

06-12-2015 LETTING ITEM 114

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS
**PROPOSED
HIGHWAY PLANS**

F.A.P. ROUTE 322 (U.S. RTE. 51)
SECTION (58-20B-1)BR
PROJECT ACNHPP - 0322 (110)
SUPERSTRUCTURE REPLACEMENTS
MACON COUNTY

C-97-029-09

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	1
ILLINOIS CONTRACT NO. 74351				

* 122 + 3 = 125 TOTAL SHEETS

D-97-006-09

INDEX OF SHEETS

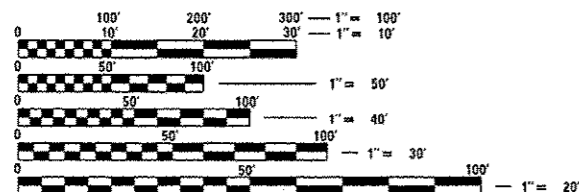
SHEET NO	TITLE
1	COVER SHEET
2	GENERAL NOTES
3-6	SUMMARY OF QUANTITIES
7-8	TYPICAL SECTIONS
9	QUANTITY SCHEDULES & PLAN DETAILS
10	CURB DETAILS
11	SURVEY TIES
12	SOUTH CROSSOVER PLAN & PROFILE
13	SOUTH CROSSOVER ELEVATIONS & OFFSETS
14	NORTH CROSSOVER PLAN & PROFILE
15	NORTH CROSSOVER ELEVATIONS & OFFSETS
16-18	TRAFFIC CONTROL - CROSSOVER CONSTRUCTION
19-27	STAGE 1 CONSTRUCTION DETAILS
28-36	STAGE 2 CONSTRUCTION DETAILS
37	EROSION CONTROL
* 38-105	BRIDGE REHABILITATION DETAILS
106-122	CROSS SECTIONS

* INCLUDES SHEETS 103A - 103C.



ADT = 4,250 (2013)

SN 058-0098 (NB)
SN 058-0099 (SB)

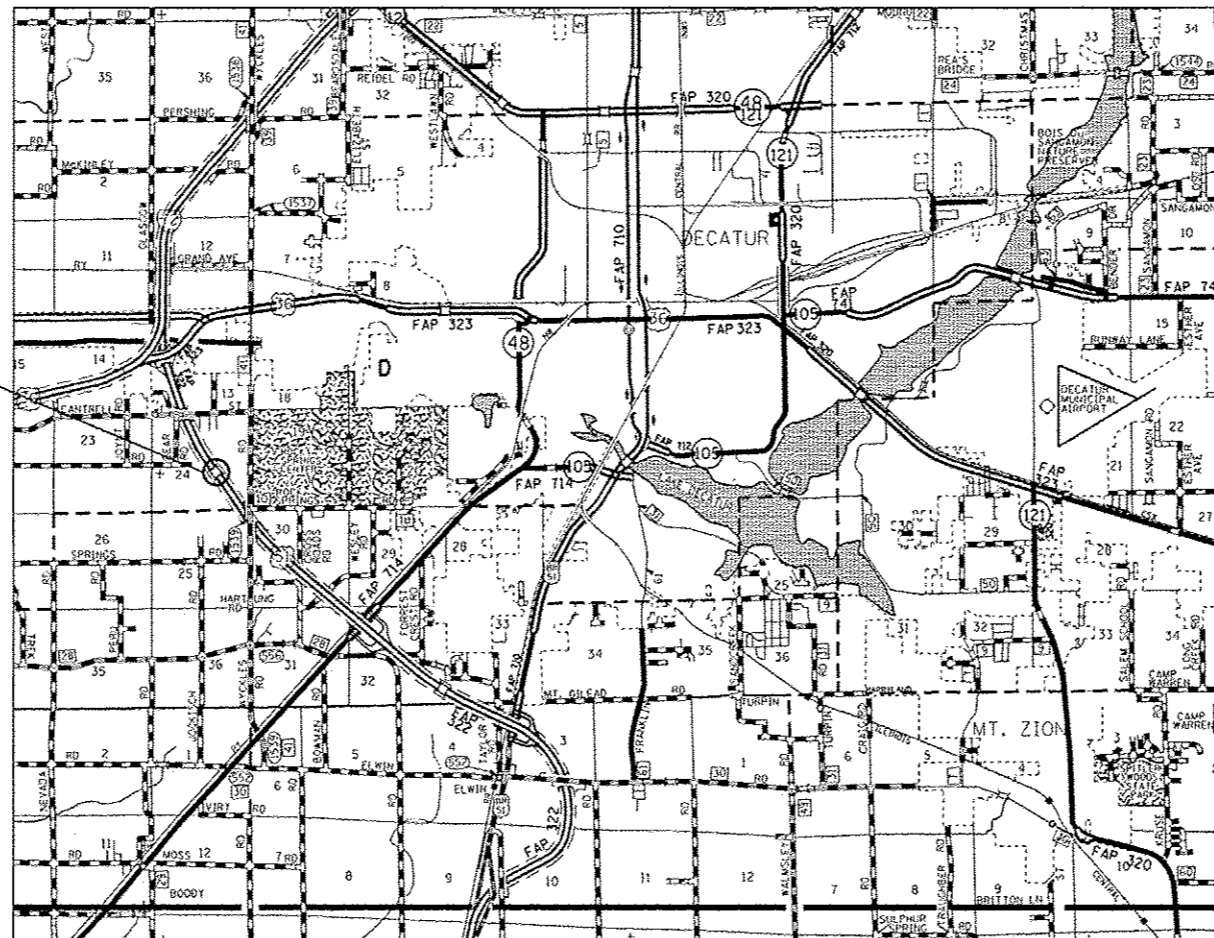


FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123
OR 811

PROJECT ENGINEER: MARK DAUGHERTY
PROJECT MANAGER: BRIAN LEWIS

CONTRACT NO. 74351



GROSS LENGTH = 7,061 FT. = 1.34 MILE
NET LENGTH = 3,749 FT. = 0.71 MILE

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

SUBMITTED March 20 2015
Ron L. Drishell
DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER

May 8 2015
John D. Baranzelli P.E.
ENGINEER OF DESIGN AND ENVIRONMENT

May 8 2015
Omar Osman P.E.
DIRECTOR OF HIGHWAYS, CHIEF ENGINEER

PRINTED BY THE AUTHORITY
OF THE STATE OF ILLINOIS

URBAN 80% FED
20% STATE

URBAN 80% FED
20% STATE

SUMMARY OF QUANTITIES			TOTAL QUANTITIES	CONSTRUCTION TYPE CODE		
CODE NO	ITEM	UNIT		0014		
20200100	EARTH EXCAVATION	CU YD	1398	1398		
20400800	FURNISHED EXCAVATION	CU YD	5040	5040		
21101505	TOPSOIL EXCAVATION AND PLACEMENT	CU YD	1834	1834		
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	600	600		
28000305	TEMPORARY DITCH CHECKS	FOOT	60	60		
28000500	INLET AND PIPE PROTECTION	EACH	9	9		
28100109	STONE RIPRAP, CLASS A5	SO YD	489	489		
28200200	FILTER FABRIC	SO YD	489	489		
35101600	AGGREGATE BASE COURSE, TYPE B 4"	SO YD	8996	8996		
35501332	HOT-MIX ASPHALT BASE COURSE, 12"	SO YD	7495	7495		
40600275	BITUMINOUS MATERIALS (PRIME COAT)	POUND	4862	4862		
40701926	HOT-MIX ASPHALT PAVEMENT (FULL-DEPTH), 12 1/4"	SO YD	7203	7203		
42001430	BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE)	SO YD	112	112		
44000300	CURB REMOVAL	FOOT	15	15		

SUMMARY OF QUANTITIES			TOTAL QUANTITIES	CONSTRUCTION TYPE CODE		
CODE NO	ITEM	UNIT		0014		
44004250	PAVED SHOULDER REMOVAL	SO YD	10419	10419		
44200529	CLASS A PATCHES, TYPE II, 8 INCH	SO YD	56	56		
44200533	CLASS A PATCHES, TYPE III, 8 INCH	SO YD	16	16		
44200535	CLASS A PATCHES, TYPE IV, 8 INCH	SO YD	409	409		
44213000	PATCHING REINFORCEMENT	SO YD	481	481		
44213200	SAW CUTS	FOOT	1394	1394		
44213204	TIE BARS 3/4"	EACH	148	148		
48101200	AGGREGATE SHOULDERS, TYPE B	TON	4446	4446		
48102100	AGGREGATE WEDGE SHOULDER, TYPE B	TON	22	22		
48203021	HOT-MIX ASPHALT SHOULDERS, 6"	SO YD	354	354		
50101500	REMOVAL OF EXISTING SUPERSTRUCTURES	EACH	4	4		
50102400	CONCRETE REMOVAL	CU YD	77.8	77.8		
50104400	CONCRETE HEADWALL REMOVAL	EACH	10	10		
50104720	REMOVAL OF EXISTING CONCRETE DECK	EACH	2	2		

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		DATE -	REVISED -
Default	PLOT DATE = 3/28/2015		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	158-20B-1BR	Macon	122	3
			CONTRACT NO. 74351	
ILLINOIS FED. AID PROJECT				

SUMMARY OF QUANTITIES			URBAN		80% FED 20% STATE	
CODE NO	ITEM	UNIT	TOTAL QUANTITIES	CONSTRUCTION TYPE CODE		
				0014		
50200100	STRUCTURE EXCAVATION	CU YD	50	50		
50300100	FLOOR DRAINS	EACH	48	48		
50300225	CONCRETE STRUCTURES	CU YD	115.3	115.3		
50300255	CONCRETE SUPERSTRUCTURE	CU YD	2313.2	2313.2		
50300260	BRIDGE DECK GROOVING	SQ YD	6860	6860		
50300300	PROTECTIVE COAT	SQ YD	8564	8564		
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1	1		
50500505	STUD SHEAR CONNECTORS	EACH	33504	33504		
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	649160	649160		
50800515	BAR SPLICERS	EACH	184	184		
51500100	NAME PLATES	EACH	2	2		
52000110	PREFORMED JOINT STRIP SEAL	FOOT	356.5	356.5		
52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	84	84		
52100020	ELASTOMERIC BEARING ASSEMBLY, TYPE II	EACH	24	24		

SUMMARY OF QUANTITIES			URBAN		80% FED 20% STATE	
CODE NO	ITEM	UNIT	TOTAL QUANTITIES	CONSTRUCTION TYPE CODE		
				0014		
52100520	ANCHOR BOLTS, 1"	EACH	264	264		
54213459	END SECTIONS 24"	EACH	4	4		
54200220	PIPE CULVERTS, CLASS D, TYPE I 15"	FOOT	84	84		
54200229	PIPE CULVERTS, CLASS D, TYPE I 24"	FOOT	1330	1330		
58700300	CONCRETE SEALER	SQ FT	1301	1301		
60100060	CONCRETE HEADWALLS FOR PIPE DRAINS	EACH	6	6		
60108100	PIPE UNDERDRAINS 4" (SPECIAL)	FOOT	234	234		
60218400	MANHOLES, TYPE A, 4'-DIAMETER, TYPE I FRAME, CLOSED LID	EACH	2	2		
60236200	INLETS, TYPE A, TYPE B GRATE	EACH	4	4		
60240301	INLETS, TYPE B, TYPE B GRATE	EACH	1	1		
60600605	CONCRETE CURB, TYPE B	FOOT	15	15		
* 63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	FOOT	675	675		
* 63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4	4		

* SPECIALTY ITEM

FILE NAME =	USER NAME = gteffennk	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUMMARY OF QUANTITIES	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
Default		CHECKED -	REVISED -			322	(58-208-118R)	Macon	122	4
		DATE -	REVISED -			CONTRACT NO. 74351				
						ILLINOIS FED. AID PROJECT				

URBAN 80% FED 20% STATE

URBAN 80% FED 20% STATE

SUMMARY OF QUANTITIES			TOTAL QUANTITIES	CONSTRUCTION TYPE CODE		
CODE NO	ITEM	UNIT		0014		
* 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL)	EACH	4	4		
	TANGENT					
63200310	GUARDRAIL REMOVAL	FOOT	575	575		
63800920	MODULAR CLARE SCREEN SYSTEM, TEMPORARY	FOOT	5450	5450		
64200116	SHOULDER RUMBLE STRIPS, 16 INCH	FOOT	9694	9694		
66700305	PERMANENT SURVEY MARKERS, TYPE II	EACH	7	7		
67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	27	27		
67100100	MOBILIZATION	L SUM	1	1		
70100410	TRAFFIC CONTROL AND PROTECTION, STANDARD	EACH	2	2		
	701416					
70100420	TRAFFIC CONTROL AND PROTECTION, STANDARD	EACH	3	3		
	701411					
70100700	TRAFFIC CONTROL AND PROTECTION, STANDARD	L SUM	1	1		
	701406					
70100800	TRAFFIC CONTROL AND PROTECTION, STANDARD	L SUM	1	1		
	701401					
70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	2	2		

SUMMARY OF QUANTITIES			TOTAL QUANTITIES	CONSTRUCTION TYPE CODE		
CODE NO	ITEM	UNIT		0014		
70107007	PAVEMENT MARKING BLACKOUT TAPE, 7"	FOOT	1668	1668		
70300100	SHORT TERM PAVEMENT MARKING	FOOT	12000	12000		
70301000	WORK ZONE PAVEMENT MARKING REMOVAL	SQ FT	4973	4973		
70400100	TEMPORARY CONCRETE BARRIER	FOOT	5450	5450		
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	5450	5450		
70500100	TEMPORARY STEEL PLATE BEAM GUARDRAIL, TYPE A	FOOT	175	175		
70500665	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	2	2		
72500100	OBJECT MARKER - TYPE I	EACH	128	128		
72800100	TELESCOPING STEEL SIGN SUPPORT	FOOT	160	160		
* 78009004	MODIFIED URETHANE PAVEMENT MARKING - LINE 4"	FOOT	116000	116000		
* 78009006	MODIFIED URETHANE PAVEMENT MARKING - LINE 6"	FOOT	14500	14500		
* 78100105	RAISED REFLECTIVE PAVEMENT MARKER (BRIDGE)	EACH	44	44		
* 78200410	GUARDRAIL MARKERS, TYPE A	EACH	24	24		
* 78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	6	6		

* SPECIALTY ITEM

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		DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES			
SCALE:	SHEET	OF	SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	058-208-1BR	Macon	122	5
			CONTRACT NO. 74351	
ILLINOIS FED. AID PROJECT				

SUMMARY OF QUANTITIES			URBAN	80% FED 20% STATE		
CODE NO	ITEM	UNIT	TOTAL QUANTITIES	CONSTRUCTION TYPE CODE		
				0014		
78300100	PAVEMENT MARKING REMOVAL	SO FT	5068	5068		
X2501000	SEEDING, CLASS 2 (SPECIAL)	ACRE	3	3		
X7010410	SPEED DISPLAY TRAILER	CAL MO	38	38		
X7015005	CHANGEABLE MESSAGE SIGN	CAL DA	28	28		
X7050167	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 1, SPECIAL (TANGENT)	EACH	2	2		
* X7270008	BREAKAWAY SIGN SUPPORT COUPLER	EACH	32	32		
X7810400	TEMPORARY RAISED PAVEMENT MARKER	EACH	580	580		
* X7830070	GROOVING FOR RECESSED PAVEMENT MARKING 5"	FOOT	3308	3308		
* X7830074	GROOVING FOR RECESSED PAVEMENT MARKING 7"	FOOT	420	420		
Z0001899	JACK AND REMOVE EXISTING BEARINGS	EACH	24	24		
Z0001903	STRUCTURAL STEEL REMOVAL	POUND	6120	6120		
Z0004552	APPROACH SLAB REMOVAL	SO YD	384	384		
Z0007112	CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES	L SUM	1	1		

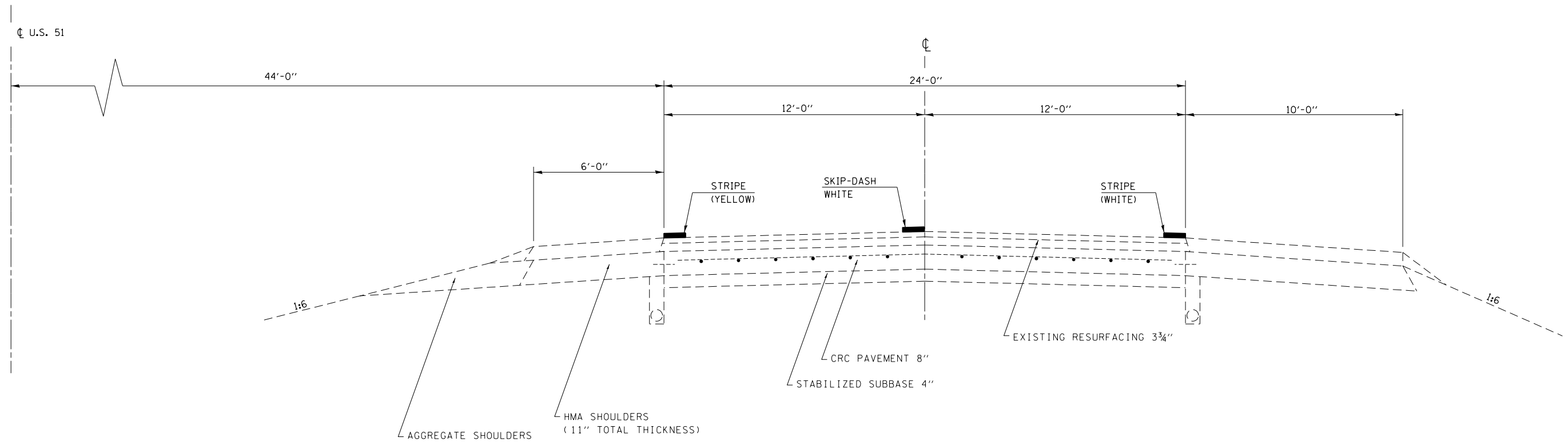
SUMMARY OF QUANTITIES			URBAN	80% FED 20% STATE		
CODE NO	ITEM	UNIT	TOTAL QUANTITIES	CONSTRUCTION TYPE CODE		
				0014		
Z0010501	CLEANING AND PAINTING STEEL BRIDGE NO. 1	L SUM	1	1		
Z0010502	CLEANING AND PAINTING STEEL BRIDGE NO. 2	L SUM	1	1		
Z0012754	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SO FT	324	324		
Z0018004	DRAINAGE SCUPPERS, DS-12	EACH	8	8		
Z0049799	PROTECTING OR RESETTING SURVEY MARKERS	EACH	7	7		
Z0065700	SLOPE WALL REPAIR	SO YD	49	49		
Z0076600	TRAINEES	HOUR	1500	1500		
Z0076604	TRAINEES TRAINING PROGRAM GRADUATE	HOUR	1500	1500		

* SPECIALTY ITEM

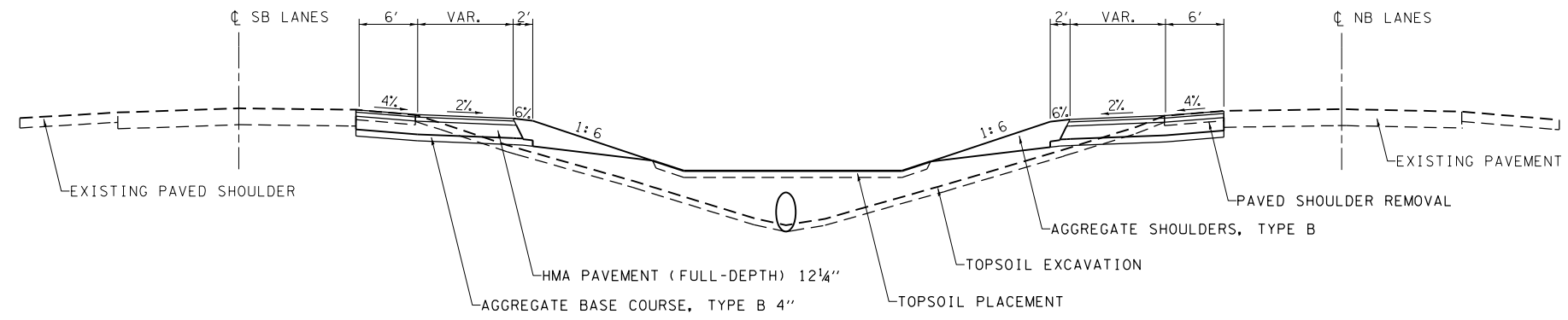
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						ILLINOIS FED. AID PROJECT					

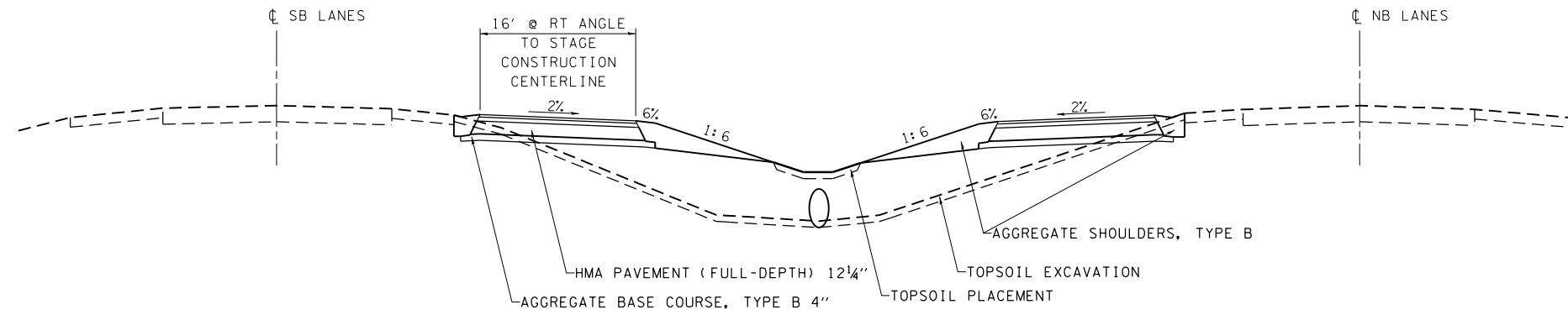
EXISTING TYPICAL SECTION



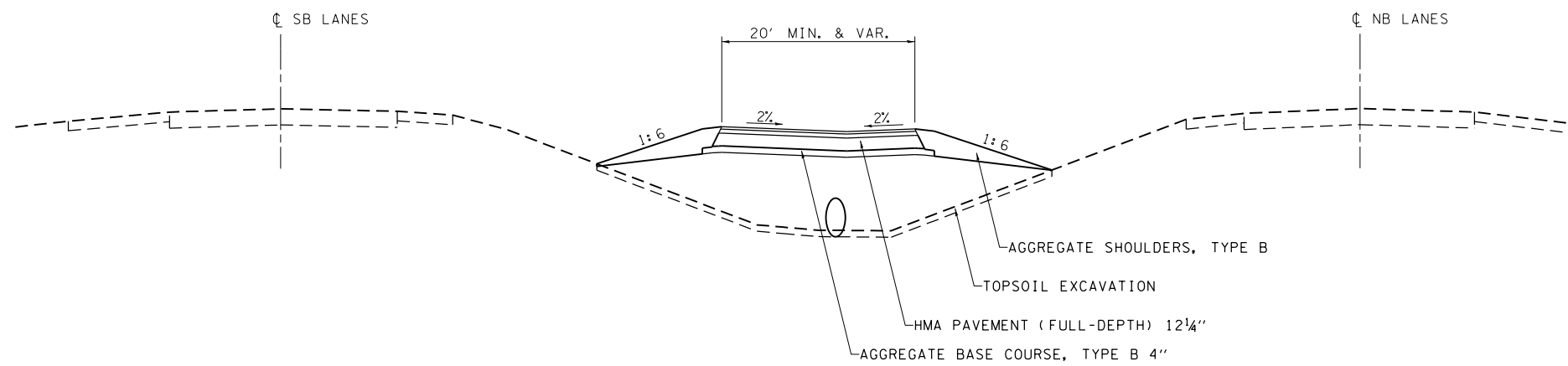
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	PLOT DATE = 3/20/2015	DATE -	REVISED -		SCALE:	SHEET	OF	SHEETS	STA.	TO	STA.		



STA 315+70 TO STA 320+70
 STA 325+30 TO STA 330+30
 STA 371+70 TO STA 376+70
 STA 381+30 TO STA 754+34



STA 320+70 TO STA 322+26
 STA 323+74 TO STA 325+30
 STA 376+70 TO STA 378+26
 STA 379+74 TO STA 381+30



STA 322+26 TO STA 323+74
 STA 378+26 TO STA 379+74

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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

PROPOSED CROSSOVER TYPICAL SECTIONS

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	8
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	

GUARDRAIL REMOVAL

DESCRIPTION	STATIONING		SIDE	LENGTH (FOOT)
	FROM	TO		
NORTHBOUND				
BRIDGE SN 058-0098	340+97	342+68	LT	175
BRIDGE SN 058-0098	341+70	342+83	RT	112.5
NORTHBOUND TOTAL				287.5
SOUTHBOUND				
BRIDGE SN 058-0099	350+05	351+19	RT	112.5
BRIDGE SN 058-0099	350+20	351+92	LT	175
SOUTHBOUND TOTAL				287.5
PROJECT TOTAL				575

APPROACH SLAB REMOVAL

DESCRIPTION	STATION ALONG C.L. LANES		AREA SQ YD
	FROM	TO	
SOUTHBOUND			
BRIDGE SN 058-0099	342+10	342+46	96
BRIDGE SN 058-0099	350+02	350+38	96
SOUTHBOUND TOTAL			192
NORTHBOUND			
BRIDGE SN 058-0098	342+51	342+87	96
BRIDGE SN 058-0098	350+42	350+78	96
NORTHBOUND TOTAL			192
PROJECT TOTAL			384

GUARDRAIL SCHEDULE

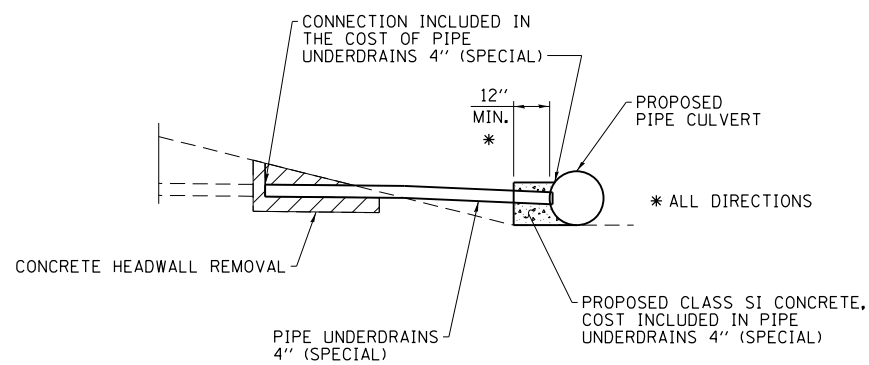
DESCRIPTION	LOCATION	FOOT	EACH	EACH	EACH	EACH	SQ YD
NORTHBOUND SN 058-0098	SW CORNER (P.L.)	187.5	1	1	5	1	95
	SE CORNER (D.L.)	150	1	1	4	1	82
NORTHBOUND SUBTOTAL		337.5	2	2	9	2	177
SOUTHBOUND SN 058-0099	NW CORNER (D.L.)	150	1	1	4	1	82
	NE CORNER (P.L.)	187.5	1	1	5	1	95
SOUTHBOUND SUBTOTAL		337.5	2	2	9	2	177
PROJECT TOTAL		675	4	4	18	4	354

SURVEY MARKERS

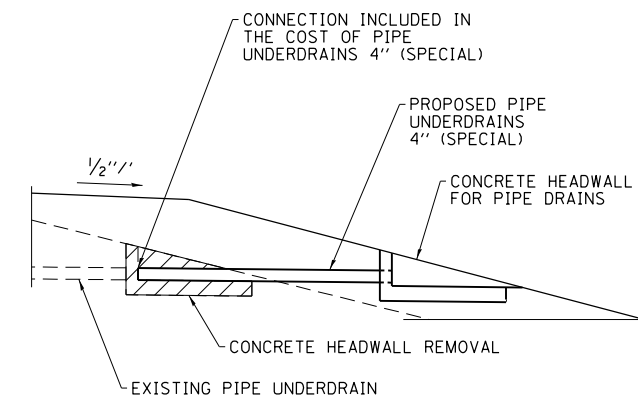
STATION	PROTECTING OR RESETTING SURVEY MARKERS	PERMANENT SURVEY MARKERS, TYPE II
	EACH	EACH
313+26.15	1	1
328+00.00	1	1
342+00.00	1	1
353+16.13	1	1
359+49.53	1	1
365+80.04	1	1
383+96.40	1	1

EARTHWORK SCHEDULE

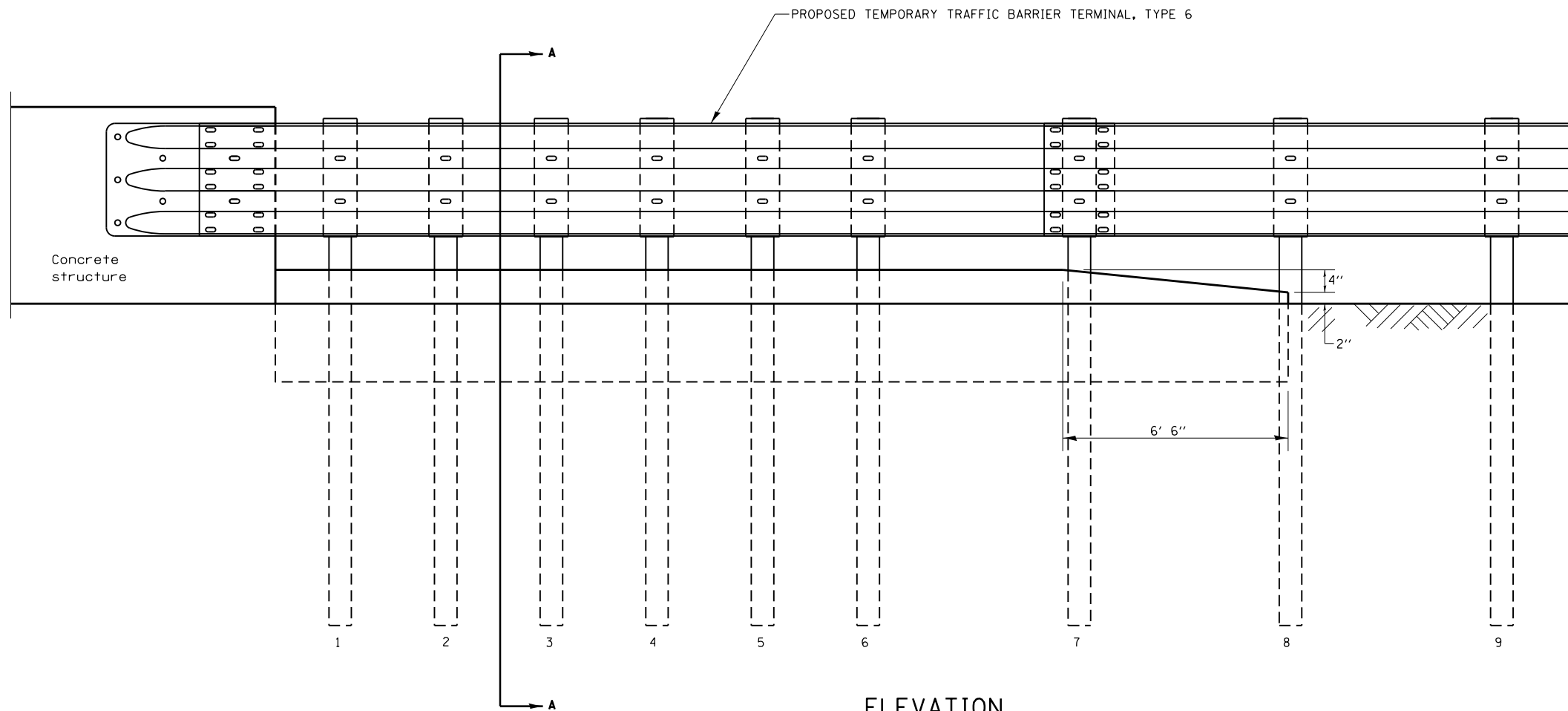
LOCATION	EARTH EXCAVATION CU. YD.	EXCAVATION TO BE USED IN EMBANKMENT ADJUSTED FOR SHRINKAGE CU. YD.	EMBANKMENT CU. YD.	EARTHWORK BALANCE WASTE (+) SHORTAGE (-) CU. YD.	TOPSOIL EXCAVATION AND PLACEMENT CU. YD.
CROSSOVERS	1398	1049	6089	-5040	1834



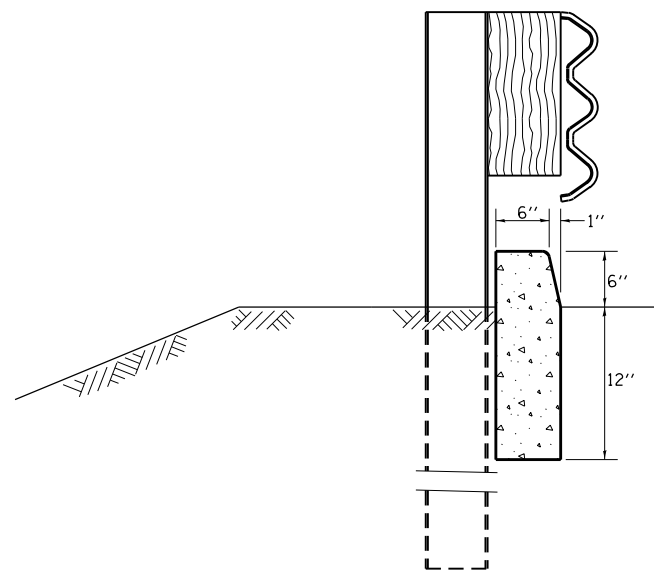
UNDERDRAIN CONNECTION TO MEDIAN CULVERT



UNDERDRAIN EXTENSION AT CROSSOVERS



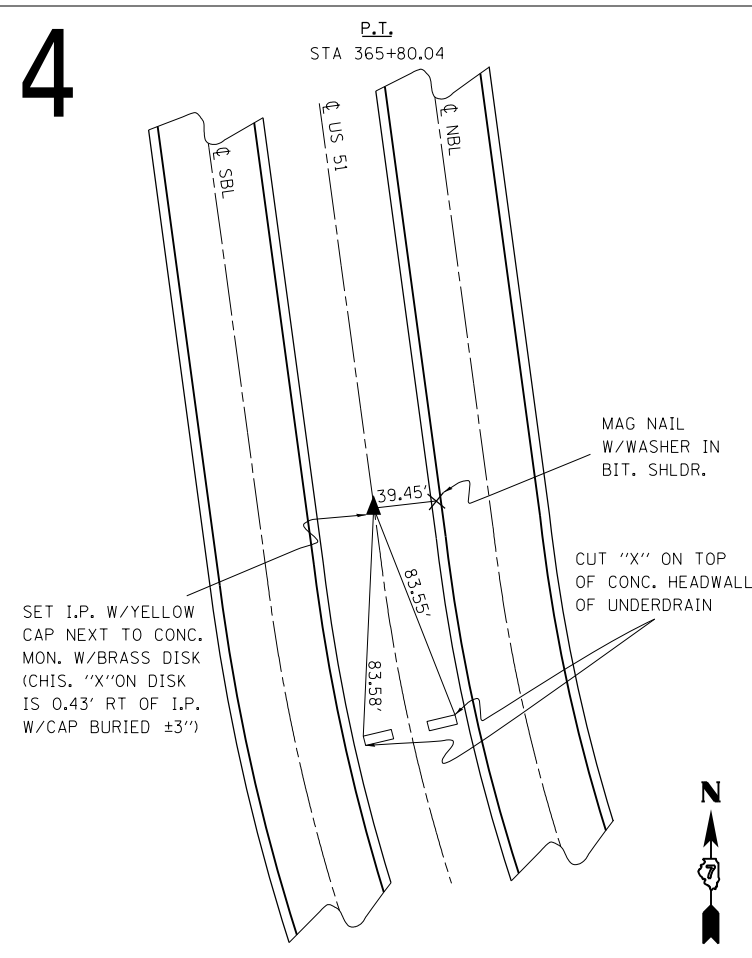
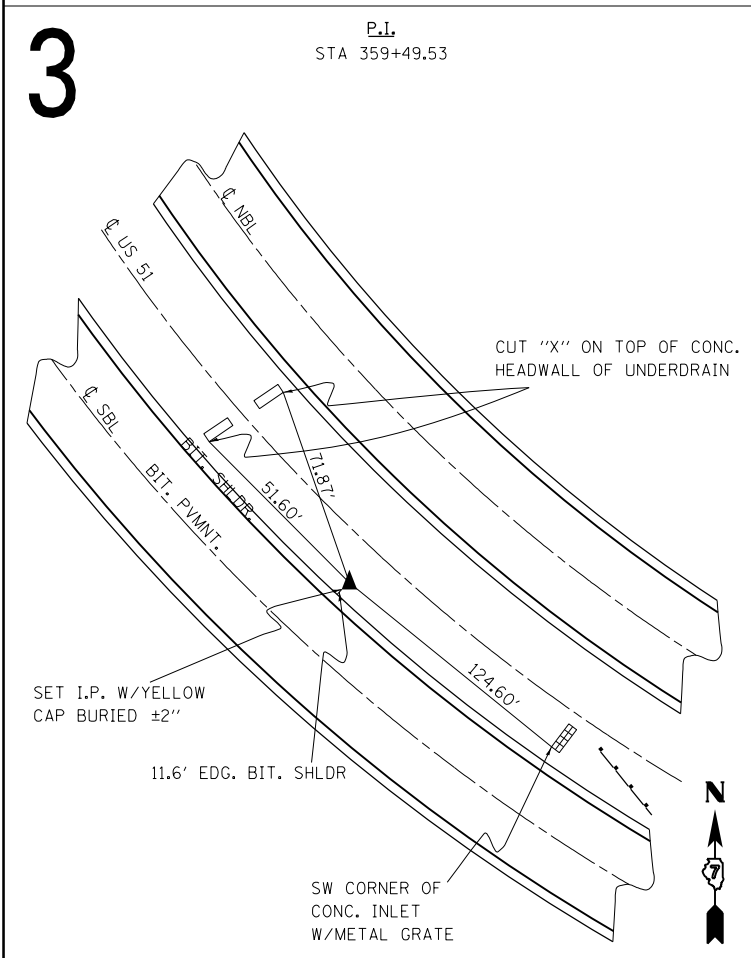
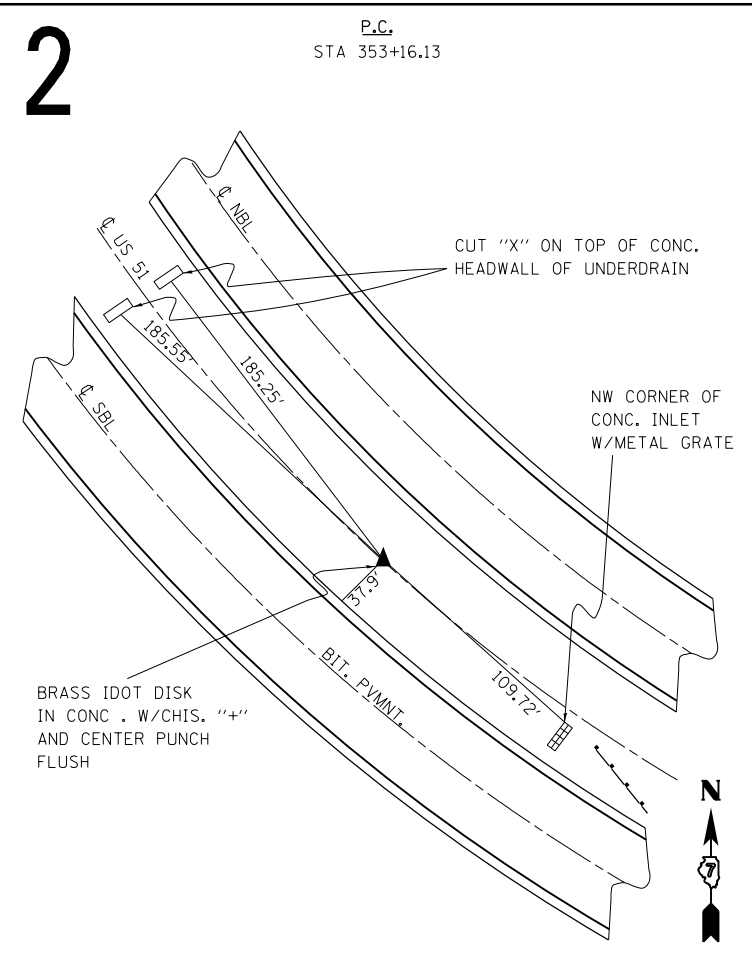
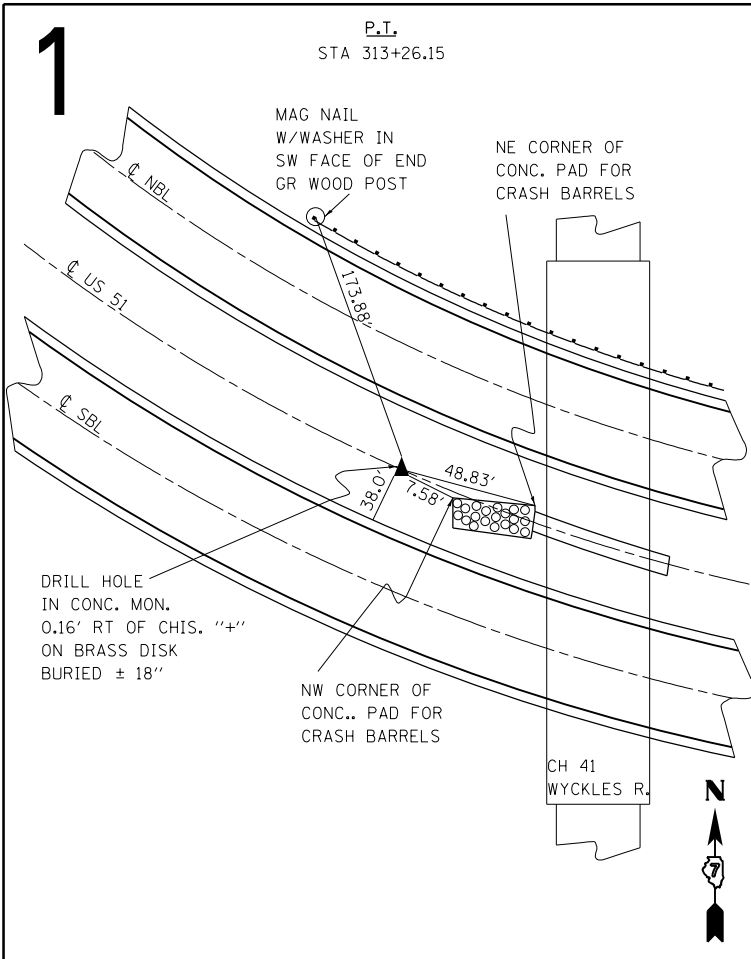
ELEVATION



SECTION A-A

NOTE: CONSTRUCT CURB AT SOUTHEAST CORNER OF SOUTHBOUND STRUCTURE ONLY.

FILE NAME =	USER NAME = steffenmk	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CURB DETAILS				F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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Default	PLOT SCALE = 100.0000' / 1in.	DATE -	REVISED -		SCALE: SHEET OF SHEETS STA. TO STA.				CONTRACT NO. 74351				
	PLOT DATE = 3/20/2015				ILLINOIS FED. AID PROJECT								



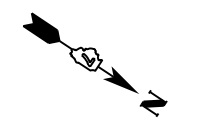
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -
	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

SURVEY TIES

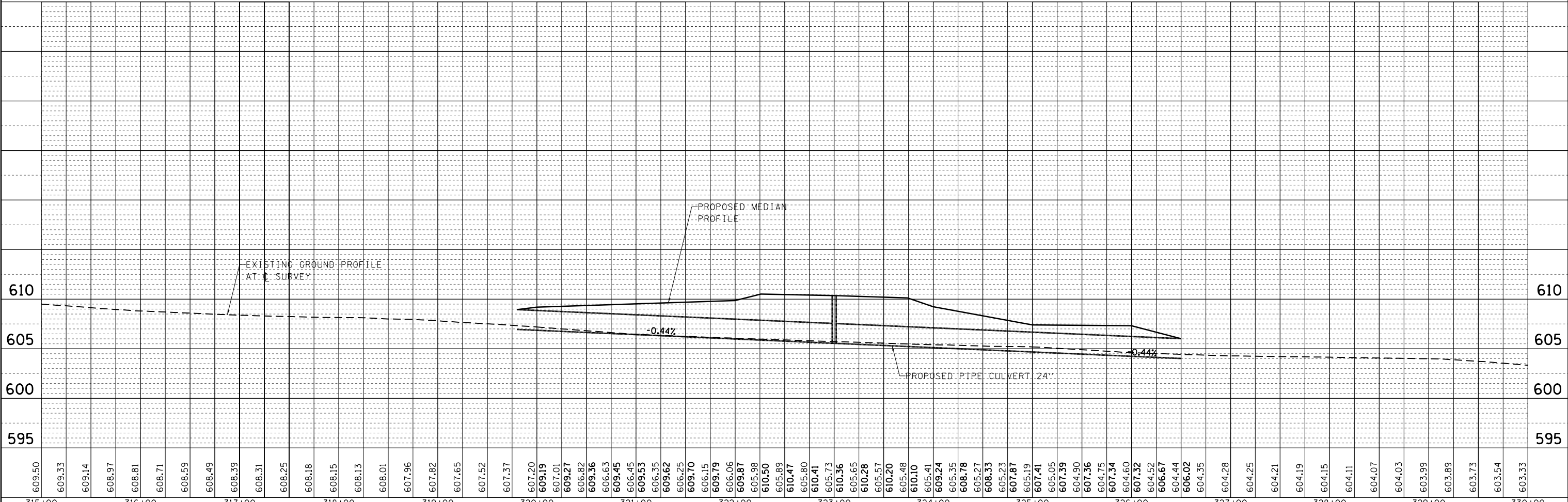
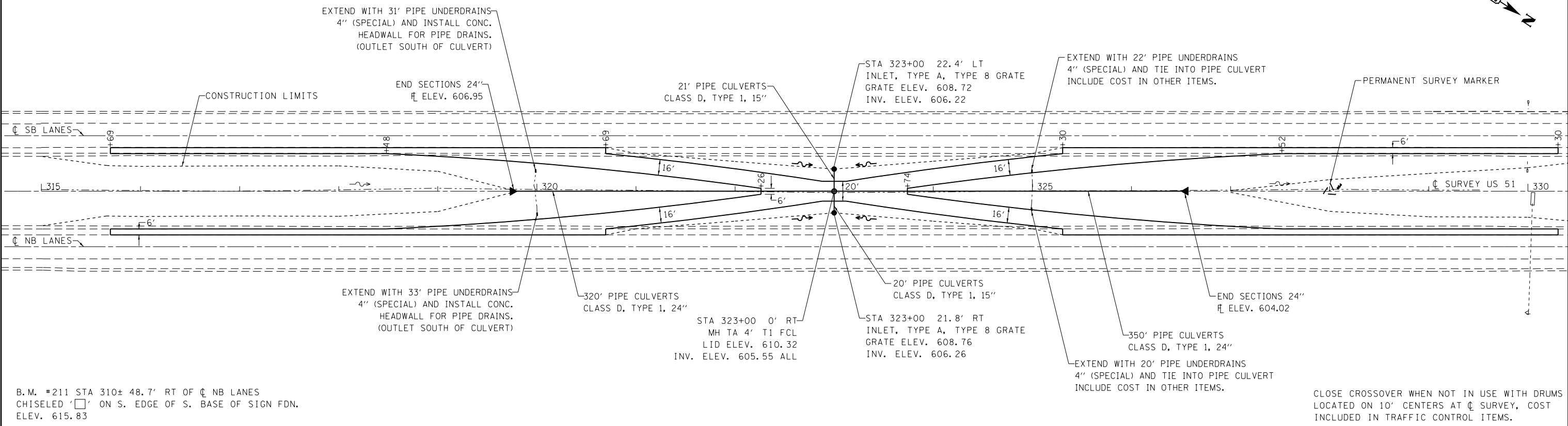
SCALE:	SHEET	OF	SHEETS	STA.	TO	STA.
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	11
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	



PLAN	SURVEYED	DATE
	PLOTTED	BY
	ALIGNED	
	CHECKED	
	FILED	
	NO.	

PROFILE	SURVEYED	DATE
	PLOTTED	BY
	GRADES CHECKED	
	STRUCTURE	
	NOT AT THIS OFFICE	
	NO.	

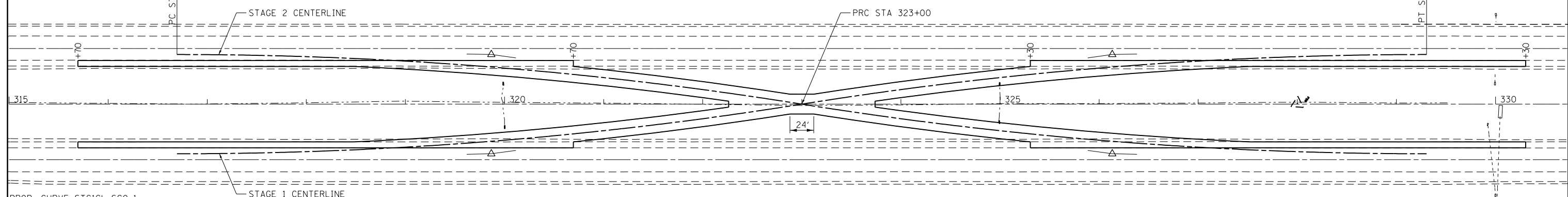


FILE NAME =	USER NAME = steffennk	DESIGNED -	REVISED -	PLAN & PROFILE				F.A.P. RE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
Default				SOUTH CROSSOVER				322	(58-20B-1)BR	Macon	122	12
				SCALE: SHEET OF SHEETS STA. TO STA.				CONTRACT NO. 74351				
											ILLINOIS FED. AID PROJECT	

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

PROP. CURVE STG2CL-SCO-1
 PI STA. = 319+86.74 50.00' LT
 $\Delta = 9^{\circ} 04' 07''$ (RT)
 $D = 1^{\circ} 25' 57''$
 $R = 4,000.00'$
 $T = 317.22'$
 $L = 633.12'$
 $E = 12.56'$

PROP. CURVE STG1CL-SCO-2
 PI STA. = 326+13.26 50.00' LT
 $\Delta = 9^{\circ} 04' 07''$ (RT)
 $D = 1^{\circ} 25' 57''$
 $R = 4,000.00'$
 $T = 317.22'$
 $L = 633.12'$
 $E = 12.56'$



PROP. CURVE STG1CL-SCO-1
 PI STA. = 319+86.74 50.00' RT
 $\Delta = 9^{\circ} 04' 07''$ (LT)
 $D = 1^{\circ} 25' 57''$
 $R = 4,000.00'$
 $T = 317.22'$
 $L = 633.12'$
 $E = 12.56'$

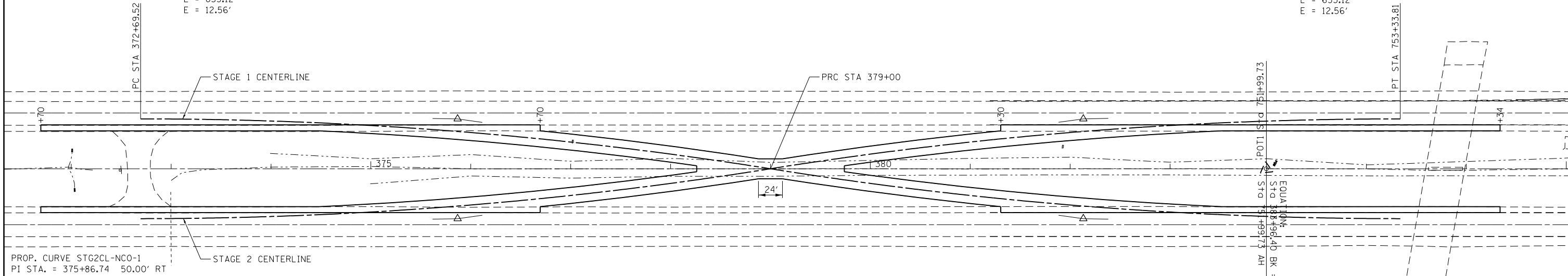
PROP. CURVE STG2CL-SCO-2
 PI STA. = 326+13.26 50.00' RT
 $\Delta = 9^{\circ} 04' 07''$ (LT)
 $D = 1^{\circ} 25' 57''$
 $R = 4,000.00'$
 $T = 317.22'$
 $L = 633.12'$
 $E = 12.56'$

SOUTH CROSS OVER ELEVATION AND OFFSET DATA

C. L. STATION U. S. 51	STAGE 1 CENTERLINE	STAGE 2 CENTERLINE	E. O. P. LEFT OF C. L.		E. O. P. LEFT OF C. L.		BREAK POINT LEFT		E. O. P. RIGHT OF C. L.		E. O. P. RIGHT OF C. L.		BREAK POINT RIGHT	
	OFFSET (FT)	OFFSET (FT)	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.
318+50	45.93	-45.93	-37.92	612.53			-38.00	612.53			37.92	612.46	38.00	612.46
318+75	44.72	-44.72	-36.71	612.45			-38.00	612.48			36.71	612.37	38.00	612.40
319+00	43.35	-43.35	-35.34	612.36			-38.00	612.41			35.34	612.29	38.00	612.34
319+25	41.83	-41.83	-33.82	612.23			-38.00	612.31			33.82	612.22	38.00	612.30
319+50	40.15	-40.15	-32.13	612.09			-38.00	612.20			32.13	612.14	38.00	612.26
319+75	38.32	-38.32	-30.29	611.95			-38.00	612.10			30.29	612.06	38.00	612.21
320+00	36.32	-36.32	-28.30	611.81			-38.00	612.00			28.30	611.98	38.00	612.17
320+25	34.17	-34.17	-26.14	611.71			-38.00	611.94			26.14	611.83	38.00	612.06
320+50	31.86	-31.86	-23.83	611.60			-38.00	611.89			23.83	611.67	38.00	611.96
320+75	29.40	-29.40	-21.35	611.51	-37.44	611.83			37.44	611.84	21.35	611.52		
321+00	26.77	-26.77	-18.72	611.39	-34.82	611.71			34.82	611.68	18.72	611.36		
321+25	23.98	-23.98	-15.93	611.24	-32.04	611.56			32.04	611.53	15.93	611.21		
321+50	21.04	-21.04	-12.98	611.08	-29.10	611.40			29.10	611.37	12.98	611.05		
321+75	17.93	-17.93	-9.87	610.92	-26.00	611.25			26.00	611.22	9.87	610.90		
322+00	14.67	-14.67	-6.60	610.76	-22.74	611.09			22.74	611.06	6.60	610.74		
322+25	11.24	-11.24	-3.16	610.63	-19.32	610.96			19.32	610.92	3.16	610.59		
322+50	7.66	-7.66			-15.74	610.81			15.74	610.76			1.25	610.47
322+75	3.91	-3.91			-12.00	610.67			12.00	610.61			1.50	610.40
323+00	0.00	0.00			-10.00	610.56			10.00	610.49			1.75	610.32
323+25	-3.91	3.91			-12.00	610.49			12.00	610.47			0.50	610.24
323+50	-7.66	7.66			-15.74	610.45	-0.75	610.15	15.74	610.48				
323+75	-11.24	11.24	-3.16	610.09	-19.32	610.42			19.32	610.49	3.16	610.17		
324+00	-14.67	14.67	-6.60	610.05	-22.74	610.38			22.74	610.50	6.60	610.17		
324+25	-17.93	17.93	-9.87	610.05	-26.00	610.37			26.00	610.48	9.87	610.15		
324+50	-21.04	21.04	-12.98	610.04	-29.10	610.37			29.10	610.45	12.98	610.12		
324+75	-23.98	23.98	-15.93	610.04	-32.04	610.36			32.04	610.41	15.93	610.09		
325+00	-26.77	26.77	-18.72	610.03	-34.82	610.35			34.82	610.38	18.72	610.05		
325+25	-29.40	29.40	-21.35	610.02	-37.44	610.34			37.44	610.35	21.35	610.03		
325+50	-31.86	31.86	-23.83	610.01			-38.00	610.29			23.83	610.00	38.00	610.28
325+75	-34.17	34.17	-26.14	610.00			-38.00	610.24			26.14	609.97	38.00	610.21
326+00	-36.32	36.32	-28.30	609.98			-38.00	610.18			28.30	609.94	38.00	610.13
326+25	-38.32	38.32	-30.29	609.94			-38.00	610.10			30.29	609.92	38.00	610.07
326+50	-40.15	40.15	-32.13	609.89			-38.00	610.01			32.13	609.90	38.00	610.02
326+75	-41.83	41.83	-33.82	609.85			-38.00	609.93			33.82	609.88	38.00	609.96
327+00	-43.35	43.35	-35.34	609.80			-38.00	609.84			35.34	609.85	38.00	609.91
327+25	-44.72	44.72	-36.71	609.75			-38.00	609.78			36.71	609.80	38.00	609.83
327+50	-45.93	45.93	-37.92	609.72			-38.00	609.72			37.92	609.75	38.00	609.75

PROP. CURVE STG1CL-NC0-1
 PI STA. = 375+86.74 50.00' LT
 $\Delta = 9^\circ 04' 07''$ (RT)
 D = 1° 25' 57"
 R = 4,000.00'
 T = 317.22'
 L = 633.12'
 E = 12.56'

PROP. CURVE STG2CL-NC0-2
 PI STA. = 382+13.26 50.00' LT
 $\Delta = 9^\circ 04' 07''$ (RT)
 D = 1° 25' 57"
 R = 4,000.00'
 T = 317.22'
 L = 633.12'
 E = 12.56'



PROP. CURVE STG2CL-NC0-1
 PI STA. = 375+86.74 50.00' RT
 $\Delta = 9^\circ 04' 07''$ (LT)
 D = 1° 25' 57"
 R = 4,000.00'
 T = 317.22'
 L = 633.12'
 E = 12.56'

PROP. CURVE STG1CL-NC0-2
 PI STA. = 382+13.26 50.00' RT
 $\Delta = 9^\circ 04' 07''$ (LT)
 D = 1° 25' 57"
 R = 4,000.00'
 T = 317.22'
 L = 633.12'
 E = 12.56'

NORTH CROSS OVER ELEVATION AND OFFSET DATA

C. L. STATION	STAGE 1 CENTERLINE	STAGE 2 CENTERLINE	E. O. P. LEFT OF C. L.		E. O. P. LEFT OF C. L.		BREAK POINT LEFT		E. O. P. RIGHT OF C. L.		E. O. P. RIGHT OF C. L.		BREAK POINT RIGHT	
U. S. 51	OFFSET (FT)	OFFSET (FT)	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.	OFFSET (FT)	ELEV.
374+50	-45.93	45.93	-37.92	633.56			-38.00	633.56			37.92	633.41	38.00	633.42
374+75	-44.72	44.72	-36.71	633.89			-38.00	633.92			36.71	633.73	38.00	633.76
375+00	-43.35	43.35	-35.34	634.23			-38.00	634.29			35.34	634.06	38.00	634.11
375+25	-41.83	41.83	-33.82	634.52			-38.00	634.61			33.82	634.37	38.00	634.46
375+50	-40.15	40.15	-32.13	634.82			-38.00	634.94			32.13	634.69	38.00	634.81
375+75	-38.32	38.32	-30.29	635.09			-38.00	635.26			30.29	635.00	38.00	635.16
376+00	-36.32	36.32	-28.30	635.39			-38.00	635.58			28.30	635.32	38.00	635.51
376+25	-34.17	34.17	-26.14	635.71			-38.00	635.97			26.14	635.64	38.00	635.88
376+50	-31.86	31.86	-23.83	636.06			-38.00	636.35			23.83	635.95	38.00	636.24
376+75	-29.40	29.40	-21.35	636.38	-37.44	636.70			37.44	636.59	21.35	636.26		
377+00	-26.77	26.77	-18.72	636.72	-34.82	637.05			34.82	636.90	18.72	636.57		
377+25	-23.98	23.98	-15.93	636.99	-32.04	637.32			32.04	637.18	15.93	636.86		
377+50	-21.04	21.04	-12.98	637.28	-29.10	637.61			29.10	637.47	12.98	637.15		
377+75	-17.93	17.93	-9.87	637.55	-26.00	637.87			26.00	637.75	9.87	637.43		
378+00	-14.67	14.67	-6.60	637.83	-22.74	638.15			22.74	638.03	6.60	637.70		
378+25	-11.24	11.24	-3.16	638.08	-19.32	638.40			19.32	638.29	3.16	637.96		
378+50	-7.66	7.66			-15.74	638.67			15.74	638.54			3.25	638.29
378+75	-3.91	3.91			-12.00	638.91			12.00	638.80			2.75	638.61
379+00	0.00	0.00			-10.00	639.21			10.00	639.09			3.00	638.95
379+25	3.91	-3.91			-12.00	639.60			12.00	639.47			3.25	639.29
379+50	7.66	-7.66			-15.74	640.04			15.74	639.89			3.75	639.65
379+75	11.24	-11.24	-3.16	640.14	-19.32	640.46			19.32	640.30	3.16	639.98		
380+00	14.67	-14.67	-6.60	640.57	-22.74	640.89			22.74	640.71	6.60	640.39		
380+25	17.93	-17.93	-9.87	640.95	-26.00	641.27			26.00	641.13	9.87	640.81		
380+50	21.04	-21.04	-12.98	641.34	-29.10	641.66			29.10	641.55	12.98	641.23		
380+75	23.98	-23.98	-15.93	641.71	-32.04	642.03			32.04	641.96	15.93	641.64		
381+00	26.77	-26.77	-18.72	642.09	-34.82	642.41			34.82	642.37	18.72	642.05		
381+25	29.40	-29.40	-21.35	642.50	-37.44	642.82			37.44	642.79	21.35	642.47		
381+50	31.86	-31.86	-23.83	642.92			-38.00	643.20			23.83	642.90	38.00	643.18
381+75	34.17	-34.17	-26.14	643.33			-38.00	643.57			26.14	643.31	38.00	643.55
382+00	36.32	-36.32	-28.30	643.74			-38.00	643.94			28.30	643.73	38.00	643.92
382+25	38.32	-38.32	-30.29	644.15			-38.00	644.31			30.29	644.10	38.00	644.26
382+50	40.15	-40.15	-32.13	644.57			-38.00	644.69			32.13	644.48	38.00	644.60
382+75	41.83	-41.83	-33.82	644.97			-38.00	645.06			33.82	644.85	38.00	644.94
383+00	43.35	-43.35	-35.34	645.38			-38.00	645.44			35.34	645.22	38.00	645.27
383+25	44.72	-44.72	-36.71	645.72			-38.00	645.75			36.71	645.59	38.00	645.62
383+50	45.93	-45.93	-37.92	646.07			-38.00	646.07			37.92	645.96	38.00	645.97

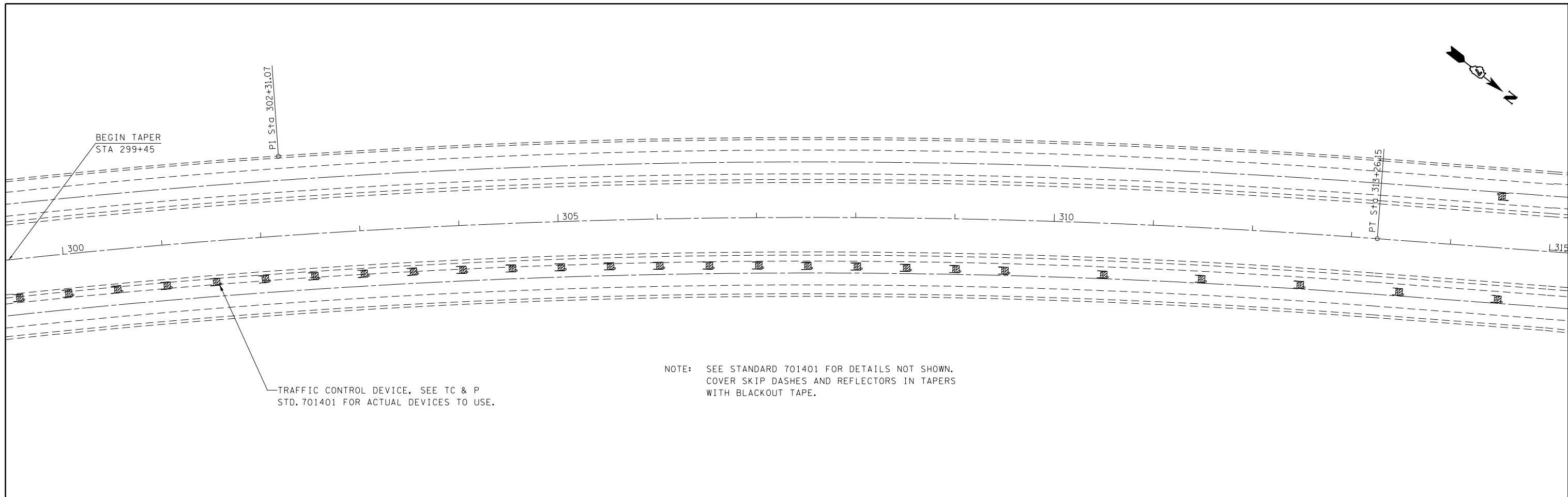
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Default	PLOT DATE = 3/20/2015	DATE -	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

NORTH MEDIAN CROSSOVER
 ELEVATIONS AND OFFSETS

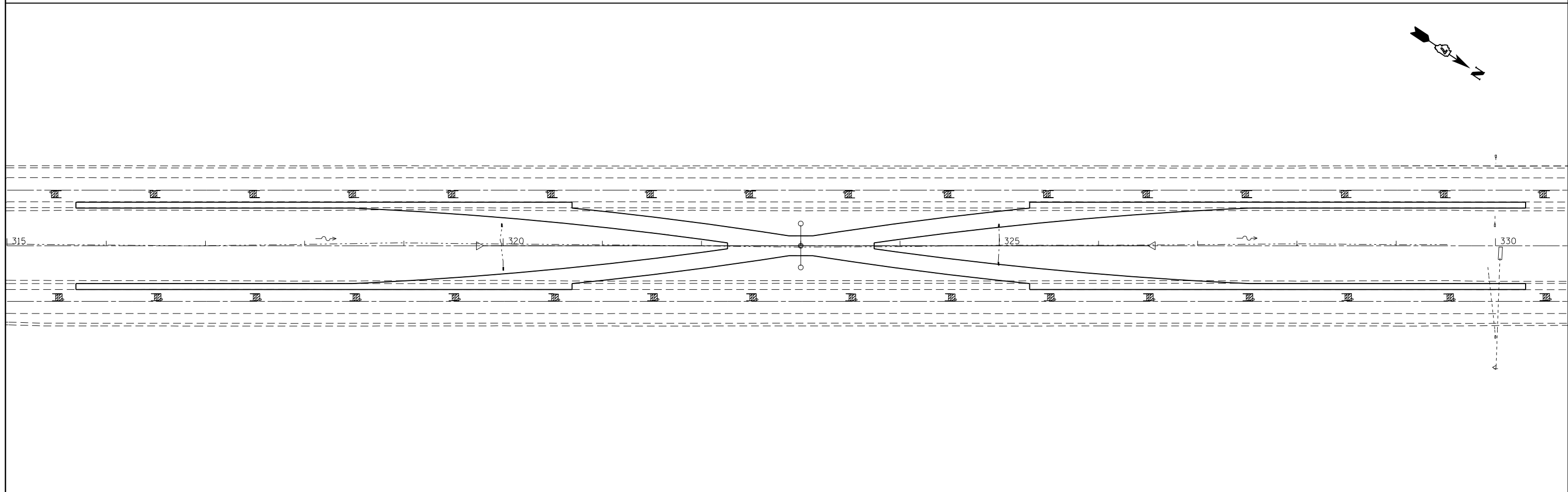
SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	15
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	





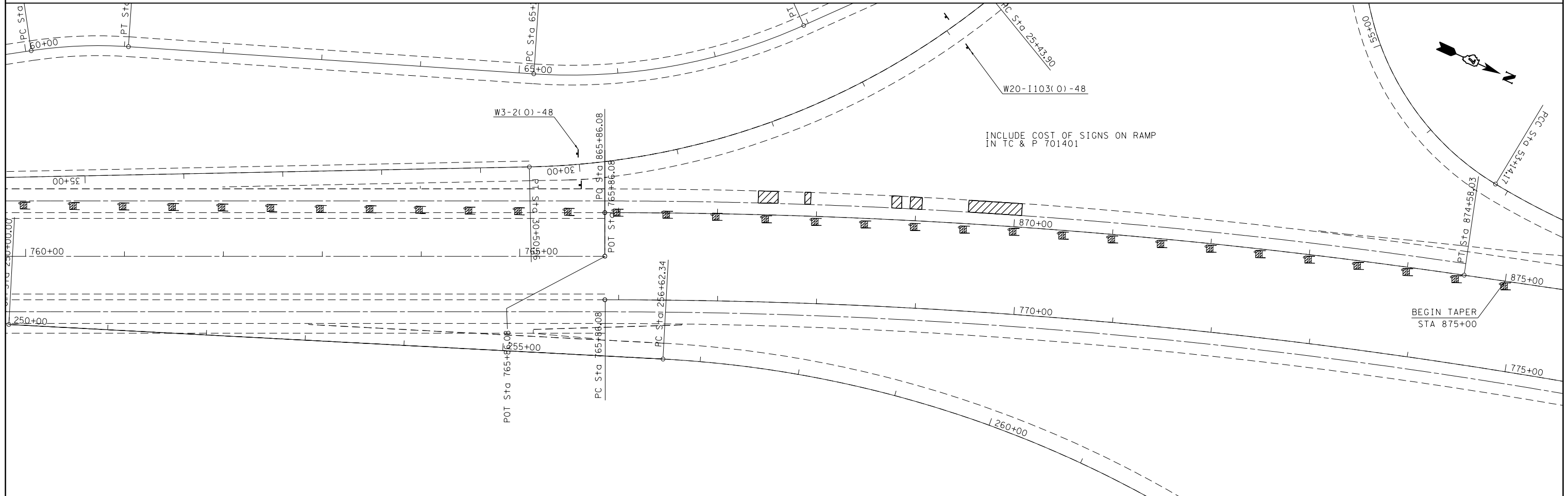
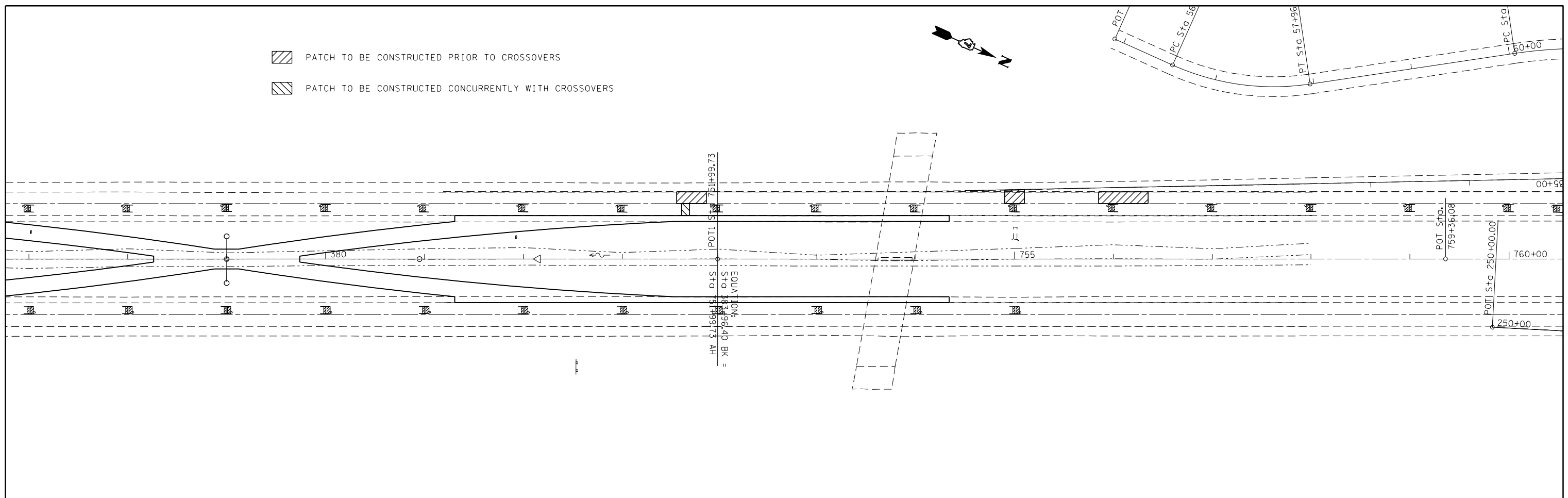
TRAFFIC CONTROL DEVICE, SEE TC & P
STD. 701401 FOR ACTUAL DEVICES TO USE.

NOTE: SEE STANDARD 701401 FOR DETAILS NOT SHOWN.
COVER SKIP DASHES AND REFLECTORS IN TAPERS
WITH BLACKOUT TAPE.



FILE NAME =	USER NAME = steffenmk	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TRAFFIC CONTROL CROSSOVER CONSTRUCTION				F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -		SCALE: SHEET OF SHEETS STA. TO STA.				CONTRACT NO. 74351				
	PLOT DATE = 3/20/2015	DATE -	REVISED -		ILLINOIS FED. AID PROJECT								

-  PATCH TO BE CONSTRUCTED PRIOR TO CROSSOVERS
-  PATCH TO BE CONSTRUCTED CONCURRENTLY WITH CROSSOVERS



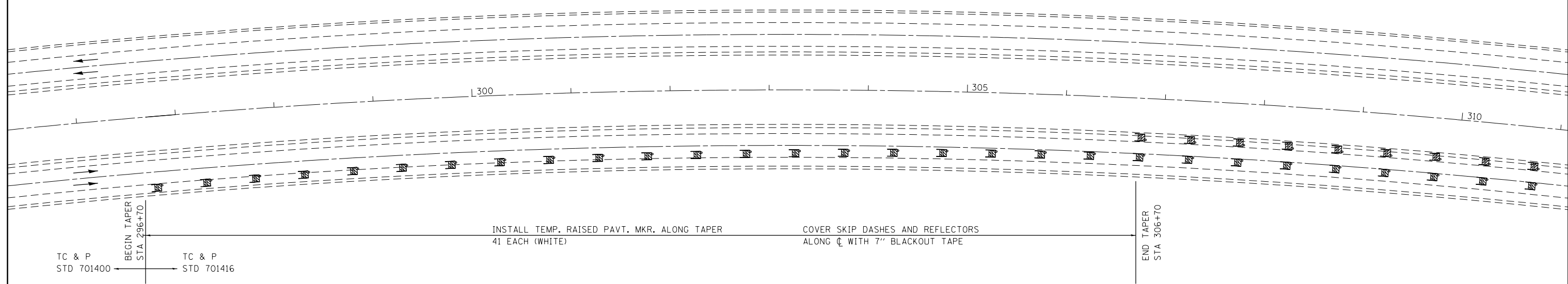
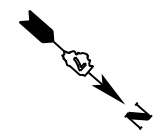
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -
	PLOT DATE = 3/23/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TRAFFIC CONTROL
CROSSOVER CONSTRUCTION**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	18
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	

SCALE: SHEET OF SHEETS STA. TO STA.



INSTALL TEMP. RAISED PAVT. MKR. ALONG TAPER
41 EACH (WHITE)

COVER SKIP DASHES AND REFLECTORS
ALONG \downarrow WITH 7" BLACKOUT TAPE

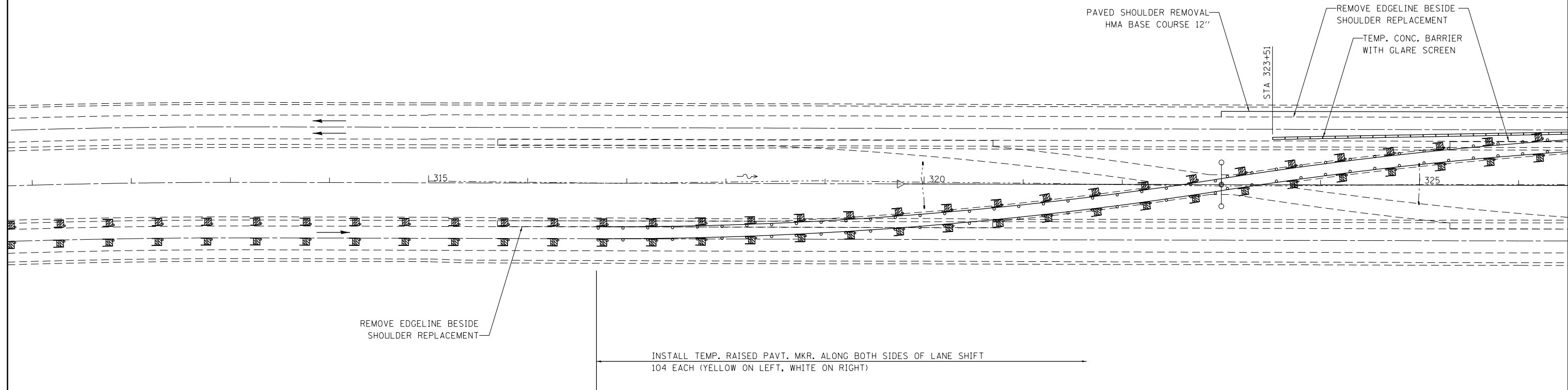
NOTE:
SEE STD 701416 FOR ACTUAL DEVICES TO BE
USED AND FOR DETAILS NOT SHOWN.

FILE NAME =	USER NAME = steffenmk	DESIGNED -	REVISED -
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -
	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

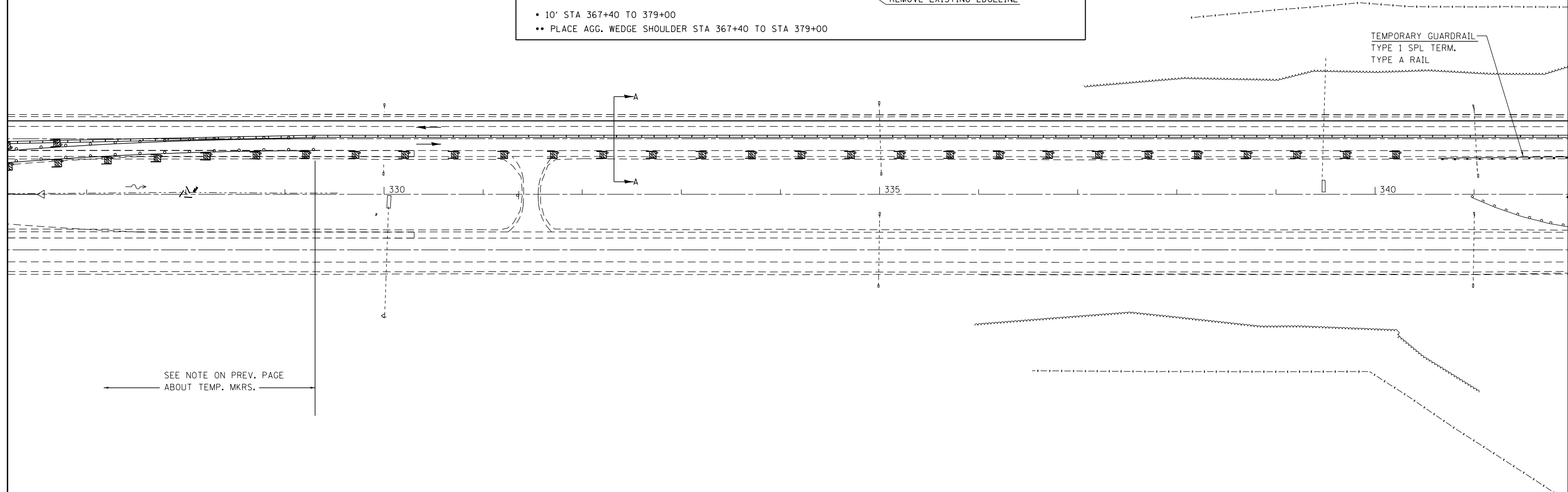
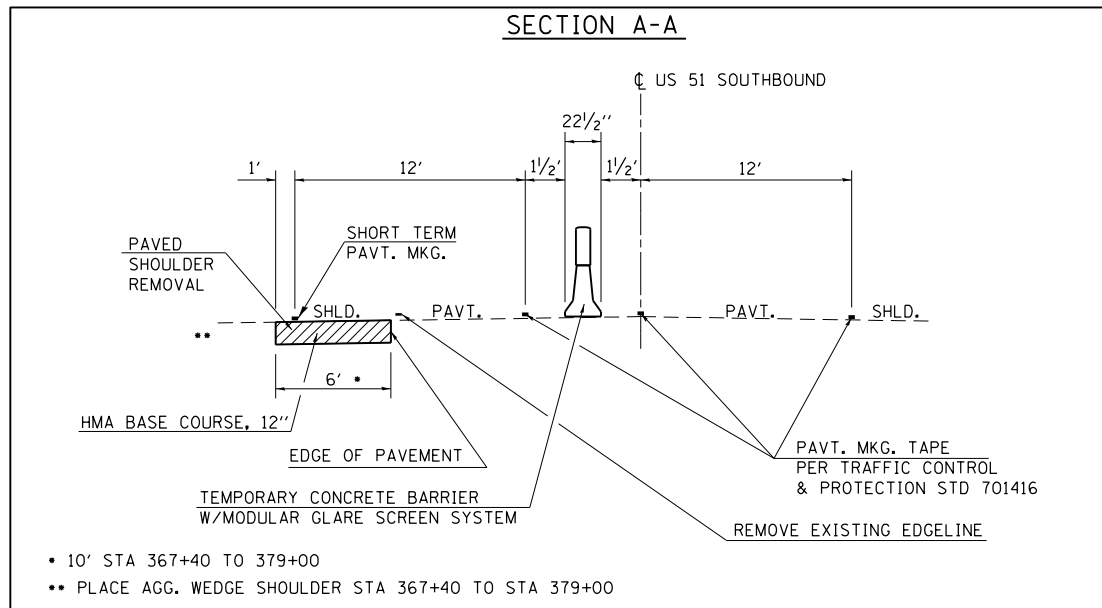
TRAFFIC CONTROL - STAGE 1			
SCALE:	SHEET	OF	SHEETS
	STA.	TO	STA.

F.A.P. R.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	19
ILLINOIS FED. AID PROJECT			CONTRACT NO.	74351

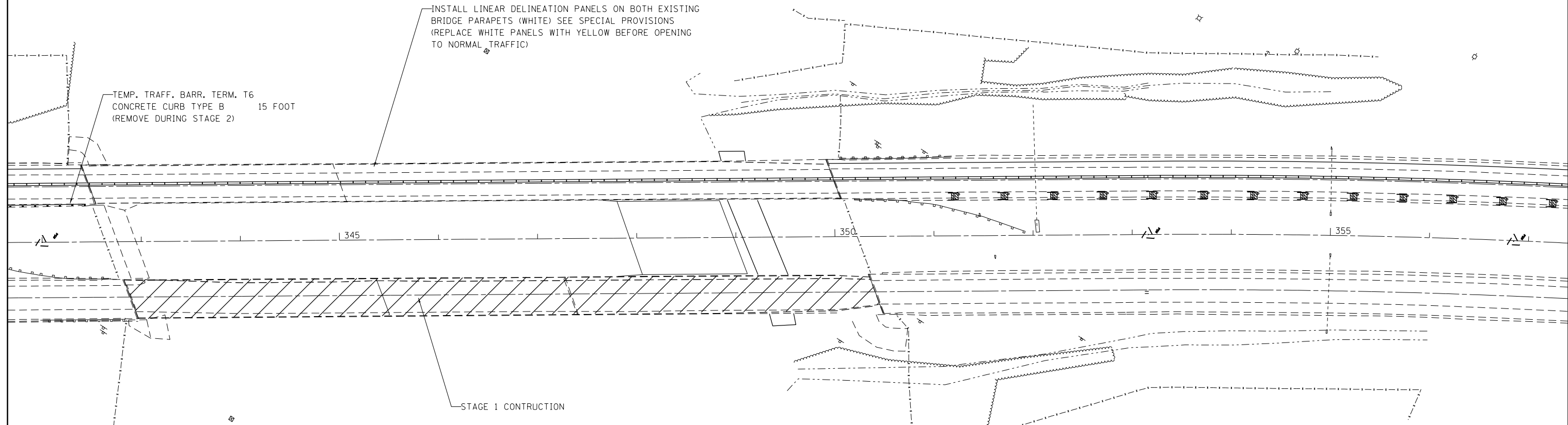


NOTE:
 BACK TO BACK REQUIREMENT FOR VERTICAL
 PANELS ON TC & P STD. 701416 WILL NOT APPLY.

FILE NAME =	USER NAME = steffenmk	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TRAFFIC CONTROL - STAGE 1				F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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Default	PLOT SCALE = 100.0000' / in.	DATE -	REVISED -		CONTRACT NO. 74351				ILLINOIS FED. AID PROJECT				
	PLOT DATE = 3/20/2015	DATE -	REVISED -		SCALE:	SHEET	OF	SHEETS	STA.	TO	STA.		



