

GENERAL NOTES

ALL REINFORCEMENT BARS SHALL BE LAPPED 24 DIAMETERS UNLESS OTHERWISE SHOWN. FIELD CONNECTIONS SHALL BE BOLTED USING HIGH STRENGTH BOLTS. BOLTS 3/4" Ø, OPEN HOLES 13/16" Ø, UNLESS OTHERWISE NOTED.

FIELD WELDING OF CONSTRUCTION ACCESSORIES WILL NOT BE PERMITTED IN THE BOTTOM OF FLANGE OF BEAMS OR GIRDERS NOR ON THE TOP FLANGE OF A DISTANCE EQUAL TO ONE-FOURTH THE SPAN LENGTH EACH WAY FROM THE PIER SUPPORTS. FIELD WELDING IN OTHER AREAS WILL BE PERMITTED ONLY WHEN APPROVED BY THE ENGINEER.

ANCHOR BOLTS SHALL BE SET BEFORE BOLTING DIAPHRAGMS OVER SUPPORTS.

SLOPE WALL SHALL BE REINFORCED WITH WELDED WIRE FABRIC 6" x 6" MESH, WEIGHING 50# PER 100 SQ. FT..

THE EMBANKMENT CONFIGURATION SHOWN SHALL BE THE MINIMUM EMBANKMENT THAT MUST BE CONSTRUCTED PRIOR TO CONSTRUCTION OF THE ABUTMENTS.

THE CONTRACTOR SHALL DRIVE ONE STEEL TEST PILES IN A PERMANENT LOCATION, AT ABUTMENT 'A' AS DIRECTED BY THE ENGINEER BEFORE ORDERING THE REMAINDER OF THE PILES.

THE BASIC LEAD SILICO CHROMATE PAINT SYSTEM SHALL BE USED FOR SHOP AND FIELD PAINTING OF STRUCTURAL STEEL.

THE CONCRETE RAIL SECTION ABOVE THE MANDATORY CONST. JOINT AT THE TOP OF THE SLAB SHALL BE CONSTRUCTED OF CLASS X CONCRETE, EXCEPT THE AGGREGATES SHALL CONFORM TO THE REQUIREMENTS OF HANDRAIL CONCRETE.

PROTECTIVE COAT SHALL NOT BE APPLIED TO SURFACES TO WHICH WATERPROOFING MEMBRANE SYSTEM IS APPLIED.

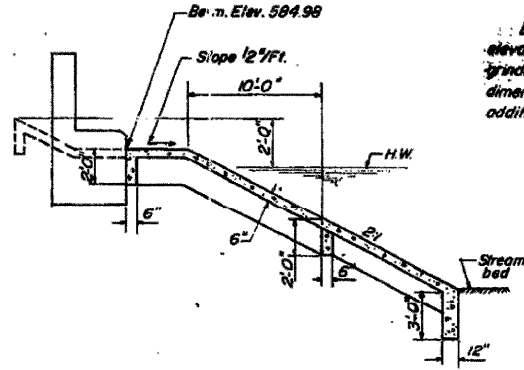
LAYOUT OF SLOPE WALLS MAY BE VARIED IN THE FIELD TO SUIT GROUND CONDITIONS AS DIRECTED BY THE ENGINEER.

CURVE DATA FA 403

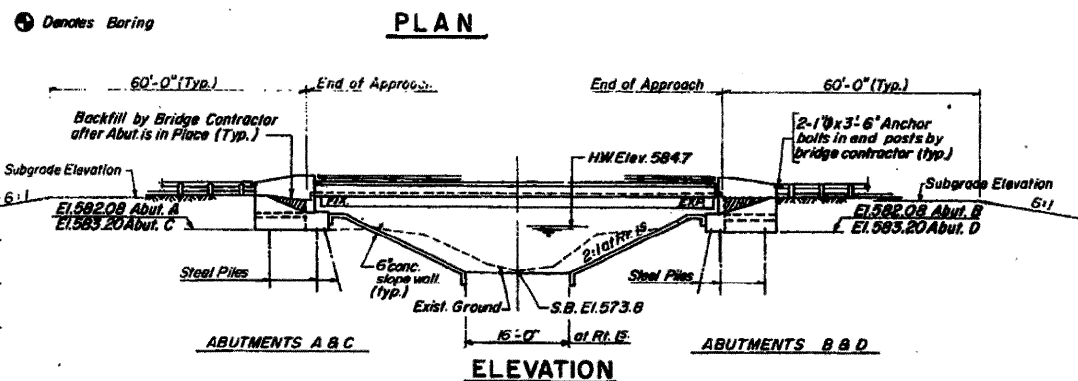
R1.Sta.732+54.457
 $\Delta = 53^{\circ}27'12''$
 $D = 1^{\circ}00'00''$
 $T = 2885019$
 $L = 5345333$
 $R = 572958$
 $SE = 0.04 \text{ 1/ft.}$

WATERWAY DATA

Drainage Area 4800 Acres.
 Character Hilly
 Required Opening 400-Sq.Ft.
 Provided Opening 412 Sq.Ft.
 Bottom of Channel El.573.8
 Q100 2000 cfs



Bearing steel surfaces shall be constructed or adjusted to elevations within a tolerance of ± 1/8 inch. Adjustment shall be by grinding the surface or by shimming the bearing. Two 1/2" adjustment dimensions of the bottom bearing plate, shall be provided for adjustment in addition to all other plates or shims.



