SHALL BE REMOVED AS EARTH EXCAVATION AND NO COMPENSATION WILL BE ALLOWED FOR ADDITIONAL LABOR OR EQUIPMENT REQUIRED.

ALL WASTE OR UNDESIRABLE MATERIAL AS IDENTIFIED BY THE ENGINEER SHALL BE DISPOSED OF OUTSIDE THE LIMITS OF THE RIGHT OF WAY AT THE CONTRACTOR'S EXPENSE.

ALL EXISTING PRIVATELY OWNED UTILITIES REQUIRING ADJUSTMENT WILL BE MADE BY THE UTILITY COMPANY INVOLVED. WHERE NO PROVISIONS HAVE BEEN MADE FOR ADJUSTMENTS ON THE PLANS, NO ADDITIONAL COMPENSATION WILL BE ALLOWED DUE TO DELAYS OR INCONVENIENCES CAUSED BY THE SAID UTILITY ADJUSTMENTS.

THE PROFILE GRADE ELEVATIONS SHOWN ON THE PLAN AND PROFILE SHEETS AND IN THE STATION CROSS SECTIONS ARE TO THE TOP OF THE FINISHED SURFACE.

ALL EXISTING DRAINAGE STRUCTURES NOT BEING REMOVED BY THE CONTRACTOR THAT ARE DAMAGED DURING CONSTRUCTION SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE.

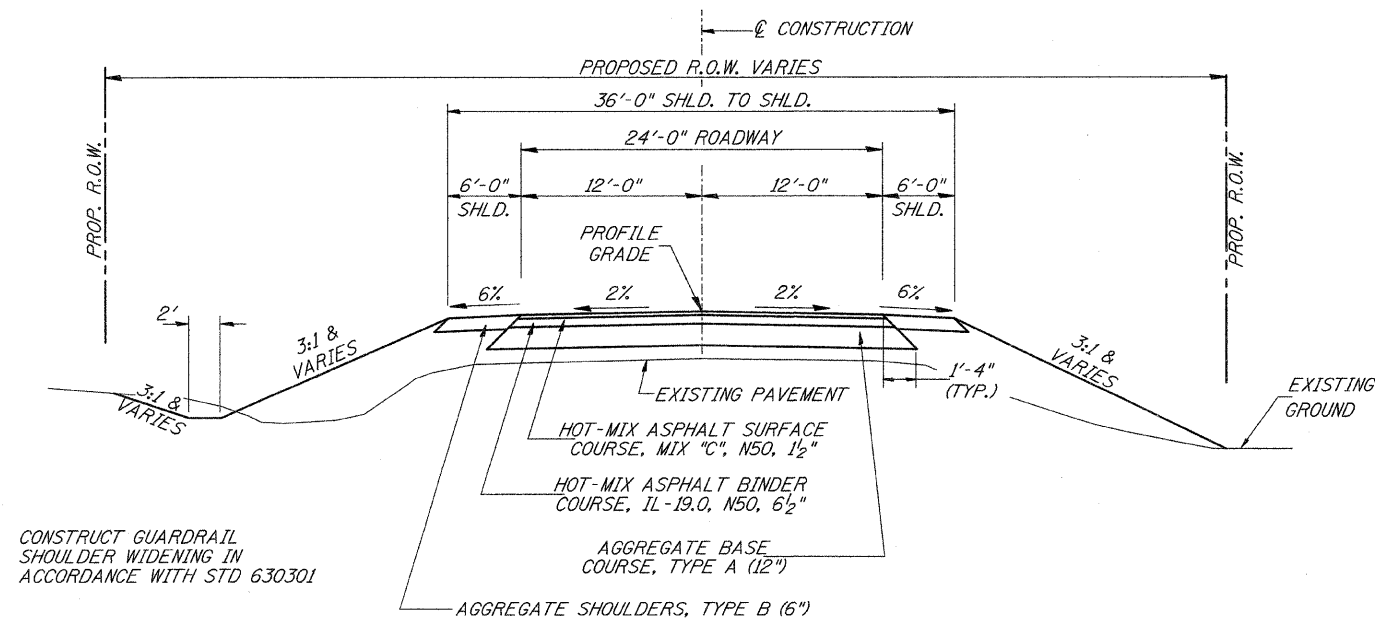
ANY REFERENCE TO STANDARDS THROUGHOUT THE PLANS SHALL BE INTERPRETED TO BE THE LATEST STANDARDS OF THE ILLINOIS DEPARTMENT OF TRANSPORTATION

THE LOCATION OF UNDERGROUND UTILITIES SHOWN ON THE PLANS REPRESENTS THE BEST KNOWLEDGE OF THE COUNTY. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LOCATIONS OF UNDERGROUND INSTALLATIONS BEFORE STARTING CONSTRUCTION OPERATIONS. THE CONTRACTOR SHALL INDEMNIFY THE COUNTY, ITS OFFICERS AND EMPLOYEES AGAINST ALL CLAIMS DUE TO DAMAGE TO CORPORATE OR PRIVATE PROPERTY RESULTING FROM HIS CONSTRUCTION OPERATIONS AS DESCRIBED IN ARTICLES 107.20 AND 107.26 OF THE STANDARD SPECIFICATIONS.

THE CONTRACTOR MAY BE REQUIRED TO CONDUCT SOME OF HIS GRADING AND TRENCHING OPERATIONS AROUND TRANSMISSION POLES AND UNDER TRANSMISSION LINES. THE ADDED COST OF SO DOING SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION.

WHERE SECTION OR SUBSECTION MONUMENTS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE REMOVED. THE CONTRACTOR SHALL PROTECT AND PRESERVE PROPERTY MARKERS UNTIL THE OWNER, AN AUTHORIZED SURVEYOR, OR AGENT, HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATION.

ALL ELEVATIONS SHOWN REFER TO U.S.G.S. MEAN SEA LEVEL DATUM.



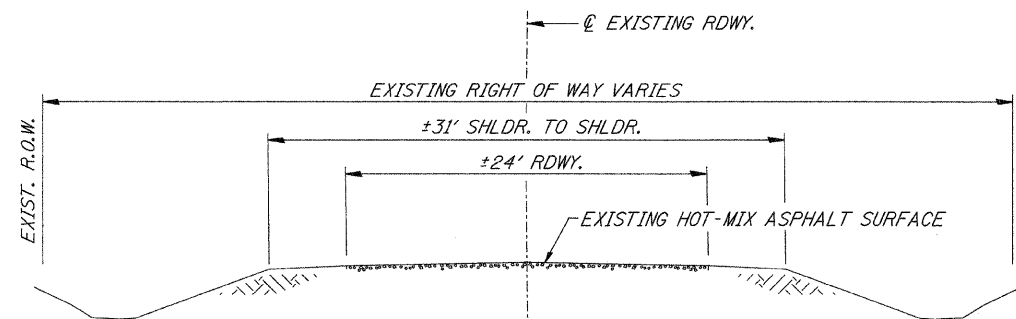
PROPOSED TYPICAL SECTION

STA. 12+25.00 TO STA. 19+12.19
STA. 21+11.53 TO STA. 31+25.00
EXCEPT TRANSITIONS

STA. 18+97.19 TO STA. 19+27.19
STA. 20+96.53 TO STA. 21+26.53
BRIDGE APPROACH PAVEMENT

BRIDGE OMISSION
STA. 19+27.19 TO STA. 20+96.53

CONSTRUCT GUARDRAIL SHOULDER WIDENING IN ACCORDANCE WITH STD 630301



EXISTING TYPICAL SECTION

**GENERAL NOTES, DETAILS,
AND TYPICAL SECTIONS**

SUMMARY OF QUANTITIES

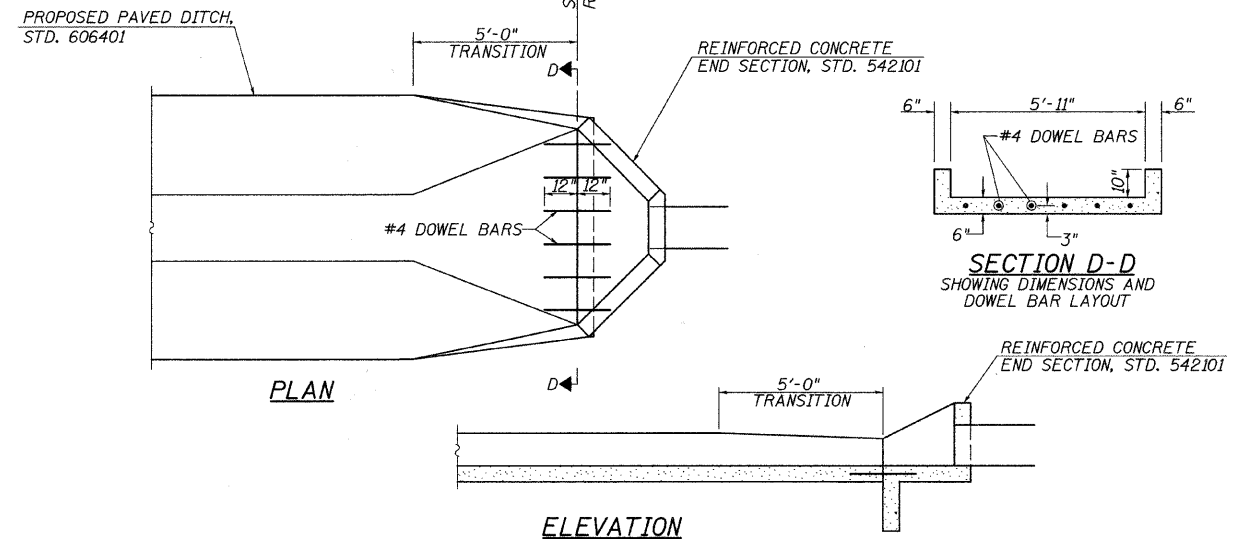
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	3
FED. ROAD DIST. NO. 7		BLANCK	PROJECT BRS-1365(111)	CONTRACT #87368
*05-00626-00-BR				

CODE NO.	ITEM	UNIT	X081-2A		TOTAL
			MAJOR BRIDGE FUNDS	NON-FEDERAL PARTICIPATION	
20100210	TREE REMOVAL (OVER 15 UNITS DIAMETER)	UNIT	18		18
20200100	EARTH EXCAVATION	CU YD	3,093	2,087	5,180
20200200	ROCK EXCAVATION	CU YD	10		10
20300100	CHANNEL EXCAVATION	CU YD	2,620		2,620
① 20400800	FURNISHED EXCAVATION	CU YD	10,030		10,030
① 20700400	POROUS GRANULAR EMBANKMENT, SPECIAL	CU YD	127		127
① 25001000	SEEDING, CLASS 2 (SPECIAL)	ACRE	2.7	0.5	3.2
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	1,250	250	1,500
28000300	TEMPORARY DITCH CHECKS	EACH	86	8	94
28000400	PERIMETER EROSION BARRIER	FOOT	680	160	840
28000500	INLET AND PIPE PROTECTION	EACH	1	1	2
28000510	INLET FILTERS	EACH	2		2
28100109	STONE RIPRAP, CLASS A5	SQ YD	825		825
28300400	AGGREGATE DITCH	TON	724		724
28200200	FILTER FABRIC	SQ YD	2,625		2,625
①	RELOCATE EXISTING RIPRAP	SQ YD	400		400
35100100	AGGREGATE BASE COURSE, TYPE A	TON	2,154	1,133	3,287
40600100	BITUMINOUS MATERIALS (PRIME COAT)	GALLON	1,672	951	2,623
40603080	HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	TON	1,153	520	1,673
40603310	HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N50	TON	260	118	378
40800050	INCIDENTAL HOT-MIX ASPHALT SURFACING	TON	15	75	90
42001165	BRIDGE APPROACH PAVEMENT	SQ YD	222		222
① 44004000	PAVED DITCH REMOVAL	FOOT	1,332	199	1,531
48101200	AGGREGATE SHOULDERS, TYPE B	TON	463	181	644
48203021	HOT-MIX ASPHALT SHOULDERS, 6"	SQ YD	67		67
① 50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1		1
① 50105220	PIPE CULVERT REMOVAL	FOOT	62		62
50200100	STRUCTURE EXCAVATION	CU YD	125		125
50300225	CONCRETE STRUCTURES	CU YD	237.7		237.7
50300255	CONCRETE SUPERSTRUCTURE	CU YD	230.1		230.1
50300260	BRIDGE DECK GROOVING	SQ YD	770		770
50300280	CONCRETE ENCASEMENT	CU YD	4.2		4.2
50300300	PROTECTIVE COAT	SQ YD	1,254	203	1,457
50400805	FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE I-BEAMS, 36 IN.	FOOT	999		999
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	64,130		64,130
50800515	BAR SPLICERS	EACH	68		68
51201400	FURNISHING STEEL PILES HP10X42	FOOT	245		245
51201610	FURNISHING STEEL PILES HP12X63	FOOT	504		504
51202305	DRIVING PILES	FOOT	245		245
51203400	TEST PILE STEEL HP10X42	EACH	2		2
51204650	PILE SHOES	EACH	12		12
51500100	NAME PLATES	EACH	1		1
52100530	ANCHOR BOLTS, 1 1/4"	EACH	8		8
54200430	PIPE CULVERTS, TYPE 1 RCCP 15"	FOOT	78		78
54215415	CAST-IN-PLACE REINFORCED CONCRETE END SECTIONS 15"	EACH	1		1
54215547	METAL END SECTIONS 12"	EACH	2		2
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	67		67
60105000	PIPE DRAINS, CORRUGATED STEEL OR ALUMINUM ALLOY 12"	FOOT	84		84
60109580	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	150		150
60603800	COMBINATION CONCRETE CURB AND GUTTER, TYPE B-6.12	FOOT	79		79
60616110	PAVED DITCH, TYPE A-30	FOOT	288	220	508
60900140	TYPE B INLET BOX, STANDARD 609006	EACH	2		2
* 63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	FOOT	412.5		412.5
* 63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4		4
* 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	4		4
63200310	GUARDRAIL REMOVAL	FOOT	1,737		1,737
67100100	MOBILIZATION	L SUM	1		1
① 70103700	TRAFFIC CONTROL COMPLETE	L SUM	1		1
* 78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	5,520	2,080	7,600
* 78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	4		4
* 78200405	GUARDRAIL MARKERS	EACH	10		10
* 78200520	BARRIER WALL MARKERS, TYPE B	EACH	4		4
X5020501	UNDERWATER STRUCTURE EXCAVATION PROTECTION, LOCATION 1	EACH	1		1
X5020502	UNDERWATER STRUCTURE EXCAVATION PROTECTION, LOCATION 2	EACH	1		1
Z0013798	CONSTRUCTION LAYOUT	L SUM	1		1
① Z0065000	SETTING PILES IN ROCK	EACH	16		16

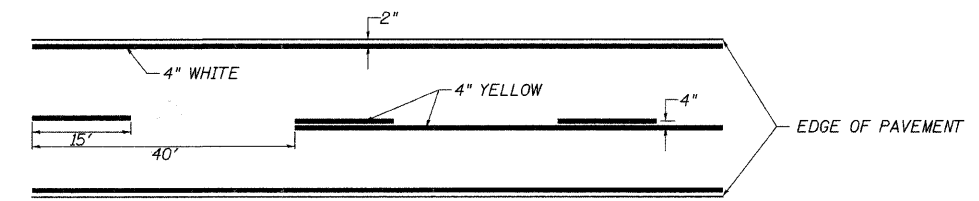
CONSTRUCTION CODE TYPE: X081-2A

① SEE SPECIAL PROVISIONS * SPECIALTY ITEMS

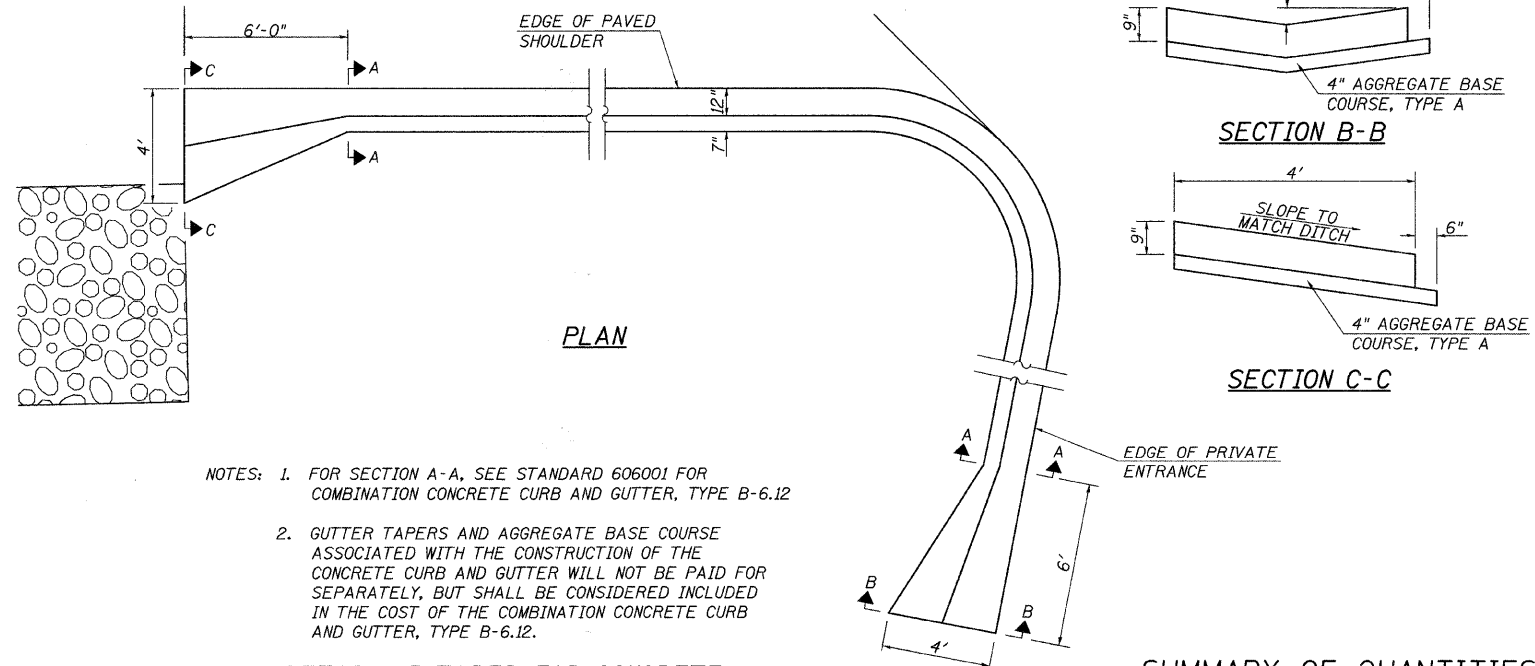
- NOTES:
1. MAJOR BRIDGE FUNDS LIMITS FROM STA. 12+25.00 TO STA. 26+05.00.
 2. NON-FEDERAL PARTICIPATION LIMITS FROM STA. 26+05.00 TO STA. 31+25.00.



DETAIL OF PAVED DITCH CONNECTION TO END SECTION



PAVEMENT STRIPING DETAIL



- NOTES:
1. FOR SECTION A-A, SEE STANDARD 606001 FOR COMBINATION CONCRETE CURB AND GUTTER, TYPE B-6.12
 2. GUTTER TAPERS AND AGGREGATE BASE COURSE ASSOCIATED WITH THE CONSTRUCTION OF THE CONCRETE CURB AND GUTTER WILL NOT BE PAID FOR SEPARATELY, BUT SHALL BE CONSIDERED INCLUDED IN THE COST OF THE COMBINATION CONCRETE CURB AND GUTTER, TYPE B-6.12.

DETAIL OF TAPER FOR CONCRETE CURB AND GUTTER, TYPE B-6.12

SUMMARY OF QUANTITIES AND DETAILS

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	4
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT BRS-1365(111)	CONTRACT #87368
*05-00626-00-BR				

PAVEMENT SCHEDULE

STATION TO STATION	WIDTH	LENGTH	AGGREGATE BASE COURSE, TYPE A TON	PRIME COAT GALLON 0.50 GAL/SQ YD	HOT-MIX ASPHALT BINDER CSE TON 112#/SQ YD/IN	HOT-MIX ASPHALT SURF CSE TON 112#/SQ YD/IN	INC HOT-MIX ASPHALT SURF TON 112#/SQ YD/IN	BRIDGE APPROACH PAVEMENT SQ YD
12+25.00 - 12+75.00	24.98' AVG.	50.00'	88					
12+75.00 - 18+97.19	26.33'	622.19'	1,147					
21+26.53 - 30+75.00	26.33'	948.47'	1,749					
30+75.00 - 31+25.00	25.38' AVG.	50.00'	89					
12+25.00 - 12+75.00	24.00' AVG.	50.00'		67				
12+75.00 - 18+97.19	25.33'	622.19'		875				
21+26.53 - 30+75.00	25.33'	948.47'		1,334				
30+75.00 - 31+25.00	24.40' AVG.	50.00'		68				
12+25.00 - 12+75.00	23.44' AVG.	50.00'			48			
12+75.00 - 18+97.19	24.79'	622.19'			625			
21+26.53 - 30+75.00	24.79'	948.47'			952			
30+75.00 - 31+25.00	23.84' AVG.	50.00'			48			
12+25.00 - 12+75.00	22.78' AVG.	50.00'				11		
12+75.00 - 18+97.19	24.13'	622.19'				141		
21+26.53 - 30+75.00	24.13'	948.47'				215		
30+75.00 - 31+25.00	23.18' AVG.	50.00'				11		
18+97.19 - 19+27.19	33.33'	30.00'					111	
20+96.53 - 21+26.53	33.33'	30.00'					111	
ENT. - 24+67 RT	21.00' AVG.	41.00'	37	57		15		
ENT. - 26+40 LT	42.00'	37.50'	92	107		36		
ENT. - 28+54 RT	24.00'	28.00'	41	47		16		
ENT. - 28+92 LT	24.00'	26.76'	44	51		17		
MB TURNOUT 26+76 LT	4.00'	24.50'		6		2		
MB TURNOUT 28+88 RT	4.00'	22.91'		5		2		
MB TURNOUT 29+31 LT	4.00'	25.50'		6		2		
TOTAL			3,287	2,623	1,673	378	90	222

PIPE CULVERTS, TYPE 1 RCCP 15"

STATION	SIDE	FOOT
24+67	RIGHT	78
TOTAL		78

CAST-IN-PLACE REINFORCED CONCRETE END SECTIONS 15"

STATION	SIDE	EACH
24+78	RIGHT	1
TOTAL		1

APPROACH PAVEMENT DRAINS

STATION	SIDE	METAL END SECTIONS 12" (EACH)	PIPE DRAINS, CORRUGATED STEEL OR ALUMINUM ALLOY 12" (FOOT)	TYPE B INLET BOX, STANDARD 609006 (EACH)
19+04.00	RIGHT	1	38	1
19+23.00	LEFT	1	46	1
TOTAL		2	84	2

TREE REMOVAL

STATION	OFFSET	SIDE	OVER 15 UNITS
24+98	63'	LEFT	18
TOTAL			18

AGGREGATE DITCH**

STATION TO STATION	SIDE	WIDTH	TON	FILTER FABRIC (SQ YD)
12+25 - 20+00	LEFT	8'	371	719
12+25 - 16+00	RIGHT	8'	181	349
20+83 - 24+00	RIGHT	8'	152	294
24+10 - 24+50	LEFT	8'	20	39
TOTAL			724	1,401

** Aggregate Ditch shall conform to material type A-4 of the Standard Specifications

PAVED DITCH, TYPE A-30

STATION TO STATION	SIDE	PAVED DITCH, TYPE A-30	PROTECTIVE COAT (SQ YD)
24+50 - 25+89	LEFT	155'	143
24+80 - 28+25	RIGHT	345'	319
24+86	RIGHT	8'	8
TOTAL		508'	470

PAVED DITCH REMOVAL

STATION TO STATION	SIDE	FOOT
12+25 - 15+61	LEFT	336
12+25 - 16+00	RIGHT	375
20+54 - 24+38	RIGHT	385
24+72 - 25+69	LEFT	107
24+76 - 28+04	RIGHT	328
TOTAL		1,531

PAINT PAVEMENT MARKING - LINE 4"

STATION TO STATION	SIDE	DESCRIPTION	FOOT
12+25 - 31+25	LEFT	WHITE EDGE LINE	1,900
12+25 - 31+25	©	DOUBLE YELLOW	3,800
12+25 - 31+25	RIGHT	WHITE EDGE LINE	1,900
TOTAL			7,600

INLET FILTERS

STATION	SIDE	EACH
19+04	RIGHT	1
19+23	LEFT	1
TOTAL		2

PIPE CULVERT REMOVAL

© STATION	SIZE	SIDE	FOOT
24+63	18"	LEFT	24
24+67	15"	RIGHT	38
TOTAL			62

TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT

SIDE	STATION TO STATION	EACH
LEFT	15+56.02 - 16+06.02	1
RIGHT	17+87.07 - 18+37.07	1
RIGHT	21+30.20 - 21+80.20	1
LEFT	22+36.65 - 22+86.65	1
TOTAL		4

BARRIER WALL AND GUARDRAIL MARKERS

STATION TO STATION	SIDE	GUARDRAIL MARKERS (EACH)	BARRIER WALL MARKERS, TYPE B (EACH)
18+37.07 - 21+30.20	RIGHT	3	2
16+06.02 - 22+36.65	LEFT	7	2
TOTAL		10	4

EARTHWORK SUMMARY

STATION TO STATION	EARTH EXCAVATION	CHANNEL EXCAVATION	STRUCTURE EXCAVATION	FILL	WASTE (SHORTAGE)
	CU YD	CU YD	CU YD	CU YD	CU YD
RDWY 12+25.00 - 19+27.19	1,865			9,926	(8,527)
RDWY 20+96.53 - 31+25.00	3,314			3,988	(1,503)
CHANNEL STRUCTURE		2,620	125		
TOTAL	5,179	2,620	125	13,914	(10,030)
USE	5,180	2,620	125		(10,030)

(© 25% SHRINKAGE)

GUARDRAIL REMOVAL

STATION TO STATION	SIDE	FOOT
14+31 - 19+81	LEFT	550
14+53 - 19+65	RIGHT	512
20+19 - 21+44	RIGHT	125
20+34 - 25+84	LEFT	550
TOTAL		1,737

PERIMETER EROSION BARRIER

STATION TO STATION	SIDE	FOOT
16+00 - 19+15	RIGHT	345
21+10 - 24+10	LEFT	335
27+75 - 29+25	LEFT	160
TOTAL		840

TEMPORARY DITCH CHECKS

STATION TO STATION	SIDE	EACH
12+50 - 14+00	RIGHT	7
14+20 - 16+00	RIGHT	10
12+50 - 14+00	LEFT	6
14+25 - 15+00	LEFT	4
15+10 - 16+50	LEFT	15
17+88 - 19+25	LEFT	2
20+00	LEFT	1
20+83 - 21+50	RIGHT	5
21+75 - 23+75	RIGHT	9
25+10 - 27+90	RIGHT	9
24+10	LEFT	1
24+50 - 25+50	LEFT	21
25+63 - 25+75	LEFT	2
27+75	LEFT	1
31+25	RIGHT	1
TOTAL		94

TRAFFIC BARRIER TERMINAL, TYPE 6

SIDE	STATION TO STATION	EACH
RIGHT	18+74.57 - 19+18.32	1
LEFT	18+93.52 - 19+37.27	1
RIGHT	20+86.45 - 21+30.20	1
LEFT	21+05.40 - 21+49.15	1
TOTAL		4