FILE NAME =	USER NAME = teasleyck	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TRAFFIC CONTROL - STAGE 1			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
pw:\IL\084EBIDINTEG.illinois.gov\PI\DOT\Documents\DOT Offices\District 7\Projects\74351\Drawings\CAD\Sheets\0774351-shr-staging.dwg	PLotted SCALE = 100.0000' / in.	CHECKED -	REVISED -					322	(58-20B-1)BR	Macon	122	21
Default	PLOT DATE = 3/23/2015	DATE -	REVISED -		SCALE:	SHEET	OF	SHEETS	STA.	TO STA.	CONTRACT NO. 74351	
										ILLINOIS FED. AID PROJECT		



STAGE 1 TEMPORARY GUARDRAIL SCHEDULE

DESCRIPTION	LOCATION	TEMPORARY STEEL PLATE BEAM GUARDRAIL, TYPE A (FOOT)	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (TANGENT) (EACH)	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 6 (EACH)	GUARDRAIL MARKERS, TYPE A (EACH)	TERMINAL MARKER - DIRECT APPLIED (EACH)
SOUTHBOUND BRIDGE SN 058-0099	SE CORNER	87.5	1	1	3	1

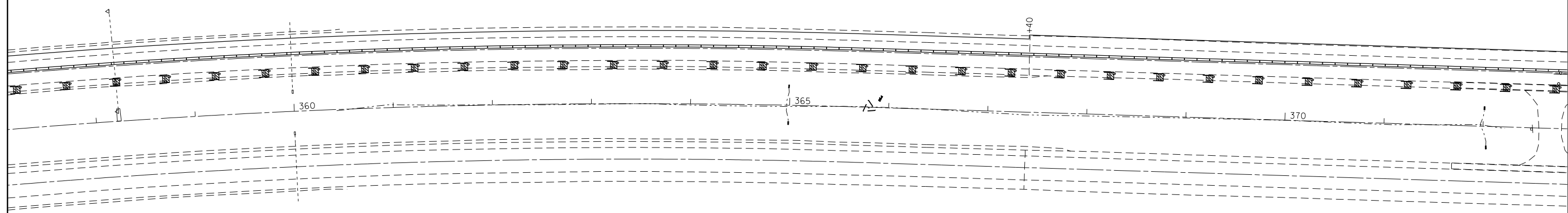
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -
	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 1

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	22
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	

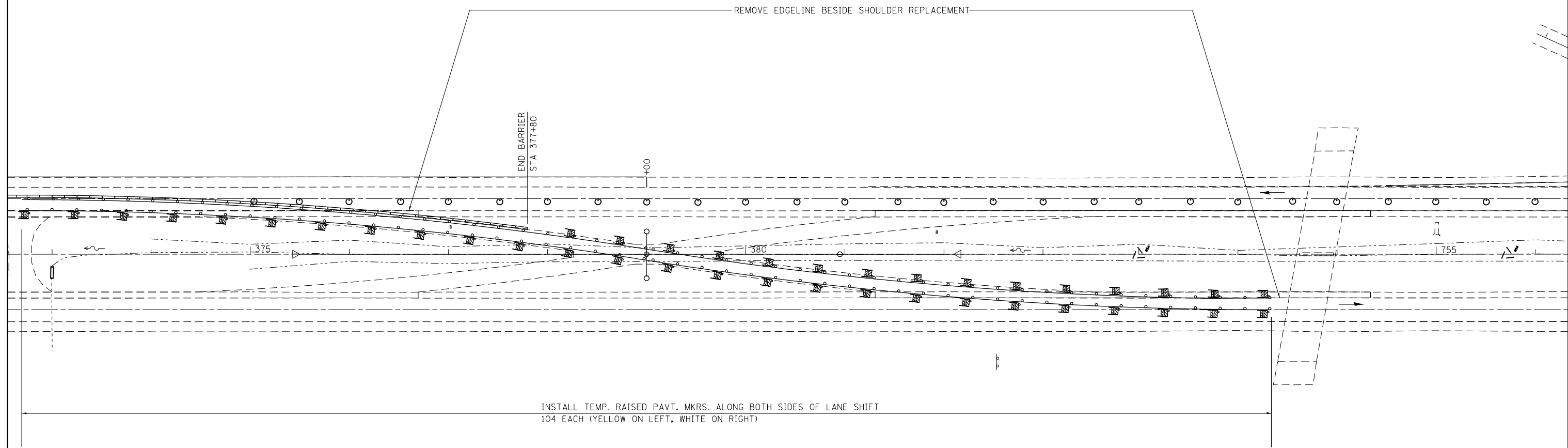
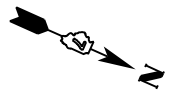


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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 1			
SCALE:	SHEET	OF	SHEETS
	STA.	TO	STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	23
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	

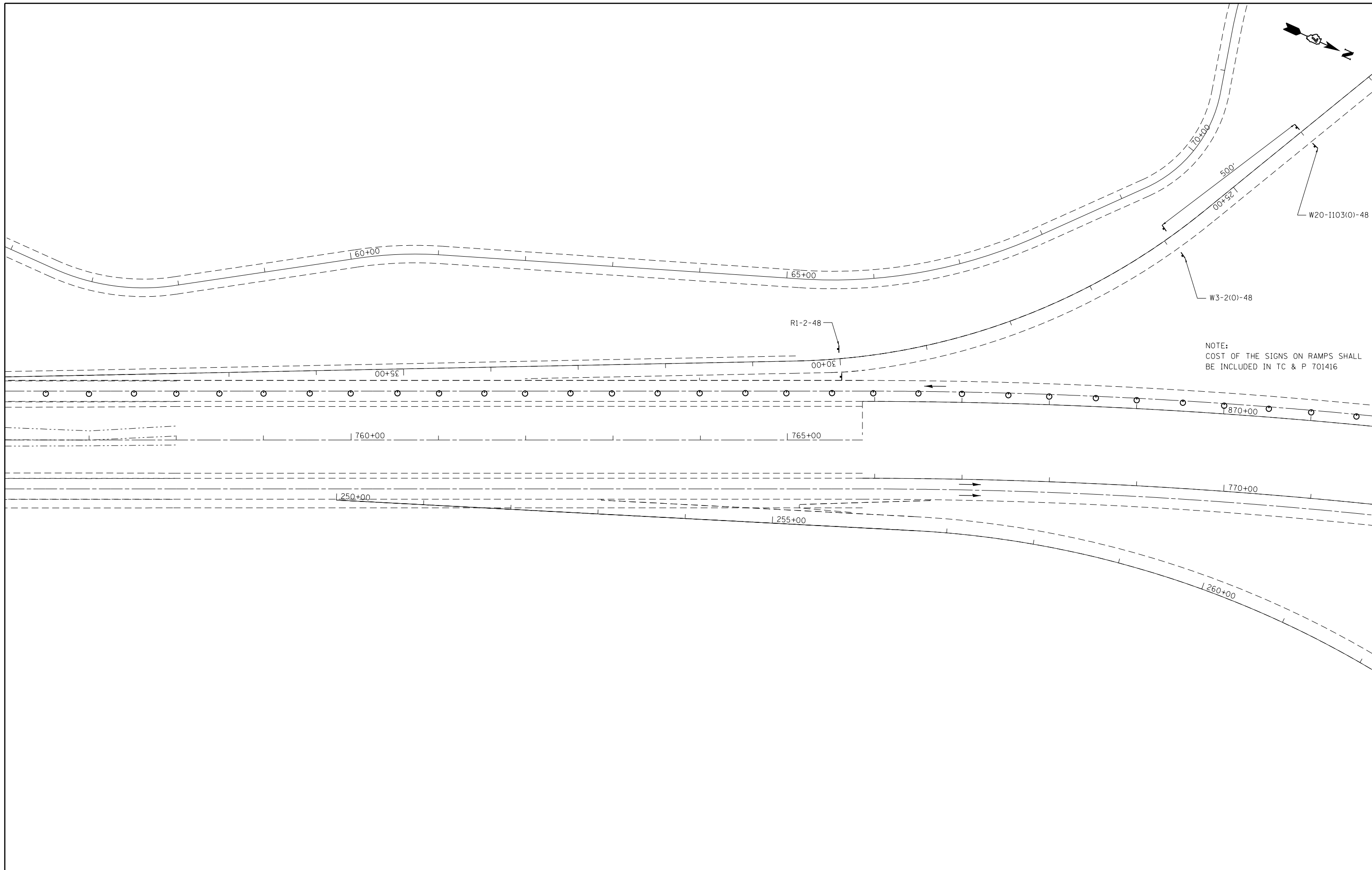


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	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 1			
SCALE:	SHEET	OF	SHEETS
	STA.		TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	24
				CONTRACT NO. 74351
ILLINOIS FED. AID PROJECT				



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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 1

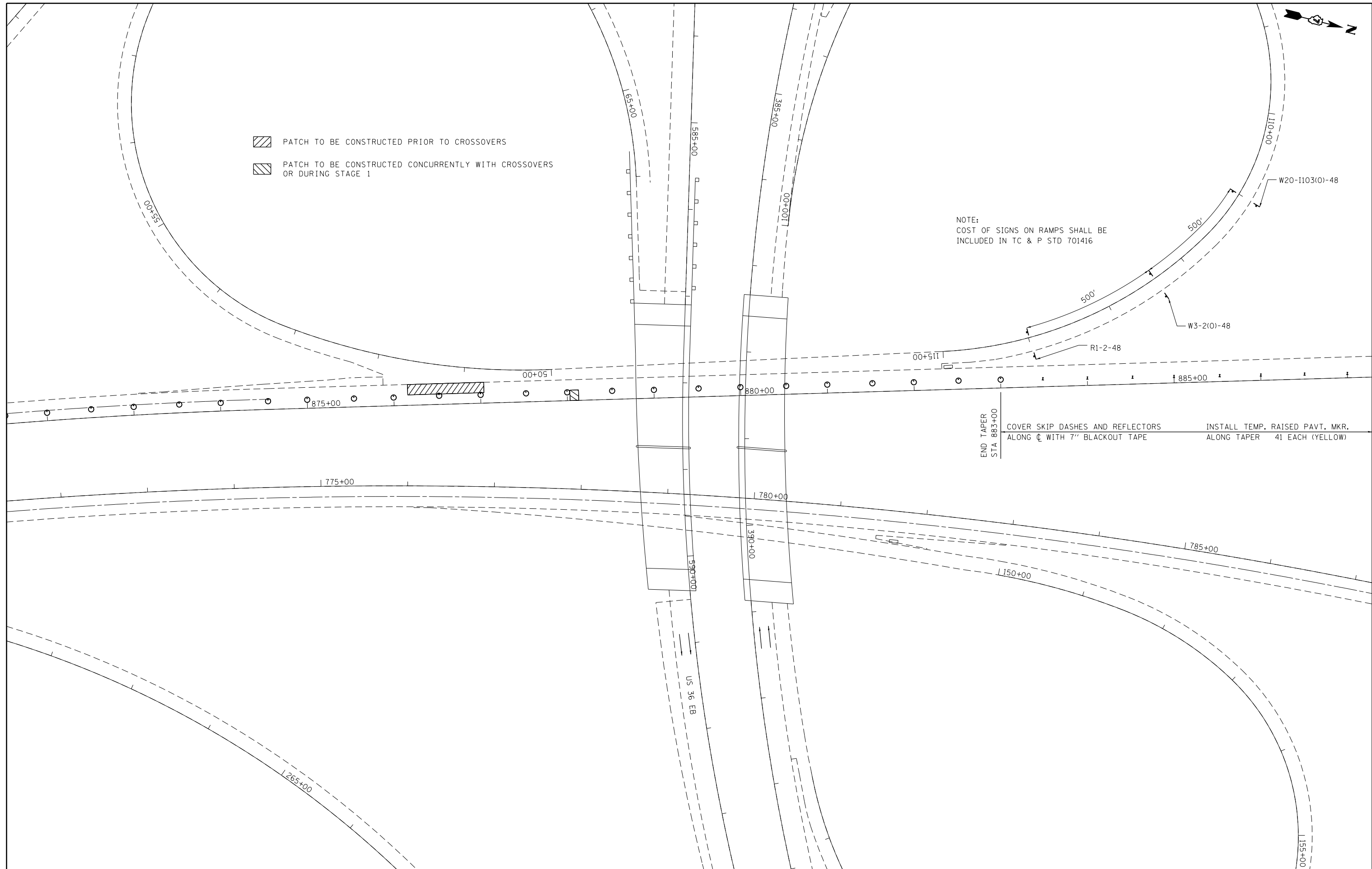
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	25
ILLINOIS FED. AID PROJECT			CONTRACT NO.	74351



- PATCH TO BE CONSTRUCTED PRIOR TO CROSSOVERS
- PATCH TO BE CONSTRUCTED CONCURRENTLY WITH CROSSOVERS OR DURING STAGE 1

NOTE:
COST OF SIGNS ON RAMPS SHALL BE INCLUDED IN TC & P STD 701416



END TAPER
STA 883+00

COVER SKIP DASHES AND REFLECTORS ALONG ϕ WITH 7" BLACKOUT TAPE
INSTALL TEMP. RAISED PAVT. MKR. ALONG TAPER 41 EACH (YELLOW)

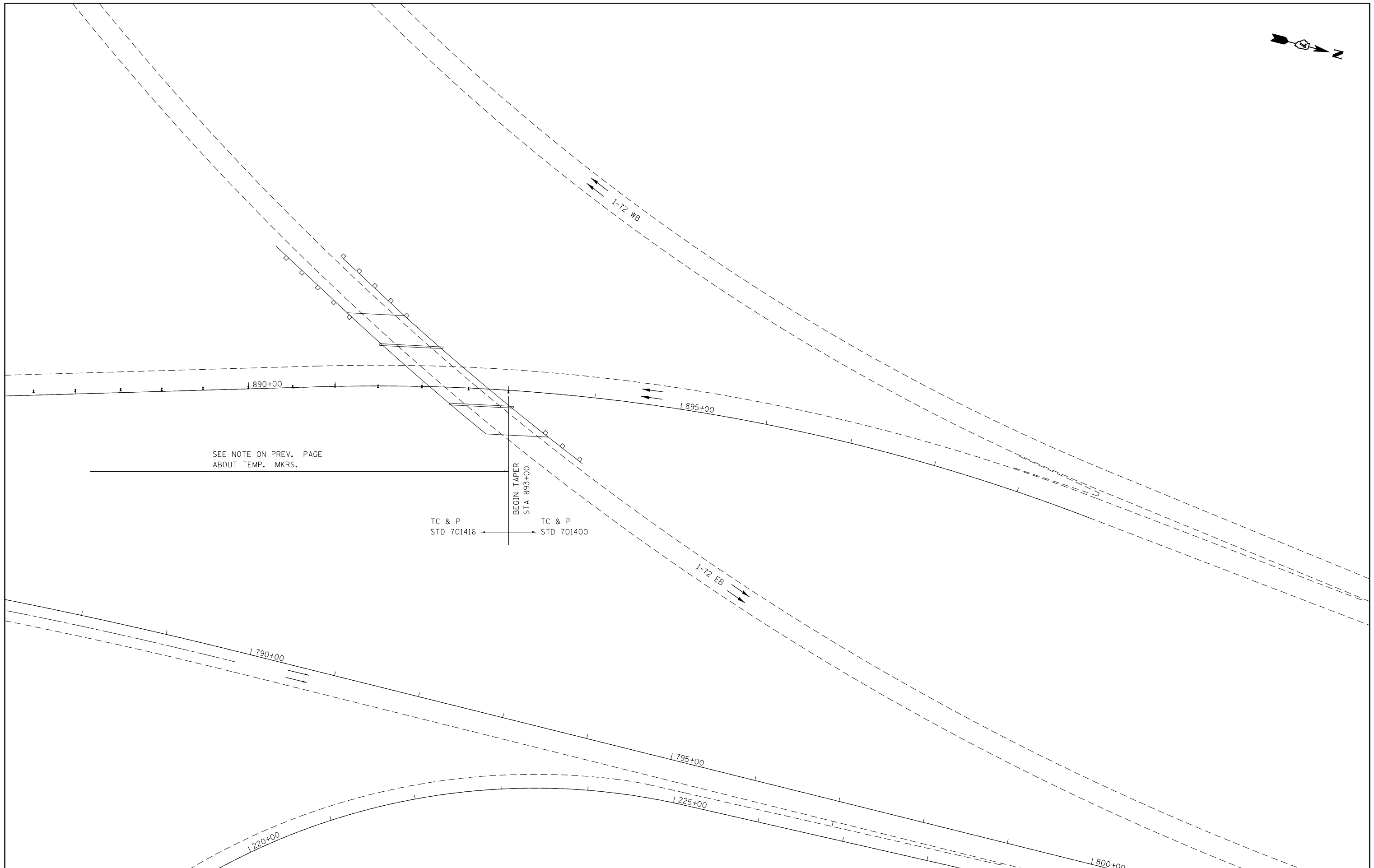
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	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 1

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	26
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	

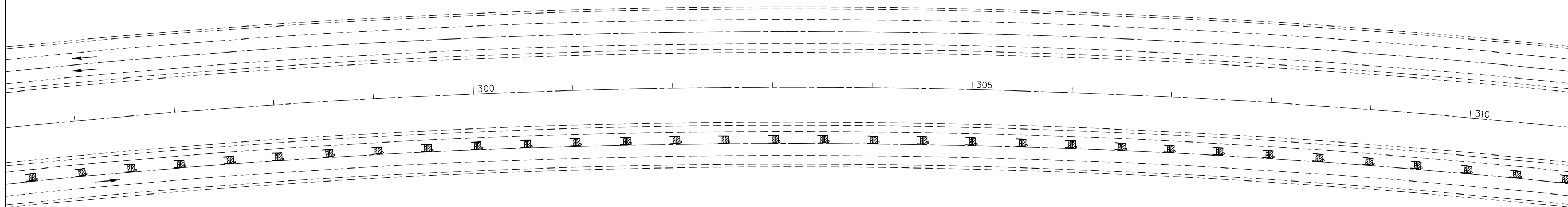
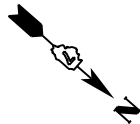


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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 1			
SCALE:	SHEET	OF SHEETS	STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	27
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	



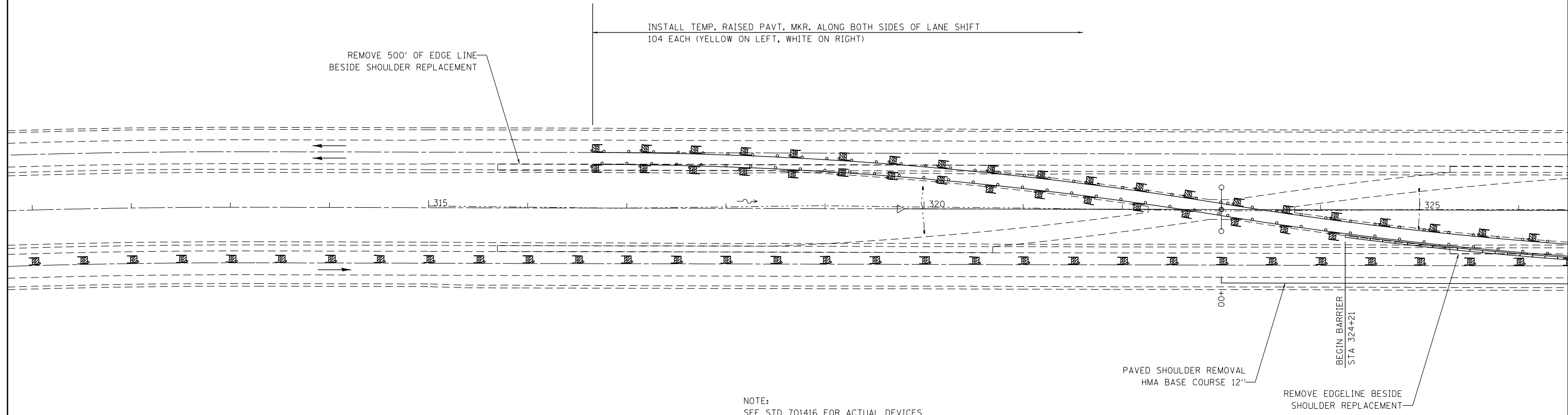
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -
	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 2

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	29
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	



NOTE:
 SEE STD 701416 FOR ACTUAL DEVICES
 TO BE USED AND FOR DETAILS NOT SHOWN.
 BACK TO BACK REQUIREMENT FOR
 VERTICAL PANELS WILL NOT APPLY.

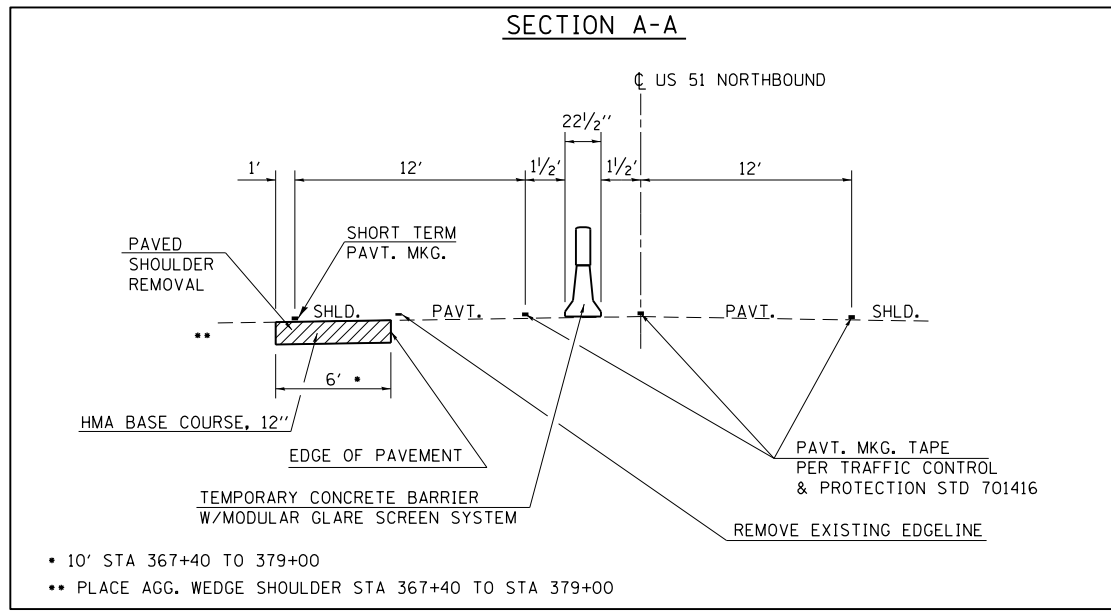
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -
	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

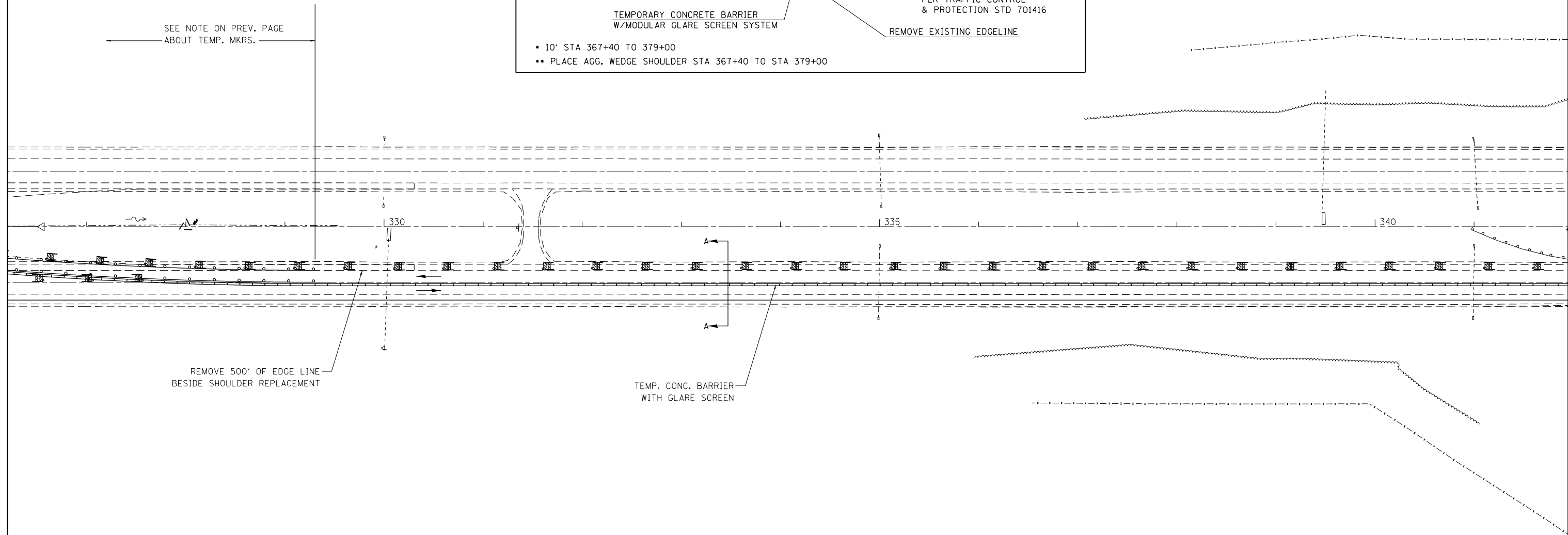
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SCALE: SHEET OF SHEETS STA. TO STA.

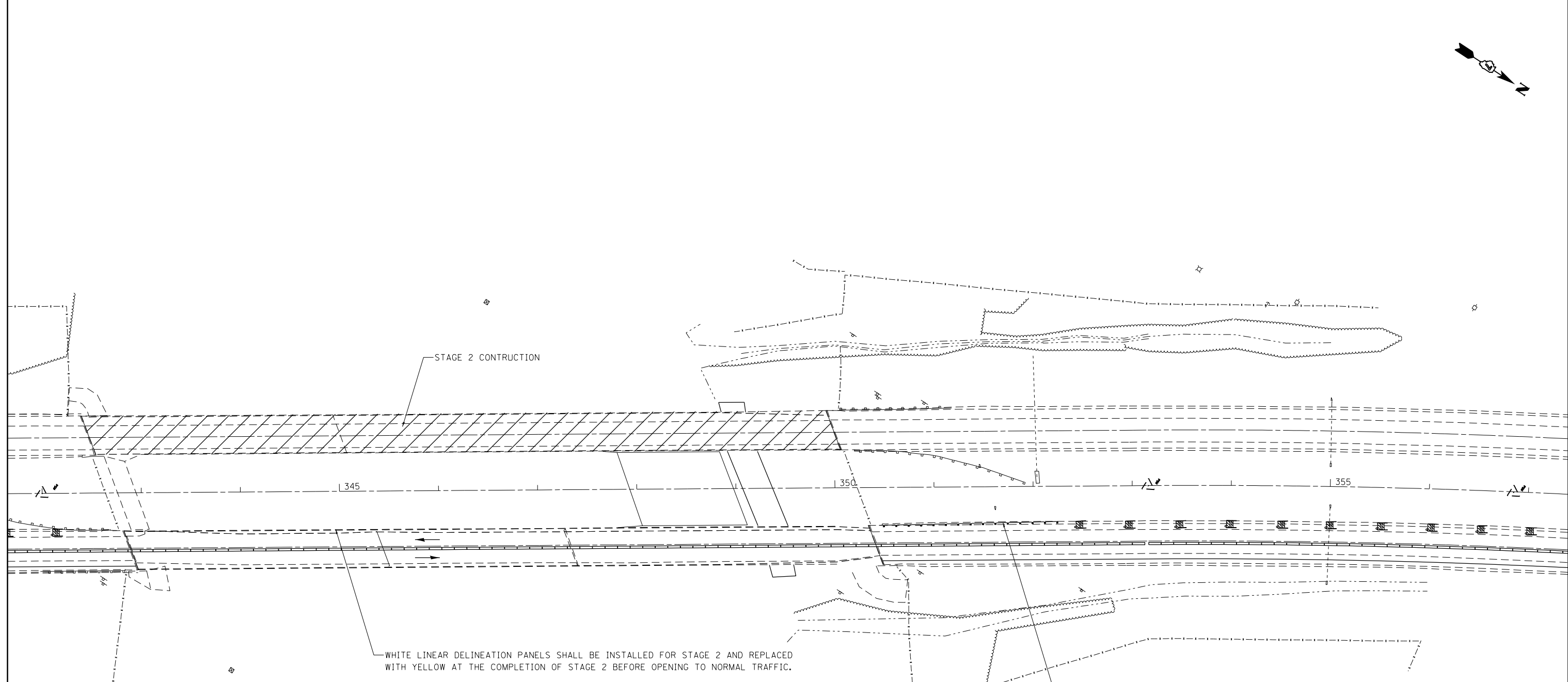
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322	(58-20B-1)BR	Macon	122	30
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	



SEE NOTE ON PREV. PAGE ABOUT TEMP. MKRS.



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					CONTRACT NO. 74351				ILLINOIS FED. AID PROJECT				
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	PLOT DATE = 3/20/2015	DATE -	REVISED -										



WHITE LINEAR DELINEATION PANELS SHALL BE INSTALLED FOR STAGE 2 AND REPLACED WITH YELLOW AT THE COMPLETION OF STAGE 2 BEFORE OPENING TO NORMAL TRAFFIC.

STAGE 2 TEMPORARY GUARDRAIL SCHEDULE

DESCRIPTION	LOCATION	TEMPORARY STEEL PLATE BEAM GUARDRAIL, TYPE A (FOOT)	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (TANGENT) (EACH)	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 6 (EACH)	GUARDRAIL MARKERS, TYPE A (EACH)	TERMINAL MARKER - DIRECT APPLIED (EACH)
NORTHBOUND BRIDGE SN 058-0098	NW CORNER	87.5	1	1	3	1

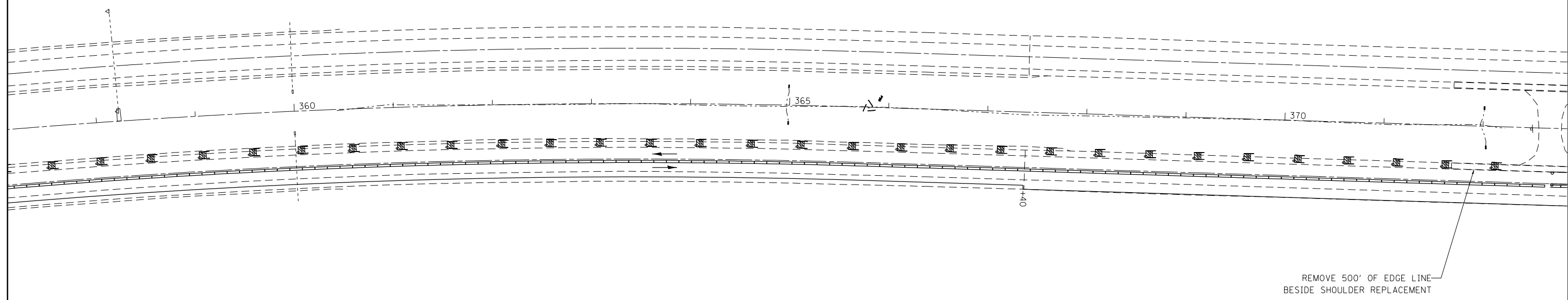
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	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 2

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	32
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	



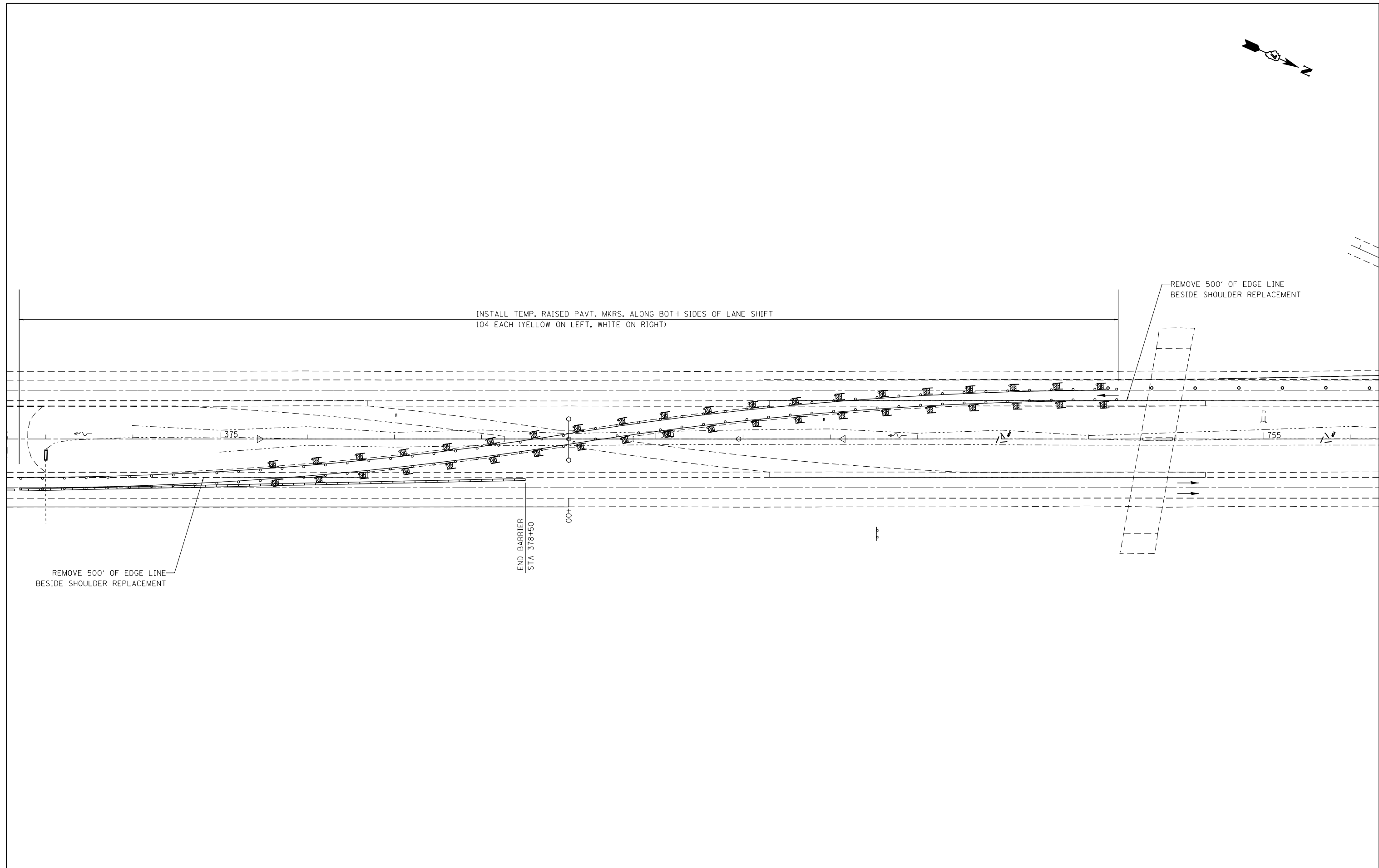
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	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 2

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	33
				CONTRACT NO. 74351
ILLINOIS FED. AID PROJECT				



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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

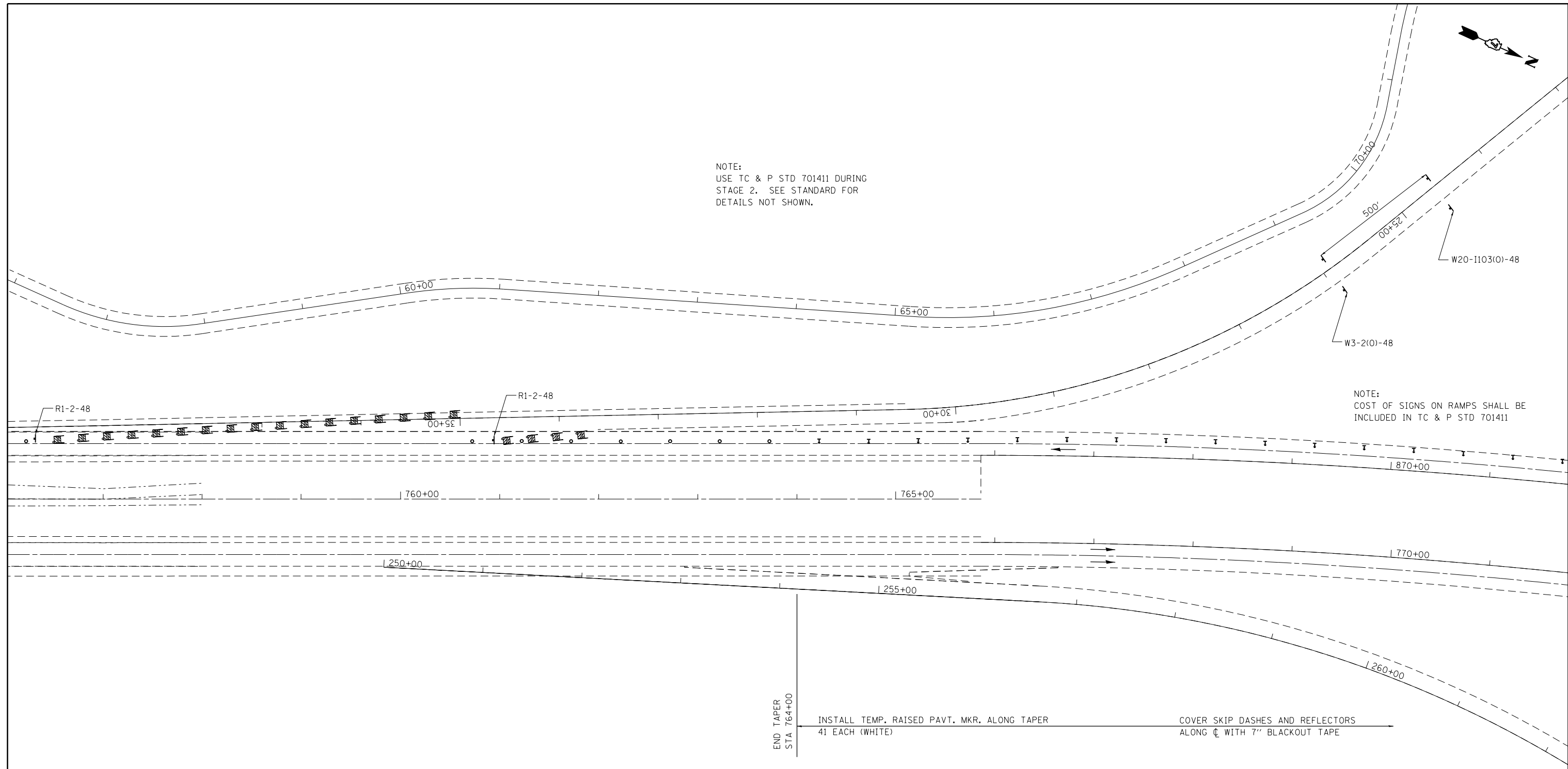
TRAFFIC CONTROL - STAGE 2			
SCALE:	SHEET	OF	SHEETS
	STA.	TO	STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	34
				CONTRACT NO. 74351
ILLINOIS FED. AID PROJECT				



NOTE:
 USE TC & P STD 701411 DURING
 STAGE 2. SEE STANDARD FOR
 DETAILS NOT SHOWN.

NOTE:
 COST OF SIGNS ON RAMPS SHALL BE
 INCLUDED IN TC & P STD 701411



END TAPER
 STA 764+00

INSTALL TEMP. RAISED PAVT. MKR. ALONG TAPER
 41 EACH (WHITE)

COVER SKIP DASHES AND REFLECTORS
 ALONG C WITH 7" BLACKOUT TAPE

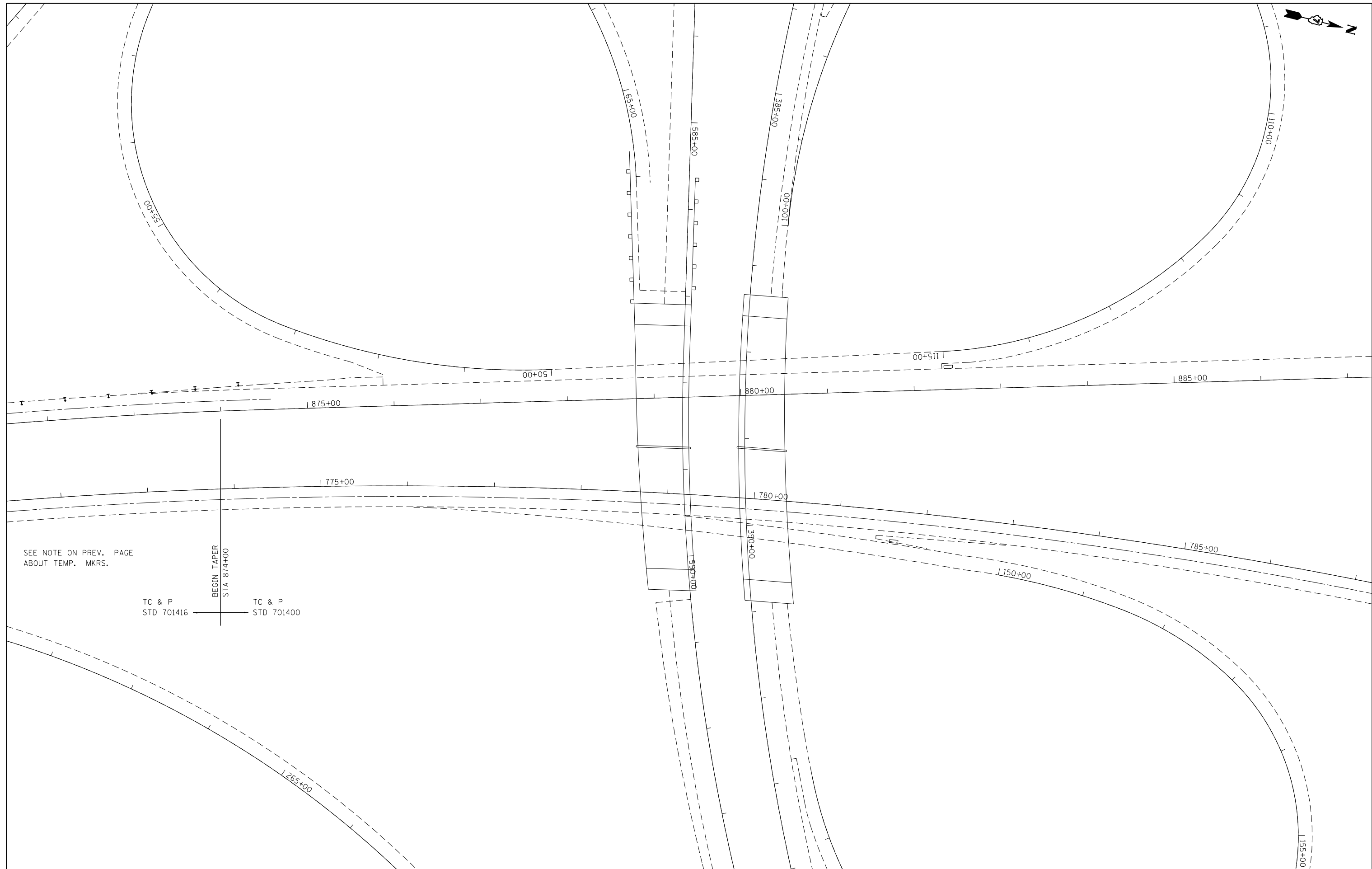
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -
	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 2

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	Macon	122	35
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	



SEE NOTE ON PREV. PAGE
ABOUT TEMP. MKRS.

TC & P
STD 701416 ← → TC & P
STD 701400

BEGIN TAPER
STA 1874+00

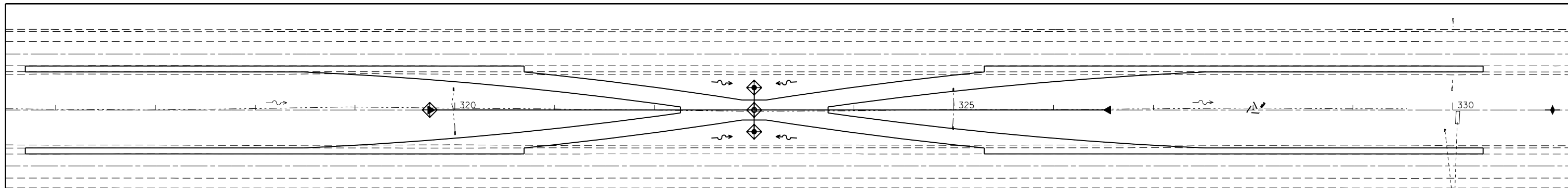
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Default	PLOT SCALE = 100.0000' / in.	CHECKED -	REVISED -
	PLOT DATE = 3/20/2015	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TRAFFIC CONTROL - STAGE 2

SCALE: SHEET OF SHEETS STA. TO STA.

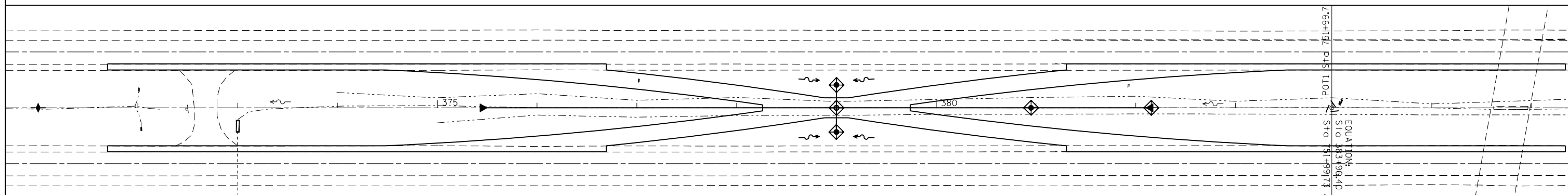
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322	(58-20B-1)BR	Macon	122	36
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	



LOCATION	TEMPORARY EROSION CONTROL SEEDING	TEMPORARY DITCH CHECKS	INLET AND PIPE PROTECTION
	POUND	FOOT	EACH
SOUTH CROSSOVER STA 331+00 LEFT, RIGHT & CENTER STA. 323+00 STA 319+80	300	30	3 1

- LEGEND**
- ◆ TEMPORARY DITCH CHECK
 - ◇ INLET AND PIPE PROTECTION

SOUTH CROSSOVER



EQUATION:
STG 333+96.4/D BK =
STG 311+99.7/AH

LOCATION	TEMPORARY EROSION CONTROL SEEDING	TEMPORARY DITCH CHECKS	INLET AND PIPE PROTECTION
	POUND	FOOT	EACH
NORTH CROSSOVER STA 371+00 LEFT, RIGHT & CENTER STA. 379+00 STA 380+95 STA 382+10	300	30	3 1 1

- LEGEND**
- ◆ TEMPORARY DITCH CHECK
 - ◇ INLET AND PIPE PROTECTION

NORTH CROSSOVER

Benchmark: Chiseled square at the Northwest Corner of S.N. 058-0099, Station 349+90.93;
Offset 85.89' left; Elevation 602.80

Existing Structures: S.N. 058-0098 (N.B.) and 058-0099 (S.B.) built in 1975 as F.A. Route 412, Section 58-20B-1, at Sta. 346+44.50. The existing dual structures are 10 span reinforced concrete decks on P.P.C. I-beams in Spans 1-7 and composite steel plate girders in positive moment regions in Spans 8-10. The superstructure is supported by pile bent abutments and solid wall piers supported by concrete piles.

The back to back abutment measures 755'-0" and the out to out of deck is 42'-0". Traffic to be maintained utilizing cross-over construction.

No Salvage

STATION 346+44.50
REBUILT 20 BY
STATE OF ILLINOIS
F.A.P. RTE. 322 SEC. (58-20B-1)BR
LOADING HL-93
STRUCTURE NO. 058-0098 (N.B.)

STATION 346+44.50
REBUILT 20 BY
STATE OF ILLINOIS
F.A.P. RTE. 322 SEC. (58-20B-1)BR
LOADING HL-93
STRUCTURE NO. 058-0099 (S.B.)

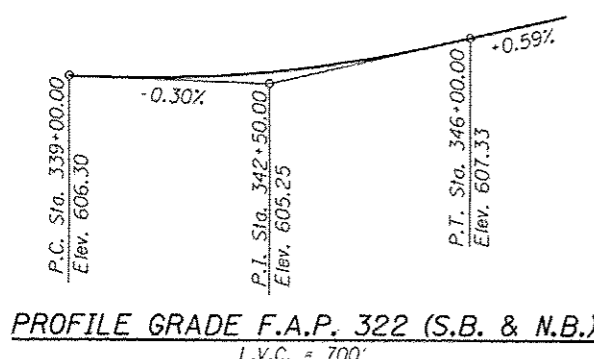
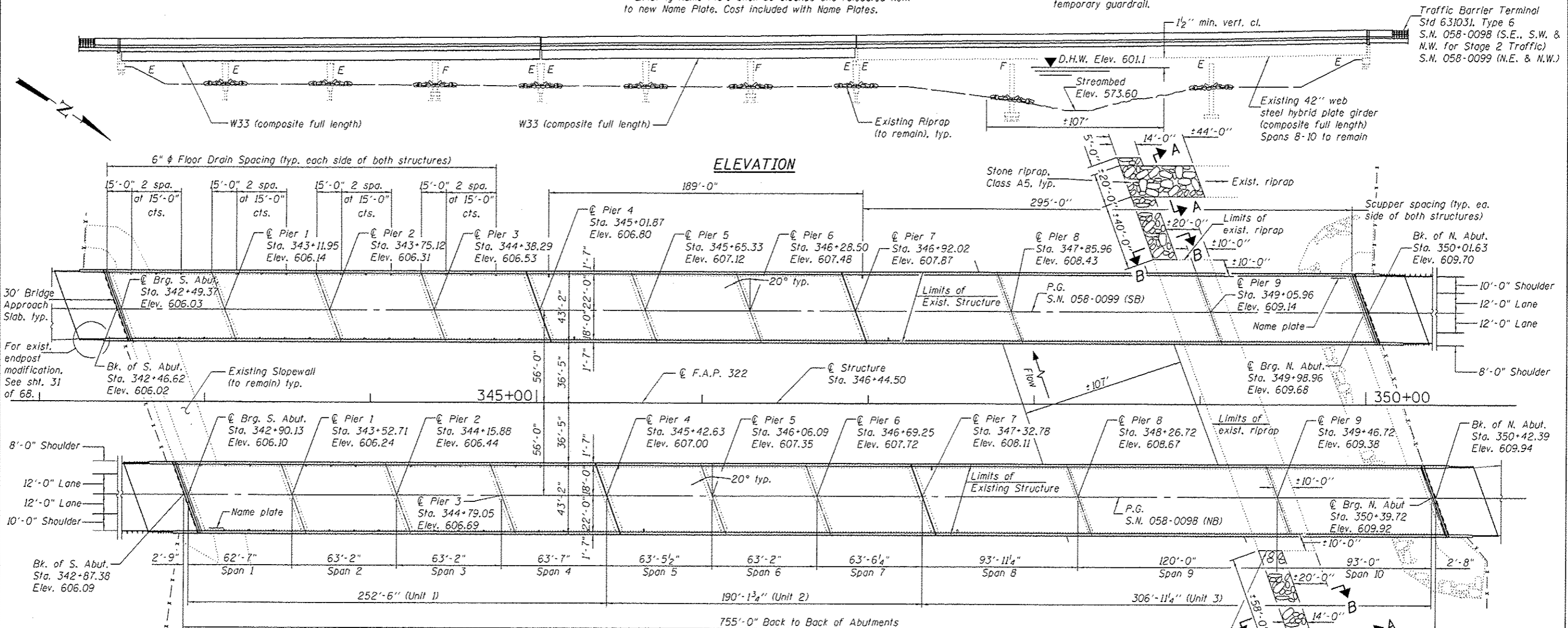
NAME PLATE
See Std. 515001

NAME PLATE
See Std. 515001

Existing Name Plate shall be cleaned and relocated next to new Name Plate. Cost included with Name Plates.

SCOPE OF WORK

- In Spans 1-4, remove the existing PPC I-Beam superstructure and replace with a continuous steel beam superstructure.
- In Spans 5-7, remove the existing PPC I-Beam superstructure and replace with a continuous steel beam superstructure.
- In Spans 8-10, remove & replace the reinforced concrete deck and make beams composite full length.
- Replace all rocker bearings with elastomeric expansion bearings at Pier 7 and the North Abutment.
- Replace existing steel diaphragms at Pier 7 and at North Abutment.
- In Spans 8-10, clean and paint existing steel superstructure.
- Perform substructure and slope wall repairs.
- Remove and replace approach slabs and modify existing wingwalls to provide for road widening.
- Modify existing southeast endpost on S.B. structure to accommodate temporary guardrail.



Note: See sheet 2 of 68 for Sections A-A and B-B.

DAVID CARL PUZEY
081-005470
SPRINGFIELD ILLINOIS
LICENSED STRUCTURAL ENGINEER
STATE OF ILLINOIS
Expires 11/30/16

SEISMIC DATA
Existing Substructure:
Seismic Performance Category (SPZ) = A
Horizontal Bedrock Acceleration Coefficient = 0.048g
Site Coefficient = 1.5

LOADING HL-93
Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS
AASHTO LRFD Bridge Design Specifications, 6th edition

DESIGN STRESSES

FIELD UNITS (New Construction)
f'c = 3,500 psi
fy = 60,000 psi (Reinforcement)
fy = 50,000 psi (M270 Grade 50)

FIELD UNITS (Existing Construction)
f'c = 3,500 psi
fy = 40,000 psi (Reinforcement)
fy = 50,000 psi (M223 Grade 50)
fy = 36,000 psi (M183 Grade 36)



GENERAL PLAN & ELEVATION
U.S. ROUTE 51 OVER SANGAMON RIVER
F.A.P. 322 SEC. (58-20B-1)BR
MACON COUNTY
STATION 346+44.50
STRUCTURE NO. 058-0098 (N.B.)
STRUCTURE NO. 058-0099 (S.B.)

DESIGNED: A.R. B.../A.R. SHEP... CHECKED: JOY / ARS / NRB DRAWN: h.t. duong CHECKED: NRB / JOY / SMO / GRA	EXAMINED: [Signature] PASSED: [Signature]	DATE: 5/6/15	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	F.A.P. RTE. 322 SECTION (58-20B-1)BR COUNTY MACON TOTAL SHEETS 122 SHEET NO. 122 CONTRACT NO. 74351
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GENERAL NOTES

Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts 1/2" φ, holes 5/8" φ, unless otherwise noted. Calculated weight of Structural Steel = 812380 lbs (M 270 Grade 50). Calculated weight of Structural Steel = 91900 lbs (M 270 Grade 36). No field welding is permitted except as specified in the contract documents. Reinforcement bars designated (E) shall be epoxy coated. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding 1/4 inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications. If the Contractor elects to use cantilever forming brackets on the exterior beams or girders, the brackets shall be placed at the same locations as required for the hardwood blocks in Article 503.06(b) of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and first interior beam at each of these additional bracket locations. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work. Concrete sealer shall be applied to the front face of abutment backwall, front and top face of new concrete at Pier 7 (N.B. lane) and top surface of Pier 4 (N.B. lane). The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. The Organic Zinc Rich Primer / Epoxy / Urethane Paint System shall be used for painting of new structural steel except where otherwise noted. The entire system shall be shop applied, with the exception of the exterior surface and the bottom of the bottom flange of fascia beams, masked off connection surfaces, field installed fasteners and damaged areas shall be touched up in the field. The color of the final finish coat for all interior steel surfaces shall be Gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Gray, Munsell No. 5B 7/1. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer. Slopewall shall be reinforced with welded wire fabric, 6" x 6" - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft. Cleaning and painting of the existing structural steel shall be as specified in the special provision for "Cleaning and Painting Existing Steel Structures". All existing steel shall be cleaned per Near White Blast Cleaning - SSPC-SP10. All existing steel shall be painted according to the requirements of Paint System 1-OZ/E/U. The color of the final finish coat for all interior steel surfaces shall be Gray, Munsell No 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Gray, Munsell No. 5B 7/1. Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8 inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings. The SSPC QP-1 and QP-2 Contractor Certifications are required for this contract.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Stone Riprap, Class A5	Sq. Yd.		489	489
Filter Fabric	Sq. Yd.		489	489
Removal of Existing Superstructures	Each	4		4
Concrete Removal	Cu. Yd.		77.8	77.8
Removal of Existing Concrete Deck	Each	2		2
Structure Excavation	Cu. Yd.		50	50
Floor Drains	Each	48		48
Concrete Structures	Cu. Yd.		115.3	115.3
Concrete Superstructure	Cu. Yd.	2313.2		2313.2
Bridge Deck Grooving	Sq. Yd.	6860		6860
Protective Coat	Sq. Yd.	8564		8564
Furnishing and Erecting Structural Steel	L. Sum	1		1
Stud Shear Connectors	Each	33504		33504
Structural Steel Removal	Pound	6120		6120
Reinforcement Bars, Epoxy Coated	Pound	640580	8580	649160
Bar Splicers	Each	184		184
Name Plates	Each	2		2
Preformed Joint Strip Seal	Foot	356.5		356.5
Elastomeric Bearing Assembly, Type I	Each	84		84
Elastomeric Bearing Assembly, Type II	Each	24		24
Anchor Bolt 1"	Each		264	264
Concrete Sealer	Sq. Ft.		1301	1301
Drainage Scupper, DS-12	Each	8		8
Jack & Remove Existing Bearings	Each		24	24
Structural Repair of Concrete (Depth ≤ than 5")	Sq. Ft.		324	324
Slopewall Repair	Sq. Yd.		49	49
Cleaning and Painting Steel Bridge, No. 1	L. Sum	1		1
Cleaning and Painting Steel Bridge, No. 2	L. Sum	1		1
Containment and Disposal of Lead Paint Cleaning Residues	L. Sum	1		1

WATERWAY INFORMATION

Drainage Area = 1040 mi² Low Grade Elev. 605.81 ft. @ Sta. 342+00

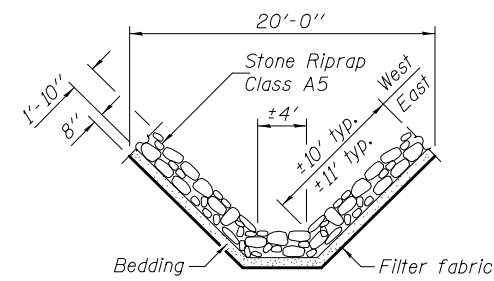
Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft.		Nat. H.W.E.	Head - Ft.		Headwater El.	
			Exist.	Prop.		Exist.	Prop.	Exist.	Prop.
	10	17200	6572	6572	598.1	1.2	1.2	599.3	599.3
Design	50	25800	8551	8551	601.1	1.4	1.4	602.5	602.5
Base	100	29400	9320	9320	602.5	1.7	1.7	604.2	604.2
Overtopping									
Max. Calc.	500	38100	9859	9859	604.6	0.4	0.4	605.0	605.0

DESIGN SCOUR ELEVATION TABLE

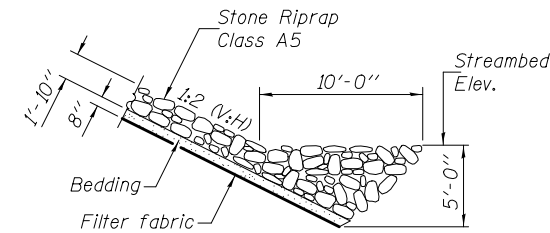
Design Scour Elevation (ft.)	Structure	S. Abut.	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Pier 8	Pier 9	N. Abut.
058-0098 NB		597.7	585.3	585.4	584.8	584.5	584.3	584.4	585.1	573.5	572.7	600.5
058-0099 SB		597.6	585.2	585.3	584.6	584.2	584.0	584.0	584.7	573.2	572.4	600.2

INDEX OF SHEETS

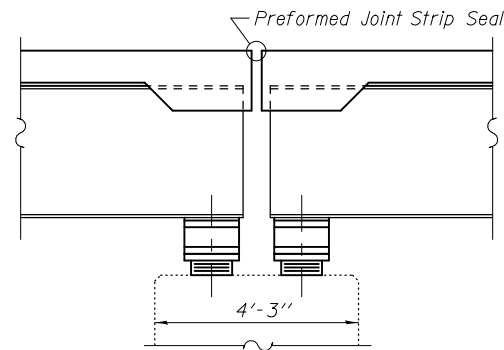
- 1 General Plan & Elevation
- 2 General Data
- 3 Top of Slab Elevations
- 4-6 Top of Slab Elevations - Spans 1 thru 4 - Unit 1 (N.B.)
- 7-8 Top of Slab Elevations - Spans 5 thru 7 - Unit 2 (N.B.)
- 9-11 Top of Slab Elevations - Spans 8 thru 10 - Unit 3 (N.B.)
- 12-14 Top of Slab Elevations - Spans 1 thru 4 - Unit 1 (S.B.)
- 15-16 Top of Slab Elevations - Spans 5 thru 7 - Unit 2 (S.B.)
- 17-19 Top of Slab Elevations - Spans 8 thru 10 - Unit 3 (S.B.)
- 20 Top of South Approach Slab Elevations (N.B.)
- 21 Top of North Approach Slab Elevations (N.B.)
- 22 Top of South Approach Slab Elevations (S.B.)
- 23 Top of North Approach Slab Elevations (S.B.)
- 24 Superstructure - Spans 1 thru 4 - Unit 1 (N.B.)
- 25 Superstructure - Spans 5 thru 7 - Unit 2 (N.B.)
- 26 Superstructure - Spans 8 thru 10 - Unit 3 (N.B.)
- 27 Superstructure - Spans 1 thru 4 - Unit 1 (S.B.)
- 28 Superstructure - Spans 5 thru 7 - Unit 2 (S.B.)
- 29 Superstructure - Spans 8 thru 10 - Unit 3 (S.B.)
- 30-31 Superstructure Details - Spans 1 thru 4 - Unit 1
- 32-33 Superstructure Details - Spans 5 thru 7 - Unit 2
- 34-35 Superstructure Details - Spans 8 thru 10 - Unit 3
- 36-38 Bridge Approach Slab Details
- 39 Preformed Joint Strip Seal
- 40 Drainage Scupper DS-12
- 41 Structural Steel - Spans 1 thru 4 - Unit 1
- 42 Structural Steel Details - Spans 1 thru 4 - Unit 1
- 43 Structural Steel - Spans 5 thru 7 - Unit 2
- 44 Structural Steel Details - Spans 5 thru 7 - Unit 2
- 45 Structural Steel - Spans 8 thru 10 - Unit 3
- 46-48 Structural Steel Details - Spans 8 thru 10 - Unit 3
- 49-51 Bearing Details - Unit 1
- 52-54 Bearing Details - Unit 2
- 55-56 Bearing Details - Unit 3
- 57 Concrete Removal At Abutments (N.B.)
- 58 Concrete Removal At Abutments (S.B.)
- 59 South Abutment (N.B.)
- 60 North Abutment (N.B.)
- 61 South Abutment (S.B.)
- 62 North Abutment (S.B.)
- 63 Pier 4 (N.B.)
- 64 Pier 7 (N.B.)
- 65 Slopewall Repair
- 66-66C Structural Repair of Concrete
- 67 Bar Splicer Assembly & Mechanical Splicer Details
- 68 Concrete Parapet Slipforming Option



**SECTION A-A
DITCH CROSS SECTION**

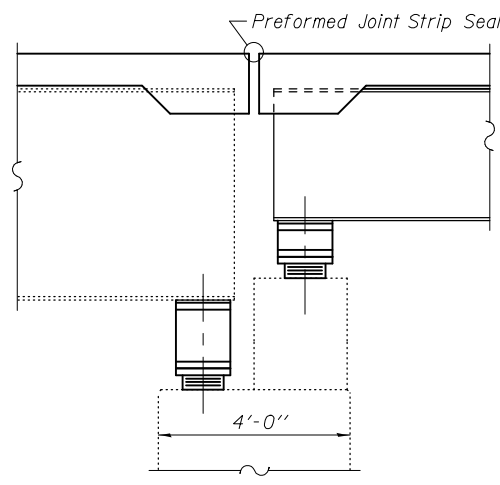


SECTION B-B



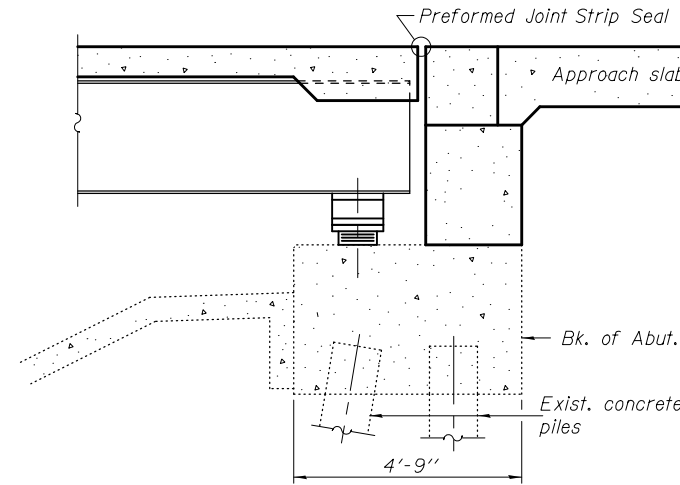
SECTION THRU PIER 4

(Horiz. dim. @ Rt. L's - Looking East)



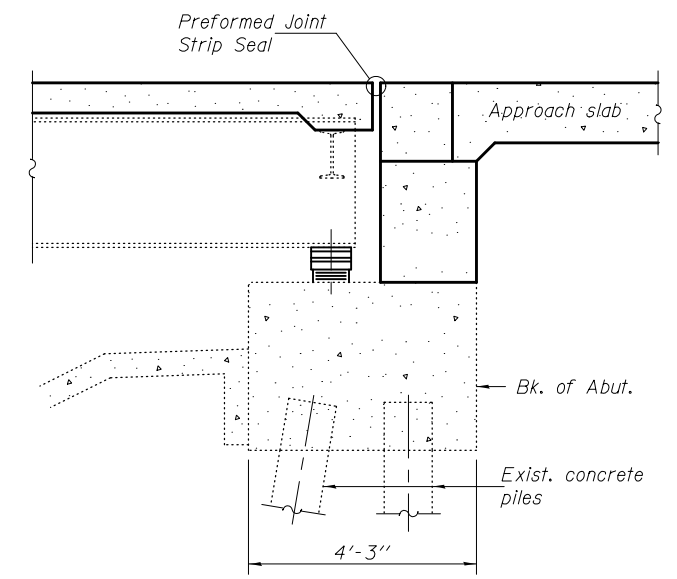
SECTION THRU PIER 7

(Horiz. dim. @ Rt. L's - Looking East)



SEC. THRU SOUTH ABUT.

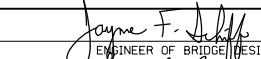

(Horiz. dim. @ Rt. L's - Looking East)



SEC. THRU NORTH ABUT.

(Horiz. dim. @ Rt. L's - Looking West)

DESIGNED - N. R. Barnett/A. R. Shebib	EXAMINED	DATE - MAY 6, 2015
CHECKED - JOV/ARS/NRB	PASSED	REVISOR
DRAWN - h.t. duong		REVISOR
CHECKED - NRB/JOV/JMO/GRA		

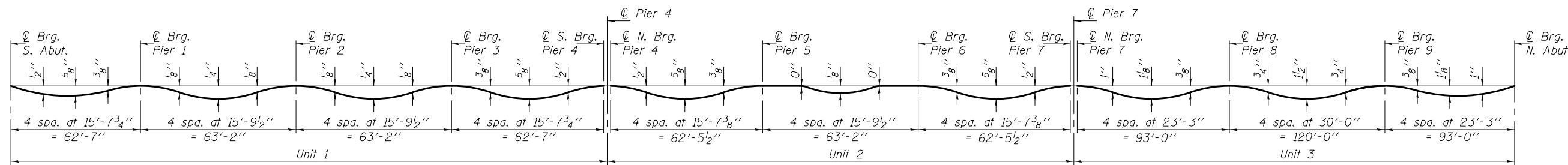

 ENGINEER OF BRIDGE DESIGN

 ACTING ENGINEER OF BRIDGES AND STRUCTURES

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**GENERAL DATA
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

SHEET NO. 2 OF 68 SHEETS

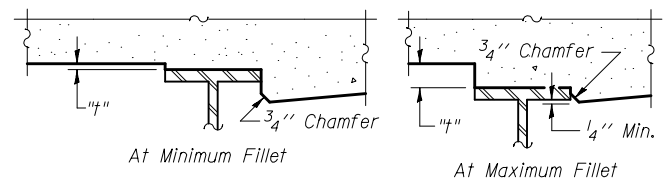
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	39
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				



DEAD LOAD DEFLECTION DIAGRAM

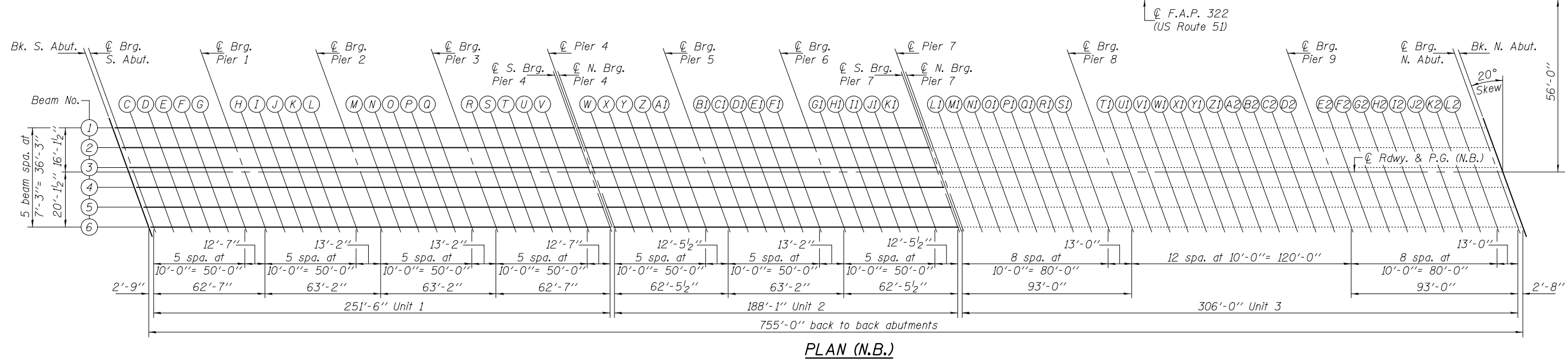
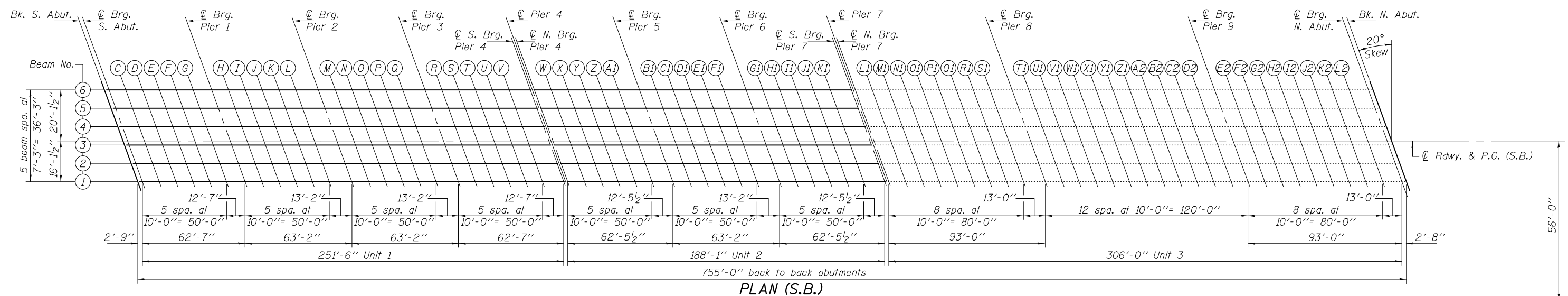
(Includes weight of concrete only.)

Note: The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown on sheets 4-19 of 68.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on sheets 4-19 of 68, minus slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS



DESIGNED - N. R. Barnett/A. R. Shebib	EXAMINED - <i>Joey F. [Signature]</i>	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TOP OF SLAB ELEVATIONS STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 40	
CHECKED - JOV/ARS/NRB	PASSED - <i>Carl [Signature]</i>	REVISOR			CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT		
DRAWN - h.t. duong	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISOR			SHEET NO. 3 OF 68 SHEETS					
CHECKED - NRB/JOV/JMO/GRA		REVISOR								

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34281.51	-16.13	605.81	605.81
☉ Brg. S. Abut.	34284.26	-16.13	605.81	605.81
C	34294.26	-16.13	605.83	605.86
D	34304.26	-16.13	605.85	605.90
E	34314.26	-16.13	605.88	605.92
F	34324.26	-16.13	605.90	605.94
G	34334.26	-16.13	605.92	605.94
☉ Brg. Pier 1	34346.84	-16.13	605.96	605.96
H	34356.84	-16.13	605.98	605.99
I	34366.84	-16.13	606.01	606.02
J	34376.84	-16.13	606.04	606.06
K	34386.84	-16.13	606.07	606.09
L	34396.84	-16.13	606.11	606.11
☉ Brg. Pier 2	34410.01	-16.13	606.15	606.15
M	34420.01	-16.13	606.19	606.19
N	34430.01	-16.13	606.22	606.23
O	34440.01	-16.13	606.26	606.28
P	34450.01	-16.13	606.30	606.31
Q	34460.01	-16.13	606.34	606.34
☉ Brg. Pier 3	34473.18	-16.13	606.40	606.40
R	34483.18	-16.13	606.44	606.46
S	34493.18	-16.13	606.48	606.52
T	34503.18	-16.13	606.53	606.58
U	34513.18	-16.13	606.58	606.62
V	34523.18	-16.13	606.63	606.66
☉ S. Brg. Pier 4	34535.76	-16.13	606.69	606.69
☉ Pier 4	34536.76	-16.13	606.69	606.69

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34284.15	-8.88	605.95	605.95
☉ Brg. S. Abut.	34286.90	-8.88	605.95	605.95
C	34296.90	-8.88	605.97	606.00
D	34306.90	-8.88	605.99	606.04
E	34316.90	-8.88	606.02	606.06
F	34326.90	-8.88	606.04	606.08
G	34336.90	-8.88	606.06	606.09
☉ Brg. Pier 1	34349.48	-8.88	606.10	606.10
H	34359.48	-8.88	606.13	606.13
I	34369.48	-8.88	606.15	606.16
J	34379.48	-8.88	606.18	606.20
K	34389.48	-8.88	606.22	606.23
L	34399.48	-8.88	606.25	606.26
☉ Brg. Pier 2	34412.65	-8.88	606.29	606.29
M	34422.65	-8.88	606.33	606.34
N	34432.65	-8.88	606.37	606.38
O	34442.65	-8.88	606.41	606.42
P	34452.65	-8.88	606.45	606.46
Q	34462.65	-8.88	606.49	606.49
☉ Brg. Pier 3	34475.82	-8.88	606.54	606.54
R	34485.82	-8.88	606.59	606.60
S	34495.82	-8.88	606.63	606.66
T	34505.82	-8.88	606.68	606.73
U	34515.82	-8.88	606.72	606.77
V	34525.82	-8.88	606.77	606.81
☉ S. Brg. Pier 4	34538.40	-8.88	606.84	606.84
☉ Pier 4	34539.40	-8.88	606.84	606.84

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34286.79	-1.63	606.07	606.07
☉ Brg. S. Abut.	34289.54	-1.63	606.07	606.07
C	34299.54	-1.63	606.09	606.12
D	34309.54	-1.63	606.11	606.16
E	34319.54	-1.63	606.13	606.18
F	34329.54	-1.63	606.16	606.20
G	34339.54	-1.63	606.18	606.21
☉ Brg. Pier 1	34352.12	-1.63	606.22	606.22
H	34362.12	-1.63	606.25	606.25
I	34372.12	-1.63	606.28	606.28
J	34382.12	-1.63	606.31	606.32
K	34392.12	-1.63	606.34	606.35
L	34402.12	-1.63	606.37	606.38
☉ Brg. Pier 2	34415.29	-1.63	606.42	606.42
M	34425.29	-1.63	606.45	606.46
N	34435.29	-1.63	606.49	606.50
O	34445.29	-1.63	606.53	606.54
P	34455.29	-1.63	606.57	606.58
Q	34465.29	-1.63	606.61	606.61
☉ Brg. Pier 3	34478.45	-1.63	606.67	606.67
R	34488.45	-1.63	606.71	606.73
S	34498.45	-1.63	606.76	606.79
T	34508.45	-1.63	606.80	606.85
U	34518.45	-1.63	606.85	606.90
V	34528.45	-1.63	606.90	606.93
☉ S. Brg. Pier 4	34541.04	-1.63	606.96	606.96
☉ Pier 4	34542.04	-1.63	606.97	606.97

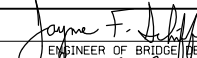

☉ ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34287.38	0.00	606.09	606.09
☉ Brg. S. Abut.	34290.13	0.00	606.10	606.10
C	34300.13	0.00	606.12	606.14
D	34310.13	0.00	606.14	606.18
E	34320.13	0.00	606.16	606.21
F	34330.13	0.00	606.19	606.22
G	34340.13	0.00	606.21	606.23
☉ Brg. Pier 1	34352.71	0.00	606.24	606.24
H	34362.71	0.00	606.27	606.28
I	34372.71	0.00	606.30	606.31
J	34382.71	0.00	606.33	606.35
K	34392.71	0.00	606.37	606.38
L	34402.71	0.00	606.40	606.41
☉ Brg. Pier 2	34415.88	0.00	606.44	606.44
M	34425.88	0.00	606.48	606.49
N	34435.88	0.00	606.52	606.53
O	34445.88	0.00	606.56	606.57
P	34455.88	0.00	606.60	606.61
Q	34465.88	0.00	606.64	606.64
☉ Brg. Pier 3	34479.05	0.00	606.69	606.69
R	34489.05	0.00	606.74	606.76
S	34499.05	0.00	606.78	606.82
T	34509.05	0.00	606.83	606.88
U	34519.05	0.00	606.88	606.92
V	34529.05	0.00	606.93	606.96
☉ S. Brg. Pier 4	34541.63	0.00	606.99	606.99
☉ Pier 4	34542.63	0.00	607.00	607.00

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34289.43	5.63	606.01	606.01
☉ Brg. S. Abut.	34292.18	5.63	606.01	606.01
C	34302.18	5.63	606.03	606.06
D	34312.18	5.63	606.06	606.10
E	34322.18	5.63	606.08	606.13
F	34332.18	5.63	606.10	606.14
G	34342.18	5.63	606.13	606.15
☉ Brg. Pier 1	34354.76	5.63	606.16	606.16
H	34364.76	5.63	606.19	606.19
I	34374.76	5.63	606.22	606.23
J	34384.76	5.63	606.25	606.27
K	34394.76	5.63	606.28	606.30
L	34404.76	5.63	606.32	606.33
☉ Brg. Pier 2	34417.93	5.63	606.36	606.36
M	34427.93	5.63	606.40	606.41
N	34437.93	5.63	606.44	606.45
O	34447.93	5.63	606.48	606.49
P	34457.93	5.63	606.52	606.53
Q	34467.93	5.63	606.56	606.56
☉ Brg. Pier 3	34481.09	5.63	606.62	606.62
R	34491.09	5.63	606.66	606.68
S	34501.09	5.63	606.71	606.74
T	34511.09	5.63	606.75	606.80
U	34521.09	5.63	606.80	606.85
V	34531.09	5.63	606.85	606.88
☉ S. Brg. Pier 4	34543.68	5.63	606.92	606.92
☉ Pier 4	34544.68	5.63	606.92	606.92

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED 
 ENGINEER OF BRIDGE DESIGN
 PASSED 
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS - SPANS 1 THRU 4 - UNIT 1 (N.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	42
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	

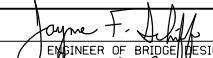

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34292.07	12.88	605.90	605.90
☉ Brg. S. Abut.	34294.82	12.88	605.90	605.90
C	34304.82	12.88	605.92	605.95
D	34314.82	12.88	605.94	605.99
E	34324.82	12.88	605.97	606.02
F	34334.82	12.88	605.99	606.03
G	34344.82	12.88	606.02	606.04
☉ Brg. Pier 1	34357.40	12.88	606.05	606.05
H	34367.40	12.88	606.08	606.08
I	34377.40	12.88	606.11	606.12
J	34387.40	12.88	606.14	606.16
K	34397.40	12.88	606.18	606.19
L	34407.40	12.88	606.21	606.22
☉ Brg. Pier 2	34420.57	12.88	606.26	606.26
M	34430.57	12.88	606.29	606.30
N	34440.57	12.88	606.33	606.34
O	34450.57	12.88	606.37	606.39
P	34460.57	12.88	606.41	606.42
Q	34470.57	12.88	606.45	606.46
☉ Brg. Pier 3	34483.73	12.88	606.51	606.51
R	34493.73	12.88	606.55	606.57
S	34503.73	12.88	606.60	606.63
T	34513.73	12.88	606.65	606.70
U	34523.73	12.88	606.70	606.74
V	34533.73	12.88	606.75	606.78
☉ S. Brg. Pier 4	34546.32	12.88	606.81	606.81
☉ Pier 4	34547.32	12.88	606.82	606.82

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34294.70	20.13	605.75	605.75
☉ Brg. S. Abut.	34297.45	20.13	605.76	605.76
C	34307.45	20.13	605.78	605.80
D	34317.45	20.13	605.80	605.84
E	34327.45	20.13	605.82	605.87
F	34337.45	20.13	605.85	605.88
G	34347.45	20.13	605.87	605.90
☉ Brg. Pier 1	34360.04	20.13	605.91	605.91
H	34370.04	20.13	605.94	605.94
I	34380.04	20.13	605.97	605.98
J	34390.04	20.13	606.00	606.02
K	34400.04	20.13	606.03	606.05
L	34410.04	20.13	606.07	606.08
☉ Brg. Pier 2	34423.20	20.13	606.11	606.11
M	34433.20	20.13	606.15	606.16
N	34443.20	20.13	606.19	606.20
O	34453.20	20.13	606.23	606.24
P	34463.20	20.13	606.27	606.28
Q	34473.20	20.13	606.31	606.32
☉ Brg. Pier 3	34486.37	20.13	606.37	606.37
R	34496.37	20.13	606.42	606.43
S	34506.37	20.13	606.46	606.50
T	34516.37	20.13	606.51	606.56
U	34526.37	20.13	606.56	606.60
V	34536.37	20.13	606.61	606.64
☉ S. Brg. Pier 4	34548.96	20.13	606.67	606.67
☉ Pier 4	34549.95	20.13	606.68	606.68

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED 
 ENGINEER OF BRIDGE DESIGN
 PASSED 
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS - SPANS 1 THRU 4 - UNIT 1 (N.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	43
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34536.76	-16.13	606.69	606.69
☉ N. Brg. Pier 4	34537.76	-16.13	606.70	606.70
W	34547.76	-16.13	606.75	606.78
X	34557.76	-16.13	606.80	606.85
Y	34567.76	-16.13	606.86	606.91
Z	34577.76	-16.13	606.91	606.95
A1	34587.76	-16.13	606.97	606.99
☉ Brg. Pier 5	34600.22	-16.13	607.04	607.04
B1	34610.22	-16.13	607.10	607.10
C1	34620.22	-16.13	607.16	607.16
D1	34630.22	-16.13	607.22	607.22
E1	34640.22	-16.13	607.28	607.28
F1	34650.22	-16.13	607.34	607.34
☉ Brg. Pier 6	34663.39	-16.13	607.42	607.42
G1	34673.39	-16.13	607.47	607.49
H1	34683.39	-16.13	607.53	607.57
I1	34693.39	-16.13	607.59	607.64
J1	34703.39	-16.13	607.65	607.70
K1	34713.39	-16.13	607.71	607.74
☉ S. Brg. Pier 7	34725.84	-16.13	607.78	607.78
☉ Pier 7	34726.91	-16.13	607.79	607.79

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34539.40	-8.88	606.84	606.84
☉ N. Brg. Pier 4	34540.40	-8.88	606.85	606.85
W	34550.40	-8.88	606.90	606.93
X	34560.40	-8.88	606.95	607.00
Y	34570.40	-8.88	607.01	607.06
Z	34580.40	-8.88	607.06	607.10
A1	34590.40	-8.88	607.12	607.14
☉ Brg. Pier 5	34602.86	-8.88	607.19	607.19
B1	34612.86	-8.88	607.25	607.25
C1	34622.86	-8.88	607.31	607.31
D1	34632.86	-8.88	607.37	607.37
E1	34642.86	-8.88	607.43	607.43
F1	34652.86	-8.88	607.49	607.49
☉ Brg. Pier 6	34666.02	-8.88	607.57	607.57
G1	34676.02	-8.88	607.62	607.64
H1	34686.02	-8.88	607.68	607.72
I1	34696.02	-8.88	607.74	607.80
J1	34706.02	-8.88	607.80	607.85
K1	34716.02	-8.88	607.86	607.89
☉ S. Brg. Pier 7	34728.48	-8.88	607.93	607.93
☉ Pier 7	34729.55	-8.88	607.94	607.94

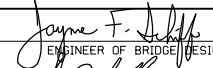

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34542.04	-1.63	606.97	606.97
☉ N. Brg. Pier 4	34543.04	-1.63	606.97	606.97
W	34553.04	-1.63	607.03	607.05
X	34563.04	-1.63	607.08	607.13
Y	34573.04	-1.63	607.14	607.19
Z	34583.04	-1.63	607.19	607.23
A1	34593.04	-1.63	607.25	607.27
☉ Brg. Pier 5	34605.50	-1.63	607.32	607.32
B1	34615.50	-1.63	607.38	607.38
C1	34625.50	-1.63	607.44	607.44
D1	34635.50	-1.63	607.50	607.50
E1	34645.50	-1.63	607.56	607.56
F1	34655.50	-1.63	607.62	607.62
☉ Brg. Pier 6	34668.66	-1.63	607.69	607.69
G1	34678.66	-1.63	607.75	607.77
H1	34688.66	-1.63	607.81	607.85
I1	34698.66	-1.63	607.87	607.92
J1	34708.66	-1.63	607.93	607.98
K1	34718.66	-1.63	607.99	608.02
☉ S. Brg. Pier 7	34731.12	-1.63	608.06	608.06
☉ Pier 7	34732.18	-1.63	608.07	608.07

☉ ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34542.63	0.00	607.00	607.00
☉ N. Brg. Pier 4	34543.63	0.00	607.00	607.00
W	34553.63	0.00	607.06	607.08
X	34563.63	0.00	607.11	607.15
Y	34573.63	0.00	607.16	607.22
Z	34583.63	0.00	607.22	607.26
A1	34593.63	0.00	607.28	607.30
☉ Brg. Pier 5	34606.09	0.00	607.35	607.35
B1	34616.09	0.00	607.41	607.41
C1	34626.09	0.00	607.47	607.47
D1	34636.09	0.00	607.53	607.53
E1	34646.09	0.00	607.59	607.59
F1	34656.09	0.00	607.65	607.65
☉ Brg. Pier 6	34669.25	0.00	607.72	607.72
G1	34679.25	0.00	607.78	607.80
H1	34689.25	0.00	607.84	607.88
I1	34699.25	0.00	607.90	607.95
J1	34709.25	0.00	607.96	608.01
K1	34719.25	0.00	608.02	608.05
☉ S. Brg. Pier 7	34731.71	0.00	608.09	608.09
☉ Pier 7	34732.78	0.00	608.10	608.10

DESIGNED - N. R. Barnett
 CHECKED - A. R. Sheblb
 DRAWN - h.t. duong
 CHECKED - NRB/JMO/GRA

EXAMINED
 PASSED

 ENGINEER OF BRIDGE DESIGN

 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS - SPANS 5 THRU 7 - UNIT 2 (N.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	44
CONTRACT NO. 74351				
SHEET NO. 7 OF 68 SHEETS				
ILLINOIS FED. AID PROJECT				

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34544.68	5.63	606.92	606.92
☉ N. Brg. Pier 4	34545.68	5.63	606.93	606.93
W	34555.68	5.63	606.98	607.01
X	34565.68	5.63	607.03	607.08
Y	34575.68	5.63	607.09	607.14
Z	34585.68	5.63	607.14	607.18
A1	34595.68	5.63	607.20	607.23
☉ Brg. Pier 5	34608.14	5.63	607.28	607.28
B1	34618.14	5.63	607.33	607.33
C1	34628.14	5.63	607.39	607.39
D1	34638.14	5.63	607.45	607.46
E1	34648.14	5.63	607.51	607.51
F1	34658.14	5.63	607.57	607.57
☉ Brg. Pier 6	34671.30	5.63	607.65	607.65
G1	34681.30	5.63	607.71	607.73
H1	34691.30	5.63	607.77	607.80
I1	34701.30	5.63	607.82	607.88
J1	34711.30	5.63	607.88	607.93
K1	34721.30	5.63	607.94	607.98
☉ S. Brg. Pier 7	34733.76	5.63	608.02	608.02
☉ Pier 7	34734.82	5.63	608.02	608.02

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34547.32	12.88	606.82	606.82
☉ N. Brg. Pier 4	34548.31	12.88	606.82	606.82
W	34558.31	12.88	606.87	606.90
X	34568.31	12.88	606.93	606.97
Y	34578.31	12.88	606.98	607.04
Z	34588.31	12.88	607.04	607.08
A1	34598.31	12.88	607.10	607.12
☉ Brg. Pier 5	34610.77	12.88	607.17	607.17
B1	34620.77	12.88	607.23	607.23
C1	34630.77	12.88	607.29	607.29
D1	34640.77	12.88	607.35	607.35
E1	34650.77	12.88	607.41	607.41
F1	34660.77	12.88	607.47	607.47
☉ Brg. Pier 6	34673.94	12.88	607.55	607.55
G1	34683.94	12.88	607.60	607.62
H1	34693.94	12.88	607.66	607.70
I1	34703.94	12.88	607.72	607.77
J1	34713.94	12.88	607.78	607.83
K1	34723.94	12.88	607.84	607.87
☉ S. Brg. Pier 7	34736.40	12.88	607.91	607.91
☉ Pier 7	34737.46	12.88	607.92	607.92

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34549.95	20.13	606.68	606.68
☉ N. Brg. Pier 4	34550.95	20.13	606.68	606.68
W	34560.95	20.13	606.74	606.77
X	34570.95	20.13	606.79	606.84
Y	34580.95	20.13	606.85	606.90
Z	34590.95	20.13	606.91	606.95
A1	34600.95	20.13	606.96	606.99
☉ Brg. Pier 5	34613.41	20.13	607.04	607.04
B1	34623.41	20.13	607.10	607.10
C1	34633.41	20.13	607.16	607.16
D1	34643.41	20.13	607.21	607.22
E1	34653.41	20.13	607.27	607.28
F1	34663.41	20.13	607.33	607.33
☉ Brg. Pier 6	34676.58	20.13	607.41	607.41
G1	34686.58	20.13	607.47	607.49
H1	34696.58	20.13	607.53	607.57
I1	34706.58	20.13	607.59	607.64
J1	34716.58	20.13	607.65	607.69
K1	34726.58	20.13	607.71	607.74
☉ S. Brg. Pier 7	34739.04	20.13	607.78	607.78
☉ Pier 7	34740.10	20.13	607.79	607.79

☉ EXISTING GIRDER 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34726.91	-16.13	607.81	607.81
☉ N. Brg. Pier 7	34727.85	-16.13	607.81	607.81
L1	34737.85	-16.13	607.87	607.90
M1	34747.85	-16.13	607.93	608.00
N1	34757.85	-16.13	607.99	608.07
O1	34767.85	-16.13	608.05	608.13
P1	34777.85	-16.13	608.11	608.19
Q1	34787.85	-16.13	608.17	608.22
R1	34797.85	-16.13	608.22	608.26
S1	34807.85	-16.13	608.28	608.30
☉ Brg. Pier 8	34820.85	-16.13	608.36	608.36
T1	34830.85	-16.13	608.42	608.44
U1	34840.85	-16.13	608.48	608.52
V1	34850.85	-16.13	608.54	608.60
W1	34860.85	-16.13	608.60	608.68
X1	34870.85	-16.13	608.65	608.76
Y1	34880.85	-16.13	608.71	608.84
Z1	34890.85	-16.13	608.77	608.88
A2	34900.85	-16.13	608.83	608.92
B2	34910.85	-16.13	608.89	608.96
C2	34920.85	-16.13	608.95	608.99
D2	34930.85	-16.13	609.01	609.03
☉ Brg. Pier 9	34940.85	-16.13	609.07	609.07
E2	34950.85	-16.13	609.13	609.14
F2	34960.85	-16.13	609.19	609.21
G2	34970.85	-16.13	609.24	609.29
H2	34980.85	-16.13	609.30	609.38
I2	34990.85	-16.13	609.36	609.45
J2	35000.85	-16.13	609.42	609.50
K2	35010.85	-16.13	609.48	609.56
L2	35020.85	-16.13	609.54	609.58
☉ Brg. N. Abut.	35033.85	-16.13	609.62	609.62
Bk. N. Abut.	35036.52	-16.13	609.63	609.63

☉ EXISTING GIRDER 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34729.55	-8.88	607.96	607.96
☉ N. Brg. Pier 7	34730.49	-8.88	607.96	607.96
L1	34740.49	-8.88	608.02	608.05
M1	34750.49	-8.88	608.08	608.15
N1	34760.49	-8.88	608.14	608.22
O1	34770.49	-8.88	608.20	608.28
P1	34780.49	-8.88	608.26	608.34
Q1	34790.49	-8.88	608.32	608.37
R1	34800.49	-8.88	608.37	608.41
S1	34810.49	-8.88	608.43	608.45
☉ Brg. Pier 8	34823.49	-8.88	608.51	608.51
T1	34833.49	-8.88	608.57	608.59
U1	34843.49	-8.88	608.63	608.67
V1	34853.49	-8.88	608.69	608.75
W1	34863.49	-8.88	608.75	608.83
X1	34873.49	-8.88	608.80	608.91
Y1	34883.49	-8.88	608.86	608.99
Z1	34893.49	-8.88	608.92	609.03
A2	34903.49	-8.88	608.98	609.07
B2	34913.49	-8.88	609.04	609.11
C2	34923.49	-8.88	609.10	609.14
D2	34933.49	-8.88	609.16	609.18
☉ Brg. Pier 9	34943.49	-8.88	609.22	609.22
E2	34953.49	-8.88	609.28	609.29
F2	34963.49	-8.88	609.34	609.36
G2	34973.49	-8.88	609.39	609.44
H2	34983.49	-8.88	609.45	609.53
I2	34993.49	-8.88	609.51	609.60
J2	35003.49	-8.88	609.57	609.65
K2	35013.49	-8.88	609.63	609.71
L2	35023.49	-8.88	609.69	609.73
☉ Brg. N. Abut.	35036.49	-8.88	609.77	609.77
Bk. N. Abut.	35039.16	-8.88	609.78	609.78

