

v431(E) (Front F v436(E) (Back F

SECTION A-A

€ Column -

- s436(E)

(EF) (EF)

v433(E) (v434(E)

5 Eq. spa. typ.

Varies

2'-10"

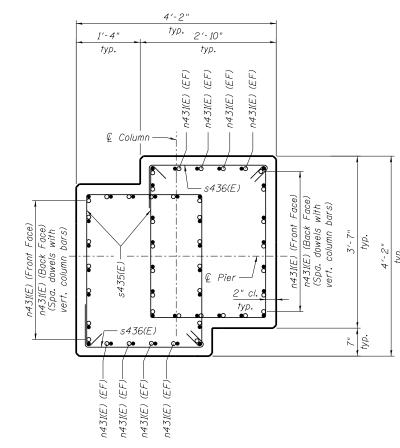
— s436(E)

(EF)

tyρ.

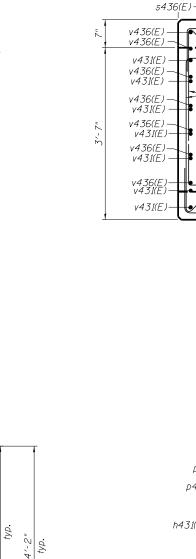
v433(E) (EF)

SECTION B-B



SECTION E-E

(Only dowels called out in this section



2'-10"

(EF) (EF)

v434(E) ,433(E)

v433(E) (EF) v434(E) (EF)

SECTION C-C

v432(E)

- € Column

v431(E)

v436(E)

-v436(E)

-v436(E)

-v431(E)

-v436(E)

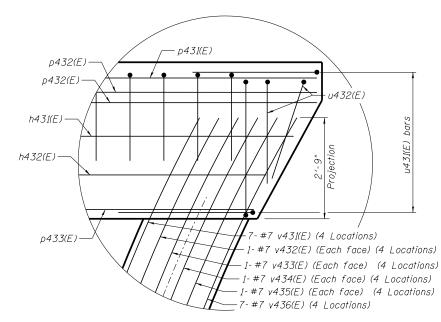
-v431(E)

-v436(E)

v431(E)

– v436(E)

- s436(E)

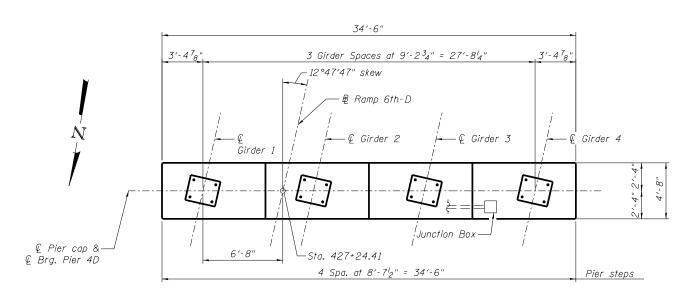


DETAIL 1

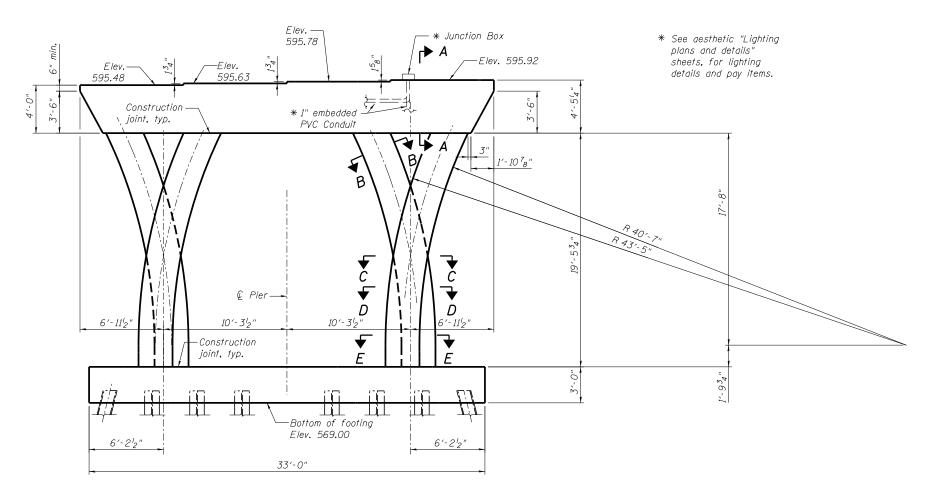
NOTE:

"EF" abbreviation indicates each face or side of the indicated column reinforcemen





PLAN OF PIER CAP



PIER 4D ELEVATION

(Looking South)

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 800601

FILE NAME = Ø81Ø187-AØ324-Ø36-Pier_4D_Layout.dgr

MODEL: Default

USER NAME = ksnider	DESIGNED - RJT/DMS	REVISED -
	CHECKED - TJJ	REVISED -
PLOT SCALE =	DRAWN - KMS	REVISED -
PLOT DATE = 1/18/2017	CHECKED - TJJ	REVISED -

DEPARTMENT OF TRANSPORTATION

PIER 4D LAYOUT 74 STRUCTURE NO. 081-0187 RAMP 6TH-D SHEET NO. SD36 OF SD44 SHEETS

ANCHOR BOLT LAYOUT

Type: HP14x73 with pile shoes

Nominal Required Bearing: 695 kips Factored Resistance Available: 486 kips

1. See sheet SD39 for pier concrete finishing notes.

bar is 2" unless noted otherwise or shown.

dressed and beveled strip unless noted otherwise.

included with the cost of Concrete Structures.

hydraulic pressure of the self consolidating concrete.

10. See foundation layout on sheet SD3 for pier layout.

9. The tremie tube shall be in place prior to placing formwork.

5. Space reinforcement in cap to miss anchor bolts.

2. For sections A-A, B-B, C-C, D-D, & E-E, See sheet SD38.

3. The minimum clear distance from the face of concrete to near reinforcing

4. All exposed corners, 90 degrees or sharper shall be filleted with a 34 "

6. The use of steel forms is required for the forming of all pier concrete surfaces from the tops of footings to the bottom of pier cap beams, including stem and pier columns. Use of medium-density overlaid (MDO) or high-density overlaid (HDO) plywood faced forms is allowed for forming of the pier cap beam. Plain plywood-faced forms will not be allowed for any portion of the pier column

7. The Contractor shall use self-consolidating concrete (SCC) in all the pier columns. The self-consolidating concrete shall conform to all requirements as specified in Section 1020 of the Standard Specifications. Cost of SCC shall be

8. The contractor shall provide adequate forms to contain the increased

PILE DATA

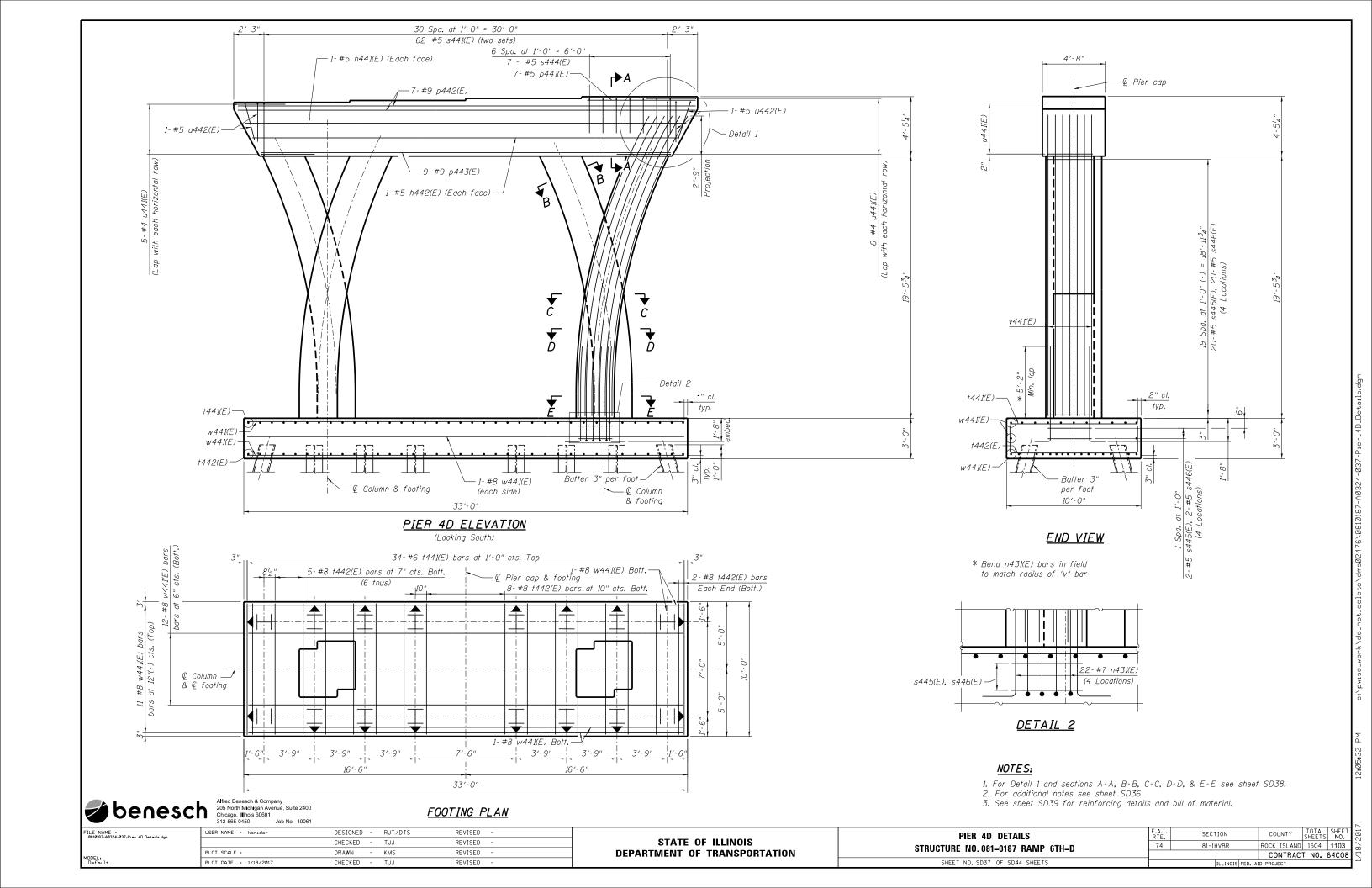
PIER NOTES:

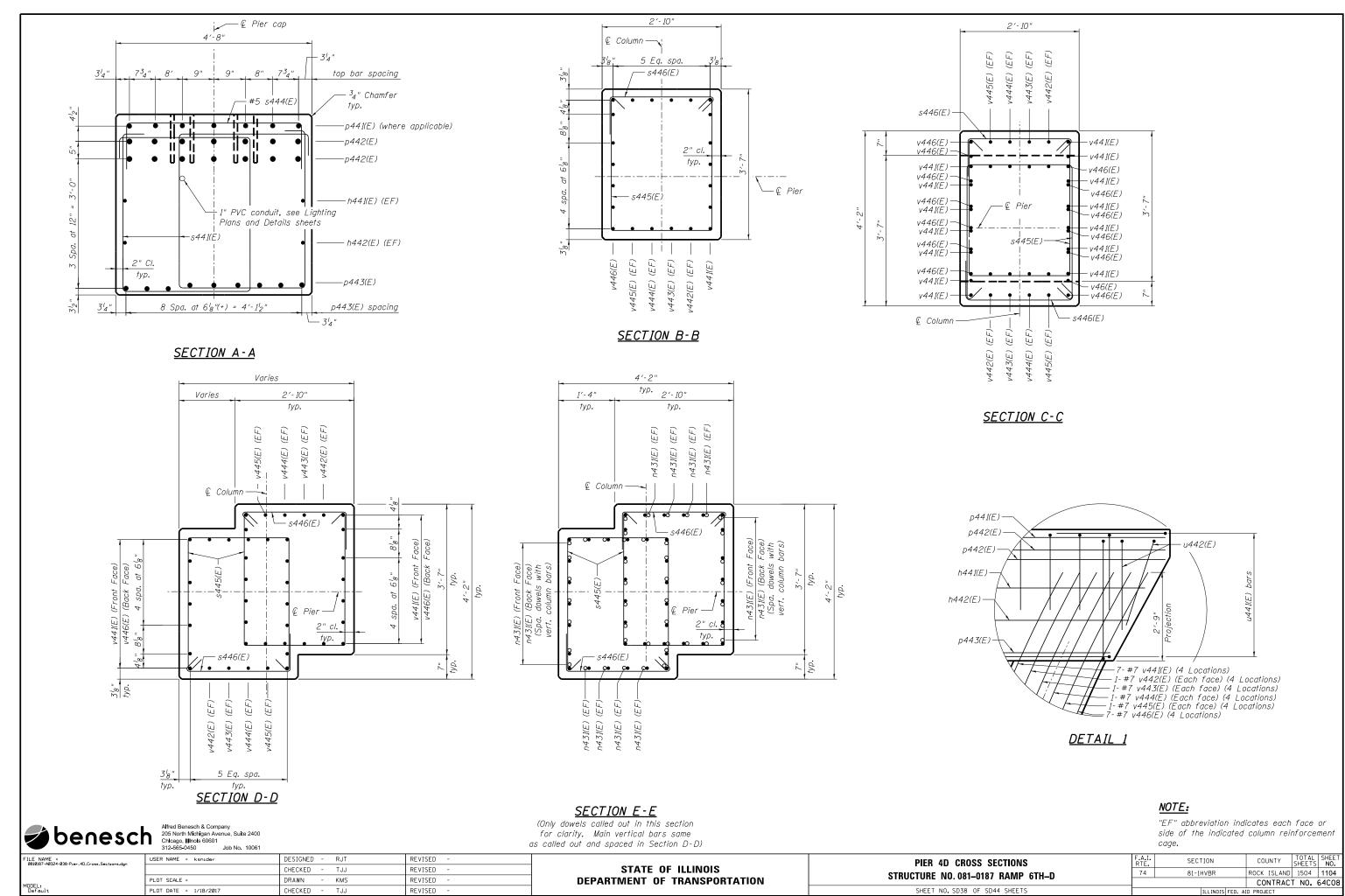
or cap surfaces.

Est. Length: 15 feet No. Production Piles: 15 No. Test Piles: 1

- © Pier & Brg.

STATE OF ILLINOIS
DEDARTMENT OF TRANSPORTATION





12:05:37 PM

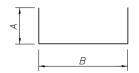
PIER 3D BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h431(E)	2	#5	32'-10"	
h432(E)	2	#5	31'-7'	
11132(2)			31 ,	
n431(E)	88	#7	8'-0"	
HIJILLY	- 00	" /	100	
p431(E)	7	#5	16 '- 11"	
p432(E)	14	#9	34'-1"	
D433(E)	9	#9	30'-4"	
p /00(L)				
s431(E)	62	#5	14'-6"	П
s434(E)	15	#5	8'-4"	
s435(E)	84	#5	9'-11"	
s436(E)	84	#5	3'-10"	L
t431(E)	34	#6	9′-8"	
t432(E)	42	#8	14'-8"	
u431(E)	11	#4	8'-4"	ш
u432(E)	4	#5	10'-4"	
v431(E)	28	#7	22'-4"	
v432(E)	8	#7	22'-4"	
v433(E)	8	#7	22'-5"	
v434(E)	8	#7	22'-5"	
v435(E)	8	#7	22'-5"	
v436(E)	28	#7	22'-6"	
w431(E)	29	#8	32′-6"	
			1	
Concrete St			Cu. Yd.	84.5
Reinforceme			Pound	15,320
Epoxy Coate				
Structure E			Cu. Yd.	20
Furn, Steel	Piles HP	14x/3	Foot	270
Driving Piles	5		Foot	270
Lact Dila C				
Pile Shoes	teel HP14	x73	Each Each	1 16

PIER 4D BILL OF MATERIAL

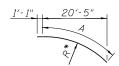
Bar No. Size Length Shape h441(E) 2 #5 32'-10" — n431(E) 88 #7 8'-0" — p441(E) 7 #5 8'-3" — p442(E) 14 #9 34'-1" — p443(E) 9 #9 30'-4" — s441(E) 14 #9 34'-1" — s441(E) 62 #5 14'-6" — s445(E) 88 #5 9'-11" — s445(E) 88 #5 3'-10" — t441(E) 34 #6 9'-8" — t441(E) 42 #8 14'-8" — v441(E) 4 #5 10'-4" — v445(E) 8 #7 23'-1" — v445(E) 8 #7 23'-2" — v445(E) 8 #7 23'-2" — <				_	
h441(E) 2 #5 32'-10" — h442(E) 2 #5 31'-7' — n431(E) 88 #7 8'-0" — p441(E) 7 #5 8'-3" — p442(E) 14 #9 34'-1" — p443(E) 9 #9 30'-4" — s441(E) 62 #5 14'-6" — s445(E) 88 #5 9'-11" — s445(E) 88 #5 9'-11" — s445(E) 88 #5 9'-11" — s445(E) 88 #5 3'-10" — u441(E) 11 #4 8'-4" — u441(E) 11 #4 8'-4" — u442(E) 8 #7 23'-2" — v443(E) 8 #7 23'-2" — v445(E) 8 #7 23'-2" — v445(E) 8 #7 23'-2" — v445(E) 8 <	Bar	No.	Size	Length	Shape
n43(E) 2 #5 31'-7' — n43(E) 88 #7 8'-0" — p441(E) 7 #5 8'-3" — p442(E) 14 #9 34'-1" — p443(E) 9 #9 30'-4" — s44(E) 7 #5 8'-4" — s445(E) 88 #5 9'-11" — s445(E) 88 #5 9'-11" — t441(E) 34 #6 9'-8" — t441(E) 34 #6 9'-8" — t442(E) 42 #8 14'-8" — v441(E) 11 #4 8'-4" — v442(E) 8 #7 23'-1" — v443(E) 8 #7 23'-2" — v445(E) 8 #7 23'-2" — v445(E) 8 #7 23'-2" — v445(E) 8 #7 23'-3" — w441(E) 29 #8<	h441(E)	2	#5	32'-10"	
n431(E) 88 #7 8'-0"				31'-7'	
p44I(E) 7 #5 8'-3" — p442(E) 14 #9 34'-1" — p443(E) 9 #9 30'-4" — s441(E) 62 #5 14'-6" _ s445(E) 88 #5 9'-11" _ s446(E) 88 #5 3'-10" _ t441(E) 34 #6 9'-8" _ t441(E) 42 #8 14'-8" _ u441(E) 41 #4 8'-4" _ u441(E) 4 #5 10'-4" _ v442(E) 8 #7 23'-1" _ v442(E) 8 #7 23'-2" _ v443(E) 8 #7 23'-2" _ v445(E) 8 #7 23'-2" _ v445(E) 8 #7 23'-2" _ v445(E) 8 #7 23'-2" _ v44		_		1 .	
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p442(E) 14 #9 34'-1" — p443(E) 9 #9 30'-4" — s441(E) 7 #5 8'-4" — s445(E) 88 #5 9'-11" — s445(E) 88 #5 9'-11" — s446(E) 88 #5 3'-10" — t441(E) 34 #6 9'-8" — t442(E) 42 #8 14'-8" — u441(E) 11 #4 8'-4" — v441(E) 4 #5 10'-4" — v442(E) 8 #7 23'-1" — v443(E) 8 #7 23'-2" — v445(E) 28 #7 23'-2" — w4	HTJIL	00	" /	0 0	
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s44!(E) 62 #5 14'-6" □ s444(E) 7 #5 8'-4" □ s445(E) 88 #5 9'-1!" □ s446(E) 88 #5 3'-10" □ t441(E) 34 #6 9'-8" — t441(E) 42 #8 14'-8" □ u441(E) 11 #4 8'-4" □ u442(E) 4 #5 10'-4" □ v442(E) 8 #7 23'-1" □ v443(E) 8 #7 23'-2" □ v445(E) 8 #7 23'-2" □ v445(E) 8 #7 23'-2" □ v446(E) 28 #7 23'-3" □ w441(E) 29 #8 32'-6" □ w445(E) 29					
\$444(E) 7 #5 8'-4"	p443(E)	9	#9	30'-4"	
\$444(E) 7 #5 8'-4"					
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t441(E) 34 #6 9'-8" — t442(E) 42 #8 14'-8" — u441(E) 11 #4 8'-4" — v441(E) 28 #7 23'-1" — v442(E) 8 #7 23'-2" — v443(E) 8 #7 23'-2" — v445(E) 8 #7 23'-2" — v446(E) 28 #7 23'-3" — w441(E) 29 #8 32'-6" — w441(E) 29 #8 32'-6" — Concrete Structures Cu. Yd. 84.9 Reinforcement Bars, Epoxy Coated Pound 15,370 Structure Excavation Cu. Yd. 96 Furn. Steel Piles HPl4x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HPl4x73 Each 1	s445(E)	88	#5	9'-11"	
t441(E) 34 #6 9'-8" — t442(E) 42 #8 14'-8" — u441(E) 11 #4 8'-4" — v441(E) 28 #7 23'-1" — v442(E) 8 #7 23'-2" — v443(E) 8 #7 23'-2" — v445(E) 8 #7 23'-2" — v446(E) 28 #7 23'-3" — w441(E) 29 #8 32'-6" — w441(E) 29 #8 32'-6" — Concrete Structures Cu. Yd. 84.9 Reinforcement Bars, Epoxy Coated Pound 15,370 Structure Excavation Cu. Yd. 96 Furn. Steel Piles HPl4x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HPl4x73 Each 1	s446(E)	88	#5	3'-10"	ا
t442(E) 42 #8 14'-8"					
t442(E) 42 #8 14'-8"	1441(E)	34	#6	9'-8"	
u441(E) 11 #4 8'-4" u442(E) 4 #5 10'-4" v441(E) 28 #7 23'-1" v443(E) 8 #7 23'-2" v445(E) 8 #7 23'-2" v445(E) 8 #7 23'-2" v446(E) 28 #7 23'-3" w441(E) 29 #8 32'-6" w441(E) <				14'-8"	1 1
v442(E) 4 #5 10'-4"					
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v443(E) 8 #7 23'-2" v444(E) 8 #7 23'-2" v445(E) 8 #7 23'-2" v446(E) 28 #7 23'-3" w441(E) 29 #8 32'-6" w441(E) 29 #8 32'-6" Concrete Structures Cu. Yd. 84.9 Reinforcement Bars, Epoxy Coated Pound 15.370 Structure Excavation Cu. Yd. 96 Furn. Steel Piles HP14x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HP14x73 Each 1					
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w441(E) 29 #8 32'-6" — w441(E) 29 #8 32'-6" — Concrete Structures Cu. Yd. 84.9 Reinforcement Bars, Epoxy Coated Pound 15,370 Structure Excavation Cu. Yd. 96 Furn. Steel Piles HP14x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HP14x73 Each 1		8	#7		
Concrete Structures Cu. Yd. 84.9 Reinforcement Bars, Epoxy Coated Structure Excavation Cu. Yd. 96 Furn. Steel Piles HP14x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HP14x73 Each 1	v446(E)	28	#7	23'-3"	/
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Concrete Structures Cu. Yd. 84.9 Reinforcement Bars, Epoxy Coated Structure Excavation Cu. Yd. 96 Furn. Steel Piles HP14x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HP14x73 Each 1	w441(E)	29	#8	32'-6"	
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Reinforcement Bars, Epoxy Coated Structure Excavation Furn. Steel Piles HP14x73 Driving Piles Test Pile Steel HP14x73 Foot Pound 15,370 Cu. Yd. 96 225 Foot 225 Foot 1		L			
Epoxy Coated Pound 15,370 Structure Excavation Cu. Yd. 96 Furn. Steel Piles HP14x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HP14x73 Each 1				Cu. Yd.	84.9
Structure Excavation Cu. Yd. 96 Furn. Steel Piles HP14x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HP14x73 Each 1				Pound	<i>15,370</i>
Furn. Steel Piles HP14x73 Foot 225 Driving Piles Foot 225 Test Pile Steel HP14x73 Each 1	Ctrustura E	voquation		Cu Vd	06
Driving Piles Foot 225 Test Pile Steel HP14x73 Each 1					
Test Pile Steel HP14x73 Each 1			14X13		
Pile Shoes Each 16		teel HP14	x73		
	Pile Shoes			Each	16

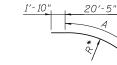
BENT BAR DETAILS



BARS s434(E). t432(E). <u>u431(E). u432(E)</u> s444(E), †442(E), u441(E) & u442(E)

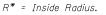
BAR	Α	В	LENGTH
s434(E)	2'-0"	4'-4"	8′-4"
t432(E)	2'-6"	9′-8"	14'-8"
u431(E)	2'-0"	4'-4"	8'-4"
u432(E)	3'-0"	4'-4"	10'-4"
s444(E)	2'-0"	4'-4"	8'-4"
t442(E)	2'-6"	9′-8"	14'-8"
u441(E)	2'-0"	4'-4"	8'-4"
u442(E)	3′-0"	4'-4"	10'-4"





BARS v150(E)-v155(E) BARS v160(E)-v165(E)

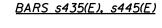
BAR	Α	R*
v431(E)	21'-3"	43′-1 ³ 8
v432(E)	21'-3"	42'-7 ⁷ 8
v433(E)	21'-4"	42′-2 ³ 8
v434(E)	21'-4"	41'-8 ³ 4
v435(E)	21'-4"	41'-34'
v436(E)	21'-5"	40′-9 ⁵ 8

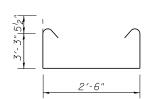


BAR	Α	R*
v441(E)	21'-3"	43'-1 ³ 8
v442(E)	21'-3"	42'-77
v443(E)	21'-4"	42'-23
v444(E)	21'-4"	41'-834
v445(E)	21'-4"	41'-34
v446(E)	21'-5"	40′-9 ⁵ 8

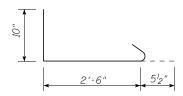
 R^* = Inside Radius.



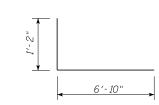




3'-1'2" BARS \$431(E) & \$441(E)



BARS \$436(E), \$446(E)



BAR n431(E)

PIER CONCRETE FINISH NOTES

If form ties are used in forming the pier, arrange ties to be regularly spaced and in a consistent geometric grid pattern. Do not locate ties at edges of concrete rustucations.

Following form removal, a rubbed surface finish in accordance with Article 503.15 (b) of the Standard Specifications shall be required but with the following additional requirements:

- 1. Demonstrate hole and void patching operations in accordance with Article 503.15 (b) of the standard Specifications on a four foot section of vertical pier concrete located in an inconspicuous area. Begin patching demonstration by using a mortar mix comprised of 1 part white cement, 2 parts standard portland cement, 6 parts mortar sand, and water. The quantity of water used shall produce a mortar consistency as dry as possible to use effectively.
- 2. When patching test areas have set, saturate with water and rub with a fine carborundum stone until surfaces are smooth in texture. Remove loose powder and other contaminants by rubbing with burlap and rinsing with water. After surfaces have dried, patch color and texture of surfaces will be reviewed by the engineer. Patches should match or be slightly lighter than surrounding concrete, If results are unsatisfactory, adjust patching mortar mix proportions and perform another demonstration until results are deemed satisfactory by the engineer.
- 3. Use the patching mortar mix proportions that are approved by the engineer as a result of the satisfactory demonstration. Do not use patching mortar that is more than 1 hour old.
- 4. Finished pier concrete shall be smooth and show no wood grain or other texture from the face of the forms used. All costs for repair or covering wood grain or other textures on these surfaces shall be the responsibility of the Contractor.

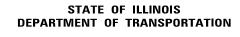
NOTE:

All dimensions are out to out.

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 800601 Chicago, Illinois 800601

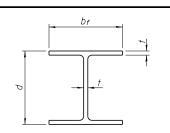
FILE NAME = Ø8lØ187-AØ324-Ø39-Pier_Bar_List.dgn
MODEL: Default

512-505-0450 00D140. 10001		
USER NAME = ksnider	DESIGNED - DTS/DMS	REVISED -
	CHECKED - RJT	REVISED -
PLOT SCALE =	DRAWN - KMS	REVISED -
PLOT DATE = 1/18/2017	CHECKED - RJT	REVISED -
	-	



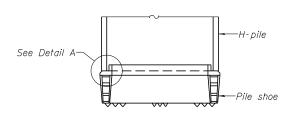
 						F MATI	
	SHEET	NΩ.	SD39	ΩF	SD44	SHEETS	

F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
74	81-1HVBR		ROCK ISLAND	1504	1105
			CONTRAC	T NO.	64C08
	ILLINOIS	FED. A	ID PROJECT		

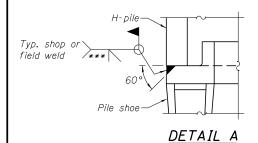


STEEL PILE TABLE

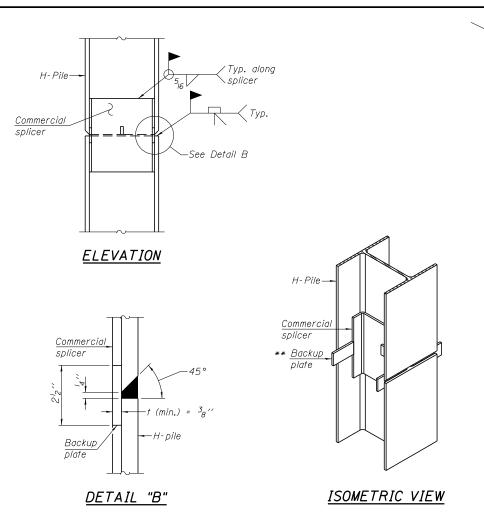
Designation	Depth d	Flange width b _f	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 4 ′′	14 ⁷ 8′′	¹³ 16 ′′	30′′
x102	14′′	14 ³ 4 ′′	1/16 ''	30′′
x89	13 ⁷ 8′′	14 ³ 4′′	58′′	30′′
x73	13 ⁵ 8′′	14 ⁵ 8 ′′	2"	30′′
HP 12x84	1214''	1214''	1/16 ′′	24''
x74	1218''	1214''	58′′	24''
x63	12''	12 % ′′	2"	24''
x53	11 ³ 4′′	12''	⁷ 16 ′′	24''
HP 10x57	10′′	1014''	916 ′′	24''
x42	934"	10 % ′′	7 ₁₆ ′′	24''
HP 8x36	8''	818''	⁷ 16 ′′	18′′



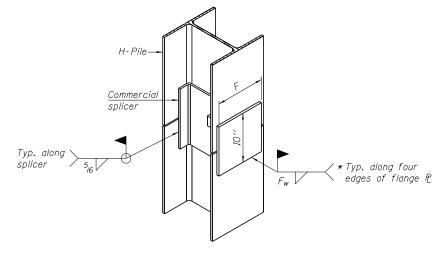
ELEVATION



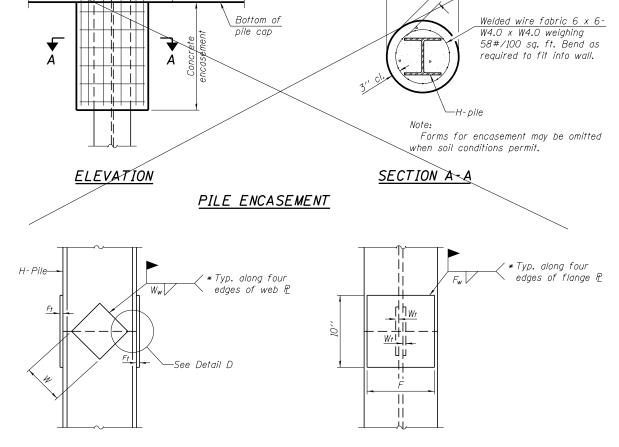
H-PILE SHOE ATTACHMENT



WELDED COMMERCIAL SPLICE

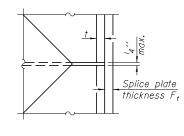


ISOMETRIC VIEW



ELEVATION

END VIEW



DETAIL D

Designation	F	F _t	F _w	W	W _f	W _w
HP 14x117	12 ½''	1''	⁷ 8′′	734''	⁵ 8′′	12''
x102	1212''	⁷ 8′′	34''	734''	58′′	12"
x89	1212''	34''	116 ′′	7 ³ 4′′	58′′	2"
x73	1212''	58′′	916 ′′	7 ³ 4′′	58′′	2"
HP 12x84	10′′	78′′	6 ′′	612''	58′′	2"
x74	10′′	78′′	^{II} 16 ′′	612"	58′′	12"
х63	10′′	58′′	2"	612''	2"	38''
x53	10′′	58′′	2"	612''	2"	38''
HP 10x57	8''	34''	916 ''	54"	2"	38''
x42	8''	58′′	916 ′′	54''	2"	38''
HP 8x36	7''	58′′	7 ₁₆ ′′	414''	2"	38''

WELDED PLATE FIELD SPLICE

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds ${}^{l}_{4}{}^{\prime\prime}$ from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.

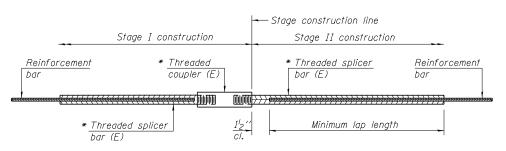
The steel H-piles shall be according to AASHTO M270 Grade 50.



)			
	F-	HP	

1-27-12 *** Weld size per pile shoe manufacture	r (⁵ 16′′ min.).
---	------------------------------

_	012 000 0100 0001101 10001								
ILE NAME = 0810187-40324-040-HP_Pile_Details.don	USER NAME = ksnider	DESIGNED - RJT	REVISED -		HP PILE DETAILS	F.A.I.	SECTION	COUNTY TOTAL SHEET	1 <u>0</u>
colors reserve to the same same sage.		CHECKED - AJK	REVISED -	STATE OF ILLINOIS	STRUCTURE NO. 081-0187 RAMP 6TH-D	74	81-1HVBR	ROCK ISLAND 1504 1106	2
ionei .	PLOT SCALE =	DRAWN - KMS	REVISED -	DEPARTMENT OF TRANSPORTATION	SINUCIUNE NU. UOI-UIO/ NAIVIP DIN-D			CONTRACT NO. 64C08	ξ
Default	PLOT DATE = 1/18/2017	CHECKED - AJK	REVISED -		SHEET NO. SD40 OF SD44 SHEETS		ILLINOIS FED. A	ID PROJECT	



STANDARD BAR SPLICER ASSEMBLY

		Minim	num Lap Len	gths		
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6
3, 4	1'-5''	1'-11''	2'-1''	2'-4''	2'-7''	2'-11''
5	1'-9''	2'-5"	2'-7''	2'-11''	3'-3''	3′-8′′
6	2'-1''	2'-11''	3'-1''	3′-6′′	3′-10′′	4'-5''
7	2'-9''	3'-10''	4'-2"	4'-8''	5′-2′′	5′-10′′
8	3′-8′′	5′-1′′	5′-5′′	6'-2''	6′-9′′	7′-8′′
9	4'-7''	6′-5′′	6′-10′′	7′-9′′	8'-7''	9'-8''

Table 1: Black bar, 0.8 Class C

Table 2: Black bar, Top bar lap, 0.8 Class C

Table 3: Epoxy bar, 0.8 Class C

Table 4: Epoxy bar, Top bar lap, 0.8 Class C

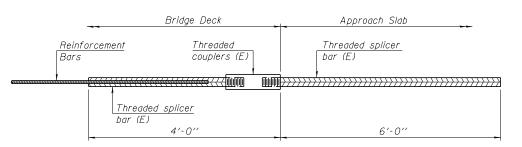
Table 5: Epoxy bar, Class C

Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Table for minimum lap length



BAR SPLICER ASSEMBLY FOR #5 BAR ON INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

No, required =

USER NAME = ksnider

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chlcago, Illinois 60601 Job No. 10061

PLOT DATE = 1/18/2017

BSD-1

DESIGNED -CHECKED -DRAWN

CHECKED -

1-27-12

RJT	REVISED	-	
AJK	REVISED	-	
KMS	REVISED	-	
AJK	REVISED	-	

Threaded Form coupler (E) <u>Template</u> Threaded splicer bar (E) <u>''A ''</u> Stage construction line - Positive stop or end of approach slab Threaded coupler (E) Threaded splicer Form bar (E) "B"

INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.

6'-0''

Threaded splicer

bar (E)

Approach slab

BAR SPLICER ASSEMBLY FOR

#5 BAR ON STUB ABUTMENTS

No. required = 33

1'-4 1/2"

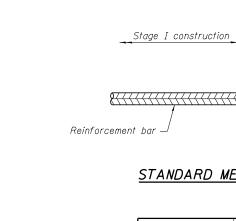
Threaded

Threaded splicer bar (E)

couplers (E)

Abutment

hatch block



STANDARD MECHANICAL SPLICER

Stage line

if applicable

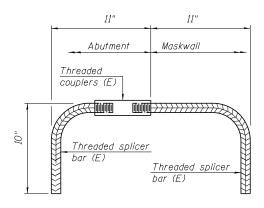
Stage II construction

Mechanical

coupler (E)

└─ Reinforcement bar

Location	Bar size	No. assemblies required



BAR SPLICER ASSEMBLY FOR #5 BAR ON MASKWALL

No. required = 17

NOTES

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements

for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

							ı
OTATE OF HUMOIO	BAR SPLICER ASSEMBLY DETAILS	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	l
STATE OF ILLINOIS	STRUCTURE NO.081-0187 RAMP 6TH-D	74	81-1HVBR	ROCK ISLAND	1504	1107	ı
DEPARTMENT OF TRANSPORTATION	OTHOUTONE NO. 001-0107 HAMII OTH-D			CONTRAC	T NO.	64C08	ı
	SHEET NO. SD41 OF SD44 SHEETS		ILLINOIS FED. A	ID PROJECT			Г

OUTE I-74 DESCRIPTION New I-74 Bridge Over Mississippi I			_ LO		ВҮ	
ECTION LOCATION (N=564389.584, E=2459470.27	3), SE	C.32,		18N, F	CORE	, 4" PM S
COUNTY Rock Island CORING METHOD NQ Core			R E	R		T
STRUCT. NOCORING BARREL TYPE & SIZE NQ Wireline Station	D E P	C O R	0 V E	Q D	T I M E	R E N G
BORING NO. PRMPD-04 Top of Rock Elev. <u>556.90</u> ft Station Begin Core Elev. <u>554.70</u> ft	T H	E	R Y			T H
Offset Ground Surface Elev. 570.50 ft	(ft)	1	(%)		(min/ft)	(tsf)
		Run 5	100	96	2.5	
	.10	+				
SHALE - medium to dark gray, soft, rock-like, thin bedded to laminated, smooth planar fractures at low to medium angles, with large limestone clasts	_	-				
531	20 —	1				
I IMESTONE - medium to dark gray, fine to coarse grained, clastic calcarenite at	<u>.20_</u> -41	0				
39.3'-41.6', dense fine limestone at 41.6'-41.8', hard, thin bedded, fresh		-				
526	70 -					
End of Boring						
	_	-				
	_	1				
	-4:	5				
	_	7				
	_					
	_	-				
	_	7				
		1				
	-6	0				
		7				
	_					
	_	-				
	_	1				
	-5	5				
Color pictures of the coresYes						
Cores will be stored for examination until The "Strength" column represents the unlaxial compressive strength of the core sar	nle /A	STM I	7.2029	1)		

301				Ne	w I-74	Page 1 of DIL BORING LOG Bridge Over Mississippi River - Illinois Approach Date 9/4/07 LOGGED BY SL
SECTION		_ 1	OCA	TION_	(N=56	4389.584, E=2459470.273), SEC.32, TWP.18N, RNG.1W, 4 th P
COUNTY Rock Island D	RILLING	ME	THOE		Н	SA, CME 550X HAMMER TYPE CME AUTOMATIC
STRUCT. NOStation	<u> </u>	D E P T	B L O W	U C S	M O I S	Surface Water Elev ft Stream Bed Elev ft
Station		Н	s (/6")	Qu (tsf)	Т	Groundwater Elev.: First Encounter 559.5 ft ↓ Upon Completion ft After Hrs. ft
CLAY - greenish gray to orange brown, some silt, some sand/gravel in matrix, slightly to		_	4			
medium plastic, medium stiff to stiff, moist			4 7	1.2 B		
		\exists	4			
- very stiff to stiff layer between '-5'		-5	6	0.7 B	12.0	
		1	4	0.7	_	
		\exists	5	В		
CLAY - medium to dark gray,	561.50	4	3	0.6	16.5	
ome sand, trace gravel, slightly o medium plastic, soft to medium tiff, moist		-10	5	В		
3		_	2 5	0.4 B		
	556.90	\dashv		В		
VEATHERED SANDSTONE		-15	50/4"			
	554.70					

The Unconfined Compressive Strength (UCS) Fallure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

	llinois Dep of Transpo vision of Highways	rtation	ROCK	CO	RE L	OG	j		Date
		_ DESCRIPTION	New I-74 Bridge Ov	ver Mississ Approach	ippi River -	Illinois	_ L0		
			ON (N=564389.584						
			NQ Core				R		СО
							E	R	Ī
STRUCT. NO	101:11.0	CORING BA Core Diam	RREL TYPE & SIZE eter 1.8	in	E	o	O V	Q	N
BORING NO	PRMPD-04	Top of Roo	ck Elev. 556.90 e Elev. 554.70	ft	T H	E	E R Y	D	E
Offset	ce Elev. 570.50	 ft			(ft		(%)	(%)	(mir
SANDSTONE - I	ight gray to light bro	ownish gray, fine gr	ained, with occasion	al to minor	554.70	Run 1	99	73	0.8
horizontal to very	y low angle planar t	o slightly irregular s	sandy rough fractures	s, fresh to	_	╢.			
	t 17.3' with smooth	planar fracture			_	7			
					_				
					2	0			
					-	_	0.1	49	0.
-day-like chale s	seem at 21.5' with r	olanar horizontal fra	cture at the seam, cl	oselv	_	Run 2	94	49	0.
spaced black bar	nding from 21.5' to	22.8', occasional ro	ck-like shale clasts		_	1			
					_	1			
					_	+			
					2	5			
					_	Run	85	78	1
-clay-like parting	gs/seams with smoo	oth planar fractures	at 26.5' and 29.2'		_	3			
					-	-			
					541.30 —	-			
LIMESTONE - gr	ray, fine to medium	grained, hard, thin	to medium bedded, o	occasional					
almy like (possible	v come healed to n	artially healed): frag	ngs and infilling, pred ctures along shale pa ightly irregular to irre	rtings are		Ĭ			
jagged, slightly w	veathered to fresh e	except at vugs		guiai ana	_	Run 4	100	100	0.8
-vuggy with oper	n and partially filled	voids at 31.5'-32.4	r		_	1			
					-	-			
					_	7			
					3	5			
						-			L

BORING NO. PRMPD-04 Station: 429+09.99 Offset: 13.14' Rt.

Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10061

E NAME = 810187-40324-042-Soil_Boring_l.dgn	USER NAME = ksnider	DESIGNED - DTS	REVISED -
olelo, Hessey eve soll-boringstagn		CHECKED - AJK	REVISED -
05	PLOT SCALE =	DRAWN - KMS	REVISED -
DEL: lefault	PLOT DATE = 1/18/2017	CHECKED - AJK	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SOIL BORING LOGS (1 OF 3)	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 081-0187 RAMP 6TH-D	74	81-1HVBR	ROCK ISLAND	1504	1108
OTHOUTONE NO. 001-0107 HANN OTH-D			CONTRAC	T NO.	64C08
SHEET NO.SD42 OF SD44 SHEETS		ILLINOIS FED. A	ID PROJECT		

	c:\pwise_work\do_not_delete\dms02476\0810187-A0324-043-Soil_Boring_2.dgn
	ö

New I-74 Bridge Over Mississippi River - Illinois LogGED BY	Illinois Depai of Transporta Division of Highways	tme	nt n		SC	OIL BORIN	G LOG	Page
COUNTY Rock Island DRILLING METHOD HSA. CME 550X HAMMER TYPE CME AUT				Ne N	ew I-74	Bridge Over Mississipp Approach	i River - Illinois	
STRUCT. NO	SECTION		LOCA	TION	(N=56	4029.213, E=2459513.	152), SEC. 32, TWP.	18N, RNG.
BORING NOPRMPD-05	COUNTY Rock Island DRILL	NG ME	THOE	o	Н	SA, CME 550X	_ HAMMER TYPE	CME AUT
BORING NOPRMPD-05	STRUCT. NO	E	L	C	0	Surface Water Elev. Stream Bed Elev.	ft ft	
After Hrs. ft	BORING NO. PRMPD-05 Station	T	w		S	Groundwater Elev.:	564.1 ft ¥	
SILT - black, with rubble (FiLL) SILT - black, with rubble (FiLL) 6 5 5 CLAY - medium gray to orange brown, slightly to medium plastic, medium stiff, moist 771.60 2 1 0.5 24.9 - 1 0.5 24.9 - 2 0.7 38.9 1 B Attempted Shelby tube at 3.5-1.05, no recovery SAND - red brown, fine grained, oose, wet Attempted Shelby tube at 11'-13', no recovery; followed up with SPT] Attempted Shelby tube at 11'-13', no recovery; followed up with SPT] SHALE - green gray, clayey, severely weathered	Ground Surface Elev. 575.10 f	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	
CLAY - medium gray to orange prown, slightly to medium plastic, medium stiff, moist Attempted Shelby tube at 3.5'-10.5'; no recovery] SAND - red brown, fine grained, cose, wet Attempted Shelby tube at 11'-13'; no recovery; followed up with SPT] Attempted Shelby tube at 11'-13'; no recovery; followed up with SPT] SHALE - green gray, clayey, severely weathered -10 561.10 1 56	course 574.	10						
2 2 3 3 3 3 3 3 3 3	SILT - black, with rubble (FILL)	_	6					
DLAY - medium gray to orange rown, slightly to medium plastic, medium stiff, moist 2			5					
Attempted Shelby tube at .5.5'-10.5', no recovery] Attempted Shelby tube at 11'-13', so recovery; followed up with series or recovery; followed up with series	CLAY - medium gray to orange	30	2					
Attempted Shelby tube at 1.5°-10.5°; no recovery] SAND - red brown, fine grained, pose, wet Attempted Shelby tube at 11°-13°; 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		-5			24.9			
Attempted Shelby tube at 1.5°-10.5°; no recovery] SAND - red brown, fine grained, pose, wet Attempted Shelby tube at 11°-13°; 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		_	1					
Attempted Shelby tube at .5:-10.5'; no recovery] 566.60 AND - red brown, fine grained, bose, wet Attempted Shelby tube at 11'-13'; 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		_			38.9			
Attempted Shelby tube at 11'-13'; or recovery; followed up with 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	3.5'-10.5'; no recovery] 566.6	60						
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		-10						
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		<u>*</u> _						
SHALE - green gray, clayey, severely weathered 561.10 1 1 23.6 -15 13 B 12 -12	no recovery; followed up with	_	1					
SHALE - green gray, clayey,	51 IJ		1					
SHALE - green gray, clayey, 3 1.4 23.6 severely weathered 12 12 12 36 Sorehole continued with rock 558.40 50/1" B		_						
30rehole continued with rock 558.40 41 1.4 B 50/1" B	SHALE - green gray, clayey,	_	-		23.6			
Borehole continued with rock 558.40 50/1" B		\Box		1.4	-			
	Borehole continued with rock							

Of Irans	Department sportation ROCK	COREL		,		Dat
JCI 174	New I-74 Bridge Over DESCRIPTIONAp	Mississippi River	- Illinoi:	s		
					OGGE	
	LOCATION (N=564029.213, E=	=2459513.152), 8	EC. 32	R.	18N, I	KN
COUNTY Rock Island	CORING METHOD NQ Core			E	R	
STRUCT. NO.	CORING BARREL TYPE & SIZE		D C	CO	à	
Station	Core Diameter 1.8 Top of Rock Elev. 561.60		E O	V	b	
BORING NO. PRMPD- Station	550 40	ft	T E	R		
Offset	75.10 ft	1	ft) (#)	1	(%)	(1
SANDSTONE - medium gray,	very fine grained, silt in matrix, abundant shale	558.40 —	Rur	1	23	f
partings, conglomeratic at 17.	,	557.10	∃ ¹			
	ed, with occasional to some thin green shale part ard, thin to medium bedded, predominantly horizo		-			
very low angle fractures, plan	ar to slightly irregular, smooth to slightly rough, fre	esh -	ゴ			
		_	-20			
		_	Run	100	95	r
		_	∃ ′			
			-			
		_	\exists			
		-	\dashv			
		_	25			
			Run	97	87	
			_ 3			
		_				
		_	\dashv			
allahib yayah for stores	a studelitas et 20 21 20 61	_	\exists			
-slightly rough fractures acros	s stylolites at 28.3-30.6		30			
		_		100	100	
thick bedded, occasional style	olites at 30.6'-35.6'		Run 4	100	100	
minor nitting with some "hirds	eye" texture from 32.1' to 35.6'	_	-			
Timor pitting with some blide	oye toxule nom oz. r to co.c				i	
			_			
		_				
		-	35			
		_	Run 5	100	84	

DOUTE	1.74	New I-74 Bridge Over Mississippi DESCRIPTION Approach	River - I	llinois			Date!	
,								
		LOCATION (N=564029.213, E=2459513.15		2.32,		<u>18N, F</u>	т	
COUNTY R	ock Island C	ORING METHOD NQ Core			R	R	CORE	1
STRUCT. NO Station	-187-88,7 -	CORING BARREL TYPE & SIZE NQ Wireline Core Diameter 1.8 in	E	CO	0 V	Q :	T I M	E
Offset		Begin Core Elev. <u>558.40</u> ft	P T H	R	E R Y	. D	E	T H
	e Elev. 575.10	with occasional to some thin green shale partings	(ft)	(#)	(%)	(%)	(min/ft)	(ts
very low angle fra (continued) -occasional soft along shale, occa -green rock-like	ctures, planar to rock-like green sh sional pitting, at shale seam with 8	35° fracture at 40.3'-40.8' ined, occasional shale partings	-40					
End of Boring		532	.50					
			_					
			45					
			-					
			크					

Color pictures of the cores Yes

Cores will be stored for examination until

The "Strength" column represents the unlaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)

-55

BORING NO. PRMPD-05 Station: 425+50.54 Offset: 36.85' Lt.



MODEL: Default

USER NAME = ksnider	DESIGNED - RJT	REVISED -
	CHECKED - AJK	REVISED -
PLOT SCALE =	DRAWN - KMS	REVISED -
PLOT DATE = 1/18/2017	CHECKED - AJK	REVISED -

STATE OF	F ILLINOIS
DEPARTMENT OF	TRANSPORTATION

SOIL BORING LOGS (2 OF 3)	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
STRUCTURE NO. 081-0187 RAMP 6TH-D	74	81-1HVBR	ROCK ISLAND	1504	1109	
			CONTRAC	T NO.	64C08	
SHEET NO. SD43 OF SD44 SHEETS		ILLINOIS FED. AI	D PROJECT			

dan
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Bo
Soil
1-044-Soil_Boring_
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BBS, form 138 (Rev. 8-99)

ROUTE	DESCR	IPTIO	Ne ^s	w I-74	Bridge Over Mississippi River - Illinois Approach LOGGED BY
ECTION	ı	LOCA.	rion_	(N=56	4254.16, E=2459482.275), SEC. 32, TWP. 18N, RNG. 1W
COUNTY Rock Island DRILL	ING ME	THOE	_	Н	SA, CME 550X HAMMER TYPE CME AUTO
Station	D E	B	U C	M	Surface Water Elev ft Stream Bed Elev ft
ORING NO. PRMPD-06	P	o W	S	S	Groundwater Elev.:
Station	Н	S	Qu	T	First Encounter559.9 ft ▼ Upon Completion ft
AVEMENT - asphalt, concrete,	ft (ft)	(/6")	(tsf)	(%)	After Hrs ft
nd base course (12" thick) 572 AND - light to medium brown,	.40	3			
ne to medium grained, loose, noist	_	2			
					-
LAY - orange brown to greenish	.40	2	0.5	19.1	
ray, some sand and gravel, ome silt, medium plastic, medium	5	2	В		
iff, moist	_	WOH.			
	\exists	2	0.8 B		
oft	\exists	WOH 1	0.4	34.4	
	-10	1	В		
[Dry unit weight = 69.6 pcf]	\exists		1		
	\dashv		0.6 B	48.3	
medium brown, fine grained and, some clay, some silt, at	_				
2.7' 559. EATHERED LIMESTONE -	60	12 46	_		
ugered through	15\	50/2"/	\dashv	_	
557. prehole continued with rock	50				
oring.	_				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

Illinois Dep of Transpo	rtation ROCK CO	RE L	90	ì		Page
Division of Highways JCI	New I-74 Bridge Over Missis:	einni Diver	Ilinois		I	Date
ROUTEI-74	DESCRIPTIONApproach	sippi Kive: -	IIIIIOI	_ L0	OGGE	D BY
SECTION	LOCATION (N=564254.16, E=245948	2.275), SEC	.32,	TWP. 1	8N, R	NG. 1
COUNTYRock Island CO	RING METHOD NQ Core			R	R	СО
STRUCT. NO.	CORING BARREL TYPE & SIZE NQ Wit	eline D	С	CO	à	T
Station	Core Diameter1.8 in	E	0	V		M
BORING NO. PRMPD-06 Station	Top of Rock Elev. 559.60 ft Begin Core Elev. 557.50 ft	P	R	E R	D .	E
Offset Ground Surface Elev. 573.40	_	H (ft)	(#)	Y (%)	(%)	(mir
LIMESTONE - gray, fine to medium	grained, occasional to some stylolites, hard, pitted		Run	87	38	5.8
	low angle fractures, primarily planar to slightly th occasional rough fractures, fresh	_	1		L	
		_	Run 2	91	51	1.9
- "birdseye" texture at 18.2'-19.0'		-				
-pitted, locally vuggy, few stylolites	at 19'-20 7'		1			
picou, rodany raggy, row etyronico	. 10-20.1	20	1			
			Run 3	100	72	2
		_				
		_				
		-25				
			Run 4	100	83	2
		_				
		544.00				
LIMESTONE - medium gray, fine to	coarse, pitted, "birdseye" texture, stylolitic, thin to	-30				
occasional rock-like shale clasts to 2 shale clasts, partings, and seams, fro	ged horizontal to very low angle fractures, " elongated, locally large clay-like to soft rock-like esh	_	Run	90	79	
,, p		_	5	30	"	,
		35				

Illinois Department of Transportation

ROCK CORE LOG

Page 3 of 3

Date ___9/5/07__

New I-74 Bridge Over Mississippi River - Illinois Approach __ DESCRIPTION___ LOGGED BY SL LOCATION (N=564254.16, E=2459482.275), SEC. 32, TWP. 18N, RNG. 1W, 4th PM CORE S

COUNTY Rock Island CORING METHOD NQ Core STRUCT. NO. ___ | CORING DARKEL | | 1.8 | in | 1. BORING NO. PRMPD-06 Ground Surface Elev. 573.40 ft (ft) (#) (%) (%) (min/ft) (tsf) LIMESTONE - medium gray, fine to coarse, pitted, "birdseye" texture, stylolitic, thin to medium bedded, irregular rough/jagged horizontal to very low angle fractures, occasional rock-like shale clasts to 2" elongated, locally large clay-like to soft rock-like shale clasts, partings, and seams, fresh (continued) Run 99 83 0.7

-abundant shale and sandstone clasts and occasional shale partings, localized deep angular pitting, locally vuggy LIMESTONE -gray, fine to medium grained, abundant green soft rock-like to clay-like shale partings and matrix infilling; fractures horizontal to 20° angle, fractures along shale partings is slight to moderately irregular, slightly rough

-40.4' to 41.4' has brecciated appearance -41.4' to 42.7' appears to be shale partings deformed by limestone clasts
End of Boring 530.70

Color pictures of the cores _____Yes
Cores will be stored for examination until____ The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BORING NO. PRMPD-06 Station: 427+75.96 Offset: 9.61' Lt.