STEEL PLATE BEAM GUARDRAIL, TYPE A

SIDE	STATION TO STATION	FOOT
LEFT	16+06.02 - 18+93.52	287.5
RIGHT	18+37.07 - 18+74.57	37.5
LEFT	21+49.15 - 22+36.65	87.5
TOTAL		412.5

INLET & PIPE PROTECTION

STATION	SIDE	EACH
24+78	RIGHT	1
31+23	LEFT	1
TOTAL		2

HOT-MIX ASPHALT SHOULDER, 6"

STATION TO STATION	SIDE	WIDTH	SQ YD
24+00 - 25+00	RIGHT	6.00'	67
TOTAL			67

COMBINATION CONCRETE CURB AND GUTTER, TYPE B-6.12




STATION TO STATION	SIDE	COMBINATION CONCRETE CURB AND GUTTER, TYPE B-6.12 (FOOT)	PROTECTIVE COAT (SQ YD)
24+00 - 24+43	RIGHT	79	18
TOTAL		79	18

AGGREGATE SHOULDERS, TYPE B

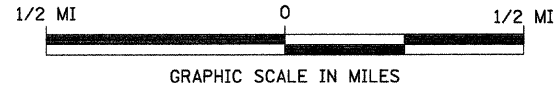
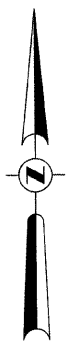
STATION TO STATION	SIDE	WIDTH	LENGTH	TON
12+25.00 - 12+75.00	LEFT	4.00' AVG.	50.00'	7
12+25.00 - 12+75.00	RIGHT	4.55' AVG.	50.00'	8
12+75.00 - 19+05.85	LEFT	6.00'	630.85'	133
12+75.00 - 18+88.53	RIGHT	6.00'	613.53'	129
18+87.25 - 19+17.25	RIGHT	1.58'	30.00'	2
19+07.13 - 19+37.13	LEFT	1.58'	30.00'	2
20+86.59 - 21+16.59	RIGHT	1.58'	30.00'	2
21+06.47 - 21+36.47	LEFT	1.58'	30.00'	2
21+17.87 - 24+00.00	RIGHT	6.00'	282.13'	60
21+35.19 - 25+91.04	LEFT	6.00'	455.85'	96
25+00.00 - 28+33.38	RIGHT	6.00'	333.38'	70
26+59.92 - 28+67.00	LEFT	6.00'	207.08'	44
28+76.00 - 30+75.00	RIGHT	6.00'	199.00'	42
29+17.00 - 30+75.00	LEFT	6.00'	158.00'	33
30+75.00 - 31+25.00	RIGHT	4.20' AVG.	50.00'	7
30+75.00 - 31+25.00	LEFT	4.25' AVG.	50.00'	7
TOTAL				644

SCHEDULES OF QUANTITIES

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	5
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT BRS-1365(111)	
*05-00626-00-BR			CONTRACT #87368	

- 1 ROAD CLOSED
2 1/4 MILES AHEAD
LOCAL TRAFFIC ONLY
R11-3
- 2 ROAD CLOSED
3 3/4 MILES AHEAD
LOCAL TRAFFIC ONLY
R11-3
- 3 ROAD CLOSED
1 1/4 MILES AHEAD
LOCAL TRAFFIC ONLY
R11-3
- 4 ROAD CLOSED
3/4 MILES AHEAD
LOCAL TRAFFIC ONLY
R11-3
- 5  ROAD CLOSED
AHEAD
W20-3
- 6  ROAD CLOSED
500 FT
W20-3
- 7  TYPE III BARRICADES

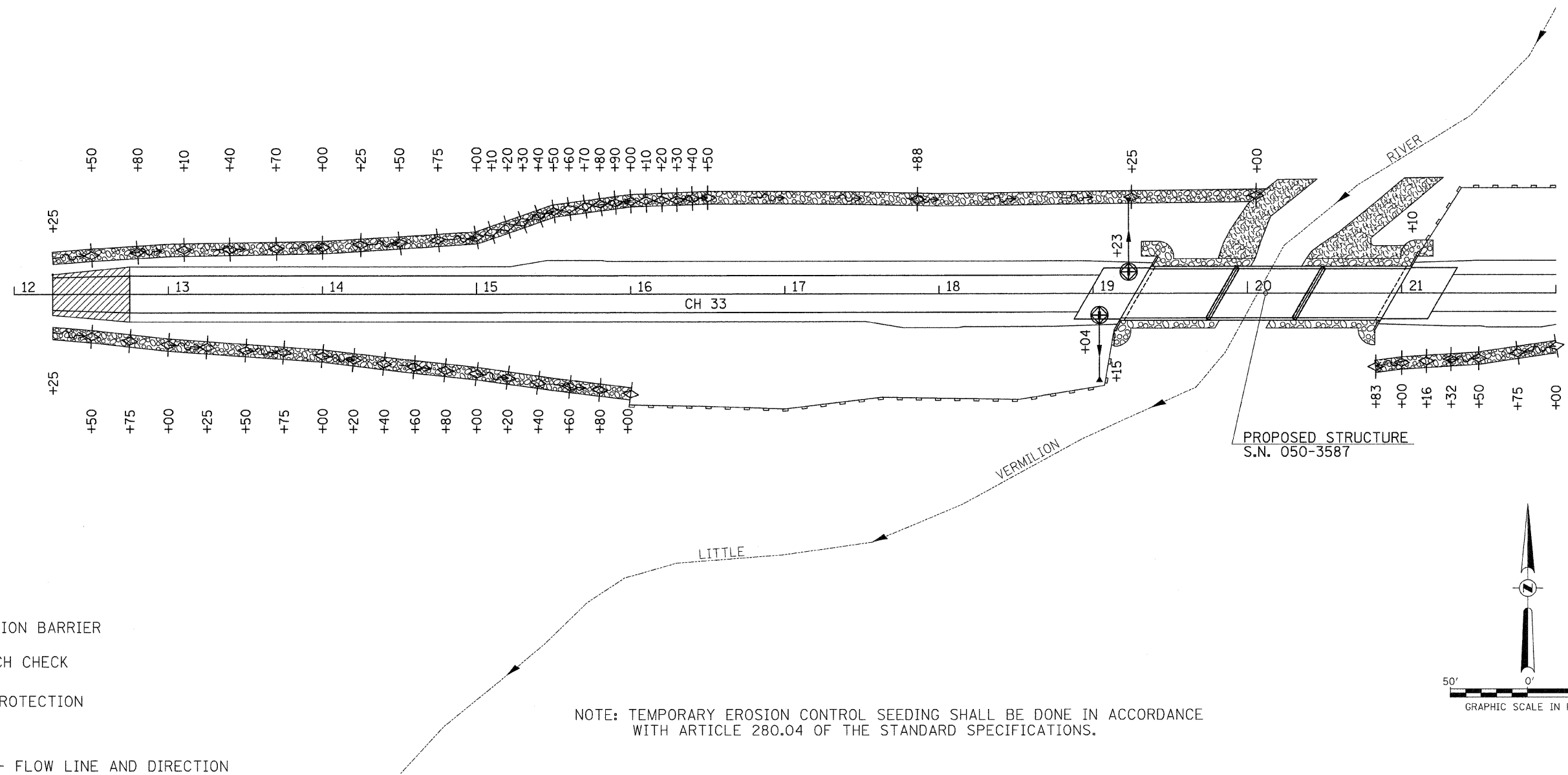
SEE STANDARD BLR 21
AND SPECIAL PROVISIONS











TRAFFIC CONTROL PLAN

T 34 N, R 1 E, 3rd PM
SECTION 14

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	6
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT BRS-1365(111)	
*05-00626-00-BR		CONTRACT *87368		



LEGEND

-  PERIMETER EROSION BARRIER
-  TEMPORARY DITCH CHECK
-  INLET & PIPE PROTECTION
-  INLET FILTERS
-  SPECIAL DITCH - FLOW LINE AND DIRECTION
-  PROPOSED PAVED DITCH
-  PROPOSED RIPRAP PLACEMENT
-  PROPOSED RELOCATED EXISTING RIPRAP

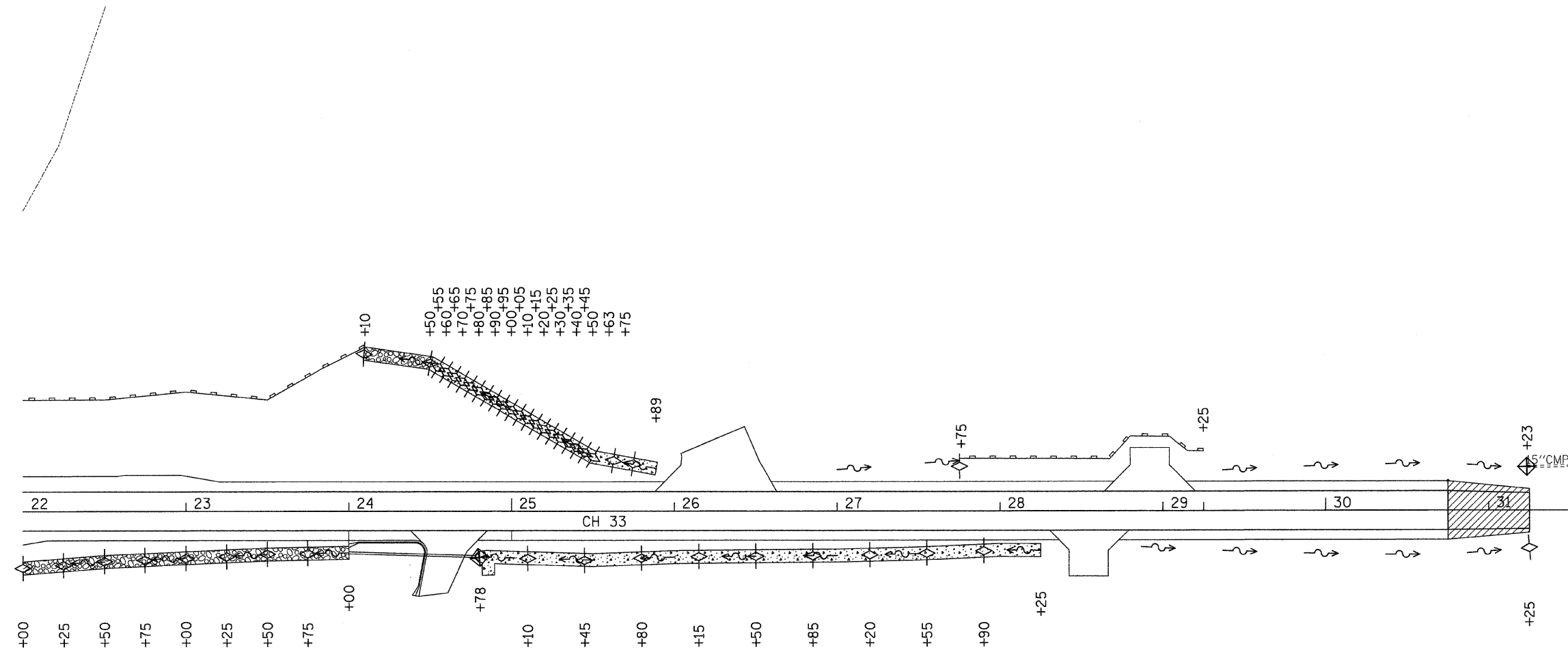
NOTE: TEMPORARY EROSION CONTROL SEEDING SHALL BE DONE IN ACCORDANCE WITH ARTICLE 280.04 OF THE STANDARD SPECIFICATIONS.

T 34 N, R 1 E, 3rd PM
SECTION 23









STORM WATER POLLUTION
PREVENTION PLAN

T 34 N, R 1 E, 3rd PM
SECTION 14

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	7
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT BRS-1365(111)	
*05-00626-00-BR			CONTRACT #87368	



LEGEND

-  PERIMETER EROSION BARRIER
-  TEMPORARY DITCH CHECK
-  INLET & PIPE PROTECTION
-  INLET FILTERS
-  SPECIAL DITCH - FLOW LINE AND DIRECTION
-  PROPOSED PAVED DITCH
-  PROPOSED RIPRAP PLACEMENT
-  PROPOSED RELOCATED EXISTING RIPRAP

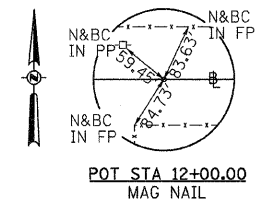
NOTE: TEMPORARY EROSION CONTROL SEEDING SHALL BE DONE IN ACCORDANCE WITH ARTICLE 280.04 OF THE STANDARD SPECIFICATIONS.

T 34 N, R 1 E, 3rd PM
SECTION 23

**STORM WATER POLLUTION
PREVENTION PLAN**

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	8
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT BRS-1365(11)		
*05-00626-00-BR			CONTRACT #87368	

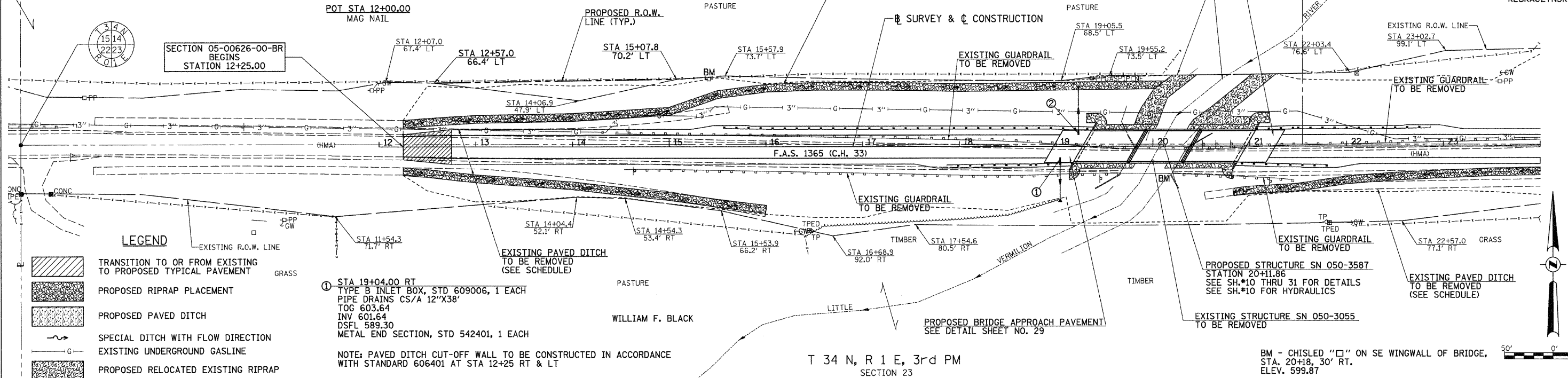
T 34 N, R 1 E, 3rd PM
SECTION 14



② STA 19+23.00 LT
TYPE B INLET BOX, STD 609006, 1 EACH
PIPE DRAINS CS/A 12"X46"
TOG 603.72
INV 601.72
DSFL 582.28
METAL END SECTION, STD 542401, 1 EACH

PROPOSED BRIDGE APPROACH PAVEMENT
SEE DETAIL SHEET NO. 29
WITHOUT CURB

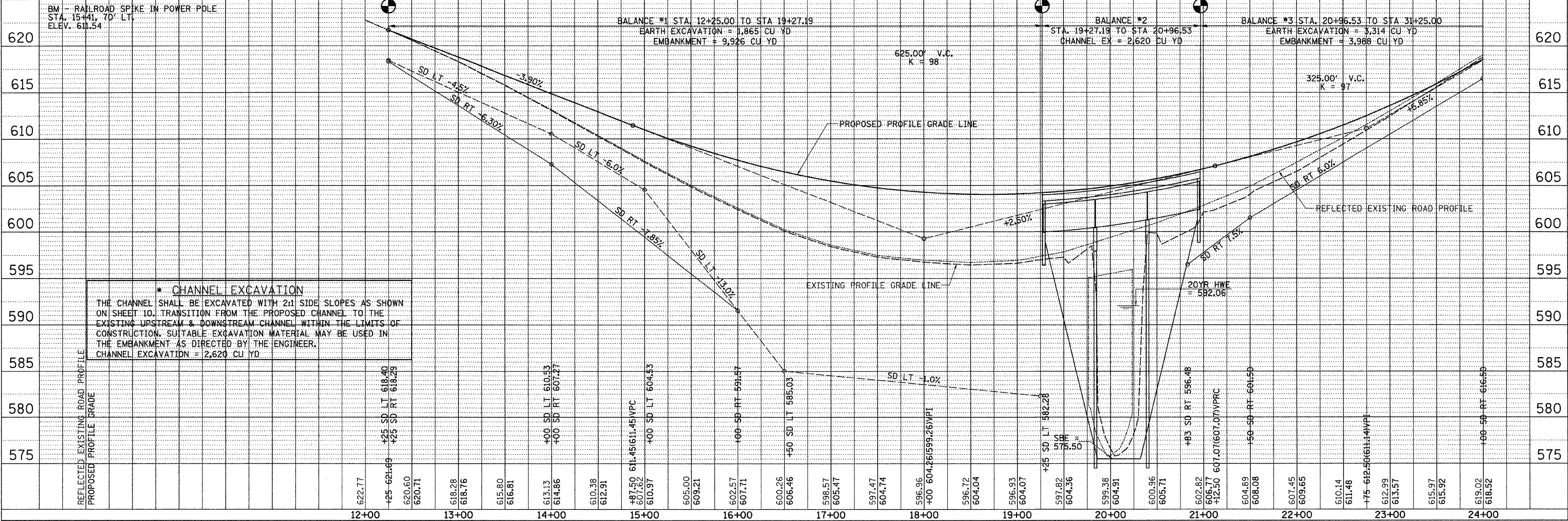
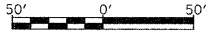
STEPHEN L. KLUKACZYNSKI



NOTE: PAVED DITCH CUT-OFF WALL TO BE CONSTRUCTED IN ACCORDANCE WITH STANDARD 606401 AT STA 12+25 RT & LT

T 34 N, R 1 E, 3rd PM
SECTION 23

BM - CHISLED "□" ON SE WINGWALL OF BRIDGE,
STA. 20+18, 30' RT.
ELEV. 599.87



*** CHANNEL EXCAVATION**
THE CHANNEL SHALL BE EXCAVATED WITH 2:1 SIDE SLOPES AS SHOWN ON SHEET 10. TRANSITION FROM THE PROPOSED CHANNEL TO THE EXISTING UPSTREAM & DOWNSTREAM CHANNEL WITHIN THE LIMITS OF CONSTRUCTION. SUITABLE EXCAVATION MATERIAL MAY BE USED IN THE EMBANKMENT AS DIRECTED BY THE ENGINEER.
CHANNEL EXCAVATION = 2,620 CU YD

DATE	BY

DATE	BY

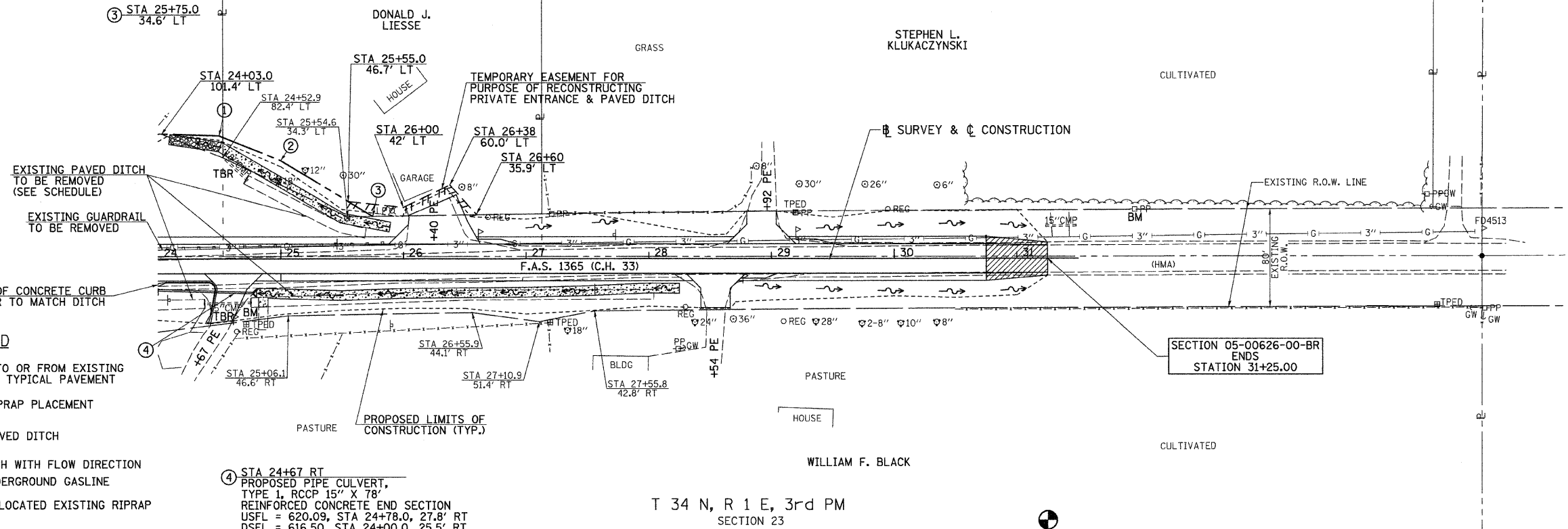
PLOT DATE = 1/23/2009
FILE NAME = I:\Bridges\2498-1\as11a\2498pp01.dgn
PLOT SCALE = 50.0000' / 1" IN.
USER NAME = jrochman

PLAN AND PROFILE STA 12+00 TO STA 24+00

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	9
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT BRS-1365(111)	
*05-00626-00-BR			CONTRACT #87368	

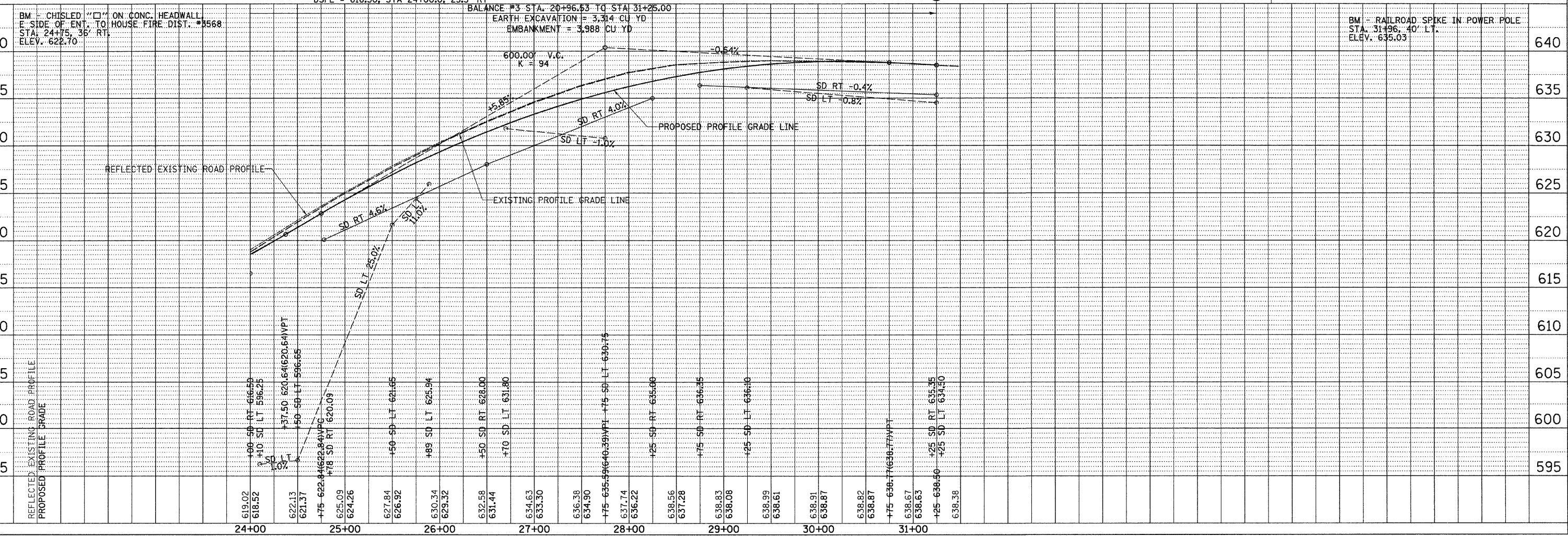
T 34 N, R 1 E, 3rd PM
SECTION 14

- ① STA 24+50.0
100.0' LT
- ② STA 25+00.0
80.0' LT
- ③ STA 25+75.0
34.6' LT



T 34 N, R 1 E, 3rd PM
SECTION 23

- ④ STA 24+67 RT
PROPOSED PIPE CULVERT,
TYPE 1, RCCP 15" X 78"
REINFORCED CONCRETE END SECTION
USFL = 620.09, STA 24+78.0, 27.8' RT
DSFL = 616.50, STA 24+00.0, 25.5' RT



PLAN AND PROFILE STA 24+00 TO STA 31+00

DATE	BY

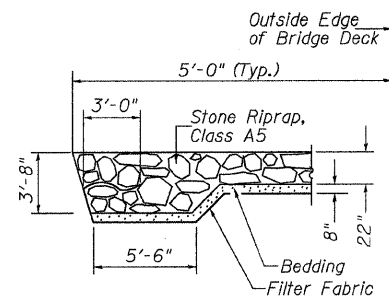
DATE	BY

PLOT DATE = 1/23/2009
FILE NAME = V:\Bridges\498-Leslie\2498pp02.dgn
SCALE = 50.0000' / 1" / IN
USER NAME = jhuchman

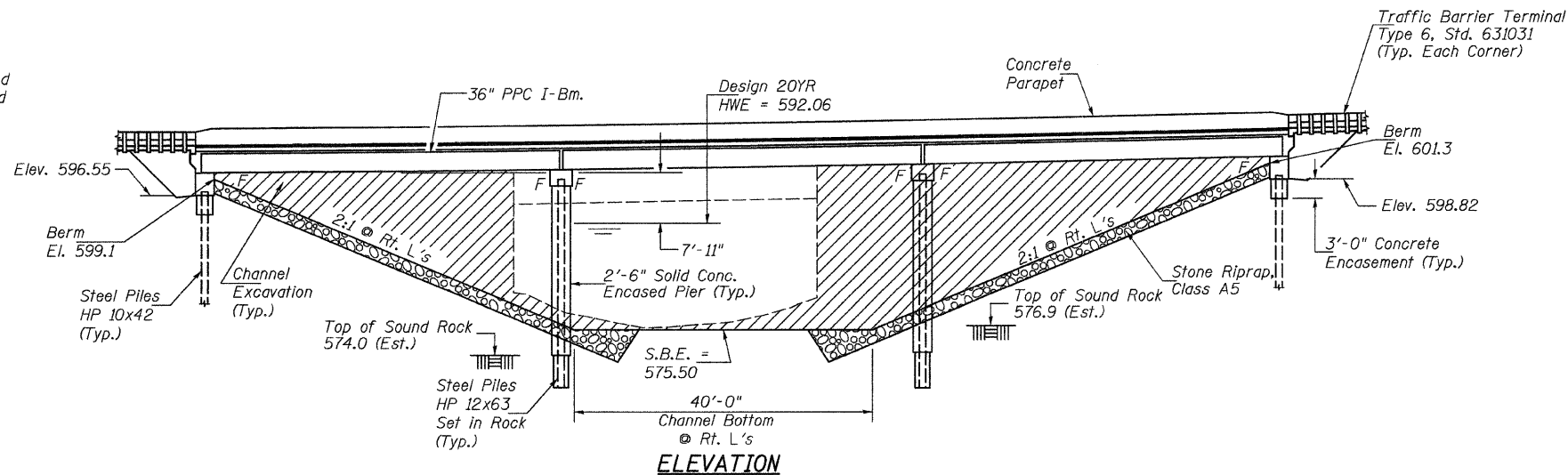
B.M.: RR Spike in Power Pole, Sta. 15+41, 70' Lt. Elev. 611.54
 RR Spike in Power Pole, Sta. 31+96, 40' Rt. Elev. 635.03

Existing Structure:
 Single span concrete T-beam superstructure supported on concrete closed abutments on timber pile supported concrete footings. The structure is ±53'-11" back to back of abutments, 25'-4" out to out of deck, and is skewed 30° left ahead. Str. No. 050-3055

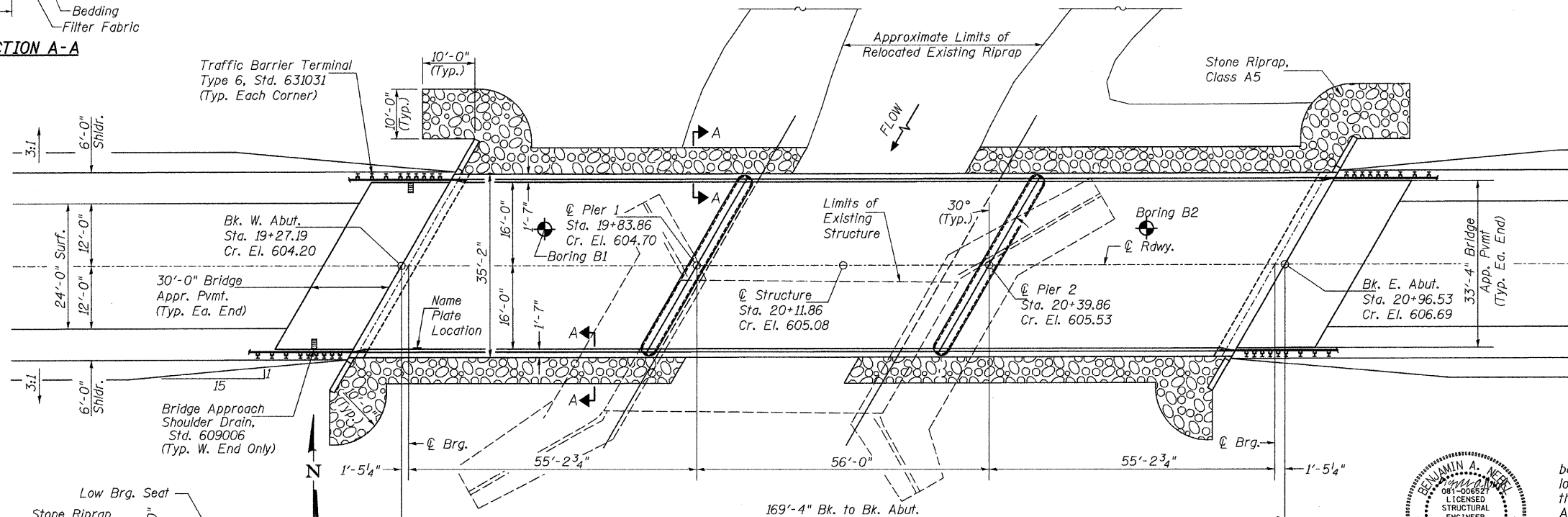
Salvage: Riprap
 Road to be closed to traffic during construction.