☉ EXISTING GIRDER 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34732.19	-1.63	608.08	608.08
☉ N. Brg. Pier 7	34733.13	-1.63	608.09	608.09
L1	34743.13	-1.63	608.15	608.18
M1	34753.13	-1.63	608.21	608.28
N1	34763.13	-1.63	608.27	608.35
O1	34773.13	-1.63	608.33	608.41
P1	34783.13	-1.63	608.39	608.46
Q1	34793.13	-1.63	608.44	608.50
R1	34803.13	-1.63	608.50	608.54
S1	34813.13	-1.63	608.56	608.58
☉ Brg. Pier 8	34826.13	-1.63	608.64	608.64
T1	34836.13	-1.63	608.70	608.72
U1	34846.13	-1.63	608.76	608.80
V1	34856.13	-1.63	608.82	608.88
W1	34866.13	-1.63	608.87	608.96
X1	34876.13	-1.63	608.93	609.04
Y1	34886.13	-1.63	608.99	609.12
Z1	34896.13	-1.63	609.05	609.16
A2	34906.13	-1.63	609.11	609.20
B2	34916.13	-1.63	609.17	609.24
C2	34926.13	-1.63	609.23	609.27
D2	34936.13	-1.63	609.29	609.31
☉ Brg. Pier 9	34946.13	-1.63	609.35	609.35
E2	34956.13	-1.63	609.41	609.42
F2	34966.13	-1.63	609.46	609.49
G2	34976.13	-1.63	609.52	609.57
H2	34986.13	-1.63	609.58	609.66
I2	34996.13	-1.63	609.64	609.73
J2	35006.13	-1.63	609.70	609.78
K2	35016.13	-1.63	609.76	609.84
L2	35026.13	-1.63	609.82	609.86
☉ Brg. N. Abut.	35039.13	-1.63	609.90	609.90
Bk. N. Abut.	35041.80	-1.63	609.91	609.91

☉ ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34732.78	0.00	608.11	608.11
☉ N. Brg. Pier 7	34733.72	0.00	608.12	608.12
L1	34743.72	0.00	608.18	608.21
M1	34753.72	0.00	608.24	608.31
N1	34763.72	0.00	608.30	608.38
O1	34773.72	0.00	608.35	608.44
P1	34783.72	0.00	608.41	608.49
Q1	34793.72	0.00	608.47	608.53
R1	34803.72	0.00	608.53	608.56
S1	34813.72	0.00	608.59	608.61
☉ Brg. Pier 8	34826.72	0.00	608.67	608.67
T1	34836.72	0.00	608.73	608.75
U1	34846.72	0.00	608.79	608.83
V1	34856.72	0.00	608.84	608.89
W1	34866.72	0.00	608.90	608.99
X1	34876.72	0.00	608.96	609.07
Y1	34886.72	0.00	609.02	609.14
Z1	34896.72	0.00	609.08	609.18
A2	34906.72	0.00	609.14	609.22
B2	34916.72	0.00	609.20	609.26
C2	34926.72	0.00	609.26	609.30
D2	34936.72	0.00	609.32	609.34
☉ Brg. Pier 9	34946.72	0.00	609.38	609.38
E2	34956.72	0.00	609.43	609.45
F2	34966.72	0.00	609.49	609.52
G2	34976.72	0.00	609.55	609.60
H2	34986.72	0.00	609.61	609.68
I2	34996.72	0.00	609.67	609.76
J2	35006.72	0.00	609.73	609.81
K2	35016.72	0.00	609.79	609.87
L2	35026.72	0.00	609.85	609.89
☉ Brg. N. Abut.	35039.72	0.00	609.92	609.92
Bk. N. Abut.	35042.39	0.00	609.94	609.94

☉ EXISTING GIRDER 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34734.83	5.63	608.04	608.04
☉ N. Brg. Pier 7	34735.77	5.63	608.04	608.04
L1	34745.77	5.63	608.10	608.14
M1	34755.77	5.63	608.16	608.23
N1	34765.77	5.63	608.22	608.30
O1	34775.77	5.63	608.28	608.36
P1	34785.77	5.63	608.34	608.42
Q1	34795.77	5.63	608.40	608.45
R1	34805.77	5.63	608.46	608.49
S1	34815.77	5.63	608.52	608.53
☉ Brg. Pier 8	34828.77	5.63	608.59	608.59
T1	34838.77	5.63	608.65	608.67
U1	34848.77	5.63	608.71	608.75
V1	34858.77	5.63	608.77	608.83
W1	34868.77	5.63	608.83	608.91
X1	34878.77	5.63	608.89	608.99
Y1	34888.77	5.63	608.95	609.07
Z1	34898.77	5.63	609.00	609.11
A2	34908.77	5.63	609.06	609.15
B2	34918.77	5.63	609.12	609.19
C2	34928.77	5.63	609.18	609.23
D2	34938.77	5.63	609.24	609.26
☉ Brg. Pier 9	34948.77	5.63	609.30	609.30
E2	34958.77	5.63	609.36	609.37
F2	34968.77	5.63	609.42	609.45
G2	34978.77	5.63	609.48	609.53
H2	34988.77	5.63	609.54	609.61
I2	34998.77	5.63	609.59	609.68
J2	35008.77	5.63	609.65	609.74
K2	35018.77	5.63	609.71	609.79
L2	35028.77	5.63	609.77	609.82
☉ Brg. N. Abut.	35041.77	5.63	609.85	609.85
Bk. N. Abut.	35044.44	5.63	609.86	609.86

☉ EXISTING GIRDER 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34737.47	12.88	607.94	607.94
☉ N. Brg. Pier 7	34738.41	12.88	607.94	607.94
L1	34748.41	12.88	608.00	608.03
M1	34758.41	12.88	608.06	608.13
N1	34768.41	12.88	608.12	608.20
O1	34778.41	12.88	608.18	608.26
P1	34788.41	12.88	608.24	608.32
Q1	34798.41	12.88	608.29	608.35
R1	34808.41	12.88	608.35	608.39
S1	34818.41	12.88	608.41	608.43
☉ Brg. Pier 8	34831.41	12.88	608.49	608.49
T1	34841.41	12.88	608.55	608.57
U1	34851.41	12.88	608.61	608.65
V1	34861.41	12.88	608.67	608.73
W1	34871.41	12.88	608.73	608.81
X1	34881.41	12.88	608.78	608.89
Y1	34891.41	12.88	608.84	608.97
Z1	34901.41	12.88	608.90	609.01
A2	34911.41	12.88	608.96	609.05
B2	34921.41	12.88	609.02	609.09
C2	34931.41	12.88	609.08	609.12
D2	34941.41	12.88	609.14	609.16
☉ Brg. Pier 9	34951.41	12.88	609.20	609.20
E2	34961.41	12.88	609.26	609.27
F2	34971.41	12.88	609.32	609.34
G2	34981.41	12.88	609.37	609.42
H2	34991.41	12.88	609.43	609.51
I2	35001.41	12.88	609.49	609.58
J2	35011.41	12.88	609.55	609.63
K2	35021.41	12.88	609.61	609.69
L2	35031.41	12.88	609.67	609.71
☉ Brg. N. Abut.	35044.41	12.88	609.75	609.75
Bk. N. Abut.	35047.08	12.88	609.76	609.76

☉ EXISTING GIRDER 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34740.10	20.13	607.80	607.80
☉ N. Brg. Pier 7	34741.05	20.13	607.81	607.81
L1	34751.05	20.13	607.86	607.90
M1	34761.05	20.13	607.92	607.99
N1	34771.05	20.13	607.98	608.06
O1	34781.05	20.13	608.04	608.13
P1	34791.05	20.13	608.10	608.18
Q1	34801.05	20.13	608.16	608.22
R1	34811.05	20.13	608.22	608.25
S1	34821.05	20.13	608.28	608.30
☉ Brg. Pier 8	34834.05	20.13	608.35	608.35
T1	34844.05	20.13	608.41	608.44
U1	34854.05	20.13	608.47	608.52
V1	34864.05	20.13	608.53	608.60
W1	34874.05	20.13	608.59	608.68
X1	34884.05	20.13	608.65	608.75
Y1	34894.05	20.13	608.71	608.83
Z1	34904.05	20.13	608.77	608.87
A2	34914.05	20.13	608.83	608.91
B2	34924.05	20.13	608.89	608.95
C2	34934.05	20.13	608.94	608.99
D2	34944.05	20.13	609.00	609.03
☉ Brg. Pier 9	34954.05	20.13	609.06	609.06
E2	34964.05	20.13	609.12	609.14
F2	34974.05	20.13	609.18	609.21
G2	34984.05	20.13	609.24	609.29
H2	34994.05	20.13	609.30	609.37
I2	35004.05	20.13	609.36	609.44
J2	35014.05	20.13	609.42	609.50
K2	35024.05	20.13	609.48	609.55
L2	35034.05	20.13	609.53	609.58
☉ Brg. N. Abut.	35047.05	20.13	609.61	609.61
Bk. N. Abut.	35049.72	20.13	609.63	609.63

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34252.49	16.13	605.76	605.76
☉ Brg. S. Abut.	34255.24	16.13	605.76	605.76
C	34265.24	16.13	605.78	605.80
D	34275.24	16.13	605.80	605.84
E	34285.24	16.13	605.81	605.86
F	34295.24	16.13	605.83	605.87
G	34305.24	16.13	605.86	605.88
☉ Brg. Pier 1	34317.82	16.13	605.88	605.88
H	34327.82	16.13	605.91	605.91
I	34337.82	16.13	605.93	605.94
J	34347.82	16.13	605.96	605.97
K	34357.82	16.13	605.99	606.00
L	34367.82	16.13	606.01	606.02
☉ Brg. Pier 2	34380.99	16.13	606.05	606.05
M	34390.99	16.13	606.09	606.09
N	34400.99	16.13	606.12	606.13
O	34410.99	16.13	606.15	606.17
P	34420.99	16.13	606.19	606.20
Q	34430.99	16.13	606.23	606.23
☉ Brg. Pier 3	34444.15	16.13	606.28	606.28
R	34454.15	16.13	606.32	606.33
S	34464.15	16.13	606.36	606.39
T	34474.15	16.13	606.40	606.45
U	34484.15	16.13	606.44	606.49
V	34494.15	16.13	606.49	606.52
☉ S. Brg. Pier 4	34506.74	16.13	606.55	606.55
☉ Pier 4	34507.74	16.13	606.55	606.55

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34249.85	8.88	605.89	605.89
☉ Brg. S. Abut.	34252.60	8.88	605.89	605.89
C	34262.60	8.88	605.91	605.94
D	34272.60	8.88	605.93	605.97
E	34282.60	8.88	605.94	605.99
F	34292.60	8.88	605.96	606.00
G	34302.60	8.88	605.98	606.01
☉ Brg. Pier 1	34315.18	8.88	606.01	606.01
H	34325.18	8.88	606.04	606.04
I	34335.18	8.88	606.06	606.07
J	34345.18	8.88	606.09	606.10
K	34355.18	8.88	606.11	606.13
L	34365.18	8.88	606.14	606.15
☉ Brg. Pier 2	34378.35	8.88	606.18	606.18
M	34388.35	8.88	606.21	606.22
N	34398.35	8.88	606.25	606.26
O	34408.35	8.88	606.28	606.29
P	34418.35	8.88	606.31	606.32
Q	34428.35	8.88	606.35	606.36
☉ Brg. Pier 3	34441.52	8.88	606.40	606.40
R	34451.52	8.88	606.44	606.46
S	34461.52	8.88	606.48	606.51
T	34471.52	8.88	606.52	606.57
U	34481.52	8.88	606.57	606.61
V	34491.52	8.88	606.61	606.64
☉ S. Brg. Pier 4	34504.10	8.88	606.67	606.67
☉ Pier 4	34505.10	8.88	606.67	606.67

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34247.21	1.63	606.00	606.00
☉ Brg. S. Abut.	34249.96	1.63	606.00	606.00
C	34259.96	1.63	606.02	606.04
D	34269.96	1.63	606.03	606.08
E	34279.96	1.63	606.05	606.10
F	34289.96	1.63	606.07	606.11
G	34299.96	1.63	606.09	606.11
☉ Brg. Pier 1	34312.54	1.63	606.12	606.12
H	34322.54	1.63	606.14	606.15
I	34332.54	1.63	606.17	606.17
J	34342.54	1.63	606.19	606.21
K	34352.54	1.63	606.22	606.23
L	34362.54	1.63	606.25	606.26
☉ Brg. Pier 2	34375.71	1.63	606.29	606.29
M	34385.71	1.63	606.32	606.32
N	34395.71	1.63	606.35	606.36
O	34405.71	1.63	606.38	606.40
P	34415.71	1.63	606.42	606.43
Q	34425.71	1.63	606.45	606.46
☉ Brg. Pier 3	34438.88	1.63	606.50	606.50
R	34448.88	1.63	606.54	606.56
S	34458.88	1.63	606.58	606.62
T	34468.88	1.63	606.63	606.67
U	34478.88	1.63	606.67	606.71
V	34488.88	1.63	606.71	606.74
☉ S. Brg. Pier 4	34501.46	1.63	606.77	606.77
☉ Pier 4	34502.46	1.63	606.77	606.77

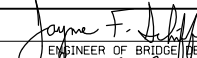

☉ ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34246.62	0.00	606.02	606.02
☉ Brg. S. Abut.	34249.37	0.00	606.03	606.03
C	34259.37	0.00	606.04	606.07
D	34269.37	0.00	606.06	606.10
E	34279.37	0.00	606.08	606.13
F	34289.37	0.00	606.10	606.13
G	34299.37	0.00	606.12	606.14
☉ Brg. Pier 1	34311.95	0.00	606.14	606.14
H	34321.95	0.00	606.17	606.17
I	34331.95	0.00	606.19	606.20
J	34341.95	0.00	606.22	606.23
K	34351.95	0.00	606.24	606.26
L	34361.95	0.00	606.27	606.28
☉ Brg. Pier 2	34375.12	0.00	606.31	606.31
M	34385.12	0.00	606.34	606.35
N	34395.12	0.00	606.37	606.38
O	34405.12	0.00	606.41	606.42
P	34415.12	0.00	606.44	606.45
Q	34425.12	0.00	606.48	606.48
☉ Brg. Pier 3	34438.29	0.00	606.53	606.53
R	34448.29	0.00	606.57	606.58
S	34458.29	0.00	606.61	606.64
T	34468.29	0.00	606.65	606.70
U	34478.29	0.00	606.69	606.74
V	34488.29	0.00	606.74	606.77
☉ S. Brg. Pier 4	34500.87	0.00	606.79	606.79
☉ Pier 4	34501.87	0.00	606.80	606.80

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34244.57	-5.63	605.93	605.93
☉ Brg. S. Abut.	34247.32	-5.63	605.94	605.94
C	34257.32	-5.63	605.95	605.98
D	34267.32	-5.63	605.97	606.01
E	34277.32	-5.63	605.99	606.03
F	34287.32	-5.63	606.00	606.04
G	34297.32	-5.63	606.02	606.05
☉ Brg. Pier 1	34309.91	-5.63	606.05	606.05
H	34319.91	-5.63	606.07	606.08
I	34329.91	-5.63	606.10	606.11
J	34339.91	-5.63	606.12	606.14
K	34349.91	-5.63	606.15	606.16
L	34359.91	-5.63	606.18	606.19
☉ Brg. Pier 2	34373.07	-5.63	606.22	606.22
M	34383.07	-5.63	606.25	606.25
N	34393.07	-5.63	606.28	606.29
O	34403.07	-5.63	606.31	606.33
P	34413.07	-5.63	606.35	606.36
Q	34423.07	-5.63	606.38	606.39
☉ Brg. Pier 3	34436.24	-5.63	606.43	606.43
R	34446.24	-5.63	606.47	606.49
S	34456.24	-5.63	606.51	606.54
T	34466.24	-5.63	606.55	606.60
U	34476.24	-5.63	606.59	606.64
V	34486.24	-5.63	606.64	606.67
☉ S. Brg. Pier 4	34498.82	-5.63	606.70	606.70
☉ Pier 4	34499.82	-5.63	606.70	606.70

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED 
 ENGINEER OF BRIDGE DESIGN
 PASSED 
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS - SPANS 1 THRU 4 - UNIT 1 (S.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	50
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34241.93	-12.88	605.81	605.81
☉ Brg. S. Abut.	34244.68	-12.88	605.82	605.82
C	34254.68	-12.88	605.83	605.86
D	34264.68	-12.88	605.85	605.89
E	34274.68	-12.88	605.86	605.91
F	34284.68	-12.88	605.88	605.92
G	34294.68	-12.88	605.90	605.92
☉ Brg. Pier 1	34307.27	-12.88	605.93	605.93
H	34317.27	-12.88	605.95	605.95
I	34327.27	-12.88	605.97	605.98
J	34337.27	-12.88	606.00	606.01
K	34347.27	-12.88	606.02	606.04
L	34357.27	-12.88	606.05	606.06
☉ Brg. Pier 2	34370.43	-12.88	606.09	606.09
M	34380.43	-12.88	606.12	606.13
N	34390.43	-12.88	606.15	606.16
O	34400.43	-12.88	606.19	606.20
P	34410.43	-12.88	606.22	606.23
Q	34420.43	-12.88	606.26	606.26
☉ Brg. Pier 3	34433.60	-12.88	606.30	606.30
R	34443.60	-12.88	606.34	606.36
S	34453.60	-12.88	606.38	606.42
T	34463.60	-12.88	606.42	606.47
U	34473.60	-12.88	606.47	606.51
V	34483.60	-12.88	606.51	606.54
☉ S. Brg. Pier 4	34496.19	-12.88	606.57	606.57
☉ Pier 4	34497.18	-12.88	606.57	606.57

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	34239.30	-20.13	605.66	605.66
☉ Brg. S. Abut.	34242.04	-20.13	605.66	605.66
C	34252.04	-20.13	605.68	605.70
D	34262.04	-20.13	605.69	605.73
E	34272.04	-20.13	605.71	605.76
F	34282.04	-20.13	605.73	605.76
G	34292.04	-20.13	605.74	605.77
☉ Brg. Pier 1	34304.63	-20.13	605.77	605.77
H	34314.63	-20.13	605.79	605.80
I	34324.63	-20.13	605.82	605.82
J	34334.63	-20.13	605.84	605.86
K	34344.63	-20.13	605.87	605.88
L	34354.63	-20.13	605.89	605.90
☉ Brg. Pier 2	34367.79	-20.13	605.93	605.93
M	34377.79	-20.13	605.96	605.97
N	34387.79	-20.13	605.99	606.00
O	34397.79	-20.13	606.03	606.04
P	34407.79	-20.13	606.06	606.07
Q	34417.79	-20.13	606.09	606.10
☉ Brg. Pier 3	34430.96	-20.13	606.14	606.14
R	34440.96	-20.13	606.18	606.20
S	34450.96	-20.13	606.22	606.25
T	34460.96	-20.13	606.26	606.31
U	34470.96	-20.13	606.30	606.35
V	34480.96	-20.13	606.35	606.38
☉ S. Brg. Pier 4	34493.55	-20.13	606.40	606.40
☉ Pier 4	34494.54	-20.13	606.41	606.41

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED *Joanne F. Joffe*
 ENGINEER OF BRIDGE DESIGN
 PASSED *Carl Ringer*
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS - SPANS 1 THRU 4 - UNIT 1 (S.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	51
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34507.74	16.13	606.55	606.55
☉ N. Brg. Pier 4	34508.74	16.13	606.56	606.56
W	34518.74	16.13	606.60	606.63
X	34528.74	16.13	606.65	606.70
Y	34538.74	16.13	606.70	606.76
Z	34548.74	16.13	606.76	606.80
A1	34558.74	16.13	606.81	606.83
☉ Brg. Pier 5	34571.20	16.13	606.88	606.88
B1	34581.20	16.13	606.93	606.93
C1	34591.20	16.13	606.99	606.99
D1	34601.20	16.13	607.05	607.05
E1	34611.20	16.13	607.11	607.11
F1	34621.20	16.13	607.17	607.17
☉ Brg. Pier 6	34634.36	16.13	607.24	607.24
G1	34644.36	16.13	607.30	607.32
H1	34654.36	16.13	607.36	607.40
I1	34664.36	16.13	607.42	607.47
J1	34674.36	16.13	607.48	607.53
K1	34684.36	16.13	607.54	607.57
☉ S. Brg. Pier 7	34696.82	16.13	607.61	607.61
☉ Pier 7	34697.88	16.13	607.62	607.62

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34505.10	8.88	606.67	606.67
☉ N. Brg. Pier 4	34506.10	8.88	606.68	606.68
W	34516.10	8.88	606.73	606.75
X	34526.10	8.88	606.78	606.82
Y	34536.10	8.88	606.83	606.88
Z	34546.10	8.88	606.88	606.92
A1	34556.10	8.88	606.93	606.95
☉ Brg. Pier 5	34568.56	8.88	607.00	607.00
B1	34578.56	8.88	607.05	607.05
C1	34588.56	8.88	607.11	607.11
D1	34598.56	8.88	607.17	607.17
E1	34608.56	8.88	607.23	607.23
F1	34618.56	8.88	607.29	607.29
☉ Brg. Pier 6	34631.73	8.88	607.36	607.36
G1	34641.73	8.88	607.42	607.44
H1	34651.73	8.88	607.48	607.52
I1	34661.73	8.88	607.54	607.59
J1	34671.73	8.88	607.60	607.65
K1	34681.73	8.88	607.66	607.69
☉ S. Brg. Pier 7	34694.18	8.88	607.73	607.73
☉ Pier 7	34695.25	8.88	607.74	607.74

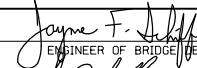

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34502.46	1.63	606.77	606.77
☉ N. Brg. Pier 4	34503.46	1.63	606.78	606.78
W	34513.46	1.63	606.83	606.85
X	34523.46	1.63	606.88	606.92
Y	34533.46	1.63	606.93	606.98
Z	34543.46	1.63	606.98	607.02
A1	34553.46	1.63	607.03	607.05
☉ Brg. Pier 5	34565.92	1.63	607.10	607.10
B1	34575.92	1.63	607.15	607.15
C1	34585.92	1.63	607.21	607.21
D1	34595.92	1.63	607.27	607.27
E1	34605.92	1.63	607.32	607.33
F1	34615.92	1.63	607.38	607.38
☉ Brg. Pier 6	34629.09	1.63	607.46	607.46
G1	34639.09	1.63	607.52	607.54
H1	34649.09	1.63	607.58	607.62
I1	34659.09	1.63	607.64	607.69
J1	34669.09	1.63	607.70	607.74
K1	34679.09	1.63	607.76	607.79
☉ S. Brg. Pier 7	34691.54	1.63	607.83	607.83
☉ Pier 7	34692.61	1.63	607.84	607.84

☉ ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34501.87	0.00	606.80	606.80
☉ N. Brg. Pier 4	34502.87	0.00	606.80	606.80
W	34512.87	0.00	606.85	606.88
X	34522.87	0.00	606.90	606.94
Y	34532.87	0.00	606.95	607.00
Z	34542.87	0.00	607.00	607.04
A1	34552.87	0.00	607.05	607.07
☉ Brg. Pier 5	34565.33	0.00	607.12	607.12
B1	34575.33	0.00	607.17	607.17
C1	34585.33	0.00	607.23	607.23
D1	34595.33	0.00	607.29	607.29
E1	34605.33	0.00	607.35	607.35
F1	34615.33	0.00	607.41	607.41
☉ Brg. Pier 6	34628.50	0.00	607.48	607.48
G1	34638.50	0.00	607.54	607.56
H1	34648.50	0.00	607.60	607.64
I1	34658.50	0.00	607.66	607.71
J1	34668.50	0.00	607.72	607.77
K1	34678.50	0.00	607.78	607.81
☉ S. Brg. Pier 7	34690.95	0.00	607.85	607.85
☉ Pier 7	34692.02	0.00	607.86	607.86

DESIGNED - N. R. Barnett
 CHECKED - A. R. Sheblb
 DRAWN - h.t. duong
 CHECKED - NRB/JMO/GRA

EXAMINED
 PASSED

 ENGINEER OF BRIDGE DESIGN

 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS - SPANS 5 THRU 7 - UNIT 2 (S.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	52
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34499.82	-5.63	606.70	606.70
☉ N. Brg. Pier 4	34500.82	-5.63	606.70	606.70
W	34510.82	-5.63	606.75	606.78
X	34520.82	-5.63	606.80	606.85
Y	34530.82	-5.63	606.85	606.90
Z	34540.82	-5.63	606.90	606.94
A1	34550.82	-5.63	606.95	606.98
☉ Brg. Pier 5	34563.28	-5.63	607.02	607.02
B1	34573.28	-5.63	607.07	607.07
C1	34583.28	-5.63	607.13	607.13
D1	34593.28	-5.63	607.19	607.19
E1	34603.28	-5.63	607.25	607.25
F1	34613.28	-5.63	607.31	607.31
☉ Brg. Pier 6	34626.45	-5.63	607.38	607.38
G1	34636.45	-5.63	607.44	607.46
H1	34646.45	-5.63	607.50	607.54
I1	34656.45	-5.63	607.56	607.61
J1	34666.45	-5.63	607.62	607.67
K1	34676.45	-5.63	607.68	607.71
☉ S. Brg. Pier 7	34688.90	-5.63	607.75	607.75
☉ Pier 7	34689.97	-5.63	607.76	607.76

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34497.18	-12.88	606.57	606.57
☉ N. Brg. Pier 4	34498.18	-12.88	606.57	606.57
W	34508.18	-12.88	606.62	606.65
X	34518.18	-12.88	606.67	606.71
Y	34528.18	-12.88	606.72	606.77
Z	34538.18	-12.88	606.77	606.81
A1	34548.18	-12.88	606.82	606.84
☉ Brg. Pier 5	34560.64	-12.88	606.89	606.89
B1	34570.64	-12.88	606.94	606.94
C1	34580.64	-12.88	607.00	607.00
D1	34590.64	-12.88	607.05	607.06
E1	34600.64	-12.88	607.11	607.12
F1	34610.64	-12.88	607.17	607.17
☉ Brg. Pier 6	34623.81	-12.88	607.25	607.25
G1	34633.81	-12.88	607.31	607.33
H1	34643.81	-12.88	607.37	607.40
I1	34653.81	-12.88	607.43	607.48
J1	34663.81	-12.88	607.49	607.53
K1	34673.81	-12.88	607.54	607.58
☉ S. Brg. Pier 7	34686.27	-12.88	607.62	607.62
☉ Pier 7	34687.33	-12.88	607.62	607.62

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 4	34494.55	-20.13	606.41	606.41
☉ N. Brg. Pier 4	34495.54	-20.13	606.41	606.41
W	34505.54	-20.13	606.46	606.49
X	34515.54	-20.13	606.51	606.55
Y	34525.54	-20.13	606.55	606.61
Z	34535.54	-20.13	606.60	606.65
A1	34545.54	-20.13	606.66	606.68
☉ Brg. Pier 5	34558.00	-20.13	606.72	606.72
B1	34568.00	-20.13	606.78	606.78
C1	34578.00	-20.13	606.83	606.83
D1	34588.00	-20.13	606.89	606.89
E1	34598.00	-20.13	606.95	606.95
F1	34608.00	-20.13	607.01	607.01
☉ Brg. Pier 6	34621.17	-20.13	607.08	607.08
G1	34631.17	-20.13	607.14	607.16
H1	34641.17	-20.13	607.20	607.24
I1	34651.17	-20.13	607.26	607.31
J1	34661.17	-20.13	607.32	607.37
K1	34671.17	-20.13	607.38	607.41
☉ S. Brg. Pier 7	34683.63	-20.13	607.45	607.45
☉ Pier 7	34684.69	-20.13	607.46	607.46

☉ EXISTING GIRDER 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34697.89	16.13	607.63	607.63
☉ N. Brg. Pier 7	34698.83	16.13	607.64	607.64
L1	34708.83	16.13	607.70	607.73
M1	34718.83	16.13	607.76	607.83
N1	34728.83	16.13	607.82	607.90
O1	34738.83	16.13	607.88	607.96
P1	34748.83	16.13	607.93	608.01
Q1	34758.83	16.13	607.99	608.05
R1	34768.83	16.13	608.05	608.09
S1	34778.83	16.13	608.11	608.13
☉ Brg. Pier 8	34791.83	16.13	608.19	608.19
T1	34801.83	16.13	608.25	608.27
U1	34811.83	16.13	608.31	608.35
V1	34821.83	16.13	608.37	608.43
W1	34831.83	16.13	608.42	608.51
X1	34841.83	16.13	608.48	608.59
Y1	34851.83	16.13	608.54	608.67
Z1	34861.83	16.13	608.60	608.71
A2	34871.83	16.13	608.66	608.75
B2	34881.83	16.13	608.72	608.79
C2	34891.83	16.13	608.78	608.82
D2	34901.83	16.13	608.84	608.86
☉ Brg. Pier 9	34911.83	16.13	608.90	608.90
E2	34921.83	16.13	608.96	608.97
F2	34931.83	16.13	609.01	609.04
G2	34941.83	16.13	609.07	609.12
H2	34951.83	16.13	609.13	609.20
I2	34961.83	16.13	609.19	609.28
J2	34971.83	16.13	609.25	609.33
K2	34981.83	16.13	609.31	609.39
L2	34991.83	16.13	609.37	609.41
☉ Brg. N. Abut.	35004.83	16.13	609.45	609.45
Bk. N. Abut.	35007.50	16.13	609.46	609.46

☉ EXISTING GIRDER 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34695.25	8.88	607.75	607.75
☉ N. Brg. Pier 7	34696.19	8.88	607.76	607.76
L1	34706.19	8.88	607.82	607.85
M1	34716.19	8.88	607.88	607.94
N1	34726.19	8.88	607.94	608.02
O1	34736.19	8.88	607.99	608.08
P1	34746.19	8.88	608.05	608.13
Q1	34756.19	8.88	608.11	608.17
R1	34766.19	8.88	608.17	608.20
S1	34776.19	8.88	608.23	608.25
☉ Brg. Pier 8	34789.19	8.88	608.31	608.31
T1	34799.19	8.88	608.37	608.39
U1	34809.19	8.88	608.43	608.47
V1	34819.19	8.88	608.48	608.55
W1	34829.19	8.88	608.54	608.63
X1	34839.19	8.88	608.60	608.71
Y1	34849.19	8.88	608.66	608.78
Z1	34859.19	8.88	608.72	608.82
A2	34869.19	8.88	608.78	608.86
B2	34879.19	8.88	608.84	608.90
C2	34889.19	8.88	608.90	608.94
D2	34899.19	8.88	608.96	608.98
☉ Brg. Pier 9	34909.19	8.88	609.02	609.02
E2	34919.19	8.88	609.07	609.09
F2	34929.19	8.88	609.13	609.16
G2	34939.19	8.88	609.19	609.24
H2	34949.19	8.88	609.25	609.32
I2	34959.19	8.88	609.31	609.40
J2	34969.19	8.88	609.37	609.45
K2	34979.19	8.88	609.43	609.51
L2	34989.19	8.88	609.49	609.53
☉ Brg. N. Abut.	35002.19	8.88	609.56	609.56
Bk. N. Abut.	35004.86	8.88	609.58	609.58

☉ EXISTING GIRDER 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34692.61	1.63	607.85	607.85
☉ N. Brg. Pier 7	34693.55	1.63	607.86	607.86
L1	34703.55	1.63	607.92	607.95
M1	34713.55	1.63	607.97	608.04
N1	34723.55	1.63	608.03	608.12
O1	34733.55	1.63	608.09	608.18
P1	34743.55	1.63	608.15	608.23
Q1	34753.55	1.63	608.21	608.27
R1	34763.55	1.63	608.27	608.30
S1	34773.55	1.63	608.33	608.35
☉ Brg. Pier 8	34786.55	1.63	608.41	608.41
T1	34796.55	1.63	608.46	608.49
U1	34806.55	1.63	608.52	608.57
V1	34816.55	1.63	608.58	608.65
W1	34826.55	1.63	608.64	608.73
X1	34836.55	1.63	608.70	608.80
Y1	34846.55	1.63	608.76	608.88
Z1	34856.55	1.63	608.82	608.92
A2	34866.55	1.63	608.88	608.96
B2	34876.55	1.63	608.94	609.00
C2	34886.55	1.63	609.00	609.04
D2	34896.55	1.63	609.05	609.08
☉ Brg. Pier 9	34906.55	1.63	609.11	609.11
E2	34916.55	1.63	609.17	609.19
F2	34926.55	1.63	609.23	609.26
G2	34936.55	1.63	609.29	609.34
H2	34946.55	1.63	609.35	609.42
I2	34956.55	1.63	609.41	609.49
J2	34966.55	1.63	609.47	609.55
K2	34976.55	1.63	609.53	609.60
L2	34986.55	1.63	609.59	609.63
☉ Brg. N. Abut.	34999.55	1.63	609.66	609.66
Bk. N. Abut.	35002.23	1.63	609.68	609.68

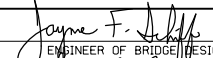

☉ ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34692.02	0.00	607.87	607.87
☉ N. Brg. Pier 7	34692.96	0.00	607.88	607.88
L1	34702.96	0.00	607.94	607.97
M1	34712.96	0.00	608.00	608.06
N1	34722.96	0.00	608.06	608.14
O1	34732.96	0.00	608.11	608.20
P1	34742.96	0.00	608.17	608.25
Q1	34752.96	0.00	608.23	608.29
R1	34762.96	0.00	608.29	608.32
S1	34772.96	0.00	608.35	608.37
☉ Brg. Pier 8	34785.96	0.00	608.43	608.43
T1	34795.96	0.00	608.49	608.51
U1	34805.96	0.00	608.55	608.59
V1	34815.96	0.00	608.60	608.67
W1	34825.96	0.00	608.66	608.75
X1	34835.96	0.00	608.72	608.83
Y1	34845.96	0.00	608.78	608.90
Z1	34855.96	0.00	608.84	608.94
A2	34865.96	0.00	608.90	608.98
B2	34875.96	0.00	608.96	609.02
C2	34885.96	0.00	609.02	609.06
D2	34895.96	0.00	609.08	609.10
☉ Brg. Pier 9	34905.96	0.00	609.14	609.14
E2	34915.96	0.00	609.19	609.21
F2	34925.96	0.00	609.25	609.28
G2	34935.96	0.00	609.31	609.36
H2	34945.96	0.00	609.37	609.44
I2	34955.96	0.00	609.43	609.52
J2	34965.96	0.00	609.49	609.57
K2	34975.96	0.00	609.55	609.63
L2	34985.96	0.00	609.61	609.65
☉ Brg. N. Abut.	34998.96	0.00	609.68	609.68
Bk. N. Abut.	35001.63	0.00	609.70	609.70

☉ EXISTING GIRDER 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34689.97	-5.63	607.77	607.77
☉ N. Brg. Pier 7	34690.91	-5.63	607.78	607.78
L1	34700.91	-5.63	607.84	607.87
M1	34710.91	-5.63	607.9	607.96
N1	34720.91	-5.63	607.96	608.04
O1	34730.91	-5.63	608.01	608.10
P1	34740.91	-5.63	608.07	608.15
Q1	34750.91	-5.63	608.13	608.19
R1	34760.91	-5.63	608.19	608.22
S1	34770.91	-5.63	608.25	608.27
☉ Brg. Pier 8	34783.91	-5.63	608.33	608.33
T1	34793.91	-5.63	608.39	608.41
U1	34803.91	-5.63	608.45	608.49
V1	34813.91	-5.63	608.50	608.57
W1	34823.91	-5.63	608.56	608.65
X1	34833.91	-5.63	608.62	608.73
Y1	34843.91	-5.63	608.68	608.80
Z1	34853.91	-5.63	608.74	608.84
A2	34863.91	-5.63	608.80	608.88
B2	34873.91	-5.63	608.86	608.92
C2	34883.91	-5.63	608.92	608.96
D2	34893.91	-5.63	608.98	609.00
☉ Brg. Pier 9	34903.91	-5.63	609.04	609.04
E2	34913.91	-5.63	609.09	609.11
F2	34923.91	-5.63	609.15	609.18
G2	34933.91	-5.63	609.21	609.26
H2	34943.91	-5.63	609.27	609.34
I2	34953.91	-5.63	609.33	609.42
J2	34963.91	-5.63	609.39	609.47
K2	34973.91	-5.63	609.45	609.53
L2	34983.91	-5.63	609.51	609.55
☉ Brg. N. Abut.	34996.91	-5.63	609.58	609.58
Bk. N. Abut.	34999.59	-5.63	609.60	609.60

DESIGNED - A. R. Shebli
 CHECKED - N. R. Barnett
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED 
 ENGINEER OF BRIDGE DESIGN
 PASSED 
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS - SPANS 8 THRU 10 - UNIT 3 (S.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	55
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	

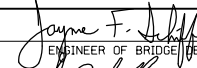

☉ EXISTING GIRDER 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34687.33	-12.88	607.64	607.64
☉ N. Brg. Pier 7	34688.28	-12.88	607.65	607.65
L1	34698.28	-12.88	607.70	607.74
M1	34708.28	-12.88	607.76	607.83
N1	34718.28	-12.88	607.82	607.90
O1	34728.28	-12.88	607.88	607.97
P1	34738.28	-12.88	607.94	608.02
Q1	34748.28	-12.88	608.00	608.06
R1	34758.28	-12.88	608.06	608.09
S1	34768.28	-12.88	608.12	608.14
☉ Brg. Pier 8	34781.28	-12.88	608.19	608.19
T1	34791.28	-12.88	608.25	608.27
U1	34801.28	-12.88	608.31	608.36
V1	34811.28	-12.88	608.37	608.44
W1	34821.28	-12.88	608.43	608.51
X1	34831.28	-12.88	608.49	608.59
Y1	34841.28	-12.88	608.55	608.67
Z1	34851.28	-12.88	608.61	608.71
A2	34861.28	-12.88	608.67	608.75
B2	34871.28	-12.88	608.72	608.79
C2	34881.28	-12.88	608.78	608.83
D2	34891.28	-12.88	608.84	608.86
☉ Brg. Pier 9	34901.28	-12.88	608.90	608.90
E2	34911.28	-12.88	608.96	608.98
F2	34921.28	-12.88	609.02	609.05
G2	34931.28	-12.88	609.08	609.13
H2	34941.28	-12.88	609.14	609.21
I2	34951.28	-12.88	609.20	609.28
J2	34961.28	-12.88	609.26	609.34
K2	34971.28	-12.88	609.31	609.39
L2	34981.28	-12.88	609.37	609.42
☉ Brg. N. Abut.	34994.28	-12.88	609.45	609.45
Bk. N. Abut.	34996.95	-12.88	609.47	609.47

☉ EXISTING GIRDER 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ Pier 7	34684.70	-20.13	607.47	607.47
☉ N. Brg. Pier 7	34685.64	-20.13	607.48	607.48
L1	34695.64	-20.13	607.54	607.57
M1	34705.64	-20.13	607.60	607.66
N1	34715.64	-20.13	607.66	607.74
O1	34725.64	-20.13	607.71	607.80
P1	34735.64	-20.13	607.77	607.85
Q1	34745.64	-20.13	607.83	607.89
R1	34755.64	-20.13	607.89	607.92
S1	34765.64	-20.13	607.95	607.97
☉ Brg. Pier 8	34778.64	-20.13	608.03	608.03
T1	34788.64	-20.13	608.09	608.11
U1	34798.64	-20.13	608.15	608.19
V1	34808.64	-20.13	608.20	608.27
W1	34818.64	-20.13	608.26	608.35
X1	34828.64	-20.13	608.32	608.43
Y1	34838.64	-20.13	608.38	608.50
Z1	34848.64	-20.13	608.44	608.54
A2	34858.64	-20.13	608.50	608.58
B2	34868.64	-20.13	608.56	608.62
C2	34878.64	-20.13	608.62	608.66
D2	34888.64	-20.13	608.68	608.70
☉ Brg. Pier 9	34898.64	-20.13	608.74	608.74
E2	34908.64	-20.13	608.79	608.81
F2	34918.64	-20.13	608.85	608.88
G2	34928.64	-20.13	608.91	608.96
H2	34938.64	-20.13	608.97	609.04
I2	34948.64	-20.13	609.03	609.12
J2	34958.64	-20.13	609.09	609.17
K2	34968.64	-20.13	609.15	609.23
L2	34978.64	-20.13	609.21	609.25
☉ Brg. N. Abut.	34991.64	-20.13	609.28	609.28
Bk. N. Abut.	34994.31	-20.13	609.30	609.30

DESIGNED - A. R. Sheblb
 CHECKED - N. R. Barnett
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

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 PASSED 
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS - SPANS 8 THRU 10 - UNIT 3 (S.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	56
ILLINOIS FED. AID PROJECT			CONTRACT NO. 74351	

WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34251.36	-18.00	605.72
A	34261.36	-18.00	605.73
B	34271.36	-18.00	605.75
North end of S. Appr. Slab	34281.36	-18.00	605.77

WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34253.54	-12.00	605.85
A	34263.54	-12.00	605.86
B	34273.54	-12.00	605.88
North end of S. Appr. Slab	34283.54	-12.00	605.90

☉ ROADWAY & PROFILE GRADE

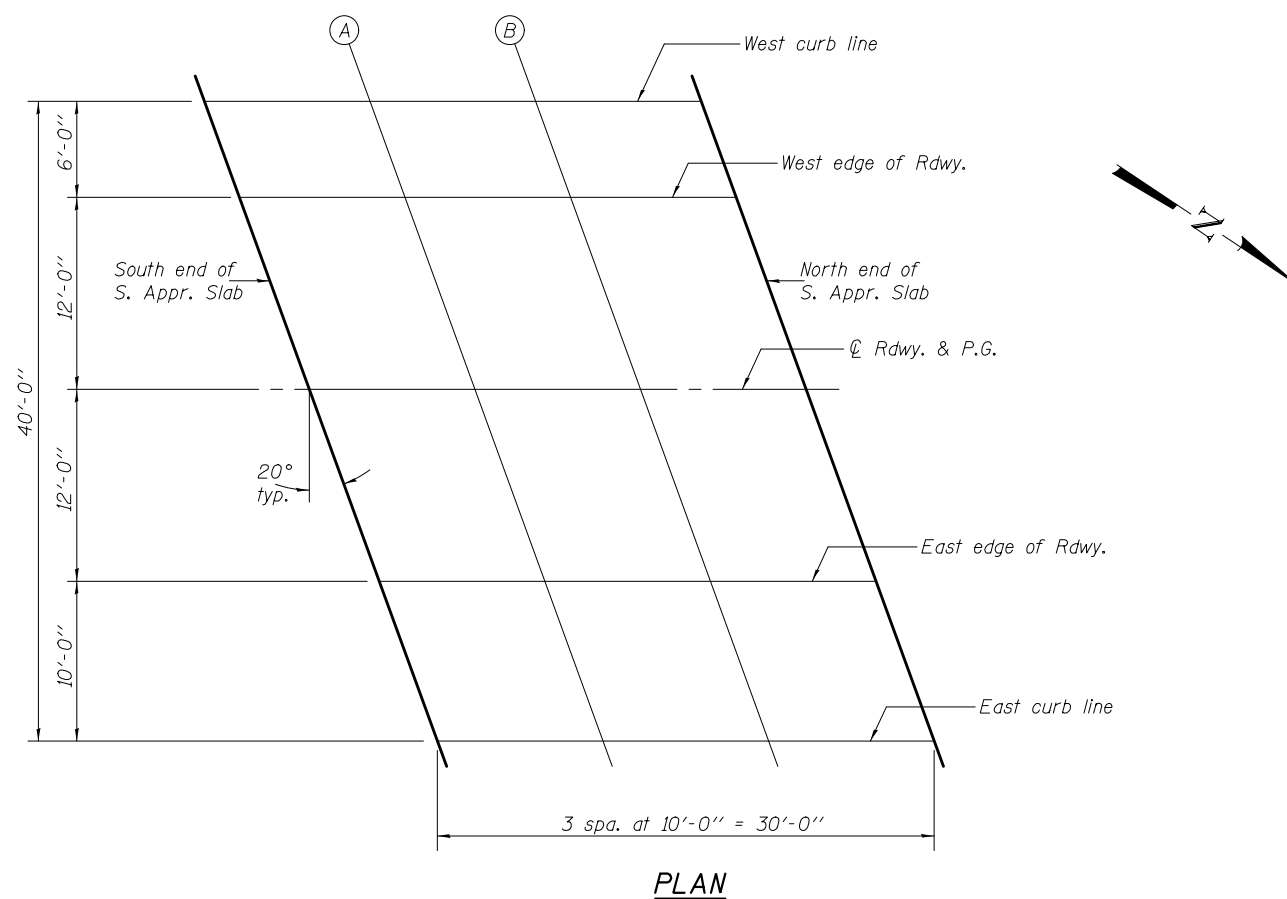
Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34257.91	0.00	606.04
A	34267.91	0.00	606.06
B	34277.91	0.00	606.07
North end of S. Appr. Slab	34287.91	0.00	606.09

EAST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34262.28	12.00	605.86
A	34272.28	12.00	605.88
B	34282.28	12.00	605.89
North end of S. Appr. Slab	34292.28	12.00	605.91

EAST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34265.92	22.00	605.66
A	34275.92	22.00	605.68
B	34285.92	22.00	605.69
North end of S. Appr. Slab	34295.92	22.00	605.71



DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED *Joanne F. Jolly*
 ENGINEER OF BRIDGE DESIGN
 PASSED *Carl Pinger*
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

TOP OF SOUTH APPROACH SLAB ELEVATIONS (N.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	57
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	35035.30	-18.00	609.59
M2	35045.30	-18.00	609.64
N2	35055.30	-18.00	609.70
North end of N. Appr. Slab	35065.30	-18.00	609.76

WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	35037.48	-12.00	609.72
M2	35047.48	-12.00	609.78
N2	35057.48	-12.00	609.84
North end of N. Appr. Slab	35067.48	-12.00	609.90

☉ ROADWAY & PROFILE GRADE

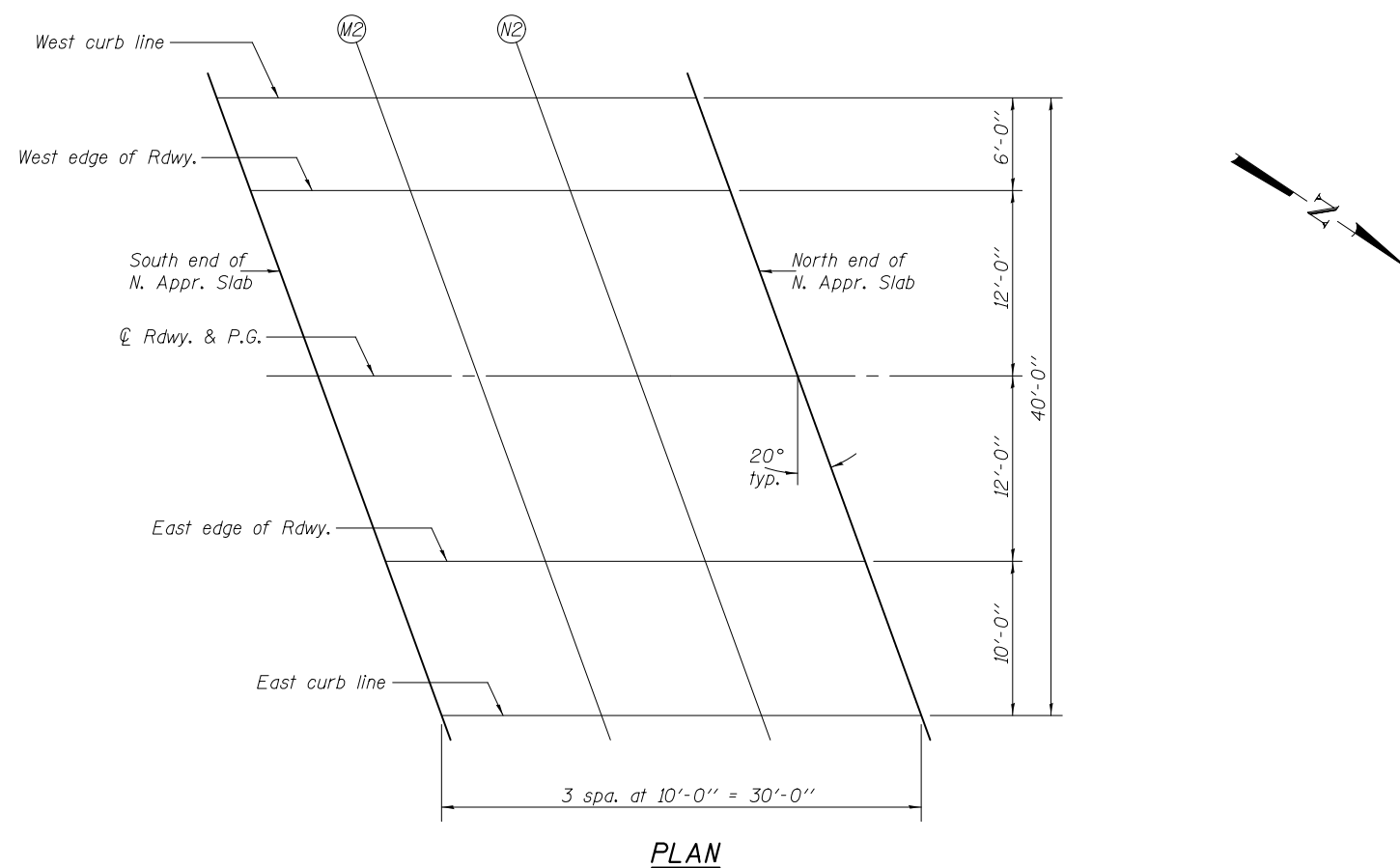
Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	35041.85	0.00	609.94
M2	35051.85	0.00	610.00
N2	35061.85	0.00	610.05
North end of N. Appr. Slab	35071.85	0.00	610.11

EAST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	35046.22	12.00	609.78
M2	35056.22	12.00	609.83
N2	35066.22	12.00	609.89
North end of N. Appr. Slab	35076.22	12.00	609.95

EAST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	35049.86	22.00	609.59
M2	35059.86	22.00	609.65
N2	35069.86	22.00	609.71
North end of N. Appr. Slab	35079.86	22.00	609.77



PLAN

DESIGNED - A. R. Shebli
 CHECKED - N. R. Barnett
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

TOP OF NORTH APPROACH SLAB ELEVATIONS (N.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	58
CONTRACT NO. 74351				

WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34209.14	-22.00	605.58
A	34219.14	-22.00	605.59
B	34229.14	-22.00	605.61
North end of S. Appr. Slab	34239.14	-22.00	605.62

WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34212.78	-12.00	605.80
A	34222.78	-12.00	605.81
B	34232.78	-12.00	605.82
North end of S. Appr. Slab	34242.78	-12.00	605.83

☉ ROADWAY & PROFILE GRADE

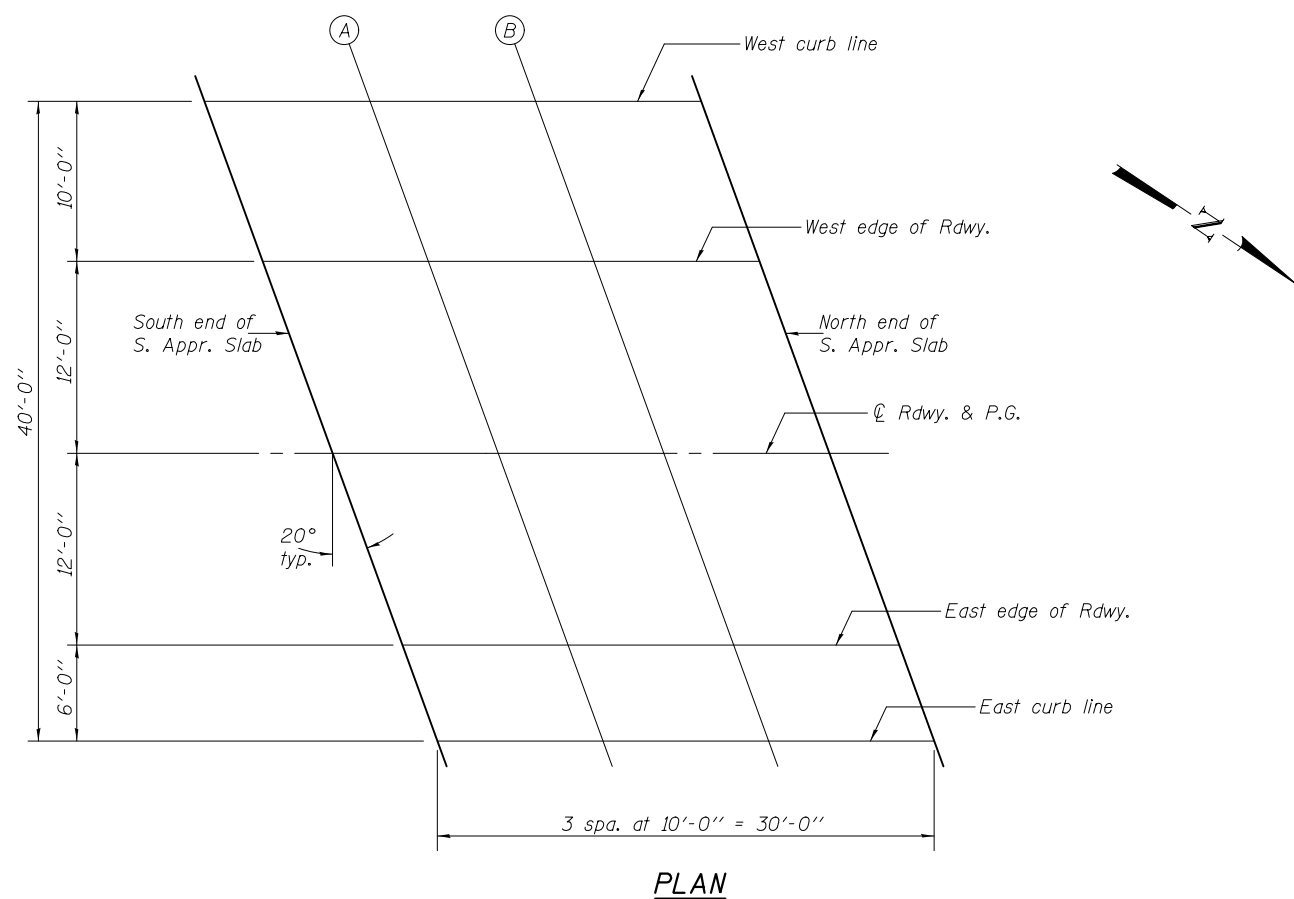
Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34217.15	0.00	605.99
A	34227.15	0.00	606.00
B	34237.15	0.00	606.01
North end of S. Appr. Slab	34247.15	0.00	606.02

EAST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34221.52	12.00	605.81
A	34231.52	12.00	605.82
B	34241.52	12.00	605.83
North end of S. Appr. Slab	34251.52	12.00	605.84

EAST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South end of S. Appr. Slab	34223.70	18.00	605.68
A	34233.70	18.00	605.69
B	34243.70	18.00	605.71
North end of S. Appr. Slab	34253.70	18.00	605.72



DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED *Joanne F. [Signature]*
 ENGINEER OF BRIDGE DESIGN
 PASSED *Carl [Signature]*
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

TOP OF SOUTH APPROACH SLAB ELEVATIONS (S.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	59
				CONTRACT NO. 74351

WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	34993.08	-22.00	609.25
M2	35003.08	-22.00	609.31
N2	35013.08	-22.00	609.37
North end of N. Appr. Slab	35023.08	-22.00	609.43

WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	34996.72	-12.00	609.48
M2	35006.72	-12.00	609.54
N2	35016.72	-12.00	609.60
North end of N. Appr. Slab	35026.72	-12.00	609.66

☉ ROADWAY & PROFILE GRADE

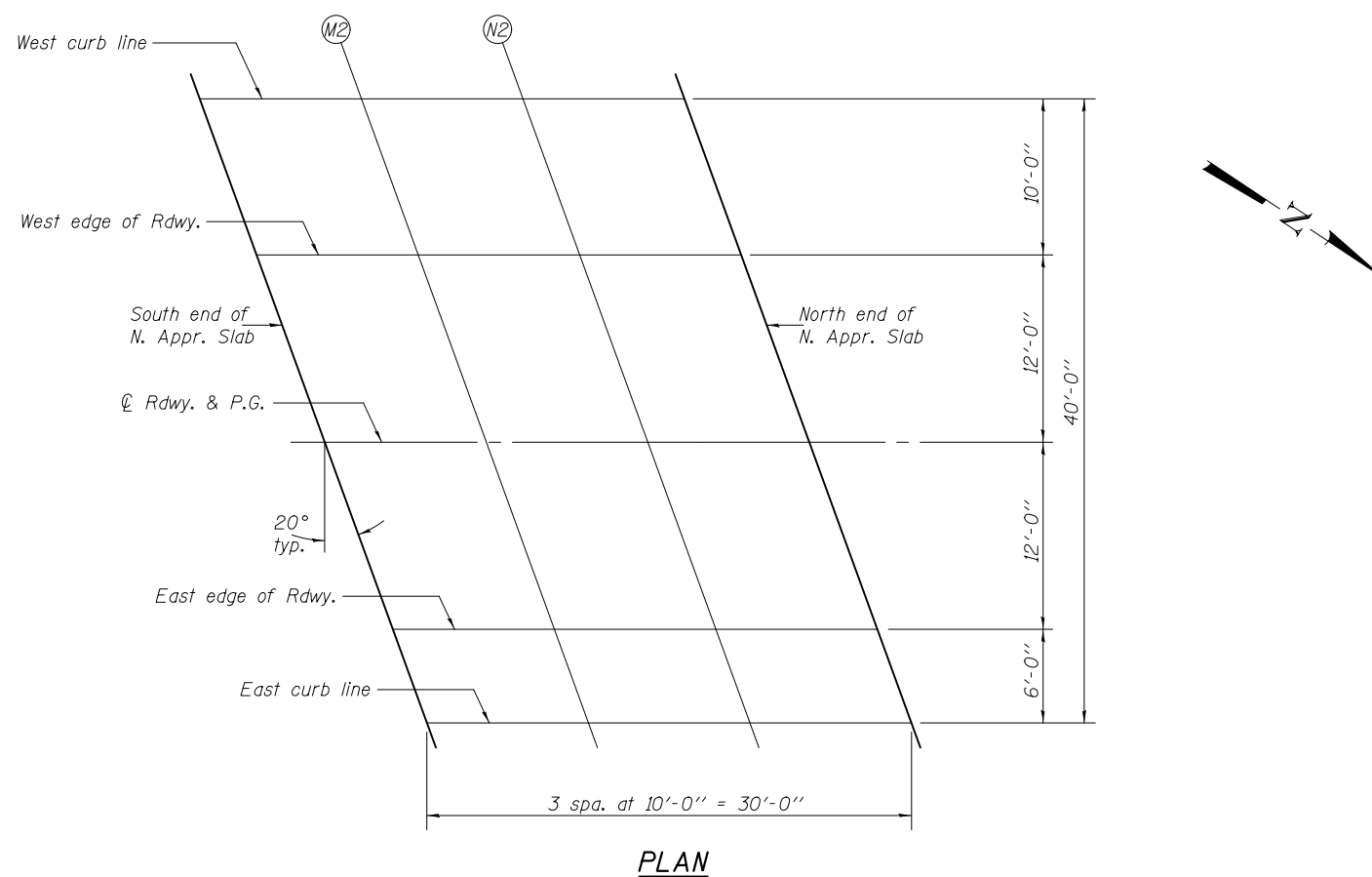
Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	35001.09	0.00	609.70
M2	35011.09	0.00	609.76
N2	35021.09	0.00	609.81
North end of N. Appr. Slab	35031.09	0.00	609.87

EAST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	35005.46	12.00	609.53
M2	35015.46	12.00	609.59
N2	35025.46	12.00	609.65
North end of N. Appr. Slab	35035.46	12.00	609.71

EAST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
South end of N. Appr. Slab	35007.64	18.00	609.42
M2	35017.64	18.00	609.48
N2	35027.64	18.00	609.54
North end of N. Appr. Slab	35037.64	18.00	609.60



PLAN

DESIGNED - A. R. Shebli
 CHECKED - N. R. Barnett
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

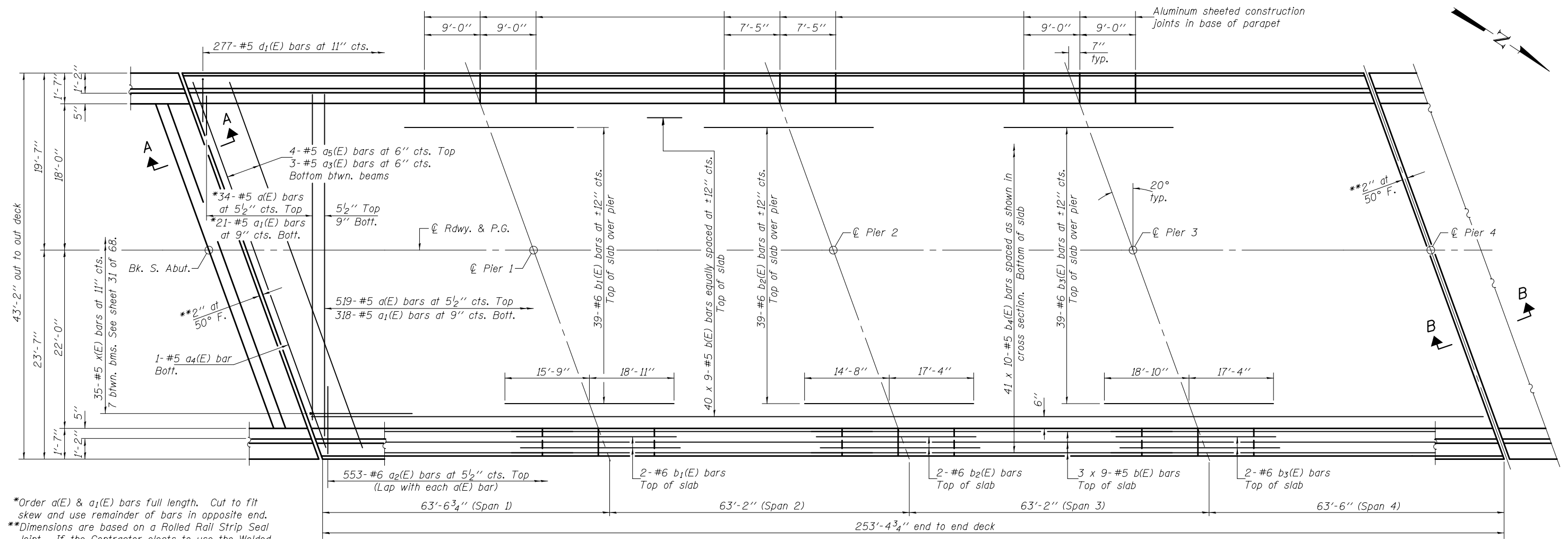
DATE - MAY 6, 2015
 REVISED
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

TOP OF NORTH APPROACH SLAB ELEVATIONS (S.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

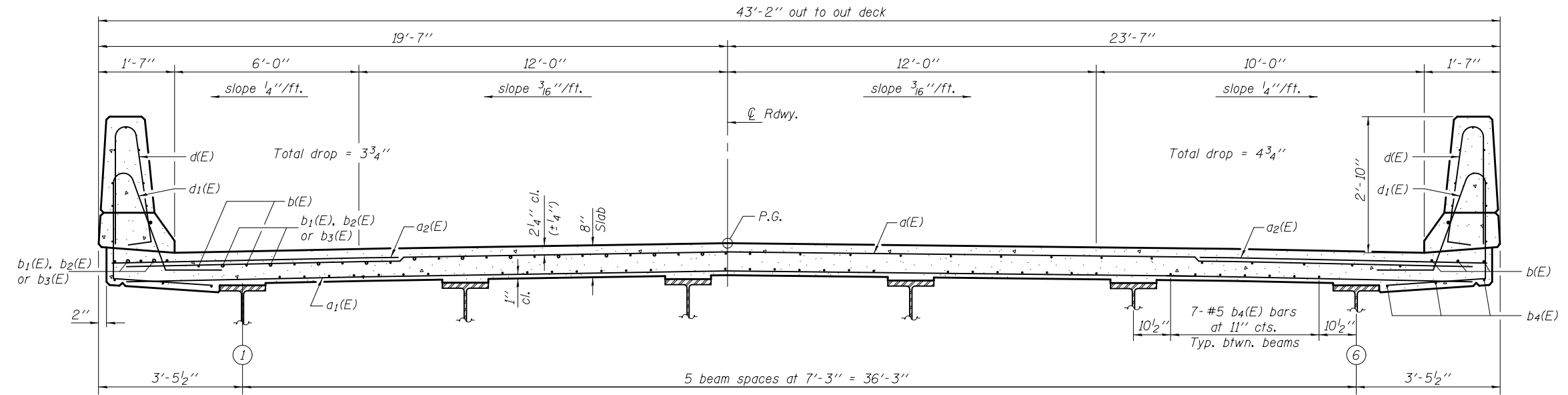
SHEET NO. 23 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	60
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				



PLAN

*Order a(E) & a1(E) bars full length. Cut to fit skew and use remainder of bars in opposite end.
 **Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Seal Joint, the opening and deck dimensions shall be adjusted to satisfy the details on base sheet EJ-SSJ on sheet 39 of 68.

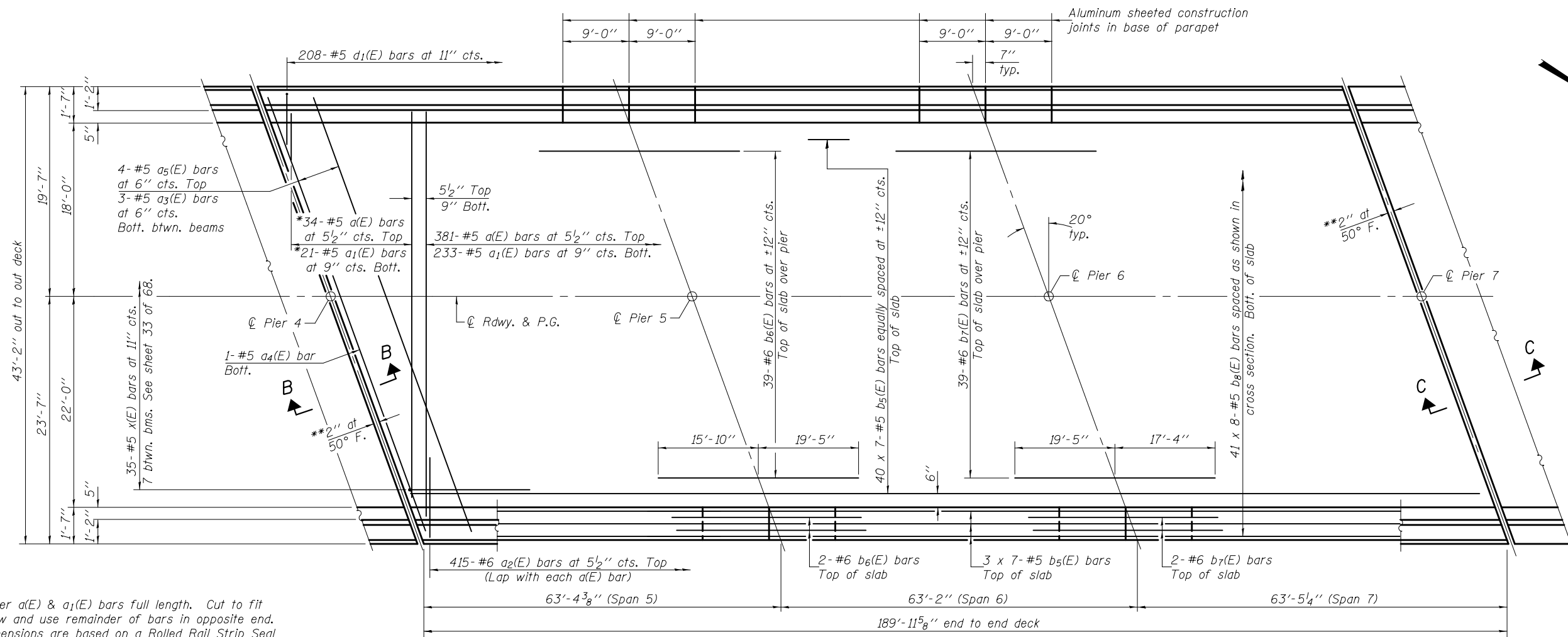


CROSS SECTION
(Looking North)

Notes:
 See sheets 30 & 31 of 68 for superstructure details and Bill of Material.
 Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
 See sheet 30 of 68 for parapet reinforcement.
 See sheet 31 of 68 for Sections A-A & B-B.

MIN. BAR LAP
 #5 bar = 3'-3"

DESIGNED - N. R. Barnett	EXAMINED - <i>Joey F. [Signature]</i>	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE - SPANS 1 THRU 4 - UNIT 1 (N.B.) STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE. 322	SECTION (58-208-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 61	
CHECKED - J. D. Ortiz-Varela	PASSED - <i>Carl [Signature]</i>	REVISOR			CONTRACT NO. 74351					
DRAWN - h.t. duong	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISOR			SHEET NO. 24 OF 68 SHEETS					
CHECKED - NRB/JOV/GRA					ILLINOIS FED. AID PROJECT					

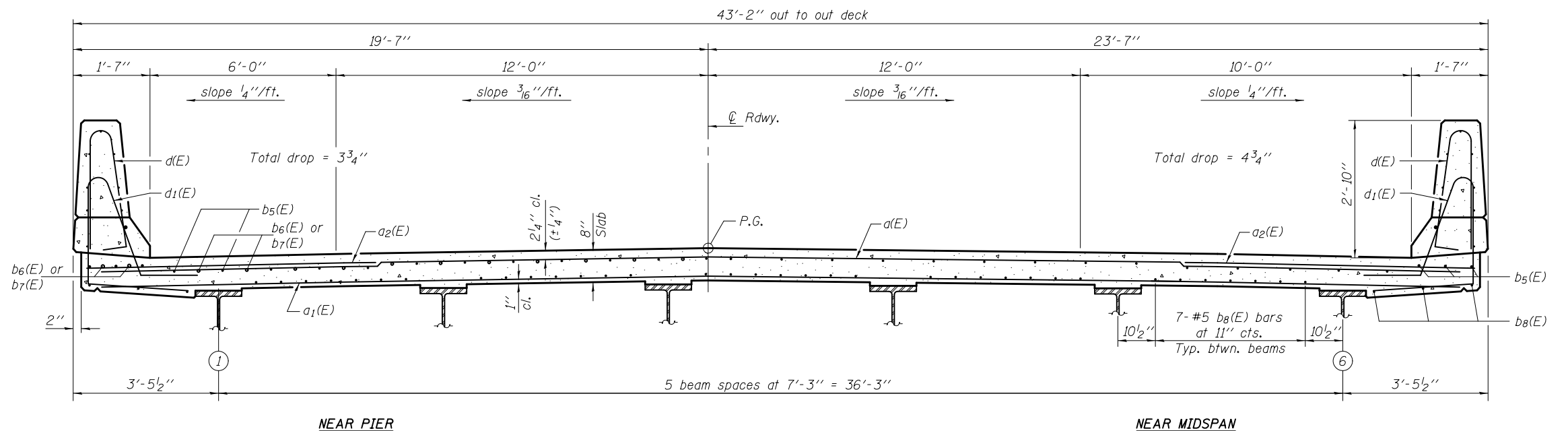


PLAN

*Order a(E) & a₁(E) bars full length. Cut to fit skew and use remainder of bars in opposite end.
*Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Seal Joint, the opening and deck dimensions shall be adjusted to satisfy the details on base sheet EJ-SSJ on sheet 39 of 68.

Notes:
See sheets 32 & 33 of 68 for superstructure details and Bill of Material.
Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
See sheet 32 of 68 for parapet reinforcement.
See sheet 33 of 68 for Sections B-B & C-C.

MIN. BAR LAP
#5 bar = 3'-3"



CROSS SECTION
(Looking North)

DESIGNED - N. R. Barnett
CHECKED - A. R. Sheblb
DRAWN - h.t. duong
CHECKED - NRB/JMO/GRA

EXAMINED
PASSED
ENGINEER OF BRIDGE DESIGN
ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
REVISED
REVISED

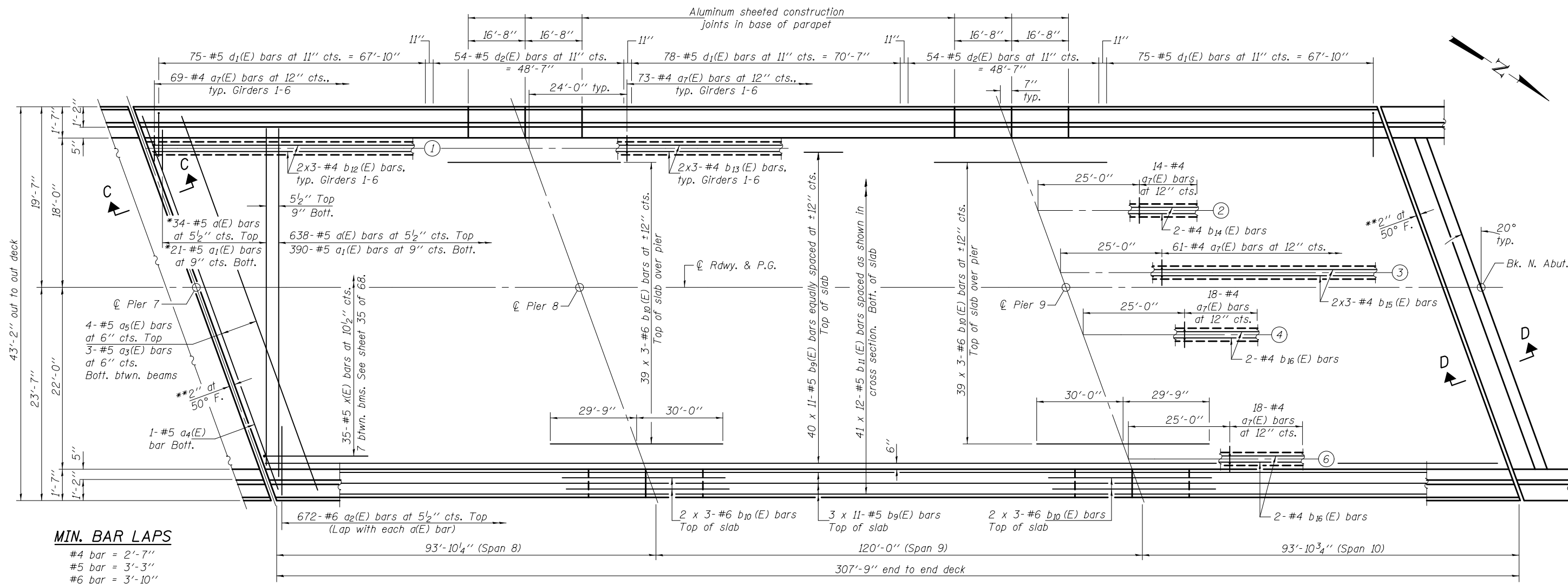
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE - SPANS 5 THRU 7 - UNIT 2 (N.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 25 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	62
CONTRACT NO. 74351				

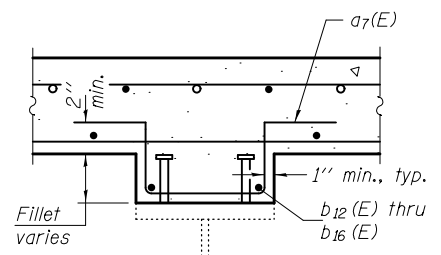
ILLINOIS FED. AID PROJECT



MIN. BAR LAPS

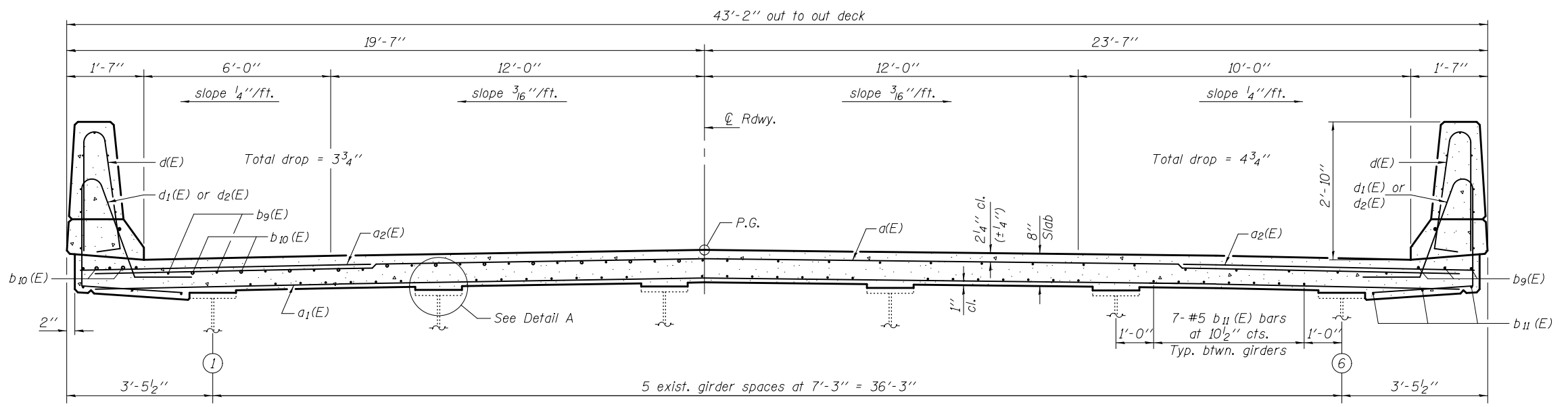
- #4 bar = 2'-7"
- #5 bar = 3'-3"
- #6 bar = 3'-10"

*Order a(E) & a₁(E) bars full length. Cut to fit skew and use remainder of bars in opposite end.
 **Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Seal Joint, the opening and deck dimensions shall be adjusted to satisfy the details on base sheet EJ-SSJ on sheet 39 of 68.



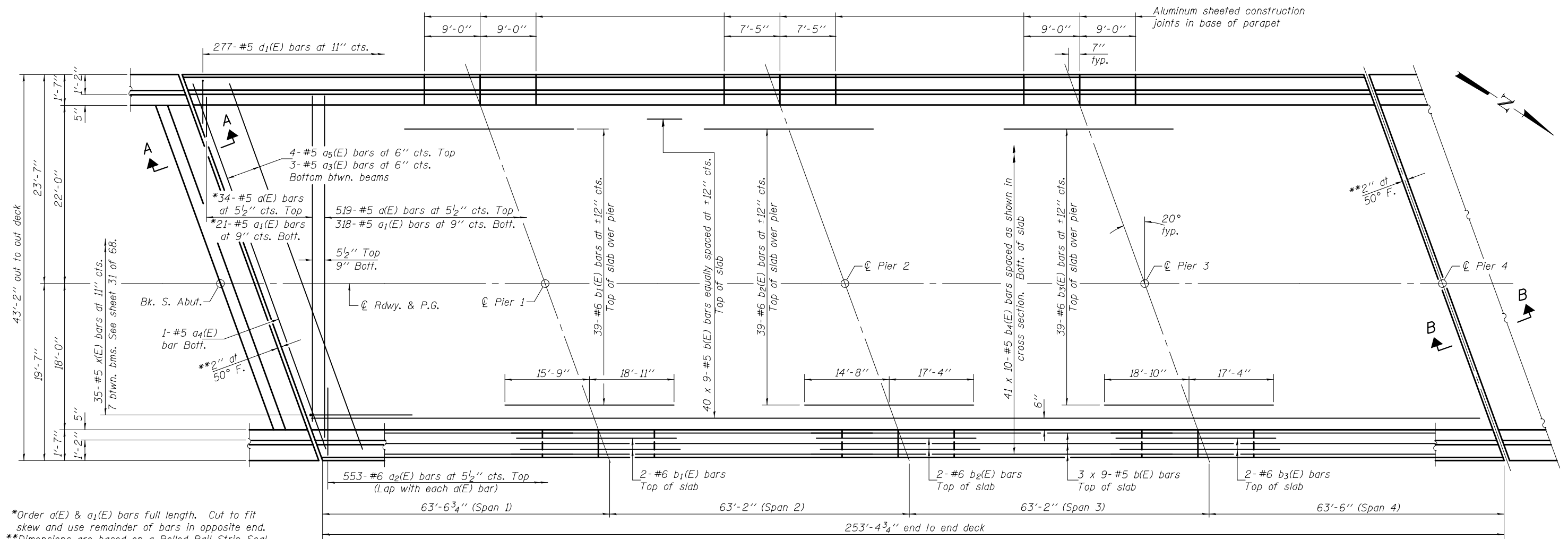
DETAIL A

Notes:
 See sheets 34 & 35 of 68 for superstructure details and Bill of Material.
 Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
 See sheet 34 of 68 for parapet reinforcement.
 See sheet 35 of 68 for Sections C-C & D-D.



CROSS SECTION
(Looking North)

DESIGNED - A. R. Sheblb	EXAMINED - <i>Jayne F. Joffe</i>	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE - SPANS 8 THRU 10 - UNIT 3 (N.B.) STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 63	
CHECKED - N. R. Barnett	PASSED - <i>Carl P. King</i>	REVISED			CONTRACT NO. 74351					
DRAWN - h.t. duong	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED			ILLINOIS FED. AID PROJECT					
CHECKED - NRB/JOV/GRA					SHEET NO. 26 OF 68 SHEETS					

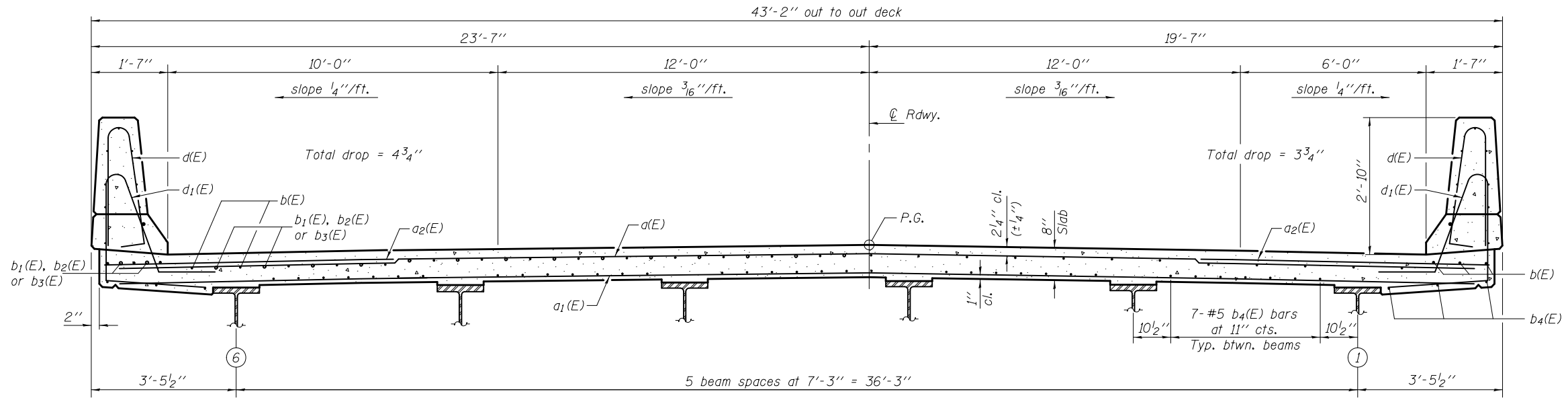


*Order a(E) & a₁(E) bars full length. Cut to fit skew and use remainder of bars in opposite end.
 **Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Seal Joint, the opening and deck dimensions shall be adjusted to satisfy the details on base sheet EJ-SSJ on sheet 39 of 68.

PLAN

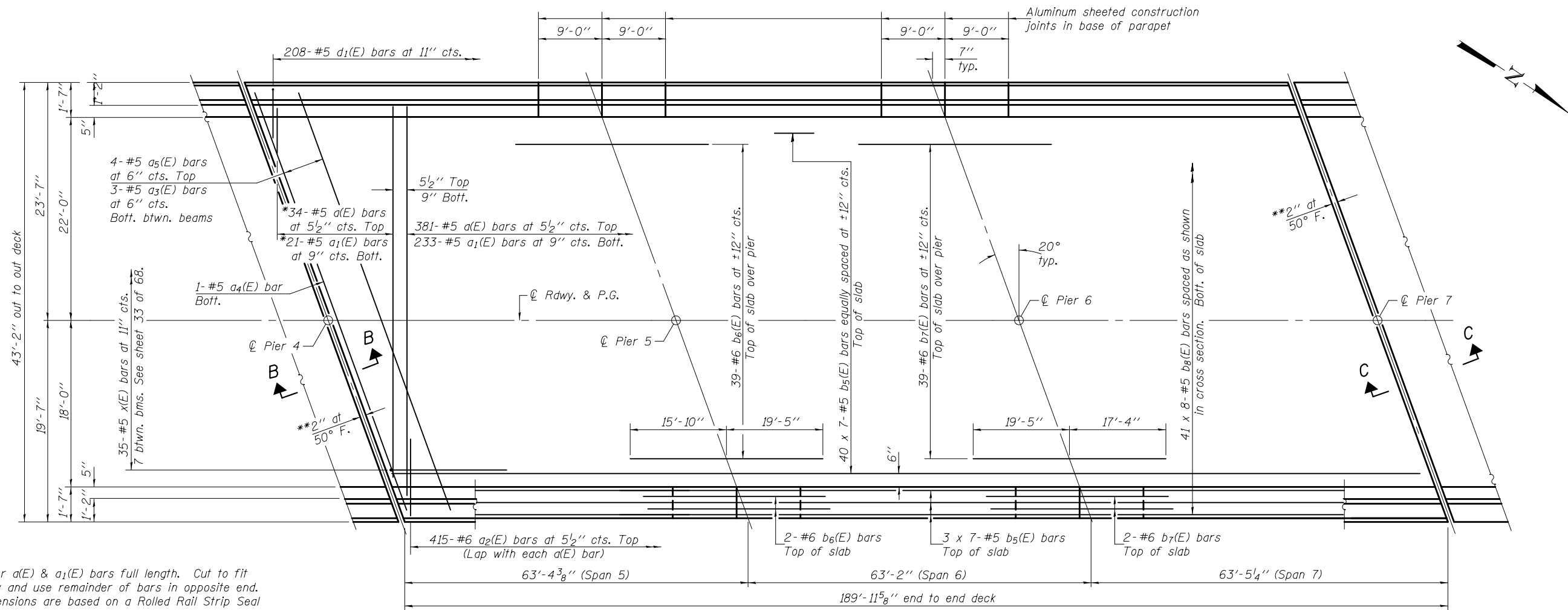
MIN. BAR LAP
 #5 bar = 3'-3"

Notes:
 See sheets 30 & 31 of 68 for superstructure details and Bill of Material.
 Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
 See sheet 30 of 68 for parapet reinforcement.
 See sheet 31 of 68 for Sections A-A & B-B.



CROSS SECTION
 (Looking North)

DESIGNED - N. R. Barnett	EXAMINED - <i>Joanne F. [Signature]</i>	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE - SPANS 1 THRU 4 - UNIT 1 (S.B.) STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 64	
CHECKED - J. D. Ortiz-Varela	PASSED - <i>Carl [Signature]</i>	REVISOR			CONTRACT NO. 74351					
DRAWN - h.t. duong	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISOR			SHEET NO. 27 OF 68 SHEETS					
CHECKED - NRB/JOV/GRA					ILLINOIS FED. AID PROJECT					

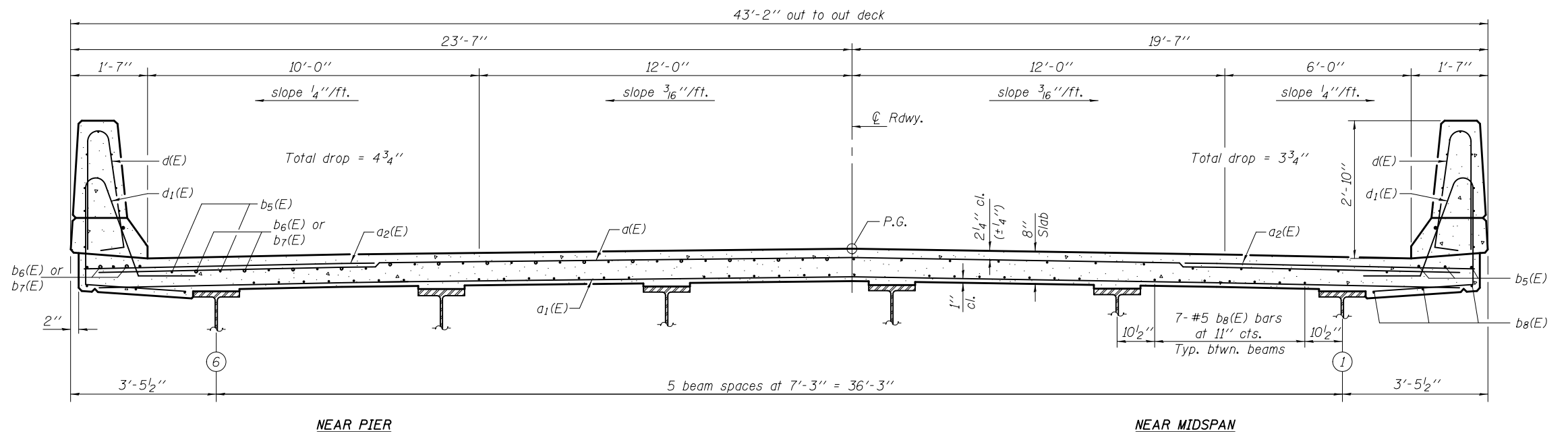


*Order a(E) & a₁(E) bars full length. Cut to fit skew and use remainder of bars in opposite end.
**Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Seal Joint, the opening and deck dimensions shall be adjusted to satisfy the details on base sheet EJ-SSJ on sheet 39 of 68.

PLAN

Notes:
See sheets 32 & 33 of 68 for superstructure details and Bill of Material.
Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
See sheet 32 of 68 for parapet reinforcement.
See sheet 33 of 68 for Sections B-B & C-C.

