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 - 630301-06 - SHOULDER WIDENING FOR TY 1 (SPL) TERMINALS
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 - 701901-04 - TRAFFIC
 - BLR 21-9 - TRAFFIC

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
PLANS FOR PROPOSED
SURFACE TRANSPORTATION PROGRAM-BRIDGE
F.A.S. 797 (COUNTY HIGHWAY 1)
WAYNE COUNTY
SECTION 13-00126-00-BR
STRUCTURE NO. 096-3463
PROJECT NO. BRS-0797(124)
JOB NO. C-97-051-14

SCALES

- PLAN 1 INCH = 50 FEET
- PROFILE HORZ. 1 INCH = 50 FEET
- PROFILE VERT. 1 INCH = 10 FEET

SUMMARY OF QUANTITIES

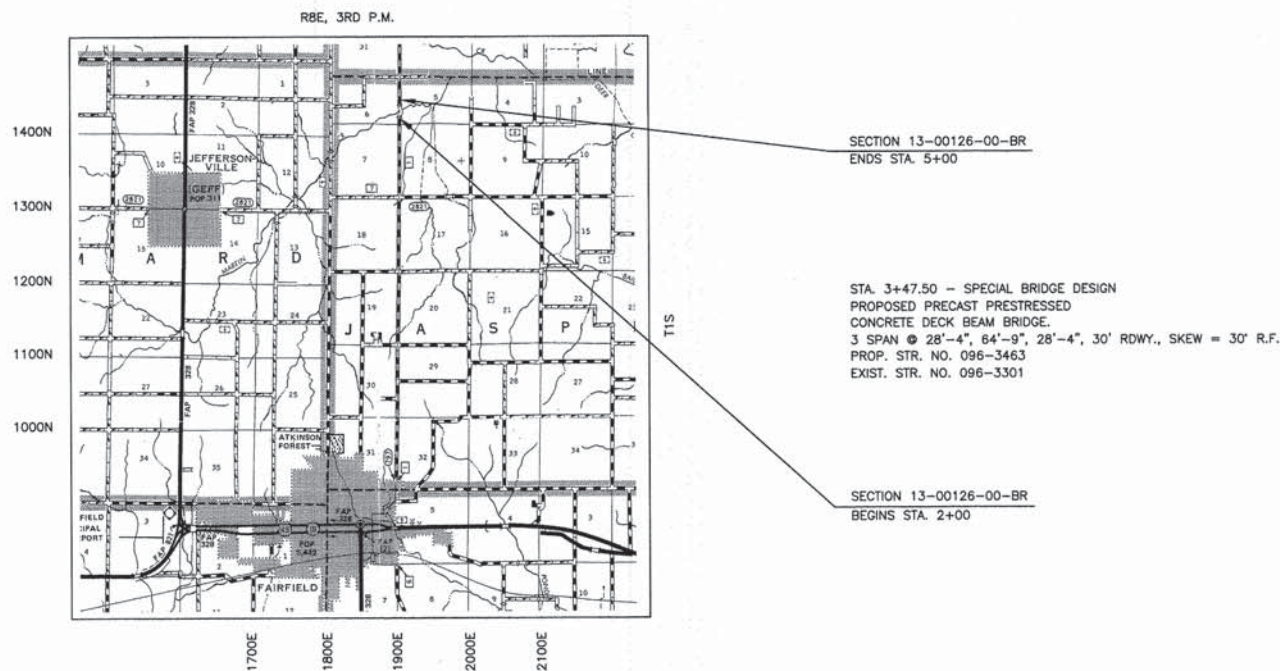
QUANTITY	UNIT	ITEM	CODE NO.
0.1	ACRE	SEEDING, CLASS 2 (SPECIAL)	X2501000
1	L SUM	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)	X7010216
302	CU YD	CHANNEL EXCAVATION	20300100
60	FOOT	PERIMETER EROSION BARRIER	28000400
284	TON	STONE DUMPED RIPRAP, CLASS A4	28100807
74	SQ YD	PORTLAND CEMENT CONCRETE BASE COURSE 8"	35300300
380	POUND	BITUMINOUS MATERIALS (PRIME COAT)	40600275
244	SQ YD	HOT-MIX ASPHALT SURFACE REMOVAL, BUTT JOINT	40600982
144	TON	HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N70	40603315
40	TON	AGGREGATE SHOULDERS, TYPE B	48101200
1	EACH	REMOVAL OF EXISTING STRUCTURES	50100100
70.6	CU YD	CONCRETE STRUCTURES	50300225
34.6	CU YD	CONCRETE ENCASEMENT	50300280
1,700	SQ FT	PRECAST PRESTRESSED CONCRETE DECK BEAMS (17" DEPTH)	50400305
1,943	SQ FT	PRECAST PRESTRESSED CONCRETE DECK BEAMS (27" DEPTH)	50400505
8,120	POUND	REINFORCEMENT BARS	50800105
247	FOOT	STEEL RAILING, TYPE SM	50901050 Δ
715	FOOT	FURNISHING STEEL PILES HP 12X53	51201600
869	FOOT	FURNISHING STEEL PILES HP 14X73	51201800
1584	FOOT	DRIVING PILES	51202305
1	EACH	TEST PILE STEEL HP 12X53	51203600
1	EACH	TEST PILE STEEL HP 14X73	51203800
22	EACH	PILE SHOES	51204650
1	EACH	NAME PLATES	51500100
1096	FOOT	PORTLAND CEMENT MORTAR FAIRING COURSE	58300100
54.7	CU YD	CONTROLLED LOW-STRENGTH MATERIAL	59300100
4	EACH	TRAFFIC BARRIER TERMINAL, TYPE 6A	63100087 Δ
4	EACH	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	63100167 Δ
1	L SUM	MOBILIZATION	67100100
4	EACH	TERMINAL MARKER - DIRECT APPLIED	78201000 Δ

Δ SPECIALTY ITEMS

FUNCTIONAL CLASS: MAJOR COLLECTOR
ADT = 1250
DESIGN SPEED = 50 MPH

LOCATION MAP

APPROXIMATE SCALE: 1 INCH = 1 MILE
NET LENGTH = 300.00 L.F. = 0.057 MILES



TOLL FREE JOINT UTILITY LOCATING
INFORMATION FOR EXCAVATORS (J.U.L.I.E.)
TELEPHONE NO. 1-800-892-0123 OR 811

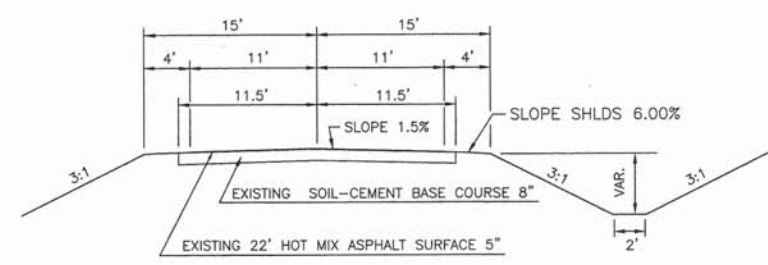


John A. Stone
02/04/2015
ILLINOIS REGISTERED PROFESSIONAL ENGINEER #55012
LICENSE EXPIRES NOVEMBER 30, 2015
PROFESSIONAL DESIGN FIRM #184-000832

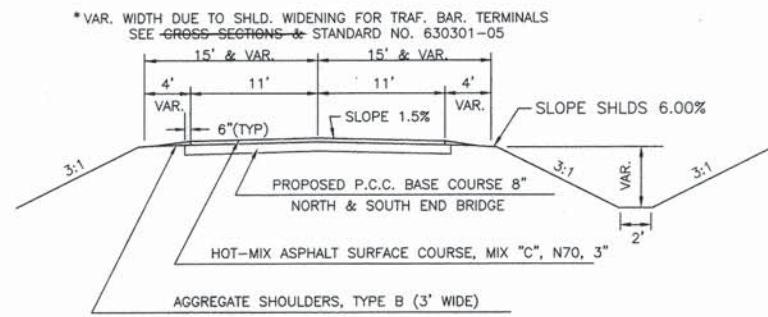
ILLINOIS DEPARTMENT OF TRANSPORTATION	
APPROVED	<i>Feb 5</i> , 2015 <i>Don Seibel</i> WAYNE COUNTY ENGINEER
PASSED	<i>2-11</i> , 2015 <i>Maureen Costello</i> DISTRICT SEVEN ENGINEER OF LOCAL ROADS & STREETS
RELEASING FOR BID BASED ON LIMITED REVIEW	
	<i>2-11</i> , 2015 <i>Roger L. Anshel</i> DEPUTY DIRECTOR OF HIGHWAYS REGION FOUR ENGINEER

SECTION	13-00126-00-BR	TOTAL SHEETS	11	SHEET NO.	2
COUNTY	WAYNE				
ROUTE	FAS 797 (C.H. 1)				
STA.	0+00	TO STA.	7+01.19		

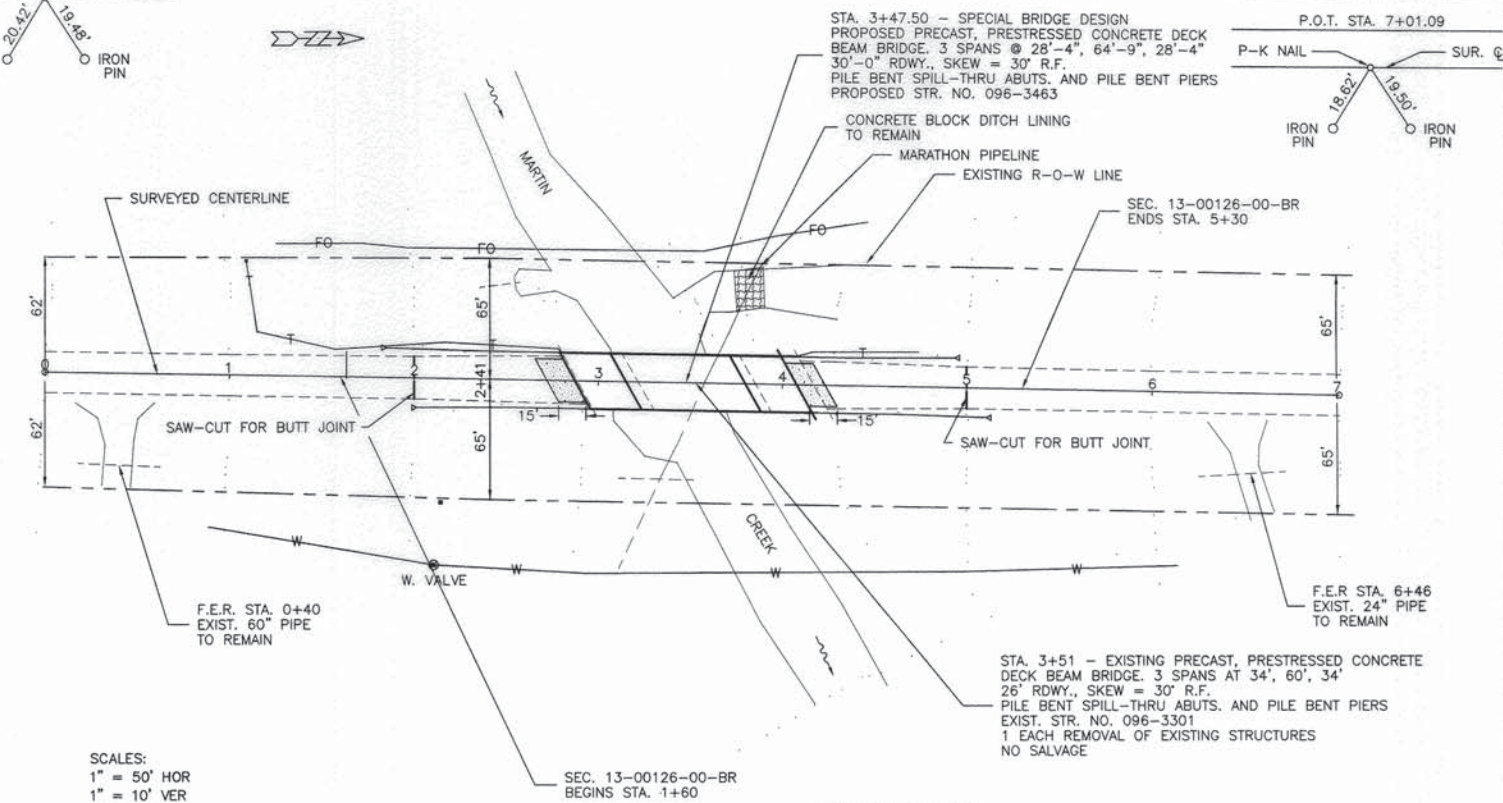
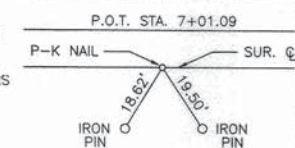
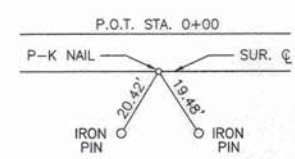
MELVIN VAUGHN