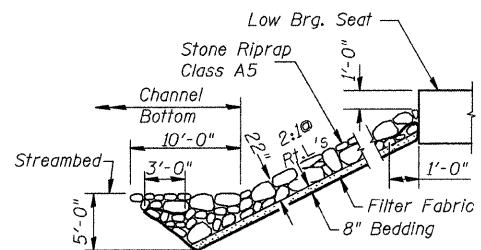
SECTION A-A



ELEVATION



PLAN



STONE RIPRAP DETAIL

WATERWAY INFORMATION

Drainage Area = 69.8 Sq. Mi. Low Grade Elev. = 604.02 @ Sta. 18+68.39

Flood Yr.	Q	C.F.S.	Opening Sq. Ft.	Nat. H.W.E.	Head - Ft.	Headwater El.			
	Exist.	Prop.	Exist.	Prop.	Exist.	Prop.			
Design	20	4,581	588	1,156	592.06	0.29	0.03	592.35	592.09
Base	100	6,499	671	1,375	594.09	0.53	0.04	594.62	594.13

Construction of this project complies with IDNR, Office of Water Resources Statewide Permit No. 2

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
 Bedrock Acceleration Coefficient (A) = 0.040 g
 Site Coefficient (S) = 1.0

LOADING HL-93

Allow 50#/sq. ft. future wearing surface.

DESIGN SPECIFICATIONS

2007 AASHTO (LRFD)

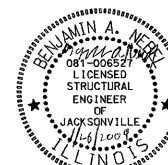
DESIGN STRESSES

FIELD UNITS

f'c = 3,500 p.s.i.
 fy = 60,000 p.s.i. (Reinforcement)

PRECAST PRESTRESSED UNITS

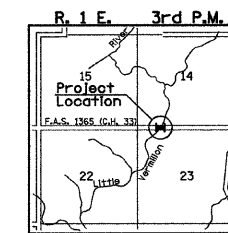
f'c = 6,000 p.s.i.
 f'cl = 5,000 p.s.i.
 f's = 270,000 p.s.i. (1/2" φ low relaxation strands)
 f'sl = 201,960 p.s.i. (1/2" φ low relaxation strands)



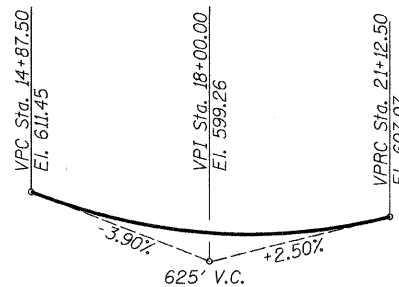
Lic Exp. 11/30/2010

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AASHTO LRFD Bridge Design Specifications. This design complies with all requirements of the current AASHTO Guide Specifications for Seismic Design of highway bridges.

Benjamin A. New 1/26/2009
 Illinois Structural No. 6527
 Expires 11/30/2010



LOCATION SKETCH



PROFILE GRADE

DESIGN SCOUR TABLE

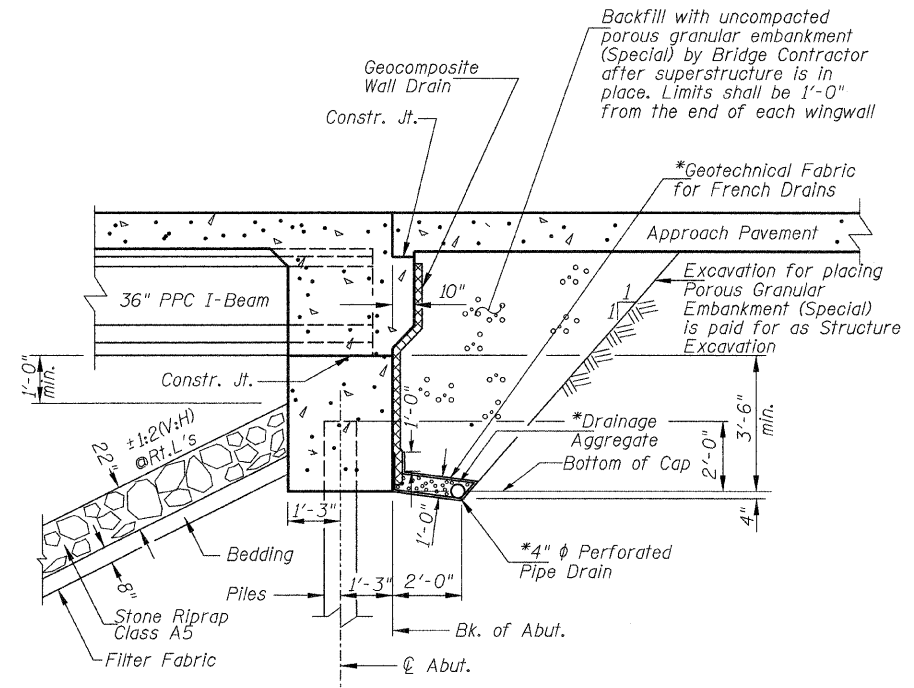
Location	W. Abut	Pier 1	Pier 2	E. Abut
Design Scour Elevation	596.6	574.0	574.5	598.8

GENERAL PLAN & ELEVATION

LITTLE VERMILION RIVER
 BUILT 200_ BY
 LASALLE COUNTY
 SEC. 05-00626-00-BR
 C.H. 33 STATION 20+11.86
 F.A. PROJ. BRS-1365(111)
 STR. NO. 050-3587 LOADING HL-93

NAME PLATE

Locate Name Plate on Face of Parapet S.W. Corner of Bridge (See Std. 515001)



*Included in the cost of Pipe Underdrains for Structures.
All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101)

SECTION THRU ABUTMENTS
(Horiz. dim. @ Rt. L's)

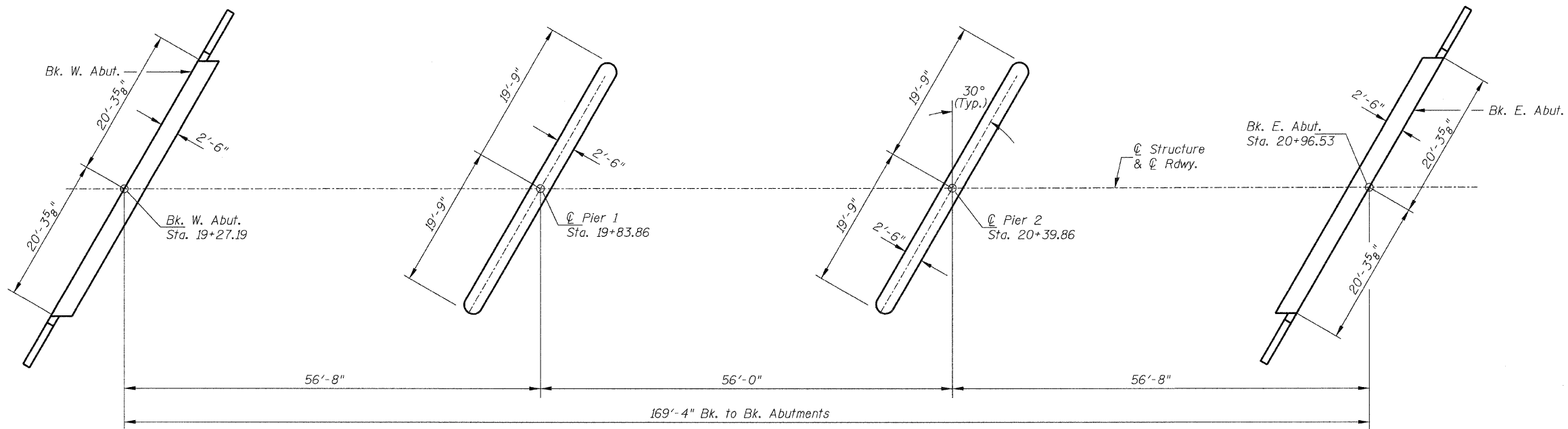
GENERAL NOTES

The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at the substructures specified or approved by the Engineer before ordering the remainder of the piles.
For Soil Boring Logs, See Sheet #'s 21 & 22 of 22.
For Existing Structure Plans, See Special Provisions.
Reinforcement Bars shall conform to the requirements of ASTM A706 Grade 60. See Special Provisions.
Reinforcement Bars designated (E) shall be epoxy coated.
Layout of the slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.
All embedded and separate bearing plates, side retainers, anchor bolts, nuts, washers and pintles shall be galvanized according to AASHTO M111 or M232 (as applicable).
Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure.

TOTAL BILL OF MATERIAL

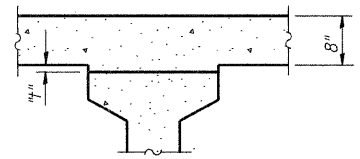
ITEM	UNIT	SUPER	SUB	TOTAL
① Removal of Existing Structures	EACH	—	—	1
Channel Excavation	CU YD	—	2,620	2,620
Structure Excavation	CU YD	—	125	125
Rock Excavation	CU YD	—	10	10
Concrete Structures	CU YD	—	237.7	237.7
Concrete Superstructure	CU YD	230.1	—	230.1
Bridge Deck Grooving	SQ YD	770	—	770
Protective Coat	SQ YD	969	—	969
Furnishing and Erecting Precast Prestressed Concrete I-Beams, 36 in.	FOOT	999	—	999
① Reinforcement Bars, Epoxy Coated	POUND	46,420	17,710	64,130
Name Plates	EACH	1	—	1
Furnishing Steel Piles HP10x42	FOOT	—	245	245
Test Pile Steel HP10x42	EACH	—	2	2
Furnishing Steel Piles HP12x63	FOOT	—	504	504
Driving Piles	FOOT	—	245	245
Pile Shoes	EACH	—	12	12
① Setting Piles in Rock	EACH	—	16	16
① Porous Granular Embankment, Special	CU YD	—	127	127
① Pipe Underdrains for Structures 4"	FOOT	—	150	150
Geocomposite Wall Drain	SQ YD	—	67	67
Concrete Encasement	CU YD	—	4.2	4.2
Stone Riprap, Class A5	SQ YD	—	825	825
Filter Fabric	SQ YD	—	1,225	1,225
Anchor Bolts, 1/4"φ	EACH	—	8	8
① Underwater Structure Excavation Protection, Location 1 (Pier #1)	EACH	—	1	1
① Underwater Structure Excavation Protection, Location 2 (Pier #2)	EACH	—	1	1
Bar Splicers	EACH	68	—	68
Bridge Approach Pavement	SQ YD	—	—	222
① Relocate Existing Riprap	SQ YD	—	400	400

① See Special Provisions



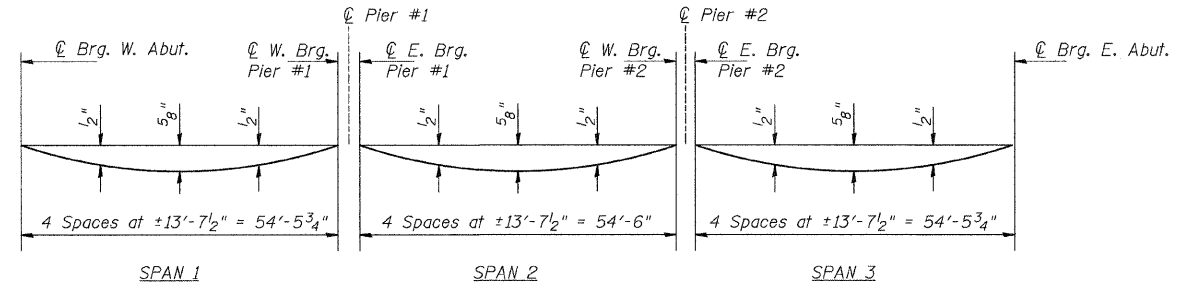
FOOTING LAYOUT

FOOTING LAYOUT, GENERAL NOTES, DETAILS, & BILL OF MATERIALS



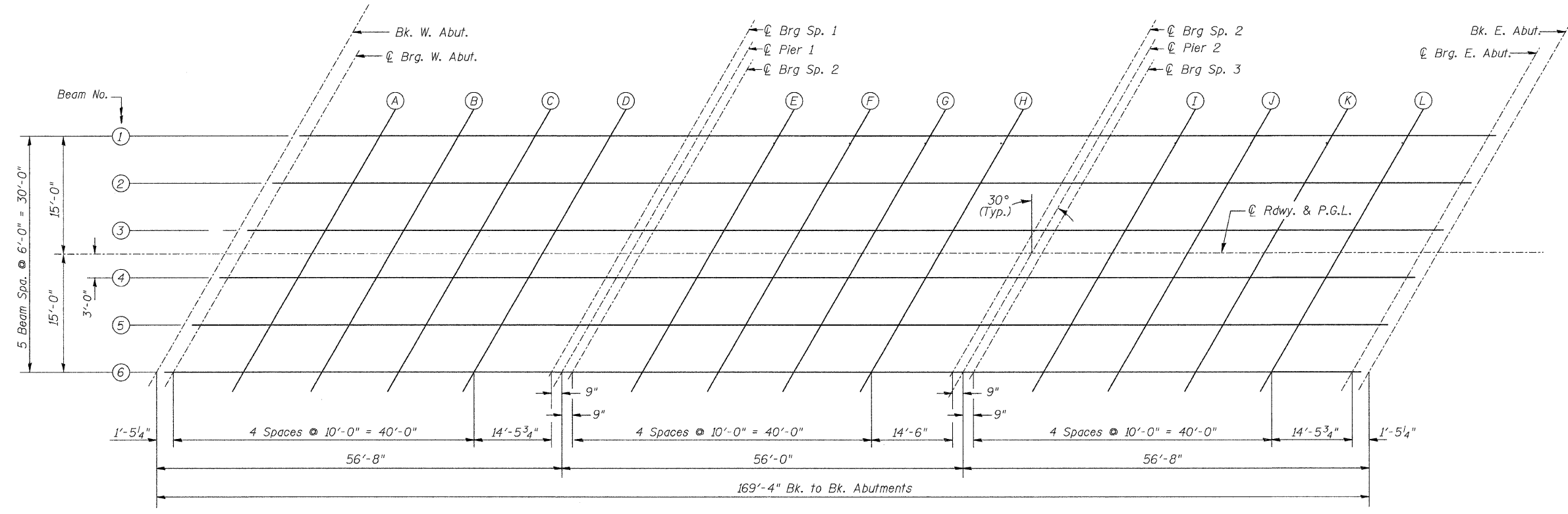
To determine "t": After all precast prestressed beams have been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflections" shown on sheets 4 of 22, minus the 8" deck thickness, equals the fillet heights "t" above top flanges of beams.

FILLET HEIGHTS



DEAD LOAD DEFLECTION DIAGRAM
(Includes weight of concrete deck and parapets, excluding beams)

Note: The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown.



PLAN



TOP OF SLAB ELEVATIONS

BEAM #1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1935.85	-15.00	604.01	604.01
CL Brg W. Abut.	1937.29	-15.00	604.02	604.02
A	1947.29	-15.00	604.10	604.12
B	1957.29	-15.00	604.18	604.23
C	1967.29	-15.00	604.28	604.33
D	1977.29	-15.00	604.39	604.42
CL Brg	1991.77	-15.00	604.56	604.56
Pier 1	1992.52	-15.00	604.57	604.57
CL Brg	1993.27	-15.00	604.58	604.58
E	2003.27	-15.00	604.71	604.74
F	2013.27	-15.00	604.85	604.90
G	2023.27	-15.00	605.01	605.05
H	2033.27	-15.00	605.17	605.21
CL Brg	2047.77	-15.00	605.43	605.43
Pier 2	2048.52	-15.00	605.44	605.44
CL Brg	2049.27	-15.00	605.45	605.45
I	2059.27	-15.00	605.64	605.67
J	2069.27	-15.00	605.85	605.89
K	2079.27	-15.00	606.06	606.10
L	2089.27	-15.00	606.28	606.31
CL Brg E. Abut.	2103.75	-15.00	606.62	606.62
Bk E. Abutment	2105.19	-15.00	606.65	606.65

BEAM #2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1932.39	-9.00	604.09	604.09
CL Brg W. Abut.	1933.83	-9.00	604.10	604.10
A	1943.83	-9.00	604.18	604.20
B	1953.83	-9.00	604.26	604.30
C	1963.83	-9.00	604.35	604.40
D	1973.83	-9.00	604.45	604.49
CL Brg	1988.31	-9.00	604.62	604.62
Pier 1	1989.06	-9.00	604.63	604.63
CL Brg	1989.81	-9.00	604.64	604.64
E	1999.81	-9.00	604.77	604.79
F	2009.81	-9.00	604.91	604.95
G	2019.81	-9.00	605.06	605.10
H	2029.81	-9.00	605.22	605.25
CL Brg	2044.31	-9.00	605.47	605.47
Pier 2	2045.06	-9.00	605.48	605.48
CL Brg	2045.81	-9.00	605.50	605.50
I	2055.81	-9.00	605.68	605.71
J	2065.81	-9.00	605.88	605.92
K	2075.81	-9.00	606.09	606.13
L	2085.81	-9.00	606.30	606.34
CL Brg E. Abut.	2100.28	-9.00	606.64	606.64
Bk E. Abutment	2101.73	-9.00	606.67	606.67

BEAM #3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1928.92	-3.00	604.16	604.16
CL Brg W. Abut.	1930.37	-3.00	604.17	604.17
A	1940.37	-3.00	604.24	604.27
B	1950.37	-3.00	604.32	604.36
C	1960.37	-3.00	604.41	604.45
D	1970.37	-3.00	604.51	604.54
CL Brg	1984.84	-3.00	604.67	604.67
Pier 1	1985.59	-3.00	604.68	604.68
CL Brg	1986.34	-3.00	604.69	604.69
E	1996.34	-3.00	604.81	604.84
F	2006.34	-3.00	604.95	604.99
G	2016.34	-3.00	605.10	605.14
H	2026.34	-3.00	605.25	605.29
CL Brg	2040.84	-3.00	605.50	605.50
Pier 2	2041.59	-3.00	605.51	605.51
CL Brg	2042.34	-3.00	605.52	605.52
I	2052.34	-3.00	605.71	605.73
J	2062.34	-3.00	605.90	605.94
K	2072.34	-3.00	606.10	606.15
L	2082.34	-3.00	606.32	606.35
CL Brg E. Abut.	2096.82	-3.00	606.65	606.65
Bk E. Abutment	2098.26	-3.00	606.68	606.68

PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1927.19	0.00	604.20	604.20
CL Brg W. Abut.	1928.63	0.00	604.21	604.21
A	1938.63	0.00	604.27	604.30
B	1948.63	0.00	604.35	604.39
C	1958.63	0.00	604.44	604.48
D	1968.63	0.00	604.53	604.57
CL Brg	1983.11	0.00	604.69	604.69
Pier 1	1983.86	0.00	604.70	604.70
CL Brg	1984.61	0.00	604.71	604.71
E	1994.61	0.00	604.84	604.86
F	2004.61	0.00	604.97	605.01
G	2014.61	0.00	605.11	605.16
H	2024.61	0.00	605.27	605.30
CL Brg	2039.11	0.00	605.51	605.51
Pier 2	2039.86	0.00	605.53	605.53
CL Brg	2040.61	0.00	605.54	605.54
I	2050.61	0.00	605.72	605.75
J	2060.61	0.00	605.91	605.95
K	2070.61	0.00	606.11	606.16
L	2080.61	0.00	606.33	606.36
CL Brg E. Abut.	2095.09	0.00	606.65	606.65
Bk E. Abutment	2096.53	0.00	606.69	606.69

BEAM #4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1925.46	3.00	604.14	604.14
CL Brg W. Abut.	1926.90	3.00	604.15	604.15
A	1936.90	3.00	604.22	604.24
B	1946.90	3.00	604.29	604.33
C	1956.90	3.00	604.38	604.42
D	1966.90	3.00	604.47	604.51
CL Brg	1981.38	3.00	604.63	604.63
Pier 1	1982.13	3.00	604.64	604.64
CL Brg	1982.88	3.00	604.65	604.65
E	1992.88	3.00	604.77	604.79
F	2002.88	3.00	604.90	604.94
G	2012.88	3.00	605.04	605.09
H	2022.88	3.00	605.20	605.23
CL Brg	2037.38	3.00	605.44	605.44
Pier 2	2038.13	3.00	605.45	605.45
CL Brg	2038.88	3.00	605.46	605.46
I	2048.88	3.00	605.64	605.67
J	2058.88	3.00	605.83	605.88
K	2068.88	3.00	606.03	606.08
L	2078.88	3.00	606.24	606.28
CL Brg E. Abut.	2093.35	3.00	606.57	606.57
Bk E. Abutment	2094.80	3.00	606.60	606.60

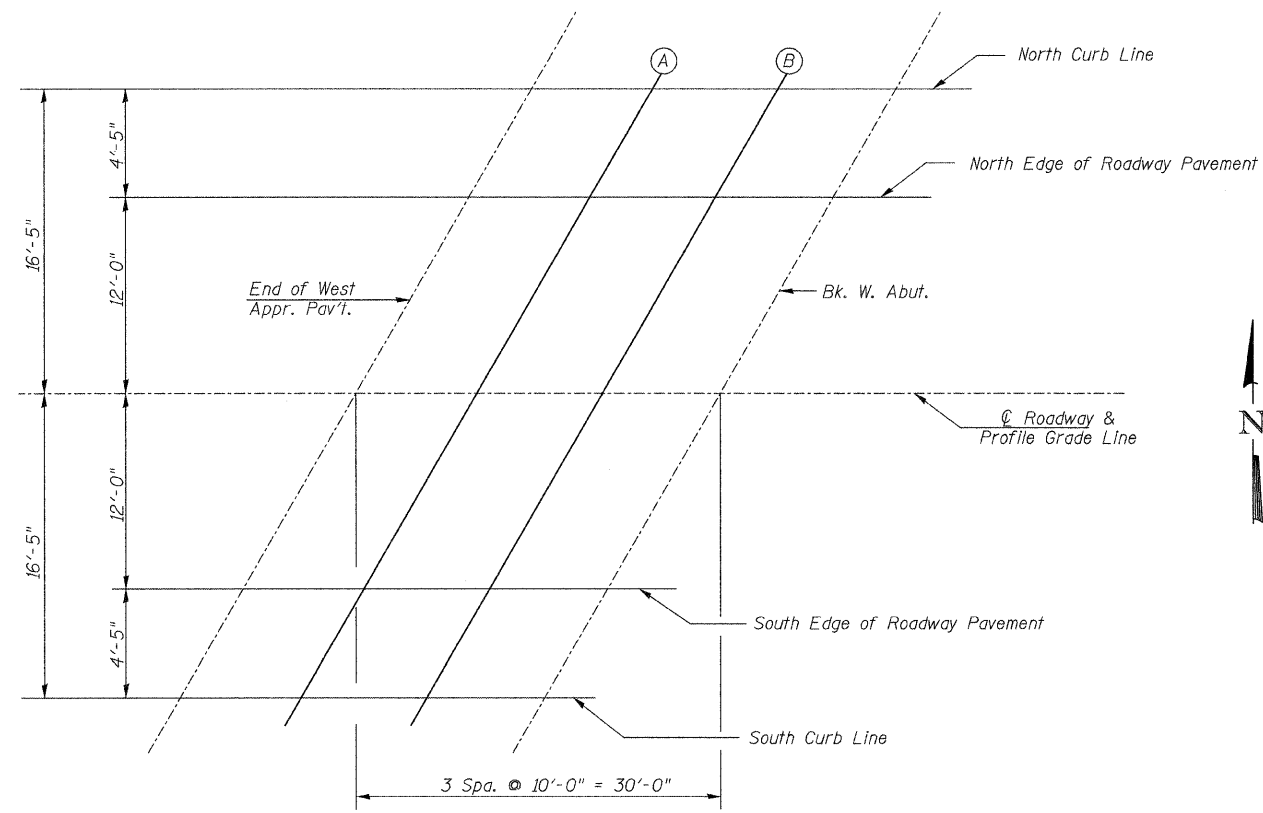
BEAM #5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1921.99	9.00	604.03	604.03
CL Brg W. Abut.	1923.44	9.00	604.04	604.04
A	1933.44	9.00	604.10	604.13
B	1943.44	9.00	604.17	604.22
C	1953.44	9.00	604.26	604.30
D	1963.44	9.00	604.35	604.38
CL Brg	1977.91	9.00	604.50	604.50
Pier 1	1978.66	9.00	604.51	604.51
CL Brg	1979.41	9.00	604.52	604.52
E	1989.41	9.00	604.63	604.66
F	1999.41	9.00	604.76	604.81
G	2009.41	9.00	604.90	604.95
H	2019.41	9.00	605.05	605.09
CL Brg	2033.91	9.00	605.29	605.29
Pier 2	2034.66	9.00	605.30	605.30
CL Brg	2035.41	9.00	605.31	605.31
I	2045.41	9.00	605.49	605.51
J	2055.41	9.00	605.68	605.72
K	2065.41	9.00	605.87	605.92
L	2075.41	9.00	606.08	606.11
CL Brg E. Abut.	2089.89	9.00	606.40	606.40
Bk E. Abutment	2091.33	9.00	606.43	606.43