MIN. BAR LAP
#5 bar = 3'-3"



CROSS SECTION
(Looking North)

DESIGNED - N. R. Barnett
CHECKED - A. R. Sheblb
DRAWN - h.t. duong
CHECKED - NRB/JMO/GRA

EXAMINED
PASSED
ENGINEER OF BRIDGE DESIGN
ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
REVISED
REVISED

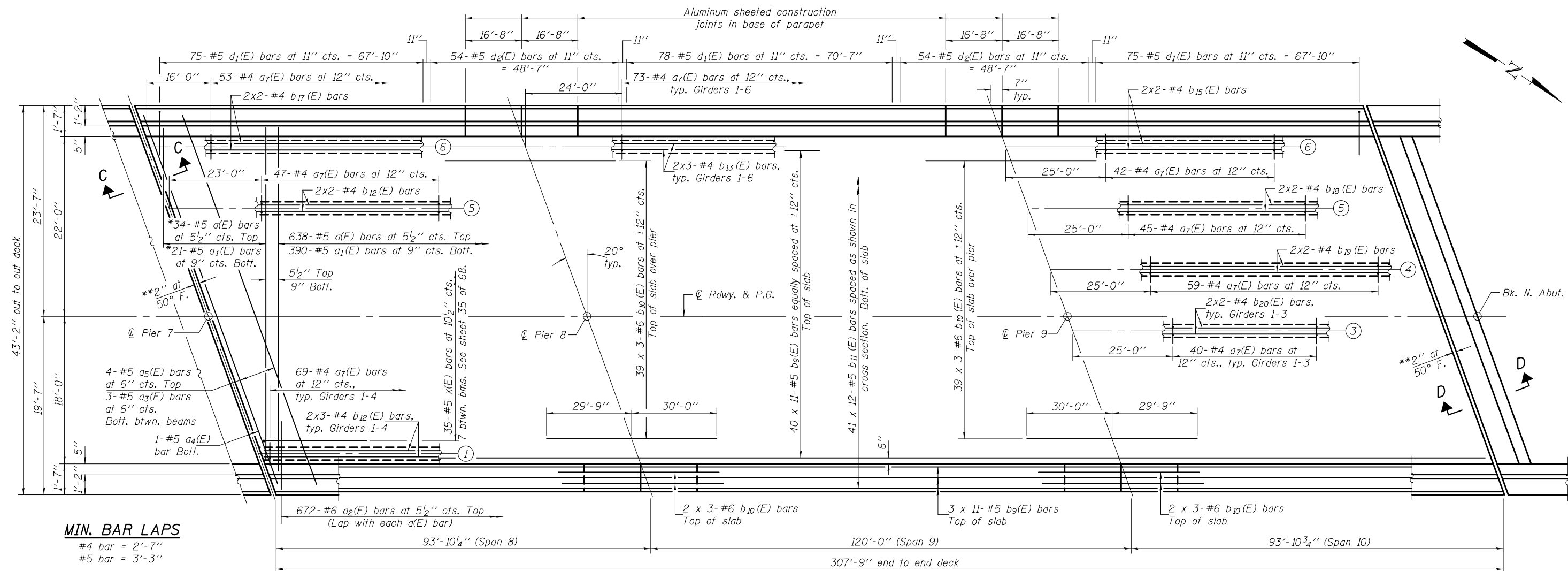
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE - SPANS 5 THRU 7 - UNIT 2 (S.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 28 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	65
CONTRACT NO. 74351				

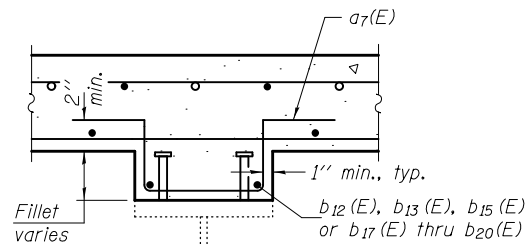
ILLINOIS FED. AID PROJECT



MIN. BAR LAPS

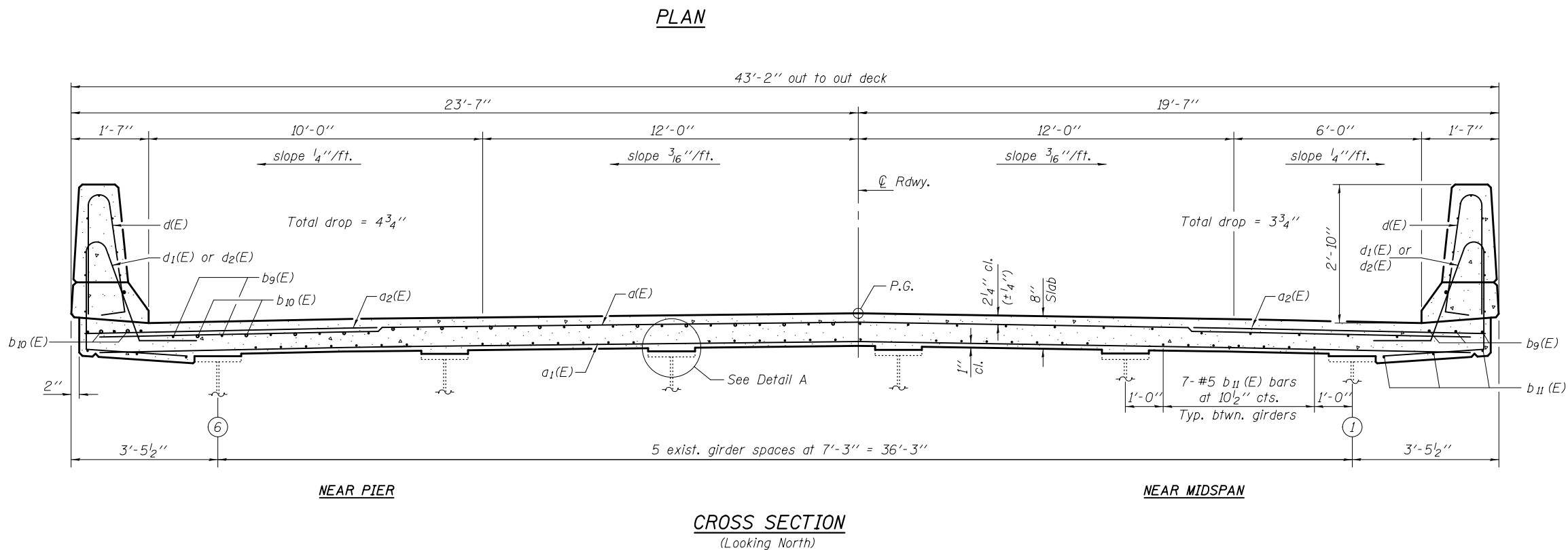
#4 bar = 2'-7"
#5 bar = 3'-3"

*Order a(E) & a₁(E) bars full length. Cut to fit skew and use remainder of bars in opposite end.
**Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Seal Joint, the opening and deck dimensions shall be adjusted to satisfy the details on base sheet EJ-SSJ on sheet 39 of 68.



DETAIL A

Notes:
See sheets 34 & 35 of 68 for superstructure details and Bill of Material.
Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
See sheet 34 of 68 for parapet reinforcement.
See sheet 35 of 68 for Sections C-C & D-D.



CROSS SECTION
(Looking North)

DESIGNED - A. R. Sheblb	EXAMINED - <i>Jayne F. Joffe</i>
CHECKED - N. R. Barnett	PASSED - <i>Carl Pung</i>
DRAWN - h.t. duong	
CHECKED - NRB/JOV/GRA	

DATE - MAY 6, 2015
REVISOR
REVISOR

DATE - MAY 6, 2015
REVISOR
REVISOR

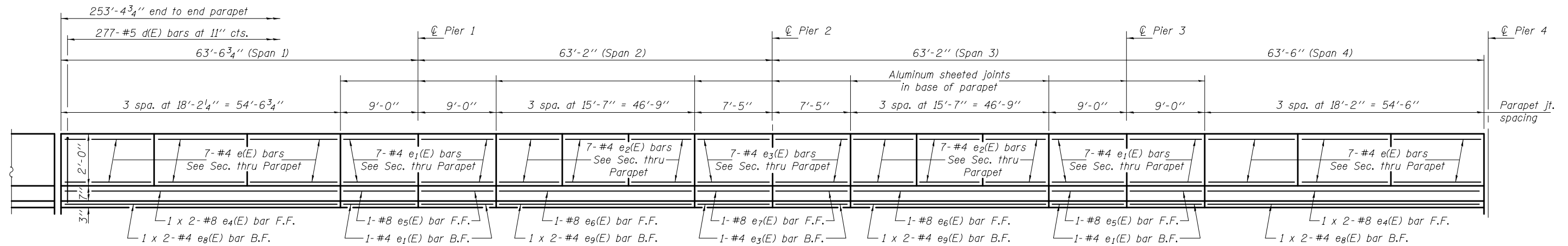
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE - SPANS 8 THRU 10 - UNIT 3 (S.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 29 OF 68 SHEETS

F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 66
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

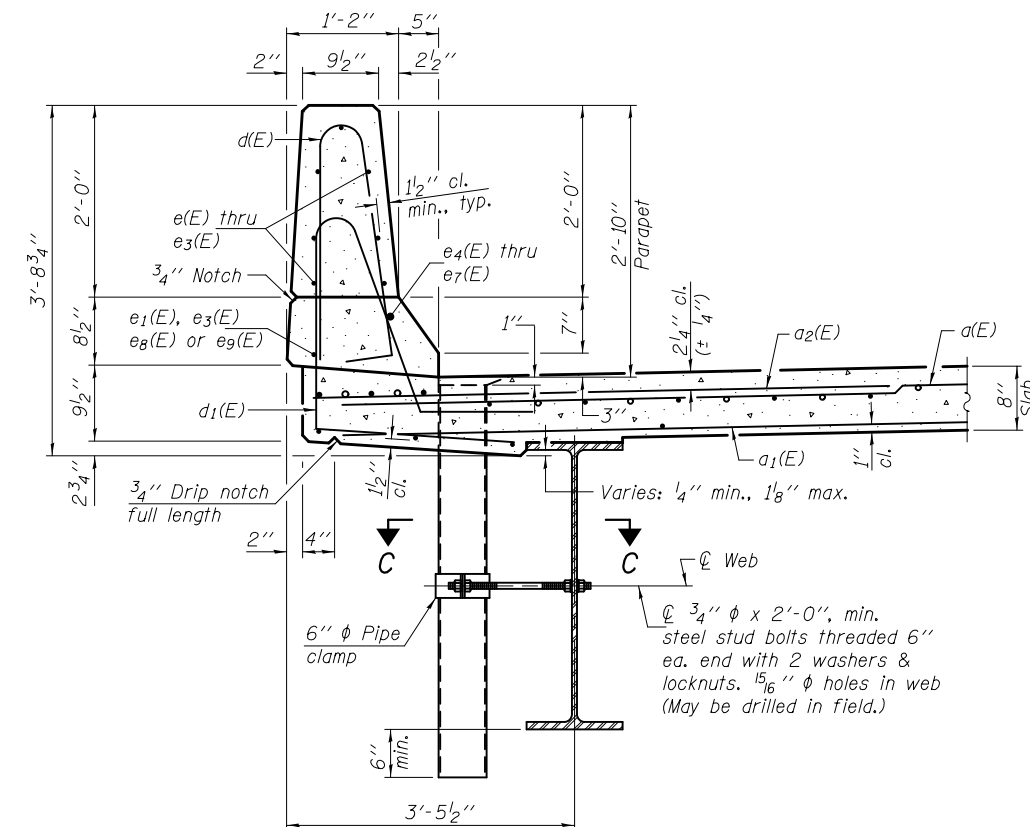


INSIDE ELEVATION OF WEST PARAPET - SPANS 1 THRU 4 - UNIT 1

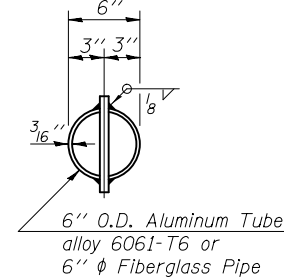
(Looking West - East parapet similar)

MIN. BAR LAPS

(Parapet)
 #4 bar = 2'-0"
 #8 bar = 5'-2"

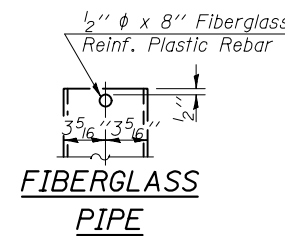


SECTION THRU PARAPET

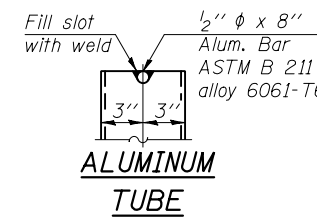


TOP PLAN

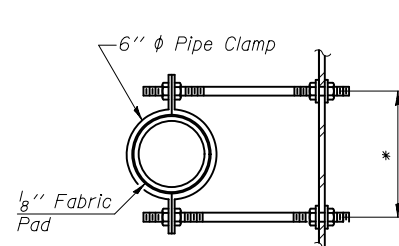
(Showing Aluminum Tube)



FIBERGLASS PIPE

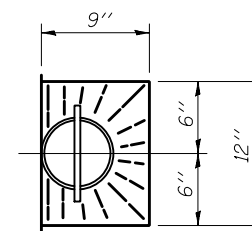


ALUMINUM TUBE

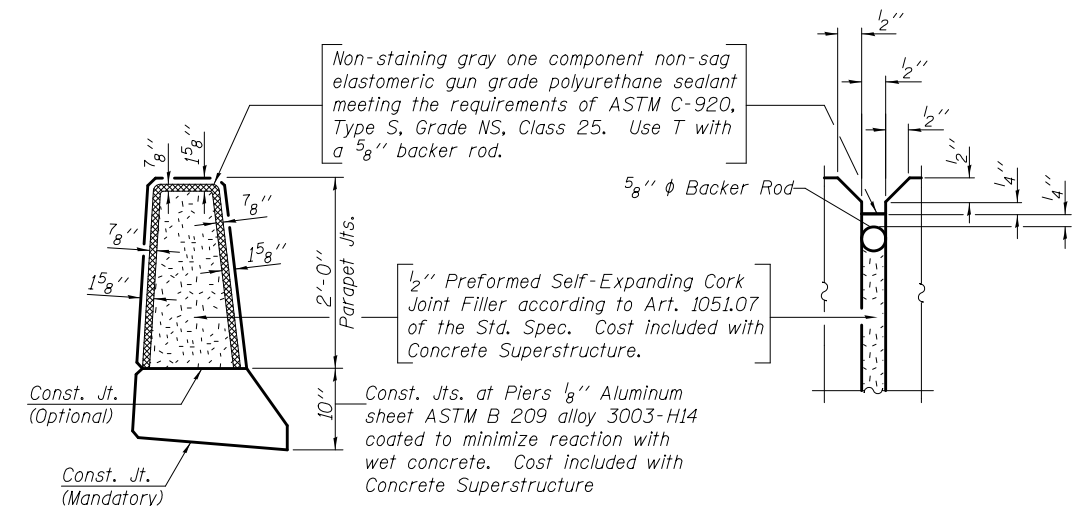


SECTION C-C

*Dimension as required by Pipe Clamp



TOP PLAN



PARAPET JOINT DETAILS

Notes:

Drains shall be located clear of all diaphragms.
 The exterior surfaces of the floor drains shall be painted with the finish coat as specified in Article 506 of the Standard Specifications. The exterior surfaces of the drains shall be cleaned according to the Society of Protective Coatings' Spec. SSPC-SPI prior to painting.
 Fiberglass pipe shall conform to ASTM D 2996, with short-time rupture strength hoop tensile stress of 30,000 p.s.i. minimum.
 Galvanize clamping device according to AASHTO M232. Cost of clamping device and inserts is included with Floor Drains.

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED - *Joanne F. Duff*
 ENGINEER OF BRIDGE DESIGN
 PASSED - *Carl Perry*
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

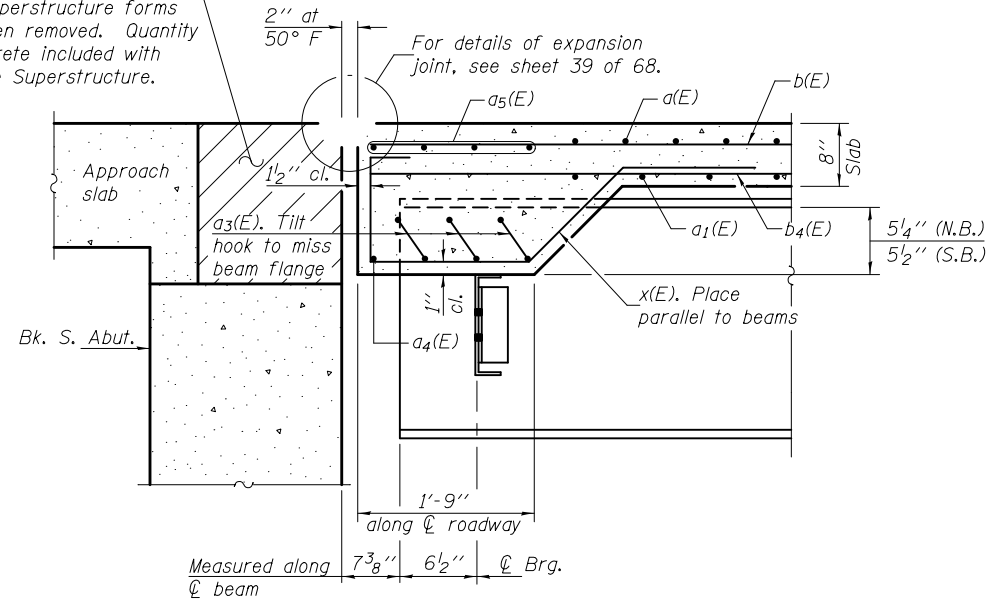
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**SUPERSTRUCTURE DETAILS - SPANS 1 THRU 4 - UNIT 1
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

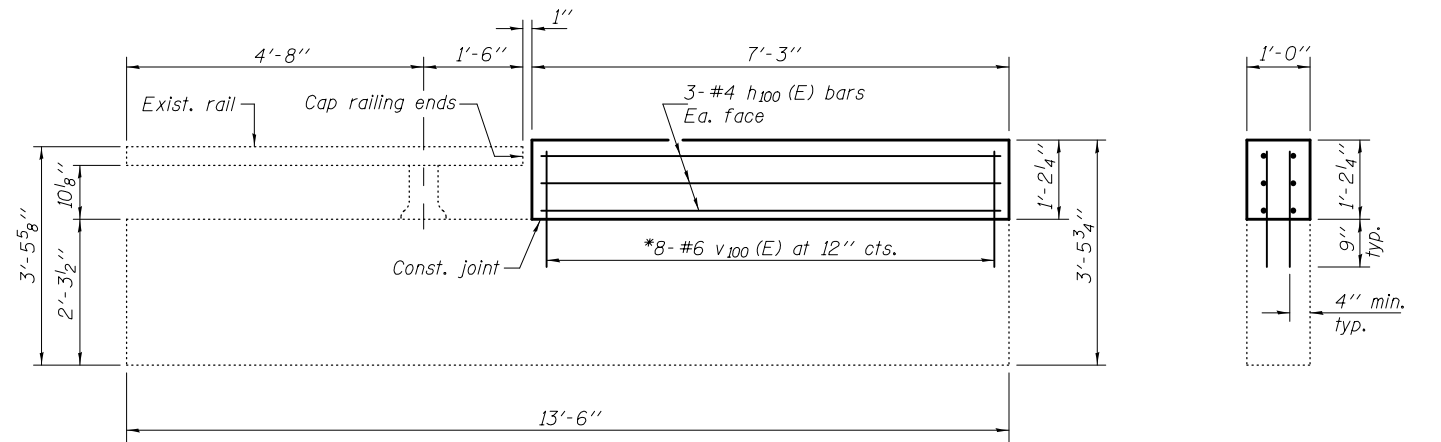
SHEET NO. 30 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	67
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				

Hatched area to be poured after superstructure forms have been removed. Quantity of concrete included with Concrete Superstructure.



SECTION A-A



EXISTING END POST MODIFICATION FOR TEMPORARY GUARDRAIL - STAGE I TRAFFIC

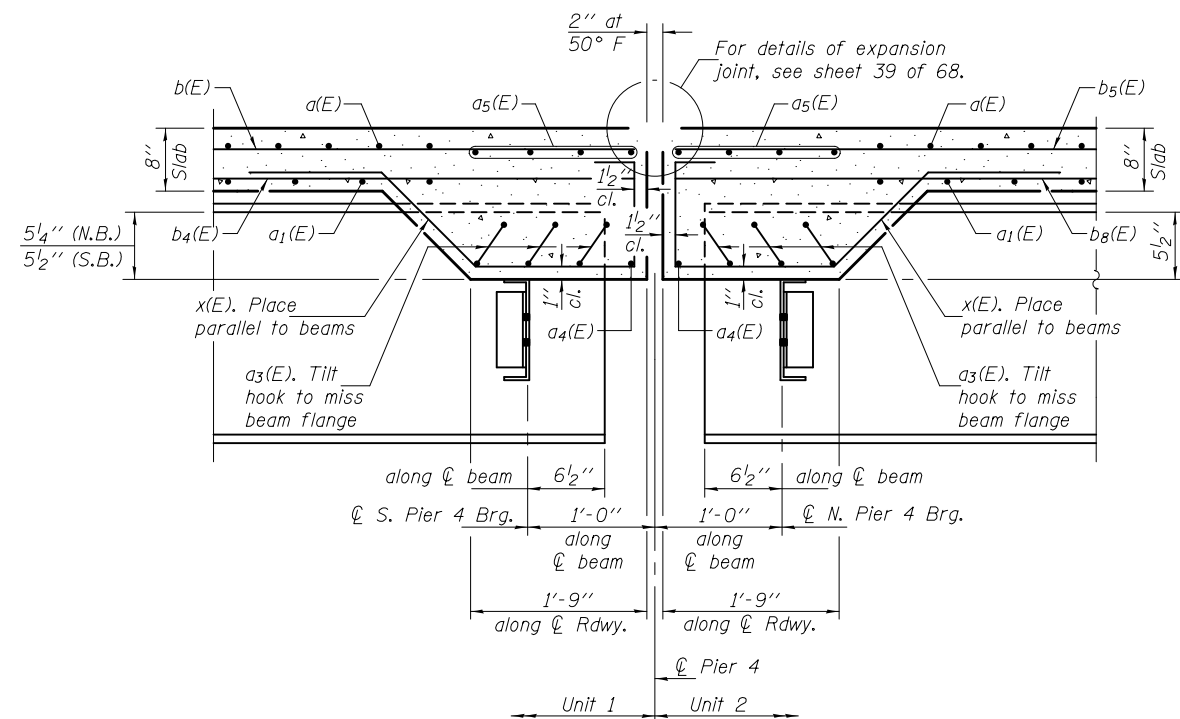
Existing East Endpost - South Abutment (S.B.)
(Looking East)

*Epoxy grout v100 (E) bars in 9" min. drilled holes according to Section 584 of the Standard Specifications.

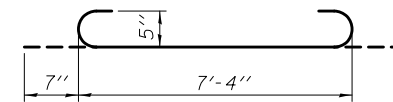
TWO SUPERSTRUCTURES (N.B. & S.B.) - UNIT 1
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	1106	#5	42'-6"	—
a1(E)	678	#5	40'-10"	—
a2(E)	2212	#6	6'-6"	—
a3(E)	60	#5	8'-6"	U
a4(E)	4	#5	38'-3"	—
a5(E)	16	#5	45'-3"	—
b(E)	828	#5	31'-1"	—
b1(E)	86	#6	34'-8"	—
b2(E)	86	#6	32'-0"	—
b3(E)	86	#6	36'-2"	—
b4(E)	820	#5	28'-2"	—
d(E)	1108	#5	5'-7"	L
d1(E)	1108	#5	8'-2"	L
e(E)	168	#4	17'-10"	—
e1(E)	128	#4	8'-9"	—
e2(E)	168	#4	15'-4"	—
e3(E)	64	#4	7'-2"	—
e4(E)	16	#8	29'-8"	—
e5(E)	16	#8	8'-9"	—
e6(E)	8	#8	46'-6"	—
e7(E)	8	#8	7'-2"	—
e8(E)	16	#4	28'-2"	—
e9(E)	16	#4	24'-3"	—
h100 (E)	6	#4	7'-0"	—
v100 (E)	16	#6	1'-9"	—
x(E)	140	#5	6'-5"	U
Reinforcement Bars, Epoxy Coated		Pound	190180	
Concrete Superstructure		Cu. Yds.	389.9	

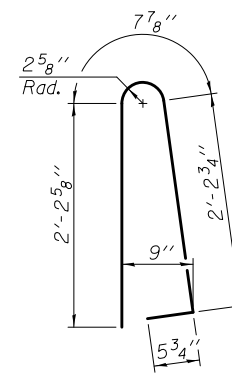
Bars indicated thus 1 x 2-#8 etc. indicates 1 line of bars with 2 lengths per line.



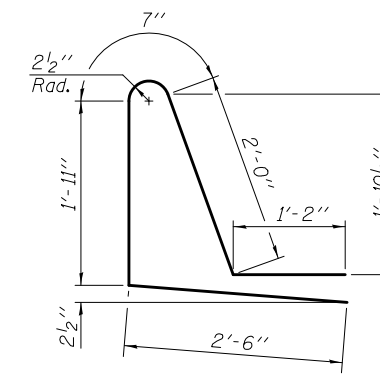
SECTION B-B



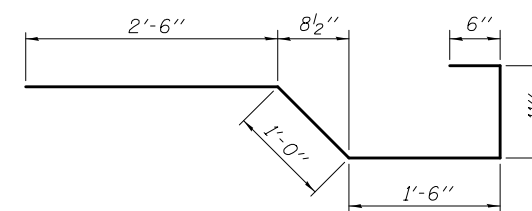
BAR a3(E)



BAR d(E)



BAR d1(E)



BAR x(E)

DESIGNED - N. R. Barnett
CHECKED - J. D. Ortiz-Varela
DRAWN - h.t. duong
CHECKED - NRB/JOV/GRA

EXAMINED
PASSED
ENGINEER OF BRIDGE DESIGN
ACTING ENGINEER OF BRIDGES AND STRUCTURES

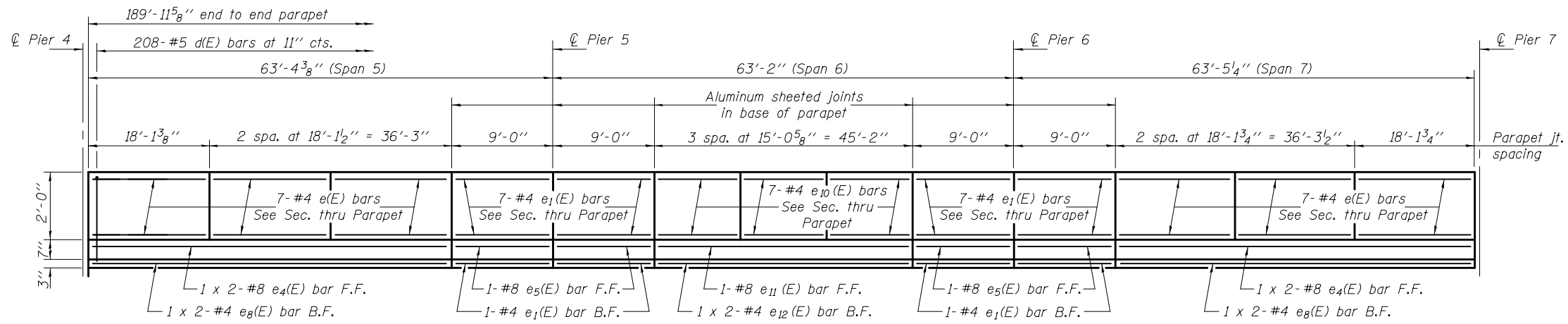
DATE - MAY 6, 2015
REVISED
REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS - SPANS 1 THRU 4 - UNIT 1
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

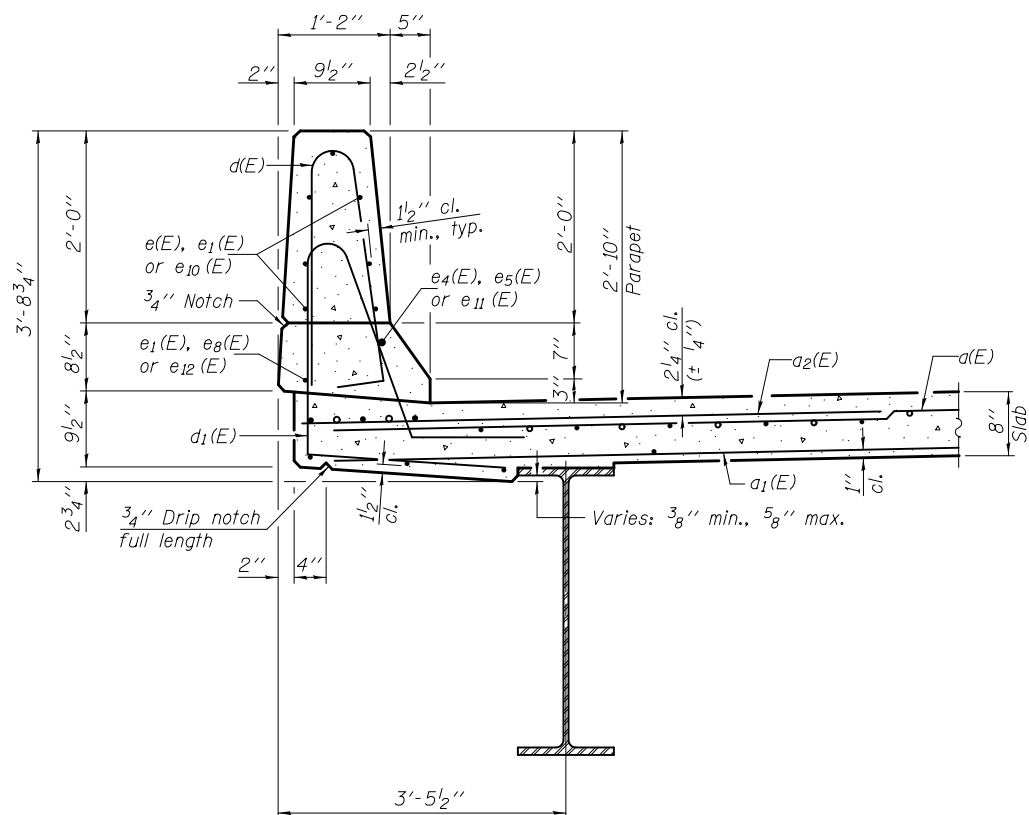
SHEET NO. 31 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	68
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				

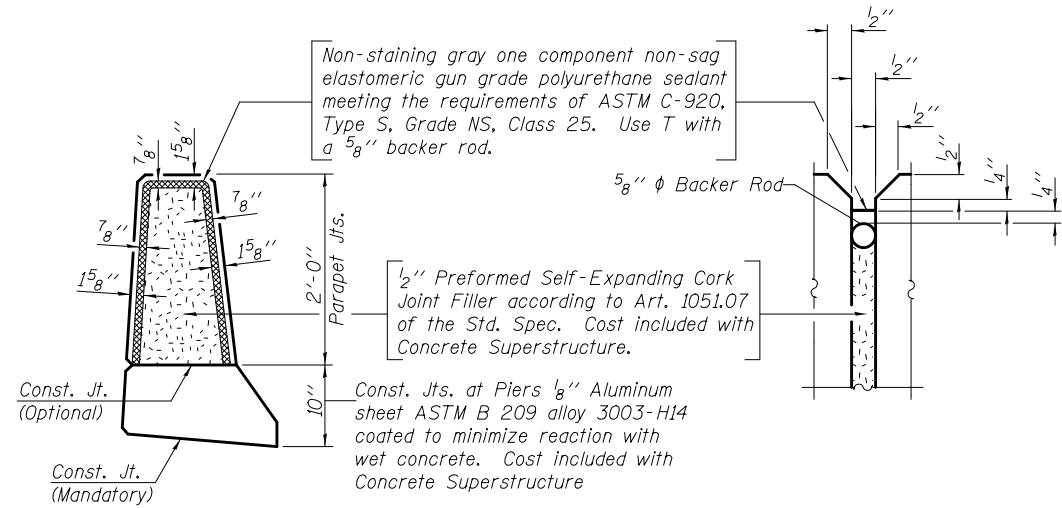


INSIDE ELEVATION OF WEST PARAPET - SPANS 5 THRU 7 - UNIT 2
 (Looking West - East parapet similar)

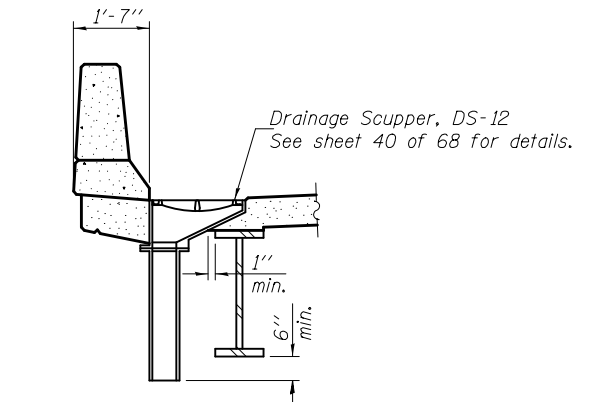
MIN. BAR LAPS
 (Parapet)
 #4 bar = 2'-0"
 #8 bar = 5'-2"



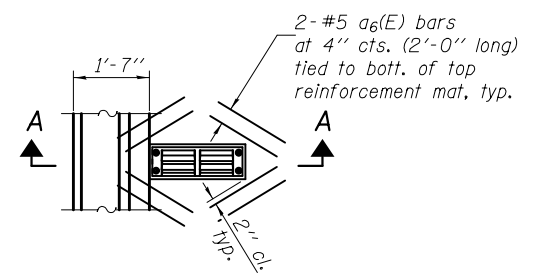
SECTION THRU PARAPET



PARAPET JOINT DETAILS



SECTION A-A



PLAN

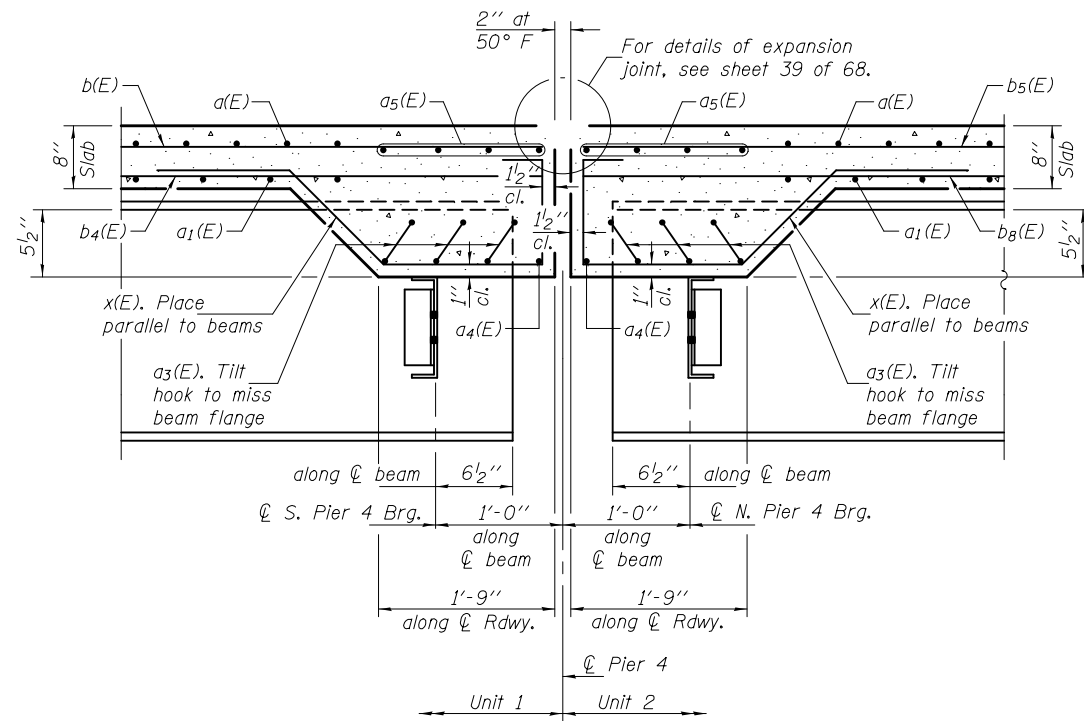
Note:
 Cut longitudinal reinforcement to clear drainage scuppers.

DESIGNED - N. R. Barnett	EXAMINED - <i>Joanne F. [Signature]</i>	DATE - MAY 6, 2015
CHECKED - A. R. Sheblb	PASSED - <i>Carl [Signature]</i>	REVISED
DRAWN - h.t. duong	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED
CHECKED - NRB/JMO/GRA		

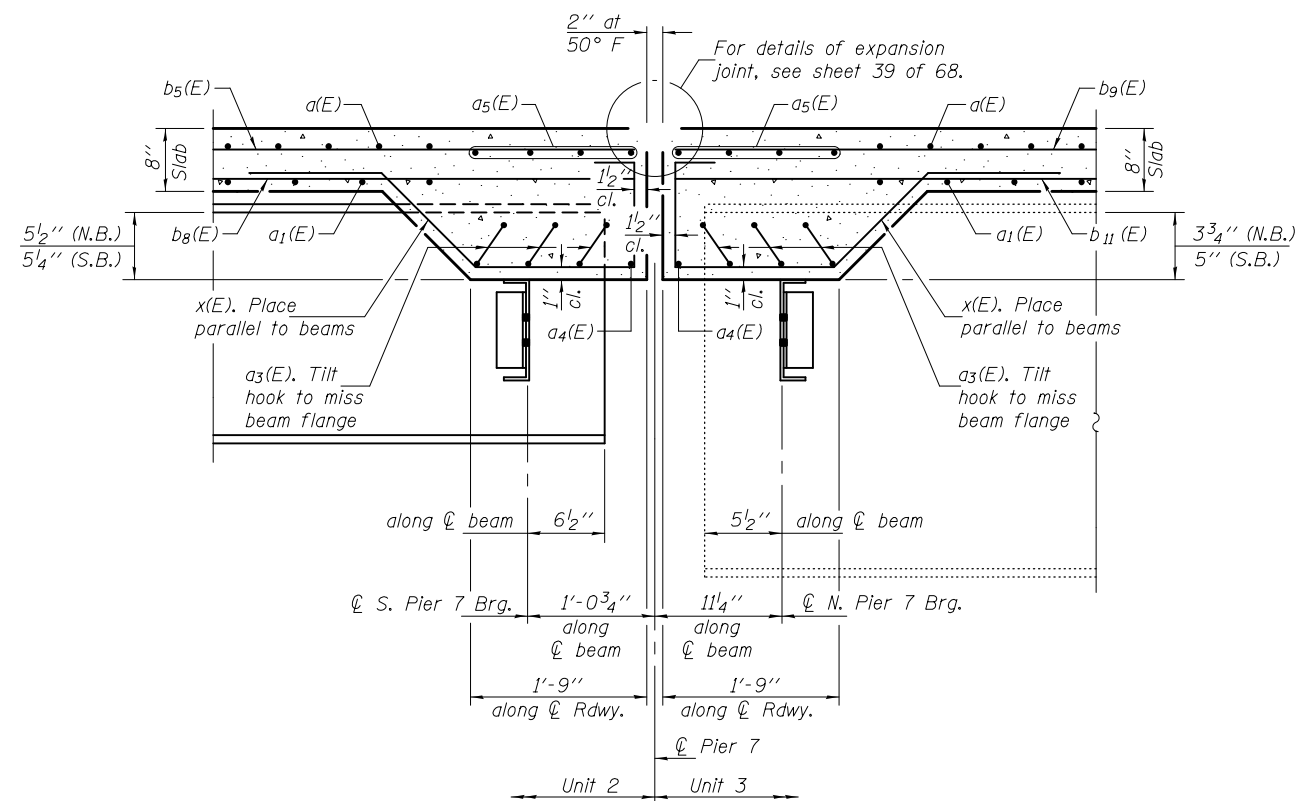
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS - SPANS 5 THRU 7 - UNIT 2
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

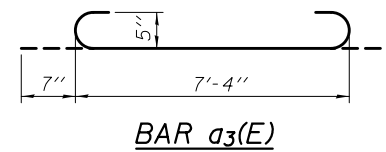
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	69
CONTRACT NO. 74351				



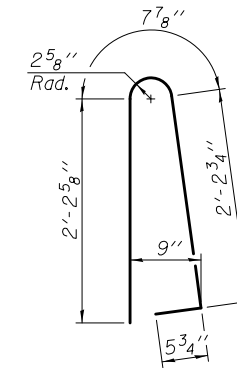
SECTION B-B



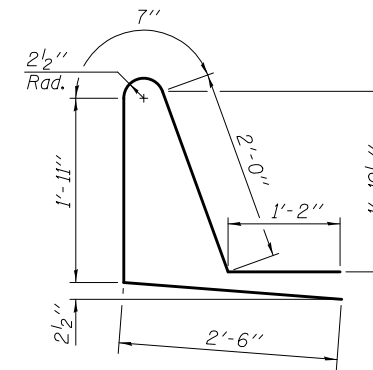
SECTION C-C



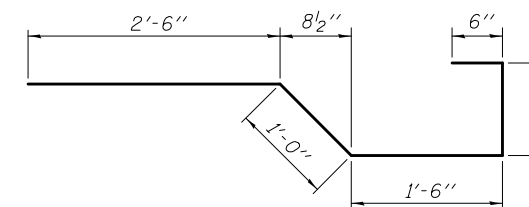
BAR a3(E)



BAR d(E)



BAR d1(E)



BAR x(E)

TWO SUPERSTRUCTURES
(N.B. & S.B.) - UNIT 2
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	830	#5	42'-6"	—
a1(E)	508	#5	40'-10"	—
a2(E)	1660	#6	6'-6"	—
a3(E)	60	#5	8'-6"	U
a4(E)	4	#5	38'-3"	—
a5(E)	16	#5	45'-3"	—
a6(E)	32	#5	2'-0"	—
b5(E)	644	#5	30'-0"	—
b6(E)	86	#6	35'-3"	—
b7(E)	86	#6	36'-9"	—
b8(E)	656	#5	26'-8"	—
d(E)	832	#5	5'-7"	U
d1(E)	832	#5	8'-2"	U
e(E)	168	#4	17'-10"	—
e1(E)	128	#4	8'-9"	—
e4(E)	16	#8	29'-8"	—
e5(E)	16	#8	8'-9"	—
e8(E)	16	#4	28'-2"	—
e10(E)	84	#4	14'-9"	—
e11(E)	4	#8	44'-11"	—
e12(E)	8	#4	23'-6"	—
x(E)	140	#5	6'-5"	U
Reinforcement Bars, Epoxy Coated			Pound	142840
Concrete Superstructure			Cu. Yds.	503.7

Bars indicated thus 1 x 2-#8 etc. indicates 1 line of bars with 2 lengths per line.

DESIGNED - N. R. Barnett
CHECKED - A. R. Sheblb
DRAWN - h.t. duong
CHECKED - NRB/JMO/GRA

EXAMINED
PASSED
ENGINEER OF BRIDGE DESIGN
ACTING ENGINEER OF BRIDGES AND STRUCTURES

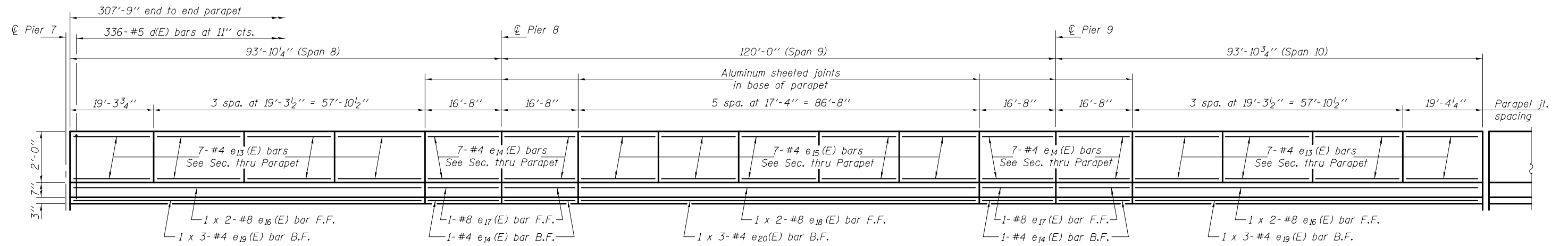
DATE - MAY 6, 2015
REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

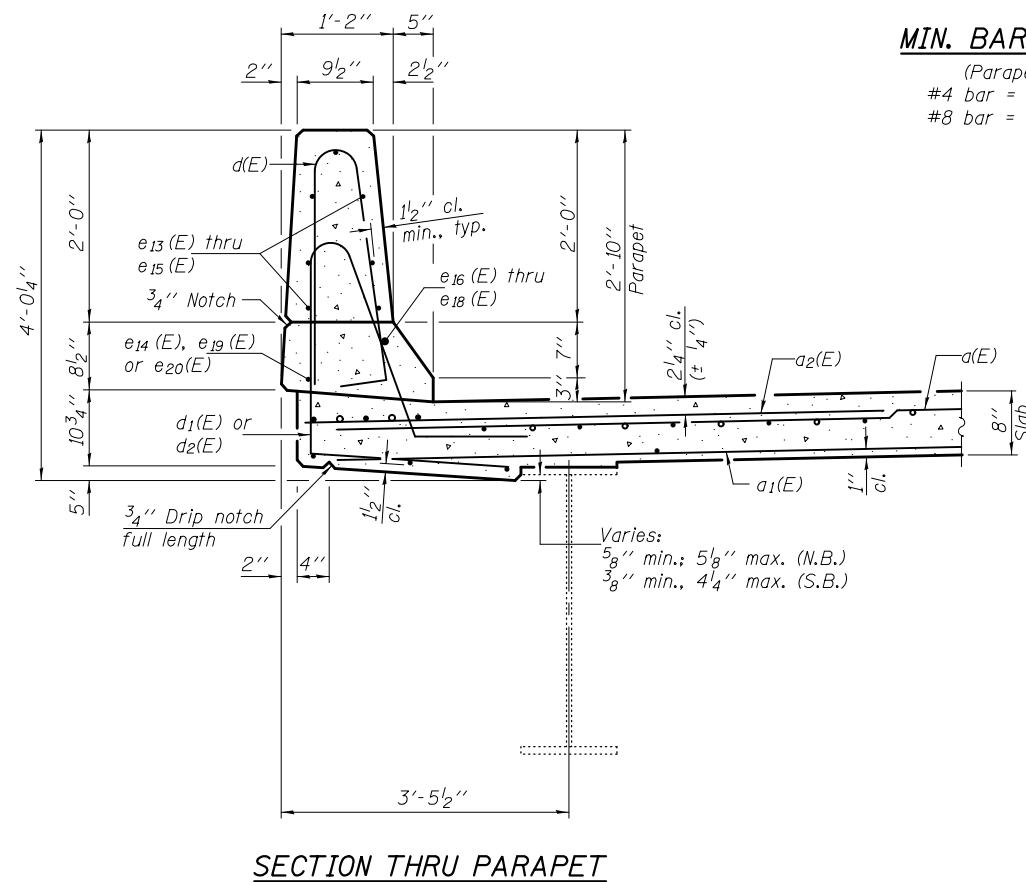
SUPERSTRUCTURE DETAILS - SPANS 5 THRU 7 - UNIT 2
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 33 OF 68 SHEETS

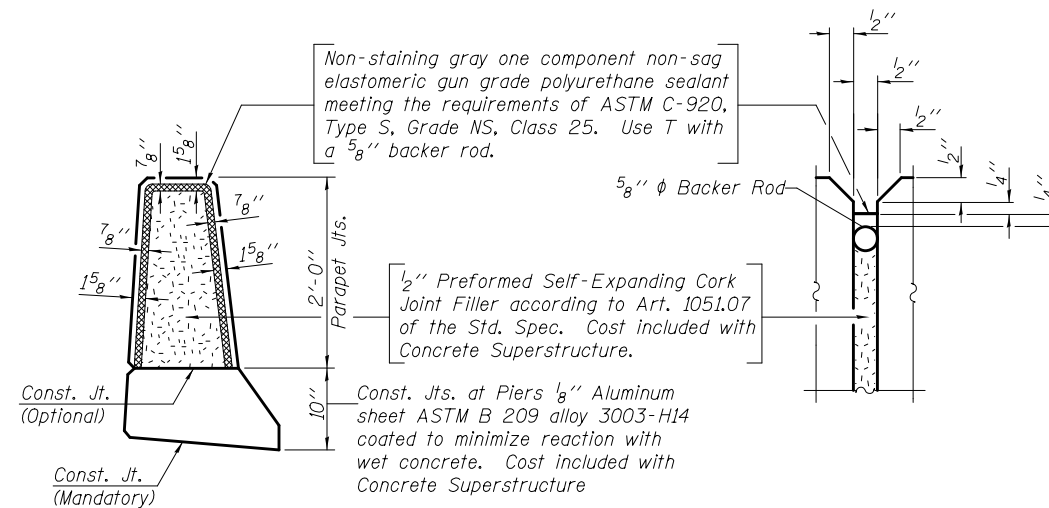
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	70
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				



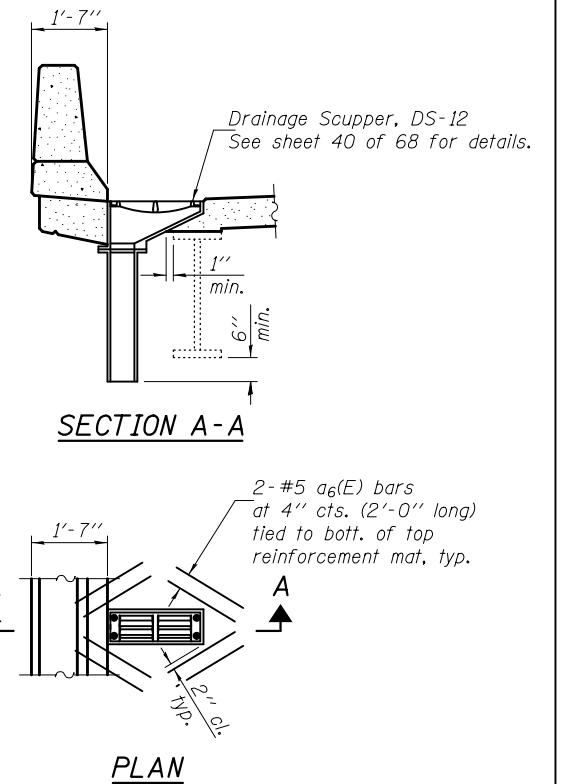
INSIDE ELEVATION OF WEST PARAPET - SPANS 8 THRU 10 - UNIT 3
(Looking West - East parapet similar)



MIN. BAR LAPS
(Parapet)
#4 bar = 2'-0"
#8 bar = 5'-2"

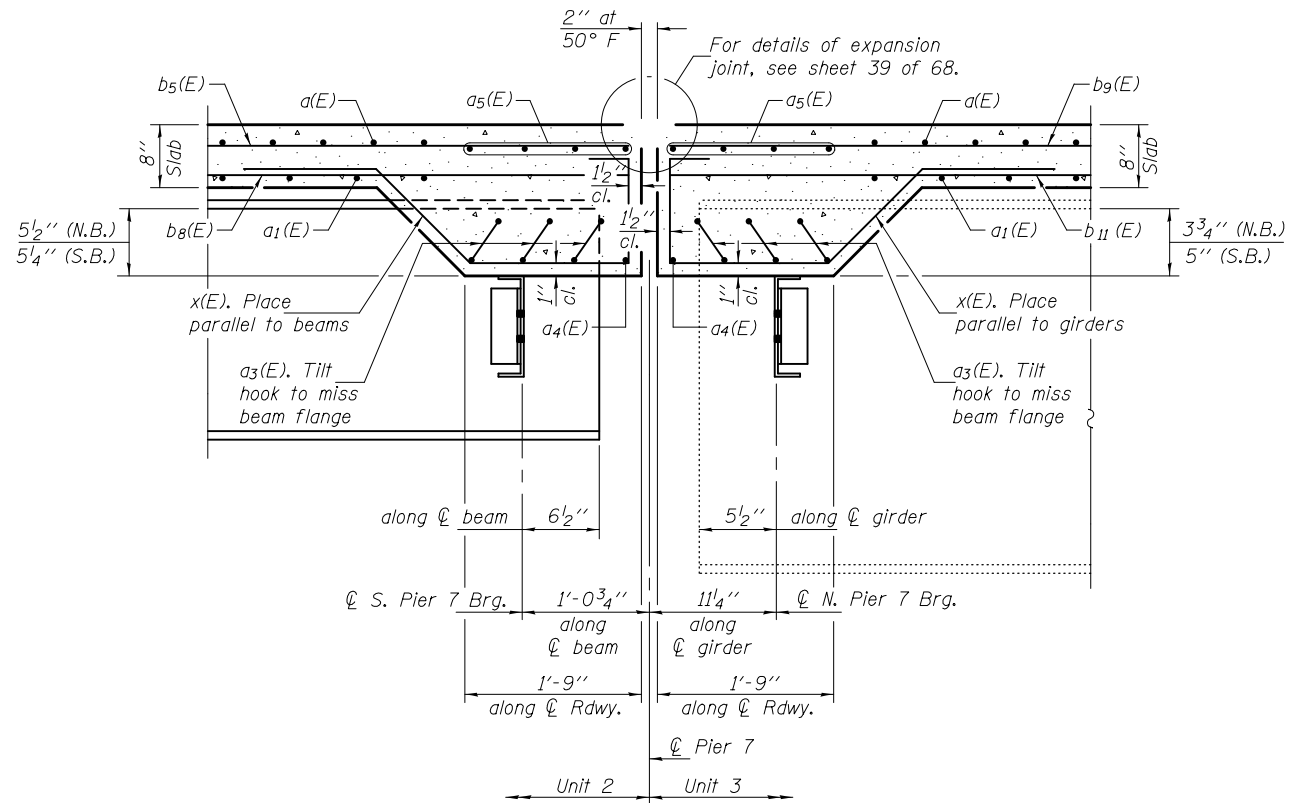


PARAPET JOINT DETAILS



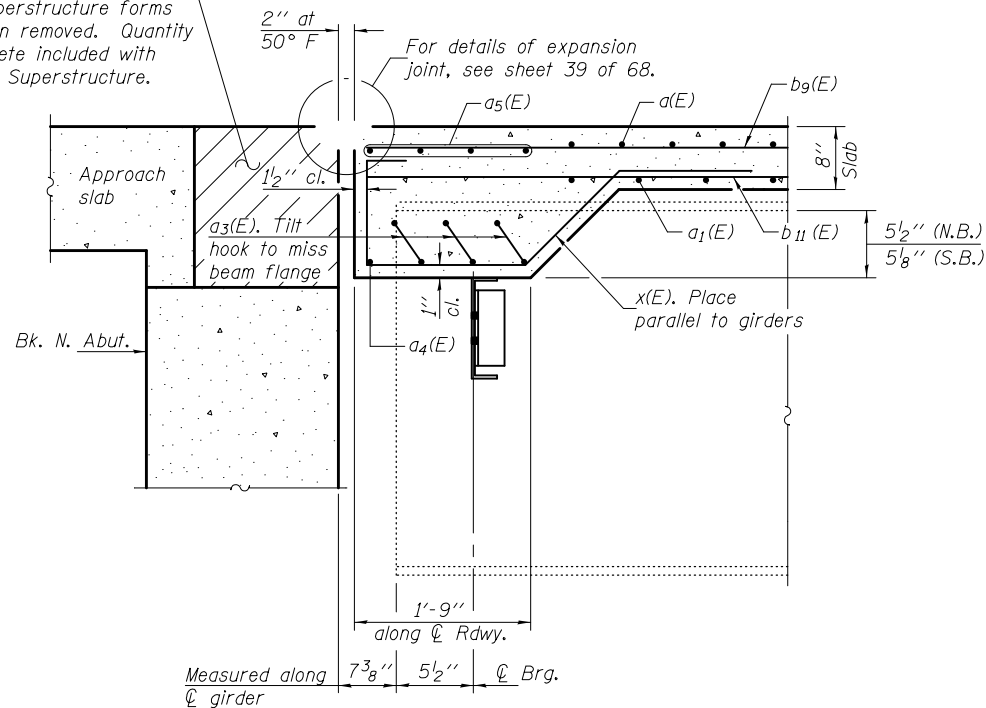
Note:
Cut longitudinal reinforcement to clear drainage scuppers.

DESIGNED - A. R. Shebli	EXAMINED - <i>Joanne F. [Signature]</i>	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE DETAILS - SPANS 8 THRU 10 - UNIT 3 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
CHECKED - N. R. Barnett	PASSED - <i>Carl [Signature]</i>	REVISED			322	(58-20B-1)BR	MACON	122	71	
DRAWN - h.t. duong	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED			CONTRACT NO. 74351					
CHECKED - NRB/JOV/GRA		REVISED			SHEET NO. 34 OF 68 SHEETS					

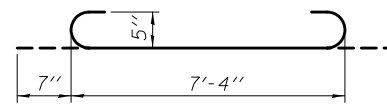


SECTION C-C

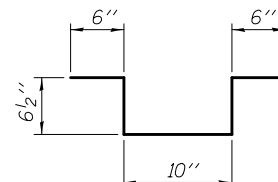
Hatched area to be poured after superstructure forms have been removed. Quantity of concrete included with Concrete Superstructure.



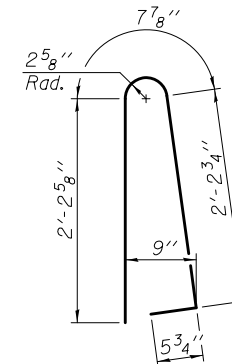
SECTION D-D



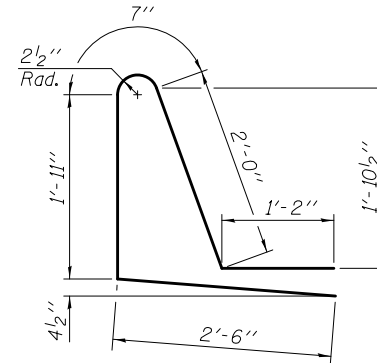
BAR a3(E)



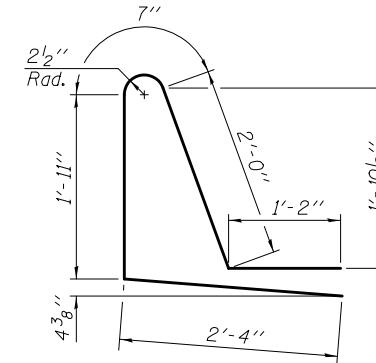
BAR a7(E)



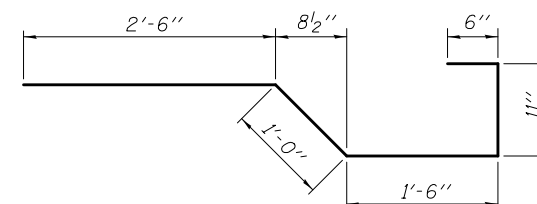
BAR d(E)



BAR d1(E)



BAR d2(E)



BAR x(E)

**TWO SUPERSTRUCTURES
(N.B. & S.B.) - UNIT 3
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a(E)	1344	#5	42'-6"	—
a1(E)	822	#5	40'-10"	—
a2(E)	2688	#6	6'-6"	—
a3(E)	60	#5	8'-6"	—
a4(E)	4	#5	38'-3"	—
a5(E)	16	#5	45'-3"	—
a6(E)	32	#5	2'-0"	—
a7(E)	2043	#4	2'-11"	—
b9(E)	1012	#5	31'-0"	—
b10(E)	516	#6	22'-6"	—
b11(E)	984	#5	28'-8"	—
b12(E)	64	#4	24'-5"	—
b13(E)	72	#4	25'-9"	—
b14(E)	2	#4	13'-0"	—
b15(E)	10	#4	21'-10"	—
b16(E)	4	#4	17'-0"	—
b17(E)	4	#4	27'-4"	—
b18(E)	4	#4	23'-4"	—
b19(E)	4	#4	30'-4"	—
b20(E)	4	#4	20'-10"	—
d(E)	1344	#5	5'-7"	—
d1(E)	912	#5	8'-2"	—
d2(E)	432	#5	8'-0"	—
e13(E)	224	#4	19'-0"	—
e14(E)	128	#4	16'-5"	—
e15(E)	140	#4	17'-1"	—
e16(E)	16	#8	41'-1"	—
e17(E)	16	#8	16'-5"	—
e18(E)	8	#8	45'-10"	—
e19(E)	24	#4	27'-0"	—
e20(E)	12	#4	30'-2"	—
x(E)	140	#5	6'-5"	—
Reinforcement Bars, Epoxy Coated		Pound	238760	
Concrete Superstructure		Cu. Yds.	868.4	

Bars indicated thus 1 x 2-#8 etc. indicates 1 line of bars with 2 lengths per line.

DESIGNED - A. R. Shebli
 CHECKED - N. R. Barnett
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS - SPANS 8 THRU 10 - UNIT 3
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

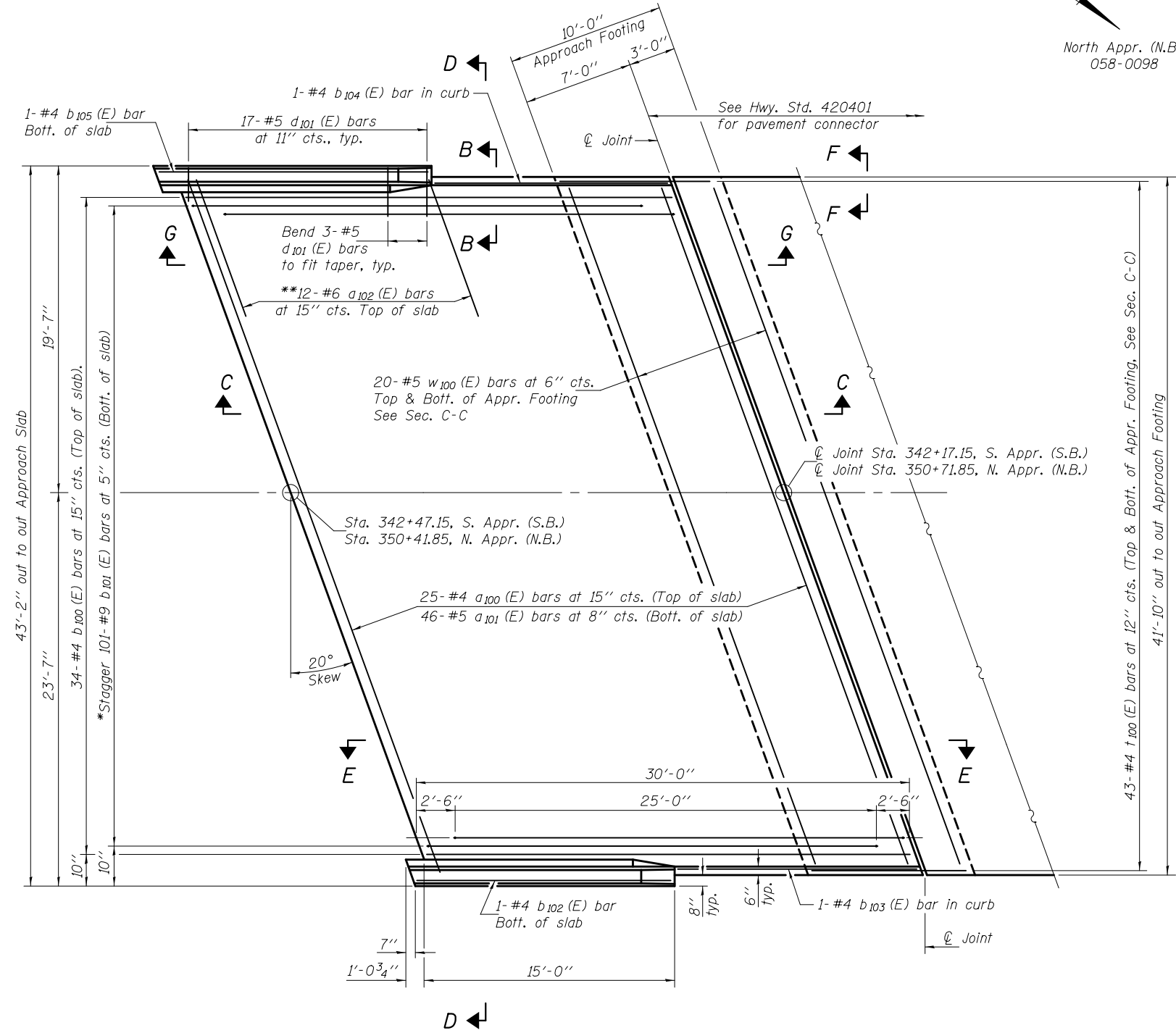
SHEET NO. 35 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	72
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

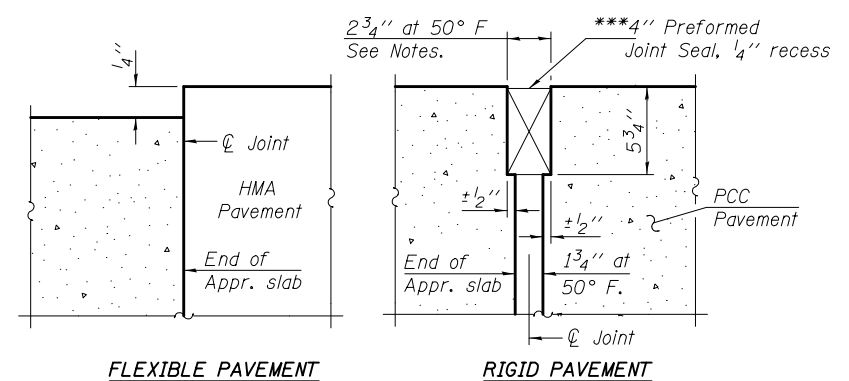
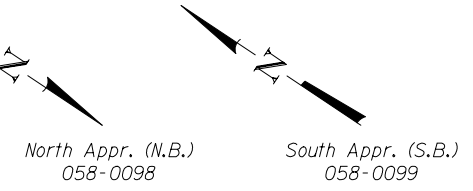
Notes:
 See sheet 38 of 68 for Sections C-C & D-D and View E-E.
 a_{100} (E) and a_{101} (E) bar spacings measured along \varnothing Rdwy.
 The joint opening shall be determined per Article 520.04 except that on jointless structures, the distance described as the bridge length between the nearest fixed bearings each way from the joint shall be taken as half the bridge length plus the approach slab length. The minimum dimension shall be $1\frac{1}{2}$ " for installation purposes.

*** Cost included with Concrete Superstructure.

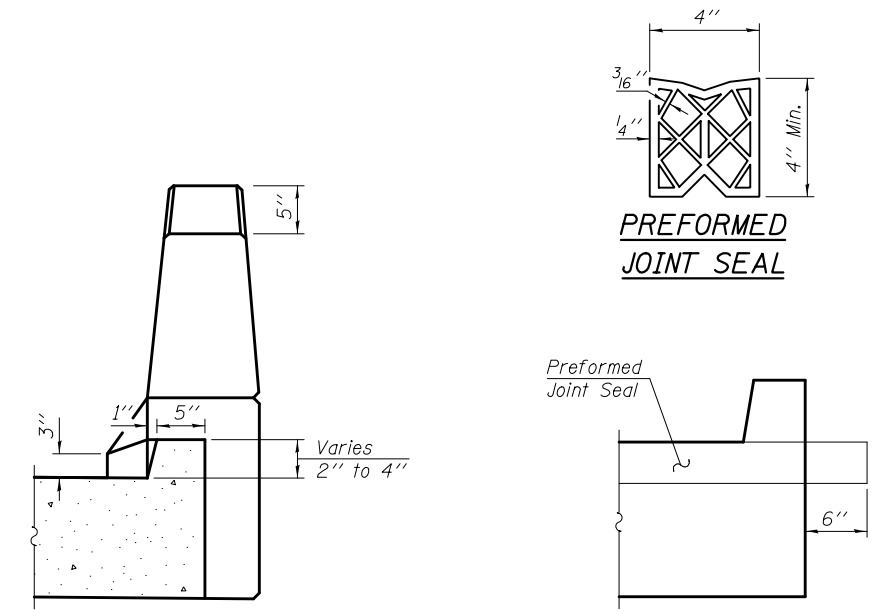


PLAN

*Tilt #9 b_{101} (E) bars as required to maintain clearance.
 **Space between a_{100} (E) bars, typ. each parapet.



DETAIL A



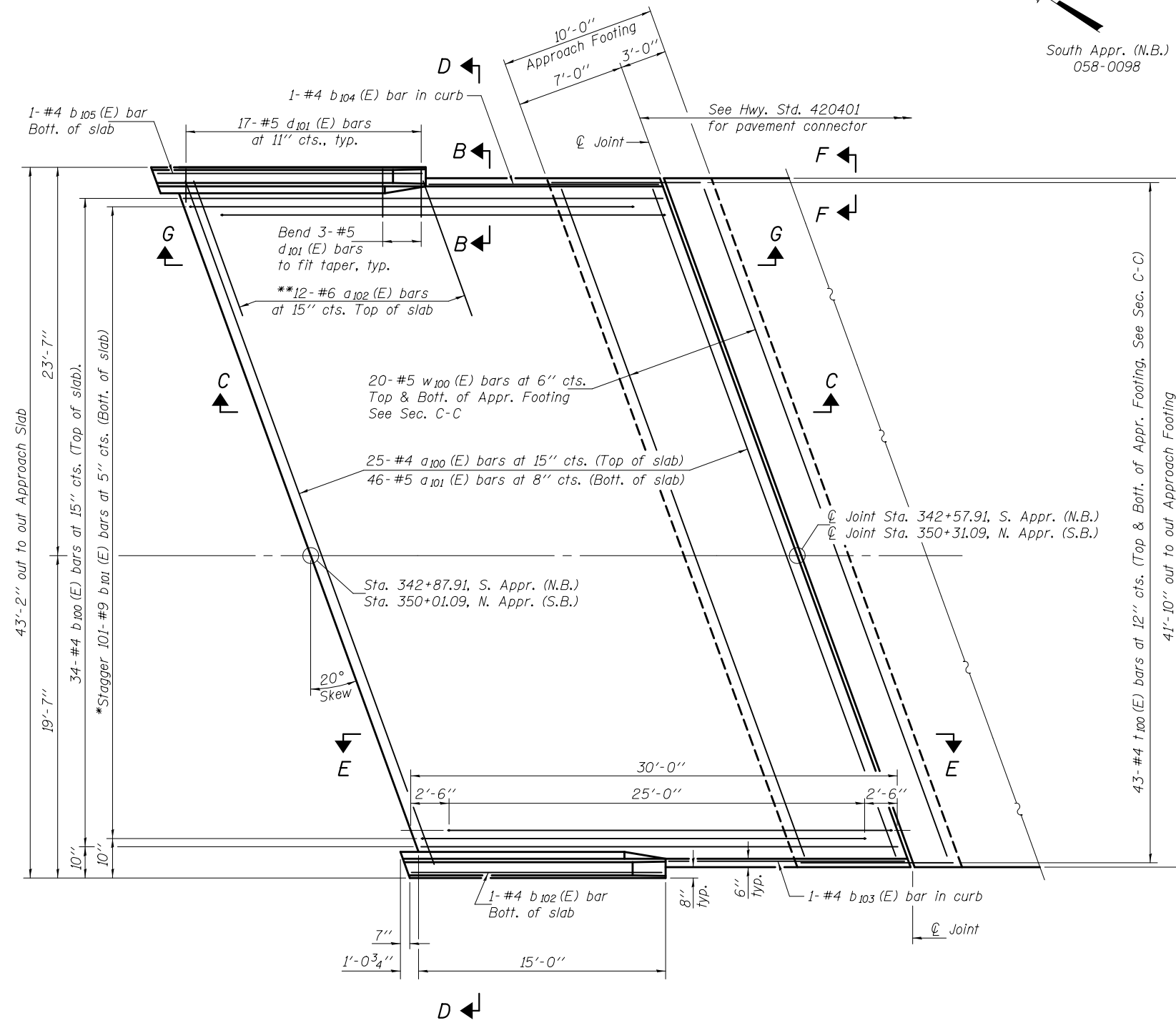
VIEW B-B

VIEW F-F

DESIGNED - N. R. Barnett	EXAMINED - <i>Joey F. Joffe</i> ENGINEER OF BRIDGE DESIGN	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CHECKED - J. D. Ortiz-Varela	PASSED - <i>Carl Pung</i> ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED			322	(58-20B-1)BR	MACON	122	73
DRAWN - h.t. duong		REVISED			CONTRACT NO. 74351				
CHECKED - NRB/JOV/GRA		REVISED			SHEET NO. 36 OF 68 SHEETS				
				ILLINOIS FED. AID PROJECT					

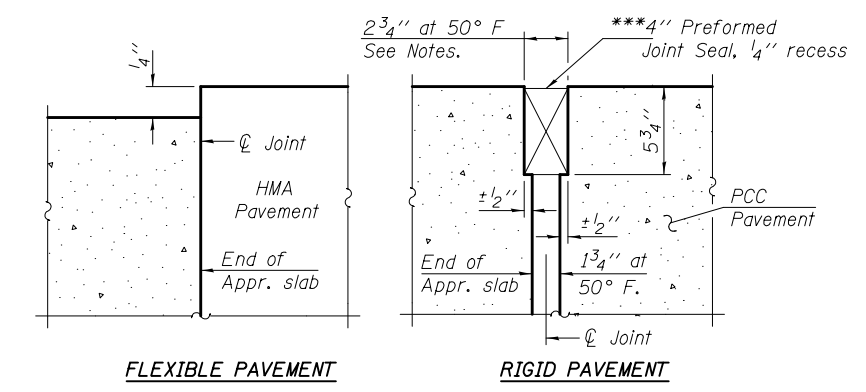
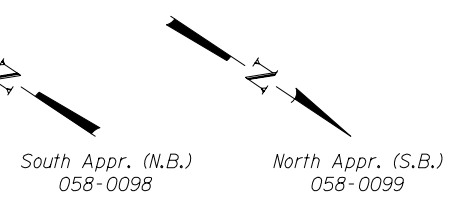
Notes:
 See sheet 38 of 68 for Sections C-C & D-D and View E-E.
 a_{100} (E) and a_{101} (E) bar spacings measured along \varnothing Rdwy.
 The joint opening shall be determined per Article 520.04 except that on jointless structures, the distance described as the bridge length between the nearest fixed bearings each way from the joint shall be taken as half the bridge length plus the approach slab length. The minimum dimension shall be $1\frac{1}{2}$ " for installation purposes.

*** Cost included with Concrete Superstructure.

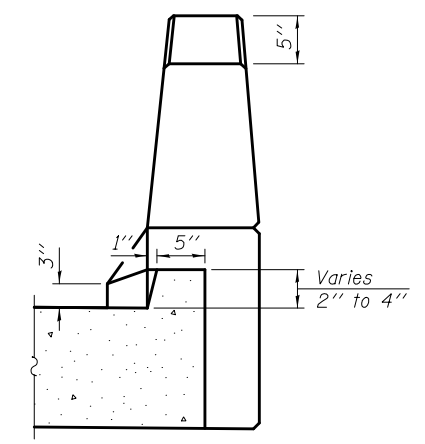
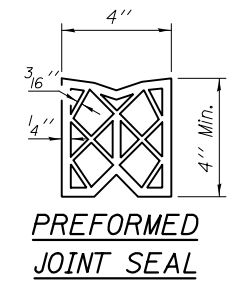


PLAN

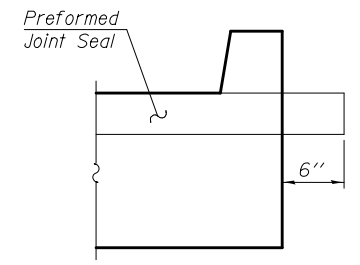
*Tilt #9 b_{101} (E) bars as required to maintain clearance.
 **Space between a_{100} (E) bars, typ. each parapet.



DETAIL A

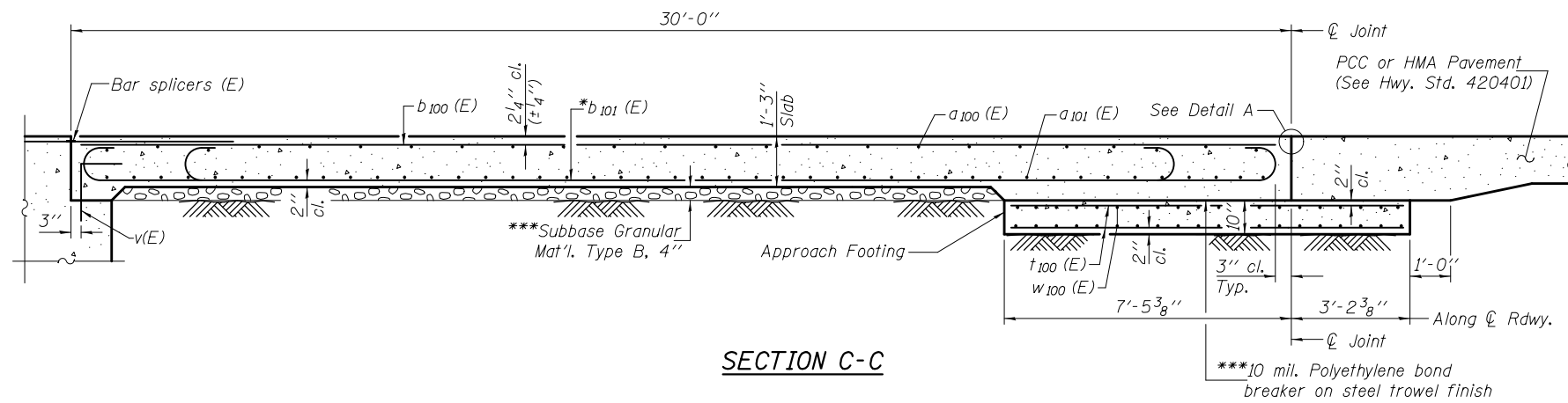


VIEW B-B

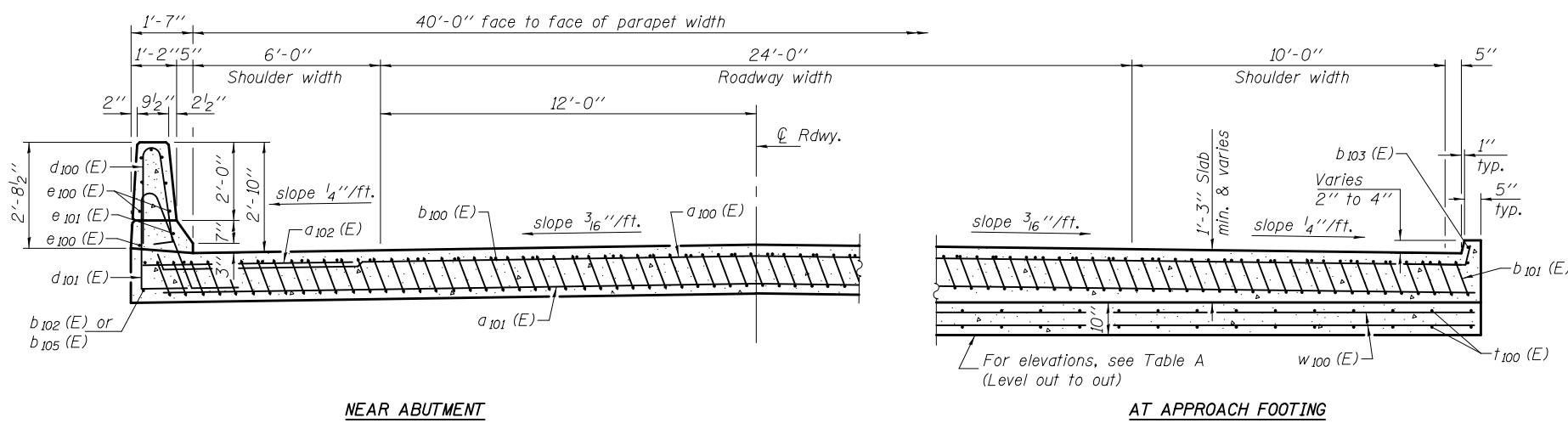
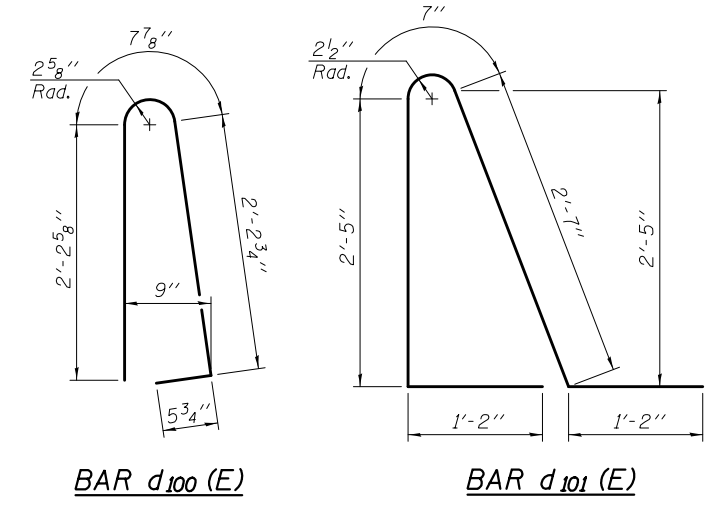


VIEW F-F

DESIGNED - N. R. Barnett	EXAMINED - <i>Joanne F. Joffe</i> ENGINEER OF BRIDGE DESIGN	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
CHECKED - J. D. Ortiz-Varela	PASSED - <i>Carl Pung</i> ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED			322	(58-20B-1)BR	MACON	122	74	
DRAWN - h.t. duong		REVISED			CONTRACT NO. 74351					
CHECKED - NRB/JOV/GRA					SHEET NO. 37 OF 68 SHEETS					
				ILLINOIS FED. AID PROJECT						



Notes:
 See sheets 36 & 37 of 68 for Detail A and View B-B.
 Approach slab and parapet concrete shall be paid for as Concrete Superstructure.
 Approach footing concrete shall be paid for as Concrete Structures.
 Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
 For v(E) bar details, see sheets 61 & 62 of 68.
 The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
 Cost of excavation for approach footing included with Concrete Structures.
 For additional parapet details, see sheets 36 & 37 of 68.



*Tilt #9 b100(E) bars as required to maintain clearance.
 **Cut e100(E) and e101(E) bars to fit.
 ***Cost included with Concrete Superstructure.

**FOUR APPROACHES
 BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a100 (E)	100	#4	44'-9"	—
a101 (E)	184	#5	44'-2"	—
a102 (E)	96	#6	6'-6"	—
b100 (E)	136	#4	29'-8"	—
b101 (E)	404	#9	29'-9"	—
b102 (E)	4	#4	15'-2"	—
b103 (E)	4	#4	14'-10"	—
b104 (E)	4	#4	14'-5"	—
b105 (E)	4	#4	16'-3"	—
d100 (E)	152	#5	5'-7"	—
d101 (E)	136	#5	7'-11"	—
e100 (E)	64	#4	15'-9"	—
e101 (E)	8	#8	15'-9"	—
t100 (E)	344	#4	10'-3"	—
w100 (E)	160	#5	44'-2"	—
Concrete Superstructure			Cu. Yd.	262.9
Concrete Structures			Cu. Yd.	55.0
Reinforcement Bars, Epoxy Coated			Pound	68870

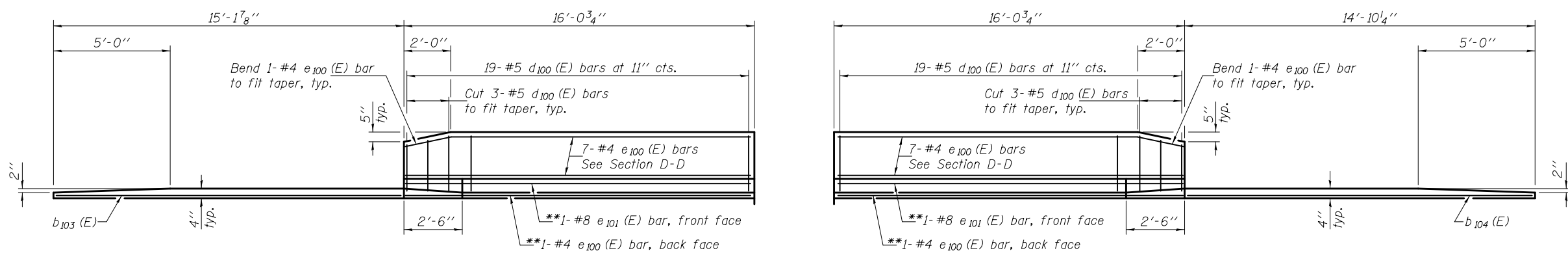
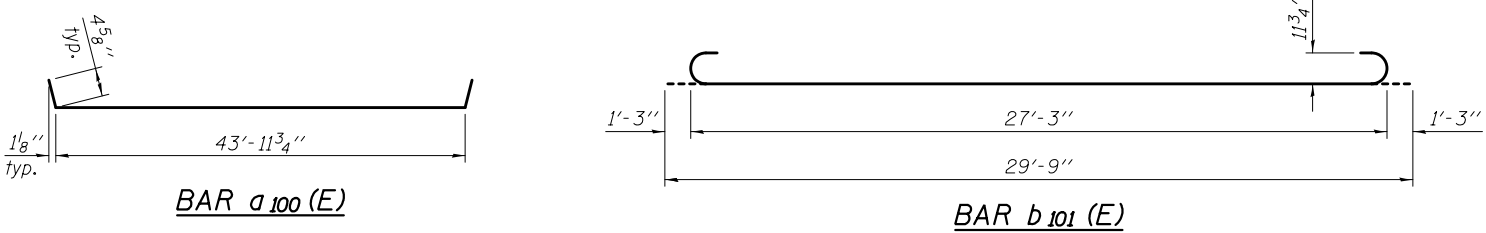
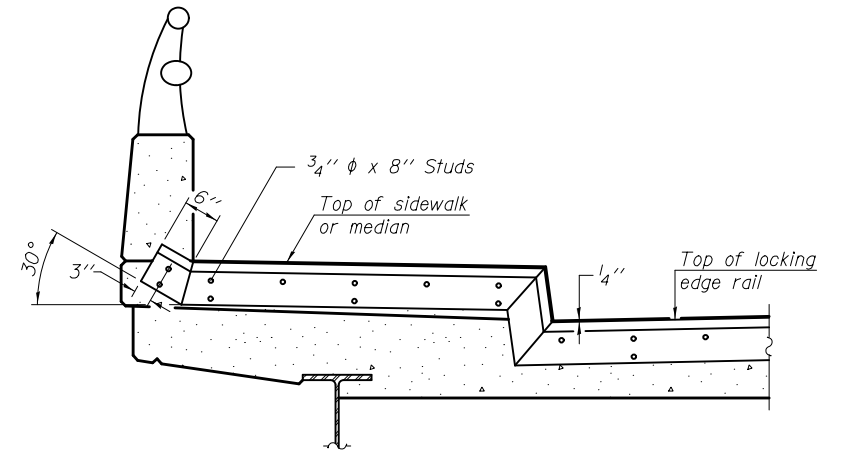
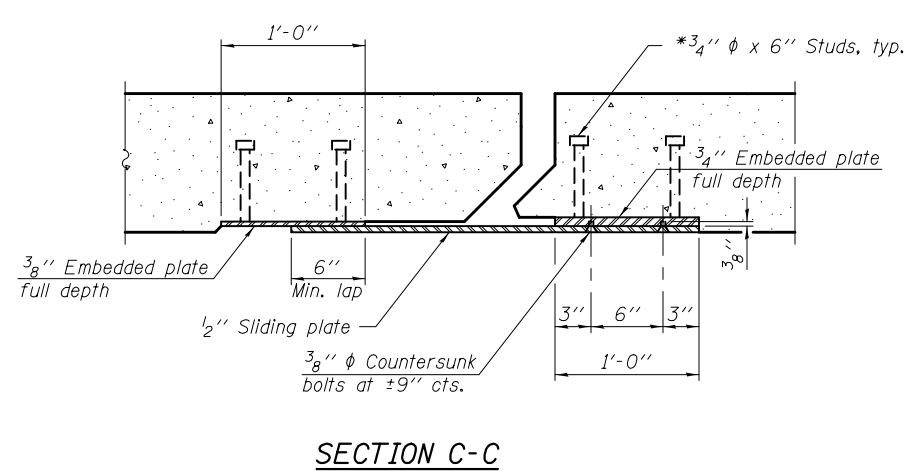
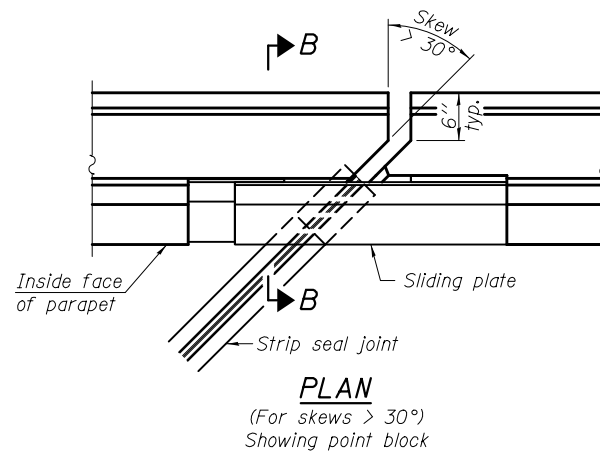
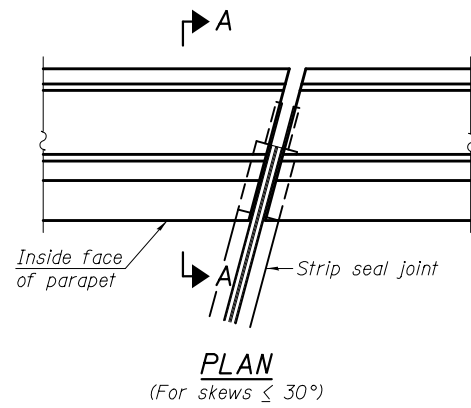


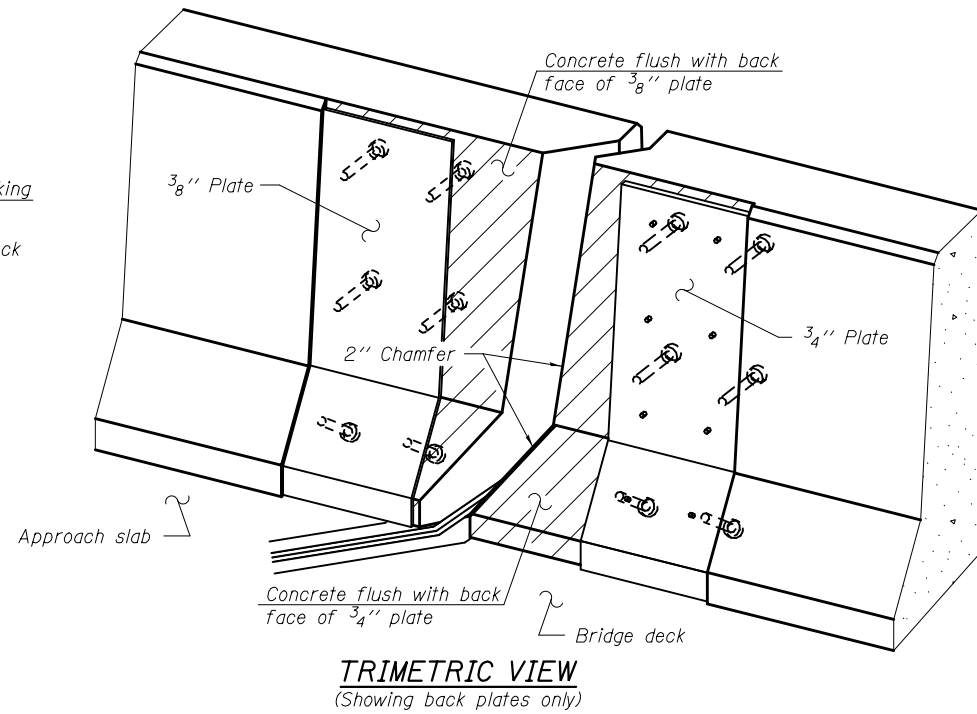
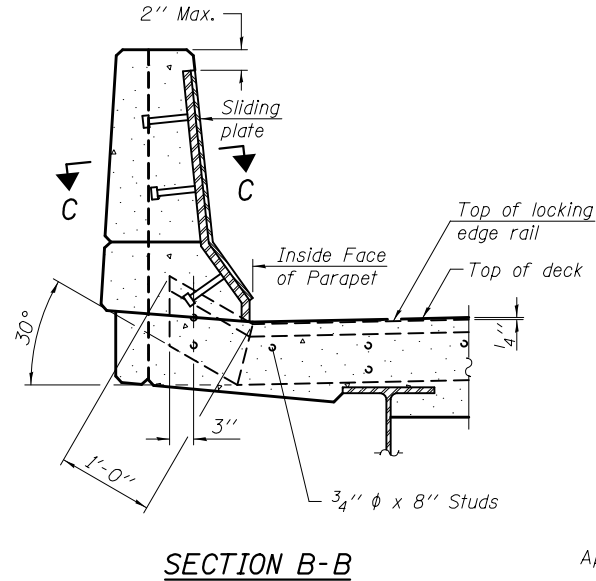
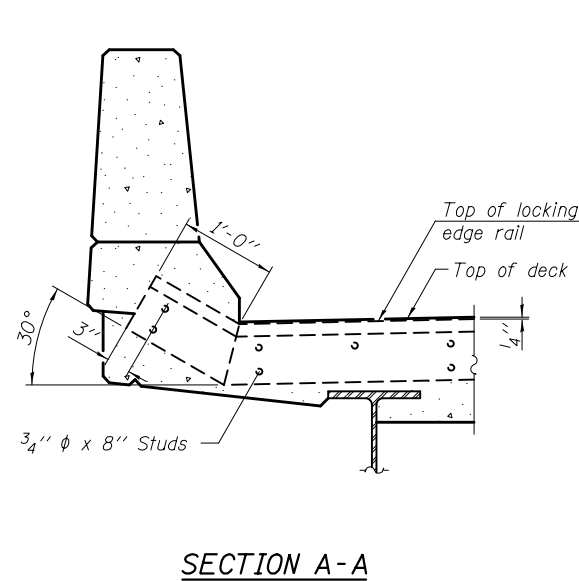
TABLE A

Location	Bottom of Footing Elev.
South Approach (S.B.)	603.48
North Approach (S.B.)	607.33
South Approach (N.B.)	603.56
North Approach (N.B.)	607.66

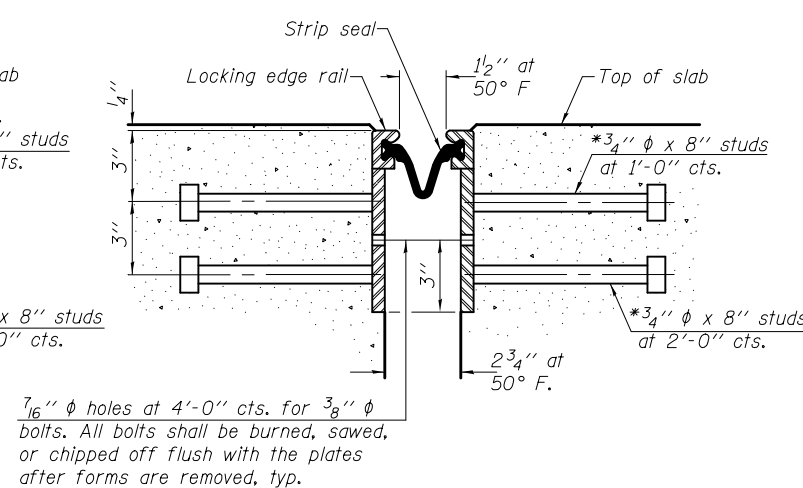
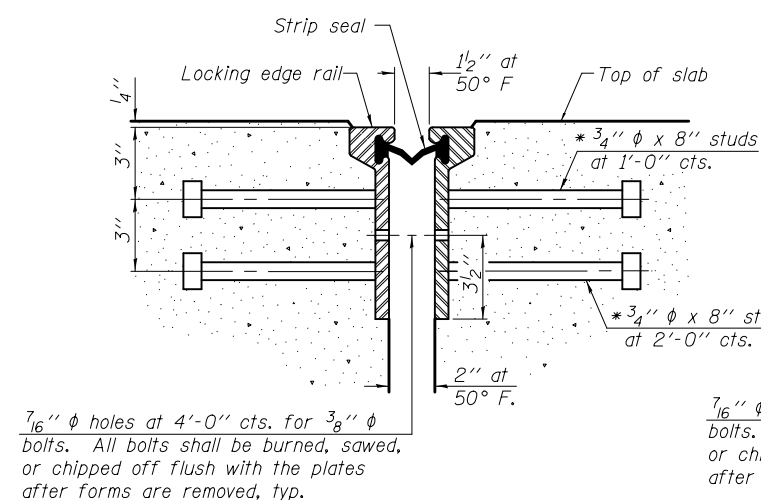




TYPICAL END TREATMENT AT SIDEWALK OR MEDIAN
 Shorter plates with a single row of studs at 12 inch cts. may be necessary on medians which are shallower than 9 inch. See manufacturer's recommendation.