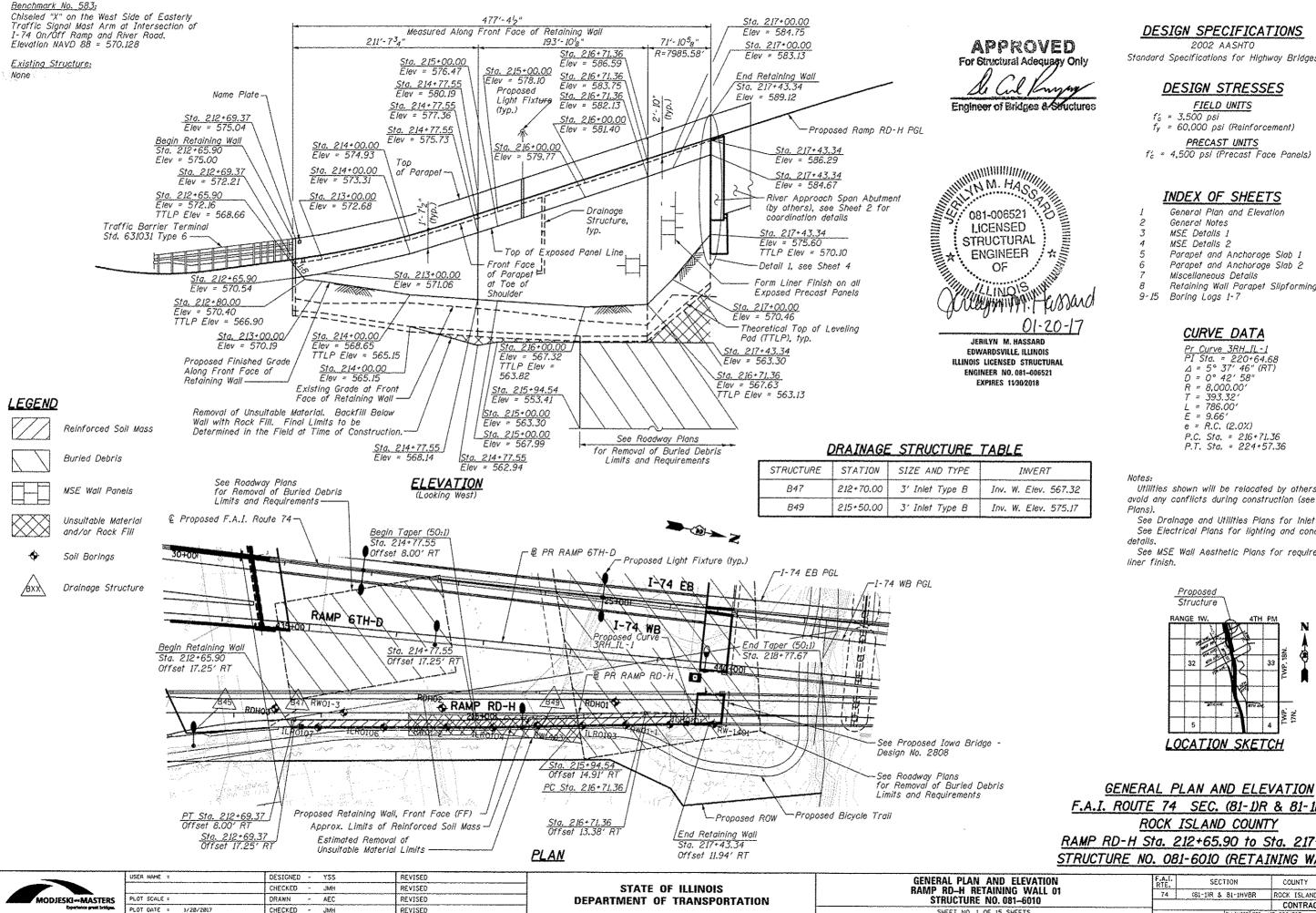
Alfred Benesch & Company
205 North Michigan Alvenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10061

FILE NAME = 0810187-A0324-044-Soil_Boring_3.dgn

USER NAME = ksnider	DESIGNED -	-	RJT	REVISED -
	CHECKED -	-	AJK	REVISED -
PLOT SCALE =	DRAWN -	-	KMS	REVISED -
PLOT DATE = 1/18/2017	CHECKED -	-	AJK	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION COUNTY SOIL BORING LOGS (3 OF 3) 74 81-1HVBR ROCK ISLAND 1504 1110 STRUCTURE NO. 081-0187 RAMP 6TH-D CONTRACT NO. 64CO8 SHEET NO. SD44 OF SD44 SHEETS



Standard Specifications for Highway Bridges

f'_c = 4,500 psi (Precast Face Panels)

Retaining Wall Parapet Slipforming Option

Utilities shown will be relocated by others to avoid any conflicts during construction (see Utility

See Drainage and Utilities Plans for Inlet details. See Electrical Plans for lighting and conduit

See MSE Wall Aesthetic Plans for required form

LOCATION SKETCH

F.A.I. ROUTE 74 SEC. (81-1)R & 81-1HVBR

RAMP RD-H Sta. 212+65.90 to Sta. 217+43.34 STRUCTURE NO. 081-6010 (RETAINING WALL 01)

PLOT DATE = 1/28/2817 CHECKED - JMH SHEET NO. 1 OF 15 SHEETS

COUNTY ROCK ISLAND 1504 1111 CONTRACT NO. 64CO

GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Wall stations and offsets are given to the front face (FF) of the wall and are measured from the Ramp RD-H baseline, except as noted. FF of the wall is to be considered edge of panel or form liner.
- 3. See Special Provision for Mechanically Stabilized Earth Retaining Walls for design and construction requirements.
- Wall construction shall not begin until after ground improvement for the unsuitable material and debris has been completed in the area of the new walls.

MSE WALL SETTLEMENT

1. The Top of Exposed Panel Elevations shown on these plans are final elevations after any settlement. The MSE wall supplier is alerted to to the fact that 4.0 inches of settlement are anticipated from Sta. 212+65.90 to Sta. 217+43.34 and shall take appropriate measures to accommodate the settlement in the wall design.

CONTRACTOR COORDINATION REQUIREMENTS

"Contractor" (responsible for construction of SN 081-6010) shall coordinate with "Bridge Contractor" (responsible for construction of the River Approach Bridge in a separate contract). Construction of the wall near the bridge abutment shall follow the steps outlined below:

$\underline{\textit{CONSTRUCTION SEQUENCE}}$

- 1. Contractor shall construct complete Rock Fill ground improvement within the limits shown in the plans or as directed by the Engineer.
- 2. Contractor shall construct MSE wall and place backfill up to the elevation of the bottom of abutment and wingwalls. Note that the abutment and wingwalls are adjacent to, but outside the limits of the MSE wall.
- 3. Bridge Contractor shall drive piles and construct abutment, wingwalls, and maskwalls.
- 4. Contractor shall resume and complete construction of MSE Walls, placement of backfill, and construction of coping.

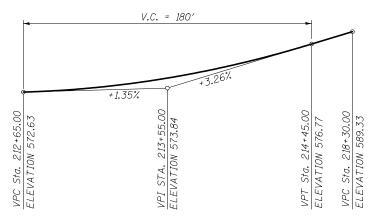
TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Structure Excavation	Cu. Yd.	270
Removal and Disposal of Unsuitable Material for Structures	Cu. Yd.	177
Concrete Superstructure	Cu. Yd.	263.7
Protective Coat	Sq. Yd.	603
Reinforcement Bars, Epoxy Coated	Pound	40,100
Name Plates	Each	1
Mechanically Stabilized Earth Retaining Wall	Sq. Ft.	5,612
Rock Fill	Cu. Yd.	243
· ·		

STATION 212+65.90
BUILT 201_ BY
STATE OF ILLINOIS
F.A.I. RT. 74
SEC. (81-1)R & 81-1HVBR
LOADING HS-20
STR. NO. 081-6010

NAME PLATE

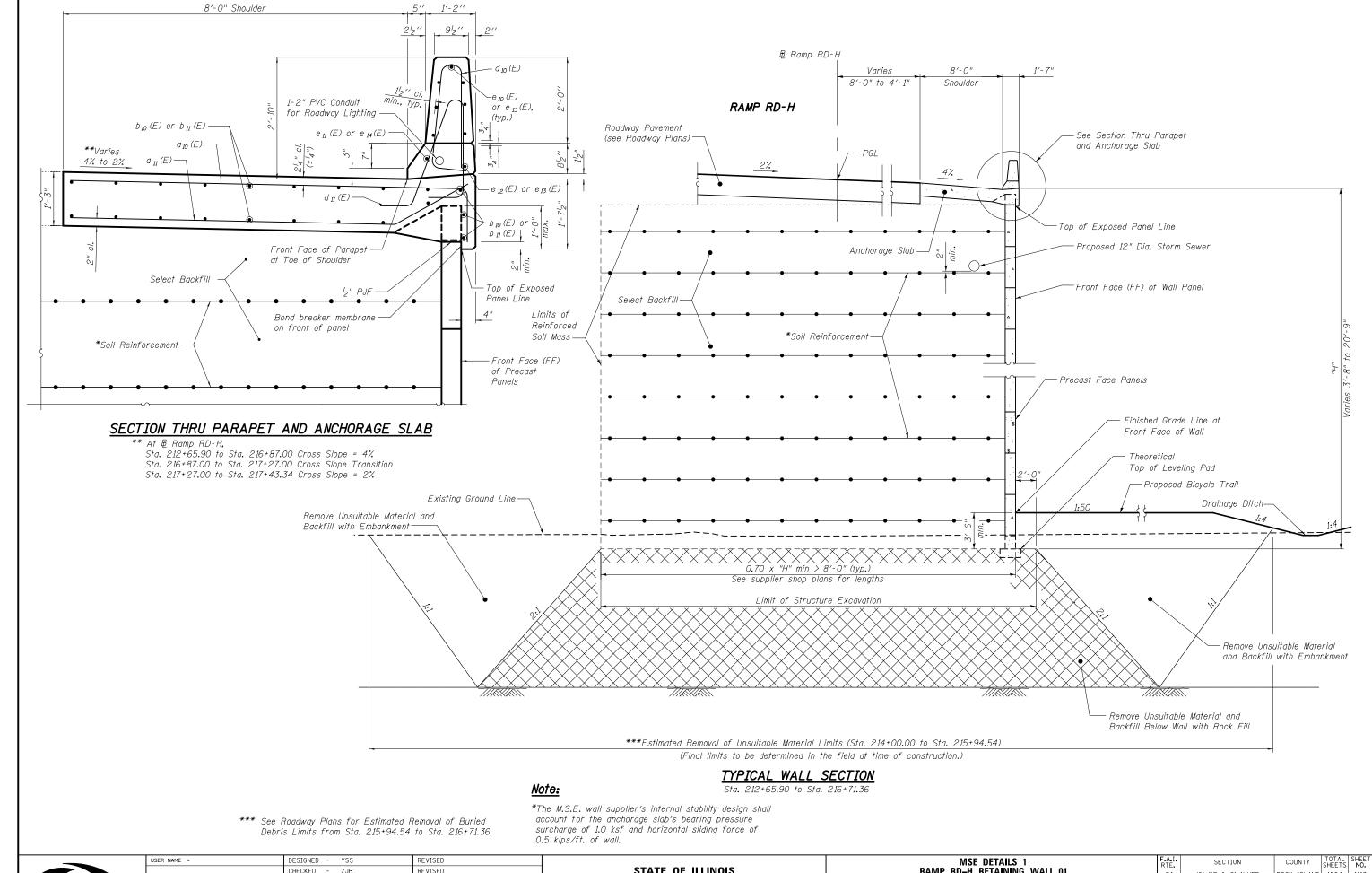
See Std. 51500



PROFILE GRADE

MODJESKI ■ MASTERS
Experience great bridges.

USER NAME =	DESIGNED - YSS	REVISED	
	CHECKED - JMH	REVISED	
PLOT SCALE =	DRAWN - MLA	REVISED	
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	

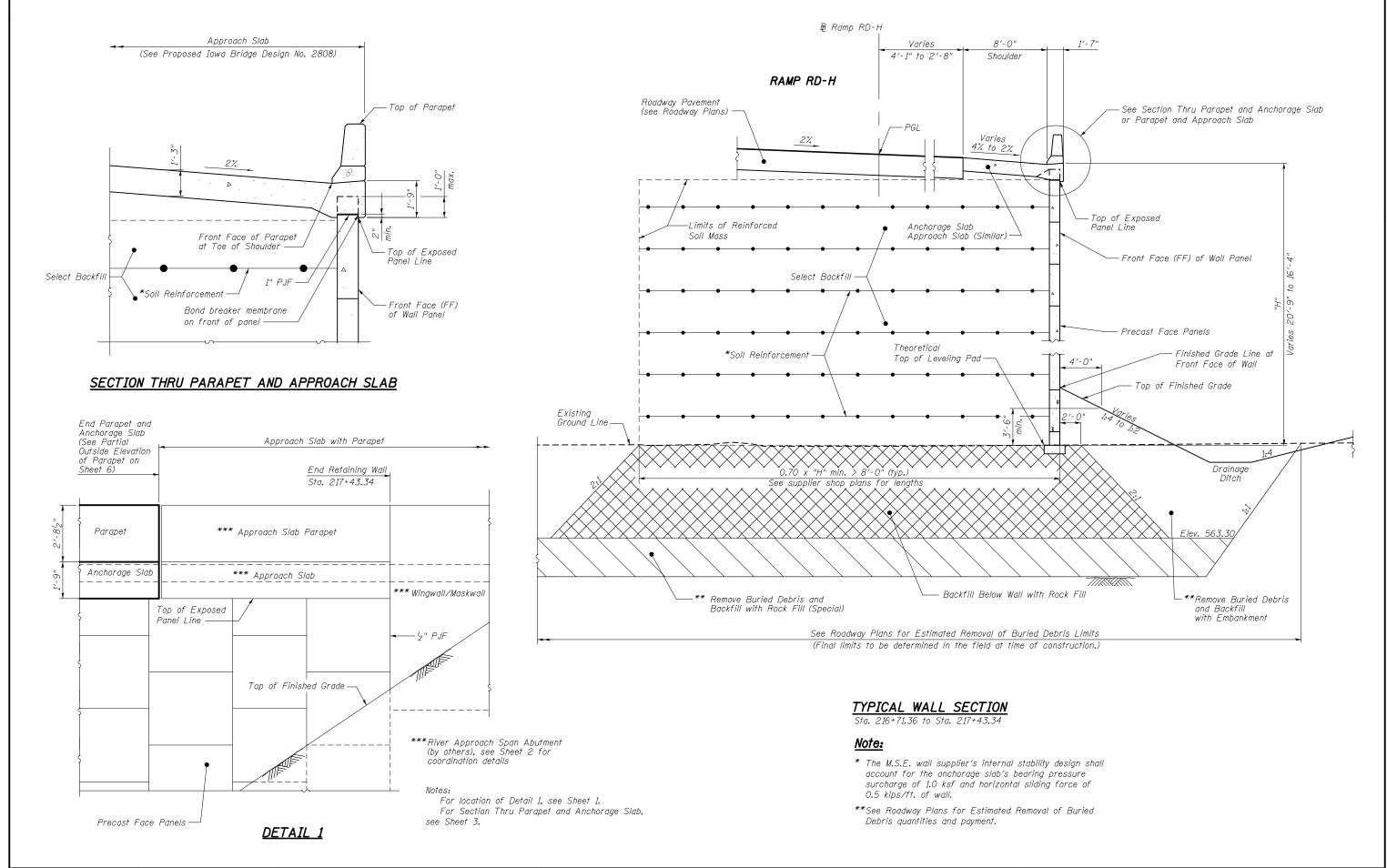


MODJESKI -- MASTERS

CHECKED - ZJB REVISED DRAWN MLA REVISED PLOT DATE = 1/20/2017 CHECKED -REVISED YSS

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** MSE DETAILS 1
RAMP RD-H RETAINING WALL 01 74 (81-1)R & 81-1HVBR STRUCTURE NO. 081-6010 SHEET NO. 3 OF 15 SHEETS

ROCK ISLAND 1504 | 1113 CONTRACT NO. 64CO8



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Experience great bridges.

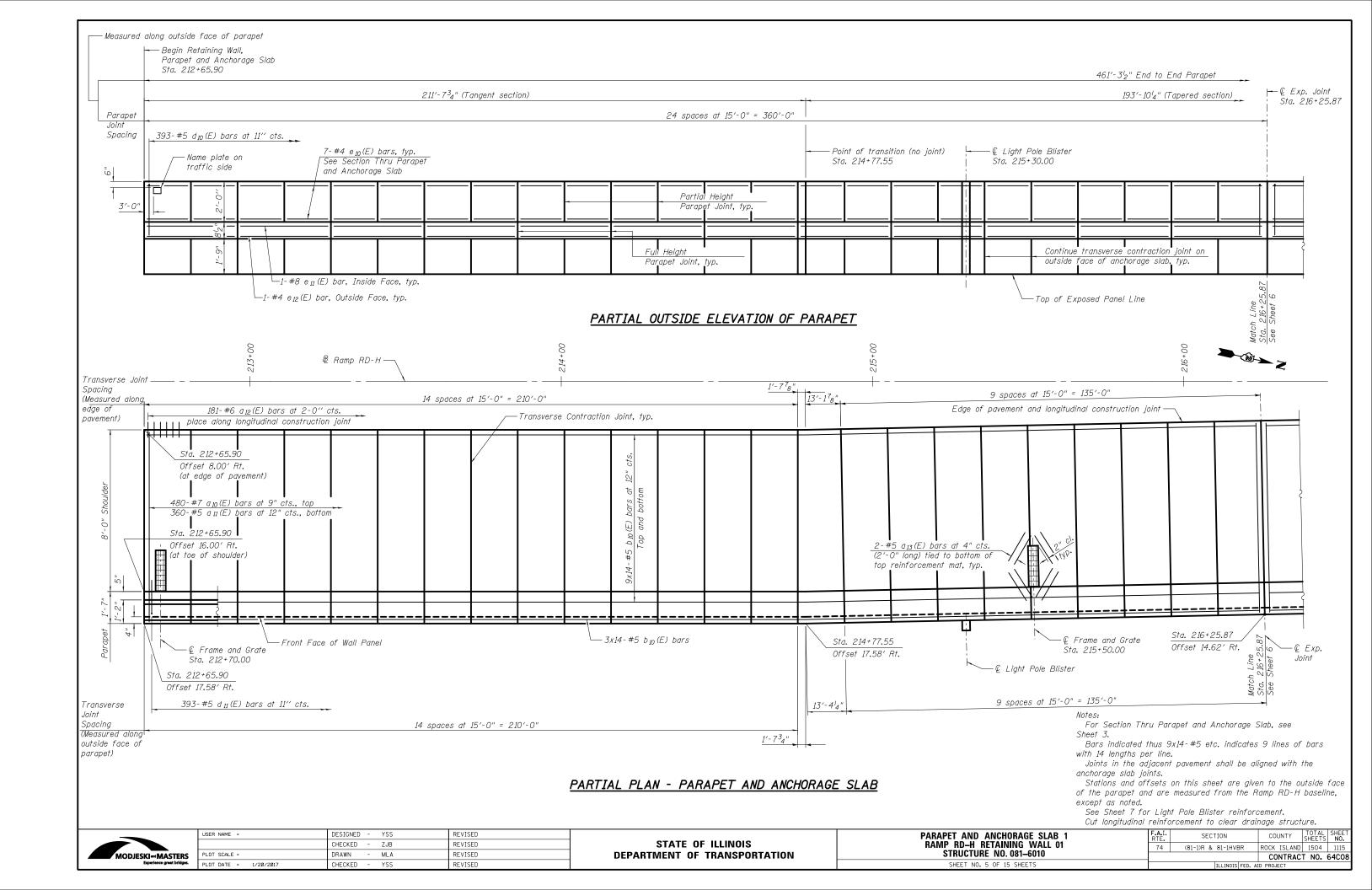
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

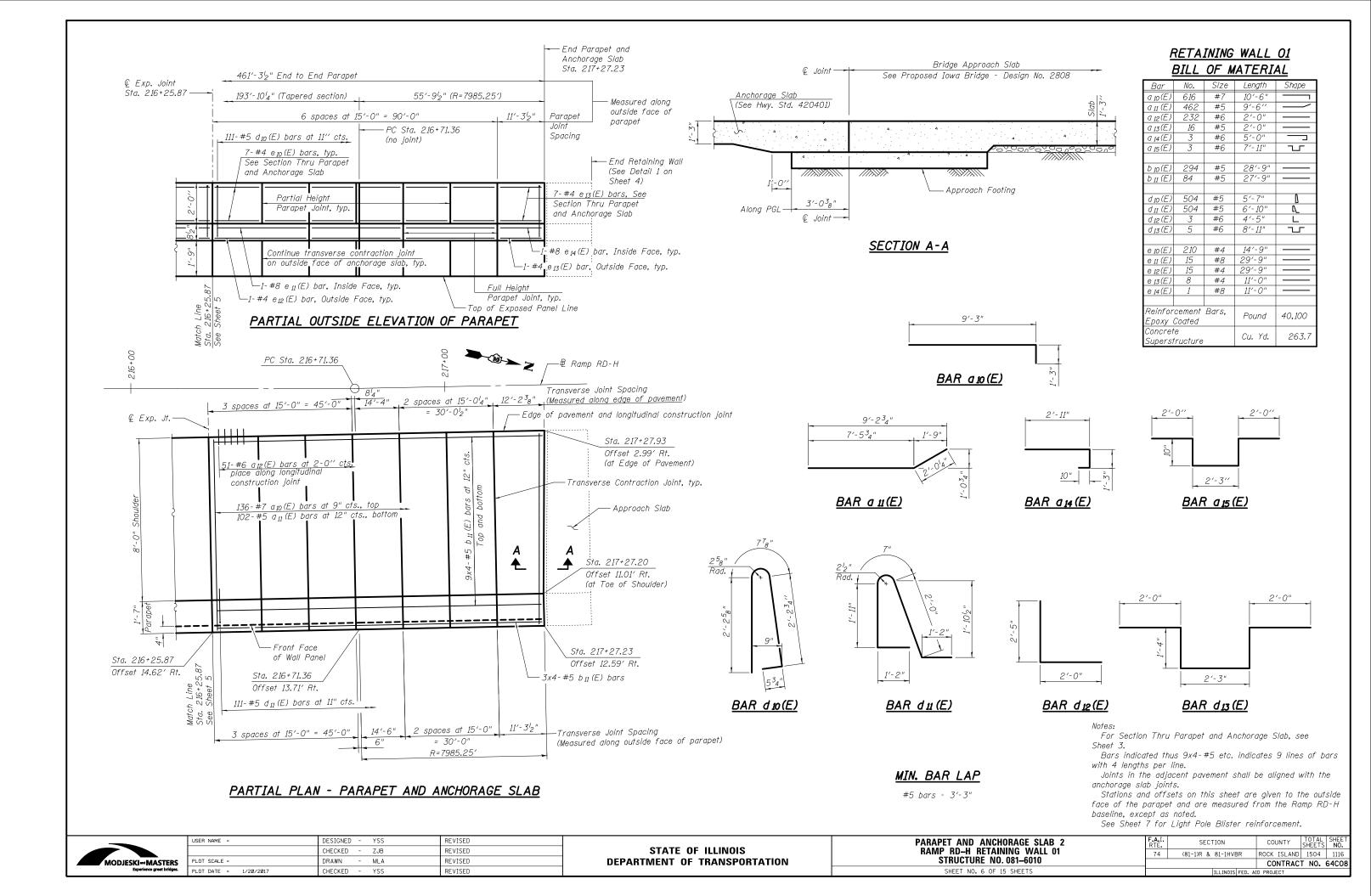
MSE DETAILS 2
RAMP RD-H RETAINING WALL 01
STRUCTURE NO. 081-6010
SHEET NO. 4 OF 15 SHEETS

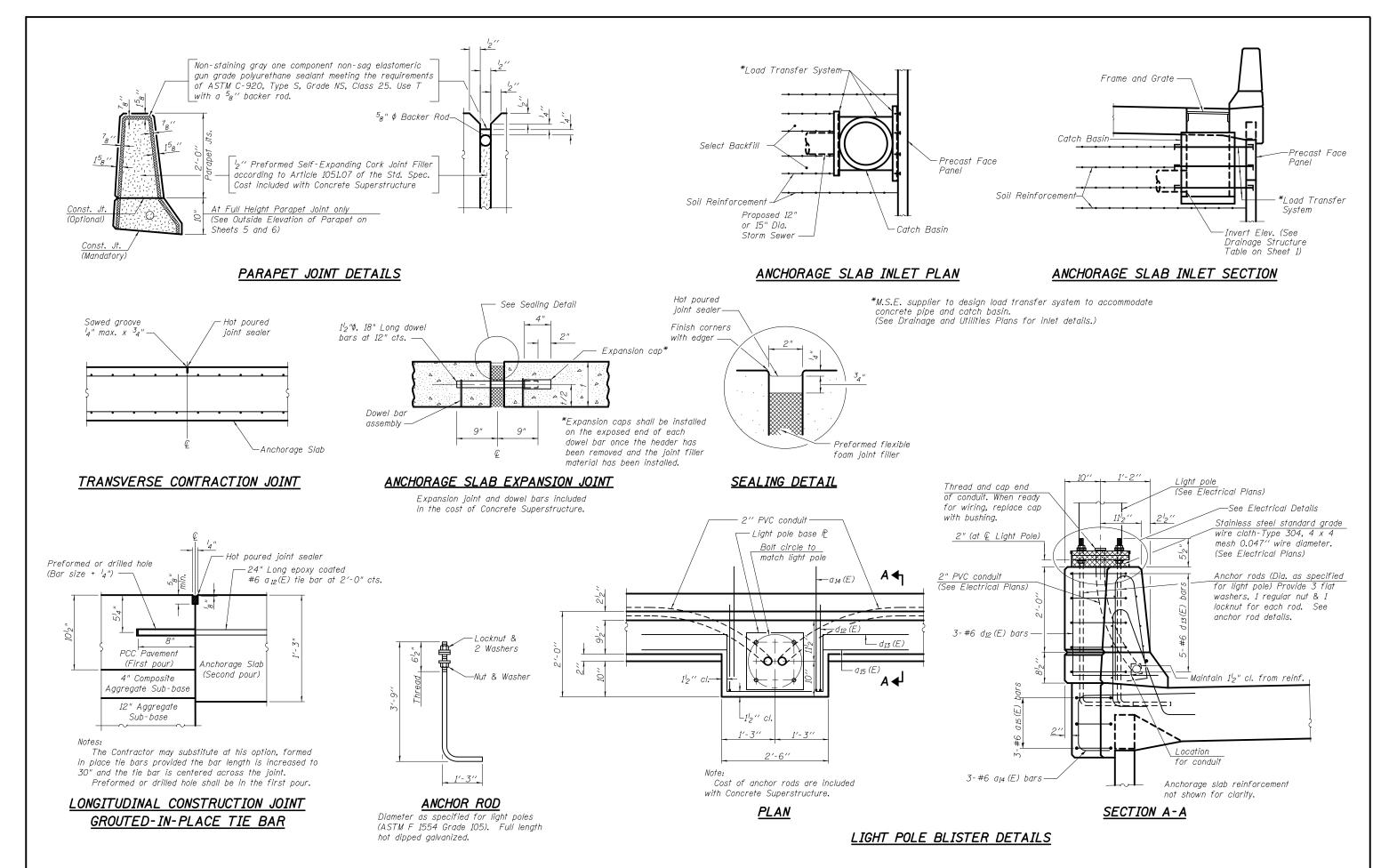
F.A.I. SECTION COUNTY TOTAL SHEETS NO.

74 (81-1)R & 81-1HVBR ROCK ISLAND 1504 1114

CONTRACT NO. 64C08





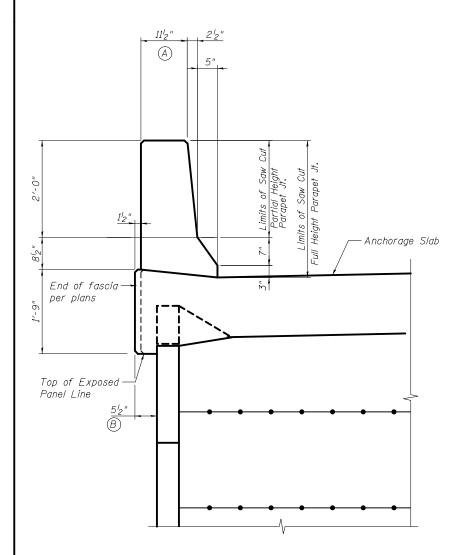


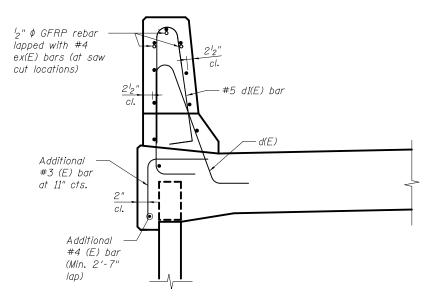
MODJESKI == MASTERS

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

MISCELLANEOUS DETAILS
RAMP RD—H RETAINING WALL 01
STRUCTURE NO. 081—6010

SHEET NO. 7 OF 15 SHEETS

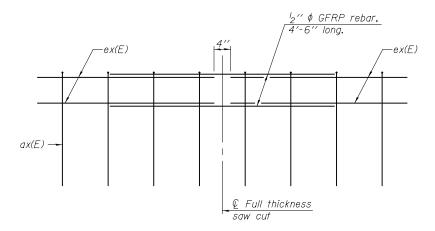




<u>SECTION</u>

(Showing reinforcement clearances for slip forming and additional reinforcement)

SECTION THRU PARAPET AND ANCHORAGE SLAB



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)



Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450

PLOT DATE = 1/20/2017	CHECKED - SLD	REVISED
PLOT SCALE =	DRAWN - KMP	REVISED
	CHECKED - SLD	REVISED
USER NAME =	DESIGNED - KMP	REVISED

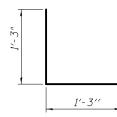
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

RETAINING WALL PARAPET SLIPFORMING OPTION RAMP RD-H RETAINING WALL 01 STRUCTURE NO. 081-6010 SHEET NO. 8 OF 15 SHEETS

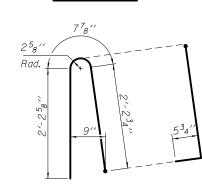
GENERAL NOTES

All dimensions shall remain the same as shown on superstructure details, except dimensions A and B which are to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension A equals 0.016 cu. yds./ft.

Full thickness saw cut at all joint locations in lieu of cork joint filler.



#3 (E) BAR



ALTERNATE BAR #5-d1(E)

(When conduit is present)

(A)	Illinois Department of Transportation
	CH2M HILL

Page <u>1</u> of <u>2</u>

	Division of Highways CH2M HILL			_						Date	9/2	0/07
					Ne	w I-74	Bridge Over Mississippi River -	Illinois				
ROUTE	I-74	DE	SCR	IPTIO	N		Approach	L	OGG	ED BY	F. <i>F</i>	breu
SECTION _	I-74 Bridge over Mi River	ssissippi	_ ı	OCA	TION _	(N=56	5384.43, E=2459285.013), SEC	. 32, TWP .	18N, I	RNG.	1W, 4 th	PM
COUNTY	Rock Island [ORILLING	G ME	THOE		ŀ	HSA, CME 55 HAMI	MER TYPE	CN	/IE AU	TOMA	TIC
			D E P	B L O	U C S	M 0 1	Surface Water Elev Stream Bed Elev.	ft	D E P	B L O	U C S	M 0 1
Station Offset	<u>ILR0101</u> 216+99 12' Rt.		H	W S	Qu	S T	Groundwater Elev.: First Encounter56 Upon Completion	ft	H	W S	Qu	S T
Ground Sur	face Elev. 568.6	7 ft	(ft)	(/6")	(tsf)	(%)	After Hrs	ft	(ft)	(/6")	(tsf)	(%)
Topsoil with gravel, b		567.67	_					547.67	_			
Fill: Silty Sar	nd With Gravel(SM))		5			Silty Clay With Gravel(CL-M	_)				
silt and some	arse to fine sand wit coarse to fine avel with brick	th	_	6 14			dark gray, wet, trace sand, littl gravel, hard thin wire strand embedded in tube	е	=		4.5 P	
fragments, dr		565.67		18				545.67				
Fill: Silty Sar	nd(SM)	_	_	14			Silty Sand (SM) Light gray, moist, very dense,	fino	_	50/4		
trace to little o	gravel, light gray,			23		17.0	sands with silt, trace medium	and				
	very dark brown to very loose to dense		_	14			coarse sand, trace fine gravel		_			
	nt petroleum odor	,	5	15			possible completely weathere	t	-25			
	nt potroioum ouor		_				sandstone		_			
								542.50				
few gravel-siz	ed brick fragments			4			Borehole continued with rock	042.00				
				4			coring.					
Samples 2, 3: performed	grain size analses			2			-		_			
occasional wo	and matter		<u> </u>	1								
occasional we	od matter		_	1		50.0			_			
			_	1		00.0			-			
			-	1					-30			
			10	<u> </u>	-				30			
			_						_			
Silty Eine to	Coarse Sand(SM)	557.67		0					_			
	very dark gray to		_	1		43.0			_			
black, wet, ve			_	1		45.0			_			
	•		_	1					_			
CI- 5												
performed	ain size analysis		_	1		90.0			_			
periorifica				1		90.0			_			
			_	1					_			
01 01117		553.67	-15	0	<u> </u>				-35			
Clayey Silt (N	/IL, CL-ML) ce gravel, dark gray	,	_						_			
to black, soft	ce graver, dark gray wet											
to black, 30it			_	2		L						
				0		67.0						
			_	1					_			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

(BB)	Illinois Dep of Transpo	artment	D00	/ OOF	_ ·				Р	age <u>2</u>	of
	Division of Highways	rtation	RUC	K COF	<⊨ I	_(JĠ			ate 9)/20.
	CH2M HILL		New I-74 Bridge	Over Mississ	ippi Rive	er - II	linois				
ROUTE	I-74 74 Bridge over Missis	_ DESCRIPTION	N	Approach				_ LO	GGE	BY <u>F.</u>	AŁ
SECTION	74 Bridge over Missis River	LOCAT	TION (N=565384.4	3, E=2459285	.013), S	EC.	32, T \	NP . 18	8N, RN	IG. 1W,	4 th I
COUNTYF	Rock Island CO	RING METHOD	Double tube, 10 ft	core barrel, N	NQ wirel	ine,	diamo	nd Poit	R	CORE	
CTRUCT NO		CODING D	ARREL TYPE & SIZ	·-				С		т	
Station						D E	CO	0 V	Q	M	
		Core Dian	neter	in		P	R	Ě	D	E	
BORING NO	ILR0101 216+99 12' Rt.	_ Top of Ro	ock Elev542.5	<u>υ</u> π 0 #		т	E	R		-	
Station	216+99 127 Pt	Begin Co	re Elev	<u> </u>		н		Υ			
Ground Surfa	ice Elev. 568.67	- ft				(ft)	(#)	(%)	(%)	(min/ft)	(
Sandstone	<u> </u>				542.50	_	Q-R	95	53		14
	ım to fine grained, lig Horizontal 10° fractu										
undulating, little	hard impermeable g	ires, rough and si irav clav infilling 1	/4" thick at 24" from	top, surfaces,		_					
stained greenis	h gray from 0-24" and	d dark gray from 2	24" to bottom, fractu	res at 0-20",	540.22	_					
slightly altered j	oint walls with little cl	lay infilling at frac	tures, hard clay infil	ing at 20" to		-					
rock coring at 1	nealed at joint walls a	ind slightly altered	i joint walls Started	preparing to	7						
Limestone	300				J	-30					
fine grained, lig	ht gray, slightly weatl	hered to unweath	ered, moderately st	ong rock							
					537.50	_					
Sandstone						_1	Q-R	2 96	69		2
Gray to light gra	ay, medium to fine gr	ained, smooth to	rough texture, slight	weathered to)	_					
	athered from 5" to 18' consolidated with infil			tone		_					
	per foot average	ining over time 10.	_ -			_					
						_					
						_					
						-3F					
						-30					
						_					
						_					
						-40					
					507.50	-					
End of Boring					527.50						⊢
Ling of Borning											
									l	1	

Color pictures of the cores _____

Cores will be stored for examination until___

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date 9/20/07

CH2M HILL				No		Bridge Over Mississippi	Divor Illinois	
ROUTE	DE	SCR	IPTIO	N Ne	w I-74	Approach	L(DGGED BY F. Abreu
I-74 Bridge over Mis	iggissis							
SECTION River			LOCA	TION _	(N=56	5290.255, E=2459318.6	346), SEC. 32, TWP.	18N, RNG. 1W, 4 th PM
COUNTY Rock Island D	RILLING	G ME	THOE	·—	-	HSA, CME 55	HAMMER TYPE	CME AUTOMATIC
		D	В	U	м			
STRUCT. NO.		E		C	Ö	Surface Water Elev	ft	
Station		F	٥	s	Ĭ	Stream Bed Elev.	ft	
DODING NO. II DO400		Ϊ́τ	w	١٠	s	Groundwater Elev.:		
BORING NO. ILR0103 215+99		Н	s	Qu	Ť	First Encounter _	E60 0 # ▼	
Station 215+99 Offset 14' Rt.						Upon Completion	1t <u>▼</u>	
Ground Surface Elev. 565.75		(ft)	(/6")	(tsf)	(%)	After Hrs.		
Silty Sand and Gravel(GM)	<u> </u>	<u>'</u>	<u> </u>	· ,	· ,	Anti	<u>``</u>	
Hole offset 5 feet southwest of		_	1					
proposed boring location	564.75		12					
Silty Medium to Coarse Sand	_	_	14					
With Gravel (SM)			16					
Very dark gray with brown, dry, dense, faint petroleum odor		_	42					
Fill: Silty Fine to Medium Sand	562.75		3					
(SM)		_	3					
Very dark brown to black, loose,			2					
wet, faint petroleum odor, loose			1 _					
		▼ -5	4					
	550.75	_	1					
Fill: Clayey Silt(CH)	559.75		0					
Very dark brown to black, moist.		_	1		106.0			
loose to very loose			2		100.0			
wood matter possible old railroad		_	1					
tie, no odor Sample 3: Atterberg limits	557.75		1					
performed		_	1		50.0			
Very Silty Fine to Coarse Sand		_	1 1		00.0			
(SM)		-10	+ :					
Little gravel, very dark brown to		-10	+-					
black, moist to wet, loose		-	1					
with brick fragments		_	1					
Tried to obtain ST from 11 to 13		_	1		28.0			
feet but encountered coarse		_	1					
material. Bag sample at 12 ft Sample 4: grain size analysis	552 75	_	1					
performed	552.75 552.50		50/3					
Silty Fine Sand(SM)	-1							
Light gray, wet, very dense, trace		_	1					
medium sand, possibly highly		-15	1					
weathered sandstone	_		1					
End of Boring		_	1					
		_	1					
		_	1					
			1					
		_	1					
		_	1					
		_	1					
			1					
		_	1	1	l	ll .		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



Sample 6: grain size analysis performed

USER NAME =	DESIGNED - JAK	REVISED	
	CHECKED - YSS	REVISED	
PLOT SCALE =	DRAWN - MLA	REVISED	
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	

W	Illinois De of Transpe Division of Highways CH2M HILL	partment ortation
POLITE	1.74	DECCRIPTION

Page <u>1</u> of <u>3</u>

CH2M HILL	-,-						Date9/20/07
OUTE 1-74	DES	CRIPTIO			Bridge Over Mississippi Approach		LOGGED BY F. Abreu
I-74 Bridge ov		_ LOCA	TION_	(N=56	5194.129, E=2459353.65	58), SEC. 32, TW	P. 18N, RNG. 1W, 4 th PM
OUNTY Rock Island	DRILLING	METHO		H	HSA, CME 55	HAMMER TYP	E _ CME AUTOMATIC
TRUCT. NOStation		D B L P O	U C S	M 0 1	Surface Water Elev Stream Bed Elev		
ORING NO. ILR01 Station 214+3 Offset 17' R	97 t.	T W H S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	ft	Z
Ground Surface Elev ravel (GM)		(ft) (/6")	(tsf)	(%)	After Hrs	ft	
ay to dark gray mottled wi ellowish orange, moist to d edium stiff, few coarse to t	ry, fine -	3 2 5					

		_	1		
	565.99		1		
gray to dark gray mottled with			3		
yellowish orange, moist to dry,			2		
medium stiff, few coarse to fine			5	1	
subangular to subrounded gravel,		_	6		
little coarse to fine sands, loose	563.99		-		
with cinder block fragments.	1		1	1	
Rough drilling and chattering 3.0'	1		3		
bgs	1			1	
Fine to Coarse Sand and Gravel	_	_	5		
(GP-GM)		▼ -5	20		
With possible limestone rock and		_	1		
silty sand seams/layers, light gray,	560.99				
wet to loose to medium dense			6	1	
Sample 2: grain size analysis	1	_	6		
performed	1			1	
Fine to Coarse Silty Sand(SM)	J	_	10		
little gravel, with silty clay layers,	558.99		3	1	
	-		4		
light gray, wet, medium dense	1	_	3	2.0	
Sample 3: grain size analysis	1	_	4		
performed			4	P	
Clay (CH)		-10	4		
Greenish gray, dry, non plastic,		-10	⊢	-	
moderate to strong cementation,		_	1	1	
medium stiff to stiff, orange brown	555.99				
stains, oxidized, possible native			50/4		
soil, trace fine sands, possible	1	_	1	1	
glacial till, weathered	1	_	ł	1	
	1	_	1	1	
Sample 4: Atterberg limit test			l		
performed			1		
Silty Sand (SM)	_	_	1		
Uniform light gray, wet, loose to	552.82		1		
medium dense, fine sands with	302.02	٠_		1	
silt, trace medium to fine	1	-15	1	1	
subangular gravels, little medium	1	-13	ł		
sands, possible completely	1	_	1		
weathered sandstone		_	1	1	
	J		1		
Borehole continued with rock		_	1		
coring.			1		
		_	1	1	
			l		
		_	1	1	
		_	1	1	1

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

P	Illinois De of Transpo Orleam of Highways CH2M HILL	partmen ortation
ROUTE	I-74	DESCRIPT
_	I-74 Bridge over Mis	ssissippi

Page <u>2</u> of <u>3</u>

ROCK CORE LOG Date 9/20/07 New I-74 Bridge Over Mississippi River - Illinois Approach LOGGED BY F. Abreu SECTION LOCATION (N=565194.129, E=2459353.658), SEC. 32, TWP. 18N, RNG. 1W, 4th PM COUNTY Rock Island CORING METHOD Double tube, 10 ft core barrel, NQ wireline, diamond bit STRUCT. NO. _ CORING BARREL TYPE & SIZE_
 Core Diameter
 in

 Top of Rock Elev.
 552.82
 ft

 Begin Core Elev.
 545.74
 ft
 BORING NO. ___
 BORING NO.
 ILR0104

 Station
 214+97

 Offset
 17' Rt.
 (ft) (#) (%) (%) (min/ft) (tsf) Ground Surface Elev. 566.99 ft Silty Sand
Top 24": Light gray, uniform, fine sands with silts, wet,
Remainder: Sandstone, light grain, rough to smooth texture, slightly weathered to
moderately weathered, weak to medium strong, crush rock zone from 29" to 32" from
top of clayey sandy infilling 14.17' - Bottom 13": Vertical fractures from 29" to 32",
remainder has horizontal fractures, rough to undulating fracture surfaces, little soft
clayey sand infilling materials at first from 24" to 34", surface stained greenish gray, no
rock wall contact due to silty sand seams at fractures, moderately to extremely
fractured, extremely close to close discontinuities Started rock coring at 09:11 @ 13.5
bcs 552.82 550.82 — Light gray, fine grained, slightly rough texture, weak to medium strong NO-R2 92 45 203.0 Light brown to light gray, medium to fine grained, slightly weathered to unweathered, medium to strong nock 31.25 - Horizontal to 15° fractures, rough fracture surface, varying planar and undulating fracture surfaces, little hard clay infilling material <1/8" at 64" from top that has tightly healed and created an irregular surface at the fracture with greenish gray stains, remainder of sample has no infilling material and no surface stain, sound to slightly fractured with close to wide discontinuities 100% fluid loss 45 second per foot 168.0

Color pictures of the cores _

Cores will be stored for examination until______
The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



ROCK CORE LOG

Page <u>3</u> of <u>3</u>

Date 9/20/07 New I-74 Bridge Over Mississippi River - Illinois ROUTE I-74 DESC I-74 Bridge over Mississippi River LOGGED BY F. Abreu __ DESCRIPTION___ LOCATION (N=565194.129, E=2459353.658), SEC. 32, TWP. 18N, RNG. 1W, 4th PM COUNTY Rock Island CORING METHOD Double tube, 10 ft core barrel, NQ wireline, diamond bit

STRUCT. NO. ___ CORING BARREL TYPE & SIZE_
 Core Diameter
 in

 Top of Rock Elev.
 552.82
 ft

 Begin Core Elev.
 545.74
 ft

 BORING NO.
 ILR0104

 Station
 214+97

 Offset
 17' Rt.
 (ft) (#) (%) (%) (min/ft) (tsf) Ground Surface Elev. 566.99 ft Sandstone
Light gray, fine grained, slightly rough texture, weak to medium strong (continued) -35

End of Borina

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - JAK	REVISED
	CHECKED - YSS	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

Illinois Department of Transportation

Page <u>1</u> of <u>1</u>

Division of Highways CH2M HILL	Ji lal	101	•			AL BOKIN		Date 9/19/07
ROUTEI-74			IPTIO			Bridge Over Mississippi Approach		DGGED BY F. Abreu
I-74 Bridge over Mis SECTIONRiver			OCA	TION _	(N=56	5075.678, E=2459393.5	88), SEC. 32, TWP.	18N, RNG. 1W, 4 th PM
COUNTY Rock Island D	RILLING	G ME	THOE		Н	HSA, CME 55	HAMMER TYPE	CME AUTOMATIC
STRUCT. NO		D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev Stream Bed Elev Groundwater Elev.: First Encounter _	ft	
Offset 17' Rt. Ground Surface Elev. 567.60	ft	(ft)	(/6")	(tsf)	(%)	Upon Completion _ After Hrs	ft	
Fill Gravel (GM) Gravel followed by silty sand subbase Fill Silty Sand With Gravel(SM) Reddish brick-like brittle dry clay, followed by yellowish orange	566.60 566.40		7 12 9					
mottled with brown sandy silt (ML) Sandy Silt (ML) Stiff to very stiff, non plastic, dry, Remainder: Silty Sand with Gravel (SM), dark brown, dry coarse to fine sands with silt and few medium to fine subangular	564.60	▼ 	1 1 1 1 2	1.0 P				
gravels Silty Clay (CL-ML) Dark gray, moist to wet, stiff, little fine sand, gumbo	561.60			3.5 P				
Sandy Lean Clay With Gravel (CL) Dark greenish gray, moist to dry, coarse to fine sand, coarse to fine gravel, very stiff, possible glacial till Sand and Silt(SM, ML)	559.60		1 3 2 3					
trace gravel, dark gray mottled with orange and greenish gray thereafter, moist, loose Sample 3: grain size analysis performed	556.60	_ _	3 4 13					
Very Sandy Lean Clay With Gravel (CL) Dark gray with greenish gray, moist, coarse to fine sands, coarse to fine gravel, and seams throughout, wet, medium dense, possible completely weathered sandstone, Driller notes rough drilling and heavy chattering 14' bgs, possible weathered rock End of Boring	554.35		22 50/3					
		_						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

Illinois Der of Transpo	oartı	me	ent		SO	OIL BORING LOG	<u>1</u> of <u>1</u>
ROUTE	_ DE	SCR		Ne	w I-74	Date _ Bridge Over Mississippi River - Illinois Approach LOGGED BY _f	
I-74 Bridge over Miss River	issippi	_ ı	LOCA	ION_	(N=56	5004.631, E=2459417.617), SEC. 32, TWP. 18N, RNG. 1V	V, 4 th PI
COUNTY Rock Island DF	RILLING	Э МЕ	THOD		H	HSA, CME 55 HAMMER TYPE CME AUTO	MATIC
STRUCT. NO. Station BORING NO. LR0107 Station 2/2+97 Offset 17' Rt. Ground Surface Elev 567.70 Fill: Gravel (6M)	_ _	D E P T H	O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev ft Stream Bed Elev ft Groundwater Elev.: First Encounter 563.7 ft ▼ Upon Completion ft After Hrs ft	
Fill: Sandy Silt With Gravel(ML) Very dark brown, dry, loose, with occasional wood matter	566.70	_	5 4 2 3				
Sandy Silt With Clay(ML) gray, moist, very stiff	564.70	▼ 	1 3 3 4	2.3 P			
Silty and Clayey Sand(SC) dark gray, moist, loose to very loose Sample 3: grain size analysis performed	561.70 559.70	_	1 1 2 2 2		21.0		
Fine to Medium Sand With Silt (SP-SM, SM) possible old alluvium	,	-10	1 1 2 2				
Silty Fine to Coarse Sand(SM) Little gravel, brown with gray, wet, loose, possible old alluvium Sample 5: grain size analysis performed	556.70 554.70	_	1 2 7 10		16.0		
Silty Sand (SM) brown with olive gray, wet, medium dense		-15	6 11 12 50/5				
Borehole continued with rock coring.	551.45	 					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



ROCK CORE LOG

Page <u>2</u> of <u>2</u>

Date <u>9/19/07</u> New I-74 Bridge Over Mississippi River - Illinois

ROUTE	I-74		CRIPTION	New I-74	Bridge Ove Ar	er iviississ oproach	ippi Rive	er - II	iinois	_ LO	GGE	BY <u>F.</u>	Abreu
SECTION _	I-74 Bridge over River	Mississippi	LOCATIO	ON (N=565	5004.631, E	=245941	17.617),	SEC	. 32, 1	WP. 1	8N, R	NG. 1W,	4 th PM
COUNTY _	Rock Island	_ CORING N	IETHOD _	Double tube	e, 10 ft core	e barrel, l	NQ wirel	ine,	diamo	E	R	CORE	S
STRUCT. N Station	0		ORING BAR		· ·	in		D E	c o	0 V	Q	T I M	R E N
Station	D. ILR0107 212+97 17' Rt.	7	Top of Roc Begin Core	k Elev	551.45	_ ft _ ft		P T H	R E	E R Y		E	G T H
	urface Elev. 56							(ft)	(#)	(%)	(%)	(min/ft)	(tsf)
unweathered planar fractu no brown an material <1/8	with brown, fine to d, weak to mediun ire surfaces, slight d greenish gray si B" thick at top 3" o ery close to close o	n strong rock tly altered join urface stains, if sample, rem	16.25' - Ho It walls, little little greeninainder no in	rizontal to 1 or no infillionsh gray soft	10° fracture ng material t clay infillin	d, rough , little or g	551.45	-20	√Q-R	1 77	9		
slightly weat fractures, ro material, no preventing b moderately t sample, likel	with brown, mediu hered to unweath- ugh fracture surfar surface stains, sli- sack wall contact a fractured, very clo- ty mechanical frac 8-14:20	ered, weak to ces, varying u ghtly altered j at bottom half se to close dis	medium str indulated ar oint surface of sample a scontinuities	ong 20.67 od planar the s and stray t some fract most fract	' - Horizont roughout, r crushed zo tures, sligh	al to 20° no infilling ones tly to	ı	_	IQ-R	2 98	6		228.0
Average 3/5	minutes per foot							-25					
								_					
							507.00	-30					
End of Borin	g						537.03	_					
								_					

Color pictures of the cores _____

Cores will be stored for examination until ____

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - JAK	REVISED	_
	CHECKED - YSS	REVISED	
PLOT SCALE =	DRAWN - MLA	REVISED	
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	

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Page <u>1</u> of <u>1</u>

SECTION

GRAVEL

STRUCT. NO. __

| Station | BORING NO. | RDH 02 | Station | 214+39 | Offset | 6' Lt. |

FILL - Very dark brown, dry to moist, stiff, SILT with sand and gravel

Bluish gray, moist, very stiff, silty CLAY with sand

Brown, fine-grained WEATHERED SANDSTONE

End of Boring

81B

566.40

559.40

553.20

50/3"

Date 6/30/10

HANSON SOIL BORING LOG

ROUTE F.A.I. 74 DESCRIPTION I-74 Over Mississippi River LOGGED BY JMB

COUNTY Rock Island DRILLING METHOD Hollow Stem Auger HAMMER TYPE

LOCATION NE1/4 of SEC. 32, TWP. 18N, RNG. 1W, 4th P.M.