BEAM #6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1918.53	15.00	603.91	603.91
CL Brg W. Abut.	1919.97	15.00	603.92	603.92
A	1929.97	15.00	603.97	604.00
B	1939.97	15.00	604.04	604.09
C	1949.97	15.00	604.12	604.17
D	1959.97	15.00	604.21	604.24
CL Brg	1974.45	15.00	604.36	604.36
Pier 1	1975.20	15.00	604.36	604.36
CL Brg	1975.95	15.00	604.37	604.37
E	1985.95	15.00	604.49	604.51
F	1995.95	15.00	604.61	604.65
G	2005.95	15.00	604.75	604.79
H	2015.95	15.00	604.89	604.93
CL Brg	2030.45	15.00	605.12	605.12
Pier 2	2031.20	15.00	605.14	605.14
CL Brg	2031.95	15.00	605.15	605.15
I	2041.95	15.00	605.32	605.35
J	2051.95	15.00	605.50	605.55
K	2061.95	15.00	605.70	605.74
L	2071.95	15.00	605.90	605.94
CL Brg E. Abut.	2086.43	15.00	606.21	606.21
Bk E. Abutment	2087.87	15.00	606.25	606.25

TOP OF SLAB ELEVATIONS



PLAN WEST APPROACH PAVEMENT

NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
End W. App. Pvmt.	1906.67	-16.42	603.59
A	1916.67	-16.42	603.71
B	1926.67	-16.42	603.85
Bk W. Abutment	1936.67	-16.42	603.99

NORTH EDGE OF ROADWAY PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
End W. App. Pvmt.	1904.12	-12.00	603.85
A	1914.12	-12.00	603.91
B	1924.12	-12.00	603.98
Bk W. Abutment	1934.12	-12.00	604.06

PROFILE GRADE LINE

Location	Station	Offset	Theoretical Grade Elevations
End W. App. Pvmt.	1897.19	0.00	604.06
A	1907.19	0.00	604.10
B	1917.19	0.00	604.14
Bk W. Abutment	1927.19	0.00	604.20

SOUTH EDGE OF ROADWAY PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
End W. App. Pvmt.	1890.26	12.00	603.81
A	1900.26	12.00	603.85
B	1910.26	12.00	603.91
Bk W. Abutment	1920.26	12.00	603.98

SOUTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
End W. App. Pvmt.	1887.71	16.42	603.53
A	1897.71	16.42	603.64
B	1907.71	16.42	603.75
Bk W. Abutment	1917.71	16.42	603.88

TOP OF WEST APPROACH PAVEMENT ELEVATIONS

NORTH EDGE OF APPROACH PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
Bk E. Abutment	2106.15	-16.67	606.65
A	2116.15	-16.67	606.80
B	2126.15	-16.67	606.96
End E. App. Pvmt.	2136.15	-16.67	607.17

NORTH EDGE OF ROADWAY PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
Bk E. Abutment	2103.46	-12.00	606.67
A	2113.46	-12.00	606.89
B	2123.46	-12.00	607.13
End E. App. Pvmt.	2133.46	-12.00	607.38

PROFILE GRADE LINE

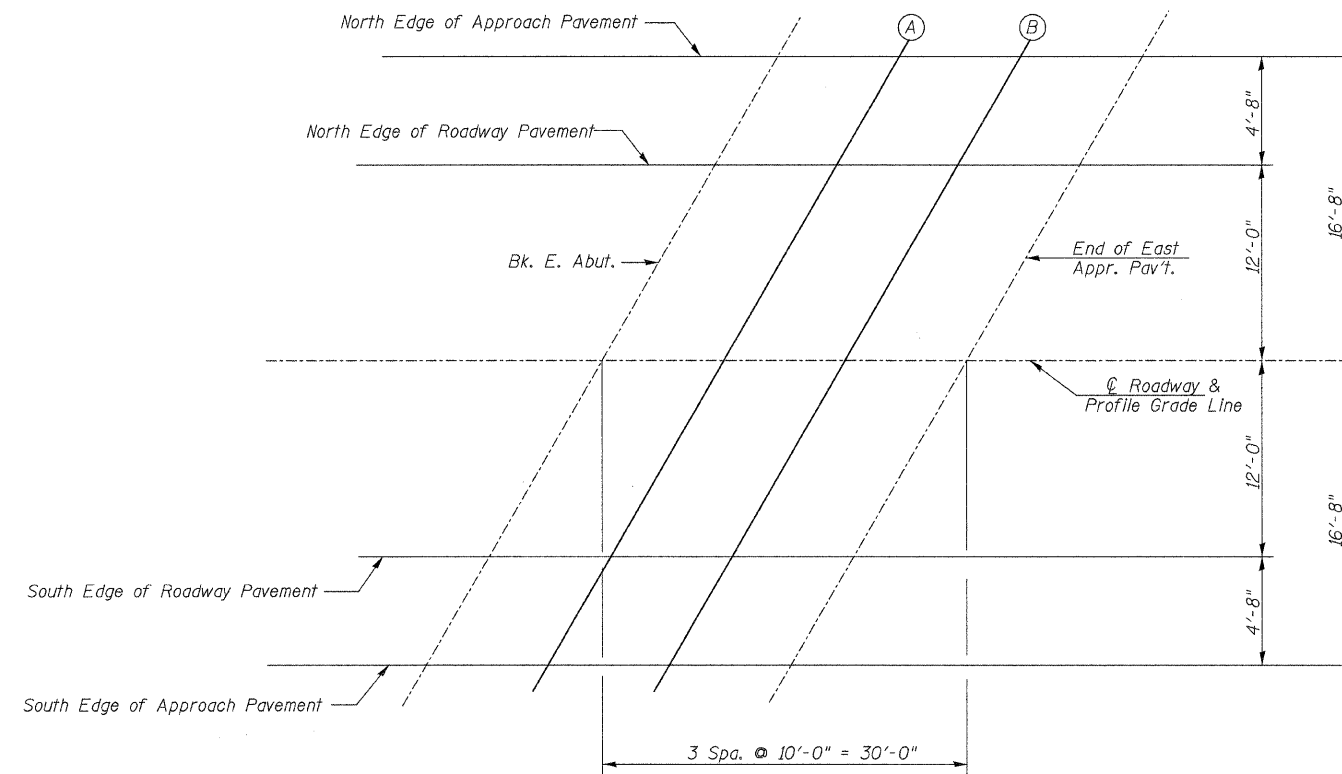
Location	Station	Offset	Theoretical Grade Elevations
Bk E. Abutment	2096.53	0.00	606.69
A	2106.53	0.00	606.93
B	2116.53	0.00	607.17
End E. App. Pvmt.	2126.53	0.00	607.43

SOUTH EDGE OF ROADWAY PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
Bk E. Abutment	2089.60	12.00	606.35
A	2099.60	12.00	606.56
B	2109.60	12.00	606.78
End E. App. Pvmt.	2119.60	12.00	607.01

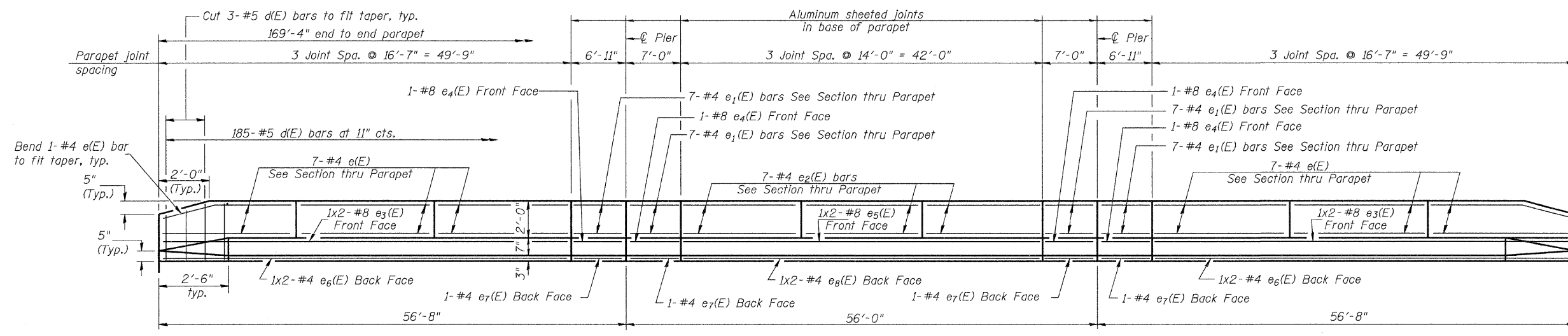
SOUTH EDGE OF APPROACH PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
Bk E. Abutment	2086.91	16.67	606.20
A	2096.91	16.67	606.34
B	2106.91	16.67	606.50
End E. App. Pvmt.	2116.91	16.67	606.65

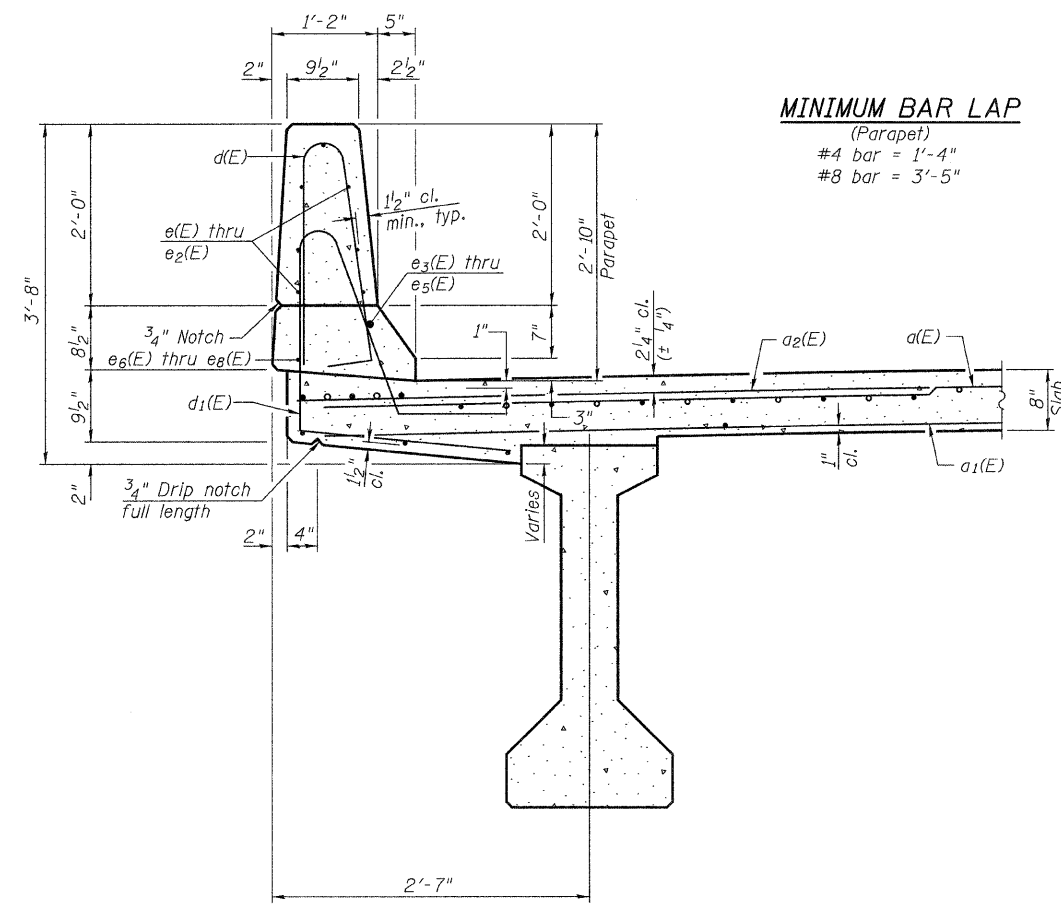


PLAN EAST APPROACH PAVEMENT

TOP OF EAST APPROACH PAVEMENT ELEVATIONS

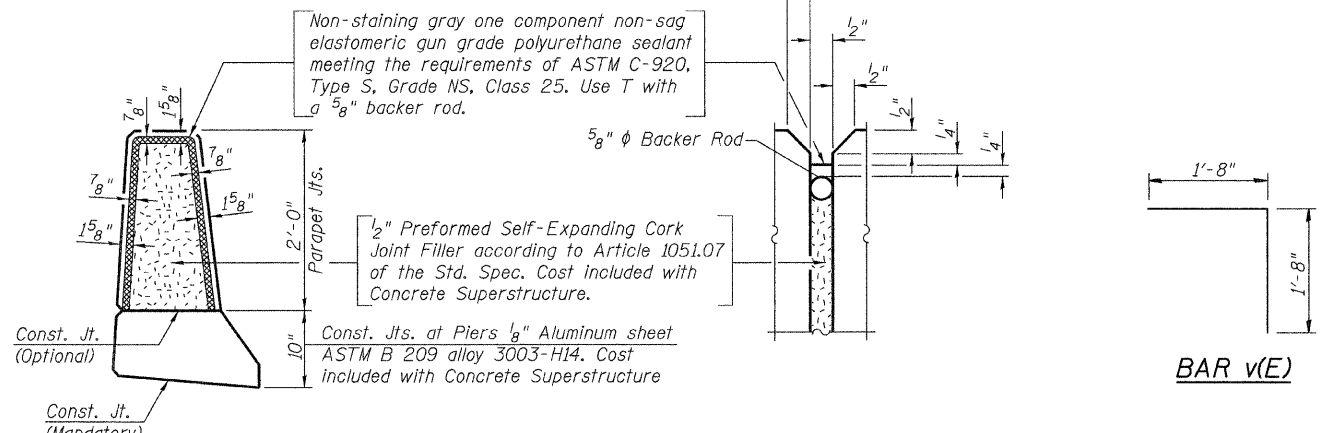


INSIDE ELEVATION OF PARAPET

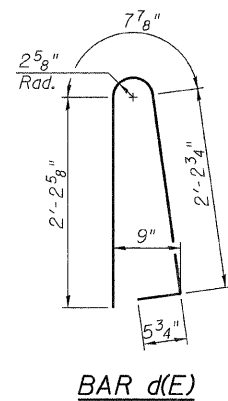


SECTION THRU PARAPET

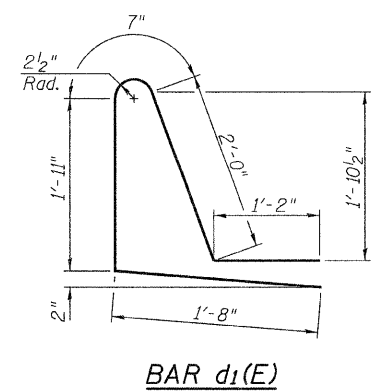
MINIMUM BAR LAP
(Parapet)
#4 bar = 1'-4"
#8 bar = 3'-5"



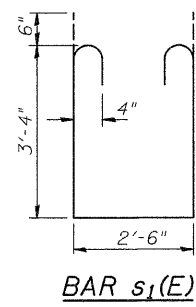
PARAPET JOINT DETAILS



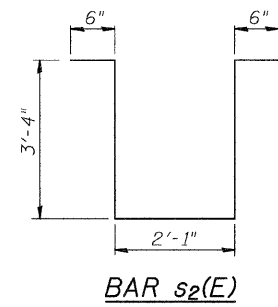
BAR d(E)



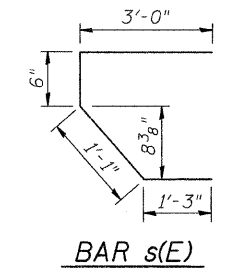
BAR d1(E)



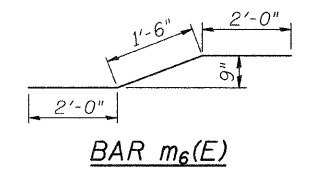
BAR s1(E)



BAR s2(E)



BAR v(E)



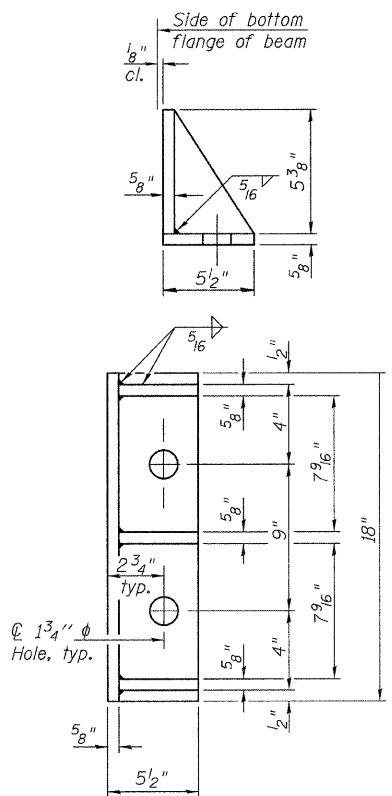
BAR m6(E)

SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	291	#5	34'-7"	—
a1(E)	204	#5	33'-7"	—
a2(E)	292	#6	6'-0"	—
a3(E)	4	#5	39'-11"	—
b(E)	152	#5	43'-7"	—
b1(E)	70	#6	26'-3"	—
b2(E)	155	#5	35'-2"	—
d(E)	370	#5	5'-7"	┘
d1(E)	370	#5	7'-4"	┘
e(E)	84	#4	16'-4"	—
e1(E)	56	#4	6'-8"	—
e2(E)	42	#4	13'-9"	—
e3(E)	8	#8	26'-6"	—
e4(E)	8	#8	6'-8"	—
e5(E)	4	#8	22'-7"	—
e6(E)	8	#4	25'-5"	—
e7(E)	8	#4	6'-8"	—
e8(E)	4	#4	21'-7"	—
m(E)	4	#6	38'-2"	—
m1(E)	6	#6	40'-4"	—
m2(E)	24	#6	9'-1"	—
m3(E)	30	#6	4'-11"	—
m4(E)	4	#6	1'-10"	—
m5(E)	40	#4	5'-11"	—
m6(E)	12	#8	5'-6"	—
s(E)	72	#5	5'-10"	┘
s1(E)	62	#4	10'-2"	┘
s2(E)	50	#4	9'-9"	┘
v(E)	68	#5	3'-4"	┘
① Reinforcement Bars, Epoxy Coated		POUND	46,420	
Concrete Superstructure		CU YD	230.1	

① See Special Provisions

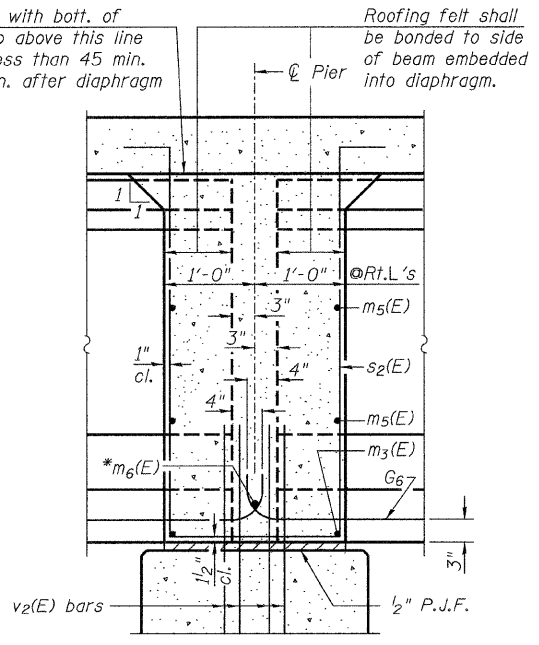
Bars indicated thus 3x2-#5 etc. indicates 3 lines of bars with 2 lengths per line.



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates. (4 Req'd.)

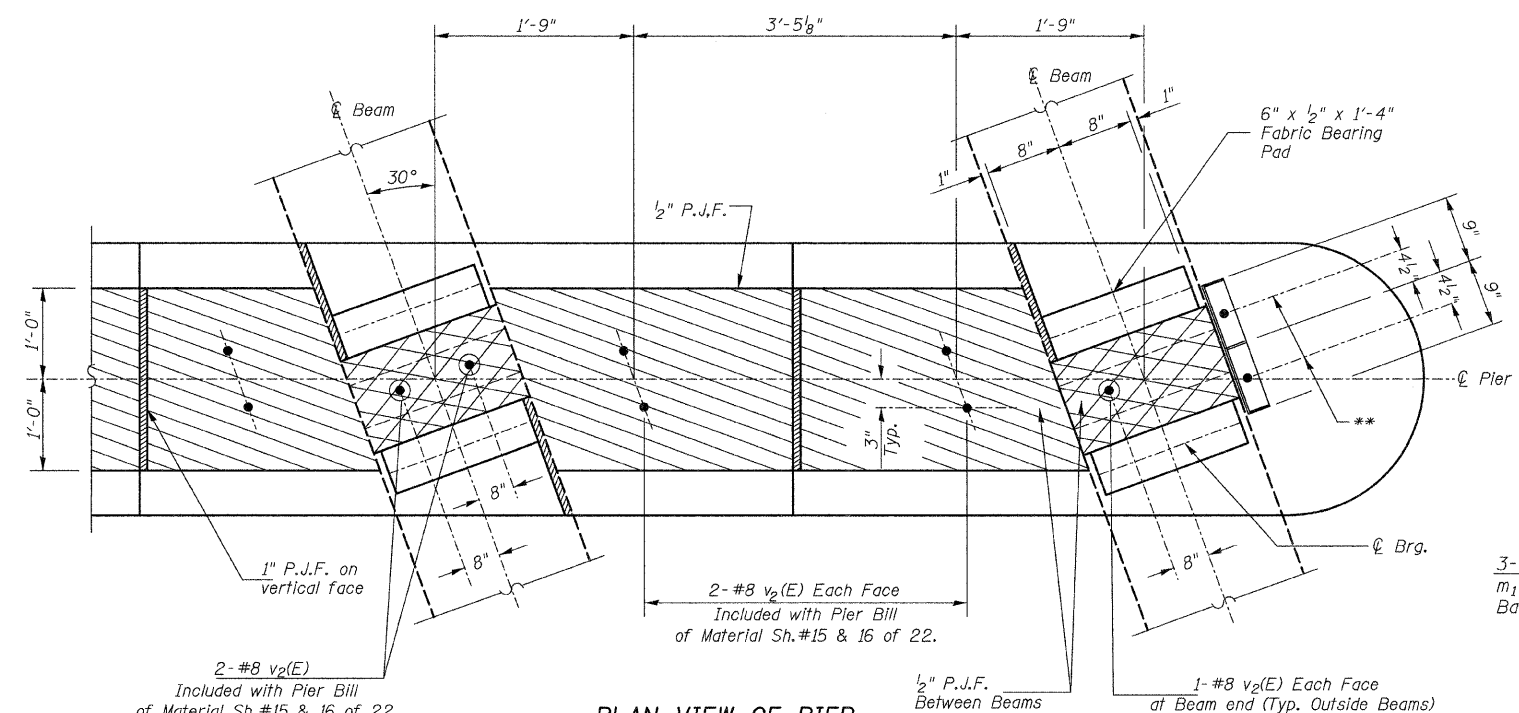
Pour diaphragm flush with bott. of slab. Concrete in slab above this line shall be placed not less than 45 min. nor more than 90 min. after diaphragm has been poured.



SECTION B-B

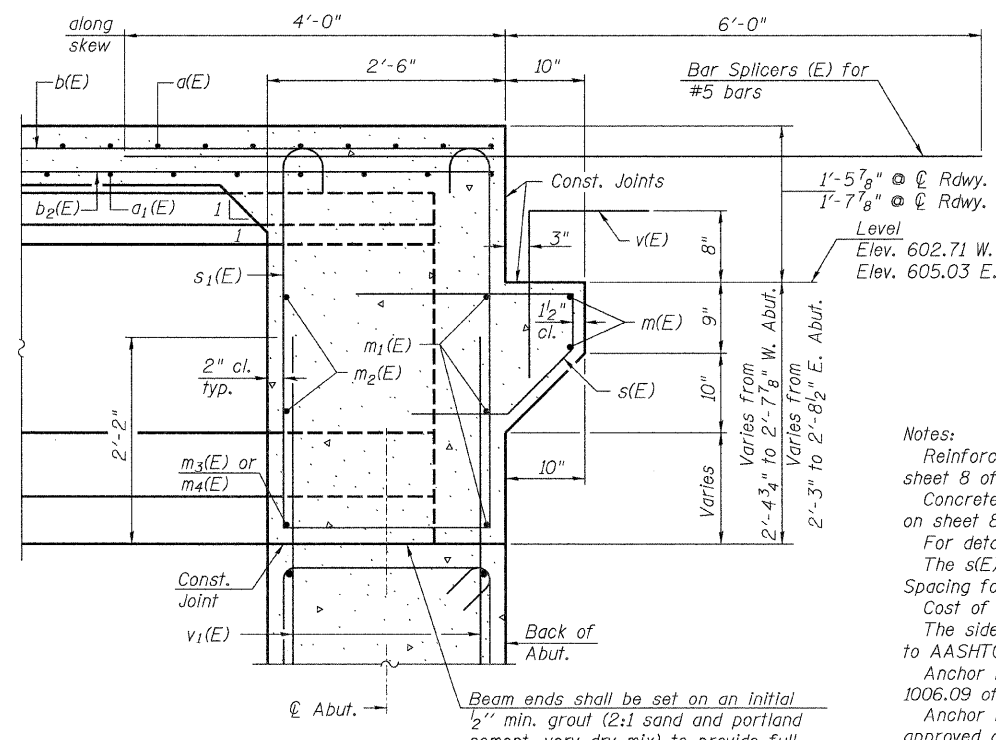
Dimensions along \bar{C} of beam, except as shown.

* Tightly fasten the #8 bars together with No. 9 wire ties.