TYPICAL CROSS SECTION OF EXISTING ROADWAY



TYPICAL CROSS SECTION OF PROPOSED ROADWAY



SCALES:
1" = 50' HOR
1" = 10' VER

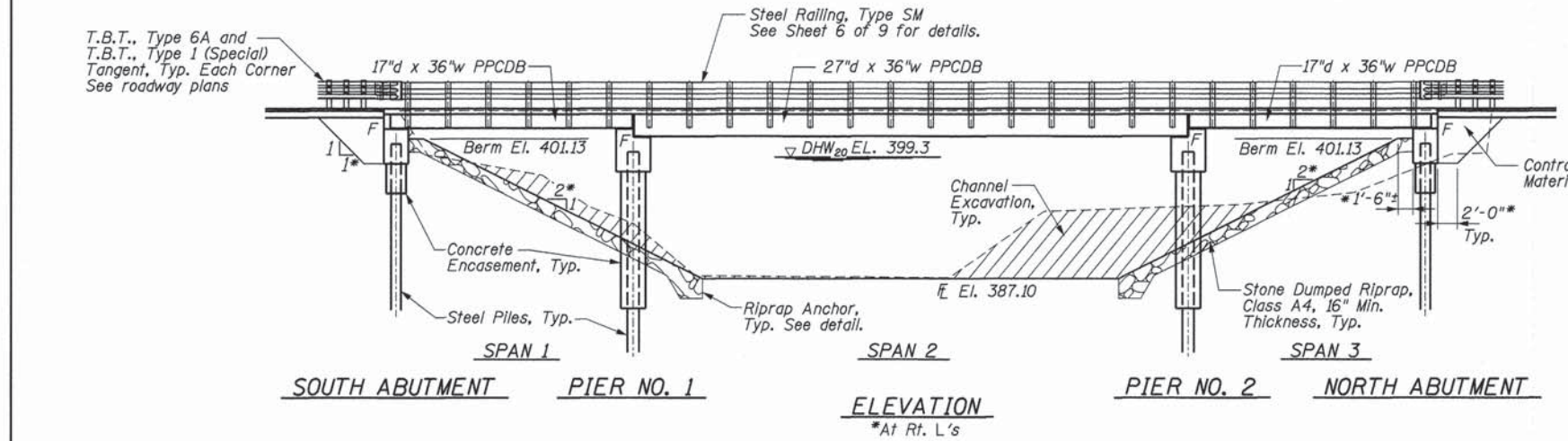
CLYDE ROBBINS

<p>PCC BASE COURSE 8"</p> <p>SOUTH END BRIDGE = 37 SQ YD NORTH END BRIDGE = 37 SQ YD TOTAL = 74 SQ YD (SEE SPECIAL PROVISIONS)</p>	<p>HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N70</p> <p>SOUTH END BRIDGE = 37 TON NORTH END BRIDGE = 37 TON OVER BRIDGE = 70 TON TOTAL = 144 TON</p>	<p>BITUMINOUS MATERIALS (PRIME COAT)</p> <p>SOUTH END BRIDGE = 190 POUND NORTH END BRIDGE = 190 POUND TOTAL = 380 POUND</p>	<p>B.M. #1 - ELEV. 400.41 P-K NAIL IN TELE. PED. POST 61.5' LT. STA. 1+09</p>	<p>TRAFFIC BARRIER TERMINALS, TYPE 6A EACH CORNER OF BRIDGE = 4 EACH</p> <p>TRAFFIC BARRIER TERMINALS, TYPE 1 (SPECIAL) TANGENT EACH CORNER OF BRIDGE = 4 EACH</p> <p>TERMINAL MARKER - DIRECT APPLIED EACH CORNER OF BRIDGE = 4 EACH</p>	<p>PERIMETER EROSION BARRIER 15 FT. Ø EACH CORNER OF BRIDGE = 60 EACH</p>	<p>EARTHWORK QUANTITIES</p> <p>CHANNEL EXCAVATION = 302 CU YD CHANNEL EXC. (ADJ. 25%) = 227 CU YD EMBANKMENT (SHLD WIDENING) = 53 CU YD EXCESS EXCAVATION = 174 CU YD</p>	<p>UTILITIES</p> <p>PIPELINE: MARATHON PIPELINE LLC PATOKA, IL 618-432-7223</p> <p>WATER: JASPER WATERWORKS FAIRFIELD, IL 618-842-2918</p> <p>TELEPHONE: FRONTIER COMMUNICATIONS 225 E. CHESTNUT ST. OLNEY, IL 618-395-6189</p>	<p>When the plans or special provisions include information pertaining to the location of underground utility facilities, such information represents only the opinion of the Wayne County Highway Department as to the location of such utilities and is only included for the convenience of the bidder.</p>	<p>EXISTING CENTERLINE GRADE</p>	<p>PROPOSED CENTERLINE GRADE (TOP 3" HMA OVERLAY)</p>	<p>20 YR. HWE 399.3</p>	<p>100 YR. HWE 399.9</p>	<p>410</p>		
									<p>410</p>	<p>400</p>	<p>400</p>	<p>400</p>	<p>400</p>	<p>400</p>	
<p>HOT-MIX ASPHALT SURFACE REMOVAL 3" TO 0" IN 50 FEET</p>	<p>EXISTING PAVEMENT</p>	<p>PROPOSED HOT MIX ASPHALT SURFACE COURSE, MIX "C", N70, 3"</p>	<p>PROPOSED HMA SURFACE COURSE, 3"</p>	<p>PROPOSED HMA SURFACE REMOVAL - BUTT JOINT = 244 SQ YD</p>	<p>CHANNEL EXCAVATION</p>	<p>F.L. 387.0</p>	<p>390</p>	<p>390</p>	<p>390</p>	<p>390</p>	<p>390</p>	<p>390</p>			
							<p>390</p>	<p>390</p>	<p>390</p>	<p>390</p>	<p>390</p>	<p>390</p>			
								<p>0+00</p>	<p>1+00</p>	<p>2+00</p>	<p>3+00</p>	<p>4+00</p>	<p>5+00</p>	<p>6+00</p>	<p>7+00</p>

DETAIL OF BUTT-JOINT STA. 2+00 AND STA. 5+00

B.M. #1 - Elev. 400.41
Mag nail in tel. ped.
61.5' Lt., Sta. 1+09

Existing Structure: Sta. 3+51 - Existing precast prestressed concrete deck beam bridge, 3 Spans at 34', 60', 34' 26' Rdwy., Skew 30° R.F. Pile bent spill-thru abutments and pile bent piers. To be removed. Exist. Str. No. 096-3301. No Salvage.



LOADING HL-93

50#/sq. ft. included in dead load for future wearing surface.

DESIGN SPECIFICATIONS

2010 (5th Ed.) AASHTO LRFD Bridge Design Specifications

DESIGN STRESSES

FIELD UNITS

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (reinforcement)

PRECAST PRESTRESSED UNITS

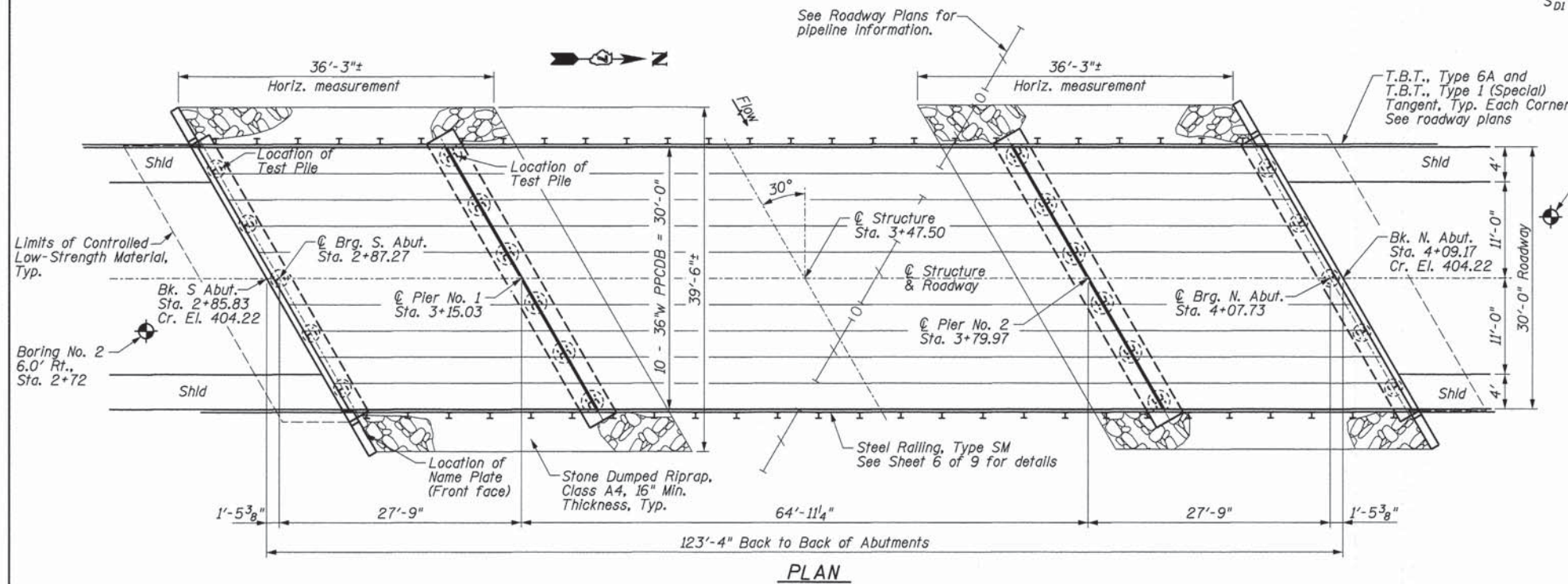
$f'_c = 6,000$ psi
 $f'_{ci} = 5,000$ psi
 $f_{pu} = 270,000$ psi ($\frac{1}{2}$ " ϕ low lax. strands)
 $f_{pbt} = 201,960$ psi ($\frac{1}{2}$ " ϕ low lax. strands)
 $f_y = 60,000$ psi (reinforcement)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 2
Soil Site Classification = C
 $S_{D1} = 0.198$ $S_{D5} = 0.554$

BILL OF MATERIALS (BRIDGE ONLY)

ITEM	UNIT	TOTAL
Channel Excavation	Cu Yd	302
Stone Dumped Riprap, Class A4	Ton	284
Bituminous Materials (Prime Coat)	Pound	93
Hot-Mix Asphalt Surface Course, Mixture "C", N70	Ton	70
Removal of Existing Structures	Each	1
Concrete Structures	Cu Yd	70.6
Concrete Encasement	Cu Yd	34.6
PPCDB (17" Depth)	Sq Ft	1700
PPCDB (27" Depth)	Sq Ft	1943
Reinforcement Bars	Pound	8120
Steel Railing, Type SM	Foot	247
Furnishing Steel Piles HP12x53	Foot	715
Furnishing Steel Piles HP14x73	Foot	869
Driving Piles	Foot	1584
Test Pile Steel HP12x53	Each	1
Test Pile Steel HP14x73	Each	1
Pile Shoes	Each	22
Name Plates	Each	1
Portland Cement Mortar Fairing Course	Foot	1096
Controlled Low-Strength Material	Cu Yd	54.7



DESIGN SCOUR TABLE

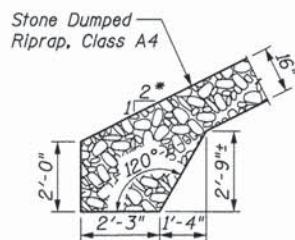
Location	Design Scour Elevation
S. Abut.	398.6
Pier 1	383.6
Pier 2	383.6
N. Abut.	398.6

WATERWAY INFORMATION

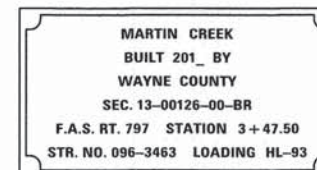
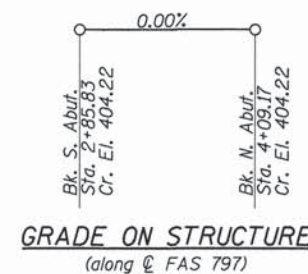
Drainage Area = 22.2 sq. mi. Low Grade Elev. 402.8 @ Sta. 7+00

Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft. Exist.	Prop.	Nat. H.W.E.	Head - Ft. Exist.	Prop.	Headwater El. Exist.	Prop.
Design	20	3117	642	752	399.3	0.7	0.4	400.0	399.7
Base	100	4680	686	790	399.9	1.3	0.8	401.2	400.7
Base	500	6240	837	813	400.4	1.9	1.4	402.3	401.8

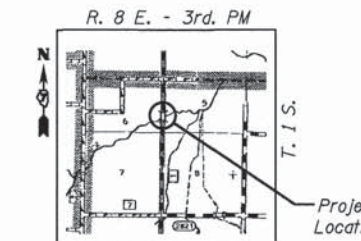
Waterway Information as calculated by Connor & Connor, Inc.



RIPRAP ANCHOR DETAIL



NAME PLATE
See Std. 515001

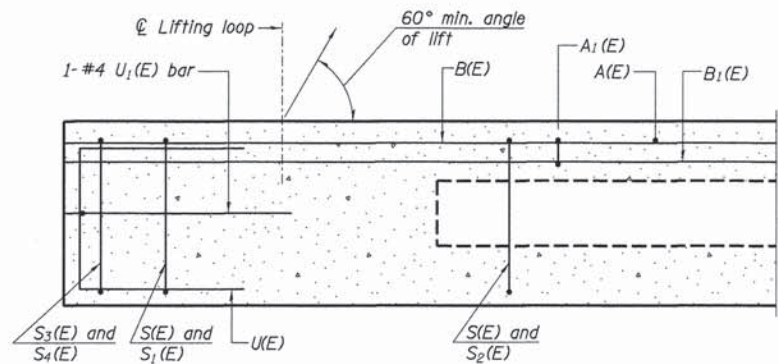


LOCATION SKETCH

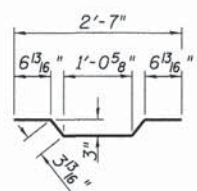
I certify that to the best of knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AASHTO Standard Specifications for Highway Bridges.



