

GENERAL NOTES

ALL REINFORCEMENT BARS SHALL BE LAPPED 24 DIAMETERS UNLESS OTHERWISE SHOWN. FIELD CORRECTIONS SHALL BE BOLTED USING HIGH STRENGTH BOLTS. BOLTS 3/4" Ø, OPEN HOLES 13/16" Ø, UNLESS OTHERWISE NOTED.

THE BASIC LEAD SILICO CHROMATE PAINT SYSTEM SHALL BE USED FOR SHOP AND FIELD PAINTING OF STRUCTURAL STEEL.

FIELD WELDING OF CONSTRUCTION ACCESSORIES WILL NOT BE PERMITTED TO THE BOTTOM OF FLANGE OF BEAMS OR GIRDERS NOR TO THE TOP FLANGE FOR A DISTANCE EQUAL TO ONE-FOURTH THE SPAN LENGTH EACH WAY FROM THE PIER SUPPORTS. FIELD WELDING IN OTHER AREAS WILL BE PERMITTED ONLY WHEN APPROVED BY THE ENGINEER.

ANCHOR BOLTS SHALL BE SET BEFORE BOLTING DIAPHRAGMS OVER SUPPORTS.

SLOPE WALL SHALL BE REINFORCED WITH WELDED WIRE FABRIC 6" x 6" MESH, WEIGHING 58# PER 100 SQ. FT..

THE EMBANKMENT CONFIGURATION SHOWN SHALL BE THE MINIMUM EMBANKMENT THAT MUST BE CONSTRUCTED PRIOR TO CONSTRUCTION OF THE ABUTMENTS.

THE CONTRACTOR SHALL DRIVE FOUR TIMBER TEST PILES, ONE AT EACH PIER AND FOUR CONCRETE TEST PILES, ONE AT EACH ABUTMENT, IN A PERMANENT LOCATION, AS DIRECTED BY THE ENGINEER BEFORE ORDERING THE REMAINDER OF THE PILES. The main load carrying member components subject to the Supplemental Requirements for Notch Toughness are the flanges, webs, and splice plates of the steel girders or wide flange beams.

THE CONCRETE RAIL SECTION ABOVE THE MANDATORY CONST. JOINT AT THE TOP OF THE SLAB SHALL BE CONSTRUCTED OF CLASS X CONCRETE, EXCEPT THE AGGREGATES SHALL CONFORM TO THE REQUIREMENTS OF MAINBELL CONCRETE. PROTECTIVE COAT SHALL NOT BE APPLIED TO SURFACES TO WHICH WATERPROOFING MEMBRANE SYSTEM IS APPLIED.



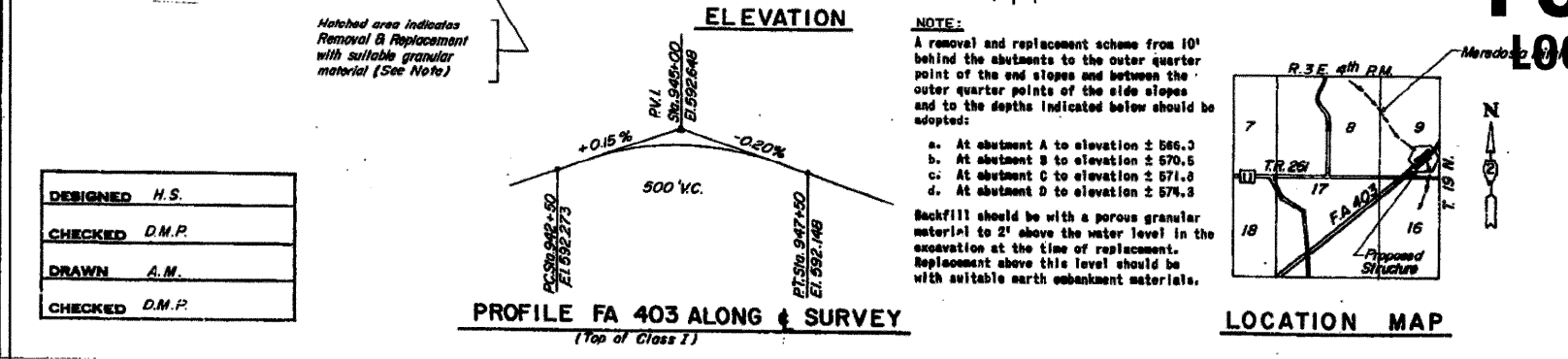
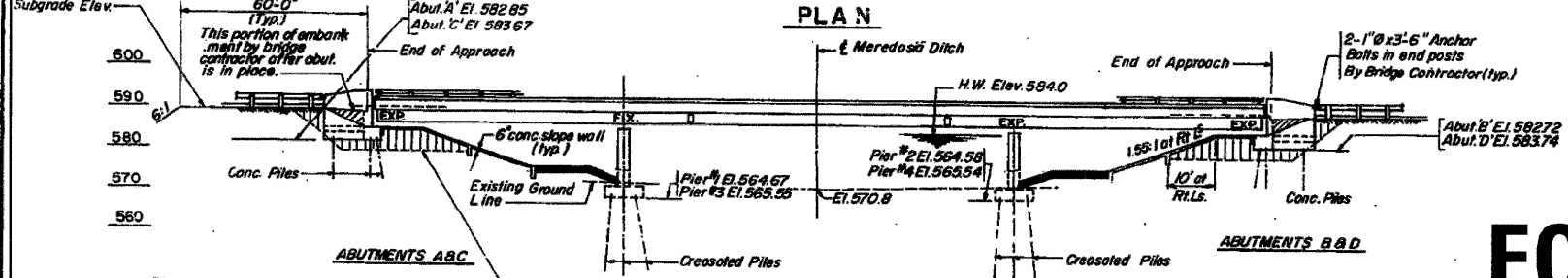
CURVE DATA FA 403
 P1.Sta.944+82.56
 $\Delta = 15^\circ-28'-08"$
 $D = 0^\circ-45'$
 $T = 1037.57$
 $L = 2062.52$
 $R = 7639.44$
 $S.E. = 0.0327/FI.$
 $E = 70.14$