Notes:
 The strip seal shall be made continuous and shall have a minimum thickness of 1/4 inch. The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.
 The Locking Edge Rails depicted are conceptual only, except for the minimum dimensions shown. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities.
 The manufacturer's recommended installation methods shall be followed.
 The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.
 All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications. Maximum space between rail segments shall be 3/16 inch, sealed with a suitable sealant. Joints in rails within 10 ft. of curbs shall be welded.
 Parapet plates and anchorage studs for skews > 30 degrees included in the cost of Preformed Joint Strip Seal.



SECTION THRU ROLLED RAIL JOINT

SECTION THRU WELDED RAIL JOINT

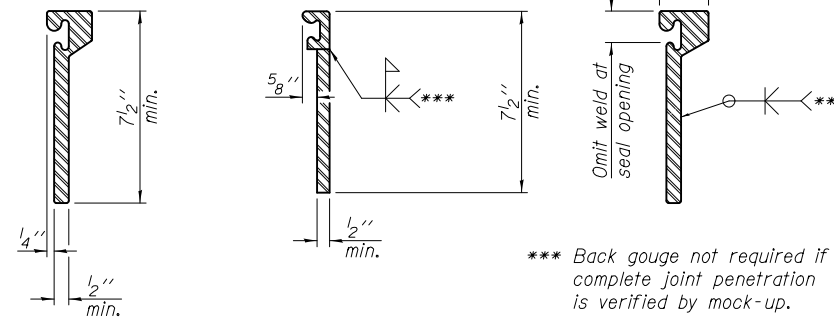
ROLLED EXTRUDED RAIL WELDED RAIL

LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

7/16 inch diameter holes at 4'-0 inch cts. for 3/8 inch diameter bolts. All bolts shall be burned, sawed, or chipped off flush with the plates after forms are removed, typ.

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



LOCKING EDGE RAILS

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	356.5

EJ-SSJ

1-27-12

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED

Signature of **Jayne F. Joffe**
 ENGINEER OF BRIDGE DESIGN
 Signature of **Carl Pinger**
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

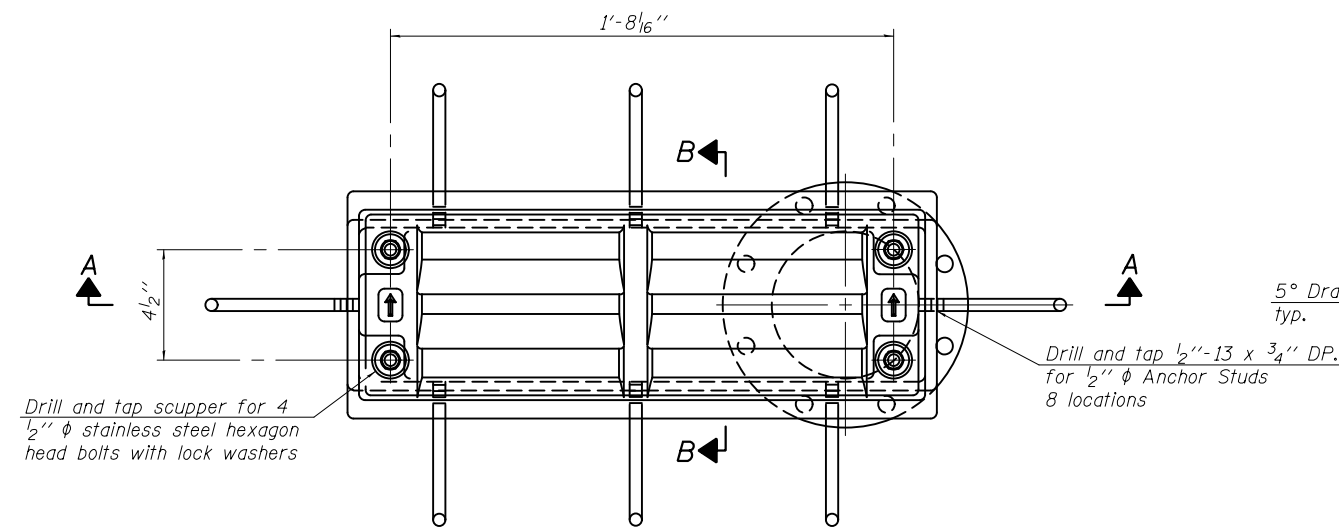
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

PREFORMED JOINT STRIP SEAL
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

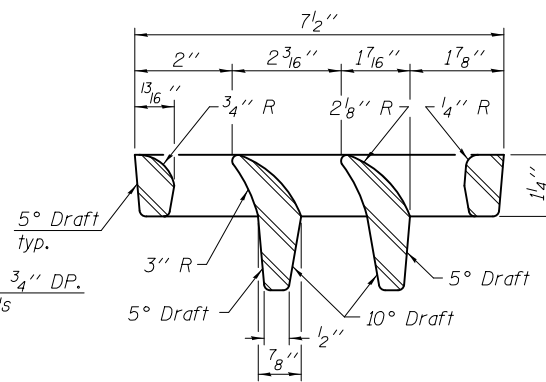
SHEET NO. 39 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	76
CONTRACT NO. 74351				

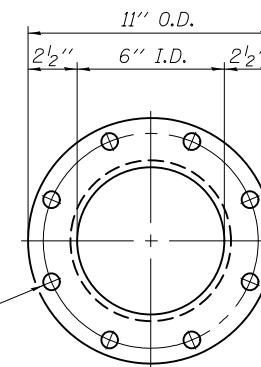
ILLINOIS FED. AID PROJECT



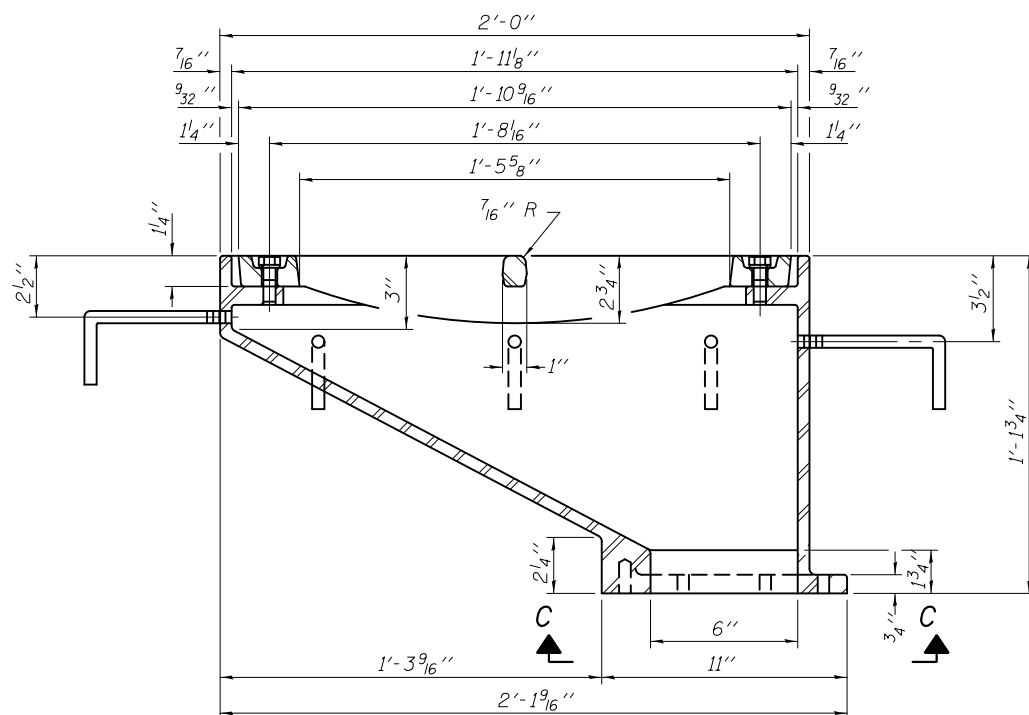
PLAN



VANE GRATE DETAIL

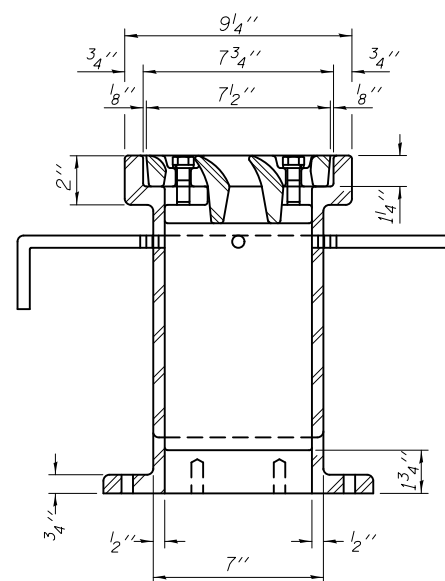


VIEW C-C

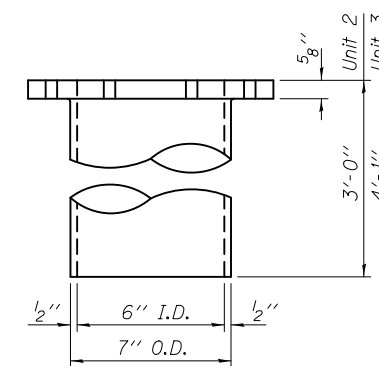


SECTION A-A

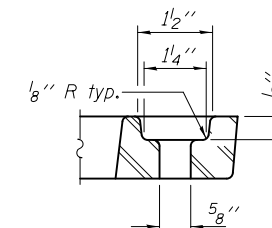
See sheet 32 of for scupper location relative to parapet.



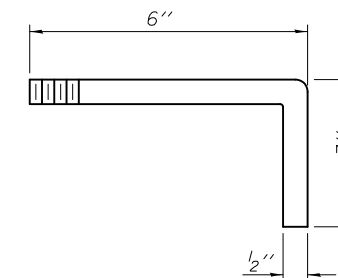
SECTION B-B



DOWNSPOUT



BOLT HOLE DETAIL



ANCHOR STUD DETAIL

Drill and tap 8 holes for 1/2"-13 bolts on a 9 1/2" φ bolt circle. (2 blind holes are 1 1/4" deep, 6 thru holes)

Notes:

All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.

Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.

Downspouts located on the exterior side of a painted steel fascia beam shall be painted with the finish coat specified for the exterior side of the fascia beam.

As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.

Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO M111.

The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.

Cost of the Grate, Frame, Downspout, Anchor Studs, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-12.

Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel equivalent.

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scupper, DS-12	Each	8

DS-12

7-1-10

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED

Joanne F. Joffe
 ENGINEER OF BRIDGE DESIGN
 Carl Pung
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015

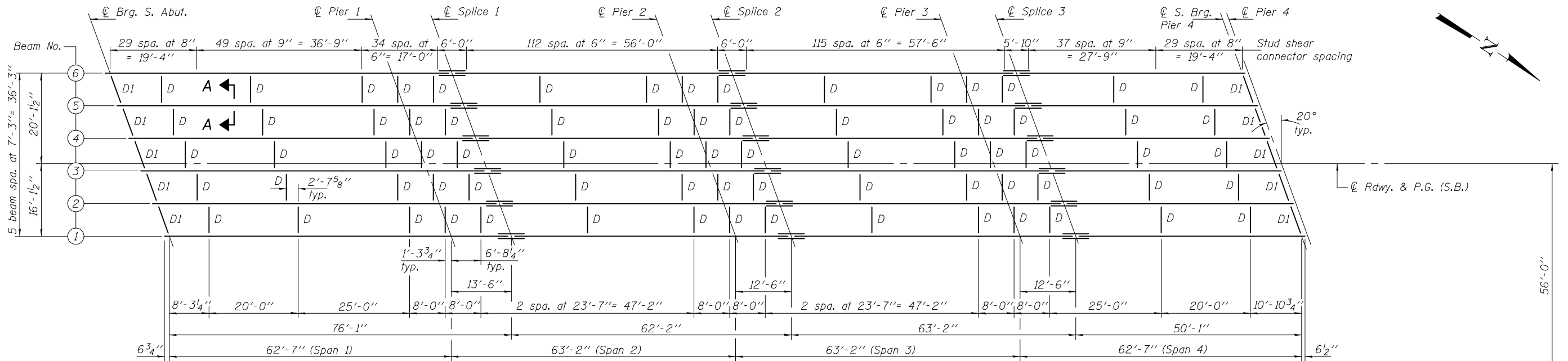
REVISED
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

DRAINAGE SCUPPER, DS-12
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 40 OF 68 SHEETS

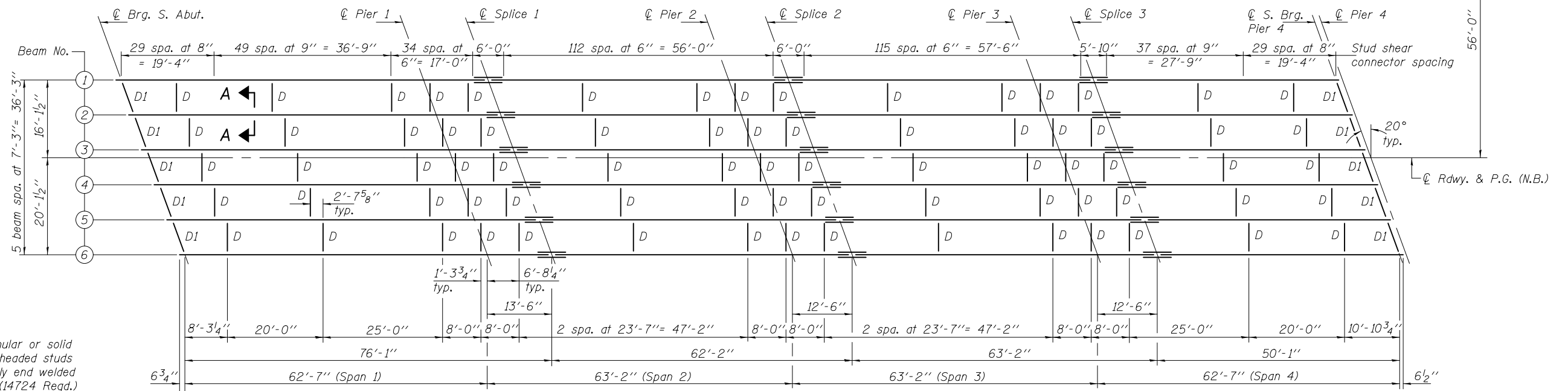
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	77
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				



PLAN (S.B.)

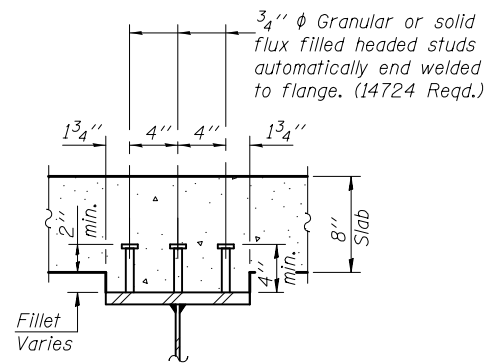
All beams are W33x141, AASHTO M 270 Grade 50 (NTR).

℄ F.A.P. 322
(US Route 51)



PLAN (N.B.)

All beams are W33x141, AASHTO M 270 Grade 50 (NTR).



SECTION A-A

DESIGNED - N. R. Barnett
CHECKED - J. D. Ortiz-Varela
DRAWN - h.t. duong
CHECKED - NRB/JOV/GRA

EXAMINED
PASSED
ENGINEER OF BRIDGE DESIGN
ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
REVISED
REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**STRUCTURAL STEEL - SPANS 1 THRU 4 - UNIT 1
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

SHEET NO. 41 OF 68 SHEETS

F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 78
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				

***TOP OF BEAM ELEVATIONS - SPANS 1 THRU 4 (S.B.)**

Location	℄ Brg. S. Abut.	℄ Brg. Pier 1	℄ Splice 1	℄ Brg. Pier 2	℄ Splice 2	℄ Brg. Pier 3	℄ Splice 3	℄ S. Brg. Pier 4
Beam 6	604.95	605.01	605.02	605.16	605.19	605.37	605.41	605.69
Beam 5	605.10	605.17	605.18	605.32	605.35	605.53	605.57	605.86
Beam 4	605.23	605.29	605.30	605.44	605.47	605.65	605.70	605.98
Beam 3	605.29	605.36	605.37	605.51	605.55	605.73	605.77	606.06
Beam 2	605.17	605.24	605.26	605.40	605.44	605.62	605.67	605.95
Beam 1	605.05	605.12	605.14	605.28	605.31	605.50	605.50	605.84

*For fabrication use only.

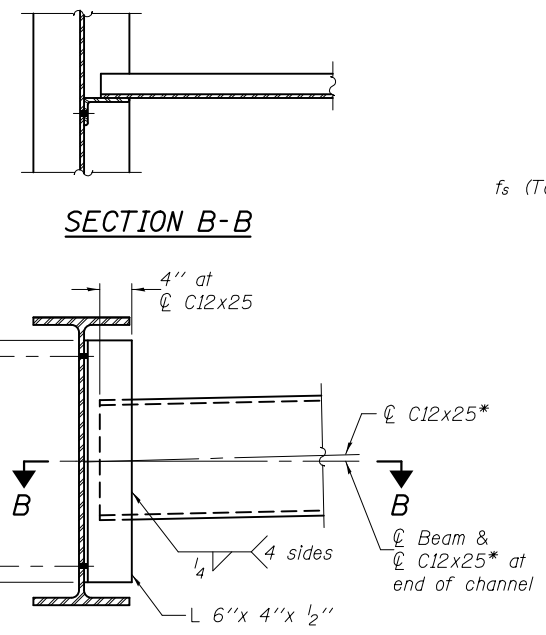
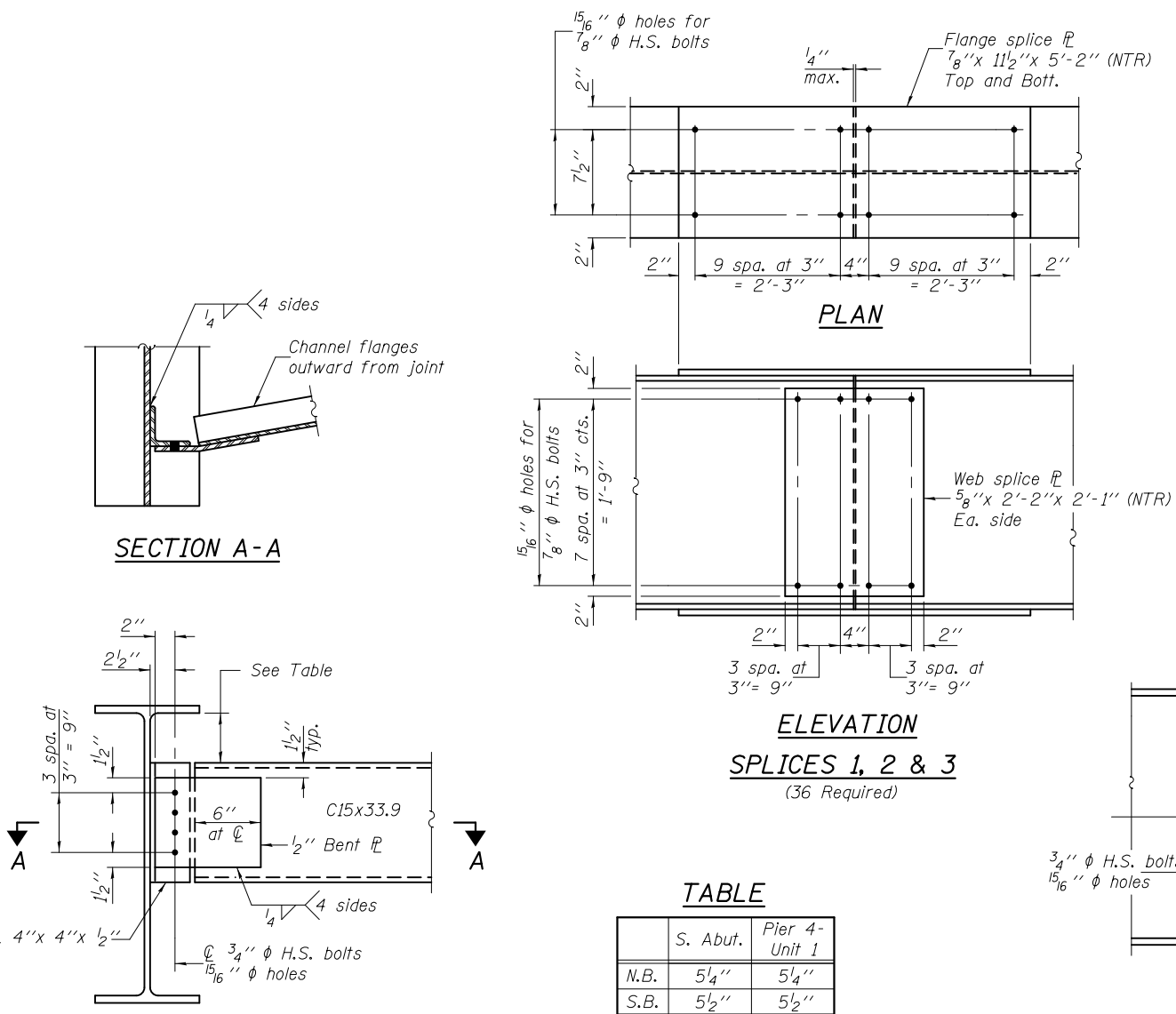
***TOP OF BEAM ELEVATIONS - SPANS 1 THRU 4 (N.B.)**

Location	℄ Brg. S. Abut.	℄ Brg. Pier 1	℄ Splice 1	℄ Brg. Pier 2	℄ Splice 2	℄ Brg. Pier 3	℄ Splice 3	℄ S. Brg. Pier 4
Beam 1	605.08	605.19	605.21	605.37	605.41	605.62	605.67	605.96
Beam 2	605.21	605.33	605.36	605.52	605.56	605.77	605.82	606.11
Beam 3	605.34	605.45	605.48	605.64	605.68	605.89	605.94	606.23
Beam 4	605.28	605.39	605.42	605.59	605.63	605.84	605.89	606.19
Beam 5	605.17	605.28	605.31	605.48	605.52	605.74	605.79	606.08
Beam 6	605.03	605.14	605.17	605.34	605.38	605.60	605.65	605.94

*For fabrication use only.

	0.4 Sp. 1 or 0.6 Sp. 4	Pier 1 or Pier 3	0.5 Sp. 2 or Sp. 3	Pier 2
I_s	(in ⁴) 7450	7450	7450	7450
$I_c(n)$	(in ⁴) 19743	19743	19743	19743
$I_c(3n)$	(in ⁴) 14495	14495	14495	14495
$I_c(cr)$	(in ⁴)			
S_s	(in ³) 448	448	448	448
$S_c(n)$	(in ³) 654	654	654	654
$S_c(3n)$	(in ³) 591	591	591	591
$S_c(cr)$	(in ³)			
DC1	(k/ft) .867	.867	.867	.867
M_{DC1}	(k) 261.2	365.8	124.8	249.5
DC2	(k/ft) 0.150	0.150	0.150	0.150
M_{DC2}	(k) 45.2	63.3	21.6	43.2
DW	(k/ft) 0.363	0.363	0.363	0.363
M_{DW}	(k) 109.3	153.2	52.2	104.5
$M_{\xi + IM}$	(k) 786.0	696.6	639.4	631.2
M_u (Strength I)	(k) 1922.5	1985.2	1380.3	1627.2
$\phi_r M_n$	(k) 3308.8	2366.0	3308.8	2366.0
f_s DC1	(ksi) 7.0	9.8	3.3	6.7
f_s DC2	(ksi) 0.9	1.3	0.4	0.9
f_s DW	(ksi) 2.2	3.1	1.1	2.1
f_s ($\xi + IM$)	(ksi) 14.4	12.8	11.7	11.6
f_s (Service II)	(ksi) 28.8	30.8	20.0	24.8
$0.95R_h F_y f$	(ksi) 47.5	47.5	47.5	47.5
f_s (Total)(Strength I)	(ksi)			
$\phi_r F_n$	(ksi)			
V_r	(k) 27.2	27.6	17.1	28.4

	Abut. or Pier 4	Pier 1 or Pier 3	Pier 2
R_{DC1}	(k) 22.1	64.6	53.0
R_{DC2}	(k) 3.7	10.8	8.8
R_{DW}	(k) 8.9	26.0	21.4
$R_{\xi + IM}$	(k) 79.2	108.1	105.0
R_{Total}	(k) 113.9	209.5	188.2



I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in⁴ and in³).

$I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in⁴ and in³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1} : Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2} : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW} : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

$M_{\xi + IM}$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

$1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{\xi + IM}$

$\phi_r M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft.).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

M_{DC1} / S_{nc}

f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).

$M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.

f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

$M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

f_s ($\xi + IM$): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).

$M_{\xi + IM} / S_c(n)$ or $M_{\xi + IM} / S_c(cr)$ as applicable.

f_s (Service II): Sum of stresses as computed below (ksi).

$f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (\xi + IM)$

$0.95R_h F_y f$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).

$1.25 (f_s DC1 + f_s DC2) + 1.5 f_s DW + 1.75 f_s (\xi + IM)$

$\phi_r F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

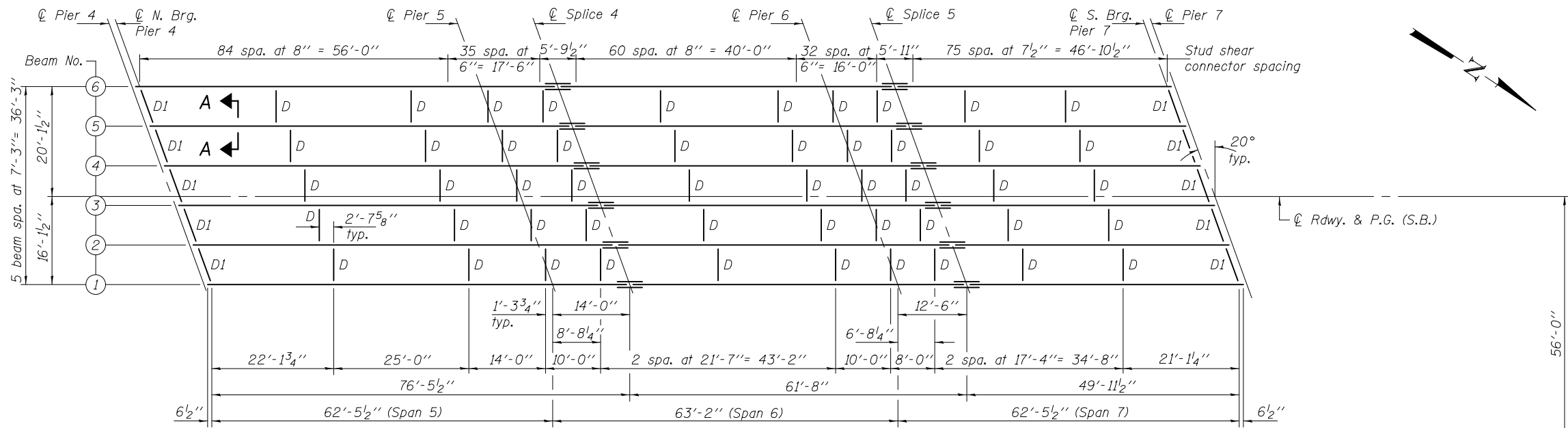
V_r : Maximum factored shear range in span computed according to Article 6.10.10.

Notes: Load carrying components designated "NTR" shall conform to the Impact Testing Requirements, Zone 2.

All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor rods.

Two hardened washers required for each set of oversized holes. All splice plates shall be AASHTO M 270 Grade 50.

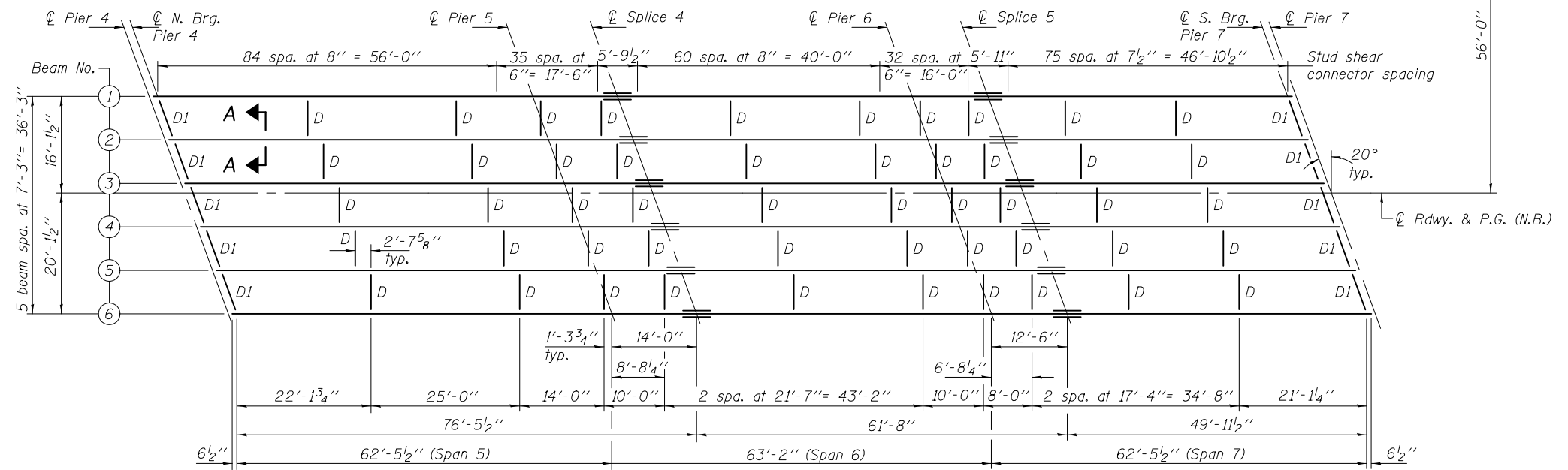
*Alternate channel C12x30 is permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.



PLAN (S.B.)

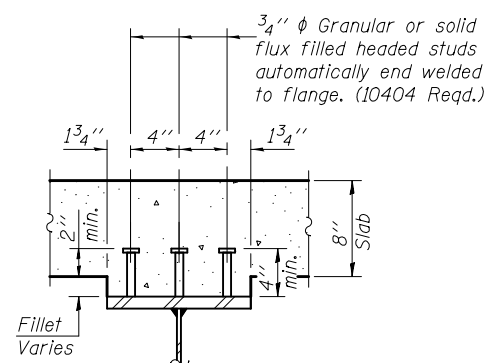
All beams are W33x141, AASHTO M 270 Grade 50 (NTR).

℄ F.A.P. 322
(US Route 51)



PLAN (N.B.)

All beams are W33x141, AASHTO M 270 Grade 50 (NTR).



SECTION A-A

DESIGNED - N. R. Barnett
CHECKED - A. R. Sheblb
DRAWN - h.t. duong
CHECKED - NRB/JMO/GRA

EXAMINED	DATE - MAY 6, 2015
PASSED	REVISOR
	REVISOR

REVISOR
REVISOR

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**STRUCTURAL STEEL - SPANS 5 THRU 7 - UNIT 2
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

SHEET NO. 43 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	80
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

***TOP OF BEAM ELEVATIONS - SPANS 5 THRU 7 (S.B.)**

Location	℄ N. Brg. Pier 4	℄ Brg. Pier 5	℄ Splice 4	℄ Brg. Pier 6	℄ Splice 5	℄ S. Brg. Pier 7
Beam 6	605.70	605.96	606.02	606.31	606.38	606.72
Beam 5	605.85	606.12	606.18	606.47	606.54	606.89
Beam 4	605.99	606.26	606.32	606.61	606.68	607.03
Beam 3	606.07	606.33	606.39	606.68	606.75	607.11
Beam 2	605.97	606.24	606.30	606.59	606.66	607.01
Beam 1	605.84	606.12	606.18	606.47	606.54	606.89

*For fabrication use only.

***TOP OF BEAM ELEVATIONS - SPANS 5 THRU 7 (N.B.)**

Location	℄ N. Brg. Pier 4	℄ Brg. Pier 5	℄ Splice 4	℄ Brg. Pier 6	℄ Splice 5	℄ S. Brg. Pier 7
Beam 1	605.99	606.28	606.35	606.64	606.71	607.07
Beam 2	606.14	606.43	606.50	606.79	606.86	607.22
Beam 3	606.26	606.55	606.62	606.91	606.99	607.35
Beam 4	606.22	606.51	606.58	606.87	606.94	607.31
Beam 5	606.11	606.41	606.48	606.77	606.84	607.20
Beam 6	605.97	606.27	606.34	606.63	606.70	607.07

*For fabrication use only.

		0.4 Sp. 5 or 0.6 Sp. 7	Pier 5 or Pier 6	0.5 Sp. 6
I_s	(in ⁴)	7450	7450	7450
$I_c(n)$	(in ⁴)	1974.3	1974.3	1974.3
$I_c(3n)$	(in ⁴)	14495	14495	14495
$I_c(cr)$	(in ⁴)			
S_s	(in ³)	448	448	448
$S_c(n)$	(in ³)	654	654	654
$S_c(3n)$	(in ³)	591	591	591
$S_c(cr)$	(in ³)			
DC1	(k/ft)	.867	.867	.867
M _{DC1}	(k)	269.2	341.7	90.7
DC2	(k/ft)	0.150	0.150	0.150
M _{DC2}	(k)	46.6	59.1	15.7
DW	(k/ft)	0.363	0.363	0.363
M _{DW}	(k)	112.7	143.1	38.0
M _{℄ + IM}	(k)	786.7	691.9	636.3
M _u (Strength I)	(k)	1940.5	1926.5	1303.5
φ _r M _n	(k)	3302.2	2743.1	3429.0
f _s DC1	(ksi)	7.2	9.2	2.4
f _s DC2	(ksi)	0.9	1.2	0.3
f _s DW	(ksi)	2.3	2.9	0.8
f _s (℄ + IM)	(ksi)	14.4	12.7	11.7
f _s (Service II)	(ksi)	29.1	29.8	18.7
0.95R _h F _{yf}	(ksi)	47.5	47.5	47.5
f _s (Total)(Strength I)	(ksi)			
φ _r F _n	(ksi)			
V _r	(k)	27.2	28.2	28.2

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in⁴ and in³).

$I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in⁴ and in³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{℄ + IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).
 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{℄ + IM}$

φ_rM_n: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft.).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
 M_{DC1} / S_{nc}

f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
 $M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.

f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
 $M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

f_s (℄ + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
 $M_{℄ + IM} / S_c(n)$ or $M_{℄ + IM} / S_c(cr)$ as applicable.

f_s (Service II): Sum of stresses as computed below (ksi).
 $f_{sDC1} + f_{sDC2} + f_{sDW} + 1.3 f_s(℄ + IM)$

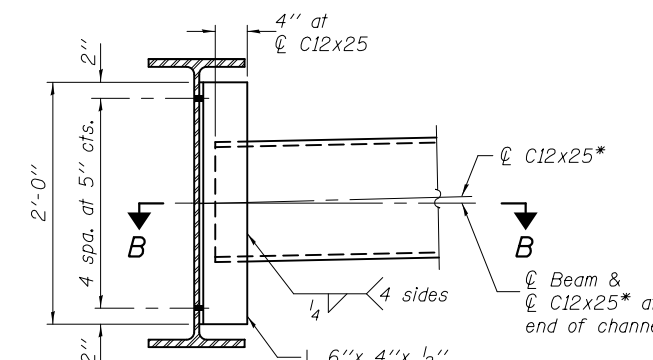
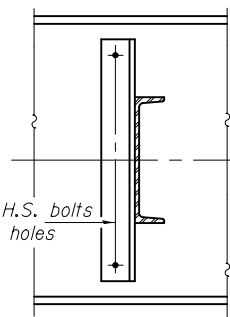
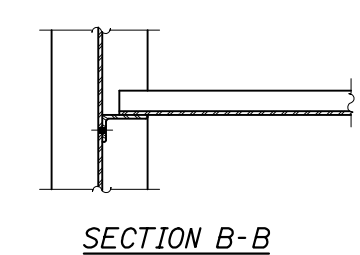
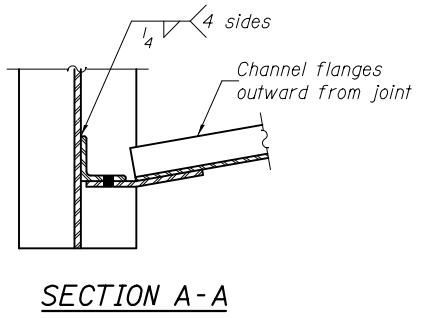
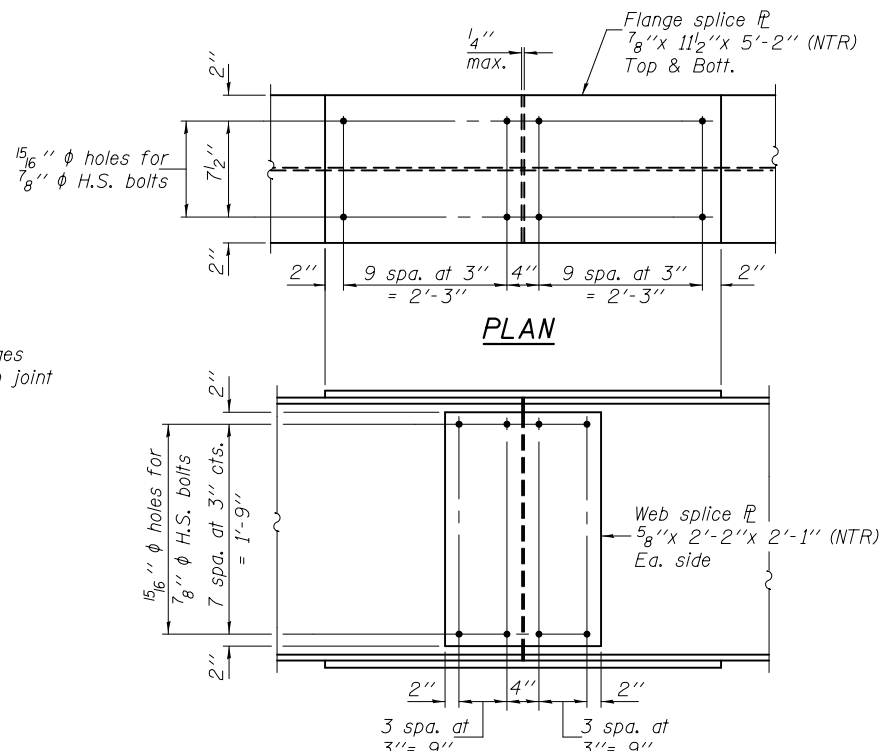
0.95R_hF_{yf}: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
 $1.25 (f_{sDC1} + f_{sDC2}) + 1.5 f_{sDW} + 1.75 f_s(℄ + IM)$

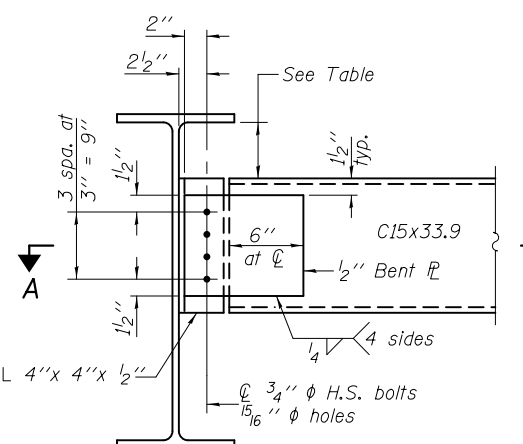
φ_rF_n: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

V_r: Maximum factored shear range in span computed according to Article 6.10.10.

	Pier 4 or Pier 7	Pier 5 or Pier 6
R _{DC1}	(k) 22.4	62.2
R _{DC2}	(k) 3.7	10.4
R _{DW}	(k) 9.0	25.1
R _{℄ + IM}	(k) 79.2	107.3
R _{Total}	(k) 114.3	205.0

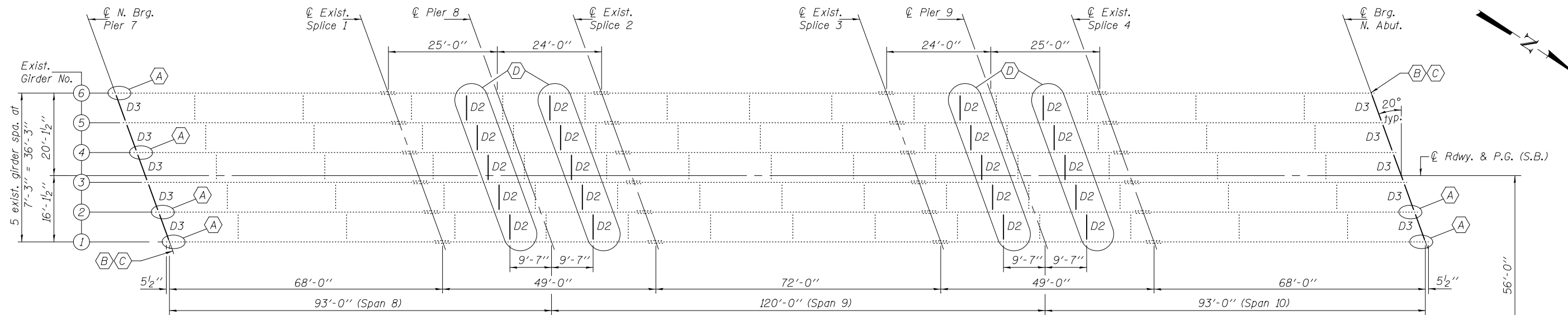


	Pier 4- Unit 2	Pier 7- Unit 2
N.B.	5 1/2"	5 1/2"
S.B.	5 1/2"	5 1/4"

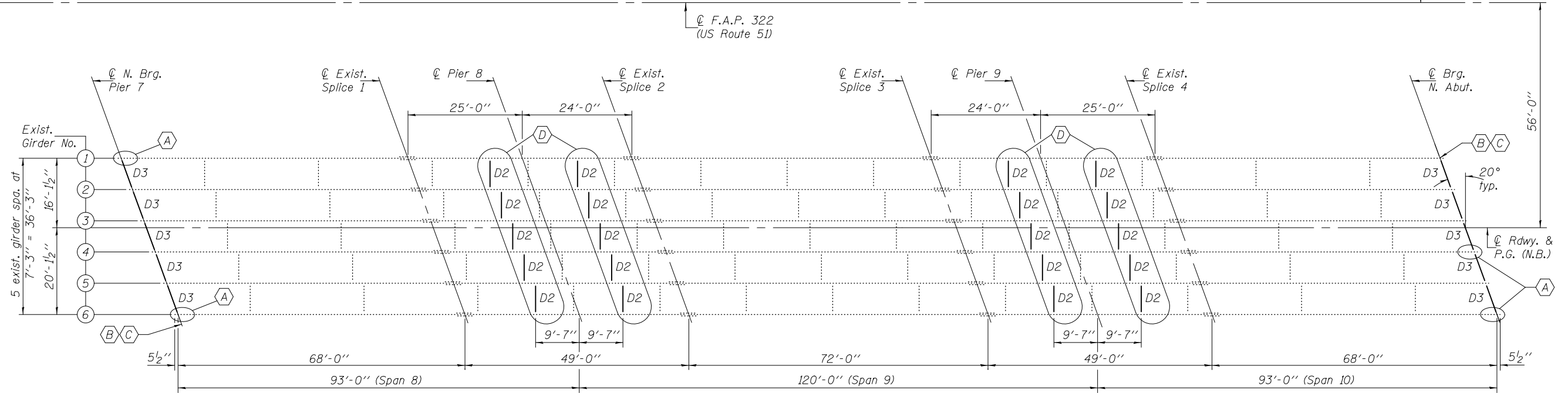


*Alternate channel C12x30 is permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.

Notes: Load carrying components designated "NTR" shall conform to the Impact Testing Requirements, Zone 2. All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor rods. Two hardened washers required for each set of oversized holes. All splice plates shall be AASHTO M 270 Grade 50.



PLAN (S.B.)



PLAN (N.B.)

Notes:

New structural steel, including repair plates, are not to be paid for separately but shall be included in the lump sum unit cost of Furnishing and Erecting Structural Steel.

Existing diaphragms to be replaced are W14x43. Existing diaphragm removal shall be paid for at the unit cost per pound for Structural Steel Removal.

LEGEND

- (A) Beam end repair.
- (B) Diaphragm replacement (typ. all diaphragms at N. Brg. Pier 7 and N. Abut. N.B. & S.B.)
- (C) Bearing replacement (typ. all bearings at N. Brg. Pier 7 and N. Abut. N.B. & S.B.)
- (D) New diaphragm installation.

DESIGNED - A. R. Shebli
CHECKED - N. R. Barnett
DRAWN - h.t. duong
CHECKED - NRB/JOV/GRA

EXAMINED	 ENGINEER OF BRIDGE DESIGN
PASSED	
	 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
REVISED
REVISED

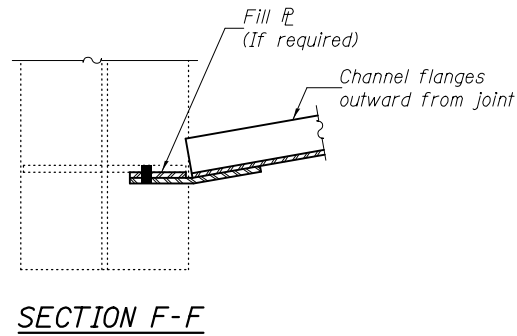
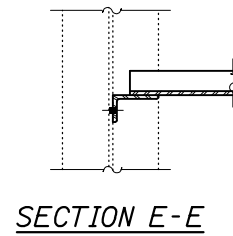
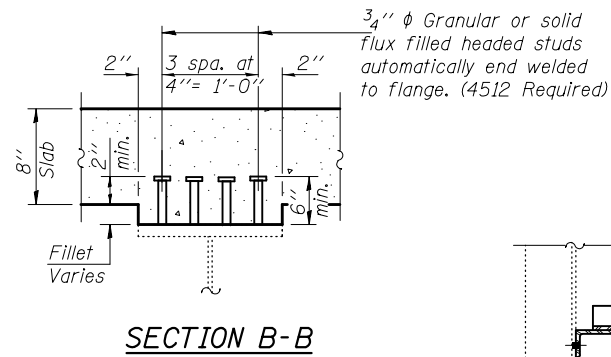
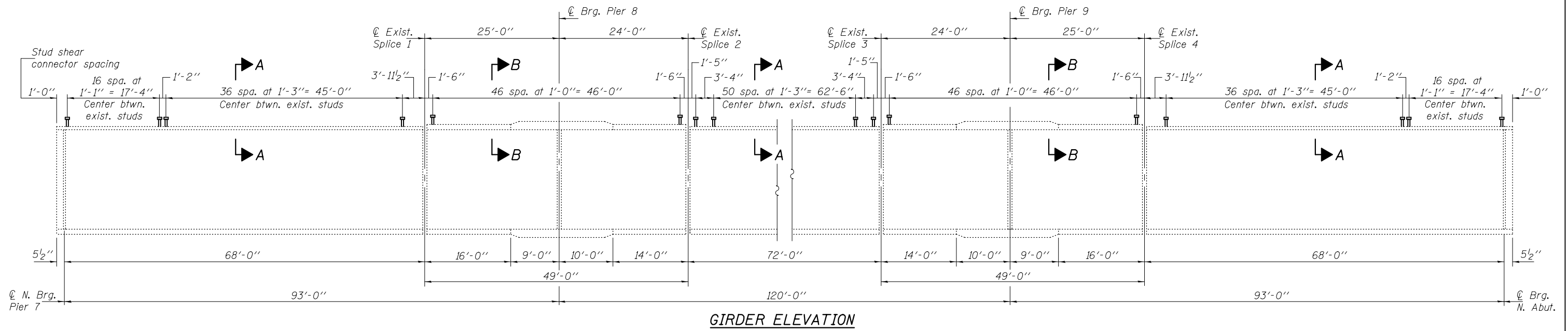
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL - SPANS 8 THRU 10 - UNIT 3
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 45 OF 68 SHEETS

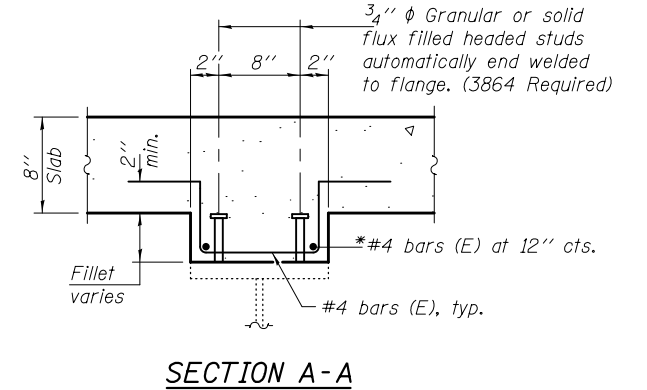
F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 82
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

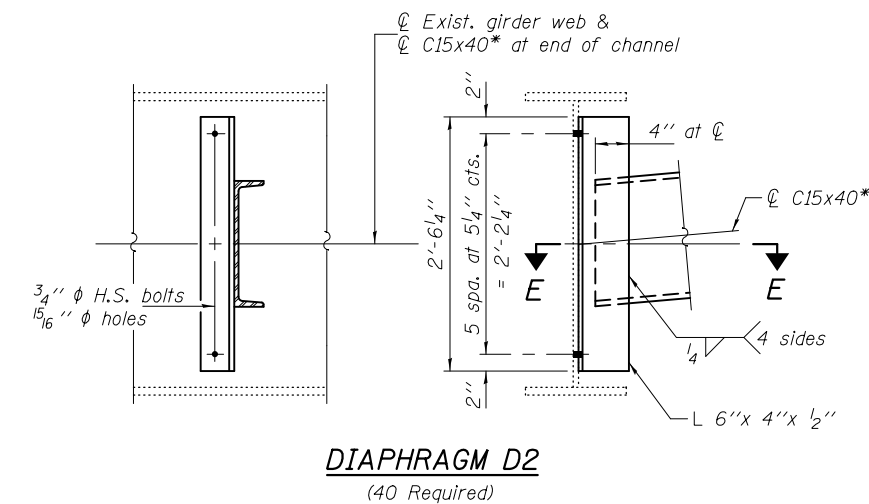


TABLE

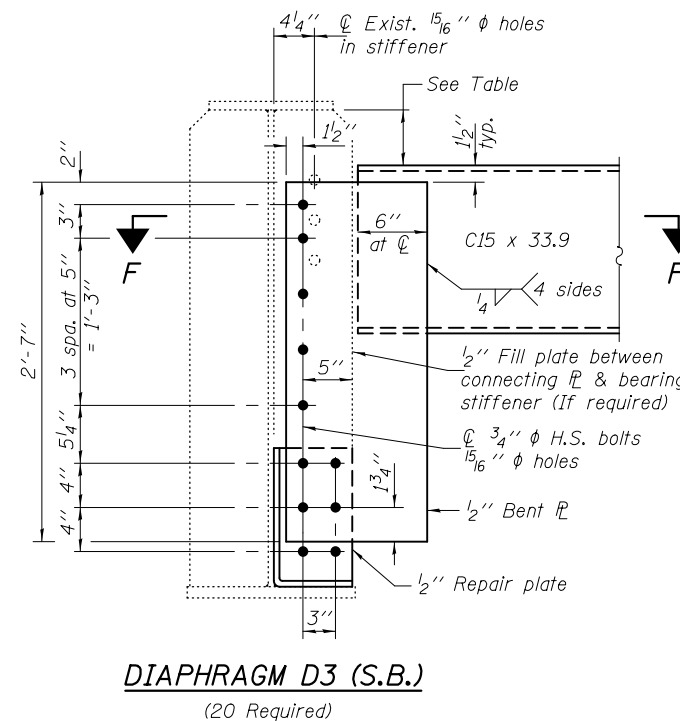
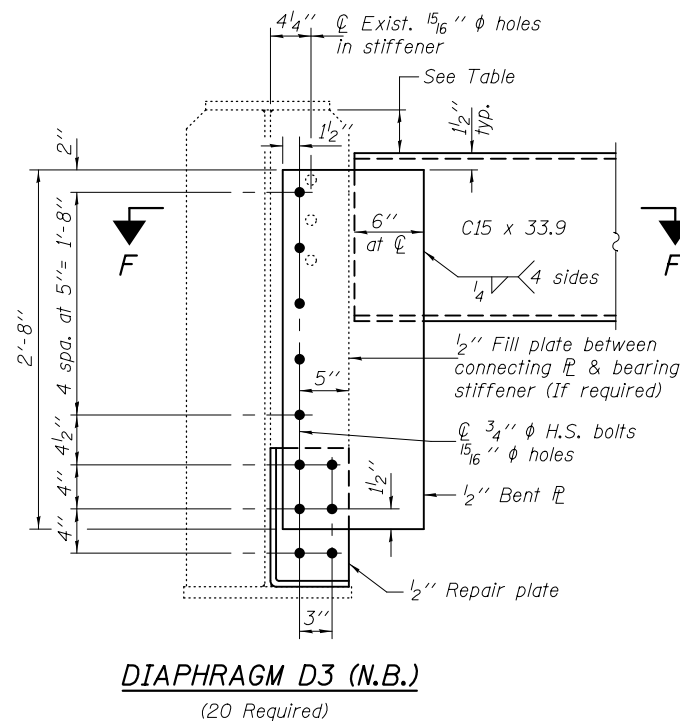
	Pier 7- Unit 3	N. Abut.
N.B.	3 3/4"	5 1/2"
S.B.	5"	5 1/8"



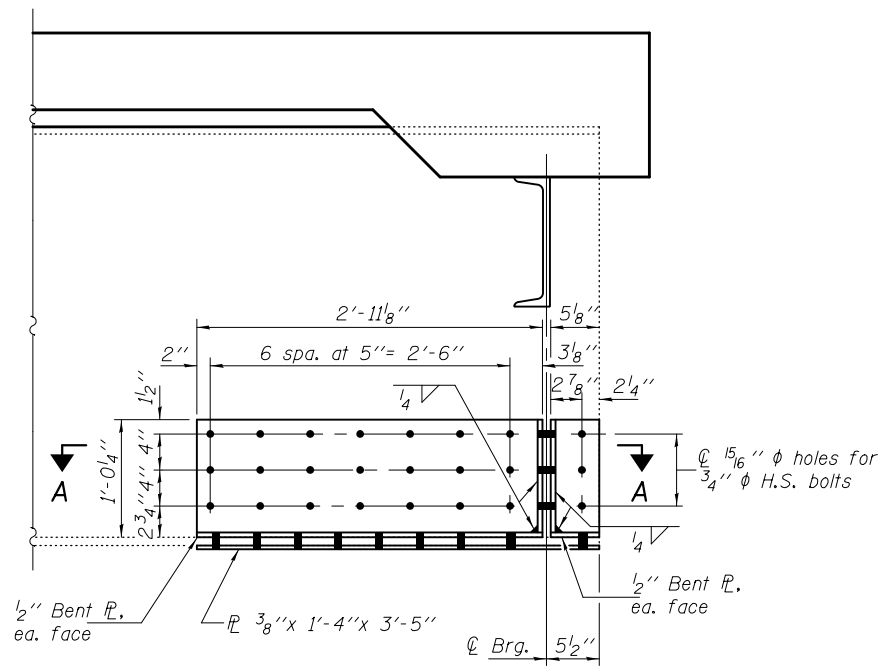
*This bar may be placed inside the shear stud when minimum clearances cannot be satisfied.



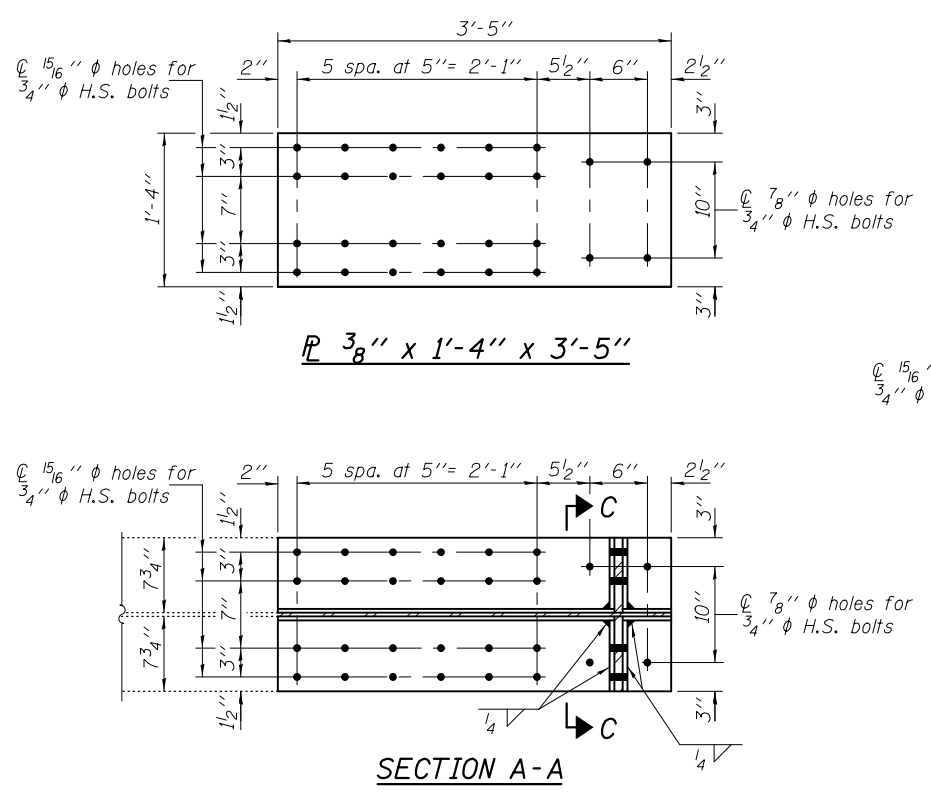
*Alternate channel C15x50 is permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no extra cost to the Department.



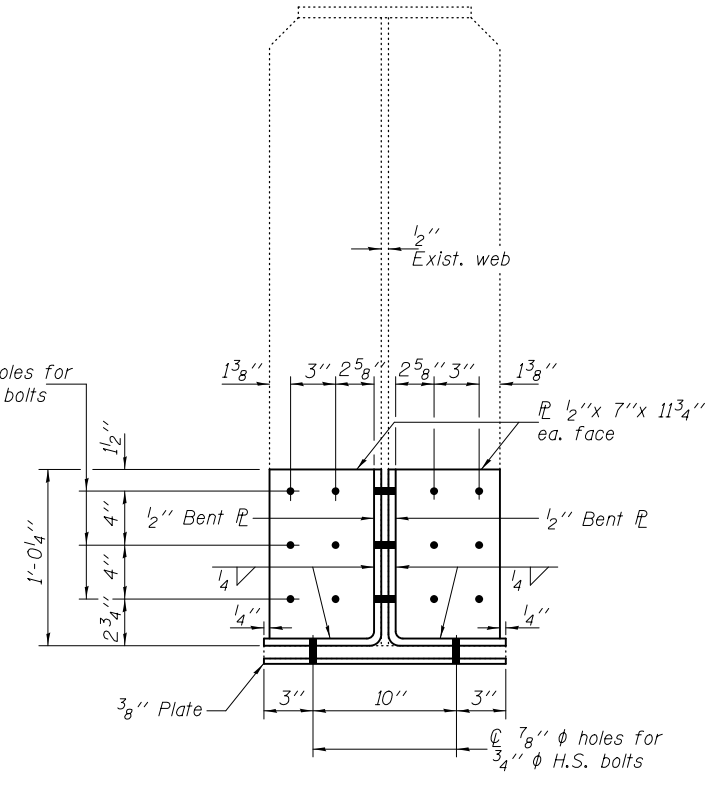
Notes:
1 5/16 inch diameter holes in 1/2 inch bent plates shall be shop drilled and used as a template for field drilling holes in the existing bearing stiffener.
See sheets 47 & 48 of 68 for girder end repair details.



GIRDERS 4 & 6 - ELEVATION AT NORTH ABUT. (N.B.)
GIRDERS 1 & 6 - ELEVATION AT PIER 7 (N.B.)



SECTION A-A



SECTION C-C

EXTERIOR BEAM MOMENT TABLE - SPANS 8 THRU 10				
	0.4 Sp. 8 or 0.6 Sp. 10	Pier 8 or Pier 9	0.5 Sp. 9	
I_s	(in ⁴)	13370	27839	13370
$I_c(n)$	(in ⁴)	36862	55731	36862
$I_c(3n)$	(in ⁴)	26722	41756	26722
$I_c(cr)$	(in ⁴)	—	32943	—
S_s	(in ³)	553	1230	553
$S_c(n)$	(in ³)	2488	1532	2488
$S_c(3n)$	(in ³)	1184	1414	1184
$S_c(cr)$	(in ³)	—	1280	—
DC1	(k/ft)	0.880	1.002	0.880
MDC1	(k)	464.8	1132.7	447.5
DC2	(k/ft)	0.150	0.150	0.150
MDC2	(k)	81.3	185.9	86.7
DW	(k/ft)	0.363	0.363	0.363
MDW	(k)	196.8	449.9	209.9
$M_{\pm} + IM$	(k)	1438.7	1753.4	1504.9
M_u (Strength I)	(k)	3495.6	5391.6	3616.2
$\phi_r M_n$	(k)	4556.6	5600.6	4556.6
f_s DC1	(ksi)	10.1	11.1	9.7
f_s DC2	(ksi)	0.8	1.7	0.9
f_s DW	(ksi)	2.0	4.2	2.1
f_s ($\pm + IM$)	(ksi)	6.9	16.4	7.3
f_s (Service II)	(ksi)	21.9	38.3	22.2
$0.95R_n F_y f$	(ksi)	32.7	46.6	32.7
f_s (Total)(Strength I)	(ksi)	—	—	—
$\phi_r F_n$	(ksi)	—	—	—
V_r	(k)	33.1	33.9	33.9

INTERIOR BEAM REACTION TABLE - SPANS 8 THRU 10			
	Pier 7 or N. Abut.	Pier 8 or Pier 9	
R_{DC1}	(k)	29.9	113.0
R_{DC2}	(k)	5.0	18.0
R_{DW}	(k)	12.1	43.4
$R_{\pm} + IM$	(k)	110.8	176.4
R_{Total}	(k)	157.8	350.8

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in⁴ and in³).

$I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in⁴ and in³).

DC1: Un-factored non-composite dead load (kips/ft.).

MDC1: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

$M_{\pm} + IM$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

$1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{\pm} + IM$

$\phi_r M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft.).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

M_{DC1} / S_{nc}

f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).

$M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.

f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

$M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

f_s ($\pm + IM$): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).

$M_{\pm} + IM / S_c(n)$ or $M_{\pm} + IM / S_c(cr)$ as applicable.

f_s (Service II): Sum of stresses as computed below (ksi).

$f_{sDC1} + f_{sDC2} + f_{sDW} + 1.3 f_s (\pm + IM)$

$0.95R_n F_y f$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

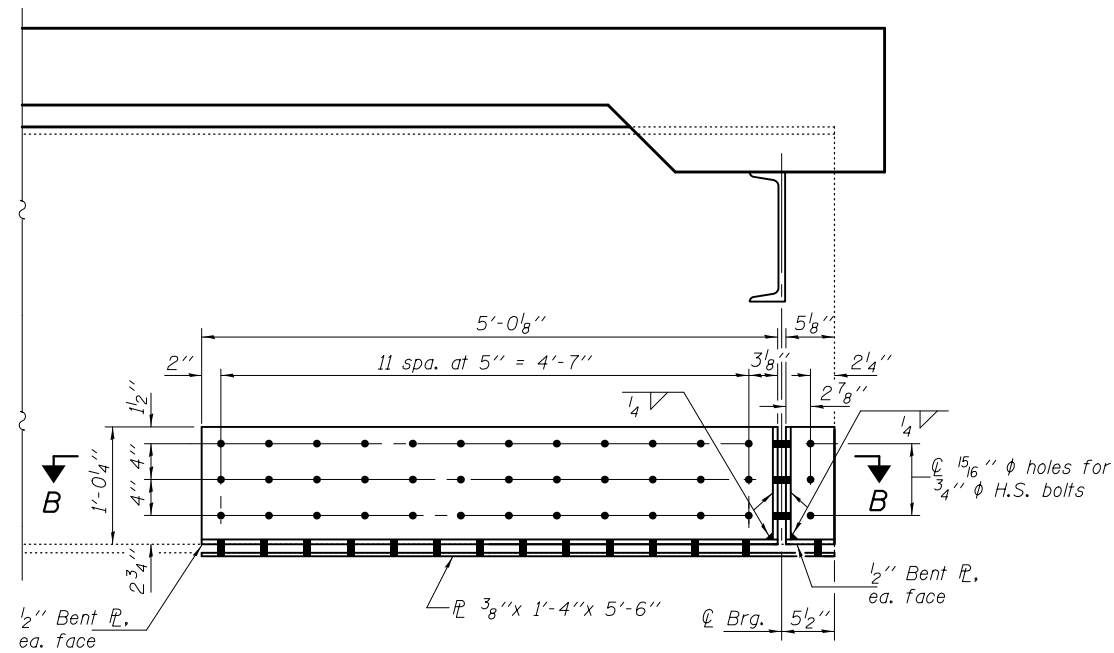
f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).

$1.25 (f_{sDC1} + f_{sDC2}) + 1.5 f_{sDW} + 1.75 f_s (\pm + IM)$

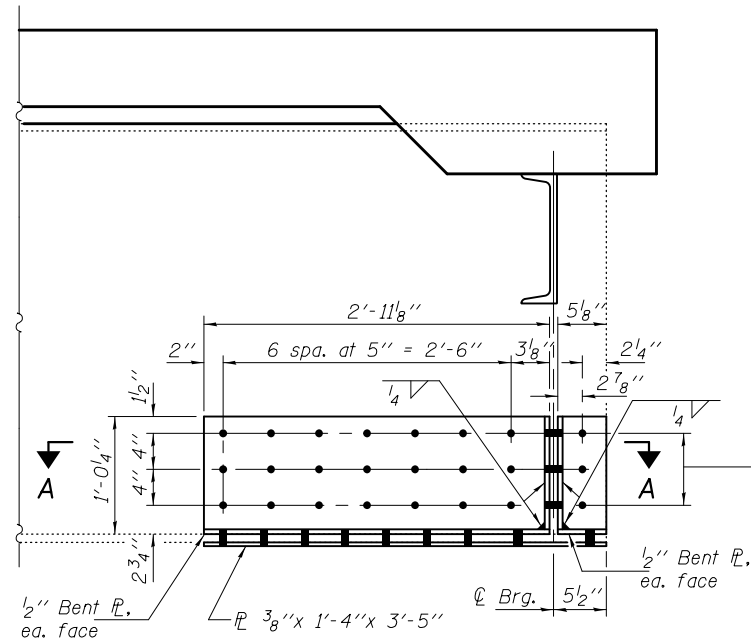
$\phi_r F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

V_r : Maximum factored shear range in span computed according to Article 6.10.10.

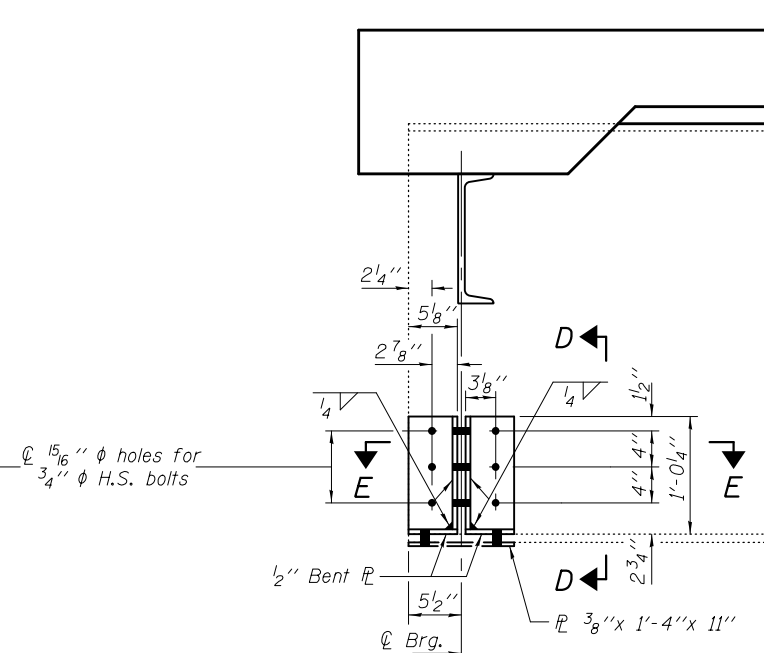
Note: 15/16" ϕ holes in 1/2" plates and 3/8" plates for girder end repairs shall be shop drilled and used as a template for field drilling holes in the existing web, flange, and bearing stiffener.



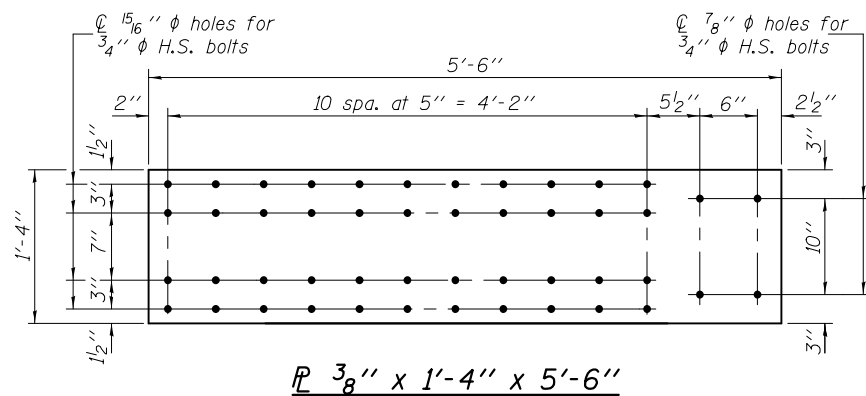
GIRDER 1 - ELEVATION AT PIER 7 (S.B.)



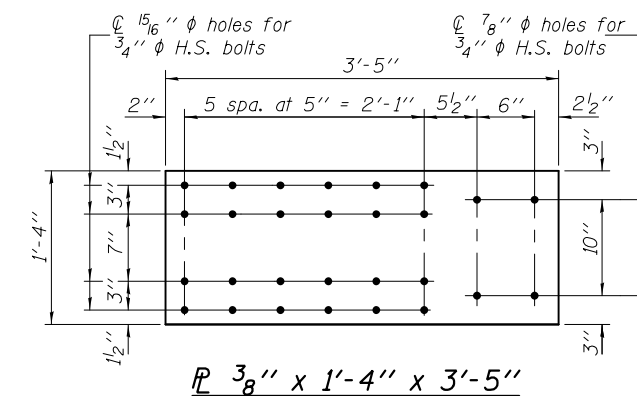
GIRDERS 1 & 2 - ELEVATION AT NORTH ABUT. (S.B.)
GIRDERS 2 & 4 - ELEVATION AT PIER 7 (S.B.)



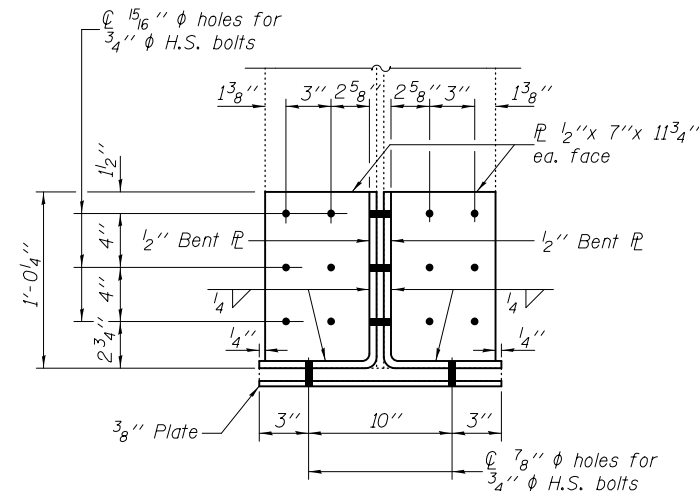
GIRDER 6 - ELEVATION AT PIER 7 (S.B.)
 (East side of girder only)



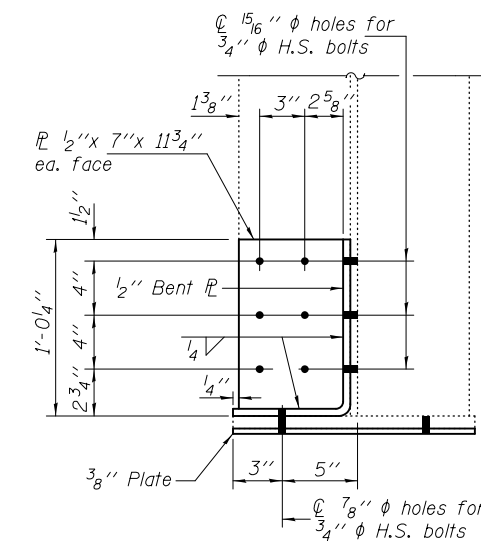
PL 3/8" x 1'-4" x 5'-6"



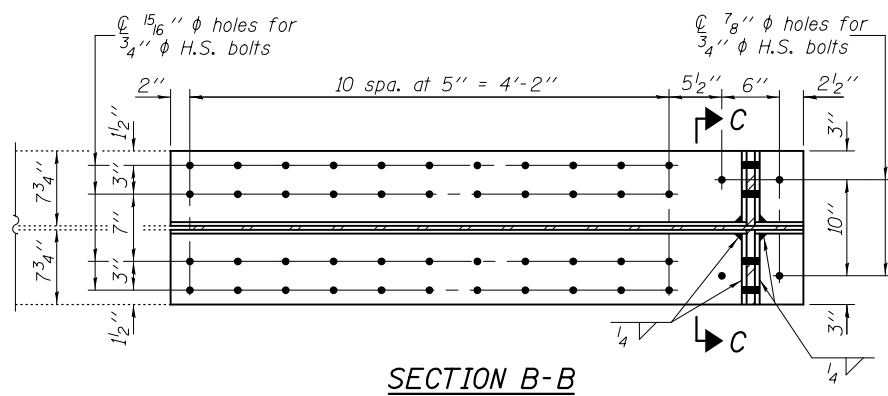
PL 3/8" x 1'-4" x 3'-5"



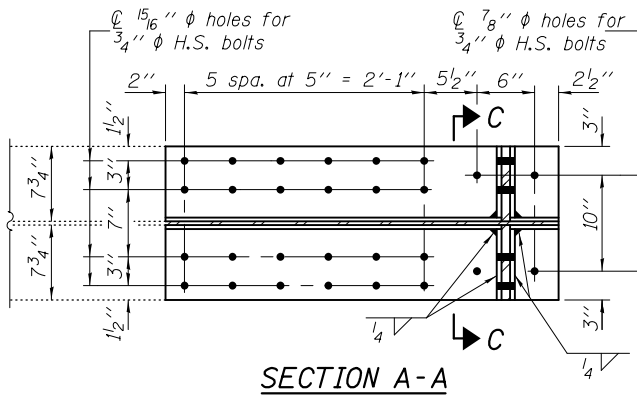
SECTION C-C



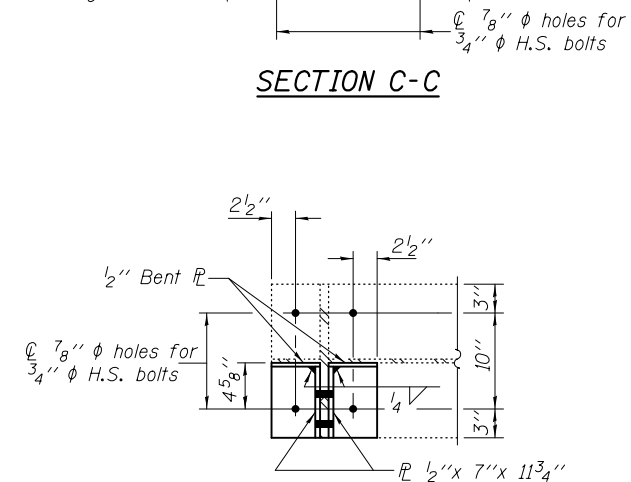
SECTION D-D



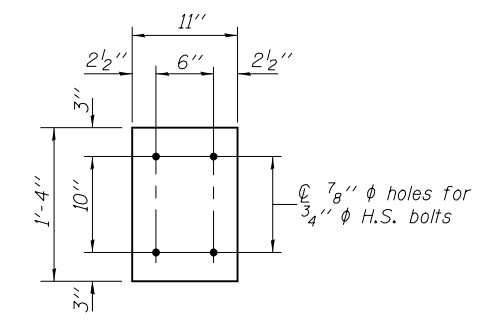
SECTION B-B



SECTION A-A



SECTION E-E



PL 3/8" x 11" x 1'-4"

Note: 15/16" diameter holes in 1/2" plates and 3/8" plates for girder end repairs shall be shop drilled and used as a template for field drilling holes in the existing web, flange, and bearing stiffener.

DESIGNED - A. R. Shebli
 CHECKED - N. R. Barnett
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED
 ENGINEER OF BRIDGE DESIGN
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

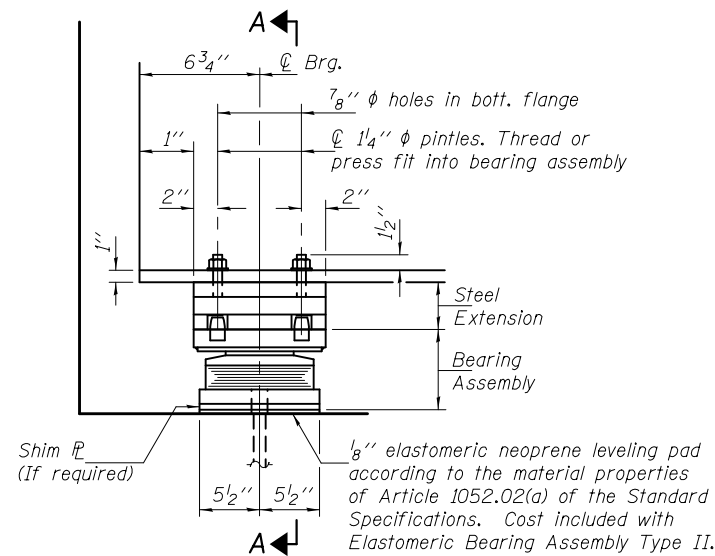
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL DETAILS - SPANS 8 THRU 10 - UNIT 3
 STRUCTURE NO. 058-0099 (S.B.)

SHEET NO. 48 OF 68 SHEETS

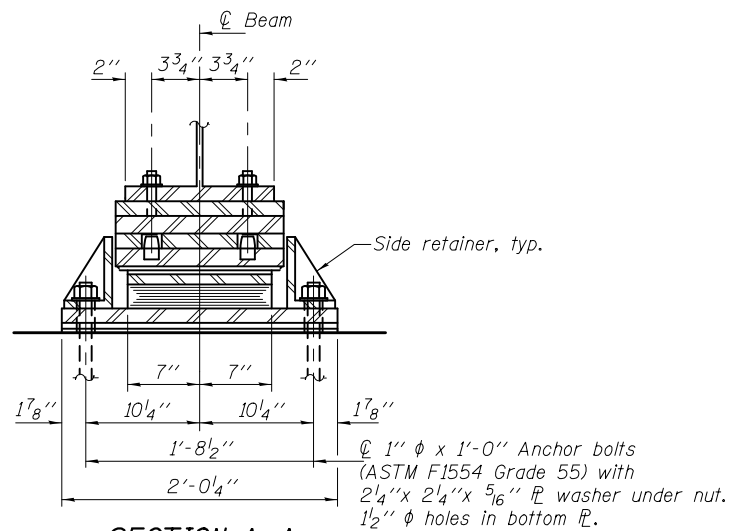
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	85
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

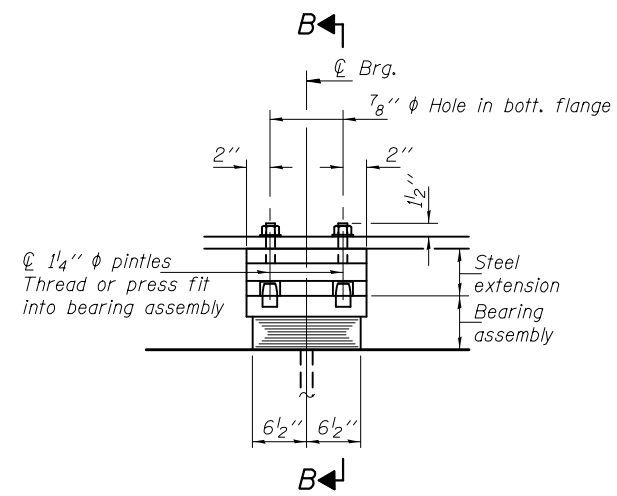


ELEVATION AT SOUTH ABUT.

TYPE II ELASTOMERIC EXP. BRG.

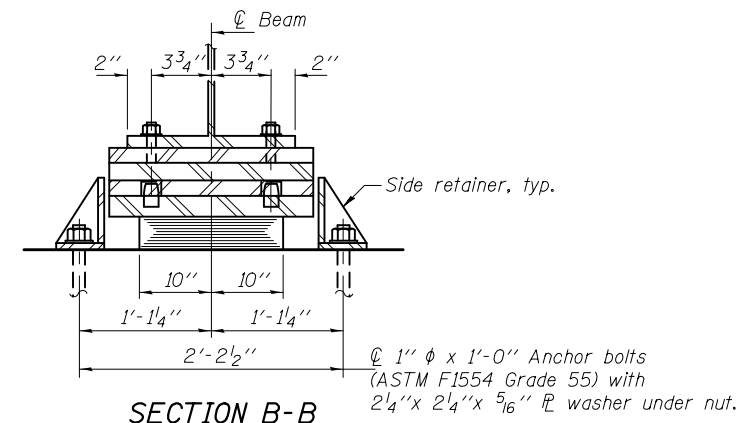


SECTION A-A

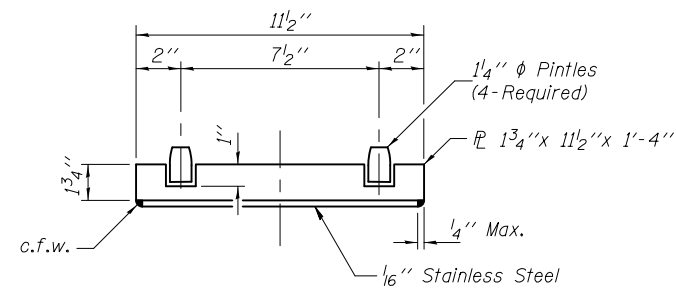


ELEVATION AT PIER 1

TYPE I ELASTOMERIC EXP. BRG.