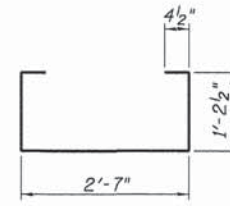
William D. Lueking
William D. Lueking
02/04/2015
Date of Signing
11/30/2016
Date of License Expiration



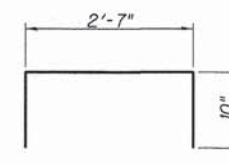
SECTION A-A



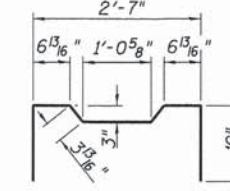
BAR A1(E)



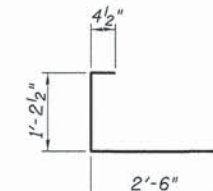
BAR S(E)



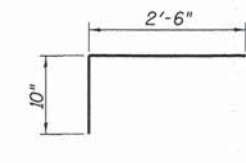
BAR S1(E)



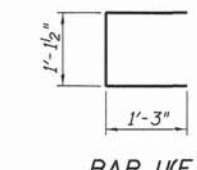
BAR S2(E)



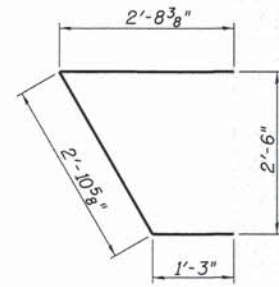
BAR S3(E)



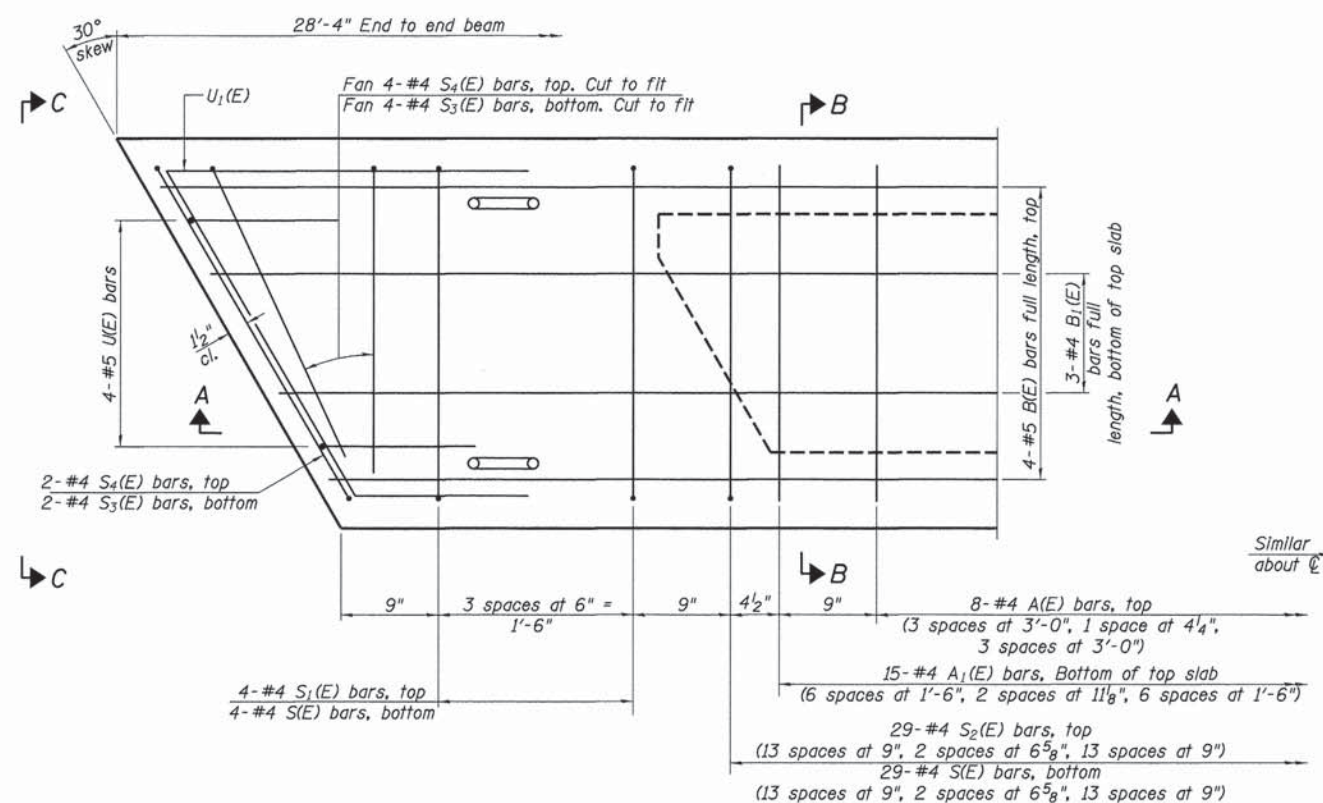
BAR S4(E)



BAR U(E)



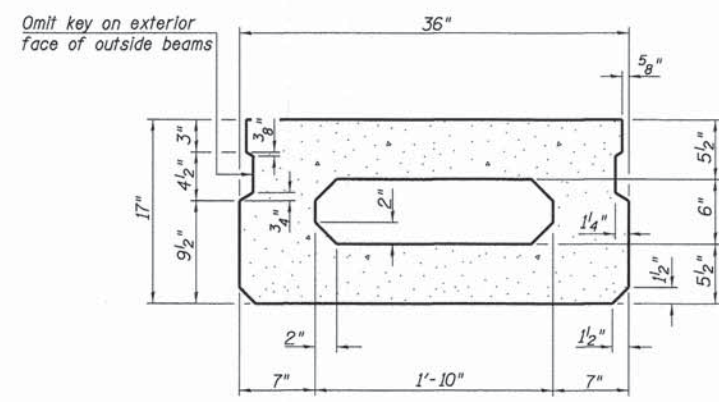
BAR U1(E)



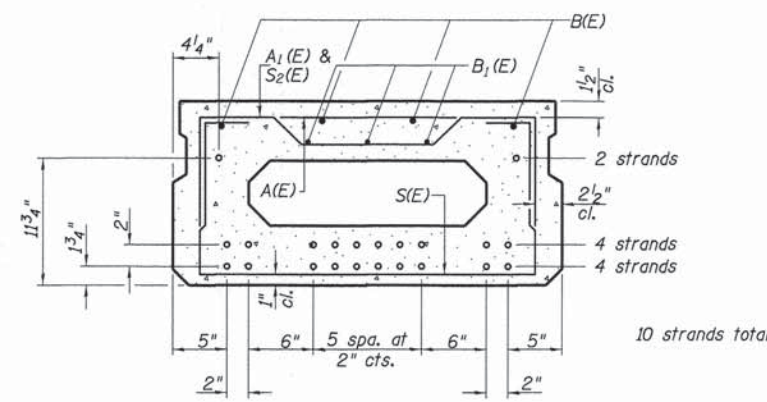
PLAN VIEW

Note: Spacing of S(E) and S(E) bars may be adjusted up to 4" in the immediate area of the transverse tie diaphragms to miss the block outs for the transverse ties.

MINIMUM BAR LAP
 #4 bar = 2'-0"
 #5 bar = 2'-6"

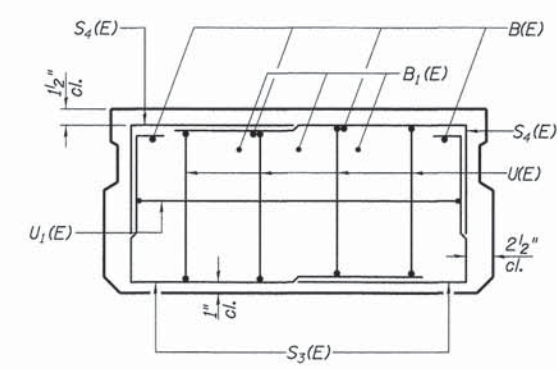


SECTION B-B (Showing dimensions)



SECTION B-B (Showing reinforcement and permissible strand locations)

Note: Place the number of strands specified in each row symmetrically about the centerline of beam in the permissible strand locations shown.



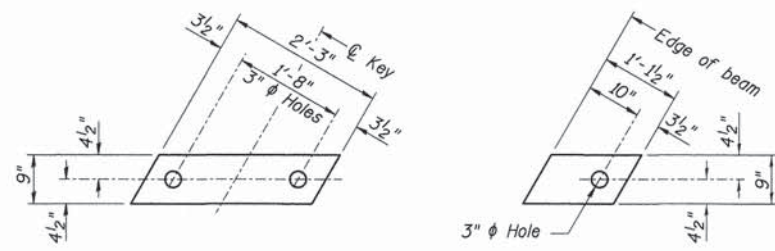
VIEW C-C

BAR LIST
ONE BEAM ONLY
 (For information only)

Bar	No.	Size	Length	Shape
A(E)	8	#4	2'-7"	—
A1(E)	15	#4	2'-10"	—
B(E)	4	#5	28'-0"	—
B1(E)	3	#4	28'-0"	—
S(E)	37	#4	5'-9"	U
S1(E)	8	#4	4'-3"	U
S2(E)	29	#4	4'-6"	U
S3(E)	12	#4	4'-1"	U
S4(E)	12	#4	3'-4"	U
U(E)	8	#5	3'-8"	U
U1(E)	2	#4	6'-10"	U

Note: See Sheet 3 of 9 for additional details and Bill of Material.

SPAN 1 OR 3

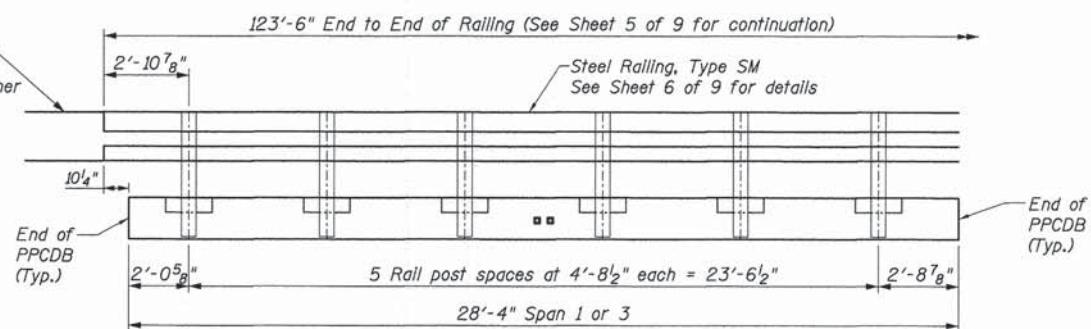


FABRIC BEARING PAD
(Interior)

FABRIC BEARING PAD
(Exterior)

All bearing pads shall be 1" thick.

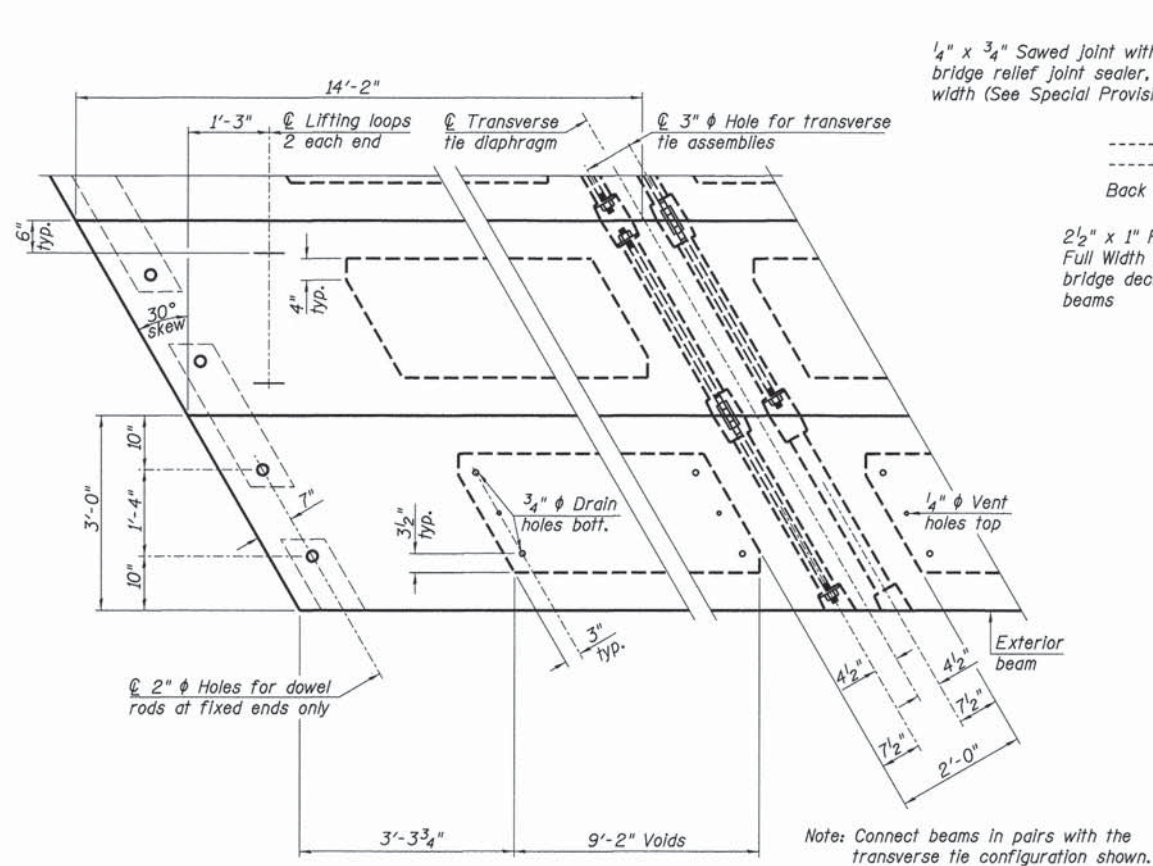
T.B.T., Type 6A and
T.B.T., Type 1 (Special)
Tangent, Typ., Each Corner
See roadway plans