PLAN VIEW OF PIER

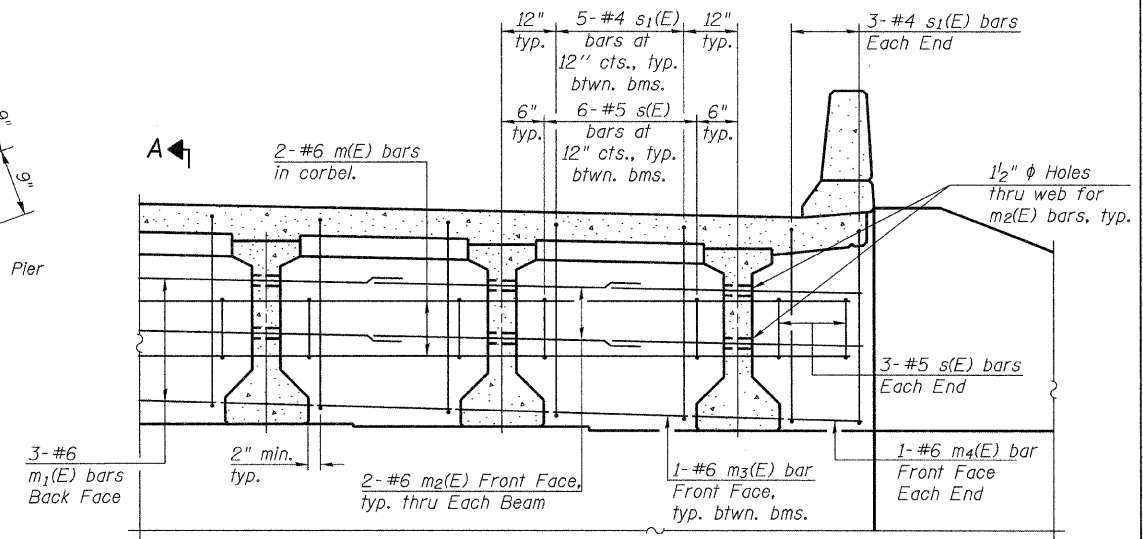
** \bar{C} 1 1/4" x 15" Anchor Bolts with 2 3/4" x 2 3/4" x 5/16" washer under nut. Holes to be drilled after beams are in place.



SECTION A-A

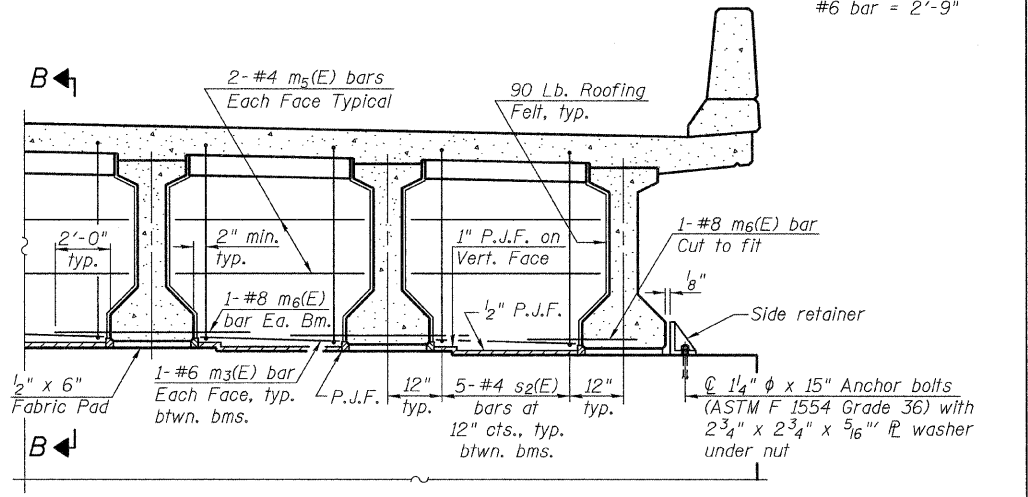
Dimensions at right angles to abutment, except as shown.

Beam ends shall be set on an initial 2" min. grout (2:1 sand and portland cement, very dry mix) to provide full bearing. Any excess grout squeezed out from under the beam shall be removed. Cost included with Concrete Structures.



DIAPHRAGM ELEVATION AT ABUTMENT

MIN. BAR LAP
#6 bar = 2'-9"



DIAPHRAGM AT PIER

Notes:
Reinforcement bars in diaphragm are billed with superstructure on sheet 8 of 22.
Concrete in diaphragm is included with Concrete Superstructure on sheet 8 of 22.
For details of bars s(E), s1(E) and s2(E) see sheet 8 of 22.
The s(E), s1(E) and s2(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.
Cost of 90 Lb. roofing felt is included with Concrete Superstructure.
The side retainer shall be galvanized after shop fabrication according to AASHTO M 111.
Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications.
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (Fy=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Anchor bolts for side retainers may be either cast in place or installed in holes drilled after the supporting member is in place and prior to pouring the deck.
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
Cost of side retainers shall be included with Concrete Structures.

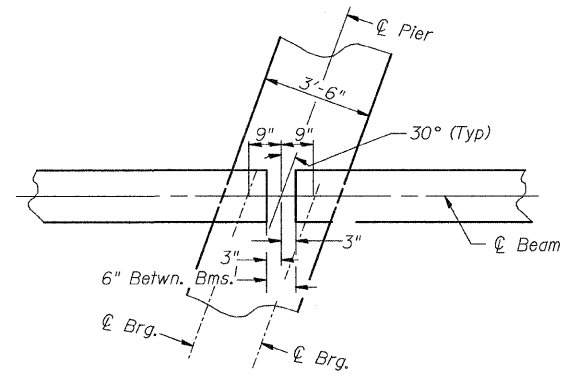
DIAPHRAGM AND BEARING PAD DETAILS

	0.4 Sp. 1 0.6 Sp. 3	Pier 1 or 2	0.5 Sp. 2
I	(in ⁴) 48,648	-	48,648
I'	(in ⁴) 291,149	-	291,149
S_b	(in ³) 3,165	-	3,165
S_b'	(in ³) 6,000	-	6,000
S_t	(in ³) 2,358	-	2,358
S_t'	(in ³) 25,964	-	25,964
$DC1$	(k/ft) 1.00	-	1.00
M_{DC1}	(k) 381	-	392
$DC2$	(k/ft) 0.15	0.15	0.15
M_{DC2}	(k) 37	46	13
DW	(k/ft) 0.30	0.30	0.30
M_{DW}	(k) 73	92	26
$M_{\Sigma + Imp}$	(k) 600	454	485

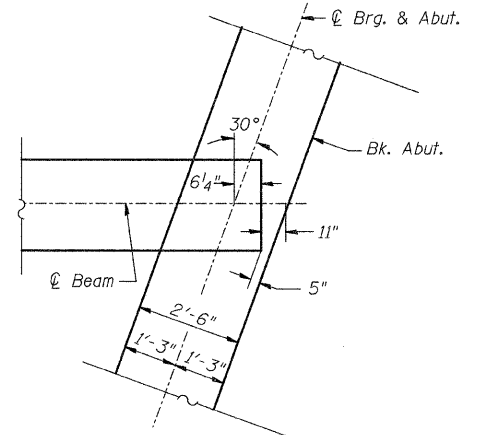
	Abut.	Pier 1 Span 1 Pier 2 Span 3	Pier 1 Span 2 Pier 2 Span 2
R_{DC1}	(k) 27.6	27.6	28.0
* R_{DC2}	(k) 3.3	4.6	4.6
* R_{DW}	(k) 6.6	9.2	9.2
* $R_{\Sigma + Imp}$	(k) 49.1	44.1	44.1
R_{Total}	(k) 86.6	85.5	85.9

* The total R_{DC2} , R_{DW} and $R_{\Sigma + Imp}$ are assumed to be distributed evenly to each bearing line at a pier regardless of the span ratios. The bearing design at a pier is based on the maximum reactions of either span.

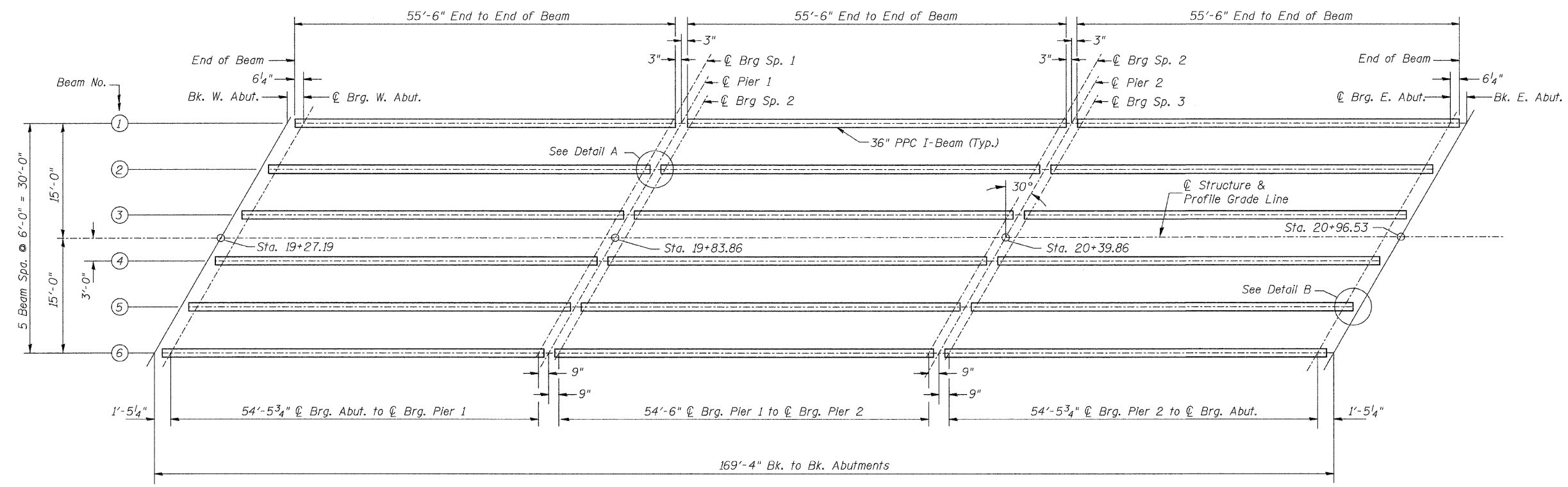
- I : Non-composite moment of inertia of beam section (in⁴).
- I' : Composite moment of inertia of beam section (in⁴).
- S_b : Non-composite section modulus for the bottom fiber of the prestressed beam (in³).
- S_b' : Composite section modulus for the bottom fiber of the prestressed beam (in³).
- S_t : Non-composite section modulus for the top fiber of the prestressed beam (in³).
- S_t' : Composite section modulus for the top fiber of the prestressed beam (in³).
- $DC1$: Un-factored non-composite dead load (kips/ft.).
- M_{DC1} : Un-factored moment due to non-composite dead load (kip-ft.).
- $DC2$: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
- M_{DC2} : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
- DW : Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
- M_{DW} : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
- $M_{\Sigma + Imp}$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).



DETAIL A
(Typical @ Piers)

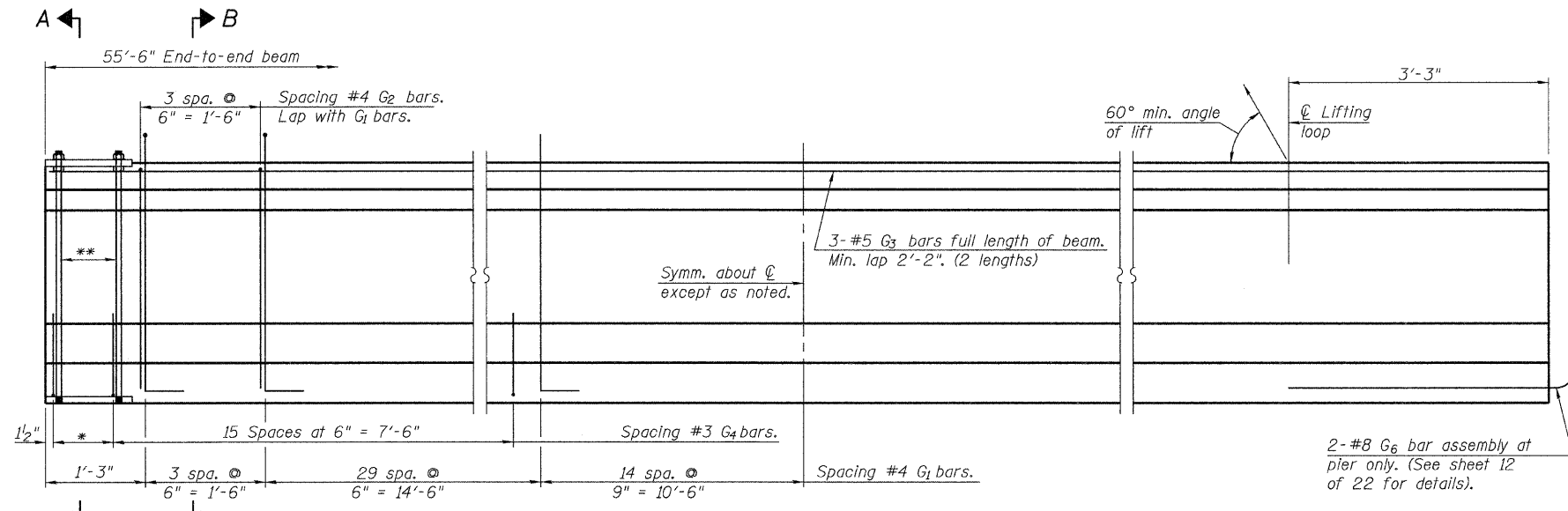


DETAIL B
(Typical @ Abutments)



FRAMING PLAN

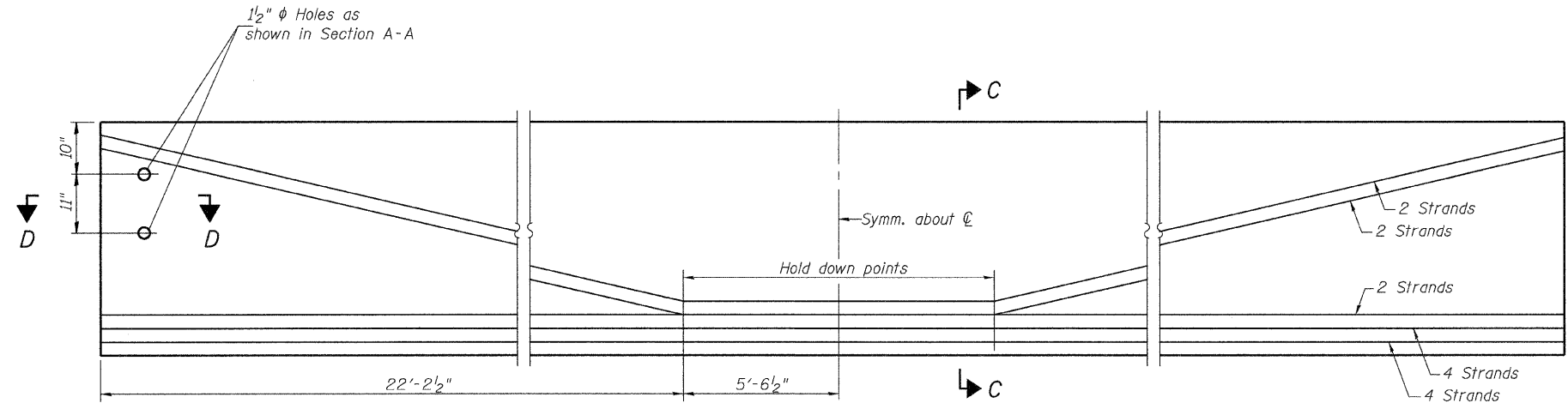
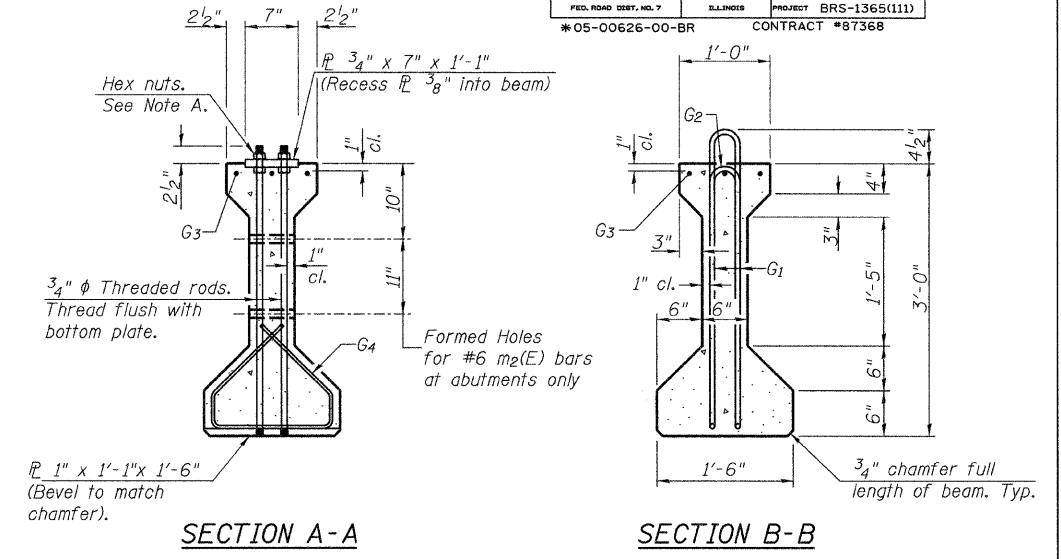
FRAMING PLAN



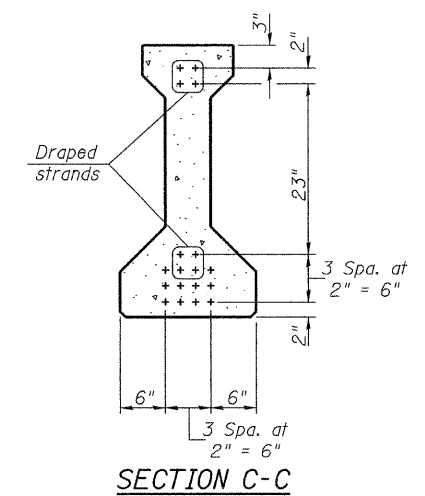
ELEVATION OF BEAM
(Showing reinforcement & dimensions)

*3 spaces at 3" = 9".
**4-3/4" ϕ threaded dowel rods at 3" cts., Each Face.

Note A:
Hex nuts (top and bottom) with lock washers (top). Only tighten sufficiently to compress lock washers.



ELEVATION OF BEAM
(Showing prestressing steel)

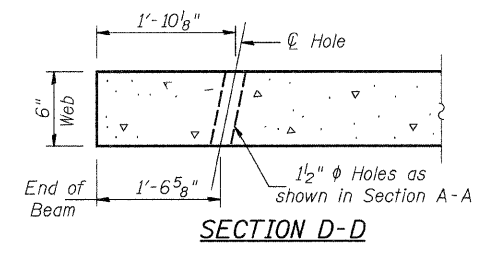


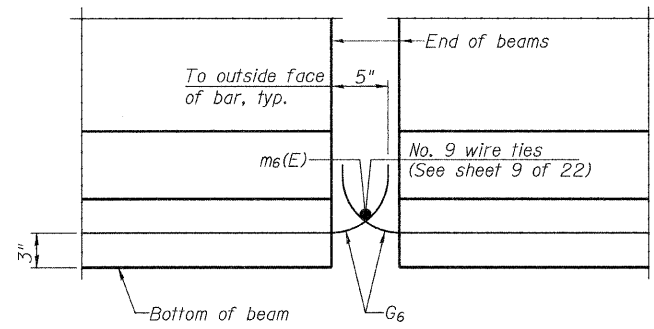
*****BAR LIST
ONE BEAM ONLY**

Bar	No.	Size	Length	Shape
G ₁	93	#4	7'-5"	Π L
G ₂	8	#4	5'-8"	Π
G ₃	6	#5	28'-9"	Π
G ₄	38	#3	4'-1"	Δ
G ₆	****2	#8	3'-9"	Π

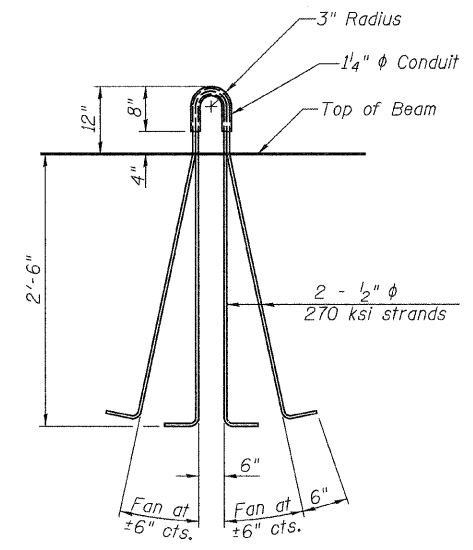
***For information only
**** 4 Bars for Center Span

Notes:
See sheet 12 of 22 for additional details and Bill of Material.
Required release strength, f'ci, shall be 5,000 psi.

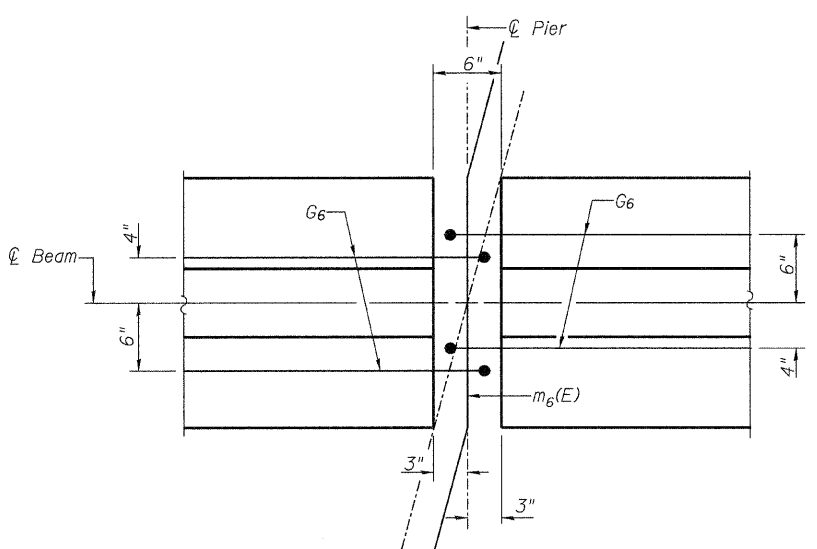




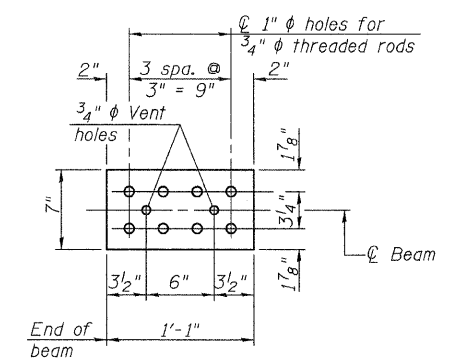
ELEVATION OF BEAM AT PIER



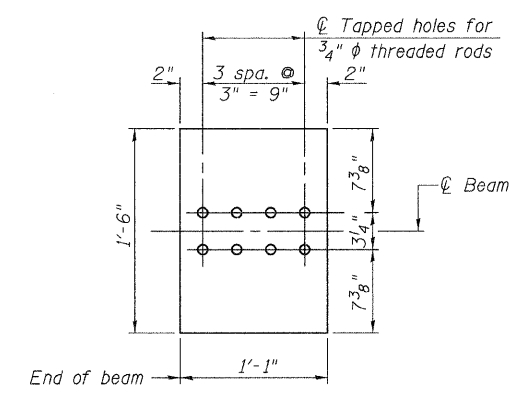
LIFTING LOOP DETAIL



PLAN OF BEAM AT PIER



TOP PLATE



BOTTOM PLATE

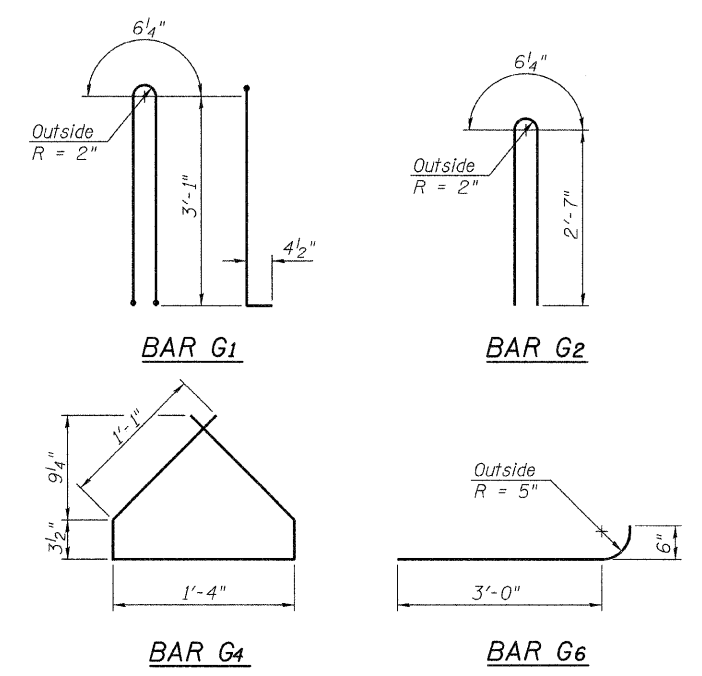
NOTES

Inserts for 3/4" ϕ threaded dowel rods, when specified, are to be two strut, coil type for interior beams and single coil, flared loop type for exterior beams.

Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be 1/2" and the nominal cross-sectional area shall be 0.153 sq. in.

Reinforcement bars shall conform to ASTM A 706, Grade 60. (See Special Provisions). A minimum 2 1/2" ϕ lifting pin shall be used to engage the lifting loops during handling. Cut G6 bars when necessary to maintain 1/2" clearance.

The top and bottom plates shall be AASHTO M270 Grade 50. The bottom plates and studs shall be galvanized according to AASHTO M111. Threaded rods shall be ASTM F 1554 Grade 55.