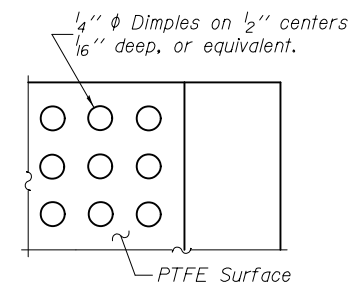


SECTION B-B

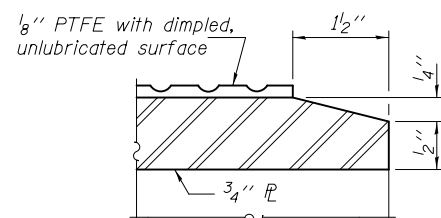


TOP BEARING ASSEMBLY

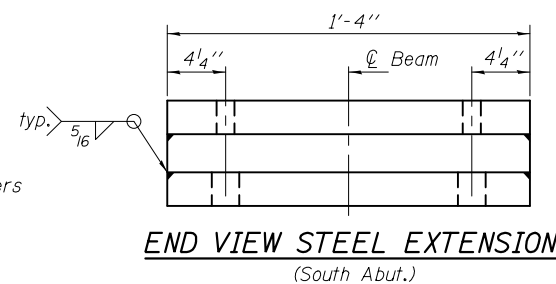
*1/8" PTFE dimpled, unlubricated



PLAN-PTFE SURFACE

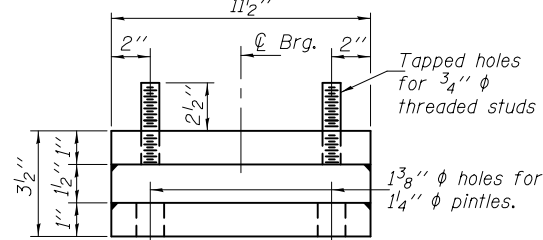


SECTION THRU PTFE



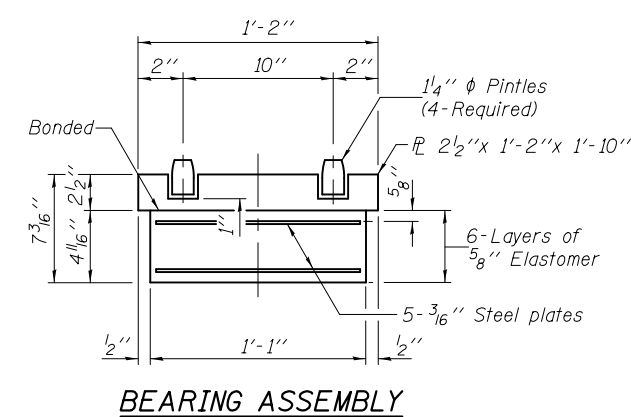
END VIEW STEEL EXTENSION

(South Abut.)



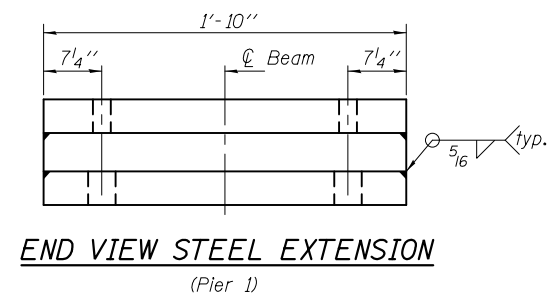
ELEVATION STEEL EXTENSION

(South Abut.)



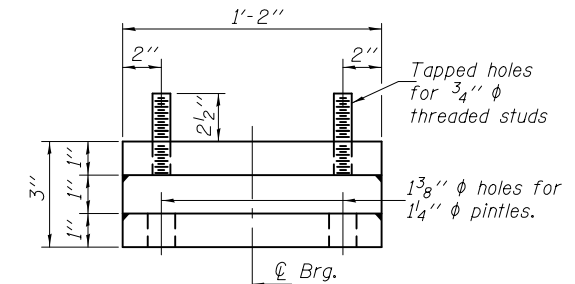
BEARING ASSEMBLY

Note: Shim plates shall not be placed under Bearing Assembly.



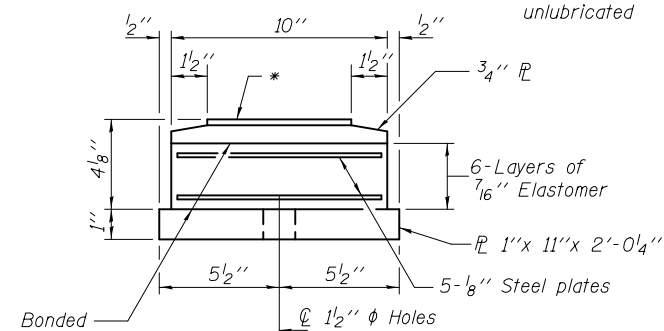
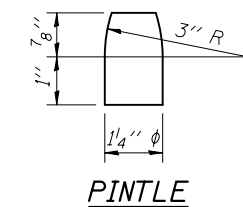
END VIEW STEEL EXTENSION

(Pier 1)

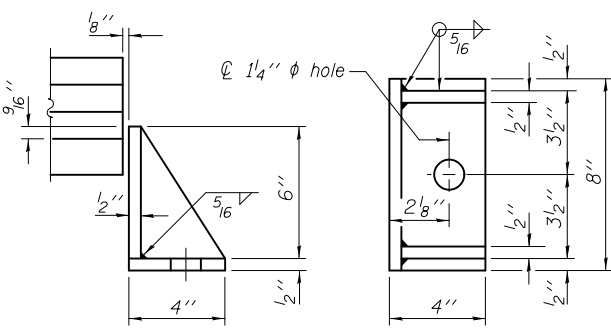


ELEVATION STEEL EXTENSION

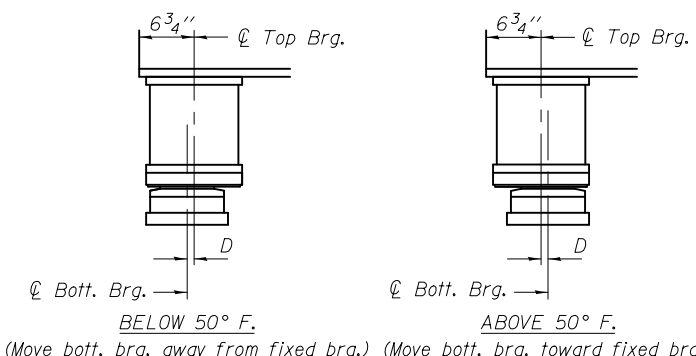
(Pier 1)



BOTTOM BEARING ASSEMBLY



Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



SETTING ANCHOR BOLTS AT EXP. BRG.

D=1/8" per each 100' of expansion for every 15° temp. change from the normal temp. of 50°F.

Notes: Anchor bolts shall be ASTM F1554 all-thread (or an Engineer approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts for side retainers for Type I bearing are to be installed in holes drilled before or after members are in place.

Anchor bolts for Type II bearings shall be placed in holes drilled in the concrete through holes in the bottom bearing plate after members are in place. Side retainers shall be placed after bolts are installed.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly.

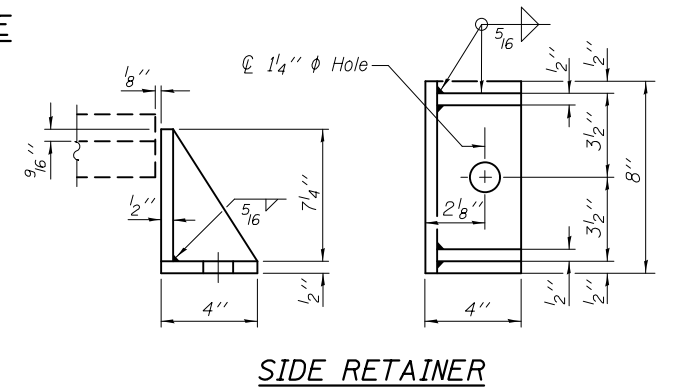
Steel extensions shall be included in the contract unit lump sum cost for Furnishing and Erecting Structural Steel.

The 1/8" PTFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces.

Bonding of 1/8" PTFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.

Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

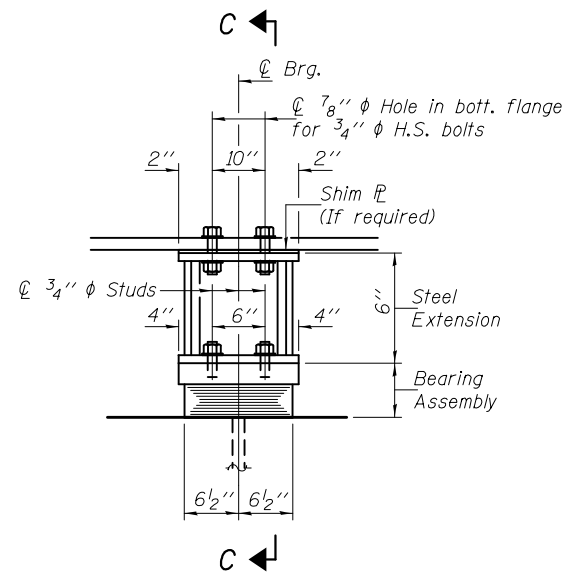
Note: The structural steel plates of the bearing assembly including steel extension shall conform to the requirements of AASHTO M 270 Grade 50.



Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

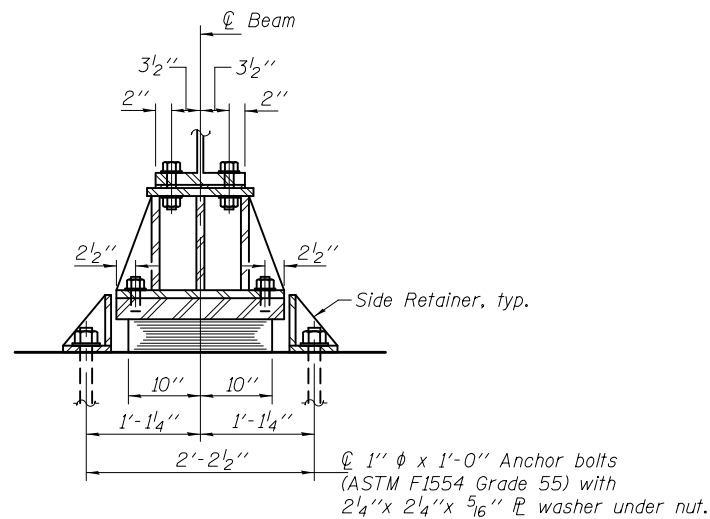
Note: The structural steel plates of the bearing assembly including steel extension shall conform to the requirements of AASHTO M 270 Grade 50.

DESIGNED - N. R. Barnett	EXAMINED - <i>Joanne F. Duff</i>	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BEARING DETAILS - UNIT 1 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 86	
CHECKED - J. D. Ortiz-Varela	PASSED - <i>Carl P. ...</i>	REVISOR			CONTRACT NO. 74351					
DRAWN - h.t. duong	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISOR			SHEET NO. 49 OF 68 SHEETS					
CHECKED - NRB/JOV/GRA					ILLINOIS FED. AID PROJECT					

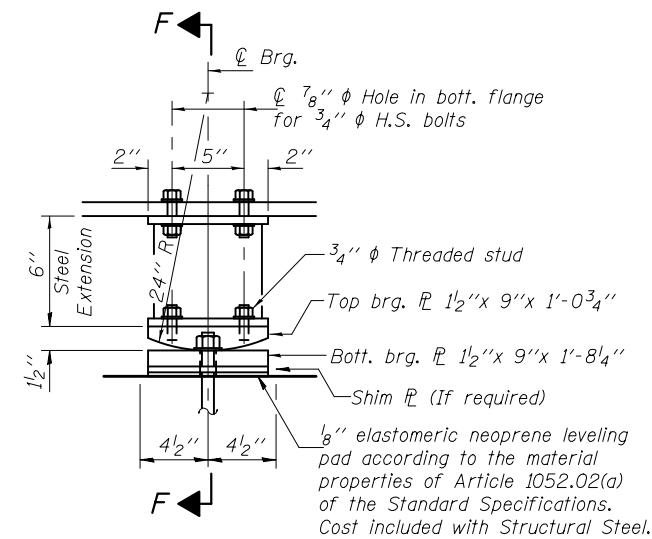


ELEVATION AT PIER 2

TYPE I ELASTOMERIC EXP. BRG.

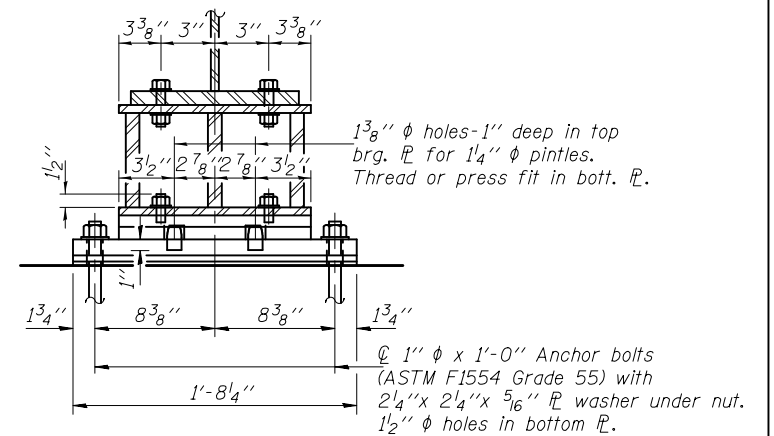


SECTION C-C

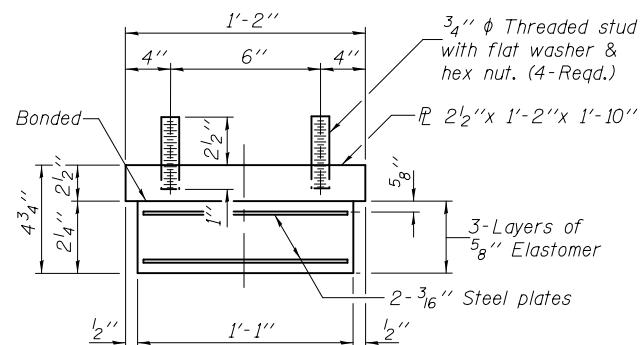


ELEVATION AT PIER 3

FIXED BEARING

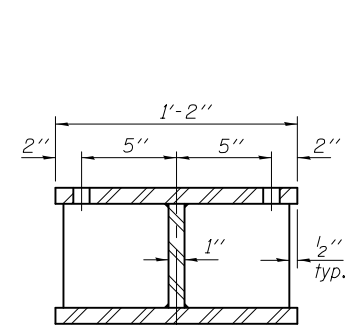


SECTION F-F

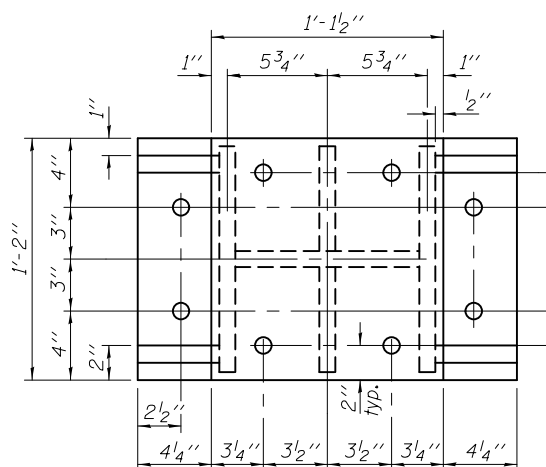


BEARING ASSEMBLY

Note: Shim plates shall not be placed under Bearing Assembly.

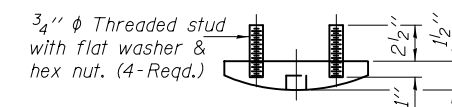


SECTION E-E

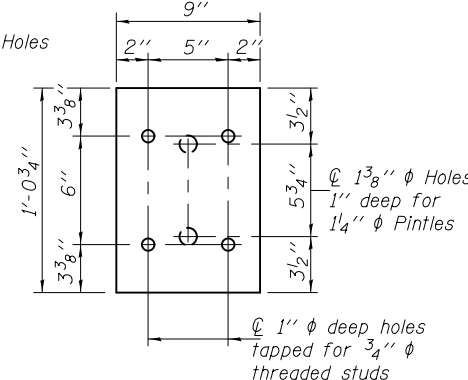


PLAN - TOP & BOTTOM PLATE

(Pier 2)

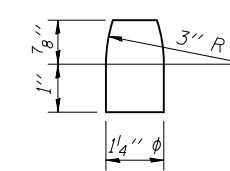


TOP BRG. PLATE DETAIL

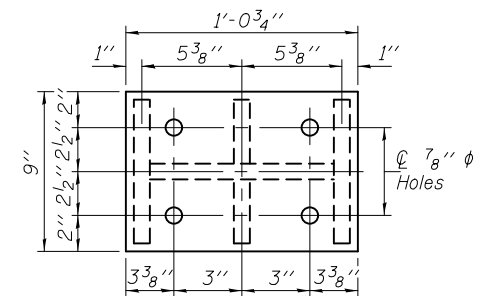


PLAN - TOP BRG. PLATE

(Pier 3)

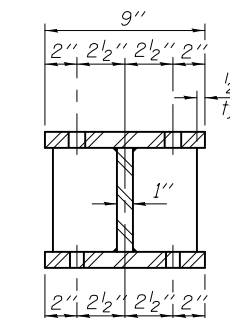


PINTLE

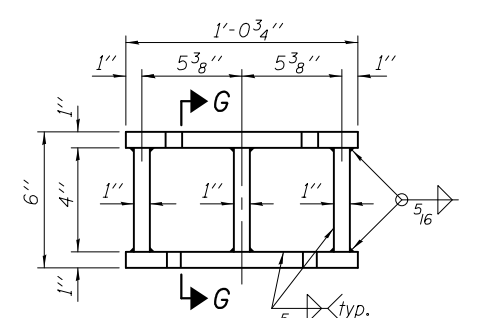


PLAN TOP & BOTTOM PLATE

(Pier 3)

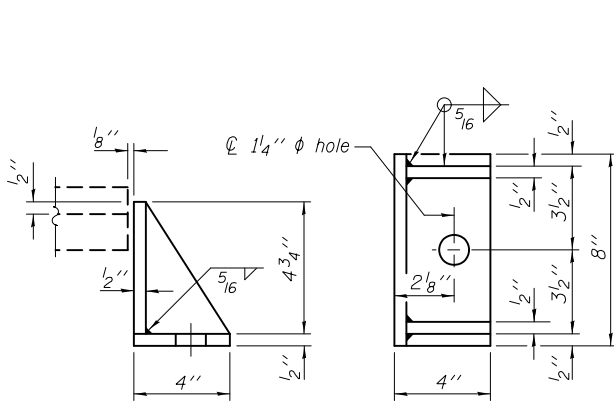


SECTION G-G



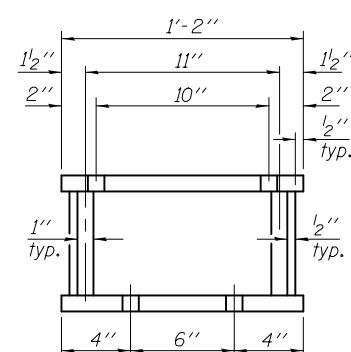
STEEL EXTENSION DETAIL

(Pier 3)

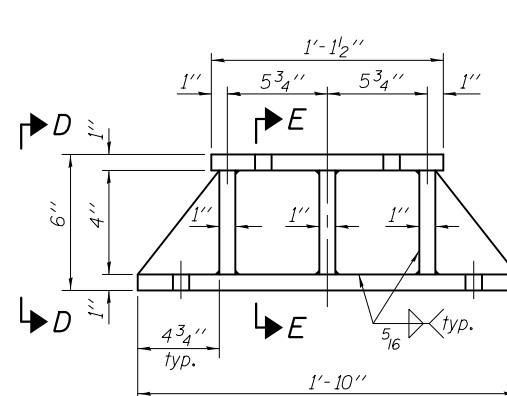


SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

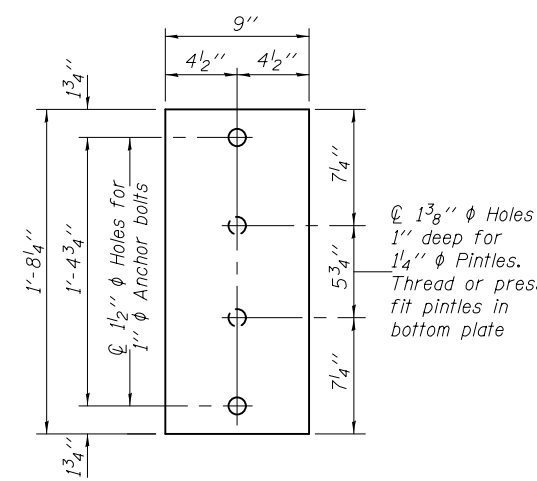


VIEW D-D



FABRICATED STEEL EXTENSION DETAIL

(Pier 2)



PLAN - BOTTOM BRG. PLATE

(Pier 3)

Notes:
 Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
 Anchor bolts for side retainers for Type I bearings are to be installed in holes drilled before or after members are in place. Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
 Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.
 Steel extensions shall be included in the contract unit lump sum cost for Furnishing and Erecting Structural Steel.
 Anchor bolts at fixed bearings are to be installed in holes drilled after the supported member is in place.
 The structural steel plates of the bearing assembly including steel extension shall conform to the requirements of AASHTO M 270 Grade 50.
 Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

DESIGNED - N. R. Barnett	EXAMINED - <i>Joey F. Joffe</i>	DATE - MAY 6, 2015
CHECKED - J. D. Ortiz-Varela	ENGINEER OF BRIDGE DESIGN	
DRAWN - h.t. duong	PASSED - <i>Carl Perry</i>	REVISED
CHECKED - NRB/JOV/GRA	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED

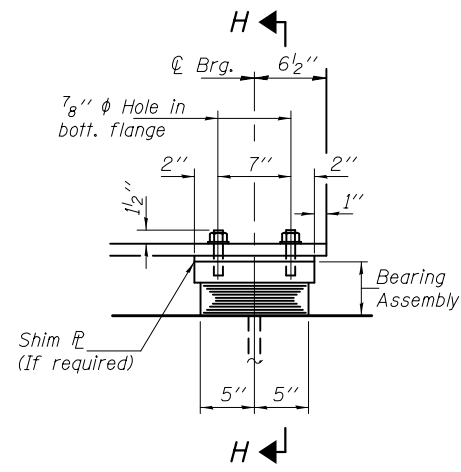
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DATE - MAY 6, 2015
REVISED
REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

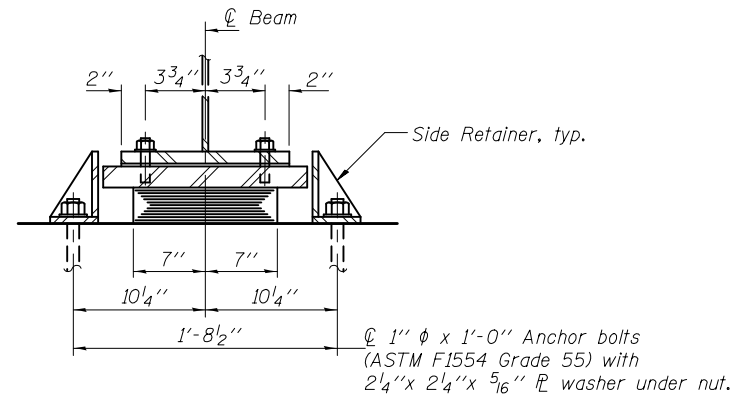
BEARING DETAILS - UNIT 1
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)
SHEET NO. 50 OF 68 SHEETS

F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 87
CONTRACT NO. 74351				ILLINOIS FED. AID PROJECT

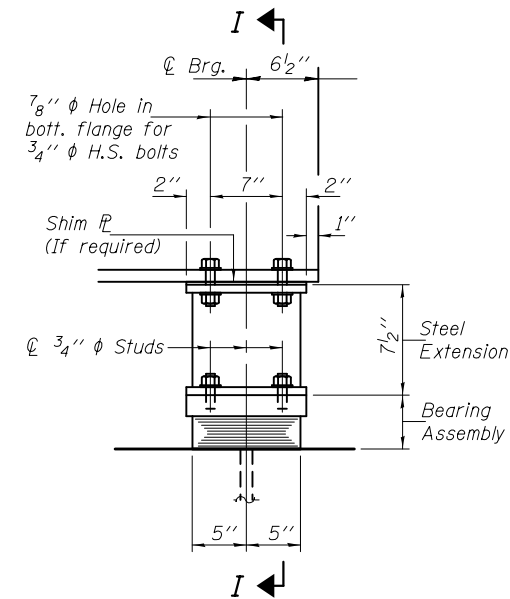


ELEVATION AT S. BRG. PIER 4 (N.B.)

TYPE I ELASTOMERIC EXP. BRG.

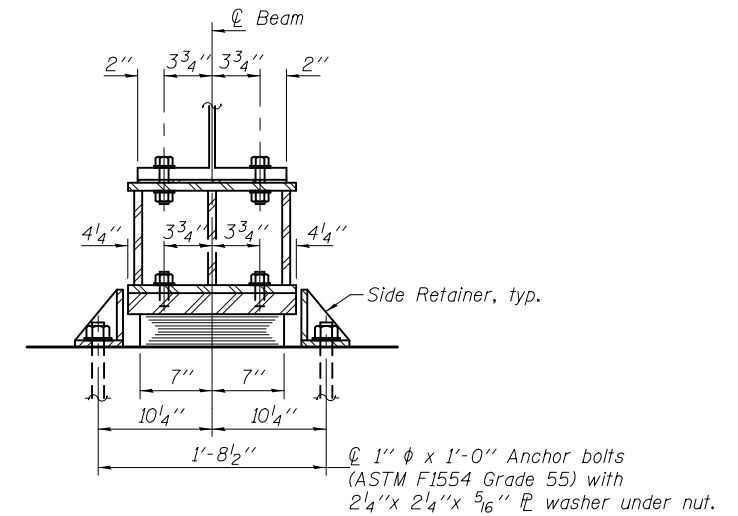


SECTION H-H

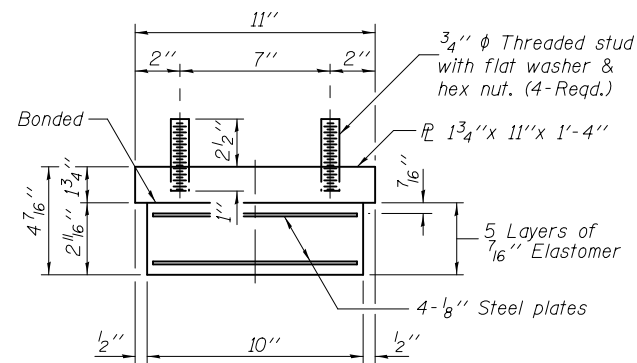


ELEVATION AT S. BRG. PIER 4 (S.B.)

TYPE I ELASTOMERIC EXP. BRG.

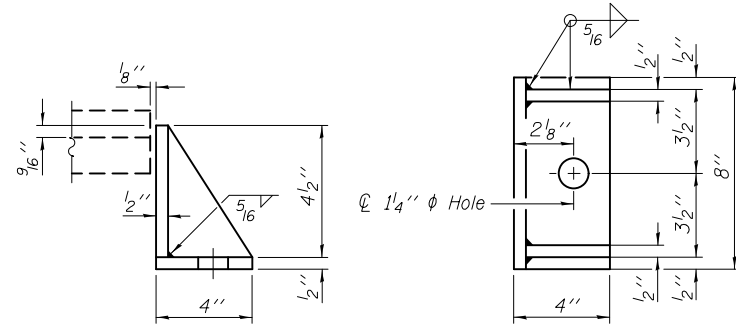


SECTION I-I



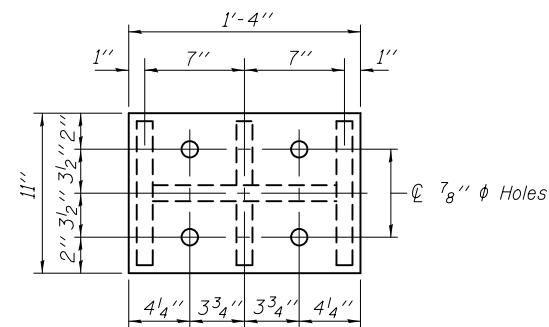
BEARING ASSEMBLY

Note: Shim plates shall not be placed under Bearing Assembly.



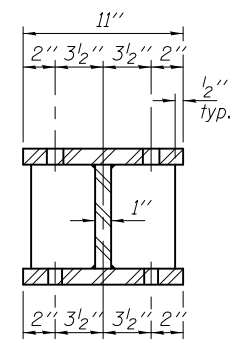
SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

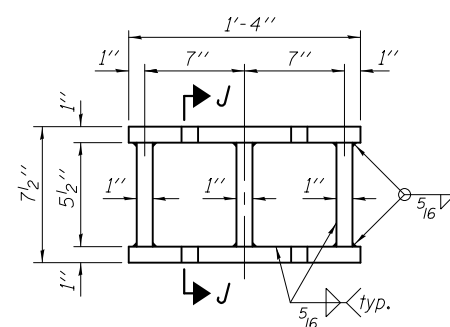


PLAN TOP & BOTTOM PLATE

Notes:
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Anchor bolts for side retainers may be cast in place or installed in holes drilled before or after members are in place.
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.
The structural steel plates of the bearing assembly including steel extension shall conform to the requirements of AASHTO M 270 Grade 50.
Steel extensions shall be included in the contract unit lump sum cost for Furnishing and Erecting Structural Steel.
Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.



SECTION J-J



FABRICATED STEEL EXTENSION DETAIL

*SHIM PLATE THICKNESS

S.N. 058-0098 (N.B.)						
	Beam 1	Beam 2	Beam 3	Beam 4	Beam 5	Beam 6
☉ Brg. S. Abut.	—	—	—	—	1/8"	1/8"
Pier 1	—	—	—	—	—	—
Pier 2	—	—	—	—	—	—
Pier 3	—	—	—	—	—	—
☉ Brg. Pier 4 (Unit 1)	1/4"	1/4"	1 3/4"	1/4"	1/4"	1/4"

*SHIM PLATE THICKNESS

S.N. 058-0099 (S.B.)						
	Beam 6	Beam 5	Beam 4	Beam 3	Beam 2	Beam 1
☉ Brg. S. Abut.	—	1/8"	1/8"	—	—	—
Pier 1	—	—	—	—	—	—
Pier 2	—	—	—	—	—	—
Pier 3	—	—	—	—	—	—
☉ Brg. Pier 4 (Unit 1)	1/4"	1/4"	1/8"	1/8"	1/8"	3/8"

*Cost included with Furnishing and Erecting Structural Steel.

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type I	Each	36
Elastomeric Bearing Assembly Type II	Each	12
Anchor Bolts 1"	Each	120

DESIGNED - N. R. Barnett
CHECKED - J. D. Ortiz-Varela
DRAWN - h.t. duong
CHECKED - NRB/JOV/GRA

EXAMINED
PASSED
ENGINEER OF BRIDGE DESIGN
ACTING ENGINEER OF BRIDGES AND STRUCTURES

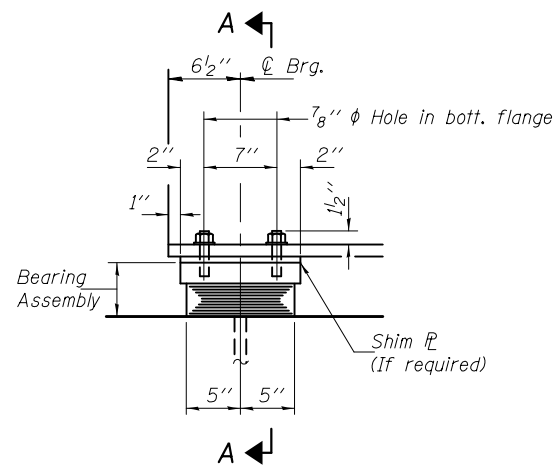
DATE - MAY 6, 2015
REVISED
REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BEARING DETAILS - UNIT 1
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

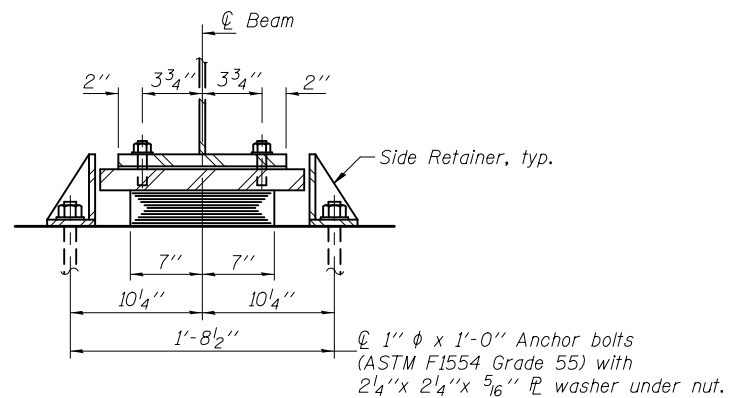
SHEET NO. 51 OF 68 SHEETS

F.A.P. R.T.E. 322
SECTION (58-20B-1)BR
COUNTY MACON
TOTAL SHEETS 122
SHEET NO. 88
CONTRACT NO. 74351
ILLINOIS FED. AID PROJECT

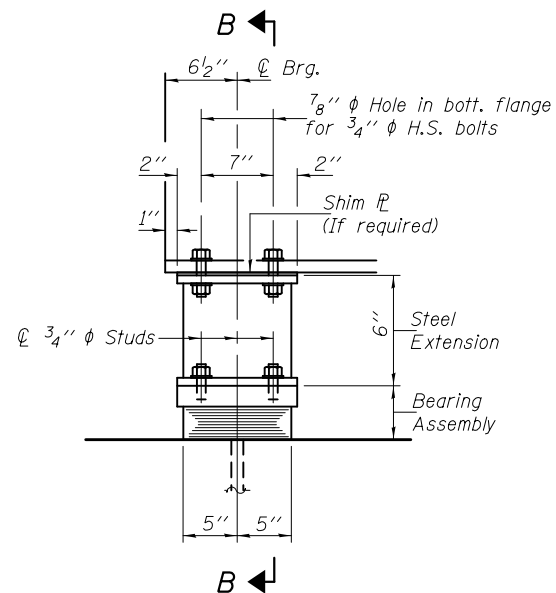


ELEVATION AT N. BRG. PIER 4 (N.B.)

TYPE I ELASTOMERIC EXP. BRG.

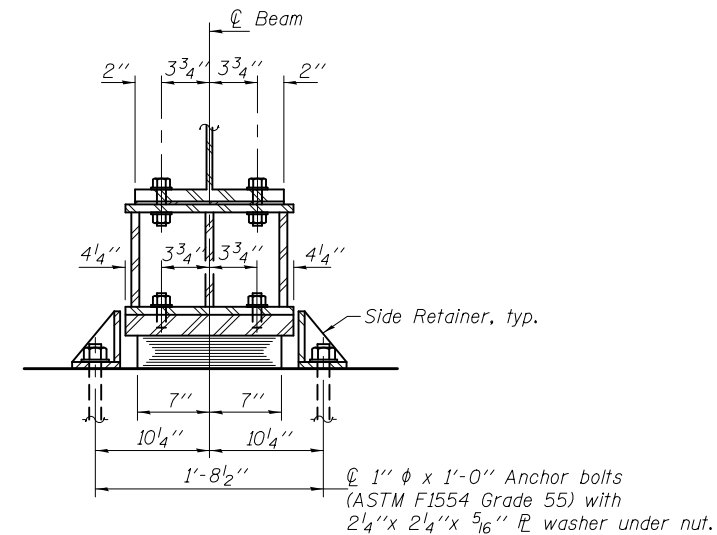


SECTION A-A

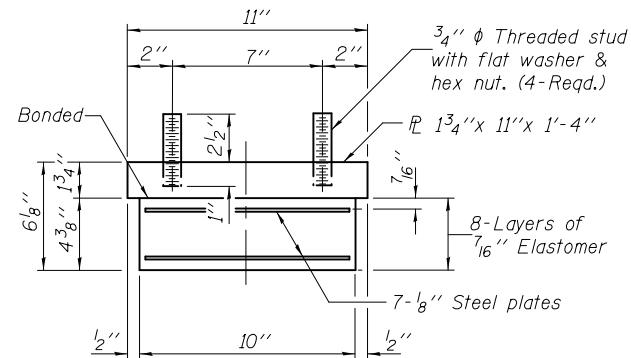


ELEVATION AT N. BRG. PIER 4 (S.B.)

TYPE I ELASTOMERIC EXP. BRG.

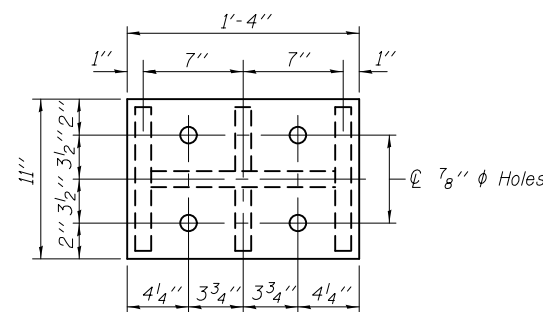


SECTION B-B



BEARING ASSEMBLY

Note: Shim plates shall not be placed under Bearing Assembly.



PLAN TOP & BOTTOM PLATE

Notes:
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts for side retainers may be cast in place or installed in holes drilled before or after members are in place.

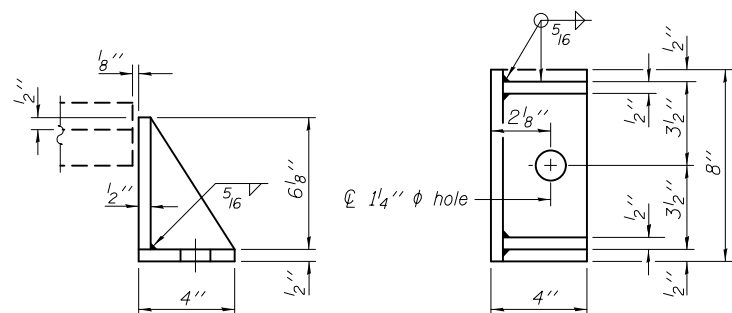
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.

The structural steel plates of the bearing assembly including steel extension shall conform to the requirements of AASHTO M 270 Grade 50.

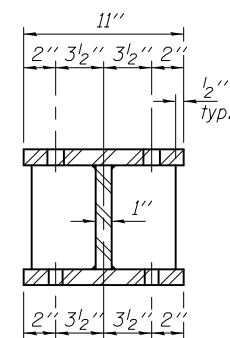
Steel extensions shall be included in the contract unit lump sum cost for Furnishing and Erecting Structural Steel.

Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

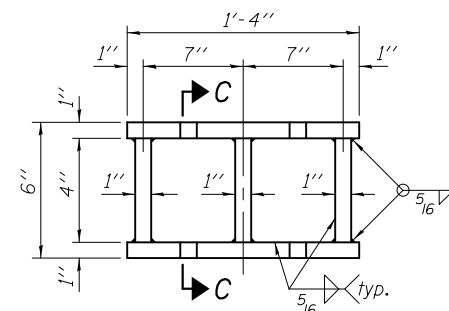


SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



SECTION C-C



FABRICATED STEEL EXTENSION DETAIL

DESIGNED - N. R. Barnett	EXAMINED - <i>Joey F. [Signature]</i>	DATE - MAY 6, 2015
CHECKED - A. R. Sheblb	PASSED - <i>Carl [Signature]</i>	REVISIONS
DRAWN - h.t. duong	ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISIONS
CHECKED - NRB/JMO/GRA		

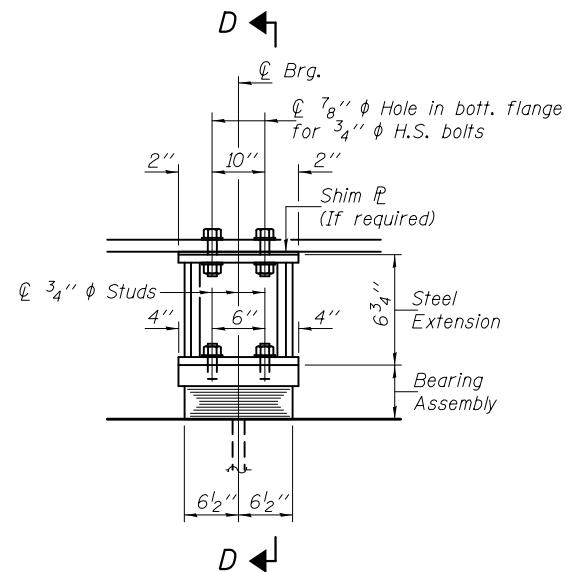
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BEARING DETAILS - UNIT 2
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

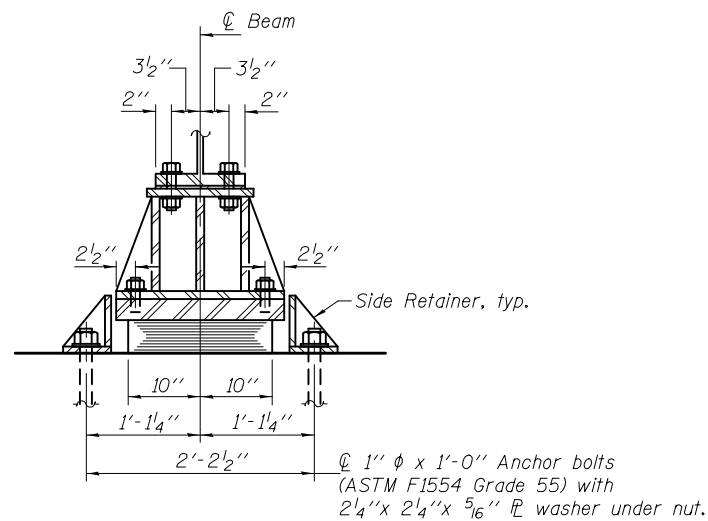
SHEET NO. 52 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	89
CONTRACT NO. 74351				

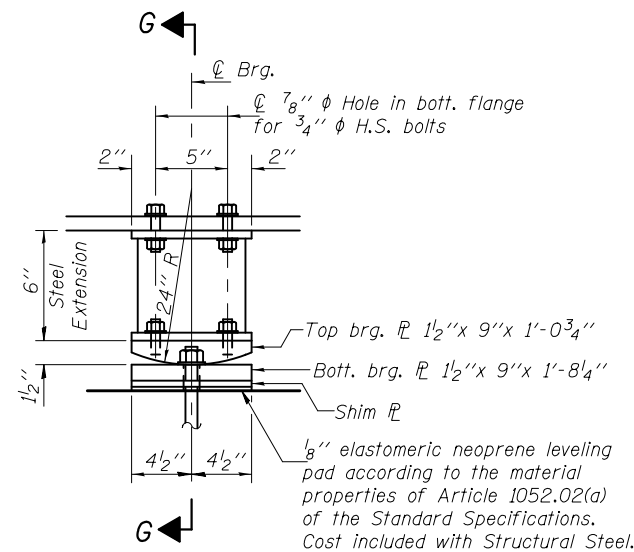
ILLINOIS FED. AID PROJECT



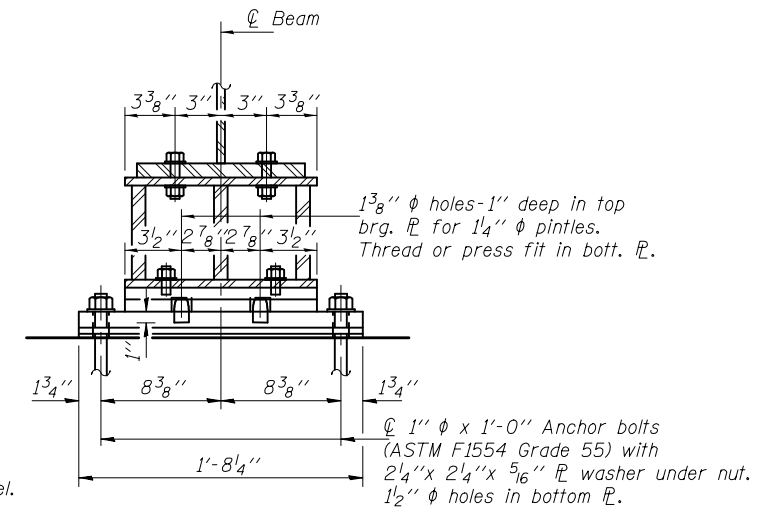
ELEVATION AT PIER 5



SECTION D-D

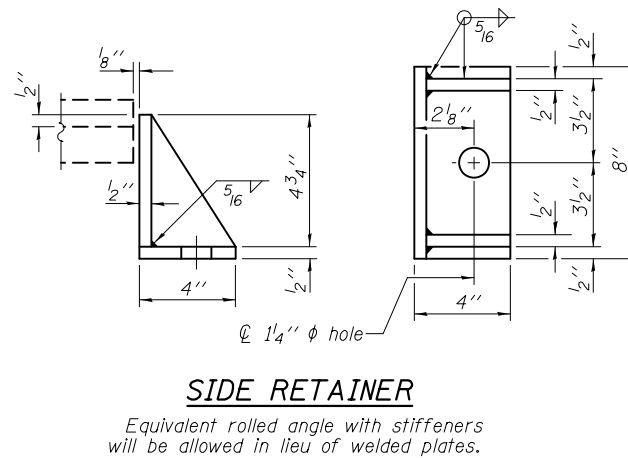
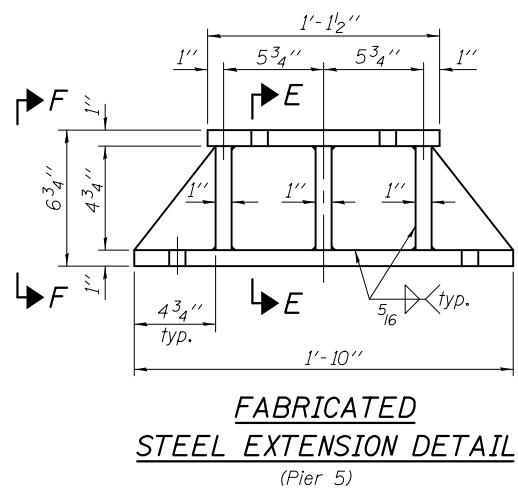
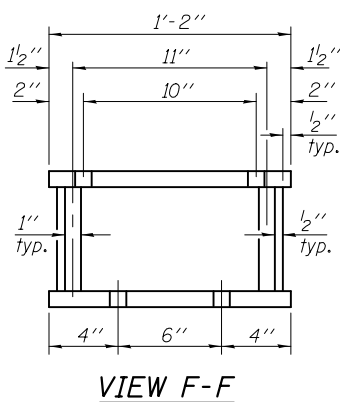
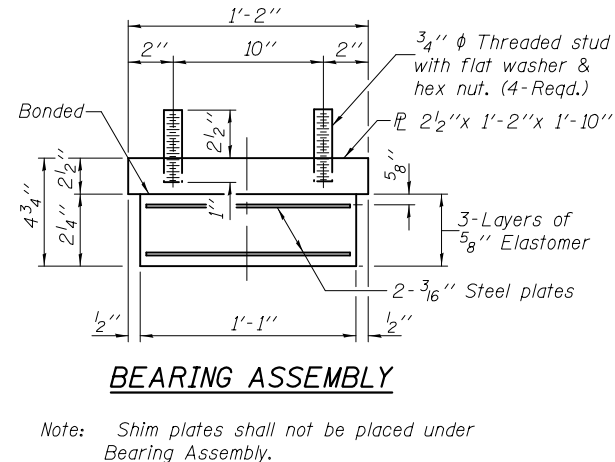
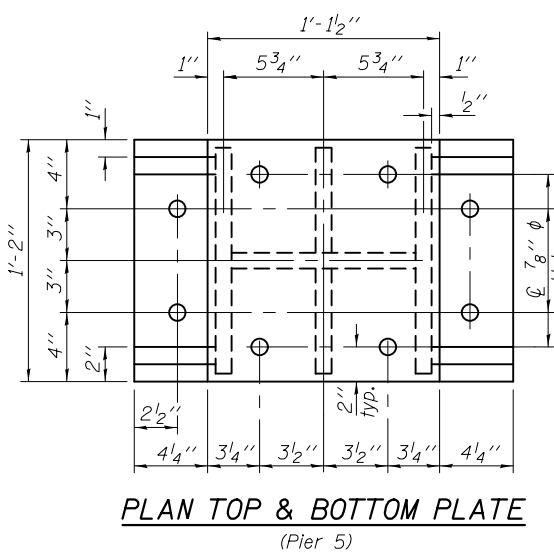
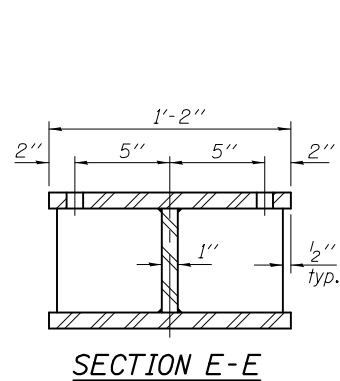


ELEVATION AT PIER 6

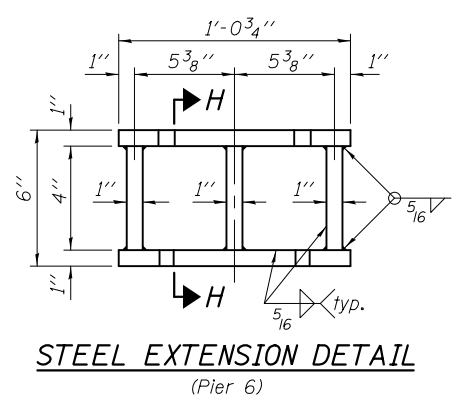
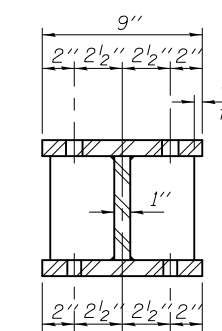
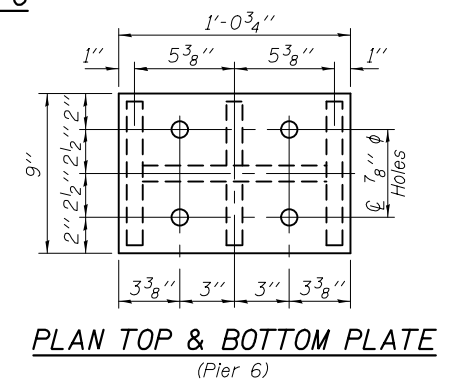
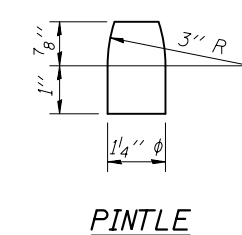
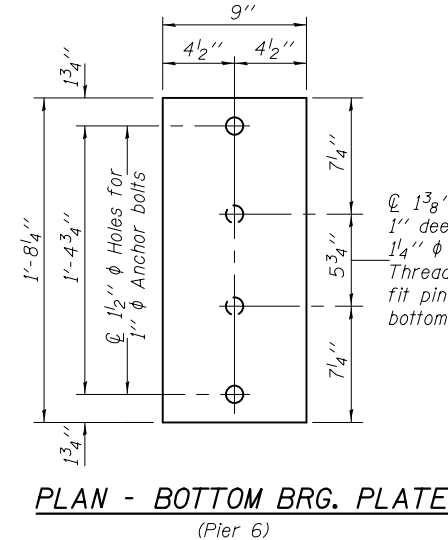
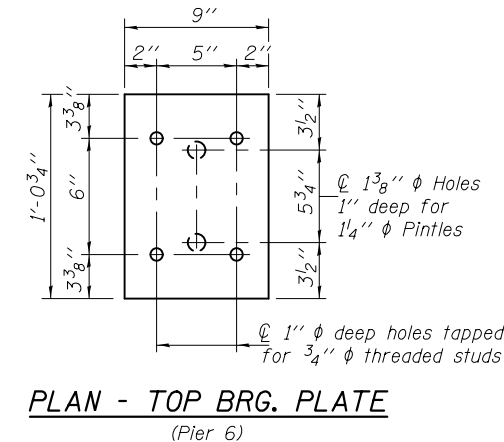
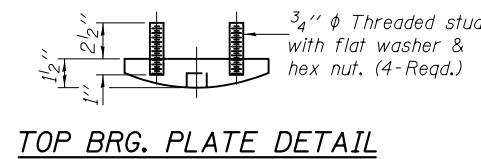


SECTION G-G

TYPE I ELASTOMERIC EXP. BRG.

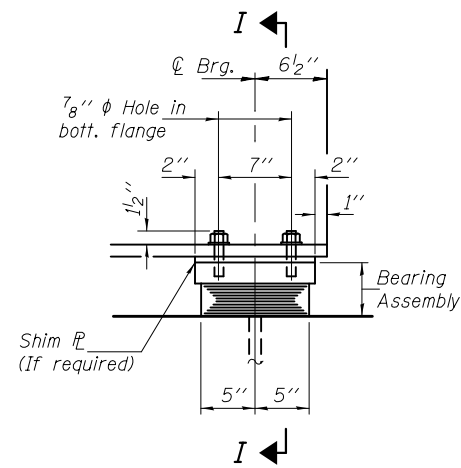


FIXED BEARING



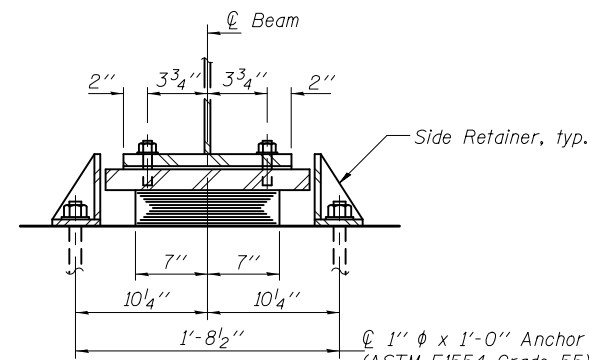
Notes:
 Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
 Anchor bolts for side retainers for Type I bearings are to be installed in holes drilled before or after members are in place. Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
 Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.
 Steel extensions shall be included in the contract unit lump sum cost for Furnishing and Erecting Structural Steel.
 Anchor bolts at fixed bearings are to be installed in holes drilled after the supported member is in place.
 The structural steel plates of the bearing assembly including steel extension shall conform to the requirements of AASHTO M 270 Grade 50.
 Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

DESIGNED - N. R. Barnett	EXAMINED - <i>Joanne F. Duff</i> ENGINEER OF BRIDGE DESIGN	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BEARING DETAILS - UNIT 2 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. R.T.E. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 90
CHECKED - A. R. Sheblb	PASSED - <i>Carl Pung</i> ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED			CONTRACT NO. 74351				
DRAWN - h.t. duong		REVISED			ILLINOIS FED. AID PROJECT				
CHECKED - NRB/JMO/GRA					SHEET NO. 53 OF 68 SHEETS				



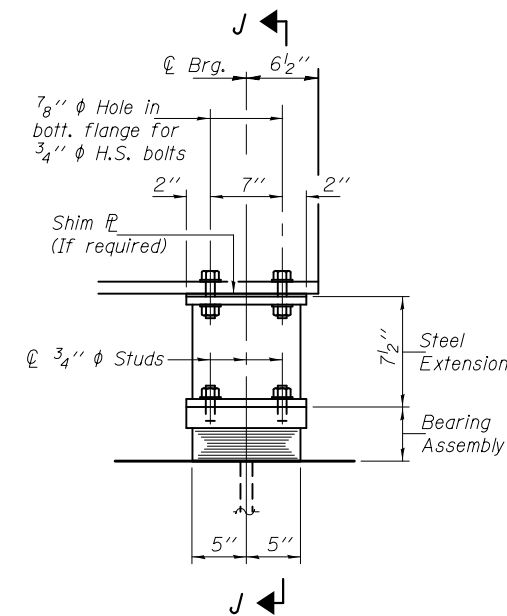
ELEVATION AT S. BRG. PIER 7 (N.B.)

TYPE I ELASTOMERIC EXP. BRG.



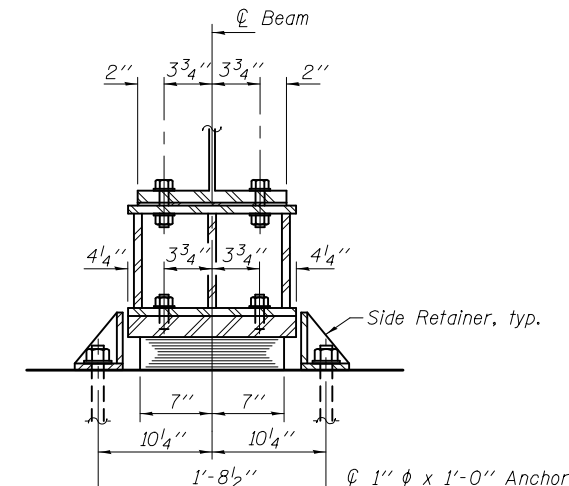
SECTION I-I

1" ϕ x 1'-0" Anchor bolts (ASTM F1554 Grade 55) with 2 1/4" x 2 1/4" x 5/16" PL washer under nut.



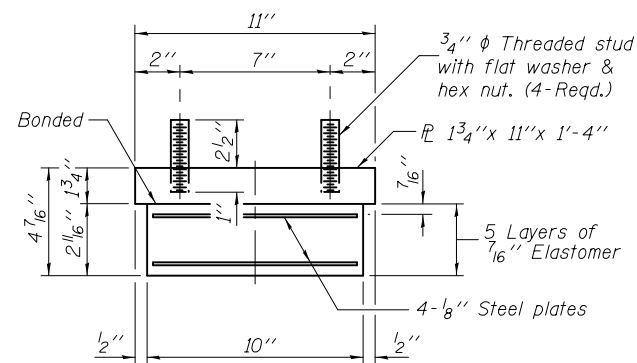
ELEVATION AT S. BRG. PIER 7 (S.B.)

TYPE I ELASTOMERIC EXP. BRG.



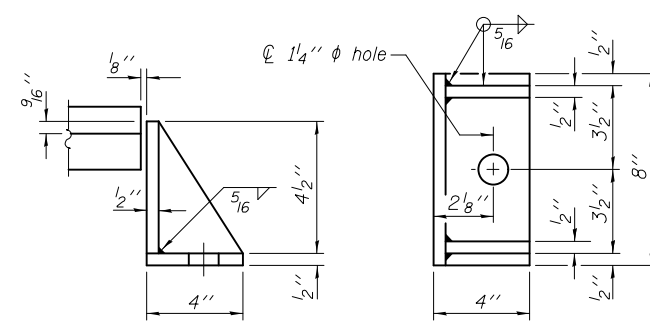
SECTION J-J

1" ϕ x 1'-0" Anchor bolts (ASTM F1554 Grade 55) with 2 1/4" x 2 1/4" x 5/16" PL washer under nut.



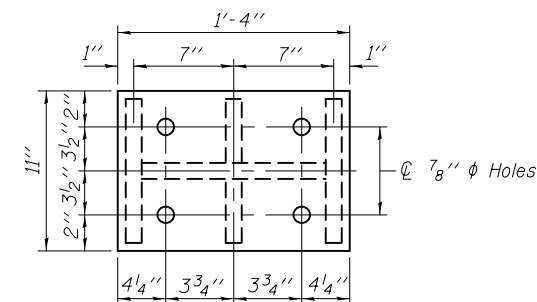
BEARING ASSEMBLY

Note: Shim plates shall not be placed under Bearing Assembly.



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



PLAN TOP & BOTTOM PLATE

Notes:
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts for side retainers may be cast in place or installed in holes drilled before or after members are in place.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.

The structural steel plates of the bearing assembly including steel extension shall conform to the requirements of AASHTO M 270 Grade 50.

Steel extensions shall be included in the contract unit lump sum cost for Furnishing and Erecting Structural Steel.

Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

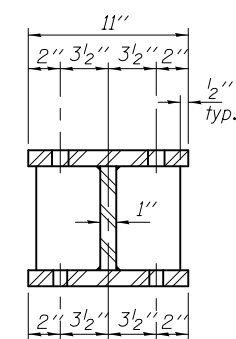
***SHIM PLATE THICKNESS**

S.N. 058-0098 (N.B.)						
	Beam 1	Beam 2	Beam 3	Beam 4	Beam 5	Beam 6
Abutment Pier 4 (Unit 2)						
Pier 5						
Pier 6	1/2"	1/2"	1/2"	1/2"	1/2"	1/2"
Pier 7			1/2"			

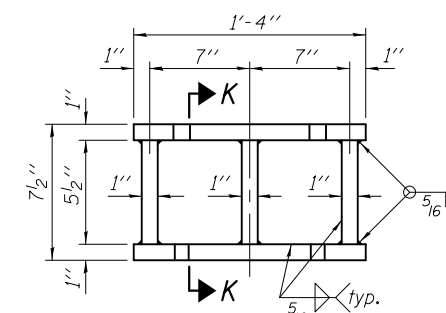
***SHIM PLATE THICKNESS**

S.N. 058-0099 (S.B.)						
	Beam 6	Beam 5	Beam 4	Beam 3	Beam 2	Beam 1
Abutment Pier 4 (Unit 2)	1/8"			1/8"	1/8"	1/4"
Pier 5						
Pier 6	1/2"	1/2"	1/2"	1/2"	1/2"	1/2"
Pier 7						

*Cost included with Furnishing and Erecting Structural Steel.



SECTION K-K



STEEL EXTENSION DETAIL

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type I	Each	36
Anchor Bolts 1"	Each	96

DESIGNED - N. R. Barnett
CHECKED - A. R. Sheblb
DRAWN - h.t. duong
CHECKED - NRB/JMO/GRA

EXAMINED
PASSED
ENGINEER OF BRIDGE DESIGN
ACTING ENGINEER OF BRIDGES AND STRUCTURES

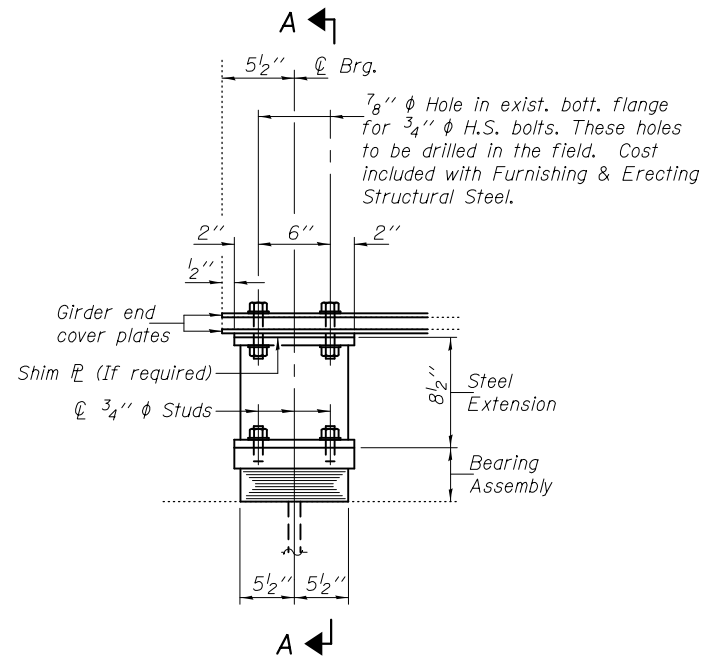
DATE - MAY 6, 2015
REVISED
REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BEARING DETAILS - UNIT 2
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

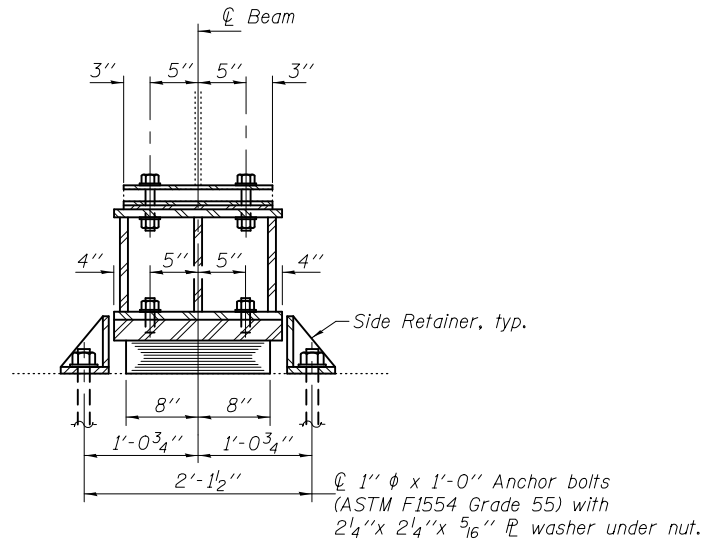
SHEET NO. 54 OF 68 SHEETS

F.A.P. RTE. 322
SECTION (58-20B-1)BR
COUNTY MACON
TOTAL SHEETS 122
SHEET NO. 91
CONTRACT NO. 74351
ILLINOIS FED. AID PROJECT

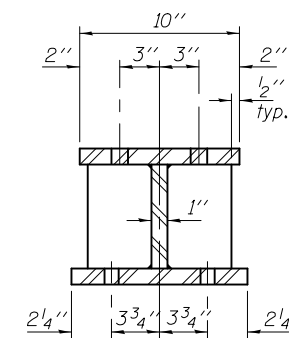


ELEVATION AT N. BRG. PIER 7

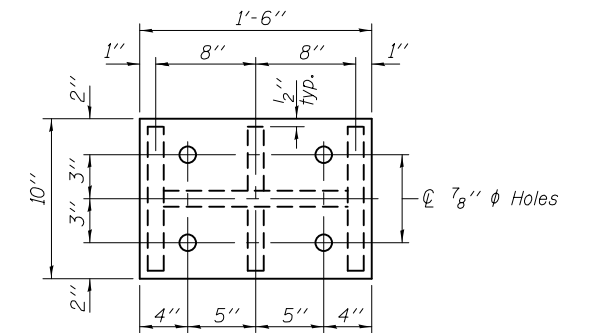
TYPE I ELASTOMERIC EXP. BRG.



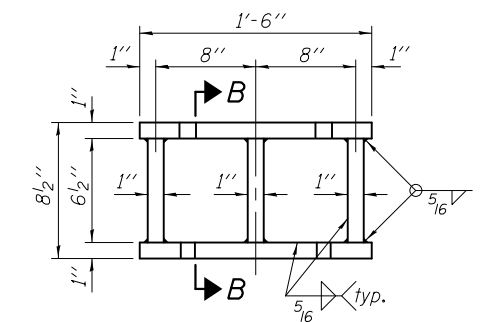
SECTION A-A



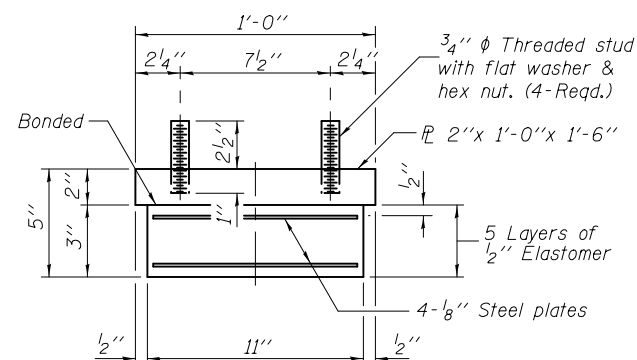
SECTION B-B



PLAN - TOP EXTENSION PLATE

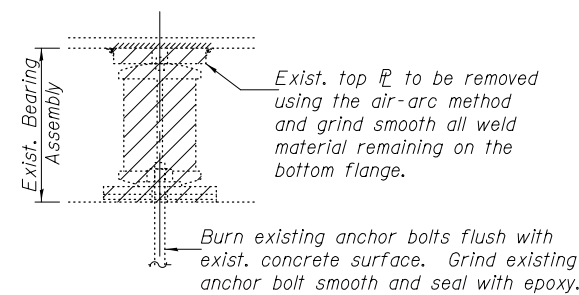


STEEL EXTENSION DETAIL



BEARING ASSEMBLY

Note: Shim plates shall not be placed under Bearing Assembly.



EXISTING BEARING REMOVAL DETAILS

AT PIER 7

Cost included with Jack & Remove Existing Bearings.

Indicate bearing removal

JACK & REMOVE EXISTING BEARING PROCEDURE

- The Contractor shall submit for approval by the Engineer, plans for jacking existing beams and installing new bearings prior to commencing any related work. The dead road reaction per beam (weight of steel only) is 8.4 K at Pier 7. Minimum jack capacity is 13.0 K at Pier 7.
- Prior to ordering any material, the Contractor shall verify steel extension and fill plate thickness required at each bearing.
- Jacking and removing existing bearings shall be done after the existing deck is removed and prior to placing the new deck.
- Jacking lifts shall be limited in accordance with the special provision "Jack and Remove Existing Bearings".
- The new bearings, plates, and steel extensions shall be in place and the jack shall be lowered before the new concrete deck is poured.

Notes:

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts for side retainers for Type I bearings are to be installed in holes drilled before or after members are in place.

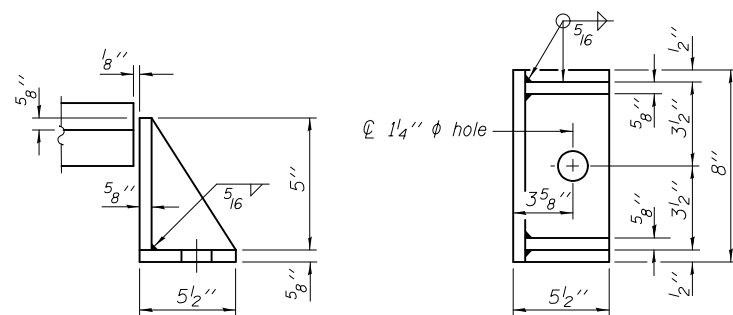
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.

The structural steel plates of the bearing assembly including steel extension shall conform to the requirements of AASHTO M 270 Grade 50.

Steel extensions shall be included in the contract unit lump sum cost for Furnishing and Erecting Structural Steel.

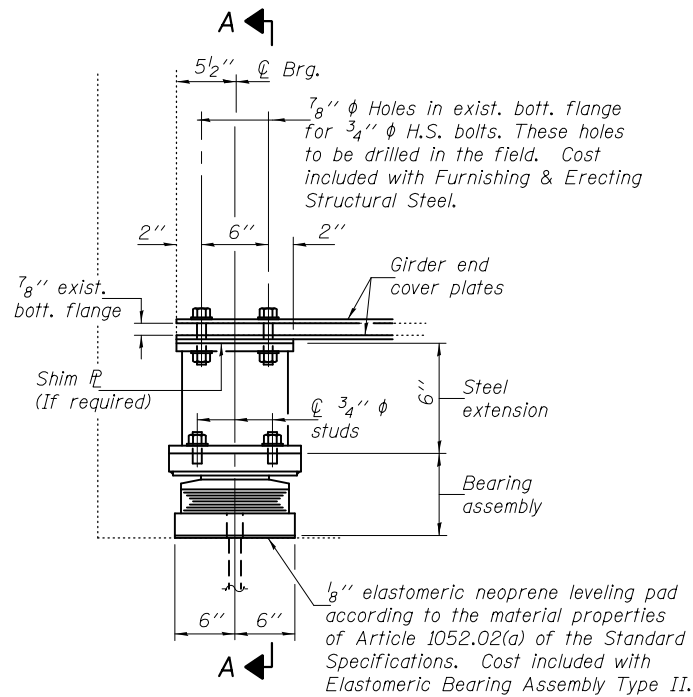
Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.



SIDE RETAINER

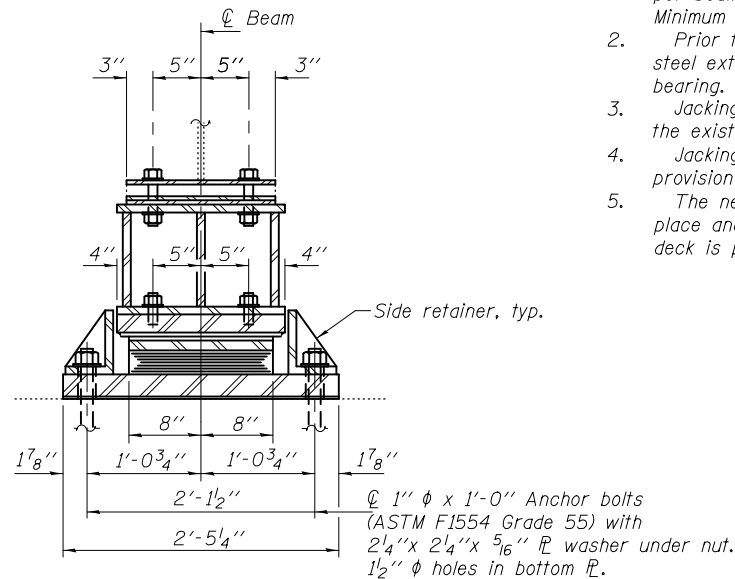
Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

DESIGNED - A. R. Shebli	EXAMINED - <i>Joanne F. DeWitt</i> ENGINEER OF BRIDGE DESIGN	DATE - MAY 6, 2015	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BEARING DETAILS - UNIT 3 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)	F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 92	
CHECKED - N. R. Barnett	PASSED - <i>Carl Pung</i> ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED			CONTRACT NO. 74351					
DRAWN - h.t. duong		REVISED			ILLINOIS FED. AID PROJECT					
CHECKED - NRB/JOV/GRA					SHEET NO. 55 OF 68 SHEETS					



ELEVATION AT NORTH ABUT.

TYPE II ELASTOMERIC EXP. BRG.



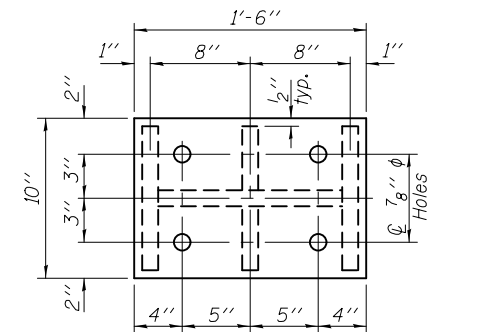
SECTION A-A

JACK & REMOVE EXISTING BEARING PROCEDURE

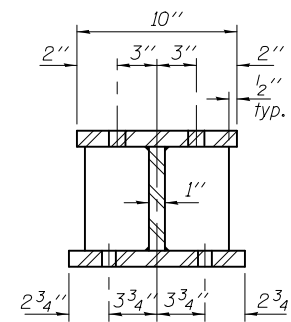
1. The Contractor shall submit for approval by the Engineer, plans for jacking existing beams and installing new bearings prior to commencing any related work. The dead road reaction per beam (weight of steel only) is 8.4 K at North Abutment. Minimum jack capacity is 13.0 K at North Abutment.
2. Prior to ordering any material, the Contractor shall verify steel extension and fill plate thickness required at each bearing.
3. Jacking and removing existing bearings shall be done after the existing deck is removed and prior to placing the new deck.
4. Jacking lifts shall be limited in accordance with the special provision "Jack and Remove Existing Bearings".
5. The new bearings, plates, and steel extensions shall be in place and the jack shall be lowered before the new concrete deck is poured.

BILL OF MATERIAL

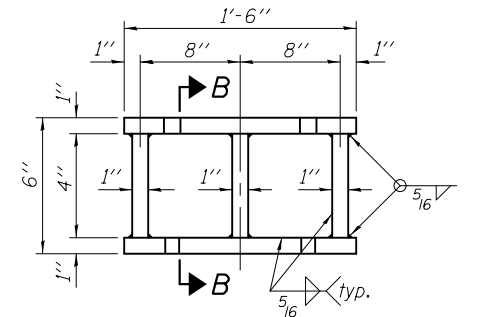
Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	12
Elastomeric Bearing Assembly, Type II	Each	12
Jack and Remove Existing Bearings	Each	24
Anchor Bolts 1"	Each	48



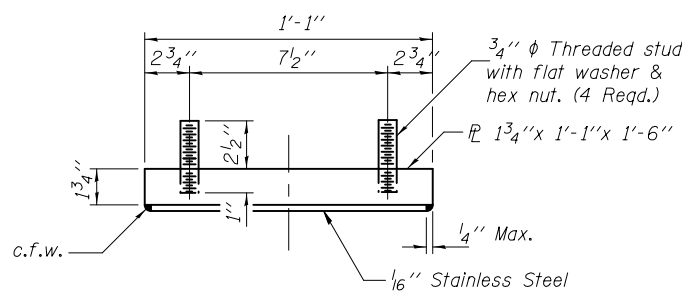
PLAN - TOP EXTENSION PLATE



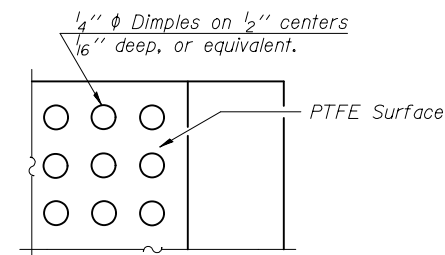
SECTION B-B



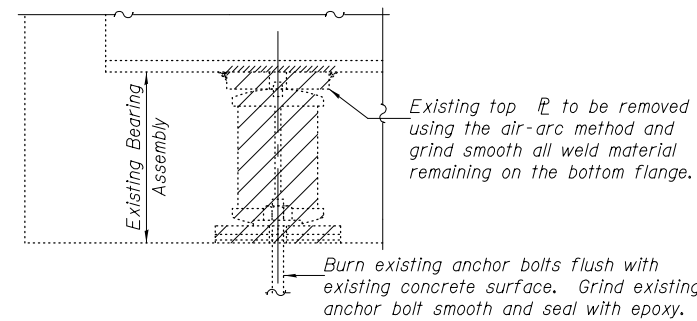
STEEL EXTENSION DETAIL



TOP BEARING ASSEMBLY



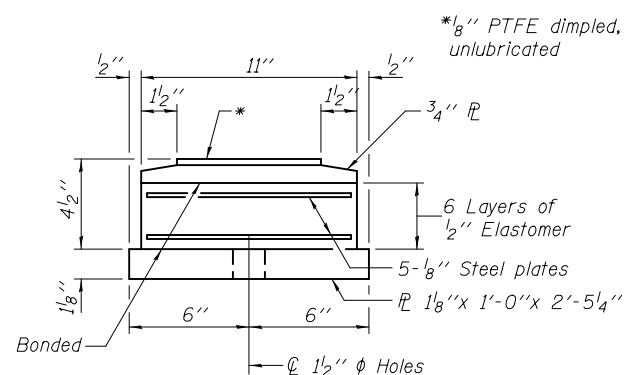
PLAN-PTFE SURFACE



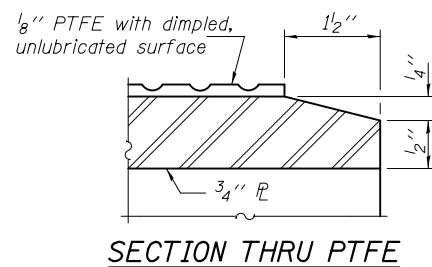
EXISTING BEARING REMOVAL DETAILS AT NORTH ABUTMENT

Cost included with Jack & Remove Existing Bearings.

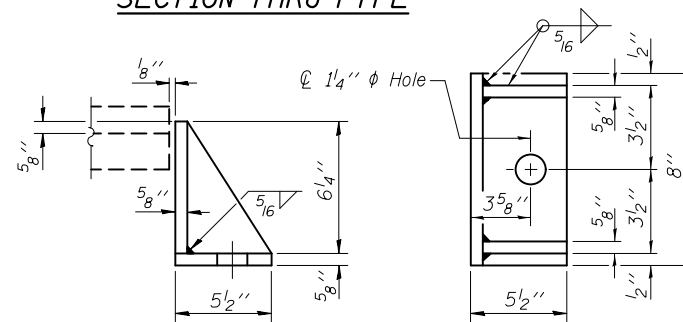
Indicate bearing removal



BOTTOM BEARING ASSEMBLY

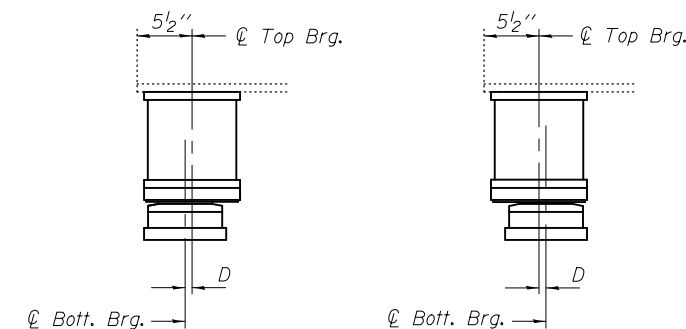


SECTION THRU PTFE



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



BELOW 50° F. (Move bott. brg. away from fixed brg.) ABOVE 50° F. (Move bott. brg. toward fixed brg.)

SETTING ANCHOR BOLTS AT EXP. BRG.

D = 1/8" per each 100' of expansion for every 15° temp. change from the normal temp. of 50°F.

Notes: Anchor bolts shall be ASTM F1554 all-thread (or an Engineer approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts for Type II bearings shall be placed in holes drilled in the concrete through holes in the bottom bearing plate after members are in place. Side retainers shall be placed after bolts are installed.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type II.

Steel extensions shall be included in the contract unit lump sum cost for Furnishing and Erecting Structural Steel.

The 1/8" PTFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces.

Bonding of 1/8" PTFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.

Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

DESIGNED - A. R. Shebli
 CHECKED - N. R. Barnett
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED
 JOYNE F. SHELLY
 ENGINEER OF BRIDGE DESIGN
 CARL L. BRYAN
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

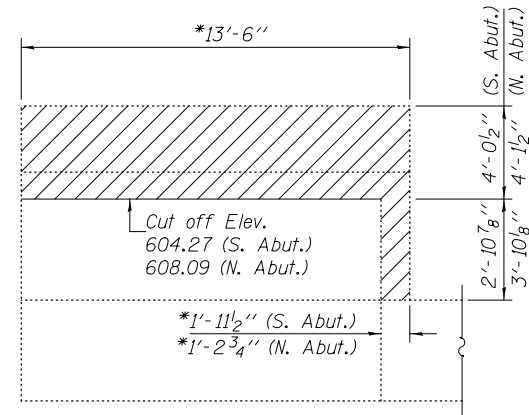
BEARING DETAILS - UNIT 3
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 56 OF 68 SHEETS

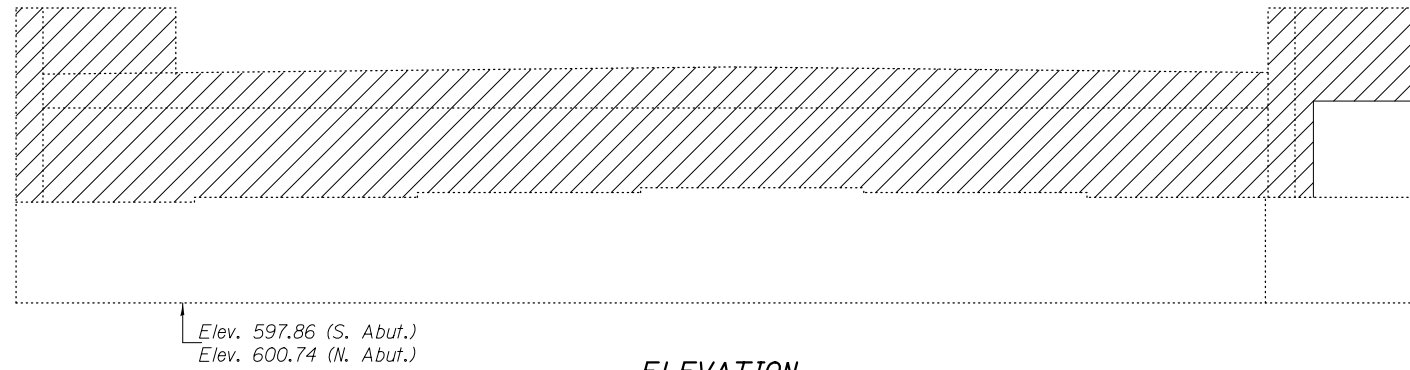
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	93
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

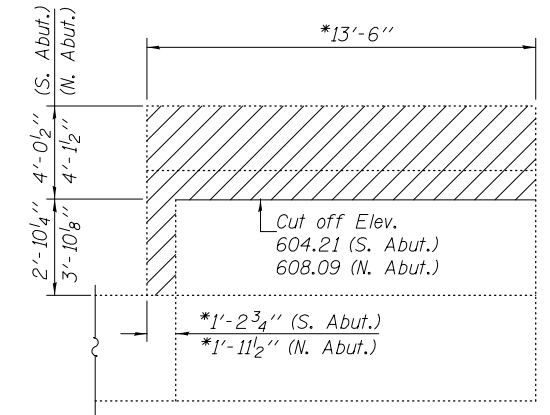
*Measured along outside face of wall.



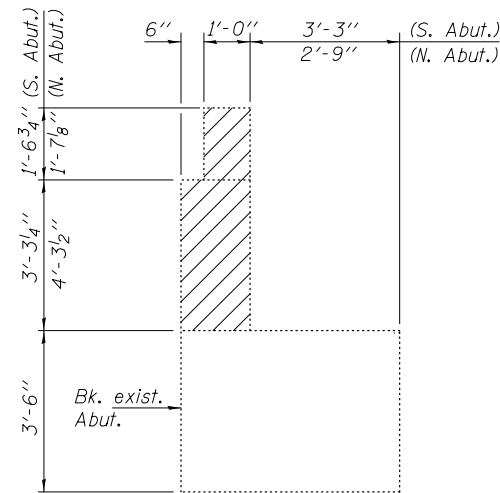
WEST WINGWALL - ELEVATION



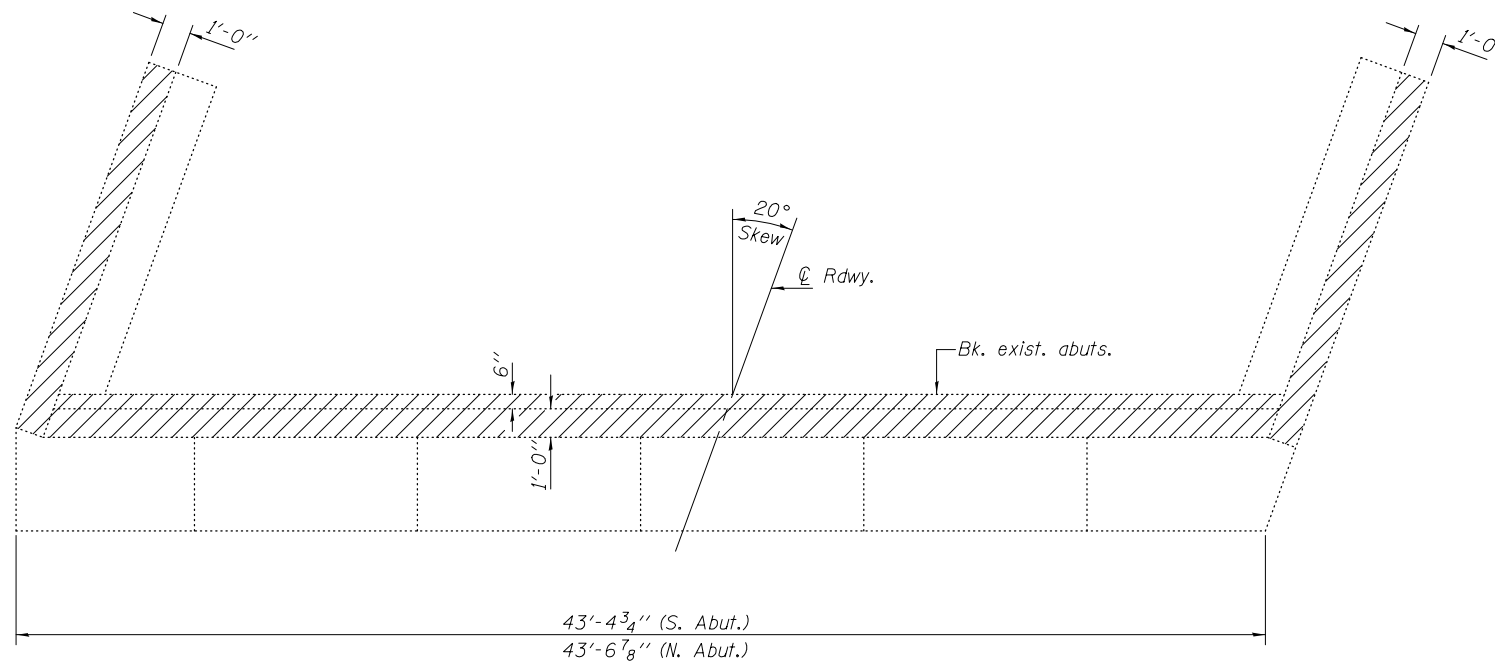
ELEVATION
(North Abut. shown - Looking North;
South Abut. - Looking South)



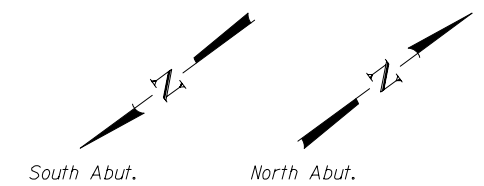
EAST WINGWALL - ELEVATION



SECTION THRU ABUT.



PLAN



Notes: Existing horizontal reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal.
Existing reinforcement not extending into new construction shall be cut off and covered with a 2" layer of cement grout. Cost shall be included with the cost of Concrete Removal.
Hatched areas indicate the limits of concrete removal.

**TWO ABUTMENTS
BILL OF MATERIAL**

Item	Unit	Total
Concrete Removal	Cu. Yd.	30.5

DESIGNED - N. R. Barnett
CHECKED - J. D. Ortiz-Varela
DRAWN - h.t. duong
CHECKED - NRB/JOV/GRA

EXAMINED
PASSED
ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
REVISED
REVISED

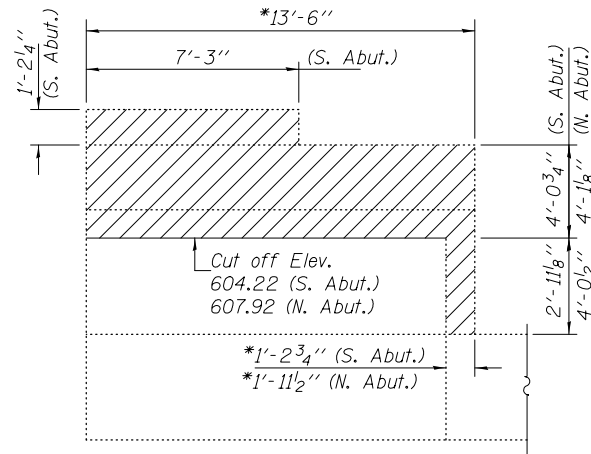
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CONCRETE REMOVAL AT ABUTMENTS (N.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

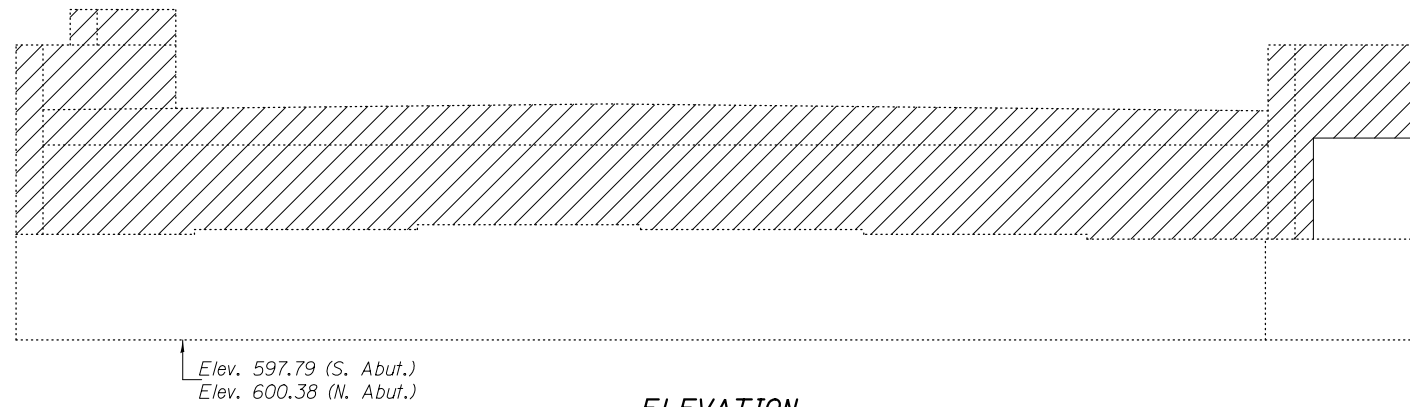
SHEET NO. 57 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	94
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				

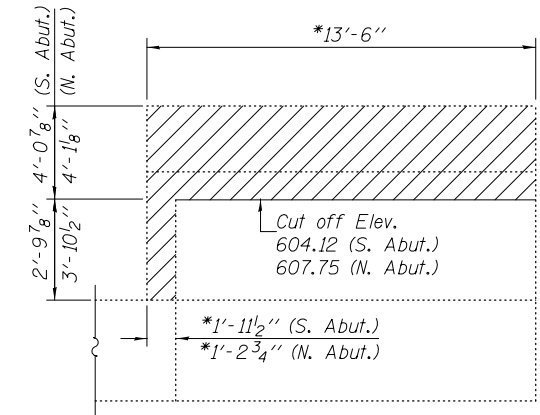
*Measured along outside face of wall.