ABUTMENT

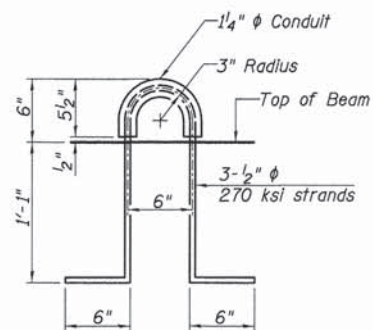
ELEVATION

PIER

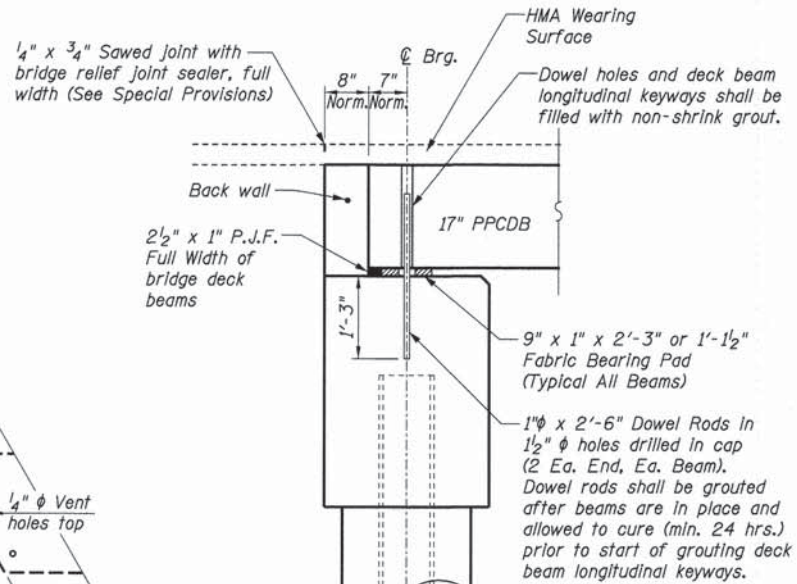


PLAN VIEW

Note: Connect beams in pairs with the transverse tie configuration shown.

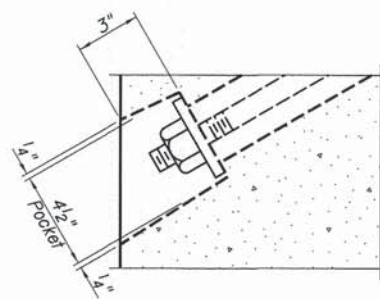


LIFTING LOOP DETAIL

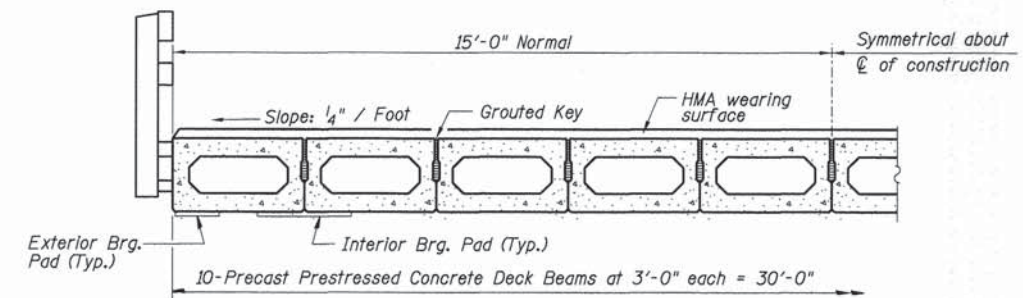


FIXED BEARING ABUTMENT

(Normal to \mathcal{E})
See Sheet 5 of 9 for Pier Bearing Detail.

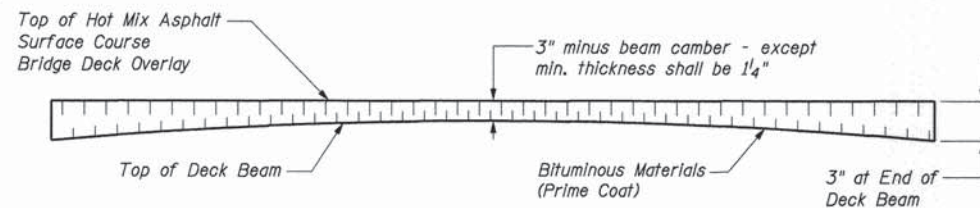


SECTION A-A

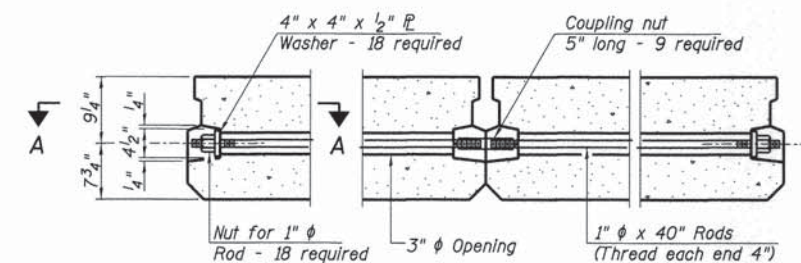


TYP. HALF CROSS SECTION

See Sheet 6 of 9 for the details showing the mounting of posts and rails to the PPCDB.



PROFILE OF OVERLAY



TYPICAL TRANSVERSE TIE ASSEMBLY

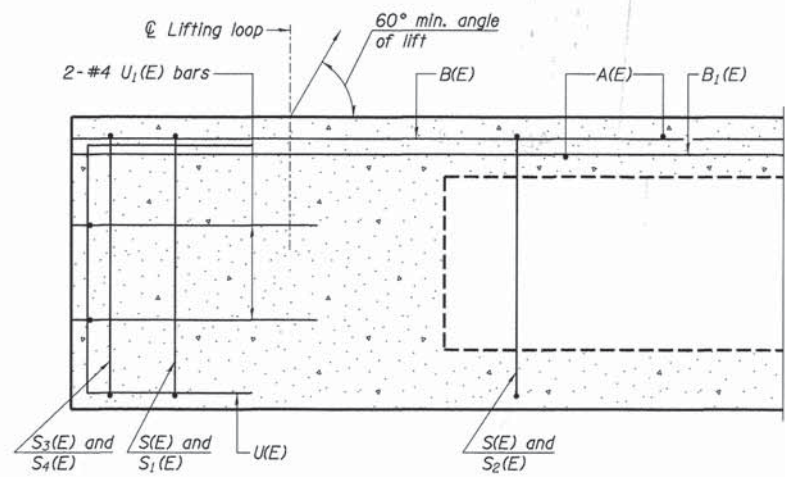
NOTES

- Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be 1/2" and the nominal cross-sectional area shall be 0.153 sq. in.
- The 1" ϕ rods in the transverse tie assembly shall be tightened to a snug fit and the threads set. Pockets on exterior faces of bridge shall be filled with grout after transverse tie assembly is in place.
- Reinforcement bars shall conform to ASTM A 706, Grade 60 (IL Modified).
- Two 1/8" fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location.
- A minimum 2 1/2" ϕ lifting pin shall be used to engage the lifting loops during handling.
- Corrosion Inhibitor, per Article 1020.05(b)(12) and 1021.06 of the Standard Specifications, shall be used in the concrete for precast prestressed concrete deck beams.
- Compressive strength of prestressed concrete, $f'c$, shall be 6000 psi.
- Compressive strength of prestressed concrete at release, $f'ci$, shall be 5000 psi.

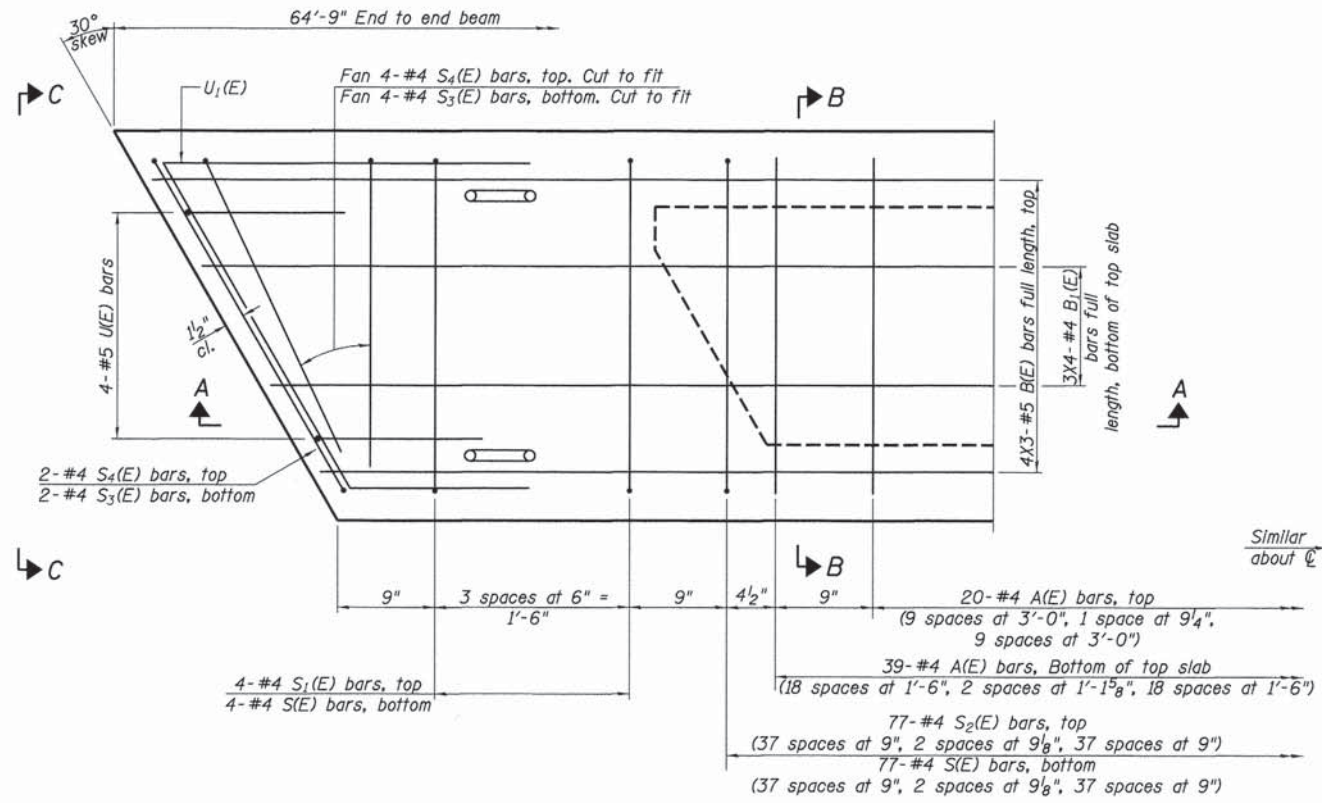
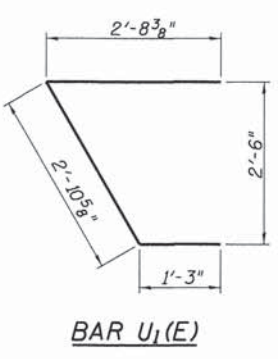
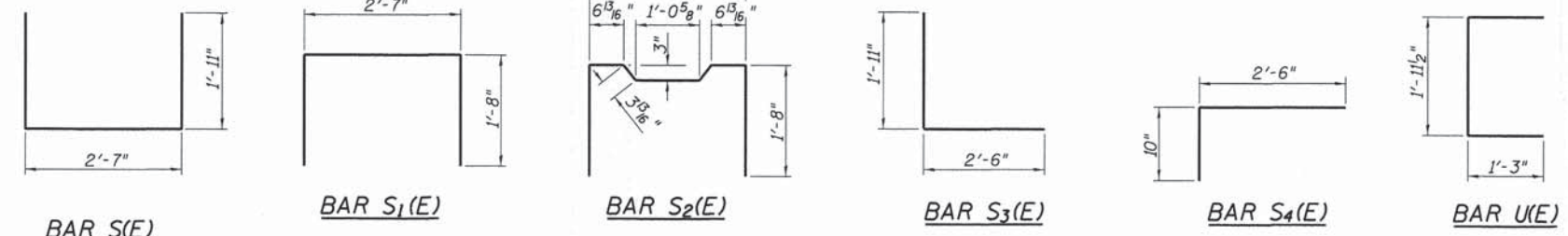
BILL OF MATERIAL - ONE SPAN

Precast Prestressed Conc. Deck Bms. (17" depth)	Sq. Ft.	850
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SPAN 1 OR 3



SECTION A-A

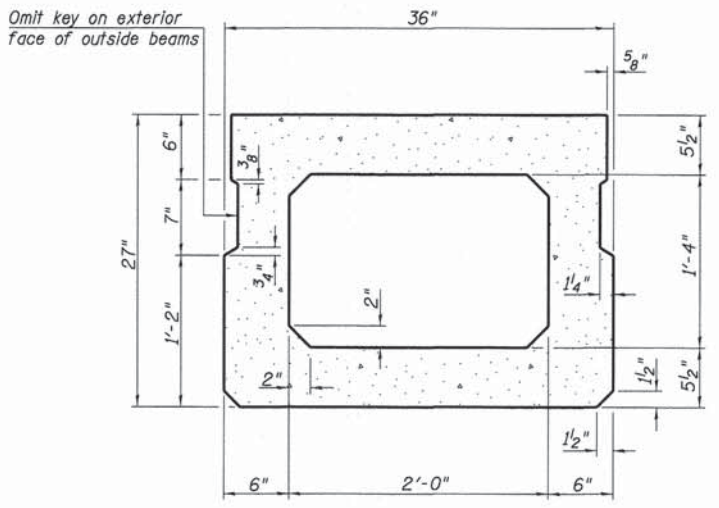


PLAN VIEW

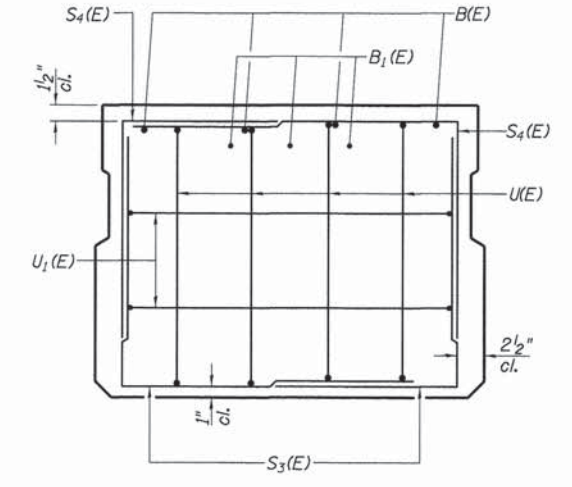
Note: Spacing of S(E) and S (E) bars may be adjusted up to 4" in the immediate area of the transverse tie diaphragms to miss the block outs for the transverse ties.