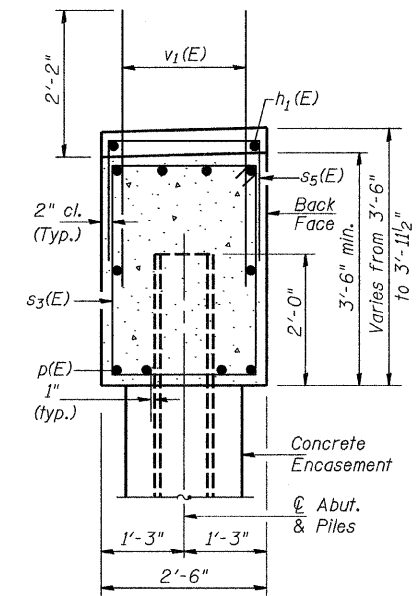
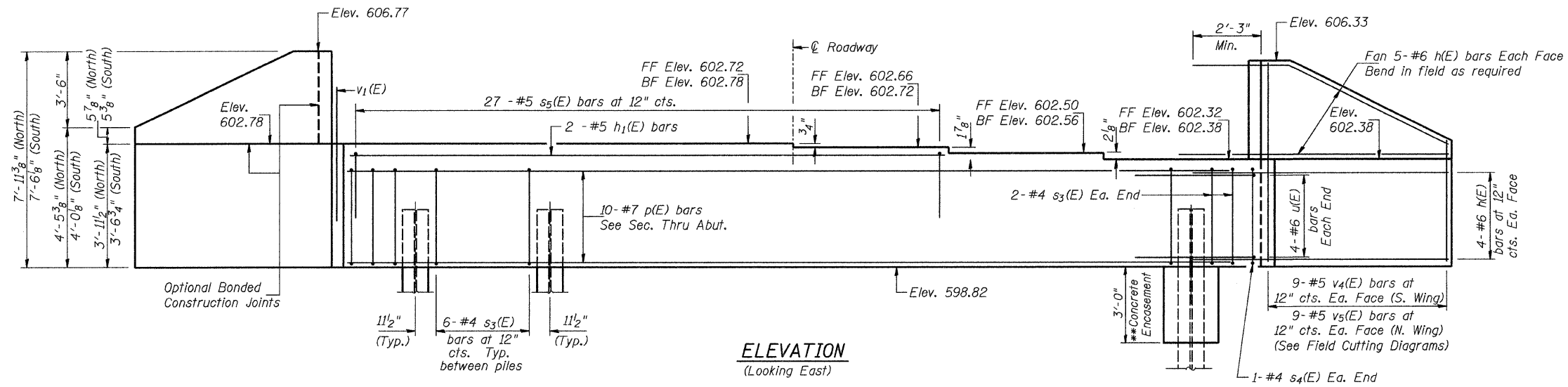
BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete I-Beams, 36"	FOOT	999

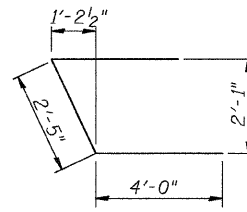
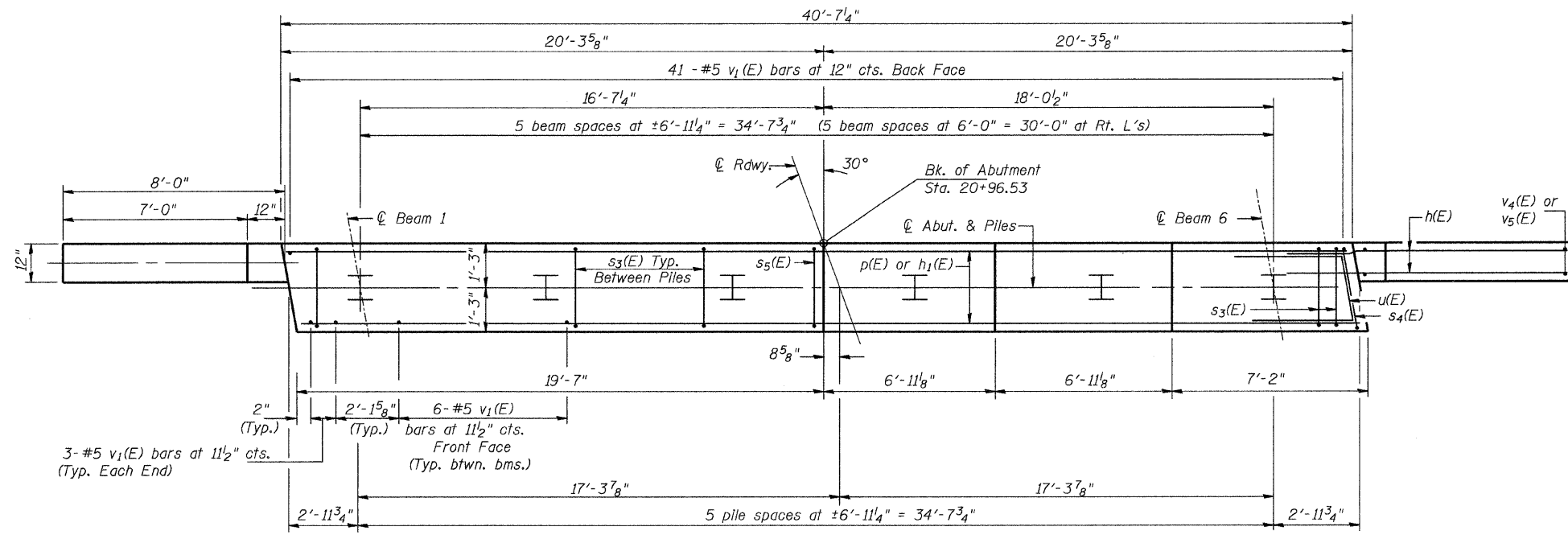
Notes:
 Four steps monolithically with cap.
 All edges shall have standard $\frac{3}{4}$ " chamfer.
 See Sh 17 of 22 for pile details.

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	23
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT	BRS-1365(111)
*05-00626-00-BR		CONTRACT #87368		

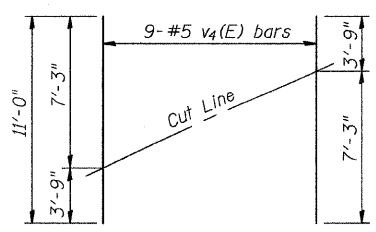
SHEET NO. 14
OF 22 SHEETS



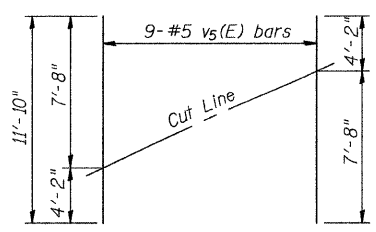
**See Sheet 17 of 22 for details.



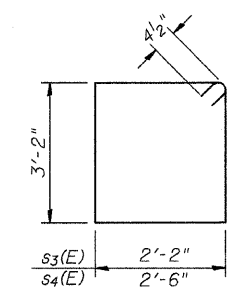
PILE DATA
 Type & Size: Steel HP10x42 w/ Pile Shoes
 Nominal Required Bearing: 335 kips
 Factored Resistance Available: 167 kips
 Est. Length: 24'
 No. Required: 6 (Includes 1 test pile)



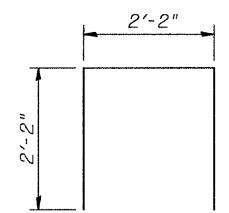
FIELD CUTTING DIAGRAM
 Order v4(E) full length. Cut as shown and use remainder of bars in opposite face.



FIELD CUTTING DIAGRAM
 Order v5(E) full length. Cut as shown and use remainder of bars in opposite face.



BARS s3(E) & s4(E)



BAR s5(E)

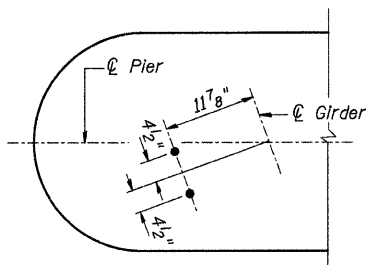
**EAST ABUTMENT
 BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h(E)	36	#6	10'-3"	—
h1(E)	2	#4	26'-3"	—
p(E)	10	#7	40'-4"	—
s3(E)	34	#4	11'-5"	□
s4(E)	2	#4	12'-1"	□
s5(E)	27	#4	6'-6"	□
u(E)	8	#6	10'-5"	U
v1(E)	77	#5	4'-4"	—
v4(E)	9	#5	11'-0"	—
v5(E)	9	#5	11'-10"	—
Concrete Structures		CU YD	18.2	
Reinforcement Bars, Epoxy Coated		POUND	2,490	
Structure Excavation		CU YD	65	
Furnishing Steel Piles HP10x42		FOOT	120	
Test Pile Steel HP10x42		EACH	1	
Pile Shoes		EACH	6	
Driving Piles		FOOT	120	
Concrete Encasement		CU YD	2.1	

① See Special Provisions

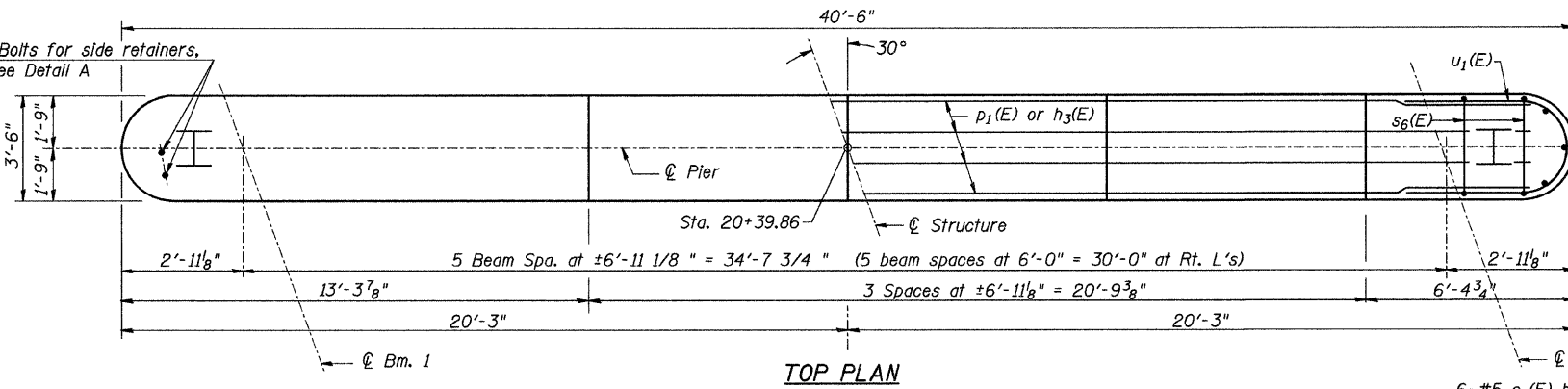
EAST ABUTMENT

Notes:
All edges shall have standard $\frac{3}{4}$ " chamfer.
Pour steps monolithically with cap.
Space reinforcement to miss anchor bolts.
See Sh 17 of 22 for pile details.

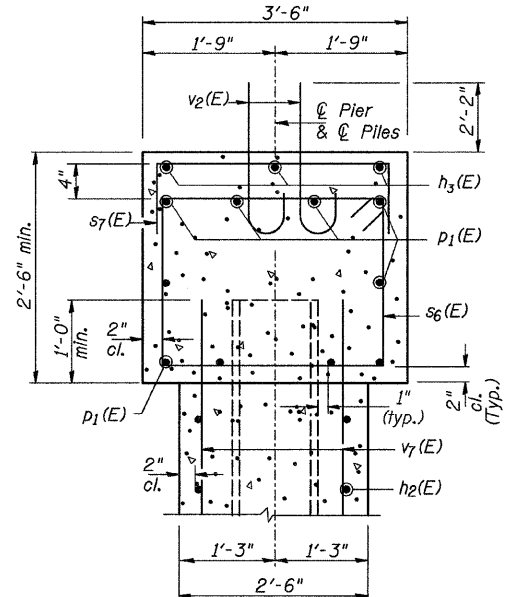


DETAIL A
ANCHOR BOLT LOCATION

$1\frac{1}{4}$ " ϕ x 15" Anchor Bolts for side retainers.
Ea. End of Pier, See Detail A



TOP PLAN

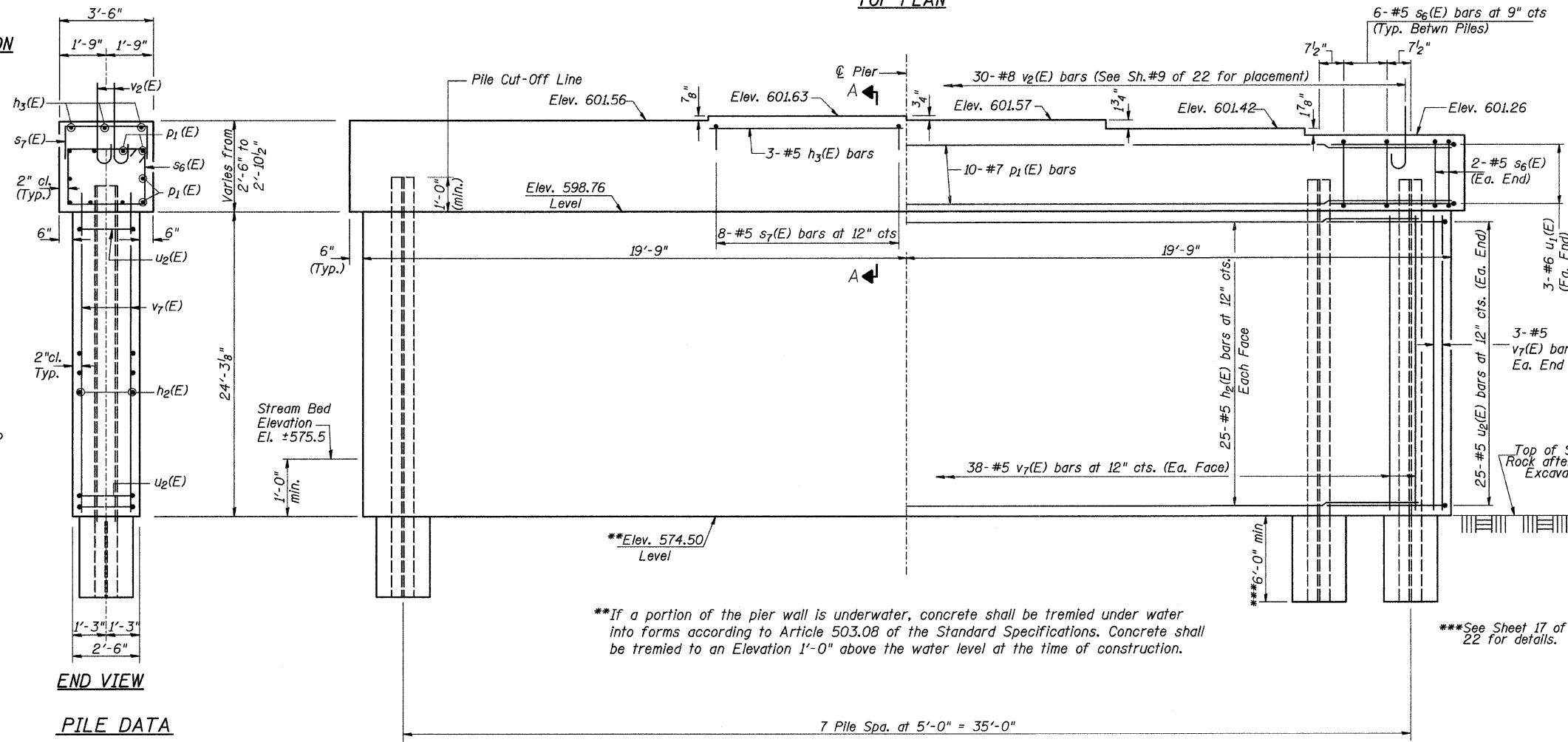


SECTION A-A

PIER #2 - BILL OF MATERIAL

BAR	NO.	SIZE	LENGTH	SHAPE
$h_2(E)$	50	#5	37'-0"	—
$h_3(E)$	3	#5	6'-7"	—
$p_1(E)$	10	#7	37'-0"	—
$s_6(E)$	46	#5	11'-5"	□
$s_7(E)$	8	#5	7'-6"	□
$u_1(E)$	6	#6	12'-0"	U
$u_2(E)$	50	#5	10'-5"	U
$v_2(E)$	30	#8	4'-7"	J
$v_7(E)$	82	#5	25'-3"	—
Concrete Structures			CU YD	101.8
① Reinforcement Bars, Epoxy Coated			POUND	6,500
Rock Excavation			CU YD	10
Structure Excavation			CU YD	20
Furnishing Steel Piles HP12x63			FOOT	256
① Setting Piles in Rock			EACH	8
① Underwater Structure Excavation Protection, Location 2			EACH	1

① See Special Provisions

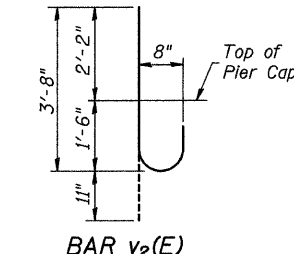


ELEVATION
(Looking East)

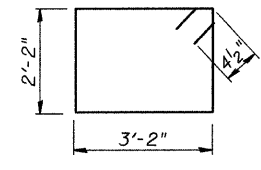
END VIEW

PILE DATA

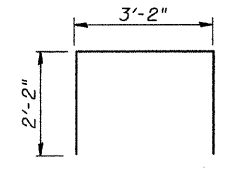
Type & Size: Steel HP12x63
Nominal Required Bearing: Set in Rock (497 kips)
Factored Resistance Available: (249 kips)
Est. Length: 32'
No. Required: 8



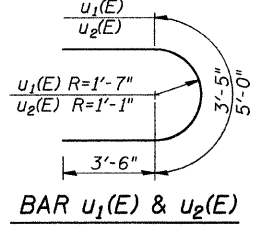
BAR $v_2(E)$



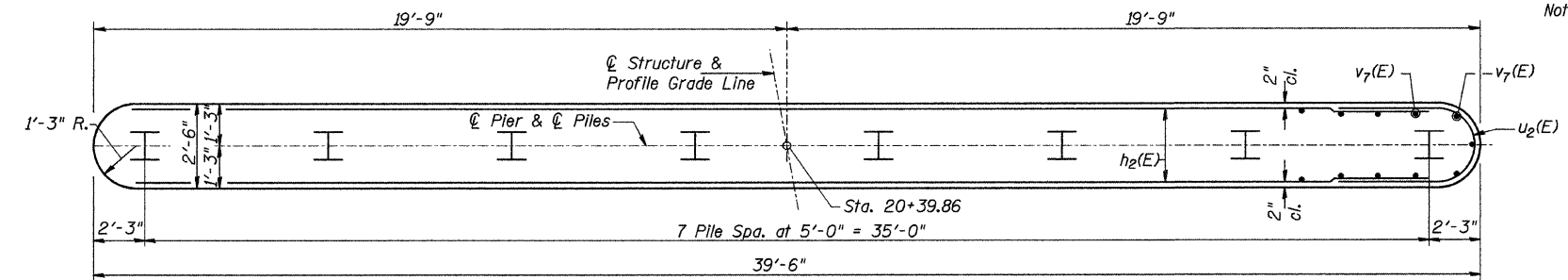
BAR $s_6(E)$



BAR $s_7(E)$



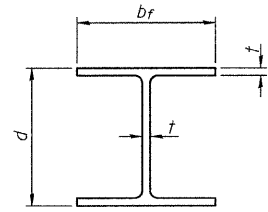
BAR $u_1(E)$ & $u_2(E)$



FOOTING PLAN

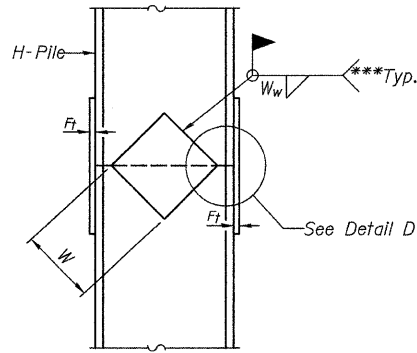
Note:
The Pier Stem at each location shall rest on sound rock, but at an elevation no higher than 1' below the proposed streambed elevation. Should rock be encountered above this elevation, the rock shall be excavated to this minimum elevation to provide a level bearing for the stem. Should sound rock be encountered below this elevation the stem shall be extended to sound rock which has been excavated to provide a level bearing. Bottom of stem elevations shown on the plan sheets represent conditions anticipated based on the available subsurface information. Should either stem need to be adjusted from the elevations shown, the stem height and effected reinforcement lengths and/or number of bars required shall be adjusted accordingly.

PIER #2

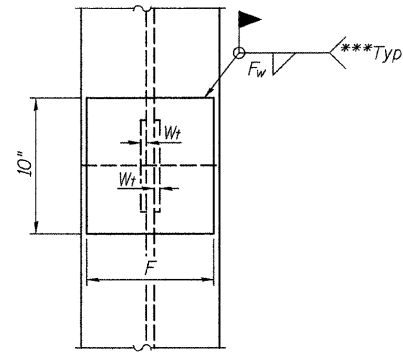


STEEL PILE TABLE

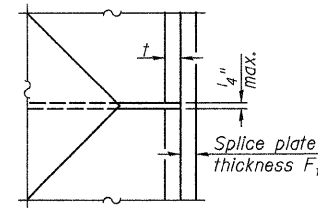
Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	13/16"	30"
x102	14"	14 3/4"	1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



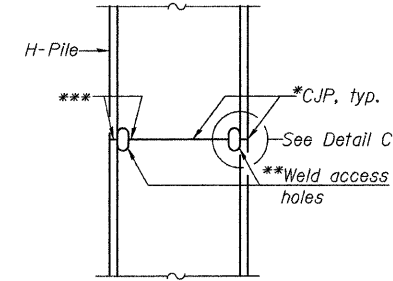
ELEVATION



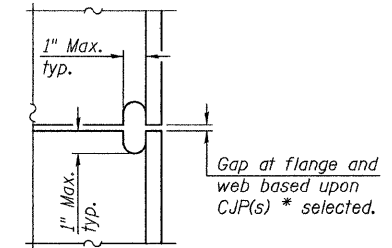
END VIEW



DETAIL D



ELEVATION



DETAIL C

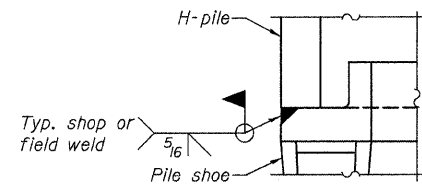
COMPLETE PENETRATION WELD SPLICE

*Use joint conforming to Figure 3.4 in AWS D1.1, Structure Welding Code - Steel.
 **Preparation per Fig. 5.2 in AWS D1.1, Structure Welding Code - Steel.
 ***Interrupt welds 1/4" from end of each pile.

Designation	F	Ft	Fw	W	Wt	Ww
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5 1/2"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5 1/2"	1/2"
x89	12 1/2"	3/4"	1/16"	7 3/4"	5 1/2"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5 1/2"	1/2"
HP 12x84	10"	7/8"	1/16"	6 1/2"	5 1/2"	1/2"
x74	10"	7/8"	1/16"	6 1/2"	5 1/2"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

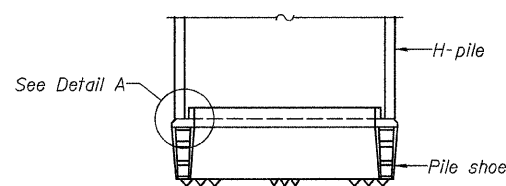
WELDED PLATE FIELD SPLICE

Note:
 The steel H-piles shall be according to AASHTO M270 Grade 50.

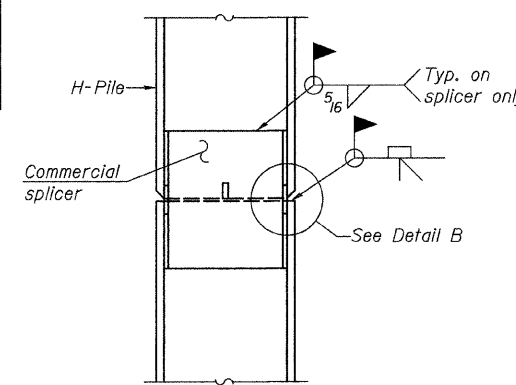


DETAIL A

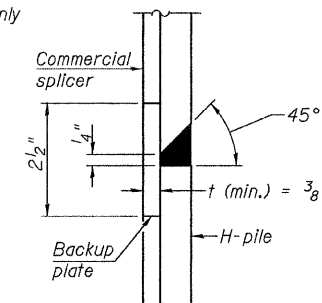
H-PILE SHOE ATTACHMENT



ELEVATION

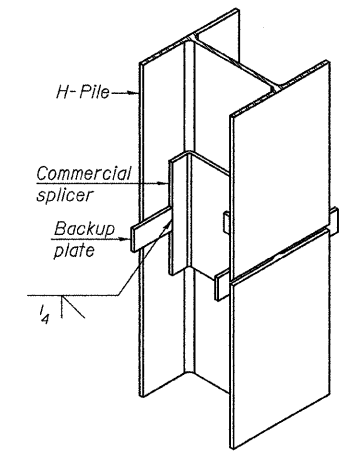


ELEVATION

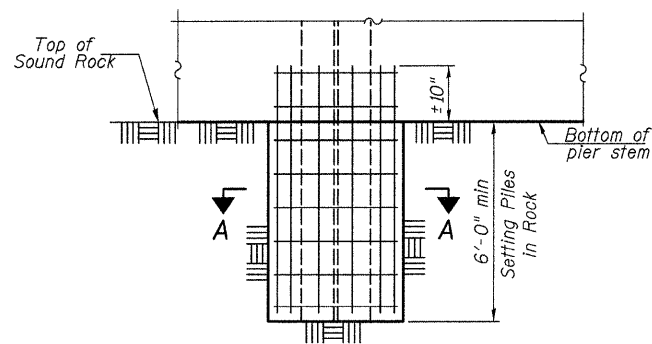


DETAIL "B"

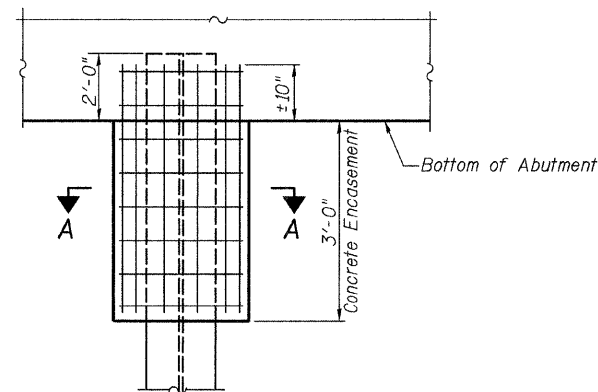
WELDED COMMERCIAL SPLICE



ISOMETRIC VIEW

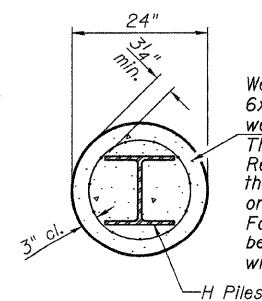


PIER ELEVATION



ABUTMENT ELEVATION

PILE ENCASEMENT



SECTION A-A

Welded wire fabric
 6x6-W4.0xW4.0
 weighing 58#/100 sq. ft.
 The cost of Excavation and Reinforcement is included with the cost of Concrete Encasement or Setting Piles in Rock as applicable. Forms for encasement may be omitted when soil conditions will permit.