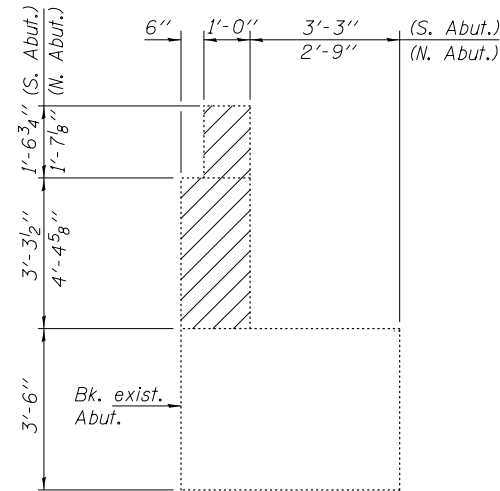
EAST WINGWALL - ELEVATION



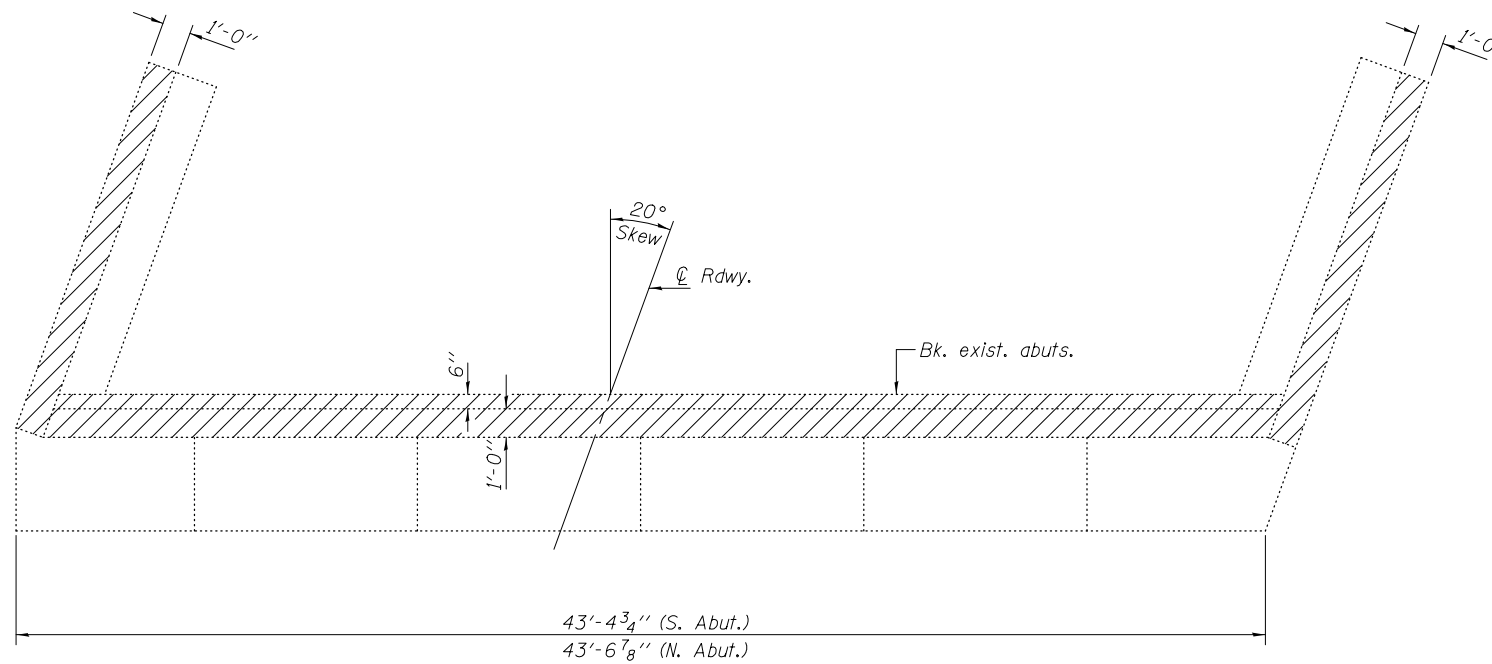
ELEVATION
(South Abut. shown - Looking South;
North Abut. - Looking North)



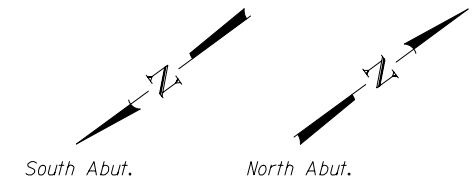
WEST WINGWALL - ELEVATION



SECTION THRU ABUT.



PLAN



Notes: Existing horizontal reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal.
Existing reinforcement not extending into new construction shall be cut off and covered with a 2" layer of cement grout. Cost shall be included with the cost of Concrete Removal.
Hatched areas indicate the limits of concrete removal.

**TWO ABUTMENTS
BILL OF MATERIAL**

Item	Unit	Total
Concrete Removal	Cu. Yd.	31.1

DESIGNED - N. R. Barnett
CHECKED - J. D. Ortiz-Varela
DRAWN - h.t. duong
CHECKED - NRB/JOV/GRA

EXAMINED
PASSED
ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
REVISED
REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

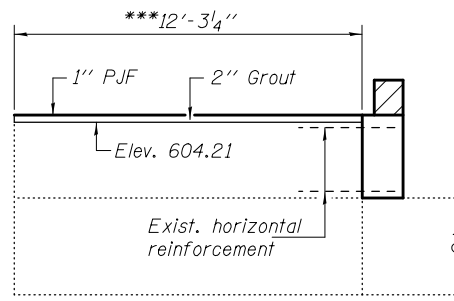
**CONCRETE REMOVAL AT ABUTMENTS (S.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

SHEET NO. 58 OF 68 SHEETS

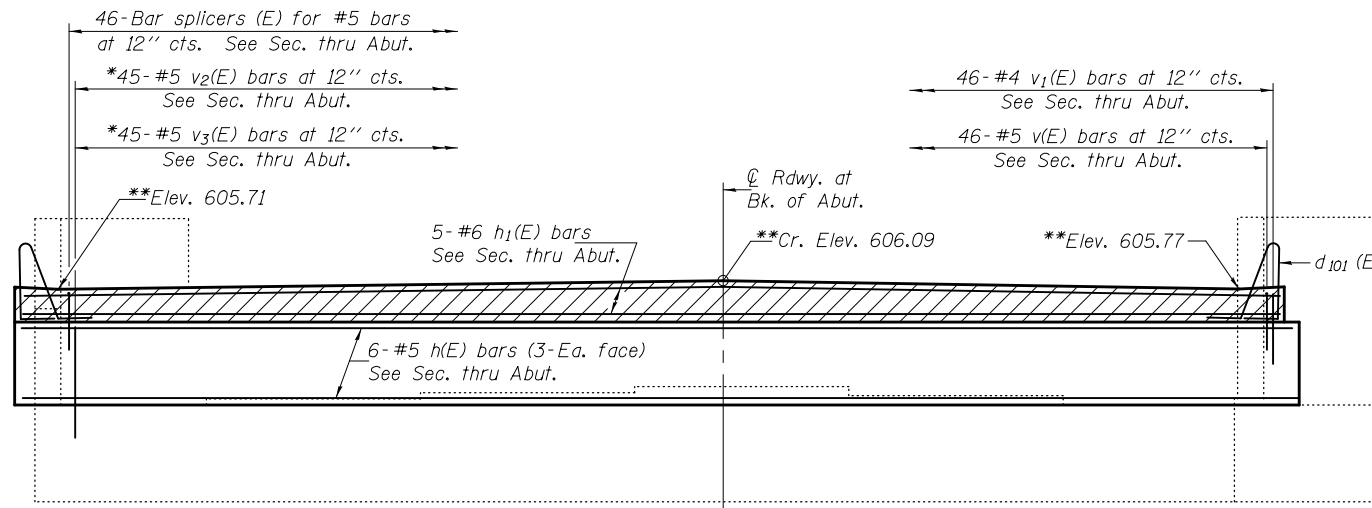
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	95
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

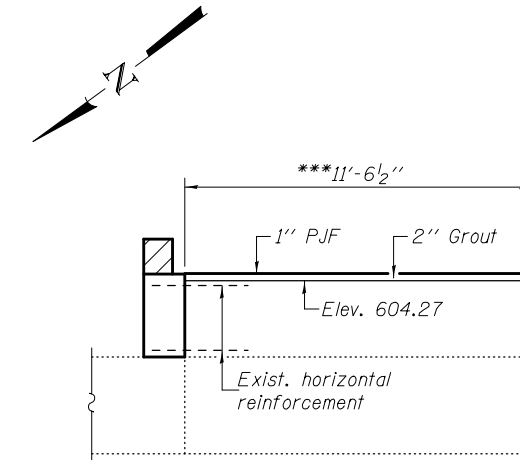
*Epoxy grout $v_2(E)$ and $v_3(E)$ bars in 9" min. drilled holes according to Section 584 of the Standard Specs.
 **Elevations at front face of backwall.
 ***Measured along outside face of wall.



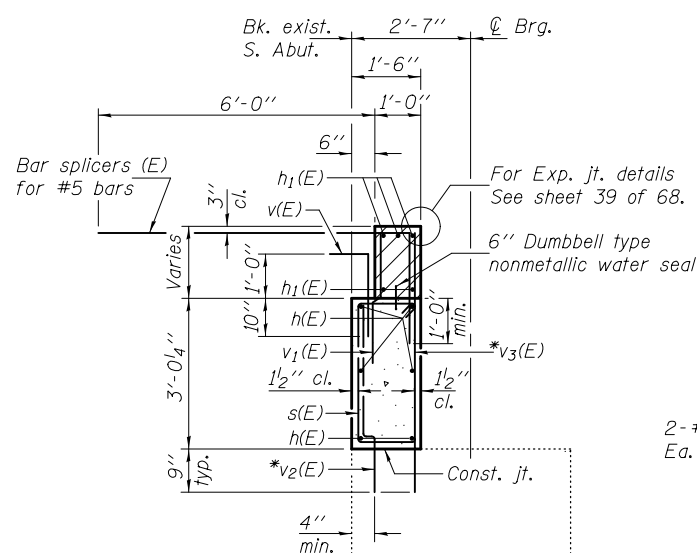
EAST WINGWALL - ELEVATION



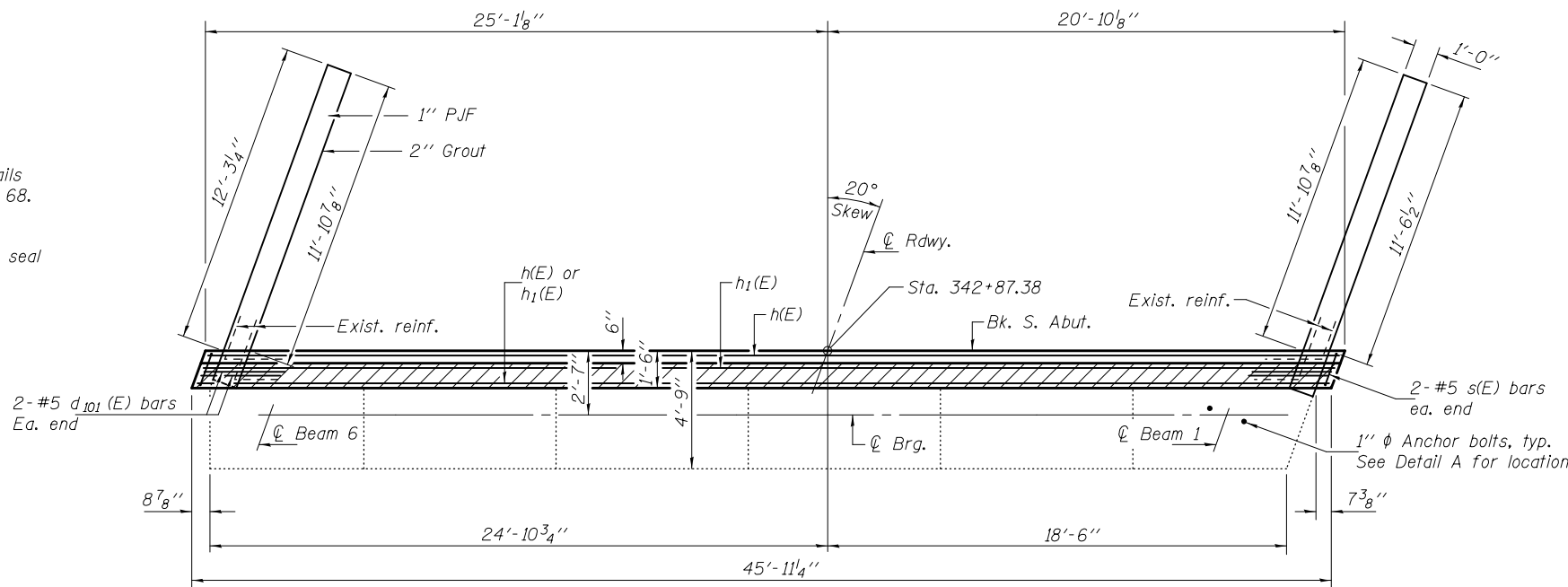
ELEVATION



WEST WINGWALL - ELEVATION



SECTION THRU ABUT.



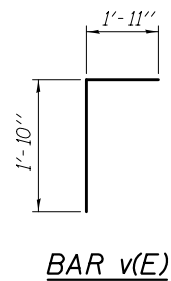
PLAN

**SOUTH ABUTMENT (N.B.)
BILL OF MATERIAL**

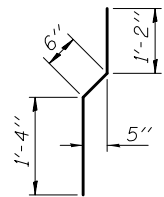
Bar	No.	Size	Length	Shape
$h(E)$	6	#5	45'-7"	—
$h_1(E)$	5	#6	45'-7"	—
$d_{101}(E)$	4	#5	7'-11"	⌒
$s(E)$	4	#5	8'-11"	⊞
$v(E)$	46	#5	3'-9"	┌
$v_1(E)$	46	#4	3'-0"	┌
$v_2(E)$	45	#5	3'-11"	┌
$v_3(E)$	45	#5	5'-0"	┌
Structure Excavation		Cu. Yd.	10.0	
Concrete Structures		Cu. Yd.	7.7	
Reinforcement Bars, Epoxy Coated		Pound	1390	
Concrete Sealer		Sq. Ft.	210.0	

For details of bar splicers, see sheet 67 of 68.
 For $d_{101}(E)$ bar bending diagram, see sheet 38 of 68.

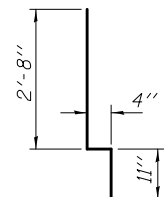
Notes: Hatched area to be poured after superstructure false work has been removed. Quantity of concrete included with Concrete Superstructure.
 Existing reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal.



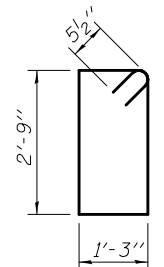
BAR $v(E)$



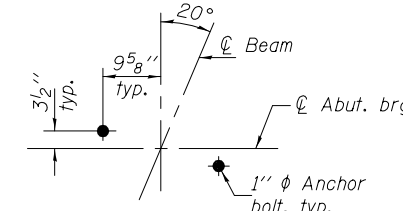
BAR $v_1(E)$



BAR $v_2(E)$



BAR $s(E)$



DETAIL A

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED - *Joanne F. [Signature]*
 PASSED - *Carl [Signature]*
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

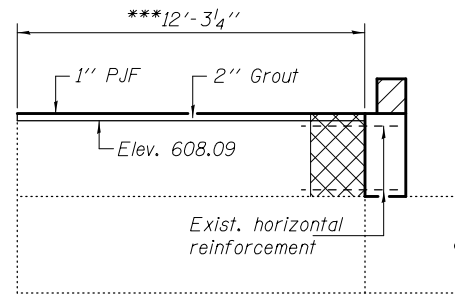
**SOUTH ABUTMENT (N.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

SHEET NO. 59 OF 68 SHEETS

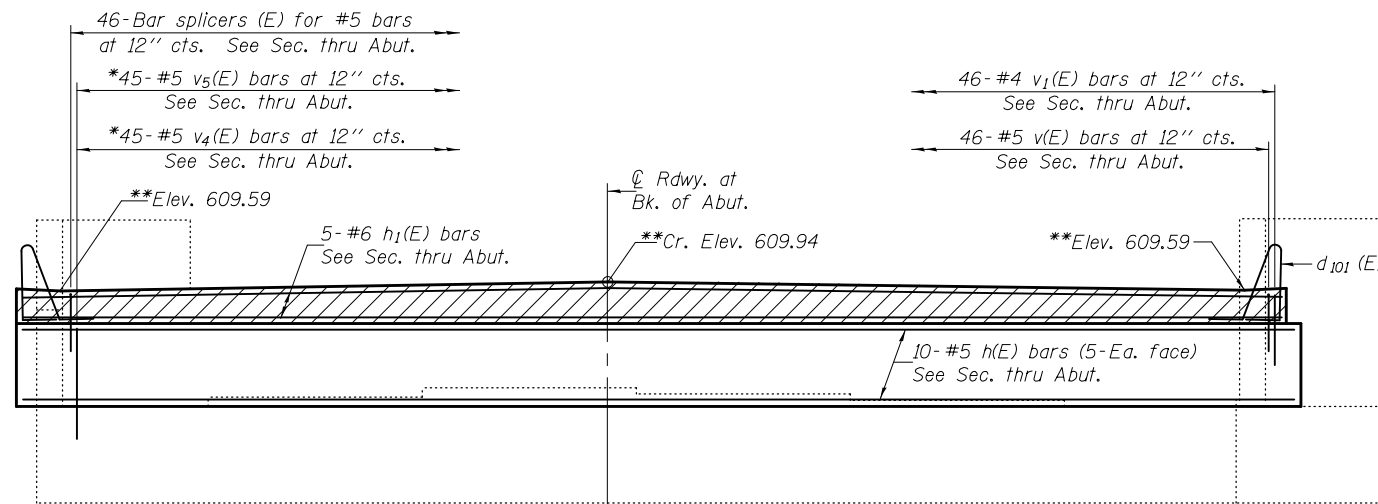
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	96
				CONTRACT NO. 74351

ILLINOIS FED. AID PROJECT

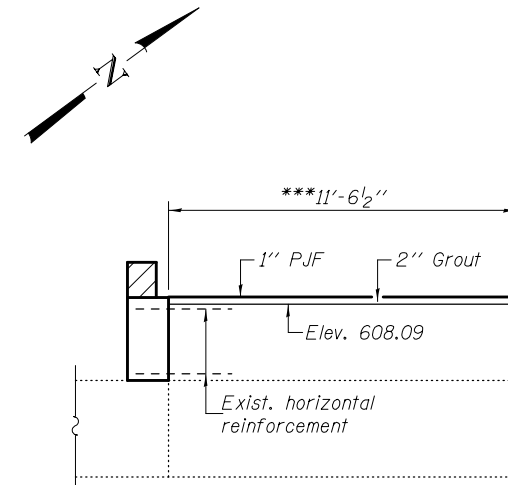
*Epoxy grout $v_4(E)$ and $v_5(E)$ bars in 9" min. drilled holes according to Section 584 of the Standard Specs.
 **Elevations at front face of backwall.
 ***Measured along outside face of wall.



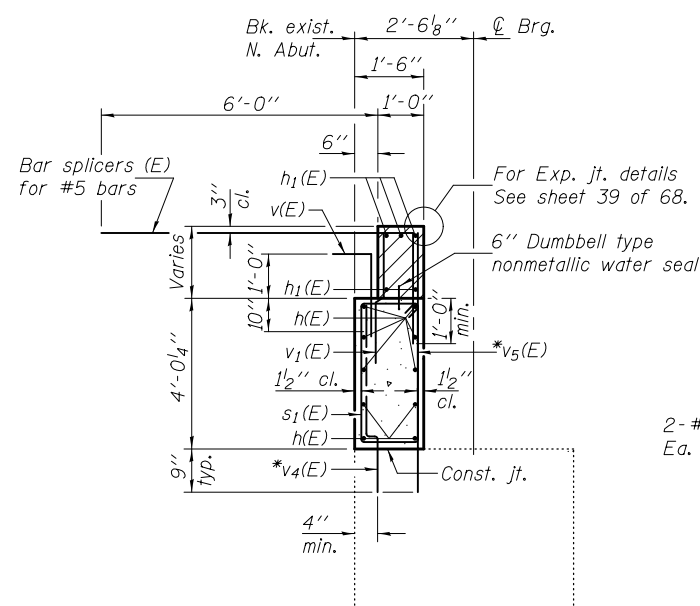
WEST WINGWALL - ELEVATION



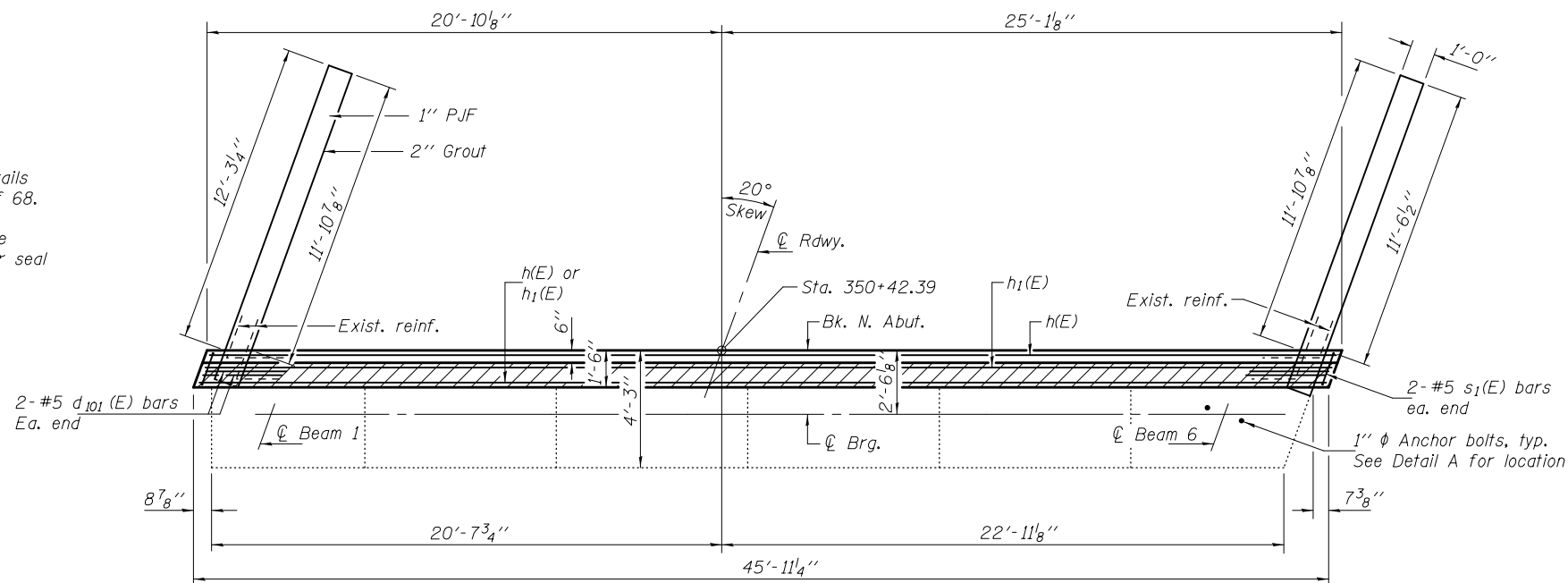
ELEVATION



EAST WINGWALL - ELEVATION



SECTION THRU ABUT.



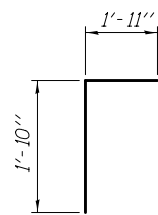
PLAN

**NORTH ABUTMENT (N.B.)
BILL OF MATERIAL**

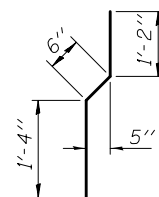
Bar	No.	Size	Length	Shape
$h(E)$	10	#5	45'-7"	—
$h_1(E)$	5	#6	45'-7"	—
$d_{101}(E)$	4	#5	7'-11"	⏏
$s_1(E)$	4	#5	10'-9"	⏏
$v(E)$	46	#5	3'-9"	┌
$v_1(E)$	46	#4	3'-0"	┌
$v_4(E)$	45	#5	4'-11"	┌
$v_5(E)$	45	#5	5'-11"	┌
Structure Excavation		Cu. Yd.	14.0	
Concrete Structures		Cu. Yd.	10.3	
Reinforcement Bars, Epoxy Coated		Pound	1680	
Concrete Sealer		Sq. Ft.	256.0	

For details of bar splicers, see sheet 67 of 68.
 For $d_{101}(E)$ bar bending diagram, see sheet 38 of 68.

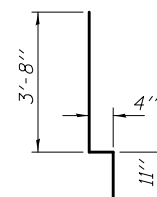
Notes: Hatched area to be poured after superstructure false work has been removed. Quantity of concrete included with Concrete Superstructure.
 Cross-hatched area indicates location of structural repair of concrete. See sheet 66B of 68.
 Existing reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal.



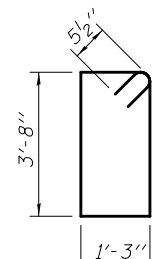
BAR $v(E)$



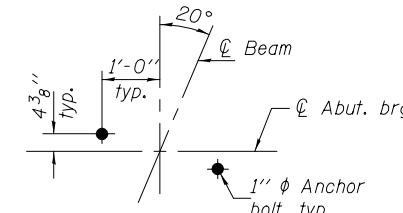
BAR $v_1(E)$



BAR $v_4(E)$



BAR $s_1(E)$



DETAIL A

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

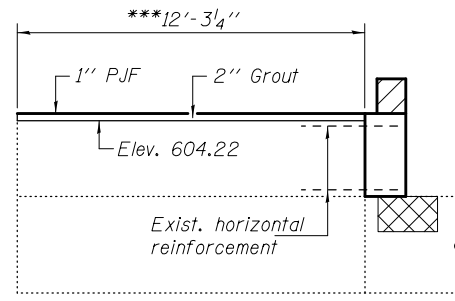
NORTH ABUTMENT (N.B.)
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 60 OF 68 SHEETS

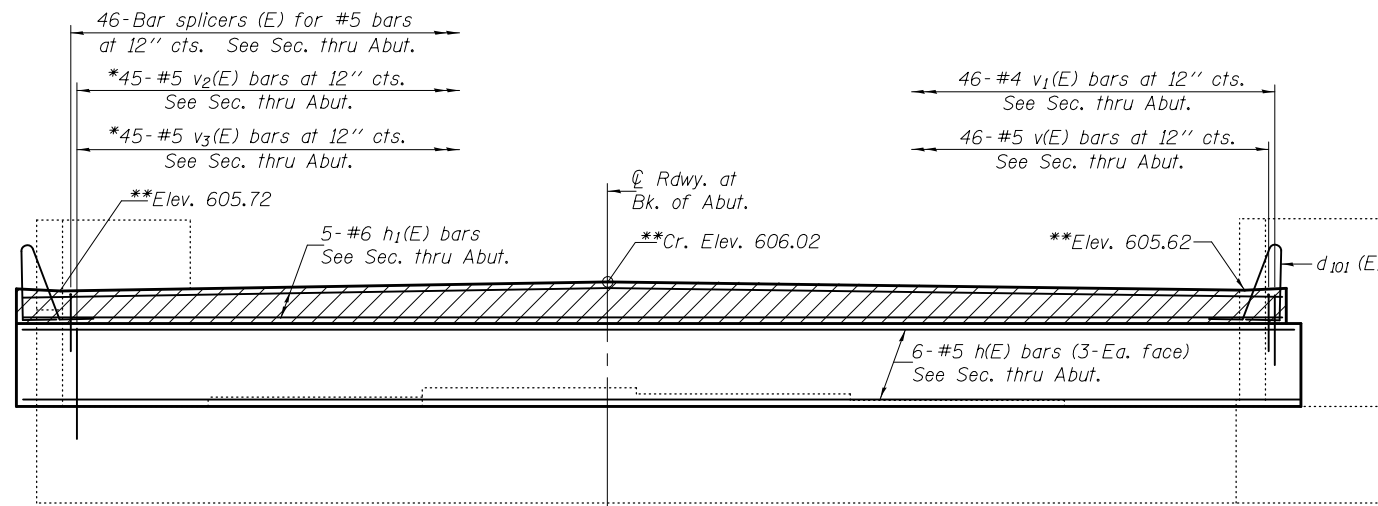
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	97
				CONTRACT NO. 74351

ILLINOIS FED. AID PROJECT

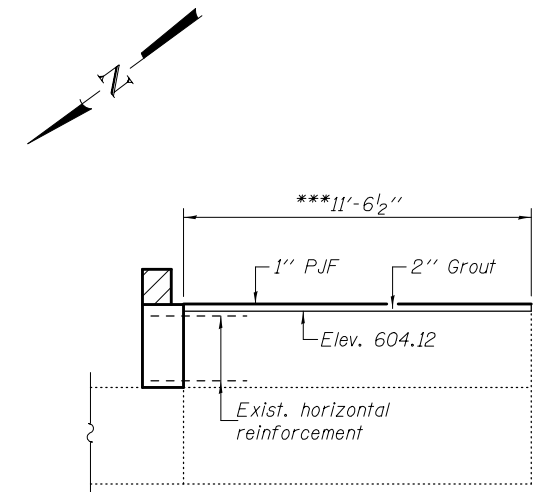
*Epoxy grout $v_2(E)$ and $v_3(E)$ bars in 9" min. drilled holes according to Section 584 of the Standard Specs.
 **Elevations at front face of backwall.
 ***Measured along outside face of wall.



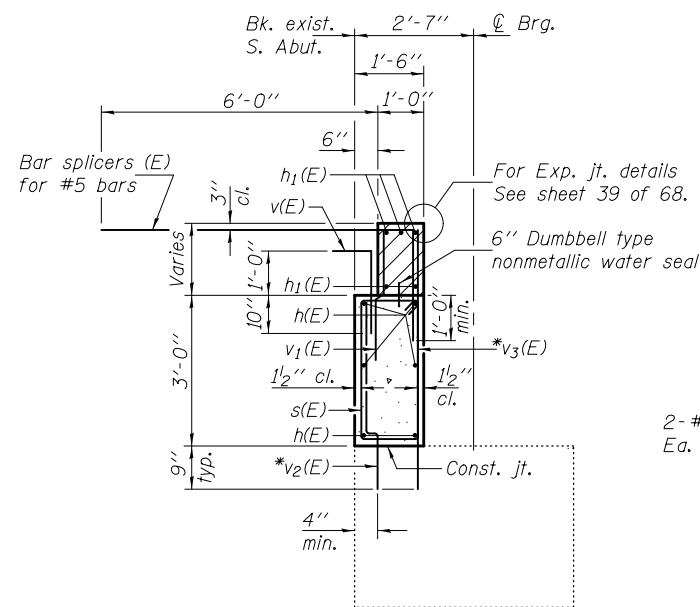
EAST WINGWALL - ELEVATION



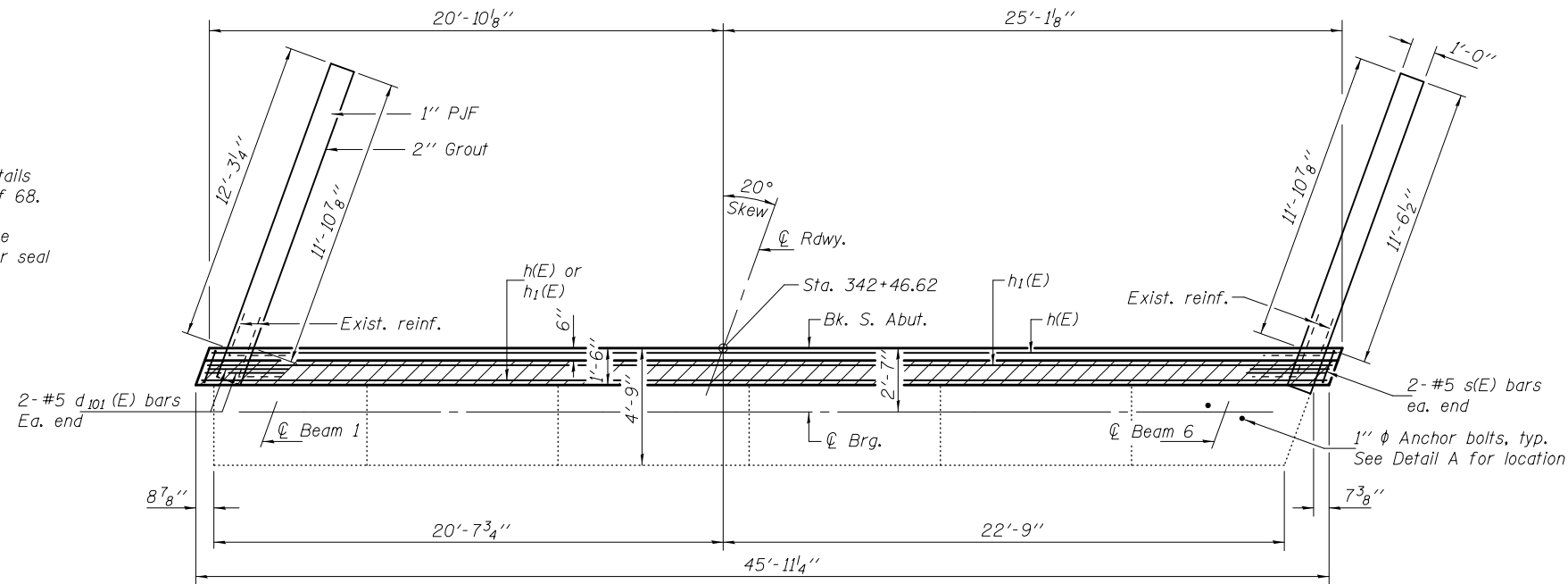
ELEVATION



WEST WINGWALL - ELEVATION



SECTION THRU ABUT.



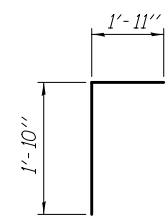
PLAN

**SOUTH ABUTMENT (S.B.)
BILL OF MATERIAL**

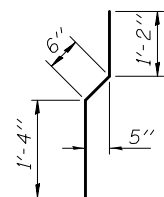
Bar	No.	Size	Length	Shape
$h(E)$	6	#5	45'-7"	—
$h_1(E)$	5	#6	45'-7"	—
$d_{101}(E)$	4	#5	7'-11"	⤴
$s(E)$	4	#5	8'-11"	⊠
$v(E)$	46	#5	3'-9"	┌
$v_1(E)$	46	#4	3'-0"	┌
$v_2(E)$	45	#5	3'-11"	┌
$v_3(E)$	45	#5	5'-0"	┌
Structure Excavation		Cu. Yd.	11.0	
Concrete Structures		Cu. Yd.	7.7	
Reinforcement Bars, Epoxy Coated		Pound	1390	
Concrete Sealer		Sq. Ft.	209.1	

For details of bar splicers, see sheet 67 of 68.
 For $d_{101}(E)$ bar bending diagram, see sheet 38 of 68.

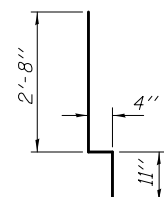
Notes: Hatched area to be poured after superstructure false work has been removed. Quantity of concrete included with Concrete Superstructure.
 Cross-hatched area indicates location of structural repair of concrete. See sheet 66 of 68.
 Existing reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal.



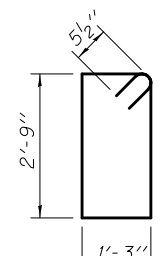
BAR $v(E)$



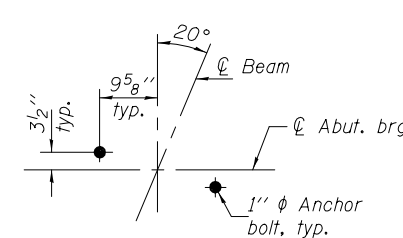
BAR $v_1(E)$



BAR $v_2(E)$



BAR $s(E)$



DETAIL A

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

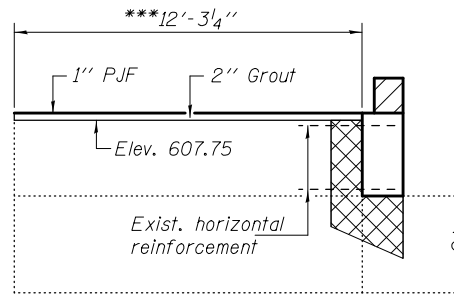
**SOUTH ABUTMENT (S.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

SHEET NO. 61 OF 68 SHEETS

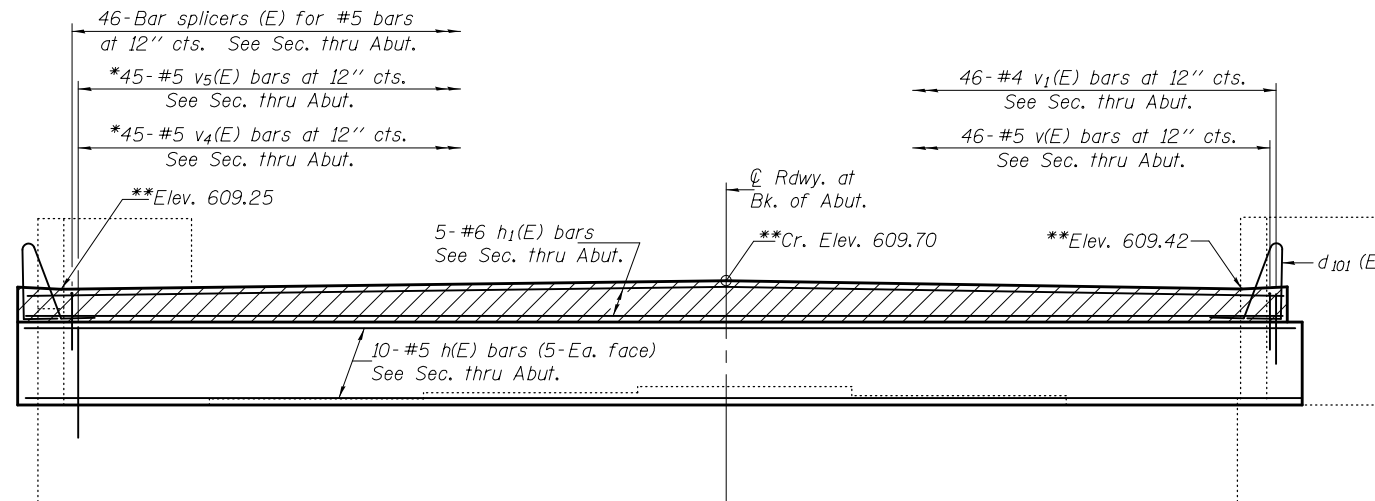
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	98
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

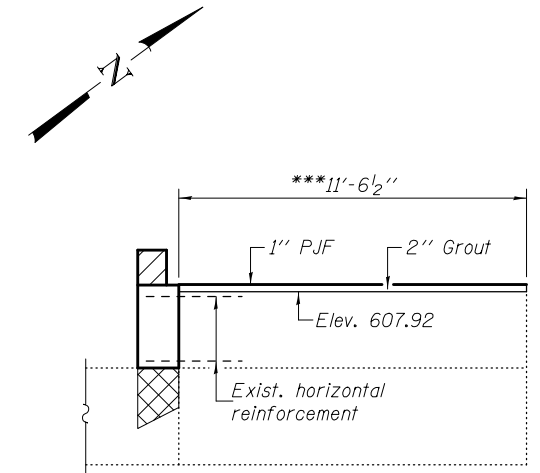
*Epoxy grout $v_4(E)$ and $v_5(E)$ bars in 9" min. drilled holes according to Section 584 of the Standard Specs.
 **Elevations at front face of backwall.
 ***Measured along outside face of wall.



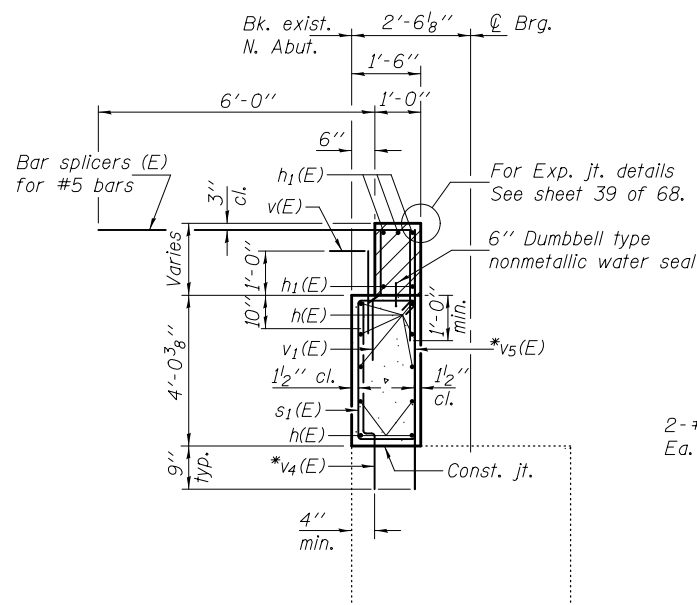
WEST WINGWALL - ELEVATION



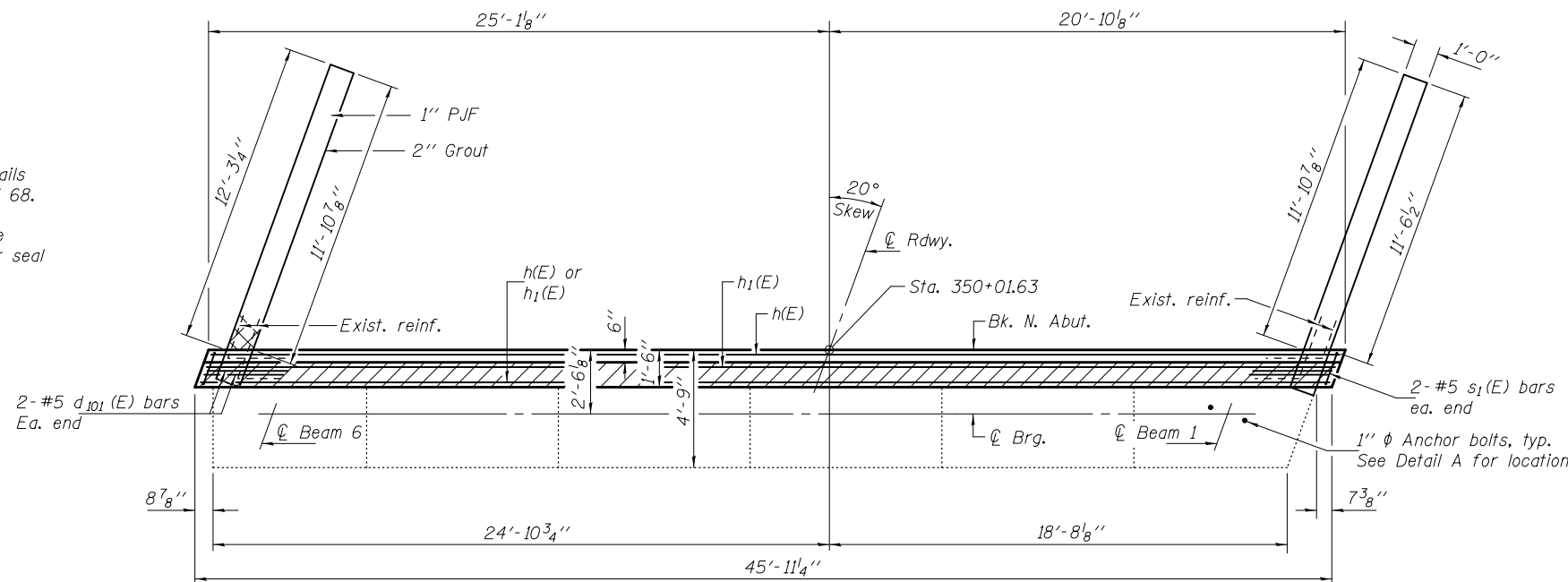
ELEVATION



EAST WINGWALL - ELEVATION



SECTION THRU ABUT.



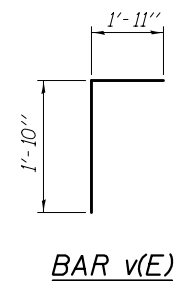
PLAN

**NORTH ABUTMENT (S.B.)
 BILL OF MATERIAL**

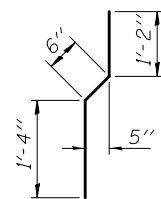
Bar	No.	Size	Length	Shape
$h(E)$	10	#5	45'-7"	—
$h_1(E)$	5	#6	45'-7"	—
$d_{101}(E)$	4	#5	7'-11"	⌋
$s_1(E)$	4	#5	10'-9"	⌋
$v(E)$	46	#5	3'-9"	┌
$v_1(E)$	46	#4	3'-0"	┌
$v_4(E)$	45	#5	4'-11"	┌
$v_5(E)$	45	#5	5'-11"	┌
Structure Excavation		Cu. Yd.	14.0	
Concrete Structures		Cu. Yd.	10.3	
Reinforcement Bars, Epoxy Coated		Pound	1680	
Concrete Sealer		Sq. Ft.	256.5	

For details of bar splicers, see sheet 67 of 68.
 For $d_{101}(E)$ bar bending diagram, see sheet 38 of 68.

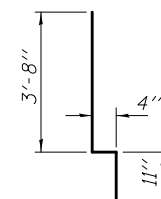
Notes: Hatched area to be poured after superstructure false work has been removed. Quantity of concrete included with Concrete Superstructure.
 Cross-hatched area indicates location of structural repair of concrete. See sheet 66 of 68.
 Existing reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal.



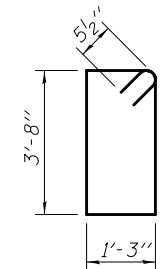
BAR $v(E)$



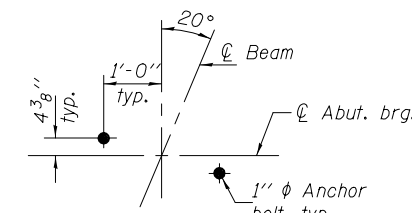
BAR $v_1(E)$



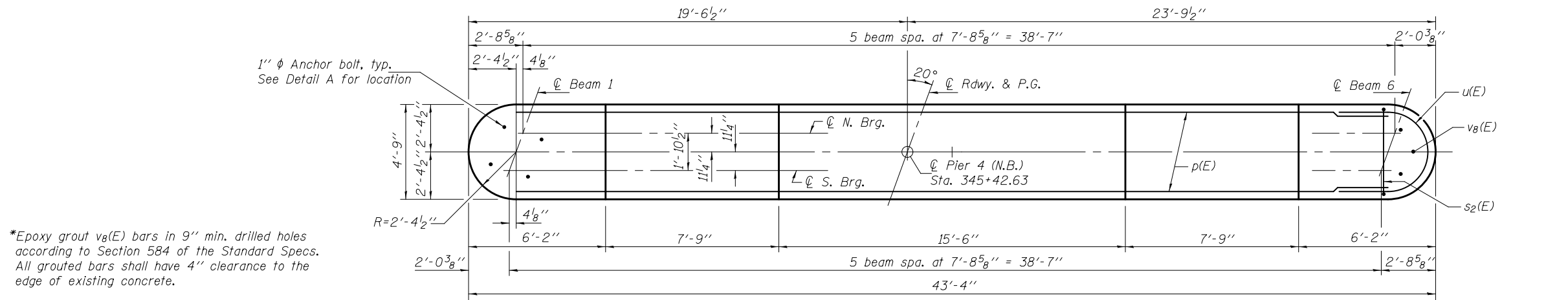
BAR $v_4(E)$



BAR $s_1(E)$

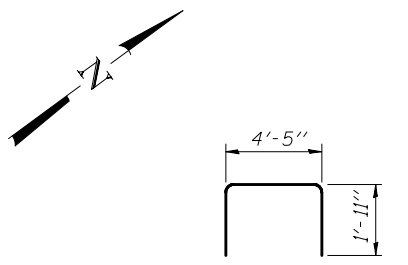


DETAIL A

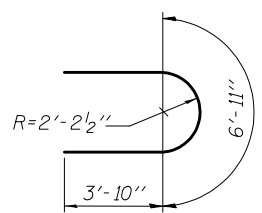


TOP PLAN

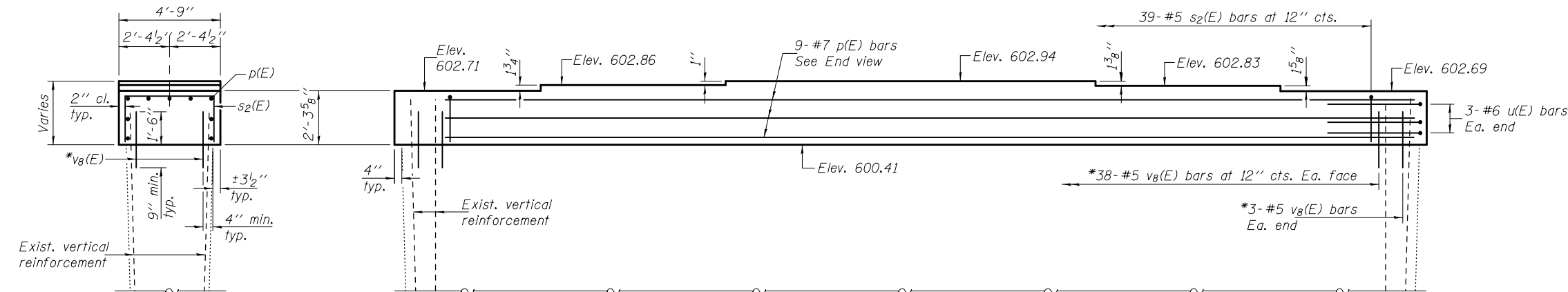
*Epoxy grout v₈(E) bars in 9" min. drilled holes according to Section 584 of the Standard Specs. All grouted bars shall have 4" clearance to the edge of existing concrete.



BAR s₂(E)



BAR u(E)



ELEVATION

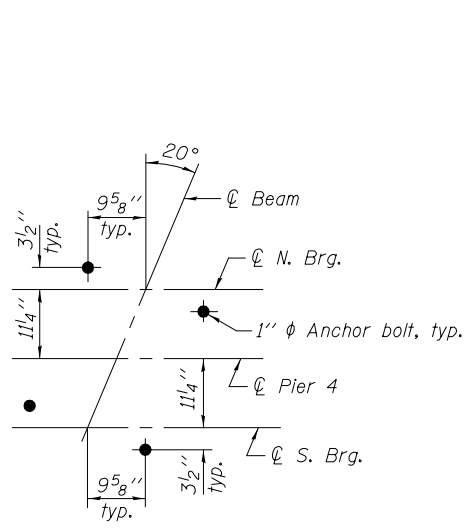
(Looking North)

END VIEW

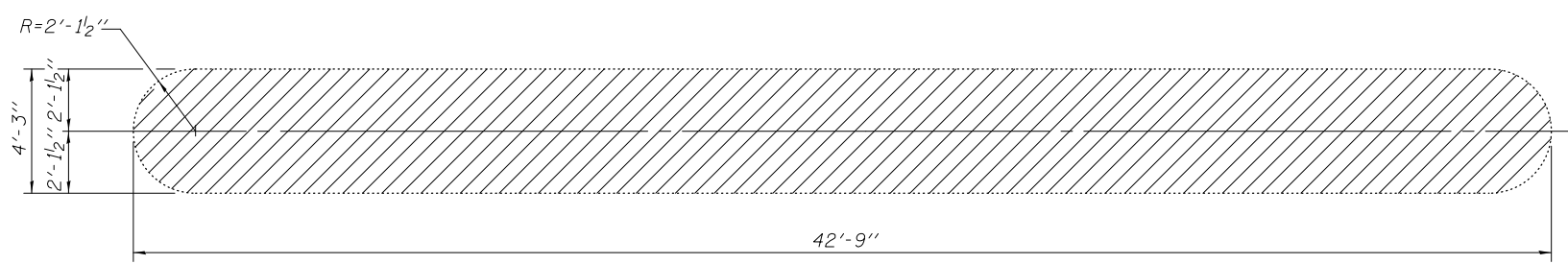
**PIER 4 (N.B.)
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
p(E)	9	#7	38'-7"	—
s ₂ (E)	39	#5	8'-3"	⊏
u(E)	6	#6	14'-7"	⊏
v ₈ (E)	82	#5	2'-3"	—
Concrete Structures			Cu. Yd.	18.1
Concrete Removal			Cu. Yd.	12.5
Reinforcement Bars, Epoxy Coated			Pound	1370

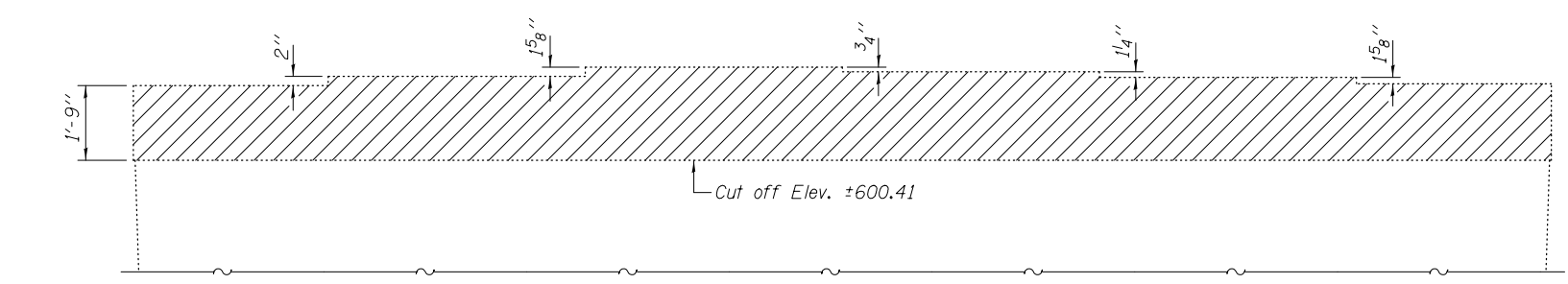
Notes: Space reinforcement in cap to miss anchor bolts. Pour steps monolithically with cap. Existing vertical reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal. Hatched areas indicate the limits of Concrete Removal.



DETAIL A

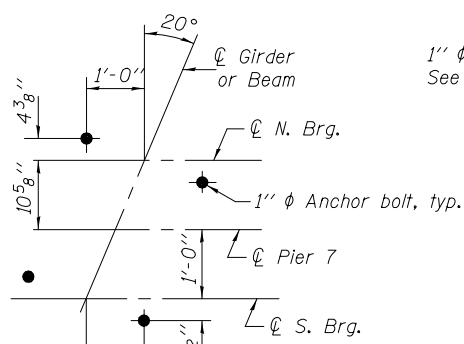


PLAN - CONCRETE REMOVAL

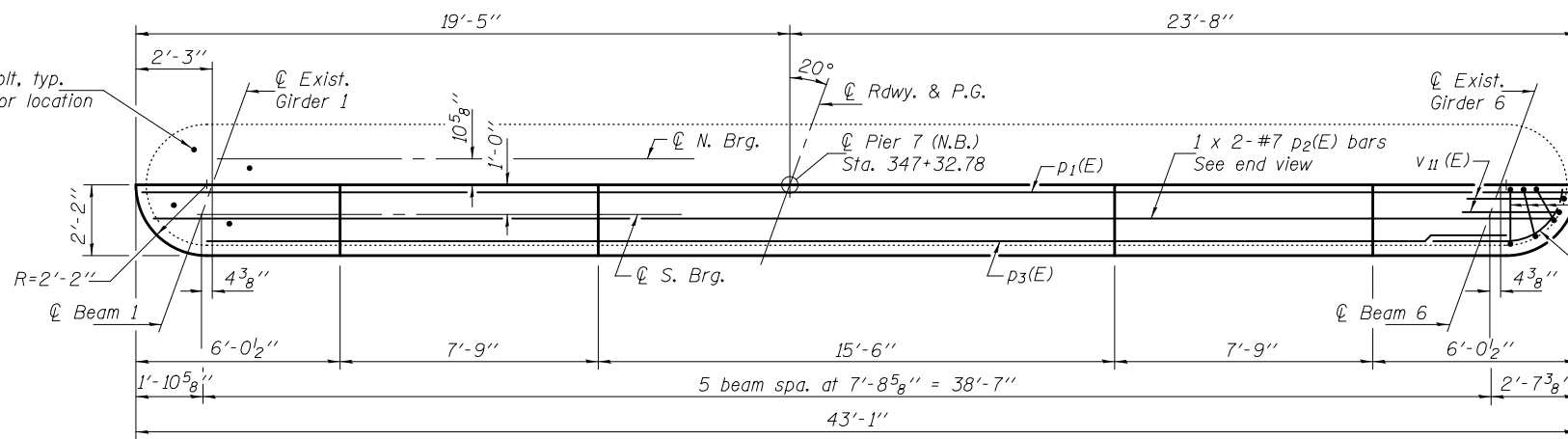


ELEVATION - CONCRETE REMOVAL

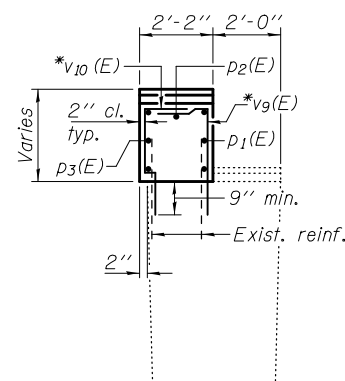
(Looking North)



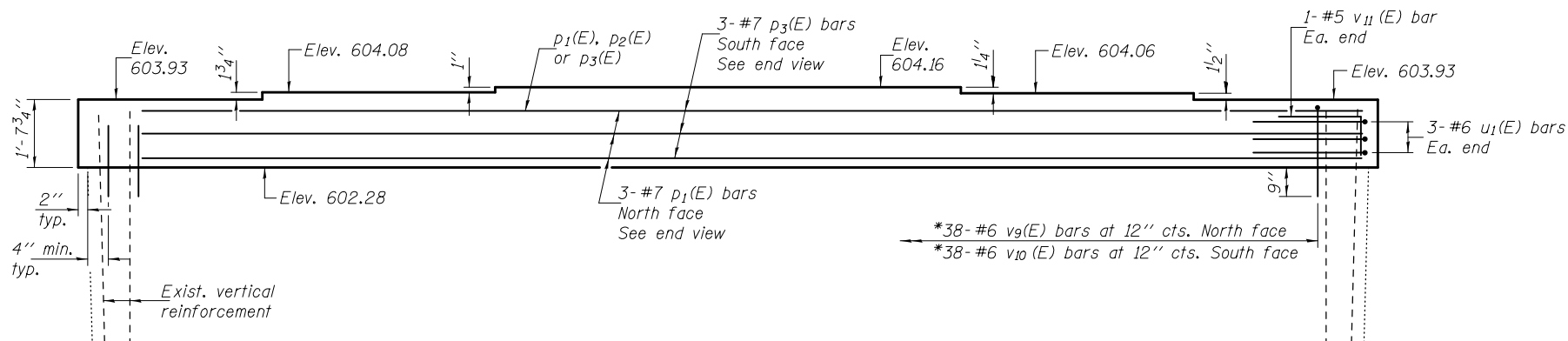
DETAIL A



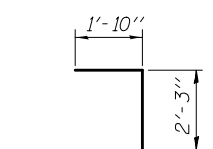
TOP PLAN



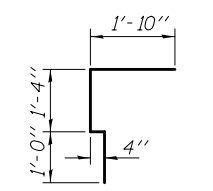
END VIEW



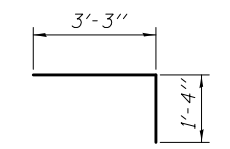
ELEVATION
(Looking North)



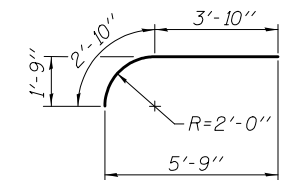
BAR v9(E)



BAR v10(E)



BAR v11(E)

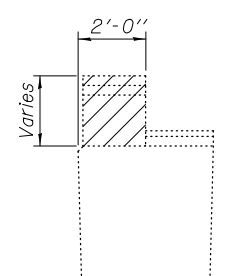


BAR u1(E)

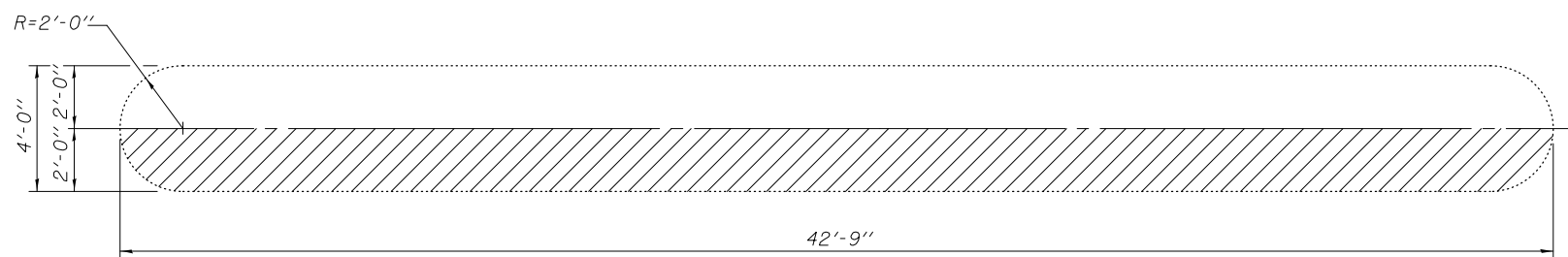
*Epoxy grout v9(E) & v10(E) bars in 9" min. drilled holes according to Section 584 of the Standard Specs. All grouted bars shall have 4" clearance to the edge of existing concrete.
**Cut to fit.

MIN. BAR LAPS

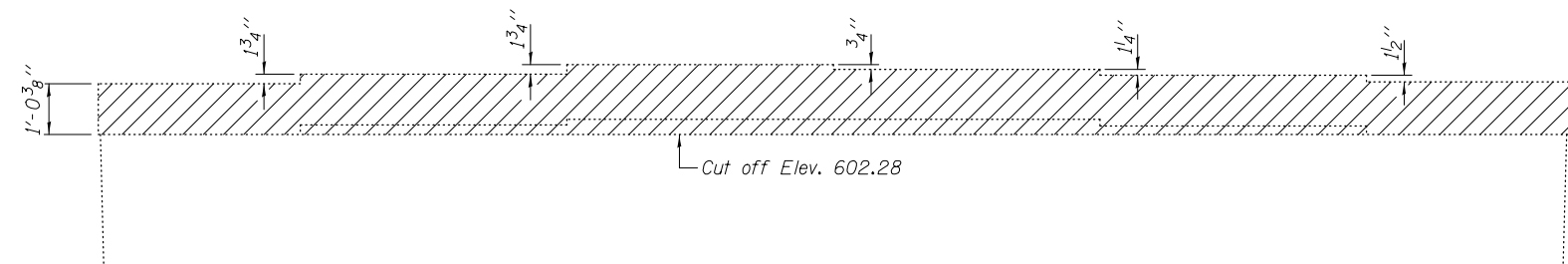
#5 bar = 3'-3"
#7 bar = 5'-10"



END VIEW -
CONCRETE REMOVAL



PLAN - CONCRETE REMOVAL

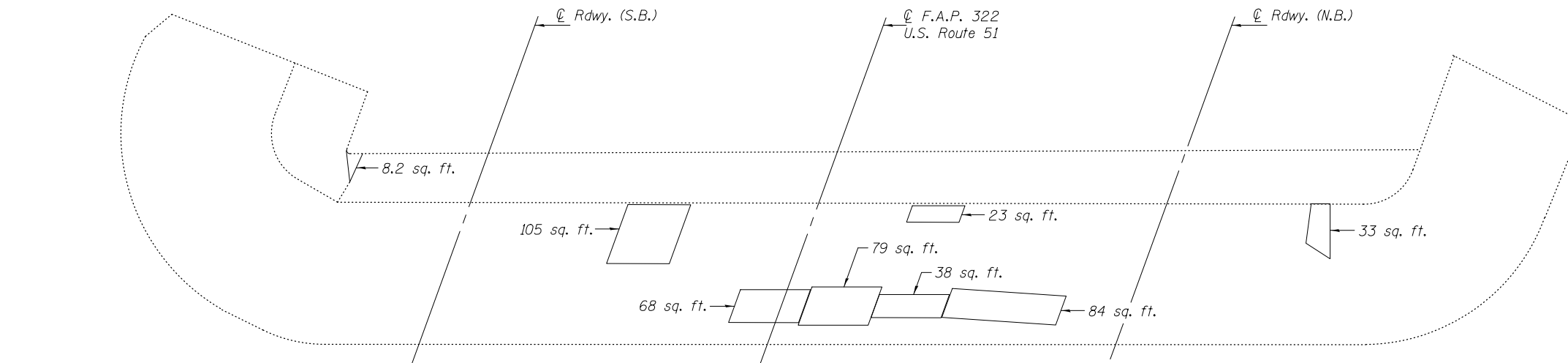


ELEVATION - CONCRETE REMOVAL
(Looking North)

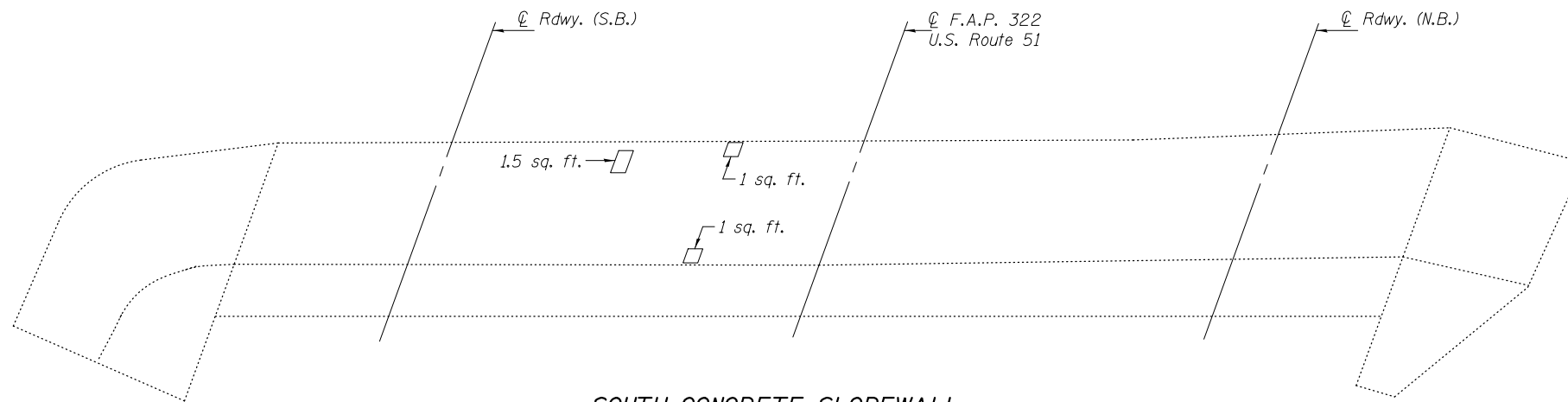
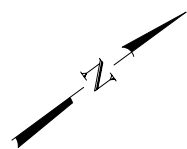
PIER 7 (N.B.)
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
p1(E)	3	#7	42'-9"	—
p2(E)	2	#7	23'-4"	—
p3(E)	3	#7	38'-9"	—
u1(E)	6	#6	6'-8"	⌋
v9(E)	44	#5	4'-1"	⌋
v10(E)	44	#5	4'-6"	⌋
v11(E)	4	#5	4'-7"	⌋
Concrete Structures		Cu. Yd.	6.2	
Concrete Removal		Cu. Yd.	3.7	
Reinforcement Bars, Epoxy Coated		Pound	1070	

Notes: Space reinforcement in cap to miss anchor bolts. Pour steps monolithically with cap. Existing vertical reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal. Hatched areas indicate the limits of Concrete Removal.



NORTH CONCRETE SLOPEWALL



SOUTH CONCRETE SLOPEWALL

TWO SLOPEWALL REPAIRS
BILL OF MATERIAL

Item	Unit	Total
Slopedwall Repair	Sq. Yd.	49.1

Notes:
 Quantities for Slopedwall Repair are based on estimated areas shown in the Bridge Condition Report dated January, 2010.
 Actual locations and areas of Slopedwall Repair are to be determined by the Engineer in the field.

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

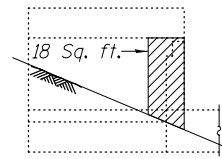
EXAMINED
 PASSED
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

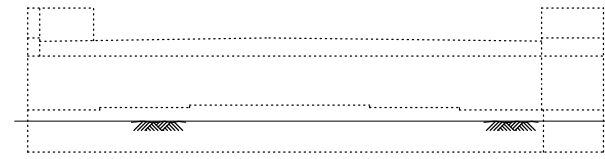
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SLOPEWALL REPAIR
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)
 SHEET NO. 65 OF 68 SHEETS

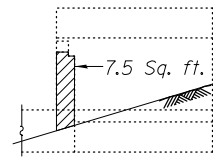
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	102
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				



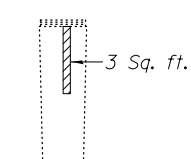
**ELEVATION -
N.W. WINGWALL**



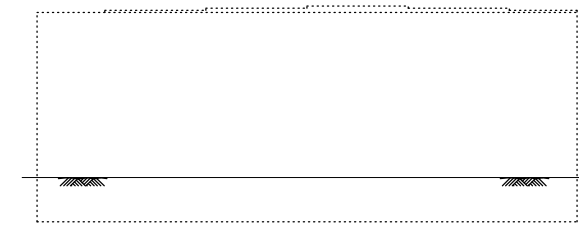
**ELEVATION - NORTH ABUTMENT
(Looking North)**



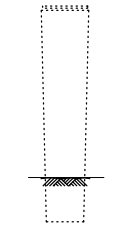
**ELEVATION -
N.E. WINGWALL**



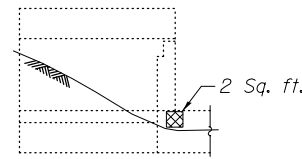
WEST END



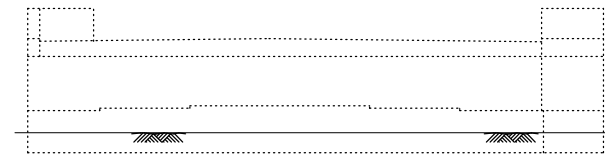
**ELEVATION - PIER 1
(Looking North)**



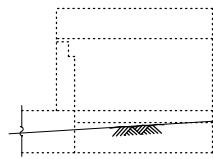
EAST END



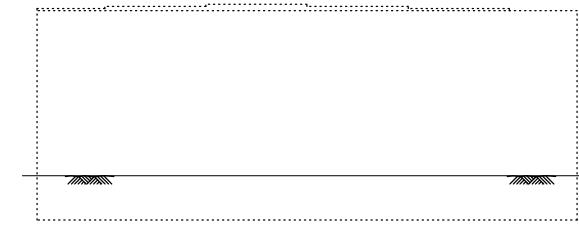
**ELEVATION -
S.E. WINGWALL**



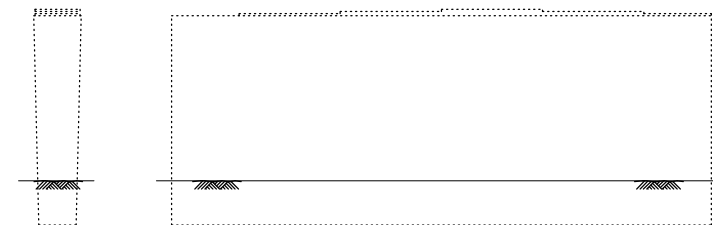
**ELEVATION - SOUTH ABUTMENT
(Looking South)**



**ELEVATION -
S.W. WINGWALL**



**ELEVATION - PIER 1
(Looking South)**

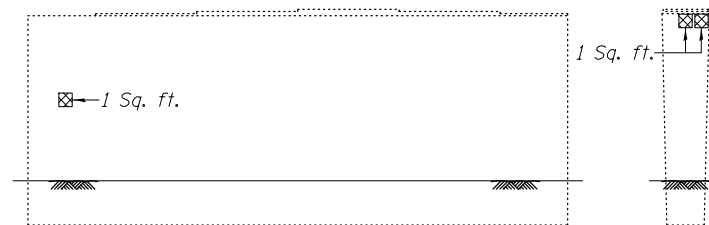


WEST END

**ELEVATION - PIER 2
(Looking North)**

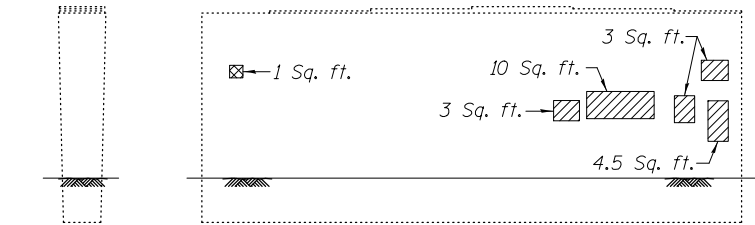
EAST END

WEST END



**ELEVATION - PIER 3
(Looking North)**

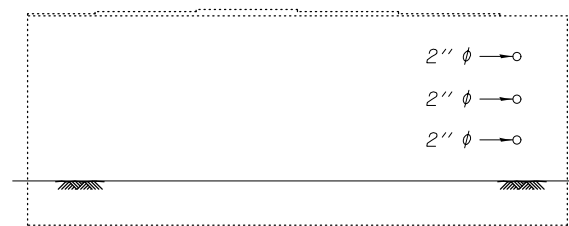
EAST END



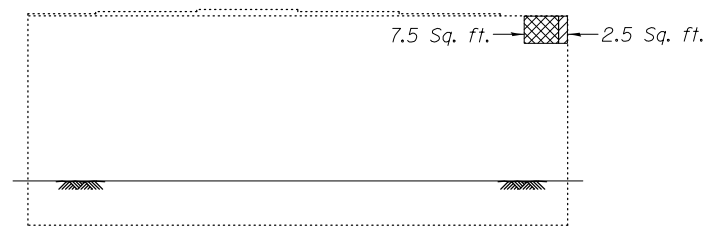
WEST END

**ELEVATION - PIER 4
(Looking North)**

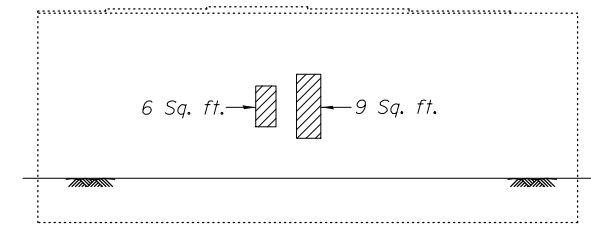
EAST END



**ELEVATION - PIER 2
(Looking South)**



**ELEVATION - PIER 3
(Looking South)**



**ELEVATION - PIER 4
(Looking South)**

LEGEND

- Unsound concrete
- Spalled concrete

DESIGNED -
CHECKED -
DRAWN - h.t. duong
CHECKED -

EXAMINED -
PASSED -
ACTING ENGINEER OF BRIDGES AND STRUCTURES

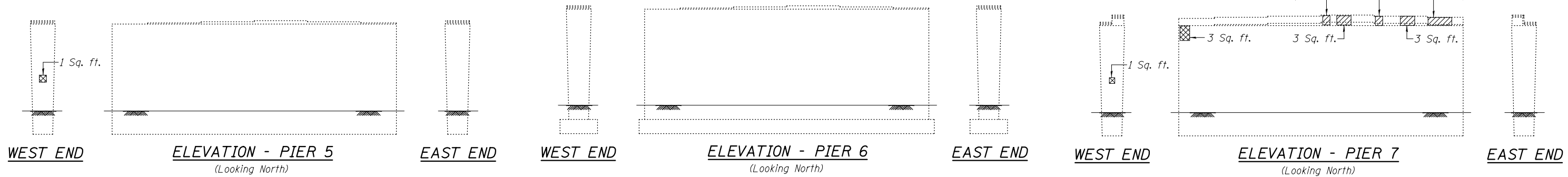
DATE - MAY 6, 2015
REVISED
REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**STRUCTURAL REPAIR OF CONCRETE (S.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

SHEET NO. 66 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	103
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				



WEST END

ELEVATION - PIER 5

(Looking North)

EAST END

WEST END

ELEVATION - PIER 6

(Looking North)

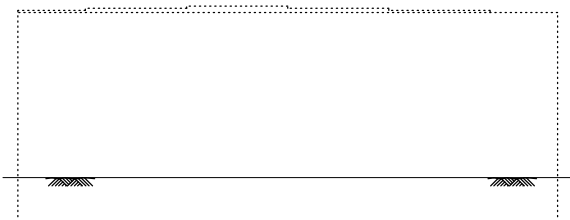
EAST END

WEST END

ELEVATION - PIER 7

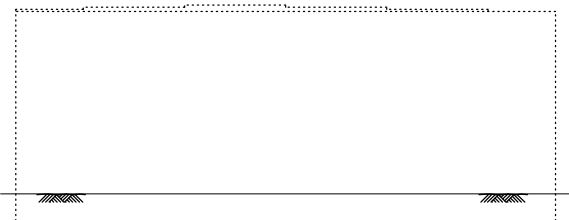
(Looking North)

EAST END



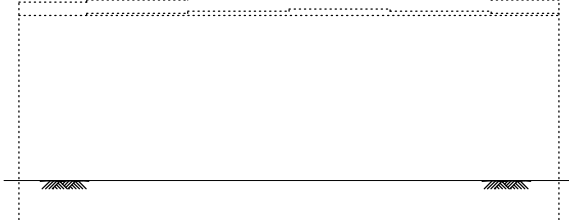
ELEVATION - PIER 5

(Looking South)



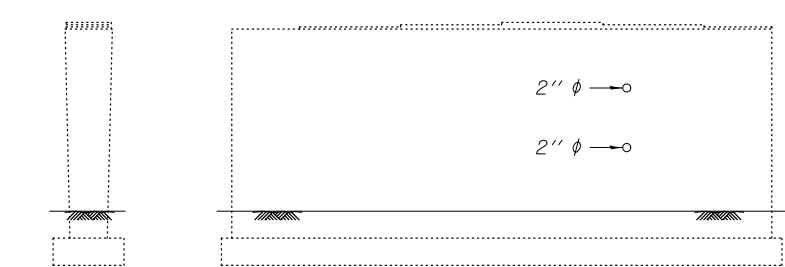
ELEVATION - PIER 6

(Looking South)



ELEVATION - PIER 7

(Looking South)

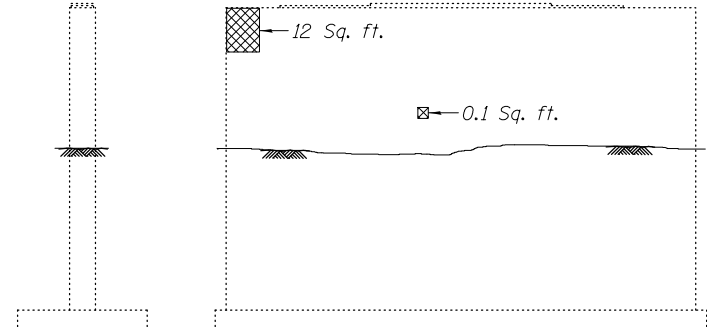


WEST END

ELEVATION - PIER 8

(Looking North)

EAST END

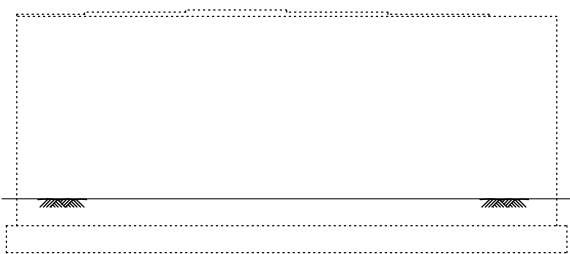


WEST END

ELEVATION - PIER 9

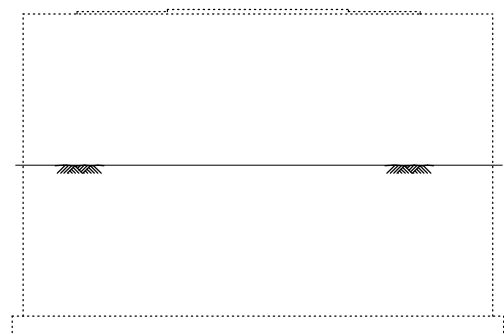
(Looking North)

EAST END



ELEVATION - PIER 8

(Looking South)



ELEVATION - PIER 9

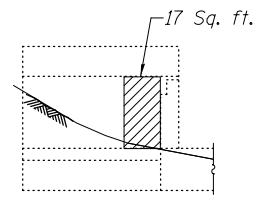
(Looking South)

**STRUCTURAL REPAIR
OF CONCRETE (SQ. FT.)
STRUCTURE NO. 058-0099 (S.B.)**

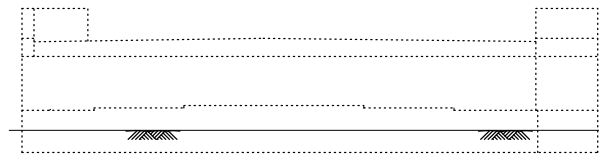
Location	Total (Sq. Ft.)
S. Abut.	2.0
Pier 1	3.0
Pier 2	0.1
Pier 3	13.0
Pier 4	39.5
Pier 5	1.0
Pier 6	0.0
Pier 7	19.0
Pier 8	0.1
Pier 9	12.1
N. Abut.	22.5

LEGEND

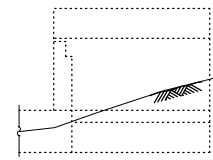
- Unsound concrete
- Spalled concrete



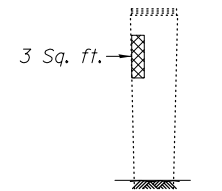
**ELEVATION -
N.W. WINGWALL**



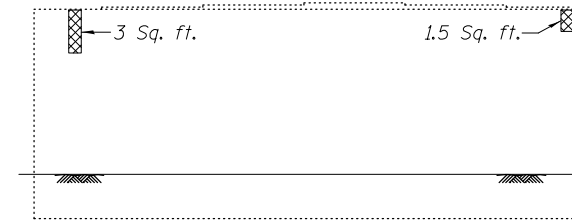
**ELEVATION - NORTH ABUTMENT
(Looking North)**



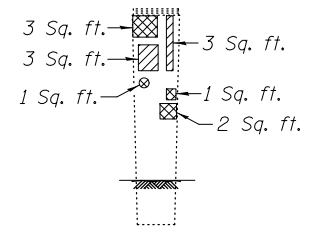
**ELEVATION -
N.E. WINGWALL**



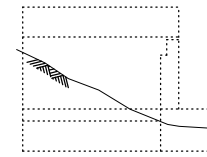
WEST END



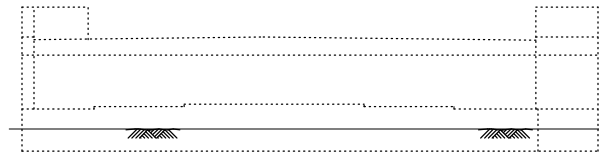
**ELEVATION - PIER 1
(Looking North)**



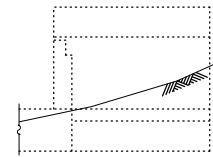
EAST END



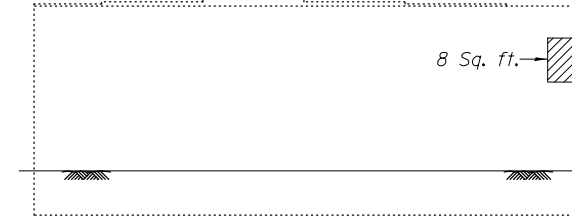
**ELEVATION -
S.E. WINGWALL**



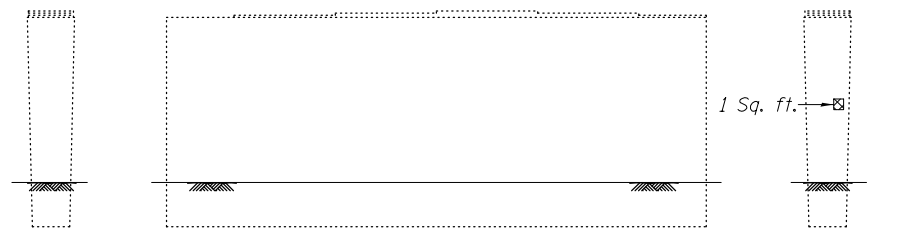
**ELEVATION - SOUTH ABUTMENT
(Looking South)**



