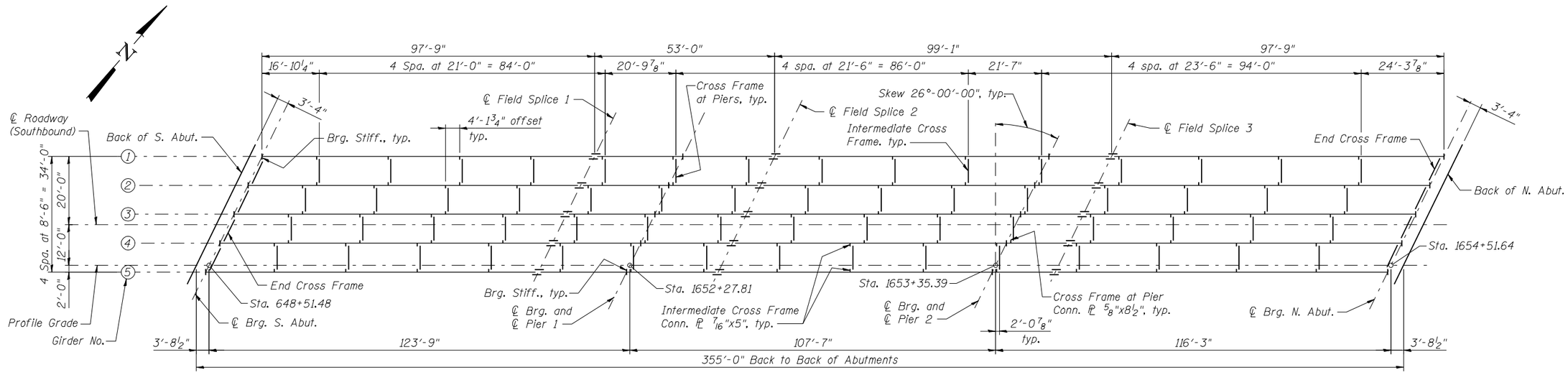

FRAMING PLAN

INTERIOR GIRDER MOMENT TABLE						
		0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.6 Sp. 3
I_s	(in^4)	29007	45007	25665	45007	26837
$I_c(n)$	(in^4)	81701	54884	69599	54884	73704
$I_c(3n)$	(in^4)	58640	54884	50959	54884	53604
$I_c(cr)$	(in^4)	35343	54922	31169	54922	32628
S_s	(in^3)	1194	1450	979	1450	1051
$S_c(n)$	(in^3)	1724	1986	1431	1986	1528
$S_c(3n)$	(in^3)	1571	1986	1300	1986	1391
$S_c(cr)$	(in^3)	1298	1999	1067	1999	1145
DC1	(k/')	1.10	1.16	1.08	1.16	1.08
M_{DC1}	('k)	1364	1731	-16	1547	1175
DC2	(k/')	0.21	0.21	0.21	0.21	0.21
M_{DC2}	('k)	266	313	9	287	231
DW	(k/')	0.38	0.38	0.38	0.38	0.38
M_{DW}	('k)	477	560	17	515	414
M_{L+IM}	('k)	1916	2032	1296	1990	1725
M_u (Strength I)	('k)	6106	6951	2285	6548	5397
$\phi_f M_n$	('k)	8223	8300	7622	8315	7295
f_s DC1	(ksi)	13.7	14.3	-0.2	12.8	13.4
f_s DC2	(ksi)	2.0	1.9	0.1	1.7	2.0
f_s DW	(ksi)	3.6	3.4	0.2	3.1	3.6
f_s (L+IM)	(ksi)	13.3	12.3	10.9	12.0	13.5
f_s (Service II)	(ksi)	36.7	35.6	14.2	33.3	36.6
$0.95R_hF_{yf}$	(ksi)	47.5	47.5	47.5	47.5	47.5
f_s (Total)(Strength I)	(ksi)	48.5	46.8	19.1	43.9	48.3
$\phi_f F_n$	(ksi)	50.0	50.0	50.0	50.0	50.0
V_f	(k)	26.9	30.4	24.7	29.6	26.3

INTERIOR GIRDER REACTION TABLE HL93 Loading (Unfactored)					
		S. Abut.	Pier 1	Pier 2	N. Abut.
R_{DC1}	(k)	56.7	147.4	137.8	52.1
R_{DC2}	(k)	10.7	27.4	26.2	9.8
R_{DW}	(k)	19.2	49.2	46.9	17.6
R_{LL+IM}	(k)	93.2	167.7	174.0	102.1
R_{Total}	(k)	179.8	391.7	384.9	181.6

I_s, S_s: Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads ($in.^4$ and $in.^3$).

$I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads ($in.^4$ and $in.^3$).

$I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads ($in.^4$ and $in.^3$).

$I_c(cr)$, $S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads ($in.^4$ and $in.^3$).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{L+IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).
$1.25\ (M_{DC1} + M_{DC2}) + 1.5\ M_{DW} + 1.75\ M_{L+IM}$

$\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
M_{DC1} / S_{nc}

f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
$M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.

f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
$M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

f_s (L+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
$M_{L+IM} / S_c(n)$ or $M_{DW} / S_c(cr)$ as applicable.

f_s (Service II): Sum of stresses as computed below (ksi).
$f_{sDC1} + f_{sDC2} + f_{sDW} + 1.3\ f_s(L+IM)$

$0.95R_hF_yf$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
$1.25\ (f_{sDC1} + f_{sDC2}) + 1.5\ f_{sDW} + 1.75\ f_s(L+IM)$

$\phi_f F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

V_f: Maximum factored shear range in span computed according to Article 6.10.10.

Note:
M_L and R_L include the effects of centrifugal force and superelevation.

Notes:
For Structural Steel notes, see sheet C32.
All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.