Bars Indicated thus: 4x3-#5 etc. Indicates 4 lines of bars with 3 lengths per line.

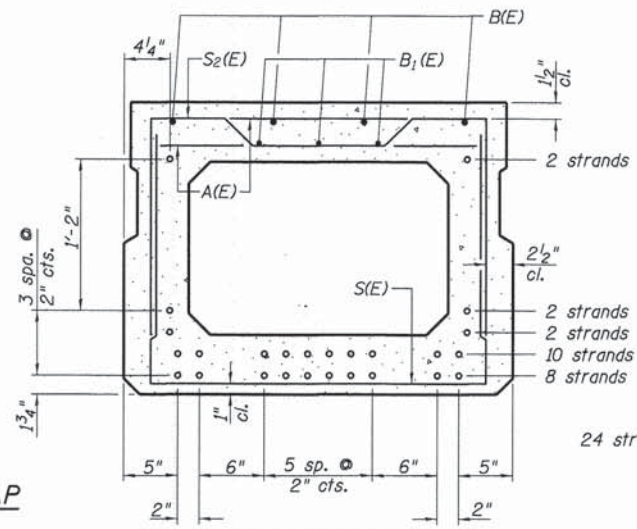
MINIMUM BAR LAP
 #4 bar = 2'-0"
 #5 bar = 2'-6"



SECTION B-B
(Showing dimensions)



VIEW C-C



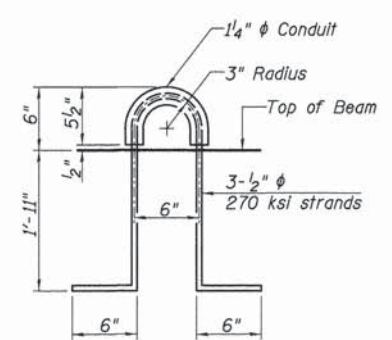
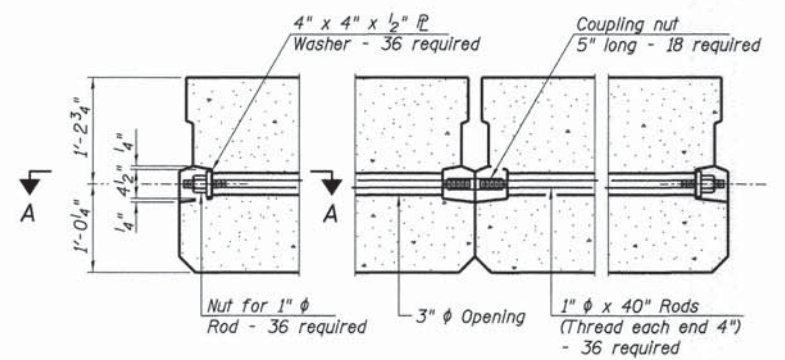
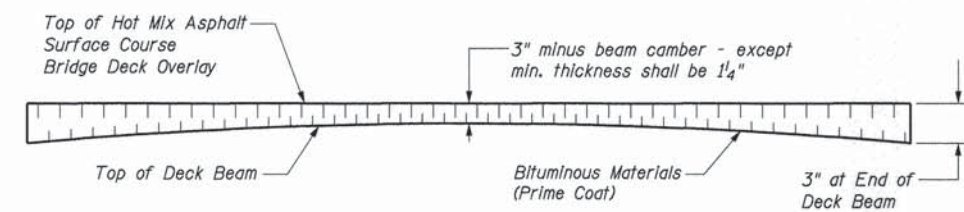
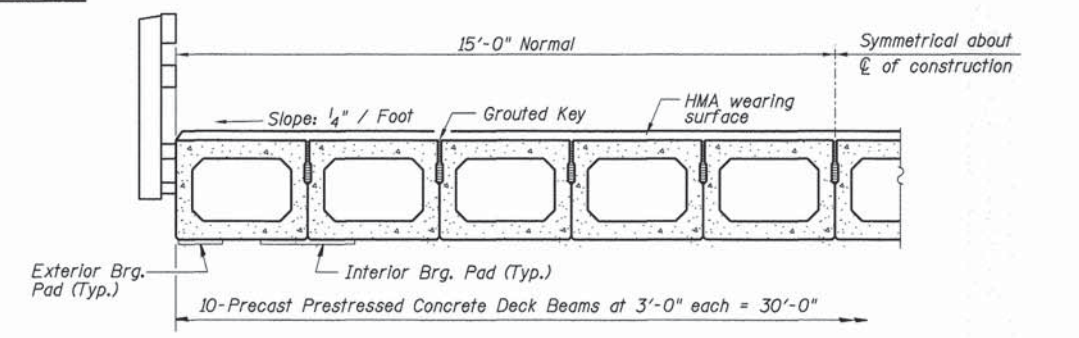
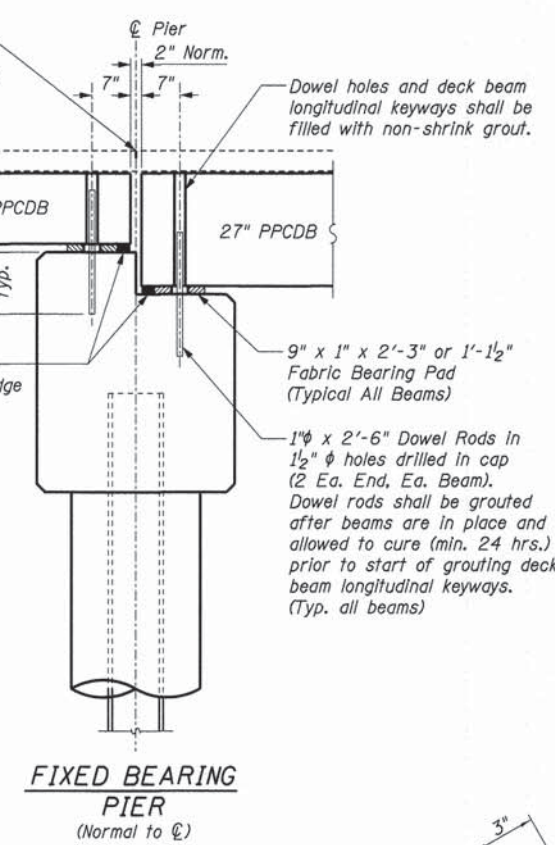
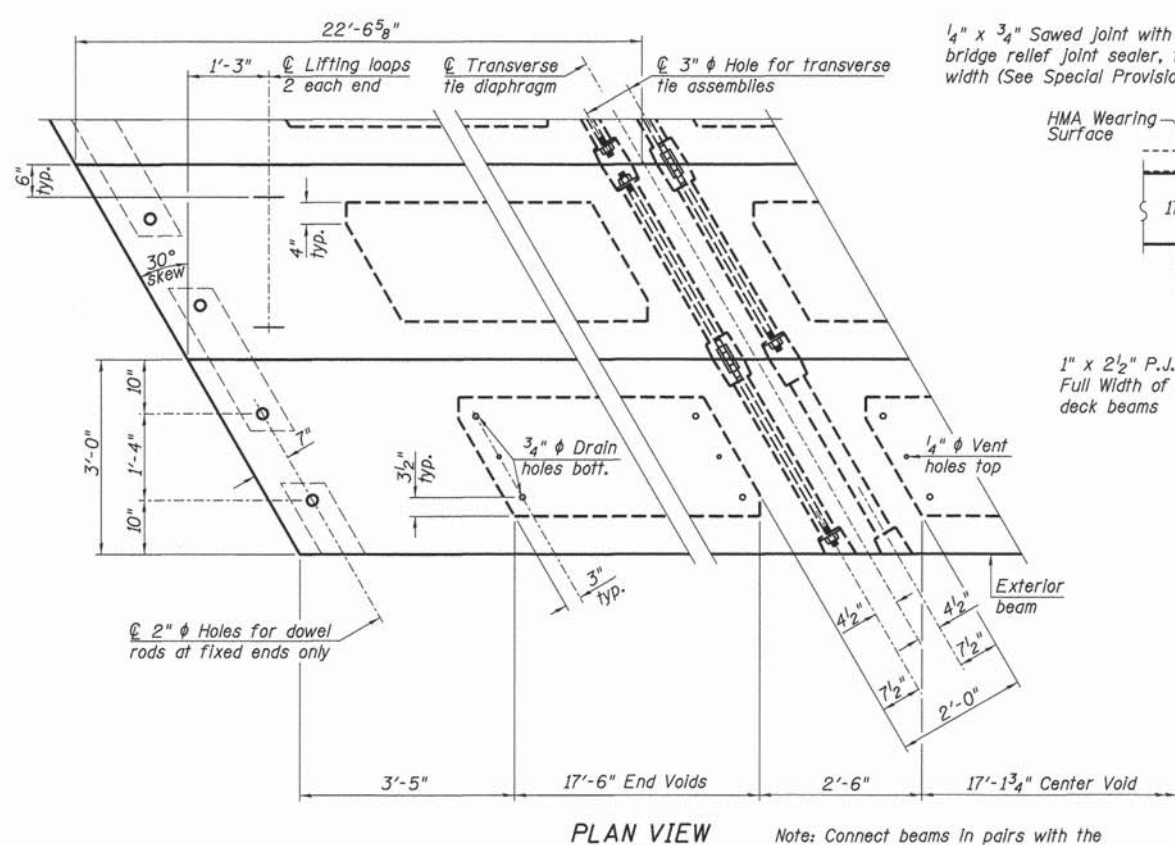
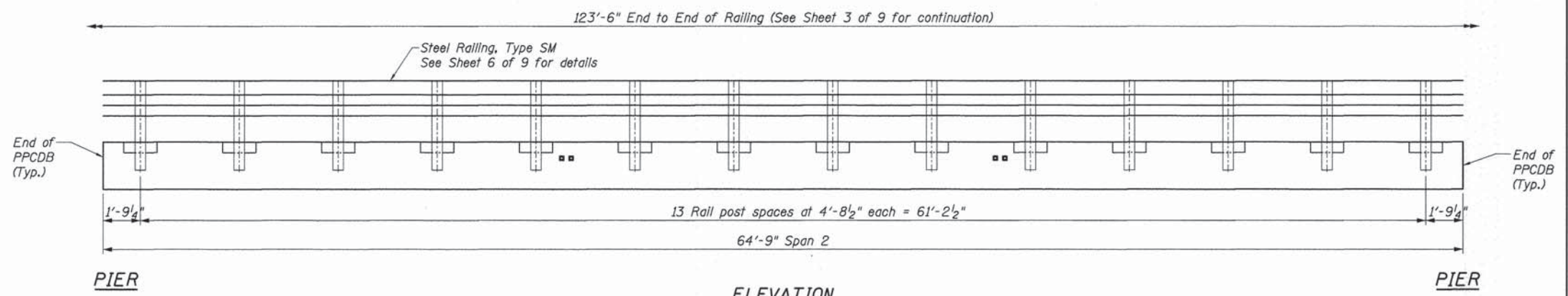
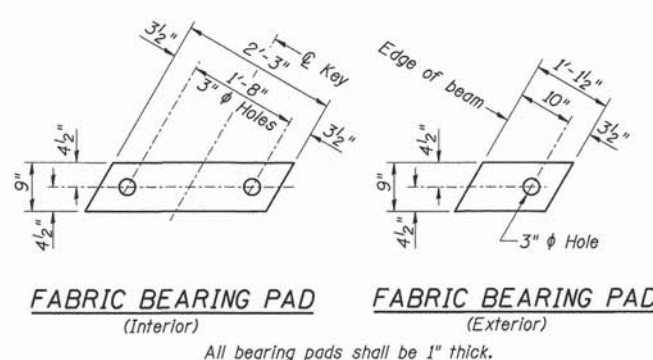
SECTION B-B
(Showing reinforcement and permissible strand locations)
 Note: Place the number of strands specified in each row symmetrically about the centerline of beam in the permissible strand locations shown.