**ELEVATION -
S.W. WINGWALL**



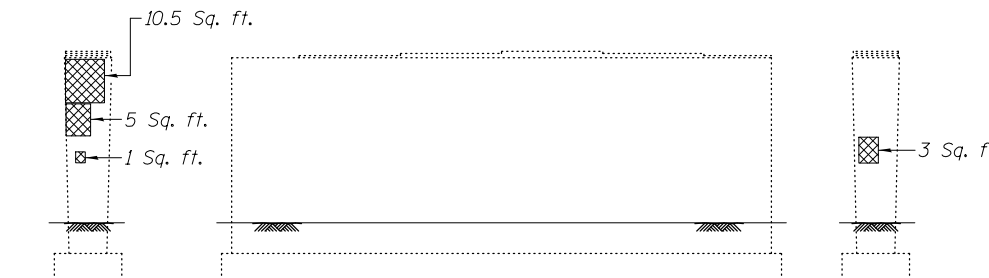
**ELEVATION - PIER 1
(Looking South)**



WEST END

**ELEVATION - PIER 2
(Looking North)**

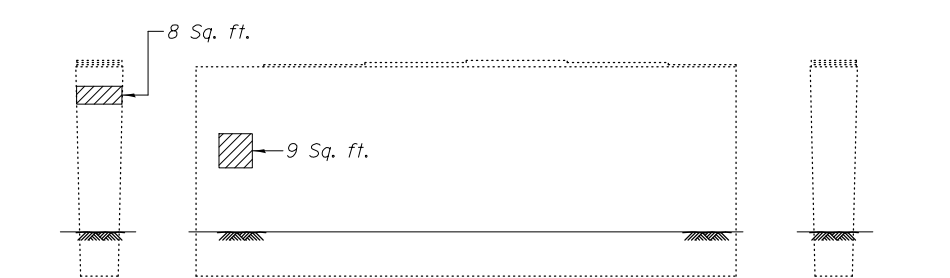
EAST END



WEST END

**ELEVATION - PIER 3
(Looking North)**

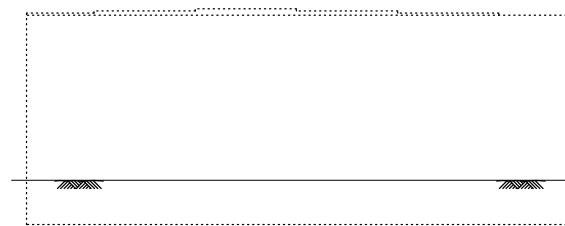
EAST END



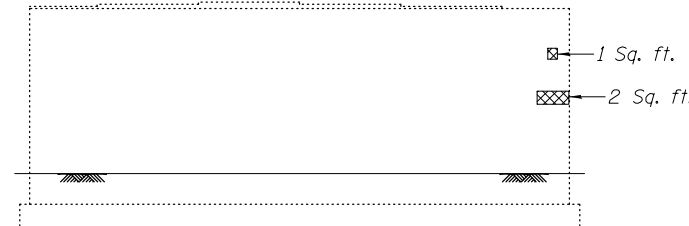
WEST END

**ELEVATION - PIER 4
(Looking North)**

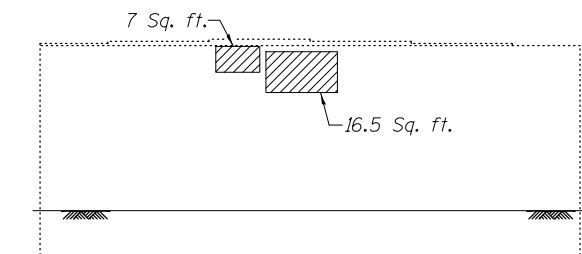
EAST END



**ELEVATION - PIER 2
(Looking South)**



**ELEVATION - PIER 3
(Looking South)**



**ELEVATION - PIER 4
(Looking South)**

LEGEND

- Unsound concrete
- Spalled concrete

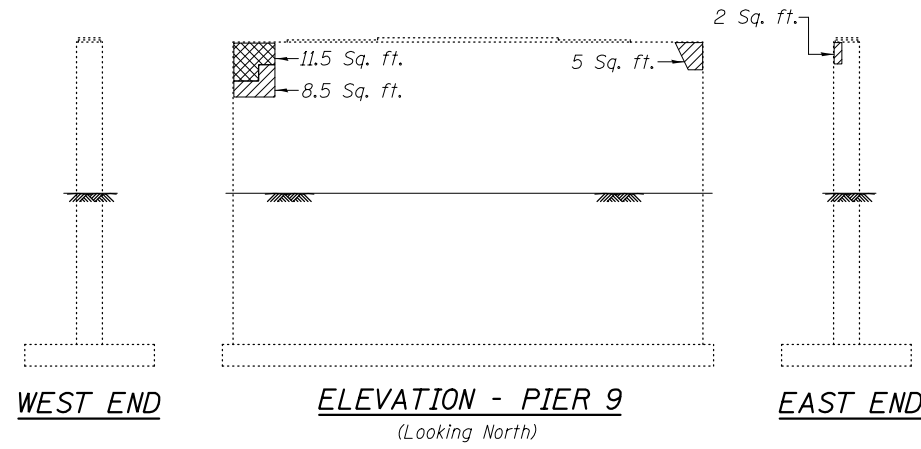
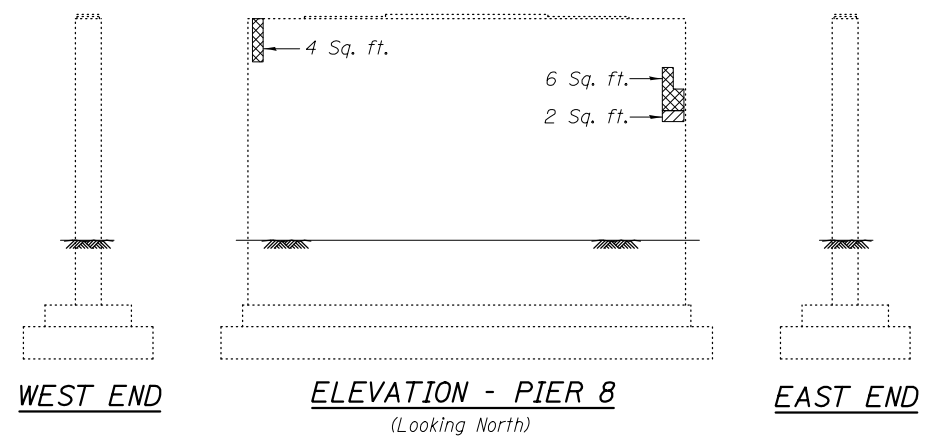
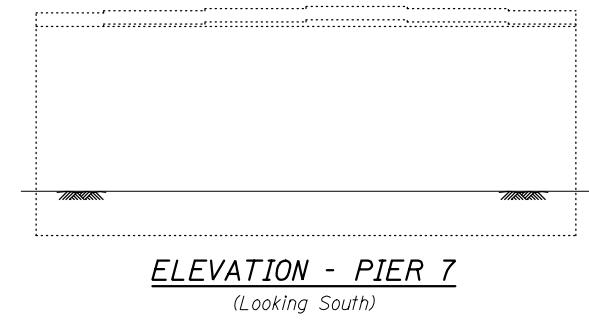
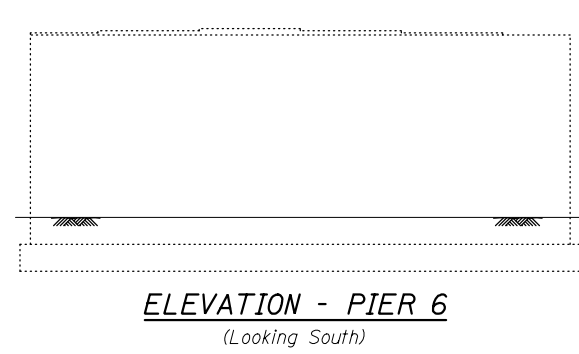
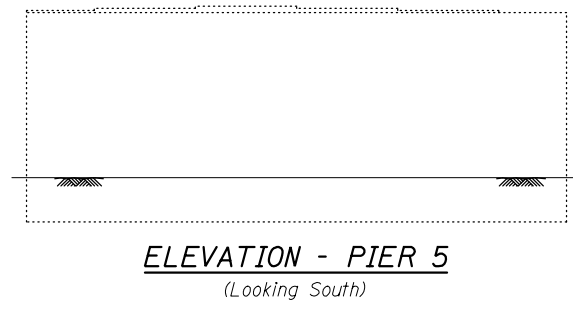
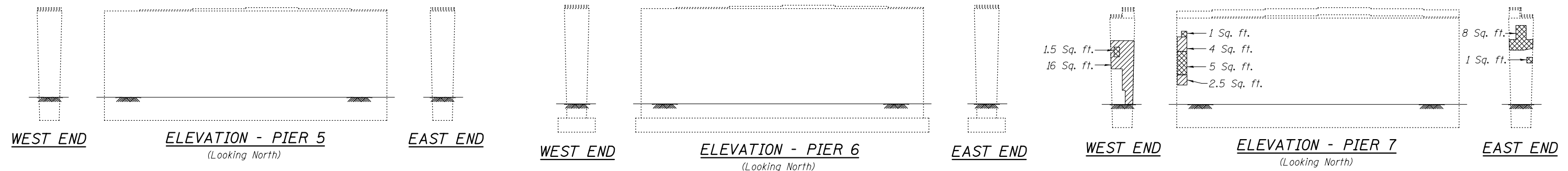
DESIGNED -	EXAMINED -	DATE - MAY 6, 2015
CHECKED -	 ENGINEER OF BRIDGE DESIGN ACTING ENGINEER OF BRIDGES AND STRUCTURES	REVISED
DRAWN - h.t. duong		REVISED
CHECKED -		REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**STRUCTURAL REPAIR OF CONCRETE (N.B.)
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

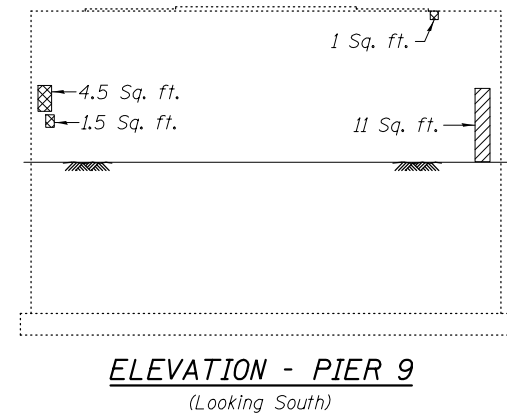
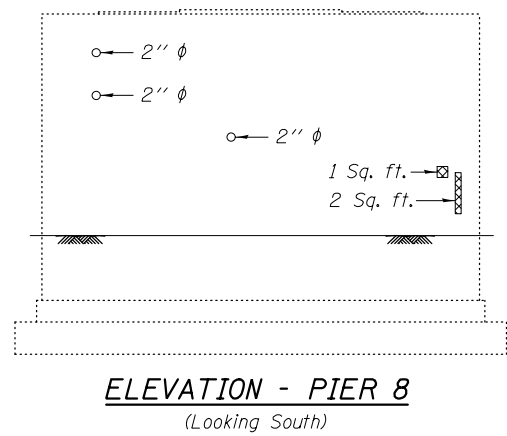
SHEET NO. 66B OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	103B
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				

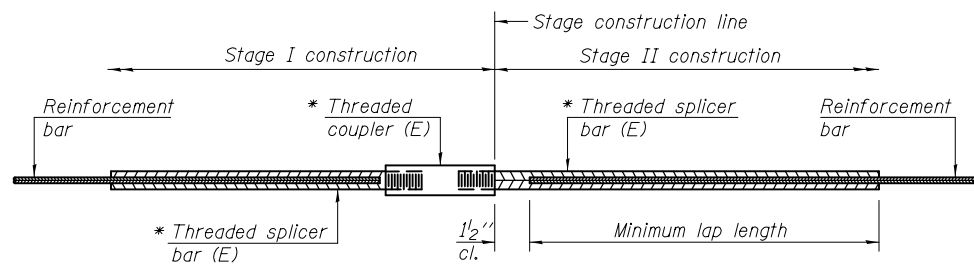


**STRUCTURAL REPAIR
OF CONCRETE (SQ. FT.)
STRUCTURE NO. 058-0098 (N.B.)**

Location	Total (Sq. Ft.)
S. Abut.	0.0
Pier 1	28.5
Pier 2	1.0
Pier 3	22.6
Pier 4	40.5
Pier 5	0.0
Pier 6	0.0
Pier 7	39.0
Pier 8	15.1
Pier 9	45.0
N. Abut.	17.0



LEGEND
 Unsound concrete
 Spalled concrete



STANDARD BAR SPLICER ASSEMBLY

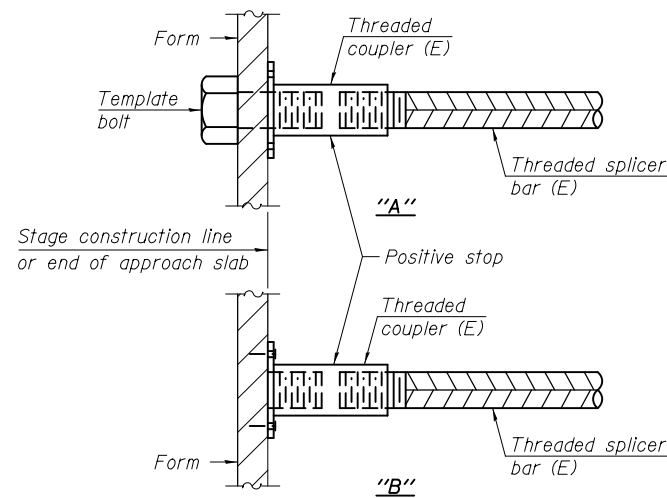
Minimum Lap Lengths						
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6
3, 4	1'-5"	1'-11"	2'-1"	2'-4"	2'-7"	2'-11"
5	1'-9"	2'-5"	2'-7"	2'-11"	3'-3"	3'-8"
6	2'-1"	2'-11"	3'-1"	3'-6"	3'-10"	4'-5"
7	2'-9"	3'-10"	4'-2"	4'-8"	5'-2"	5'-10"
8	3'-8"	5'-1"	5'-5"	6'-2"	6'-9"	7'-8"
9	4'-7"	6'-5"	6'-10"	7'-9"	8'-7"	9'-8"

- Table 1: Black bar, 0.8 Class C
- Table 2: Black bar, Top bar lap, 0.8 Class C
- Table 3: Epoxy bar, 0.8 Class C
- Table 4: Epoxy bar, Top bar lap, 0.8 Class C
- Table 5: Epoxy bar, Class C
- Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + 1/2" + thread length

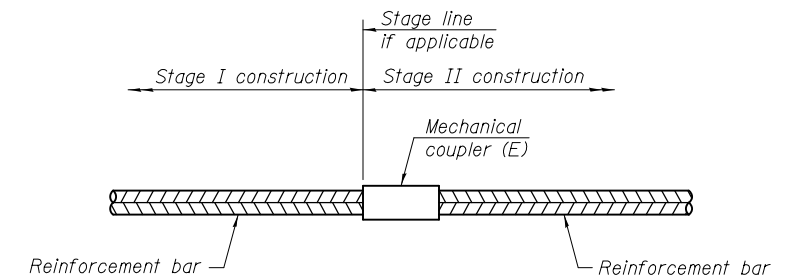
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Table for minimum lap length



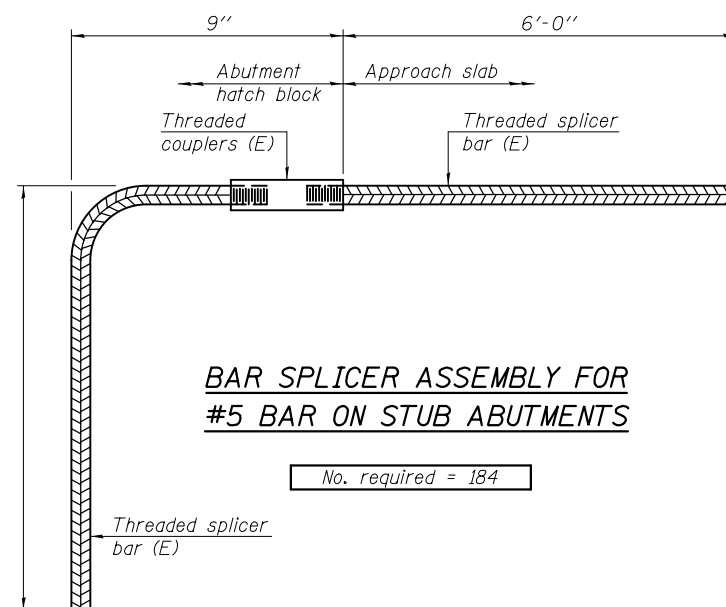
INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
 (E) : Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



NOTES

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
 All reinforcement shall be lapped and tied to the splicer bars.
 Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
 See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

8-31-12

DESIGNED - N. R. Barnett
 CHECKED - J. D. Ortiz-Varela
 DRAWN - h.t. duong
 CHECKED - NRB/JOV/GRA

EXAMINED
 PASSED
 ACTING ENGINEER OF BRIDGES AND STRUCTURES

DATE - MAY 6, 2015
 REVISED
 REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

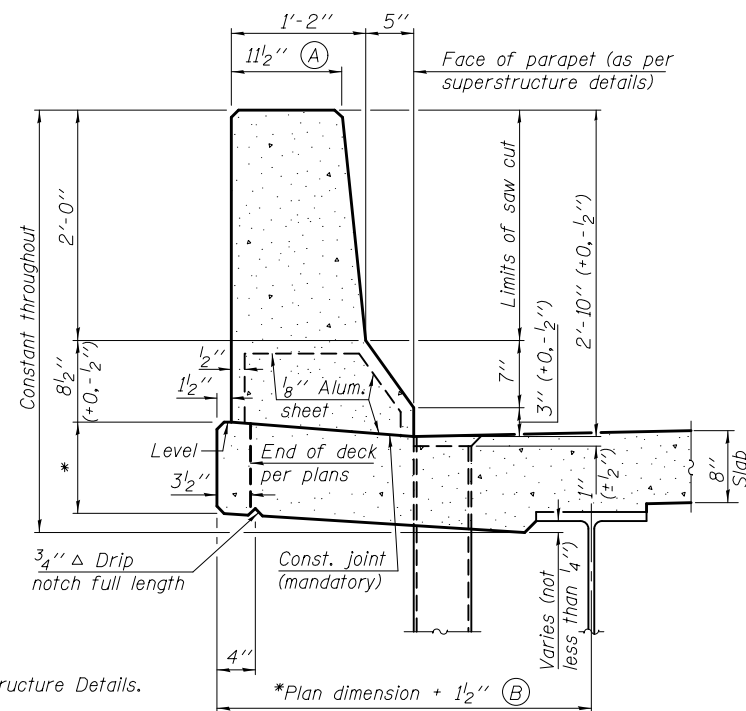
BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
 STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)

SHEET NO. 67 OF 68 SHEETS

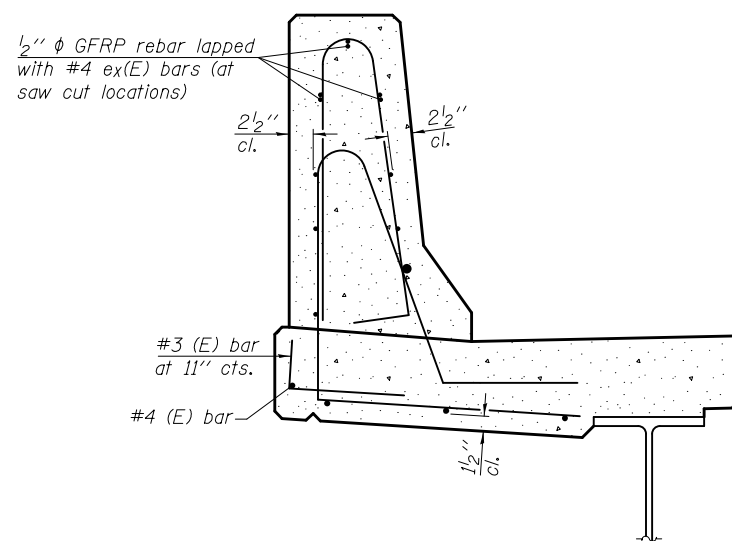
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	104
CONTRACT NO. 74351				
ILLINOIS FED. AID PROJECT				

GENERAL NOTES

All dimensions shall remain the same as shown on superstructure details, except dimensions A and B which are to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension A and B = 0.0165 cu. yds./ft. for 34" parapet or = 0.0223 cu. yds./ft. for 42" parapet. Place aluminum sheet in curb portion at and near piers. Full thickness saw cut at all joint locations in lieu of cork joint filler. Steel superstructure shown. Other superstructure types similar.

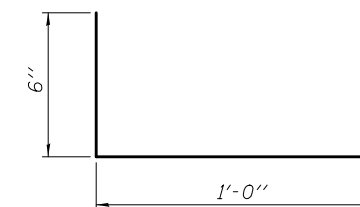


34" F SHAPE PARAPET SECTION
(Showing dimensions)

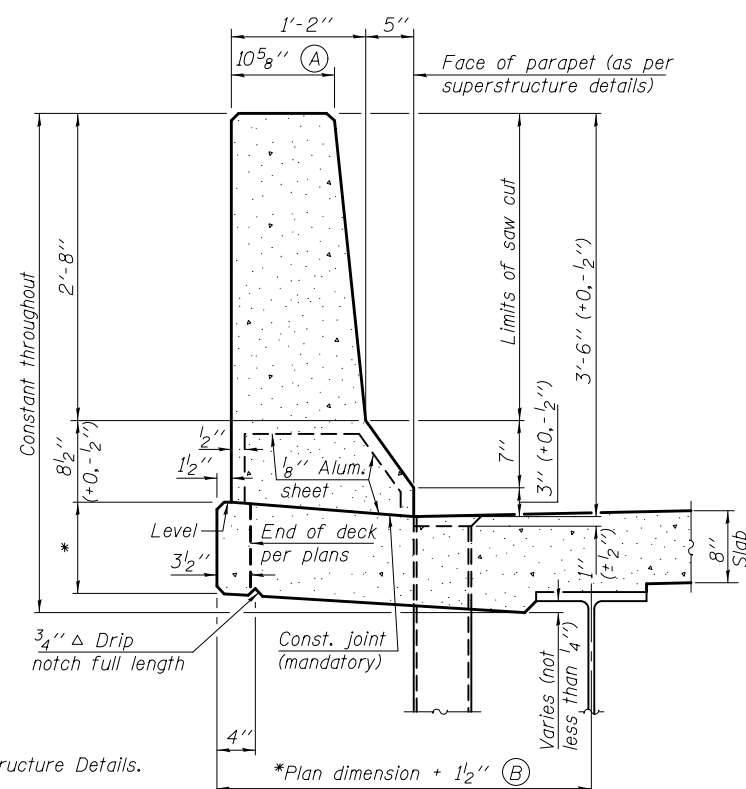


SECTION

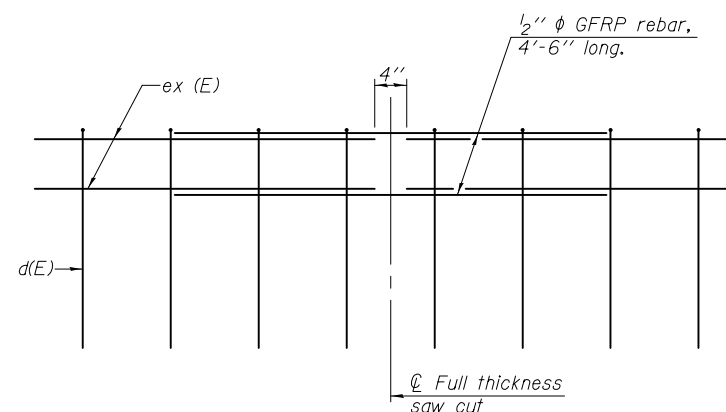
(34" parapet shown - 42" parapet similar)
(Showing reinforcement clearances for slip forming and additional reinforcement bars)



#3 (E) BAR

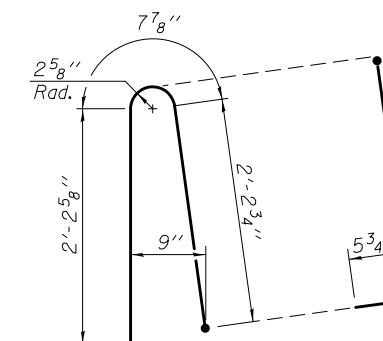


42" F SHAPE PARAPET SECTION
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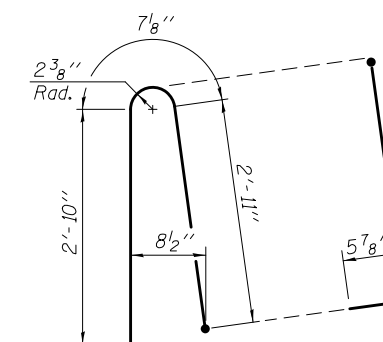


GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)



ALTERNATE BAR d(E)
(For 34" parapet when conduit is present)



ALTERNATE BAR d(E)
(For 42" parapet when conduit is present)

SFP 34-42

8-16-12

DESIGNED - N. R. Barnett
CHECKED - J. D. Ortiz-Varela
DRAWN - h.t. duong
CHECKED - NRB/JOV/GRA

EXAMINED	DATE - MAY 6, 2015
PASSED	REVISOR
	REVISOR

DATE - MAY 6, 2015
REVISOR
REVISOR

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**CONCRETE PARAPET SLIPFORMING OPTION
STRUCTURE NO. 058-0098 (N.B.) & 058-0099 (S.B.)**

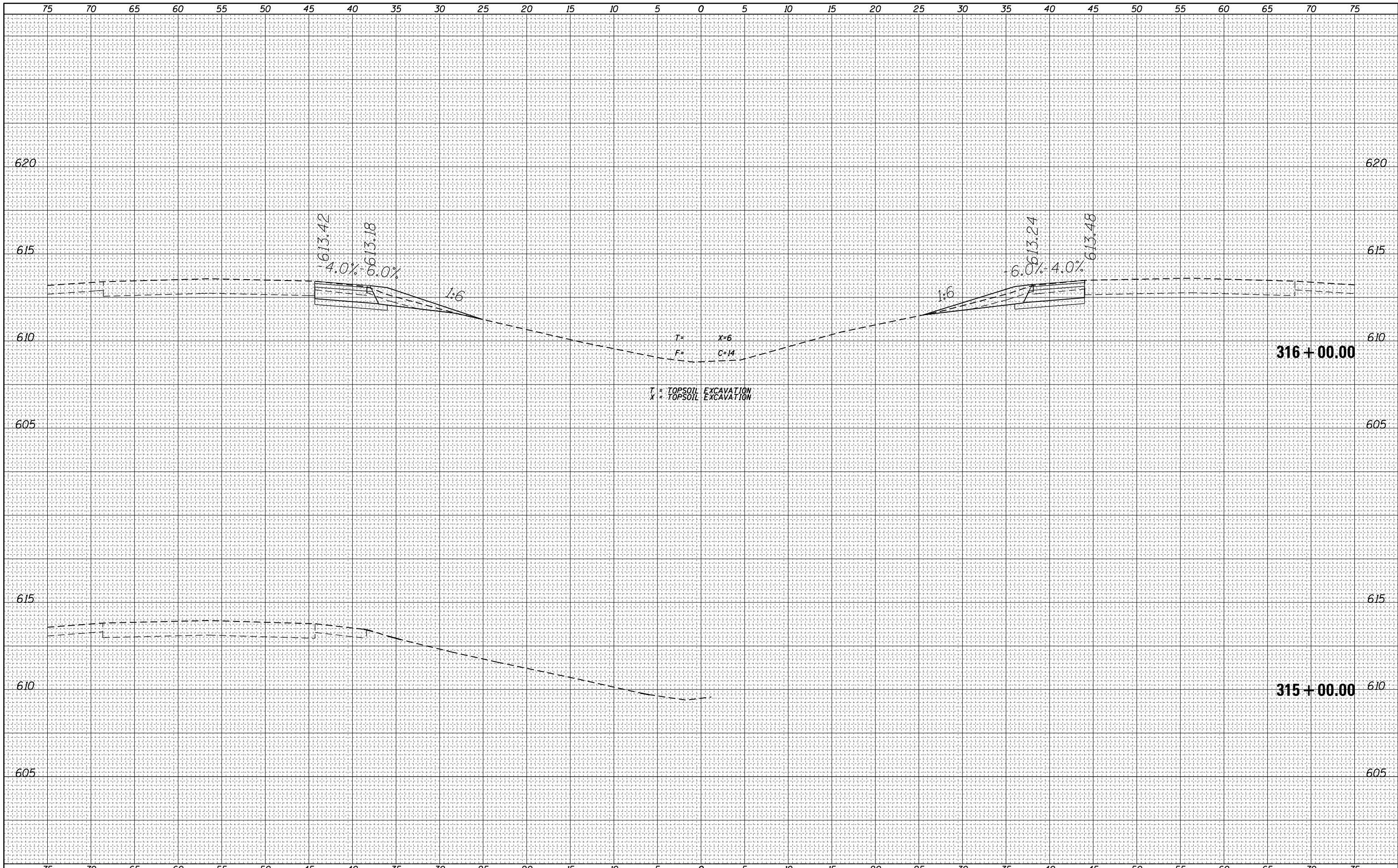
SHEET NO. 68 OF 68 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	105
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

BY: _____ DATE: _____
 SURVEYED _____
 PLOTTED _____
 TEMPLATE _____
 AREAS CHECKED _____
 FINAL SURVEY NO. _____

BY: _____ DATE: _____
 SURVEYED _____
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 TEMPLATE _____
 AREAS CHECKED _____
 ORIGINAL SURVEY NO. _____



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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

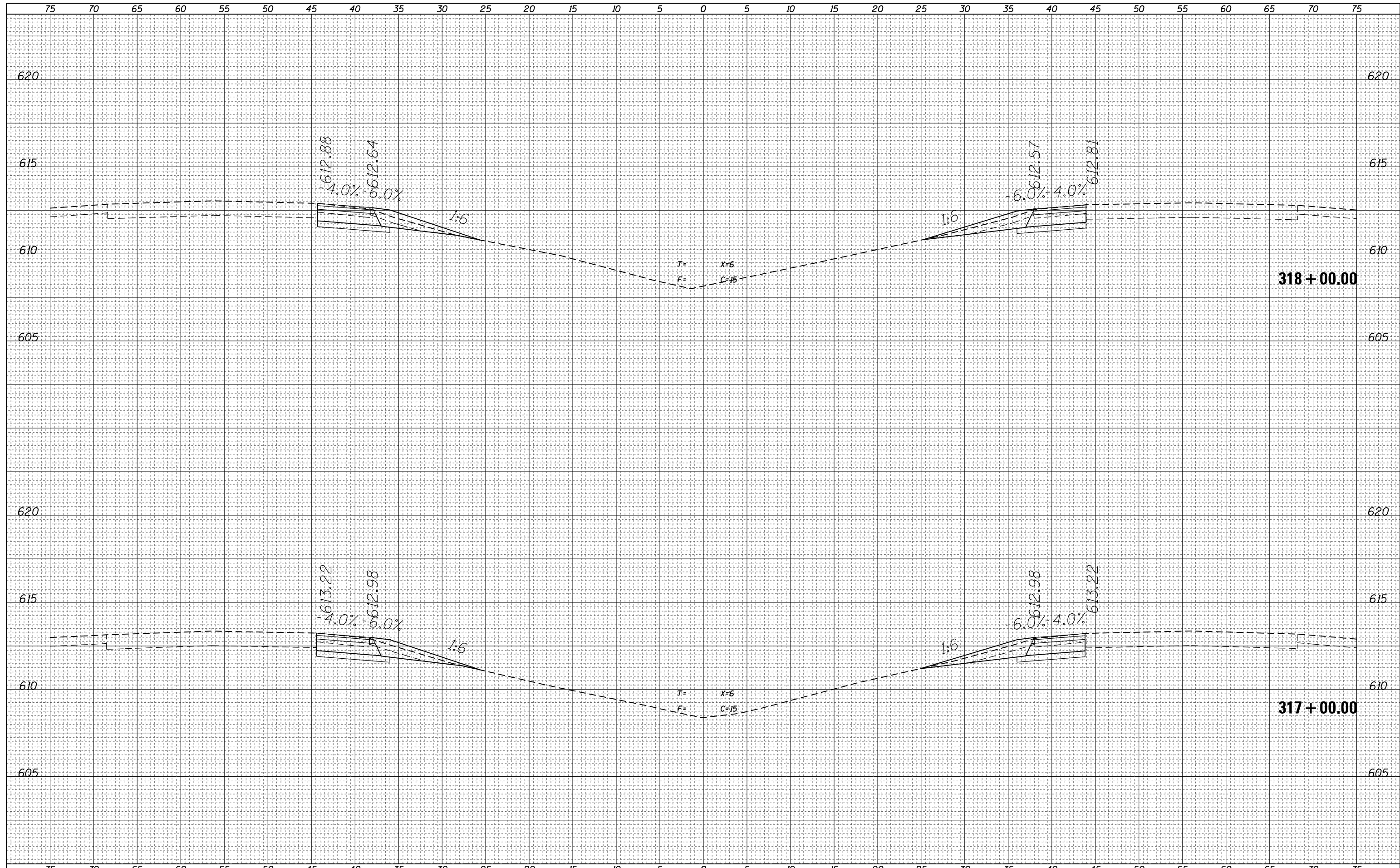
CROSS SECTIONS

SCALE: SHEET OF SHEETS STA. 315+00.00 TO STA. 316+00.00

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322	(58-20B-1)BR	MACON	122	106
			CONTRACT NO. 74351	

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NOTE BOOK	PLOTTED
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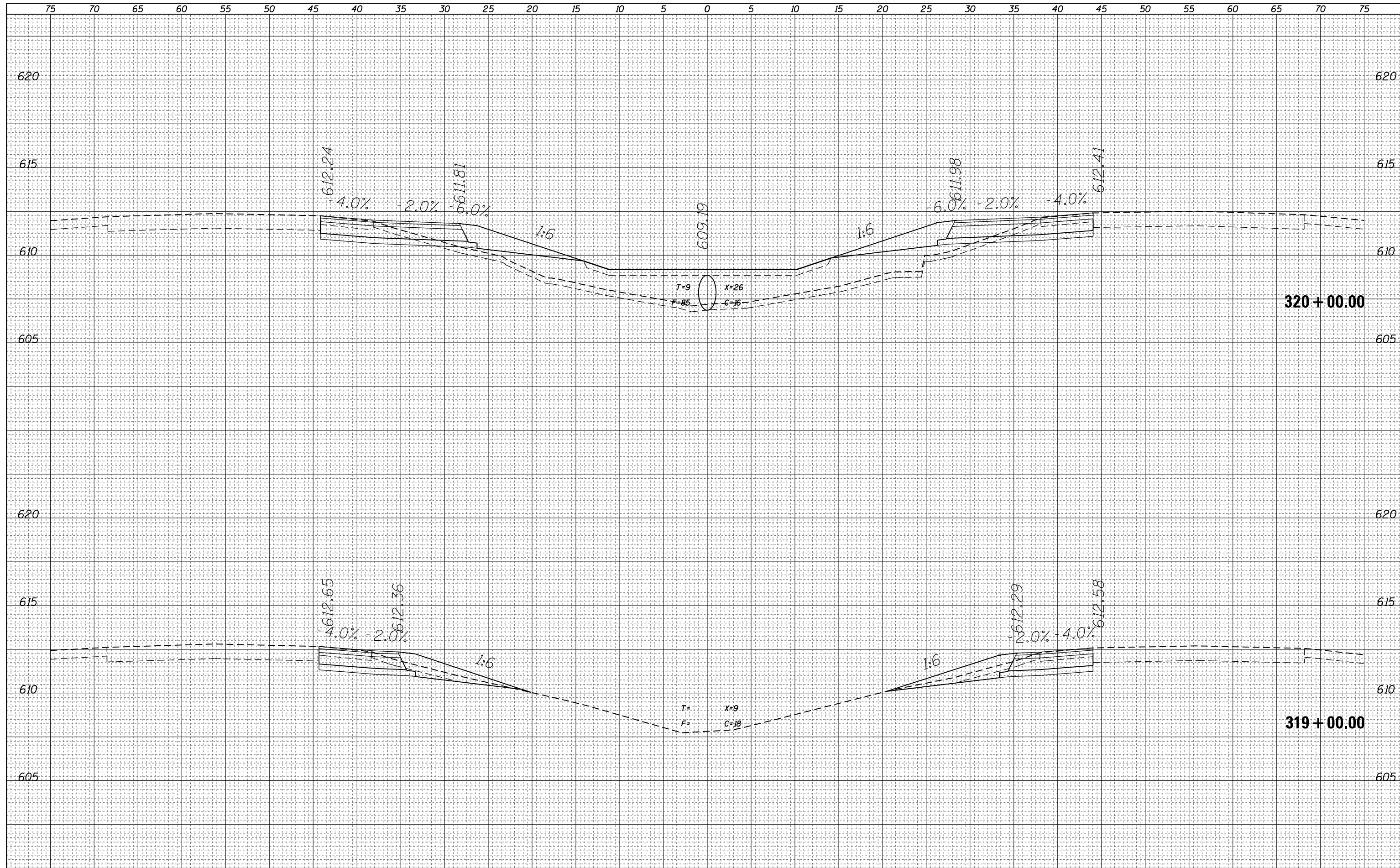
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FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
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ORIGINAL SURVEY	SURVEYED
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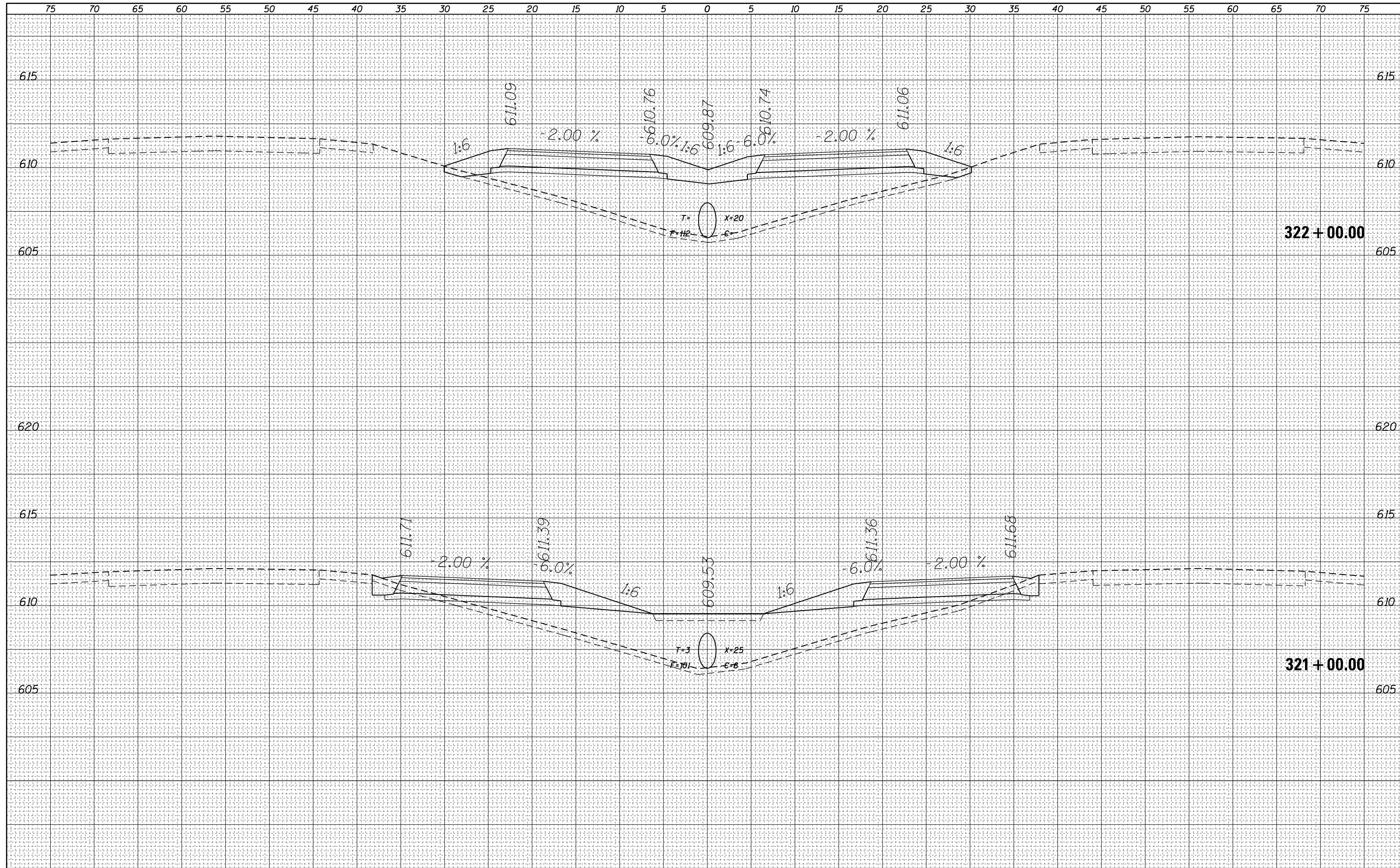
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

CROSS SECTIONS	
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	108
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	

DATE	BY
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NOTE BOOK	
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NOTE BOOK	
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS		SHEET NO.																			
322	(58-20B-1)BR	MACON	122		109																			
					CONTRACT NO. 74351																			
ILLINOIS FED. AID PROJECT																								
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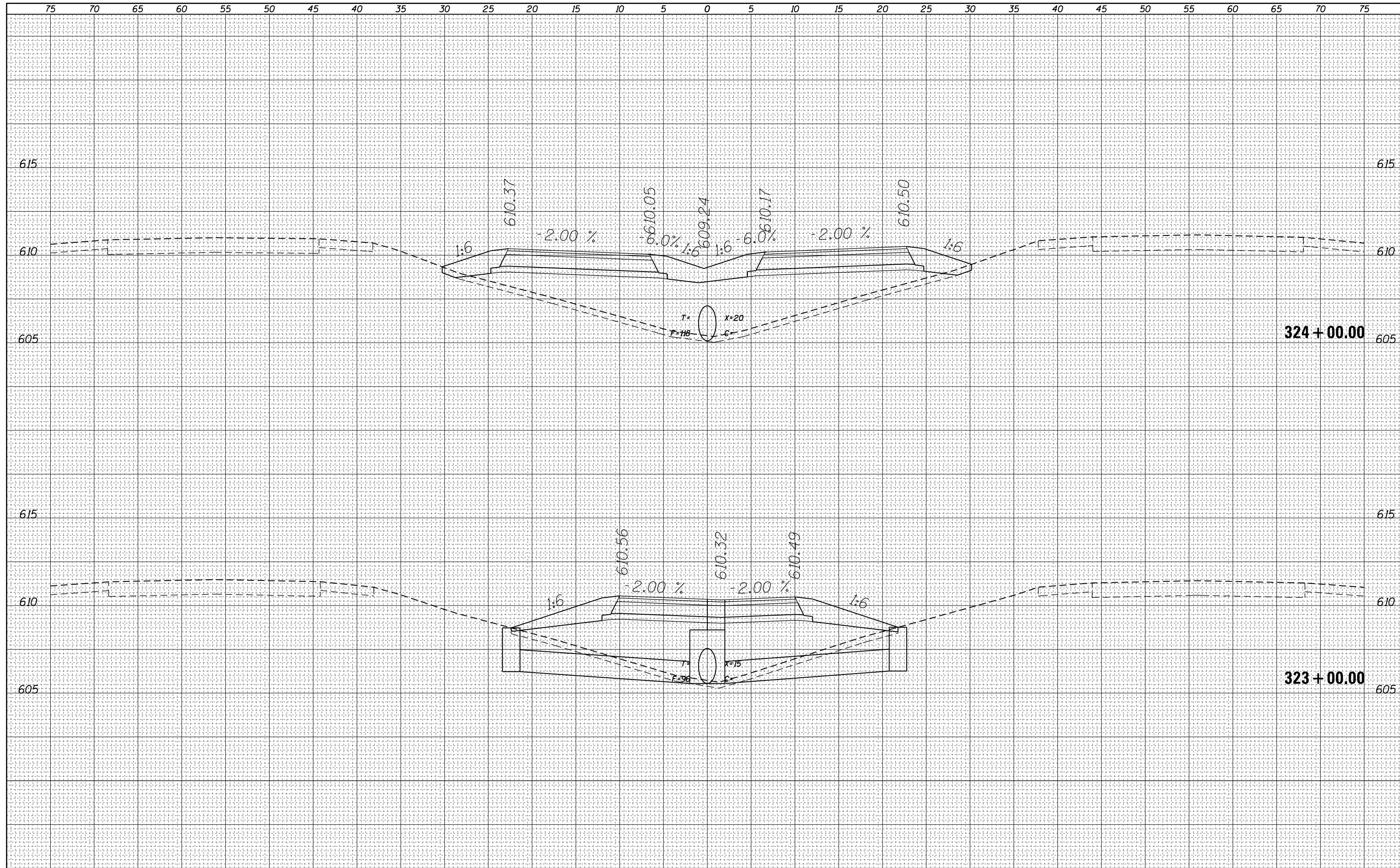
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

CROSS SECTIONS

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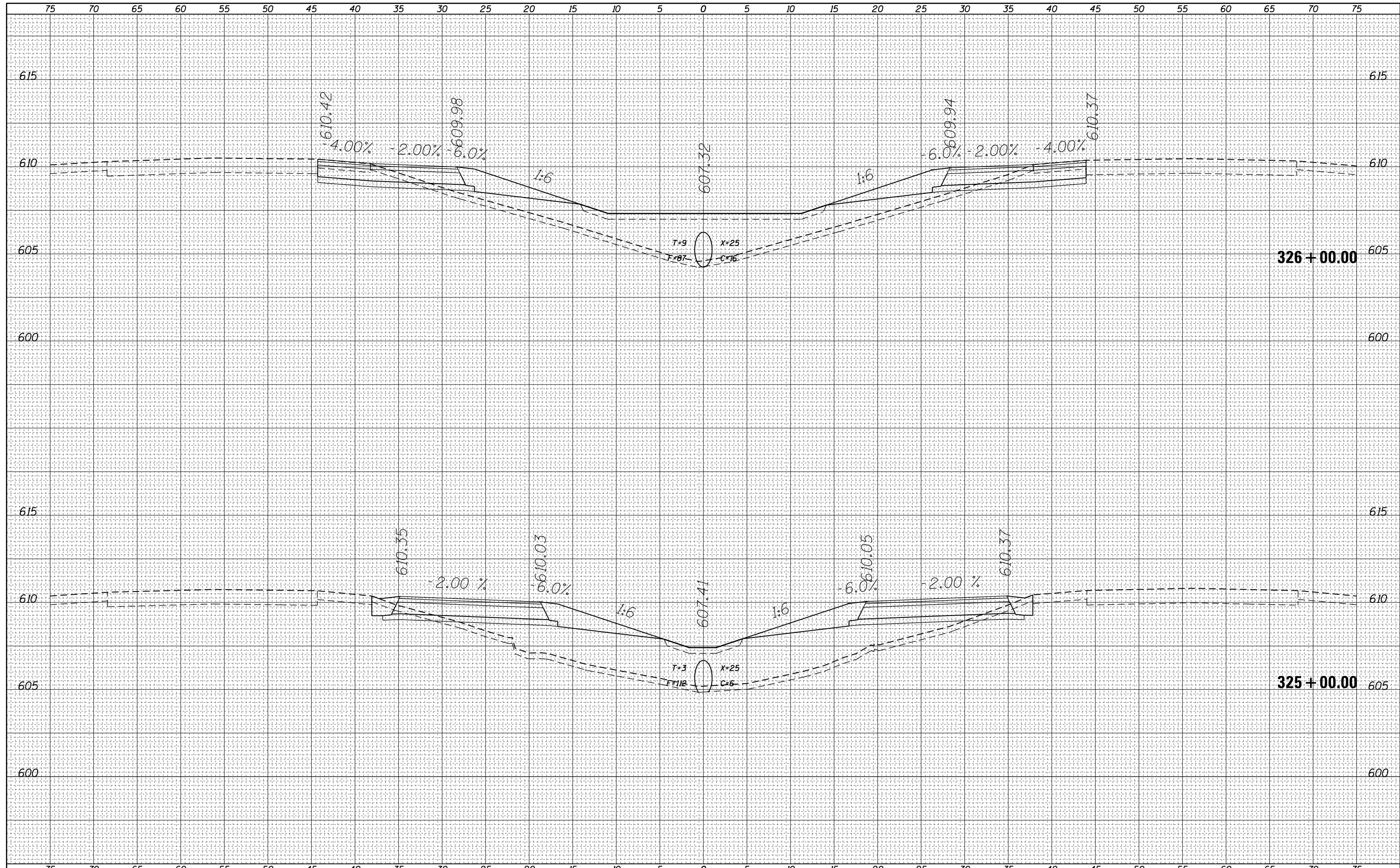
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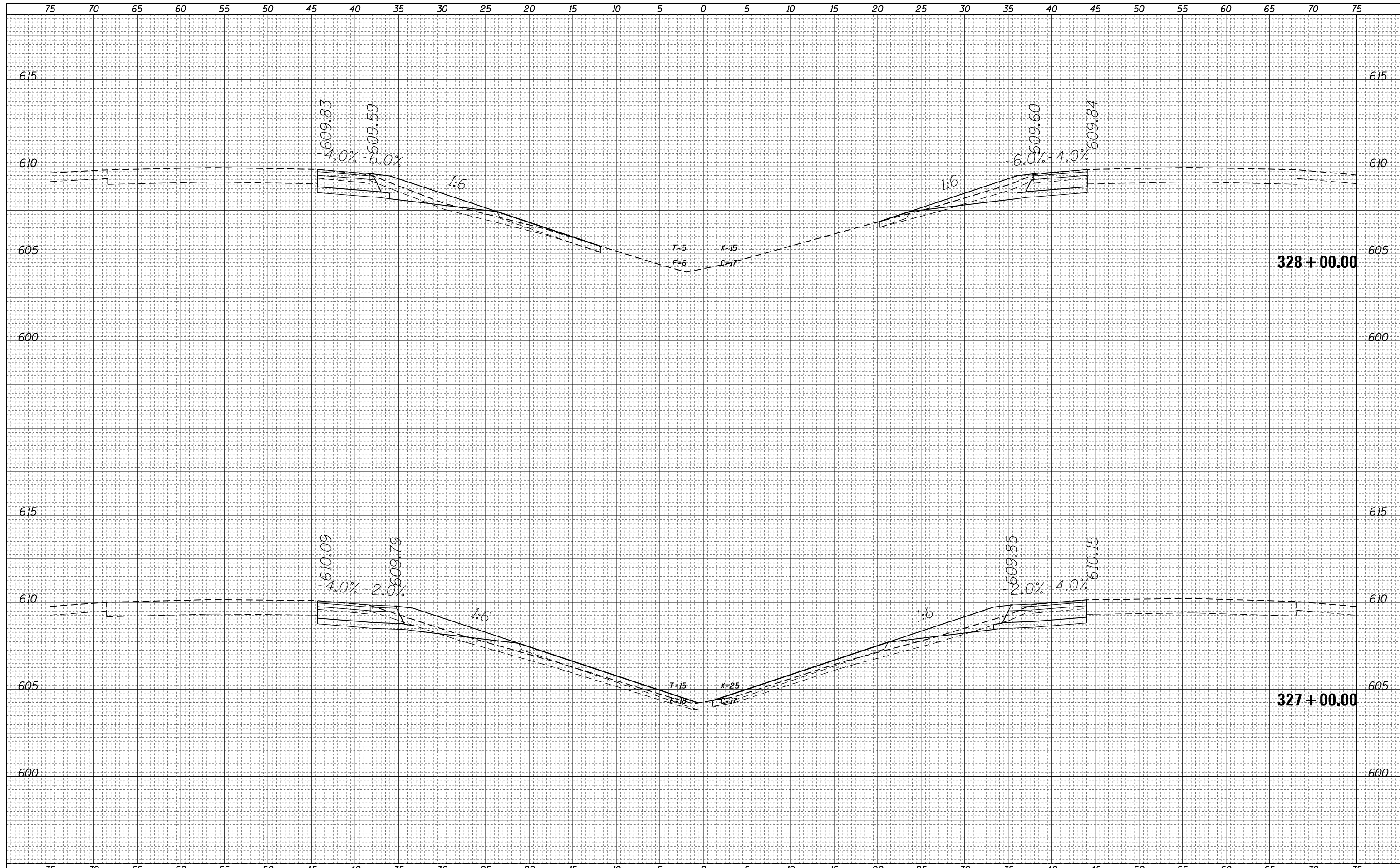
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

CROSS SECTIONS
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	111
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	

DATE	
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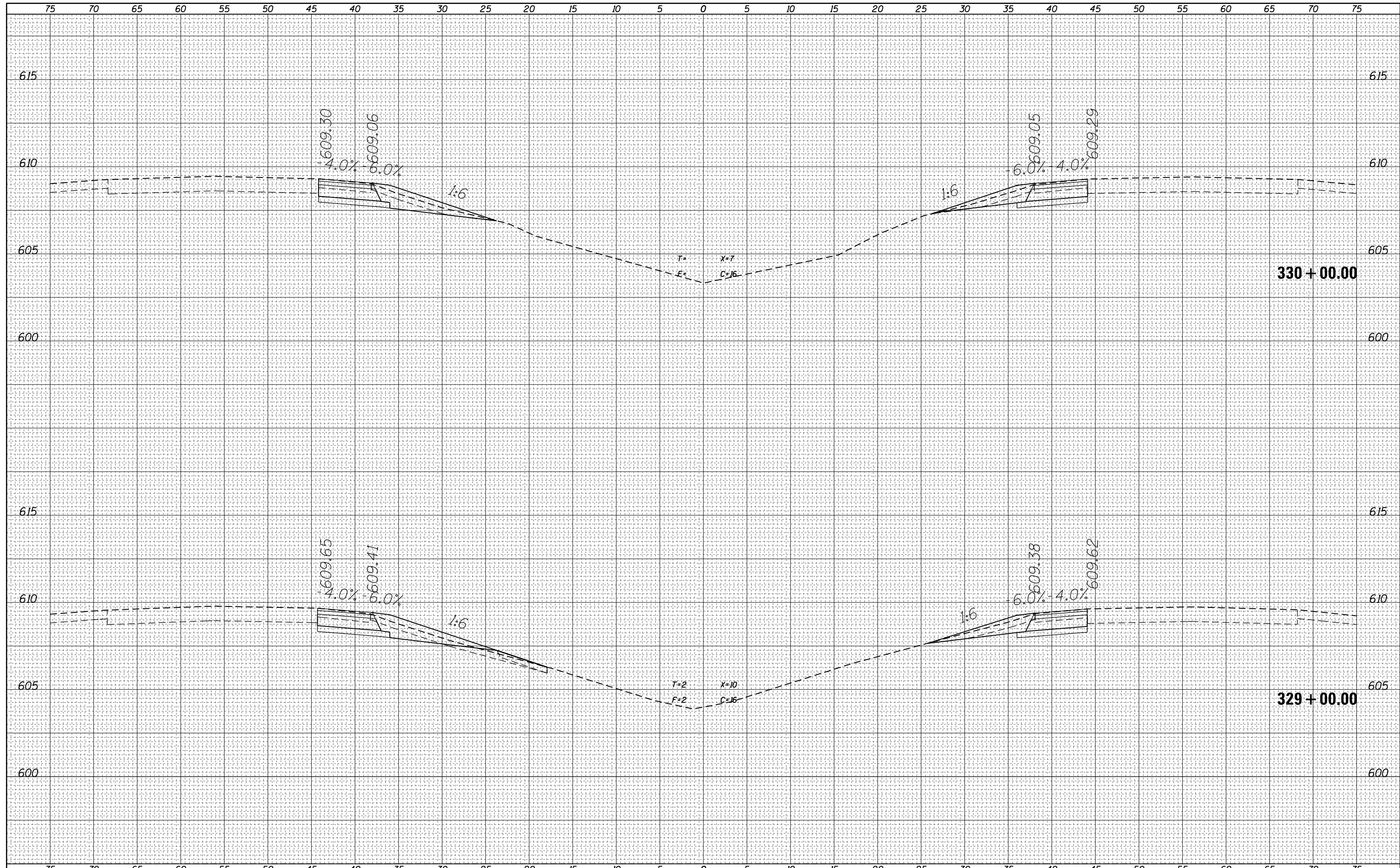
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DATE	BY
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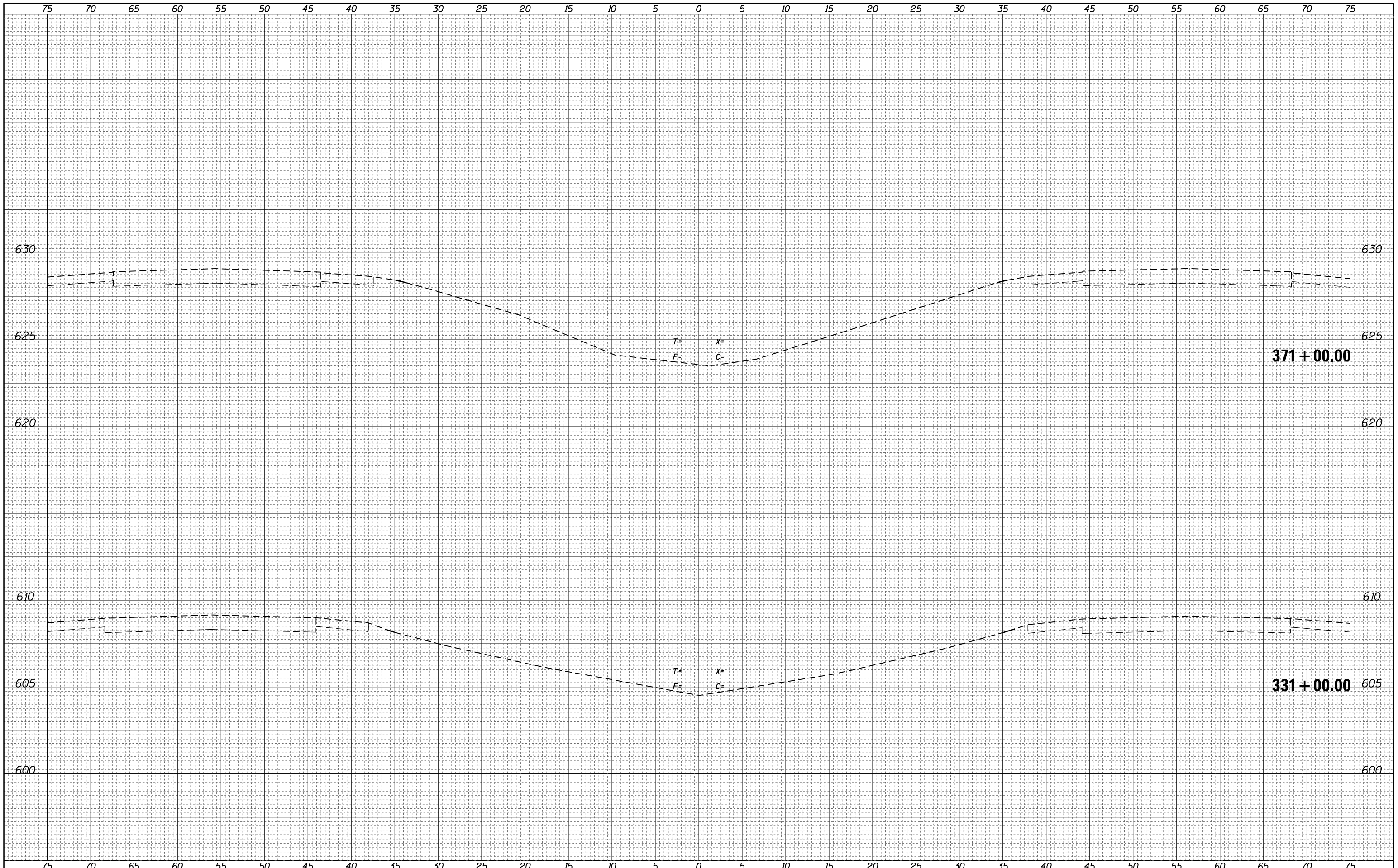
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NOTE BOOK	AREAS CHECKED
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Default	DOT Offices\District 7\Projects\74351\CADD\Drawings\Drawings\74351-sht-xxsht	CHECKED -	REVISIED -		SCALE:	SHEET	OF	SHEETS	STA. 329+00.00	TO STA. 330+00.00	322	(58-20B-1)BR	MACON	122	113
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	PLOT DATE = 3/20/2015										ILLINOIS FED. AID PROJECT				

DATE	BY	FINAL SURVEY NO.	SURVEYED PLOTTED	TEMPLATES	AREAS CHECKED

DATE	BY	ORIGINAL SURVEY NO.	SURVEYED PLOTTED	TEMPLATES	AREAS CHECKED



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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

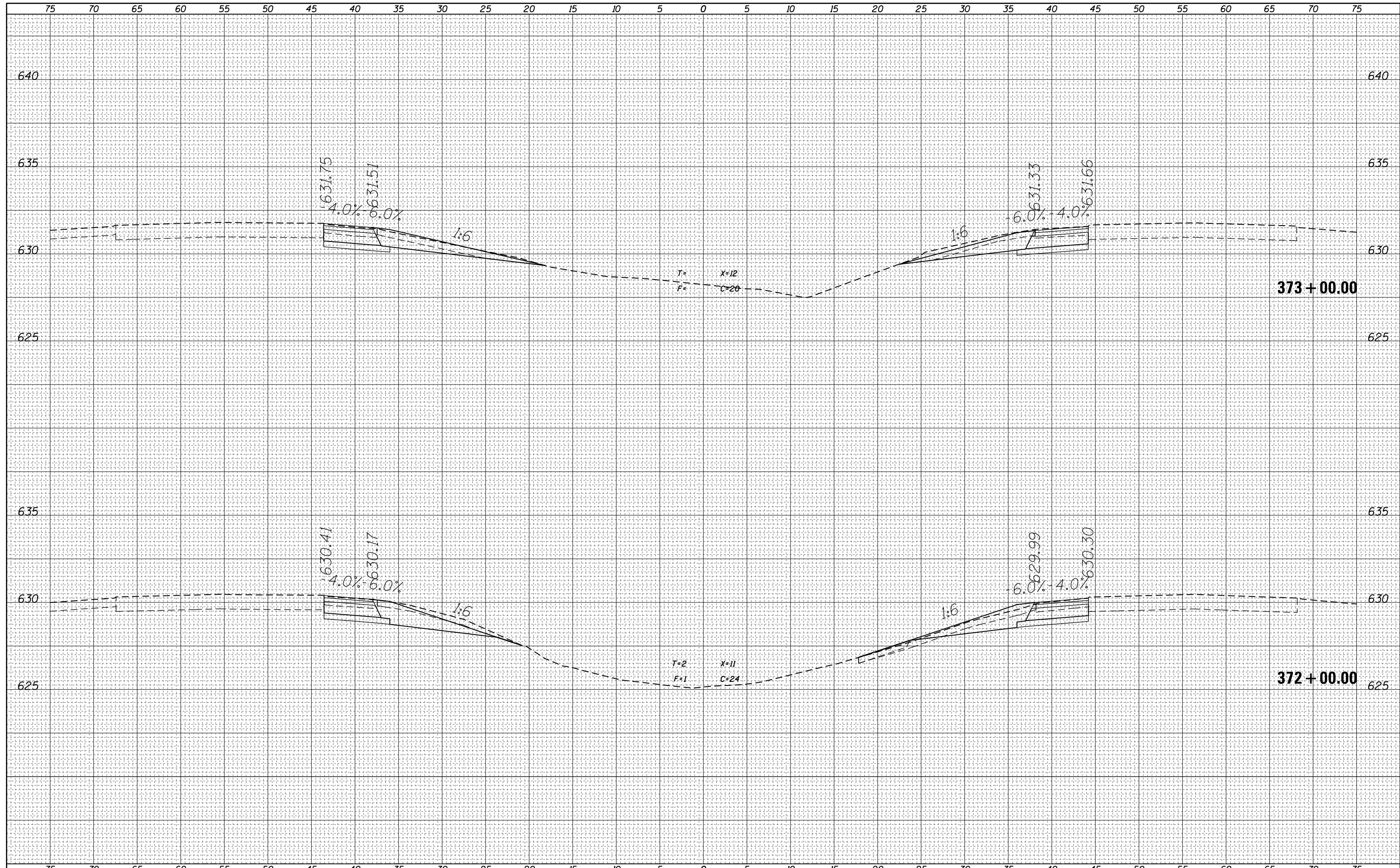
CROSS SECTIONS

SCALE:	SHEET	OF	SHEETS	STA. 331+00.00	TO STA. 371+00.00
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	114
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	

DATE	
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FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
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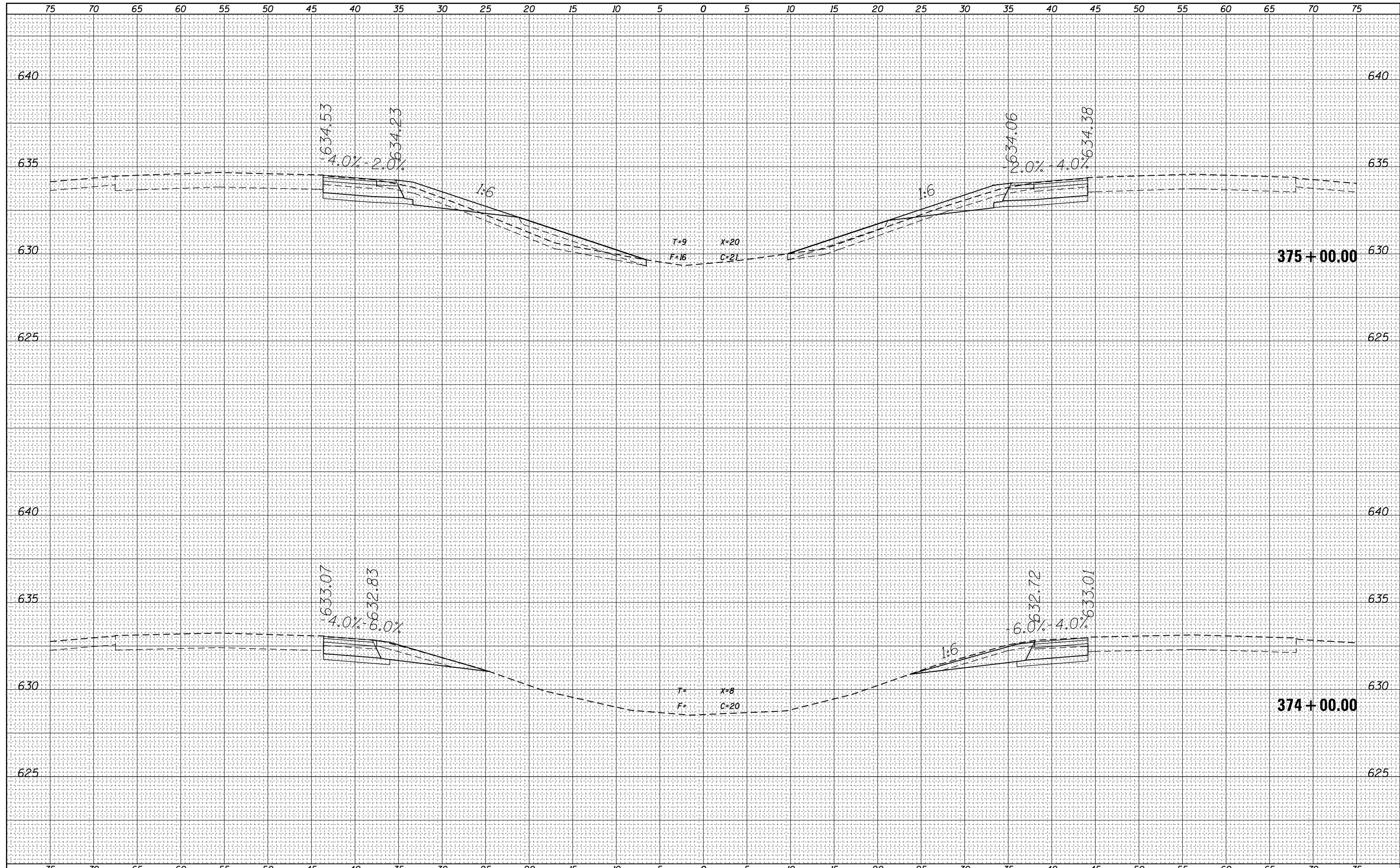
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\\IL084EBIDINTEG.Illinois.gov\PIWIDOT\Documents\IDOT Offices\District 7\Projects\74351\CADD\Drawings\Drawings\74351-sht-xxsht	DATE	CHECKED -	REVISIED -		
Default	PLOT DATE = 3/20/2015	REVISIED -	REVISIED -		
				F.A.P. RTE. 322 SECTION (58-20B-1)BR COUNTY MACON TOTAL SHEETS 122 SHEET NO. 115 CONTRACT NO. 74351 ILLINOIS FED. AID PROJECT	

DATE	
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FINAL SURVEY	SURVEYED
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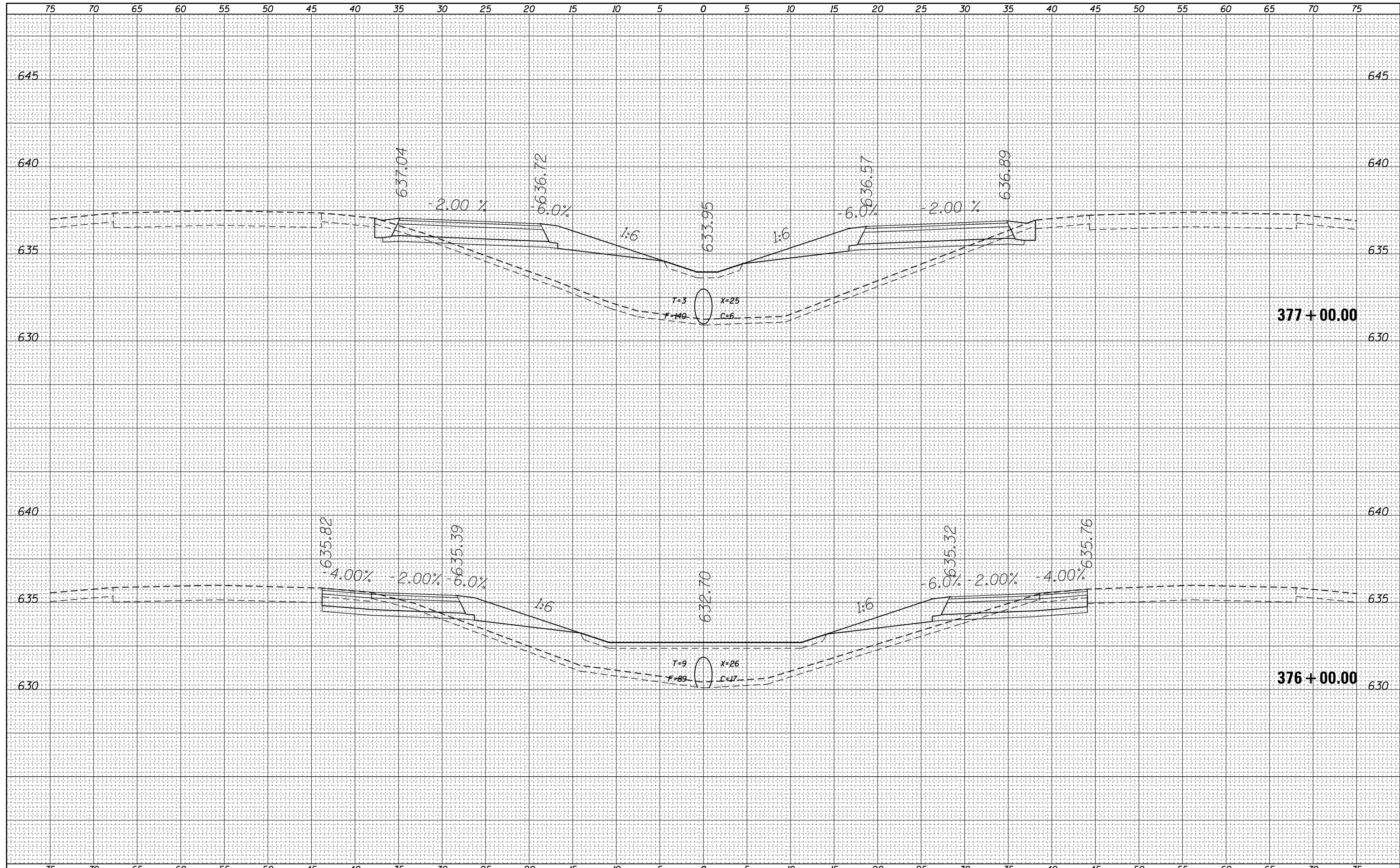
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FILE NAME =	USER NAME = steffenk	DESIGNED -	REVISIED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CROSS SECTIONS		F.A.P. RTE. 322	SECTION (58-20B-1)BR	COUNTY MACON	TOTAL SHEETS 122	SHEET NO. 116	
Default	DOT Offices\District 7\Projects\74351\CADD\Drawings\Drawings\74351-sht-xxsst	CHECKED -	REVISIED -		SCALE:	SHEET	OF	SHEETS	STA. 374+00.00	TO STA. 375+00.00	CONTRACT NO. 74351	
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	PLOT DATE = 3/20/2015											

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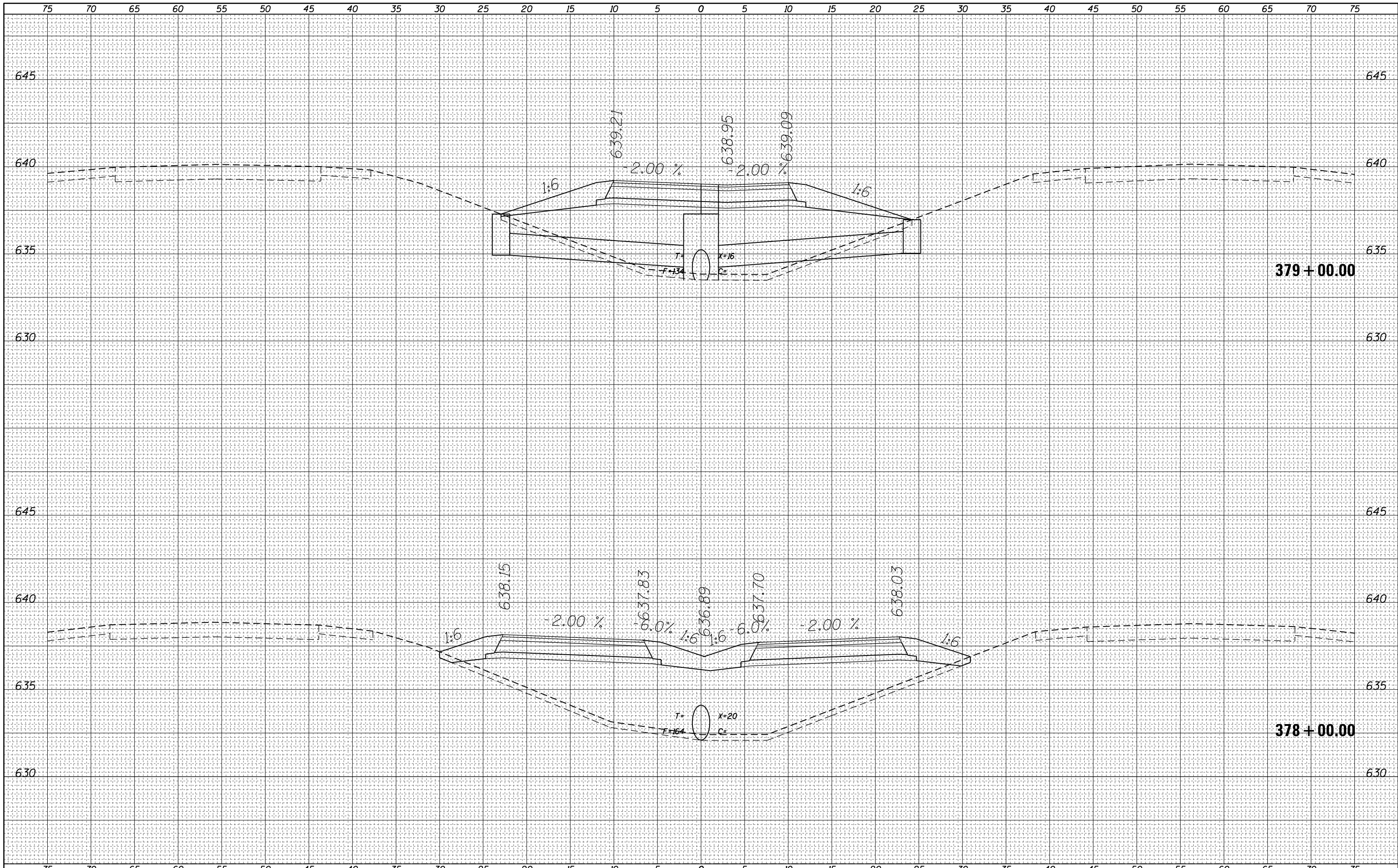
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FILE NAME =	USER NAME = steffennk	DESIGNED -	REVISIED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CROSS SECTIONS SCALE: SHEET OF SHEETS STA. 376+00.00 TO STA. 377+00.00	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
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STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

CROSS SECTIONS

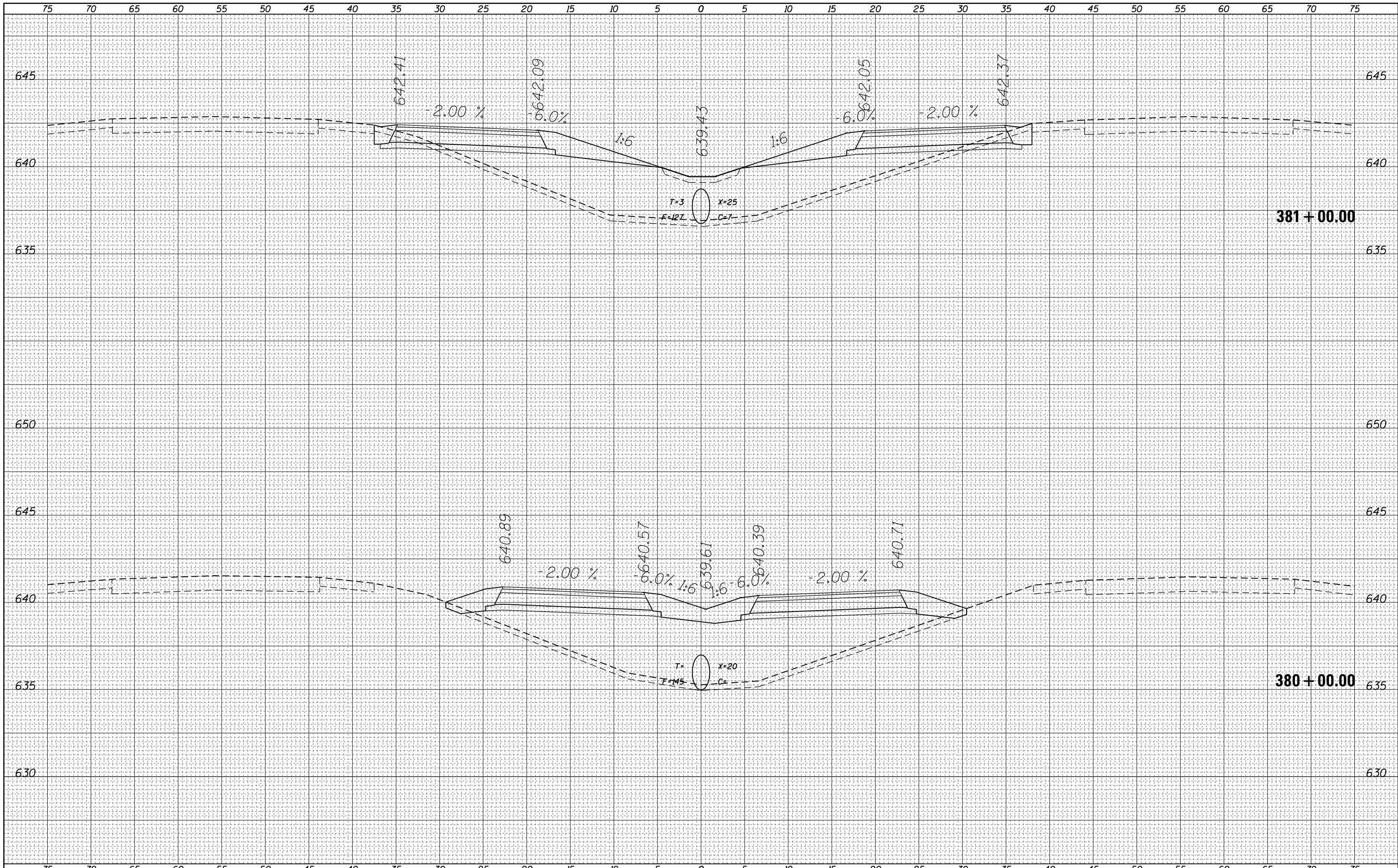
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	118
CONTRACT NO. 74351				

ILLINOIS FED. AID PROJECT

DATE	
BY	
FINAL SURVEY NO.	
SURVEYED PLOTTED	
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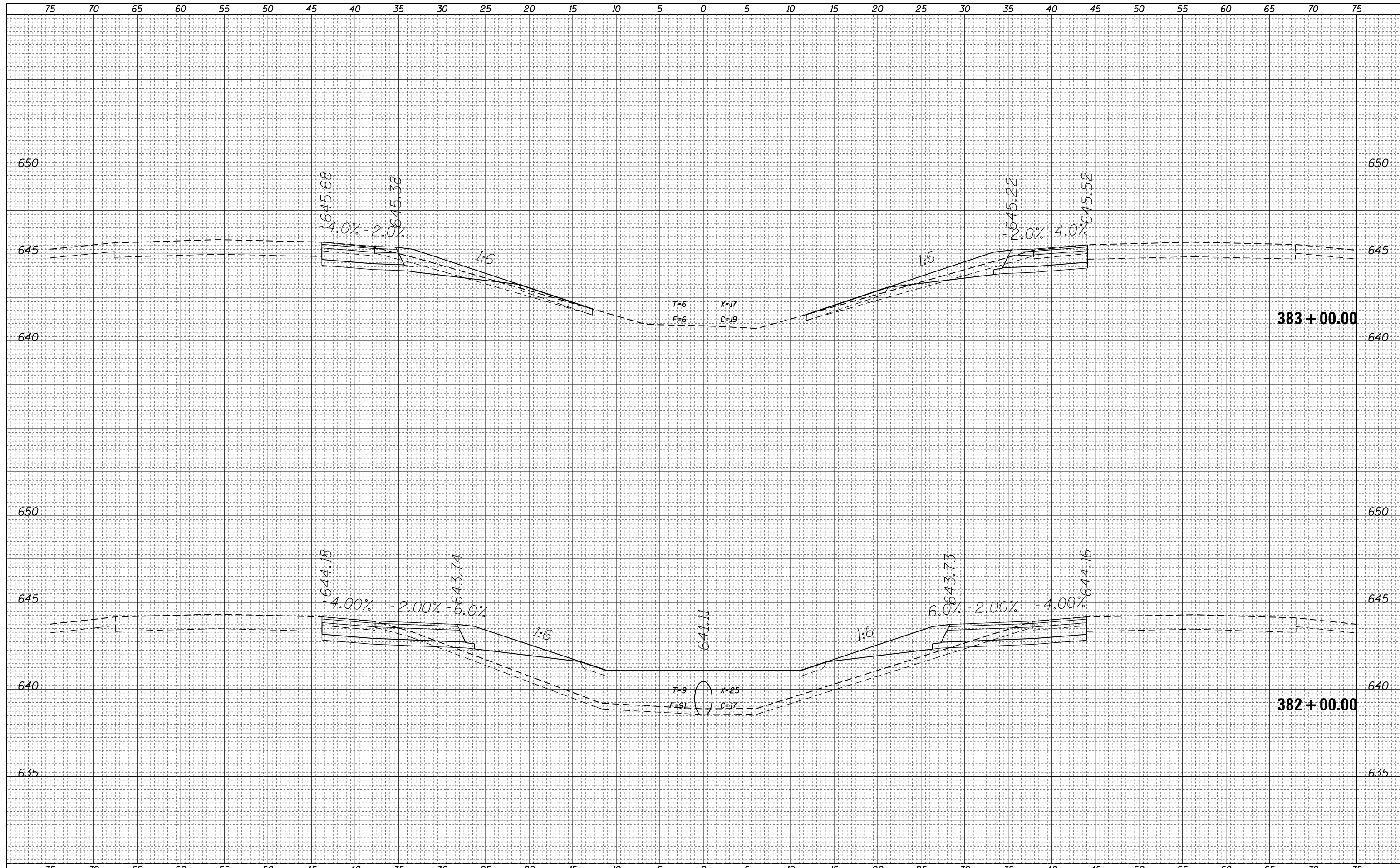
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CROSS SECTIONS
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
322	(58-20B-1)BR	MACON	122	119
CONTRACT NO. 74351			ILLINOIS FED. AID PROJECT	

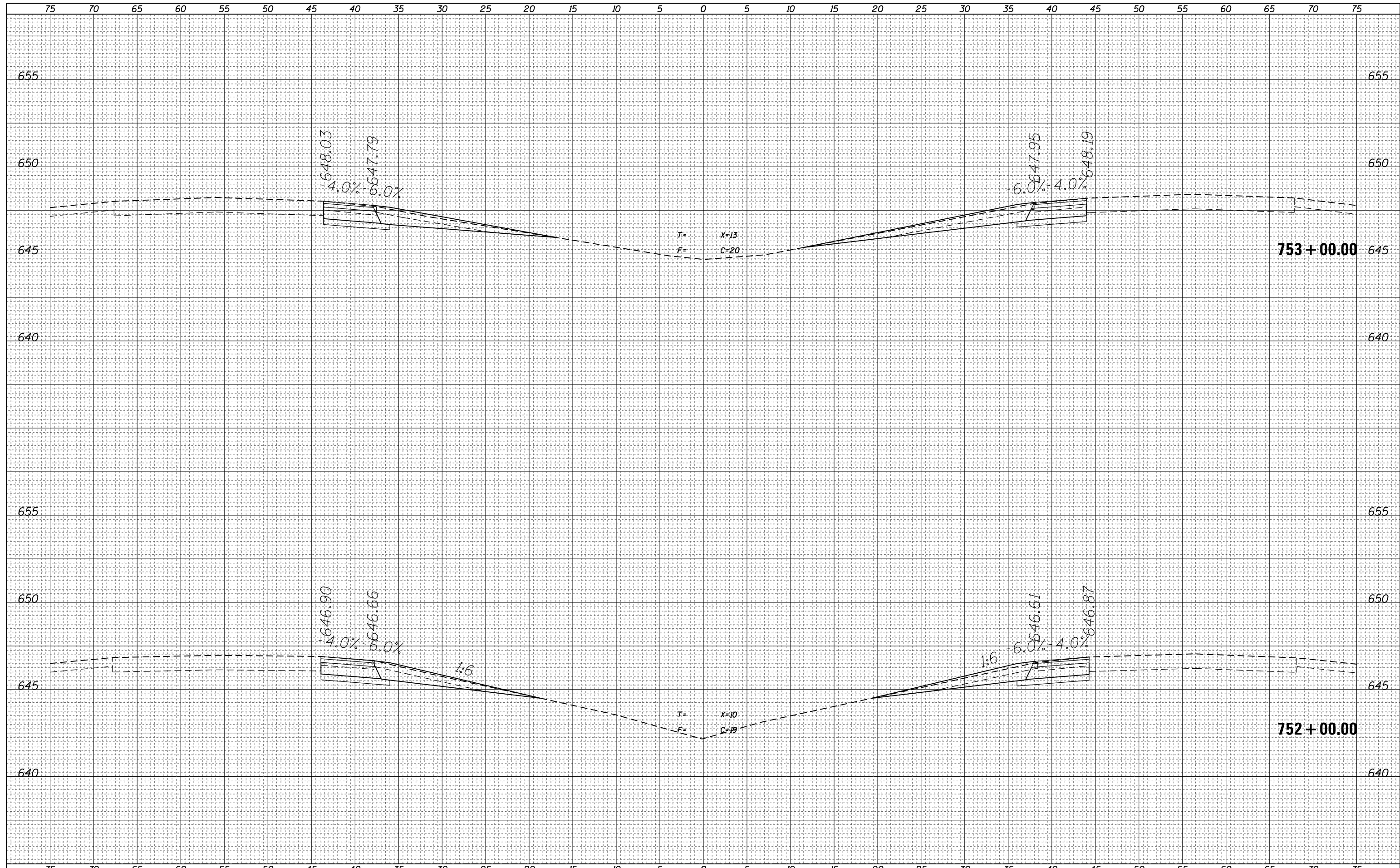
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ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
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	AREAS CHECKED



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FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED

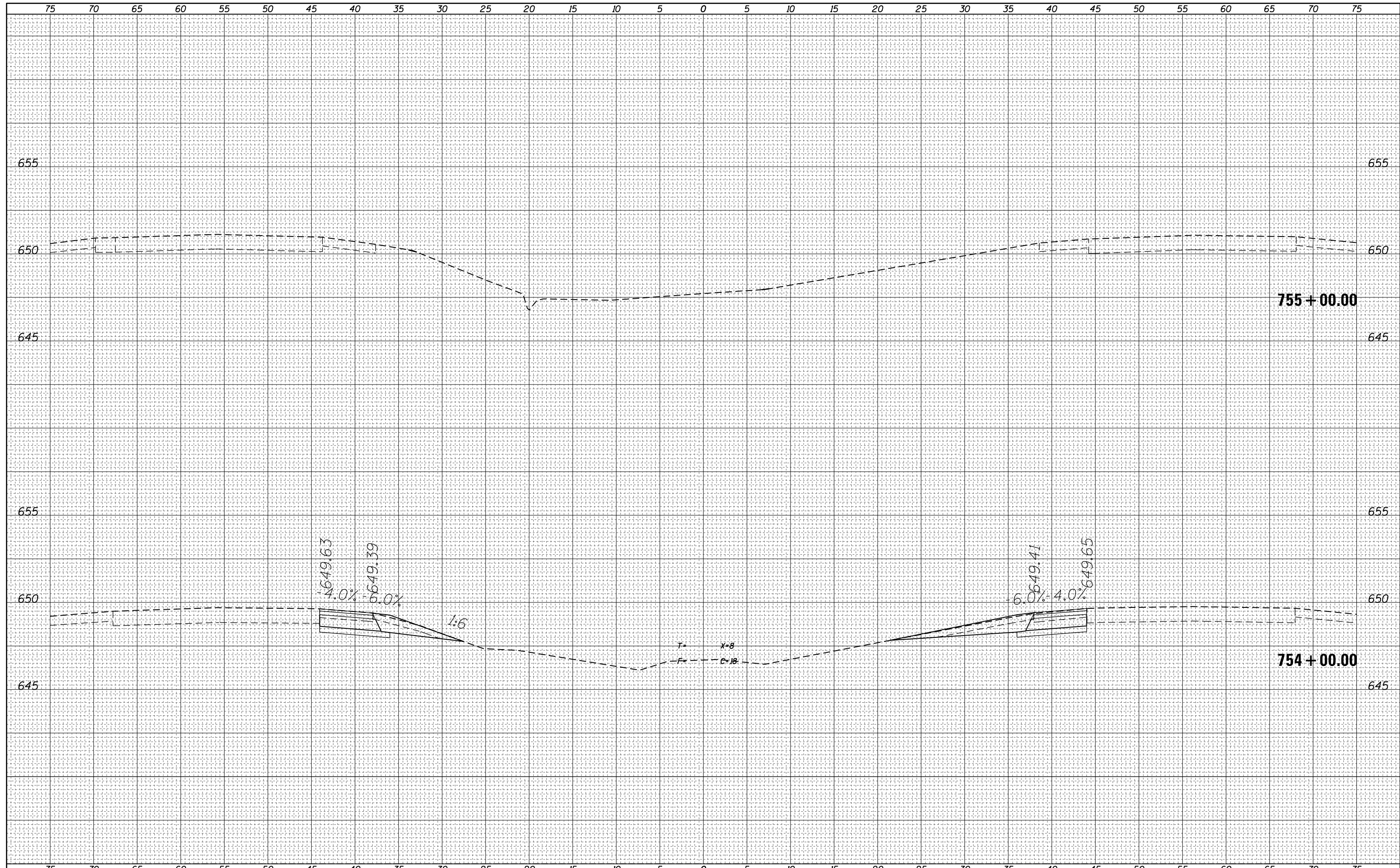
DATE	
BY	
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED



FILE NAME =	USER NAME = steffemk	DESIGNED -	REVISIED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CROSS SECTIONS SCALE: SHEET OF SHEETS STA. 752+00.00 TO STA. 753+00.00	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
Default	DOT Offices\District 7\Projects\74351\CADD\Drawings\Drawings\74351-sht-xxsht	REVISIED -	REVISIED -			322	(58-20B-1)BR	MACON	122	121	
	PLOT SCALE = 10.0000' / in.	CHECKED -	REVISIED -			CONTRACT NO. 74351					
	PLOT DATE = 3/20/2015	DATE -	REVISIED -			ILLINOIS FED. AID PROJECT					

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
FINAL SURVEY	
NOTE BOOK	
NO.	

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
ORIGINAL SURVEY	
NOTE BOOK	
NO.	



FILE NAME =	USER NAME = steffennk	DESIGNED -	REVISIED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CROSS SECTIONS SCALE: SHEET OF SHEETS STA. 754+00.00 TO STA. 755+00.00	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
Default	DOT Offices\District 7\Projects\74351\CADD\Drawings\Drawings\74351-sht-xxsht	REVISIED -	REVISIED -			322	(58-20B-1)BR	MACON	122	122	
	PLOT SCALE = 10.0000" / in.	CHECKED -	REVISIED -			CONTRACT NO. 74351					
	PLOT DATE = 3/20/2015	DATE -	REVISIED -			ILLINOIS FED. AID PROJECT					