Page	1	of	1

Date 6/28/10

CONTRACTHANSON

Date 6/28/10 LOGGED BY JMB ROUTE F.A.I. 74 DESCRIPTION I-74 Over Mississippi River 81B LOCATION NE1/4 of SEC. 32, TWP. 18N, RNG. 1W, 4th P.M. COUNTY Rock Island DRILLING METHOD Hollow Stem Auger HAMMER TYPE Stream Bed Elev. First Encounter
Upon Completion
After ____ Hrs. (ft) (/6") (tsf) (%) TOPSOIL FILL - Brown and gray, moist, stiff, silty, sandy, lean CLAY with rock and brick fragments Dark brown, moist, soft, SILT with fine-grained sand and rock fragments, tree roots 4 Gravish brown, wet, loose,

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Grayish Drown, wet, loose, well-graded, fine- to medium-grained SAND Grayish brown, wet, medium-dense, well-graded, medium-to coarse-grained SAND End of Boring

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

SECTION 81B		LOCAT	ION _	NE1/4 (of SEC. 32, TWP. 18N,	RNG. 1W, 4th P.M.	
COUNTY Rock Island D	RILLING ME	THOD	_	Но	low Stem Auger	_ HAMMER TYPE	Auto
STRUCT. NO. Station RDH 01 BORING NO. 216+36 Station 216+36 Offset 13'Lt. Ground Surface Elev. 565.7	E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft ft ∑	
FILL - Dark to very dark brown, moist to wet, soft and loose, silt, fine- to coarse-grained sand and gravel, with degrading plywood, particle board, timber, lumber, bituminous materials, metal scraps, cinder blocks, and brick fragments, petroleum odor	- - 2- - - - 4- - - - - -	4 5 5 5	1.50P	18			
	8— 	3 2 1	0.30P	35 68 44			
Gray, fine- to medium-grained WEATHERED SANDSTONE Gray, fine-grained SANDSTONE	12— 12— 14— 551.20 549.70 16—	4 13 16	0.80P	17			
Glay, inte-grained SANUS TONE	J 0 4 0 . 0 0	30/11					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99) The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

MODJESKI--MASTERS

	USER NAME =	DESIGNED - JAK	REVISED	
		CHECKED - YSS	REVISED	
;	PLOT SCALE =	DRAWN - MLA	REVISED	
	PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	1

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

BURING LUGS 4	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
RAMP RD-H RETAINING WALL 01	74	(81-1)R & 81-1HVBR	ROCK ISLAND	1504	1122
STRUCTURE NO. 081–6010			CONTRAC	T NO.	64C08
SHEET NO. 12 OF 15 SHEETS		ILLINOIS FED. AI	D PROJECT		

()	HANSON
	HANSUN

Page <u>1</u> of <u>1</u> Date __6/30/10

HANSON

SOIL BORING LOG

HANSON Page $\underline{1}$ of $\underline{1}$

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

				I	Date 6/28/10								Date <u>6/28/10</u>
ROUTE F.A.I. 74 I	DESCRIPTIO	N		I-74 Over Mississippi River LOGGE	D BYJMB	F	ROUTE	F.A.I. 74 DI	SCRIPTION	ı		I-74 Over Mississippi River L	OGGED BY JMB
SECTION 81-1HVB	LOCA	TION .	NE¼	of SEC. 32, TWP. 18N, RNG. 1W, 4th P.M.		s	SECTION	81-1HVB	LOCAT	<u> NOI</u>	NE¼ o	f SEC. 32, TWP. 18N, RNG. 1W, 4th P.M	1.
COUNTY Rock Island DRILLI	NG METHO		Ho	low Stem Auger HAMMER TYPE	Auto	C	COUNTYF	Rock Island DRILLIN	G METHOD		Holl	ow Stem Auger HAMMER TYPE	Auto
STRUCT. NO.	D B E L P O T W H S	C S Qu		Surface Water Elev.		E	STRUCT. NO. Station BORING NO. Station Offset Ground Surface	RW 01-3	D B L P O T W H S (ft) (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion 563.4 ft After Hrs. ft	
GRAVEL 566. FILL - Very dark brown, wet, stiff, SILT with fine-grained sand with gravel 564.	7 5 2 5 5	1.00P	28			F	GRAVEL FILL - Very dark to very stiff, clay and gravel, orga	566.70 k brown, moist, stiff yey SILT with sand anic material	_	1.75P	17		
FILL - Very dark brown, wet, silty CLAY with fine-grained sand and gravel	7		26 20					562.4	¥	2.50\$	61		
Grayish brown, moist, stiff, silty CLAY with trace sand and gravel	6 4 5	1.60P	31				Dark brown, mo with silt	oist, sandy CLAY	6		24		
558.	50 8-							559.4 n, wet, medium	8	0.34B	22 22 21		
Brown, moist, stiff, silty CLAY with 558. sand and gravel Brown, moist, medium dense, silty SAND	00 5		30			d	dense, silty, fine	e-grained SAND	10-	0.36B 0.40P	19 19		
Gray, fine-grained, WEATHERED 556. Brown and gray, poorly cemented, fine-grained, WEATHERED SANDSTONE with gravel and grayish green clay	12 10		23				Brown, wet, me graded, SAND	556.4 dium dense, well and GRAVEL	12 12		15		
Brown, wet, poorly cemented, fine-grained, WEATHERED SANDSTONE	14—		19			g	graded, silty SA	dium dense, well ND and GRAVEL	14 — 6 9 9		16		
End of Boring	00 16 50/0	"				19	SANDSTONE End of Boring	ed, WEATHERED 551.3	50/2"		15/		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

ROUTE	F.A.I. 74	_ DESCR	IPTION	١		I-74 Over Mississippi F	River LOG	GED BYJMB
SECTION	81-1HVB		LOCAT	ION _	NE¼ c	of SEC. 32, TWP. 18N,	RNG. 1W, 4th P.M.	
COUNTY	Rock Island Di	RILLING ME	THOD		Hol	low Stem Auger	_ HAMMER TYPE	Auto
Station BORING NO Station Offset	081-6010 RW 01-1 216+48 13' Rt. Ice Elev. 565.3	E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft ft ▽	
moist to wet, so fine- to coarse- gravel, with de particle board, bituminous ma	blocks, and brick	- - 2- ∑ - 4-	6 8	0.75P 0.10P				
		- - 6- - -	3 3 1 1 1		85			
		8- - - 10-	3 2 2		26			
		 12 	2 2 5		131			
		14 <i>-</i> - - -	6 3 4		108			
Gray, fine-grain SANDSTONE End of Boring	ned, WEATHERED	549.30 548.80	24 50/1/2					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99) The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

	USER NAME =	DESIGNED - JAK	REVISED		BORING LOGS 5	F.A.I.	SECTION	COUNTY	TOTAL SHEET
		CHECKED - YSS	REVISED	STATE OF ILLINOIS	RAMP RD-H RETAINING WALL 01	74	(81-1)R & 81-1HVBR	ROCK ISLAND) 1504 1123
STERS	PLOT SCALE =	DRAWN - MLA	REVISED	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 081–6010		102 2111 01 02 2111011	CONTRAC	CT NO. 64C08
nt bridges.	PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED		SHEET NO. 13 OF 15 SHEETS		TILL INDIS FED.	AID PROJECT	

(A)	Illinois Department of Transportation
	CH2M HILL

Page <u>1</u> of <u>3</u>

	Division of Highways CH2M HILL			•						Date	10/2	25/05
ROUTE	I-74	DE	SCR	IPTIO	Ne N	w I-74	Bridge Over Mississippi River - Illino Approach	is L(ogg	ED BY	<u>L. F</u>	Hunt
	I-74 Bridge over Mis	iggissis					5431.726, E=2459268.813), SEC. 32					
							·					
COUNTY	Rock Island D	RILLING	S ME	THOE			HSA, CME 55 HAMMER	TYPE	CI	ME AU	TOMA	TIC
			D E	В	U	M	Surface Water Elev	_ ft	D E	В	U	M O
Station		_	Р	L O	s	Ĭ.	Stream Bed Elev.	_ ft	Р	L	s	Ĭ.
BORING NO.	RW1401		T H	W	Qu	S	Groundwater Elev.:		T H	W S	Qu	S
Offset	217+49 12′ Rt.	_		ľ	- Qu	١.	First Encounter559.5 Upon Completion		١	ľ	Qu	·
Ground Sur	face Elev. 568.53		(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft)	(/6")	(tsf)	(%)
	M) Gravel, sand, clav, grav brown, dn	,	_	14 12			Clay (CL) Clay, dark brown to black, moist to wet, stratified, sand		_	WOH	0	
	um dense to dense	,		10			at top 4" of sample, limestone and			WOH 50	0.0 P	
			_	9			sand for bottom 4"-5" of sample WOH = Weight of Hammer.		_			
			_	10					_			
				14 22								
				45					_	i		
			_	26					_			
			5	10 12					-25	50/3		
		562.53		8				542.53				
	nd, little to some lay, brown, moist to		_	1			Borehole continued with rock coring.		_			
9.0', wet deep	er, loose,			1 2			soming.			1		
contamination	атьп			4					_	1		
				1					_	-		
			<u>*</u>	3					_	1		
		558.53	-10						-30	1		
	(SC) Clayey sand, ark brown and white,		_	2					_	-		
wet, loose				3					_	i		
		556.53		6					_			
silt, dark brow	CL) Sandy clay and n, wet		_	2		21.4			_	-		
'				2					_	i		
Poorly Grada	d Sand(SP) Sand	554.53		2 2					_			
and gravel, tra	ice organics, dark		-15	1	-				-35	-		
brown to black	k, wet, loose		-13	2					55	1		
				2					_			
			_	<u> </u>	1				_	-		
									_	1		
									\equiv			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

P	Illinois De of Transp	epartmen oortation
ROUTE	I-74	DESCRIPT
	I-74 Bridge over M	ississippi

Page <u>2</u> of <u>3</u>

ROCK CORE LOG Date 10/25/05 New I-74 Bridge Over Mississippi River - Illinois LOGGED BY L. Hunt Approach LOCATION (N=565431.726, E=2459268.813), SEC. 32, TWP. 18N, RNG. 1W, 4th PM COUNTY Rock Island CORING METHOD NQ DOUBLE BARREL DIAMOND TIP CORE STRUCT. NO. _ CORING BARREL TYPE & SIZE_
 Core Diameter
 in

 Top of Rock Elev.
 542.53
 ft

 Begin Core Elev.
 542.53
 ft
 BORING NO. ____ RW1401
 Station
 217+49

 Offset
 12' Rt.
 (ft) (#) (%) (%) (min/ft) (tsf) Ground Surface Elev. 568.53 ft Sandstone and Shale Interbedded Sandstone and Shale, gray, fine grained, weathering: barely consolidated, seems highly weathered, no discoloration; extremely weak strength, interbedded, hummocky bedding; Shale - laminated beds; Sandstone, no apparent bedding (thick to massive), well sorted, well rounded. Auger refusal at 26' at 12.35, Begin rock core at 13.41. Horizontal fractures, extremely fractured to slightly fractured continuity, extremely close to close discontinuity, rough to smooth joints, jiorits do not seem altered, but shale is softened in joints, these could also be bedding lalanes. 542.53 R1 100 540.03 R2 100 12 Drilling water was black then dark gray for about 20 seconds at the start of rock Sandstone and Shale, gray, fine grained, see weathering above, extremely weak rock, interbedded, laminated to very thin beds, well sorted, well rounded. Horizontal fractures, extremely fractured to slightly fractured continuity, extremely flose to close discontinuity, rough to smooth joints, joints not altered, softened shale at contact points.
First 2.5' of coring R-1 occurred more rapidly than other rock coring with same rig(2.5' in 10-15 minutes). 535.03 rig(z.2' in 10-15 minutes).

Sandstone and Shale, gray, fine grained, see weathering above, interbedded, laminated to very thin beds, well sorted, well rounded; shale-extremely weak rock; sandstone-very weak rock; 33.5' to 35.66' highly shaley Drilling water turned black from shale at 33.5' for just a few seconds. Horizontal fractures, extremely fractured to sound continuity, extremely close to moderate discontinuity, rough to smooth joints, unaltered joint walls, but softened shale at contact points. Sandstone and Shale, black to dark gray, fine to medium grained, fine grained sandstone, fair amount of silt sized particles in shale, see above weathering, interbedded, laminated to very thin bedding, shale-extremely weak rock, sandstone-weak rock. Replaced drill bit at 3pm. Horizontal fractures, extremely R4 92 fractured to slightly fractured continuity, extremely close to close discontinuity, rough to smooth joints, some altering of joint walls (could be due to coring processes and strength of shale). R5 100 50 198.0 Sandstone, Shale, and Limestone, dark gray to light gray; sandstone, fine grained, see above weathering, medium strength, laminated to thin bedding, well sorted, well rounded; shale, see above weathering, laminated beds, extremely weak rock; limestone (at 42.83), fine to medium grained, slightly to moderately weathered, no apparent bedding (thin to massive). Horizontal fractures, extremely fractured to slightly fractured continuity, extremely close to close discontinuity, rough to smooth joints, some altering of joint walls (could be due to coring processes and strength of shale), Limestone, firm clay mineral coatings and sandy/gravelly material in fractures with rock wall separation <1/4" thick. Limestone Limestone, gray, fine to medium grained, unweathered to slightly weathered, strong rock, no apparent bedding (thin to massive). Horizontal fractures, sound continuity, wide discontinuity, rough to smooth joints, tightly healed joints with hard clay mineral in joints with no rock wall separation. R6 100 100 294.0

Color pictures of the cores		

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



ROCK CORE LOG

Page <u>3</u> of <u>3</u>

Date 10/25/05 New I-74 Bridge Over Mississippi River - Illinois

ROUTE	I-74		SCRIPTION_	New I-72	A Bridge Ove	pproach	Kivei - II	1111015	_ LO	GGE	BYL	. Hunt
SECTION _	I-74 Bridge over River	Mississippi	_ LOCATIO	ON (N=56	55431.726, E	E=2459268.81	13), SEC	. 32,	Γ WP . 1	8N, R	NG. 1W.	4 th PM
COUNTY _	Rock Island	CORING	METHOD _	NQ DOUE	BLE BARRE	L DIAMOND	TIP		R E	R	CORE	S T
Station	0		Core Diame	eter		in	— D E P	C O R	C O > E	Q D	T I M E	R E N G
Station	O. <u>RW1401</u> 217+49 12' Rt.		Top of Roc Begin Core	Elev	542.53	_ ft	T H	E	R Y			T H
	urface Elev. 56						(ft)	(#)	(%)	(%)	(min/ft)	(tsf)
End of Ro							-50					

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - JAK	REVISED
	CHECKED - YSS	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

W	Illinois Department of Transportation
	CH2M HILL

Page <u>1</u> of <u>1</u>

Division of Highways
CH2M HILL

I-74

DESCRIPTION

New I-74 Bridge Over Mississippi River - Illinois
Approach

LOGGED BY L. Hunt

I-74 Bridge over Mississippi

		IPTIO	Ne V	w I-74	Bridge Over Mississippi River - Illinois Approach	_ LOC	GED B	/ <u>L. I</u>	lunt
SECTION I-74 Bridge over Mississ	sippi I	LOCA	TION _	(N=56	5240.796, E=2459335.667), SEC. 32,	TWP. 18	BN, RNG	. 1W, 4	th PM
COUNTY Rock Island DRIL	LLING ME	THOD			HSA, CME 55 HAMMER T	YPE _	CME AL	JTOMA	TIC
STRUCT. NO. Station BORING NO. RW1403 Station 2/5+46 Offset 15' Rt.	- E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter 556.4 Upon Completion	ft <u>▼</u>	B E L P O T W H S	U C S Qu	M O I S T
Ground Surface Elev. 566.39 Silty Clay (CL-ML) Gravel and	_ ft (ft)	(/ 6")	(tsf)	(%)	After Hrs Bottom of Borehole at 20'. No	ft (ft) (/6")	(tsf)	(%)
silty clay, light gray and brown, dry to moist, stratified, hard	64.39	7 6 3	6.8 P		auger refusal, but hit sandstone and couldn't sample with the split spoon End of Boring	_			
Sandy Clay (CL) Sandy clay, some gravel, dark brown and black, moist, homogeneous.	_	2 6 5				_			
Silty Clay (CL-ML) Silty clay, some fine sand, dark brown, moist to wet, homogeneous. WOH =	62.39 	3 1 WOH	0.8 P	24.0		_	-25		
Weight of Hammer, Shelby sample from 4'-6' obtained in adjacent hole on 11/10/05, See lab results for consolidation data	_	WOH WOH Push WOH	0.8 P 2.7			_	_		
	_	2 3 4 3				-	-30		
	<u>▼</u> -10	7	0.7			_	-30		
Sandstone 55	54.39	6 50/3 50/0	P			_			
						_			
	15	50/1				_			
	_					_			
	_					_			
54	46.39 -20					_	-40		

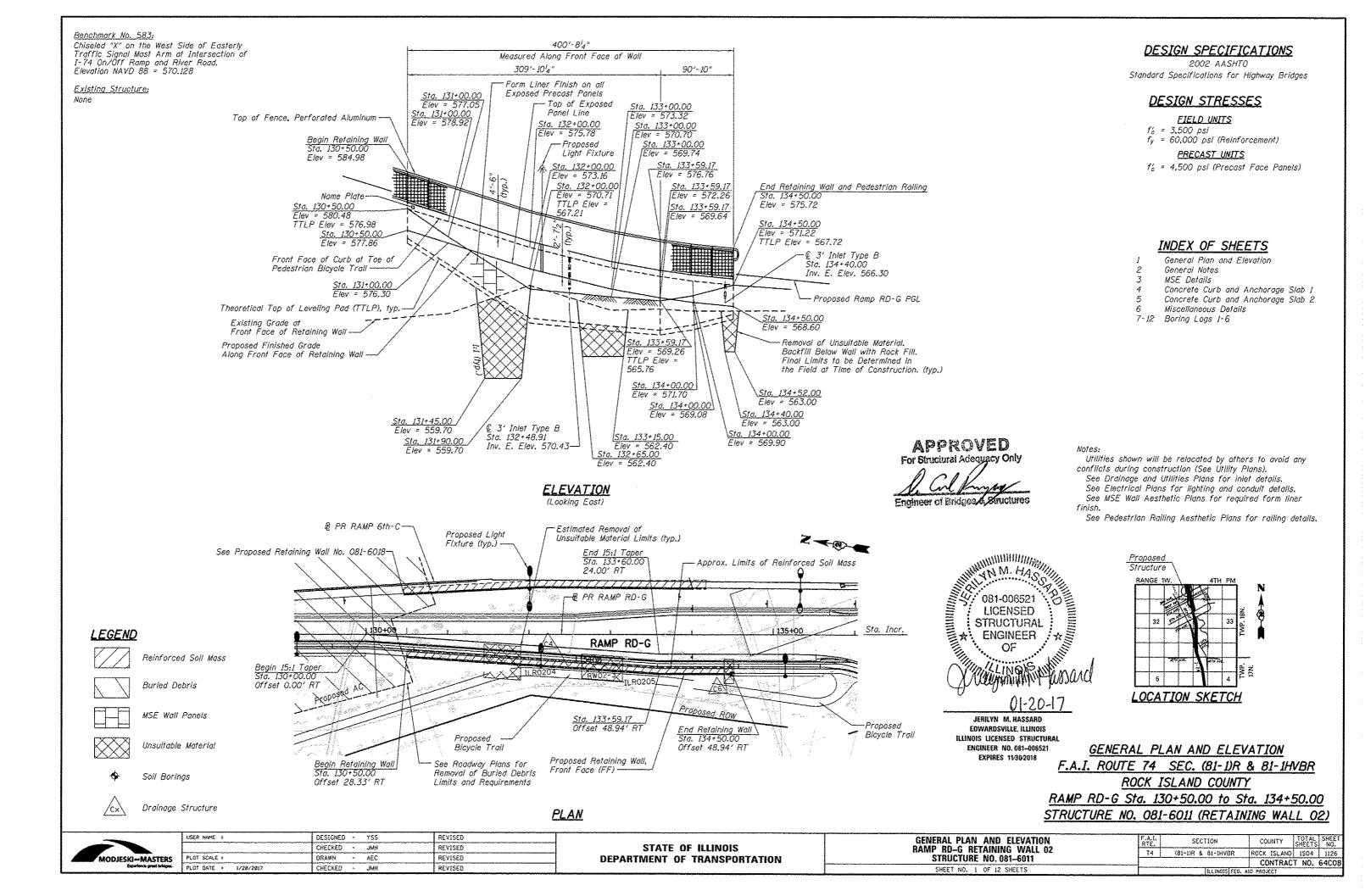
The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

MODJESKI and MASTERS Experience great bridge	

USER NAME =	DESIGNED - JAK	REVISED	
	CHECKED - YSS	REVISED	
PLOT SCALE =	DRAWN - MLA	REVISED	
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	

BORING LOGS 7				
RAMP RD-H RETAINING WALL ()1			
STRUCTURE NO. 081–6010				
SHEET NO. 15 OF 15 SHEETS				

F.A.I. RTE.	SECT	LION		СО	UNTY	TOTAL SHEETS	SHEE NO.
74	(81-1)R &	81-1HVE	3R	ROCK	ISLAND	1504	1125
				CC	NTRAC	T NO.	64CC
		ILLINOIS	FED. A	ID PROJ	ECT		



GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Wall stations and offsets are given to the front face (FF) of the wall and are measured from the Ramp RD-G baseline, except as noted. FF of the wall is to be considered edge of panel or form liner.
- 3. See Special Provision for Mechanically Stabilized Earth Retaining Walls for design and construction requirements.
- 4. Wall construction shall not begin until after ground improvement for the unsuitable material has been completed in the area of the new wall.

MSE WALL SETTLEMENT

1. The Top of Exposed Panel Elevations shown on these plans are final elevations after any settlement. The wall system supplier shall take appropriate measures to accommodate the 0 to 4 inches of settlement that are anticipated from Sta. 130+50.00 to Sta. 134+50.00.

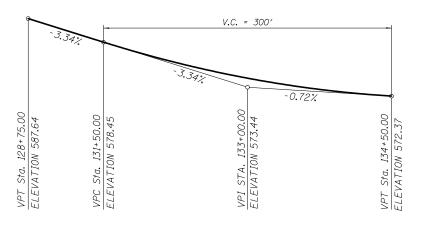
TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Structure Excavation	Cu. Yd.	252
Removal and Disposal of Unsuitable Material for Structures	Cu. Yd.	1,726*
Concrete Superstructure	Cu. Yd.	188.8
Protective Coat	Sq. Yd.	412
Reinforcement Bars, Epoxy Coated	Pound	31,680
Name Plates	Each	1
Mechanically Stabilized Earth Retaining Wall	Sq. Ft.	1,517
Rock Fill	Cu. Yd.	825**

- * Estimated quantity includes 900 cu, yds, for potential overage.
- ** Estimated quantity includes 400 cu. yds. for potential overage.

STATION 130+50.00
BUILT 201_ BY
STATE OF ILLINOIS
F.A.I. RT. 74
SEC. (81-1)R & 81-1HVBR
LOADING HS-20
STR. NO. 081-6011

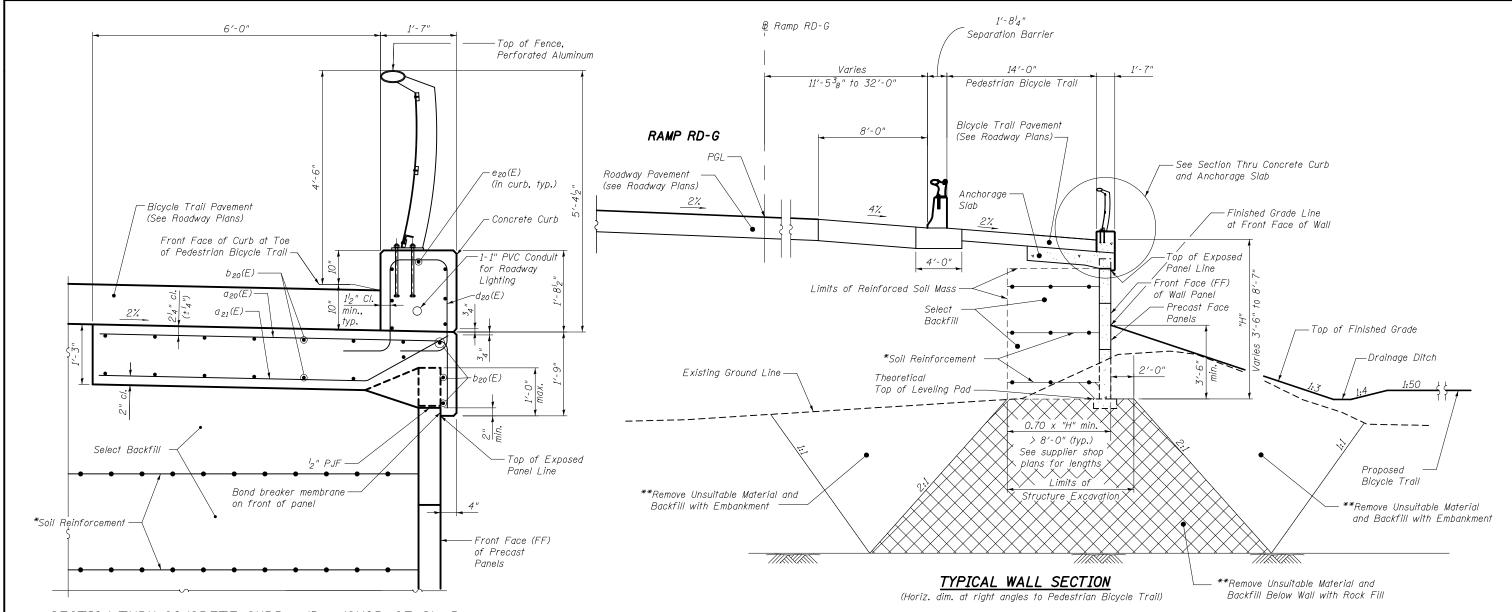
NAME PLATE



PROFILE GRADE
(Along & Ramp RD-G)

MODJESKI and MASTERS Experience great bridges	

USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED



SECTION THRU CONCRETE CURB AND ANCHORAGE SLAB

Notes:

- * The M.S.E. wall supplier's internal stability design shall account for the anchorage slab's bearing pressure surcharge of 1.0 ksf and horizontal sliding force of 0.5 kips/ft. of wall.
- *** Removal of Unsuitable Material is required from: Sta. 131+45.00 to Sta. 131+90.00 Sta. 132+65.00 to Sta. 133+15.00 Sta. 134+40.00 to Sta. 134+52.00 (See Sheet 1 for details.)

Note.

For fence, perforated aluminum, base plate and anchor bolt details, see Pedestrian Railing Aesthetic Plans.

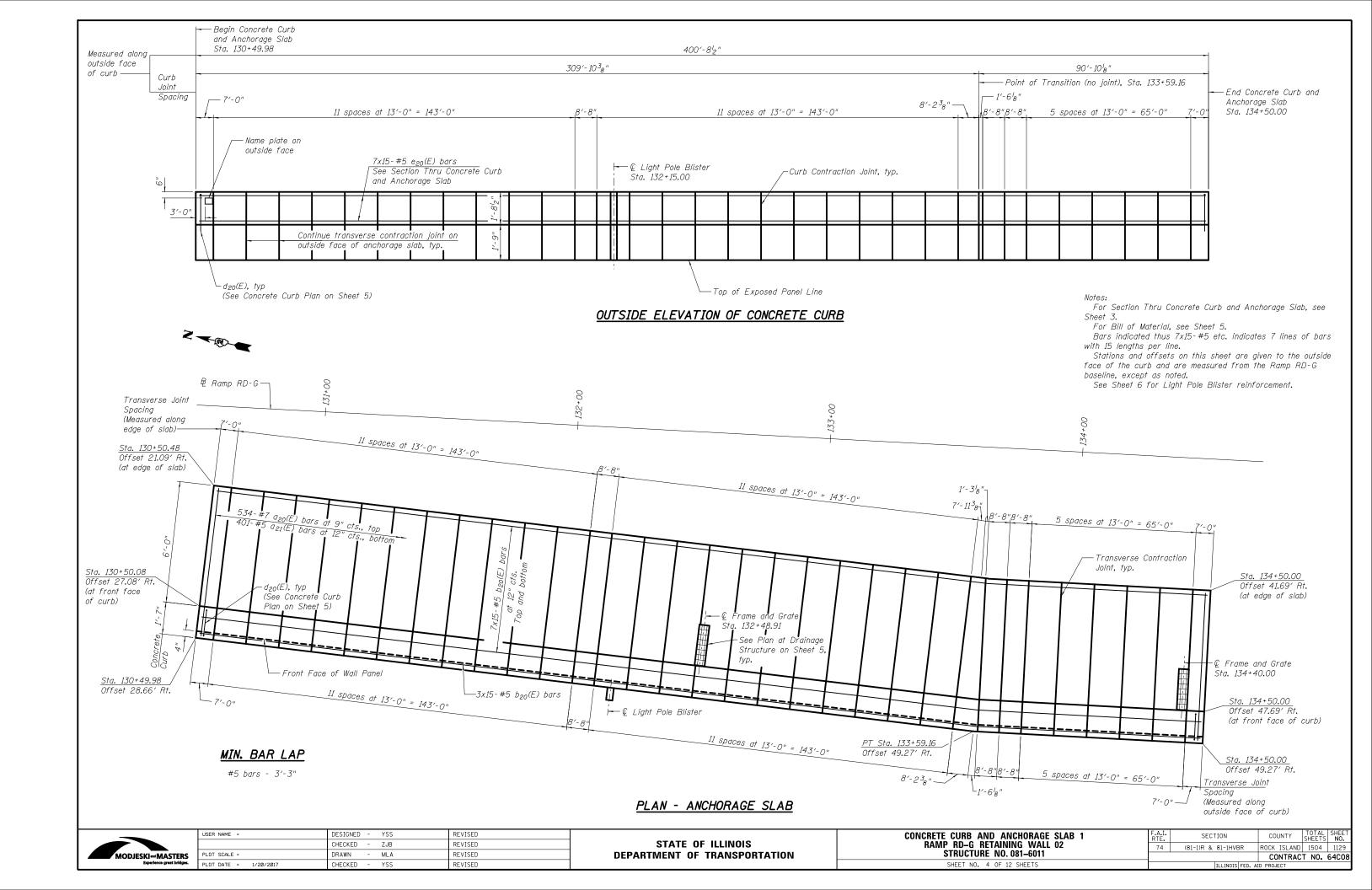


USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

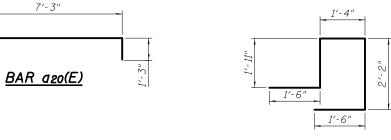
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

MSE DETAILS
RAMP RD-G RETAINING WALL 02
STRUCTURE NO. 081-6011
SHEET NO. 3 OF 12 SHEETS

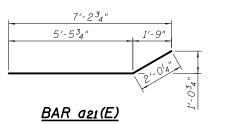
F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
74	(81-1)R & 81-1HVBR		ROCK ISLAND	1504	1128
			CONTRAC	T NO.	64C08
	ILLINOIS FED.	. A	ID PROJECT		

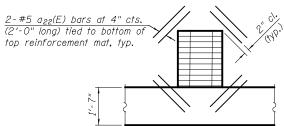


RETAINING WALL 02 7′-3" 1'-4"



BAR deo(E)

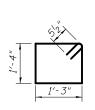


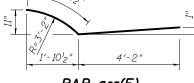


PLAN AT DRAINAGE STRUCTURE

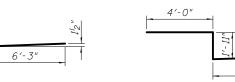
(Cut longitudinal reinforcement to clear drainage structure.)

BILL OF MATERIAL						
Bar	No.	Size	Length	Shape		
a20(E)	534	#7	8′-6"	_		
a ₂₁ (E)	401	#5	7′-6"			
a ₂₂ (E)	16	#5	2'-0"			
a23(E)	376	#5	7′-3"			
a24(E)	4	#7	10'-1"	ー		
a ₂₅ (E)	4	#5	6'-4"	_		
a ₂₆ (E)	4	#7	7′-5"	Ь		
b20(E)	255	#5	29'-9"			
d20 (E)	560	#5	8′-5"	-0		
d21 (E)	4	#7	8'-0"			
d22(E)	4	#4	6'-1"			
d23(E)	1	#5	6'-1"			
d24 (E)	1	#5	6'-3"			
d25 (E)	1	#5	6′-7"			
d26 (E)	1	#5	7′-1"			
d27 (E)	2	#5	7'-11"			
e20(E)	105	#5	29'-9"			
Reinforcement Bars,			Pound	31,680		
Ероху (, ourid	31,000		
Concrete Superstructure			Cu. Yd.	188.8		





BAR d22(E)



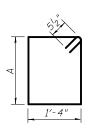
BAR a26(E)

BAR a25(E)

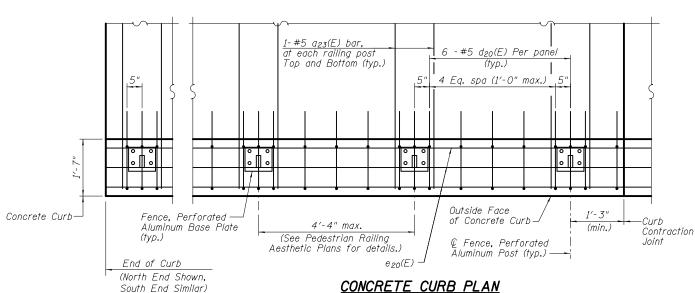
4'-2"

BAR a24(E)

Bar	A
d ₂₃ (E)	1'-3"
d ₂₄ (E)	1'-4"
d ₂₅ (E)	1'-6"
d ₂₆ (E)	1'-9"
do7(E)	2'-2"



BARS des(E) thru der(E)



CONCRETE CURB PLAN (Slab reinforcing not shown)

Details and quantities shown assume Fence, Perforated Aluminum layout consists of 93 fence panels and 94 fence post throughout the limits of the retaining wall.

Notes:

For base plate details, see Pedestrian Railing Aesthetic Plans.

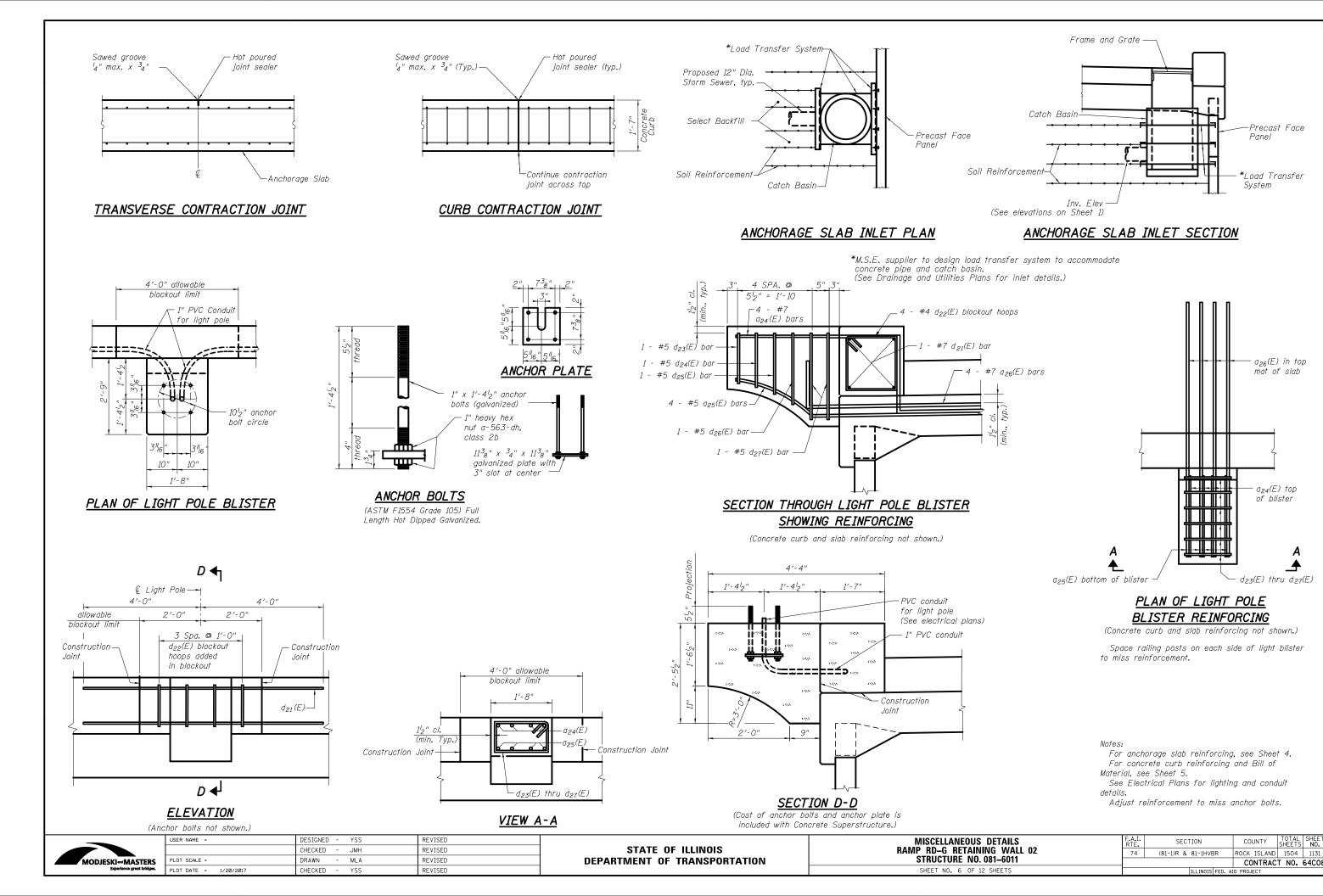


USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED
FEOT DATE = 1/20/2017	CHECKED - 133	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

CONCRETE CURB AND ANCHORAGE SLAB 2	F.A.I. RTE.	
RAMP RD-G RETAINING WALL 02	74	(81-
STRUCTURE NO. 081-6011		
CHEET NO. E OF 10 CHEETC		

SECTION COUNTY 81-1)R & 81-1HVBR ROCK ISLAND 1504 1130 CONTRACT NO. 64CO



P	Illinois Do of Transp	epartment portation
ROUTE	1-74	DESCRIPT
	I-74 Bridge over N	Mississippi

Page <u>1</u> of <u>2</u>

	or Transpo	tatioi			00	IL DOMIN	CLOO				
	ivision of Highways								Date	9/1	9/07
OUTE	I-74 74 Bridge over Miss				2000 5000	Bridge Over Mississippi Approach			ED BY		Abreu
ECTION	River		LOCAT	ION	(N=56	5232.456, E=2459065.7	(32), SEC . 32, TW	P. 18N	, RNG	. 1W, 4	1" PN
OUNTYF	Rock Island DF	RILLING ME	THOD	_	ŀ	ISA, CME 55	HAMMER TYPE	CI	ME AU	ТОМА	TIC
TRUCT. NO		_ D E	B L O	U C S	M 0	Surface Water Elev Stream Bed Elev		D E P	B L O	UCS	M 0 1
ORING NO Station Offset Ground Surfa	ILR0201-S 130 + 16 11' Lt. ce Elev. 566.39	H (ft)	W S (/6")	Qu (tsf)	S T (%)	Groundwater Elev.: First Encounter Upon Completion After Hrs.	562.4 ft ¶	T H (ft)	W S (/6")	Qu (tsf)	S T (%)
oncrete				100	-		1.50%	100	100	100	1000

Station 130 + 16 Offset 11' Lt Ground Surface Elev. 566.39	 ft	(ft)	(/6")	(tsf)	(%)	First Encounter Upon Completion After Hrs.	562.4	π <u>▼</u> ft	(ft)	(/6")	(tsf)	(%
Concrete 7" slab with rebar	565.39	_				2000000			_			
Fill: Fine to Medium Sand With	303.33		4									
Silt (SP-SM) Very dark brown, dry to moist,		. 🗆	8									
medium dense, little gravel, fine to			5									
medium sands, trace coarse	563.39		4									
sands		_	2									l.
Fill: Sandy Lean Clay (CL)		V	4	1.8								
Very dark gray mottled with greenish gray, moist to wet, stiff,		_	3	Р						1		
faint petroleum odor, trace		5	2						-25			
medium to fine gravel, with sand		_	-						-			
seams Fill: wood matter with fine to		-	2						_	-		
coarse sand, strong petroleum		-	1	_	-				-	1		
odor, saturated, possible old		_	2						_	1		
railroad ties	558.39	-	2						-	1		
Fill: Silty Sand Trace Gravel	330.33	-	2									
(SM)		_	1						_			
Top 5": Brown, wet, root matter with petroleum odor and root			2							1		
matter throughout		-10	1						-30			
Remainder: Silty Sand trace												
gravel, dark to medium gray, wet,	555.39											
non plastic, medium to fine sands, trace subrounded fine gravels,			2			E 1 (5)			_			
loose, faint petroleum odor			1			End of Boring			_			
Encountered WT at 10' bgs		-	2									
Silty Fine to Coarse Sand (SM)	553.39		15							-		
trace gravel, brown, wet, very loose to medium dense, faint		_	30									
petroleum odor, occasional root,		_	50/2						_			
possible native soil, non odorous									_			
Sandy Silt With Clay And Gravel		-15	1						-35			
(CL) Top 2": Dark brown followed by	550.56	-	1						-	1		
yellowish orange and then light												
gray at bottom 2", wet, non plastic,		-	1						_ :=			
very angular flat coarse to fine		-							-			
gravels (possible rock fragments), some medium to fine sands with												
silt and few clay, possible									_			
gumbo/residual soil Driller began		_										
to set up for rock coring at 0950		-	1									

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

CI	ivision of Highways H2M HILL		Now 1.7	4 Bridge Ov	or Minnin-i-	ni Dive-	Illine		D	ate 9	/19/0
	1-74		New I-7	A Bridge OV	Approach	pi Kiver -	IIIIIIOI	_ LO	GGED	BY F.	Ab
	'4 Bridge over Missi River		ON (N=5	65232.456,	E=2459065	.732), SE	C. 32	TWP.	18N, F	RNG. 1W	, 4 th
COUNTY R	cock Island CO	RING METHOD _	Double to	ube, 10 ft co	re barrel, No	Q wireline	e, dian	E	R	CORE	
		CORING BA	RREL TYP	E & SIZE			C	CO	Q.	T	1
Station		Core Diam	eter		in	E				M	1
BORING NO.	ILR0201-R	Top of Roo	k Elev.	550.56	ft	F		E R	D	E	
Station	ILR0201-R 130 + 16	Begin Core	e Elev	550.56	ft			Y			1
Offset	11' Lt.								(9/)	(min/ft)	
Ground Surfactions	ce Elev. 566.39	_ ft				(1	t) (#)	V 100 100	(%)	(min/ft)	(t
undulated, little sample, remain top 16", 16" to 3 no rock wall cor sound to moder run 1/2-1/2-1/4-3/4-light gray milky 23"-31.5" = end Medium to fine a strong 21.42' - 15", 51", 67" an has tightly heale stains, no infillin bedded through	water, brown water of run grained, smooth tex 15° to 45° degree fid 88" from top, harded at most fractures g and surface stain out, stiff to very stiff to moderate fracture.	impermeable clay face stains only, su bottom dark gray a rock, remainder ti lose to moderate of 2.5' down and 7'-4' ture, slightly weath actures, irregular, impermeable clay except from 45" to 51", gray clay infilling i	infilling thi urfaces sta and brown ghtly heald discontinuidation dark brown ered to ur undulating infilling 1. 51" from from 57" that is 1/2'	roughout top ained greeni coal stains, ed with coal tites 23'-86' vn to dark gr weathered, g, slickenslic /8" to 1/2" th top, dark gr o bottom thi 't to 1/4" thicl	13" of sh gray at top 30": strands, = top of eeen medium leda t11", ick that ay surface nly a c at 1.1/4		-	R2 95	67		
End of Boring						534.97	-			-	_
Life of Boiling						-					
							-				
						-					

Color pictures of the cores

Cores will be stored for examination until

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)

Illinois Department of Transportation

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date 9/18/07

New I-74 Bridge Over Mississippi River - Illinois
Approach

LOGGED BY KB 1-74 DESCRIPTION ROUTE | 1-74 | Bridge over Mississippi | 1-74 | Bridge over Mississippi | LOCATION (N=565145.331, E=2459082.04), SEC. 32, TWP. 18N, RNG. 1W, 4th PM COUNTY Rock Island DRILLING METHOD HSA, CME 55 HAMMER TYPE CME AUTOMATIC

STRUCT. NO Station			D E P	B L O	U C S	M 0 1	Surface Water Elev Stream Bed Elev	ft ft
BORING NO	ILR0203 131+05	_	H	W S	Qu	S	Groundwater Elev.:	4
Station Offset	13' Lt.	-			4.0		First Encounter Upon Completion	ft
		ft	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft
Concrete Surface: 3" of co	oncrete	567.43						
Silty Sand (SM) dark brown and moist, very loose	black, slightly e, fine to medium		-	2				

grained, low plasticity			2		
granios, ion piacaony			2		
		-	2		
		-	2		
			4		
		-5	2	1.5	
		-0	2	Р	
	561.93	-			
Sand Silt and Clay (ML)	301.93		0		
Black, moist		-	2		
NOTE: Sample 3 grain size		-	3		
analysis performed		-	3		
Clay (CH)	559.93	-	2		
Clay (CH) black, slightly moist, firm to stiff,		-	3	1.8	25.0
trace fine sand, moderate					25.0
plasticity		1	3	Р	
Rimac: Pu = 94 lbs		-10			
NOTE: Sample 4 Atterberg limits:		20	2		
LL=63, PI=46			3	1.0	23.0
			5	Р	
Rimac: Pu = 28 lbs			1		-
			2	0.5	
		-	3	Р	
		-	,,113		
brown, very dense, fine to medium			5		
grained, Same as above, sandy		-15	15		
gravel in tip, brown, very dense, fine to medium angular gravel <1"			22		
diameter	551.93				
Sandy Gravel (GP)			50/3"		
light gray, wet, very dense, fine to					
medium angular gravel, fine to coarse sand		_			
	549.93				

End of Boring

	-20			
The Unconfined Compressive Streng	th (UCS) Failure N	lode is indicated b	y (B-Bulge, S-Shear, P-Pe	netromete
he SPT (N value) is the sum of the la	ast two blow value	s in each samplin	g zone (AASHTO T206)	

BBS, from 137 (Rev. 8-99)

USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

(P)	Illinois Department of Transportation
	Division of Highways CH2M HILL

Page <u>1</u> of <u>3</u>

(A)	of Transpo	rtati	on			30	IL DOKIN	G LUG				
	Division of Highways CH2M HILL									Date	9/1	8/0
OUTE	1-74	DE	SCRI	PTION		w I-74	Bridge Over Mississipp Approach	River - Illinois	LOG	GED BY	F. A	Abre
ECTION _	I-74 Bridge over Miss River	sissippi	_ ı	OCAT	ION	(N=56	5046.146, E=2459048.2	298), SEC . 32, 1	WP . 18	N, RNG	. 1W, 4	4 th P
OUNTY _	Rock Island Di	RILLING	S ME	THOD	_	H	HSA, CME 55	HAMMER TY	PE _C	OME AU	TOMA	TIC
TRUCT. NO		_	D	В	U	М	Surface Water Elev.	fi	D		U	N
Station		-	P	0	S	1	Stream Bed Elev.		P		S	I
ORING NO.			T	W		S	Groundwater Elev.:		T	W		S
Station	131 + 97 37′ Rt.	_	Н	S	Qu	1	First Encounter		▼ H	S	Qu	'
Offset Ground Sur		— ft	(ft)	(/6")	(tsf)	(%)	Upon Completion _ After Hrs.	f	(fi	(/6")	(tsf)	(%

Station 131+97 Offset 37' Rt		Н	S	Qu	Т	First Encounter 557	<u>'.9</u> ft ▼ ft	Н	S	Qu	Т
Ground Surface Elev. 569.92	ft	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft)	(/6")	(tsf)	(%
Topsoil light brown silt, hole offset 4.5' west of marked boring location	568.92	-	3			Silty Clay (poss. weathered Shale) (CL-ML) Gray with olive green, dry to	548.67				
Fill Silt With Sand And Gravel		-	4	4.3		moist, very stiff, trace coarse		-	-		
(ML)		-	6	P.0		gravel, very brittle, shale-like cl top of Rock 18'8" bgs (continue		-	-		
Yellowish orange transitioning to brown, dry to moist, non plastic,		-	5	375		Borehole continued with rock	u)	-			
medium to fine sands, little		_	-			coring.		_	1		
angular flat coarse to fine gravels,		-						-			
possible fill, occasional root matter	r	_	2					_			
Possible underground obstruction (concrete) 4'6" bgs		-	50/2					-25			
(concrete) 4 6 bgs		-5	(-20			
	563.92	_						-	1		
Poorly Graded Medium to	000.02		1						1		
Coarse Sand (SP)		-	1					-			
Brown, dry to moist, loose to very			1						1		
loose, trace gravel NOTE: Sample 3 grain size	561.92	_	2								
analysis performed	001.02		1						1		
Very Silty Sand (SM, ML)	J	_	1			1					
Brown, moist, very loose			2						1		
Sample 4: grain size analysis	559.92	-10	100					-30			
performed	500.02	- 10	2						1		
Very Clayey Fine to Medium Sand (SC)			1	1.3		1			1		
trace coarse sand and gravel,			1	P					1		
greenish gray, moist to wet, stiff,		•	4						1		
with root matter, occasional fibers		-	_						1		
with "muck-like" appearance	556.92								1		
Sample 5: grain size anaylsis and Aterberg limit tests (LL=27, PI-12)			2								
performed	1		2	2.0	29.0	1					
Clay (CH)	_		4	P					1		
Bluish gray mottled with orange		-15	5					-35			
brown, moist to wet, very stiff, little	9					1					
coarse-fine sands, trace gravels											
possible glacial till, reddish brick											
like gravel particles											
Sample 6: grain size analysis and Atterberg limit (LL=68, PI=12)											
tests performed											
nowan province			20							1	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

ROUTE	1-74		DESC	RIPTION	I-74 Bridge Ove		i River -			GGED	BY F	Ah
	I-74 Bridge o	ver Missis	ssippi	1								
SECTION _	R	liver		LOCATION (N	=565046.146,	E=2459048.2	298), SE (2. 32,	TWP.	18N, F	RNG. 1W	, 4 th
				THOD _ Double				diamo	E	R	CORE	
STRUCT. NO)		CO	RING BARREL T	YPE & SIZE		D	С	C	à	T	
Station				ore Diameter		in	E	0	v		M	
ROPING NO	ILRO	204	To	op of Rock Elev.	548.67	ft	P	R	E	D	E	
Station	131-	97	В	egin Core Elev.	548.67	ft	T	E	R			
Offset	131 - 37'	Rt.					Н		Υ			
Ground Su	rface Elev.	569.92	ft				(ft)	(#)	(%)	(%)	(min/ft)	(1
to smooth fra discontinous joint walls, tig fractures, ho fractured, ex 3/4-1-3/4-1-1 28'-81" = top Gray to light 28'-26" - bot Kill switch or medium to fi	acture surface joints, greeni ghtly healed a rizontal beddi tremely close 1/2/6 of run gray water tom of run n rig broke, dr ne grained 2	ss, undulat sh-gray to it 12", 18" ng through to close d illing dtopp 5.83' - Ho	gray sur and 39" in nout top 2 liscontinu	es at top 18" of , clay infilling mate face stains, rock from top, bands 20" of sample, mitties Start 10:00 co. 55 am temporal and vertical fractures of the same start and control of the same start and same same same same same same same same	erial top 20", wall contact, a of sandy clay fr loderate to extr 0 rilly\ ures at top 16" o	Iltered actions at emely	41.92	NQ-R	2 56	27		
undulating s sandy clay ir clay infilling, little or no su bedding thro impermeable to medium d .75·2-1/4·2·2	urfaces, remanifilling materia 4" thick zone urface stains a ughout, thick of filling from 2 iscontinuities	inder roug al at top 4" of clay inf at top 36", continous 5" to 67" f Start 13:	th and plate of samp filling from remainded zones of tom top,	30° fracture, top anar fracture sur le, stiff to hard cl n 45" to 49" and or stains dark graf f sand clay infillir sound to modera	faces, residual lay, impermeab at bottom 4" of ay, horizontal to ng, tightly heale	soil, soft le gray sample, 30"		0				
Change to v 1-1/2·3/4·2·1	ery dark gray -3/4·2-1/4	fluid at 31	'4" bgs									
Bit pressure	- 250 psi						3					
Hole pluggin Some fluid le	ig at 32'-33' b	gs					-	-				
weak to med 65" from top With strands	dium strong ro	ck, top 4"	residual towards	top 4", remainde soil, brittle shale bottom of sample hered, medium t	-like clay infillin e, gray with ligh	g 47" and		NQ-R	3 100	48		

Top 60" Limestone
Bottom 60" Sandstone 35.83' - Horizontal to 30° fractures throughout, smooth
undulated fractures and irregular undulated fractures from 90" to 120" from top, 1/8" to
1" thick bands of hard __impermeable clay infilling throughout, tightly healed at
most fractures, bands of coal minerals at bottom 30" of sample, sound to medium

Color pictures of the cores

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



ROCK CORE LOG

Page 3 of 3

Date 9/18/07

COUNTY Ro	River	ORING METHODD	N (N=565046.14						CORE	S T
STRUCT. NO Station BORING NO Station Offset Ground Surface	ILR0204 131+97 37' Rt.	Core Diamete Top of Rock Begin Core E	Elev. 548.67	in ft	D E P T H	C O R E (#)	C O V E R Y	Q . D . (%)	T I M E	R E N G T H
gractured, moder vertical fracture a	ate to very close	discontinuties, horizor tiff to very stiff clay infil	ntal to 70" thick be ling through fract	edding, ure from 16"		77	(,	17	(,	(
to 24" from top, eminated rock co	oring at 45' 10" h	as @! 14:17								
Limestone	NAME OF THE PARTY OF	dual soil at top 4", rema	ainder slightly wea	athered.	-					
	strong rock, top	4" residual soil, brittle s			-45					
				52	4.09_					
End of Boring										
					-					
					-					
					-50					
					50					
					55					
					-55					
					-					
					_					
					-60					

Color pictures of the cores

Cores will be stored for examination until

Cores will be stored for examination until _____
The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)
BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - YSS	REVISED	Π
	CHECKED - JMH	REVISED	
PLOT SCALE =	DRAWN - MLA	REVISED	
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	

Illinois Departr of Transportati	ion	1		SC	IL BORIN	G LOG	
CH2M HILL			Ne	w I-74	Bridge Over Mississipp	oi River - Illinois	Date9/18/0
ROUTE DE	SCR	IPTION		200 500	Approach	L(OGGED BYKB
SECTION I-74 Bridge over Mississippi	_ 1	OCAT	ION _	(N=56	4896.826, E=2459062.	562), SEC. 32, TWP.	. 18N, RNG . 1W, 4 th F
COUNTY Rock Island DRILLING	S ME	THOD	_		HSA, CME 55	_ HAMMER TYPE	CME AUTOMATIC
STRUCT. NO.	D E		U	M	Surface Water Elev.	ft	
Station	P	0	S	I	Stream Bed Elev.	π	
BORING NO. ILR0205	T	W		S	Groundwater Elev.:		
BORING NO. ILR0205 Station 133 + 47 Offset 47' Rt	Н	S	Qu	Т	First Encounter	559.9 ft ▼	
Offset 47' Rt.	(64)	//em	(4-6)	(0/)	Upon Completion After Hrs.	ft	
Ground Surface Elev. 567.92 ft	(π)	(/6")	(tsf)	(%)	After Hrs.	ft	
Concrete 567.42							
3" of concrete Silty Fine to Medium Sand (SM)	_						
black, slightly moist, loose, black , slightly moist	-						
slightly moist	-	1 2					
	=	3					
Sandy Silt (ML)							
Sandy Sift (ML) black, slightly moist, very soft to	-	0	4.0				
stiff	-5	0	1.0 P				
		0					
	100	0					
		2					
559.92							
Silt (ML)	-	3					
dark greenish and brown, loose to medium dense, moist, trace fine		3	1.5				
sand	-	3	Р				
	10						
sandstone in tip pale		5					
		12					
	_	16					
555.92 Shale		4					
pale olive brown, dense, moderate	-	9			-		
plasticity	_	30					
	-	-					
	-	50/2					
	-15		1.5				
552.42	-		P				
End of Boring							
	_						
					II .		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

of Transpo	ıtatı	JII			50	DIL BORIN	CLOO	Date 9/21/0
ROUTE I-74	DES	SCRI	PTION	Nev	w I-74	Bridge Over Mississippi Approach	i River - Illinois	
I-74 Bridge over Miss SECTION River	issippi	_ L	OCAT	ION _	(N=56	4822.636, E=2459073.6	518), SEC. 32, TWP	. 18N, RNG . 1W, 4 th I
COUNTY Rock Island DF	RILLING	ME	THOD			HSA, CME 55	HAMMER TYPE	CME AUTOMATION
STRUCT. NO		D E P T H	B L O W S	U C S	M O I S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter	ft	
Offset 48' Rt. Ground Surface Elev. 568.24	— ft	(ft)	(/6")	(tsf)	(%)	First Encounter Upon Completion After Hrs.	ft ft	
Concrete		(-)	V - 7		3.552	Alterins.		
7" concrete underlain by dark brown silty clay with sand	567.24	-						
Fill Silty Clay With Sand (CL-ML)			2					
Dark brown, dry to moist, medium			3					
stiff to stiff, with reddish brick		-	3 2					
material top 1" of sample contained crushed concrete	565.24		2					
Fill Sandy Silt (ML)	564.49	-	2	1.8		-		
Brown to dark brown, dry to moist,			2	P				
non plastic, loose		-5	3					
Fill: Clayey Silt (ML) Very dark gray to black, moist, low		-0	-			1		
plasticity, stiff, non odorous, trace fine sand		<u>~</u>						
Fill Silty Fine to Medium Sand (SM)		_		1.0 P				
Gray to brown, moist, trace coarse sand	560.24							
Medium to Coarse Sand Little Silt			2			1		
And Gravel (SM)	559.44		36			1		
dark brown mottled with orange		-	47					
brown, wet, loose Sample 3 (8'-10'): grain size		-10	32					
analysis performed								
Clayey Sand With Silt (SC)			50/2					
light gray with greenish gray,		-	30/2					
moist, very dense Silty Fine to Medium Sand (SM)		-						
light gray to white with yellowish		_						
orange streaks, moist to wet, very			32					
dense, possible completely		-	15			1		
weathered sandstone little coarse sands, trace fine			50/4					
gravels, possible completely		-15						
weathered sandstone				7				
Driller notes rough drilling and	552.57		1					
chatter 11.0' bgs, possible weathered rock								
Light gray, moist to wet, very			1					
dense, medium to fine sands with								
silt, trace coarse sands, strong								
cementation, coarse to fine angular flat gravels and sandstone								
fragments, little clay (possible								
infilling) throughout, possible								
completely weathered sandstone		-20						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