**BAR LIST
 ONE BEAM ONLY**
 (For Information only)

Bar	No.	Size	Length	Shape
A(E)	59	#4	2'-7"	—
B(E)	12	#5	23'-2"	—
B1(E)	12	#4	17'-8"	—
S(E)	85	#4	6'-5"	U
S1(E)	8	#4	5'-11"	U
S2(E)	77	#4	6'-2"	U
S3(E)	12	#4	4'-5"	U
S4(E)	12	#4	4'-2"	U
U(E)	8	#5	4'-6"	U
U1(E)	4	#4	6'-10"	U

Note: See Sheet 5 of 9 for additional details and Bill of Material.

SPAN 2



NOTES

Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be 1/2" and the nominal cross-sectional area shall be 0.153 sq. in.

The 1" ϕ rods in the transverse tie assembly shall be tightened to a snug fit and the threads set. Pockets on exterior faces of bridge shall be filled with grout after transverse tie assembly is in place.

Reinforcement bars shall conform to ASTM A 706, Grade 60 (IL Modified).

Two 1/8" fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location.

A minimum 2 1/2" ϕ lifting pin shall be used to engage the lifting loops during handling.

Corrosion Inhibitor, per Article 1020.05(b)(12) and 1021.06 of the Standard Specifications, shall be used in the concrete for precast prestressed concrete deck beams.

Compressive strength of prestressed concrete, $f'c$, shall be 6000 psi.

Compressive strength of prestressed concrete at release, $f'cl$, shall be 5000 psi.

BILL OF MATERIAL

Precast Prestressed Conc. Deck Bms. (27" depth)	Sq. Ft.	1943
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SPAN 2

RHUTASEL and ASSOCIATES, INC.
 CONSULTING ENGINEERS • LAND SURVEYORS
 CENTRALIA, ILLINOIS FREEBURG, ILLINOIS
 ILLINOIS DESIGN FIRM LICENSE NO. 184-000287

DESIGNED - BLT	REVISED -
DRAWN - JN	REVISED -
CHECKED - WDL	REVISED -
DATE - 02/04/2015	REVISED -

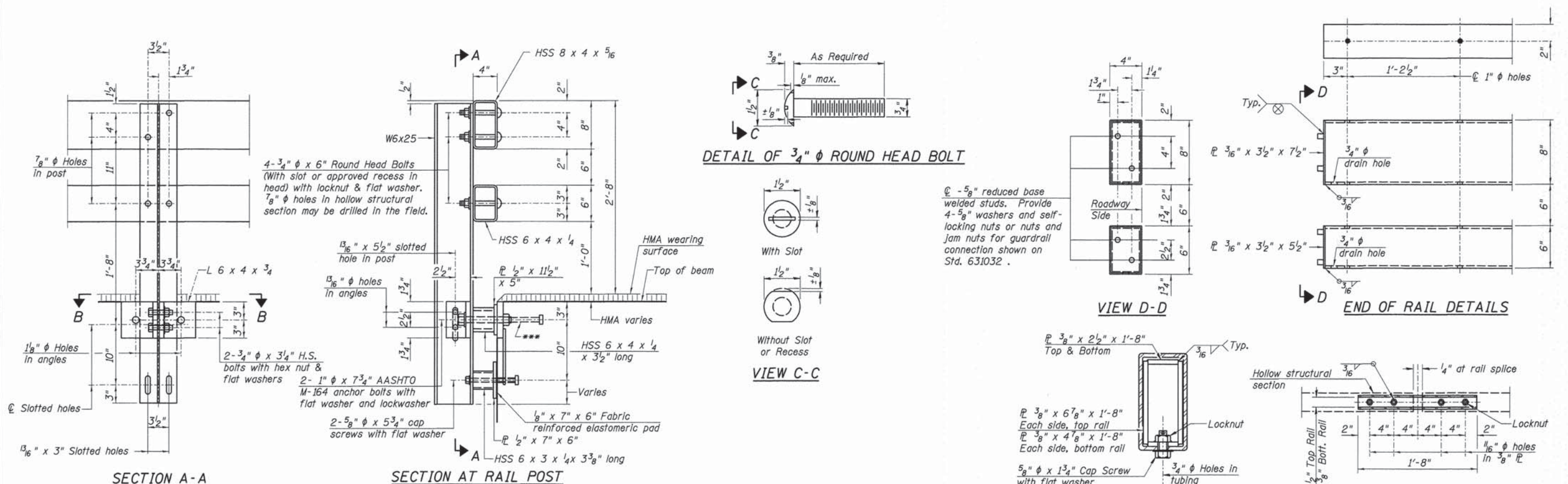
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PRECAST PRESTRESSED CONCRETE DECK BEAM DETAILS

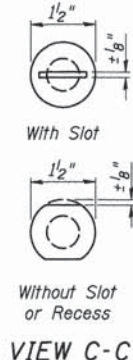
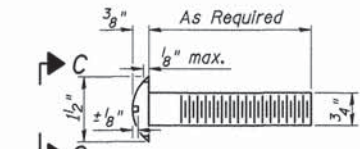
SCALE: NTS

BRIDGE SHEET 5 OF 9

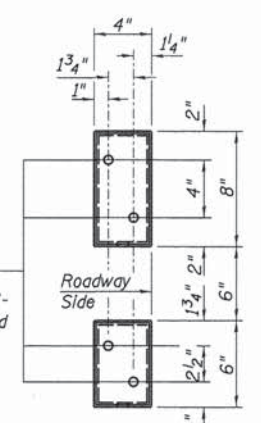
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CONTRACT NO. 95755			
RAAI JOB NO. 53414 ILLINOIS FED. AID PROJECT			



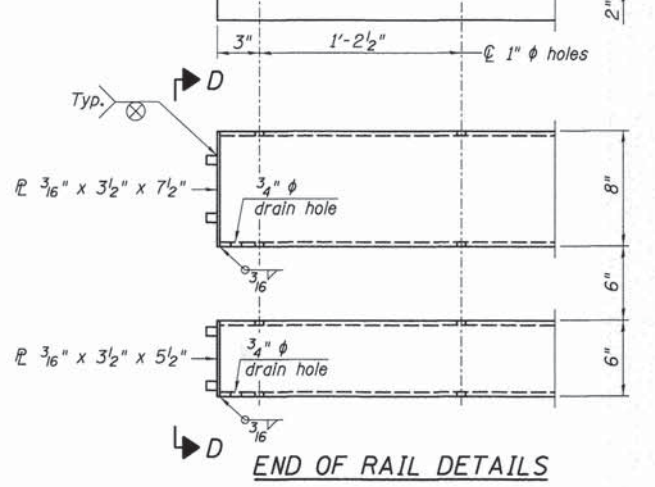
DETAIL OF 3/4" ϕ ROUND HEAD BOLT



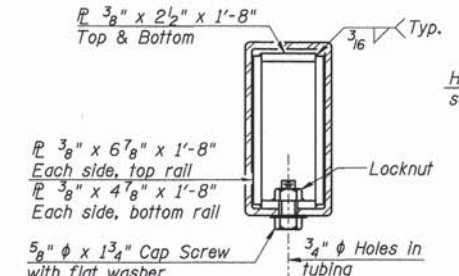
VIEW C-C



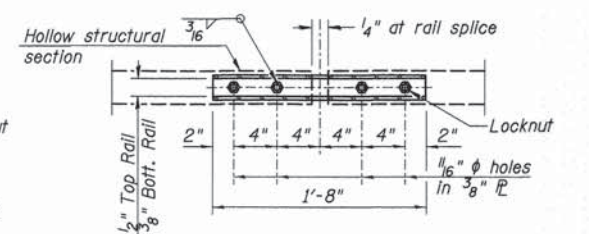
VIEW D-D



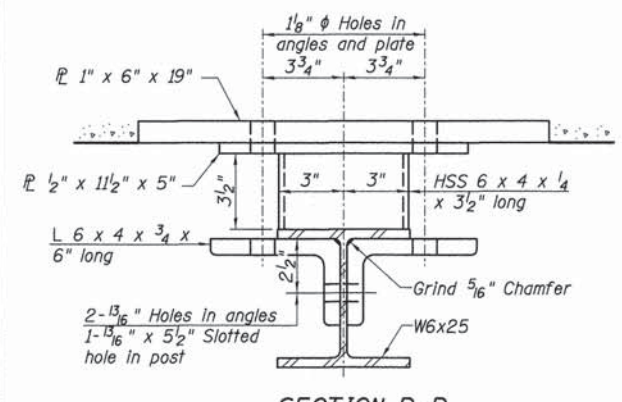
END OF RAIL DETAILS



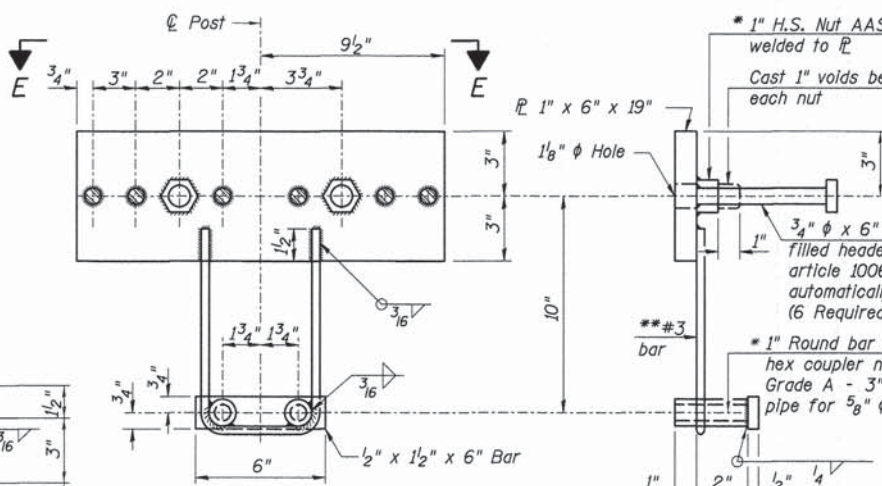
SECTION AT RAIL SPLICE



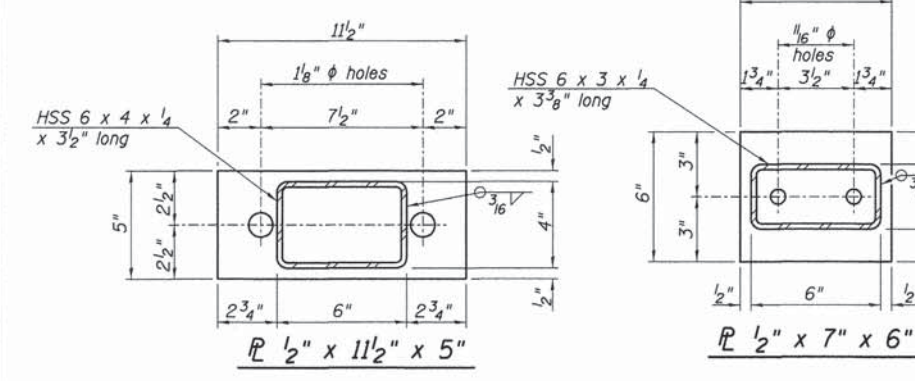
PLAN-BOTT. SPLICE TYPICAL



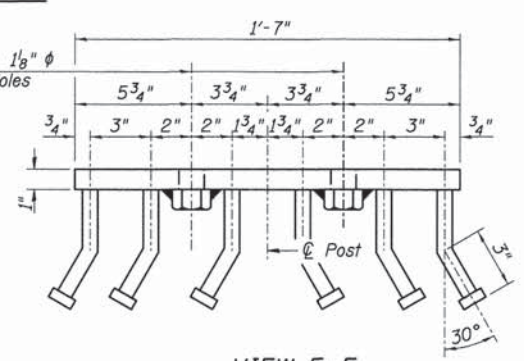
SECTION B-B



ANCHOR DEVICE



HSS 6 x 3 x 1/4 x 3 3/8 long



VIEW E-E

Notes:
 All field drilled holes shall be coated with an approved zinc rich paint before erection.
 For multi-span bridges, sufficient 1/4" x 6" x 1'-2" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Steel Railing, Type SM.
 All steel rail members shall be galvanized according to Article 509.05 of the Standard Specifications.
 *** The studs of the anchor devices shall be placed below the top reinforcement bars and the outermost longitudinal reinforcement bar shall be placed directly above the studs of the rail post anchor device.

BILL OF MATERIAL

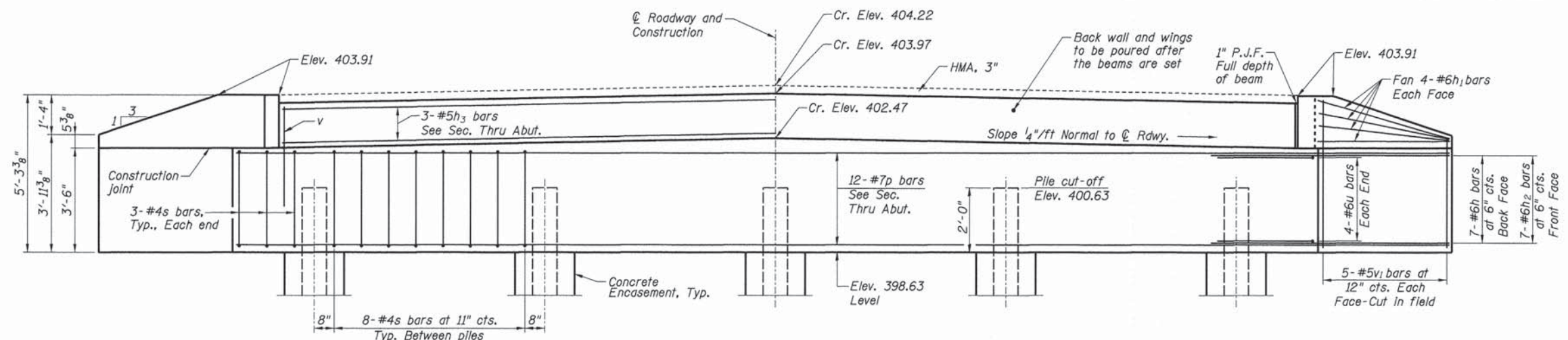
Item	Unit	Quantity
Steel Railing, Type SM	Foot	247

See Sheets 3 and 5 of 9 for post spacing.