TYPICAL SECTION THRU SLOPE WALL

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER STRUCT.	SUB STRUCT.	TOTAL
PROTECTIVE COAT	SQ. YDS.	184	--	184
CLASS "X" CONCRETE	CY. YDS.	209.2	219.7	428.9
STRUCTURAL STEEL	L. SWM	008	--	008
ALUMINUM RAILING	LIN. FT.	326	--	326
REINFORCEMENT BARS	LBS.	47,880	16,500	64,380
STUD SHEAR CONNECTORS	EACH	2,184	--	2,184
STEEL PILES HP8 x 36	LIN. FT.	--	817	817
TEST PILE (STEEL) HP8 x 36	EACH	--	1	1
NAME PLATES	EACH	1	--	1
SLOPE WALL 6"	SQ. YDS.	--	1,638	1,638
BIT. CONC. SURFACE COURSE CLASS 1	TWS	60.0	--	60.0
WATERPROOFING MEMBRANE SYS.	SQ. YDS.	704.0	--	704.0
PREFORMED JOINT SEALER 2 1/2"	LIN. FT.	94.0	--	94.0
PERMANENT B.M. TYPE 1	EACH	1	--	1

* CALCULATED WEIGHT OF STRUCTURAL STEEL = 234,200 LBS.

NOTES:

DESIGN LOADING:
 HS 20-44 And Allowance For 25 PS.F Future Wearing Surface.

DESIGN STRESSES:
 $f_c = 1400 \text{ P.S.I.}$ Except As follows:
 $f_c = 1200 \text{ P.S.I.}$ For Deck Slab
 $f_c = 1000 \text{ P.S.I.}$ For Conc. In Contact With Earth
 $f_s = 27,000 \text{ P.S.I.}$ - M222 Structural Steel
 $f_s = 20,000 \text{ P.S.I.}$ Reinforcement Steel
 $v = 75 \text{ P.S.I.}$ Allowable Shear in Footings
 $n = 10$
 Allowable Live Load Deflection = $L/1200$ (Composite)

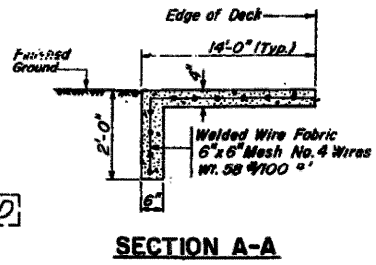
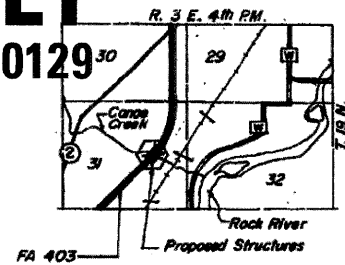
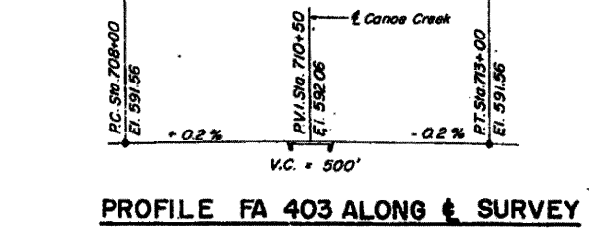
DESIGN SPECIFICATIONS: AASHTO 1973 As Applicable.
 For Footing Layout see Sheet #3

STATION 710+50.00
 BUILT BY
 STATE OF ILLINOIS
 FA 403 SECTION 161-1B-2
 FA 403 OVER CANOE CREEK
 LOADING HS 20

NAME PLATE
 SEE STD 2113

FOR INFORMATION ONLY
 LOCATION 4 Structure Numbers 081-0128 & 0129

DESIGNED	D.N.
CHECKED	H.S.
DRAWN	A.M.
CHECKED	H.S.



AS REVISED

Signature: *Samuel M. Priddy*

GENERAL PLAN & ELEVATION
 FA 403 SECTION 161-1B-2
 FA 403 OVER CANOE CREEK
 ROCK ISLAND COUNTY
 STATION 710+50.00

** FAI ROUTES 74 & 88
 ** D2 BRIDGE PAINTING 2008-1