(V)	Illinois Department of Transportation
	Division of Highways CH2M HILL

ROCK CORE LOG

Page <u>2</u> of <u>2</u>

Date 9/21/07

ROUTE	1-74	DESCRIPTION		4 Bridge Ov		ppi River - I			GGET	BY F	Abre
	I-74 Bridge over				трргоаоп				OOLL	, DI	71010
	River	LOCATIO	ON (N=5	64822.636,	E=245907	3.618), SEC	2. 32,	TWP.	18N, F	RNG. 1W	, 4th F
COUNTY	Rock Island	CORING METHOD _	Double tu	ube, 10 ft co	re barrel, N	NQ wireline,	diamo	E	R	CORE	S
STRUCT NO		CORING BAR	REI TYE	F & SIZE		F 25		C		T	R
			WILL III	L & SIZL _		D	C	0	Q	1	E
Otation		Core Diame	eter _		in	E	0	V		M	N
BORING NO.	ILR0206	Top of Roc	k Elev	552.57	ft	P	R	E R	D	E	G
Station	134 + 22	Begin Core	Elev	552.57	ft	Н	E	Y			Н
Offset	48' Rt.										
Ground Sur	face Elev. 56	8.24 ft				(ft)	(#)	(%)	(%)	(min/ft)	(ts
Sandstone						552.57	NQ-R	1 90	7		
greenish grey crushed rock discontin fractured 23 / 23 / 25 / 25 / 25 / 25 / 25 / 25 /	or surface stains, 1-2mm thick-enou nuous joints, extr-98"= top of run foot m of run 24 nangular to angul thy weathered to the shale-like spangular to angul es surfaces, slight joint with very sent rock wall con 3/4 min/foot aver de 6" from bottom to light gray, smoot light gray, s	dar, Remainder: Sandstounweathered, medium sith that has hardened and far, Remainder: Horizon thy altered joint walls 14 tiff to hard shale-like cligatact, surfaces stained datage	o 41", sand ontact at incontinuitie one, gray in trong, roci formed a situation 20 "" to 24" fro y breaks to ark gray po	dy clay mate remaining fr is, moderate with dark grak appears to solid rock [2] fractures, room top, Rem hick enough ossibly do to	arial and actures, a to fine ay, fine be 20.67' - sugh to hainder a (<1/4")	-28	NQ-R		18		
from top 25.67' - 10° to no infilling ma blotches of g make a conti CR: 1 min/fo	o 30° fractures, r aterial, gray surfa reenish gray clay nuous sample fro ot average	ough and irregular undul oce staining only, slightly that has possibly filled v om 6" to 34" from top, tig	ating fract altered to reins in ro	ture surface altered join	s, little or it walls, lened to	-3/537.41	0				
End of Boring	9					_					
						-	1				
						-					
						7					
						-	1				
						-					
						5	+				
						- 5					
						3	5				

Color pictures of the cores _____
Cores will be stored for examination until
The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - YSS	REVISED
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PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

C.F	HANSON
~	

Page <u>1</u> of <u>1</u> Date 6/29/10

CONTRACTHANSON

ROUTE _____ F.A.I. 74

TOPSOIL FILL - Very dark brown, moist, very stiff, silty, fine- to medium-grained SAND and GRAVEL

Dark brown, wet, well-graded, SAND with trace silt

Gray, hard, very fine-grained, WEATHERED SANDSTONE End of Boring

Brown, wet, medium- to coarse-grained SAND

Grayish green, moist, very stiff, fat \$\frac{564.00}{\nabla}\$\$\$

Creosote timber

81B COUNTY Rock Island DRILLING METHOD

565.00

560.50

558.50

557.50

556.80

7 2.00P 13

5 0.81B 30

SECTION ___

SOIL BORING LOG

Hollow Stem Auger

Page <u>1</u> of <u>1</u>

Date 6/28/10 DESCRIPTION I-74 Over Mississippi River LOGGED BY JMB LOCATION NE1/4 of SEC. 32, TWP. 18N, RNG. 1W, 4th P.M. HAMMER TYPE

SOIL BORING LOG

HANSON

Page <u>1</u> of <u>1</u>

							Date 6/29/10
ROUTE F.A.I. 74	DESCI	RIPTION	N		I-74 Over Mississippi I	River LOG	GED BYJMB
SECTION 81-1HVB		LOCA	TION	NE¼	of SEC. 32, TWP. 18N,	RNG. 1W, 4th P.M.	
COUNTY Rock Island D	RILLING M	ETHOD		Но	llow Stem Auger	HAMMER TYPE	Auto
STRUCT. NO. 081-6011 Station RW 02-1 BORING NO. RW 02-1 Station 130+12 Offset 25' Rt. Ground Surface Elev. 566.2	E P T H	O W	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft ft ▽	
	565.70						
FILL - Dark to very dark brown, moist to wet, soft and loose, silt, fine- to coarse-grained sand and gravel, with degrading plywood, particle board, timber, lumber,	2-	4 5 2		11			
bituminous materials, metal scraps, cinder blocks, and brick	-	-					
fragments, petroleum odor	₽	2 2 5	0.50P	20			
	6-	4 5	1.75P	18			
	-	6			_		
	8-	_					
	-	2 3 3		196			
	555.70 10-						
Brownish gray, wet, dense, clayey, silty, fine-grained SAND with trace gravel Gray, fine-grained, WEATHERED SANDSTONE	554.70	10 12 14	0.75P	20			
	-	-					
End of Boring	552.20 ₁₄	50/5"		17			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

ROUTE F.A.I. 74	DES	CRIF	PTION	ı		I-74 Over Mississippi	River	LOGGI	ED BY _	JMB
SECTION 81B		_ L0	OCAT	ION	NE¼ d	of SEC. 32, TWP. 18N,	RNG. 1W, 4t	h P.M.		
COUNTY Rock Island D	RILLING	MET	HOD		Hol	llow Stem Auger	HAMMER T	YPE	Auto)
STRUCT. NO. Station BORING NO. RDG 01		D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After 24 Hrs.	563.4	ft ft ▽		
Brown, moist, medium, sandy, lean CLAY										
		2-	4 4 50/2"	1.60P	19					
CONCRETE	567.40									
Brown, moist, soft, sandy, lean CLAY	566.90 566.40	4—	7 11	1.75P	25					
Very dark brown, dry to moist, very stiff, sandy SILT		7	15							
		6								
		_+		0.61S	16 21					
Dark brown, moist to wet, stiff, sandy SILT	563.40	-		0.013						
Very dark brown, wet, soft, sandy, clayey SILT	562.40	8—_			28					
		-01								
Gray, moist, stiff, lean CLAY with silt	<u>559.40</u> 1	12-	3 5 6	1.15B	28					
Gray, moist, stiff, lean CLAY with silt and fine-grained sand	556.90 1	14-	3 5 7	1.10B	29					
Cross band fire resided	554.40	16—								
Gray, hard, fine-grained, WEATHERED SANDSTONE		1	50/3"							
	551.80	8-								
End of Boring		1	50/1"							
End of Boring	301.00		50/1"							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99) The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS from BBS, from 137 (Rev. 8-99)

MODJESKI MASTERS

USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

BORING LOGS 4 RAMP RD-G RETAINING WALL 02 STRUCTURE NO. 081–6011	
CHEET NO 10 OF 10 CHEETC	

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
74	(81-1)R & 81-1HVBR	ROCK ISLAND	1504	1135
		CONTRAC	T NO.	64C0
	ILLINOIS FED. AI	ID PROJECT		

CA	HANSON
~	MINION

Page <u>1</u> of <u>1</u>

SECTION

BORING NO. ____ Station ____

FILL - Soil

CONCRETE

HANSON SOIL BORING LOG

| Station | 132+75 | | Offset | 40' Rt. | | H | S | Qu | I | | Hirst Encounce. | Upon Completion After Hrs. |

3 1.75P 30

0.41B 28

567.40

566.90

560.40

554.20

81-1HVB

STRUCT. NO. 081-6011

FILL - Gray, moist, loose, silty SAND, creosote wood pieces,

metal scraps, brick and concrete

Dark brown and dark gray, moist, soft to stiff, lean CLAY with silt

Gravish green, moist, stiff to very

Gray, fine-grained, WEATHERED SANDSTONE 12

Dark gray, WEATHERED SHALE

stiff, lean CLAY with trace silt

End of Boring

COUNTY Rock Island DRILLING METHOD

RW 02-3 132+79

 ROUTE
 F.A.I. 74
 DESCRIPTION
 I-74 Over Mississippi River
 LOGGED BY
 JMB

LOCATION NE1/4 of SEC. 32, TWP. 18N, RNG. 1W, 4th P.M.

Hollow Stem Auger

Page <u>1</u> of <u>1</u> Date 6/29/10

BBS from 137 (Rev. 8-99)

HAMMER TYPE

HANSON SOIL BORING LOG Date 6/28/10 ROUTE F.A.I. 74 DESCRIPTION I-74 Over Mississippi River LOGGED BY ___JMB LOCATION NE1/4 of SEC. 32, TWP. 18N, RNG. 1W, 4th P.M. SECTION 81-1HVB COUNTY Rock Island DRILLING METHOD Hollow Stem Auger STRUCT. NO. 081-6011 Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion (ft) (/6") (tsf) (%) After ____ CONCRETE 567.70 Very dark brown, moist, soft, silty, lean CLAY with fine-grained sand 2 0.25P 28 565.00 Very dark brown, moist, stiff, silty, lean CLAY with trace very fine-grained sand 0.26B 30 0.53B 26 1.79B 22 1.27S 19 2.50P 17 Brown, wet, silty, fine- to medium-grained SAND Brown, moist, medium dense, silty, fine-grained SAND with 558.00 gravel
Gray, WEATHERED SILTSTONE 12 50/5" 9 Gray, fine-grained, WEATHERED 554.50 SANDSTONE End of Boring

Page <u>1</u> of <u>1</u>

BBS, from 137 (Rev. 8-99)

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

COUNTY Rock Island DI	RILLING	ME	THOD		Но	llow Stem Auger	HAMMER 1	YPE	Auto
STRUCT. NO. 081-6011 Station BORING NO. RW 02-2	_	D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.			/ tuto
Station		T H (ft)	W S (/6")	Qu (tsf)	S T (%)	Groundwater Elev.: First Encounter Upon Completion After Hrs.	557.3	ft ft ∑ ft	
CONCRETE	567.80	_						·	
Brown, slightly moist, very stiff, clayey SILT with sand		2-	5 3 2	2.25P	15				
Dark brown, moist, soft, silty, lean CLAY with trace sand	564.80	4-	2 2 4	0.44B	27				
		6-		0.49B	29	-			
Grayish green, moist, stiff, silty,	559.70	8-		1.80B 2.75B					
ean CLAY		- 10-				-			
	556.30	∑ - 12-	6 8 8	0.40B	32				
Brown, wet, medium dense, silty, medium-grained SAND with gravel Gray, fine-grained, WEATHERED	555.80	_							
SANDSTONE End of Boring	554.20	14 —	15	1.76S	10				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

The Unconfined Compressive Strength (UCS) The SPT (N value) is the sum of the last two b			enetrometer)
, , , , , , , , , , , , , , , , , , , ,			DDS from 127 (Dov. 9.00)

REVISED USER NAME = DESIGNED - YSS CHECKED - JMH REVISED MODJESKI === MASTERS DRAWN MLA REVISED PLOT DATE = 1/20/2017 CHECKED -YSS

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** BORING LOGS 5
RAMP RD-G RETAINING WALL 02 SECTION COUNTY 74 (81-1)R & 81-1HVBR ROCK ISLAND 1504 1136 STRUCTURE NO. 081-6011 CONTRACT NO. 64CO8 SHEET NO. 11 OF 12 SHEETS

Illinois Dep of Transpo Division of Highways CHZM HILL	oartmo rtatio	ent n			OIL BORING LOG			12/3	
OUTEI-74	sissippi		ı	0000 0000	Bridge Over Mississippi River - Illinois Approach				
ECTION River		LOCAT	ION _	(N=56	5278.629, E=2459057.591), SEC. 32, TWP.	18N, I	RNG.	1W, 4	1 th PN
OUNTY Rock Island DI	RILLING M	ETHOD	_	H	HSA, CME 55 HAMMER TYPE	СМЕ	E AUT	ГОМА	TIC
TRUCT. NO. Station ORING NO. RW1501 Station 129 + 69 Offset 10' Lt.		L O W	U C S	M O I S T	Surface Water Elev. ft Stream Bed Elev. ft Groundwater Elev.: First Encounter 562.7 ft Upon Completion ft	D E P T H	B L O W S	U C S Qu	M O I S T
Ground Surface Elev. 570.72	ft (f	t) (/6")	(tsf)	(%)	After Hrs ft	(ft) (1.00	(tsf)	(%
Clayey Gravel (GC) Clayey ravel to fine grained sand, dark rown to brown, dry to moist, tratified. Borehole moved out of nounds of debris for safety easons silfty Sand (SM) Fine grained	568.72	9 12 9 8 3 4			Clay to Silt (CL-ML) Clay to silt, 550.22 dark gray brown to light gray, wet, poss, whrd shale Auger refusal at 20.5' Decontaminate equipment starting at 8:56 am. End of Boring	-	50/5		
and, brown, dry to moist, omogeneous.	-	3 2			End of Boring				
etroleum odor from 4-6'.		1 -5 1 1				-25			
NOH = Weight of Hammer	_	WOH WOH 1 WOH		19.0					
Clay (CH) Clay, dark brown to	562.72 ▼	2				_			
lack, moist, homogeneous. Shelby tube from 8ft-10ft obtained from adjacent boring. CD Triaxial est and Atterberg limit (LL=50, Pl=23) test performed.		WOH WOH 2		56.0		-30			
clayey Sand (SC) Clayey sand, ace organics, dark brown to rown, moist to 12.0', wet deeper, omogeneous	- -	WOH WOH				=			
	556.72	6 4		48.0		-			
and and Clay (SP,SC) Sand ind clay, trace gravel, trace irganics, dark brown, wet, omogeneous		3 15 1 10				-35			
		15 33							
						=			
	550.72	-20				-40			

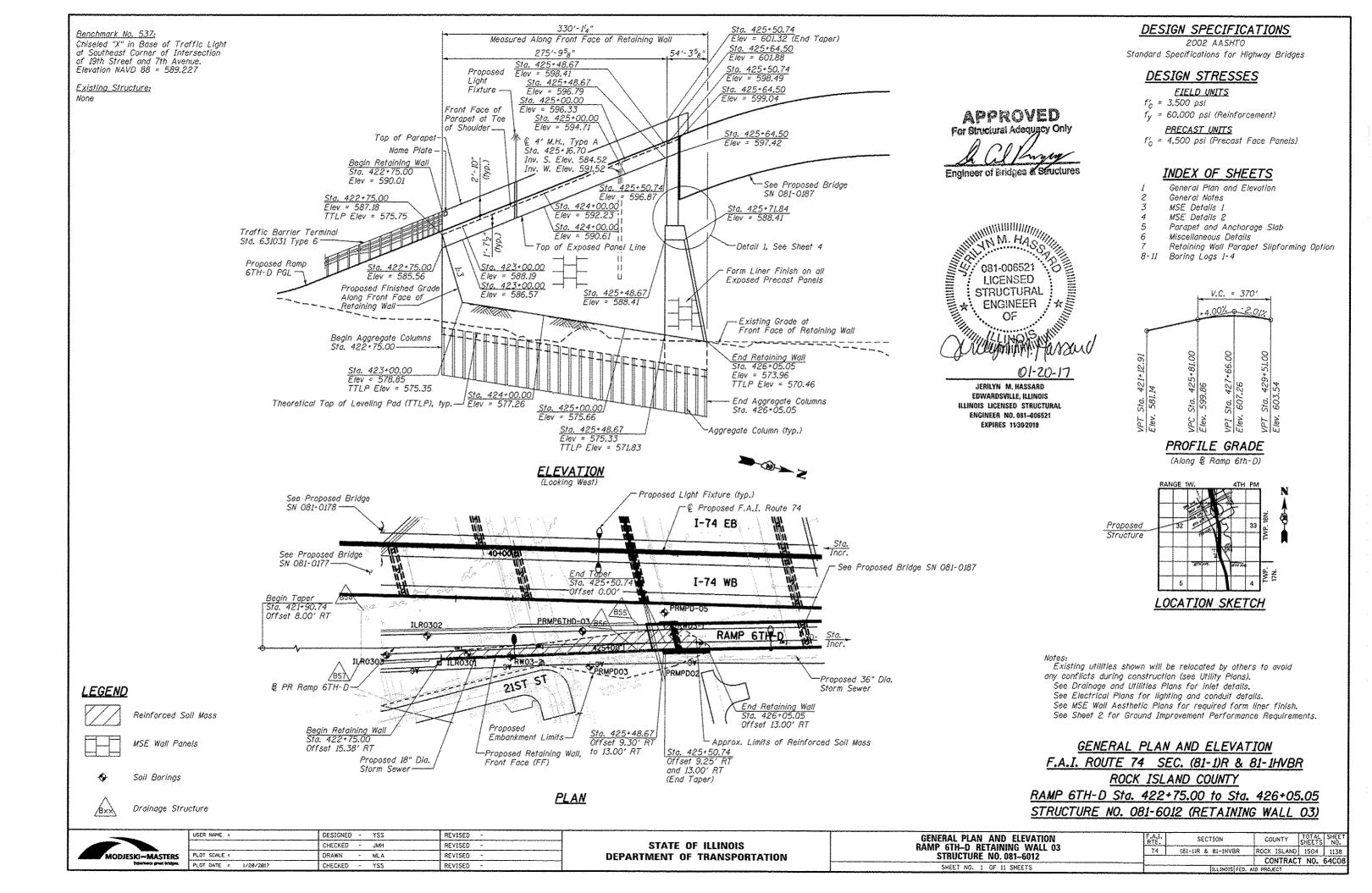
The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

MODJESKI == MASTERS
Experience great bridges.

	USER NAME =	DESIGNED - YSS	REVISED	
		CHECKED - JMH	REVISED	
5	PLOT SCALE =	DRAWN - MLA	REVISED	
	PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BORING LOGS 6
RAMP RD-G RETAINING WALL 02
STRUCTURE NO. 081-6011
SHEET NO. 12 OF 12 SHEETS



GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Wall stations and offsets are given to the front face (FF) of the wall and are measured from the Ramp 6TH-D baseline, except as noted. FF of the wall is to be considered edge of panel or form liner.
- 3. See Special Provision for Mechanically Stabilized Earth Retaining Walls and Aggregate Column Ground Improvement for design and construction requirements.
- 4. For existing soils laboratory data, see the Geotechnical Investigation Laboratory Data Special Provision.
- 5. The piles for SN 081-0187 are located within the reinforced soil mass. See SN 081-0187 plans for additional pile requirements.
- 6. Wall system supplier shall coordinate proposed wall configuration with Aggregate Column Ground Improvement subcontractor.
- 7. Wall construction shall not begin until after Aggregate Column Ground Improvement has been completed in the area of the new wall.
- 8. Obstructions such as old foundations, pavements, utilities, etc. that are within the area to be treated with Aggregate Column Ground Improvement shall be removed by others.
- 9. See SN 081-0187 plans for maskwall details.

GROUND IMPROVEMENT PERFORMANCE REQUIREMENTS

- 1. Minimum factor of safety for global slope stability shall be 1.5.
- 2. Allowable bearing pressure (with F.S.) shall be equal to or greater than the equivalent uniform service bearing pressure of 5000 psf.

Minimum factor of safety against equivalent uniform service bearing pressure shall be 2.0 if a load test is performed.

Minimum factor of safety against equivalent uniform service bearing pressure shall be 2.5 if a load test is not performed.

- 3. Total settlement measured at the theoretical top of leveling pad shall not exceed
- 4. Total settlement measured on the pavement shall not exceed 1.0 inch.
- 5. Differential settlement measured along the theoretical top of leveling pad shall not exceed 1/100.
- 6. The assumed structure life for settlement computations shall be 75 years.
- 7. Contractor's verification program shall include monitoring points or other instrumentation to demonstrate compliance with the stated performance requirements.
- 8. The Shop Drawings and construction procedures submittal shall indicate the sequence of construction within the limits of Aggregate Column Ground Improvement. The aggregate column installation shall be coordinated with utility removal, structure removals, proposed utility installation, and bridge pile driving.
- 9. Aggregate columns shall be installed before the bridge piles are driven; however, the piles shall not be driven through the aggregate of an installed column. The aggregate column layout shall provide clearance for the bridge piles.

MSE WALL SETTLEMENT

1. The Top of Exposed Panel Elevations shown on these plans are final elevations after any settlement. The wall settlement will be determined by the ground improvement design. The wall system supplier shall coordinate with Aggregate Column Ground Improvement subcontractor to accommodate this settlement in the wall design.

REVISED

REVISED

REVISED

REVISED

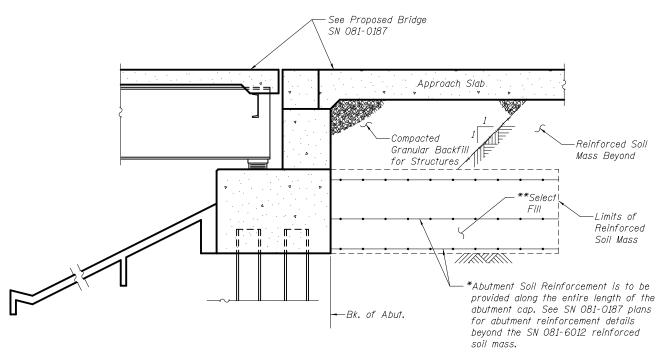
TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Structure Excavation	Cu. Yd.	368
Concrete Superstructure	Cu. Yd.	148.3
Protective Coat	Sq. Yd.	339
Reinforcement Bars, Epoxy Coated	Pound	22,630
Name Plates	Each	1
Aggregate Column Ground Improvement	L. Sum	0.57
Mechanically Stabilized Earth Retaining Wall	Sq. Ft.	5791

^{*} See proposed retaining wall S.N. 081-6019 for remainder of L. Sum quantity.

STATION 422+75.00 BUILT 201_ BY STATE OF ILLINOIS F.A.I. RT. 74 SEC. (81-1)R & 81-1HVBR LOADING HS-20 STR. NO. 081-6012

NAME PLATE



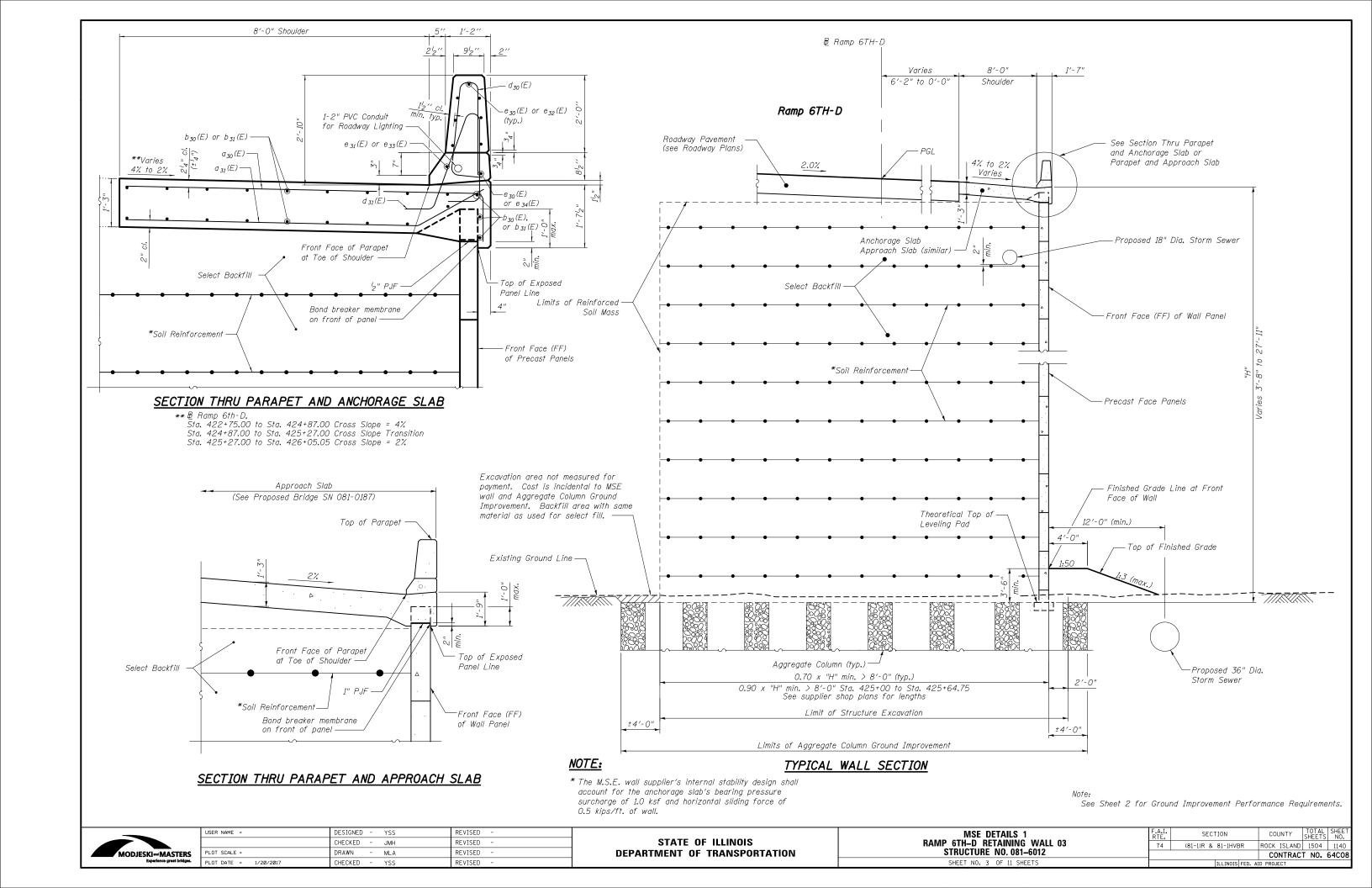
SECTION THRU PILE SUPPORTED STUB ABUTMENT

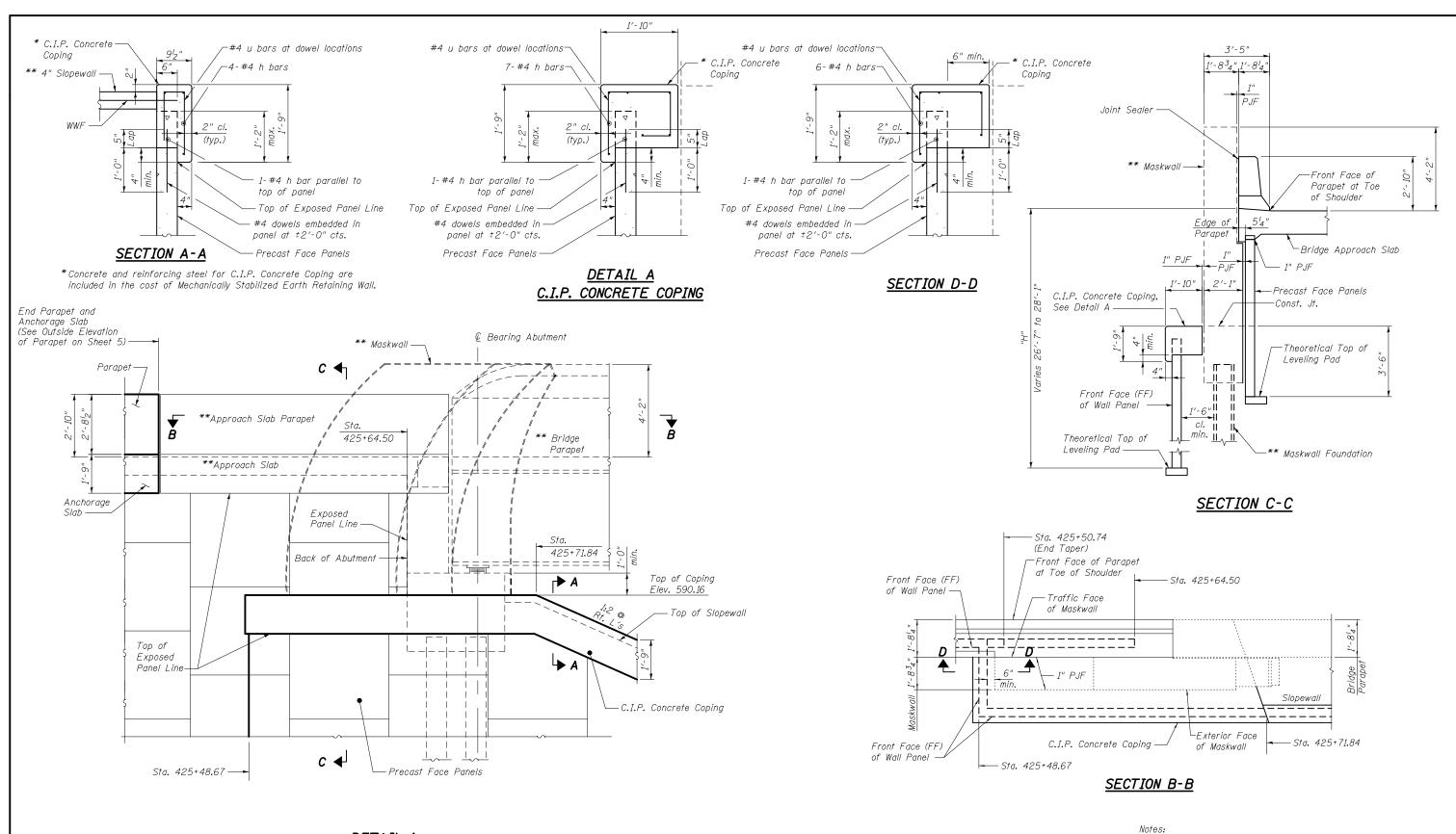
- * The M.S.E. wall supplier shall design the abutment soil reinforcement to resist a horizontal force of 3.0 kips/ft of abutment. Cost shall be included with the cost of "Mechanically Stabilized Earth Retaining Wall".
- ** Select fill shall be placed in all areas beneath the proposed abutments regardless of the limits of the reinforced soil mass.

	USER NAME =	DESIGNED	-	YSS
		CHECKED	-	JMH
MODJESKI MASTERS PLOT SCALE =		DRAWN	-	MLA
Experience great bridges.	PLOT DATE = 1/20/2017	CHECKED	-	YSS

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** GENERAL NOTES
RAMP 6TH-D RETAINING WALL 03 **STRUCTURE NO. 081–6012** SHEET NO. 2 OF 11 SHEETS

SECTION COUNTY 74 (81-1)R & 81-1HVBR ROCK ISLAND | 1504 | 1139 CONTRACT NO. 64C08





DETAIL 1

(Maskwall foundation not shown for clarity.)

** See Proposed Bridge SN 081-0187

The soil reinforcement limits for the upper and lower MSE walls shall meet the design requirements provided within the Typical Sections. The width of the lower wall soil reinforcement shall be designed based on "H" as dimensioned in Section C-C. The width of the upper wall soil reinforcement shall be designed based on the height from the upper wall Theoretical Top of Leveling Pad to the Toe of Shoulder and shall be equal to or greater than the limit of soil reinforcement required for the lower MSE wall. For location of Detail 1, see Sheet 1.

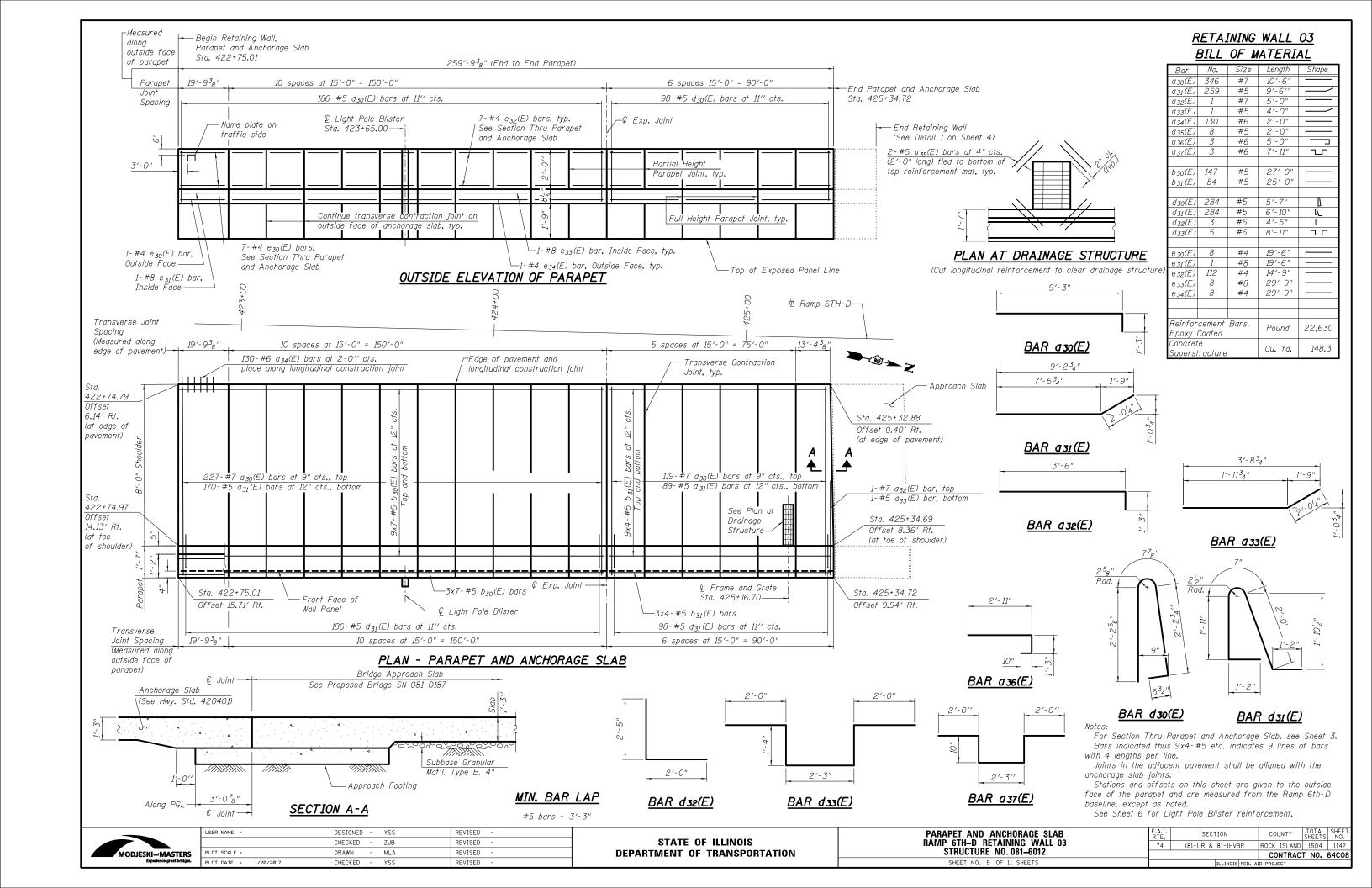


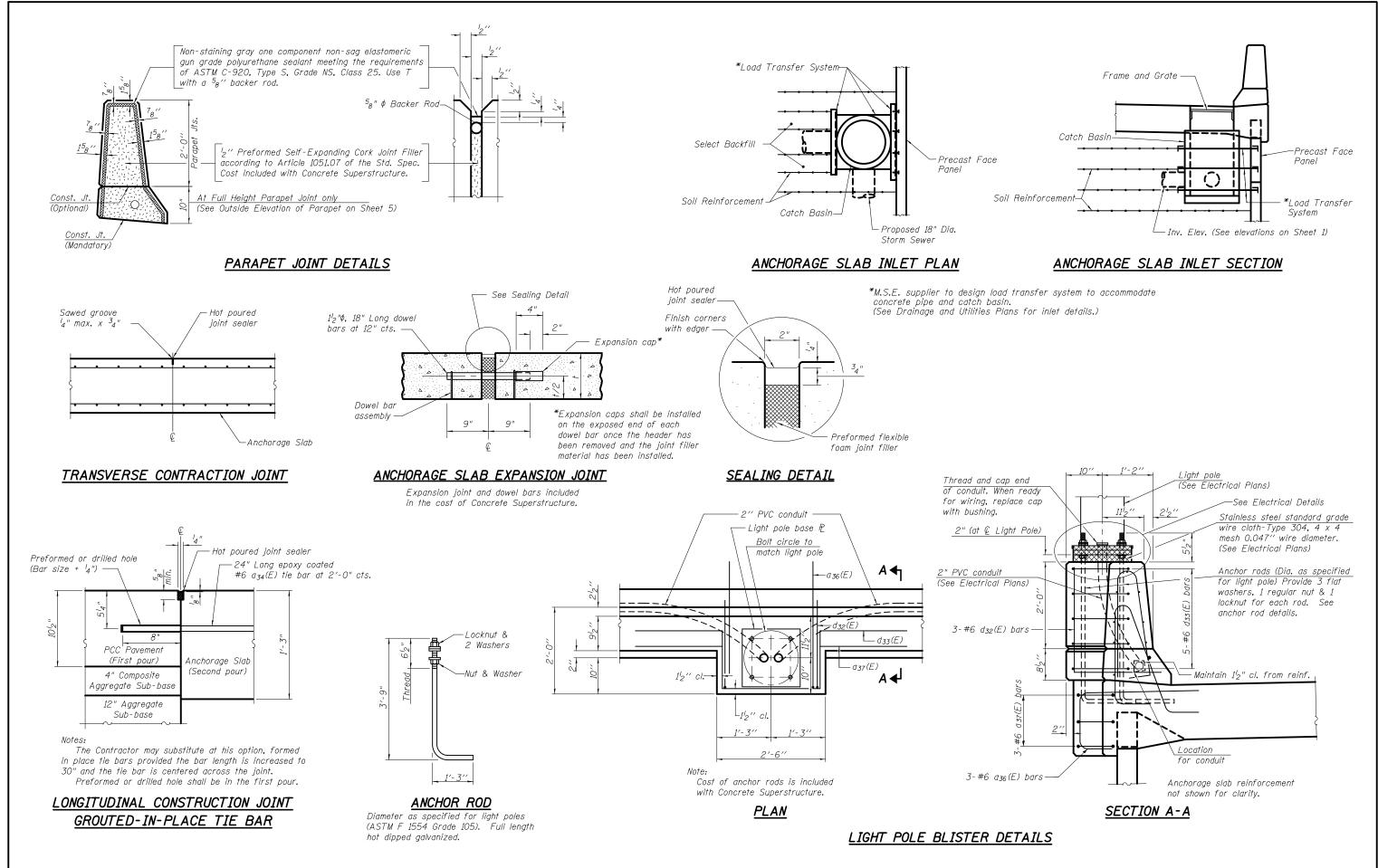
USER NAME =	DESIGNED - YSS	REVISED -
	CHECKED - JMH	REVISED -
PLOT SCALE =	DRAWN - MLA	REVISED -
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

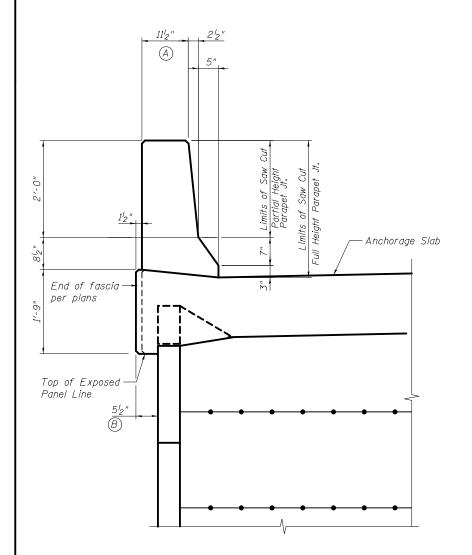
MSE DETAILS 2 RAMP 6TH-D RETAINING WALL 03							
STRUCTURE NO. 081-6012							
SHEET NO 4 OF 11 SHEETS							

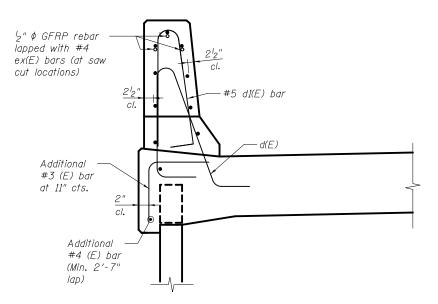
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.		
74	(81-1)R & 81-1HVBR	ROCK ISLAND	1504	1141		
		CONTRAC	T NO.	64C08		
	ILLINOIS FED.	AID PROJECT				





	USER NAME =	DESIGNED - YSS	REVISED -		MISCELLANEOUS DETAILS	F.A.I. SECTION	COUNTY	TOTAL SHEET:	SHEE NO.
		CHECKED - JMH	REVISED -	STATE OF ILLINOIS	RAMP 6TH-D RETAINING WALL 03	74 (81-1)R & 81-1HV	BR ROCK ISLAND	.D 1504	1143
MODJESKI and MASTERS Experience great bridges.	PLOT SCALE =	DRAWN - MLA	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 081-6012		CONTRAC	CT NO.	64C08
Experience great bridges.	PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED -		SHEET NO. 6 OF 11 SHEETS	ILLINOIS	FED. AID PROJECT		

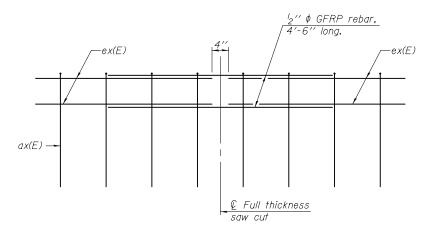




SECTION

(Showing reinforcement clearances for slip forming and additional reinforcement)

SECTION THRU PARAPET AND ANCHORAGE SLAB



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)



USER NAME =	DESIGNED - KMP	REVISED -
	CHECKED - SLD	REVISED -
PLOT SCALE =	DRAWN - KMP	REVISED -
PLOT DATE = 1/20/2017	CHECKED - SLD	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

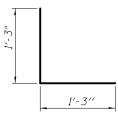
RETAINING WALL PARAPET SLIPFORMING OPTION RAMP 6TH-D RETAINING WALL 03 STRUCTURE NO. 081-6012 SHEET NO. 7 OF 11 SHEETS

SECTION COUNTY 74 (81-1)R & 81-1HVBR ROCK ISLAND 1504 1144 CONTRACT NO. 64C08

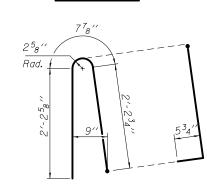
GENERAL NOTES

All dimensions shall remain the same as shown on superstructure details, except dimensions A and B which are to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension A equals 0.016 cu. yds./ft.

Full thickness saw cut at all joint locations in lieu of cork joint filler.



#3 (E) BAR



ALTERNATE BAR #5-d1(F)

(When conduit is present)

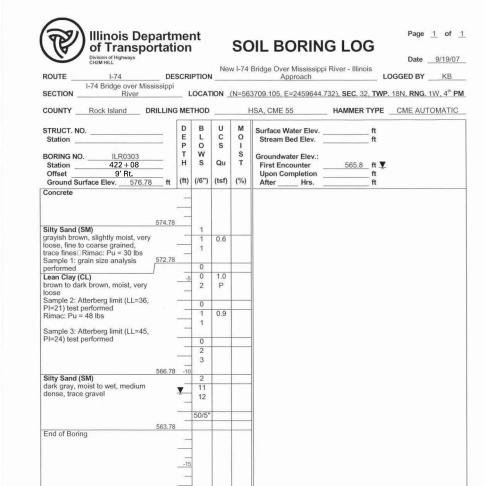
Division of Highways CH2M HILL			Nev	v I-74	Bridge Over Mississipp	i River - Illinois	Date 9/19/0
ROUTE 1-74 II-74 II-74 Bridge over Mississip	ni						
SECTION River	1	OCAT	ION _	(N=56	3805.847, E=2459619.4	115), SEC. 32, TWP	. 18N, RNG. 1W, 4 th P
COUNTY Rock Island DRILLI	NG ME	THOD			HSA, CME 55	HAMMER TYPE	CME AUTOMATIC
STRUCT. NO.	D	В	U	M	Surface Water Elev.	ft	
Station	E	L	C	0	Stream Bed Elev.	ft	
BORING NOILR0301	T	w	0	s	Groundwater Elev.:		
Station 423 + 08 Offset 9' Rt.	Н	S	Qu	Т	First Encounter	564.8 ft ▼	
	(64)	//em	/4-D	(0/)	Upon Completion	ft	
Ground Surface Elev. 575.78 1	t (11)	(/6")	(tsi)	(%)	After Hrs.	ft	
Fill: Brick And Silty Sand	_						
		-					
	100						
		5					
		3					
	_	3					
	_	2			-		
		3					
		3					
569	78						
Lean Clay (CL) yellowish brown, moist to 11.0',	_	2					
wet deeper, loose, moderate	-	2					
plasticity, trace sand	-	-					
Sample 4: Atterberg limit test	-	2			1		
performed (LL=31, PI=11)		3			1		
	7_0	3					
	10				-		
		-			-		
	Ā	1					
563	78						
Fine to Coarse Silty Sand (SM)		3			1		
yellowish brown, little gravel, loose Sample 6 (12'13.5'); grain size	_	4					
analysis performed		4					
Fine to Coarse Sand (SP)		30			1		
light brown, wet, dense to very	-15	50/2"			1		
dense, trace silt	-						
559	.78						
End of Boring	-	-					
	_						
	-						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

Illinois Dep. of Transpoi	rtati	on			SC	OIL BORING LOG	Date	9/2	6/07
ROUTE 1-74	DES	CRI	PTION	Ne	w I-74	Bridge Over Mississippi River - Illinois Approach LOC	GED BY		
SECTION I-74 Bridge over Missi River	ssippi	L	OCAT	ION _	(N=56	3752.136, E=2459609.413), SEC . 32, TWP . 1	8N, RN 0	6. 1W,	4 th P
COUNTY Rock Island DR	ILLING	ME	THOD			HSA, CME 55 HAMMER TYPE	CME AL	JTOMA	TIC
STRUCT. NO. Station BORING NO. ILR0302 Station 422 + 58 Offset 14' Lt.	_	D E P T H	O W S	U C S	M O I S T	Stream Bed Elev. ft Groundwater Elev.:	D B L P O T W H S	U C S	N C I
Ground Surface Elev. 576.65	ft	(ft)	(/6")	(tsf)	(%)	After ft	ft) (/6")	(tsf)	(%
Topsoil dark brown	575.65					Weathered Sandstone Top 2" medium to fine gravel sized very angular light gray rock 555.75	_		
Silt (ML) Dark brown to yellow orange, dry, loose, little medium to fine sand			3 4	4.5 P		fragments, possible lightly weathered rok with silt and clay, wet, some coarse to fine sands			
francisco construction			4			remainder: light gray with greenish gray streaks, dry, hard,			
trace coarse sand			5	4.5	-	impermeable, silt with fine sands,	-		
		-5	5	P		very strong cementation, possible completely weathered sandstone (continued)			
	570.65	_				Borehole continued with rock-	-		
Lean Clay > (ML-CL)	370.03		4			-			
Little gravel, medium brown with yellowish orange streaks, dry to moist, very loose to loose			4 3 4	1.5 P		-			
Sample 4 (8'-10'): Atterberg limit		_	2		-	-	-		
test performed (LL=35, PI=20)		-	1	1.5					
		▼ -10	2 2	Р			-30		
	565.65	_					-		
Very Silty Fine to Coarse Sand	000100					_			
(SM) Gray with mingled brown, little						-			
gravel, wet, possible gumbo, tried		-					-		
to obtain ST from 11' to 13' but coarse grained soil at 12'			5			-			
prevented from push a full sample Shelby tube recovery collected in	562.65	77	7			1 _			
bag sample, coarse gravels and		-	14				0.5		
fine cobbles from 12' to 13' (heavy grinding)		15	14		-	-	-35		
Sample 5 (13'-15'): grain size	560.45	-					-		
analysis performed Clayey Fine to Coarse Sand (SC)	300.45		36	2.0		4			
little gravel, greenish gray with		_	45 50/4	0.8		_	-		
gray streaks, dry, trace fine sands, medium dense, possible		-	50,4				-		
weathered rock		-	50/5						
Sample 6 (16'-18'): grain size analysis and Atterberg limit		_				-	-		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

MODJESKI ™ MASTERS	
Experience great bridges.	