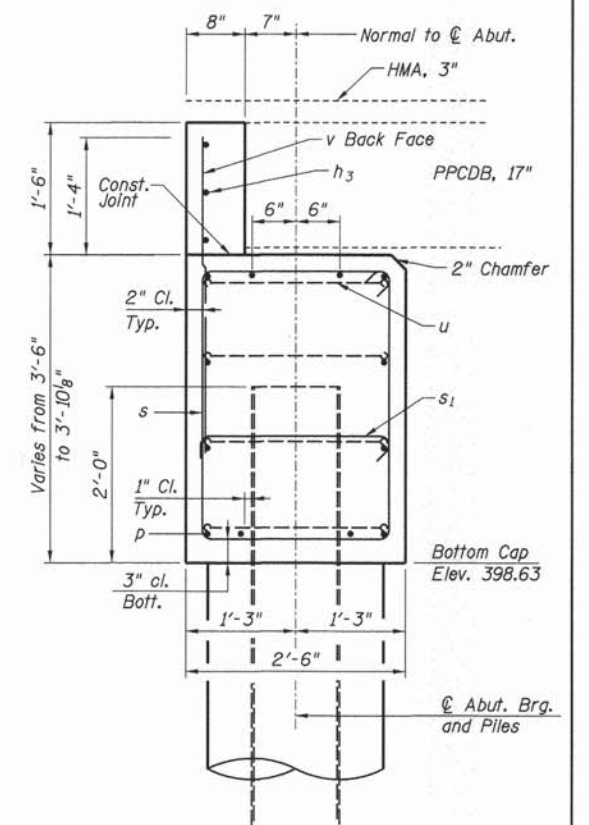
** Threaded areas shall be plugged or blocked off during casting of beam. Galvanized after fabrication.

** Whenever the lower insert assemblies interfere with strand locations, the #3 bars shall be cut and adjusted in order to allow raising or lowering of the lower inserts. Maximum adjustment not to exceed 1/2".

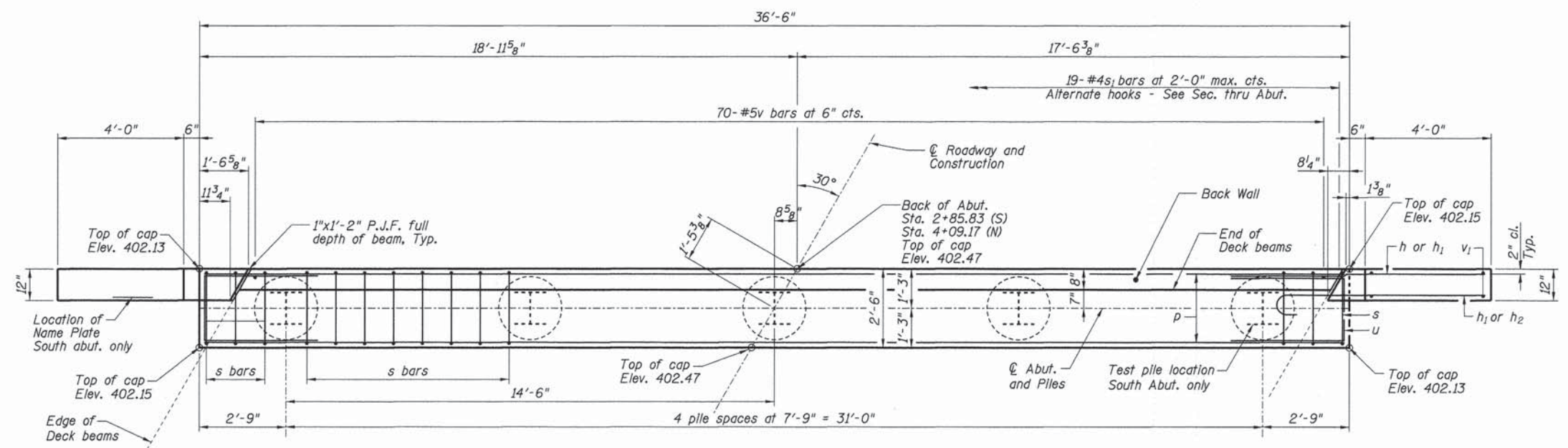
DESIGNED	BLT	REVISED	
DRAWN	JN	REVISED	
CHECKED	WDL	REVISED	
DATE	02/04/2015	REVISED	



ELEVATION



SEC. THRU ABUT.
(Normal to $\text{\textcircled{C}}$)



PLAN

BILL OF MATERIAL FOR ONE ABUTMENT

Bar	No.	Size	Length	Shape
h	14	#6	9'-0"	—
h ₁	16	#6	5'-6"	CUT IN FIELD
h ₂	14	#6	7'-3"	C
h ₃	3	#5	34'-3"	—
p	12	#7	36'-2"	—
s	38	#4	11'-3"	□
s ₁	19	#4	3'-3"	□
u	8	#6	9'-3"	—
v	70	#5	3'-6"	—
v ₁	20	#5	5'-0"	CUT IN FIELD
Concrete Structures		Cu. Yd.	15.4	
Concrete Encasement		Cu. Yd.	1.8	
Reinforcement Bars		Pound	2270	
Furnishing Steel	Foot	S. Abut.	320	
Piles HP12x53	Foot	N. Abut.	395	
Driving Piles	Foot	S. Abut.	320	
		N. Abut.	395	
Test Pile Steel	Each	S. Abut.	1	
HP 12x53	Each	N. Abut.	0	
Pile Shoes	Each	S. Abut.	5	
		N. Abut.	5	

For details of Piles and Pile Encasement, see HP Pile Details Sheet.

GENERAL NOTES

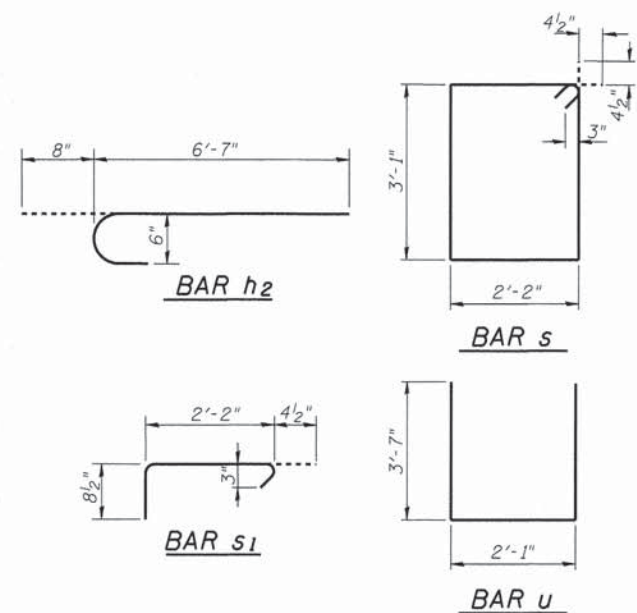
- Reinforcement bars shall conform to the requirements of ASTM A 706 Grade 60 (IL Modified).
- All exposed edges shall have standard 3/4" chamfer, unless otherwise noted or as directed by the Engineer.
- All clearances between rebar and form surface shall be 2", unless otherwise noted.
- Space reinforcement in cap to miss PPCDB dowel rods.
- The Steel H-piles shall be according to AASHTO M270 Grade 50.
- The Contractor shall drive one (1) Test Pile of the size and location indicated and as directed by the Engineer before ordering the remainder of the piles.
- The Test Pile shall be driven to 110 percent of the Nominal Required Bearing indicated in the pile data information.
- The position of the 90° & 135° hooked ends of the s₁ bar shall be alternated between adjacent bars.
- The back wall and portion of the wingwalls above the construction joint shall be cast against the in-place deck beams.

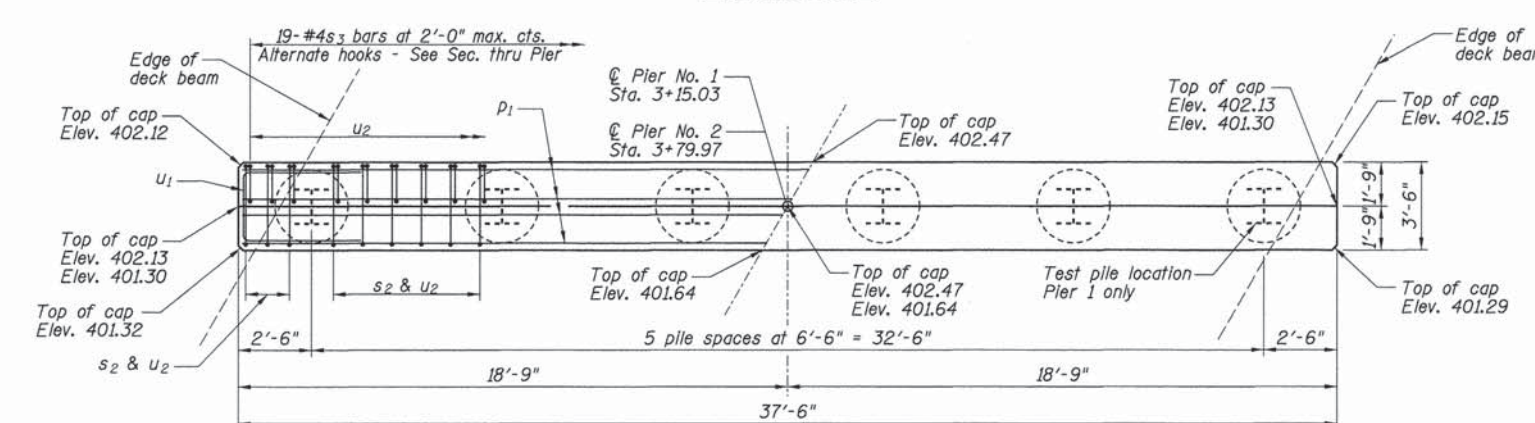
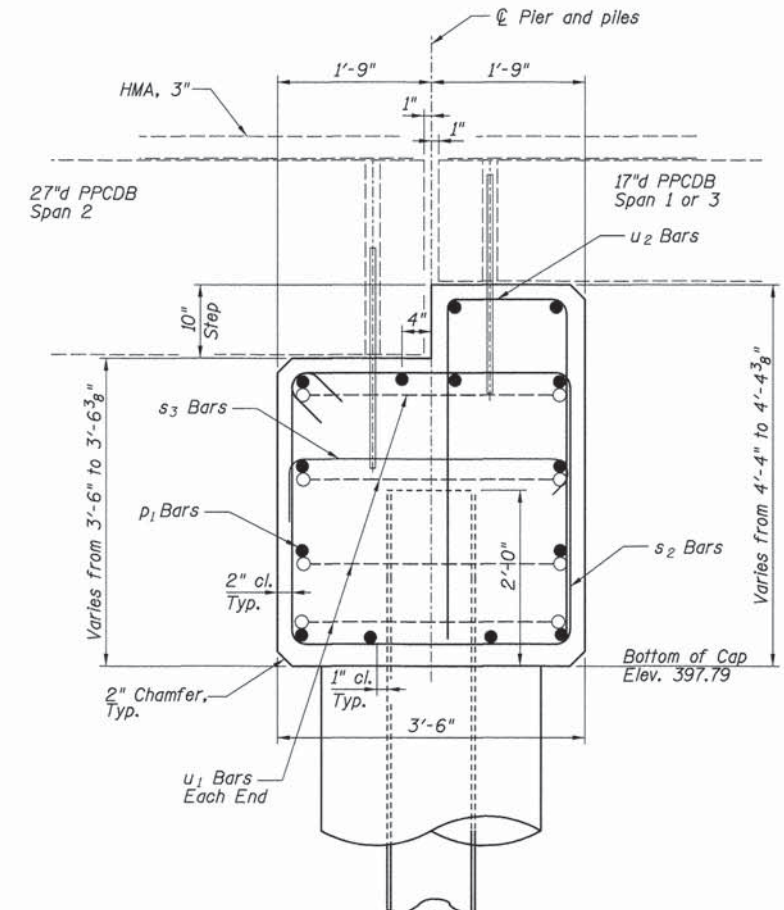
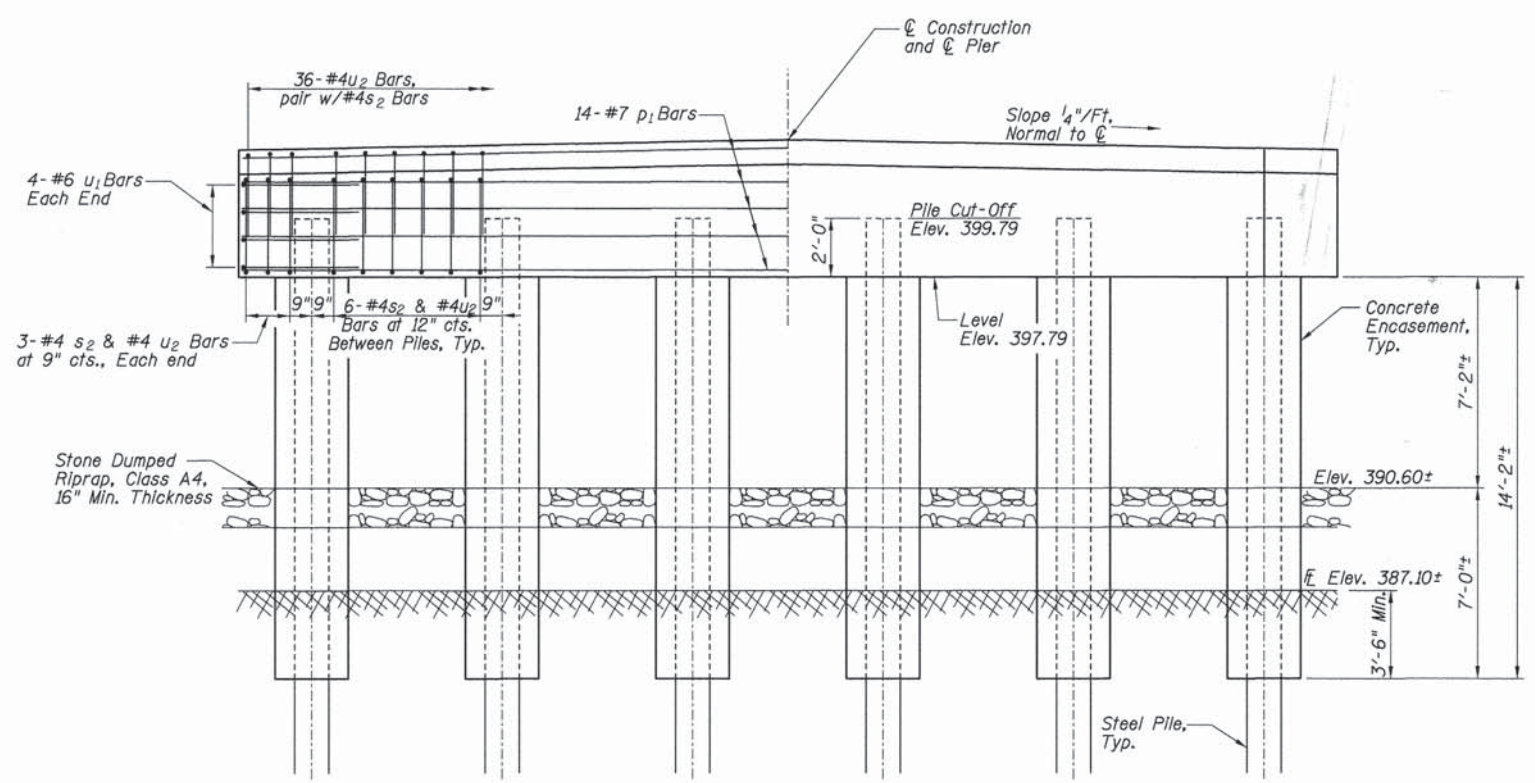
PILE DATA SOUTH ABUTMENT