WATERWAY DATA
 Drainage Area 36.55 Sq. Mi.
 Character Flat Cultivated Land
 Required Opening 765 Sq. Ft.
 Bottom of Channel El. 570.8
 Design Discharge 3514 cfs.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER STRUCT.	SUB STRUCT.	TOTAL
STRUCTURE EXCAVATION	CU. YDS.	---	625	625
PROTECTIVE COAT	SQ. YDS.	445	---	445
CLASS "X" CONCRETE	CU. YDS.	591.0	1,049.8	1,630.8
STRUCTURAL STEEL	L. SHK.	0.26	---	0.26
ALUMINUM RAILING	LIN. FT.	920	---	920
REINFORCEMENT BARS	LBS.	189,280	27,040	216,320
STUD SHEAR CONNECTORS	EACH	6,300	---	6,300
CROCKETED PILES (20.1' TO 38')	LIN. FT.	---	2,920	2,920
CONCRETE PILES	LIN. FT.	---	2,740	2,740
TEST PILES (CONCRETE)	EACH	---	4	4
TEST PILES (TIMBER)	EACH	---	4	4
SLOPE WALL (6")	SQ. YDS.	1,573	---	1,573
BIT. CONC. SURFACE COURSE CLASS I	TONS	166	---	166
WATERPROOFING MEMBRANE SYSTEM	SQ. YDS.	1,967	---	1,967
NEOPRENE EXP. JOINT 2"	LIN. FT.	124	---	124
NEOPRENE EXP. JOINT 2 1/2"	LIN. FT.	130	---	130
REINFORC. & REPLACEMENT OF GRANULAR MAT'L.	CU. YDS.	---	3,800	3,800
METAL SHOTS	EACH	---	212	212
CROCKETED PILES (UP TO 20')	LIN. FT.	---	646	646
PERMANENT B.M. TYPE I	EACH	1	---	1
NAME PLATES	EACH	1	---	1
ROCK FILL SPECIAL	TON	---	2,364	2,364

* CALCULATED PLAN WEIGHT OF STRUCTURAL STEEL = 628,240 LBS.

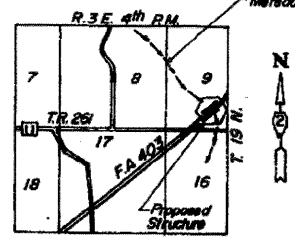


DESIGNED	H.S.
CHECKED	D.M.P.
DRAWN	A.M.
CHECKED	D.M.P.

NOTE:
 A removal and replacement scheme from 10' behind the abutments to the outer quarter point of the end slopes and between the outer quarter points of the side slopes and to the depths indicated below should be adopted:

- At abutment A to elevation ± 586.3
- At abutment B to elevation ± 570.5
- At abutment C to elevation ± 571.8
- At abutment D to elevation ± 574.3

Backfill should be with a porous granular material to 2' above the water level in the excavation at the time of replacement. Replacement above this level should be with suitable earth embankment materials.



FOR INFORMATION ONLY

LOCATION A Structure Numbers 081-0134 & 0135

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION
 DESIGNER: [Signature]
 CHECKED: [Signature]
 DRAWN: [Signature]
 PROJECT: FA PROJ. EBRF-403-1(C)
 LOADING HS 20
 NAME PLATE SEE STD 2113

GENERAL PLAN & ELEVATION
 FA 403 SECTION 161-1B-4
 FA 403 OVER MEREDOSIA DITCH
 ROCK ISLAND & WHITE SIDE COUNTY
 STATION 944+69.00