USER NAME =	DESIGNED - YSS	REVISED -
	CHECKED - JMH	REVISED -
PLOT SCALE =	DRAWN - MLA	REVISED -
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

BORING LOGS 1 RAMP 6TH-D RETAINING WALL 03 STRUCTURE NO. 081-6012
SHEET NO. 8 OF 11 SHEETS

.I. E.	SECTION	COUNTY TOTAL SHEET NO.
4	(81-1)R & 81-1HVBR	ROCK ISLAND 1504 1145
		CONTRACT NO. 64CO
	ILLINOIS FED.	AID PROJECT

Illinois Depar of Transporta	ation	1		SC	IL BORIN	G LOG	
Division of Highways CH2M HILL						· D:	Date11/1/0
OUTE 1-74	DESCR	IPTION	Ne	W I-74	Bridge Over Mississipp Approach	of River - Illinois	LOGGED BY _ L. Hur
I-74 Bridge over Mississin	igo						WP. 18N, RNG. 1W, 4 th l
OUNTY Rock Island DRILL	ING ME	THOD	-	1	HSA, CME 55	_ HAMMER TYP	PE CME AUTOMATION
TRUCT. NO.	D	В	U	М	Surface Water Elev.	ft	
Station	E	L	C	0	Stream Bed Elev.	ft	
ORING NO. PRMPD02	T	w	3	S	Groundwater Elev.:		
Station 425 + 74	н	S	Qu	Т	First Encounter	ft	
Offset 26' Rt	ft (ft)	(/6")	(tsf)	(%)	Upon Completion After Hrs.	ft	
lay (CL) little gravel, trace sand,		, ,		100	Aiter Tiro.		
ark brown, dry to moist, firm to							
un	-	6	1.3				
	_	4	P				
		4					
	_	WOH 1	0.9				
		1	P				
	_ <	2					
	-	1	0.6	13.0			
	-	1	P	15.0			
		2					
		WOH					
	-	WOH 8					
	.20	16					
ilty Clay (CL) trace sand and ravel, gray mottled orange brown	-	WOH		37.0			
nd dark brown, moist, soft to rm. Shelby tube sample T-1 from		WOH WOH	D	37.0			
'-11' from adjacent location		1					
aving mc: 28%, dry density: 4.5pcf and UC: 920psf	_	WOH	0.8				
and oo. ozopol	-	50/3	P				
		50/2					
Sorehole continued with rock).70	50/2					
oring.	-						
	15	5					
	-	-					
	3===						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Illinois Department of Transportation Page <u>1</u> of <u>1</u> **SOIL BORING LOG** Date ___11/1/05 New I-74 Bridge Over Mississippi River - Illinois
Approach

LOGGED BY
L. Hunt 1-74 DESCRIPTION I-74 Bridge over Mississippi River SECTION _ LOCATION (N=563955.586, E=2459599.114), SEC. 32, TWP. 18N, RNG. 1W, 4th PM COUNTY Rock Island DRILLING METHOD HSA, CME 55 HAMMER TYPE CME AUTOMATIC STRUCT. NO. BORING NO. PRMPD03 T W S I Groundwater Elev.:
Station 424 + 58 Under T Size T S P O S T W H S Qu 561.4 ft <u>▼</u> __ft _ft (ft) (/6") (tsf) (%) Clay (CL)trace to little silt, trace sand, dark brown, dry to moist, stiff No Sample. Silty Clay (CL, CL-ML) gray brown, mottled orange brown and dark brown, dry to moist, soft to Auger refusal at 28.5'; end of borehole.
End of Boring Sandy Clay to Sand (SC) Sandy Clay to Sand, gray, moist to wet Siltstone Siltstone, little sand, gray, moist, homogeneous.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

USER NAME =	DESIGNED - YSS	REVISED -
	CHECKED - JMH	REVISED -
PLOT SCALE =	DRAWN - MLA	REVISED -
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED -

BBS, from 137 (Rev. 8-99)

BORING LOGS 2	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
RAMP 6TH-D RETAINING WALL 03	74	(81-1)R & 81-1HVBR	ROCK ISLAND	1504	1146
STRUCTURE NO. 081-6012			CONTRAC	T NO.	64C08
SHEET NO. 9 OF 11 SHEETS		ILLINOIS FED. A	ID PROJECT		

Illinois Dep of Transpo Division of Highways CHZM HILL						IL BORIN		Date9/7/0
ROUTE1-74	DES	SCRIE	PTION	Ne	w I-74	Bridge Over Mississipp Approach	oi River - Illinois	OGGED BYSL
I-74 Bridge over Miss ECTION River	issippi	L	OCAT	ION	(N=564	4029.213. F=2459513.	152) SEC 32 TWP	. 18N. RNG . 1W. 4 th
COUNTY Rock Island DF						HSA, CME 55		
COUNTY ROCK ISIANU DR	CILLING							CIVIE AUTOMATI
STRUCT. NO		D E	B L	U	M	Surface Water Elev.	ft	
Station 437+80.7		P	0	S	ĭ	Stream Bed Elev.	ft	
BORING NO. PRMPD-05		T	w		s	Groundwater Elev.:		
Station 425 + 51		н	S	Qu	Т	First Encounter	5641 ft V	
Offset 37' Lt						Upon Completion		
Ground Surface Elev. 575.10	ft	(ft)	(/6")	(tsf)	(%)	After Hrs.		
PAVEMENT - asphalt and base	- 107							
course	574.10							
SILT - black, with rubble (FILL)		_	4					
		_	6					
		-	5					
	F74.00	_						
CLAY - medium gray to orange	571.60	-	2					
prown, slightly to medium plastic,		-	1	0.5	24.9			
medium stiff, moist		-5	2	В	21.0			
		-5	-					
			1					
			2	0.7	38.9			
			1	В				
Attempted Shelby tube at								
3.5'-10.5'; no recovery]	566.60							
SAND - red brown, fine grained,								
oose, wet								
		-10						
Attempted Shelby tube at 11'-13';		Ā	1					
no recovery; followed up with		-	1					
SPT]		-	1					
		- 1	-					
		_						
	561.10		1					
SHALE - green gray, clayey,	3070		3	1.4	23.6			
severely weathered		-15	13	В				
			12					
	558.40		41	1.4				
Borehole continued with rock	330.40		50/1"	В	1			
coring.								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

	Division of Highways CH2M HILL		New I-74 Bridge C	war Mississ	inni Piver	- Illinoi		D	ate	9/7/07
ROUTE	I-74	DESCRIPTION _		Approach		1111101	_ LO	GGED	BY	SL
SECTION _	I-74 Bridge over Missis River	LOCATION	N _(N=564029.21	3, E=24595	13.152), S	EC. 32,	TWP.	18N, F	RNG. 1W	, 4 th PN
STRUCT. NO	Rock Island COF		REL TYPE & SIZE	NQ Wir		D C E O	R E C O V	R · Q	T I M	S T R E N
BORING NO. Station	425 + 51	Top of Rock Begin Core E	Elev. 561.60		- 1	P R T E	R Y		E	G T H
Offset Ground Sur	37' Lt. face Elev. 575.10	ft			(ft) (#)	(%)	(%)	(min/ft)	(tsf)
SANDSTON	E - medium gray, very f glomeratic at 17.5'-18.1			ale	558.40 — 557.10	Rur 1	82	23	1	
and seams, I	- gray, fine grained, wi ocally stylolitic, hard, th le fractures, planar to sl	in to medium bedde	d, predominantly I	norizontal to	_	-20				
						Rur 2	100	95	1.2	
					7					
					_	-25				
					-	Rur 3	97	87	1	
					-					1081.
-slightly roug	gh fractures across style	olites at 28.3'-30.6'				-30				
-thick bedde	d, occasional stylolites	at 30.6'-35.6'			-	Rui 4	100	100	2.6	
-minor pitting	g with some "birdseye" t	exture from 32.1' to	35.6'		_					
					1-	-35				
					_	- Rui	100	84	1.3	

CHZM HILL. New I-74 Bridge Over Mississing	ni River III	inois		D	ate 9	/7/07
OUTE I-74 DESCRIPTION Approach	pritiver - III	111013		GGED	BY	SL
I-74 Bridge over Mississippi River LOCATION (N=564029.213, E=245951:	3.152), SEC	. 32,	TWP.	18N, F	NG. 1W	4 th PI
OUNTY Rock Island CORING METHOD NQ Core			R E	R	CORE	S
TRUCT. NO. CORING BARREL TYPE & SIZE NQ Wire	D E P T H (ft)	C O R E (#)	COVERY	Q D	T I M E	R E N G T H
IMESTONE - gray, fine grained, with occasional to some thin green shale partings and seams, locally stylolitic, hard, thin to medium bedded, predominantly horizontal to ery low angle fractures, planar to slightly irregular, smooth to slightly rough, fresh continued) occasional soft rock-like green shale partings and clasts in limestone with fractures long shale, occasional pitting, at 38.9'-40.3' green rock-like shale seam with 85° fracture at 40.3'-40.8'	-40					
medium gray, fine to medium grained, occasional shale partings	532.50					
	45					

Color pictures of the cores Yes

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)

MODJESKI and MASTERS

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PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED -	

CE	HANSON
- T	HANSUN

Page 1 of 1

~								
							Date _	7/1/10
ROUTE F.A.I. 74	DES	CRIPT	ION _		I-74 Over Mississippi	River L	OGGED BY	JMB
SECTION 81B/8	1-HVB	LOC	CATION	NE 1	/4, SEC. 32, TWP. 18N,	, RNG. 1W, 4th P.N	1.	
COUNTYRock Island	DRILLING	METH	OD _	Н	ollow Stem Auger	HAMMER TYPE	Aut	.0
STRUCT. NO. Station BORING NO. PRMP 6t	h D-03 49 t. 576.4 ft	E I P (T \ H :	B U L C O S W S Q	O I S	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft 562.4 ft ▽		
FILL - Very dark brown, mo wet, medium to stiff, silty, le CLAY	an	.	3 0.6	5B 39				
FILL - Brown, iron staining moist, medium, sandy, clay SILT with trace gravel, iron metal debris	ey and	4-	7 3 3 3	19				
Brown, moist, medium, silty	567.40 CLAY	-	3 3 3 3	50	-			
Brown, wet, LIMESTONE fragments	562.90 1-	¥ 1	7 3 0		_			
Gray, weathered LIMESTO clayey shale filled voids	559.40 NE, 558.30 1:	8 (FC	/1"/					
End of Boring		/20						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

CON HANSON

SOIL BORING LOG

Page 1 of 1 Date ___7/1/10

HANSON

SOIL BORING LOG

DOUTE 5.4.74			DT101			1740 - 14 - 1 - 1	D:	Date _	
ROUTE F.A.I. 74									
SECTION 81-1HVB		_ ι	OCA1	ION _	NE¼ c	of SEC. 32, TWP. 18N,	RNG. 1W, 4th P.	М.	
COUNTY Rock Island D	RILLING	ME	THOD		Hol	low Stem Auger	HAMMER TYPE	Aut	ю.
STRUCT. NO. 081-6012 Station RW 03-2 BORING NO. RW 03-2 Station 423+60 Offset 14' Rt. Ground Surface Elev. 575.2		D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft ft	<u>Z</u>	
TOPSOIL	574.95	-							
Very dark brown, moist, soft to medium stiff, silty, lean CLAY			3 2 3		26				
	571.70	_							
Dark gray, moist, soft to medium stiff, silty, lean CLAY with trace very-fine grained SAND	071.70	4-	3 2 3	1.50P	21				
Gray, wet, stiff, silty CLAY	569.70	_							
Gray, wet, still, slity CEA1		6-		0.89S	22				
		-		0.000	23				
		_		0.90B					
		8-							
		Ž.			24				
		_			25				
		-			26				
		10 — —			20				
	563.70	_	7		9				
Brown, wet, dense, silty, fine- to medium-grained SAND and grave with limestone fragments		12-	8 10						
		_							
		14— -	10 18 23		13				
		_							
		16—	20 23 24						
	EE7 00	_	27						
Very dark gray, WEATHERED	557.20	18—	-						
SHALE	555.70	_	25 50/5"	2.23S	17				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

SECTION 81-1HVB COUNTY Rock Island D							
STRUCT. NO081-6012	Г	D	В	U	М	Surface Water Elev.	 Auto
Station RW 03-1 BORING NO. RW 03-1 Station 425+60 Offset 9' Lt. Ground Surface Elev. 574.2		E P T H	U O W S (/6")	C S Qu (tsf)	O I S T (%)	Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	
ASPHALT	573.70						
FILL - Dark brown, SILT with fine-grained sand and gravel		2-	11 6 6		7		
FILL - Brown and gray, moist, loose, very-fine to medium-grained SAND and SILT with gravel	570.70	4— - -	4 4 2	3.50P	19		
	567.20	6-		0.32S 0.60P	19		
FILL - Gray, moist, loose, silty, medium-grained SAND with clay, wood debris				1.00P 0.37S	22		
Gray with brown mottles, silty, lean CLAY with fine-grained sand		- - 10-		0.29S 0.55S	31		
Gray, wet, very soft, clayey SILT	563.20	 - 12	1 4 7		62		
INTACT ROCK End of Boring	561.20 560.70		50/0"				

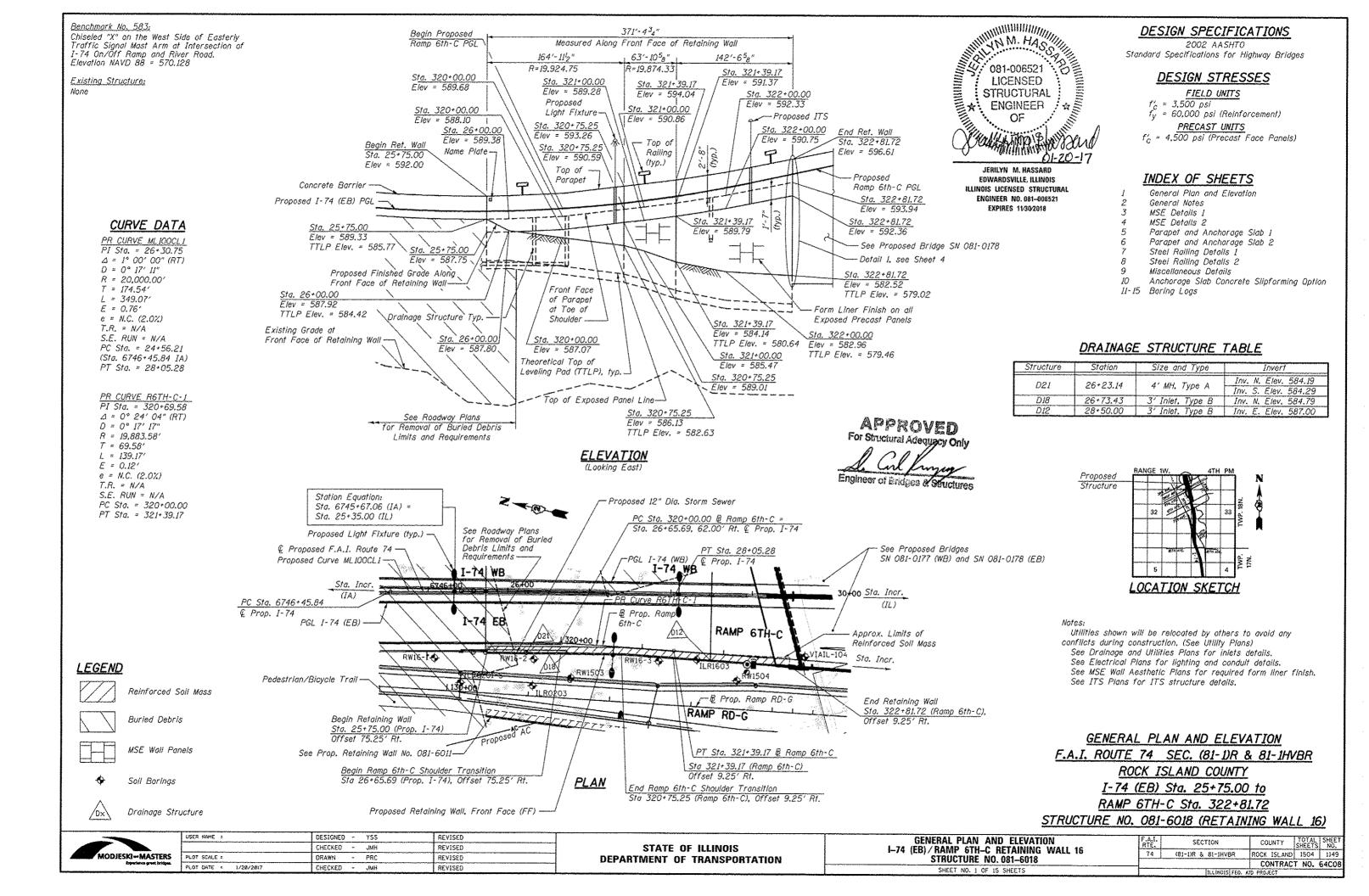
The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

MODJESKI and MASTERS Experience great bridges.
--

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BORING LOGS 4 RAMP 6TH-D RETAINING WALL 03 STRUCTURE NO. 081-6012	
CHEET NO. 11 OF 11 CHEETC	

RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
74	(81-1)R & 81-1HVBR	ROCK ISLAND	1504	1148
		CONTRAC	T NO.	64C0
	ILLINOIS FED. A	ID PROJECT		



GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Wall stations and offsets are given to the front face (FF) of the wall and are measured from the Centerline F.A.I Route 74 and Ramp 6th-C baseline, except as noted. FF of the wall is to be considered edge of panel or form liner.
- 3. See Special Provision for Mechanically Stabilized Earth Retaining Walls for design and construction requirements.
- 4. Wall construction shall not begin until after ground improvement for the buried debris has been completed in the area of the new wall.
- 5. Slipforming of the aesthetic parapet is permitted.

MSE WALL SETTLEMENT

1. The Top of Exposed Panel Elevations shown on these plans are final elevations after any settlement. The wall system supplier shall take appropriate measures to accommodate the 0 to 5 inches of settlement that are anticipated from Sta. 25+75.00 to Sta. 322+81.72.

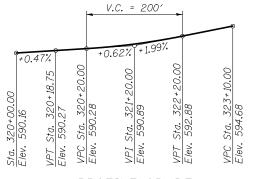
TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Concrete Superstructure	Cu. Yd.	193.5
Protective Coat	Sq. Yd.	391
Reinforcement Bars, Epoxy Coated	Pound	40,390
Name Plates	Each	1
Steel Railing (Special)	Ft.	355
Mechanically Stabilized Earth Retaining Wall	Sq. Ft.	2,853

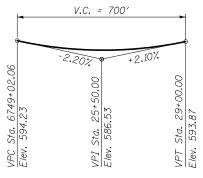
STATION 25+75.00
BUILT 201_ BY
STATE OF ILLINOIS
F.A.I. RT. 74
SEC. (81-1)R & 81-1HVBR
LOADING HS-20
STR. NO. 081-6018

NAME PLATE

See Std. 515001



 $\frac{PROFILE \ GRADE}{\text{(Along } \mathbb{E} \ Ramp \ 6th-C)}$



PROFILE GRADE

(Along & Proposed F.A.I. Route 74)

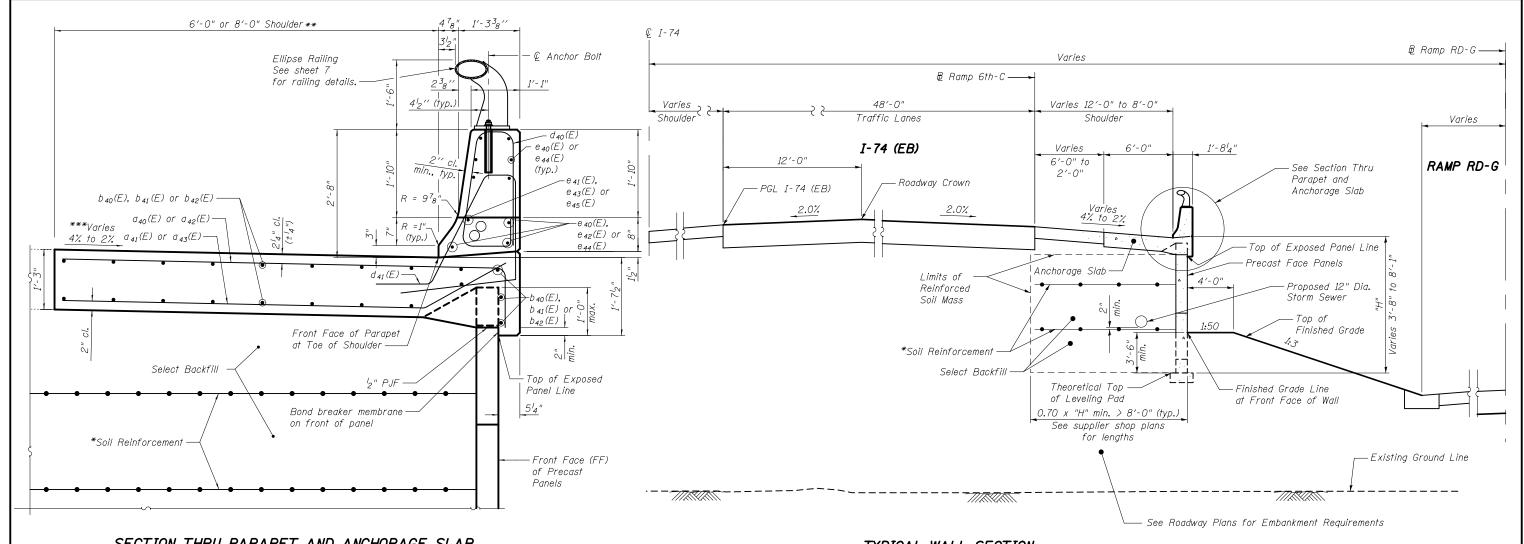
MODJESKI and MASTERS Experience great bridges.	

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL NOTES
I-74 (EB) / RAMP 6TH-C RETAINING WALL 16
STRUCTURE NO. 081-6018

SHEET NO. 2 OF 15 SHEETS



SECTION THRU PARAPET AND ANCHORAGE SLAB

** At Front Face of Wall, Sta. 25+75.00 to Sta. 320+75.25, Anchor Slab Width = 6'-0" Sta. 320+75.25 to Sta. 322+81.72, Anchor Slab Width = 8'-0"

*** At B Ramp 6th-C, Sta. 25+75.00 to Sta. 320+08.42 Cross Slope = 4% Sta. 320+08.42 to Sta. 320+68.36 Cross Slope Transition Sta. 320+68.36 to Sta. 322+81.72 Cross Slope = 2%

TYPICAL WALL SECTION

Sta. 25+75.00 (I-74) to Sta. 320+75.25 (Ramp 6th-C)

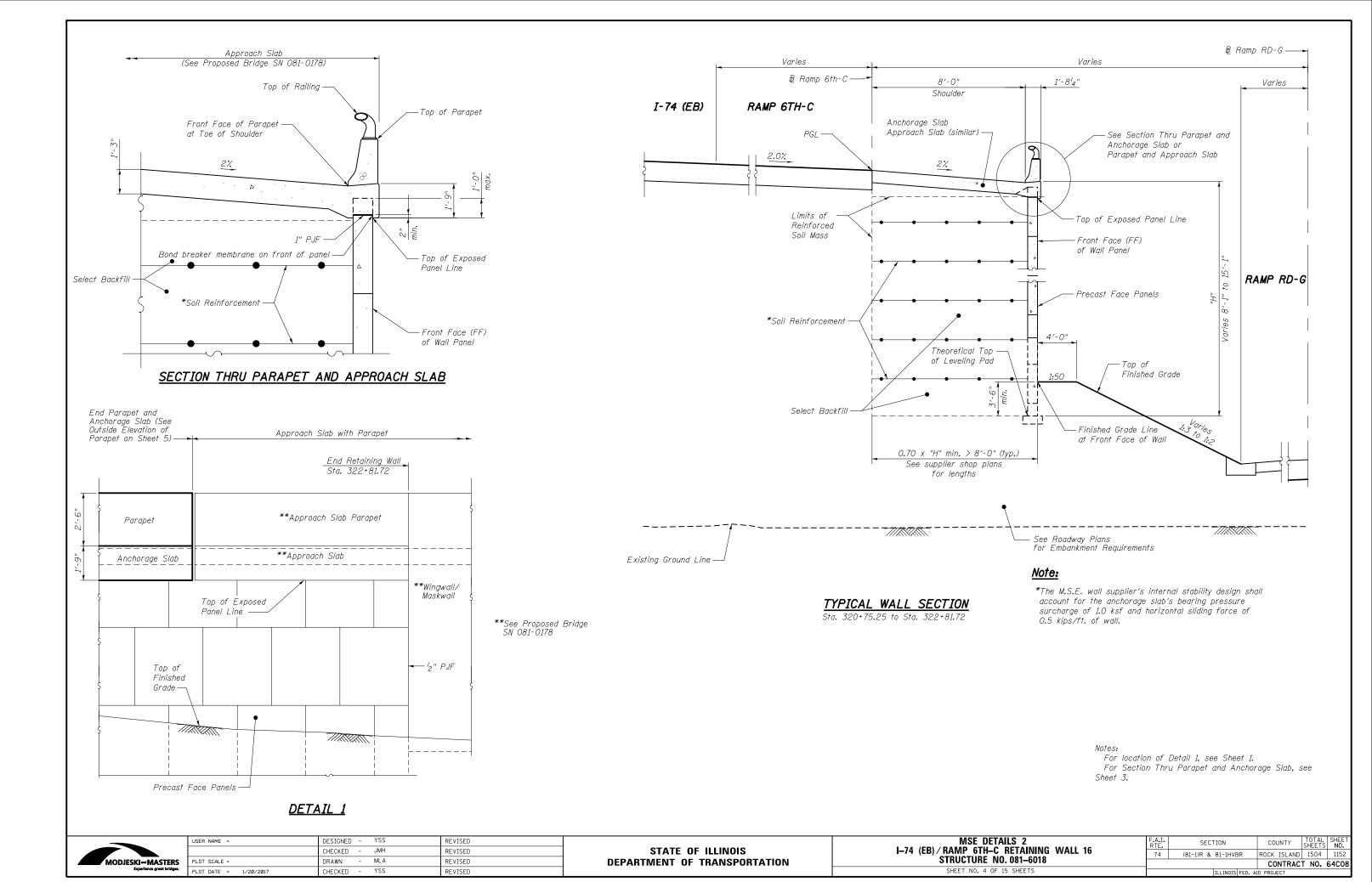
Note:

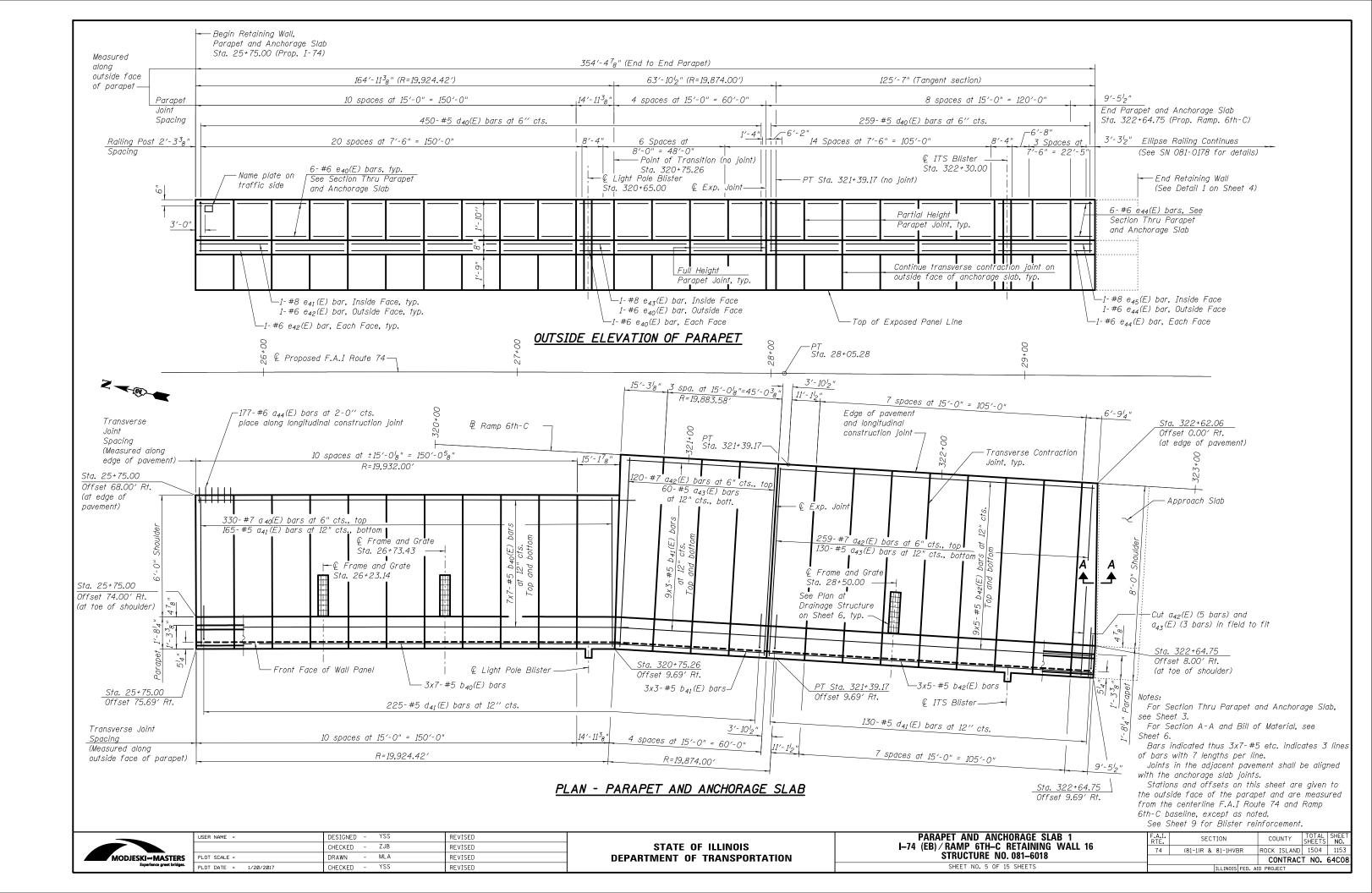
*The M.S.E. wall supplier's internal stability design shall account for the anchorage slab's bearing pressure surcharge of 1.0 ksf and horizontal sliding force of 0.5 kips/ft. of wall.

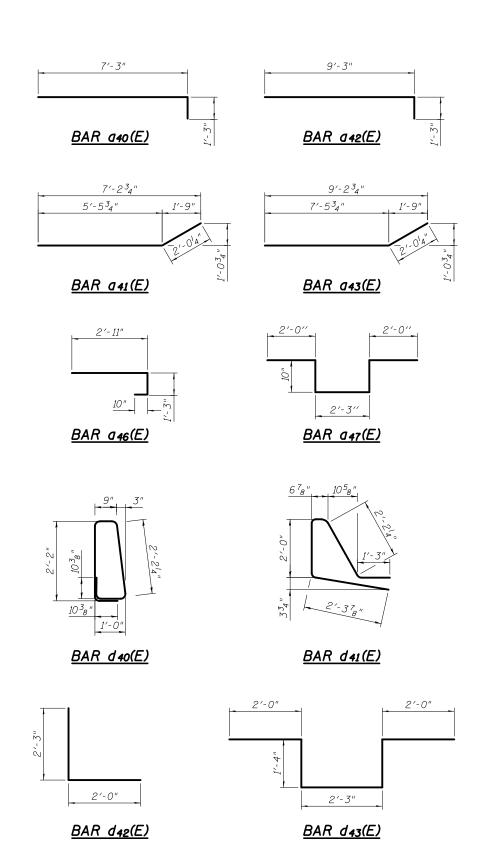


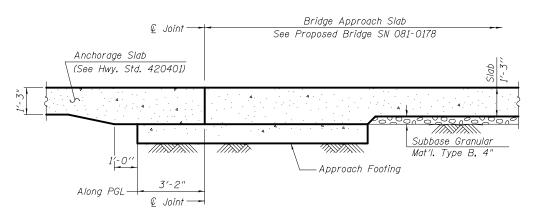
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PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
74	(81-1)R & 81-1HVBR	ROCK ISLAND	1504	1151
		CONTRAC	T NO.	64C0
	TILINOIS FED. A	ID PROJECT		

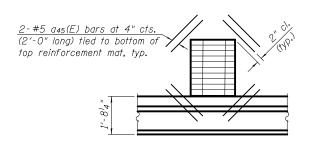








SECTION A-A



PLAN AT DRAINAGE STRUCTURE

(Cut longitudinal reinforcement to clear drainage structure.)

RETAINING WALL 16 BILL OF MATERIAL

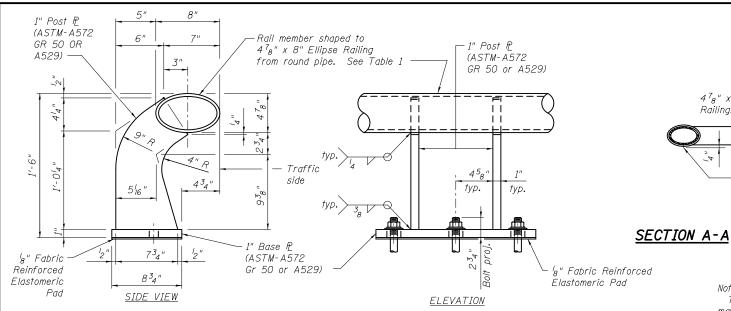
Bar	No.	Size	Length	Shape
a 40(E)	330	#7	8′-6"	
041(E)	<i>1</i> 65	#5	7′-6"	
a 42(E)	379	#7	10′-6"	
a 43(E)	190	#5	9'-6''	
044(E)	177	#6	2'-0"	
045(E)	24	#5	2'-0"	
a 46(E)	6	#6	5′-0"	
a 47(E)	6	#6	7′-11"	ᇺ
b 40(E)	119	#5	27'-0"	
b41(E)	63	#5	22′-3"	
b 42(E)	105	#5	28'-6"	
d40(E)	709	#5	7′-10"]
d41(E)	355	#5	8'-4"	[_
d42(E)	6	#6	4'-3"	L
d43(E)	10	#6	8′-11"	7
e 40(E)	141	#6	14'-9"	
e 41(E)	11	#8	29′-9"	
e 42(E)	33	#6	29′-9"	
e 43(E)	1	#8	14'-9"	
e 44(E)	9	#6	9'-2"	
e 45(E)	1	#8	9'-2"	
Reinforcement Bars,			Pound	40,390
Epoxy Coated			, 53/10	,0,000
Concrete Cu. Yd. 193.5				1935
Superstructure Ca. 7a. 155.5				

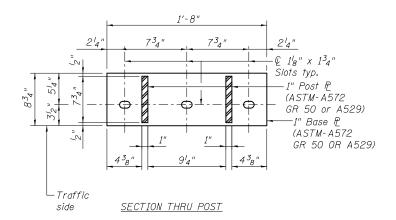
MIN. BAR LAP #5 bars - 3'-3"

Note: For location of Section A-A, see Sheet 5.

MODJESKI of MASTERS Productor great bidges
Experience great bridges.

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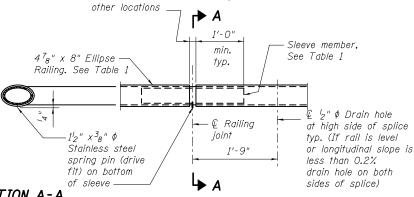




ELLIPTICAL TUBE WITH RAIL POST AND ANCHORAGE DETAILS

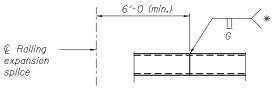
TABLE 1							
<i>APPROVE</i>	APPROVED RAILING MATERIAL						
4 ⁷ 8"x 8"	Sleeve Membe	er					
Ellipse Railing	(at railing sp	lice)					
Material	Material	Thickness					
6" Dia, Std. Pipe	ASTM-A53-B	0.353"					
ASTM-A53 E OR S	A36 or A500 GR. B	0.339"					
GRADE B	API-5LX52	0.224"					
6" dia., 0.280"	ASTM-A53-B	0.353"					
Wall thickness	A36 or A500 GR. B	0.339"					
ASTM-A501	API-5LX52	0.224"					
6 ⁵ 8" O.D. x O.188"	ASTM-A53-B	0.339"					
Tube	A36 or A500 GR. B	0.325"					
API-5LX52	API-5LX52	0.216"					

Joint opening to match width of supporting parapet joint opening plus 38" (if slab opening occurs at same location). 1^{l_4} " min. at all



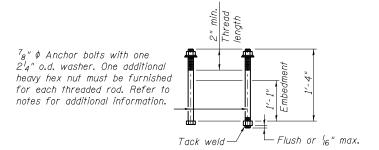
ELLIPSE RAILING SLEEVE DETAIL

The major and minor diameters of the rail member may vary +/- 3_{16} " from plan dimensions. However, the difference between the outside diameters of the sleeve and the inside diameters of the rail shall not exceed ${}^{l}_{8}$ " along the major or minor axis. The maximum gap along the 45° axis of the sleeve may be ¼" max.



RAILING SHOP SPLICE DETAIL

* Shop splice is permitted with minimum 85 percent penetration. The weld may be square groove, double vee groove, or single groove. Grind smooth.



CAST-IN-PLACE ANCHOR BOLT OPTIONS

NOTES:

See Sheet 5 of 14 for post spacing. Steel Railing (Special) shall be fabricated and installed in accordance with Article 509 of the Standard Specifications, unless otherwise noted. All steel rail elements shall be galvanized

according to Article 509.05 of the Standard Specifications.

The Steel Railing (Special) is to be bid on a per linear foot basis measured from end to end of steel railing.

Payment for Steel Railing (Special) shall include full compensation for furnishing all material, and all the equipment and labor required to erect the rail in accordance with these plans and the Standard Specifications.

Anchor bolts shall be $^{7}_{8}$ " ϕ , ASTM A-193 GR. B7, fully threaded with heavy hex nuts and one hardened washer and one $2^{-1}4$ " O.D. washer each. Embed threaded rods 10^{l}_{2} " min. into concrete parapet. Material for these items shall be in accordance with the adhesive manufacturer's requirements to be capable of obtaining an ultimate load per threaded rod of 36 kips in tension, considering spacing and edge distance. See Standard Specification 509.06 for further details on setting anchor bolts. Cost of anchor bolts included with Steel Railing (Special).

Optional cast-in-place anchor bolts to comply with ASTM F-1554 Grade 105. Hex nuts to comply with AASHTO M291, washers to comply with AASHTO M-293. Galvanizing in accordance with AASHTO M-232.

Provide one 18" and two 16" galvanized steel shims for 25% of rail posts, to be used as required. Shims shall be similar to base plates in size and holes. Cost included with Steel Railing (Special).

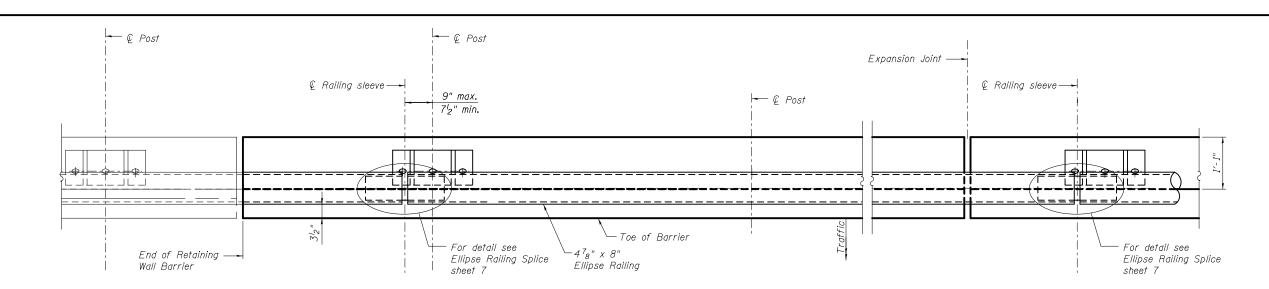
BILL OF MATERIAL

ITEM	UNIT	TOTAL
Steel Railing (Special)	Foot	355

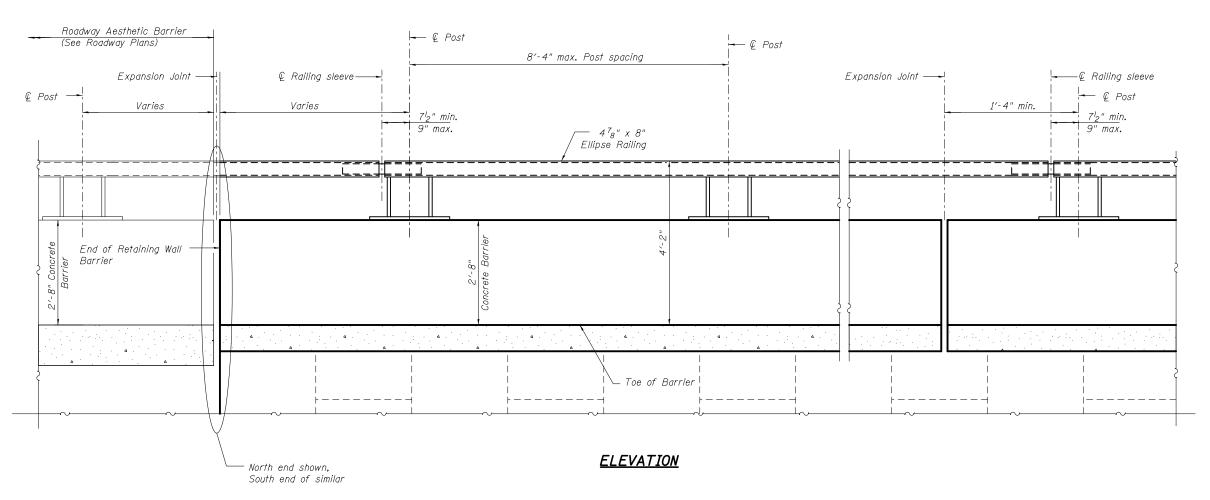


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PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

TE. 74	SECT (81-1)R &		3R	_	 UNTY ISLAND	SHEETS 1504	NO. 1155
		TI I TNOTS	EED	A 3	 NTRAC	T NO.	64C08



<u>PLAN</u>



Notes:

Edge of base plate shall not be less than 6" from any cold joint or barrier discontinuity. For post spacing, see Sheet 5.
South end of rail shall tie into railing on

S.N. 081-0178.

North end of rail shall tie into roadway

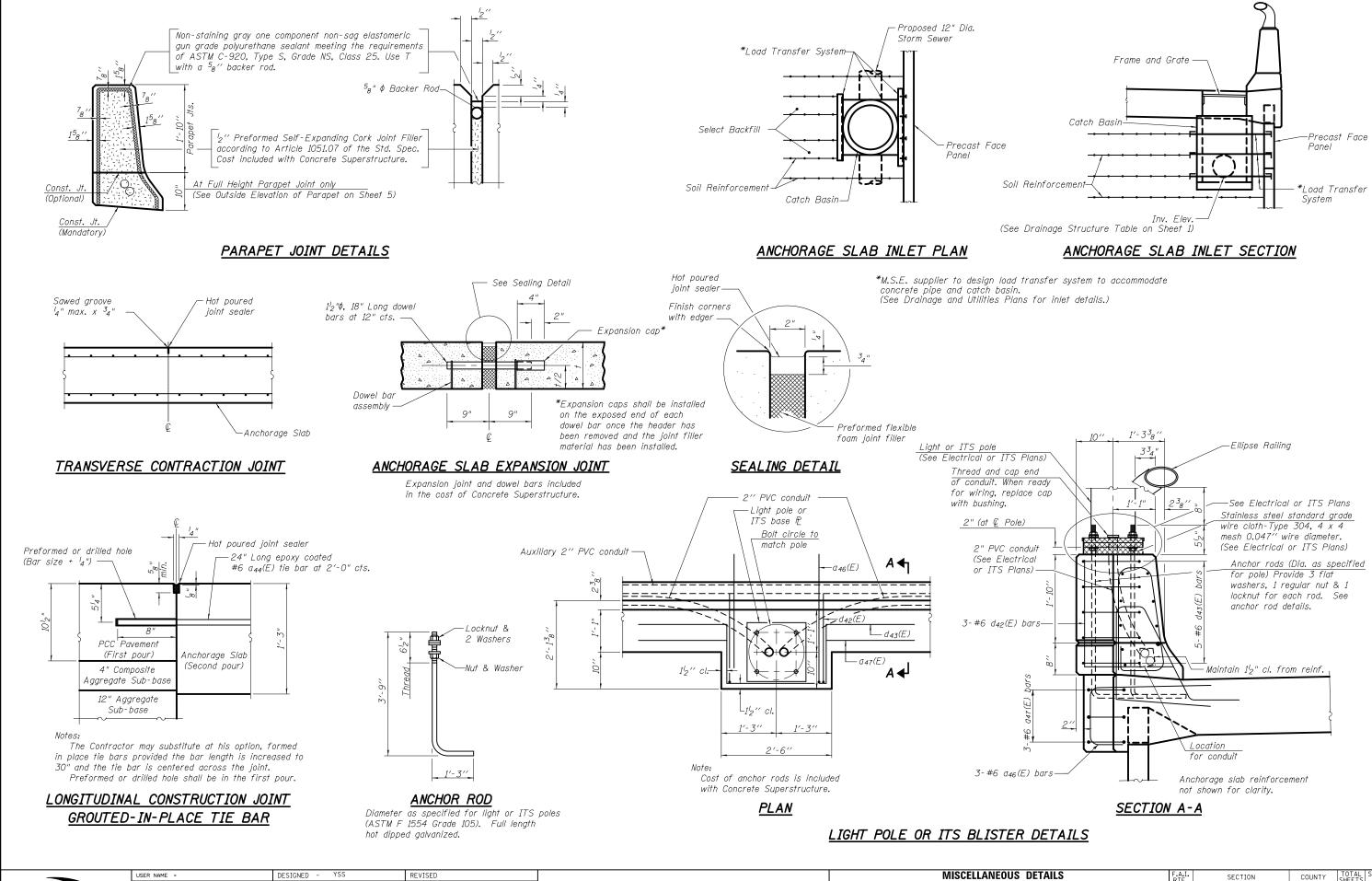
North end of rail shall tie into roadway aesthetic barrier railing.



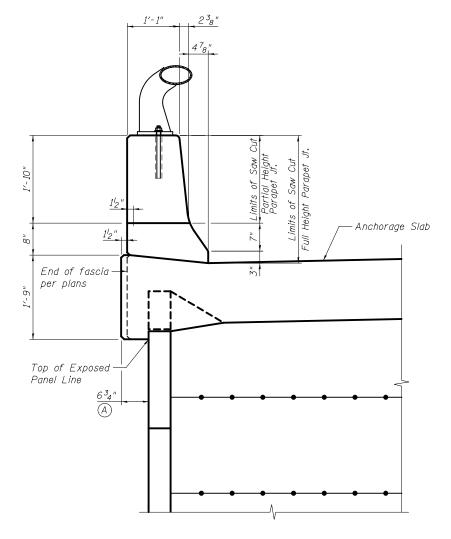
USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

STEEL RAILING DETAILS 2 I-74 (EB)/RAMP 6TH-C RETAINING STRUCTURE NO.081-6018	WALL 16
SHEET NO 8 OF 15 SHEETS	

	F.A.I. RTE.	SECTION			CO	UNTY	TOTAL SHEETS	SHEET NO.
	74	(81-1)R & 81-1H	VBR	T	ROCK	ISLAND	1504	1156
				Т	CO	NTRAC	T NO.	64C08
ı		ILLINO	IS FED.	ΑI	D PROJ	ECT		



I-74 (EB) / RAMP 6TH-C RETAINING WALL 16 JMH STATE OF ILLINOIS CHECKED REVISED 74 (81-1)R & 81-1HVBR ROCK ISLAND 1504 1157 STRUCTURE NO. 081-6018 IODJESKI - MASTERS DRAWN MLA REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 64CO SHEET NO. 9 OF 15 SHEETS LOT DATE = 1/20/2017 CHECKED REVISED

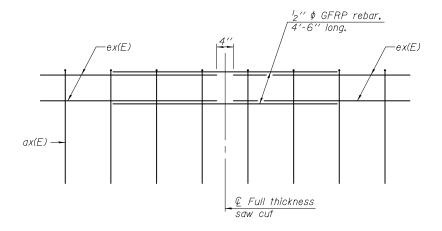


½" ∮ GFRP rebar Tapped with #4 ex(E) bars (at saw cut locations) #5 (E) bar Replace dx(E) bar - d(E) Additional #3 (E) bar at 11" cts. cl. Additional #4 (E) bar (Min. 2'-7" lap)

SECTION

(Showing reinforcement clearances for slip forming and additional reinforcement)

SECTION THRU PARAPET AND ANCHORAGE SLAB



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)

REVISED

REVISED

REVISED

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450 Job No. 10061 benesch DESIGNED -REVISED SLD CHECKED -

PLOT DATE = 1/20/2017

DRAWN

CHECKED -

RMG

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

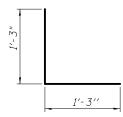
 ORAGE SLAB CONCRETE SLIPFORMING OPTION 74 (EB)/RAMP 6TH-C RETAINING WALL 16 STRUCTURE NO. 081-6018
CHEET NO 10 OF 15 CHEETS

SECTION COUNTY 74 (81-1)R & 81-1HVBR ROCK ISLAND 1504 1158 CONTRACT NO. 64C08

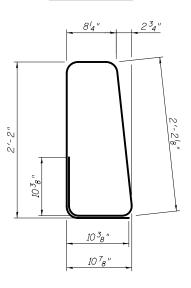
GENERAL NOTES

All dimensions shall remain the same as shown on superstructure details, except dimension A which is to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension A equals 0.008 cu. yds./ft.

Full thickness saw cut at all joint locations in lieu of cork joint filler.



#3 (E) BAR



#5 (E) BAR

(V)	Illinois Department of Transportation
	Division of Highways

Page <u>1</u> of <u>2</u>

		Date	9/19/07
ON_	New I-74 Bridge Over Mississippi River - Illinois Approach	LOGGED BY	F. Abreu

ROUTE I-74	DE	SCR	IPTIO	٧		Approach	L	OGGED B	Y F. A	Abreu
I-74 Bridge over Mis	iggissis									
SECTION River		_ ι	OCA	ION_	(N=56	5232.456, E=2459065.7	732), SEC. 32, TWP .	18N, RNG	i. 1W, 4	In PM
COUNTY Rock Island D	RILLING	3 ME	THOL			HSA, CME 55	_ HAMMER TYPE	CME AL	JTOMA	TIC
			_						Ι	
STRUCT. NO.		D	В	U	M	Surface Water Elev.	ft	D B	U	M
Station		E	L	С	0	Surface Water Elev. Stream Bed Elev.	ft	E L	С	0
		P	0	S				P 0	S	1
BORING NO. ILR0201-S		Ţ	w	_	S	Groundwater Elev.: First Encounter Upon Completion		T W		S
Station 25 + 42		н	S	Qu	Т	First Encounter	562.4 ft <u>▼</u>	H S	Qu	т
Offset111' Rt.			//OIII		(0/)	Upon Completion _	ft	/cu / //am		(0/)
BORING NO. ILR0201-S Station 25 + 42 Offset 111' Rt. Ground Surface Elev. 566.39	ft	(H)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft) (/6")	(tsf)	(%)
Concrete								•		
7" slab with rebar	565.39	_						7		
Fill: Fine to Medium Sand With			4							
Silt (SP-SM)		_	8					\neg		
Very dark brown, dry to moist,			5					\neg		
medium dense, little gravel, fine to	563.39	_	4					-		
medium sands, trace coarse sands	503.38		2					\dashv		
Fill: Sandy Lean Clay(CL)	J		4	1.8	30.0			\dashv		
Very dark gray mottled with		<u> </u>	3	P	00.0			-		
greenish gray, moist to wet, stiff,		_	-					-		
faint petroleum odor, trace		5	_2_					25		
medium to fine gravel, with sand		_						_		
seams			_					_		
Fill: wood matter with fine to		_	2							
coarse sand, strong petroleum			1							
odor, saturated, possible old railroad ties			2							
	558.39		2							
Fill: Silty Sand Trace Gravel(SM))		2							
Top 5": Brown, wet, root matter			1		40.0			7		
with petroleum odor and root matter throughout			2							
Remainder: Silty Sand trace		-10	1					-30		
gravel, dark to medium gray, wet,			Ť.					–		
non plastic, medium to fine sands,	555 20	_						\dashv		
trace subrounded fine gravels,	555.38		2					\dashv		
loose, faint petroleum odor		_	1		11.0	End of Boring		\dashv		
Encountered WT at 10' bgs]		2		' ' '			-		
Silty Fine to Coarse Sand(SM)		_	15					-		
trace gravel, brown, wet, very	553.39							_		
loose to medium dense, faint petroleum odor, occasional root.	1	_	30					\dashv		
possible native soil, non odorous	1		50/2					_		
Sandy Silt With Clay And Grave	J	_						4		
(CL)	•	15						-35		
Top 2": Dark brown followed by										
vellowish orange and then light	550.56									
gray at bottom 2", wet, non plastic	.									
very angular flat coarse to fine		_						٦		
gravels (possible rock fragments), some medium to fine sands with										
silt and few clay, possible		_						7		
gumbo/residual soil Driller began								\neg		
to set up for rock coring at 0950	1	_						\dashv		
	_		1			1			1	1

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

(A)	Illinois Department of Transportation
	CH2M HILL

I-74

Ground Surface Elev. 566.39 ft

SECTION

STRUCT. NO.

BORING NO. ___

Sandstone

(3/4-3/4')

End of Boring

I-74 Bridge over Mississippi River

DESCRIPTION

with Limestone and bands of coal towards bottom of sample, light brown with light gray, rough texture at top 32", remainder has smooth texture, medium to fine grained with little coarse grains, slightly weathered to unweathered, medium to strong, top 32". Sandstone, remainder Limestone with coal bands 15.83" - Horizontal to 15° fractures,

Saridstone, remainder Liffiestone with coal barids 15.83 – Horizontal to 15 inactures, rough planar fractures at top 32" of sample, remainder fractures are irregular and undulated, little hard greenish gray impermeable clay infilling throughout top 13" of sample, remainder: no infilling, surface stains only, surfaces stained greenish gray at top 16", 16" to 30" no stains, 30" to bottom dark gray and brown coal stains, top 30": no rock wall contact due to crushed rock, remainder tightly healed with coal strands, sound to moderate fractures, very close to moderate discontinuities 23"-86" = top of

23°-31.5" = end of run Medium to fine grained, smooth texture, slightly weathered to unweathered, medium strong 21 42° - 15" to 45" degree fractures, irregular, undulating, slickensided at 11", 15", 51", 67" and 88" from top, hard impermeable clay infilling 1/8" to 1/2" thick that has lightly healed at most fractures except from 45" to 51" from top, dark gray surface stains, no infilling and surface stains from 45" to 51", from 57" to bottom thinly bedded throughout, stiff to very stiff gray clay infilling that is 1/2" to 1/4" thick at fracture, sound to moderate fractures, close to wide discontinuities Average 1-1/4 minute per foot for top 5 feet, 10:20-30 (2/4 3/4")

run
1/2-1/2-1/4-3/4-3/4
light gray milky water, brown water 2.5' down and 7'-4' dark brown to dark green
23'-31.5" = end of run
23'-31.5" = end of run

COUNTY Rock Island CORING METHOD Double tube, 10 ft core barrel, NQ wireline, diamond bit

CORING BARREL TYPE & SIZE

Core Diameter ______ in Top of Rock Elev. ______ 550.56 ____ ft

Begin Core Elev. 550.56 f

ROCK CORE LOG

LOCATION (N=565232.456, E=2459065.732), SEC. 32, TWP. 18N, RNG. 1W, 4th PM

New I-74 Bridge Over Mississippi River - Illinois Approach

Page <u>2</u> of <u>2</u>

(ft) (#) (%) (%) (min/ft) (tsf)

NQ-R1 78 41

Illinois Department of Transportation Date 9/19/07

> Black, moist NOTE: Sample 3 grain size analysis performed Clay (CH) black, slightly moist, firm to stiff,

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date 9/18/07 New I-74 Bridge Over Mississippi River - Illinois DESCRIPTION____ I-74 Approach I-74 Bridge over Mississippi River LOCATION (N=565145.331, E=2459082.04), SEC. 32, TWP. 18N, RNG. 1W, 4th PM COUNTY Rock Island DRILLING METHOD HSA, CME 55 STRUCT, NO. Surface Water Elev Stream Bed Elev. | BORING NO. | ILR0203 | Station | 26 + 32 | Offset | 114' Rt. | Ground Surface Elev. | 567.93 | ft First Encounter Upon Completion After Hrs. (ft) (/6") (tsf) (%) Concrete Surface: 3" of concrete 567.43 Silty Sand (SM)
dark brown and black, slightly
moist, very loose, fine to medium grained, low plasticity Sand Silt and Clay(ML)

race fine sand, moderate plasticity Rimac: Pu = 94 lbs NOTE: Sample 4 Atterberg limits: LL=63, PI=46 Rimac: Pu = 28 lbs brown, very dense, fine to medium grained, Same as above, sandy gravel in tip, brown, very dense, fine to medium angular gravel <1" Sandy Gravel(GP) light gray, wet, very dense, fine to medium angular gravel, fine to coarse sand End of Boring

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

COUNTY

CONTRACT NO. 64CO8

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USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

(V)	Illinois Department of Transportation
	Division of Highways

Page <u>1</u> of <u>2</u>

	Date	9/24/07
e Over Mississippi River - Illinois Approach	LOGGED BY	F. Abreu

ROUTE 1-74 1		IF IIO			Арргоаст		GGED BT F. Abreu
SECTIONRiver		LOCA	TION_	(N=56	4956.216, E=2459158.4	8), SEC. 32, TWP. 18	BN, RNG. 1W, 4 th PM
COUNTY Rock Island DRILLI	NG MI	ETHO			HSA, CME 55	HAMMER TYPE _	CME AUTOMATIC
STRUCT. NOStation	P	B L O	U C S	M 0 1	Surface Water Elev. Stream Bed Elev.	ft ft	
BORING NO. ILR1603 Station 321+68 Offset 9' Rt.	T H	S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	562.3 ft ⊻	
Ground Surface Elev. 568.27 f		(/6")	(tsf)	(%)	After Hrs.	ft	
Fill Concreteunderlain with 3" concrete, silt and gravel, dark gray to black, dry, hole offset 3 feet west of proposed boring location)2 	13					
Fill Silty Fine to Coarse Sand With Gravel (SM) Very dark gray, dry, loose, occasional reddish brick fragments		3 3					
Silty to Clayey Fine Sand(SM, SC) Dark brown with dark gray, moist,	_	1	1.0 P	24.0			
stiff, possible fill, weak cementation Sample 2: grain size analysis and Atterberg limits (LL=26, Pl=10)	<u>-</u> - 	2					
test performed	_						
Possible Fill Sandy Fat Clay(CH) dark gray to greenish gray, wet, stiff to very stiff, trace gravel			1.5	23.0			
	_	2		L			
Sample 3 (8'-10'): Atterberg limits (LL=59, Pl=28) test performed	-10	3 3 3	2.5 P	26.0			
557. Weathered Sandstone	27	25					
Brown with gray, wet, coarse to	_	33		26.0			
fine sands with, coarse to fine gravels, some silt and clay, dense to very dense, Bottom 2": Sandy	_	16 50/5					
Silt, uniform gray, dry, nonplastic, silt with fine sands, possible	_	50/4	\vdash				
complete weathered sandstone, Driller reports rough drilling and chatter 12.0' bgs, possible		<u> </u>					
weathered rock Completely weathered sandstone, dry, uniform gray, medium to fine	<u> </u>						
sands with silt Possible top of rock at 13'6" bgs Sample 4 (11'-13'): grain size spallers and Attentogral limits	-	-					
analysis and Atterberg limits (LL=25, PI=7) tests performed Borehole continued with rock	_	1					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

	New I-74 Bridge Over Mississ DESCRIPTION Approach	sippi River - II	linois		C	Page <u>2</u> Date <u>9</u>	/24/07
I-74 Bridge over Mississip SECTION River	opi LOCATION (N=564956.216, E=245915	58.48), SEC.	32, T	WP. 18	3N, R	IG . 1W,	4 th PM
STRUCT. NOStation	Core Diameter In Top of Rock Elev. <u>552.60</u> ft Begin Core Elev. <u>552.60</u> ft		C O R E	E C O V E R Y	R Q D	CORE T I M E (min/ft)	T R E N G T H
sample, smooth to rough texture, slightl water action, slightly weathered, weak t gray blotches at bottom 15" of sample close to close discontinuities, horizontal fracture surfaces, undulated, greenish g 1" thick at 80% of fractures surfaces, st		552.60	NQ-R	1 92	51		472.0

Color pictures of the cores	
Caraa will be atered for ave	mination until

End of Boring

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)

Page <u>2</u> of <u>2</u>

472.0

542.60



SOIL BORING LOG

Page <u>1</u> of <u>1</u> Date __10/25/05_

(ft) (/6") (tsf) (%)

New I-74 Bridge Over Mississippi River - Illinois ROUTE I-74 I-74 Bridge over Mississippi River _ DESCRIPTION____ Approach LOCATION (N=565101.511, E=2459102.047), SEC. 32, TWP. 18N, RNG. 1W, 4th PM COUNTY Rock Island DRILLING METHOD HSA, CME 55 ft E L C O P O S I T W S S Qu T M Surface Water Elev. STRUCT. NO. Stream Bed Elev. First Encounter
Upon Completion
After ____ Hrs.