Type: Steel HP12x53
 Nominal Required Bearing: 419 kips
 Factored Resistance Available: 230 kips
 Estimated Length: 80'/pile
 No. Production Piles w/Pile Shoes: 4
 No. Test Piles w/Pile Shoes: 1

PILE DATA NORTH ABUTMENT

Type: Steel HP12x53
 Nominal Required Bearing: 419 kips
 Factored Resistance Available: 230 kips
 Estimated Length: 79'/pile
 No. Production Piles w/Pile Shoes: 5
 No. Test Piles w/Pile Shoes: 0





PILE DATA PIER NO. 1

Type:	Steel HP14x73
Nominal Required Bearing:	419 kips
Factored Resistance Available:	173 kips
Estimated Length:	79'/pile
No. Production Piles w/Pile Shoes	5
No. Test Piles w/Pile Shoes	1

PILE DATA PIER NO. 2

Type:	Steel HP14x73
Nominal Required Bearing:	419 kips
Factored Resistance Available:	173 kips
Estimated Length:	79'/pile
No. Production Piles w/Pile Shoes	6
No. Test Piles w/Pile Shoes	0

GENERAL NOTES

Reinforcement bars shall conform to the requirements of ASTM A 706 Grade 60 (IL Modified).

All exposed edges shall have standard 3/4" chamfer, unless otherwise noted or as directed by the Engineer.

All clearances between rebar and form surface shall be 2", unless otherwise noted.

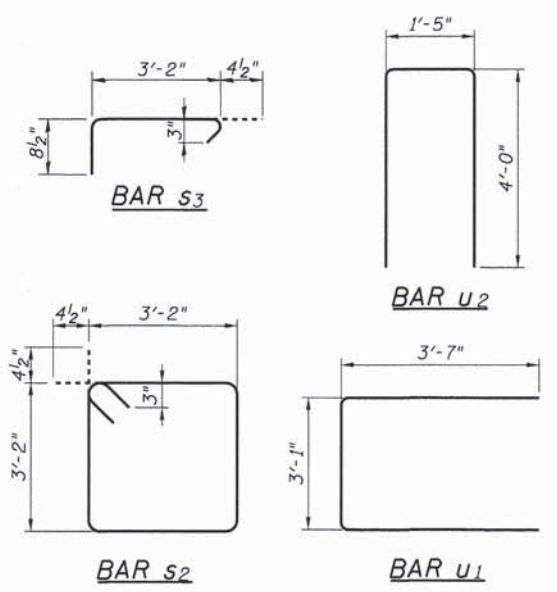
Space reinforcement in cap to miss PPCDB dowel rods.

The Steel H-piles shall be according to AASHTO M270 Grade 50.

The Contractor shall drive one (1) Test Pile of the size and location indicated and as directed by the Engineer before ordering the remainder of the piles.

The Test Pile shall be driven to 110 percent of the Nominal Required Bearing indicated in the pile data information.

The position of the 90° & 135° hooked ends of the s₃ bar shall be alternated between adjacent bars.

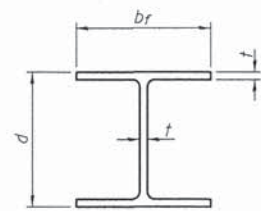


BILL OF MATERIAL FOR ONE PIER

Bar	No.	Size	Length	Shape
p ₁	14	#7	37'-2"	—
s ₂	36	#4	13'-5"	□
s ₃	19	#4	4'-3"	┌
u ₁	8	#6	10'-3"	┌
u ₂	36	#4	9'-5"	┌
Concrete Structures Cu. Yd. 19.9				
Concrete Encasement Cu. Yd. 15.5				
Reinforcement Bars Pound 1790				
Furnishing Steel Foot Pier 1 395				
Piles, HP14x73 Foot Pier 2 474				
Driving Piles Foot Pier 1 395				
Foot Pier 2 474				
Test Pile Steel HP14x73 Each Pier 1 1				
Each Pier 2 0				
Pile Shoes Each Pier 1 6				
Each Pier 2 6				

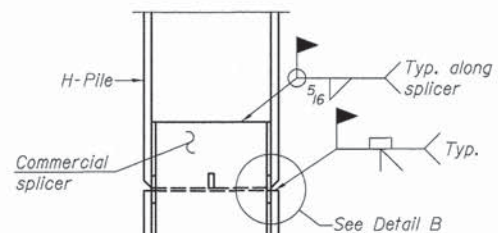
For details of Piles and Pile Encasement, see HP Pile Details Sheet.

DESIGNED -	BLT	REVISED -	
DRAWN -	JN	REVISED -	
CHECKED -	WDL	REVISED -	
DATE -	02/04/2015	REVISED -	

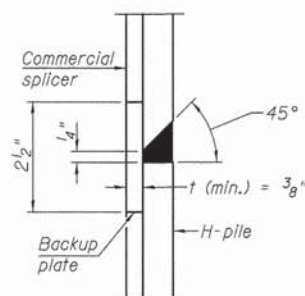


STEEL PILE TABLE

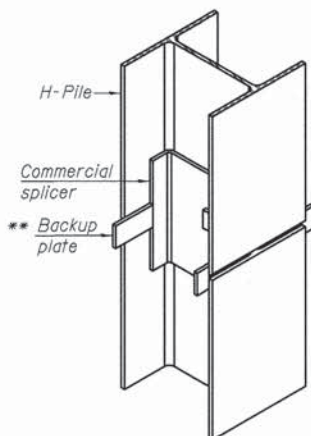
Designation	Depth <i>d</i>	Flange width <i>b_f</i>	Web and Flange thickness <i>t</i>	Encasement diameter <i>A</i>
HP 14x117	14 1/4"	14 7/8"	1 3/16"	30"
x102	14"	14 3/4"	1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	11/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



ELEVATION

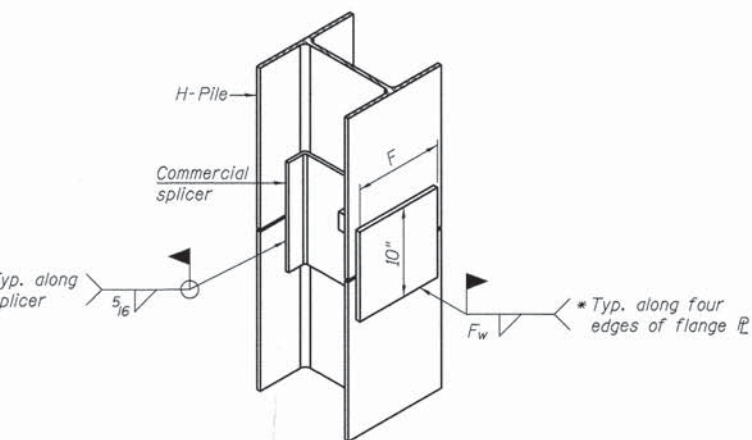


DETAIL "B"



ISOMETRIC VIEW

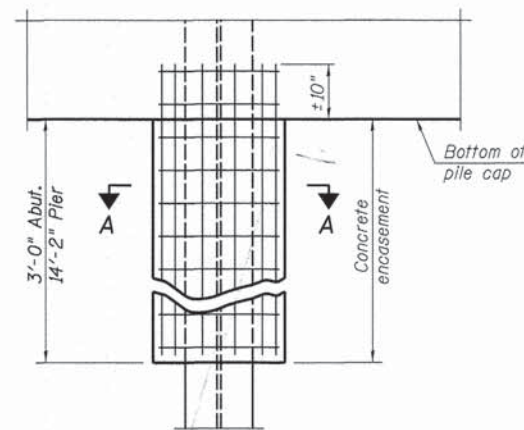
WELDED COMMERCIAL SPLICE



ISOMETRIC VIEW

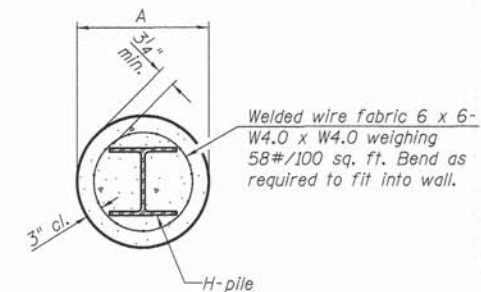
WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds 1/4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (5/16" min.).



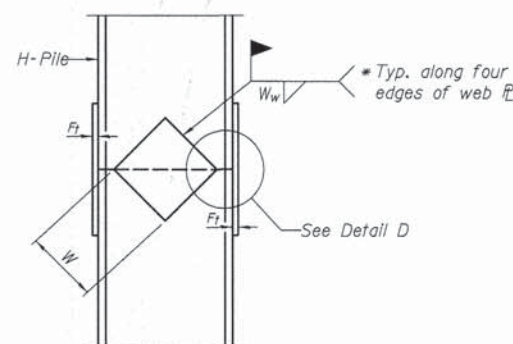
ELEVATION

PILE ENCASEMENT

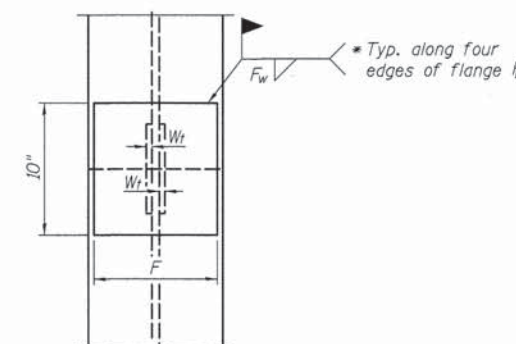


SECTION A-A

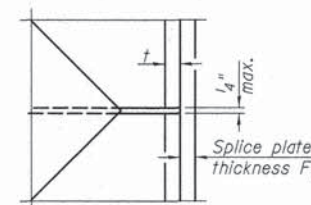
Note:
Forms for encasement may be omitted when soil conditions permit.



ELEVATION



END VIEW

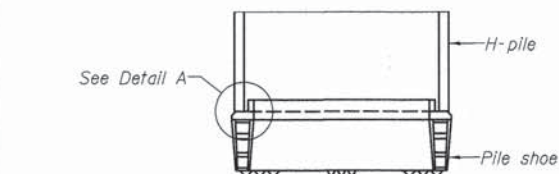


DETAIL D

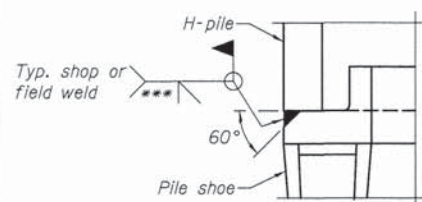
WELDED PLATE FIELD SPLICE

Designation	<i>F</i>	<i>F_t</i>	<i>F_w</i>	<i>W</i>	<i>W_t</i>	<i>W_w</i>
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5/8"	1/2"
x89	12 1/2"	3/4"	11/16"	7 3/4"	5/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5/8"	1/2"
HP 12x84	10"	7/8"	11/16"	6 1/2"	5/8"	1/2"
x74	10"	7/8"	11/16"	6 1/2"	5/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

Note:
The steel H-piles shall be according to AASHTO M270 Grade 50.



ELEVATION



DETAIL A

H-PILE SHOE ATTACHMENT