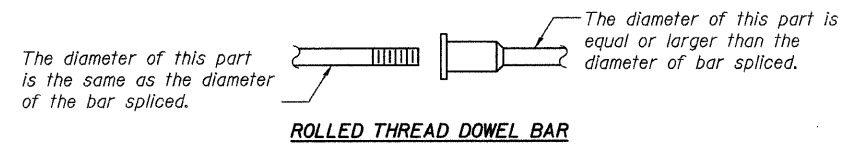
NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.
 Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length.
 All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.
 Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.
 Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

- ① Minimum Capacity = $1.25 \times f_y \times A_t$
(Tension in kips)
- ② Minimum *Pull-out Strength = $0.66 \times f_y \times A_t$
(Tension in kips)

Where f_y = Yield strength of lapped reinforcement bars in ksi.
 A_t = Tensile stress area of lapped reinforcement bars.
 * = 28 day concrete

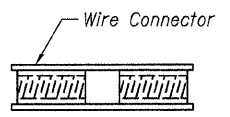
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Strength Requirements	
		Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension
#4	1'-8"	14.7	7.9
#5	2'-0"	23.0	12.3
#6	2'-7"	33.1	17.4
#7	3'-5"	45.1	23.8
#8	4'-6"	58.9	31.3
#9	5'-9"	75.0	39.6
#10	7'-3"	95.0	50.3
#11	9'-0"	117.4	61.8



ROLLED THREAD DOWEL BAR



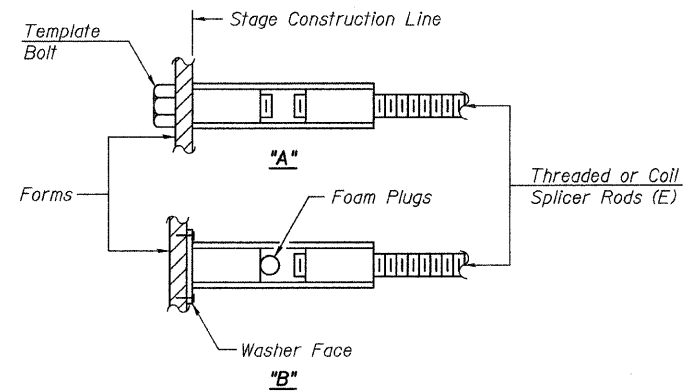
**** ONE PIECE**



WELDED SECTIONS

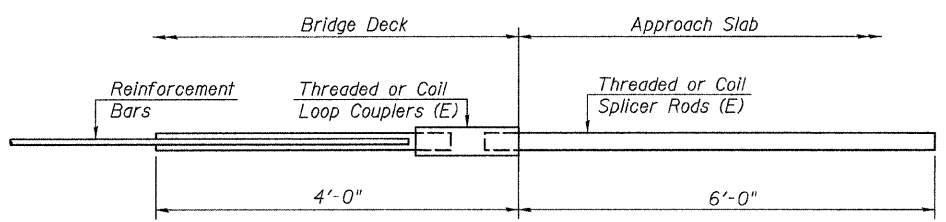
BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



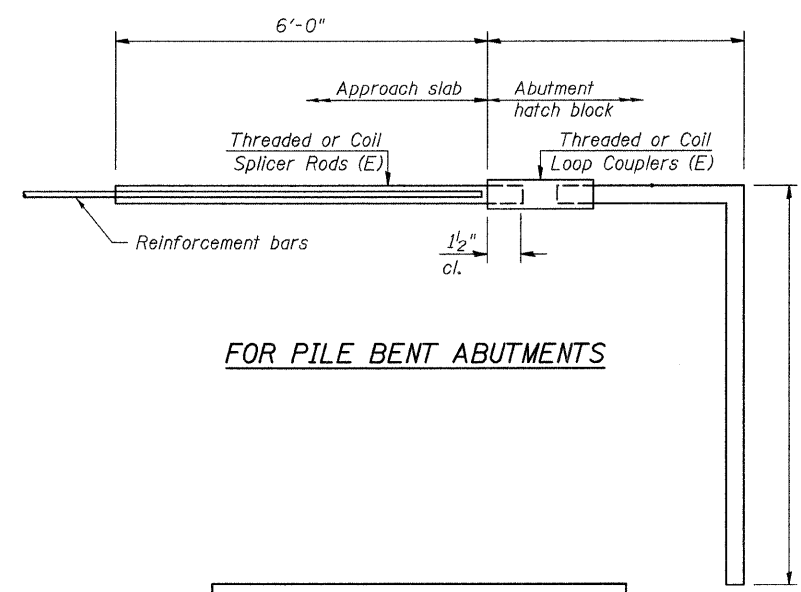
INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
 (E) : Indicates epoxy coating.



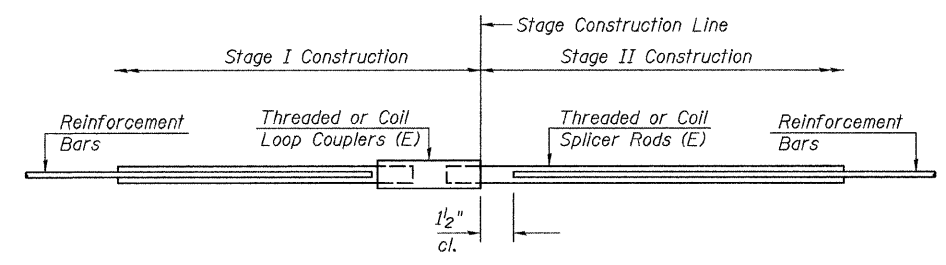
FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required = 68



FOR PILE BENT ABUTMENTS

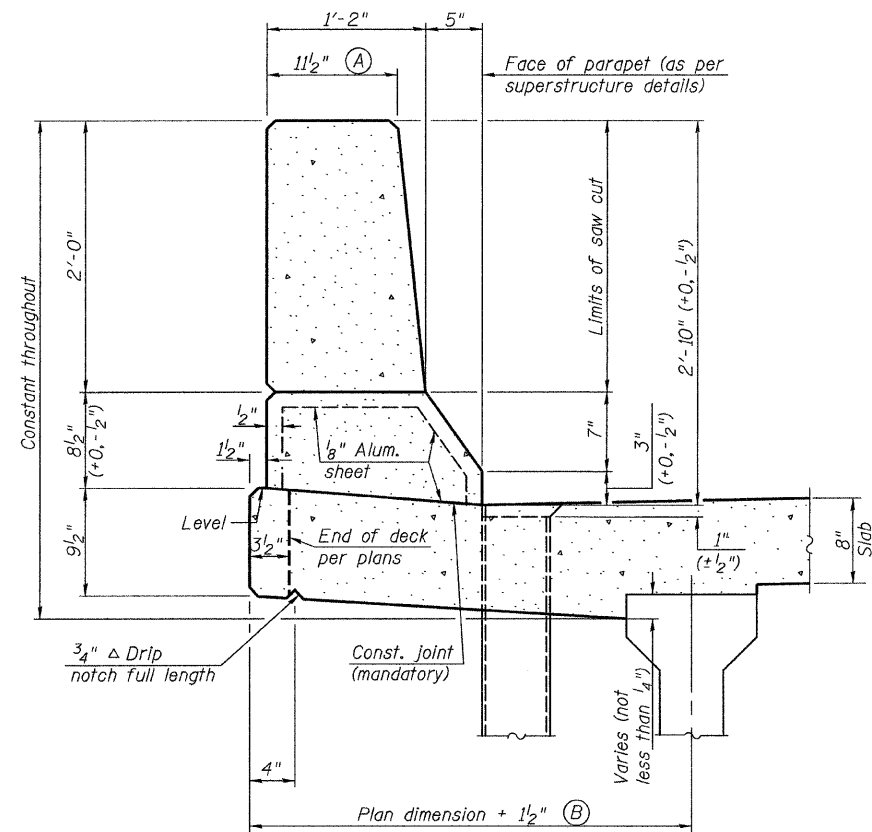
Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required = 0



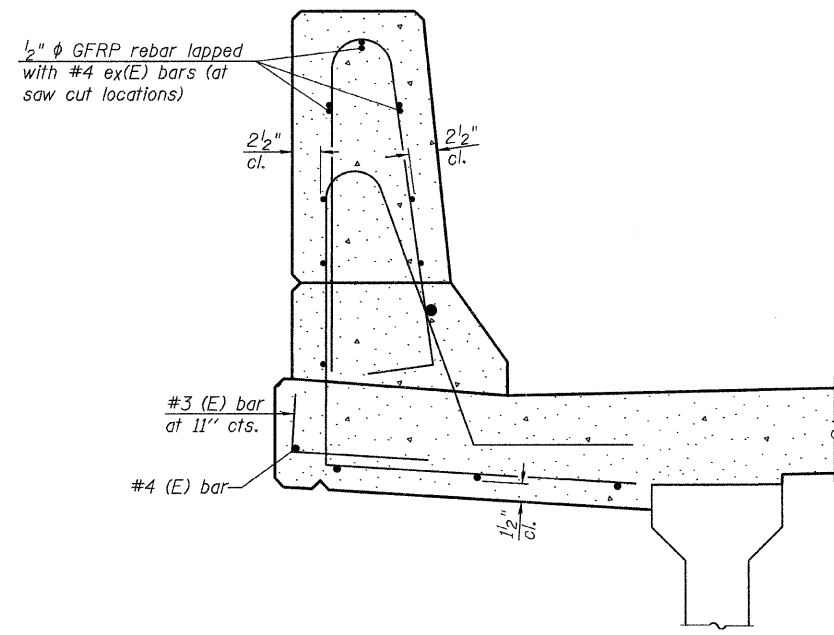
STANDARD

Bar Size	No. Assemblies Required	Location

BAR SPLICER ASSEMBLY DETAILS



SECTION
(Showing dimensions)

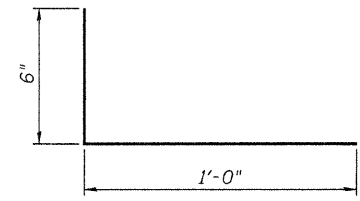


SECTION
(Showing reinforcement clearances for slip forming and additional reinforcement bars)

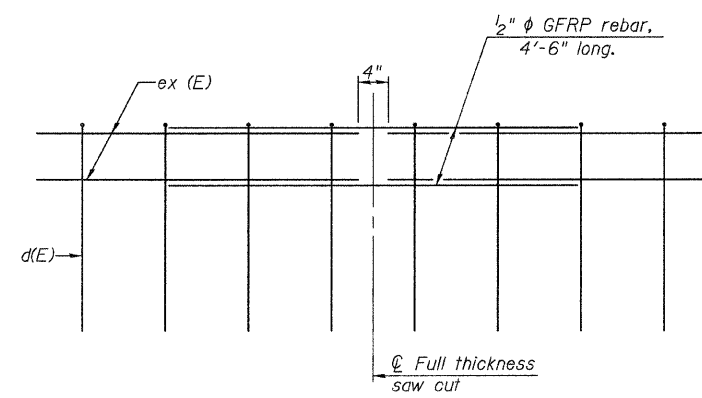
GENERAL NOTES

All dimensions shall remain the same as shown on contract plans, except dimensions (A) and (B) which are to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension (A) and (B) = 0.0165 cu. yds./ft. of parapet.

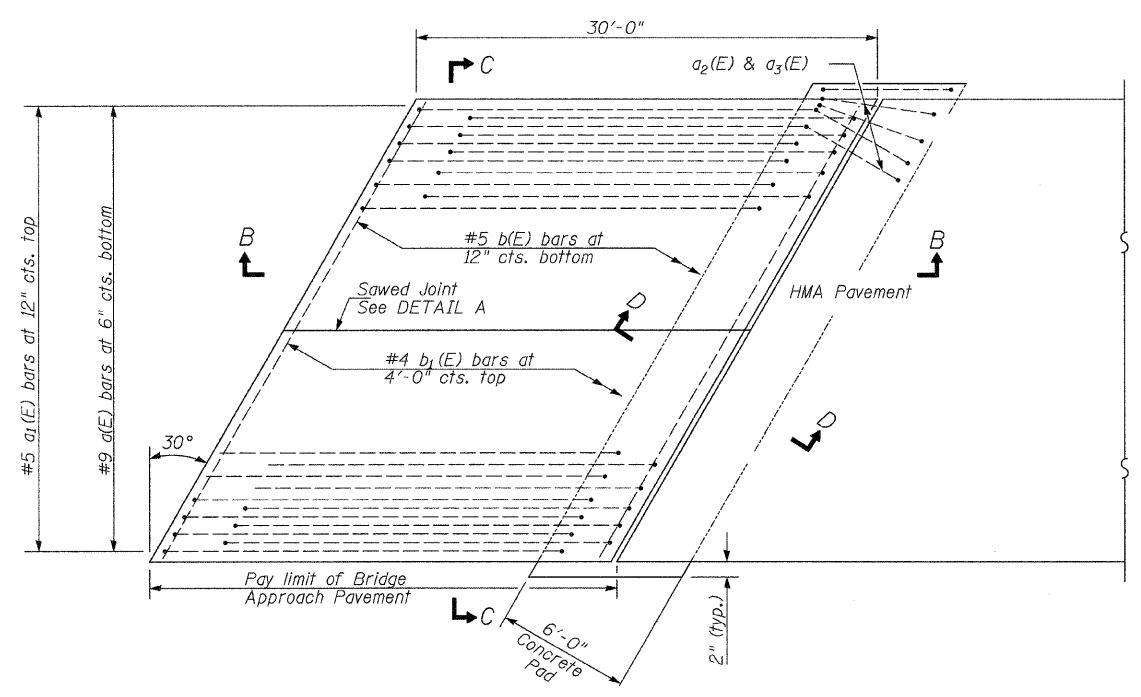
Place aluminum sheet in curb portion at and near piers. Full thickness saw cut at all joint locations in lieu of cork joint filler.



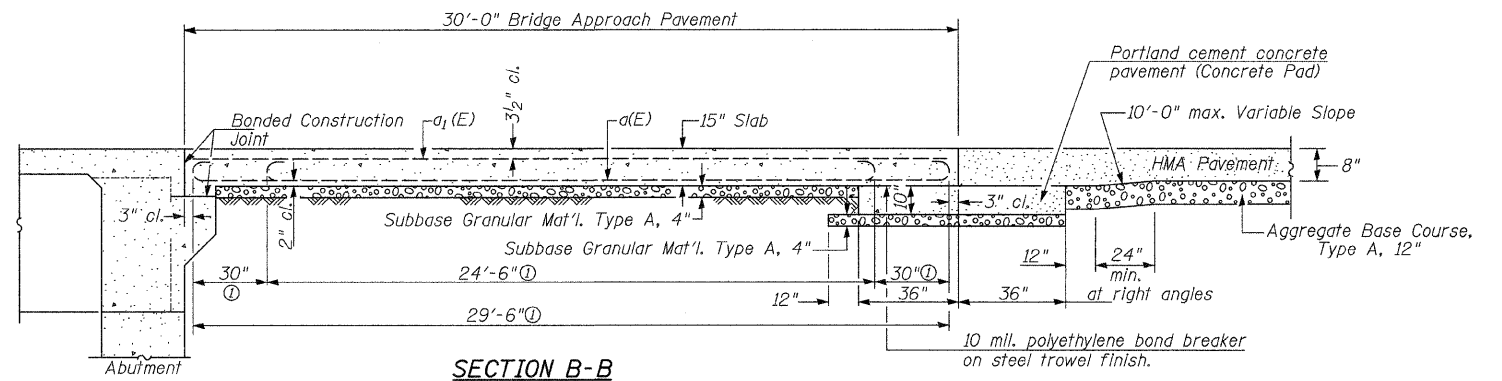
#3 (E) BAR



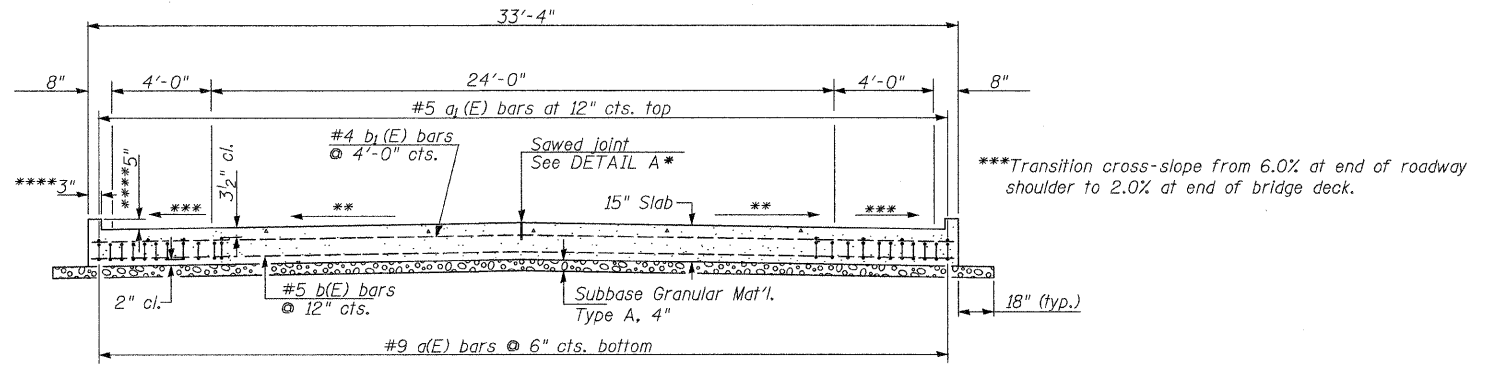
GFRP REBAR STIFFENING DETAIL
(Place as shown in parapet section at each parapet joint location.)



PLAN



SECTION B-B
⓪ Stagger #9 a(E) bars as shown on plan - full width

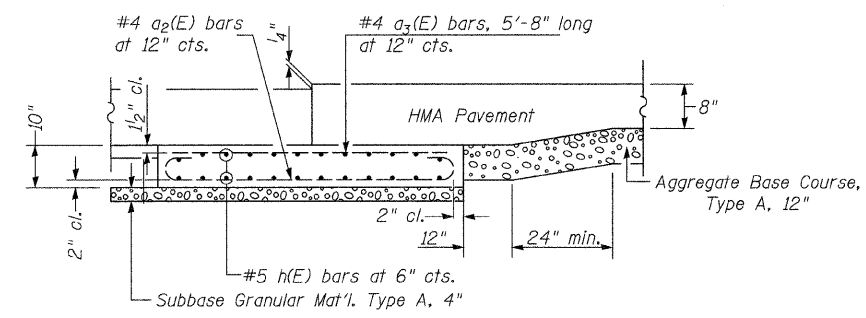


SECTION C-C
(See Plan for Dimensions not shown)

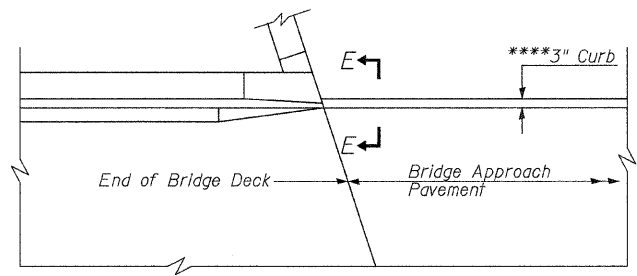
**Transition cross-slope from 2.0% at end of roadway pavement to 1.5% at end of bridge deck.
****No curb on east approach pavement.

BILL OF MATERIAL

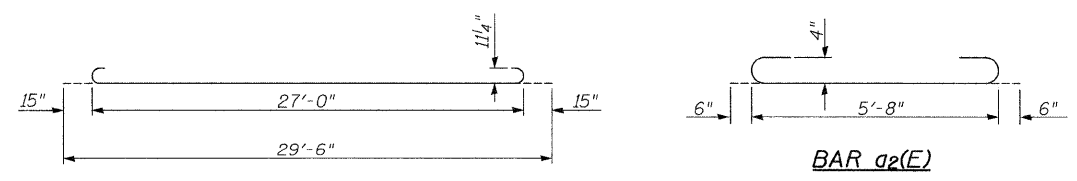
Item	Unit	Quantity
Bridge Approach Pavement	SQ YD	222



SECTION D-D
(Showing reinforcement)

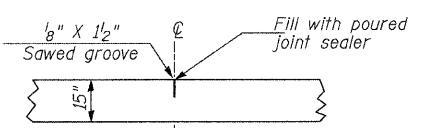


**PARAPET TO CURB TRANSITION
INTEGRAL ABUTMENT**

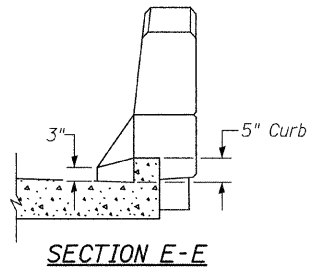


BAR a1(E)

BAR a2(E)



DETAIL A
(Reinforcement Not Shown)




SECTION E-E

DESIGN STRESSES

$f_y = 60,000$ p.s.i.
 $f'_c = 3,500$ p.s.i.
 $n = 8.5$

**BRIDGE APPROACH
PAVEMENT DETAILS**


Note:
Cost of Concrete, Reinforcement and Subbase Granular Material for Approach Pavement and Concrete Pad is included in cost of Bridge Approach Pavement.

 Midwest Testing Services, Inc. 3705 Progress Blvd. Peru, IL 61354	BORING LOG		Phone: 815-223-6696
	Sheet 1 of 2		Fax: 815-223-6659
Client: <u>Hutchison Engineering, Inc.</u>	Boring No. <u>B-1</u>		e-mail: mts37@comcast.net
Project Name: <u>Sec 05-00626-00-BR</u>	Surface Elev. <u>598.03</u>		
Project Site: <u>CH-33 Over Little Vermilion River</u>	Auger Depth <u>34'</u>	Rotary Depth <u>NA</u>	
<u>LaSalle County, Illinois</u>	Start Date <u>05/03/08</u>	Finish Date <u>05/03/08</u>	

Location: 6' Lt of Centerline CH-33
Station 19+55

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Grain Size Log	Depth in feet	SAMPLES						DRILLED BY	REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)		
598.03										Randy Satranski Diedrich D-120	
597.03	Stiff Brown Sandy To Gravely Loam		1								
596.03			2								
595.03			3	1	SS	1.8	13	S	16		
594.03	Medium Brown Sandy Loam		4								
593.03			5								
592.03			6	2	SS	1.4	11	S	12		
591.03	Medium To Stiff Brownish Gray Silty Clay Loam		7								
590.03			8								
589.03			9	3	SS	1.2	10	B	20		
588.03	Very Dense Brown Coarse Gravel And Cobbles		10								
587.03			11								
586.03			12	4	SS	---	12	---	22		
585.03			13								
584.03			14								
583.03			15	5	SS	---	14	---	19		
582.03			16								
581.03			17								
580.03			18	6	SS	0.8	7	B	24		
579.03			19								
578.03			20								
			21	7	SS	1.3	10	B	21		
		22	8	SS	---	55	---	---			


Groundwater Data: Static water level after auger removal - elevation 581.0.
Comments:

 Midwest Testing Services, Inc. 3705 Progress Blvd. Peru, IL 61354	BORING LOG		Phone: 815-223-6696
	Sheet 2 of 2		Fax: 815-223-6659
Client: <u>Hutchison Engineering, Inc.</u>	Boring No. <u>B-1</u>		e-mail: midwest.07@iosightbb.com
Project Name: <u>Sec 05-00626-00-BR</u>	Surface Elev. <u>598.03</u>		
Project Site: <u>CH-33 Over Little Vermilion River</u>	Auger Depth <u>34'</u>	Rotary Depth <u>NA</u>	
<u>LaSalle County, Illinois</u>	Start Date <u>05/03/08</u>	Finish Date <u>05/03/08</u>	

Location: _____

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Grain Size Log	Depth in feet	SAMPLES						DRILLED BY	REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)		
577.03										Randy Satranski Diedrich D-120	
576.03	Very Dense Brown Coarse Gravel And Cobbles		22								
575.03	Very Dense Yellowish Brown Weathered Sandstone		23	9	SS	---	78	---	---		
574.03	Refusal With Hollow Stem Auger		24								
573.03			25								
572.03			26								
571.03	Very Dense Pale Yellow And White Sandstone		27								
570.03			28								
569.03			29								
568.03	Penetrated With Rock Bit		30								
567.03			31								
566.03			32								
565.03	Bottom of Boring		33								
564.03			34								
563.03			35								
562.03		36									
561.03		37									
560.03		38									
559.03		39									
558.03		40									
557.03		41									

Groundwater Data: Static water level after auger removal - elevation 581.0.
Comments:

 Midwest Testing Services, Inc. 3705 Progress Blvd. Peru, IL 61354	BORING LOG		Phone: 815-223-6696
	Sheet <u>1</u> of <u>2</u>		Fax: 815-223-6659
			e-mail: mts37@comcast.net


Client: Hutchison Engineering, Inc.
 Project Name: Sec 05-00626-00-BR
 Project Site: CH-33 Over Little Vermilion River
LaSalle County, Illinois

Boring No. B-2
 Surface Elev. 601.87
 Auger Depth 35' Rotary Depth NA
 Start Date 05/03/08 Finish Date 05/03/08

Location: 7' Lt of Centerline CH-33
Station 20+70

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES						DRILLED BY		REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)		
601.87											Randy Saffanski Diedrich D-120	
600.87	Very Stiff To Stiff Black Brown And Gray Gravelly Clay	[Graphic Log]	1	SS	2.1	14	S	18				
599.87												
598.87												
597.87	Medium Brown Sandy Loam	[Graphic Log]	2	SS	1.5	10	B	20				
596.87												
595.87												
594.87	Stiff Gray Sandy Clay Loam	[Graphic Log]	3	SS	---	15	---	18				
593.87												
592.87												
591.87	Stiff Gray Sandy Clay	[Graphic Log]	4	SS	---	17	---	22				
590.87												
589.87												
588.87	Stiff Gray Sandy Clay	[Graphic Log]	5	SS	1.3	10	B	19				
587.87												
586.87												
585.87	Stiff Gray Sandy Clay	[Graphic Log]	6	SS	1.4	10	B	19				
584.87												
583.87												
582.87	Stiff Gray Sandy Clay	[Graphic Log]	7	SS	1.5	12	B	21				
581.87												
581.87			8	SS	1.8	13	B	20				

Groundwater Data: Static water level after auger removal - elevation 581.0.
 Comments:

 Midwest Testing Services, Inc. 3705 Progress Blvd. Peru, IL 61354	BORING LOG		Phone: 815-223-6696
	Sheet <u>2</u> of <u>2</u>		Fax: 815-223-6659
			e-mail: midwest.07@insightbb.com

Client: Hutchison Engineering, Inc.
 Project Name: Sec 05-00626-00-BR
 Project Site: CH-33 Over Little Vermilion River
LaSalle County, Illinois

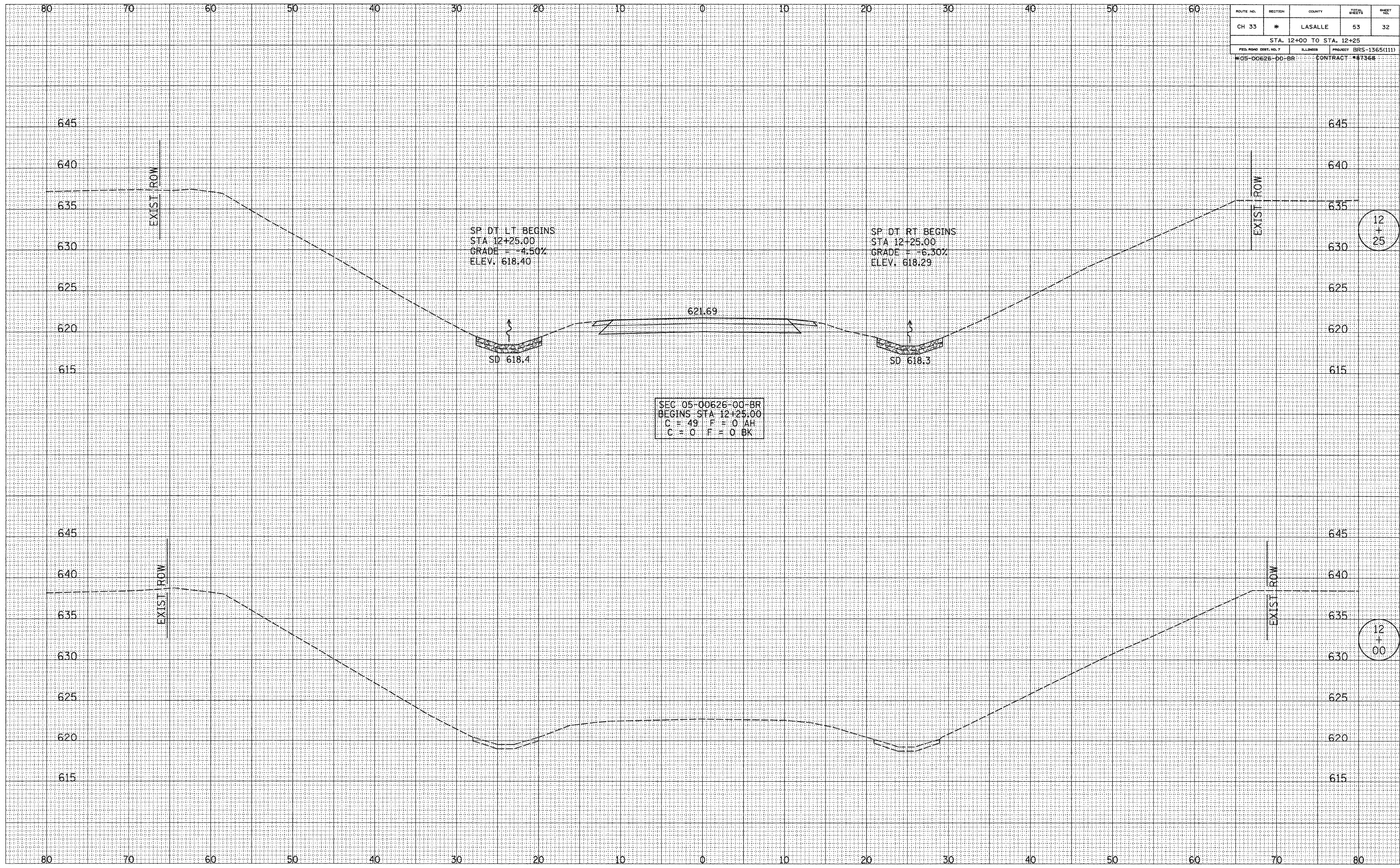
Boring No. B-2
 Surface Elev. 601.87
 Auger Depth 35' Rotary Depth NA
 Start Date 05/03/08 Finish Date 05/03/08