Fill (GC) Clayey gravel to clayey sand, trace brick, dark brown, dry to moist, stratified. Auger refusal at 20' End of Boring Sandy to Silty Clay (CL, CL-ML) trace to little gravel and silt, trace organics, dark brown to brown, dry to moist, very stiff 4 3 -5 3 2.5 Sandy Clay (CL) trace gravel, dark brown to gray brown, dry to moist, soft to firm, encountered hard material at 6', moved borehole 3' west and started 1 0.5 30.0 sampling again at 6'
Silty Clay (CL-ML) Silty clay, trace gravel, gray brown, dry to moist to wet, homogeneous, stiff

(ft) (/6") (tsf) (%)

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

9 33 36

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USER NAME =	DESIGNED - YSS	REVISED	
00211 111112	CHECKED - JMH	REVISED	
PLOT SCALE =	DRAWN - MLA	REVISED	DE
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	76

Poorly Graded Sand with Silt (SP-SM) little gravel, light gray and brown, wet, homogeneous,

	Date _	10/27/05	
inois	LOGGED BY	I Hunt	

ROUTE I-74		RIPTIO	Ne N	w I-74	Bridge Over Mississippi Approach	River - Illinois	DGGED BY L. Hunt
I-74 Bridge over Mis SECTION River	ssissippi	LOCA	TION_	(N=56	4902.45, E=2459144.517	7), SEC . 32, TWP . 1	8N, RNG. 1W, 4 th PM
COUNTY Rock Island D	RILLING N	METHO			HSA, CME 55	HAMMER TYPE	CME AUTOMATIC
STRUCT. NO. Station RW1504 Station 322 + 19	E	ΓŴ	U C S Qu	M O I S T	Surface Water Elev Stream Bed Elev Groundwater Elev.: First Encounter		
Offset 31' Rt. Ground Surface Elev. 567.96		ft) (/6")	(tsf)	(%)	Upon Completion After Hrs	ft	
Concrete 1' of concrete and crushed rock.	566.96	_					
Clayey Sand (SC) Clayey Sand, few gravel, dark brown and brown dry to moist, homogeneous.	, <u> </u>	5 5 5 5					
Sandy Clay (CL) Sandy Clay, some silt, few gravel, dark brown, dry to moist, homogeneous.	564.96	4 3 2	0.7 P				
Sandy Clay, some silt, trace gravel, black, dry to moist, homogeneous.		-5 2 2 2 2	0.2 P				
Clayey Silt to Silty Clay(MH - CL Clayey Silt to Silty Clay, trace gravel, gray brown, dry to moist, stratified.	560.96 -) 558.96	2 2 3 4 4	1.1 P				
Sand to Shale (SC) Sand to Shale, gray, wet, stratified. Water at 10' while drilling	<u>¥</u> .	2					
Shale Poss. shale	556.96	50/0					
Auger refusal at 14'; end of borehole. End of Boring	553.96						
	_ <u></u>	-15					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

Page <u>1</u> of <u>1</u>	S HANSON
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SOIL BORING LOG

Page 1 of 1

ROUTE	F.A.I. 74	DE	SCRI	PTION	ı		I-74 Over Mississippi F	River	LOGGED BYJME
SECTION	81-1HVB		_ L	OCAT	ION	NE¼ d	of SEC. 32, TWP. 18N,	RNG. 1W, 4th P.	M.
COUNTYF	Rock Island D	RILLING	ME.	THOD		Hol	llow Stem Auger	HAMMER TYPE	. Auto
Station BORING NO Station Offset	081-6018 RW 16-1 25+10 81' Rt. ce Elev. 570.1		D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft ft	<u>Z</u>
FILL - Dark to v	ery dark brown,		. ,	, ,	,	. ,	74101 11101	···	
moist to wet, so	oft and loose, silt, grained sand and								
gravel, with deg	rading plywood,		_	6 8		10			
particle board,			2-	8					
bituminous mat scraps, cinder b	erials, metal blocks, and brick		-	_					
fragments, petr	oleum odor		-						
			1-	12		8			
			٦_	12 15					
			_	woh		18			
			-	woh		10			
			6-	50/1"	_				
			_						
			8-						
			-	4	1.75P	17			
			_	5	1.70				
			-	7					
			10—						
			∇						
			-						
			12—						
			-						
			14—						
			·· –						
			_						
			-						
			16—	8		6			
		553.10		16 50/0"					
Gray, fine-grain	ed, LIMESTONE		_	(50/0")					
		FF4 C0	18—						
End of Boring		551.60		50/0"	-				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

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Experience great bridges.	

USER NAME =	DESIGNED - YSS	REVISED	
	CHECKED - JMH	REVISED	
PLOT SCALE =	DRAWN - MLA	REVISED	
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED	

BORING LOGS 3	F.A.I. RTE.			
-74 (EB)∕RAMP 6TH-C RETAINING WALL 16 [
STRUCTURE NO. 081-6018				
CHEET NO 13 OF 15 CHEETS				

(F)	HANSON
CIT	HANSON

Page <u>1</u> of <u>1</u>

									Date	6/29/1
ROUTE	F.A.I. 74	DES	SCRI	PTION	N		I-74 Over Mississippi I	River	LOGGED BY	JMB
SECTION	81-1HVB		_ ι	OCA1	ION	NE¼	of SEC. 32, TWP. 18N,	RNG. 1W, 4th F	P.M.	
COUNTY _	Rock Island Di	RILLING	ME	THOD		Но	low Stem Auger	_ HAMMER TYP	PEAu	to
Station BORING NO	0. 081-6018 . RW 16-2		D E P T	B L O W	U C S	M O I S	Surface Water Elev. Stream Bed Elev.			
Station Offset	26+33 82' Rt.		H	S	Qu	T	Groundwater Elev.: First Encounter	ft		
	rface Elev. 567.4	ft					Upon Completion	558.4 ft	$\overline{\Delta}$	
			(ft)	(/6")	(tsf)	(%)	After Hrs.	ft		
TOPSOIL		√ 567.15								
	dark brown, wet, stiff sandy SILT with trace fragments		2-	8 5 3	2.25P	19				
			_							
			4-	2 2 3	1.25P	22				
		561.90	_							
	moist, lean CLAY	301.30	6-							
with silt			- 0			16				
			_			27				
			_							
Grav. moist.	stiff, CLAY with silt	559.40	8-		2.22B	27				
J. a.y.,	o, o		∇^{-}			24				
			_							
		556.90	10-							
	medium stiff, CLAY	330.90	-							
with very fine	e-grained sand		_		0.92B	28				
			12-	7						
			_	9						
		553.90	_							
	very dense, silty,			50/5"		30				
coarse-grain GRAVEL	ed SAND and	552.90	14—							
Brown, WEA	THERED	J	_							
LIMESTONE			-							
End of Borin	a	551.30	16—	50/1"	\vdash	23				
LING OF BOTTI	9			20.7	1					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

A	11
C. T.	HANSON

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

ROUTE	F.A.I. 74	DE	SCRI	PTION	١		I-74 Over Mississippi F	River	LOGGED BY _	JMB
SECTION	81-1HVB		_ ι	OCAT	ION	NE¼ d	of SEC. 32, TWP. 18N,	RNG. 1W, 4th P	.M.	
COUNTY R	tock Island D	RILLING	ME	THOD		Hol	llow Stem Auger	HAMMER TYP	E Aut	0
STRUCT. NO. Station	081-6018		D E	B L	U	M	Surface Water Elev. Stream Bed Elev.			
BORING NO.	RW 16-3		P T	O W	S	S				
Station	27+87		H	S	Qu	T	Groundwater Elev.:			
	82' Rt. ce Elev. 569.8	_{ft}			- Qu		First Encounter Upon Completion	π 562.8 ft	∇	
Ground Garia		—"	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	<u>~</u>	
FILL - Dark brov										
medium, sandy	SILI		_	_	4 7CD	12				
CONCRETE		568.30	-	5 50/0"/	1.76B	12				
FILL - Dark brov	vn, moist, siltv	567.80	2-	30.5						
SAND and GRA			-							
			_							
			4-		1.50P	23				
			_	12 18						
			_							
		563.80	_							
FILL - Very dark	brown, moist,		6-							
soft, SILT with r and concrete fra	netal scraps, brick		∇							
	-	562.30								
silt	f, CLAY with trace		8-		1.55B	29				
			_		1.550	25				
			_							
			_ 10—							
			10-							
			_		1.36B	23				
			_	5 7	1.36B	23				
		557.60	12-	10						
Gray, moist, ver trace silt, sand a	y stiff, CLAY with		_							
,	0	556.30	_							
	RED LIMESTONE	556.10		50/2"	1.25P/	27				
End of Boring										

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

МО	DJESKI and MASTERS Experience great bridges.

USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

BORING LOGS 4	F.A.I. RTE.	
I-74 (EB)/RAMP 6TH-C RETAINING WALL 16 STRUCTURE NO.081-6018	74	(81-1)
STRUCTURE INU. UST-0018		
CHEET NO 14 OF 15 CHEETS		

of Transi	eparti oortat	me ior	nt 1		SC	IL BORIN	G LOG	Page 1 of
Division of Highways CH2M HILL								Date 8/28/0
ROUTEI-74	DE	SCR	IPTIO	N	W I-74	Bridge Over Mississipp Approach	River - Illinois L	OGGED BY SL
SECTION I-74 Bridge over N	lississippi	_ ι	OCA	TION_	(N=56	4827.741, E=2459192.0	07), SEC. 32, TWP.	18N, RNG . 1W, 4 th PN
COUNTY Rock Island	DRILLING	G ME	THOE		ŀ	HSA, CME 55	HAMMER TYPE	CME AUTOMATIC
STRUCT. NO		D E P	B L O	U C S	M 0 1	Surface Water Elev Stream Bed Elev	ft ft	
BORING NO. VIAIL-104 Station 323 + 00 Offset 4' Lt.		T H	w	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	559.7_ ft <u>▼</u>	
Ground Surface Elev. 568.	20ft		(/6")	(tsf)	(%)	After Hrs.	ft	
ASPHALT + BASE COURSE - (to 6" thick)	3" 567.70	_						
SILT - black, sandy, and gravel, moist (FILL)		_	3					
most (FILL)		_	11 12					
	565.20							
CLAY - reddish brown to greeni brown, silty, medium plastic,	sh	_	3					
medium stiff to soft, moist.		_	3	1.0	17.0			
		5	3	Р				
		_	_					
	561.10	_	3	0.3	18.9			
SHALE - medium gray, with san partings, friable, stiff.	id	_	4	В				
	559.70	▼						
SAND - medium brown, fine to medium, some silt, loose,		_	3					
saturated.		-10	2					
- moderately well consolidated 2" seam at 10'	in 557.20	_						
SANDSTONE - moderate to severely weathered.	331.20		50/4"					
ootolog wednered.								
- augered through 11.3' to 14'		_						
- augereu imougii 11.5 l0 14	554.20	_						
Borehole continued with rock coring.		15						
		_						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

Division of Highways CH2M HILL	rtation ROCK (D	ate8	/28/0
	_ DESCRIPTION App	proach	1111013	_ LO	GGED	BY	SL
I-74 Bridge over Missi SECTION River		2459192.07), SEC.	32, T \	WP . 18	N, RN	IG. 1W, 4	4 th PN
COUNTY Rock Island CO	RING METHOD NQ Core			R	_	CORE	s
STRUCT. NO	Core Diameter	in E	C O R	ECOVE	R Q D	T I M E	T R E N G
BORING NO. VIAIL-104 Station 323+00 Offset 4' Lt. Ground Surface Elev. 568.20		ft T H (ft)	E (#)	R Y (%)	٠	(min/ft)	T H (tsi
SANDSTONE - light to medium gray partings, soft to very soft, moderate occasionally medium bedded spacir	, with numerous shale partings with fractur y well cemented, non-distinct bedding at thi ng, fractures at partings are horizontal to 10 are planar to slightly irregular and sandy ro	e at 554.20 in to15 ° planar	Run 1	100	51	2.7	`
and sindour, nactures in sandstone ocalized high angle to vertical fracti	are premark to slightly weathered.		Run 2	85	21	1.2	305
- near-vertical fracture in sandstone - thin beds of medium to dark gray	e at 19.7', sandy rough shale with numerous sand partings at 20.3'	-21.5'	Run	73	40	1.6	
		-25	3				
			Run 4	98	38	1.2	
		-30	1	i 1		1	

Color pictures of the cores Yes

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)

Illinois Department of Transportation

ROCK CORE LOG

Page $\underline{3}$ of $\underline{3}$

Division of Highways CH2M HILL	_			D	ate8	/28/07
ROUTE I-74 DESCRIPTION New I-74 Bridge Over Mississippi Riv	er - II	linois	_ LO	GGED	BY	SL
I-74 Bridge over Mississippi SECTION River LOCATION (N=564827.741, E=2459192.07), S	SEC.	32, T\	NP . 18	N, RN	I G . 1W, 4	1 th PM
COUNTY Rock Island CORING METHOD NQ Core STRUCT, NO. CORING BARREL TYPE & SIZE NQ Wireline			R E C	R	CORE T	S T R
STRUCT. NO. 29+40 CORING BARREL TYPE & SIZE NQ Wireline	D E P	C O R	O V E	Q D	I M E	E N G
Station 323+00 Begin Core Elev. 554.20 ft	H	E	R Y			T H
Ground Surface Elev. 568.20 ft	(ft)	(#)	(%)	(%)	(min/ft)	(tsf)
SANDSTONE - light to medium gray, with numerous shale partings with fracture at partings, soft to very soft, moderately well cemented, non-distinct bedding at thin to occasionally medium bedded spacing, fractures at partings are horizontal to 10° planar and smooth, fractures in sandstone are planar to slightly irregular and sandy rough,	35					
localized high angle to vertical fractures, fresh to slightly weathered. (continued)	_	Run 6	98	62	0.6	
	\equiv					
- brownish gray with occasional shale clasts, increasing to numerous clast at 40.0' - 40.3', rough horizontal fractures with localized 70° rough fracture at 39.9'	_					
527.90	-40					
SHALE - medium to dark gray. End of Boring	_					
Little of Borning	_					
	_					
	_					
	_ -45					
	_					
	_					
	_					
	-50					
	=					
	_					
	_					

Color pictures of the cores Yes

Cores will be stored for examination until

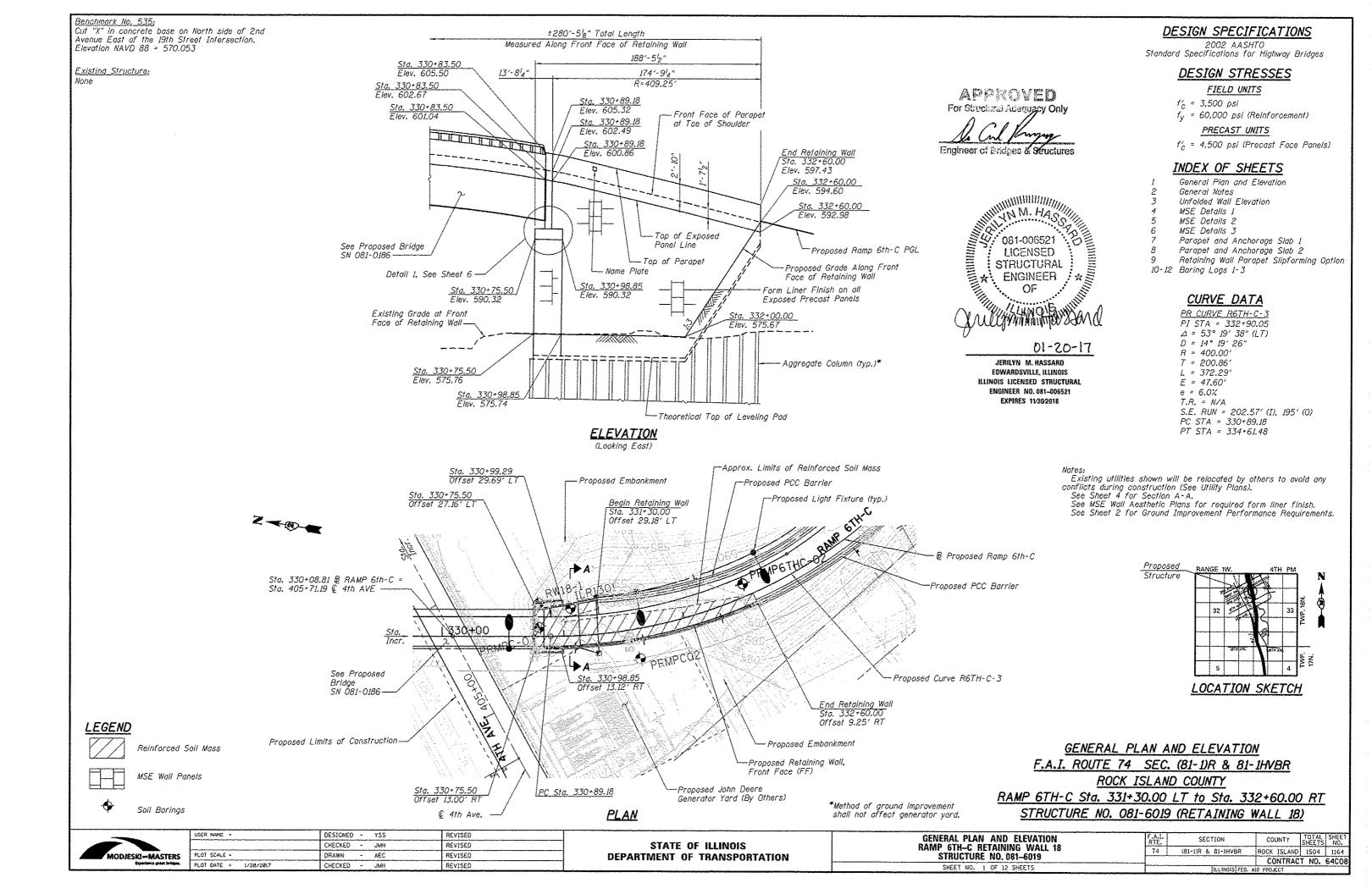
The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)

USER NAME = DESIGNED - YSS REVISED CHECKED - JMH REVISED MODJESKI and MASTERS
Experience great bridges. DRAWN - MLA REVISED PLOT DATE = 1/20/2017 CHECKED - YSS

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** BORING LOGS 5 I-74 (EB)/RAMP 6TH-C RETAINING WALL 16 STRUCTURE NO. 081-6018 SHEET NO. 15 OF 15 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHE
74	(81-1)R & 81-1HVBR	ROCK ISLAND	1504	116
		CONTRAC	T NO.	64C
	ILLINOIS FED. A	ID PROJECT		



GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Wall stations and offsets are given to the front face (FF) of the wall and are measured from the Ramp 6TH-C baseline, except as noted. FF of the wall is to be considered edge of panel or form liner.
- 3. See Special Provision for Mechanically Stabilized Earth Retaining Walls and Aggregate Column Ground Improvement for design and construction
- 4. For existing soils laboratory data, see the Geotechnical Investigation Laboratory Data Special Provision.
- 5. The piles for SN 081-0186 are located within the reinforced soil mass. See SN 081-0186 plans for additional pile requirements.
- 6. Wall system supplier shall coordinate proposed wall configuration with Aggregate Column Ground Improvement subcontractor.
- 7. Wall construction shall not begin until after Aggregate Column Ground Improvement has been completed in the area of the new wall.
- 8. See SN 081-0186 plans for maskwall details.

GROUND IMPROVEMENT PERFORMANCE REQUIREMENTS

- 1. Minimum factor of safety for global slope stability shall be 1.5.
- 2. Allowable bearing pressure (with F.S.) shall be equal to or greater than the equivalent uniform service bearing pressure as shown on Sheet 3. Intermediate values may be defined by interpolating between the values shown.

Minimum factor of safety against equivalent uniform service bearing pressure shall be 2.0 if a load test is performed.

Minimum factor of safety against equivalent uniform service bearing pressure shall be 2.5 if a load test is not performed.

- 3. Total settlement measured at the theoretical top of leveling pad shall not exceed 4.0 inches.
- 4. Total settlement measured on the pavement shall not exceed 1.0 inch.
- 5. Differential settlement measured along the theoretical top of leveling pad shall not exceed 1/100.
- 6. The assumed structure life for settlement computations shall be 75 years.
- 7. Contractor's verification program shall include monitoring points or other instrumentation to demonstrate compliance with the stated performance requirements.
- 8. The Shop Drawings and construction procedures submittal shall indicate the sequence of construction within the limits of Aggregate Column Ground Improvement. The aggregate column installation shall be coordinated with utility removal, structure removals, proposed utility installation, and bridge pile driving.
- 9. Aggregate columns shall be installed before the bridge piles are driven; however, the piles shall not be driven through the aggregate of an installed column. The aggregate column layout shall provide clearance for the bridge piles.

MSE WALL SETTLEMENT

1. The Top of Exposed Panel Elevations shown on these plans are final elevations after any settlement. The wall settlement will be determined by the ground improvement design. The wall system supplier shall coordinate with Aggregate Column Ground Improvement subcontractor to accommodate this settlement in the wall design.

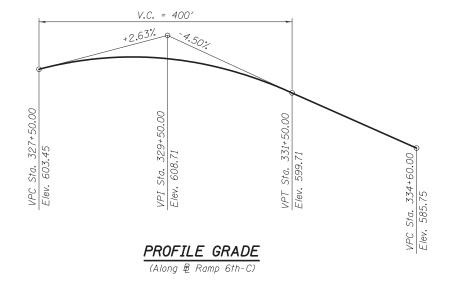
TOTAL BILL OF MATERIAL

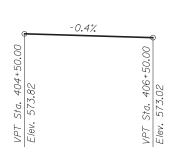
ITEM	UNIT	TOTAL
Structure Excavation	Cu. Yd.	397
Concrete Superstructure	Cu. Yd.	92.9
Protective Coat	Sq. Yd.	212
Reinforcement Bars, Epoxy Coated	Pound	14,140
Name Plates	Each	1
Aggregate Column Ground Improvement	L. Sum	0.43
Mechanically Stabilized Earth Retaining Wall	Sq. Ft.	5,419
Rock Fill	Cu. Yd.	597

^{*} See proposed retaining wall S.N. 081-6012 for remainder of L. Sum quantity.

STATION 331+30.00 BUILT 201_ BY STATE OF ILLINOIS F.A.I. RT. 74 SEC. (81-1)R & 81-1HVBR LOADING HS-20 STR. NO. 081-6019

NAME PLATE





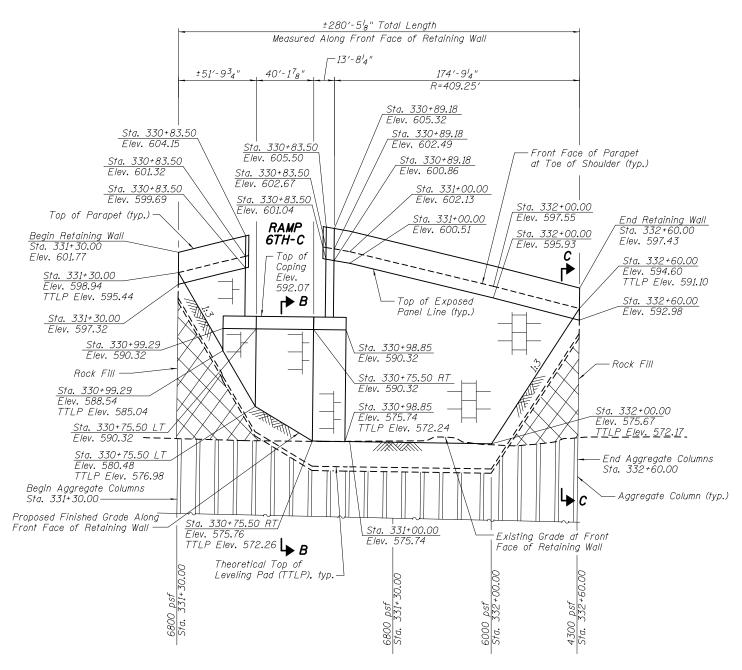
PROFILE GRADE (Along € 4th Avenue)

	USER NAME =	DE
		СН
MODJESKI === MASTERS	PLOT SCALE =	DR
		-

USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** GENERAL NOTES
RAMP 6TH-C RETAINING WALL 18 STRUCTURE NO. 081-6019 SHEET NO. 2 OF 12 SHEETS

F.A.I. SECTION COUN					TOTAL SHEETS	SHE
74	(81-1)R & 81-1HVB	ROCK	ISLAND	1504	116	
			CC	NTRAC	T NO.	64C
	TI I TNOTC	EED A	ID DDO I	гот		



UNFOLDED WALL ELEVATION

LEGEND:

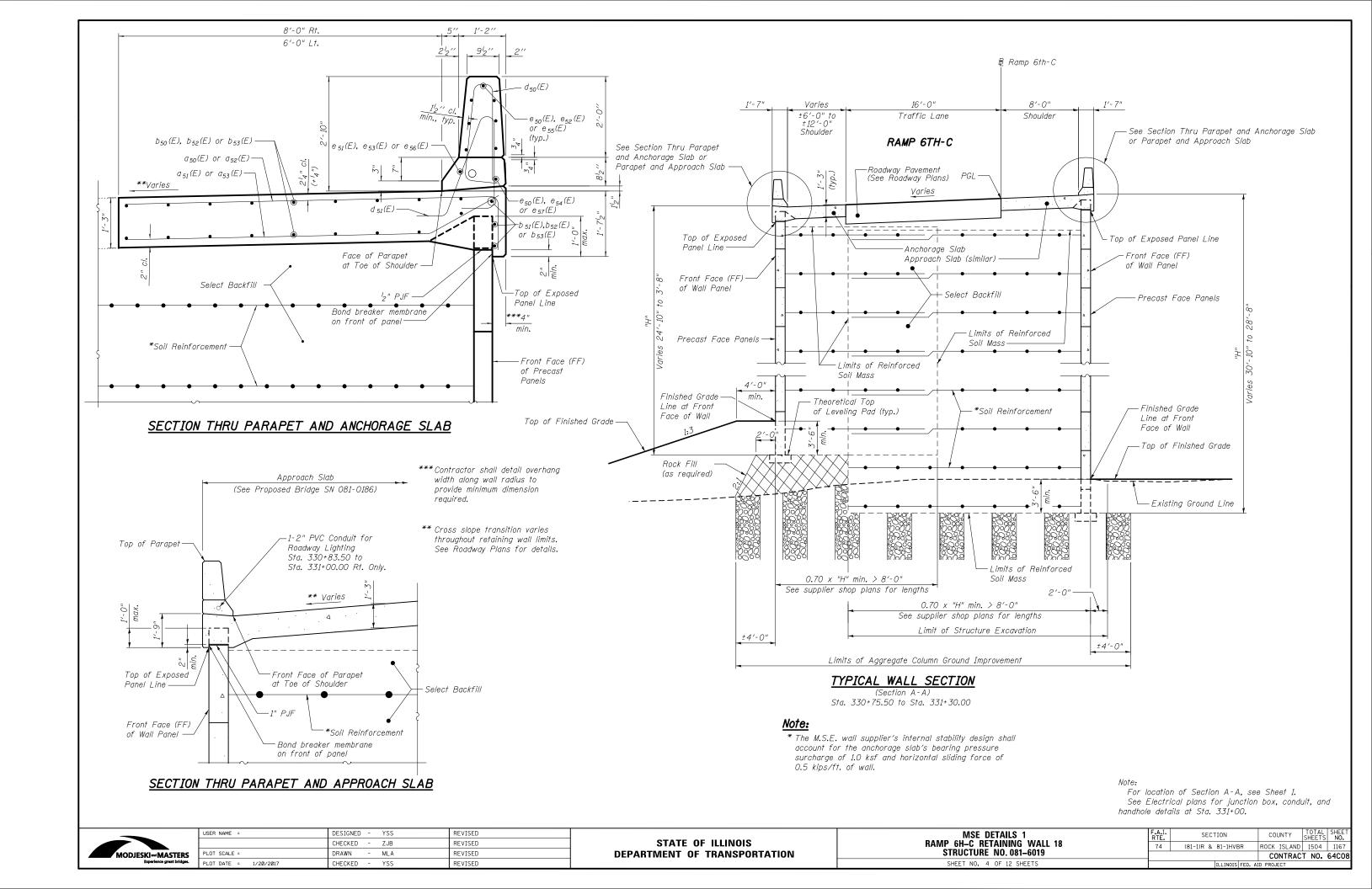
<u>6800 psf</u> Equivalent Uniform Service Bearing Pressure

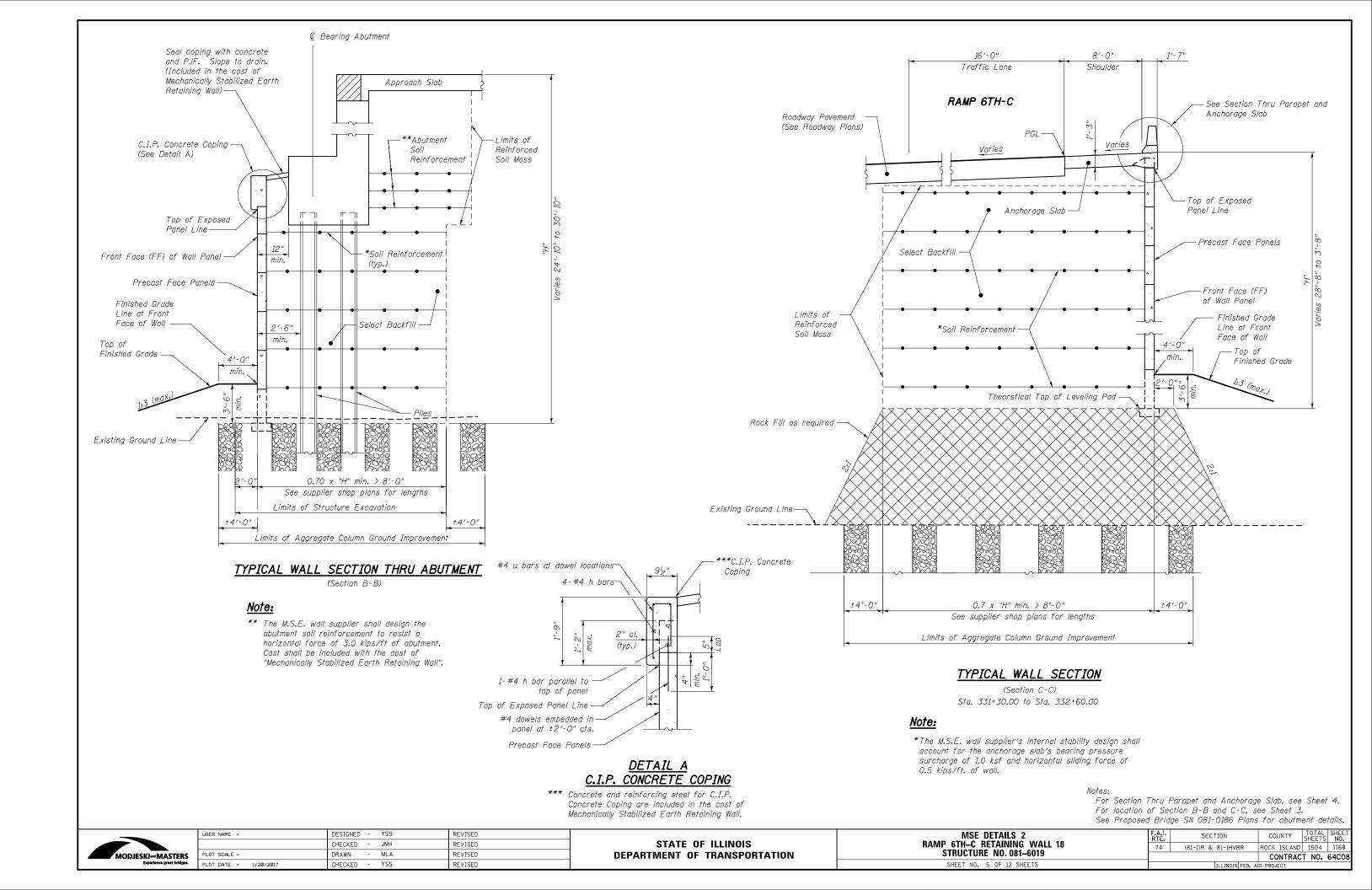
Notes:

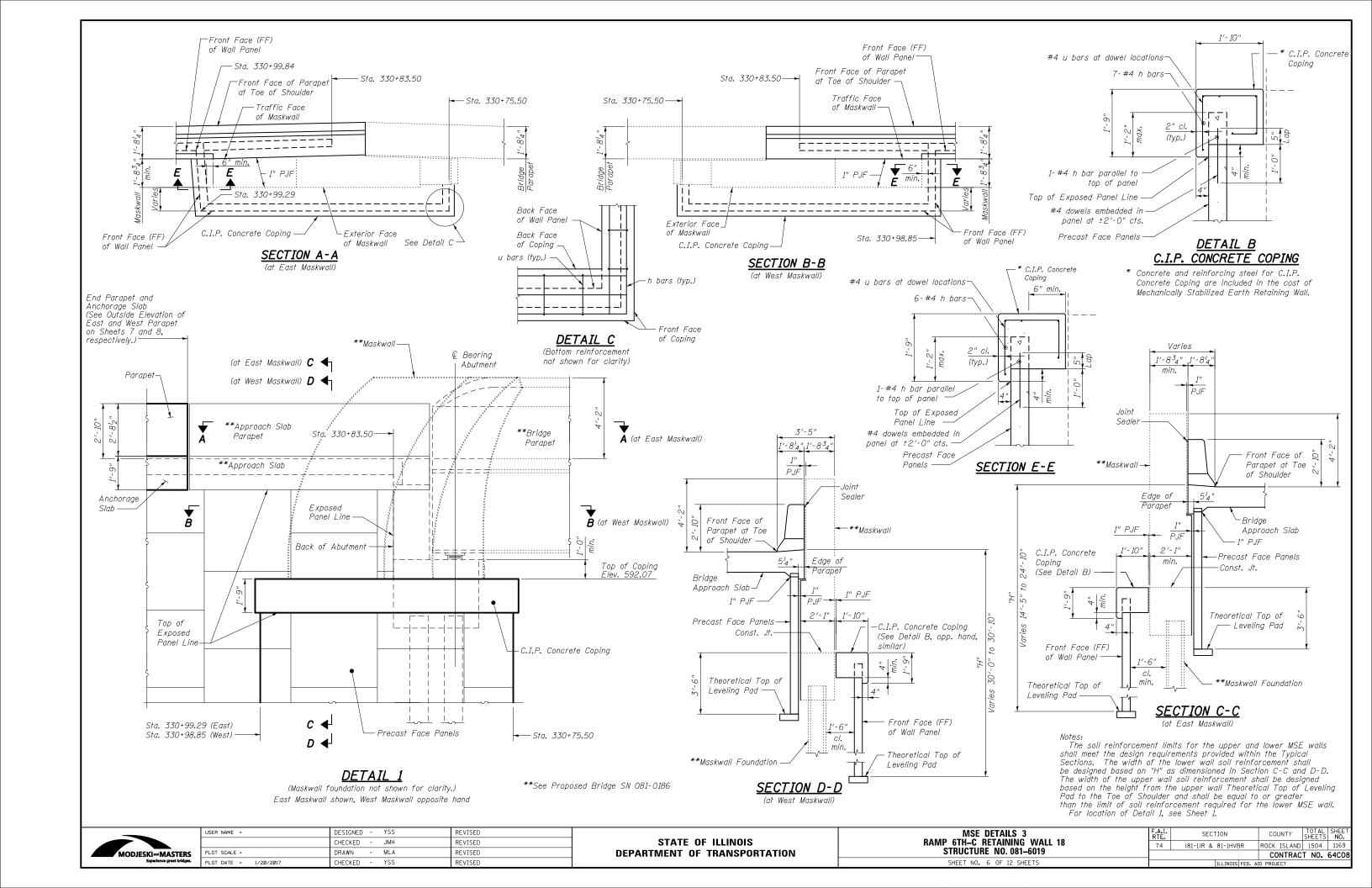
See Sheet 5 for Sections B-B and C-C.
See Sheet 2 for Ground Improvement Performance Requirements.
See Electrical plans for junction box, conduit, and handhole details at Sta. 331+00.

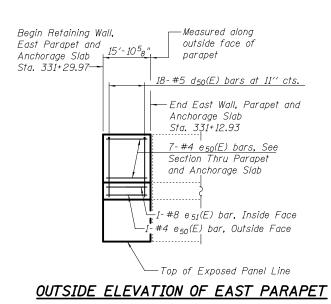
MODJESKI MASTERS

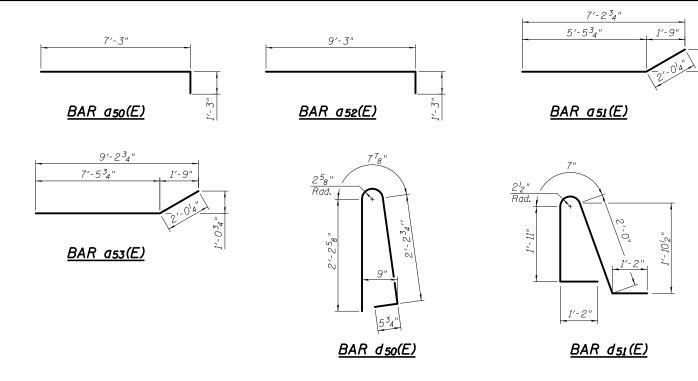
USER NAME =	DESIGNED - YSS	REVISED		UNFOLDED WALL ELEVATION	F.A.I. SECTION	COUNTY TOTAL SHEET SHEET NO.
	CHECKED - JMH	REVISED	STATE OF ILLINOIS	RAMP 6TH-C RETAINING WALL 18	74 (81-1)R & 81-1HVBR	ROCK ISLAND 1504 1166
PLOT SCALE =	DRAWN - AEC	REVISED	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 081-6019	·	CONTRACT NO. 64CO8
PLOT DATE = 1/20/2017	CHECKED - JMH	REVISED		SHEET NO. 3 OF 12 SHEETS	ILLINOIS FED.	AID PROJECT

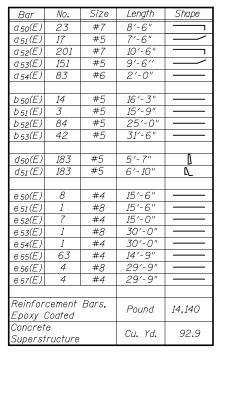






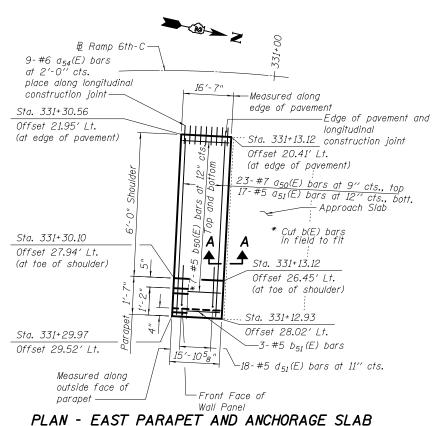


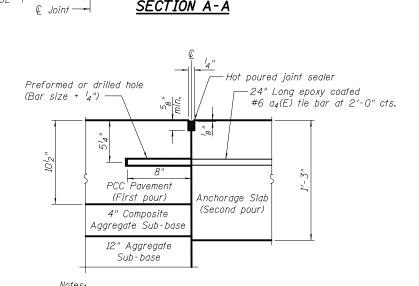




RETAINING WALL 18

BILL OF MATERIAL





The contractor may substitute at his option, formed

in place tie bars provided the bar length is increased to

Preformed or drilled hole shall be in the first pour.

30" and the tie bar is centered across the joint.

SECTION A-A

Bridge Approach Slab

See Proposed Bridge SN 081-0186

Approach Footing

Select Fill

For Section Thru Parapet and Anchorage Slab, see Sheet 4.

Joints in the adjacent pavement shall be aligned with the anchorage slab joints.

Stations and offsets on this sheet are given to the outside face of the parapet and are measured from Ramp 6th-C baseline, except as noted.

LONGITUDINAL CONSTRUCTION JOINT GROUTED-IN-PLACE TIE BAR

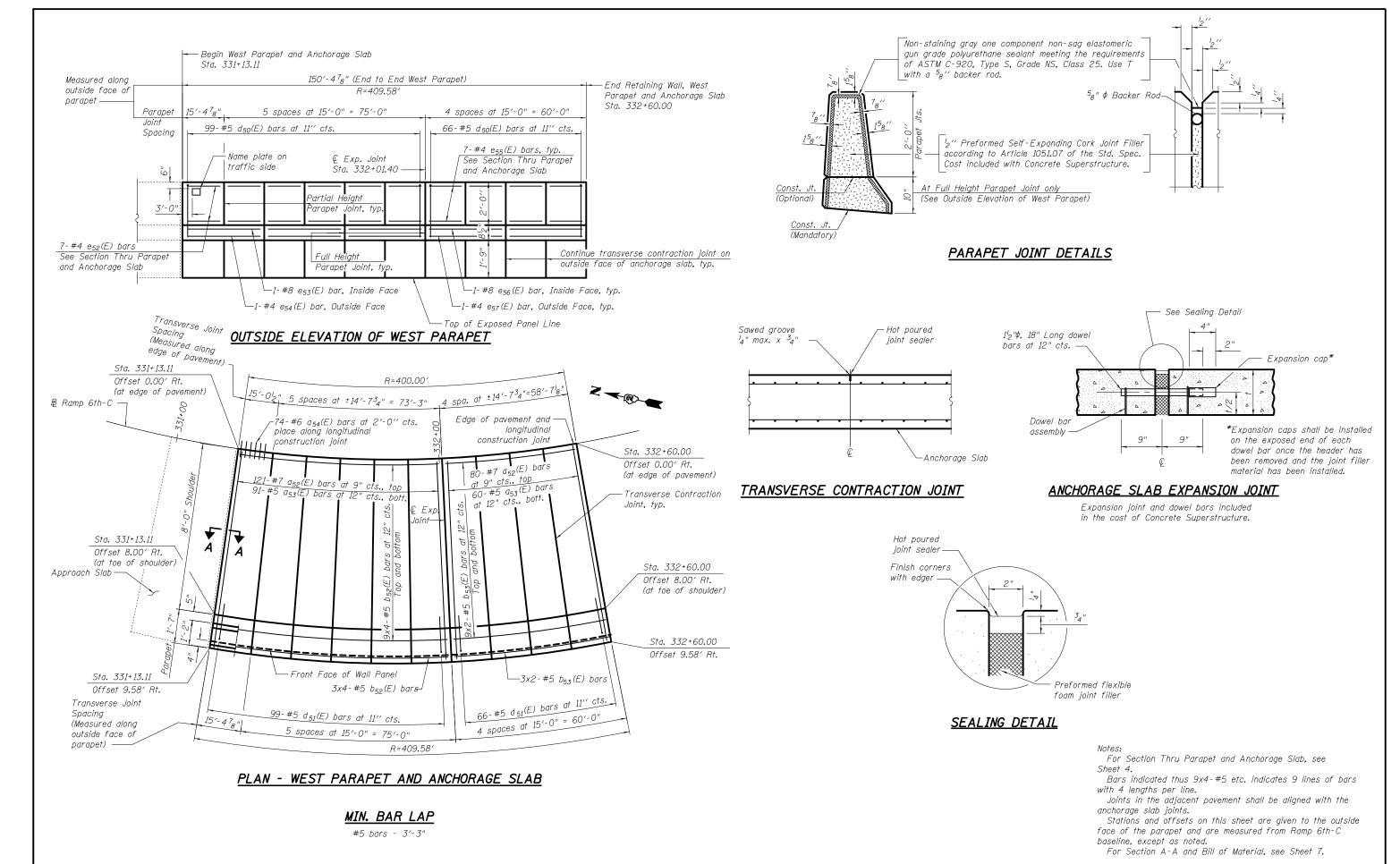
€ Joint

Anchorage Slab

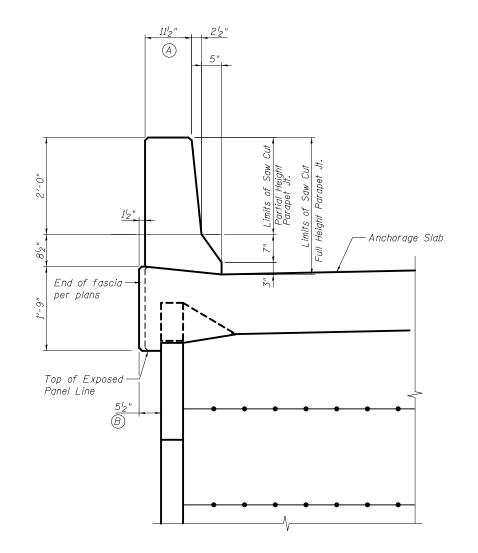
Along PGL

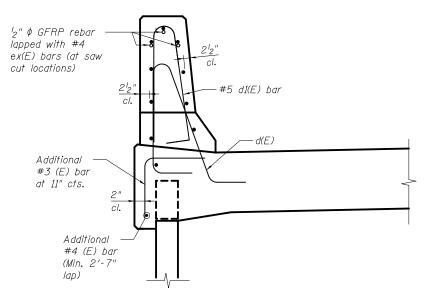
IODJESKI --- MASTEI

t bridges.	PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED		SHEET NO. 7 OF 12 SHEETS	ILLINOIS FED.	AID PROJECT		
TERS	PLOT SCALE =	DRAWN - MLA	REVISED	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 081-6019		CONTRACT NO. 64CO8		
		CHECKED - ZJB	REVISED	STATE OF ILLINOIS	RAMP 6TH-C RETAINING WALL 18	74 (81-1)R & 81-1HVBR	ROCK ISLAND 1504 1170		
	USER NAME =	DESIGNED - YSS	REVISED		PARAPET AND ANCHORAGE SLAB 1	RTF SECTION	COUNTY SHEETS NO.		



PARAPET AND ANCHORAGE SLAB 2 RAMP 6TH-C RETAINING WALL 18 USER NAME = DESIGNED -YSS REVISED SECTION COUNTY STATE OF ILLINOIS CHECKED -JMH REVISED (81-1)R & 81-1HVBR ROCK ISLAND 1504 | 1171 MODJESKI MASTERS DRAWN MLA REVISED **DEPARTMENT OF TRANSPORTATION** STRUCTURE NO. 081-6019 CONTRACT NO. 64CO SHEET NO. 8 OF 12 SHEETS LOT DATE = 1/20/2017 CHECKED -YSS REVISED

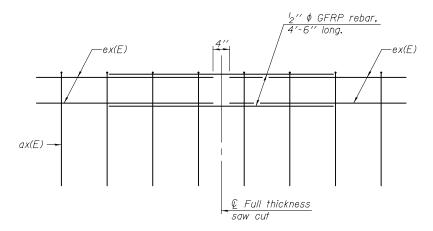




<u>SECTION</u>

(Showing reinforcement clearances for slip forming and additional reinforcement)

SECTION THRU PARAPET AND ANCHORAGE SLAB



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)



Alfred Benesch & Company 205 North Michigan Avenue, Sulte 2400 Chicago, Illinois 60601 312-565-0450 Job No. 10061



	USER NAME =	DESIGNED	-	KMP	REVISED
		CHECKED	-	SLD	REVISED
;	PLOT SCALE =	DRAWN	-	RMG	REVISED
	PLOT DATE = 1/20/2017	CHECKED	-	SLD	REVISED

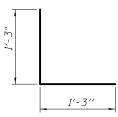
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

RETAINING WALL PARAPET SLIPFORMING OPTION RAMP 6TH-C RETAINING WALL 18 STRUCTURE NO. 081-6019 SHEET NO. 9 OF 12 SHEETS

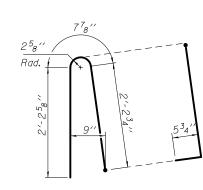
GENERAL NOTES

All dimensions shall remain the same as shown on superstructure details, except dimensions A and B which are to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension A equals 0.016 cu. yds./ft.

Full thickness saw cut at all joint locations in lieu of cork joint filler.



#3 (E) BAR



ALTERNATE BAR #5-d1(E) (When conduit is present)



SOIL BORING LOG

Page <u>1</u> of <u>1</u>

	CH2M HILL			Date _	9/19/07
			New I-74 Bridge Over Mississippi River - Illinois		
ROUTE _	I-74	_ DESCRIPTION	Approach	LOGGED BY	F. Abreu
	L 74 Bridge over Missi	issippi			

I-74 Bridge over Miss SECTIONRiver	sissippi I	OCA	TION _	(N=56	4025.307, E=2459262.179), SEC. 32	2, TWP.	18N	, RNG	. 1W, 4	th PM
COUNTY Rock Island D	RILLING ME	THOE		H	HSA, CME 55 HAMMER	TYPE	CI	ME AU	TOMA	TIC
STRUCT. NO.	— Р Т Н	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	_ ft _ ft _ ft	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)
3" Of Asphalt Surface 3" Asphalt Silty Sand (SM) grayish brown, slightly moist, medium dense, fine to coarse grained, non-plastic plasticity, Grayish brown, slightly moist, medium dense, fine to coarse, non	575.45	4 8 5			Sandstone grayish brown, stiff, No recovery 1' of laminated silt stone/sand stone in the shoe (continued) No recovery 1 1/2' of laminated grey sandstone in tip	•				
plastic, fines dark gray, loose, low plasticity, Same as above, dark gray, loose, low plasticity fines yellowish brown, very loose, Same		2 2 3					25			
as above very moist wet, loose, Same as above, more sit		1 1 2 2 2 2		23.0	End of Boring	546.95	-30			
Sandy Silt (SM) Grayish brown, moist, medium to stiff, low plasticity, fine to medium grained	563.95	0 3 3		13.0						
grayish brown, moist, firm, fine to medium grained, Same as above, stiff, grey brown siltstone at tip (4' total count pocket pen)		0 5 8								
grayish brown, stiff, No recovery 1' of laminated silt stone/sand stone in the shoe	" <u>-</u>	30								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

₹ Hanson

SOIL BORING LOG

Page $\underline{1}$ of $\underline{1}$

	HANSON			, O i		,	RING LOG				
										Date _	
ROUTE	F.A.I. 74	DES	SCR	PTION	١		I-74 Over Mississippi F	River	LOG	GED BY	JMB
SECTION _	81B		_ ı	OCAT	ION _	NE¼ o	of SEC. 32, TWP. 18N,	RNG. 1W, 41	h P.M.		
COUNTY _	Rock Island D	RILLING	ME	THOD		Hol	llow Stem Auger	_ HAMMER 1	TYPE _	Aut	0
Station BORING NO	PRMP 6th C-02	_	D E P T	B L O W	U C S	M 0 1 8	Surface Water Elev Stream Bed Elev				
Station Offset	332+55 10' Lt.	-	н		Qu	T	Groundwater Elev.: First Encounter		ft		
Ground Su	rface Elev. <u>576.5</u>	ft	(ft)	(/6")	(tsf)	(%)	First Encounter Upon Completion After Hrs.	566.5	ft ∑ ft		
ASPHALT		576.10									
Very dark bro sandy, lean (own, moist, stiff, siity,		_	4	0.95B	18					
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			2—	5	0.550	"					
			2-	6							
			_	-							
		572.50	4-								
Gray, moist, CLAY	stiff, silty, sandy, lear	1	· –		0.90B	15					
			_								
			6-								
	own mottles, moist to	570.00	-	1							
wet, soft, silt	y, lean CLAY		Ξ	1							
			8—	-							
			_	2 2	0.55B	32					
				2							
		•	1 V -								
		565.00	_		0.43B	22					
	medium dense, silty,		12—								
clayey, fine- SAND, petro	to medium-grained leum odor		_								
			-								
Drougn sunt	dense, medium- to	562.50	- 14 —	7 10		13					
coarse-grain	ed SAND and		-	20							
GRAVEL		561.00	_	E0 (01)							
End of Boring	Rock	560.50	16—	50/0"							
	-										
				l	l		1				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

MODJESKI and MASTERS Experience great bridges	

USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

DUNING LUGS I	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
RAMP 6TH-C RETAINING WALL 18	74	(81-1)R & 81-1HVBR	ROCK ISLAND	1504	1173
STRUCTURE NO. 081-6019			CONTRAC	T NO.	64CC
SHEET NO. 10 OF 12 SHEETS		ILLINOIS FED. A	ID PROJECT		

(P)	Illinois Department of Transportation
	Division of Highways

Sandstone Auger refusal and 555.95 -20 50/2

SOIL BORING LOG

Page <u>1</u> of <u>2</u>

	DESCR	IPTIO			Bridge Over Mississippi Approach		OGGED BY B. Karı
I-74 Bridge over Mississip River	pi L	OCAT	ION_	(N=56	3968.083, E=2459220.4	95), SEC. 32, TWP.	18N, RNG. 1W, 4 th
DUNTY Rock Island DRILL	NG ME	THOD		ŀ	HSA, CME 55	HAMMER TYPE	CME AUTOMATION
DRING NO. PRMPC02	D E P T	B L O W	U C S	M 0 1 s	Surface Water Elev Stream Bed Elev Groundwater Elev.:		
Station 331 + 59 Offset 24' Rt. Ground Surface Elev. 575.95	H (ft)	S (/6")	Qu (tsf)	T (%)	First Encounter Upon Completion After Hrs.		
" asphalt concrete, underlain by crushed gravel 574.	05						
scellaneous Fill Poorly graded nd, brown, moist, fine to coarse, , underlain by 3" thick brick, ay, gravel mix		5 4 5 14					
Sand, gravel, silty clay mix		28 24 18					
Concrete pieces, gravel, sand	5	8 3 11 10					
Bricks, concrete rubble, gravel, ty clay, gray, brown, moist, soft, w plasticity	_	5 6 6 4					
Reddish brown silty sandy clay, oist, soft/loose, fine sand seams th alternating silty clay seams	-10	3 4 4 3					
Gray sandy clay, moist/wet, soft, le sand and fines with iron oxide reaks with poorly graded fine to edium sand seams	=	3 3 2					
Gray/black sandy clay, oist/wet, asphalt concrete with otroleum odor	=	3 7 6					
	15	8					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

Division of Highways CH2M HILL						D	ate12	2/15/05
	DESCRIPTION	I-74 Bridge Over Mississippi Approach			LO	GGED	BY B.	Karnik
I-74 Bridge over SECTION River	Mississippi LOCATION (N	I=563968.083, E=2459220.49	95), SEC	. 32.	TWP.	18N, F	NG. 1W	, 4 th PM
COUNTY Rock Island	CORING METHOD NQ DO	DUBLE BARREL DIAMOND	TIP		R E	R	CORE	S
STRUCT. NO.		TYPE & SIZE	_ D	С	C	Q.	T	R
Station	Core Diameter	in	E	0	V		M	N
BORING NO. PRMPCO	2 Top of Rock Elev	, <u>555.95</u> ft	P	R	E R	D	E	G
Station 331 + 59 Offset 24' Rt.	Begin Core Elev.	555.95 ft	Ĥ	_	Y			н
Ground Surface Elev. 57			(ft)	(#)	(%)	(%)	(min/ft)	(tsf)
	rained, slightly weathered, weath		5.95	R-1	50	17		
	ared Horizontal fractures, no sta e at bottom 3", black sandstone							
smooth undulating joints, thin	silty infilling at 9" from the top, r	no infilling elsewhere						
			-					
Light gray, fine grained, sligh	itly weathered, weak to modera	tely strong, extremely		R-2	100	45		
ractured to sound, with shale Fractures are mostly horizonta	seams throughout, Coring rate: al, extremely close to moderate	spacing, no staining,	-					
smooth undulating joint surfactiones have silty infilling coating	es, highly fractured zones at 2'	3" and 4' 6" from the top,	_					
zones have sitty intilling coatir	ig with fractured pieces		-25					
			_					
			_	-				
			_					
	emely fractured to sound, unwe I throughout Coring rate: 14 mir		_	R-3	93	83		
fractures, no staining, smooth	undulating surfaces, discontinu	uities are extremely close	_					
to moderately spaced, snaley tightly healed joints	infilling (very thin) and coating	at some joint surfaces,						
			30					
			-					
			-					
moderately fractured to soun	nd, unweathered Coring rate: 6	minutes for 5' Horizontal	-	R-4	97	85	-	447.0
joints, no staining, smooth und	dulating joints, some joints are a oft silty infilling is present preven	at 20 degrees, no infilling			Acces			1000000
	close to moderately spaced dis		_					
			-35					
			200					
			_					
			-	-				
	shale seams, extremely fracture athered Horizontal joints, no st			R-5	77	23		



SOIL BORING LOG

Page <u>1</u> of <u>3</u>

() of Transportat	ioi	n		SC	IL BORIN	G LOG		
Division of Highways CH2M HILL			No	w I 74	Bridge Over Mississippi	Divor Illinois	Date _	9/4/07
ROUTE DE	SCR	IPTIO	N	W 1-74	Approach	Niver - Illinois	LOGGED BY	KJB
SECTION I-74 Bridge over Mississippi River	_ ı	LOCA	TION _	(N=56	4052.458, E=2459235.2	91), SEC . 32, TW	VP. 18N, RNG. 1	1W, 4 th PM
COUNTY Rock Island DRILLING	Э МЕ	THOD		ŀ	HSA, CME 55	HAMMER TYP	'E CME AUT	OMATIC
STRUCT. NO	D E P	B L O	U C S	M 0 1	Surface Water Elev Stream Bed Elev	ft ft		
BORING NO.	T H (ft)	W S (/6")	Qu (tsf)	S T (%)	Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft	Ţ	
PAVEMENT - asphalt concrete (4" 525.47 thick) SILT - yellowish brown to brown and orange-brown mottled to gray, little to some clay, powdery,		4 5 9	2.5 P					
slightly to medium plastic, medium stiff to stiff, moist - dark brown, little to some clay	_	1 1	0.5					
- some clay, medium plastic	5	2 2 2 2	0.8 P					
567.30 CLAY - tan, brown and orange, little to some fine sand, soft to medium stiff, very moist to wet.	-10	2 1 2	0.5 P					
	_	WOH 1 3	1.0 P					
SAND - black, fine to coarse, and dark gray medium to high plastic clay, very soft/loose, saturated.		WOH 2 0						
[Note: strong petroleum odor and trace free product in saturated 559.80 zone at 13.5'-15'; PID = 420 ppm] SHALE - light gray, sandy (hard clay), no laminations, dry.		20 34 60	>4.5 P					
Borehole continued with rock	_		F					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED

End of Boring
Color pictures of the cores
Cores will be stored for examination until
The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)
BBS, form 138 (Rev. 8-99)

(A)	Illinois Department of Transportation

ROCK CORF LOG

Page <u>2</u> of <u>3</u>

	NOCK COKE LOG					
			Da	ate	9/4/07	
PTION_	New I-74 Bridge Over Mississippi River - Illinois Approach	_ LO	GGED	вү	KJB	
OCATIO	ON (N=564052.458, E=2459235.291), SEC. 32, T	WP.	18N, R I	NG. 1W	, 4 th PM	
		_			_	-

ROUTEI-74 DESCRIPTION Approach	er - ı	llinois	_ LO	GGED	BY	KJB
I-74 Bridge over Mississippi SECTION River LOCATION (N=564052.458, E=2459235.291).	SEC	. 32, 1	Γ W P. 1	8N, R	NG. 1W,	4 th PM
COUNTY Rock Island CORING METHOD NQ Core			RE	R	CORE	S T
STRUCT. NO. CORING BARREL TYPE & SIZE NQ Wireline	D E P	C O R	C O > E	Q D	T I M E	R E N G
BORING NO. PRMPC-03 Top of Rock Elev. 559.80 ft	T H	Ë	R Y		-	T H
Ground Surface Elev. 575.80 ft	(ft)	(#)	(%)	(%)	(min/ft)	(tsf)
SANDSTONE - light brownish gray, fine grained, uniform grain size, well sorted, moderately well cemented, soft, localized black banding and light gray shale pod inclusions, primarily horizontal sandy rough fractures, non-distinct bedding with fractures at thin to thick bedded spacing, slightly weathered to fresh.	-20	Run 1	98	55	1.5	
	_	Run 2	100	69	8.0	
-dark gray shale bed with numerous light gray sandstone partings and seams, soft, rock-like at 21' to 22.8'	_					
	_					
	-25					
-4" thick dark gray to black sandy shale seam at 25.7' to 26.0'	_	Run	98	83	0.6	
-brown spotted/speckled fine grained sandstone at 26' to 27.3'		3				
	30					
	_	Run 4	100	85	0.6	
	_	7				
	Ξ					
	-35					
	=	Run 5	98	98	0.7	
	_	-				

Color pictures of the cores	Yes
Coree will be stored for eva	mination until

	W HILL							D	ate	<u>3/4/07</u>
ROUTE	I-74	DESCRIPTION	New I-74 Brid	ge Over Mississipp Approach	i River - II	linois	LO	GGED	BY	KJB
I-74	Bridge over Miss	sissippi								
SECTION	River	LOCATI	ON (N=564052	.458, E=2459235	291), SEC	. 32,		18N, R		\equiv
COUNTY Roo	k Island Co	ORING METHOD $_$	NQ Core				R	R	CORE	S
STRUCT NO		CODING DA	DDEL TYPE 8	CIZE NO Missella		_	c	"`	Т	R
STRUCT. NO Station	330+80			SIZE NQ Wirelin	D E	C	0 V	Q	I M	l E
		Core Diam	eter <u>1</u> k Elev. <u>559</u>	<u>.8</u> in 9.80 ft	P	R	Ě	D	E	Ğ
BORING NO Station	PRMPC-03 330 ± 80	lop of Roc	Elev557	7.40 ft	Т	E	R			1
Offset	7′ Lt.	Degin out	, <u></u>		н	ĺ	Y			١
Ground Surface		ft			(ft)	(#)	(%)	(%)	(min/ft)	(ts
		, fine grained, unifor								
		calized black bandin dy rough fractures, r				ĺ				
		acing, slightly weath			-40	ĺ				
		3, 3	,	,	_	ĺ				
						1				
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					_	ĺ				
End of Boring				5	29.80	<u> </u>				
and or borning					-					
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Color pictures of the cores Yes

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)

HANSON

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

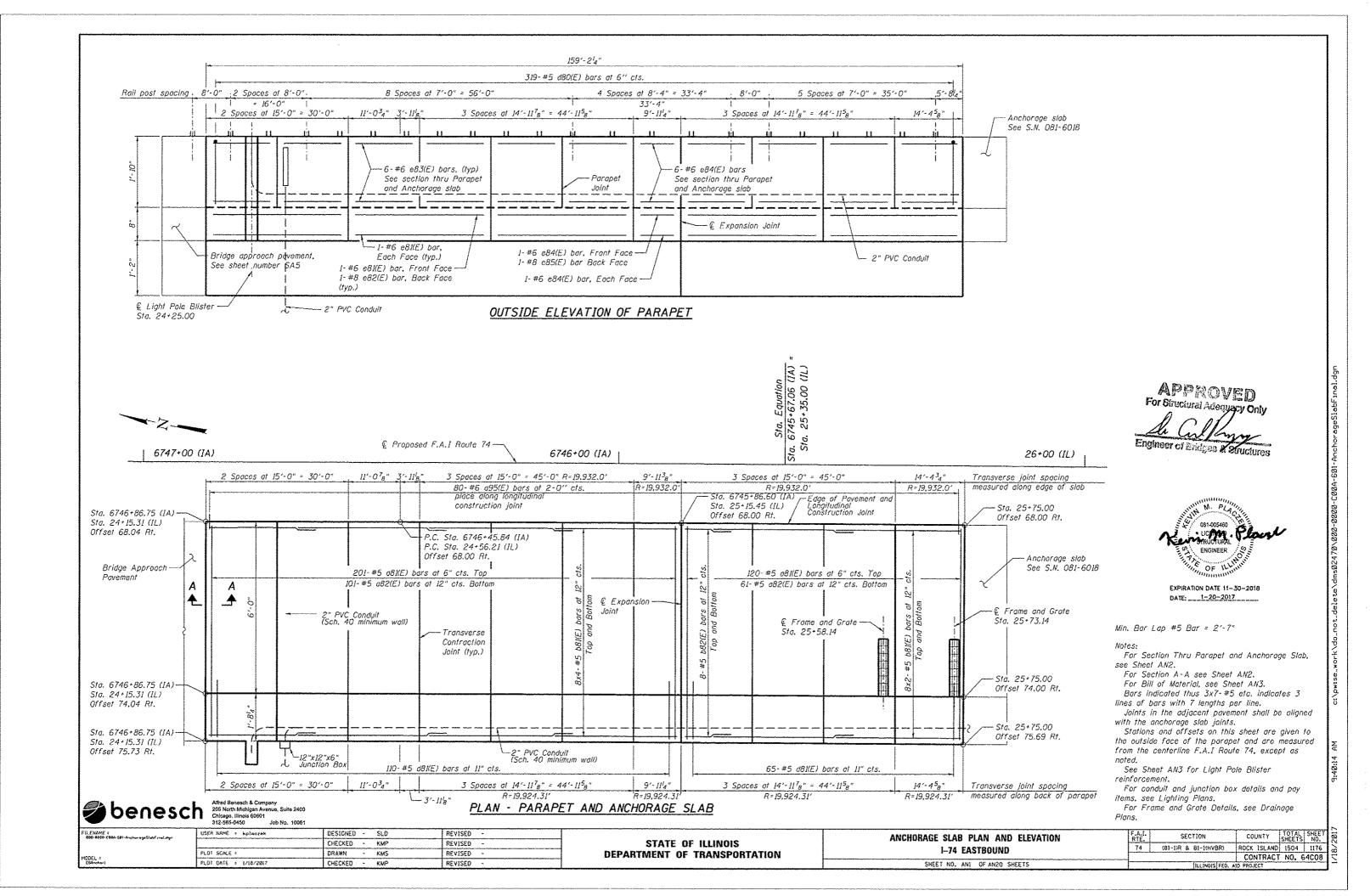
SECTION 81-1HVB			OCAT	ION	NE1/.	SEC 32 TWP 19N	RNG 1W 4	th P M		
COUNTY Rock Island DF	ILLING	ME	THOD		Hol	low Stem Auger	_ HAMMER	TYPE _	Aut	to
STRUCT. NO. 081-6019 Station RW 18-1 BORING NO. RW 18-1 Station 330+79 Offset 27' Lt. Ground Surface Elev. 576.0		D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	564.0	- _ft ft.∇		
ASPHALT	575.80	-								
FILL - Very dark brown, moist, stiff, clayey SILT with fine-grained sand and gravel, coal and cinders		2—	6 6 5		13					
		_								
		4—	2 3 3		25					
	569.50	6—		1.10B	20					
Brown and gray, moist, medium stiff, SILT with trace very fine-grained sand	503.50	_			21					
		8-			21					
		- 10 <i>-</i>			23					
	564.00	- 12	3 3		22					
Brownish gray, wet, loose, silty, fine- to medium-grained SAND	562.50	_	3							
Gray, wet, soft, SILT with fine-grained sand, petroleum odor		14—	2 2 2	0.30P	29					
Gray, WEATHERED SILTSTONE	560.50 559.50		50/5"	3.50P	7					
End of Boring	_ 50.00	_								

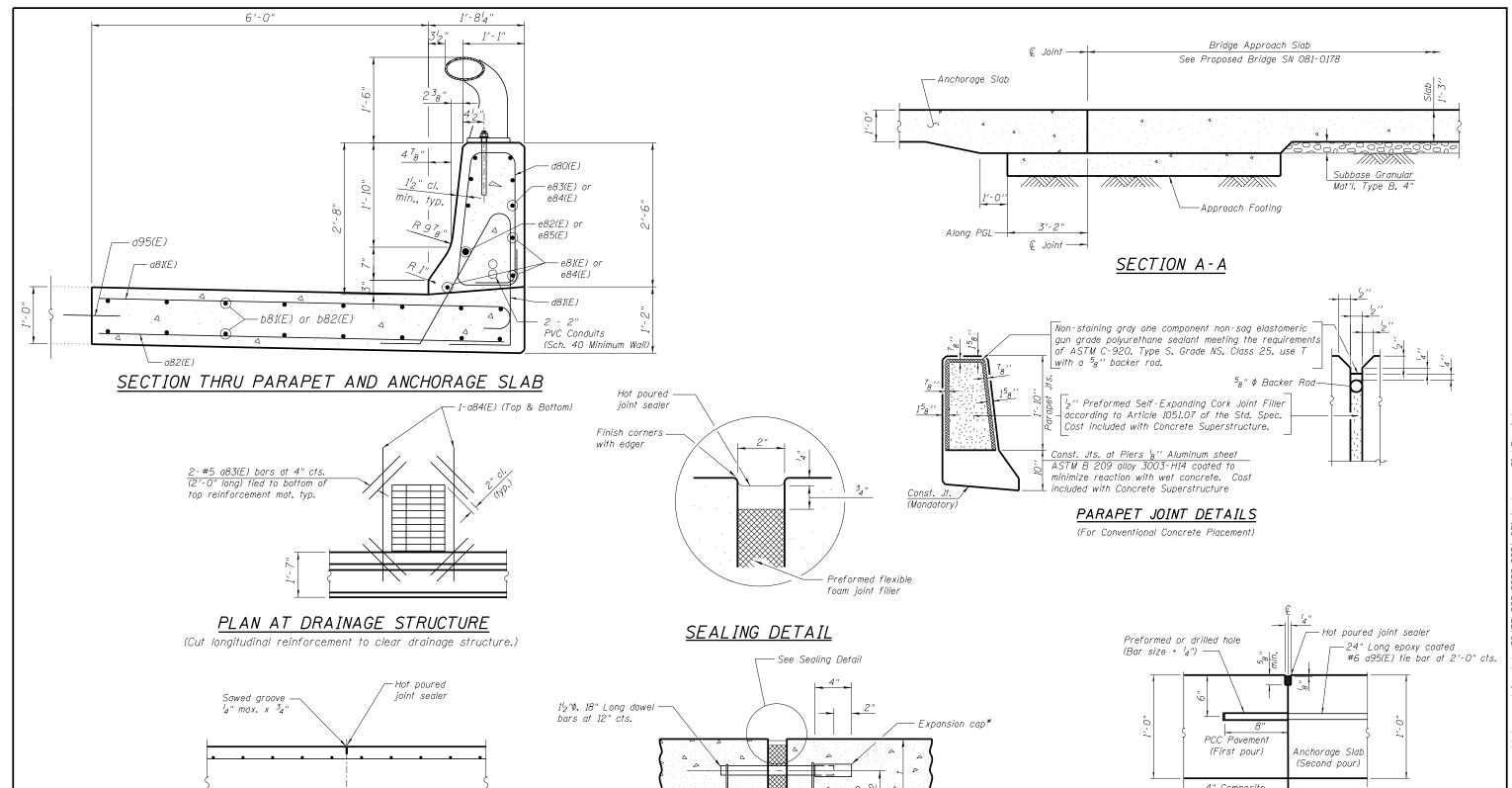
The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

MODJESKI and MASTERS Experience great bridges	

USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - MLA	REVISED
PLOT DATE = 1/20/2017	CHECKED - YSS	REVISED





TRANSVERSE CONTRACTION JOINT

Anchorage Slab

ANCHORAGE SLAB EXPANSION JOINT

*Expansion caps shall be installed

been removed and the joint filler

on the exposed end of each dowel bar once the header has

material has been installed.

Expansion joint and dowel bars included in the cost of Concrete Superstructure.

Preformed on (Bar size +		Long epo	ealer oxy coated o bar at 2'-0" cts.
1	4" Composite Aggregate Sub-base		
_	12" Aggregate Sub-base		
Notoc			

The Contractor may substitute at his option, formed in place tie bars provided the bar length is increased to 30" and the tie bar is centered across the joint. Preformed or drilled hole shall be in the first pour.

LONGITUDINAL CONSTRUCTION JOINT GROUTED-IN-PLACE TIE BAR

benescl	1	2 C 3
FILENAME = 000-0000-0004-601-AnchorageSlabFinal.dgn	USE	R
our recorder out and agent and manager		

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chlcago, Illinols 60601 Job No. 10061

USER NAME = kplaczek	DESIGNED	-	SLD	REVISED -
	CHECKED	-	KMP	REVISED -
PLOT SCALE =	DRAWN	-	KMS	REVISED -
PLOT DATE = 1/18/2017	CHECKED	-	KMP	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

Dowel bar

assembly

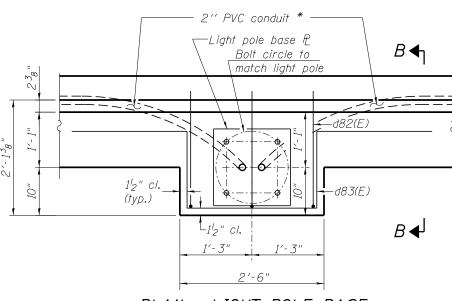
ANCHORA 1–74		SLAB D STBOUN		
SHEET NO.	VVIS	OF ANSO	SHEETS	

A.I. TE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.						
74	(81-1)R & 81-1(HVBR)	ROCK ISLAND	1504	1177						
		CONTRACT	NO. 6	4C08						
	ILLINOIS FED. AID PROJECT									

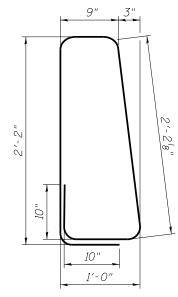
BILL OF MATERIAL EASTBOUND

Bar	No.	Size	Length	Shape
a81(E)	321	#5	7′-10"	
a82(E)	162	#5	7′-3"	
a83(E)	16	#5	2'-0"	
a84(E)	8	#5	5′-0"	
a95(E)	80	#6	2'-0"	
. 04/5	0.0		07/ 0"	
b81(E)	96	#5	27'-0"	
b82(E)	16	#5	10′-9"	
(00/5)	7.10		7/ 10/	0
d80(E)	319	#5	7′-10"	Ŋ
d81(E)	175	#5	8′-3"	
d82(E)	3	#6	4'-5"	L
d83(E)	5	#6	8'-11"	7
e81(E)	<i>1</i> 5	#6	29′-8"	
e82(E)	5	#8	29′-8"	
e83(E)	60	#6	14′-8"	
e84(E)	9	#6	9′-7"	
e85(E)	1	#8	9′-7"	
Concrete	Superstru	Cu. Yd.	66.1	
Reinforce Epoxy Co		Pound	13,790	
Protective			Sq. Yd.	182.1

Bars indicated thus 1 x 5 - #5 etc. indicates 1 line of bars with 5 lengths per line.



PLAN - LIGHT POLE BASE

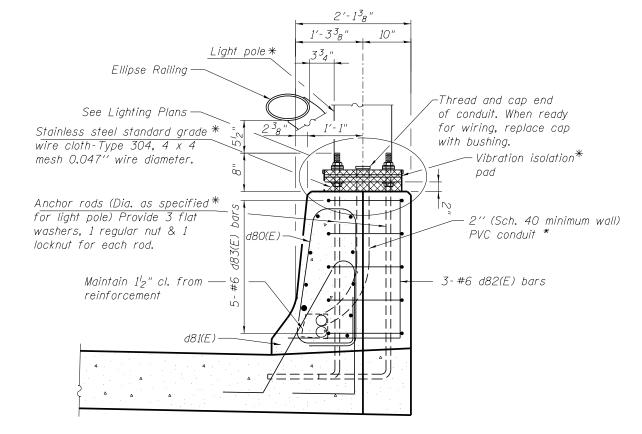




10"

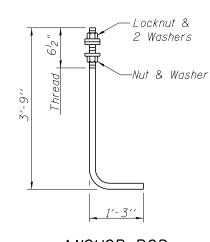
BAR d81(E)

10"



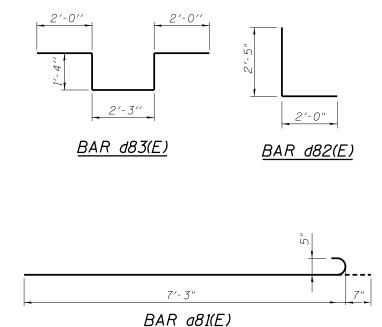
SECTION B-B

* See lighting plans for details and pay items



ANCHOR ROD

Diameter as specified for light poles (ASTM F 1554 Grade 105). Full length hot dipped galvanized. Anchor bolts included in the cost of Concrete Superstructure.

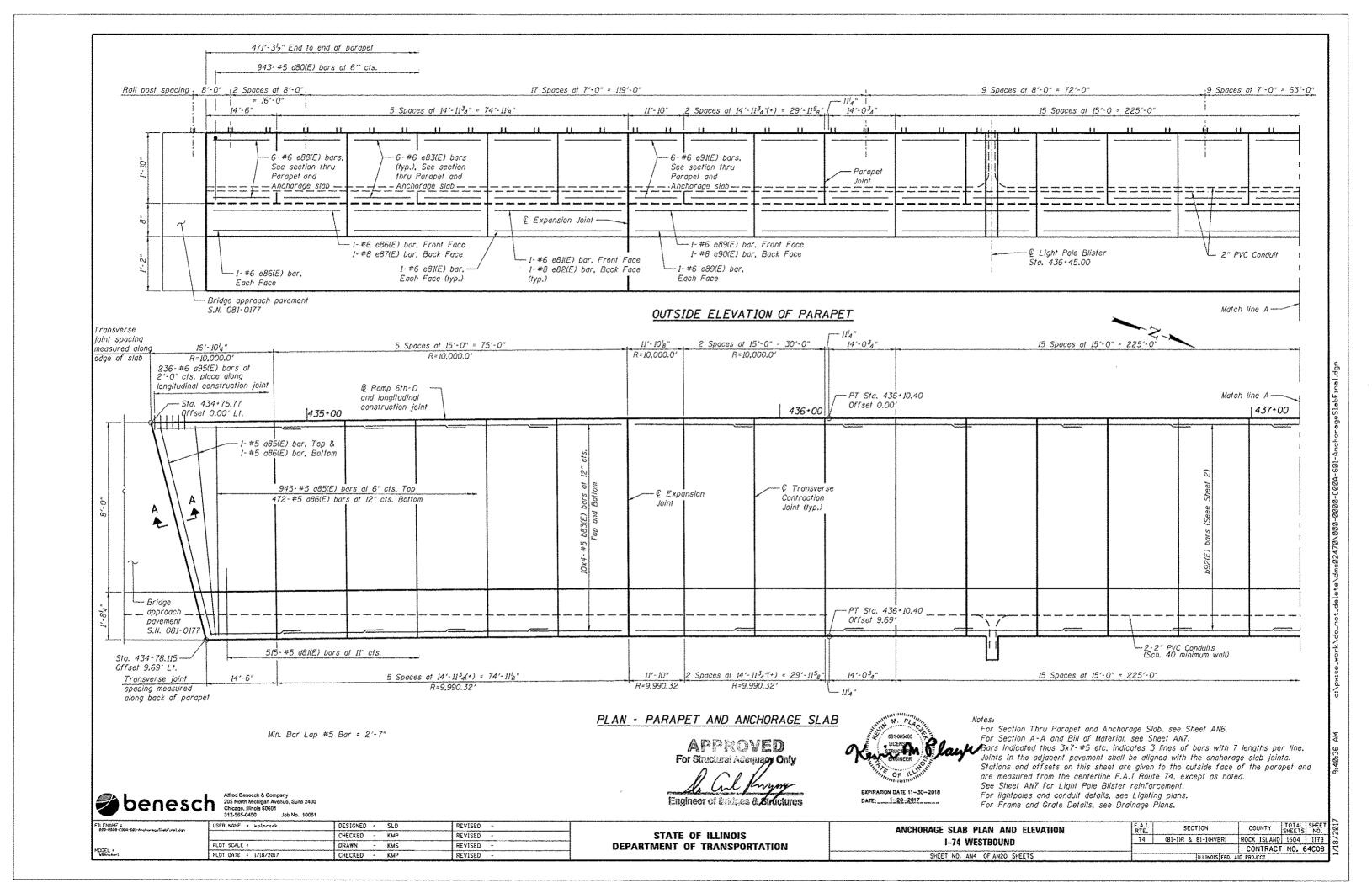


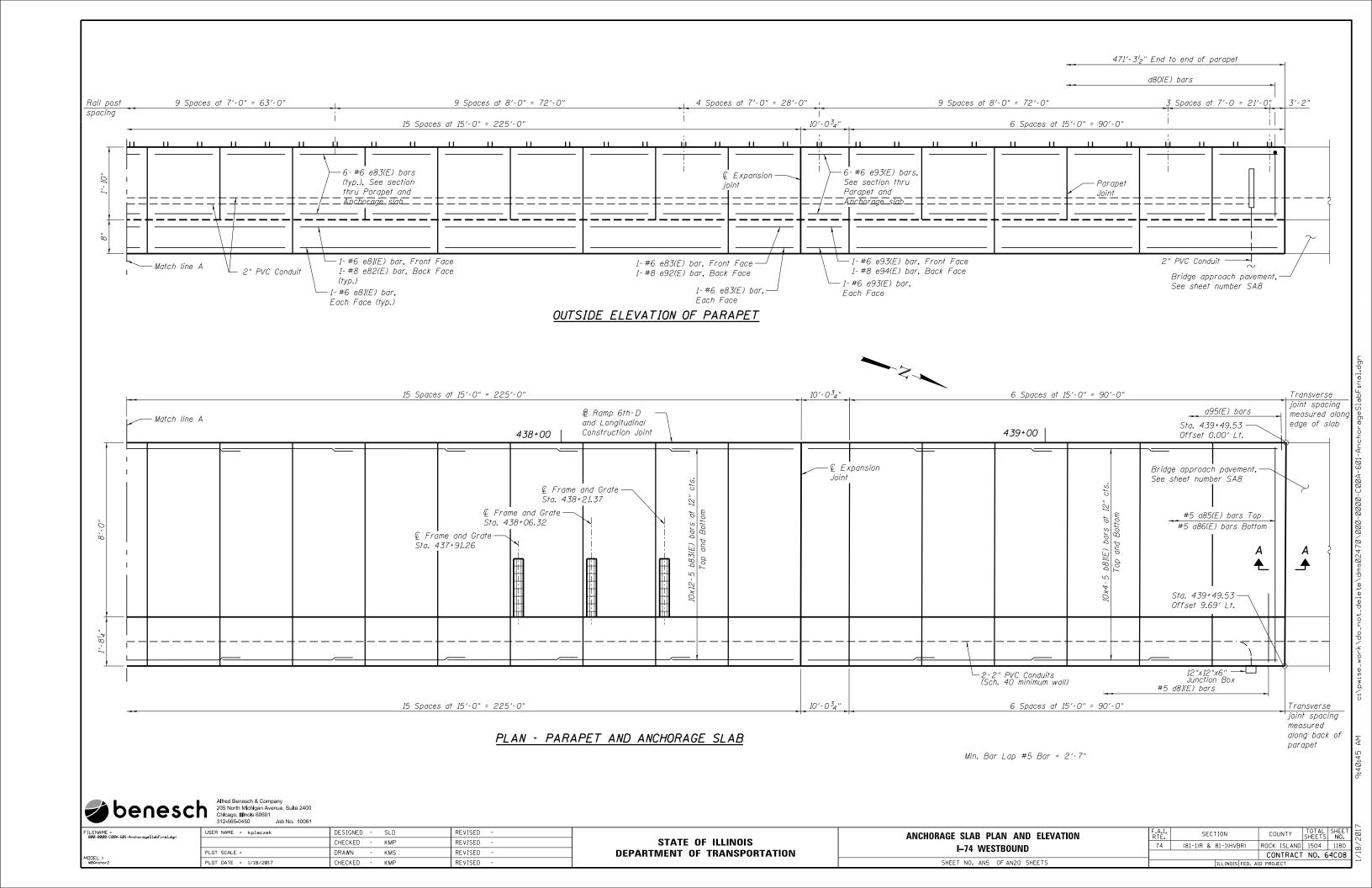


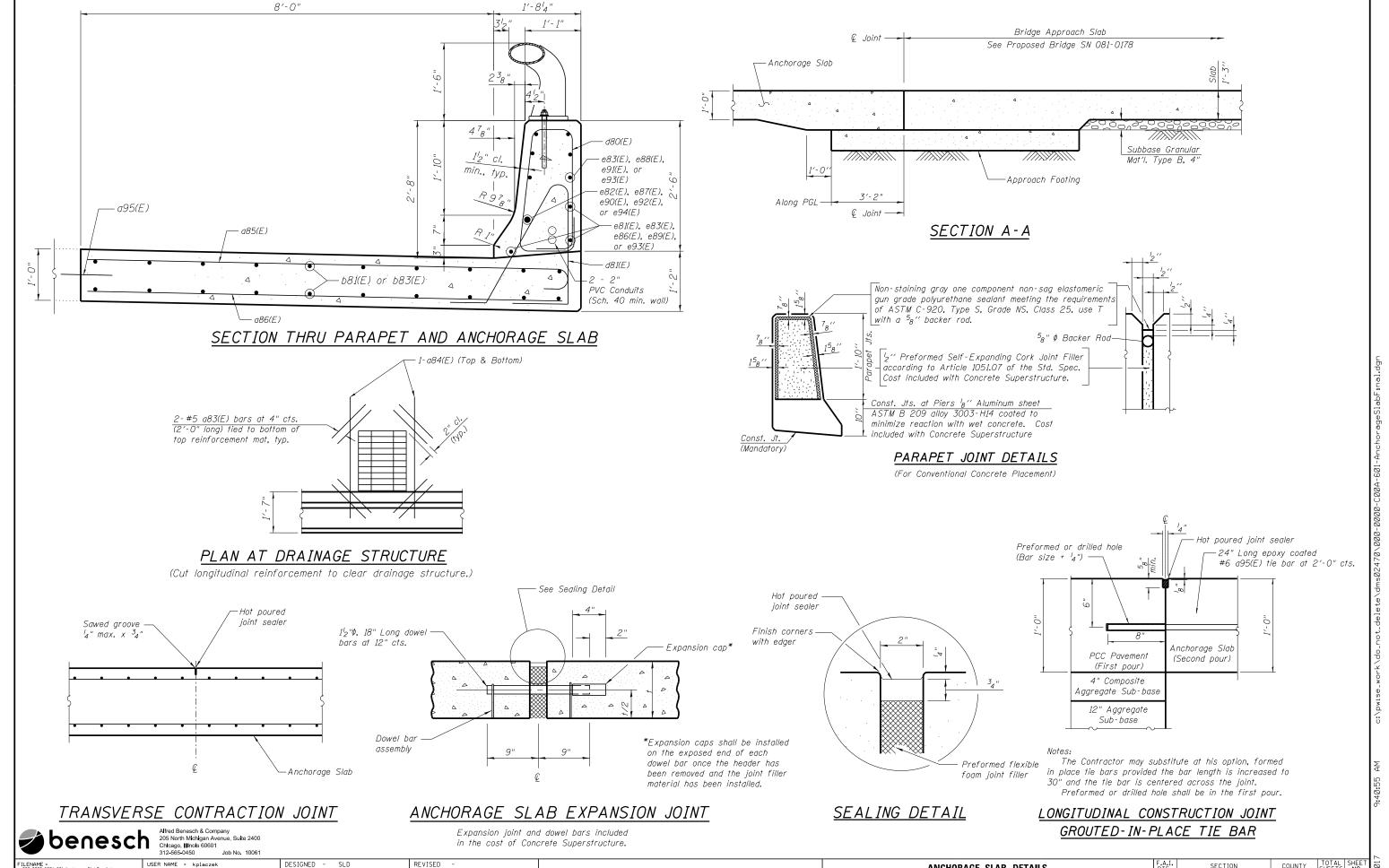
	000-0000-C00A-601-AnchorageSlabFinal.dgn	USER NAME = kplaczek	DESIGNED - SLD	REVISED -		
			CHECKED - KMP	REVISED -		
		PLOT SCALE =	DRAWN - KMS	REVISED -		
	MUDEL = EBAnchor_Det 2	PLOT DATE = 1/18/2017	CHECKED - KMP	REVISED -		

STATE OF ILLINOIS	
DEPARTMENT OF TRANSPORTATION	J

ANCHORAGE SLAB DETAILS		SECTION	COUNTY	TOTAL SHEETS	SHEE NO
I–74 EASTBOUND	74	(81-1)R & 81-1(HVBR)	ROCK ISLAND	1504	117
I-74 ENGIDOGIA			CONTRACT	NO. 6	4C0
SHEET NO ANG OF ANDO SHEETS		THE TWO IS FED. AT	ID DDO IECT		







STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

CHECKED -

CHECKED -

DRAWN

PLOT DATE = 1/18/2017

KMP

KMS

KMP

REVISED

REVISED

REVISED

ANCHORAGE SLAB DETAILS

I-74 WESTBOUND

SHEET NO. ANG OF ANZO SHEETS

(81-1)R & 81-1(HVBR) | ROCK ISLAND | 1504 | 1181 CONTRACT NO. 64C08

SECTION

74

COUNTY

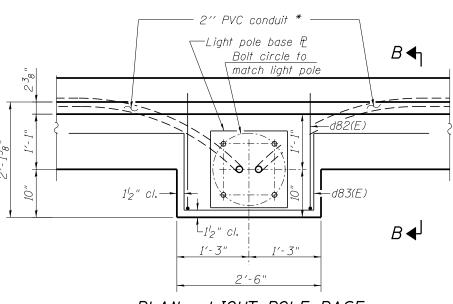
BILL OF MATERIAL **WESTBOUND**

Bar	No.	Size	Length	Shape
a83(E)	24	#5	2'-0"	
a84(E)	12	#5	5′-0"	
a85(E)	947	#5	9'-10"	
a86(E)	474	#5	9'-3"	
a95(E)	236	#6	2'-0"	
b81(E)	80	#5	27'-0"	
b83(E)	320	#5	26′-0"	
d80(E)	945	#5	7′-10"	U
d81(E)	515	#5	8'-3"	Ň
d82(E)	3	#6	4'-5"	
d83(E)	5	#6	8'-11"	7.
e81(E)	39	#6	29'-8"	
e82(E)	13	#8	29'-8"	
e83(E)	177	#6	14'-8"	
e86(E)	3	#6	29'-0"	
e87(E)	1	#8	29'-0"	
e88(E)	6	#6	14'-2"	
e89(E)	3	#6	26′-6"	
e90(E)	1	#8	26′-6"	
e91(E)	6	#6	11′-6"	
e92(E)	1	#8	14′-8"	
e93(E)	9	#6	9′-8"	
e94(E)	1	#8	9′-8"	
Concrete	Superstru	ucture	Cu. Yd.	229.3
Reinforcement Bars, Epoxy Coated			Pound	45,770
Protective	e Coat		Sq. Yd.	619.7

Bars indicated thus 1 x 5-#5 etc. indicates 1 line of bars with 5 lengths per line.

2'-0"

BAR d82(E)



PLAN - LIGHT POLE BASE

1'-338"

Light pole ₩

d80(E)

Ellipse Railing -

See Lighting Plans -

Stainless steel standard grade *

Anchor rods (Dia. as specified*

Maintain 1^{l_2} " cl. from —

wire cloth-Type 304, 4 x 4 mesh 0.047" wire diameter.

for light pole) Provide 3 flat

reinforcement

washers, 1 regular nut & 1

locknut for each rod.

2'-138"

Thread and cap end

with bushing.

pad

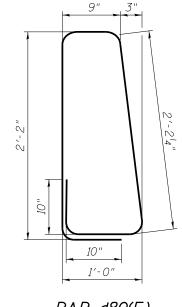
3-#6 d82(E) bars

of conduit. When ready for wiring, replace cap

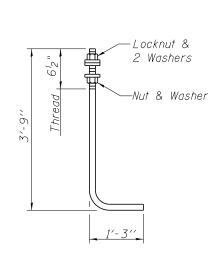
-Vibration isolation*

(Sch. 40 minimum wall) *

2" PVC conduit



BAR d80(E)

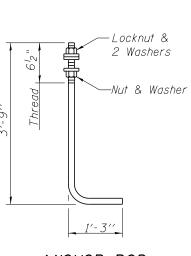


ANCHOR ROD

STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

Diameter as specified for light poles (ASTM F 1554 Grade 105). Full length hot dipped galvanized. Anchor bolts included in the cost of Concrete Superstructure.



7′-3"

2'-3"

BAR d83(E)

BAR a81(E)

SECTION B-B

* See lighting plans for details and pay items



d81(E)

USER NAME = kplaczek
PLOT SCALE =
PLOT DATE = 1/18/2017

USER NAME = kplaczek	DESIGNED - SLD	REVISED -
	CHECKED - KMP	REVISED -
PLOT SCALE =	DRAWN - KMS	REVISED -
PLOT DATE = 1/18/2017	CHECKED - KMP	REVISED -

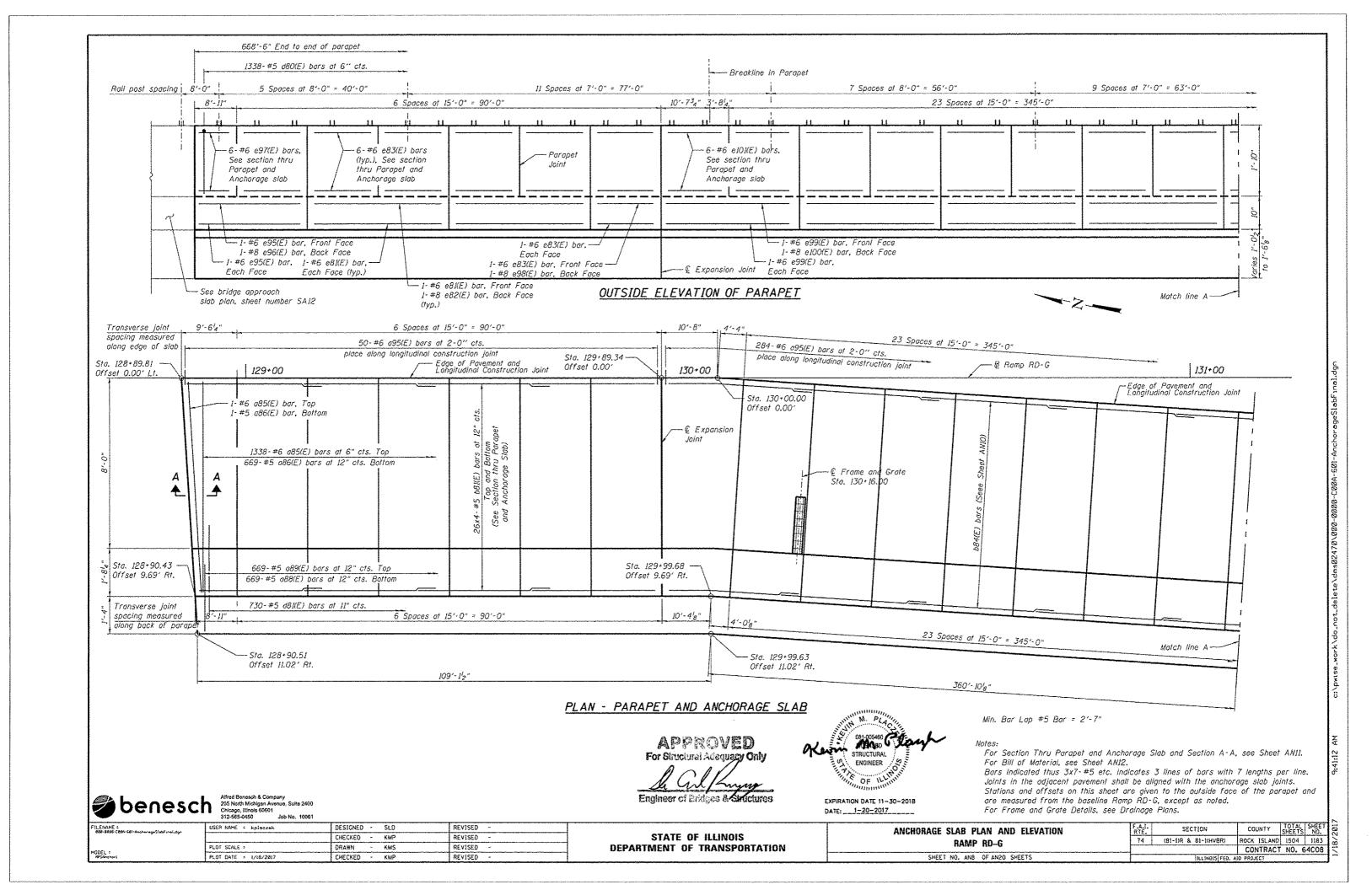
ANCHORAGE SLAB DETAILS I–74 WESTBOUND
,
SHEET NO. AN7 OF AN2O SHEETS

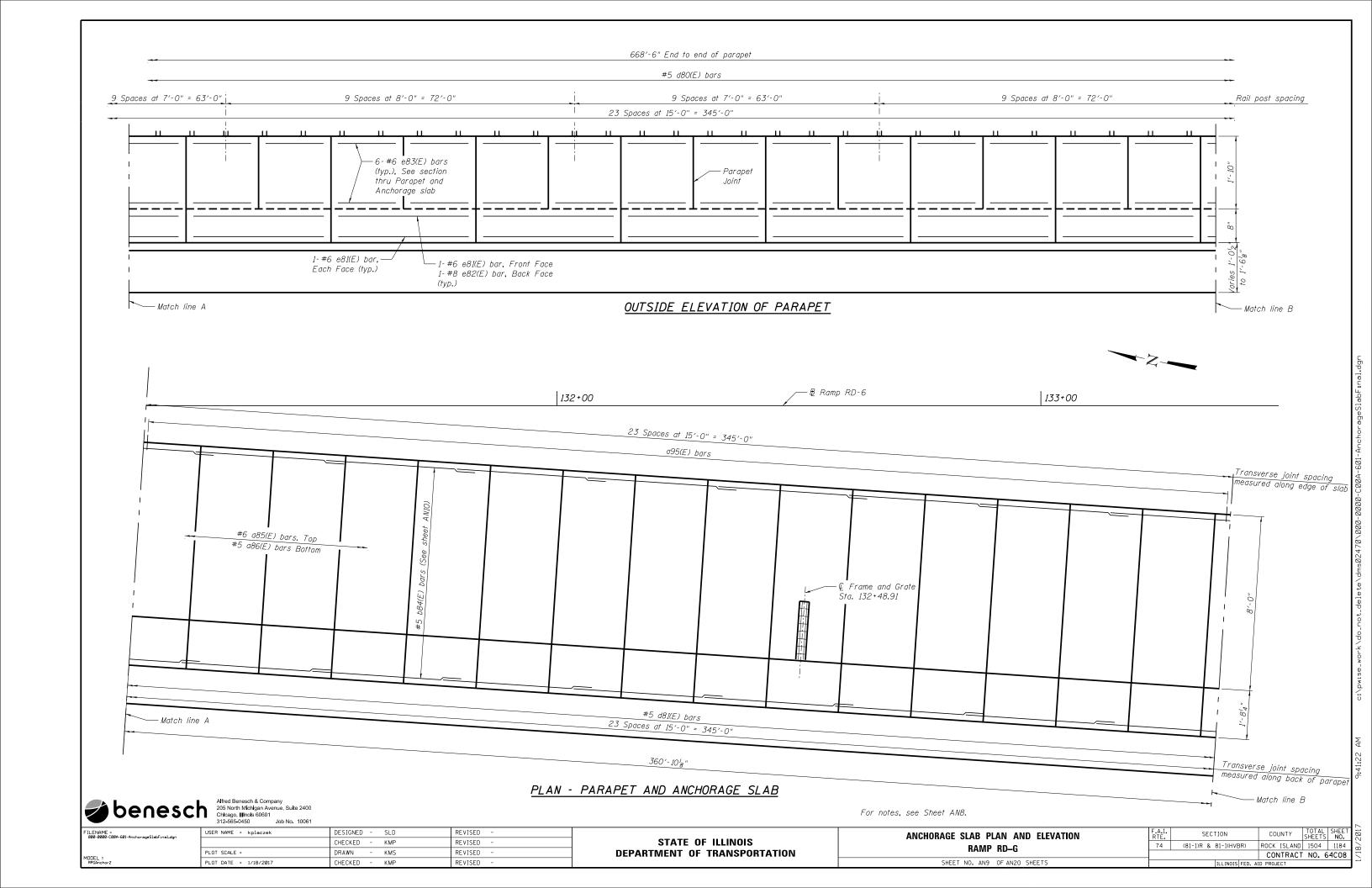
10"

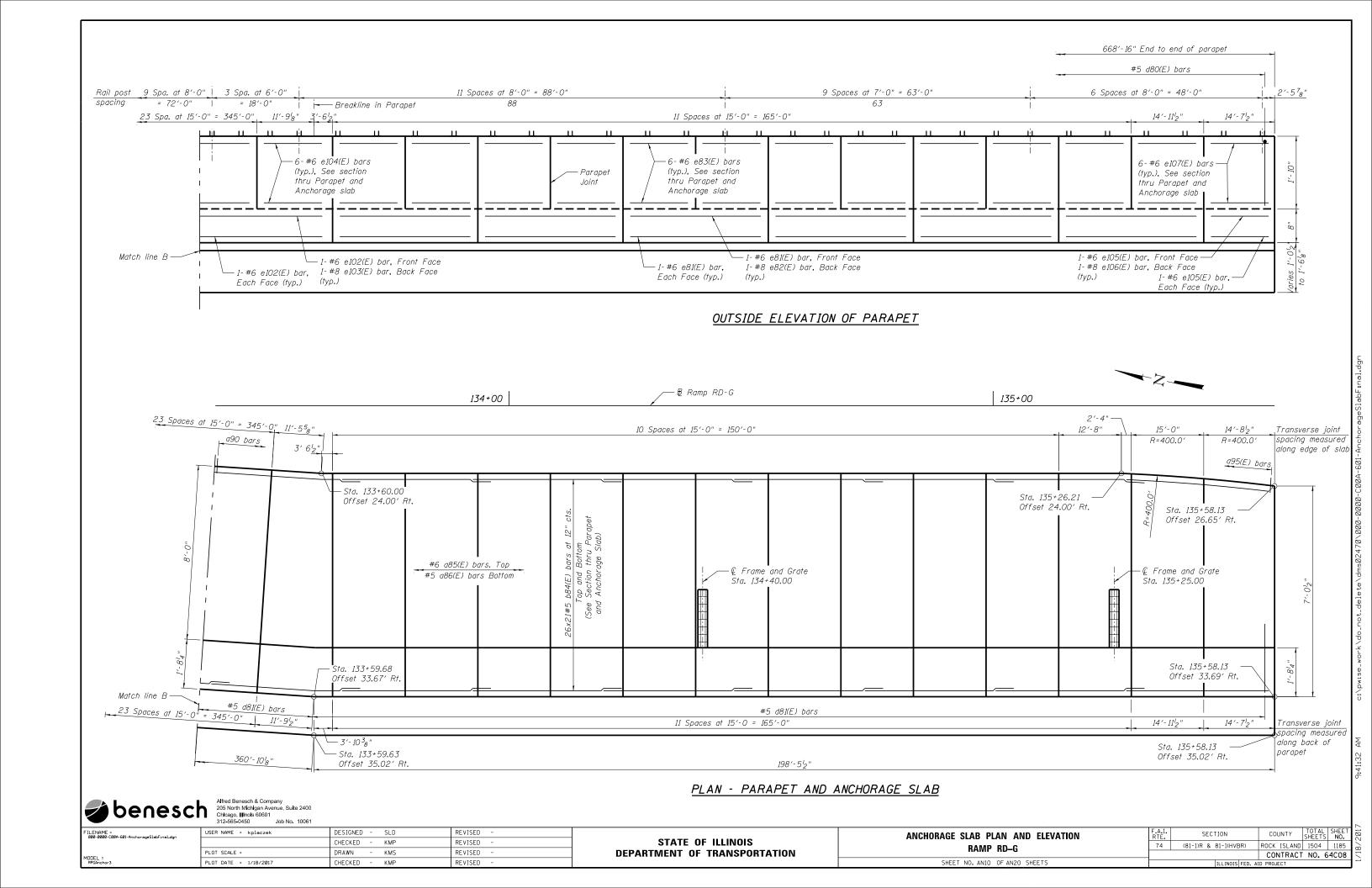
BAR d81(E)

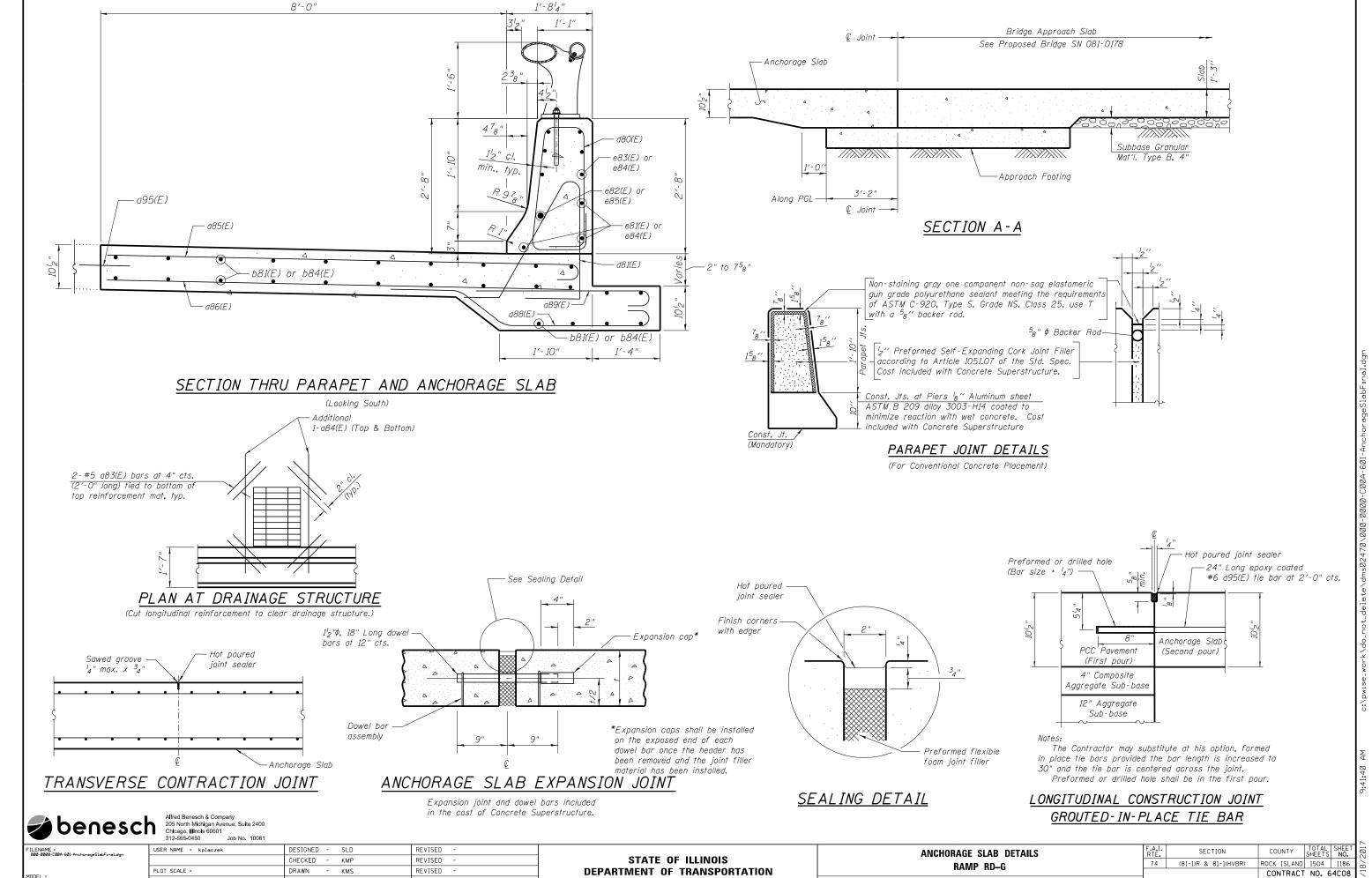
10"

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	100
74	(81-1)R & 81-1(HVBR)	ROCK ISLAND	1504	1182	0
		CONTRACT	NO. 6	4C08	-
TILL INDISCRED. AID PROJECT					









SHEET NO. AN11 OF AN20 SHEETS

PLOT DATE = 1/18/2017

CHECKED

KMP

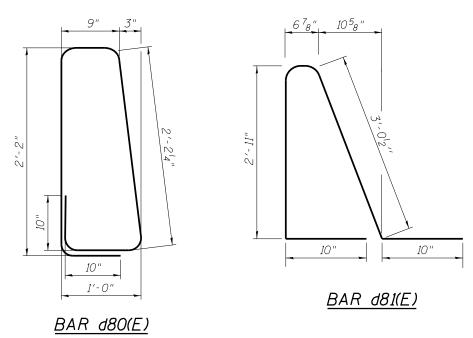
REVISED

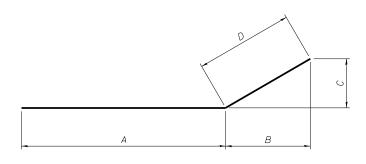
RAMP RD-G

BILL OF MATERIAL

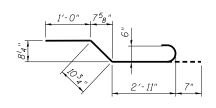
Bar	No.	Size	Length	Shape
a83(E)	32	#5	2'-0"	_
a84(E)	16	#5	5′-0"	
a85(E)	1339	#6	9'-11"	
a86(E)	670	#5	9'-3"	
a88(E)	669	#5	5′-5″	
a89(E)	669	#5	3′-9"	
a95(E)	334	#6	2'-0"	
b81(E)	104	#5	27'-0"	
b84(E)	546	#5	29'-0"	
d80(E)	1337	#5	7′- 10"	0
d81(E)	730	#5	8'-3"	Ĭ
e81(E)	54	#6	29'-8"	
e82(E)	18	#8	29'-8"	
e83(E)	249	#6	14'-8"	
e95(E)	3	#6	23'-7"	
e96(E)	1	#8	23'-7"	
e97(E)	6	#6	8"-7"	
e98(E)	1	#8	14'-8"	
e99(E)	3	#6	29'-0"	
e100(E)	1	#8	29'-0"	
e101(E)	6	#6	14'-0"	
e102(E)	3	#6	30'-0"	
e103(E)	1	#8	30'-0"	
e104(E)	6	#6	14 '- 11"	
e105(E)	3	#6	14'-3"	
e106(E)	1	#8	14'-3"	
e107(E)	6	#6	14'-3"	
Concrete	Superstru	ıcture	Cu. Yd.	334.
Reinforce Epoxy Co		Pound	81,130	
Protective			Sq. Yd.	871.4

Bars indicated thus 1 x 3-#5 etc. in 1 line of bars with 3 lengths per line.





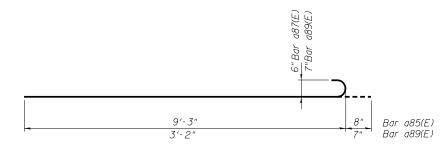
BARS e99(E) to e104(E)



<u>BAR a88(E)</u>

BARS e99(E) to e104(E)

Bar	Α	В	С	D	Length
e99(E)	18′-10"	10'-1 ³ 4"	8"	10′-2"	29'-0"
e100(E)	18′-10"	10'-1 ³ 4"	8"	10'-2"	29'-0"
e101(E)	10'-2"	3'-9 ³ 8"	3"	3′-10"	14'-0"
e102(E)	26′-4"	3'-7 ⁷ 8"	3'-7 ⁷ 8"	3′-8"	30′-0"
e103(E)	26′-4"	3'-7 ⁷ 8"	278"	3′-8"	30'-0"
e104(E)	11'-3"	3'-7 ⁷ 8"	2 ⁷ 8"	3′-8"	14'-11"



BARS a85(E) and a89(E)

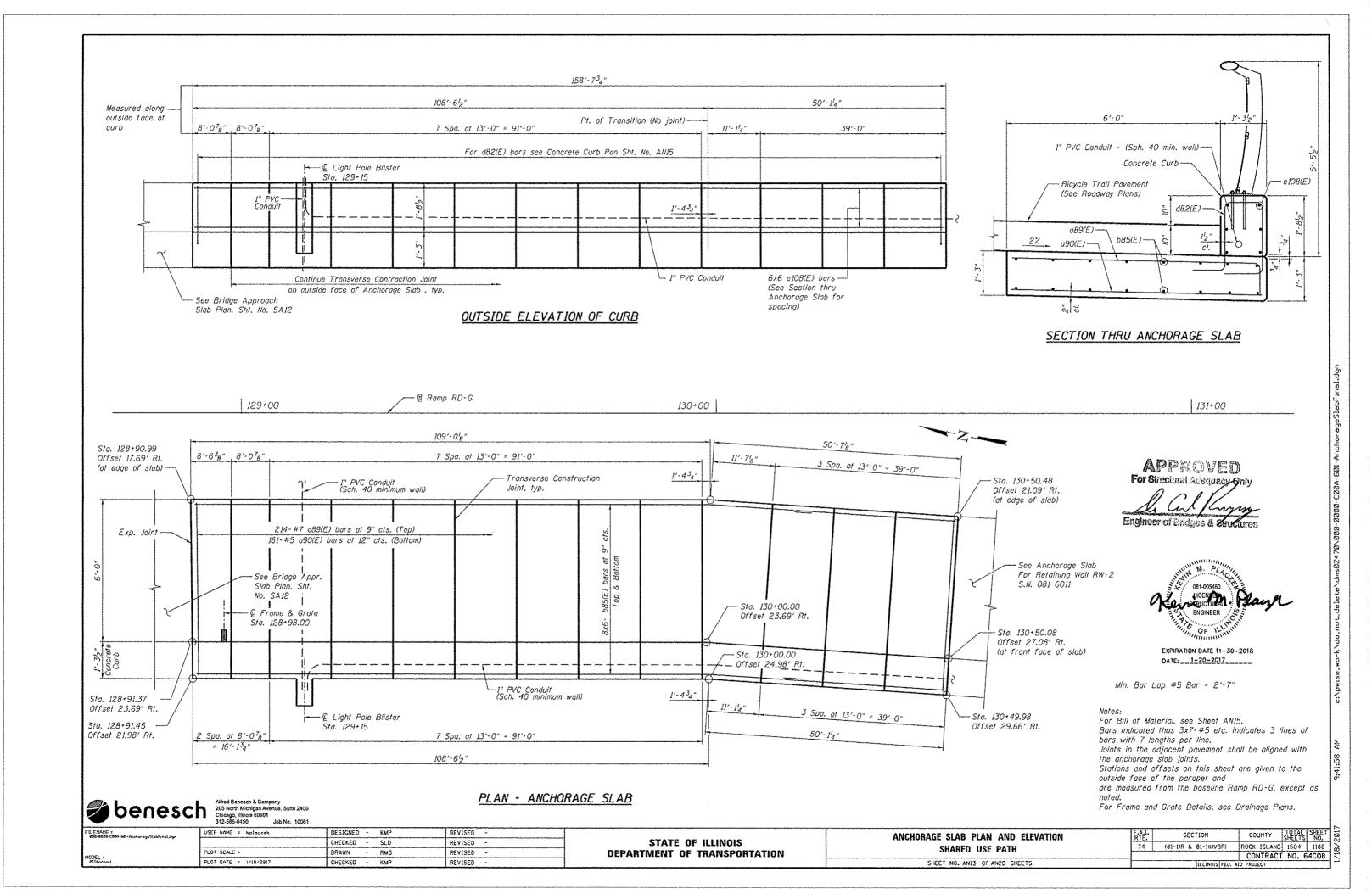


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DEL -	

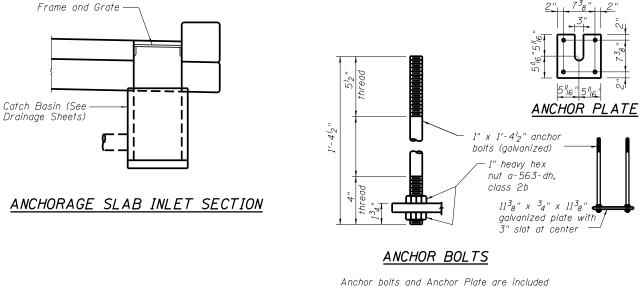
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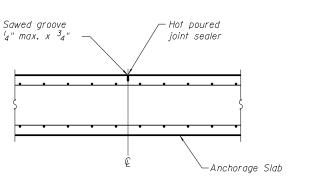
STATE O	F ILLINOIS
DEPARTMENT OF	TRANSPORTATION

ANCHORAGE SLAB DETAILS	F.A.I. RTE.	SECTION
RAMP RD-G		(81-1)R & 81-1(H
IIAWI IID-G		
CHEET NO AND OF ANDO CHEETC		

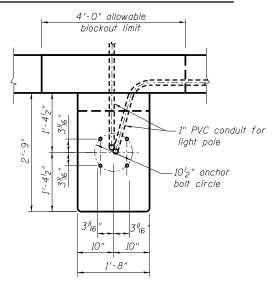




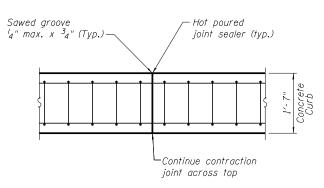




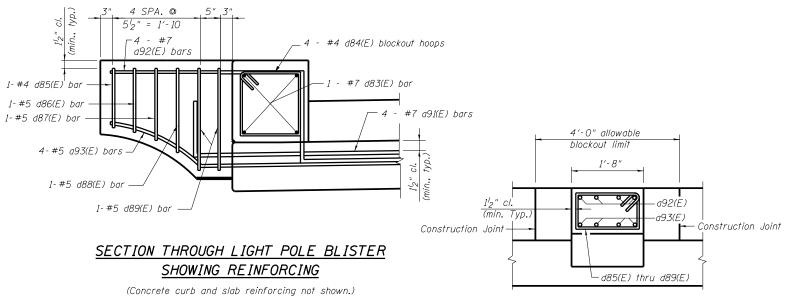
TRANSVERSE CONTRACTION JOINT

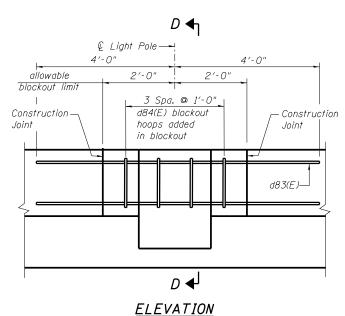


PLAN OF LIGHT POLE BLISTER

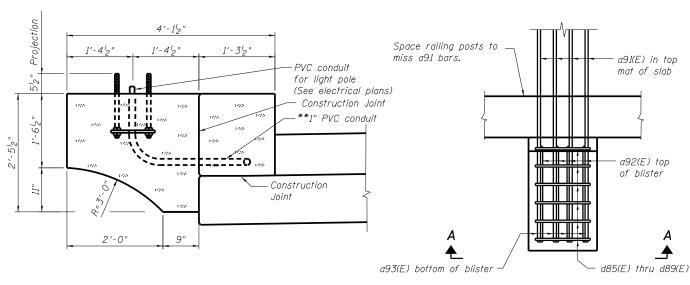


CURB CONTRACTION JOINT





(Anchor bolts not shown.)



<u>SECTION D-D</u> (Cost of anchor bolts and anchor plate is included with Concrete Superstructure.)

<u>PLAN OF LIGHT POLE</u> BLISTER REINFORCING

VIEW A-A

(Concrete curb and slab reinforcing not shown.)

Not

in the cost of Concrete Superstructure.

For anchorage slab reinforcing, see Sheet ANI3. For concrete curb reinforcing and Bill of Material, see Sheet ANI5.

See Electrical Plans for lighting and conduit details.

Space railing posts to miss a91 bars.

benesch

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chlcago, Illinols 60601 312-565-0450 Job No. 10061

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L = Anchor_Det2	PLOT DATE = 1/18/2017	CHECKED	-	KMP	REVISED	-

STATE OF ILLINOIS	
DEPARTMENT OF TRANSPORTATION	

ANCHORAGE SLAB DETAILS	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
SHARED USE PATH	74	(81-1)R & 81-1(HVBR)	ROCK ISLAND	1504	1189
SHANED USE FAIR			CONTRACT	NO. 6	4C08
SHEET NO. AN14 OF AN2O SHEETS		ILLINOIS FED. A	ID PROJECT		

11,860

68.7

159

Pound

Cu. Yd.

Sq. Yd.

Α





For base plate details, see Pedestrian Railing Detail Plans.

a89(E) 214 a90(E) 313 #5 6'-11" #7 7'-5" a91(E) 4 #7 9'-9" a92(E) 4 a93(E) #5 6'-4" #5 a94(E) b85(Е) 96 #5 28'-10" d82(E) 222 d83(E) 4 力 #7 8'-0" d84(E) 4 #5 #4 6'-1" #5 6'-3" d85(E) d86(E) 1 d87(E) 1 #5 6'-7" #5 7'-1" #5 7'-11" d88(E) d89(E) 2 e108(E) 36 #5 28′-10"

BILL OF MATERIAL

a84(E) 4

Reinforcement Bars,

Epoxy Coated

Superstructure

Protective Coat

Concrete

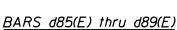
Size Length

#5 5'-0"

#7 7'-10"

d85(E) d86(E) d87(E) 1'-6" d88(E) 2'-2' d89(E) 1'-4"

Bar



6'-3"

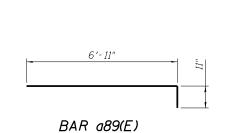
Notes:

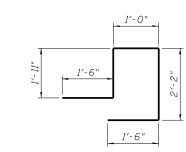
BAR a91(E)

-1- #5 a84(E) bars top and bottom 2-#5 a94(E) bars at 4" cts. (2'-0" long) tied to bottom of top reinforcement mat, typ.

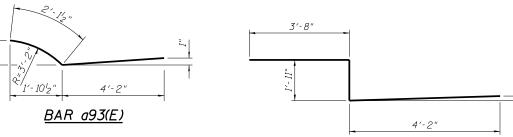
PLAN AT DRAINAGE STRUCTURE

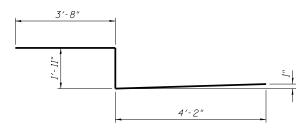
(Cut longitudinal reinforcement to clear drainage structure.) 11,860



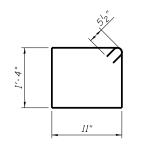


BAR d82(E)

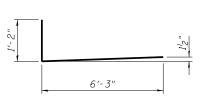




BAR a92(E)



BAR d84(E)



Affred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Michols 60601

Concrete Curb

Pedestrian Railing

Base Plate (typ.)

312-565-0450 Job No. 10061		
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PLOT DATE = 1/18/2017	CHECKED - KMP	REVISED -

1-#5 a90(E) bar, at each railing post Top and Bottom (typ.)

4'-4" max.

(See Pedestrian Railing Details)

e108(E)

CONCRETE CURB PLAN

(Slab reinforcing not shown)

Details and quantities shown assume Pedestrian Railing layout consists of 37 railing panels and 38 railing post throughout the limits of the

6 - #5 d82(E) Per panel

(typ.)

4 Eq. spa (1'-0" max.)

Outside Face

of Concrete Curb-

Post (typ.)

Pedestrian Railing

-Curb

Joint

Contraction

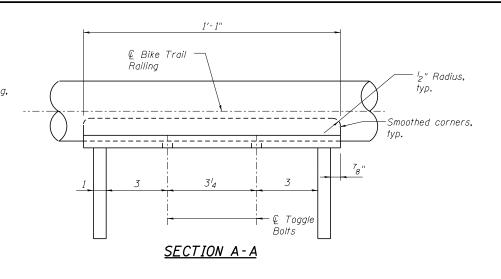
(min.)

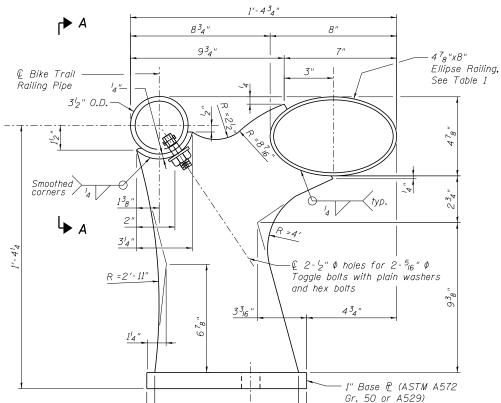
STATE OF ILLINOIS
SIAIL OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ANCHORAGE SLAB DETAILS	F.A.I. RTE.	
SHARED USE PATH	74	(81-
OHAHED OUE FAIH		
CUEET NO ANIE OF ANIOG CHEETC		

.A.I. RTE.	SECTION		CO	UNTY	TOTAL SHEETS	SHEET NO.	
74	(81-1)R & 81-1(HVBR)	1	ROCK	ISLAND	1504	1190	
CONTRACT NO. 64C08							
ILLINOIS FED. AID PROJECT							

MODEL = PEDAnchor_Det

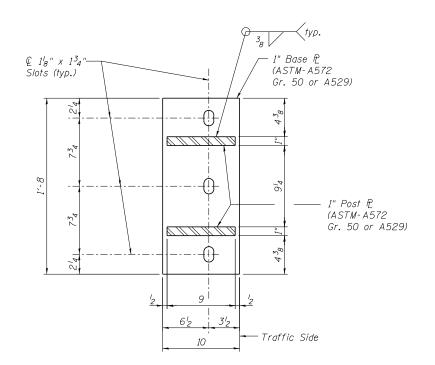




ELEVATION-RAISED TRAFFIC AND BICYCLE RAILING POST

9"

10"



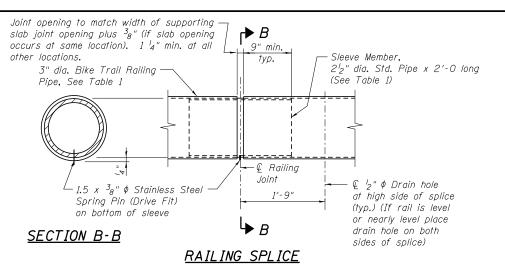
BASE PLATE DETAIL

TABLE 1						
APPRO!	/ED RAILING MATERIA	4 <i>L</i>				
4 ⁷ 8" x 8"	Sleeve Mem	ber				
Ellipse Railing	(at Railing S	plice)				
Material	Material	Thickness				
6" dia. Std. Pipe	ASTM-A53-B	0.353"				
ASTM-A53 E or S	A36 or A500 Gr. B	0.339"				
Grade B	API-5LX52 0.224"					
6" dia. , 0.280"	ASTM-A53-B	0.353"				
wall thickness	A36 or A500 Gr. B 0.339"					
ASTM-A501	API-5LX52 0.224"					
6 ⁵ 8" O.D. x O.188"	ASTM-A53-B	0.339"				
Tube	A36 or A500 Gr. B	0.325"				
API-5LX52	API-5LX52	0.216"				
3½" dia. Bike	Sleeve Mem	ber				
Trail Railing Pipe	(at Railing Splice)					
Material	Material					
3" dia. Std. Pipe	2 ¹ 2" dia. Std. Pipe					
ASTM-A53 E or S	ASTM-A53 E or S					
Grade B	Grade B	}				

BILL OF MATERIAL

ITEM		TOTAL
Structural Steel Railing, Traffic and Bicycle	Foot	703

Quantity includes railing on Ramp RD-G Bridge Approach Slab (Sht. SA3) and Ramp RD-G Anchorage Slab.



The major and minor diameters of the rail member may vary +/- 0.1875 inches from plan dimensions. However, the difference between the outside diameters of the sleeve and the inside diameters of the rail shall not exceed 0.125 inches along the major or minor axis. The maximum gap along the 45° axis of the sleeve may be I_4 " max.

STRUCTURAL STEEL RAILING, TRAFFIC & BICYCLE NOTES:

Structural steel barrier joints are to be located as shown.

The structural steel railing is to be bid on a per linear foot basis measured from end to end of steel railing.

The number of linear foot of structural steel railing installed will be paid for at the contract unit price per foot based on plan quantities.

Price bid for "Structural Steel Railing, Traffic & Bicycle" shall be full compensation for furnishing all material, and all the equipment and labor required to erect the railing in accordance with these plans and current specifications.

All railings, posts, sleeves, base plates, and shims shall be galvanized after fabrication in accordance with the requirements of ASTM A123.

Optional cast-in-place anchor bolts to comply with astm F1554 Grade 105, hex nuts to comply with ASTM A563 Grade DH. Washers to comply with ASTM F436. Galvanizing in accordance with ASTM F2329.

Anchor bolts shall be $^{7}_{8}$ " dia., A193 Gr. B7, be fully threaded with heavy hex nuts and one hardened washer and one 2 $^{l}_{4}$ " O.D. washer each. Embed threaded rods 10 $^{l}_{2}$ " min. into concrete parapet. Anchor bolts, nuts, and washers shall be galvanized in accordance with ASTM F2329. Adhesive bonding material system shall be in accordance with materials I.M. 491.11. Installed anchors shall be capable of obtaining an ultimate load per threaded rod of 36 kips in tension for the spacing and edge distance shown in the plans. Install and field test anchors in accordance with the developmental specification, "Installing Adhesive-Bonded Anchors and Dowels for Traffic Railings".

Toggle bolts shall be a type of stud and/or screw style capable of supporting a 1,000 pound load in tension when tested through a l_2 " round hole. Toggle bolts shall be galvanized in accordance with ASTM F2329. Toggle bolts may be cadmium-plated to meet the requirements of ASTM B766 "Electrodeposited Coatings of Cadmium" in place of galvanizing.

The testing of the toggle bolts shall follow the guidelines set forth in ASTM A370 with the following modifications. The bottom fixture used to grip the bolt shall allow the end pieces to spread prior to gripping it. The fixture at the top should allow centering of the bolt allowing the bolt to pulled axially. The speed of the test should be approximately ${}^{l}_{4}{}^{m}$ per minute. The results of the test shall be submitted as a certified test report to the central materials office in ames along with a certified mill test report. In addition, three sample toggle bolts shall also be submitted to the central materials office in ames for verification testing. The samples shall have the same heat number as the toggle bolts supplied.

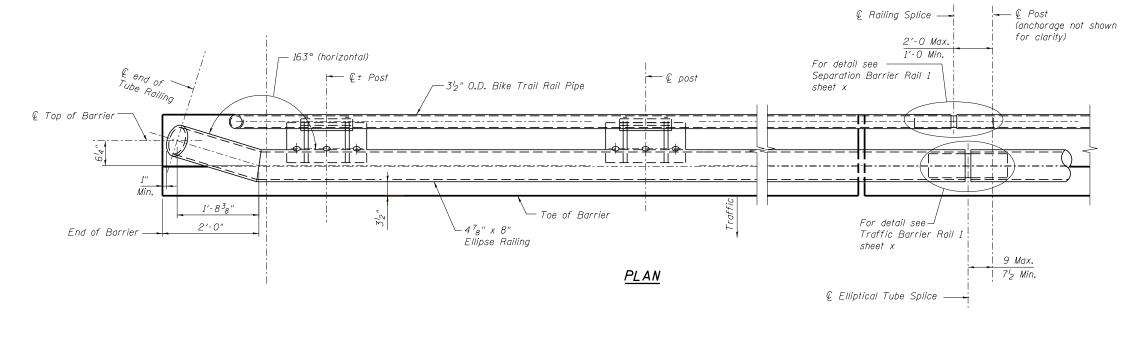
See Sheet AN18 for Ellipse Railing Splice, Railing Shop Splice and cast in place anchor bolt options details.

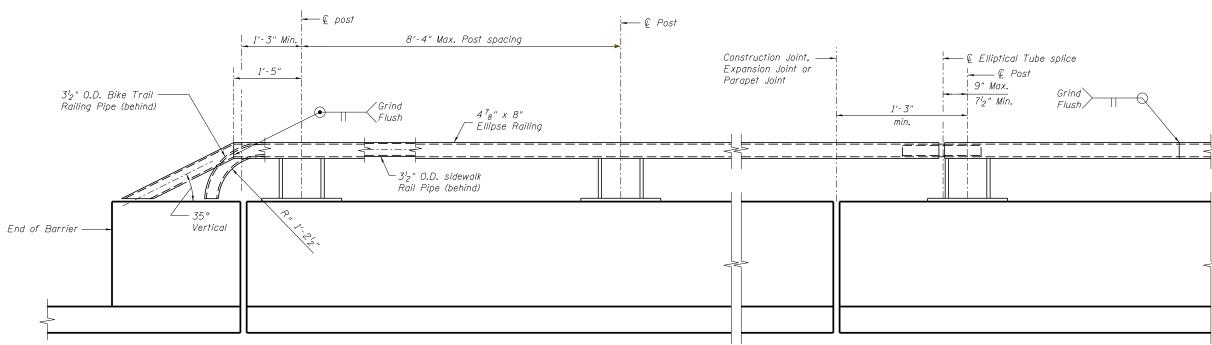
benesch 205 North Michigan Avenue, Suite 2400 Chlcago, Illinois 60601

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		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	. 100
TRAFFIC AND BICYCLE RAILING	74	(81-1)R & 81-1(HVBR)	ROCK ISLAND	1504	1191	à
			CONTRACT	NO. 6	4C08	7
SHEET NO. ANI6 OF AN20 SHEETS		ILLINOIS FED.	. AID PROJECT			-







<u>ELEVATION</u>

NOTE:

Edge of base plate shall not be less than 6" from any cold joint or barrier discontinuity including expansion joint.

|--|

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chlcago, IIIInols 60601 312-565-0450 Job No. 10061

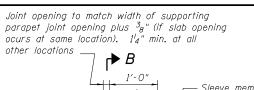
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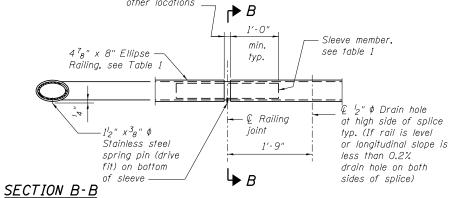
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PLOT DATE = 1/18/2017	CHECKED - KMP	REVISED -

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DEPARTMENT OF TRANSPORTATION	J

TRAFFIC AND BICYCLE RAILING	74	(8
SHEET NO. AN17 OF AN20 SHEETS		

TE. SECTION		COUNTY		SHEETS	NO			
74	(81-1)R &	81-1(HVE	R) ROCK ISLAND		1504	119		
			COI	NTRACT	NO. 6	54C0		
ILLINOIS FED. AID PROJECT								



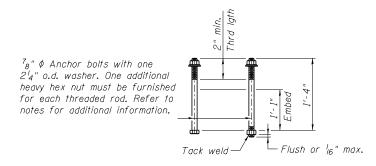


ELLIPSE RAILING SLEEVE DETAIL

The major and minor diameters of the rail member may vary $+/-\frac{3}{16}$ " from plan dimensions. not exceed $^{l}_{8}$ " along the major or minor axis.



* Shop splice is permitted with minimum 85 percent penetration. The weld may be square groove, double vee groove, or single groove. Grind smooth.



CAST-IN-PLACE ANCHOR BOLT OPTIONS

However, the difference between the outside diameters of the sleeve and the inside diameters of the rail shall The maximum gap along the 45° axis of the sleeve may be $\frac{1}{4}$ " max.

TABLE 1					
<i>APPROVE</i>	D RAILING MATERIAL				
4 ⁷ 8"x 8"	Sleeve Membe	er			
Ellipse Railing	(at railing sp	lice)			
Material	Material	Thickness			
6" Dia. Std. Pipe	ASTM-A53-B	0.353"			
ASTM-A53 E OR S	A36 or A500 GR. B	0.339"			
GRADE B	API-5LX52	0.224"			
6" dia. , 0.280"	ASTM-A53-B	0.353"			
Wall thickness	A36 or A500 GR. B	0.339"			
ASTM-A501	API-5LX52	0.224"			
6 ⁵ 8" O.D. x O.188"	ASTM-A53-B	0.339"			
Tube	A36 or A500 GR. B	0.325"			
API-5LX52	API-5LX52	0.216"			

BILL OF MATERIAL

NOTES:

509.05 of the Standard Specifications.

measured from end to end of steel railing.

otherwise noted.

Specifications.

Steel Railing (Special).

AASHTO M-232.

1. Steel Railing (Special) shall be fabricated and installed in

2. All steel rail elements shall be galvanized according to Article

3. The Steel Railing (Special) is to be bid on a per linear foot basis

4. Payment for Steel Railing (Special) shall include full compensation

5. Anchor bolts shall be $^{7}8''$ ϕ , ASTM A-193 GR. B7, fully threaded

with heavy hex nuts and one hardened washer and one 2^{l}_{4} " O.D.

Material for these items shall be in accordance with the adhesive

load per threaded rod of 36 kips in tension, considering spacing

6. Optional cast-in-place anchor bolts to comply with ASTM F-1554

comply with AASHTO M-293. Galvanizing in accordance with

7. Provide one $^{l}_{8}$ " and two $^{l}_{16}$ " galvanized steel shims for 25% of rail

posts, to be used as required. Shims shall be similar to base

plates in size and holes. Cost included with Steel Railing (Special).

Grade 105. Hex nuts to comply with AASHTO M291, washers to

manufacturer's requirements to be capable of obtaining an ultimate

and edge distance. See Standard Specification 509.06 for further

details on setting anchor bolts. Cost of anchor bolts included with

washer each. Embed threaded rods 10^{l_2} " min. into concrete parapet.

for furnishing all material, and all the equipment and labor required

to erect the rail in accordance with these plans and the Standard

accordance with Article 509 of the Standard Specifications, unless

ITEM	UNIT	TOTAL
Steel Railing (Special)	Foot	651

Quantity includes railing on I-74 WB and EB Bridge Approach Slabs (Shts. SA9 and SA6) and I-74 WB and EB Anchorage Slabs (Shts. AN4 an AN1),

Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chlcago, Illinois 60601

1" Post P

GR 50 OR

A529)

¹_R" Fabric

Reinforced

Elastomeric

Pad

(ASTM-A572

6"

516"

734"

83₄"

SIDE VIEW

7"

434"

Rail member shaped to

Traffic

side

'" Rase P

(ASTM-A572

GRr 50 or A529)

478" x 8" ellipse Railing,

from round pipe. See Table 1

Traffic

side

1" Post P

typ.

ELEVATION

1'-8"

SECTION THRU POST

ELLIPTICAL TUBE WITH RAIL POST AND ANCHORAGE DETAILS

(ASTM-A572

GR 50 or A529)

typ.

^I₈" Fabric Reinforced

Elastomeric Pad

-€ 1½" x 1¾"

−1" Post P

(ASTM-A572 GR 50 or A529) -1" Base ₱ (ASTM-A572 GR 50 OR A529)

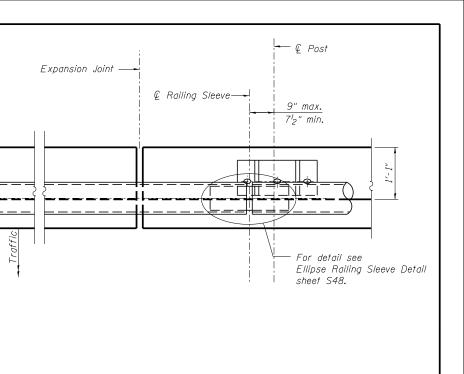
Slots typ.

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	CHECKED - KMP	REVISED -
PLOT SCALE =	DRAWN - DTS	REVISED -
PLOT DATE = 1/18/2017	CHECKED - KMP	REVISED -

Job No. 1006

TRAFFIC RAILING STRUCTURE NO. 081–0177 (WESTBOUND)	
SHEET NO. AN18 OF AN2O SHEETS	

F.A.I. RTE.				COUNTY		SHEET NO.
74	(81-1)R & 81-1(HVBR)		ROCK	ISLAND	1504	1193
			CON	NTRACT	NO. 6	4C08
	ILLINOIS FED.	A]	D PROJ	ECT		



<u>PL AN</u>

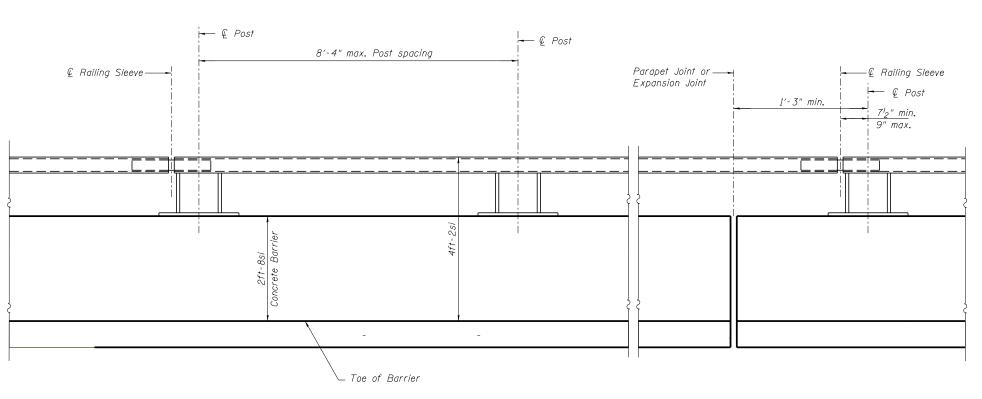
4⁷8" x 8" Ellipse Railing

└─ Toe of Barrier

4⁷8" x 8" Ellipse Railing © Post

32"

© Top of Parapet —



├─ @ Post

<u>ELEVATION</u>

NOTE:

Edge of base plate shall not be less than 6" from any cold joint or barrier discontinuity.

Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chcago, Illnols 60601
Job No. 10061
Job No. 10061

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PLOT DATE = 1/18/2017	CHECKED - KMP	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TRAFFIC RAILING Structure no.081–0177 (Westbound)							
SHE.	ET NO	. AN19	OF	AN20	SHEETS		

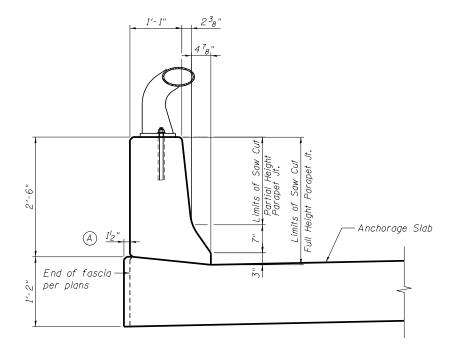
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74	(81-1)R & 81-1(HVBR)	ROCK ISLAND	1504	1
		CONTRACT	NO. 6	40
	ILLINOIS FED. A	ID PROJECT		

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COUNTY

ROCK ISLAND 1504 1195

CONTRACT NO. 64C08



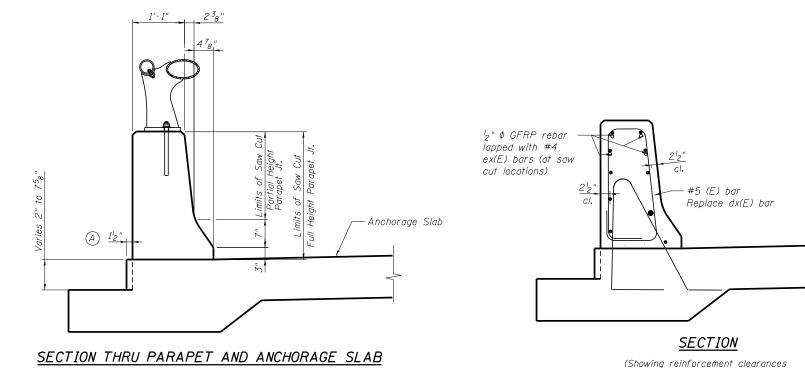
לי" φ GFRP rebar lapped with #4 ex(E) bars (at saw cut locations) - #5 (E) bar Replace dx(E) bar

SECTION THRU PARAPET AND ANCHORAGE SLAB

SECTION

(Showing reinforcement clearances for slip forming)

I-74 WB & EB ANCHORAGE SLABS

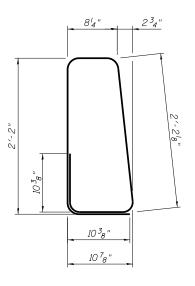


Ramp RD-G ANCHORAGE SLAB

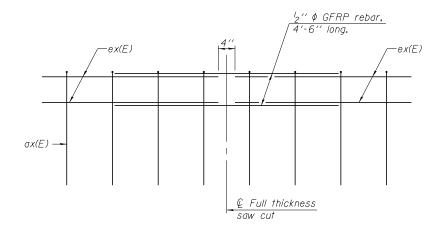
GENERAL NOTES

All dimensions shall remain the same as shown on anchorage slab details, except dimension A. Additional concrete needed to revise dimension A equals 0.005 cu. yds./ft. for I-74 WB and EB anchorage slabs and 0.02 cu. yds./ft. for Ramp RD-G anchorage slab.

Full thickness saw cut at all joint locations in lieu of cork joint filler.



#5 (E) BAR



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)

benesch Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chlcago, Illinols 60601 312-565-0450 Job No. 10061

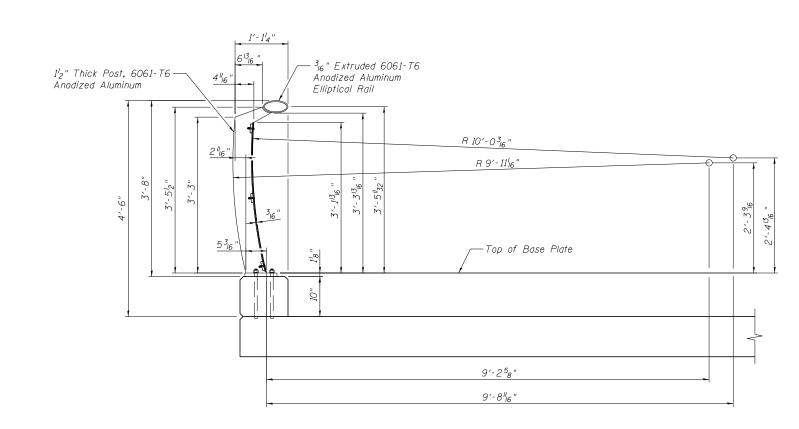
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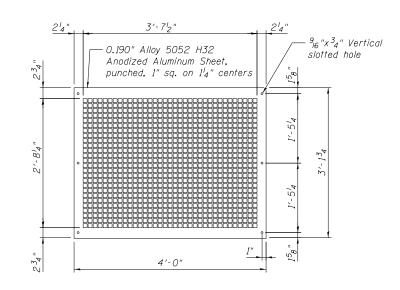
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PLOT DATE = 1/18/2017	CHECKED - SLD	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

for slip forming)

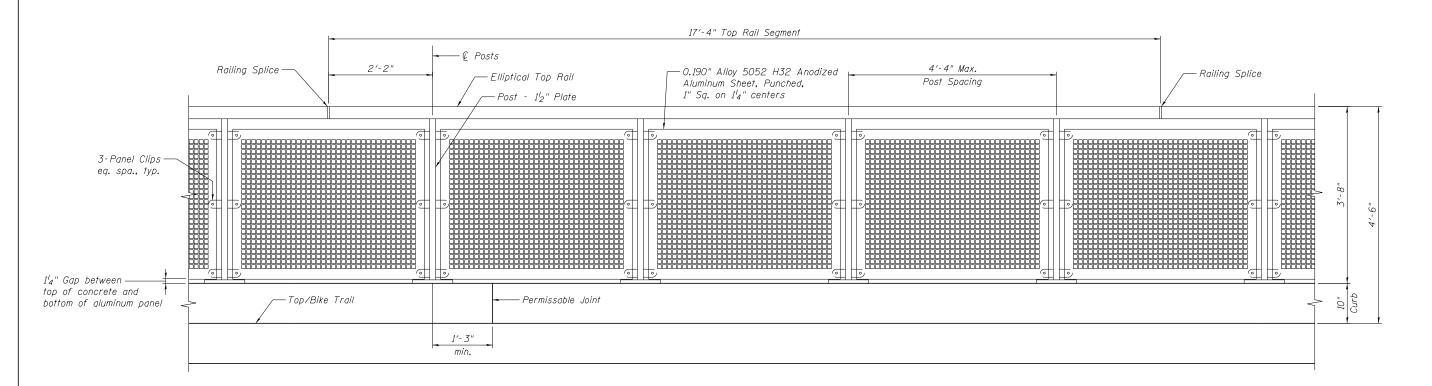
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ANCHORAGE SLAB CONCRETE SLIPFORMING OPTION		(81-1)R & 81-1(HVBR)
SHEET NO. ANZO OF ANZO SHEETS		TI I INDIS EED





PANEL LAYOUT (FLATTENED)

TYPICAL SECTION THRU RAILING



RAILING TYPICAL ELEVATION

Note: See Sheet 2 for notes.

SHIVE HATTERY

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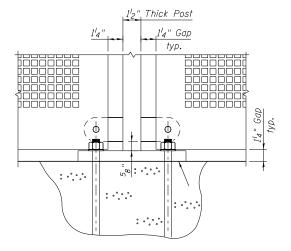
QUANTITY

570

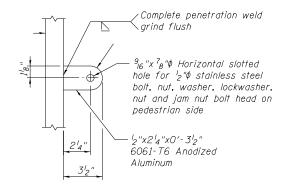
Foot

— € Baseplate 414" 414" 0.190" Punched Aluminum Sheet -4⁻¹⁵/₁₆" φ Holes for 4" $^{5}8$ " ϕ Anchor bolts 1'-0" 5,6 1/4" Gap between top of concrete and bottom of aluminum panel " Neonrene 278" Sheet - Post -€ Base Plate 4" 1"x8½"x0′-9" Base & 6061-T6 1'-0" Curb Anodized Aluminum 4"

PLAN - TYPICAL BASE PLATE DETAIL



TYPICAL DETAIL AT BOTTOM OF PANEL



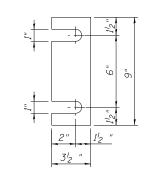
RAILING PANEL CLIP CONNECTION

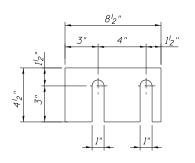
SECTION THRU BASE OF RAILING

⁵8" φ

Anchor bolts

(4 Reg'd ea.)





SHIM DETAILS

PEDESTRIAN RAILING NOTES

- 1. The Pedestrian Rail is to be bid on a lineal foot basis for each type, measured end to end of rail. The price bid for pedestrian rail of each type shall be full compensation for furnishing all material, including anchor bolts and shims, and all of the equipment and labor required to erect the rail in accordance with these plans and specifications.
- 2. All materials and workmanship shall be in accordance with Article 509 of the Standard Specifications.
- 3. Ends of rail sections are to be sawed or milled. All cut ends are to be true, smooth, and free of burrs or ragged edges.
- 4. No painting will be required.
- 5. Stainless steel bolts shall be according to ASTM A193-12b, Class 1 B8 (30 ksi minimum or 223 HB minimum) or Class 2 B8 (50 ksi minimum or 321 HB minimum), Stainless steel nuts shall be according to ASTM A194-12 Grade 8, 8M, or 8F with a UNC Series Class 2B fit. Stainless steel washers shall be plain flat, Type 304 or 3041, according to Federal Specification FF-W-92. Stainless steel bolts shall be snug tightened.
- 6. Anchor bolts shall be 58 " dia., A193-12B Gr. B7, be fully threaded with heavy hex nuts and one hardened washer and one 1^{3}_{4} " O.D. washer each. Embed threaded rods 10¹2" min. into concrete parapet. Anchor bolts, nuts, and washers shall be galvanized in accordance with ASTM F2329. Adhesive bonding material system shall be in accordance with materials I.M. 491.11. Installed anchors shall be capable of obtaining an ultimate load per threaded rod of 8 kips in tension for the spacing and edge distance shown in the plans.
- 7. Aluminum post and framing members shall comply with the requirements of ASTM B221-12 and ASTM B429-10 and be of Grade 6061-T6 and meet American National Standard dimensional tolerances for Aluminum Mill products. Aluminum Alloy 5052 H32 sheet panels shall comply with the requirements of ASTM B209-10.
- 8. Any welds with burrs on the framing member shall be ground flush. Welding shall comply with the requirements of AWS D1.2, Structural Welding Code - Aluminum.
- 9. Aluminum filler alloy ER5356 or ER5556 shall be used in accordance with Article 1094.05 of the standard specifications. Only microscopically clean welding wire (those which have been shaved after drawing) should be used, and spools of wire remaining at the end of the day's production should be sealed in polyethylene bags. Welding wire in drive rolls and gun not so protected shall be discarded.
- 10. All areas to be welded shall be brushed with stainless steel brushes immediately prior to welding. All aluminum welding shall be performed by the Gas Metal Arc Welding (GMAW) process. Only the stringer bead technique shall be used. Interpass temperature shall not exceed 200 degrees fahrenheit. All initial root passes shall not exceed $\frac{5}{16}$ inch and must penetrate the root. The convexity of a fillet weld shall not exceed is inch.

11. Post and framing members shall have a smooth, flush surface.

ITEM

Fence, Perforated Aluminum

12. Anodize coating shall be per AAMA 611-98 - Voluntary specifications for Anodized Architectural Aluminum. Use a Class 1 clear anodized finish (requires minimum coating thickness of 0.7 mil), Surface preparation shall be in accordance with ASTM D 3933-10.

PEDESTRIAN RAILING QUANTITIES

Quantity includes railing on Retaining Wall RW-02 (S.N. 081-6011),

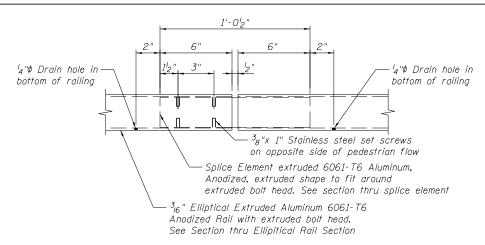
Mixed Used Path Anchorage Slab and Ramp RD-G Bridge Approach Slab.

- 13. The installed orientation of the punched panels must be punched toward the inside, with the breakout side toward the outside in case of sharp edges and for consistent appearance. Punched panels shall exhibit no burrs. If raw aluminum stock exhibits different finishes on each face, the punching shall be performed on the brightest face, which shall then be mounted toward the pedestrian side.
- 14. Provide two I_{16} inch aluminum shims of each type for each railing post, to be used as required.
- 15. Provide an l_8 inch thick neoprene sheet between concrete and shims under each rail post base plate. The neoprene sheet shall match the length and width of the masonry plate.
- 16. The neoprene sheets are to be 50, 60, or 70 Durometer hardness and shall meet the requirements of Iowa DOT Standard Specifications Section 4195.02.
- 17. Apply a neat caulk bead around plate edges. Do not contaminate surrounding concrete surfaces with caulk. Caulk shall be light grey non-slag latex marketed for outdoor use. No testing or certification is required.
- 18. Posts are to be set normal to grade.
- 19. Provide a railing mockup for review and approval. For the purposes of the mockup, one assembly including a single standard railing panel with 2 posts and a top rail will be required.
- 20. The Elliptical Rail shall be 6061-T6, 3 ₁₆" thick and shall conform to the requirements of ASTM B429-10. The heat treatment shall be in accordance with practice B918-09.
- 21. The 0.190" thick punched plate shall meet the requirements of ASTM B209-10 Alloy 5052-H32 with a minimum yield strength of 23 ksi and a minimum elongation of 9% in 2 inches. Punched plates shall have two sides standard mill finish.
- 22. The contractor shall furnish a certificate stating that each lot has been sampled, tested by a certified lab and inspected in accordance with the specification requirement of the corresponding ASTM standard.
- 23. Immediately following fabrication, protect all aluminum railing and panel surfaces from damage during shipping, handling, storage and installation. Protective measures shall remain in place until final assembly and installation. Repair or replacement of damaged components shall be at the contractor's cost and to the satisfaction of the engineer at no additional cost to the project.

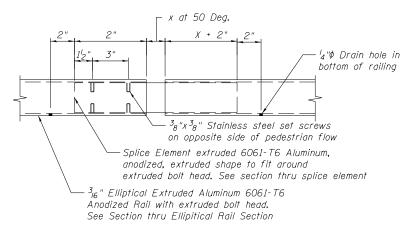
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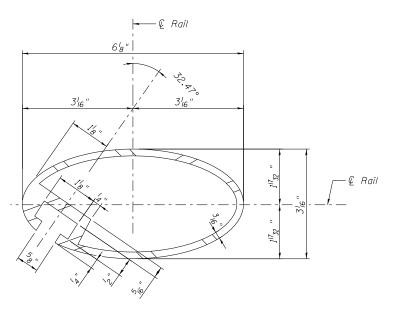
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PEDESTRIAN RAILING DETAILS 2	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	UM:
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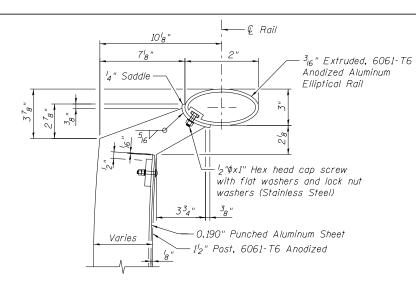
TYPICAL ELLIPTICAL RAILING SPLICE DETAIL



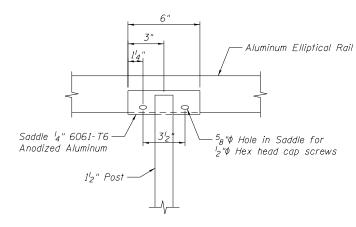
TYPICAL ELLIPTICAL RAILING EXPANSION JOINT SPLICE DETAIL



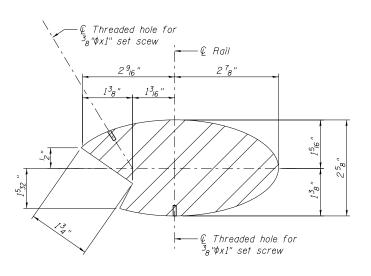
SECTION THRU ELLIPTICAL RAIL SECTION



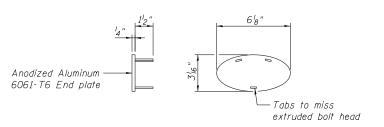
ELLIPTICAL RAILING POST CONNECTION DETAIL



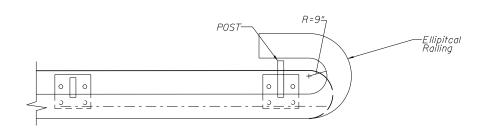
ELLIPTICAL RAILING POST CONNECTION DETAIL



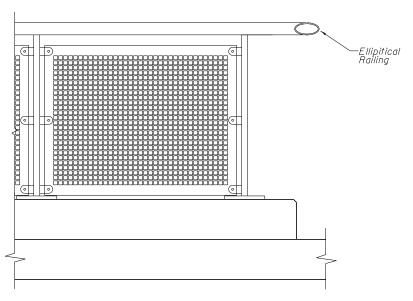
SECTION THRU SPLICE ELEMENT Note: Splice must be a sliding fit in rail section



CAST END CAP FOR ELLIPTICAL RAIL Drive Fit Type



PLAN VIEW SHORT RAILING TERMINATION DETAIL



SHORT RAILING TERMINATION DETAIL

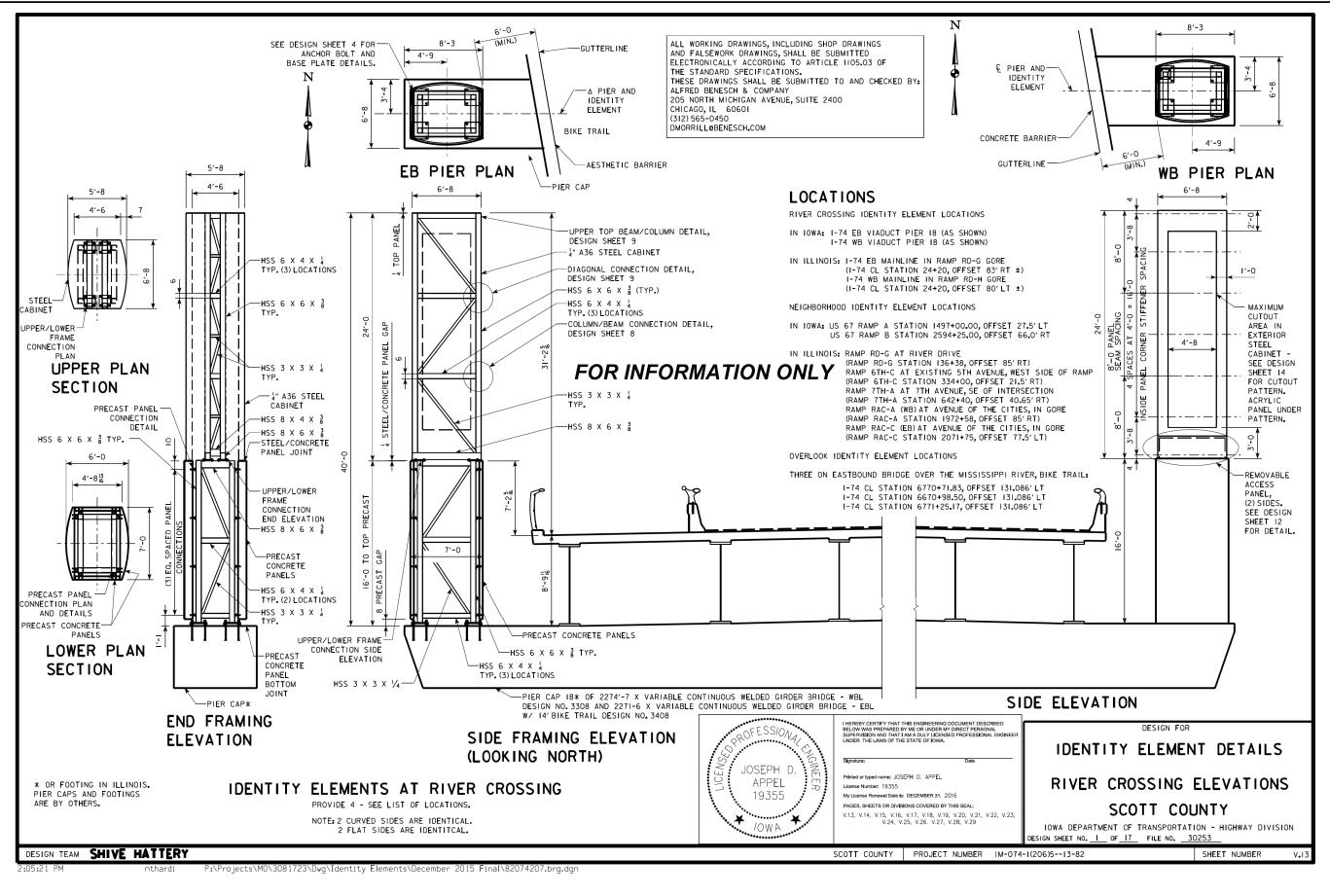
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.I. E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	CV.
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PROJECT NO. 92-032-01 CONTRACT NO. 64C08					خ ا
) RO	ROAD DIST NO 7 ILLINOIS FED AID PROJECT				



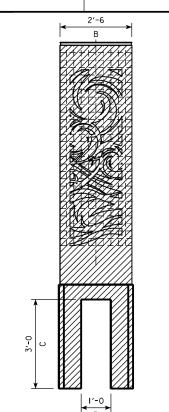


Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chlcago, Illinois 60601

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STATE OF	ILLINOIS
DEPARTMENT OF	TRANSPORTATION

RIVER CROSSING IDENTITY ELEMENT		SECTION
		(81-1)R & 81-1(HVBR
SHEET NO. 1 OF 13 SHEETS		ILLINOIS F



IDENTITY ELEMENT, **NEIGHBORHOOD** I'-0 **ELEVATION** END VIEW (3 SIDES SIMILAR)

ANTI-GRAFFITI COATING

NOTE:

FOR INFORMATION ONLY

NO ANTI-GRAFFITI COATING SHALL BE APPLIED TO THE CURVED ACRYLIC PANEL. CONTRACTOR IS RESPONSIBLE FOR PROTECTING THIS SURFACE DURING APPLICATION OF ANTI-GRAFFITI COATING TO THE REST OF THE IDENTITY ELEMENT'S VERTICAL SURFACES.

OVERLOOK DETAIL

NOTE:

ANTI-GRAFFITI COATING

ANTI-GRAFFITI SURFACE PREPARATION AND APPLICATION SHALL BE IN ACCORDANCE WITH THE "SPECIAL PROVISIONS FOR ANTI-GRAFFITI COATING". THE MATERIAL USED SHALL BE AN APPROVED TYPE IN ACCORDANCE WITH MATERIALS I.M. 491.23. ANTI-GRAFFITI COATING SHALL BE APPLIED TO SURFACES LISTED IN THE SCHEDULE AND TO THE LIMITS INDICATED IN THESE PLANS.

IDENTITY ELEMENT DETAILS

NEIGHBORHOOD ELEVATIONS SCOTT COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

SHEET NUMBER

DESIGN SHEET NO. 2 OF I7 FILE NO. 30253

DESIGN TEAM SHIVE HATTERY

STEEL-PANELS

PRECASE

CONCRETE

PRECAST PANEL-CONNECTION PLAN

UPPER PLAN SECTION

7′-0

LOWER PLAN SECTION

SEE LIGHT

DETAIL SUPPORT.

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FRAMING ELEVATION

3-SIDED IDENTITY ELEMENTS, NEIGHBORHOOD

PROVIDE (7). SEE LIST OF LOCATIONS ON DESIGN SHEET I.

NOTE: ALL 3 SIDES ARE IDENTICAL.



Job No. 1006

SEE DESIGN SHEET 4 FOR-ANCHOR BOLT AND

BASE PLATE DETAILS

CONCRETE FOOTING -

5′-3

FOUNDATION PLAN

UPPER TOP BEAM/COLUMN DETAIL, DESIGN SHEET 9

-HSS 3 X 3 X 🛔

−HSS 8 X 6 X 3

-HSS 6 X 6 X 4 TYP. STEEL/ CONCRETE PANEL

PRECASE

CONNECTION

PANEL

DETAIL

PRECAST

CONCRETE PANEL

BOTTOM JOINT

-GRADE

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SEE SHEETS V.9-V.12 FOR FOOTING DETAILS AT US 67 RAMPS A AND B AND FOR ORIENTATION OF IDENTITY ELEMENTS WITH

MAXIMUM CUTOUT AREA IN EXTERIOR STEEL CABINET -

SEE DESIGN SHEET 14 FOR CUTOUT PATTERN. ACRYLIC PANEL UNDER PATTERN.

1'-0

REMOVABLE

DETAIL, ALL (3) SIDES. SEE DESIGN SHEET 12.

SIDE ELEVATION

ACCESS

RESPECT TO ROADWAY ALIGNMENTS.

NEICHDORIGOD IDENTITY ELEMENT	F.A.I. RTE.	
NEIGHBORHOOD IDENTITY ELEMENT	74	
SHEET NO. 2 OF 13 SHEETS		

SCOTT COUNTY PROJECT NUMBER IM-074-I(206)5--I3-82

SECTION COUNTY (81-1)R & 81-1(HVBR) ROCK ISLAND 1504 1200 CONTRACT NO. 64CO8