Location: 7' Lt of Centerline CH-33
Station 20+70

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES						DRILLED BY		REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)		
580.87											Randy Saffanski Diedrich D-120	
579.87	Stiff Gray Sandy Clay	[Graphic Log]	22									
578.87	Very Dense Yellowish Brown Weathered Sandstone	[Graphic Log]	9	SS	---	100	---	---				
577.87												
576.87	Refusal With Hollow Stem Auger	[Graphic Log]	25									
575.87												
574.87												
573.87	Very Dense Pale Yellow And White Sandstone	[Graphic Log]	27									
572.87												
571.87												
570.87	Penetrated With Rock Bit	[Graphic Log]	31									
569.87												
568.87												
567.87	Bottom of Boring	[Graphic Log]	33									
566.87												
565.87												
564.87												
563.87												
562.87												
561.87												
560.87												

Groundwater Data: Static water level after auger removal - elevation 581.0.
 Comments:

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	32
STA. 12+00 TO STA. 12+25				
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-1365(111)	
K05-00625-00-BR		CONTRACT #81368		



SP. DT. LT. BEGINS
 STA 12+25.00
 GRADE = -4.50%
 ELEV. 618.40

SP. DT. RT. BEGINS
 STA 12+25.00
 GRADE = -6.30%
 ELEV. 618.29

SEC. 05-00626-00-BR
 BEGINS STA. 12+25.00
 C = 49 F = 0 AH
 C = 0 F = 0 BK

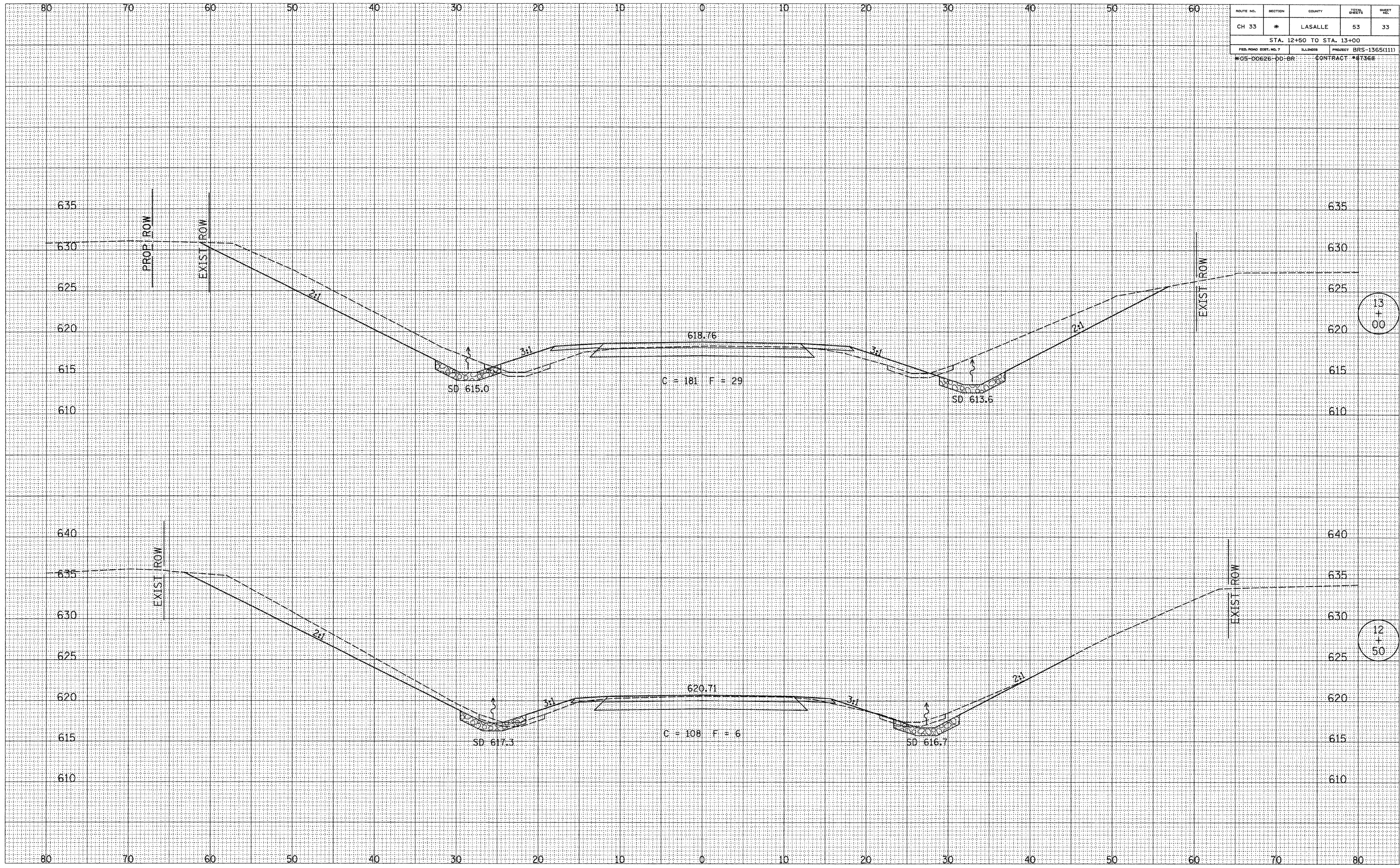
12
+
25

12
+
00

DATE	BY
FINISH SURVEY	
SURVEY PLOTTED	
TEMPLATE	
AREAS CHECKED	
NO.	

DATE	BY
ORIGINAL SURVEY	
SURVEY PLOTTED	
TEMPLATE	
AREAS CHECKED	
NO.	

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	33
STA. 12+50 TO STA. 13+00				
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-1365(111)	
*05-00626-00-BR		CONTRACT #87368		



13
+
00

12
+
50

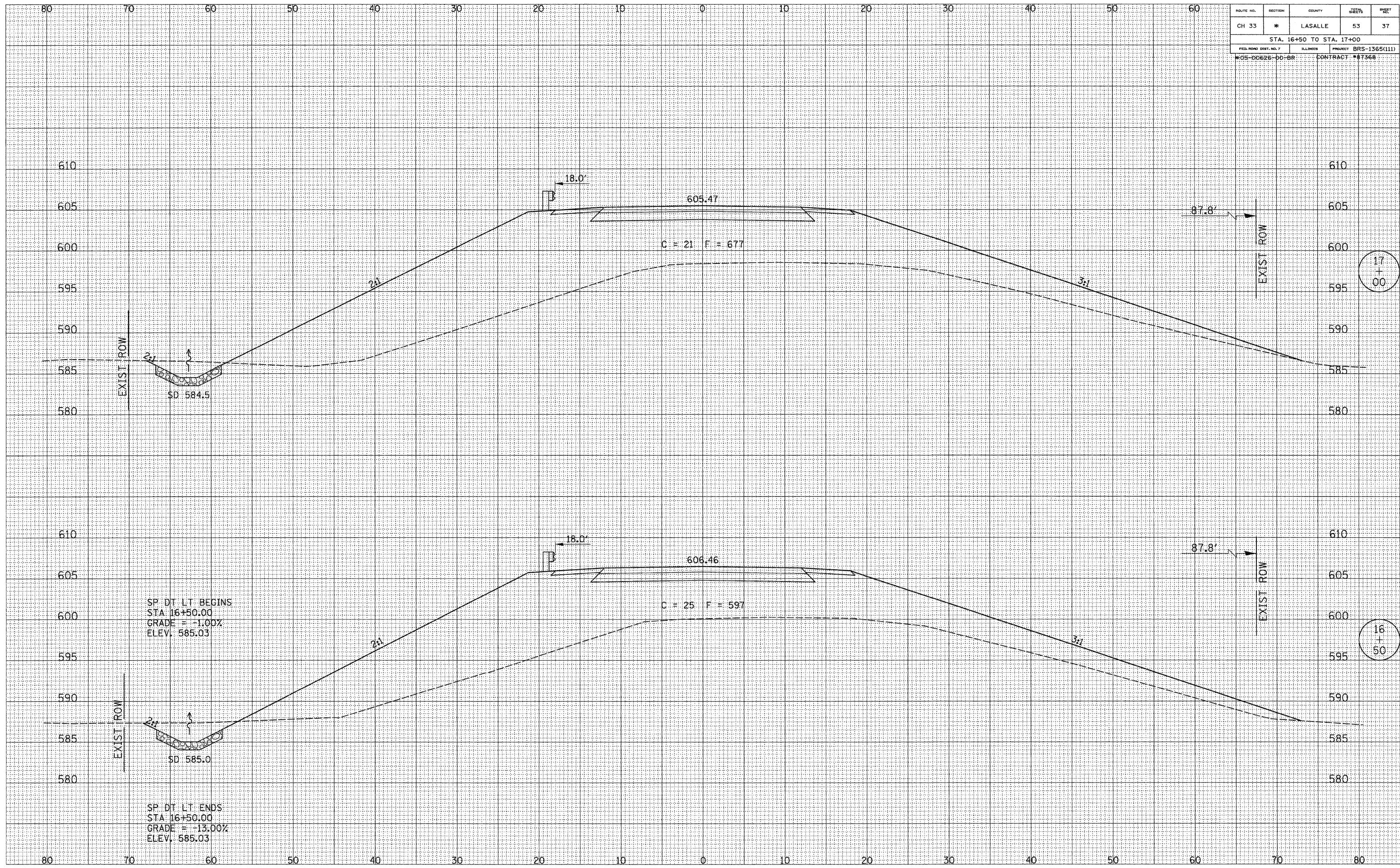
DATE	BY
FINISHED	
SURVEY	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
NO.	

DATE	BY
ORIGINAL	
SURVEY	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
NO.	

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	37
STA. 16+50 TO STA. 17+00				
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-1365(111)	
W05-0062B-00-BR		CONTRACT	#7368	

DATE	BY
DESIGNED	BY
PLOTTED	BY
TEMPLATE	BY
AREAS	BY
CHECKED	BY
NO.	

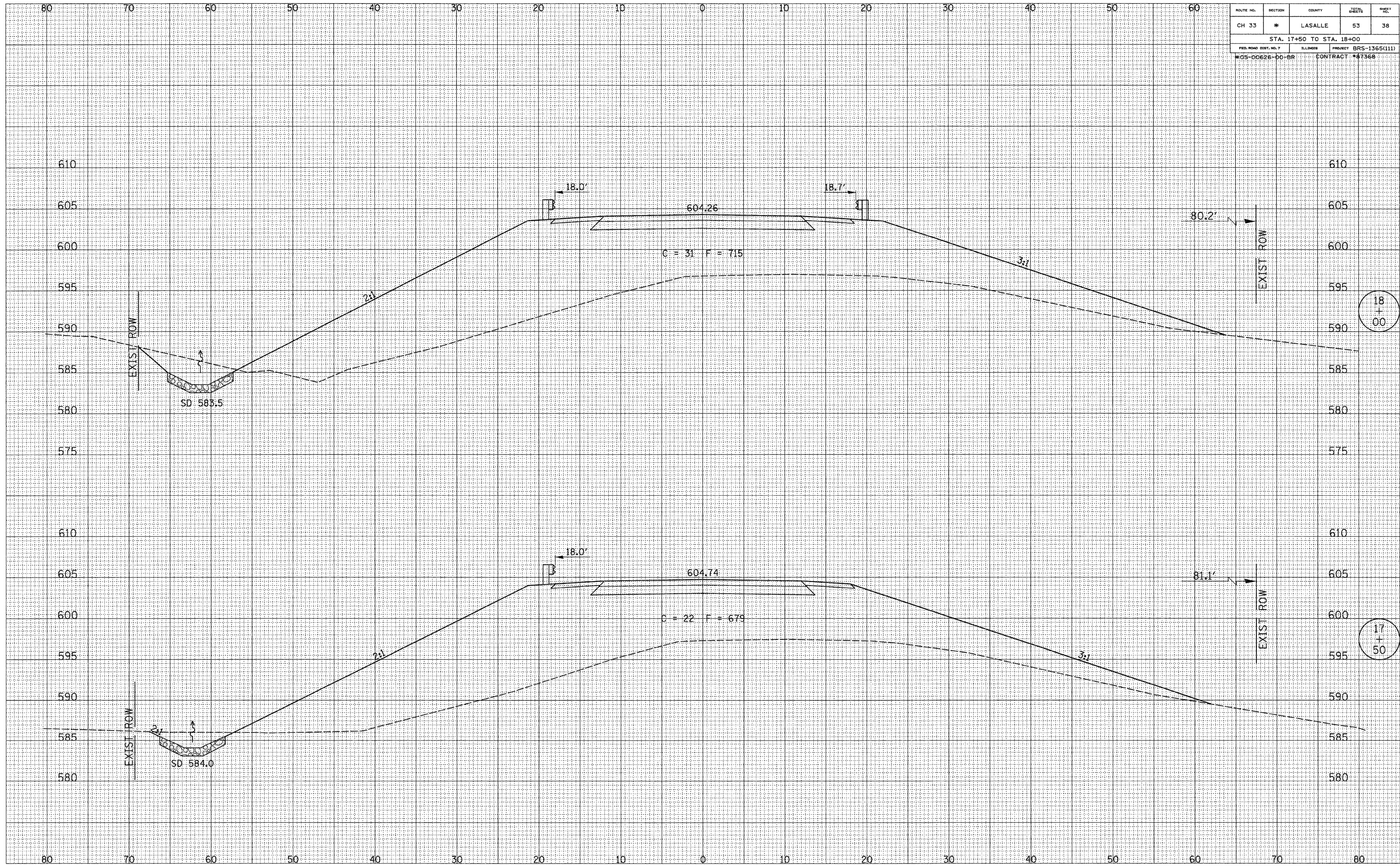
DATE	BY
DESIGNED	BY
PLOTTED	BY
TEMPLATE	BY
AREAS	BY
CHECKED	BY
NO.	



17
+
00

16
+
50

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	38
STA. 17+50 TO STA. 18+00				
FED. ROAD DIST. NO. 7	ELLINGSB	PROJECT	BRS-1365(111)	
W05-00626-DD-BR		CONTRACT: *47368		



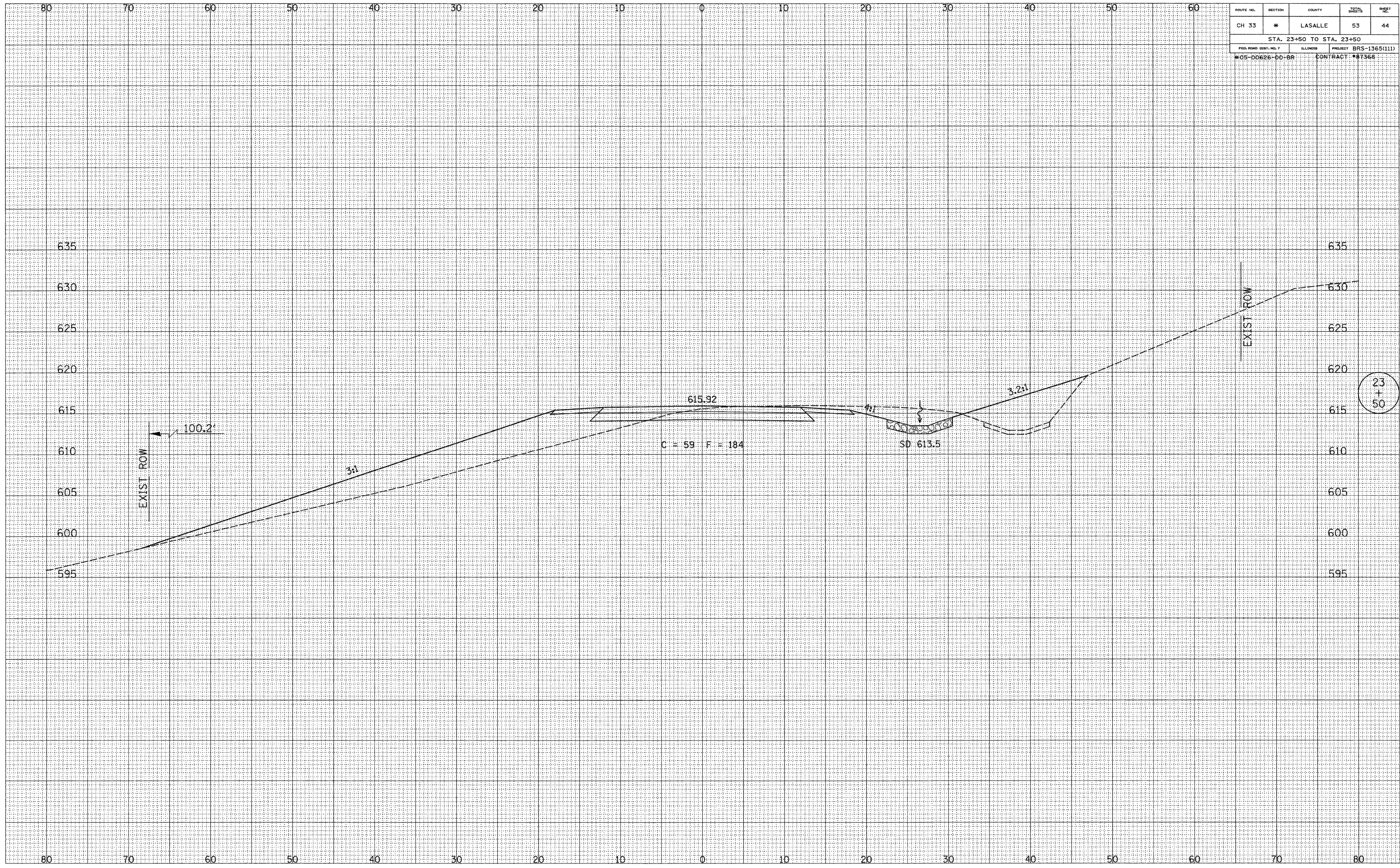
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FINAL SURVEY	CHECKED
NOTE BOOK	PLOTTED
NO.	AREAS CHECKED

DATE	BY
ORIGINAL SURVEY	CHECKED
NOTE BOOK	PLOTTED
NO.	AREAS CHECKED

18
+
00

17
+
50

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	44
STA. 23+50 TO STA. 23+50				
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-1365(111)	
W05-00626-DD-BR		CONTRACT	#1368	



DATE	BY
SURVEYED	PLOTTED
NOTE BOOK	AREAS CHECKED
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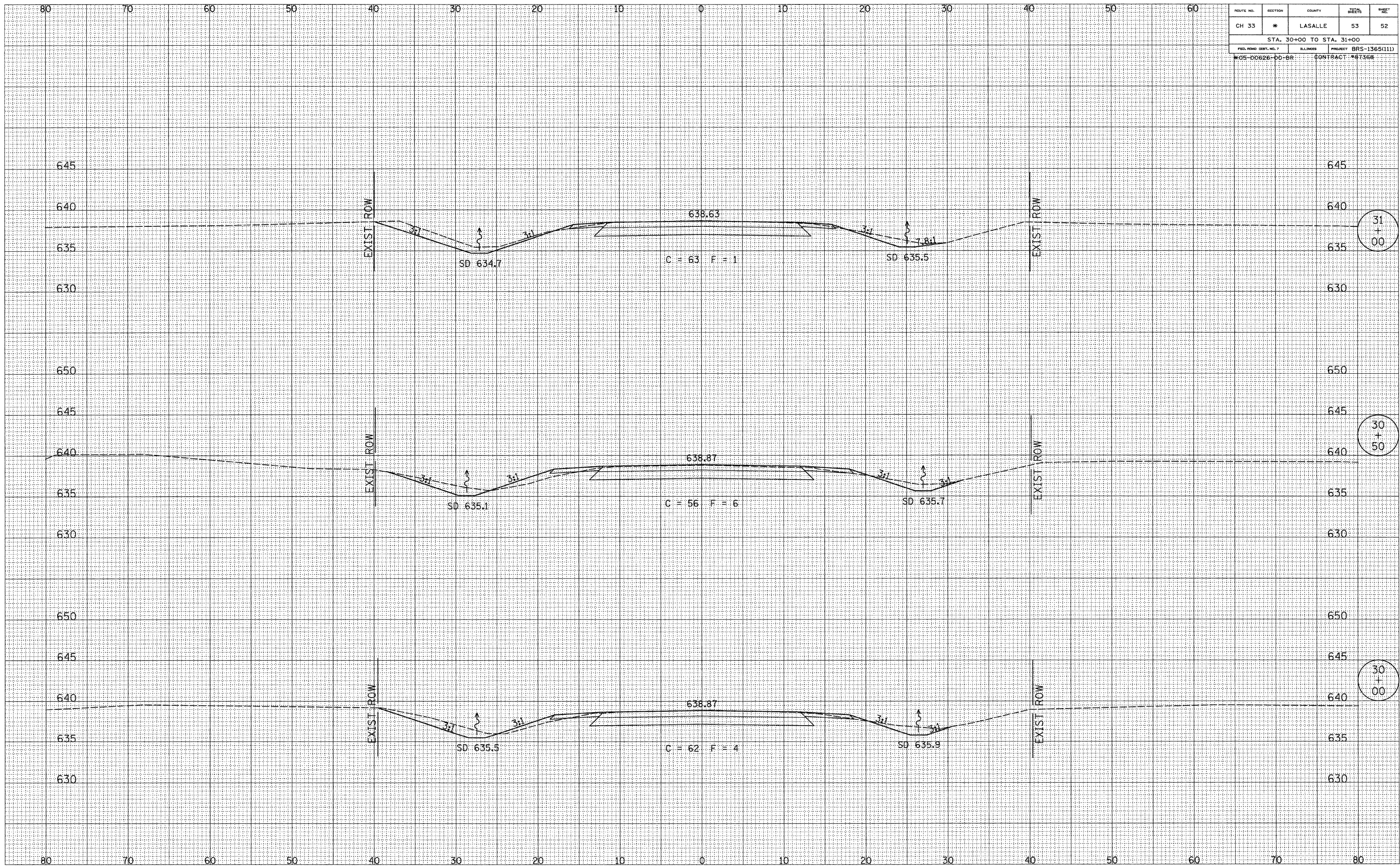
DATE	BY
ORIGINAL SURVEY	PLOTTED
NOTE BOOK	AREAS CHECKED
NO.	

23
+
50

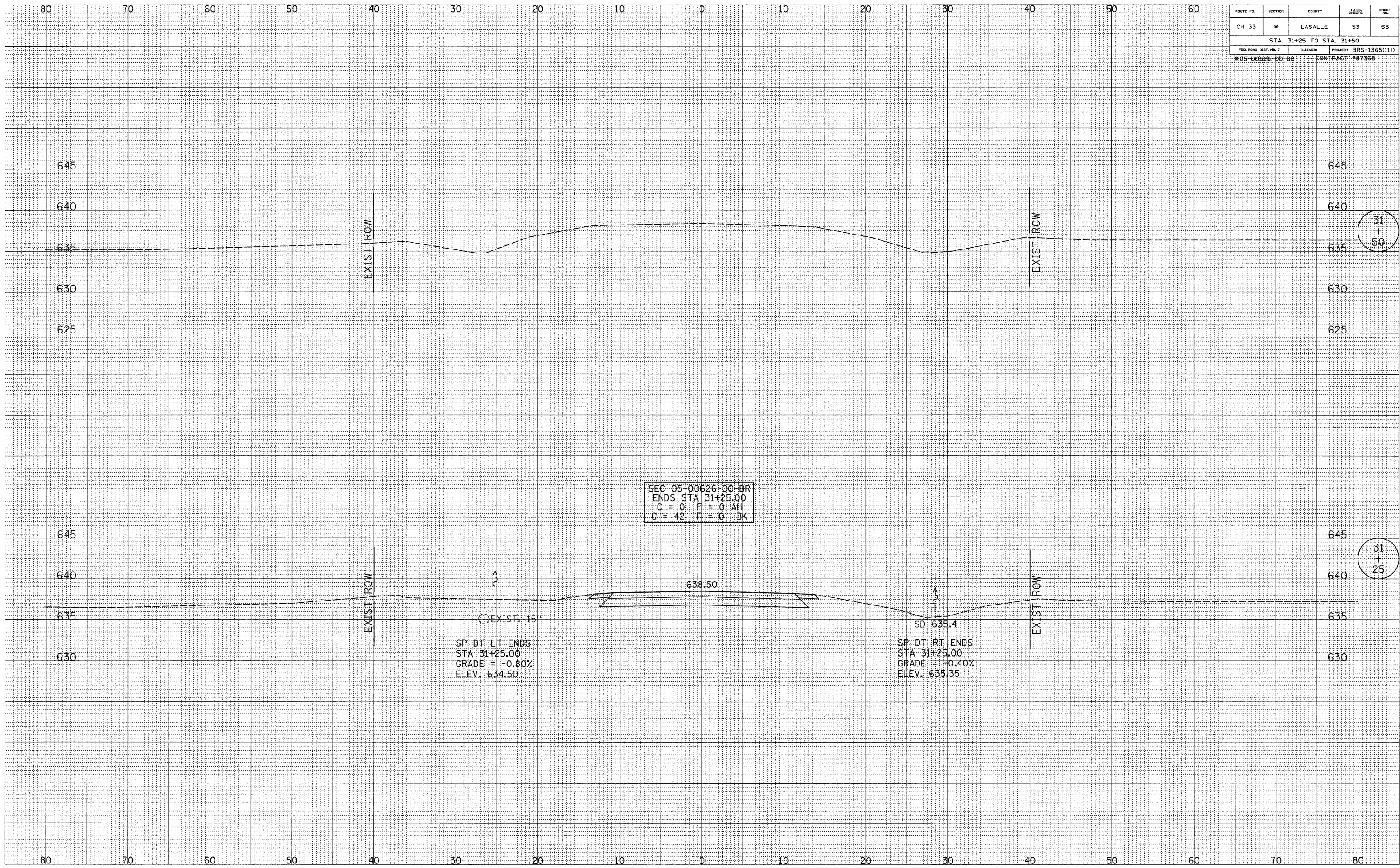
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	52
STA. 30+00 TO STA. 31+00				
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-1365(111)	
W05-00626-00-6B		CONTRACT	#47368	

BY	DATE
FINAL SURVEY	
SURVEYED	
PLOTTED	
TEMPLATE	
NOTE BOOK	
NO.	
AREAS CHECKED	

BY	DATE
ORIGINAL SURVEY	
SURVEYED	
PLOTTED	
TEMPLATE	
NOTE BOOK	
NO.	
AREAS CHECKED	



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 33	*	LASALLE	53	53
STA. 31+25 TO STA. 31+50				
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-1365(111)	
05-00626-00-BR		CONTRACT	#1368	



BY	DATE
FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED

BY	DATE
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED