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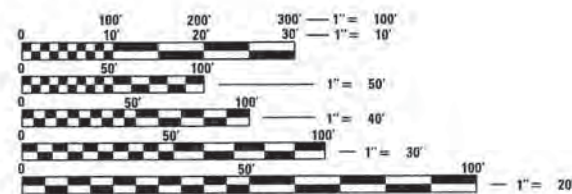
HIGHWAY STANDARDS SEE SHEET 2

**UTILITIES**

EGYPTIAN ELECTRIC COOP  
10169 OLD HIGHWAY 3  
MURPHYSBORO, IL 62966  
ATTN: BROOKE GUTHMAN  
618-684-2143

AMEREN ILLINOIS  
1050 WEST BOULEVARD  
BELLEVILLE, IL 62221  
ATTN: MARTIN FULLER  
618-236-6281

FRONTIER COMMUNICATIONS  
208 W UNION STREET  
MARION, IL 62959  
ATTN: TERRY PHILLIPS  
618-997-0679



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

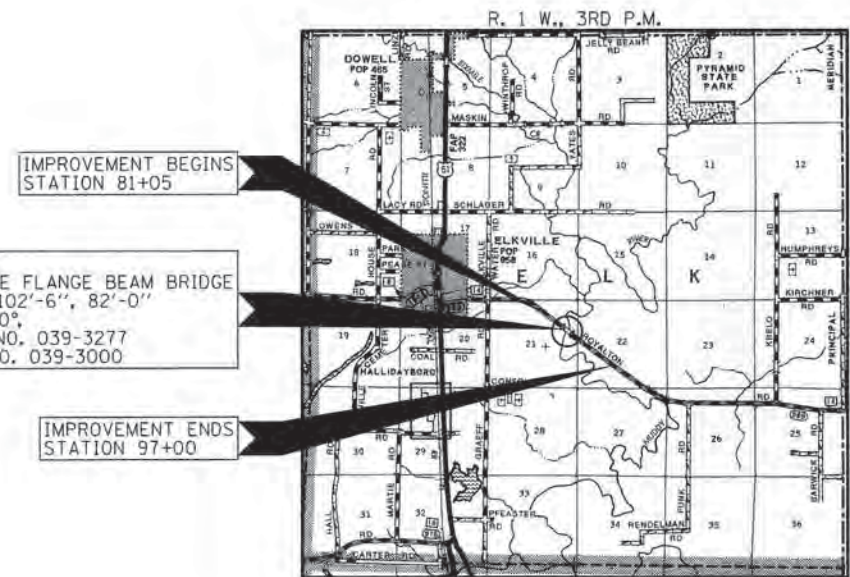
**STATE OF ILLINOIS**  
**DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**PLANS FOR PROPOSED**  
**SURFACE TRANSPORTATION PROGRAM**  
**SYSTEM BRIDGE**

**PROJECT BRS-0869(106)**  
**SECTION 10-00163-00-BR**  
**JACKSON COUNTY**  
**F.A.S. 869 / C.H. 14 / ROYALTON ROAD**  
**PROPOSED STRUCTURE NO. 039-3277**  
**C-99-542-11**

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
C.H. 14	10-00163-00-BR	JACKSON	82	1
FED. ROAD DIST. NO.		ILLINOIS CONTRACT NO.	99519	



FUNCTIONAL CLASSIFICATION: MAJOR COLLECTOR  
DESIGN SPEED: 50 MPH  
DESIGN TRAFFIC: 1695 ADT (2015)



STA. 84+22.00  
CONTINUOUS STEEL WIDE FLANGE BEAM BRIDGE  
THREE SPANS: 82'-0", 102'-6", 82'-0"  
30'-0" RDWY., SKEW = 0°  
PROPOSED STRUCTURE NO. 039-3277  
EXISTING STRUCTURE NO. 039-3000

**LOCATION MAP**  
APPROXIMATE SCALE: 0 1 MILE  
NET LENGTH OF SECTION = 1595 FEET = 0.302 MILES

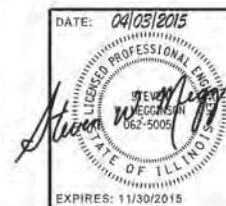


**ILLINOIS DEPARTMENT OF TRANSPORTATION**

APPROVED: *[Signature]* 4-8 2015  
COUNTY ENGINEER

PASSED: *[Signature]* April 9, 2015  
DISTRICT NINE ENGINEER OF LOCAL ROADS & STREETS

Releasing For Bid Based on Limited Review: *[Signature]* 4/9 2015  
DEPUTY DIRECTOR OF HIGHWAYS  
REGION FIVE ENGINEER  
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION



**HAMPTON, LENZINI AND RENWICK, INC.**  
CIVIL ENGINEERS - STRUCTURAL ENGINEERS - LAND SURVEYORS  
**HLR**  
3085 STEVENSON DRIVE, SUITE 201  
SPRINGFIELD, ILLINOIS 62703  
217.546.3400 www.hlrengineering.com  
184.000959  
ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORPORATION  
PROJECT NUMBER: 13.0340.130 DATE: 04/03/15

**GENERAL NOTES**

- ALL CONSTRUCTION SHALL BE DONE IN ACCORDANCE WITH THE STATE OF ILLINOIS "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, ADOPTED JANUARY 1, 2012," THESE PLANS AND THE SPECIAL PROVISIONS INCLUDED IN THE CONTRACT DOCUMENTS.
- THE LOCATIONS OF EXISTING GAS MAINS, ELECTRIC POWER LINES, TELEPHONE LINES AND OTHER UTILITIES AS SHOWN ON THE PLANS ARE BASED ON CAREFUL FIELD INVESTIGATION AND THE BEST INFORMATION AVAILABLE, BUT THE LOCATIONS ARE NOT GUARANTEED. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ASCERTAIN THEIR EXACT LOCATION FROM THE INDIVIDUAL UTILITY COMPANIES AND BY FIELD INSPECTION.
- THE AREA TO BE SEEDED SHALL CONSIST OF ALL DISTURBED EARTH SURFACES WITHIN THE RIGHT OF WAY OR AS DIRECTED BY THE ENGINEER.  
**SEEDING, CLASS 2 (SPECIAL) = 2.25 ACRES**
- THE FOLLOWING RATES OF APPLICATION HAVE BEEN USED IN CALCULATING PLAN QUANTITIES:

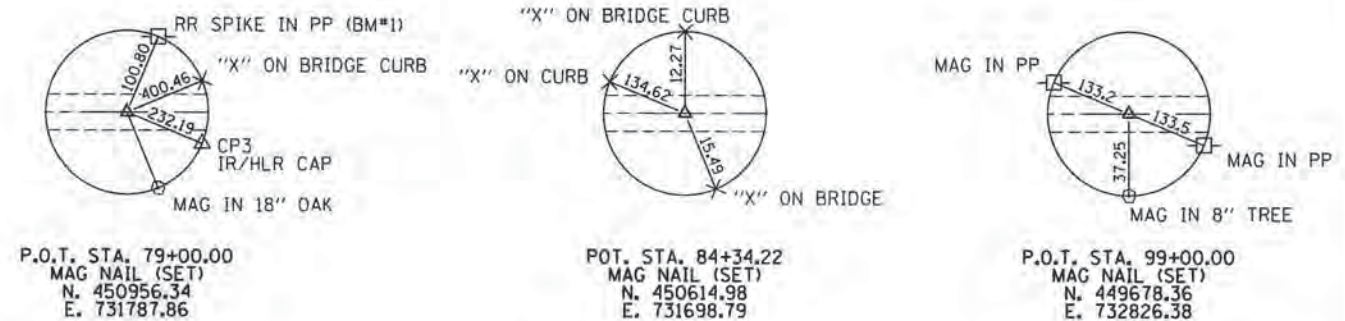
ALL HOT MIX ASPHALT	112 LBS/SQ. YD./INCH OF THICKNESS
ALL AGGREGATE	2.05 TONS/CU. YD.
BITUMINOUS MATERIALS:	
ON PAVEMENT	0.05 LB./SQ. FT.
INTERMEDIATE LIFTS (FOG COAT)	0.025 LB./SQ. FT.
ON AGGREGATE SURFACE	0.25 LB./SQ. FT.

- IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL EXISTING FIELD DIMENSIONS AND CONDITIONS PRIOR TO CONSTRUCTION AND ORDERING MATERIALS.
- AT ALL LOCATIONS WHERE THE PROPOSED HOT MIX ASPHALT OR CONCRETE PAVEMENT JOINS AN EXISTING HOT MIX ASPHALT OR CONCRETE PAVEMENT, A FULL DEPTH SAWED JOINT SHALL BE CONSTRUCTED. THE COST OF THIS JOINT WILL BE INCLUDED IN THE COST OF THE TYPE OF PAVEMENT BEING CONSTRUCTED.
- VERTICAL PANELS SHOWN ON STANDARD 701321 WILL NOT BE REQUIRED ON THE STAGE II NEW BRIDGE PARAPET. THE BARRIER WALL REFLECTORS SHALL BE INSTALLED PRIOR TO OPENING TO TRAFFIC.
- ANY TIME THE CONCRETE BARRIER IS NOT IN THE PROPER POSITION, FLAGGERS SHALL BE IN PLACE TO CONTROL TRAFFIC. THE TEMPORARY TRAFFIC SIGNALS SHALL BE TURNED OFF OR COVERED.
- ALL TEMPORARY EROSION CONTROL MEASURES SHALL BE LEFT IN PLACE UNTIL REMOVAL IS REQUIRED TO CONSTRUCT FINAL GRADE LINES.
- TREES WITHIN THE RIGHT-OF-WAY WHICH INTERFERE WITH CONSTRUCTION SHALL BE REMOVED ONLY AT THE DIRECTION OF THE ENGINEER. ANY TREES ALREADY FELLED SHALL BE MEASURED AND REMOVED AS PER THE SPECIAL PROVISIONS.
- TREE REMOVAL IS PROHIBITED BETWEEN APRIL 1 AND SEPTEMBER 30.

**HIGHWAY STANDARDS**

- 000001-06 STANDARD SYMBOLS, ABBREVIATIONS, AND PATTERNS
- 001001-02 AREAS OF REINFORCEMENT BARS
- 001006 DECIMAL OF AN INCH AND OF A FOOT
- 280001-07 TEMPORARY EROSION CONTROL SYSTEMS
- 4200401-11 BRIDGE APPROACH PAVEMENT CONNECTOR
- 515001-03 NAME PLATE FOR BRIDGES
- 630001-10 STEEL PLATE BEAM GUARDRAIL
- 630301-06 SHOULDER WIDENING FOR TYPE 1, (SPECIAL) GUARDRAIL TERMINALS
- 631032-08 TRAFFIC BARRIER TERMINAL, TYPE 6A
- 635006-03 REFLECTOR AND TERMINAL MARKER PLACEMENT
- 635011-02 REFLECTOR MARKER AND MOUNTING DETAILS
- 642006-02 SHOULDER RUMBLE STRIPS, 8 IN
- 701001-02 OFF-ROAD OPERATIONS 2L, 2W, MORE THAN 4.5M (15') AWAY
- 701006-05 OFF-ROAD OPERATIONS 2L, 2W, 4.5M (15') TO 600 MM (24") FROM PAVEMENT EDGE
- 701011-04 OFF-ROAD MOVING OPERATIONS 2L, 2W, DAY ONLY
- 701201-04 LANE CLOSURE, 2L, 2W, DAY ONLY, FOR SPEEDS > 45 MPH
- 701301-04 LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS
- 701306-03 LANE CLOSURE, 2L, 2W, SLOW MOVING OPERATIONS - DAY ONLY FOR SPEEDS > 45 MPH
- 701311-03 LANE CLOSURE, 2L, 2W, MOVING OPERATIONS, DAY ONLY
- 701321-14 LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER
- 701326-04 LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING FOR SPEEDS > 45 MPH
- 701901-04 TRAFFIC CONTROL DEVICES
- 704001-07 TEMPORARY CONCRETE BARRIER
- 780001-05 TYPICAL PAVEMENT MARKINGS

**BENCHMARKS AND TIES**



- BM\*1 - RR SPIKE IN POWER POLE  
40' LT., STA. 79+93  
ELEV. 389.38
- BM\*2 - CHISELED "X" ON N.W. CURB  
13' LT., STA. 83+00  
ELEV. 388.92
- BM\*3 - RR SPIKE IN POWER POLE  
60' RT., STA. 96+94  
ELEV. 376.83

HMA MIXTURE REQUIREMENTS				
LOCATION(S):	FAS 689/CH14/ROYALTON ROAD	FAS 689/CH14/ROYALTON ROAD	FAS 689/CH14/ROYALTON ROAD	FAS 689/CH14/ROYALTON ROAD
MIXTURE USE(S):	HMA BASE COURSE	HMA SURFACE COURSE	HMA BINDER COURSE	HMA SHOULDERS
PG:	PG 64-22	PG 64-22	PG 64-22	PG 58-22
DESIGN AIR VOIDS:	4% @ Ndes 70	4% @ Ndes 70	4% @ Ndes 70	2% @ Ndes 70
MIXTURE COMPOSITION: (MIXTURE GRADATION):	IL 19.0	IL 9.5	IL 19.0	HMA SHOULDERS
FRICTION AGGREGATE:	NONE	MIXTURE C	NONE	NONE
MIXTURE WEIGHTS:	112 LBS \ SY \ INCH THICKNESS	112 LBS \ SY \ INCH THICKNESS	112 LBS \ SY \ INCH THICKNESS	112 LBS \ SY \ INCH THICKNESS

SUMMARY OF QUANTITIES				
ITEM NO.	CODE NO.	ITEM	CONSTRUCTION TYPE CODE 0011	
			UNIT	QUANTITY
1	20100500	TREE REMOVAL, ACRES	ACRE	0.5
2	20200100	EARTH EXCAVATION	CU YD	1586
3	20300100	CHANNEL EXCAVATION	CU YD	820
4	20400800	FURNISHED EXCAVATION	CU YD	2984
5	25100115	MULCH, METHOD 2	ACRE	4.5
6	25100630	EROSION CONTROL BLANKET	SQ YD	895
7	28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	432
8	28000400	PERIMETER EROSION BARRIER	FOOT	3123
9	28100207	STONE RIPRAP, CLASS A4	TON	1486
10	28200200	FILTER FABRIC	SQ YD	1601
11	30201500	LIME	TON	144
12	35100100	AGGREGATE BASE COURSE, TYPE A	TON	464
13	35501312	HOT-MIX ASPHALT BASE COURSE, 7"	SQ YD	1108
14	40600275	BITUMINOUS MATERIALS (PRIME COAT)	POUND	3338
15	40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SQ YD	120
16	40600990	TEMPORARY RAMP	SQ YD	68
17	40603085	HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N70	TON	37
18	40603315	HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N70	TON	258
19	42001430	BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE)	SQ YD	40
20	44300200	STRIP REFLECTIVE CRACK CONTROL TREATMENT	FOOT	183
21	48101600	AGGREGATE SHOULDERS, TYPE B 8"	SQ YD	666
22	48203029	HOT-MIX ASPHALT SHOULDERS, 8"	SQ YD	505
23	50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1
24	50104650	SLOPE WALL REMOVAL	SQ YD	686
25	50200100	STRUCTURE EXCAVATION	CU YD	188
26	50200300	COFFERDAM EXCAVATION	CU YD	328
27	50201121	COFFERDAM (TYPE 2) (LOCATION - 1)	EACH	1
28	50201122	COFFERDAM (TYPE 2) (LOCATION - 2)	EACH	1
29	50300225	CONCRETE STRUCTURES	CU YD	219.2
30	50300255	CONCRETE SUPERSTRUCTURE	CU YD	243.5

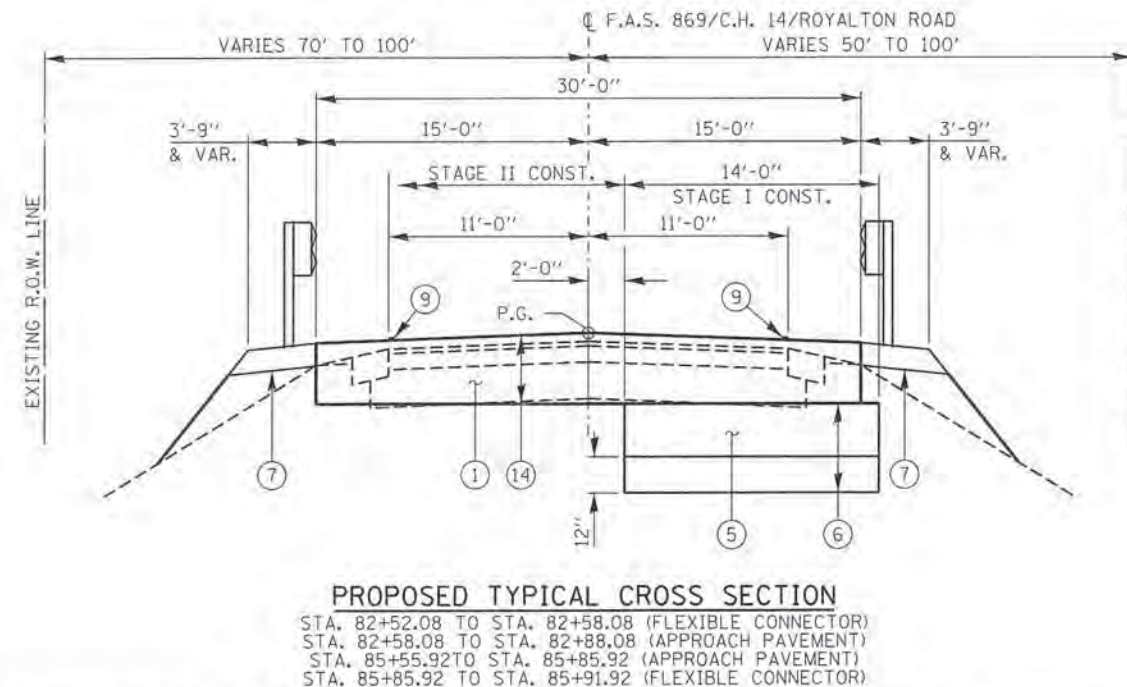
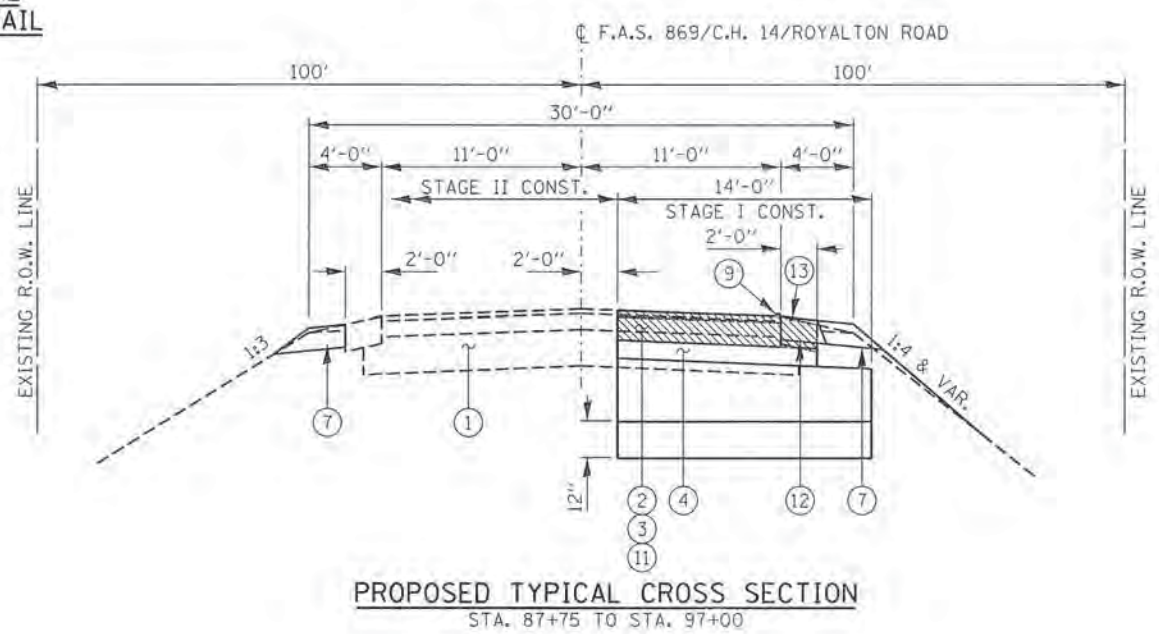
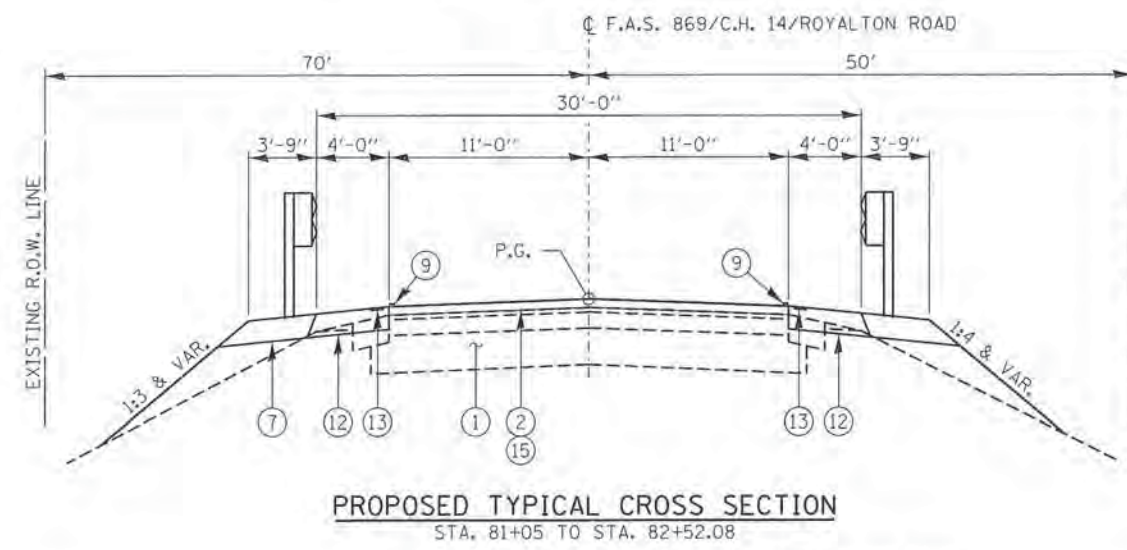
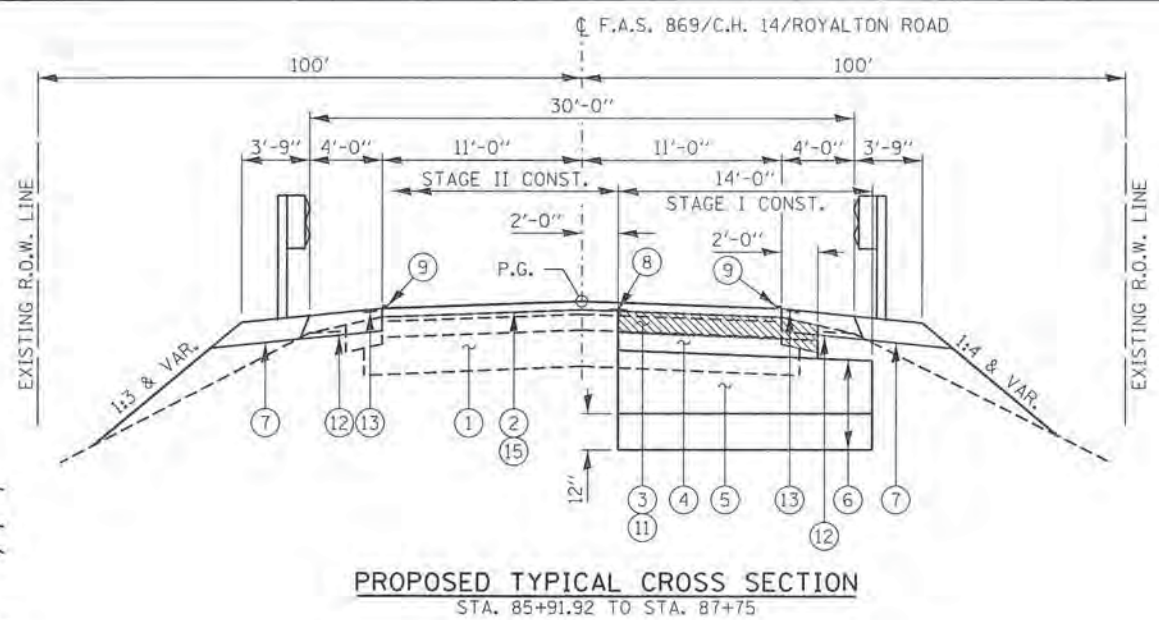
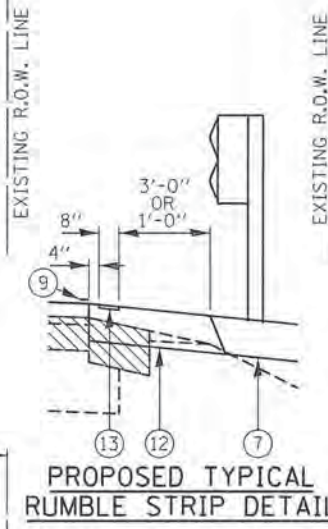
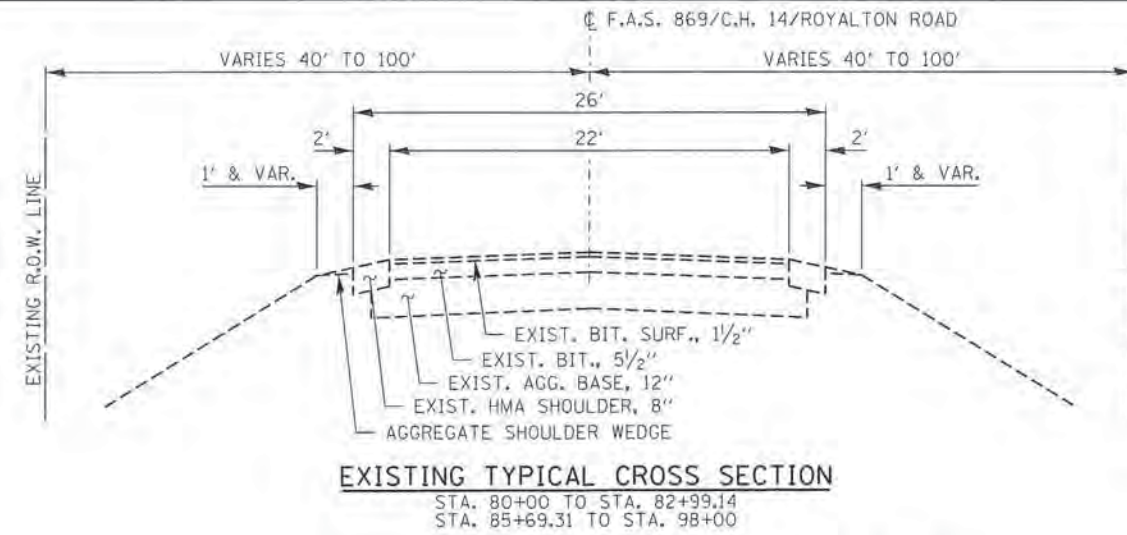
^ SEE SPECIAL PROVISIONS

SUMMARY OF QUANTITIES				
ITEM NO.	CODE NO.	ITEM	CONSTRUCTION TYPE CODE 0011	
			UNIT	QUANTITY
31	50300260	BRIDGE DECK GROOVING	SQ YD	1020
32	50300300	PROTECTIVE COAT	SQ YD	1259
33	50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1
34	50500505	STUD SHEAR CONNECTORS	EACH	4338
35	50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	84490
36	50800515	BAR SPLICERS	EACH	1096
* 37	50901050	STEEL RAILING, TYPE SM	FOOT	596
38	51202100	FURNISHING STEEL PILES HP14X117	FOOT	1332
39	51202305	DRIVING PILES	FOOT	1332
40	51204100	TEST PILE STEEL HP14X117	EACH	4
41	51204650	PILE SHOES	EACH	24
42	51500100	NAME PLATES	EACH	1
43	52000110	PREFORMED JOINT STRIP SEAL	FOOT	62
44	52100510	ANCHOR BOLTS, 3/4"	EACH	24
45	52100520	ANCHOR BOLTS, 1"	EACH	24
46	59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	58
* 47	63000003	STEEL PLATE BEAM GUARDRAIL, TYPE A, 9 FOOT POSTS	FOOT	100
* 48	63100087	TRAFFIC BARRIER TERMINAL, TYPE 6A	EACH	4
* 49	63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	4
* 50	63200310	GUARDRAIL REMOVAL	FOOT	464
51	64200108	SHOULDER RUMBLE STRIPS, 8 INCH	FOOT	1585
52	67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	9
53	67100100	MOBILIZATION	L SUM	1
^ 54	70100405	TRAFFIC CONTROL AND PROTECTION, STANDARD 701321	EACH	1
^ 55	70100450	TRAFFIC CONTROL AND PROTECTION, STANDARD 701201	L SUM	1
^ 56	70100460	TRAFFIC CONTROL AND PROTECTION, STANDARD 701306	L SUM	1
^ 57	70100500	TRAFFIC CONTROL AND PROTECTION, STANDARD 701326	L SUM	1
58	70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	10
* 59	70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	1
60	70106800	CHANGEABLE MESSAGE SIGN	CAL MO	2

^ SEE SPECIAL PROVISIONS

SUMMARY OF QUANTITIES				
ITEM NO.	CODE NO.	ITEM	CONSTRUCTION TYPE CODE 0011	
			UNIT	QUANTITY
61	70300100	SHORT TERM PAVEMENT MARKING	FOOT	72
62	70300220	TEMPORARY PAVEMENT MARKING - LINE 4"	FOOT	4392
63	70301000	WORK ZONE PAVEMENT MARKING REMOVAL	SQ FT	1479
64	70400100	TEMPORARY CONCRETE BARRIER	FOOT	1625
^ 65	70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	588
66	70600250	IMPACT ATTENUATORS, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2
67	70600350	IMPACT ATTENUATORS, RELOCATE (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2
* 68	78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	4392
* 69	78200410	GUARDRAIL MARKERS, TYPE A	EACH	14
* ^ 70	78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	4
71	78300100	PAVEMENT MARKING REMOVAL	SQ FT	791
* 72	86200300	UNINTERRUPTABLE POWER SUPPLY, EXTENDED	EACH	1
^ 73	X2501000	SEEDING, CLASS 2 (SPECIAL)	ACRE	2.25
^ 74	X4404400	PAVEMENT REMOVAL (SPECIAL)	SQ YD	1382
^ 75	X5030305	CONCRETE WEARING SURFACE, 5"	SQ YD	200
^ 76	X5040100	PRECAST BRIDGE APPROACH SLAB	SQ FT	1749
^ 77	X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	128
^ 78	Z0046304	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	124
^ 79	Z0026407	TEMPORARY SHEET PILING	SQ FT	531
80	Z0013798	CONSTRUCTION LAYOUT	L SUM	1
81	Z001900	PROCESSING LIME STABILIZED SOIL MIXTURE 36"	SQ YD	1778
	Z001600	TRAINEES	hr	500
	Z001600	TRAINEES TRAINING PROGRAM GRADUATE	hr	500

^ SEE SPECIAL PROVISIONS  
\* SPECIALTY ITEMS



- LEGEND**
- ① EXISTING PAVEMENT
  - ② HMA SURFACE COURSE MIX "C", N70, 2 1/4"
  - ③ HMA BASE COURSE, 7"
  - ④ AGGREGATE BASE COURSE, TYPE A, 6"
  - ⑤ EARTH EXCAVATION
  - ⑥ LIME STABILIZED SOIL, 36"
  - ⑦ AGGREGATE SHOULDERS, 8"
  - ⑧ STRIP REFLECTIVE CRACK CONTROL TREATMENT
  - ⑨ PAINT PAVEMENT MARKING - LINE 4"
  - ⑩ TBT TYPE 6A, SPBGR TY. A OR TBT TY. 1 (SPL) TANGENT
  - ⑪ PAVEMENT REMOVAL, SPECIAL
  - ⑫ HMA SHOULDERS 8"
  - ⑬ SHOULDER RUMBLE STRIP 8" (8" WIDE, 1/2" DEEP RUMBLE STRIP AT LOCATIONS AS DIRECTED BY ENGINEER)
  - ⑭ PRECAST BRIDGE APPROACH SLAB
  - ⑮ HMA BINDER COURSE, IL 19.0, N70

FILE NAME = 138348-shr-typsections.dgn	USER NAME *	DESIGNED - L.F.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>TYPICAL CROSS SECTIONS C.H. 14 / ROYALTON ROAD</b>			F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.		
HAMPTON, LENZINI AND RENWICK, INC. 208 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62709 ILLINOIS PROFESSIONAL DESIGN #1334 L.S. PEI & CO., INC. #84-00098	PLOT SCALE *	DRAWN - D.A.B.	REVISED -		SCALE:	SHEET NO.	OF SHEETS	STA.	TO STA.	869	10-00163-00-BR	JACKSON	82	4
	PLOT DATE = 4/6/2015	CHECKED - J.W.F.	REVISED -											
		DATE - 04/03/15	REVISED -											
										CONTRACT NO. 99519			ILLINOIS FED. AID PROJECT	

ROADWAY SCHEDULE															
LOCATION	PROCESSING LIME STABILIZED SOIL MIXTURE 36"	LIME	AGGREGATE BASE COURSE TYPE A	HOT-MIX ASPHALT BASE COURSE 7"	BITUMINOUS MATERIALS (PRIME COAT)	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	TEMPORARY RAMP	HOT-MIX ASPHALT BINDER COURSE IL 19.0, N70	HOT-MIX ASPHALT SURFACE COURSE MIX "C", N50 (2.25")	BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE)	STRIP REFLECTIVE CRACK CONTROL TREATMENT	AGGREGATE SHOULDERS TYPE B 8"	HOT-MIX ASPHALT SHOULDERS 8"	SHOULDER RUMBLE STRIPS 8 INCH	PAVEMENT REMOVAL (SPECIAL)
		30201500	35100100	35501312	40600275	40600982	40600990	40603085	40603315	42001430	44300200	48101600	48203029	64200108	X4404400
	TON	TON	TON	TON	POUND	SQ YD	SQ YD	TON	TON	SQ YD	FOOT	SQ YD	SQ YD	FOOT	SQ YD
FAS 689 \CH14\ROYALTON RD															
STA 81+00 TO STA 82+99.14					180	68	34	17	51	20		131	131	294	
STA 85+69.31 TO STA 97+00	1778	144	464	1108	3158	52	34	20	207	20	183	535	374	1291	1382
TOTAL	1,778	144	464	1,108	3,338	120	68	37	258	40	183	666	505	1,585	1,382

EARTHWORK SUMMARY							
LOCATION	EARTH EXCAVATION	CHANNEL EXCAVATION	SHRINKAGE FACTOR	% USED	EARTH EXCAVATION ADJUSTED FOR SHRINKAGE(25%)	EMBANKMENT REQUIRED	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-)
	20200100	20300100					
	CU YD	CU YD			CU YD	CU YD	CU YD
FAS 689\CH14\ROYALTON ROAD							
STA 81+05 TO STA 82+87.08	17		25.00%	100.00%	13	110	-97
STA 82+87.08 TO STA 85+56.92		820	25.00%	70.00%	574		574
STA 85+56.92 TO STA 97+00	1569		25.00%	100.00%	1177	4638	-3461
TOTAL	1586				1764	4748	-2984
					FURNISHED EXCAVATION	2984	CU.YD.

GUARDRAIL SCHEDULE						
LOCATION	STEEL PLATE BEAM GUARD RAIL TYPE A 9 FOOT POSTS	TRAFFIC BARRIER TERMINAL TYPE 6A	TRAFFIC BARRIER TERMINAL TYPE 1 SPECIAL (TANGENT)	GUARD RAIL REMOVAL	TERMINAL MARKER DIRECT APPLIED	GUARDRAIL MARKERS TYPE A
	63000001	63100087	63100167	63200310	78201000	78200410
	FOOT	EACH	EACH	FOOT	EACH	EACH
RT. STA 81+26.21 TO RT. STA 82+99.60	50	1	1	165	1	
LT. STA 81+76.21 TO LT. STA 82+99.60		1	1	66	1	
RT. STA 85+68.42 TO RT. STA 86+67.80		1	1	66	1	
LT. STA 85+68.42 TO LT. STA 87+17.8	50	1	1	167	1	
RT. STA 81+26.21 TO RT. STA 86+67.80						7
LT. STA 81+76.21 TO LT. STA 87+17.80						7
PROJECT TOTAL	100	4	4	464	4	14

PAVEMENT MARKING SCHEDULE							
LOCATION	TEMPORARY PAVEMENT MARKING - LINE 4"		PAINT PAVEMENT MARKING - LINE 4"		SHORT TERM PAVEMENT MARKING	WORK ZONE PAVEMENT MARKING REMOVAL	PAVEMENT MARKING REMOVAL
	SINGLE WHITE EDGE LINE	SKIPPED DASHED YELLOW CENTERLINE	SINGLE WHITE EDGE LINE	SKIPPED DASHED YELLOW CENTERLINE			
	70300220	70300220	78001110	78001110			
	FOOT	FOOT	FOOT	FOOT	FOOT	SQ FT	SQ FT
FAS 689\CH14\ROYALTON RD							
LT. STA 80+05.69 TO LT. STA 99+58	1,952		1,952			651	546
CL. STA 80+05.69 TO CL. STA 99+58		488		488		163	49
RT. STA 80+05.69 TO RT. STA 99+58	1,952		1,952			651	196
CL. STA 80+05 TO CL. STA 87+75					72	14	
SUBTOTAL	3904	488	3904	488	72	1479	791
TOTAL		4392		4392	72	1479	791

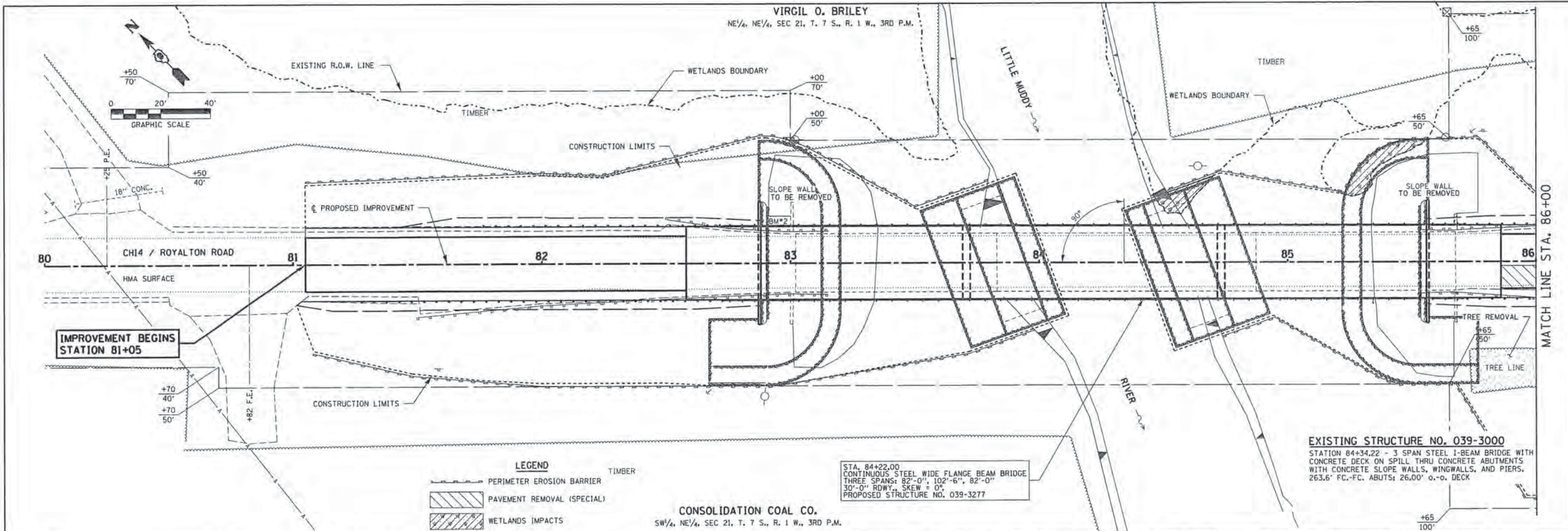
STAGING SCHEDULE					
LOCATION	TEMPORARY CONCRETE BARRIER	RELOCATE TEMPORARY CONCRETE BARRIER	TEMPORARY BRIDGE TRAFFIC SIGNALS	IMPACT ATTENUATORS TEMPORARY (NON-REDIRECTIVE) TEST LEVEL 3	IMPACT ATTENUATORS RELOCATE (NON-REDIRECTIVE) TEST LEVEL 3
	70400100	70400200	70106500	70600250	70600350
	FOOT	FOOT	EACH	EACH	EACH
FAS 689\CH14\ROYALTON RD					
STAGE I					
RT. STA 80+05.69 TO RT. STA 99+58	1625		1	2	
STAGE II					
LT. STA 80+05.69 TO LT. STA 88+50		588			2
TOTAL	1625	588	1	2	2

SEEDING SCHEDULE				
LOCATION	MULCH, METHOD 2 *	EROSION CONTROL BLANKET	TEMPORARY EROSION CONTROL SEEDING *	SEEDING CLASS 2 (SPECIAL)
	25100115	25100630	28000250	X2501000
LOCATION	ACRE	SQ YD	POUND	ACRE
FAS 6891CH14/ROYALTON ROAD				
LT. STA 81+05 TO LT. STA 82+87.08	0.18	292	18	0.09
RT. STA 81+05 TO RT. STA 82+87.08	0.22	0	22	0.11
CL STA 82+87.08 TO STA 85+56.92	0.26	0	26	0.13
LT. STA 85+56.92 TO LT. STA 97+00	0.28	166	28	0.14
RT. STA 85+56.92 TO RT. STA 97+00	3.38	437	338	1.69
TOTAL	4.32	895	432	2.16
USE	4.50	895	432	2.25

\* 2 APPLICATIONS

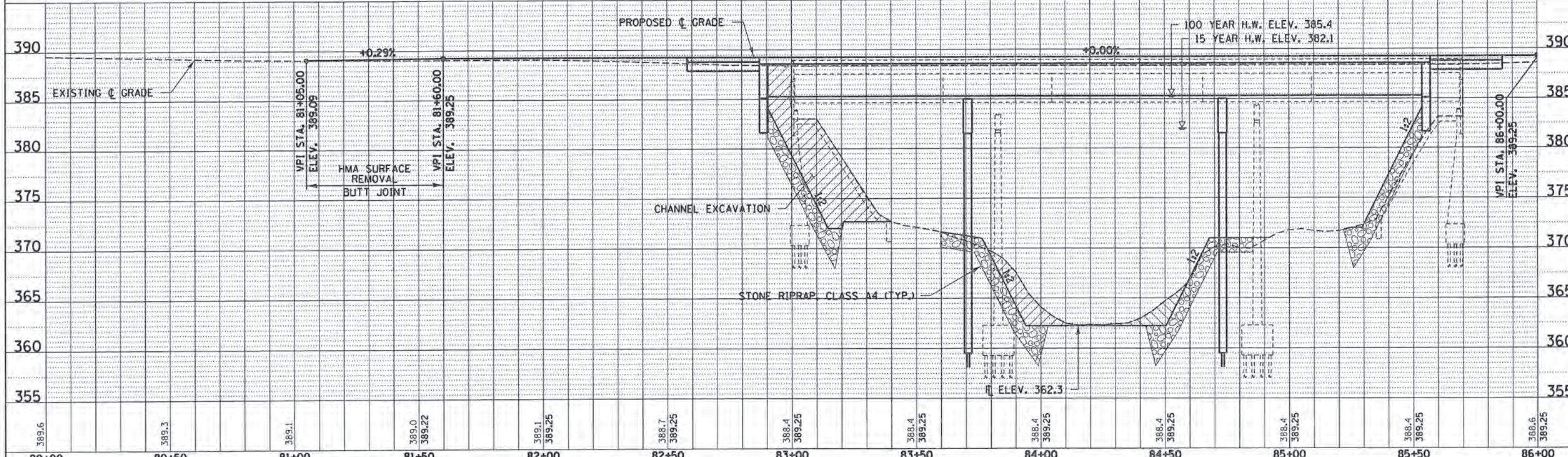
28000400 PERIMETER EROSION BARRIER	
LOCATION	FOOT
FAS 6891CH14/ROYALTON ROAD	
RT. STA 81+05 TO RT. STA 83+00	286
LT. STA 81+05 TO LT. STA 83+00	295
RT. STA 85+69 TO RT. STA 97+00	1,272
LT. STA 85+69 TO LT. STA 97+00	1,270
TOTAL	3123

20100500 TREE REMOVAL	
LOCATION	ACRE
FAS 6891CH14/ROYALTON ROAD	
RT. STA 86+29 TO RT. STA 96+03	0.30
TOTAL	0.30
USE	0.50



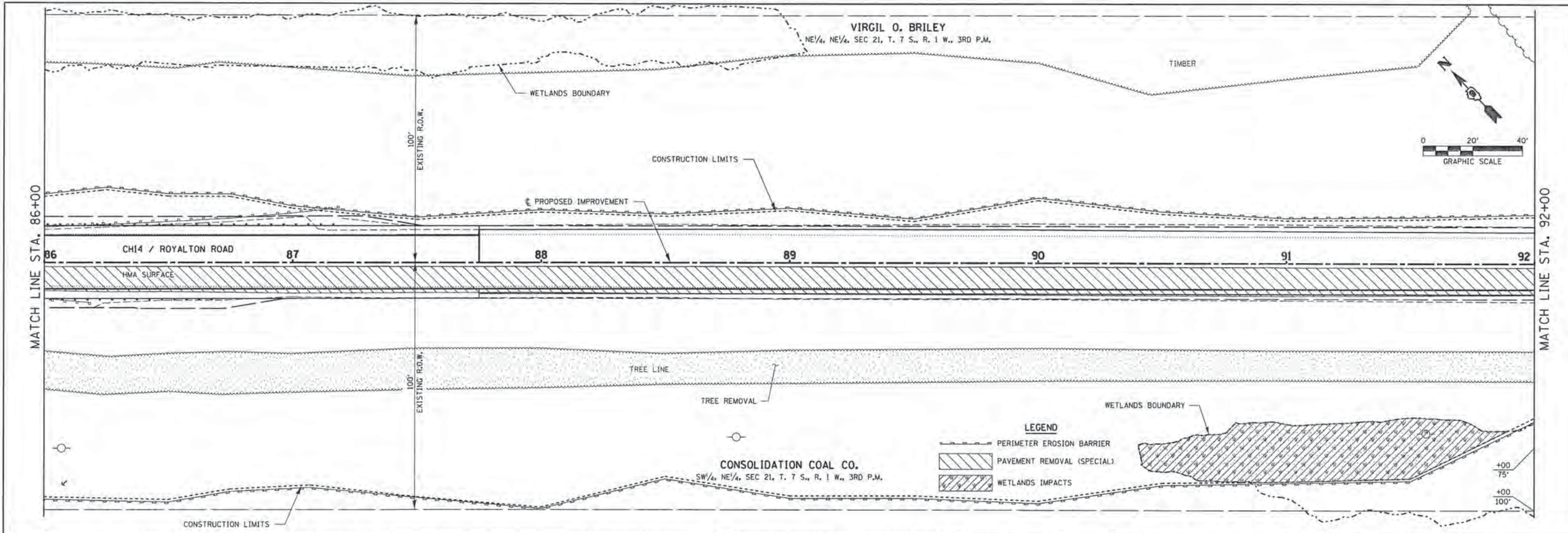
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DRAWN	
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DATE	
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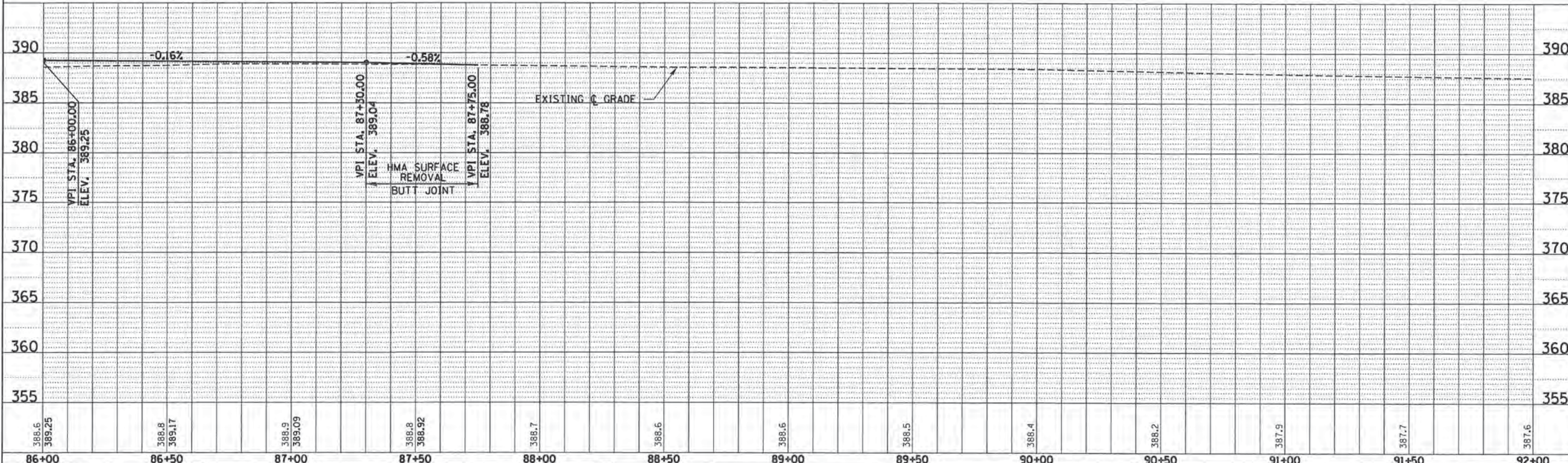


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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - L.G.C.	REVISED -		869	10-00163-00-BR	JACKSON	82	7	
3405 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703		CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519					
ILLINOIS PROFESSIONAL DESIGN FIRM L.E. / P.E. / S.E. CORP. 184-000928		DATE - 02/23/15	REVISED -		ILLINOIS FED. AID PROJECT					

PLAN	SURVEYED	DATE
	PLOTTED	BY
	NOTED	
	REVISIONS	
	NO. OF WAY CHECKED	
	ADD FILE NAME	

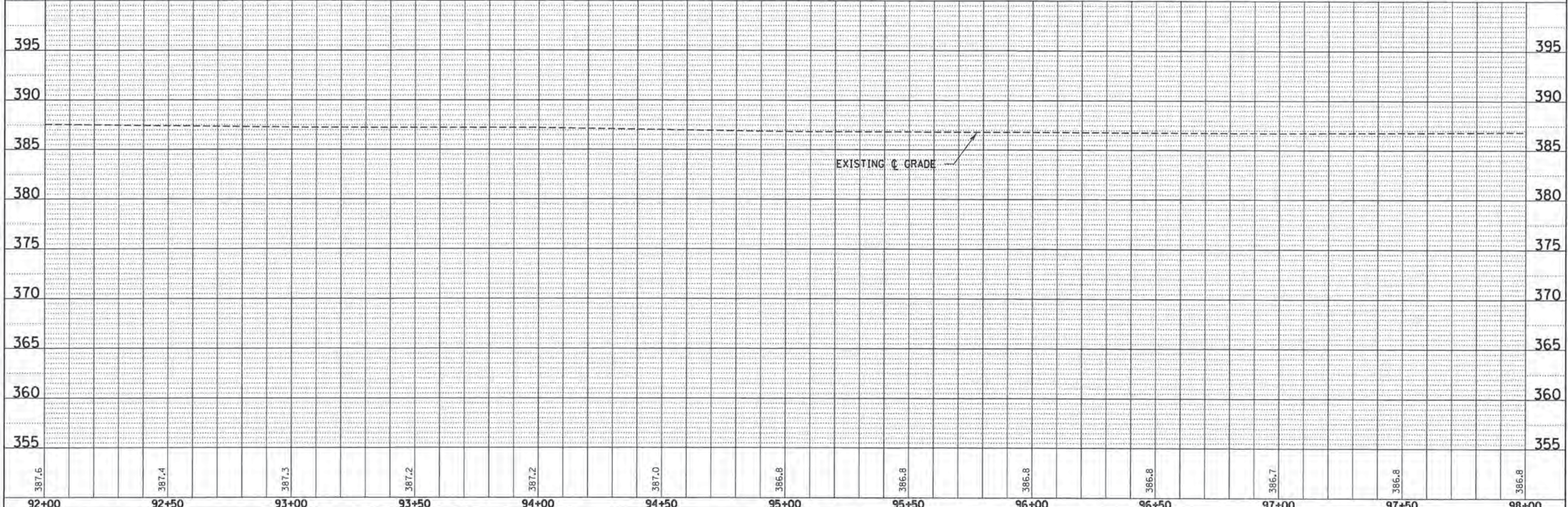
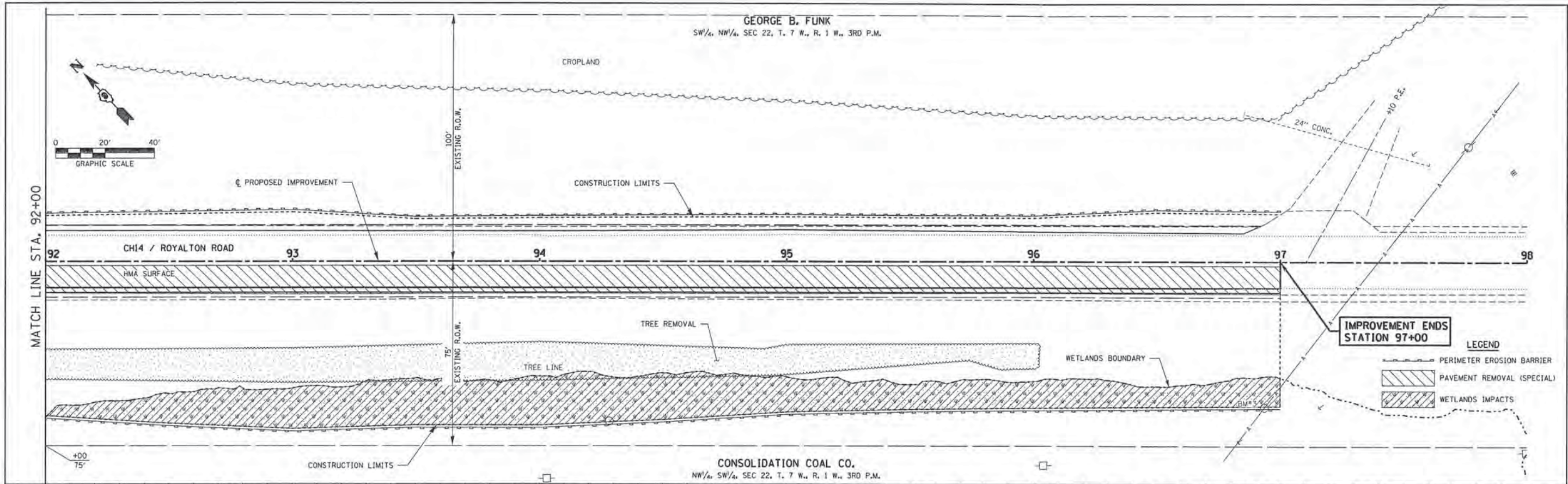


PROFILE	SURVEYED	DATE
	PLOTTED	BY
	NOTED	
	REVISIONS	
	NO. OF WAY CHECKED	
	ADD FILE NAME	



FILE NAME = 130348-shs-p8.dgn	USER NAME =	DESIGNED - L.F.S.	REVISED -	<b>STATE OF ILLINOIS</b> <b>JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>PLAN &amp; PROFILE</b> <b>C.H. 14 / ROYALTON ROAD</b>			F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - L.G.C.	REVISED -		869	10-00163-00-BR	JACKSON	82	8			
3880 ELEVATION DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62793	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519							
ILLINOIS PROFESSIONAL DESIGN FIRM L.S. / P.S. / S.E. CORP. 184.000259	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		ILLINOIS FED. AID PROJECT							

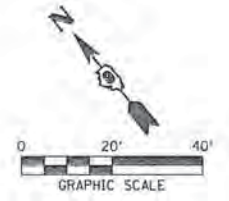
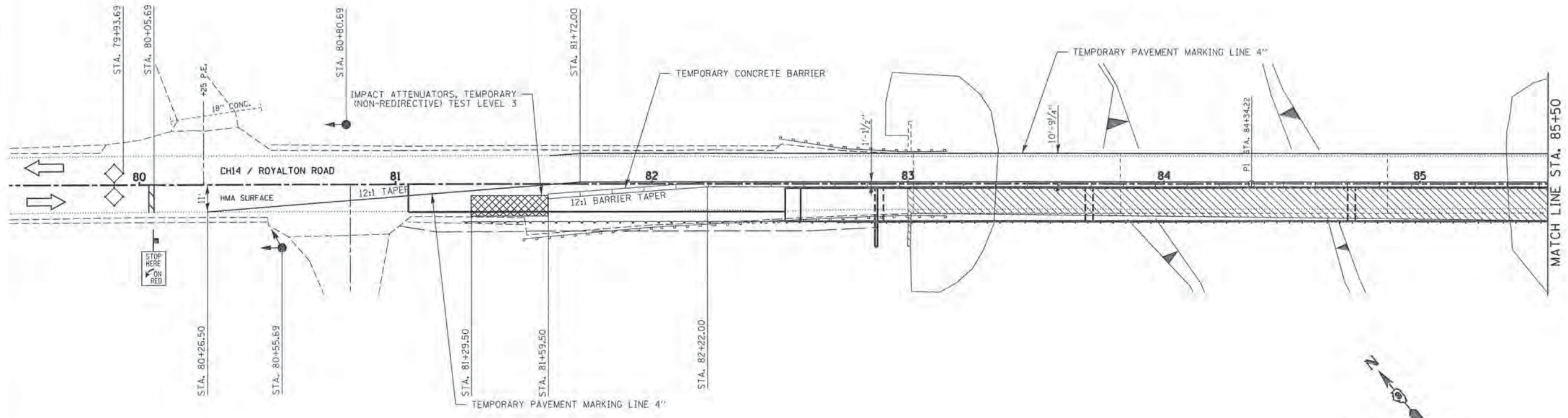




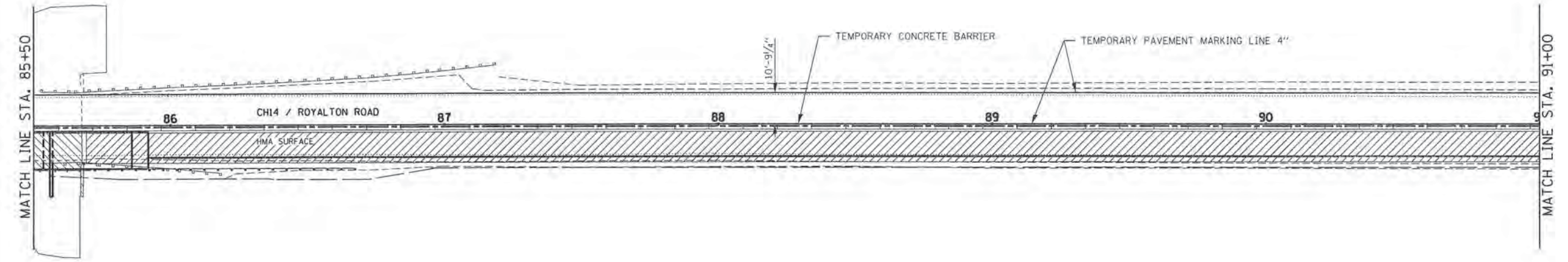
DATE	
BY	
REVISIONS	
NO.	
DESCRIPTION	
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DESCRIPTION	

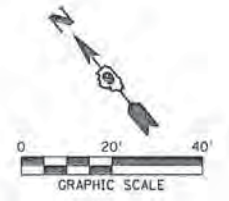
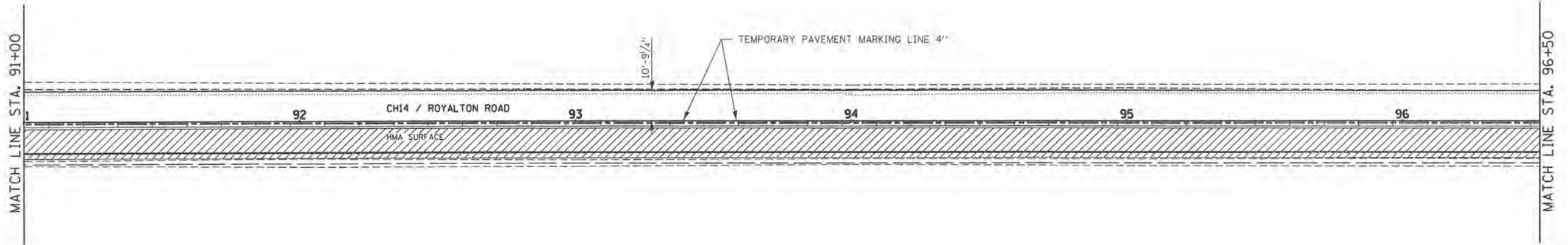
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HAMPTON, LENZINI AND RENWICK, INC.	387.4	DRAWN - L.G.C.	REVISED -		869	10-00163-00-BR	JACKSON	82	9		
387.3	CHECKED - J.W.F.	REVISED -	DATE - 02/23/15		SCALE: 20H:5V	SHEET NO. 3 OF 3 SHEETS	STA. 92+00.00 TO STA. 98+00.00	CONTRACT NO. 99519			
387.2	DATE - 02/23/15	REVISED -			ILLINOIS FED. AID PROJECT						



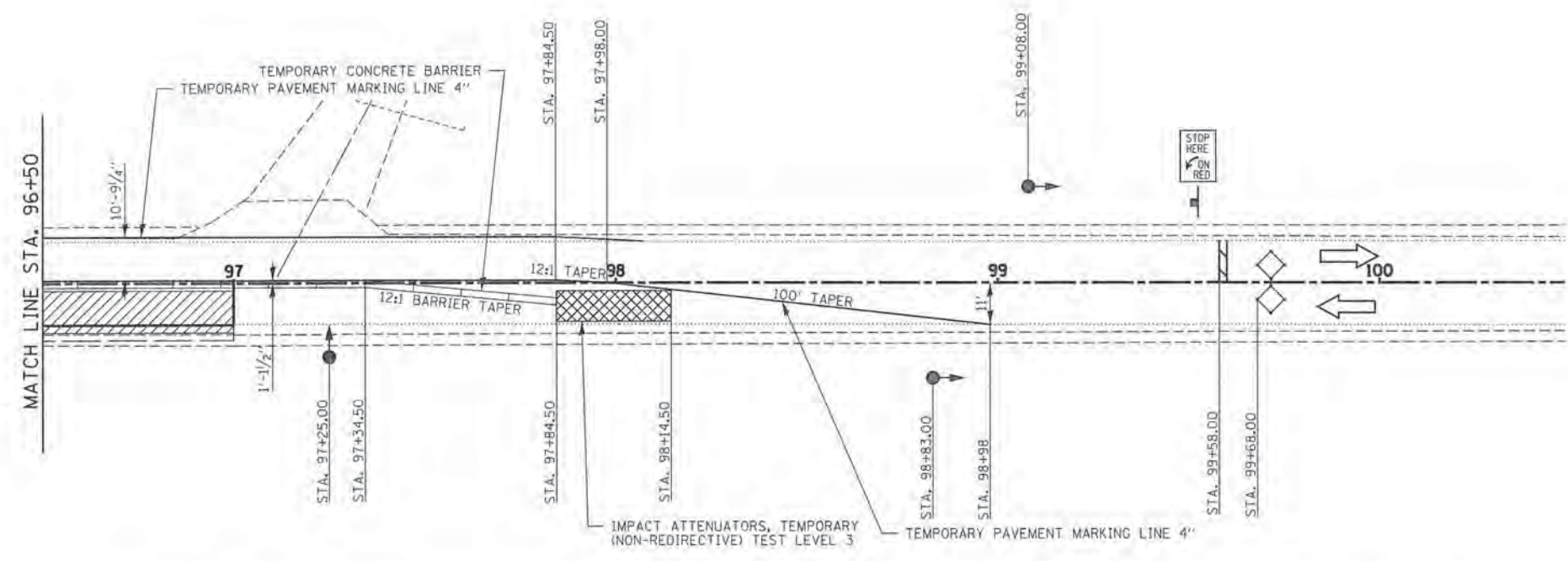
- LEGEND**
- PAVEMENT REMOVAL (SPECIAL)
  - EXISTING STRUCTURE REMOVAL



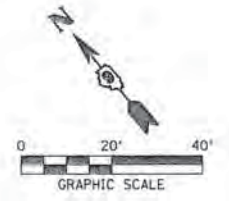
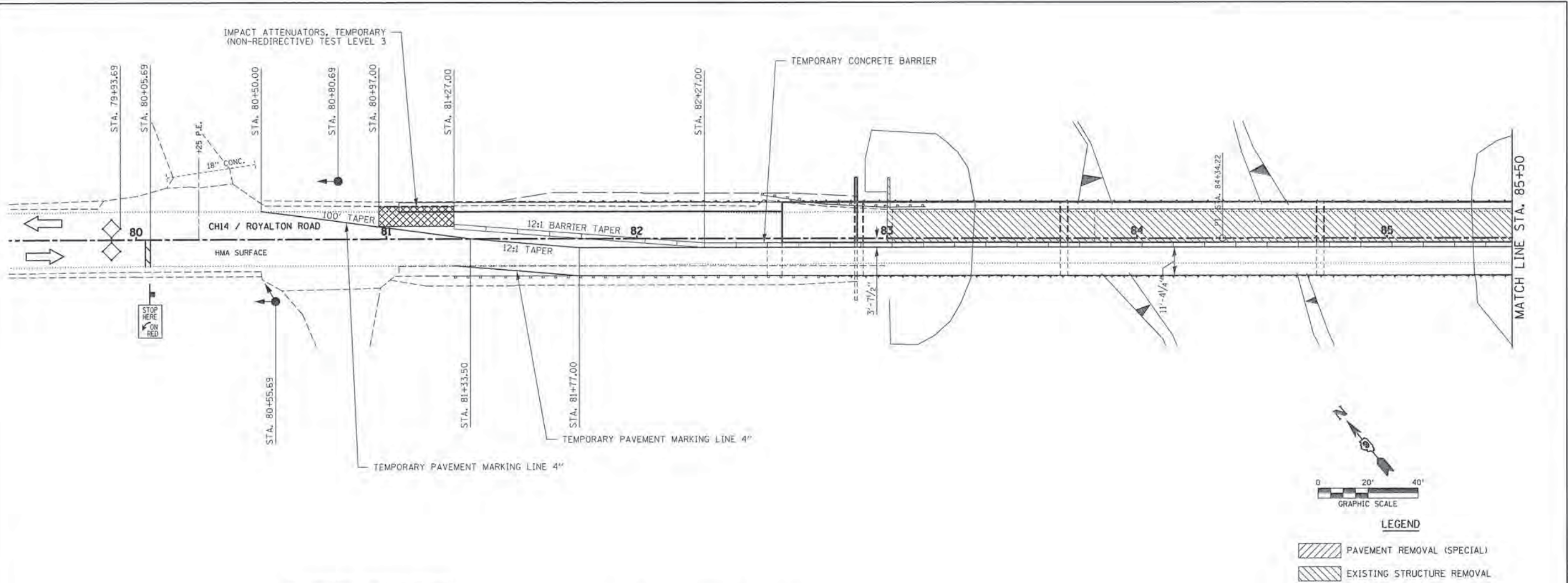
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	HAMPTON, LENZINI AND RENWICK, INC.	DRAWN - T.W.K.	REVISED -			869	10-00163-00-BR	JACKSON	82	10	
<small>250 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 154-000666</small>	PLOT SCALE =	CHECKED - J.W.F.	REVISED -			CONTRACT NO. 99519					
PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -				SCALE: SHEET NO. 1 OF 3 SHEETS STA. TO STA.		ILLINOIS FED. AID PROJECT			



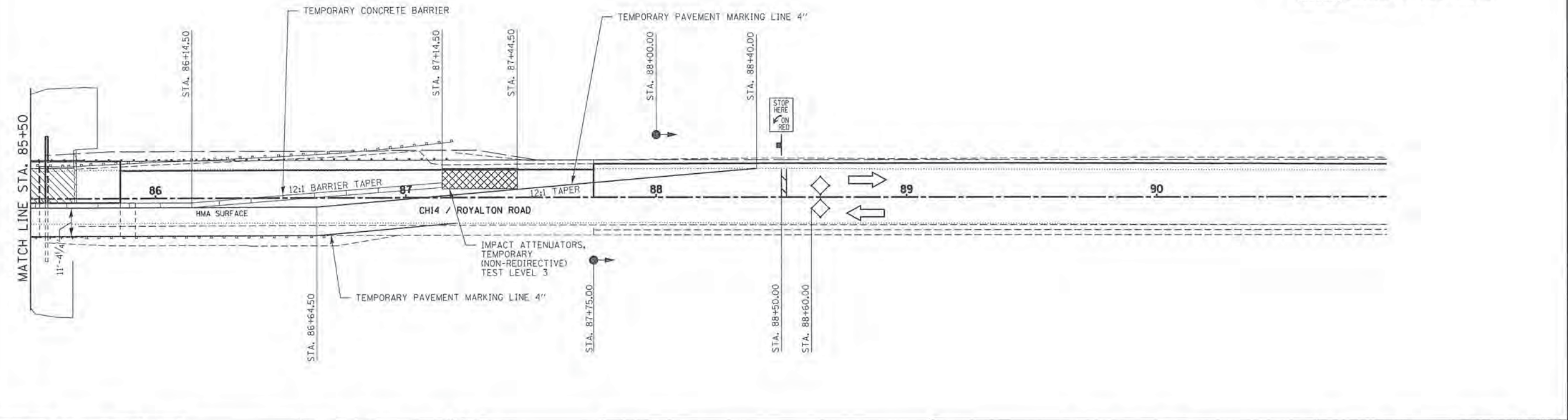
- LEGEND**
- PAVEMENT REMOVAL (SPECIAL)
  - EXISTING STRUCTURE REMOVAL



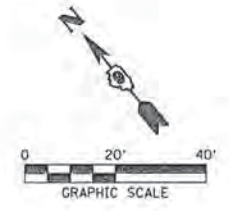
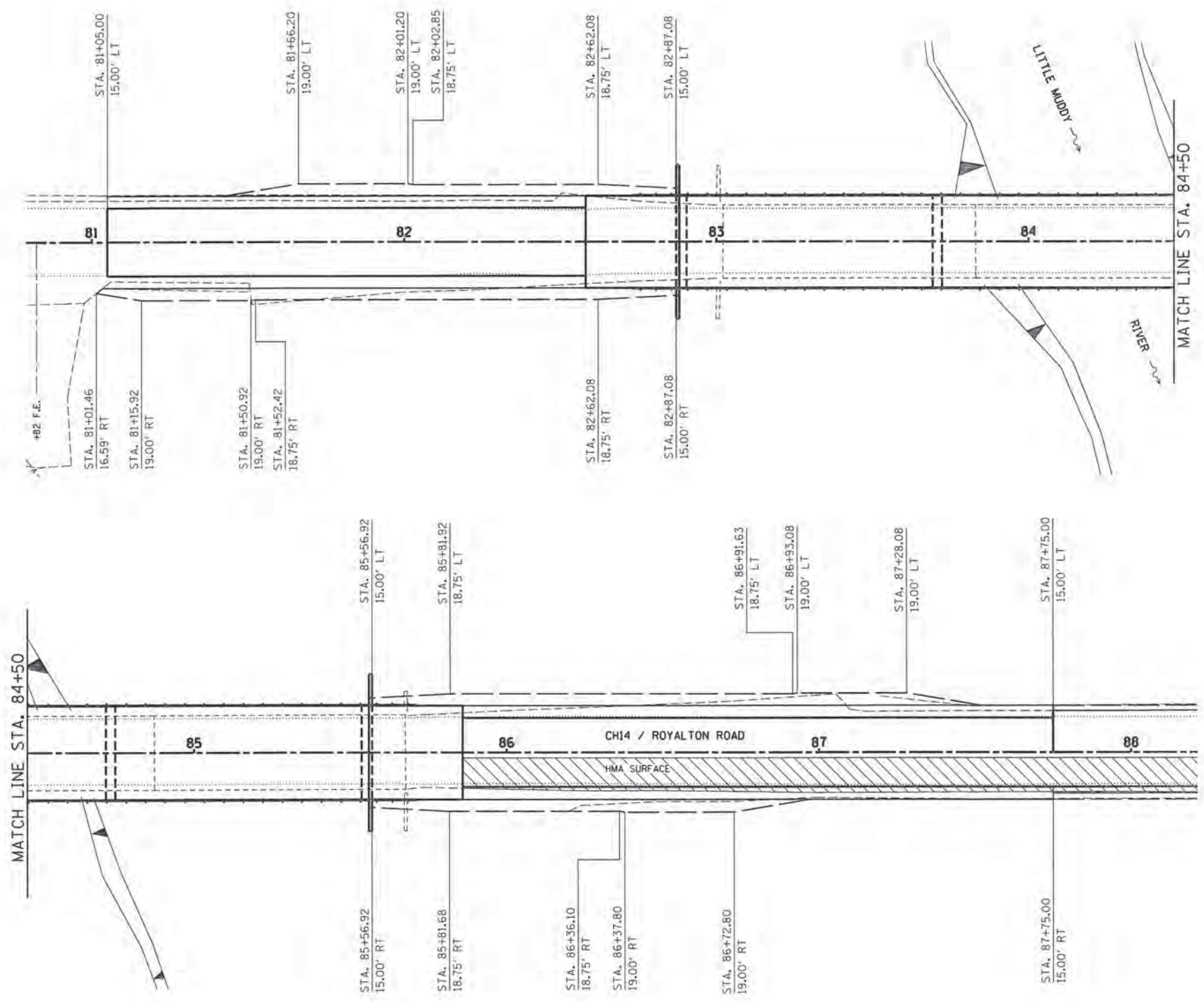
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HAMPTON, LENZIN AND RENWICK, INC. <small>2001 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62763 ILLINOIS PROFESSIONAL DESIGN FIRM LS / FE / SE CORP. 184.00003</small>	PLLOT SCALE =	DRAWN - T.W.K.	REVISED -			14	10-00163-00-BR	JACKSON	82	11		
PLLOT DATE = 4/6/2015		CHECKED - J.W.F.	REVISED -			SCALE: SHEET NO. 2 OF 3 SHEETS STA. TO STA.		ILLINOIS FED. AID PROJECT				
		DATE - 02/23/15	REVISED -									
CONTRACT NO. 99519												



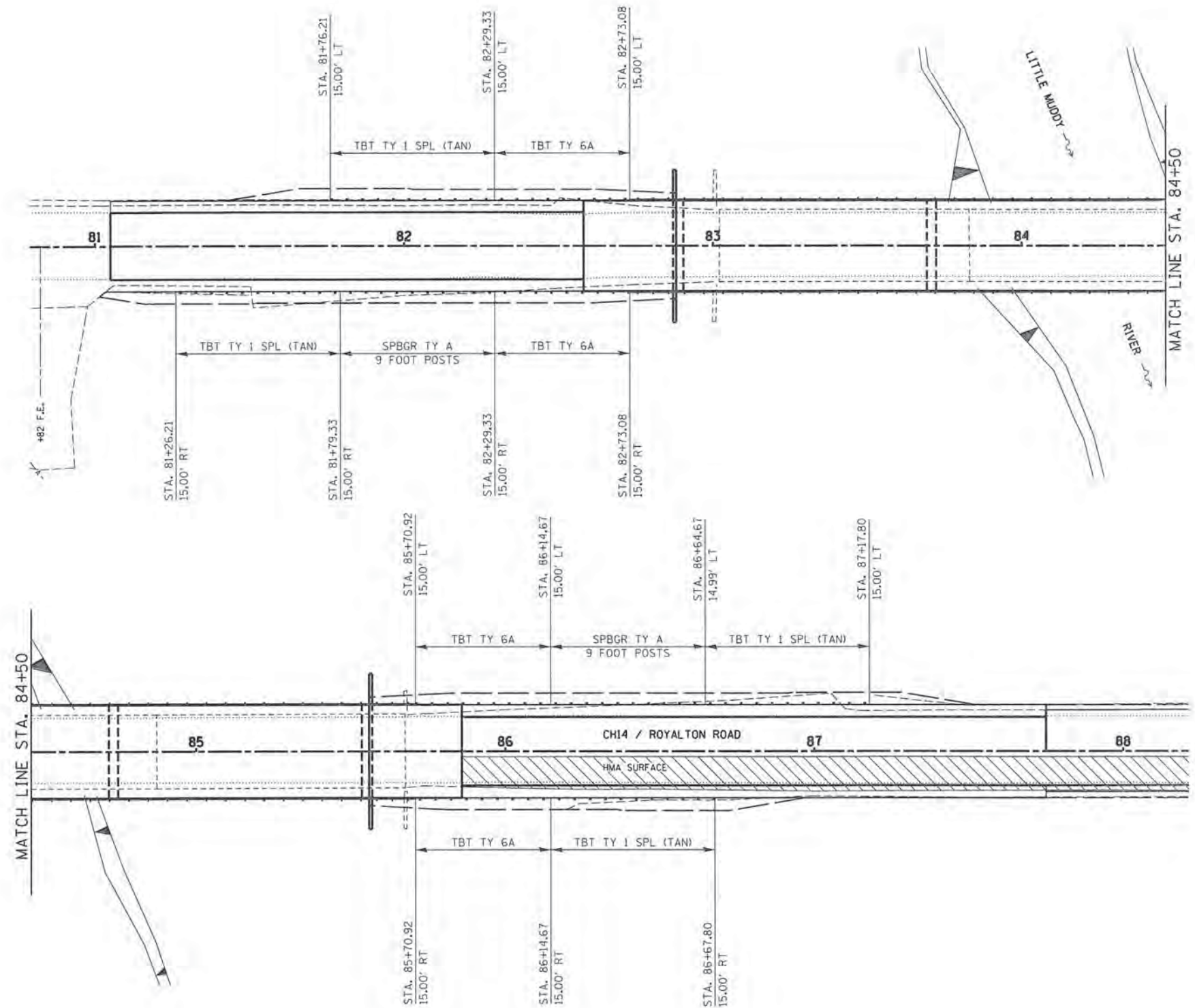
- LEGEND**
- PAVEMENT REMOVAL (SPECIAL)
  - EXISTING STRUCTURE REMOVAL



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HAMPTON, LENZINI AND RENWICK, INC. 280 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62762 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.00955	DRAWN - T.W.K.	CHECKED - J.W.F.	REVISED -			14	10-00163-00-BR	JACKSON	82	12		
PLOT SCALE =	DATE - 02/23/15	REVISED -	REVISED -			SCALE: SHEET NO. 3 OF 3 SHEETS STA. TO STA.		CONTRACT NO.				
PLOT DATE = 4/6/2015	REVISED -	REVISED -	REVISED -			ILLINOIS/FED. AID PROJECT						
SCALE: SHEET NO. 3 OF 3 SHEETS STA. TO STA.												



FILE NAME = 130340-sht-shoulder.dgn	USER NAME =	DESIGNED - L.F.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>PAVED SHOULDER LAYOUT C.H. 14 / ROYALTON ROAD</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 2881 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L3 / PE 158 CORP. 134-000933	PLOT SCALE =	DRAWN - T.W.K.	REVISED -			859	10-00163-00-BR	JACKSON	82	13	
	PLOT DATE = 4/6/2015	CHECKED - J.W.F.	REVISED -			CONTRACT NO. 99519					
		DATE - 02/23/15	REVISED -			ILLINOIS FED. AID PROJECT					
						SCALE:	SHEET NO. OF SHEETS	STA.	TO STA.		



FILE NAME = 138348-sht-guardrail.dgn	USER NAME =
HAMPTON, LENZINI AND RENWICK, INC. 3061 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62768 ILLINOIS PROFESSIONAL DESIGN FIRM LS/P/152 CORP. 194.000528	PLOT SCALE =
	PLOT DATE = 4/6/2015

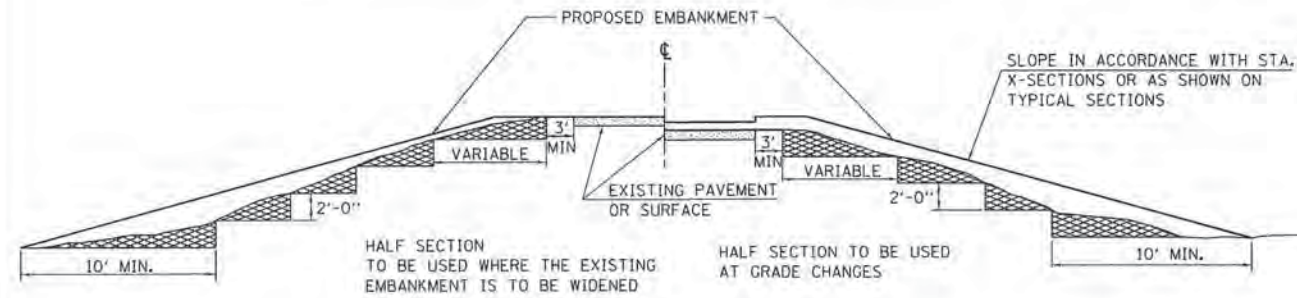
DESIGNED - L.F.S.	REVISED -
DRAWN - T.W.K.	REVISED -
CHECKED - J.W.F.	REVISED -
DATE - 02/23/15	REVISED -

**STATE OF ILLINOIS**  
**JACKSON COUNTY HIGHWAY DEPARTMENT**

<b>GUARDRAIL LAYOUT</b>			
<b>C.H. 14 / ROYALTON ROAD</b>			
SCALE:	SHEET NO.	OF SHEETS	STA. TO STA.

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
869	10-00163-00-BR	JACKSON	82	14
CONTRACT NO. 99519				
ILLINOIS FED. AID PROJECT				

**TYPICAL CROSS SECTION SHOWING  
STEP CONSTRUCTION ON EXISTING FILL**

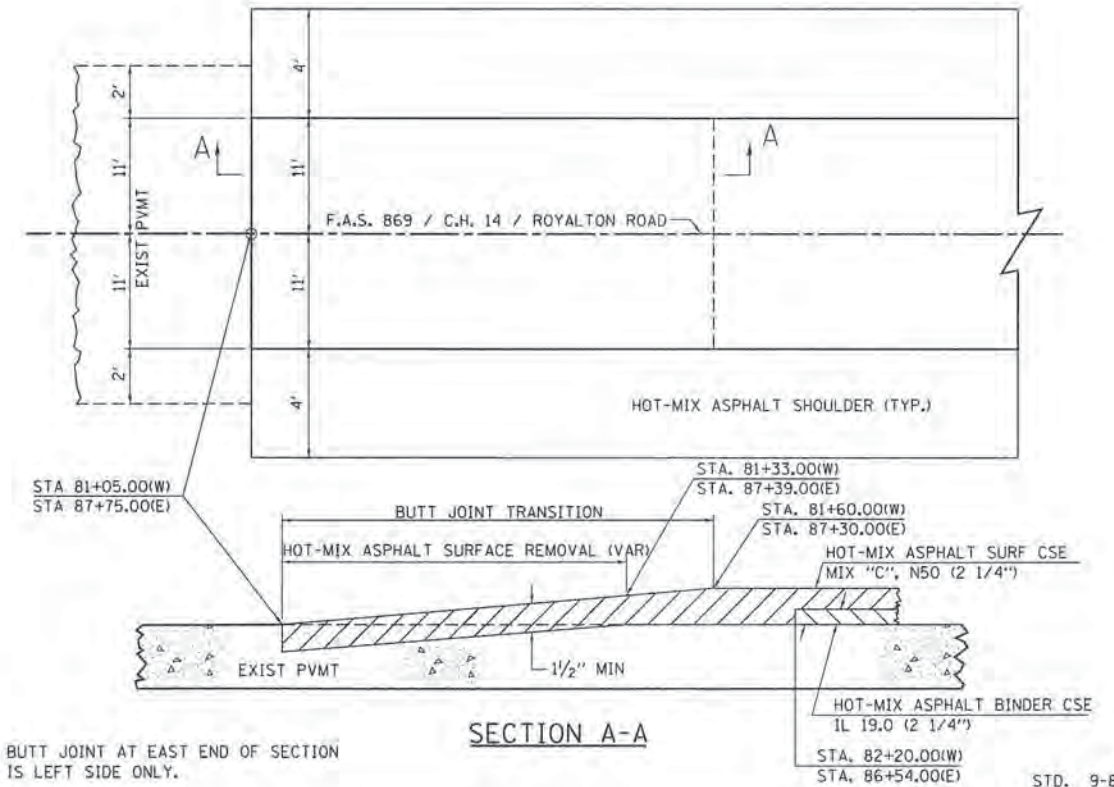


MATERIAL TO BE REMOVED AND REPLACED IN THE EMBANKMENT IN ACCORDANCE WITH ART. 205.04 OF THE STANDARD SPECIFICATION. COST TO BE INCLUDED IN THE VARIOUS ITEMS OF EXCAVATION AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED BECAUSE OF THIS WORK.

REVISIONS	
REDRAWN	2-15-89
REVISED	8-15-94
CHECKED	6-3-99
RESIZED	6-7-08

STD. 9-16

**BUTT JOINT**



REVISIONS	
DRAWN	10-17-90
REVISED	01-11-07
REVISED	3-25-08
REVISED	

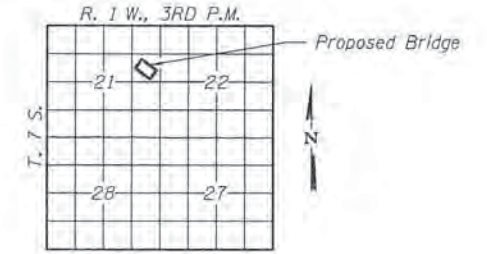
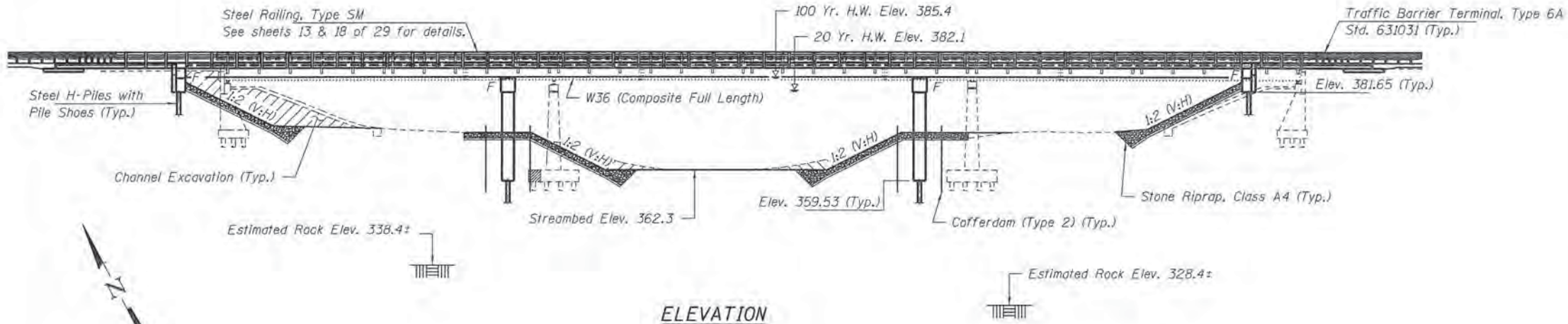
STD. 9-86

FILE NAME = 128348-sht-standards.dgn	USER NAME =	DESIGNED - L.F.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>STANDARD DETAILS DISTRICT 9 C.H. 14 / ROYALTON ROAD</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3305 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62763 ILLINOIS PROFESSIONAL DESIGN FIRM L.S. / P.E. / N.E. CORP. 194.000054	PLOT SCALE =	DRAWN - T.W.K.	REVISED -			869	10-00163-00-BR	JACKSON	82	15
PLOT DATE = 4/6/2015	DATE - 08/11/14	CHECKED - J.W.F.	REVISED -			CONTRACT NO. 99519				
		DATE - 08/11/14	REVISED -			ILLINOIS FED. AID PROJECT				
						SCALE:	SHEET NO. 1 OF 2 SHEETS	STA.	TO STA.	

BENCHMARK: Chiseled "□" on NW curb. 13' Lt. Sta. 83+00, Elev. 388.92.

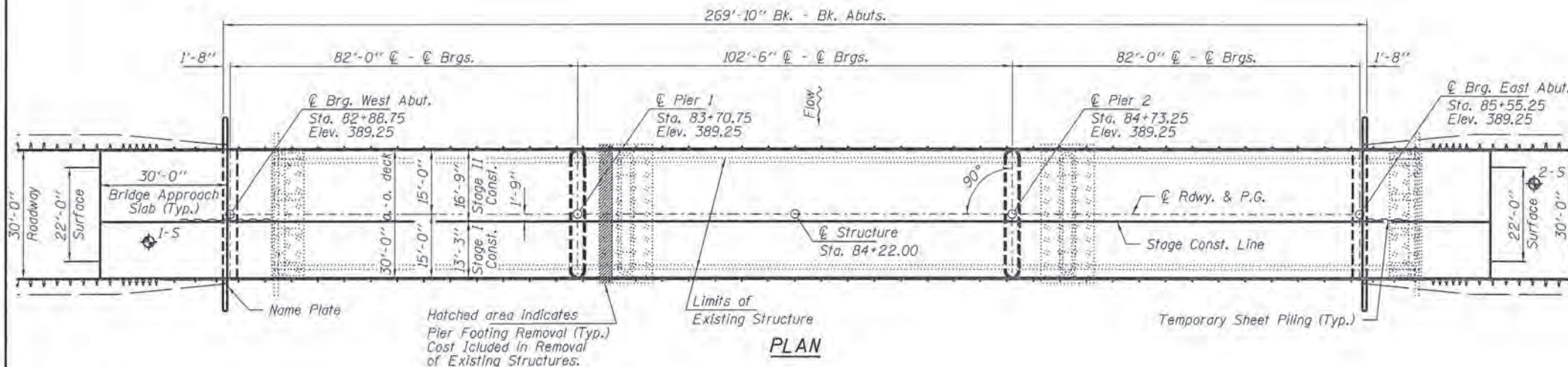
EXISTING STRUCTURE NO. 039-3000: Sta. 84+34.22 - Three span steel I-beam bridge on open abutments and solid wall concrete piers. 270.17' bk.-bk. abutts.; 26.00' o.-o. deck.

Structure to be replaced using staged construction to maintain one lane of traffic at all times.



**INDEX OF STRUCTURE SHEETS**

1. General Plan & Elevation
- 2-3. General Details
4. Stage Construction Details
5. Temporary Concrete Barrier for Stage Construction
- 6-9. Top of Slab Elevations
- 10-11. Top of Approach Slab Elevations
12. Superstructure
13. Superstructure Details
- 14-17. Precast Bridge Approach Slab
18. Steel Railing, Type SM
19. Structural Steel
- 20-21. Structural Steel Details
22. Bearing Details
23. West Abutment
24. East Abutment
25. Piers
26. HP Pile Details
27. Bar Splicer Assembly and Mechanical Splicer Details
- 28-29. Borings



LITTLE MUDDY RIVER  
BUILT 2011 BY  
JACKSON COUNTY  
C.H. 14 - F.A.S. 869  
SEC. 10-00163-00-BR  
STR. NO. 039-3277  
LOADING HL-93

**SEISMIC DATA**

Seismic Performance Zone (SPZ) = 2  
Design Spectral Acceleration at 1.0 sec. ( $S_{D1}$ ) = 0.249g  
Design Spectral Acceleration at 0.2 sec. ( $S_{D5}$ ) = 0.680g  
Soil Site Class = C

**DESIGN SPECIFICATIONS (NEW CONST.)**

2012 AASHTO LRFD Bridge Design Specification, with 2013 Interims.

**LOADING HL-93**

Allow 50#/sq. ft. for future wearing surface.

**DESIGN STRESSES**

**FIELD UNITS**

$f'_c$  = 5,000 psi (Superstructure)  
 $f'_c$  = 3,500 psi (Substructure)  
 $f_y$  = 60,000 psi (Reinf.)  
 $f_y$  = 50,000 psi (Structural Steel) (M270 Gr. 50W)

**PRECAST UNITS - (APPROACH SLAB)**

$f'_c$  = 6,000 psi  
 $f'_ci$  = 5,000 psi  
 $f_y$  = 60,000 (Reinf.)

Event/Limit State	Design Scour Elevations (ft.)				Item #
	W. Abut.	Pier 1	Pier 2	E. Abut.	
Q100	381.6	362.7	360.8	381.6	5
Q200	381.6	361.0	360.5	381.6	
Design	381.6	362.7	360.8	381.6	
Check	381.6	361.0	360.5	381.6	

**WATERWAY INFORMATION**

Drainage Area = 262 Sq. Mi. Existing Low Grade Elev. 386.7 @ Sta. 97+00  
Proposed Low Grade Elev. 386.7 @ Sta. 97+00

Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft.		Natural Head - Ft.		Headwater El.		
			Exist.	Prop.	Exist.	Prop.	Exist.	Prop.	
Design	10	10600	2380	2660	380.60	0.33	0.32	380.93	380.92
Base	20	12800	2720	3040	382.13	0.38	0.36	382.51	382.49
Max. Calc.	100	18100	3260	3640	385.36	0.09	0.08	385.45	385.44
	200	20600	3260	3640	386.70	0.26	0.25	386.96	386.95

10 Year Velocity through Existing Bridge = 4.5 Tps

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current "AASHTO LRFD Specifications."

*Scott M. Shoup*  
04/03/2015  
ILLINOIS STRUCTURAL NO. 081-6529

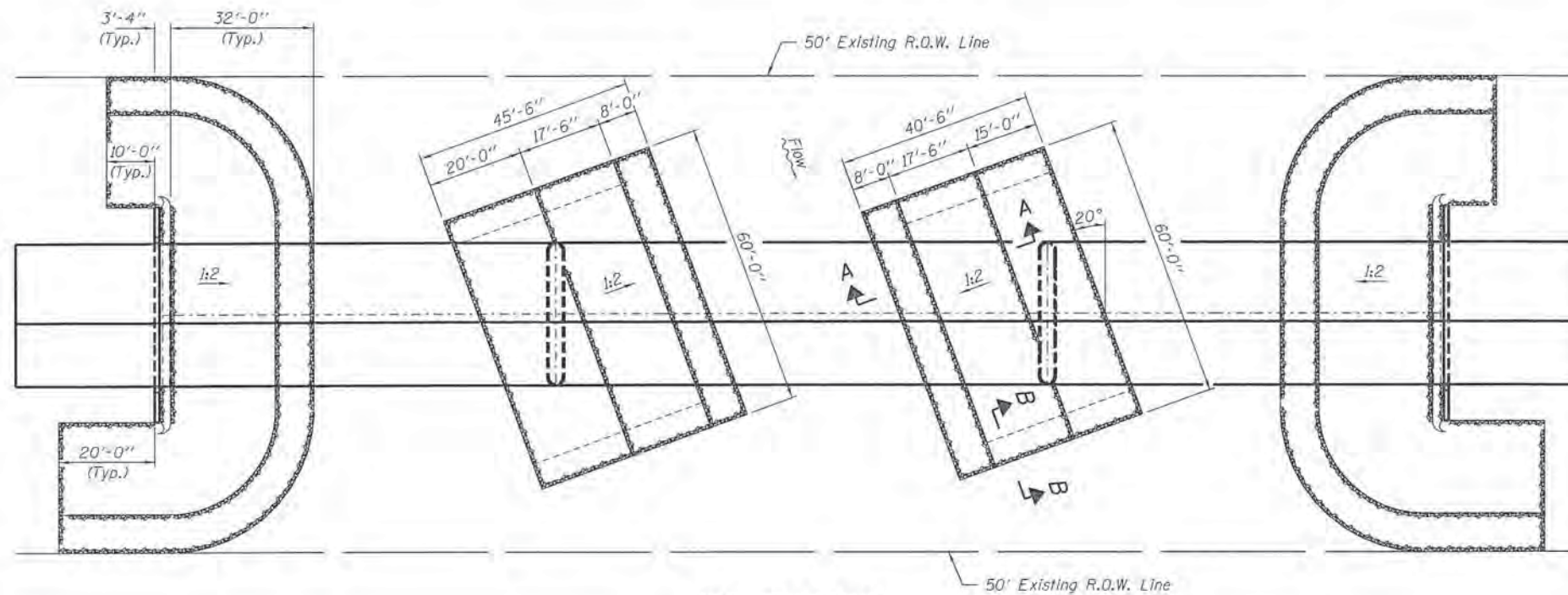


Expires 11-30-2016

**GENERAL PLAN & ELEVATION**  
**ROYALTON ROAD / FAS 869**  
**OVER LITTLE MUDDY RIVER**  
**SECTION 10-00163-00-BR**  
**JACKSON COUNTY**  
**STATION 84+32.22**  
**STRUCTURE NO. 039-3277**

FILE NAME = 130340-sh1-bridge.dgn	USER NAME =	DESIGNED - S.M.S.	REVISED -	STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT	GENERAL PLAN AND ELEVATION STRUCTURE NO. 039-3277	F.A.S.	SECTION	COUNTY	TOTAL SHEET NO.	
3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.646.3400 www.dmvengineering.com	PLOT SCALE =	CHECKED - D.W.T.	REVISED -			869	10-00163-00-BR	JACKSON	82	16
183.00358 ILLINOIS PROFESSIONAL DESIGN FIRM 13 / PG 1 OF 29 SHEETS	PLOT DATE = 4/6/2015	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 99519				
		CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT BRS-08691061				





**RIPRAP LAYOUT**

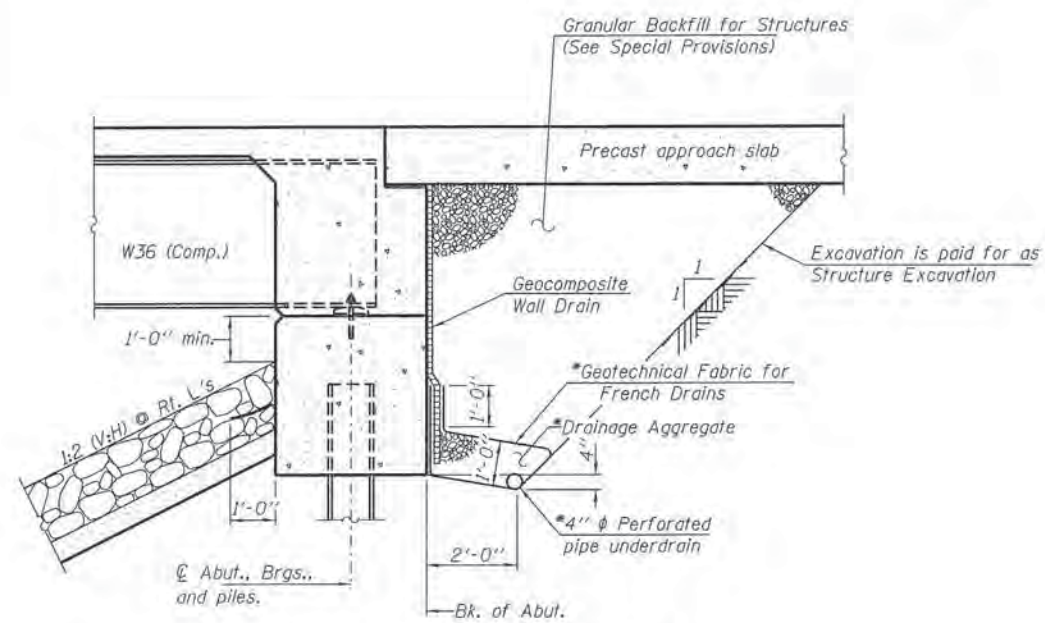


**GENERAL NOTES**

Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts in painted areas and ASTM A325 Type 3 in unpainted areas. Bolts 1/2" φ, holes 5/8" φ, unless otherwise noted.  
 Calculated weight of Structural Steel = 267,520 lbs.  
 All structural steel shall be AASHTO M 270 Grade 50W except expansion joints which shall be AASHTO M 270 Grade 50.  
 No field welding is permitted except as specified in the contract documents.  
 Reinforcement bars and Bar Splicers designated (E) shall be epoxy coated.  
 Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8" (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.  
 Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete cap plus 3 inches. Painted areas shall be primed in the shop with a Department approved zinc rich primer. Field painting will not be required.  
 Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.



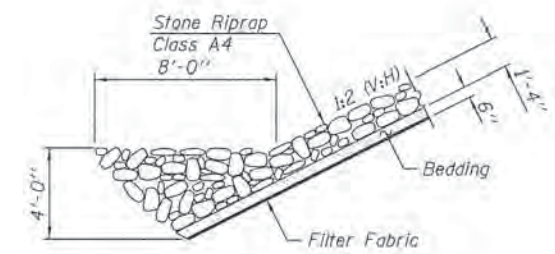
**PROFILE GRADE**  
(along roadway)



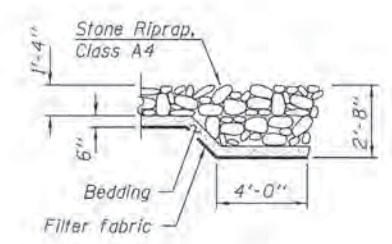
**SECTION THRU INTEGRAL ABUTMENT**  
(Horiz. dim. @ Rt. L's)

\*Included in the cost of Pipe Underdrains for Structures. (See Special Provisions)

Note:  
 All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).



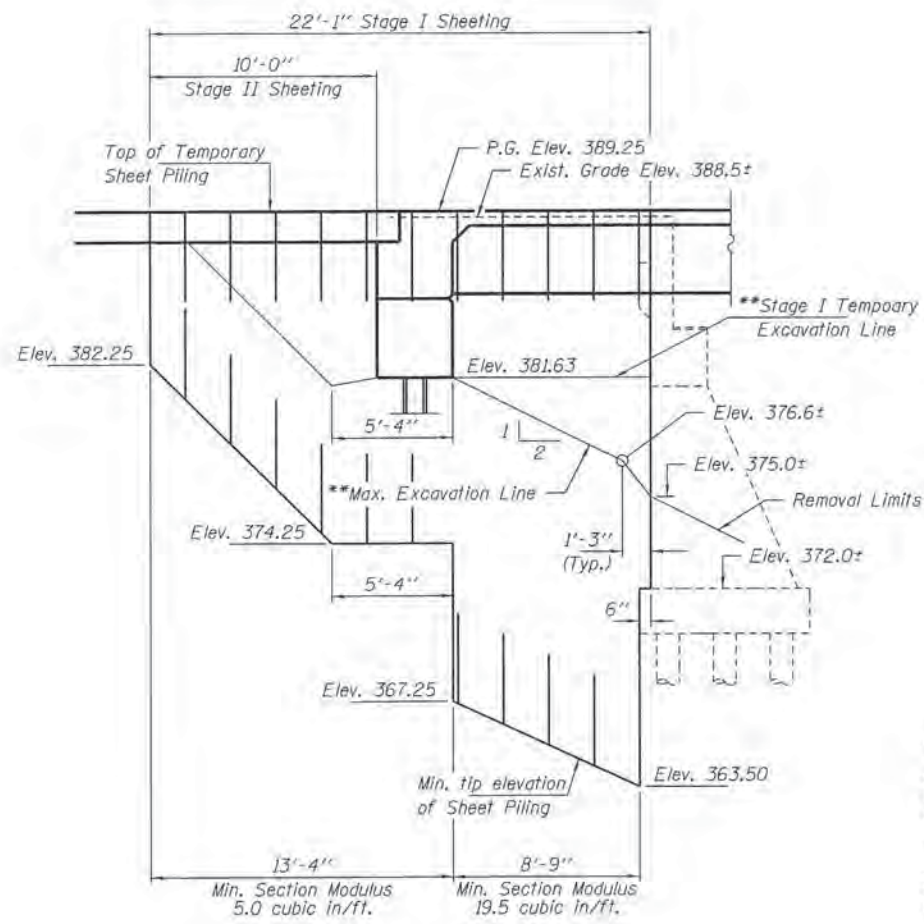
**SECTION A-A**



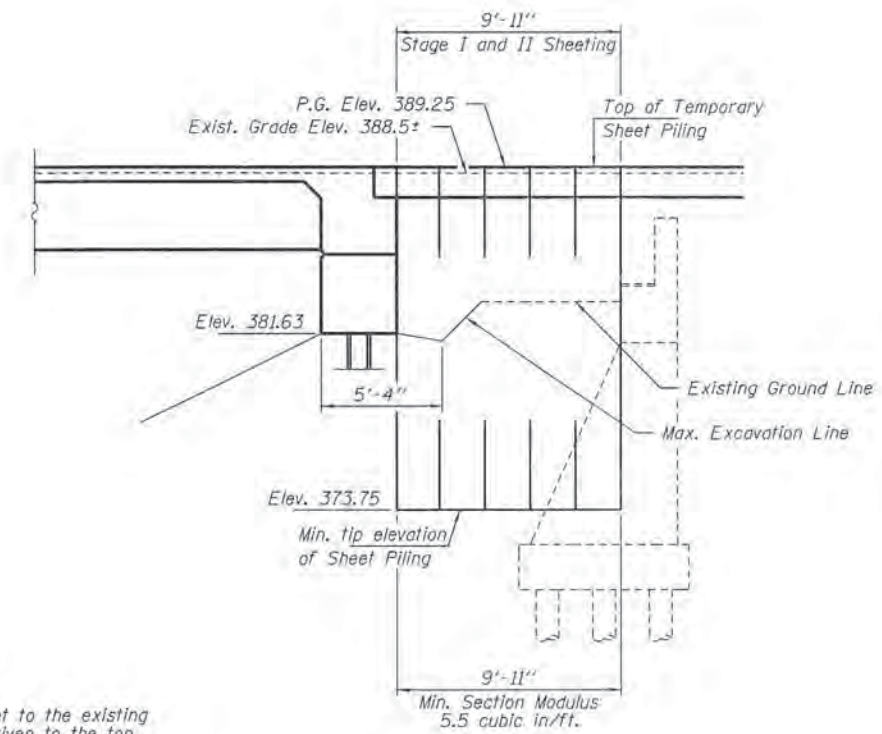
**SECTION B-B**

**TOTAL BILL OF MATERIAL**

ITEM	UNIT	SUPER	SUB	TOTAL
Channel Excavation	Cu. Yd.			820
Stone Riprap, Class A4	Ton			1,486
Filter Fabric	Sq. Yd.			1,601
Removal of Existing Structures	Each			1
Slope Wall Removal	Sq. Yd.			686
Structure Excavation	Cu. Yd.			188
Cofferdam Excavation	Cu. Yd.			328
Cofferdam (Type 2) (Location-1)	Each			1
Cofferdam (Type 2) (Location-2)	Each			1
Concrete Structures	Cu. Yd.		219.2	219.2
Concrete Superstructure	Cu. Yd.	243.5		243.5
Bridge Deck Grooving	Sq. Yd.	1.020		1.020
Protective Coat	Sq. Yd.	1.259		1.259
Furnishing and Erecting Structural Steel	L. Sum	1		1
Stud Shear Connectors	Each	4,338		4,338
Reinforcement Bars, Epoxy Coated	Pound	62,970	21,520	84,490
Bar Splicers	Each	852	244	1,096
Steel Railing, Type SM	Foot	596		596
Furnishing Steel Piles HP14x117	Foot		1,332	1,332
Driving Piles	Foot		1,332	1,332
Test Pile Steel HP14x117	Each		4	4
Pile Shoes	Each		24	24
Name Plates	Each		1	1
Preformed Joint Strip Seal	Foot		62	62
Anchor Bolts, 3/4"	Each		24	24
Anchor Bolts, 1"	Each		24	24
Geocomposite Wall Drain	Sq. Yd.		58	58
Temporary Sheet Piling	Sq. Ft.		531	531
Pipe Underdrains for Structures 4"	Foot		124	124
Concrete Wearing Surface, 5"	Sq. Yd.	200		200
Precast Bridge Approach Slab	Sq. Ft.	1,749		1,749
Granular Backfill for Structures	Cu. Yd.		128.0	128.0



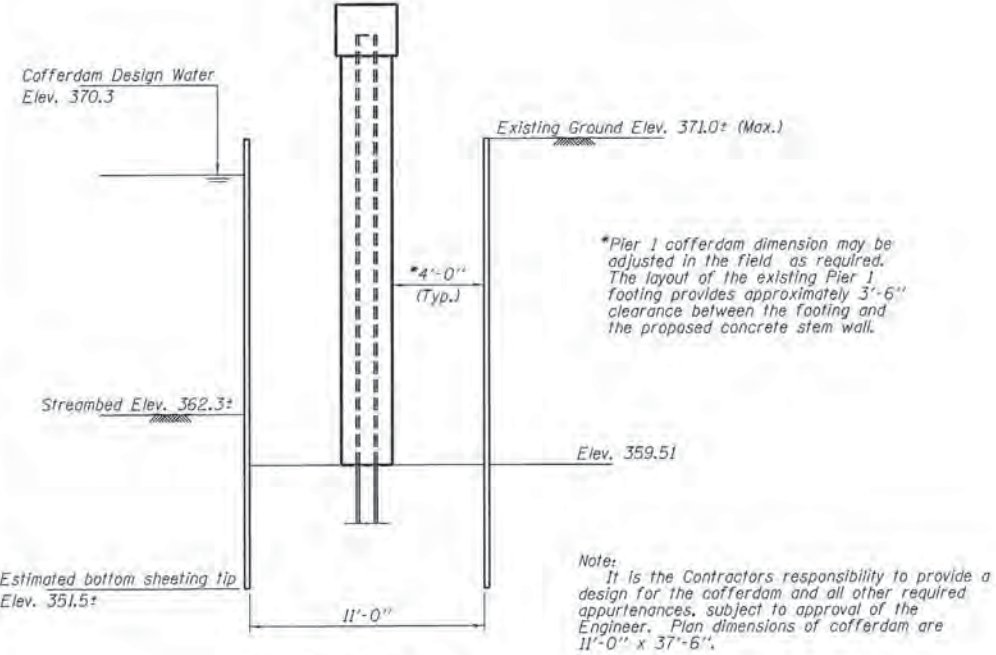
WEST ABUTMENT



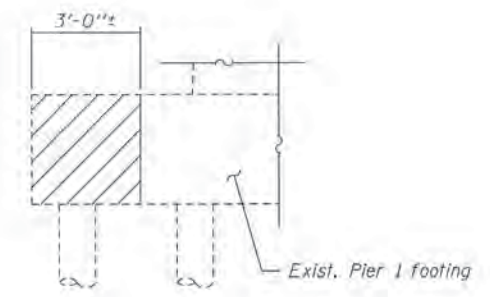
EAST ABUTMENT

Notes:  
 The Contractor shall connect the first sheet to the existing abutment wall to ensure stability of sheets driven to the top of the existing footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Piling.  
 If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the Engineer.  
 \*\*Due to existing open abutments, the Stage I Construction excavation will need to be limited to the Stage I Temporary Excavation Line at the Stage Construction Line until the Stage II removal begins.

TEMPORARY SHEET PILING AT ABUTMENTS



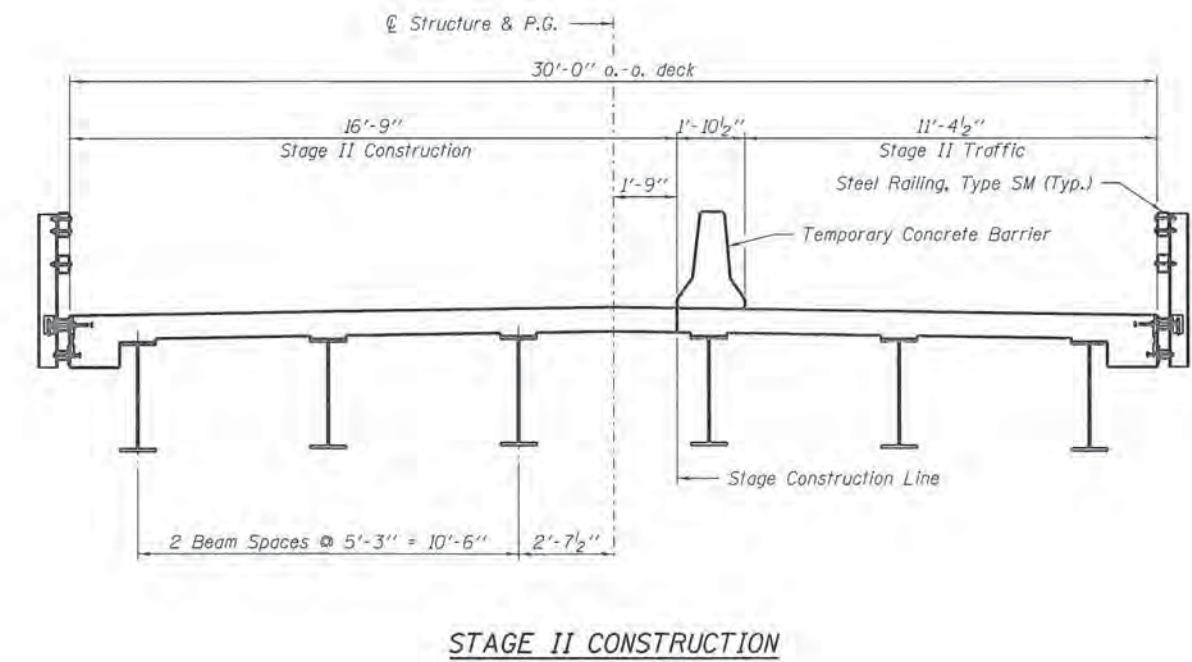
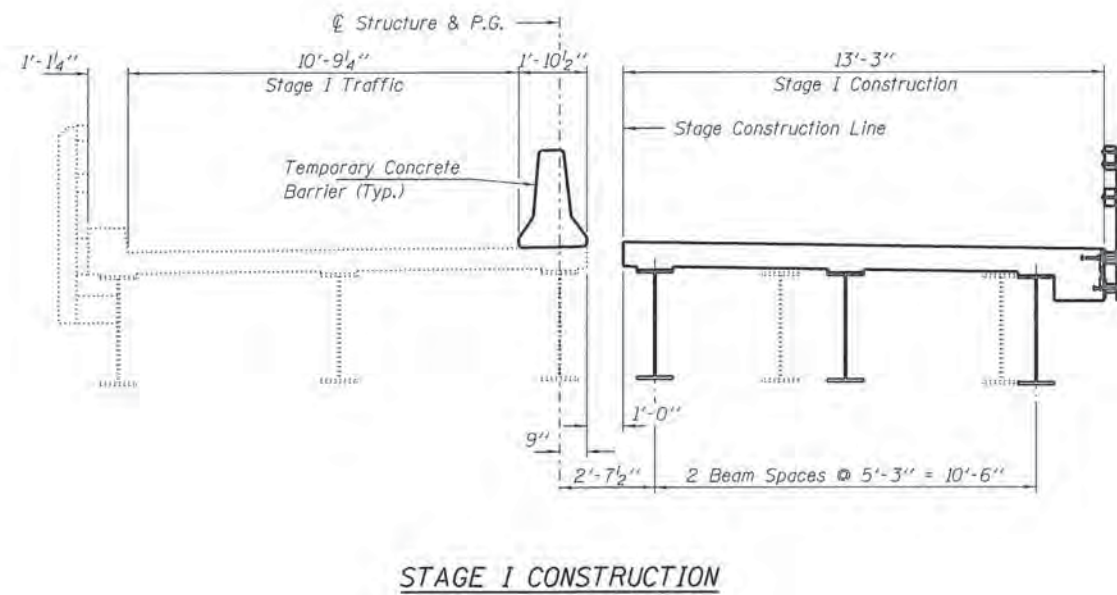
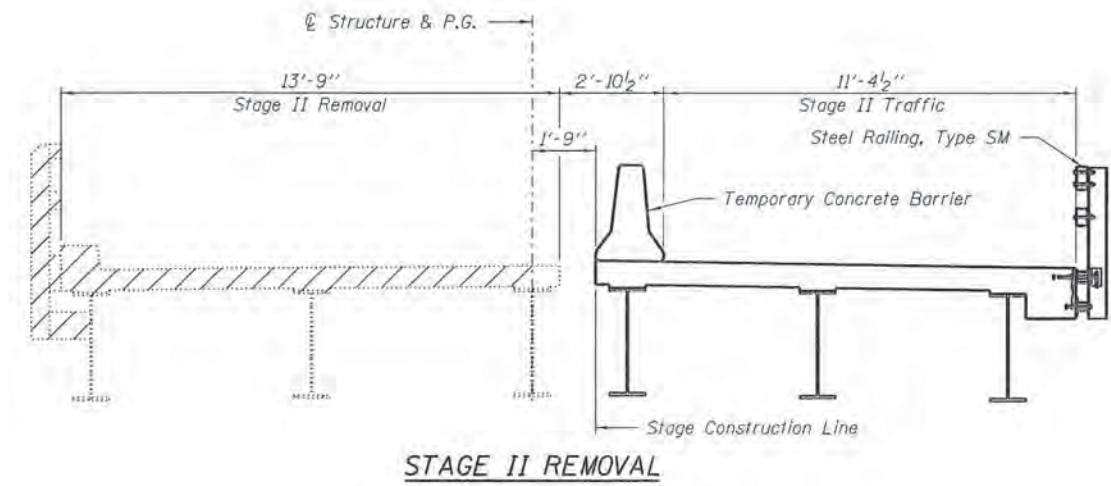
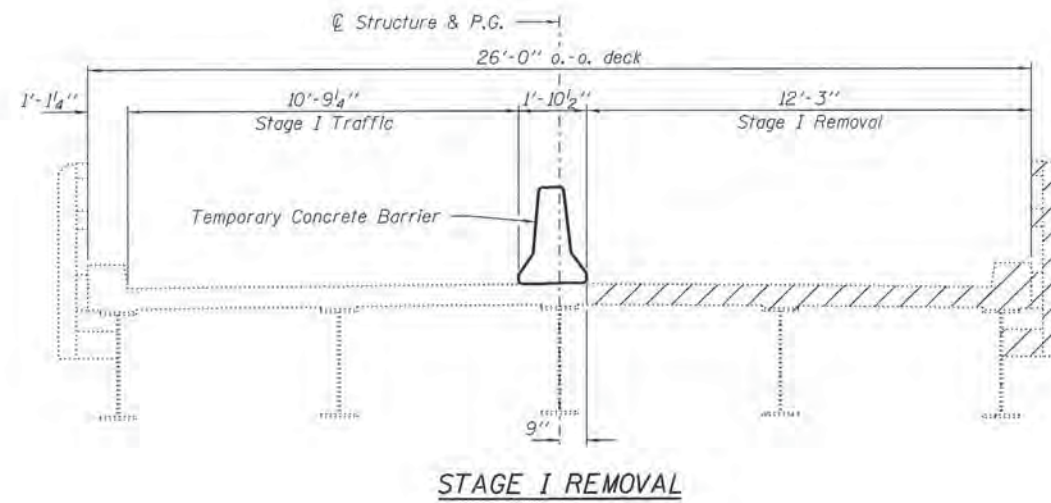
COFFERDAM DETAIL



EXISTING PIER 1 REMOVAL  
 (Required for Cofferdam installation)

Hatch area indicates Pier Footing removal.

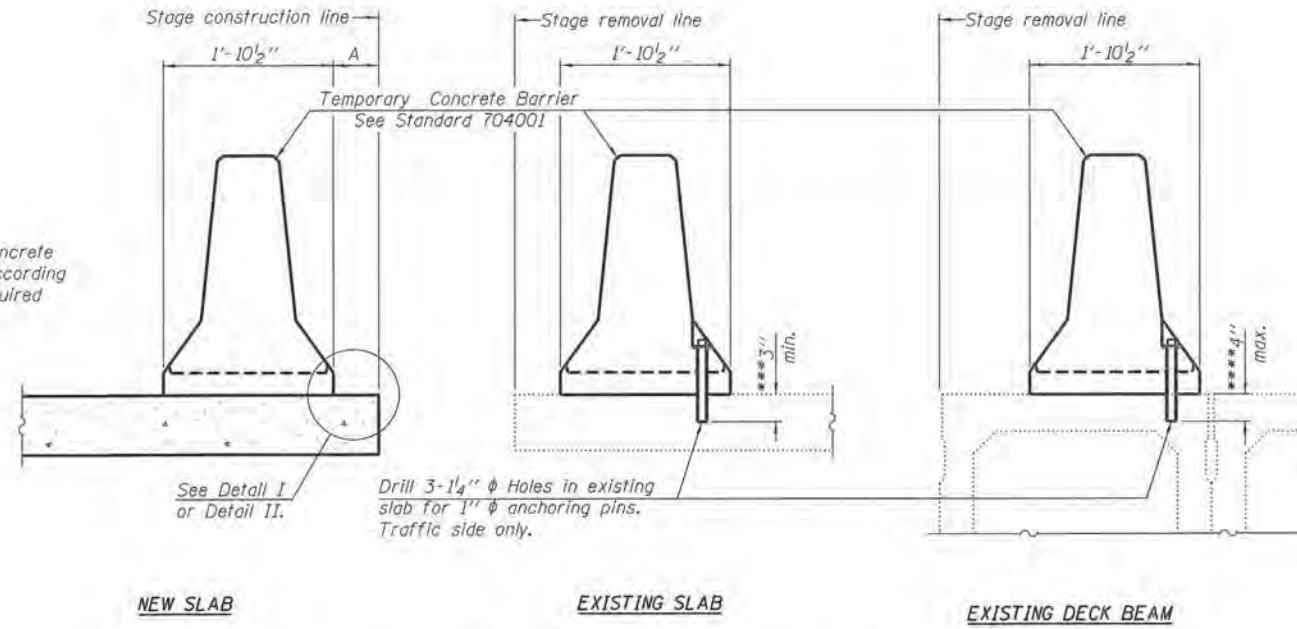
FILE NAME = 130340-ah-bridge.dgn	USER NAME =	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>GENERAL DETAILS STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
3055 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.448.3400 www.jbrengineering.com	PLDT SCALE =	CHECKED - D.W.T.	REVISED -			869	10-00163-00-BR	JACKSON	82	18	
181.000958 ILLINOIS PROFESSIONAL DESIGN FIRM L3 / PE / SE CORPORATION	PLDT DATE = 4/6/2015	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 99519					
		CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT BR5-08691061					



Notes:  
 All sections are looking East.  
 Hatched areas indicate removal.  
 See Roadway Plans for quantity of Temporary Concrete Barrier.

FILE NAME = 130340-ah-bridge.dgn	USER NAME =	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>STAGE CONSTRUCTION DETAILS STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.546.3400 www.sbrengineering.com	PLOT SCALE =	CHECKED - D.W.T.	REVISED -			859	10-00163-00-BR	JACKSON	82	19	
184.002859 ILLINOIS PROFESSIONAL DESIGN FIRM L3 I/F/E CORPORATION	PLOT DATE = 4/6/2015	DRAWN - D.A.B.	REVISED -			<b>CONTRACT NO. 99519</b>					
						SHEET NO. 4 OF 29 SHEETS					

When "A" is 3'-1" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required when "A" is greater than 3'-1".



**SECTIONS THRU SLAB OR DECK BEAM**

**NOTES**

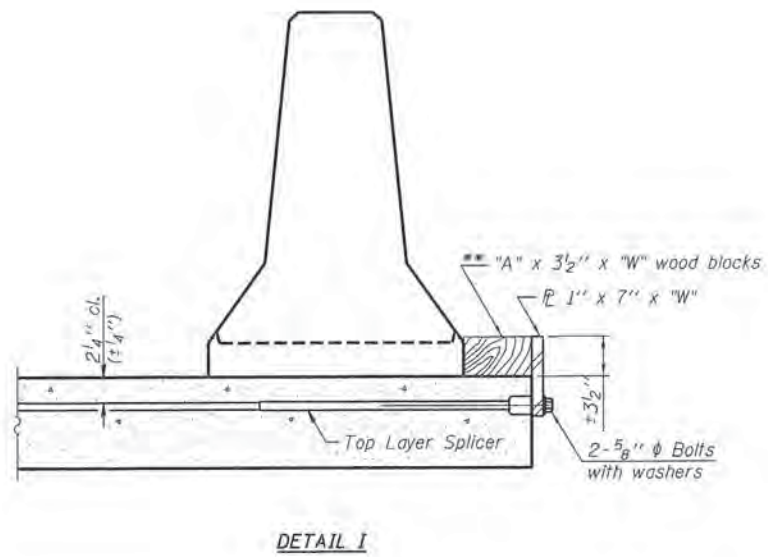
Detail I - With Bar Splicer or Couplers:  
Connect one (1) 1" x 7" x "W" steel  $\bar{C}$  to the top layer of couplers with 2- $\frac{5}{8}$ "  $\phi$  bolts screwed to coupler at approximate  $\bar{C}$  of each barrier panel.

Detail II - With Extended Reinforcement Bars:  
Connect one (1) 1" x 7" x "W" steel  $\bar{C}$  to the concrete slab or concrete wearing surface with 2- $\frac{5}{8}$ "  $\phi$  Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate  $\bar{C}$  of each barrier panel.

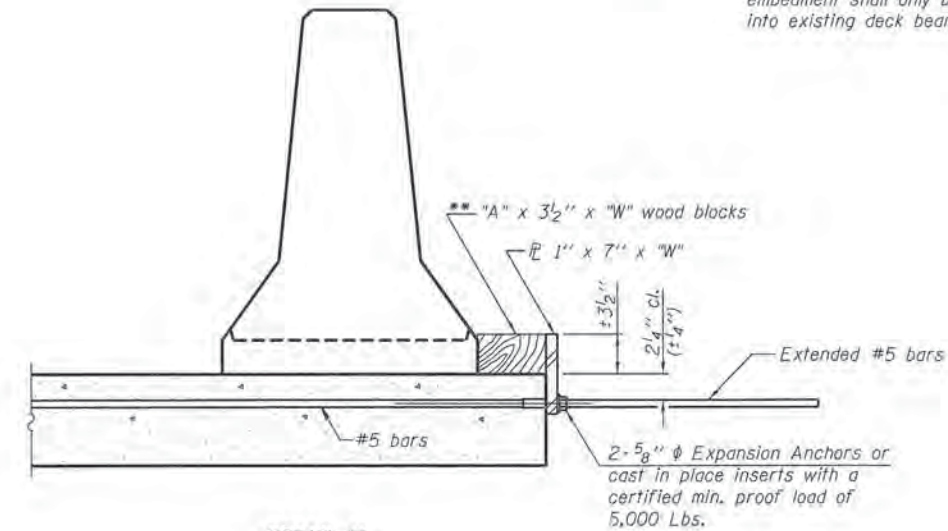
Cost of retainer assembly is included with Temporary Concrete Barrier. The 1" x 7" x "W" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

\*\*\* Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

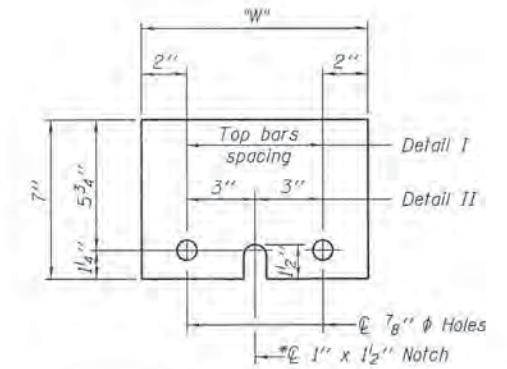
\*\*\*\* If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



**DETAIL I**



**DETAIL II**



**STEEL RETAINER  $\bar{C}$  1" x 7" x "W"**  
\* Required only with Detail II

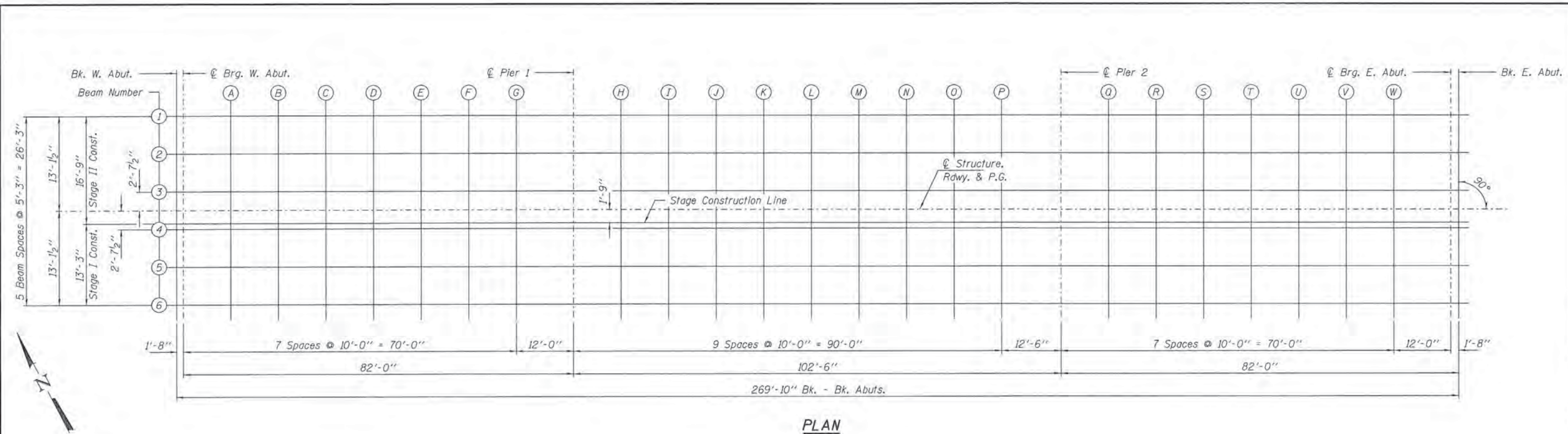
**RETAINER ASSEMBLY**

\*\* Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

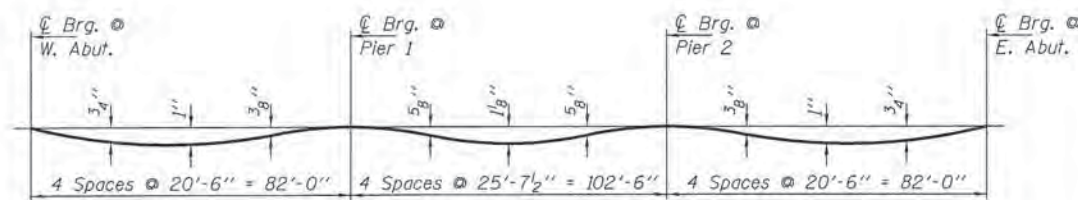
R-27

1-12-15

FILE NAME = 130348-shr-bridge.dgn	USER NAME *	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
2085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 317.548.3400 www.jhrengineering.com	CHECKED - D.W.T.	REVISED -	869			10-00163-00-BR	JACKSON	82	20	
141.00095 ILLINOIS PROFESSIONAL DESIGN FIRM LLP / PC / SC CORPORATION	DRAWN - D.A.B.	REVISED -	CONTRACT NO. 99519							
PLOT SCALE =	CHECKED - M.D.C.	REVISED -	ILLINOIS FED. AID PROJECT BR5-08691061							
PLOT DATE = 4/6/2015				SHEET NO. 5 OF 29 SHEETS						



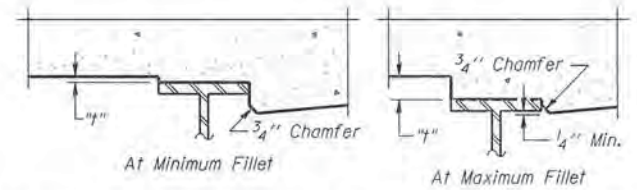
**PLAN**



**DEAD LOAD DEFLECTION DIAGRAM**

(Includes weight of concrete only.)

*Note:*  
The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown on sheets 7 thru 9 of 29.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown above. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on sheets 7 thru 9 of 29, minus slab thickness, equals the fillet heights "t" above top flange of beams.

**FILLET HEIGHTS**

FILE NAME = 130348-sht-br ridge.dgn 3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217-545-3400 www.hjrengineering.com 184.800459 ILLINOIS PROFESSIONAL DESIGN FIRM 15179 / 156 CORPORATION	USER NAME =	DESIGNED - S.M.S. CHECKED - D.W.T. DRAWN - D.A.B. CHECKED - M.D.C.	REVISED - REVISED - REVISED - REVISED -	<b>STATE OF ILLINOIS</b> <b>JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>TOP OF SLAB ELEVATIONS</b> <b>STRUCTURE NO. 039-3277</b> SHEET NO. 6 OF 29 SHEETS	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE =	PLOT DATE = 4/6/2015	869			10-00163-00-BR	JACKSON	82	21	<b>CONTRACT NO. 99519</b> <small>(ILLINOIS) FED. AID PROJECT BR3-0869106</small>

**BEAM 1**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	82+87.08	-13.13	389.04	389.04
☉ Brg. W. Abut.	82+88.75	-13.13	389.04	389.04
A	82+98.75	-13.13	389.04	389.08
B	83+08.75	-13.13	389.04	389.11
C	83+18.75	-13.13	389.04	389.13
D	83+28.75	-13.13	389.04	389.13
E	83+38.75	-13.13	389.04	389.11
F	83+48.75	-13.13	389.04	389.09
G	83+58.75	-13.13	389.04	389.06
☉ Pier 1	83+70.75	-13.13	389.04	389.04
H	83+80.75	-13.13	389.04	389.05
I	83+90.75	-13.13	389.04	389.08
J	84+00.75	-13.13	389.04	389.11
K	84+10.75	-13.13	389.04	389.13
L	84+20.75	-13.13	389.04	389.14
M	84+30.75	-13.13	389.04	389.13
N	84+40.75	-13.13	389.04	389.11
O	84+50.75	-13.13	389.04	389.09
P	84+60.75	-13.13	389.04	389.06
☉ Pier 2	84+73.25	-13.13	389.04	389.04
Q	84+83.25	-13.13	389.04	389.05
R	84+93.25	-13.13	389.04	389.08
S	85+03.25	-13.13	389.04	389.10
T	85+13.25	-13.13	389.04	389.12
U	85+23.25	-13.13	389.04	389.13
V	85+33.25	-13.13	389.04	389.12
W	85+43.25	-13.13	389.04	389.09
☉ Brg. E. Abut.	85+55.25	-13.13	389.04	389.04
Bk. E. Abut.	85+56.92	-13.13	389.04	389.04

**BEAM 2**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	82+87.08	-7.88	389.13	389.13
☉ Brg. W. Abut.	82+88.75	-7.88	389.13	389.13
A	82+98.75	-7.88	389.13	389.17
B	83+08.75	-7.88	389.13	389.20
C	83+18.75	-7.88	389.13	389.21
D	83+28.75	-7.88	389.13	389.21
E	83+38.75	-7.88	389.13	389.19
F	83+48.75	-7.88	389.13	389.17
G	83+58.75	-7.88	389.13	389.14
☉ Pier 1	83+70.75	-7.88	389.13	389.13
H	83+80.75	-7.88	389.13	389.14
I	83+90.75	-7.88	389.13	389.16
J	84+00.75	-7.88	389.13	389.19
K	84+10.75	-7.88	389.13	389.21
L	84+20.75	-7.88	389.13	389.22
M	84+30.75	-7.88	389.13	389.21
N	84+40.75	-7.88	389.13	389.20
O	84+50.75	-7.88	389.13	389.17
P	84+60.75	-7.88	389.13	389.14
☉ Pier 2	84+73.25	-7.88	389.13	389.13
Q	84+83.25	-7.88	389.13	389.14
R	84+93.25	-7.88	389.13	389.16
S	85+03.25	-7.88	389.13	389.19
T	85+13.25	-7.88	389.13	389.20
U	85+23.25	-7.88	389.13	389.21
V	85+33.25	-7.88	389.13	389.20
W	85+43.25	-7.88	389.13	389.17
☉ Brg. E. Abut.	85+55.25	-7.88	389.13	389.13
Bk. E. Abut.	85+56.92	-7.88	389.13	389.13

**BEAM 3**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	82+87.08	-2.63	389.21	389.21
☉ Brg. W. Abut.	82+88.75	-2.63	389.21	389.21
A	82+98.75	-2.63	389.21	389.25
B	83+08.75	-2.63	389.21	389.28
C	83+18.75	-2.63	389.21	389.29
D	83+28.75	-2.63	389.21	389.29
E	83+38.75	-2.63	389.21	389.28
F	83+48.75	-2.63	389.21	389.25
G	83+58.75	-2.63	389.21	389.22
☉ Pier 1	83+70.75	-2.63	389.21	389.21
H	83+80.75	-2.63	389.21	389.22
I	83+90.75	-2.63	389.21	389.24
J	84+00.75	-2.63	389.21	389.27
K	84+10.75	-2.63	389.21	389.29
L	84+20.75	-2.63	389.21	389.30
M	84+30.75	-2.63	389.21	389.30
N	84+40.75	-2.63	389.21	389.28
O	84+50.75	-2.63	389.21	389.25
P	84+60.75	-2.63	389.21	389.23
☉ Pier 2	84+73.25	-2.63	389.21	389.21
Q	84+83.25	-2.63	389.21	389.22
R	84+93.25	-2.63	389.21	389.24
S	85+03.25	-2.63	389.21	389.27
T	85+13.25	-2.63	389.21	389.29
U	85+23.25	-2.63	389.21	389.29
V	85+33.25	-2.63	389.21	389.28
W	85+43.25	-2.63	389.21	389.25
☉ Brg. E. Abut.	85+55.25	-2.63	389.21	389.21
Bk. E. Abut.	85+56.92	-2.63	389.21	389.21

☉ STRUCTURE, RDWY., & P.G.

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	82+87.08	0.00	389.25	389.25
☉ Brg. W. Abut.	82+88.75	0.00	389.25	389.25
A	82+98.75	0.00	389.25	389.29
B	83+08.75	0.00	389.25	389.32
C	83+18.75	0.00	389.25	389.34
D	83+28.75	0.00	389.25	389.33
E	83+38.75	0.00	389.25	389.32
F	83+48.75	0.00	389.25	389.29
G	83+58.75	0.00	389.25	389.27
☉ Pier 1	83+70.75	0.00	389.25	389.25
H	83+80.75	0.00	389.25	389.26
I	83+90.75	0.00	389.25	389.28
J	84+00.75	0.00	389.25	389.31
K	84+10.75	0.00	389.25	389.33
L	84+20.75	0.00	389.25	389.34
M	84+30.75	0.00	389.25	389.34
N	84+40.75	0.00	389.25	389.32
O	84+50.75	0.00	389.25	389.29
P	84+60.75	0.00	389.25	389.27
☉ Pier 2	84+73.25	0.00	389.25	389.25
Q	84+83.25	0.00	389.25	389.26
R	84+93.25	0.00	389.25	389.28
S	85+03.25	0.00	389.25	389.31
T	85+13.25	0.00	389.25	389.33
U	85+23.25	0.00	389.25	389.33
V	85+33.25	0.00	389.25	389.32
W	85+43.25	0.00	389.25	389.30
☉ Brg. E. Abut.	85+55.25	0.00	389.25	389.25
Bk. E. Abut.	85+56.92	0.00	389.25	389.25

STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	82+87.08	1.75	389.22	389.22
☉ Brg. W. Abut.	82+88.75	1.75	389.22	389.22
A	82+98.75	1.75	389.22	389.26
B	83+08.75	1.75	389.22	389.29
C	83+18.75	1.75	389.22	389.31
D	83+28.75	1.75	389.22	389.31
E	83+38.75	1.75	389.22	389.29
F	83+48.75	1.75	389.22	389.26
G	83+58.75	1.75	389.22	389.24
☉ Pier 1	83+70.75	1.75	389.22	389.22
H	83+80.75	1.75	389.22	389.23
I	83+90.75	1.75	389.22	389.26
J	84+00.75	1.75	389.22	389.28
K	84+10.75	1.75	389.22	389.31
L	84+20.75	1.75	389.22	389.32
M	84+30.75	1.75	389.22	389.31
N	84+40.75	1.75	389.22	389.29
O	84+50.75	1.75	389.22	389.27
P	84+60.75	1.75	389.22	389.24
☉ Pier 2	84+73.25	1.75	389.22	389.22
Q	84+83.25	1.75	389.22	389.23
R	84+93.25	1.75	389.22	389.26
S	85+03.25	1.75	389.22	389.28
T	85+13.25	1.75	389.22	389.30
U	85+23.25	1.75	389.22	389.30
V	85+33.25	1.75	389.22	389.29
W	85+43.25	1.75	389.22	389.27
☉ Brg. E. Abut.	85+55.25	1.75	389.22	389.22
Bk. E. Abut.	85+56.92	1.75	389.22	389.22

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	82+87.08	2.63	389.21	389.21
☉ Brg. W. Abut.	82+88.75	2.63	389.21	389.21
A	82+98.75	2.63	389.21	389.25
B	83+08.75	2.63	389.21	389.28
C	83+18.75	2.63	389.21	389.29
D	83+28.75	2.63	389.21	389.29
E	83+38.75	2.63	389.21	389.28
F	83+48.75	2.63	389.21	389.25
G	83+58.75	2.63	389.21	389.22
☉ Pier 1	83+70.75	2.63	389.21	389.21
H	83+80.75	2.63	389.21	389.22
I	83+90.75	2.63	389.21	389.24
J	84+00.75	2.63	389.21	389.27
K	84+10.75	2.63	389.21	389.29
L	84+20.75	2.63	389.21	389.30
M	84+30.75	2.63	389.21	389.30
N	84+40.75	2.63	389.21	389.28
O	84+50.75	2.63	389.21	389.25
P	84+60.75	2.63	389.21	389.23
☉ Pier 2	84+73.25	2.63	389.21	389.21
Q	84+83.25	2.63	389.21	389.22
R	84+93.25	2.63	389.21	389.24
S	85+03.25	2.63	389.21	389.27
T	85+13.25	2.63	389.21	389.29
U	85+23.25	2.63	389.21	389.29
V	85+33.25	2.63	389.21	389.28
W	85+43.25	2.63	389.21	389.25
☉ Brg. E. Abut.	85+55.25	2.63	389.21	389.21
Bk. E. Abut.	85+56.92	2.63	389.21	389.21

**BEAM 5**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	82+87.08	7.88	389.13	389.13
☉ Brg. W. Abut.	82+88.75	7.88	389.13	389.13
A	82+98.75	7.88	389.13	389.17
B	83+08.75	7.88	389.13	389.20
C	83+18.75	7.88	389.13	389.21
D	83+28.75	7.88	389.13	389.21
E	83+38.75	7.88	389.13	389.19
F	83+48.75	7.88	389.13	389.17
G	83+58.75	7.88	389.13	389.14
☉ Pier 1	83+70.75	7.88	389.13	389.13
H	83+80.75	7.88	389.13	389.14
I	83+90.75	7.88	389.13	389.16
J	84+00.75	7.88	389.13	389.19
K	84+10.75	7.88	389.13	389.21
L	84+20.75	7.88	389.13	389.22
M	84+30.75	7.88	389.13	389.21
N	84+40.75	7.88	389.13	389.20
O	84+50.75	7.88	389.13	389.17
P	84+60.75	7.88	389.13	389.14
☉ Pier 2	84+73.25	7.88	389.13	389.13
Q	84+83.25	7.88	389.13	389.14
R	84+93.25	7.88	389.13	389.16
S	85+03.25	7.88	389.13	389.19
T	85+13.25	7.88	389.13	389.20
U	85+23.25	7.88	389.13	389.21
V	85+33.25	7.88	389.13	389.20
W	85+43.25	7.88	389.13	389.17
☉ Brg. E. Abut.	85+55.25	7.88	389.13	389.13
Bk. E. Abut.	85+56.92	7.88	389.13	389.13

**BEAM 6**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	82+87.08	13.13	389.04	389.04
☉ Brg. W. Abut.	82+88.75	13.13	389.04	389.04
A	82+98.75	13.13	389.04	389.08
B	83+08.75	13.13	389.04	389.11
C	83+18.75	13.13	389.04	389.13
D	83+28.75	13.13	389.04	389.13
E	83+38.75	13.13	389.04	389.11
F	83+48.75	13.13	389.04	389.09
G	83+58.75	13.13	389.04	389.06
☉ Pier 1	83+70.75	13.13	389.04	389.04
H	83+80.75	13.13	389.04	389.05
I	83+90.75	13.13	389.04	389.08
J	84+00.75	13.13	389.04	389.11
K	84+10.75	13.13	389.04	389.13
L	84+20.75	13.13	389.04	389.14
M	84+30.75	13.13	389.04	389.13
N	84+40.75	13.13	389.04	389.11
O	84+50.75	13.13	389.04	389.09
P	84+60.75	13.13	389.04	389.06
☉ Pier 2	84+73.25	13.13	389.04	389.04
Q	84+83.25	13.13	389.04	389.05
R	84+93.25	13.13	389.04	389.08
S	85+03.25	13.13	389.04	389.10
T	85+13.25	13.13	389.04	389.12
U	85+23.25	13.13	389.04	389.13
V	85+33.25	13.13	389.04	389.12
W	85+43.25	13.13	389.04	389.09
☉ Brg. E. Abut.	85+55.25	13.13	389.04	389.04
Bk. E. Abut.	85+56.92	13.13	389.04	389.04



**NORTH EDGE OF SHOULDER**

Location	Station	Offset	Theoretical Grade Elevations
End of W. Approach Slab	82+58.08	-15.00	389.02
A	85+68.08	-15.00	389.02
B	82+78.08	-15.00	389.02
Bk. W. Abutment	82+88.08	-15.00	389.02

**NORTH EDGE OF PAVEMENT**

Location	Station	Offset	Theoretical Grade Elevations
End of W. Approach Slab	82+58.08	-11.00	389.08
A	85+68.08	-11.00	389.08
B	82+78.08	-11.00	389.08
Bk. W. Abutment	82+88.08	-11.00	389.08

**CL PROPOSED ROADWAY & P.G.**

Location	Station	Offset	Theoretical Grade Elevations
End of W. Approach Slab	82+58.08	0.00	389.25
A	85+68.08	0.00	389.25
B	82+78.08	0.00	389.25
Bk. W. Abutment	82+88.08	0.00	389.25

**STAGE CONSTRUCTION LINE**

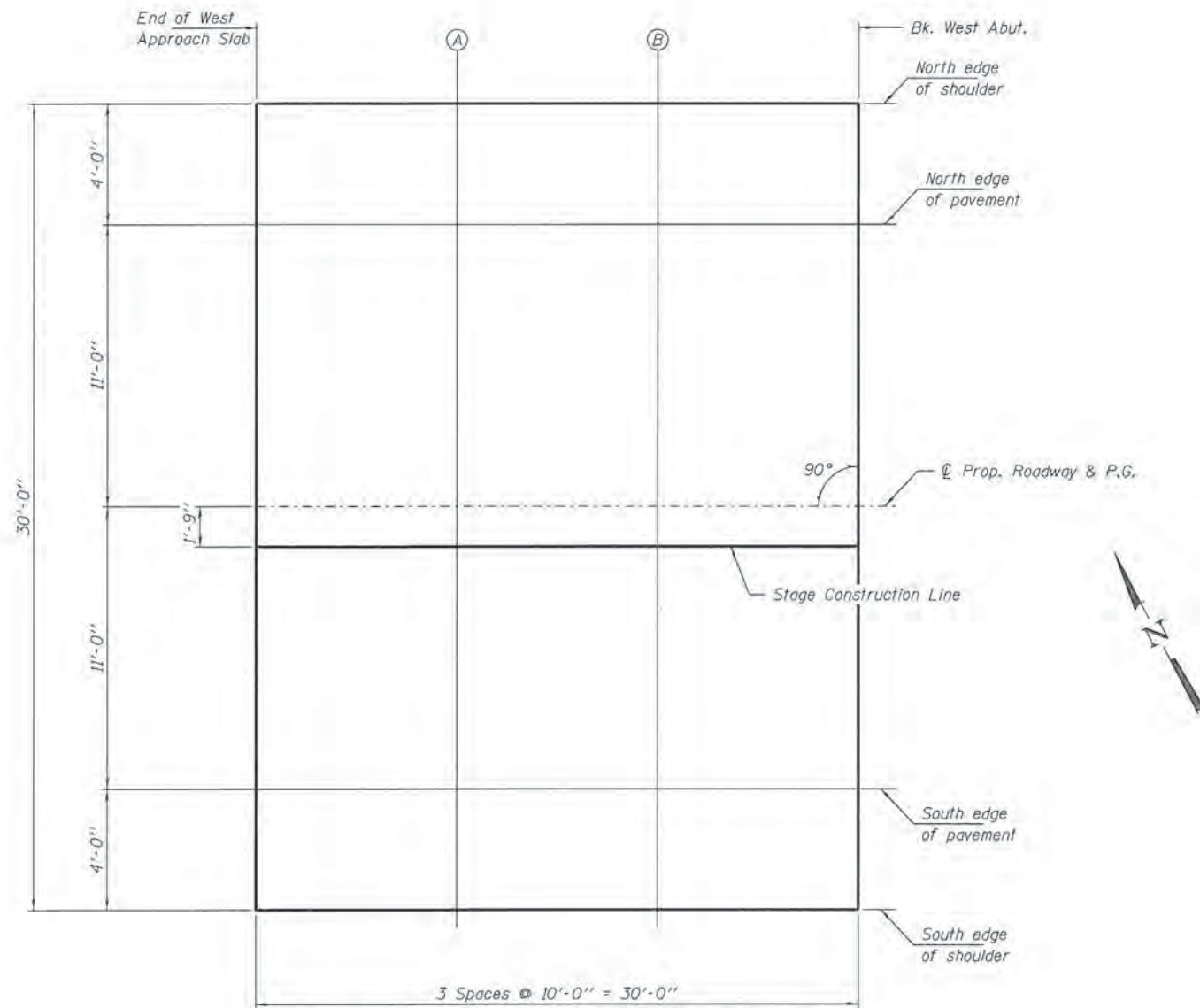
Location	Station	Offset	Theoretical Grade Elevations
End of W. Approach Slab	82+58.08	1.75	389.22
A	85+68.08	1.75	389.22
B	82+78.08	1.75	389.22
Bk. W. Abutment	82+88.08	1.75	389.22

**SOUTH EDGE OF PAVEMENT**

Location	Station	Offset	Theoretical Grade Elevations
End of W. Approach Slab	82+58.08	11.00	389.08
A	85+68.08	11.00	389.08
B	82+78.08	11.00	389.08
Bk. W. Abutment	82+88.08	11.00	389.08

**SOUTH EDGE OF SHOULDER**

Location	Station	Offset	Theoretical Grade Elevations
End of W. Approach Slab	82+58.08	15.00	389.02
A	85+68.08	15.00	389.02
B	82+78.08	15.00	389.02
Bk. W. Abutment	82+88.08	15.00	389.02



**WEST APPROACH SLAB - PLAN**

NORTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment	85+55.92	-15.00	389.02
A	85+65.92	-15.00	389.02
B	85+75.92	-15.00	389.02
Bk. E. Approach Slab	85+85.92	-15.00	389.02

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment	85+55.92	-11.00	389.08
A	85+65.92	-11.00	389.08
B	85+75.92	-11.00	389.08
Bk. E. Approach Slab	85+85.92	-11.00	389.08

☉ PROPOSED ROADWAY & P.G.

Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment	85+55.92	0.00	389.25
A	85+65.92	0.00	389.25
B	85+75.92	0.00	389.25
Bk. E. Approach Slab	85+85.92	0.00	389.25

STAGE CONSTRUCTION LINE

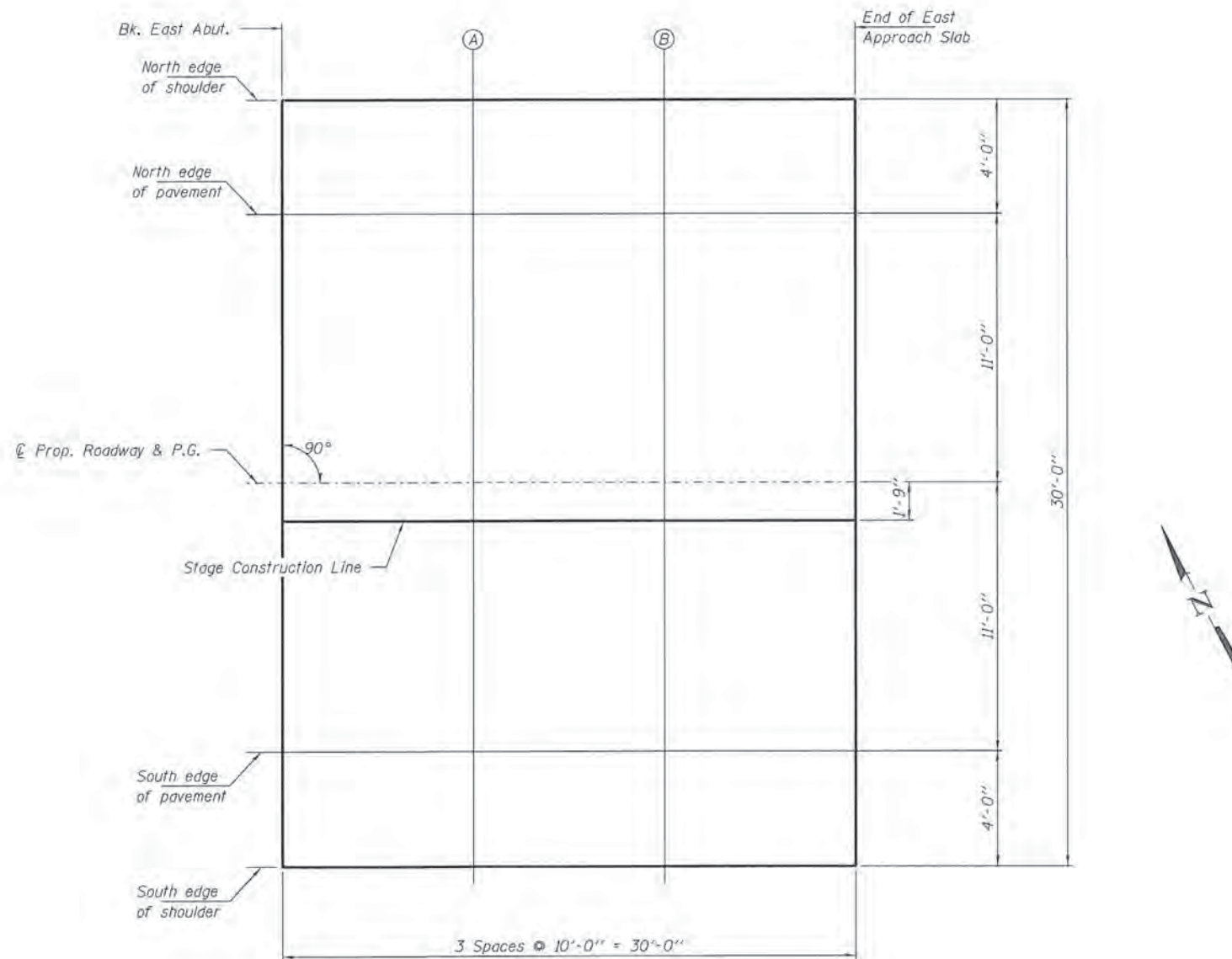
Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment	85+55.92	1.75	389.22
A	85+65.92	1.75	389.22
B	85+75.92	1.75	389.22
Bk. E. Approach Slab	85+85.92	1.75	389.22

SOUTH EDGE OF PAVEMENT

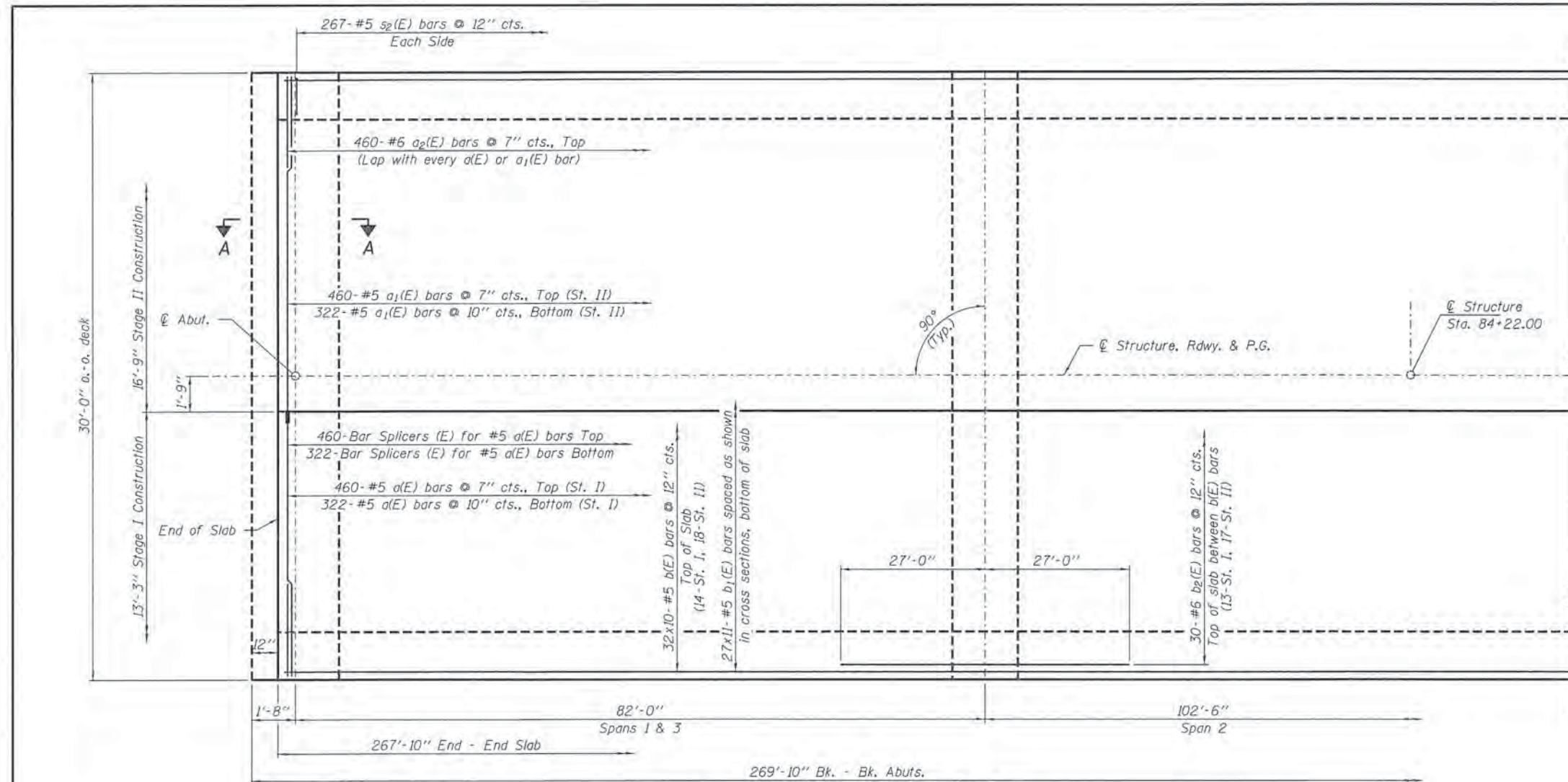
Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment	85+55.92	11.00	389.08
A	85+65.92	11.00	389.08
B	85+75.92	11.00	389.08
Bk. E. Approach Slab	85+85.92	11.00	389.08

SOUTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment	85+55.92	15.00	389.02
A	85+65.92	15.00	389.02
B	85+75.92	15.00	389.02
Bk. E. Approach Slab	85+85.92	15.00	389.02



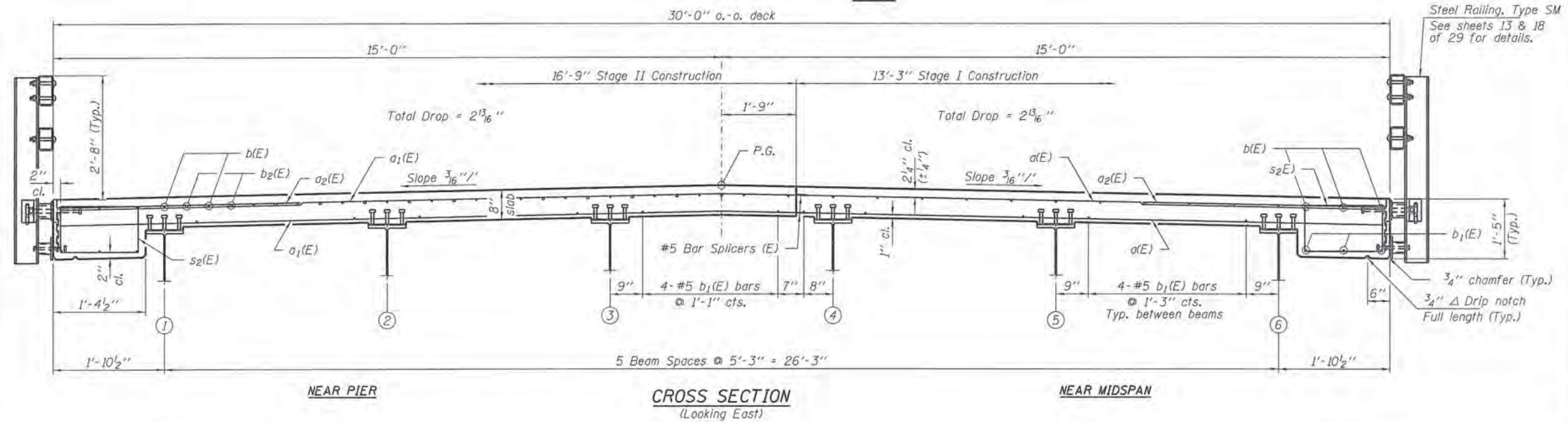
EAST APPROACH SLAB - PLAN



**MIN. BAR LAP**  
 #5 bars = 2'-7"  
 #6 bars = 3'-1"

Notes:  
 See sheet 13 of 29 for superstructure details.  
 Bars indicated thus 32x10-#5 etc. indicates 32 lines of bars with 10 lengths per line.  
 See sheet 13 of 29 for Section A-A.  
 See sheet 27 of 29 for Bar Splicer Details.

PLAN



CROSS SECTION  
(Looking East)

**SUPERSTRUCTURE  
BILL OF MATERIAL**

BAR	NO.	SIZE	LENGTH	SHAPE
a(E)	782	#5	12'-11"	—
a1(E)	782	#5	16'-5"	—
a2(E)	920	#6	6'-6"	—
b(E)	320	#5	29'-1"	—
b1(E)	297	#5	26'-8"	—
b2(E)	60	#6	54'-0"	—
m(E)	8	#6	12'-11"	—
m1(E)	24	#6	4'-10"	—
m2(E)	12	#6	1'-5"	—
m3(E)	36	#5	4'-0"	—
m4(E)	8	#6	16'-5"	—
m5(E)	6	#6	4'-0"	—
s(E)	62	#5	7'-9"	□
s1(E)	62	#5	9'-8"	□
s2(E)	534	#5	5'-2"	□
Concrete Superstructure			Cu. Yd.	243.5
Bridge Deck Grooving			Sq. Yd.	833
Protective Coat			Sq. Yd.	1,059
Reinforcement Bars, Epoxy Coated			Pound	60,480
Bar Splicers			Each	790

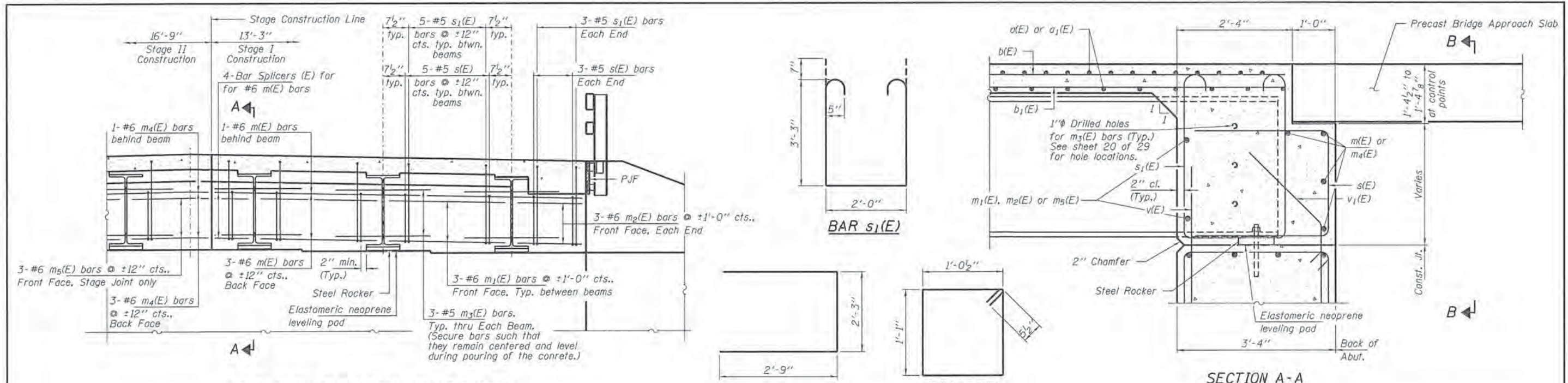
FILE NAME = 130340-ahb-bridge.dgn	USER NAME =	DESIGNED - S.M.S.	REVISED -
3065 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.244.3450 www.nhrengineering.com		CHECKED - D.W.T.	REVISED -
		DRAWN - D.A.B.	REVISED -
		CHECKED - M.D.C.	REVISED -
	PLOT SCALE =		
	PLOT DATE = 4/6/2015		

STATE OF ILLINOIS  
 JACKSON COUNTY HIGHWAY DEPARTMENT

SUPERSTRUCTURE  
 STRUCTURE NO. 039-3277

SHEET NO. 12 OF 29 SHEETS

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
869	10-00163-00-BR	JACKSON	82	27
			CONTRACT NO. 99519	
(ILLINOIS) FED. AID PROJECT BR5-08691061				

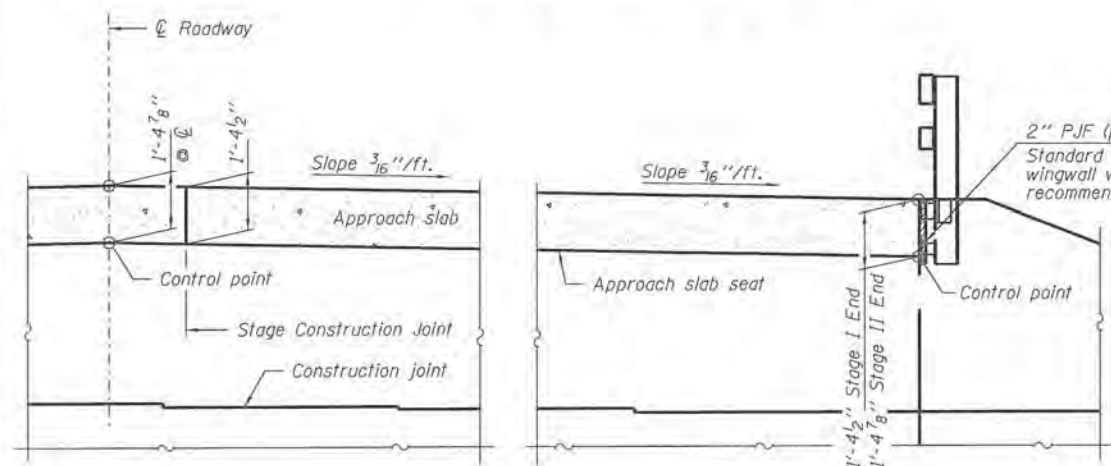


**DIAPHRAGM ELEVATION AT ABUTMENT**

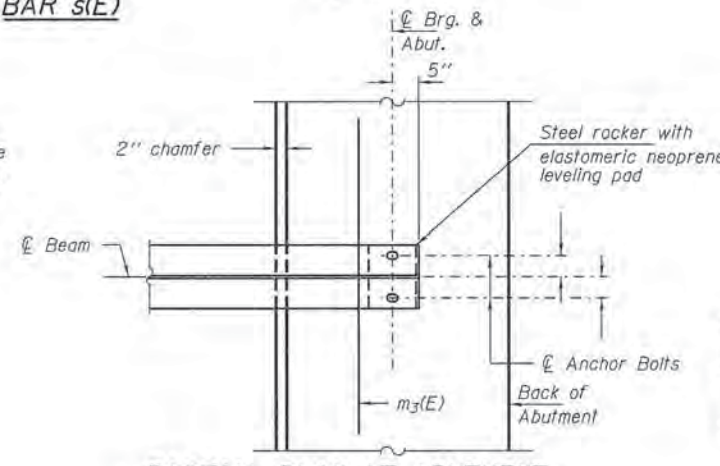
**BAR s(E)**

**BAR s2(E)**

**SECTION A-A**

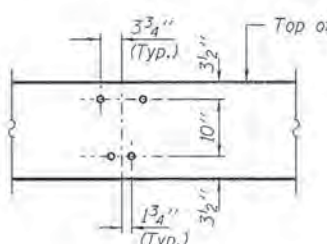


**SECTION B-B**

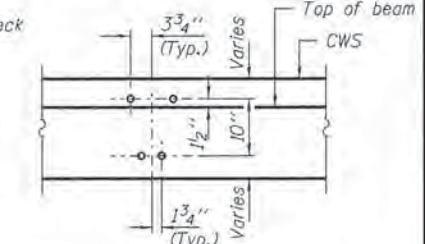


**PARTIAL PLAN AT ABUTMENT**  
(Showing bottom flange of beam)

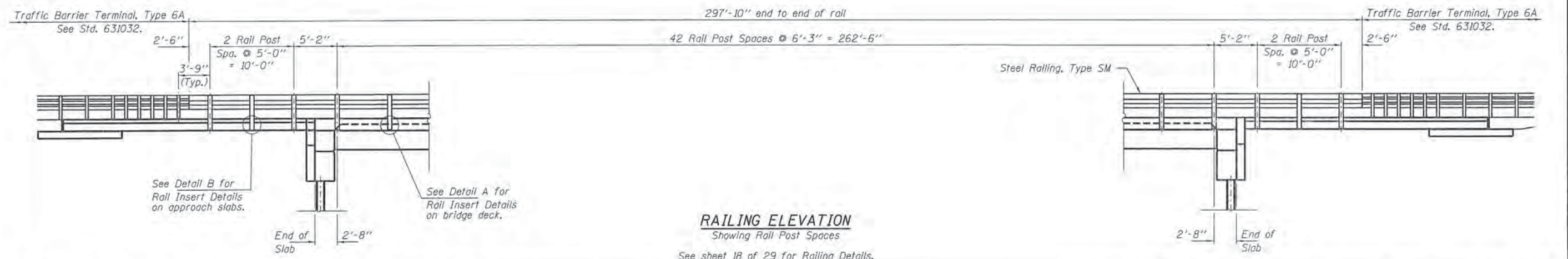
**Notes:**  
 Reinforcement bars in diaphragm are billed with superstructure on sheet 12 of 29.  
 Concrete in diaphragm is included with Concrete Superstructure on sheet 12 of 29.  
 For details of bars v(E) and v1(E) see sheet 23 & 24 of 29.  
 The approach slab seat shall have a constant slope determined from the control points shown.  
 For bearing details see sheet 22 of 29.  
 For Bar Splicer details see sheet 27 of 29.



**DETAIL A**



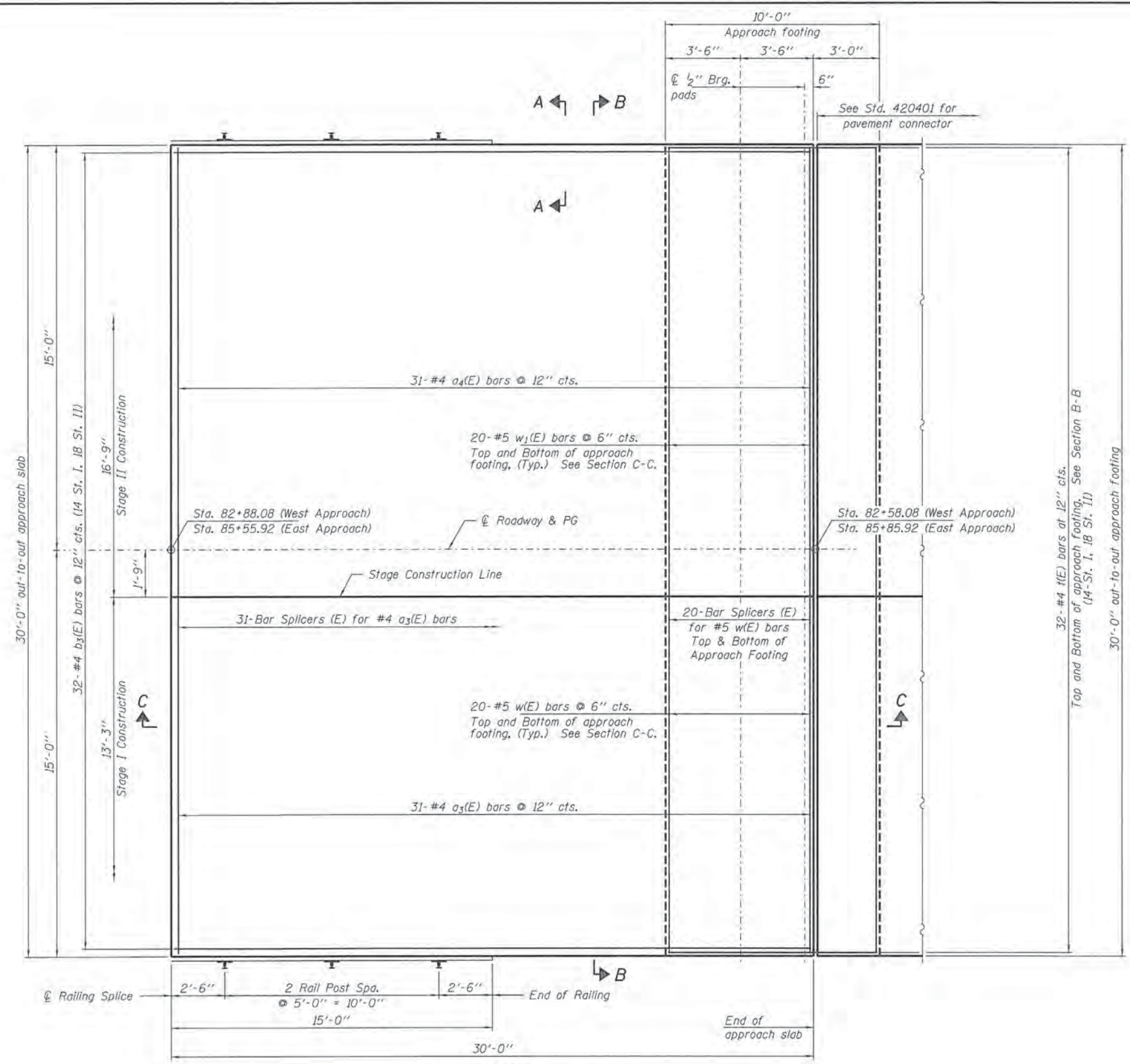
**DETAIL B**



**RAILING ELEVATION**  
(Showing Rail Post Spaces)

See sheet 18 of 29 for Railing Details.

FILE NAME * 130340-sht-bridge.dgn	USER NAME *	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>SUPERSTRUCTURE DETAILS STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.548.3400 www.hbrengineering.com		CHECKED - D.W.T.	REVISED -			869	10-00163-00-BR	JACKSON	82	28	
161.800959 ILLINOIS PROFESSIONAL DESIGN FIRM L6 / PE / SE CORPORATION	PLOT SCALE *	DRAWN - D.A.B.	REVISED -			<b>CONTRACT NO. 99519</b>					
	PLOT DATE * 4/6/2015	CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT BR5-08691061					



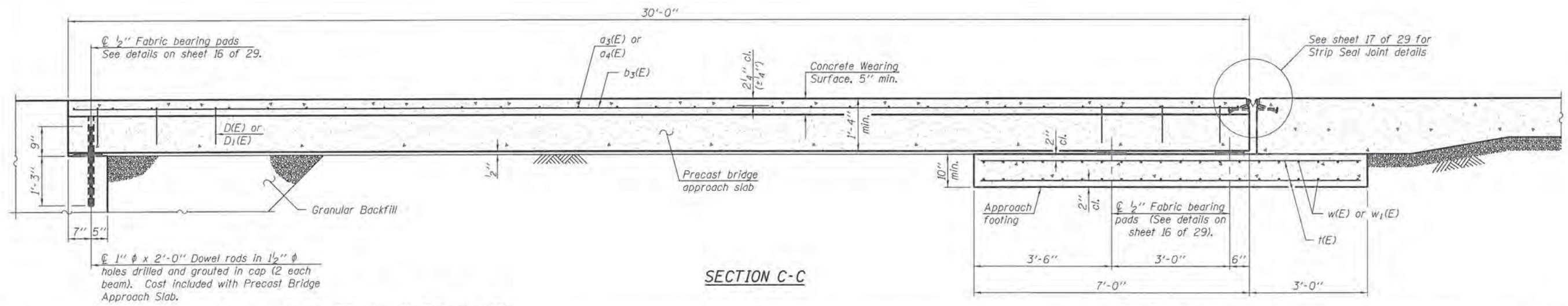
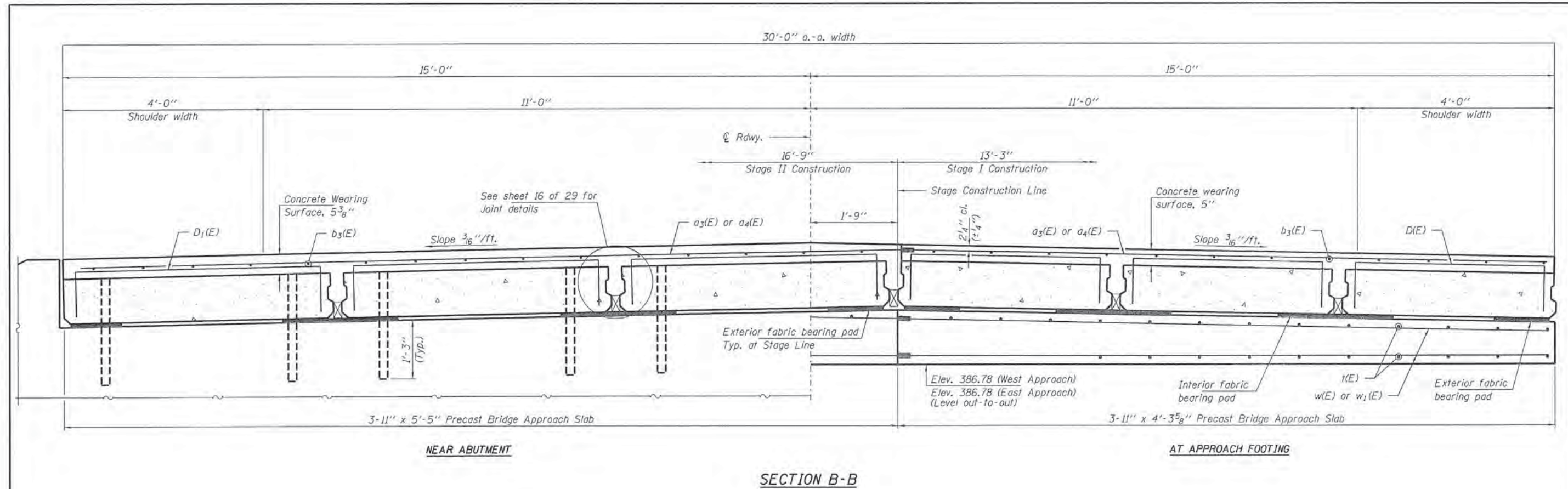
SECTION A-A

**PLAN**  
(Showing wearing surface)  
(East Approach Slab Shown)  
(West Approach Slab similar by 180° rotation)

(Beams: 36" min. width; 72" max. width)

(Sheet 1 of 4)

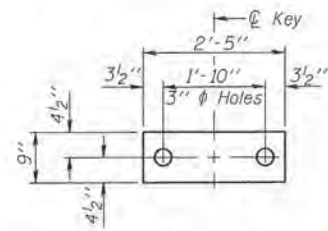
FILE NAME = 130340-sht-bridge.dgn 3095 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.544.3400 www.jiangengineering.com 184.000889 ILLINOIS PROFESSIONAL DESIGN FIRM L3 / PE / SE CORPORATION	USER NAME =	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS</b> <b>JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>PRECAST BRIDGE APPROACH SLAB</b> <b>STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE =	CHECKED - D.W.T.	REVISED -			869	10-00163-00-BR	JACKSON	82	29
PLOT DATE = 4/6/2015	CHECKED - M.D.C.	REVISED -		SHEET NO. 14 OF 29 SHEETS		CONTRACT NO. 99519		ILLINOIS FED. AID PROJECT BRS-0869106		



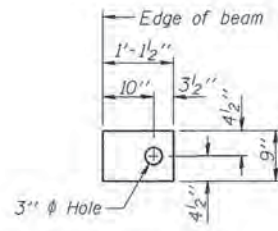
FILE NAME * 130340-sht-bridge.dgn	USER NAME *	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>PRECAST BRIDGE APPROACH SLAB STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
2083 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.546.3400 www.jhrengineering.com	PLLOT SCALE =	CHECKED - D.W.T.	REVISED -			869	10-00163-00-BR	JACKSON	82	30	
184 000959 ILLINOIS PROFESSIONAL DESIGN FIRM L6/PE/ISS CORPORATION	PLLOT DATE = 4/6/2015	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 99519					
		CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT BR5-08691061					

(Beams: 36" min. width; 72" max. width)

(Sheet 2 of 4)



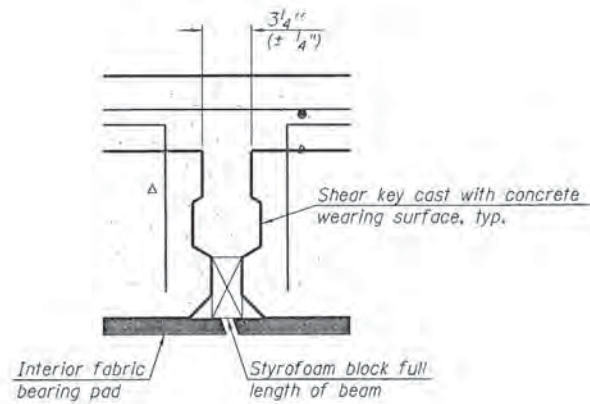
INTERIOR



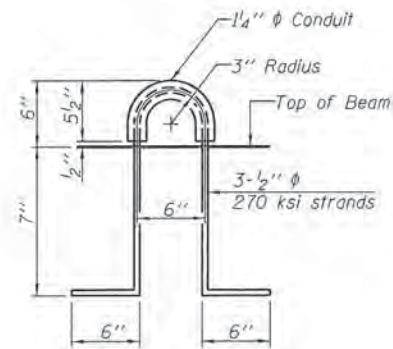
EXTERIOR

**FABRIC BEARING PAD**

Notes:  
 All bearing pads shall be 1/2" thick.  
 Omit holes for fabric bearing pads at approach slab footing end of beams.  
 Expansion bearing pad shall be bonded to the approach slab footing.

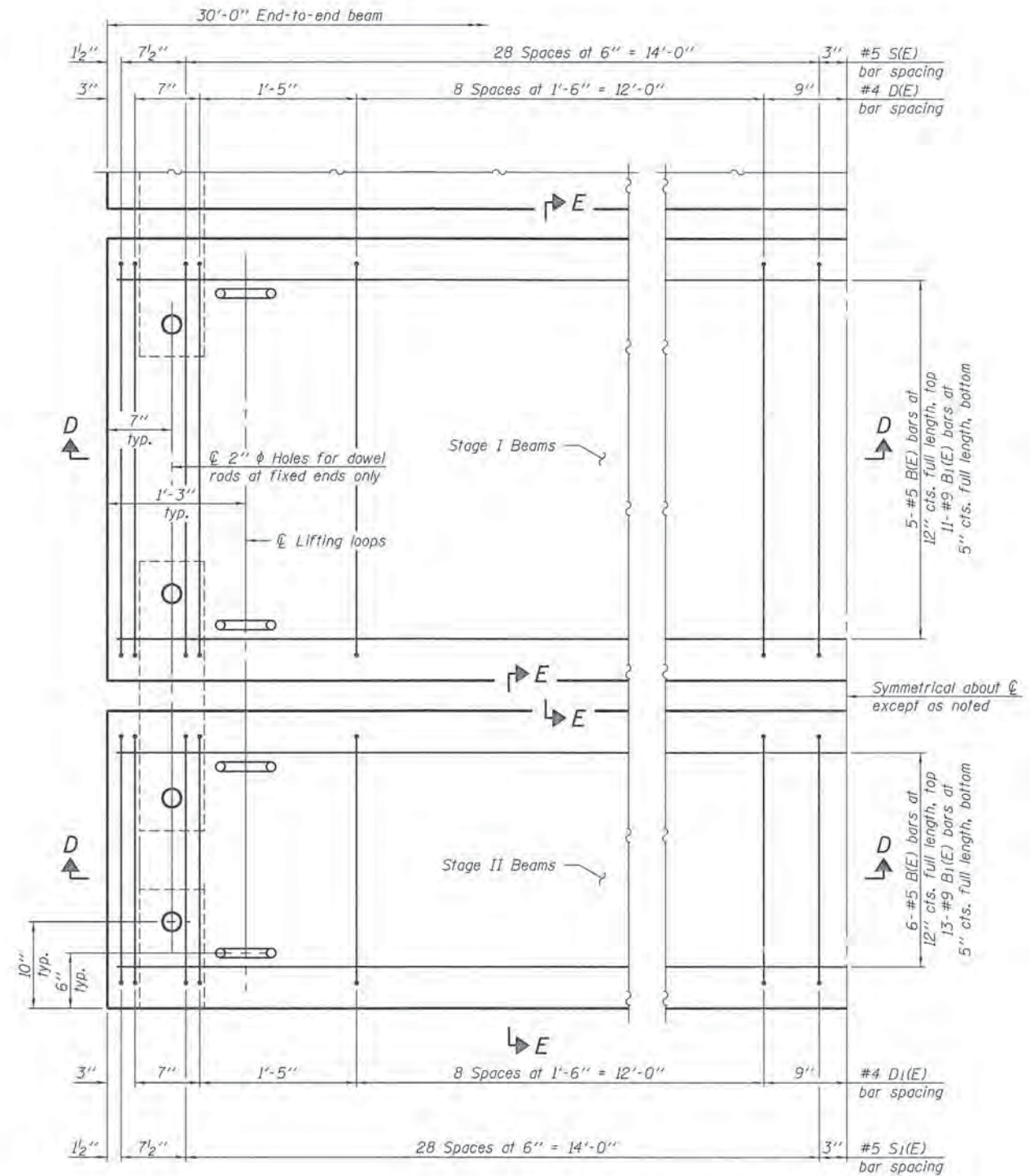


**SECTION THRU SHEAR KEY JOINT**



**LIFTING LOOP DETAIL**

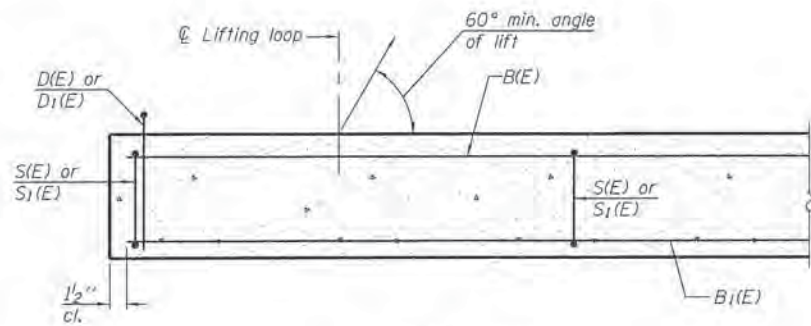
(Beams: 36" min. width; 72" max. width)



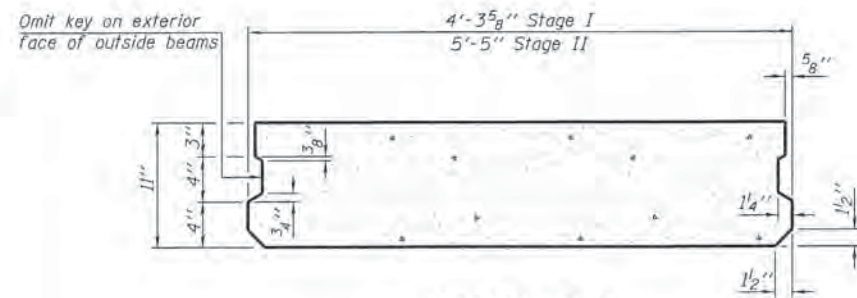
**PLAN**

(Sheet 3 of 4)

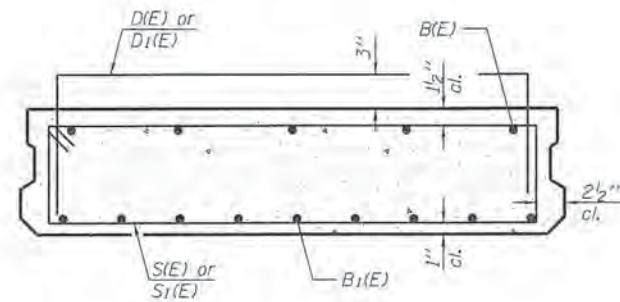
FILE NAME * 130348-shr-bridge.dgn	USER NAME *	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>PRECAST BRIDGE APPROACH SLAB STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
2065 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.545.3405 www.phengineering.com	PLOT SCALE *	CHECKED - D.W.T.	REVISED -			869	10-00163-00-BR	JACKSON	82	31	
184.00059 ILLINOIS PROFESSIONAL DESIGN FIRM L6 / PE / SE CORPORATION	PLOT DATE * 4/6/2015	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 99519					
						SHEET NO. 16 OF 29 SHEETS					
						ILLINOIS FED. AID PROJECT BRS-0869106					



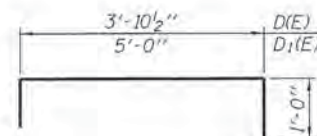
SECTION D-D



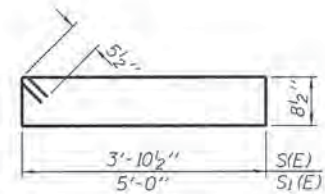
SECTION E-E  
(Showing dimensions)



SECTION E-E  
(Showing reinforcement)

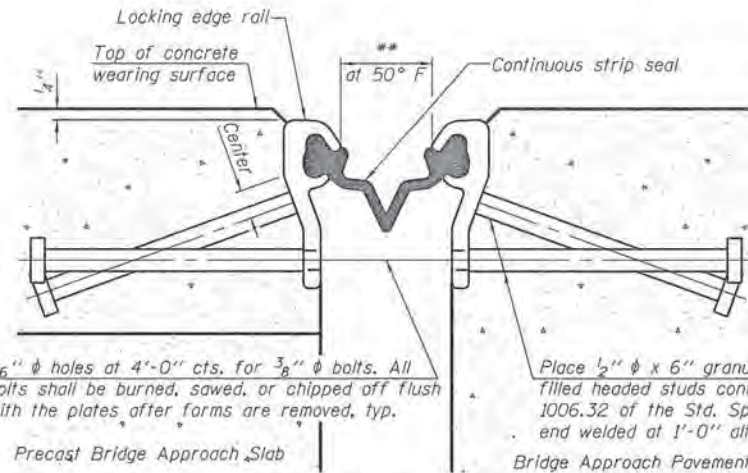


BARS D1(E) & D1(E)



BARS S1(E) & S1(E)

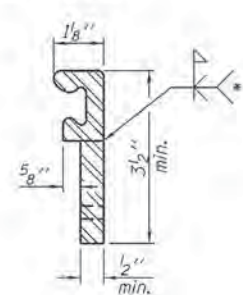
(Beams: 36" min. width; 72" max. width)



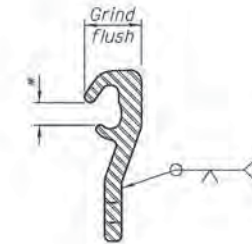
SECTION THRU STRIP  
SEAL JOINT  
(at rt. angles)



ROLLED  
(EXTRUDED) RAIL



WELDED RAIL



LOCKING EDGE  
RAIL SPLICE  
Rolled rail shown, welded  
rail similar.

LOCKING EDGE RAIL

- \* Omit weld at seal opening.
- \*\* The joint opening shall be determined per Article 520.04 except that on jointless structures, the distance described as the bridge length between the nearest fixed bearings each way from the joint shall be taken as half the bridge length plus the approach slab length. The minimum dimension shall be 1 1/2" for installation purposes.
- \*\*\* Back gouge not required if complete joint penetration is verified by mock-up.

BAR LIST  
EACH STAGE I BEAM  
(For information only)

Bar	No.	Size	Length	Shape
B(E)	5	#5	29'-8"	—
B1(E)	11	#9	29'-8"	—
D1(E)	22	#4	5'-11"	□
S1(E)	58	#5	10'-1"	□

BAR LIST  
EACH STAGE II BEAM  
(For information only)

Bar	No.	Size	Length	Shape
B(E)	6	#5	29'-8"	—
B1(E)	13	#9	29'-8"	—
D1(E)	22	#4	7'-0"	□
S1(E)	58	#5	12'-4"	□

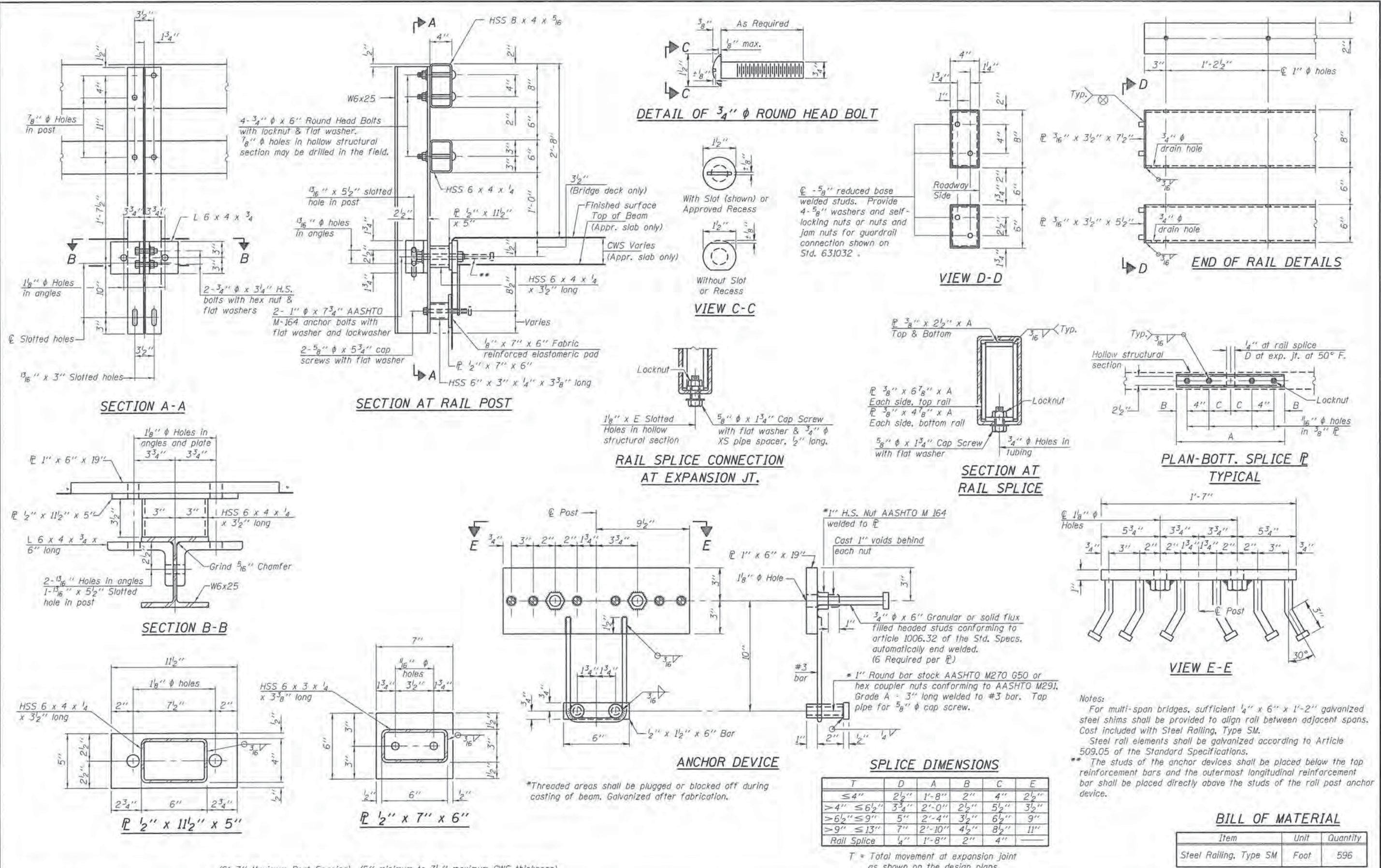
Notes:  
The precast bridge approach slab shall be according to Section 504 of the Standard Specifications and shall be paid for at the contract unit price per square foot for Precast Bridge Approach Slab.  
Cast-in-place substitution of Precast Bridge Approach Slab is not allowed. Wearing surface reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.  
Approach footing concrete shall be paid for as Concrete Structures.  
The top surface of precast bridge approach slabs shall be roughened to a depth of 1/4" according to the IDOT "Manual for Fabrication of Precast Prestressed Concrete Products."  
After precast bridge approach slab has been erected, holes shall be drilled into abutment and anchor dowels placed. Dowel holes shall be filled with non-shrink grout to top of precast slab and allowed to cure fully prior to grouting the longitudinal shear keys.  
Two 5/8" fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location. Cost included with Precast Bridge Approach Slab.  
A minimum 2 1/2" diameter lifting pins shall be used to engage the lifting loops during handling.  
Compressive strength of precast concrete, f'c shall be 6,000 psi.  
For additional rail details, see sheets 13 & 18 of 29.  
Any concrete poured monolithically with the wearing surface, such as curbs, will not be paid for separately, but will be included in the cost of Concrete Wearing Surface, 5".  
The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The strip seal shall extend 6" beyond the edge of the approach slab on each end. The configuration of the strip seal shall match the configuration of the Locking Edge Rails.  
The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed.  
The inside of the Locking Edge Rail groove shall be free of weld residue. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.  
The manufacturer's recommended installation methods shall be followed. All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.  
Maximum space between rail segments at stage lines shall be 3/16", sealed with a suitable sealant.

TWO APPROACHES  
BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
a3(E)	62	#4	12'-11"	—	
a4(E)	62	#4	16'-5"	—	
b3(E)	64	#4	29'-8"	—	
H(E)	128	#4	9'-8"	—	
w(E)	80	#5	12'-11"	—	
w1(E)	80	#5	16'-5"	—	
Concrete Structures				Cu. Yd.	21.4
Bridge Deck Grooving				Sq. Yd.	187
Protective Coat				Sq. Yd.	200
Reinforcement Bars, Epoxy Coated				Pound	5,760
Bar Splicers				Each	142
Preformed Joint Strip Seal				Foot	62
Concrete Wearing Surface, 5"				Sq. Yd.	200
Precast Bridge Approach Slab				Sq. Ft.	1,749

(Sheet 4 of 4)

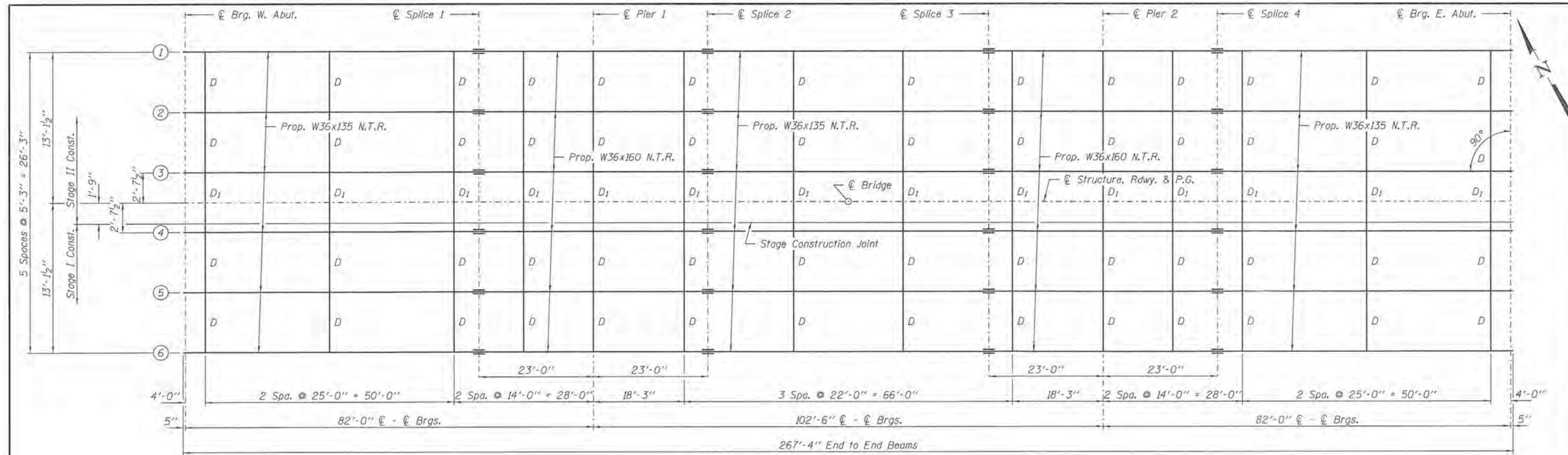




Notes:  
 For multi-span bridges, sufficient 1/4" x 6" x 1'-2" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Steel Railing, Type SM.  
 Steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.  
 \*\* The studs of the anchor devices shall be placed below the top reinforcement bars and the outermost longitudinal reinforcement bar shall be placed directly above the studs of the rail post anchor device.

**BILL OF MATERIAL**

Item	Unit	Quantity
Steel Railing, Type SM	Foot	596

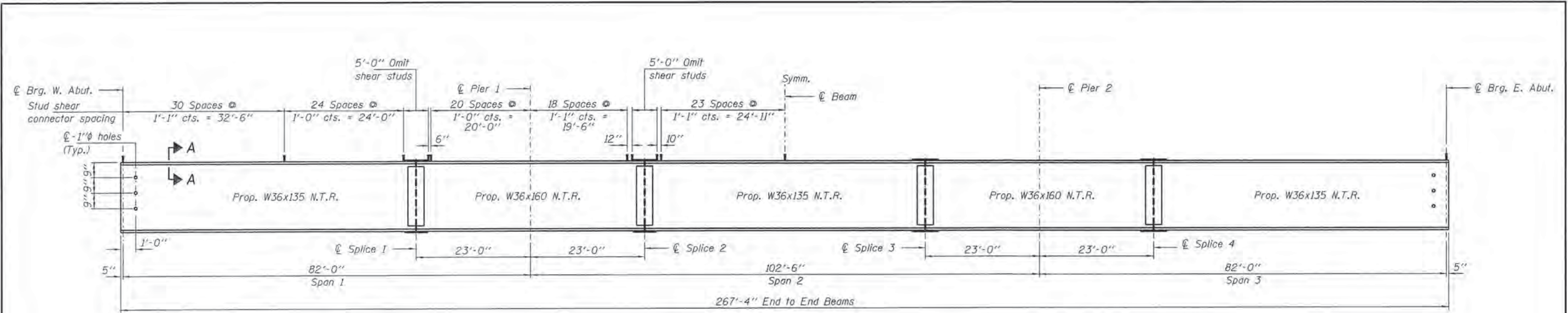


**PLAN**

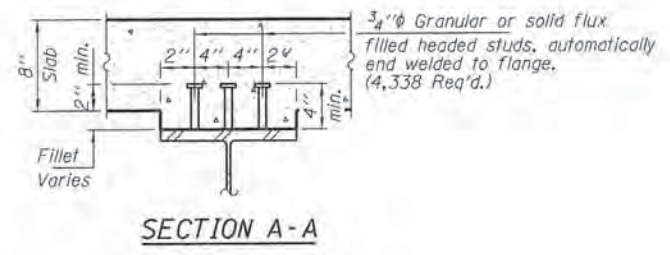
Location	℄ Brg. W. Abut.	℄ Splice 1	℄ Brg. Pier 1	℄ Splice 2	℄ Brg. Splice 3	℄ Pier 2	℄ Splice 4	℄ Brg. E. Abut.
BEAM 1	388.31	388.32	388.32	388.32	388.32	388.32	388.32	388.31
BEAM 2	388.39	388.41	388.41	388.41	388.41	388.41	388.41	388.39
BEAM 3	388.47	388.49	388.49	388.49	388.49	388.49	388.49	388.47
BEAM 4	388.47	388.49	388.49	388.49	388.49	388.49	388.49	388.47
BEAM 5	388.39	388.41	388.41	388.41	388.41	388.41	388.41	388.39
BEAM 6	388.31	388.32	388.32	388.32	388.32	388.32	388.32	388.31

**TOP OF WEB ELEVATIONS**  
(For fabrication only)  
(Does not include Dead Load Deflections)

**Notes:**  
 All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.  
 Load carrying components designated "NTR" shall conform to the Impact Testing Requirements, Zone 2.  
 All beams, diaphragms, connection plates and splices shall be M270 Grade 50W.  
 For Structural Steel details see sheets 20 & 21 of 29.

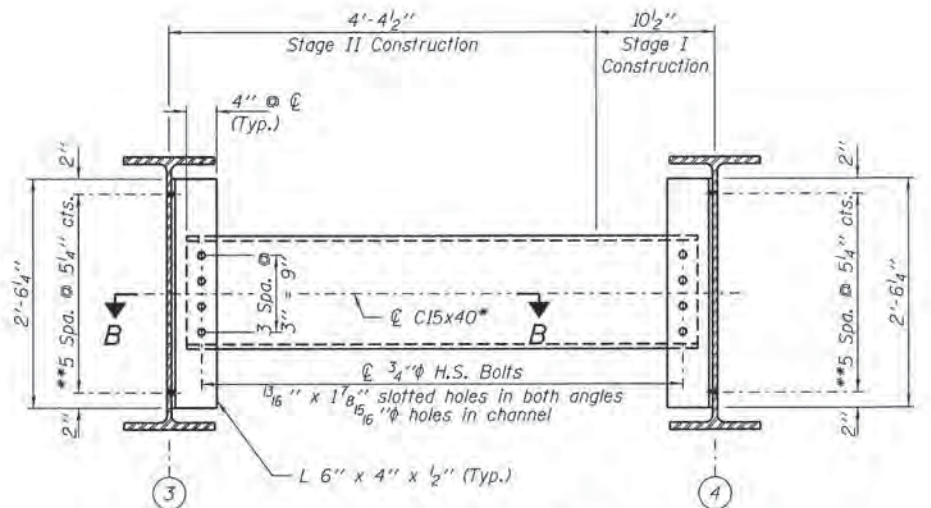
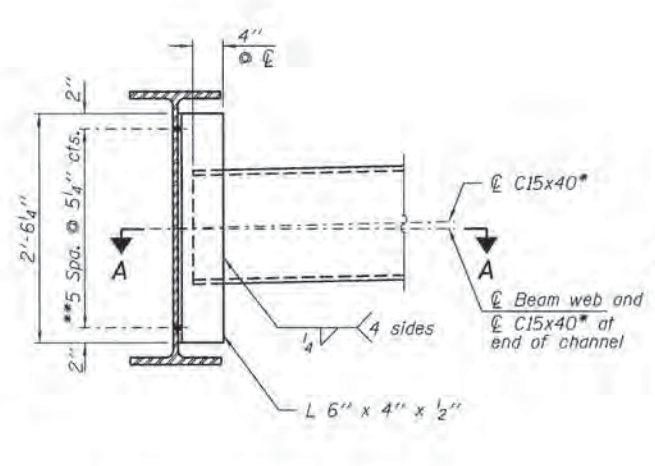
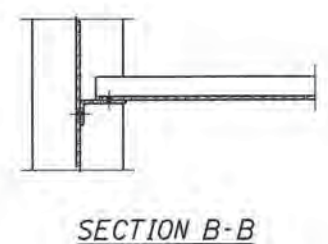
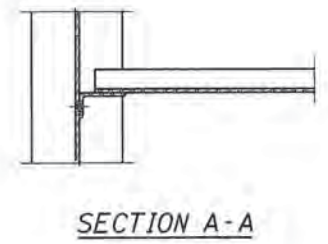


**BEAM ELEVATION**



**Notes:**

- Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
- All beams, diaphragms, connection plates and splices shall be M270 Grade 50W.
- Alternate channels (C15X50) are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.
- \*\*3/4" dia HS bolts, 5/8" dia holes.
- Two hardened washers required for each set of oversized holes.
- All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
- Bolts in slots shall be finger tight until the second stage pour is complete and fully tightened after completion of the deck pour for Stage II Construction. Position slots so bolts start at one end with no concrete load and finish near the opposite end under deck load, allowing maximum displacement without laterally stressing main members.



FILE NAME = 130340-shr-bridge.dgn	USER NAME *	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>STRUCTURAL STEEL DETAILS STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.544.5400 www.ilengineering.com		CHECKED - D.W.T.	REVISED -			869	10-00163-00-BR	JACKSON	82	35	
184.00956 ILLINOIS PROFESSIONAL DESIGN FIRM L3 / PSE INCORPORATION	PLOT SCALE =	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 99519					
	PLOT DATE = 4/6/2015	CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT BR5-08691061					
						SHEET NO. 20 OF 29 SHEETS					

INTERIOR GIRDER MOMENT TABLE				
		0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or 2	0.5 Sp. 2
$I_s$	(in <sup>4</sup> )	7,800	9,760	7,800
$I_c(n)$	(in <sup>4</sup> )	20,821	24,431	20,821
$I_c(3n)$	(in <sup>4</sup> )	15,281	17,833	15,281
$I_c(cr)$	(in <sup>4</sup> )	9,077	12,141	9,077
$S_s$	(in <sup>3</sup> )	438	542	438
$S_c(n)$	(in <sup>3</sup> )	649	775	649
$S_c(3n)$	(in <sup>3</sup> )	585	698	585
$S_c(cr)$	(in <sup>3</sup> )	420	577	420
DC1	(k/')	0.72	0.75	0.72
M <sub>DC1</sub>	(k)	317	654	290
DC2	(k/')	0.04	0.04	0.04
M <sub>DC2</sub>	(k)	16	31	15
DW	(k/')	0.27	0.27	0.27
M <sub>DW</sub>	(k)	119	236	111
M <sub>l + IM</sub>	(k)	839	961	817
M <sub>u</sub> (Strength I)	(k)	2,063	2,892	1,978
$\phi_r M_n$	(k)	3,240	-	3,263
$f_s$ DC1	(ksi)	8.7	14.5	7.9
$f_s$ DC2	(ksi)	0.3	0.6	0.3
$f_s$ DW	(ksi)	2.4	4.9	2.3
$f_s$ (l + IM)	(ksi)	15.5	20.0	15.1
$f_s$ (Service II)	(ksi)	31.6	46.0	30.2
0.95R <sub>n</sub> F <sub>yf</sub>	(ksi)	47.5	47.5	47.5
$f_s$ (Total)(Strength I)	(ksi)	-	61.2	-
$\phi_r F_n$	(ksi)	-	-	-
V <sub>r</sub>	(k)	25.2	-	27.2

INTERIOR GIRDER REACTION TABLE			
	Abut.	Pier 1 or 2	
R <sub>DC1</sub>	(k)	21.4	74.9
R <sub>DC2</sub>	(k)	1.1	3.6
R <sub>DW</sub>	(k)	8.0	27.2
R <sub>l + IM</sub>	(k)	65.3	116.7
R <sub>Total</sub>	(k)	95.8	222.4

$I_s, S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$  (Total-Strength I, and Service II) due to non-composite dead loads (in<sup>4</sup> and in<sup>3</sup>).

$I_c(n), S_c(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing  $f_s$  (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in<sup>4</sup> and in<sup>3</sup>).

$I_c(3n), S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in<sup>4</sup> and in<sup>3</sup>).

$I_c(cr), S_c(cr)$ : Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing  $f_s$  (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in<sup>4</sup> and in<sup>3</sup>).

DC1: Un-factored non-composite dead load (kips/ft.).

M<sub>DC1</sub>: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M<sub>DC2</sub>: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M<sub>DW</sub>: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M<sub>l + IM</sub>: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M<sub>u</sub> (Strength I): Factored design moment (kip-ft.).

$1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{l + IM}$

$\phi_r M_n$ : Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft.).

$f_s$  DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

$M_{DC1} / S_{nc}$

$f_s$  DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).

$M_{DC2} / S_c(3n)$  or  $M_{DC2} / S_c(cr)$  as applicable.

$f_s$  DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

$M_{DW} / S_c(3n)$  or  $M_{DW} / S_c(cr)$  as applicable.

$f_s$  (l + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).

$M_{l + IM} / S_c(n)$  or  $M_{l + IM} / S_c(cr)$  as applicable.

$f_s$  (Service II): Sum of stresses as computed below (ksi).

$f_{sDC1} + f_{sDC2} + f_{sDW} + 1.3 f_s (l + IM)$

0.95R<sub>n</sub>F<sub>yf</sub>: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

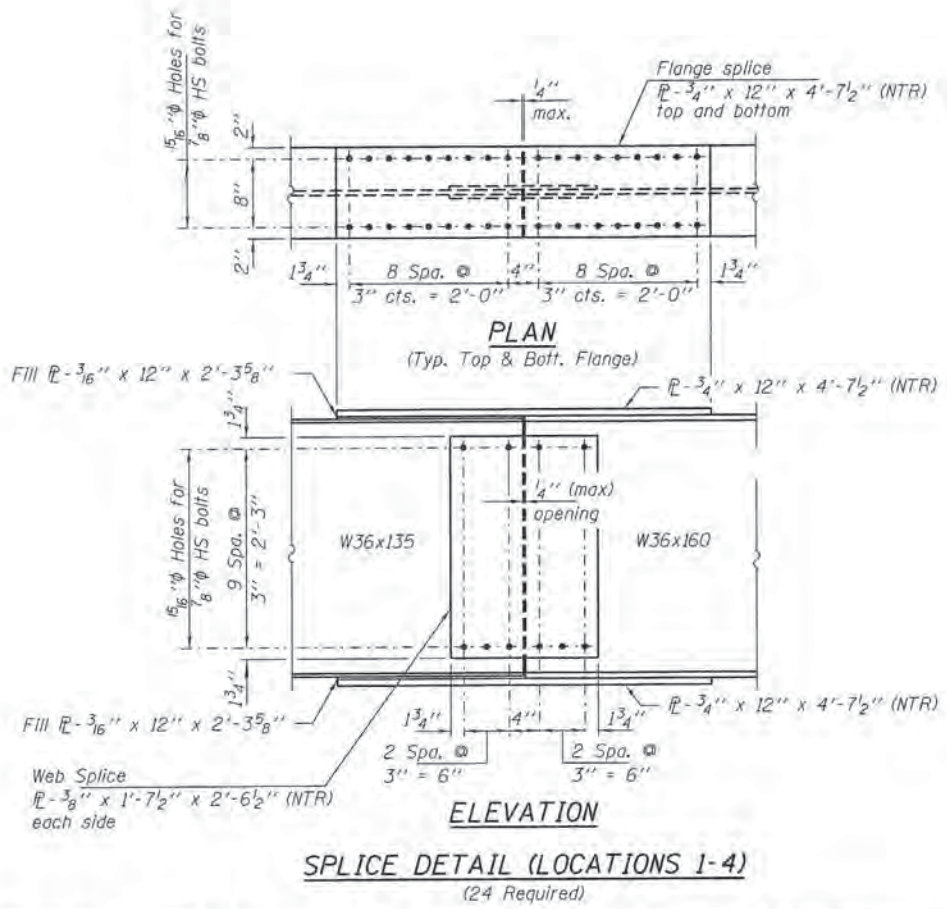
$f_s$  (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).

$1.25 (f_{sDC1} + f_{sDC2}) + 1.5 f_{sDW} + 1.75 f_s (l + IM)$

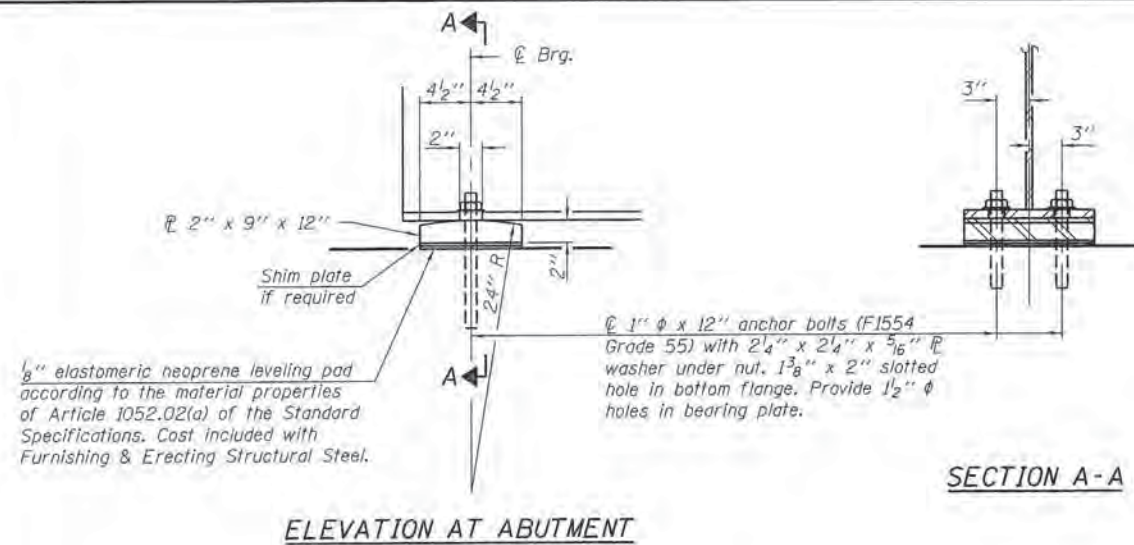
$\phi_r F_n$ : Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

V<sub>r</sub>: Maximum factored shear range in span computed according to Article 6.10.10.

Note:  
M<sub>l</sub> and R<sub>l</sub> include the effects of centrifugal force and superelevation.



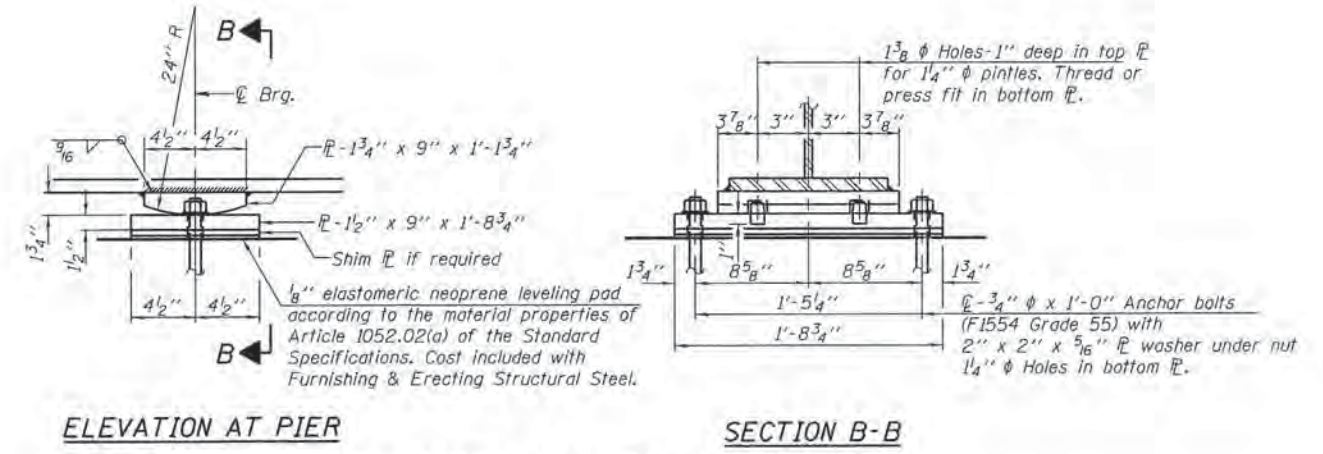
Notes:  
Load carrying components designated "NTR" shall conform to the Impact Testing Requirements, Zone 2.  
All beams and splices shall be M270 Grade 50W.



1/8" elastomeric neoprene leveling pad according to the material properties of Article 1052.02(a) of the Standard Specifications. Cost included with Furnishing & Erecting Structural Steel.

1"  $\phi$  x 12" anchor bolts (F1554 Grade 55) with 2 1/4" x 2 1/4" x 5/16"  $\mathbb{E}$  washer under nut. 1 3/8" x 2" slotted hole in bottom flange. Provide 1/2"  $\phi$  holes in bearing plate.

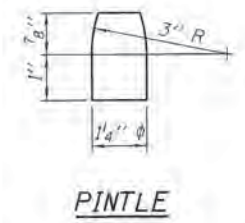
**ELEVATION AT ABUTMENT**  
**FIXED BEARING AT ABUTMENTS**  
 (12 required)



1/8" elastomeric neoprene leveling pad according to the material properties of Article 1052.02(a) of the Standard Specifications. Cost included with Furnishing & Erecting Structural Steel.

3/4"  $\phi$  Holes-1" deep in top  $\mathbb{E}$  for 1/4"  $\phi$  pintles. Thread or press fit in bottom  $\mathbb{E}$ .

**ELEVATION AT PIER**  
**FIXED BEARING AT PIERS 1 & 2**  
 (12 required)



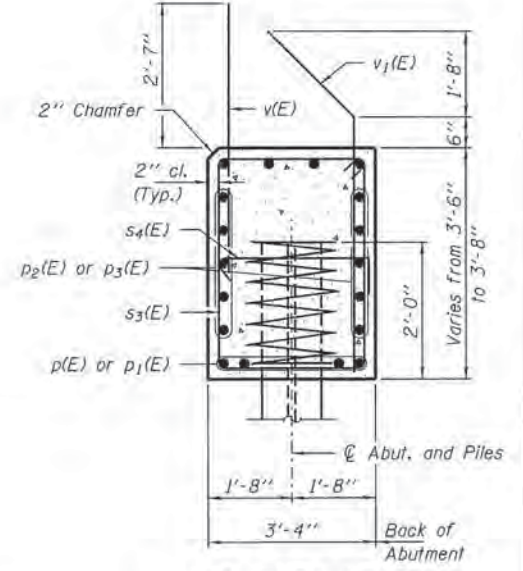
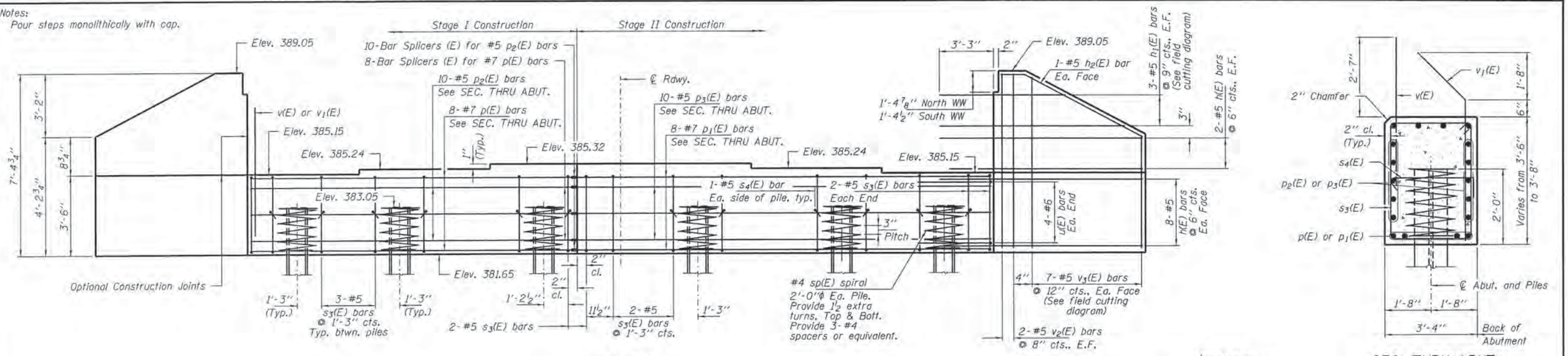
**PINTLE**

**Notes:**  
 Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.  
 Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.  
 Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.  
 Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.  
 The structural steel plates of the fixed bearings, including pintles, shall conform to the requirements of AASHTO M270 Grade 50W.  
 The anchor bolt sizes and grades shown constitute a calculated seismic structural fuse. Substitution of higher diameter and/or grade anchor bolts will not be allowed.

**BILL OF MATERIAL**

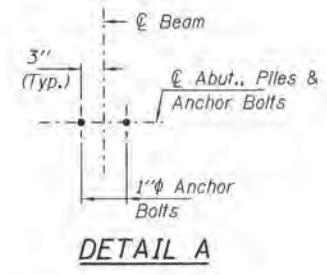
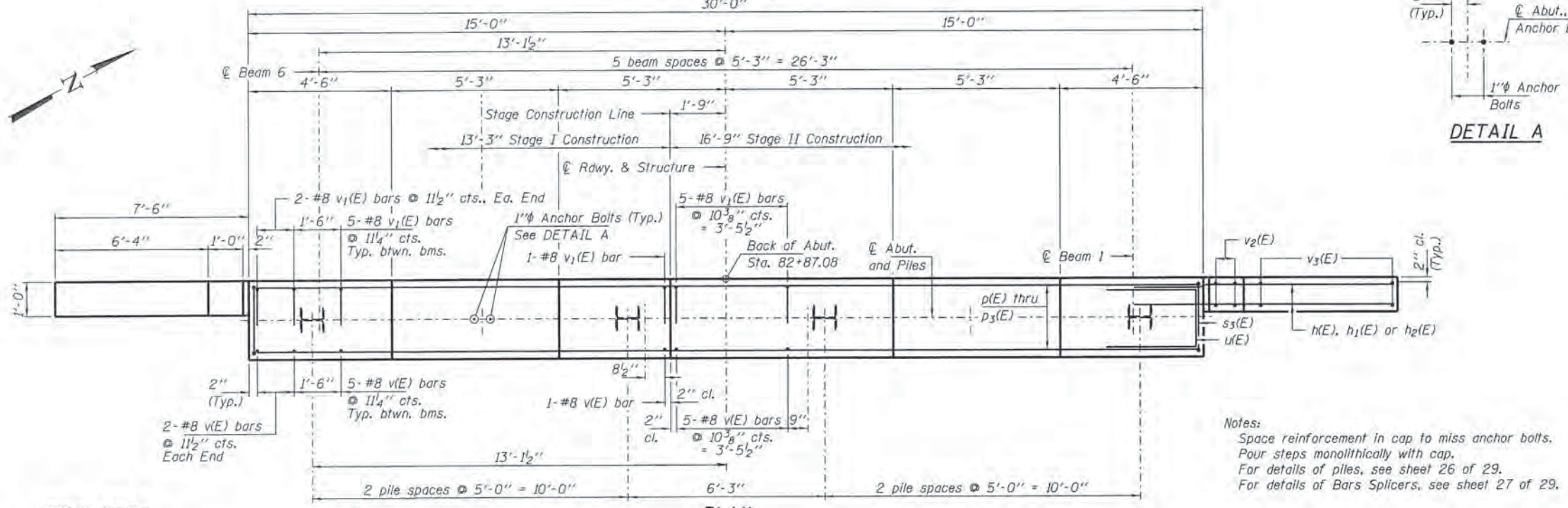
Item	Unit	Total
Anchor Bolts, 3/4"	Each	24
Anchor Bolts, 1"	Each	24

Notes:  
Pour steps monolithically with cap.



ELEVATION

SEC. THRU ABUT.



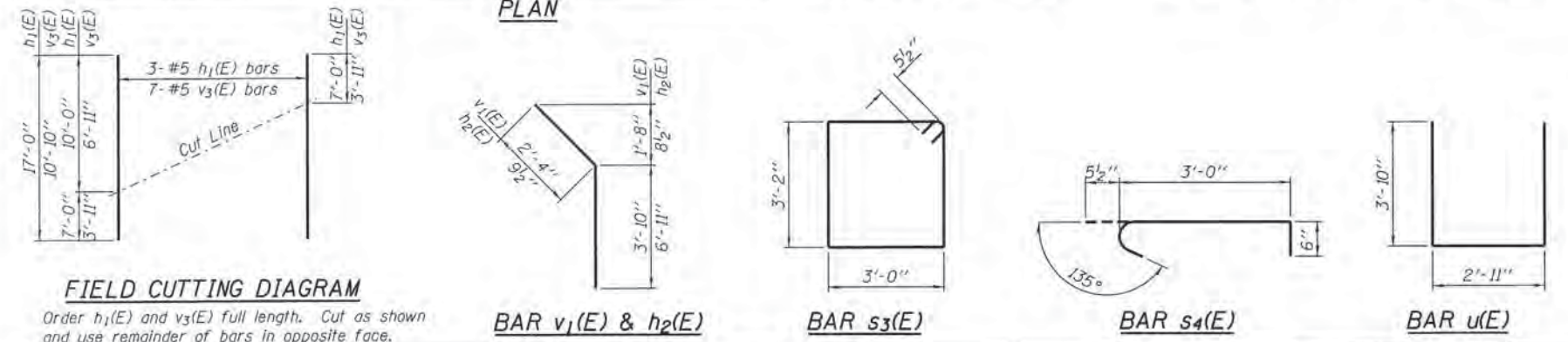
DETAIL A

PLAN

**PILE DATA**

Type: Steel HP14x117 with Pile Shoes  
Nominal Required Bearing: 929 Kips/pile  
Factored Resistance Available: 510 Kips/pile  
Est. Length: 49'  
No. Production Piles: 5  
No. Test Piles: 1

Notes:  
Space reinforcement in cap to miss anchor bolts.  
Pour steps monolithically with cap.  
For details of piles, see sheet 26 of 29.  
For details of Bars Splicers, see sheet 27 of 29.



FIELD CUTTING DIAGRAM

BAR v<sub>1</sub>(E) & h<sub>2</sub>(E)

BAR s<sub>3</sub>(E)

BAR s<sub>4</sub>(E)

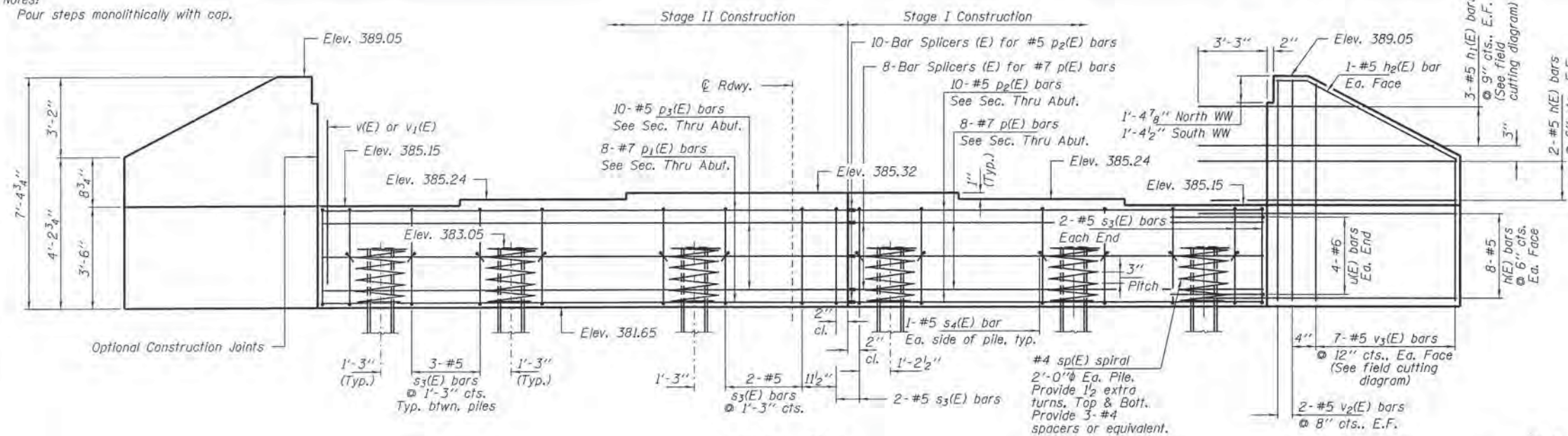
BAR u(E)

**WEST ABUTMENT  
BILL OF MATERIAL**

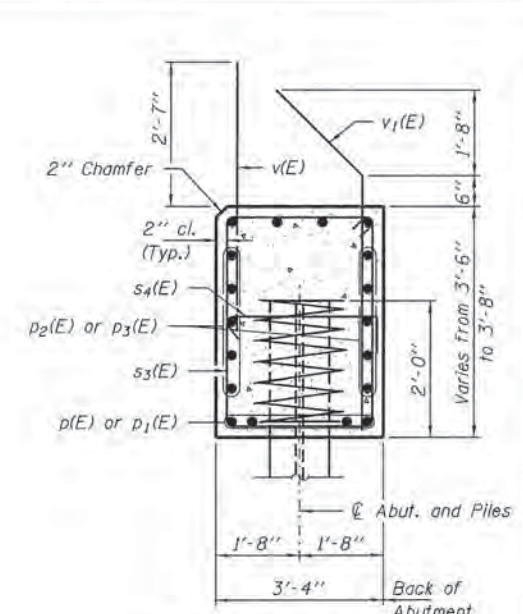
Bar	No.	Size	Length	Shape
h(E)	40	#5	10'-9"	—
h <sub>1</sub> (E)	6	#5	17'-0"	—
h <sub>2</sub> (E)	4	#5	7'-9"	—
p(E)	8	#7	12'-11"	—
p <sub>1</sub> (E)	8	#7	16'-5"	—
p <sub>2</sub> (E)	10	#5	12'-11"	—
p <sub>3</sub> (E)	10	#5	16'-5"	—
s <sub>3</sub> (E)	21	#5	13'-3"	□
s <sub>4</sub> (E)	12	#5	4'-0"	└
sp(E)	6	#4	2'-0"	WWW
u(E)	8	#6	10'-7"	—
v(E)	30	#8	5'-11"	—
v <sub>1</sub> (E)	30	#8	6'-2"	—
v <sub>2</sub> (E)	8	#5	7'-0"	—
v <sub>3</sub> (E)	14	#5	10'-10"	—
Structure Excavation		Cu. Yd.	94	
Concrete Structures		Cu. Yd.	16.5	
Reinforcement Bars, Epoxy Coated		Pound	3,300	
Bar Splicers		Each	18	
Furnishing Steel Piles HP14x117		Foot	245	
Driving Piles		Foot	245	
Test Pile Steel HP14x117		Each	1	
Pile Shoes		Each	5	

\*Length is height of spiral.

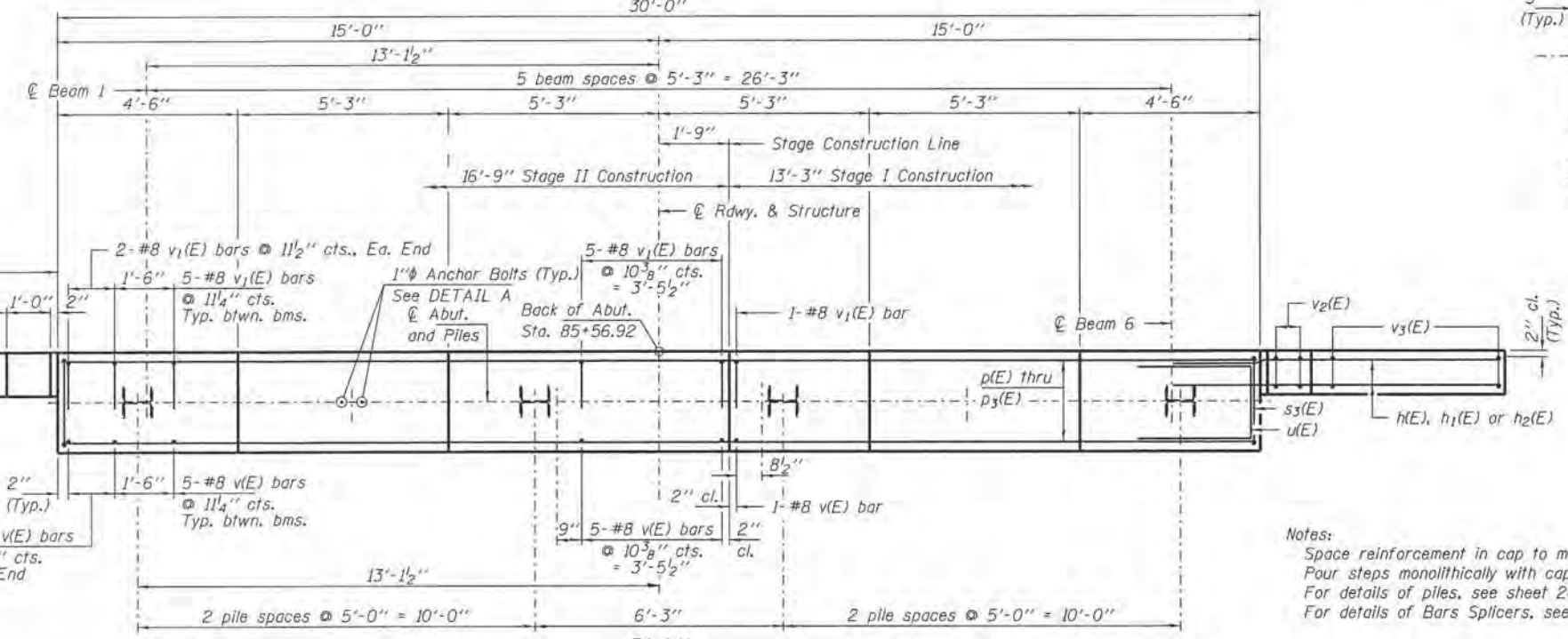
Notes:  
Pour steps monolithically with cap.



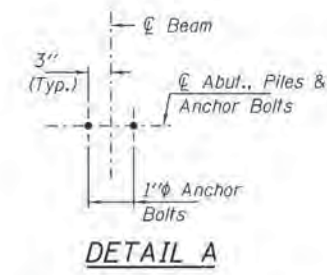
**ELEVATION**  
30'-0"



**SEC. THRU ABUT.**



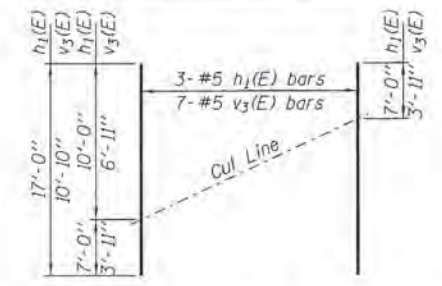
**PLAN**



**DETAIL A**

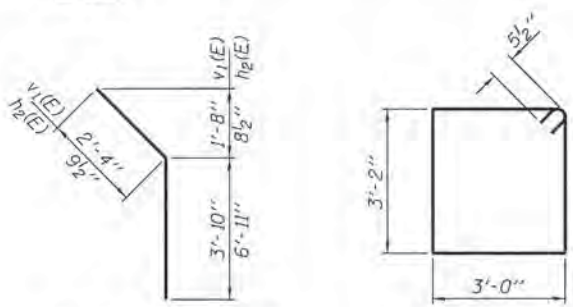
**PILE DATA**

Type: Steel HP14x117 with Pile Shoes  
Nominal Required Bearing: 929 Kips/pile  
Factored Resistance Available: 510 Kips/pile  
Est. Length: 62'  
No. Production Piles: 5  
No. Test Piles: 1

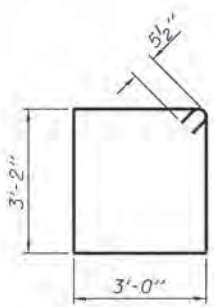


**FIELD CUTTING DIAGRAM**

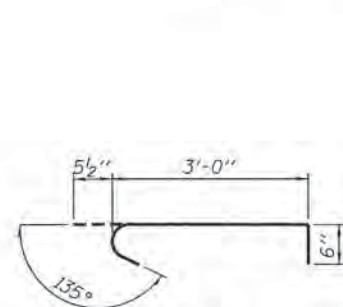
Order h1(E) and v3(E) full length. Cut as shown and use remainder of bars in opposite face.



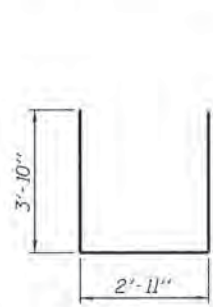
**BAR v1(E) & h2(E)**



**BAR s3(E)**



**BAR s4(E)**



**BAR u(E)**

Notes:  
Space reinforcement in cap to miss anchor bolts.  
Pour steps monolithically with cap.  
For details of piles, see sheet 26 of 29.  
For details of Bars Splicers, see sheet 27 of 29.

**EAST ABUTMENT  
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h(E)	40	#5	10'-9"	—
h1(E)	6	#5	17'-0"	—
h2(E)	4	#5	7'-9"	—
p(E)	8	#7	12'-11"	—
p1(E)	8	#7	16'-5"	—
p2(E)	10	#5	12'-11"	—
p3(E)	10	#5	16'-5"	—
s3(E)	21	#5	13'-3"	□
s4(E)	12	#5	4'-0"	┌
sp(E)	6	#4	2'-0"	WWW
u(E)	8	#6	10'-7"	—
v(E)	30	#8	5'-11"	—
v1(E)	30	#8	6'-2"	—
v2(E)	8	#5	7'-0"	—
v3(E)	14	#5	10'-10"	—
Structure Excavation		Cu. Yd.	94	
Concrete Structures		Cu. Yd.	16.5	
Reinforcement Bars, Epoxy Coated		Pound	3,300	
Bar Splicers		Each	18	
Furnishing Steel Piles HP14x117		Foot	310	
Driving Piles		Foot	310	
Test Pile Steel HP14x117		Each	1	
Pile Shoes		Each	5	

\*Length is height of spiral.

AI-2440S-0

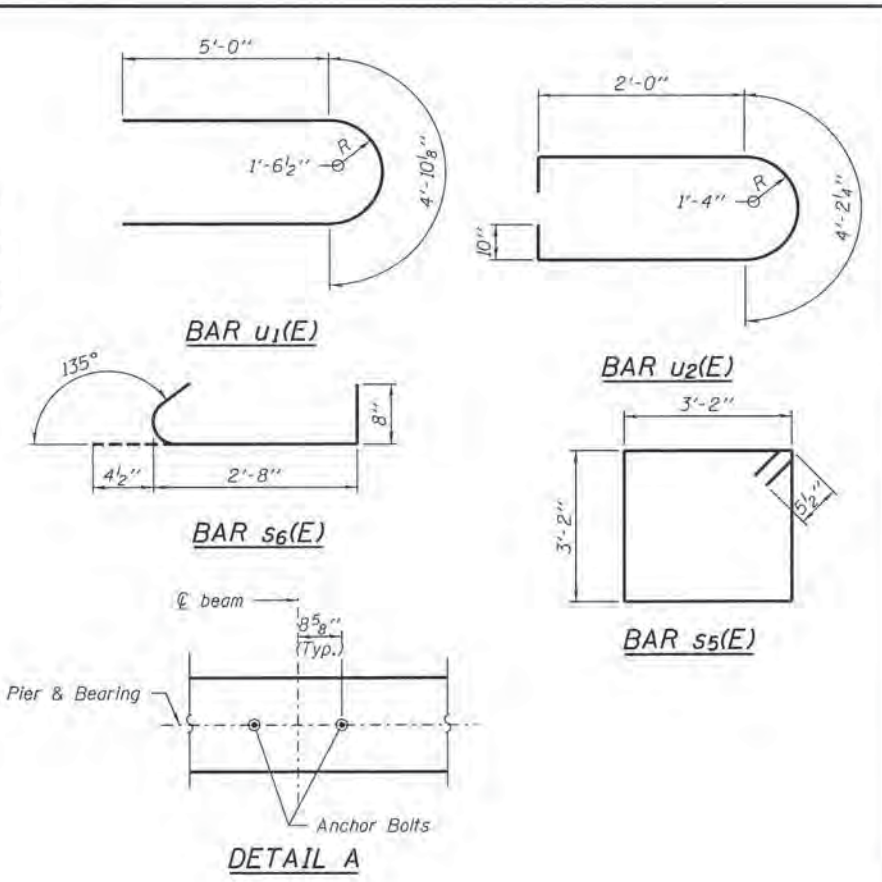
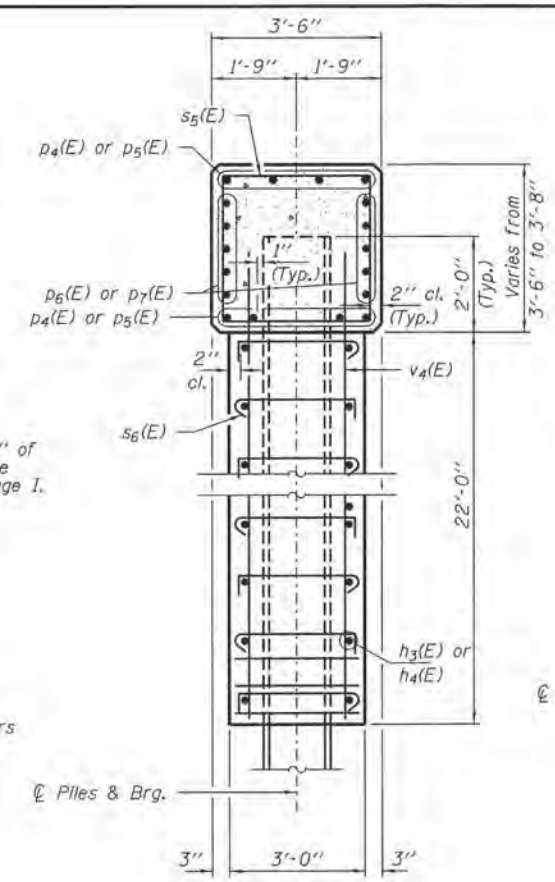
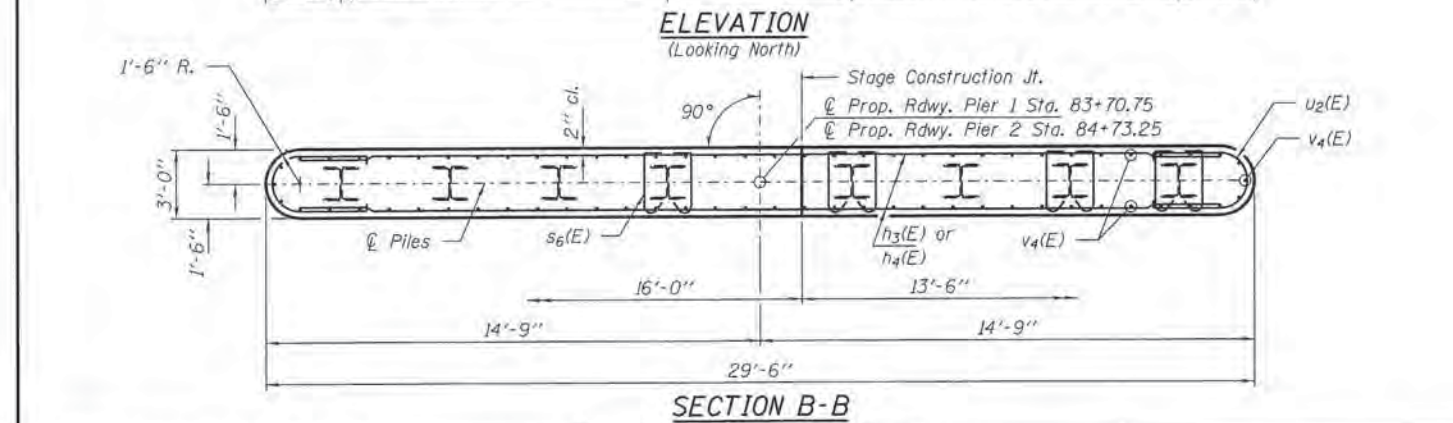
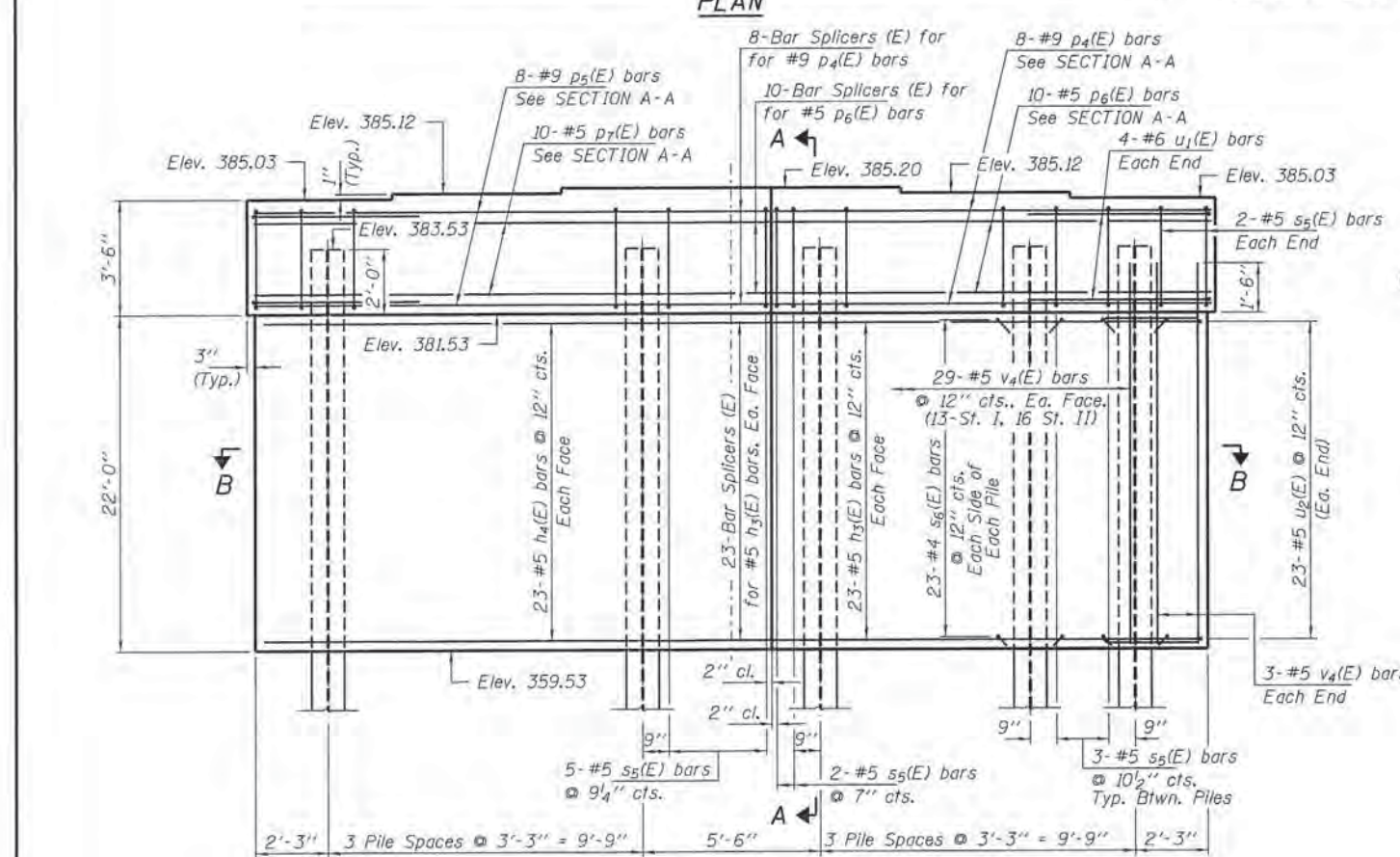
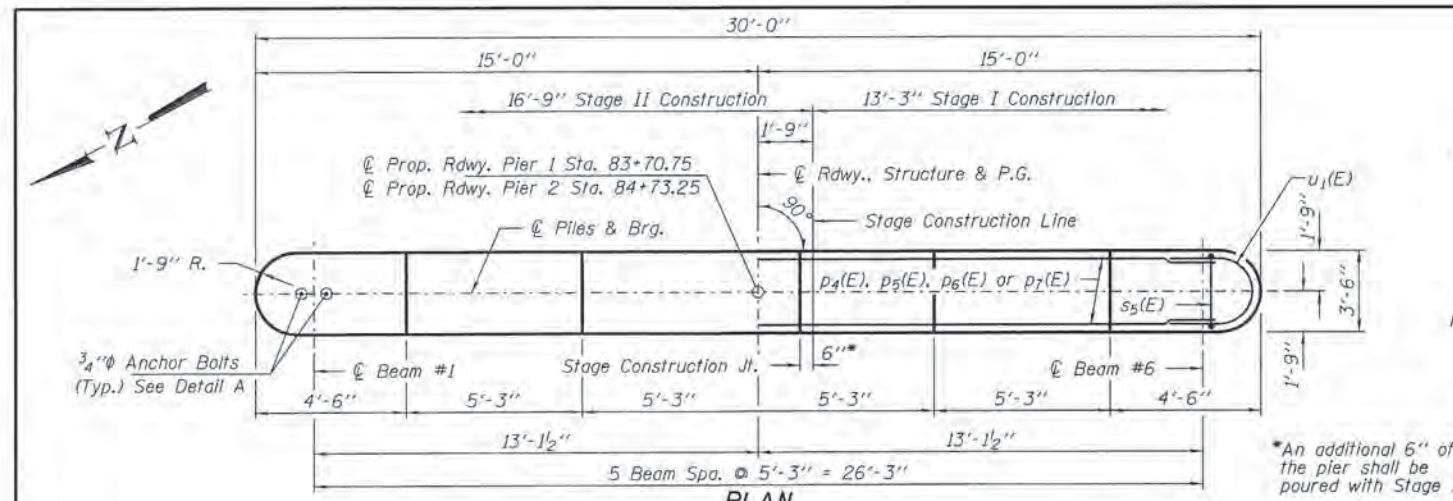
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3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.546.3450 www.hineng.com		D.W.T.	
184.00099 ILLINOIS PROFESSIONAL DESIGNERS L5 / P1 / SE CORPORATION		D.A.B.	
		M.D.C.	

STATE OF ILLINOIS  
JACKSON COUNTY HIGHWAY DEPARTMENT

EAST ABUTMENT  
STRUCTURE NO. 039-3277  
SHEET NO. 24 OF 29 SHEETS

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
B69	10-00163-00-BR	JACKSON	82	39
				CONTRACT NO. 99519
ILLINOIS FED. AID PROJECT BR5-0869106I				



**SECTION A-A**

Notes:  
 Space reinforcement in cap to miss anchor bolts.  
 Pour steps monolithically with cap.  
 For details of piles, see sheet 26 of 29.  
 For details of Bars Splicers, see sheet 27 of 29.

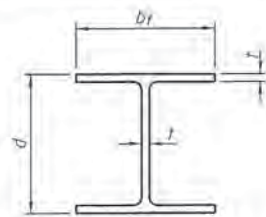
**PILE DATA**

Type: Steel HP14x117 with Pile Shoes  
 Nominal Required Bearing: 929 Kips/pile  
 Factored Resistance Available: 401 Kips/pile  
 Est. Length: 49' (Pier 1)  
 62' (Pier 2)  
 No. Production Piles: 14  
 No. Test Piles: 2 (1-Pier 1, 1-Pier 2)

**2 PIERS  
 BILL OF MATERIAL**

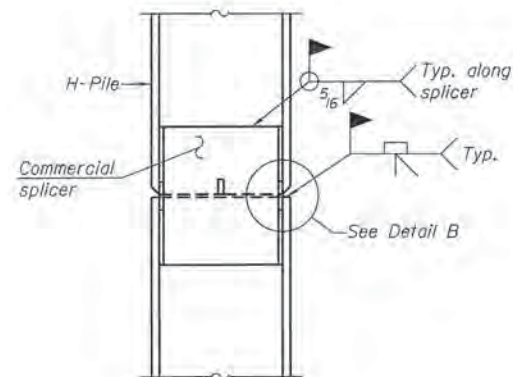
Bar	No.	Size	Length	Shape
h3(E)	92	#5	12'-0"	—
h4(E)	92	#5	14'-6"	—
p4(E)	16	#9	12'-0"	—
p5(E)	16	#9	14'-6"	—
p6(E)	20	#5	12'-0"	—
p7(E)	20	#5	14'-6"	—
s5(E)	58	#5	13'-7"	□
s6(E)	736	#4	3'-9"	⌋
u1(E)	16	#6	14'-11"	⌋
u2(E)	92	#5	9'-11"	⌋
v4(E)	128	#5	23'-6"	—
Cofferdam Excavation			Cu. Yd.	328
Concrete Structures			Cu. Yd.	164.8
Reinforcement Bars, Epoxy Coated			Pound	11,650
Bar Splicers			Each	128
Furnishing Steel Piles HP14x117			Foot	777
Driving Piles			Foot	777
Test Pile Steel HP14x117			Each	2
Pile Shoes			Each	14



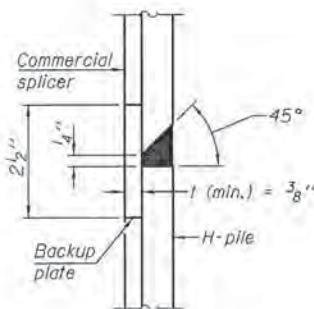


STEEL PILE TABLE

Designation	Depth d	Flange width b <sub>f</sub>	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	1 3/16"	30"
x102	14"	14 3/4"	1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"

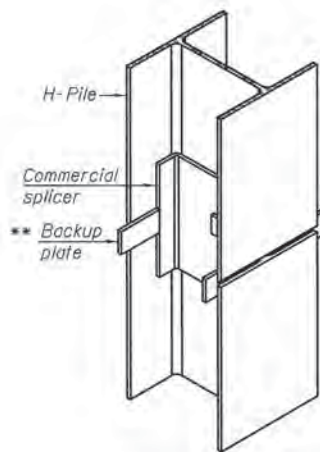


ELEVATION

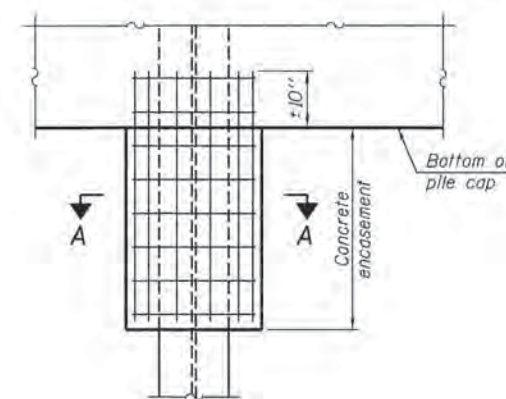


DETAIL "B"

WELDED COMMERCIAL SPLICE

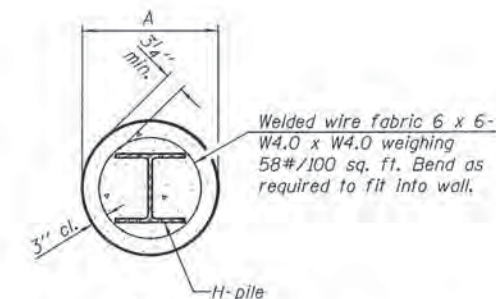


ISOMETRIC VIEW



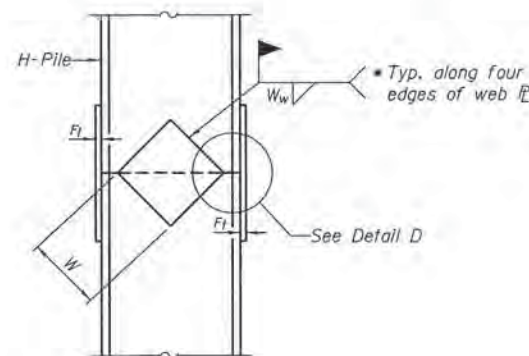
ELEVATION

PILE ENCASEMENT

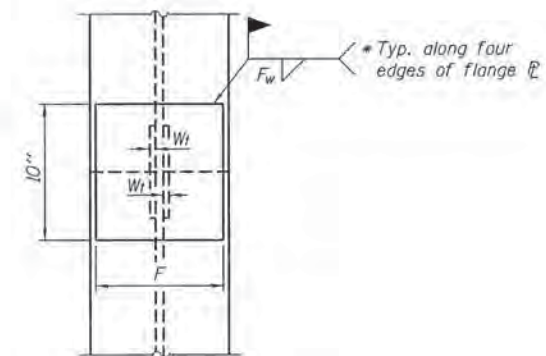


SECTION A-A

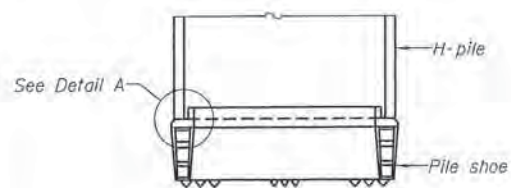
Note:  
Forms for encasement may be omitted when soil conditions permit.



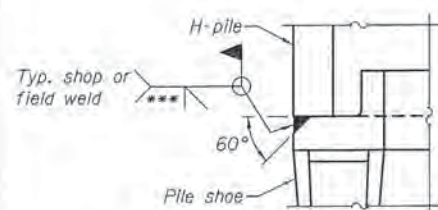
ELEVATION



END VIEW

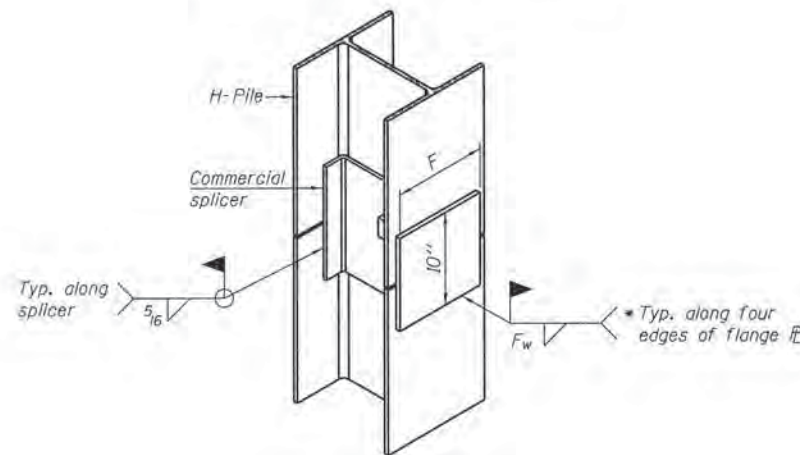


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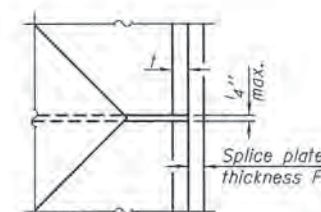


DETAIL A

H-PILE SHOE ATTACHMENT



ISOMETRIC VIEW



DETAIL D

WELDED PLATE FIELD SPLICE

Designation	F	F <sub>t</sub>	F <sub>w</sub>	W	W <sub>t</sub>	W <sub>w</sub>
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5/8"	1/2"
x89	12 1/2"	3/4"	1/16"	7 3/4"	5/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5/8"	1/2"
HP 12x84	10"	7/8"	1/16"	6 1/2"	5/8"	1/2"
x74	10"	7/8"	1/16"	6 1/2"	5/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

WELDED COMMERCIAL SPLICE ALTERNATE

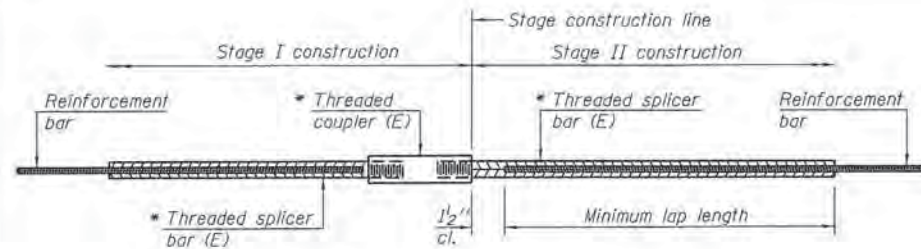
- \* Interrupt welds 1/4" from end of web and/or each flange.
- \*\* Remove portions of backup plates that extend outside the flanges.
- \*\*\* Weld size per pile shoe manufacturer (5/16" min.).

Note:  
The steel H-piles shall be according to AASHTO M270 Grade 50.

F-HP

1-27-12

FILE NAME * 130348-shr-bridge.dgn	USER NAME *	DESIGNED - S.M.S.	REVISED -	<b>STATE OF ILLINOIS JACKSON COUNTY HIGHWAY DEPARTMENT</b>	<b>HP PILE DETAILS STRUCTURE NO. 039-3277</b>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
3083 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 217.546.3400 www.nhrng.com		CHECKED - D.W.T.	REVISED -			869	10-00163-00-BR	JACKSON	82	41	
184.9595 ILLINOIS PROFESSIONAL DESIGN FIRM L3/PE/SE CORPORATION	PLOT SCALE =	DRAWN - D.A.B.	REVISED -			CONTRACT NO. 99519					
	PLOT DATE = 4/6/2015	CHECKED - M.D.C.	REVISED -			ILLINOIS FED. AID PROJECT BRS-08691061					



**STANDARD BAR SPLICER ASSEMBLY**

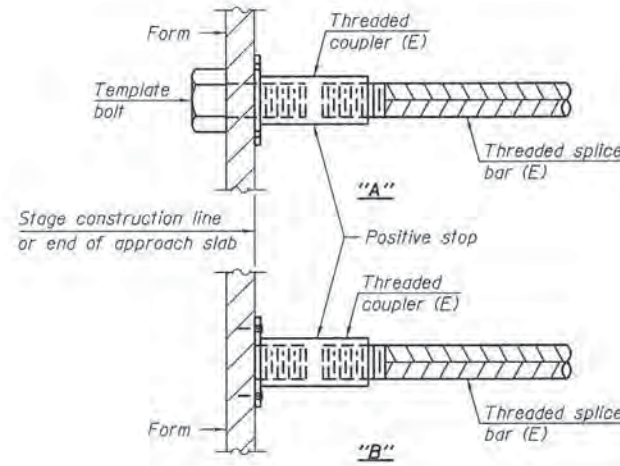
Minimum Lap Lengths						
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6
3, 4	1'-5"	1'-11"	2'-1"	2'-4"	2'-7"	2'-11"
5	1'-9"	2'-5"	2'-7"	2'-11"	3'-3"	3'-8"
6	2'-1"	2'-11"	3'-1"	3'-6"	3'-10"	4'-5"
7	2'-9"	3'-10"	4'-2"	4'-8"	5'-2"	5'-10"
8	3'-8"	5'-1"	5'-5"	6'-2"	6'-9"	7'-8"
9	4'-7"	6'-5"	6'-10"	7'-9"	8'-7"	9'-8"

- Table 1: Black bar, 0.8 Class C
- Table 2: Black bar, Top bar lap, 0.8 Class C
- Table 3: Epoxy bar, 0.8 Class C
- Table 4: Epoxy bar, Top bar lap, 0.8 Class C
- Table 5: Epoxy bar, Class C
- Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + 1 1/2" + thread length

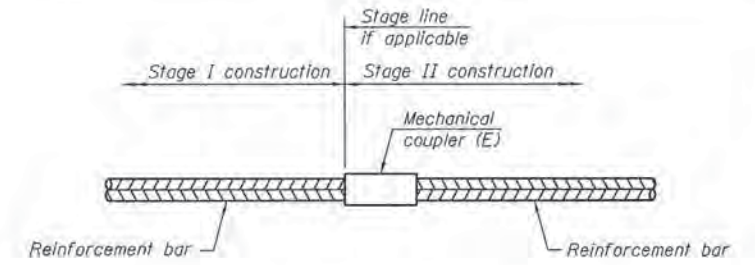
\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Table for minimum lap length
Top of slab	#5	460	3
Bottom of slab	#5	322	3
Diaphragm	#6	8	6
Approach Slab	#4	62	3
Appr. Slab Footing	#5	80	3
Abutments	#7	16	6
Abutments	#5	20	6
Piers	#9	16	6
Piers	#5	112	6



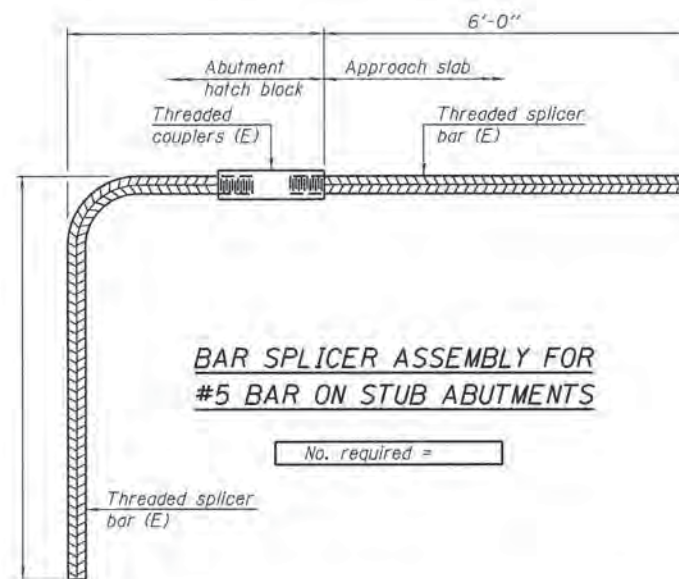
**INSTALLATION AND SETTING METHODS**

"A" : Set bar splicer assembly by means of a template bolt.  
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.  
 (E) : Indicates epoxy coating.



**STANDARD MECHANICAL SPLICER**

Location	Bar size	No. assemblies required



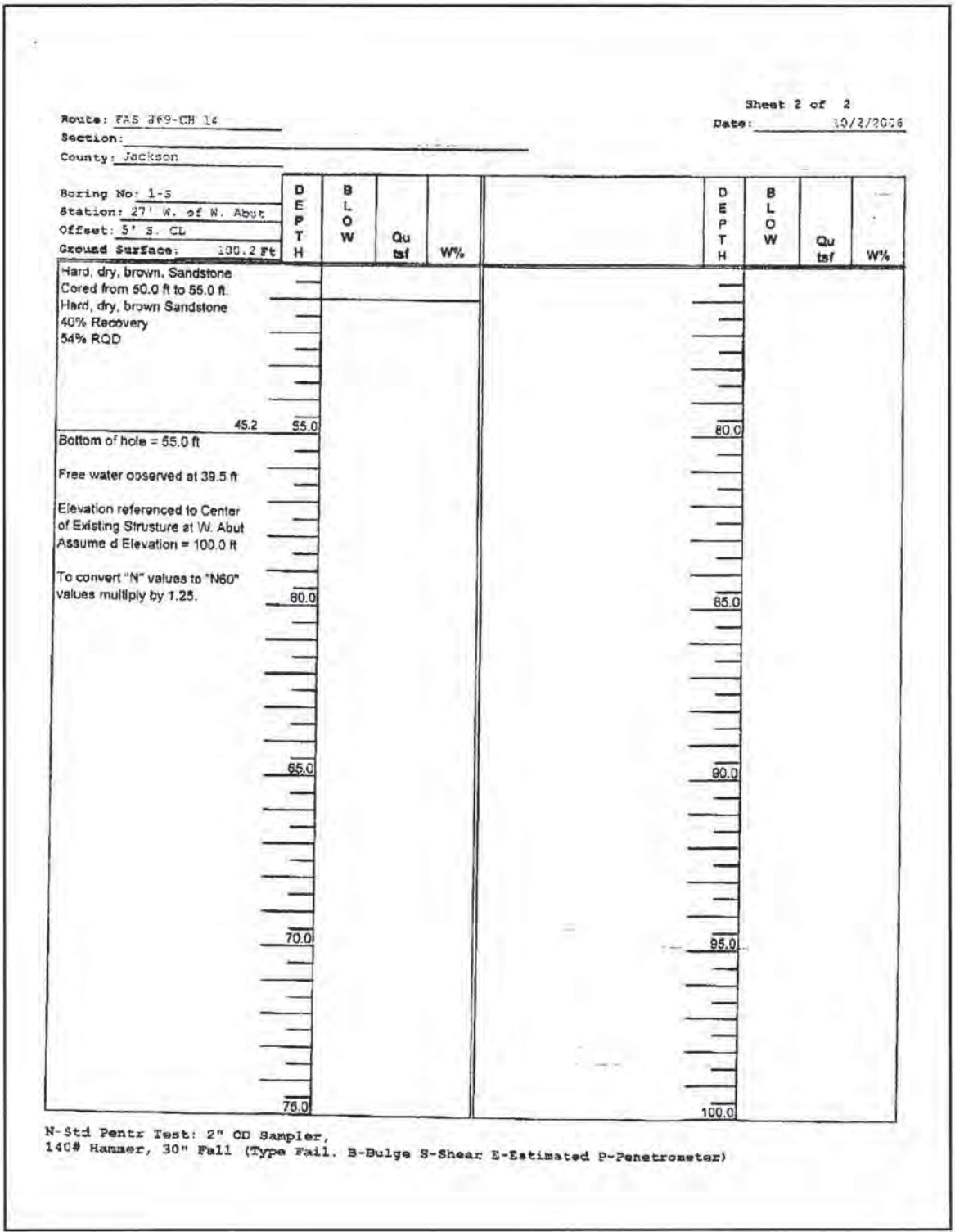
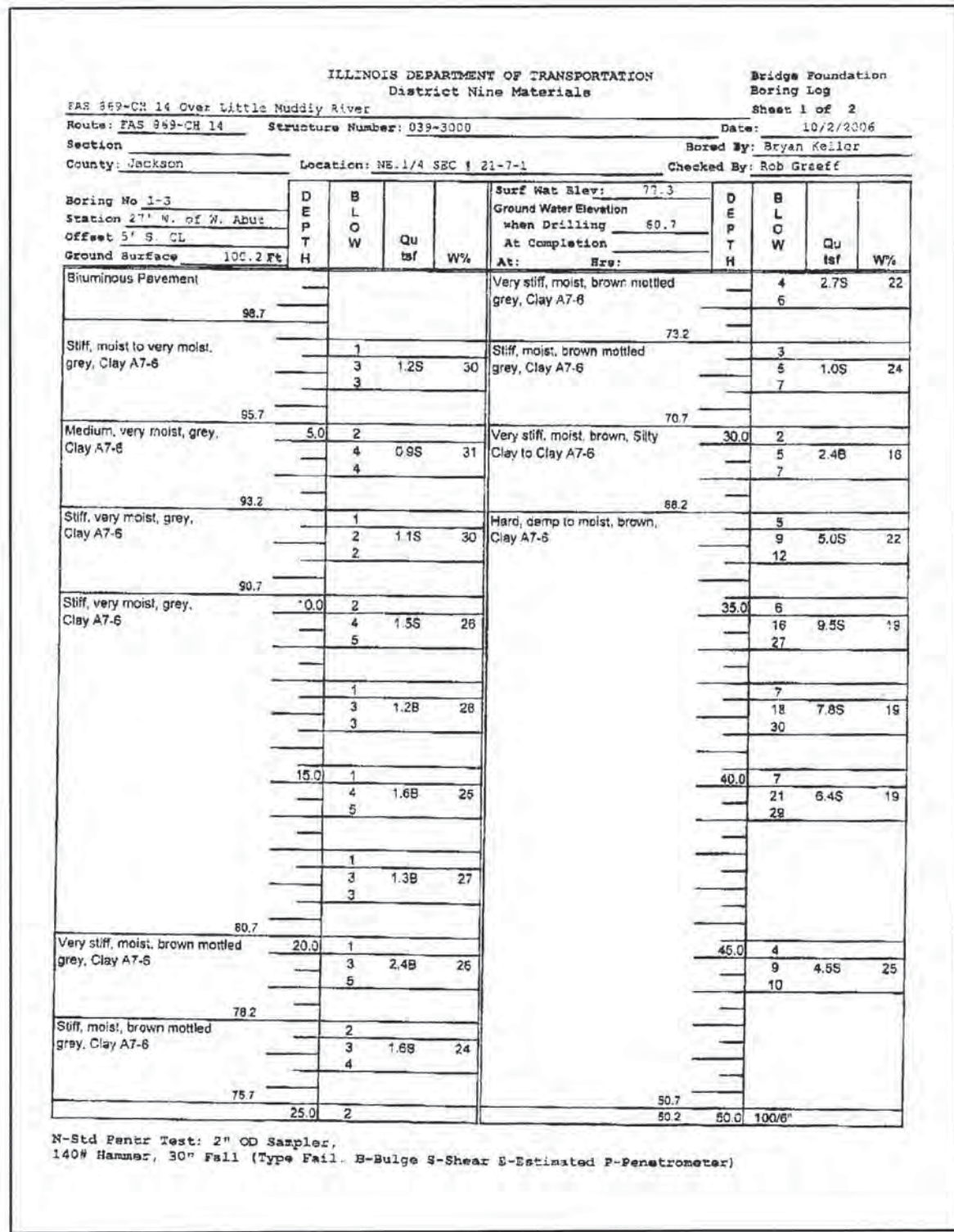
**BAR SPLICER ASSEMBLY FOR #5 BAR ON STUB ABUTMENTS**

**NOTES**

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.  
 All reinforcement shall be lapped and tied to the splicer bars.  
 Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.  
 See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

8-31-12

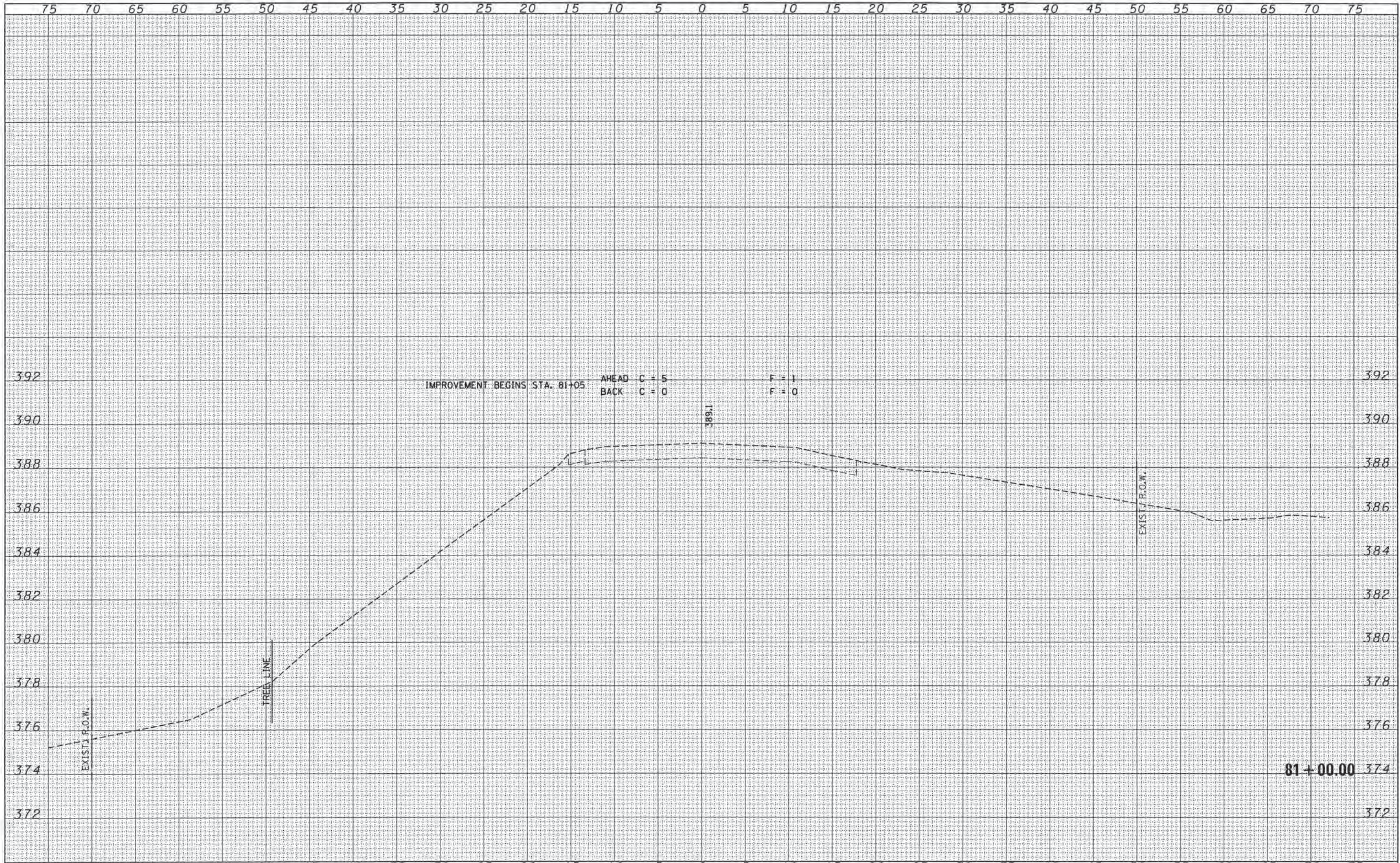


BORING 1-S



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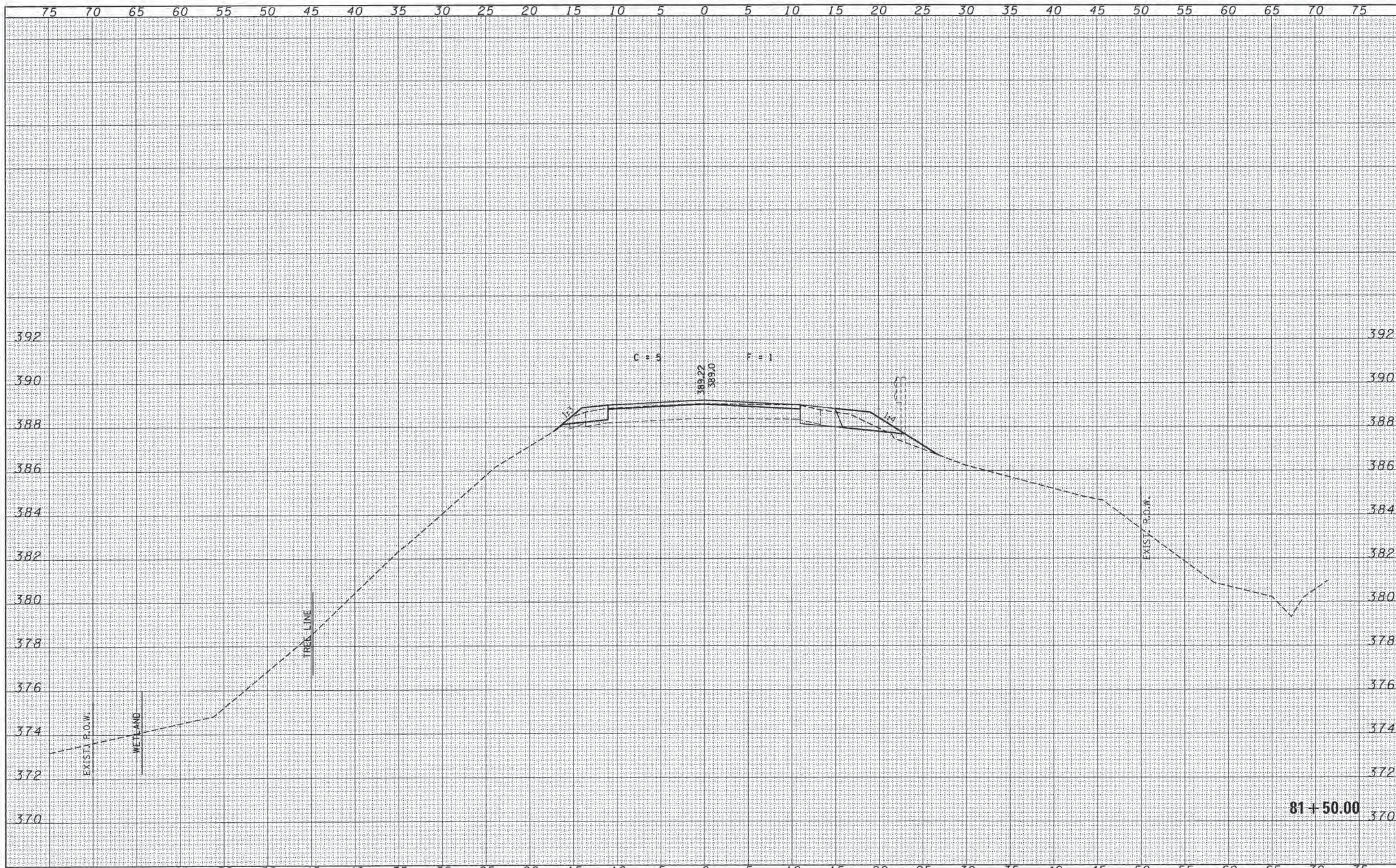
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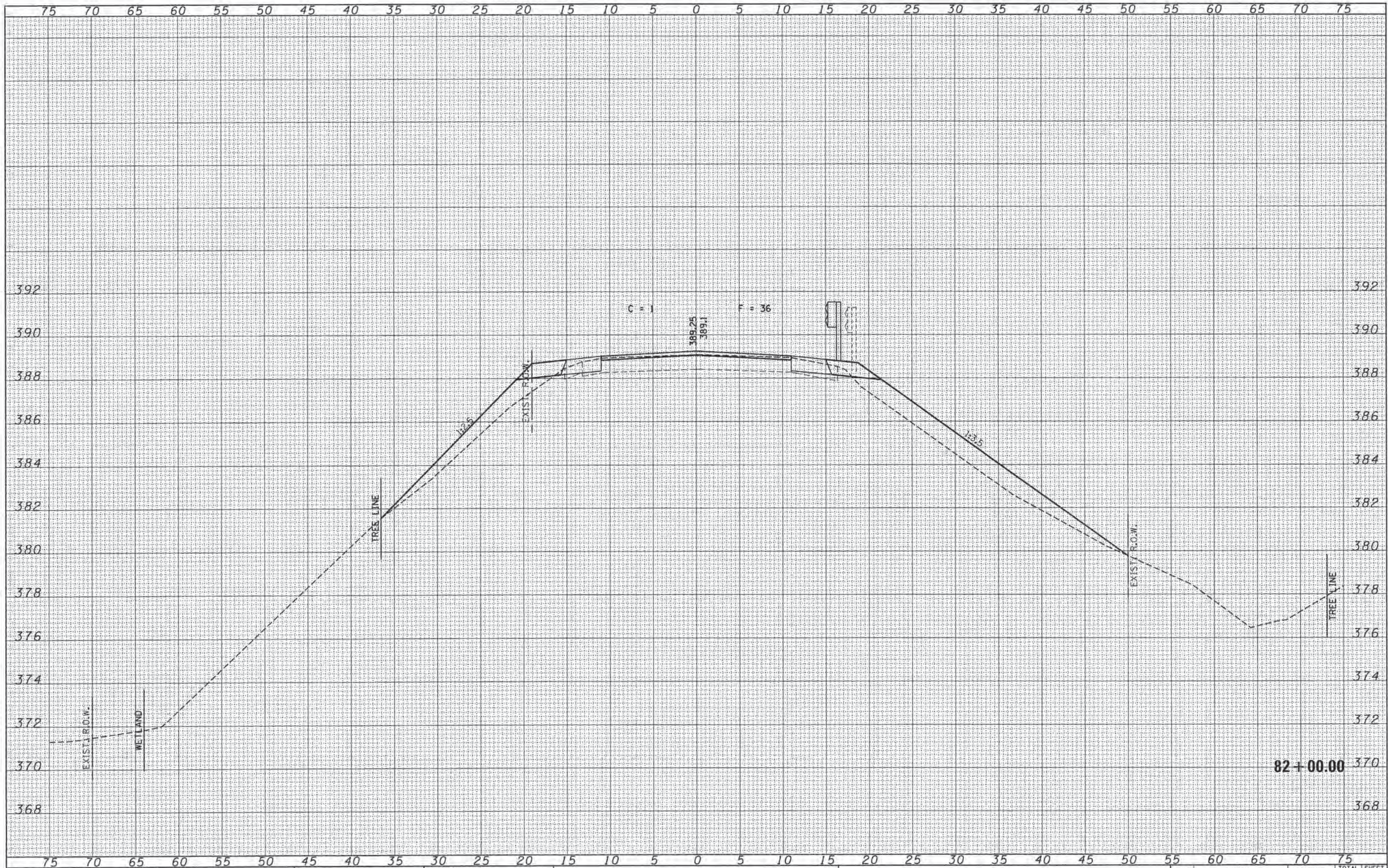
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394 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62709		CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519							
ILLINOIS PROFESSIONAL DESIGN FIRM L1 / P1 / S1 CORP. 184-078851		DATE - 02/23/15	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 81+00.00 TO STA. 81+00.00	ILLINOIS FED. AID PROJECT				

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3801 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62768	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V		SHEET NO. OF SHEETS		STA. 81+50.00 TO STA. 81+50.00	ILLINOIS FED. AID PROJECT	
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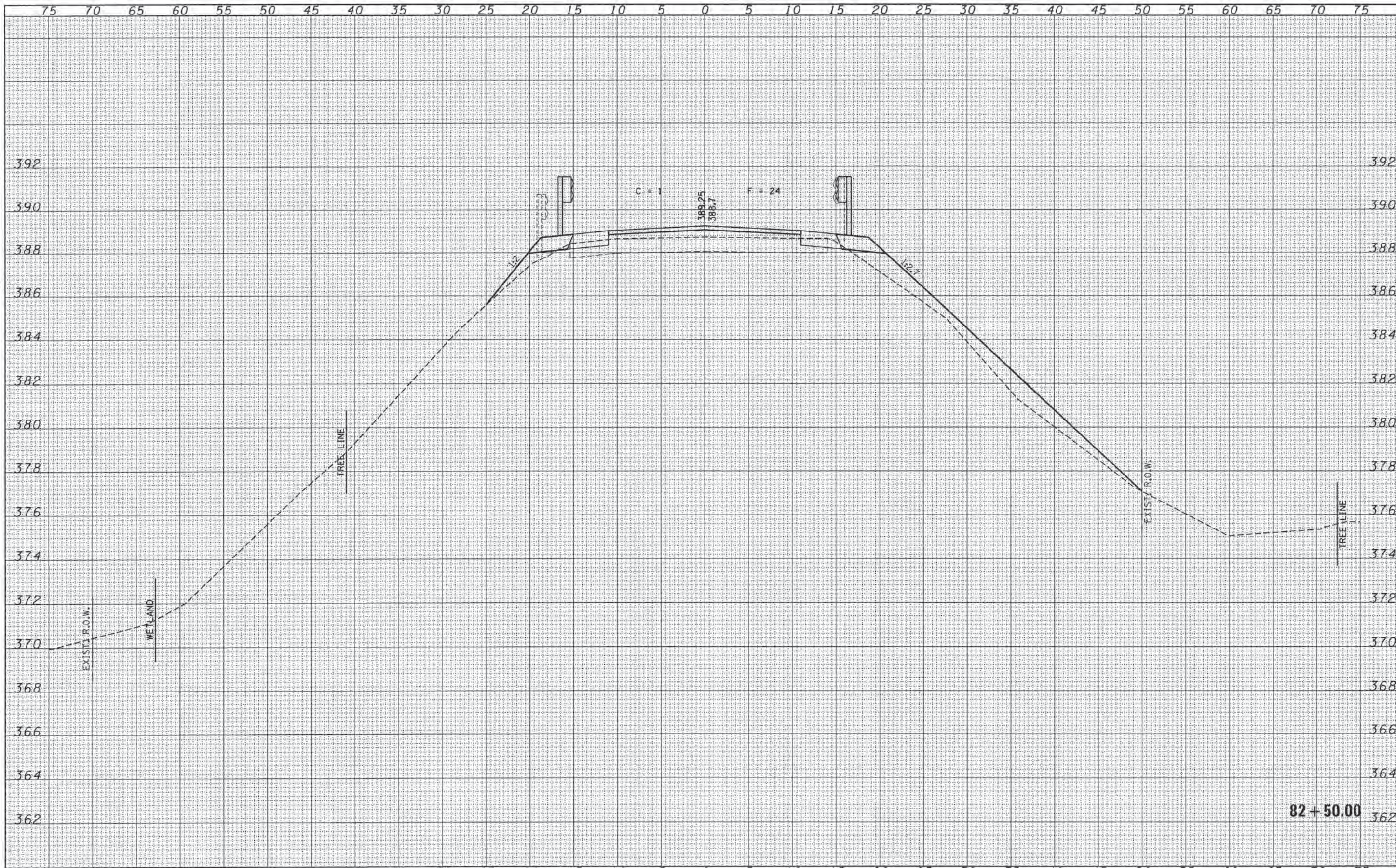
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HAMPTON, LENZINI AND RENWICK, INC. 308 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	47		
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		DATE - 02/23/15	REVISED -								

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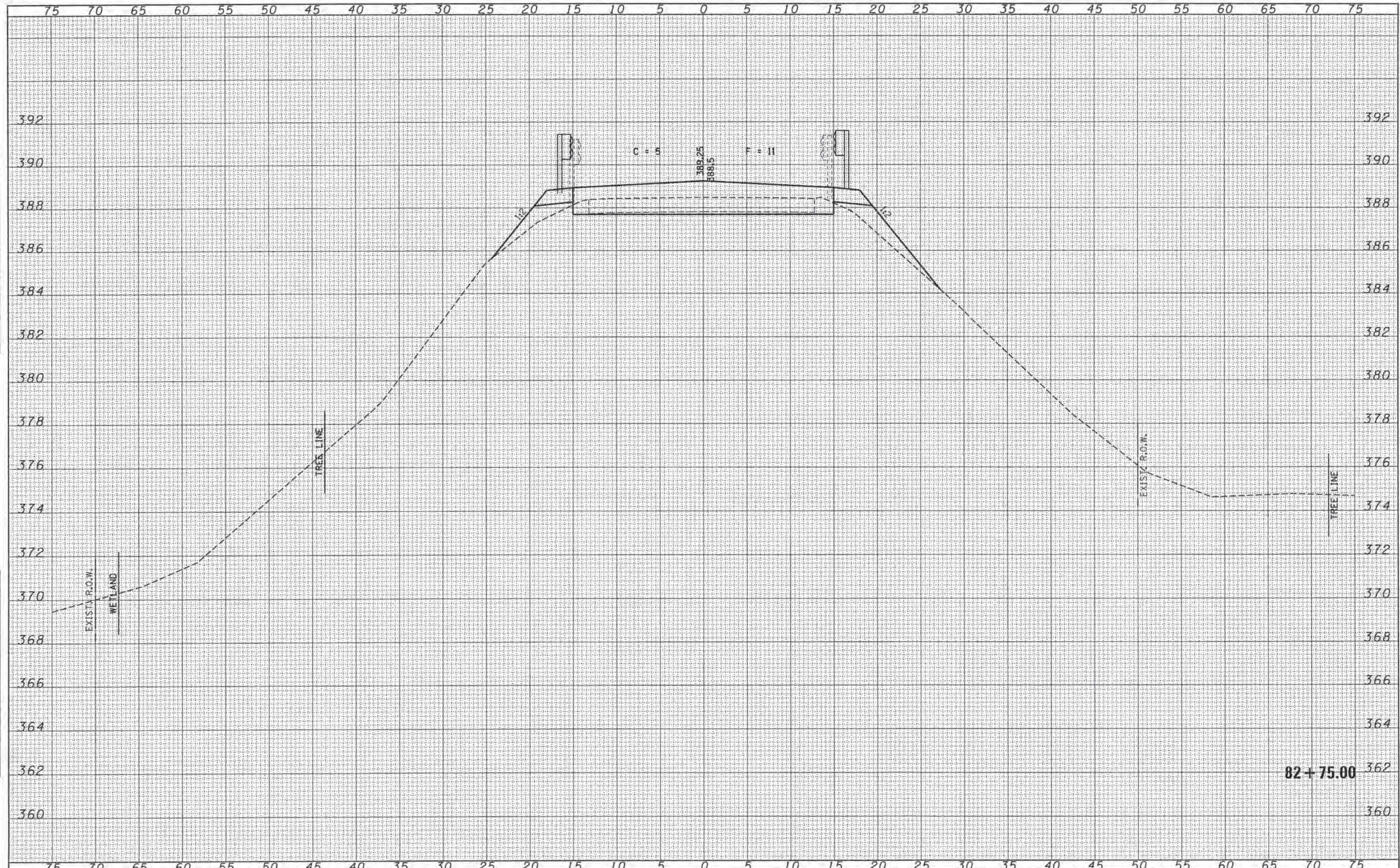
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394 STEVENSON DRIVE, SUITE 200 SPRINGFIELD, ILLINOIS 62709		CHECKED - J.W.F.	REVISED -									
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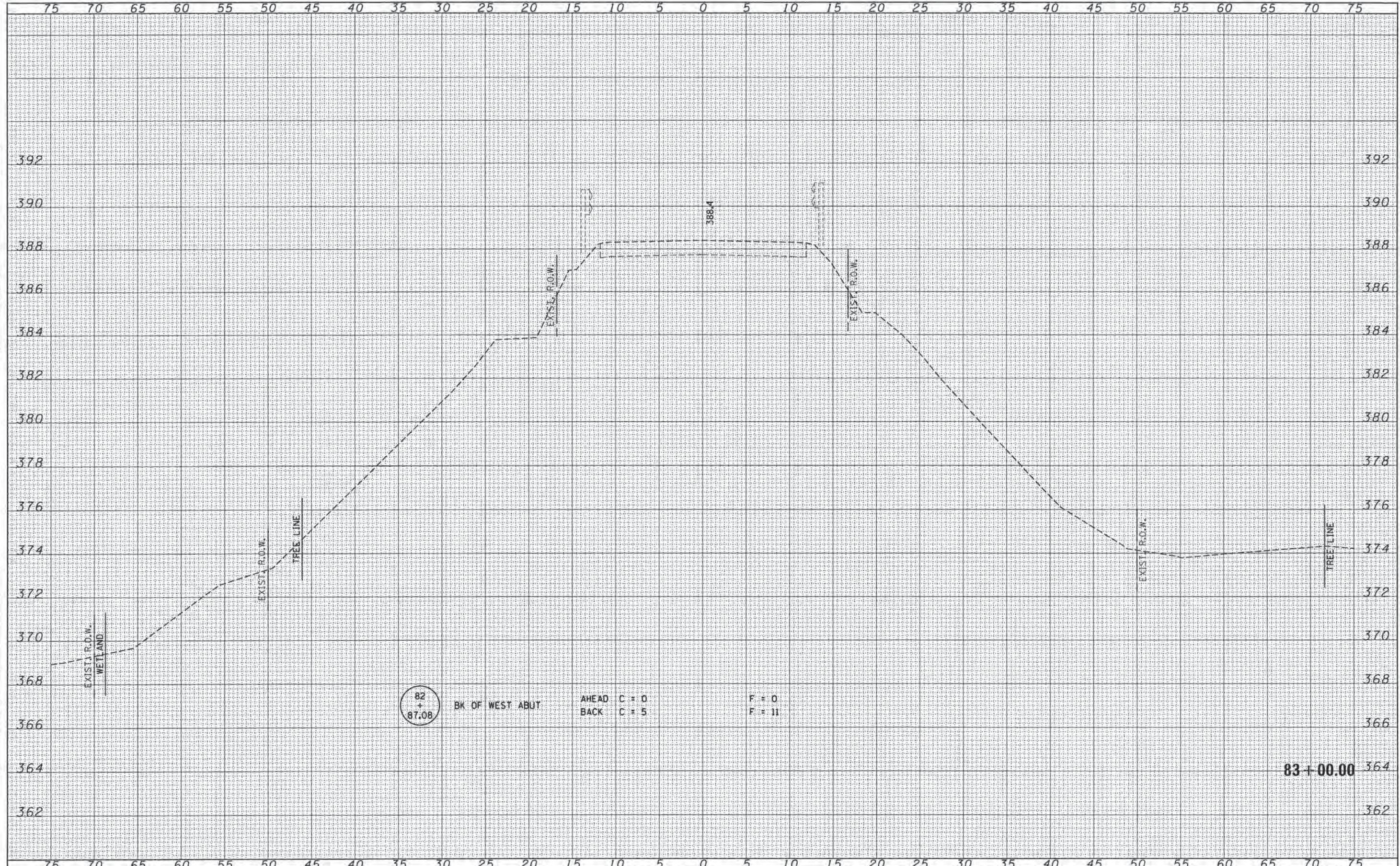


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ILLINOIS PROFESSIONAL ENGINEERING 18 1/2 E. COOK ST. SPRINGFIELD, ILL. 62702		DATE - 02/23/15	REVISED -		SHEET NO. OF SHEETS		STA. 82+75.00 TO STA. 82+75.00		(ILLINOIS) FED. AID PROJECT			



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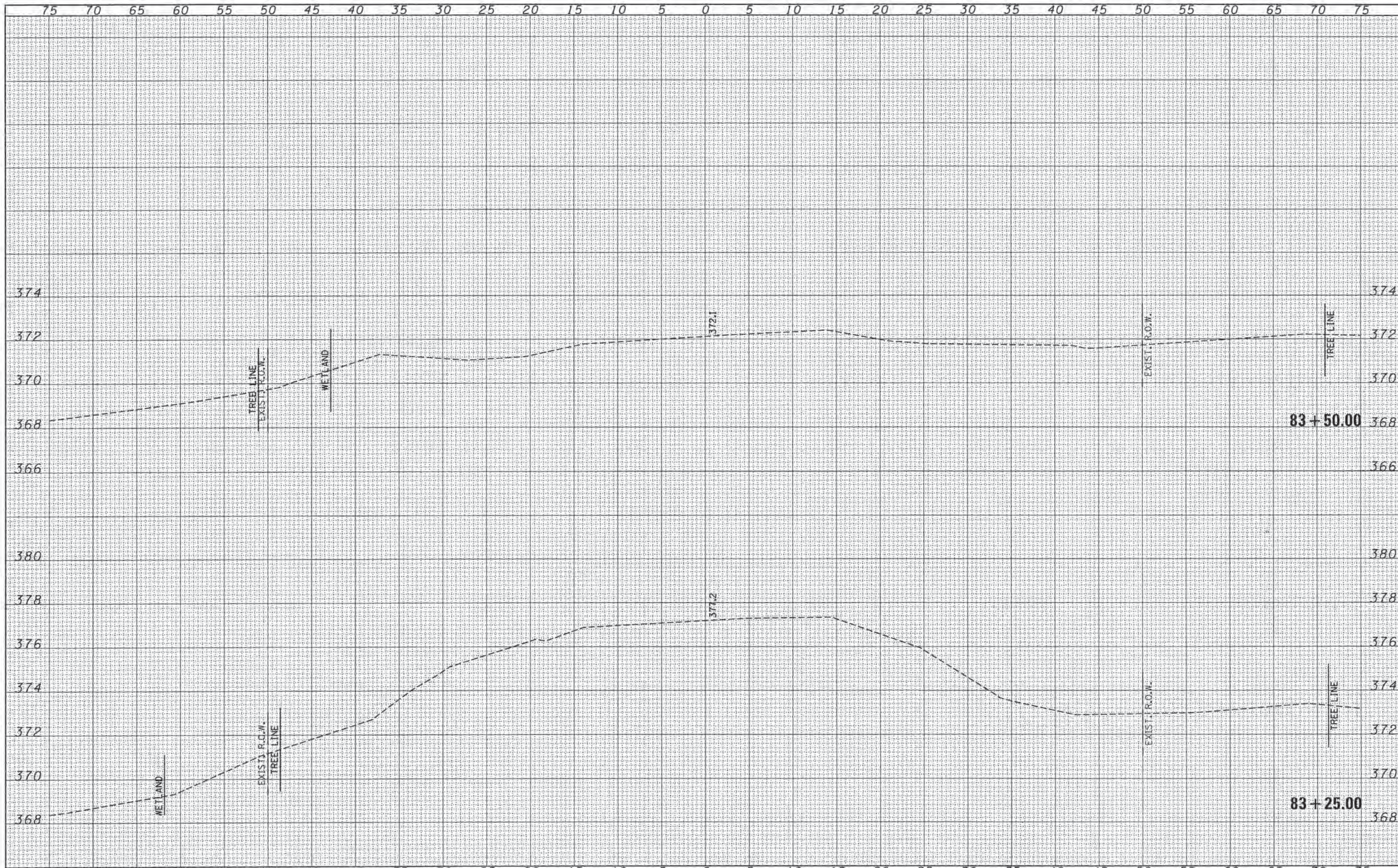
BK OF WEST ABUT

AHEAD C = 0  
 BACK C = 5  
 F = 0  
 F = 11

83+00.00

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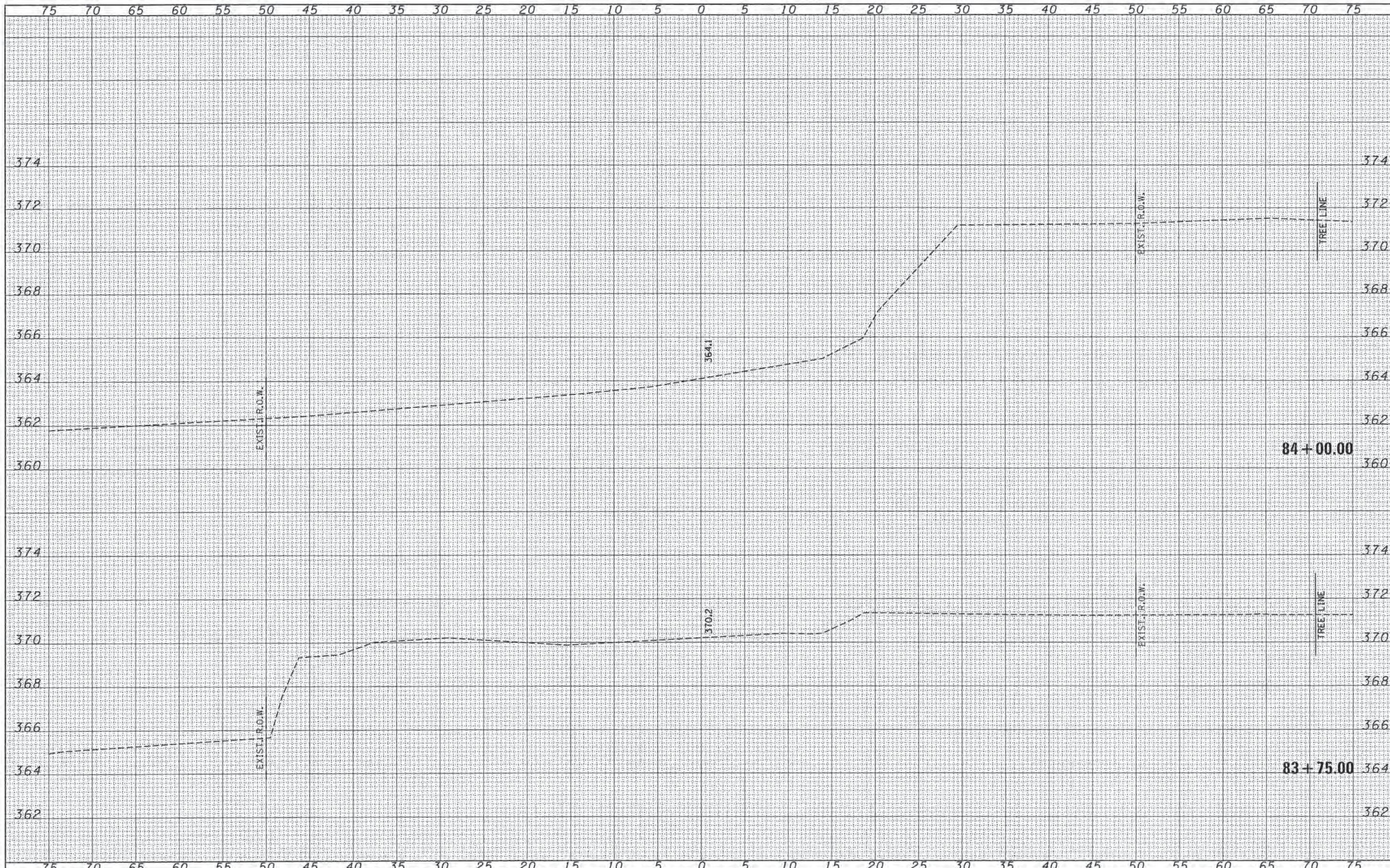
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	51				
300 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V				SHEET NO. OF SHEETS		CONTRACT NO. 99519		
ILLINOIS PROFESSIONAL DESIGN FIRM 631 P.E. / 92 C.D.P.	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		STA. 83+25.00 TO STA. 83+50.00				ILLINOIS FED. AID PROJECT				

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FINAL SURVEY	SUBMITTED
NOTE BOOK	PLOTTED
MS.	TEMPLATE
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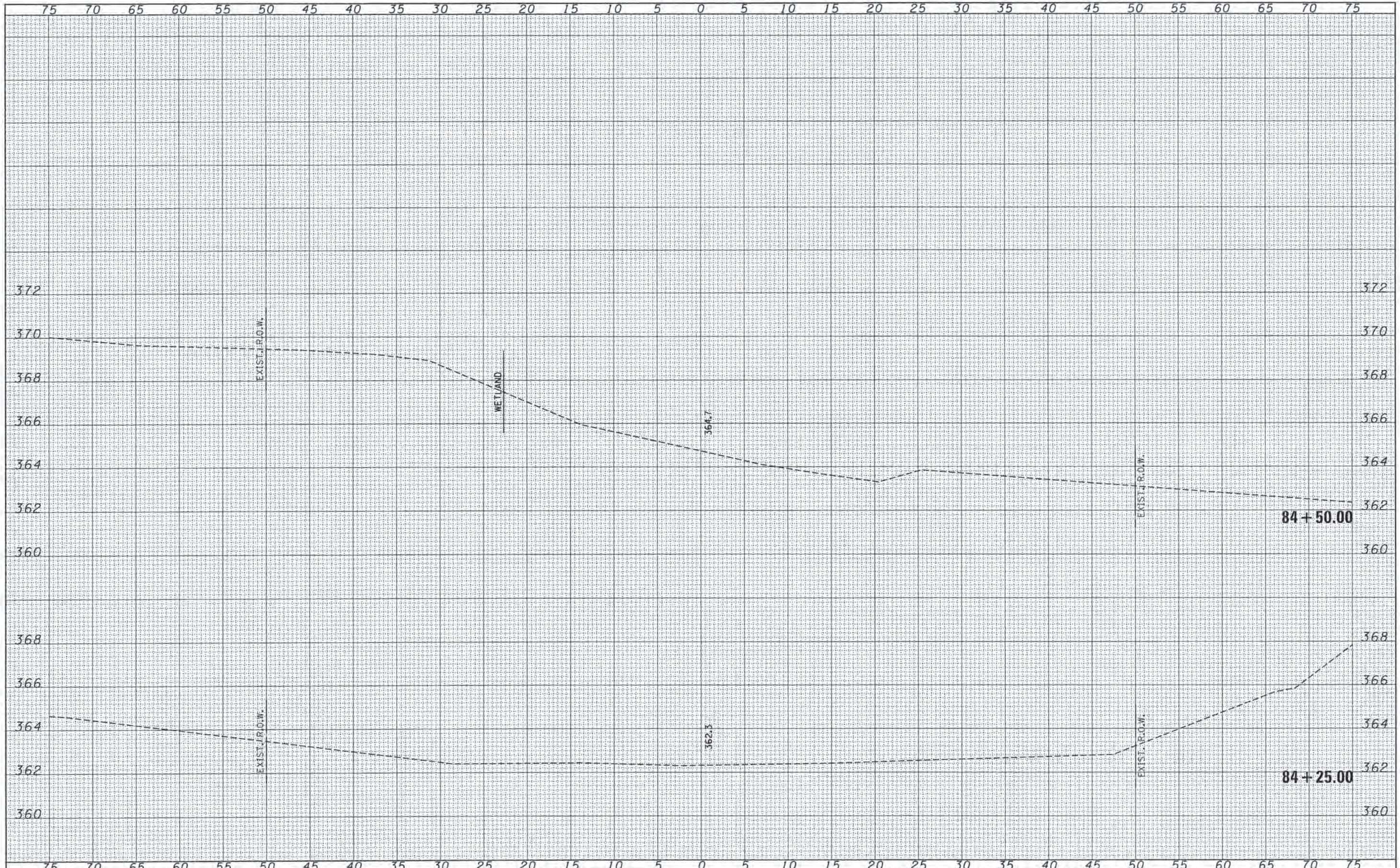
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<b>HAMPTON, LENZINI AND RENWICK, INC.</b> 201 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62702	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	52	CONTRACT NO. 99519		
<b>HLR</b> ILLINOIS PROFESSIONAL DESIGN FIRM 131 WEST 54th STREET, CHICAGO, IL 60605	PLOT DATE = 4/6/2015	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 83+75.00 TO STA. 84+00.00	ILLINOIS FED. AID PROJECT				
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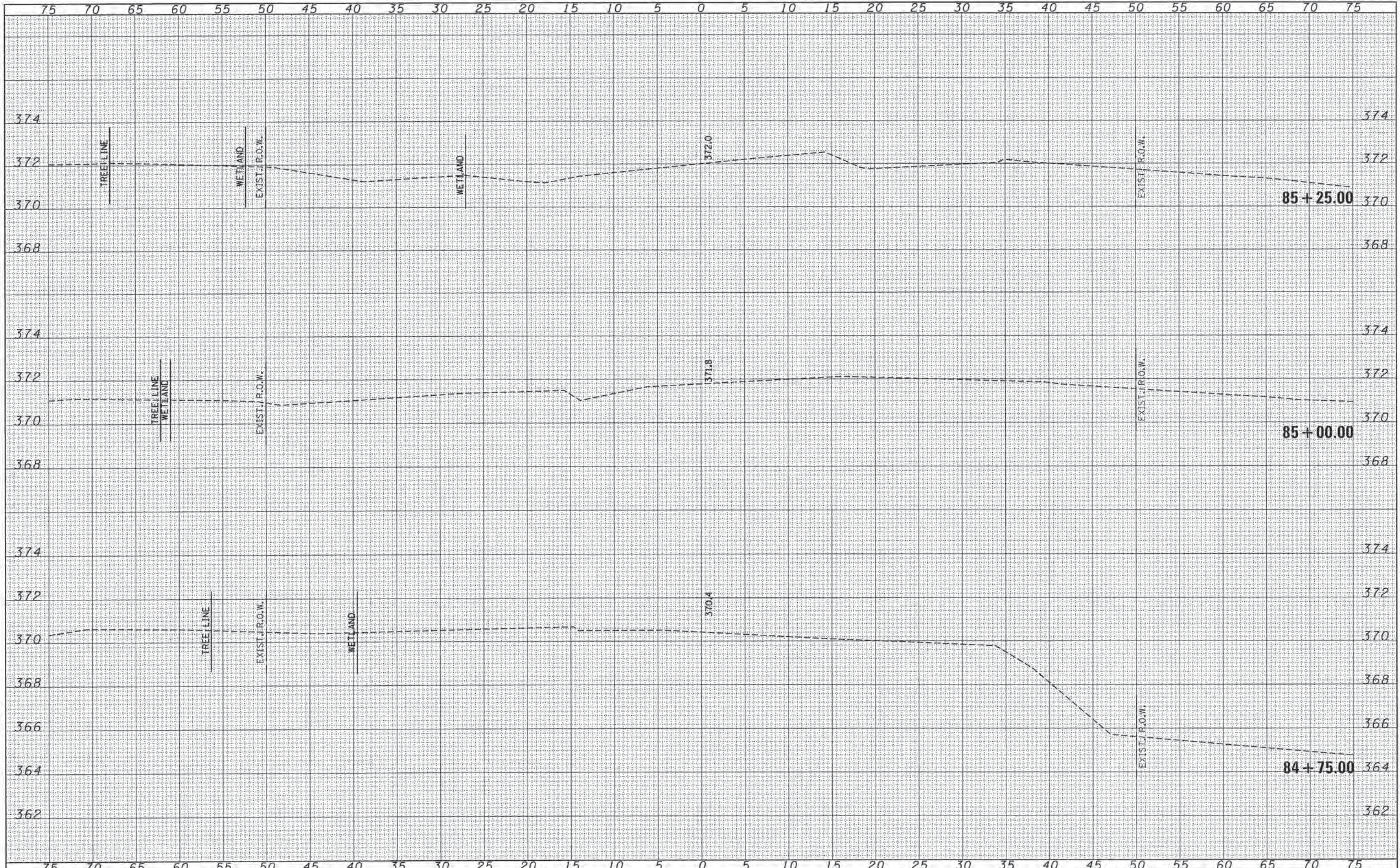
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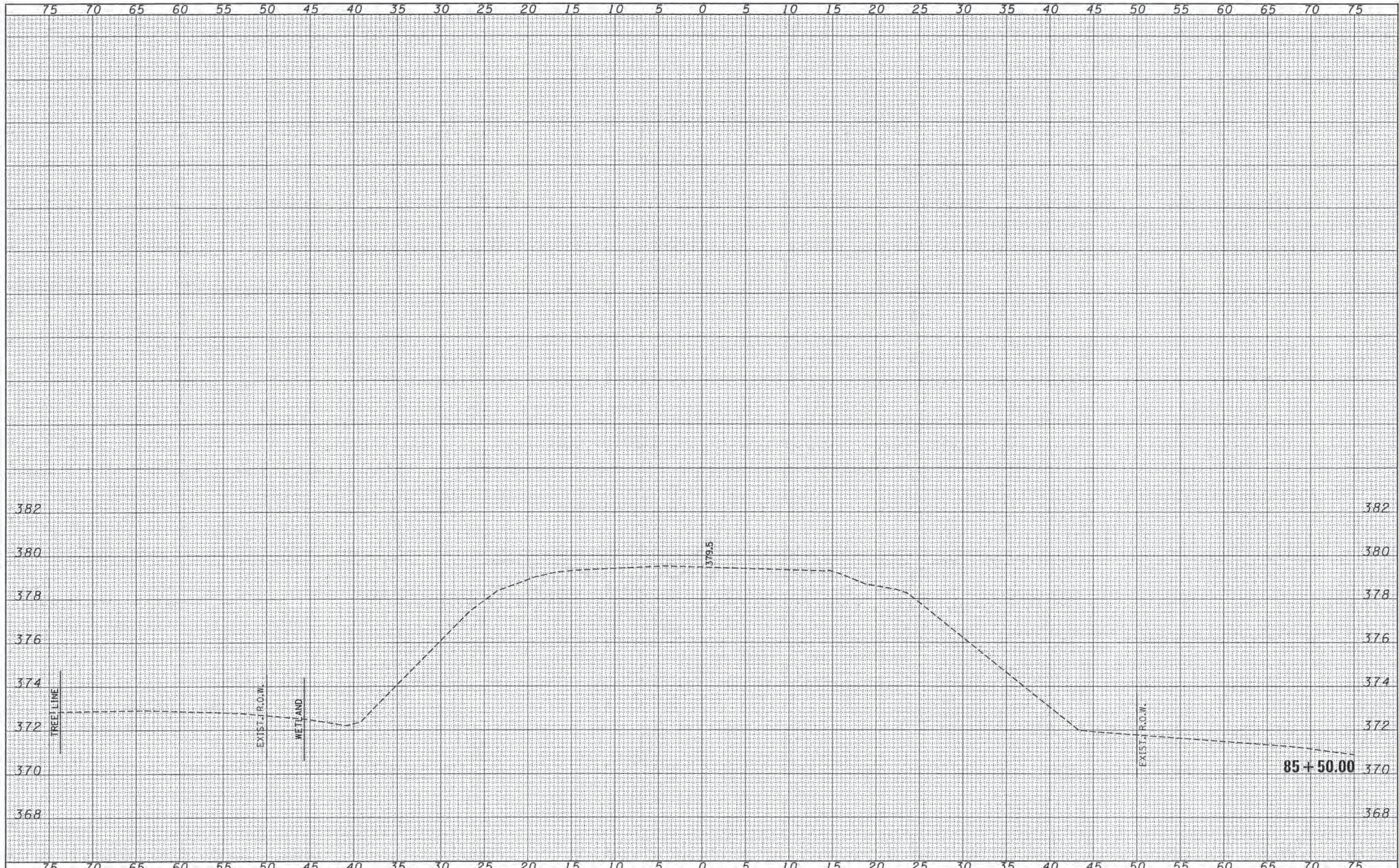
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -			869	10-00163-00-BR	JACKSON	82	53	
3841 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62709	PLOT SCALE =	CHECKED - J.W.F.	REVISED -			CONTRACT NO. 99519					
ILLINOIS PROFESSIONAL DESIGN FIRM LS + PE LLC CORP. 184.000008	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -			[ILLINOIS] FED. AID PROJECT					

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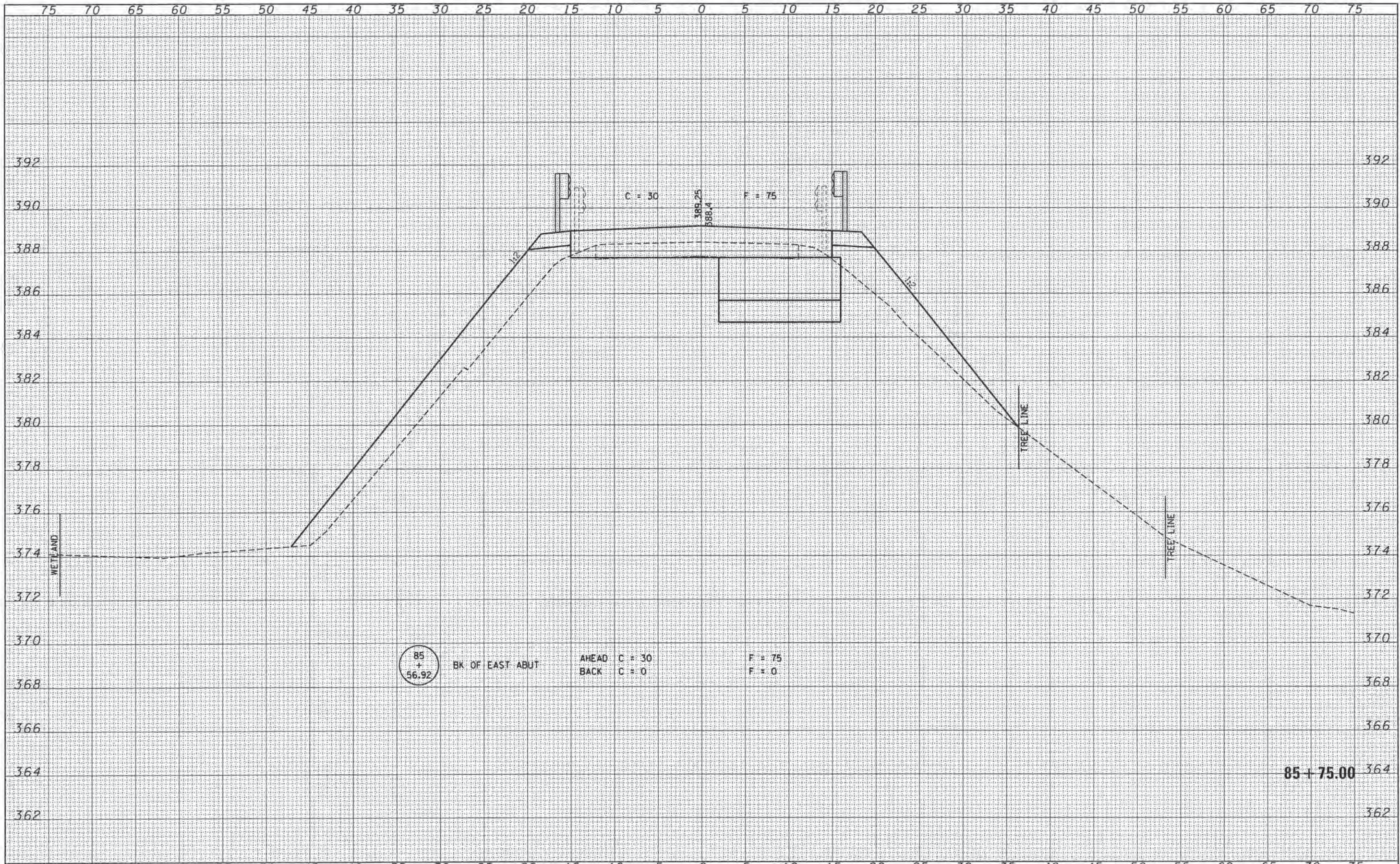
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	54	CONTRACT NO. 99519	
3800 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62781	PLLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5/42V			SHEET NO. OF SHEETS STA. 84+75.00 TO STA. 85+25.00			ILLINOIS FED. AID PROJECT
ILLINOIS PROFESSIONAL DESIGN FIRM L31/PE/SE/ENR - 04-00056	PLLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -								



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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	55				
3581 STEVENSON DRIVE, SUITE 250 SPRINGFIELD, ILLINOIS 62761		CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519								
ILLINOIS PROFESSIONAL DESIGN FIRM L3 / PLEASE CONP. 196 00089		DATE - 02/23/15	REVISED -		SCALE: 5Hx2V	SHEET NO.	OF	SHEETS	STA. 85+50.00 TO STA. 85+50.00	ILLINOIS FED. AID PROJECT			

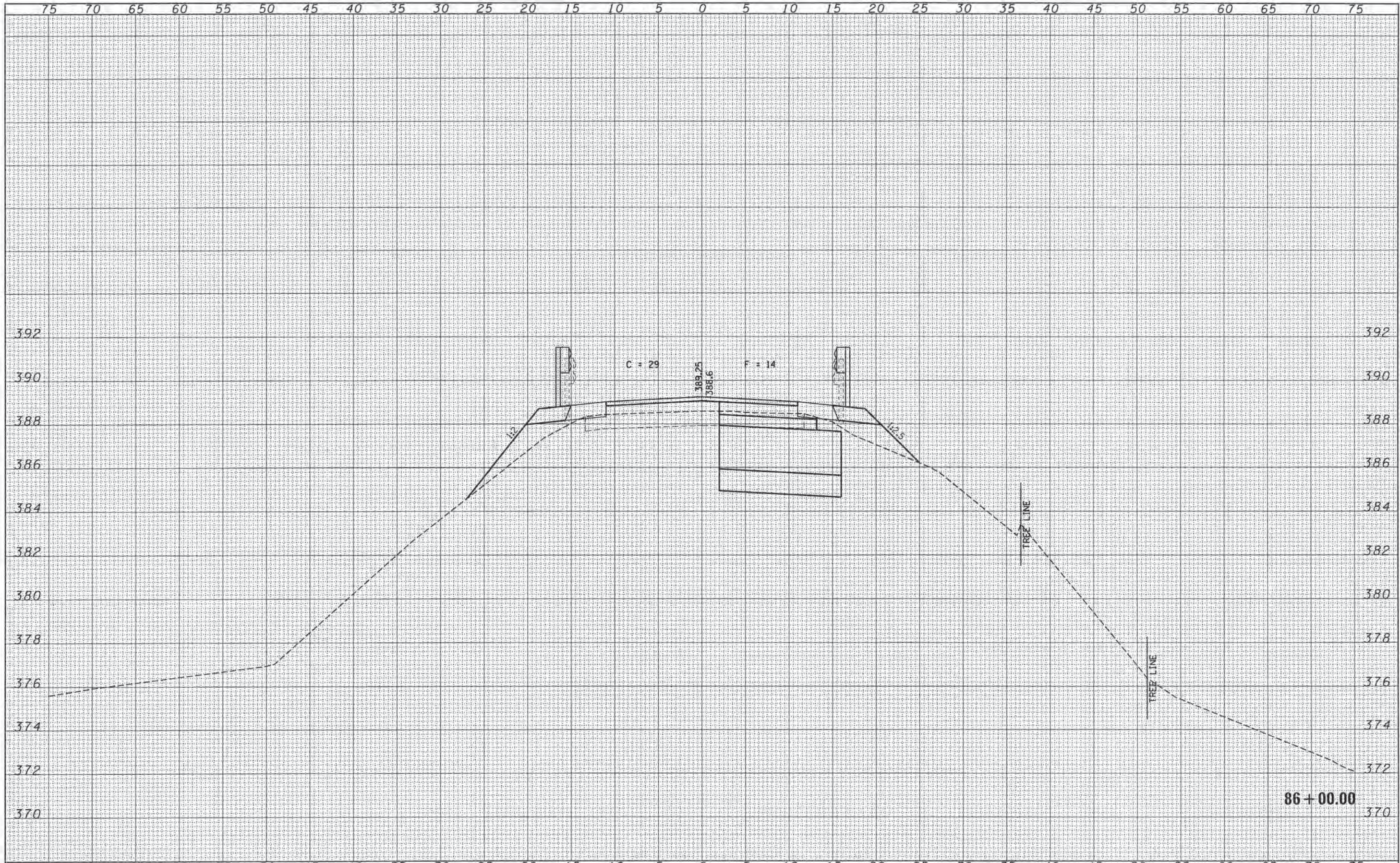


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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -			869	10-00163-00-BR	JACKSON	82	56	
330 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761		CHECKED - J.W.F.	REVISED -			SCALE: 5H:2V		SHEET NO. OF SHEETS		STA. 85+75.00 TO STA. 85+75.00	
ILLINOIS PROFESSIONAL DESIGN FIRM L3 / P1 / F88 / COMP. 16-00899		DATE - 02/23/15	REVISED -			CONTRACT NO. 99519					
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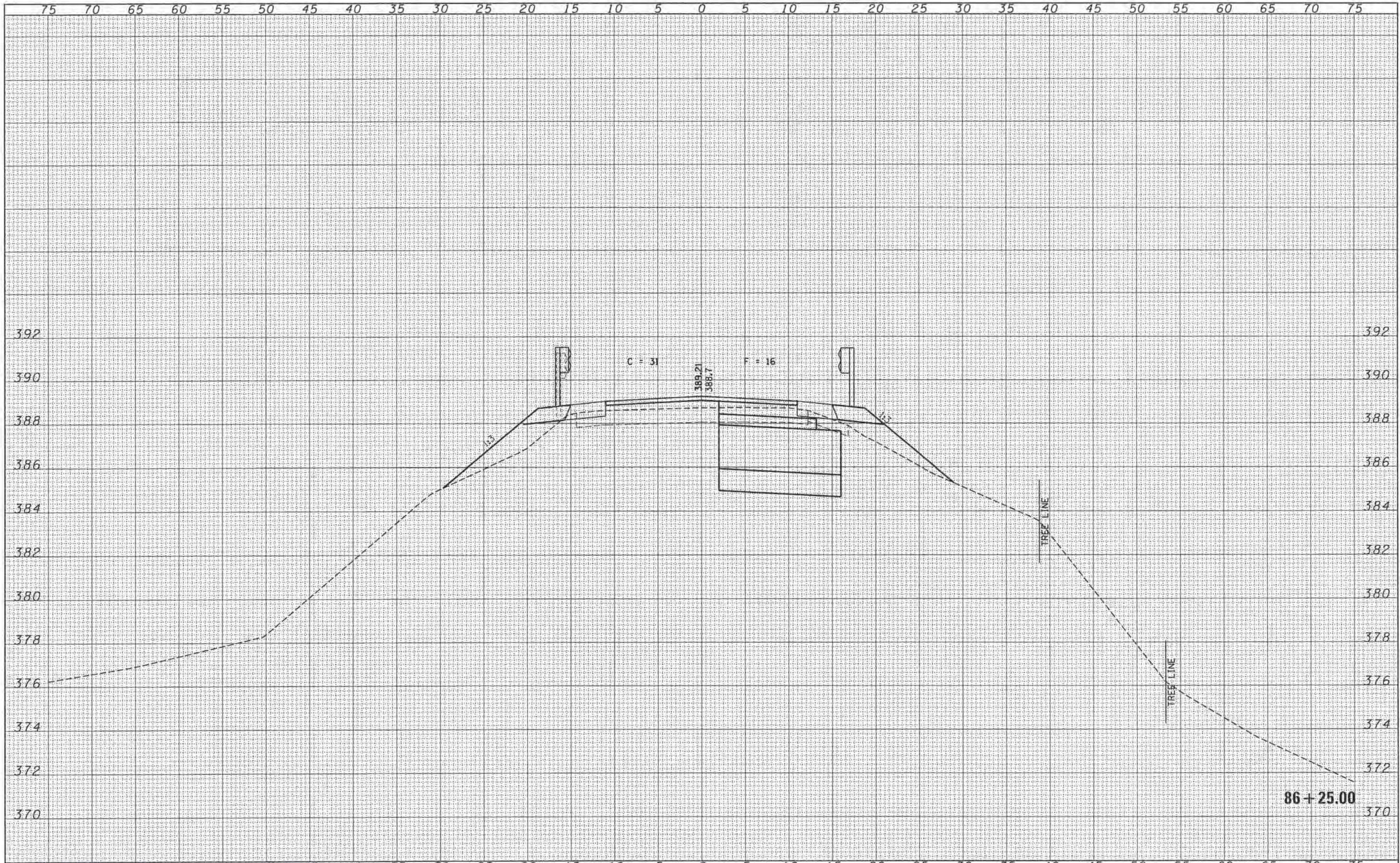




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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	57	CONTRACT NO. 99519	
391 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62768		CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 86+00.00 TO STA. 86+00.00	ILLINOIS FED. AID PROJECT			
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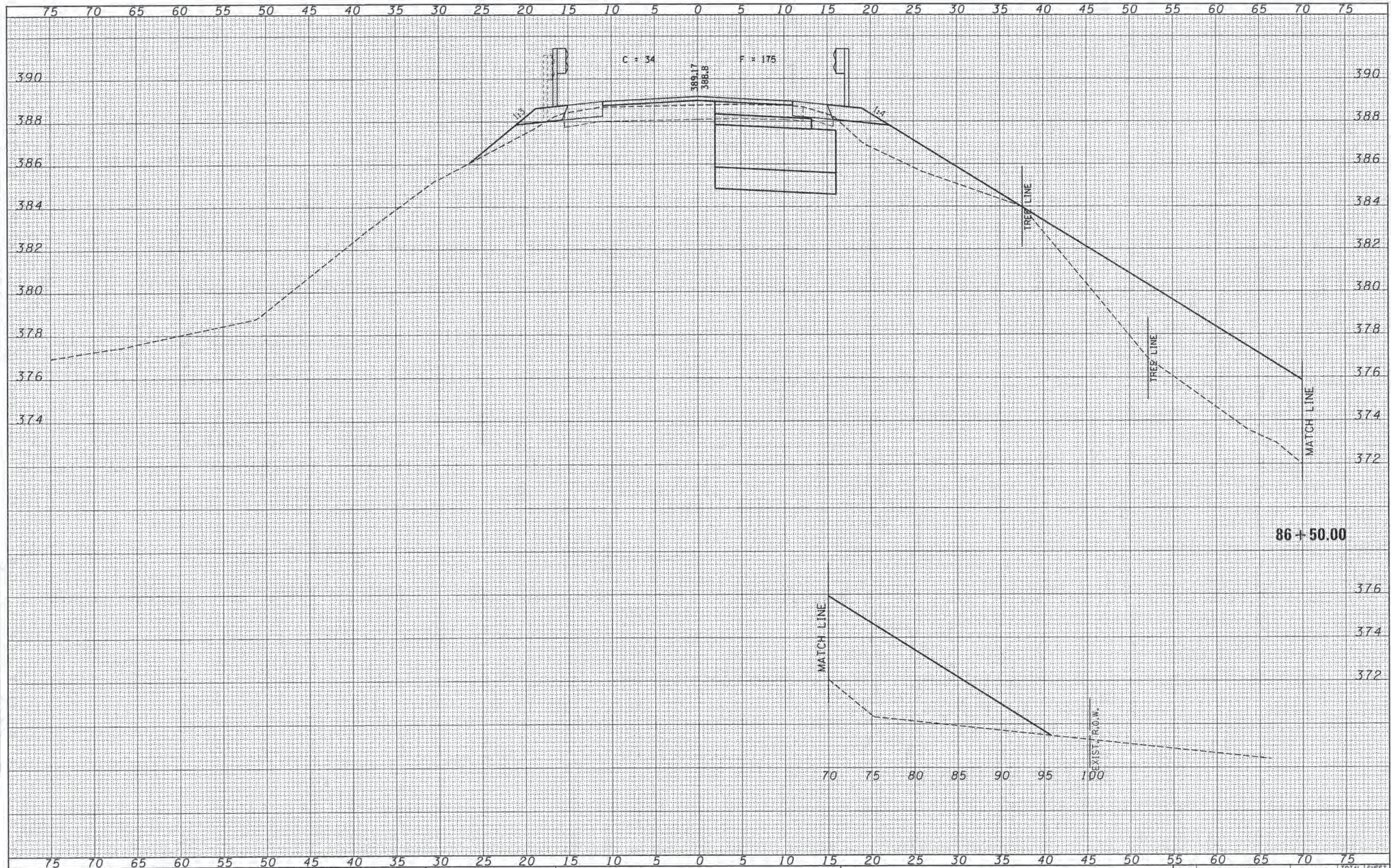
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HAMPTON, LENZINI AND RENWICK, INC. 3333 STEVENSON DRIVE, SUITE 207 SPRINGFIELD, ILLINOIS 62760	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	58				
ILLINOIS PROFESSIONAL DESIGN FIRM L3 / PE / SE CORP. 194.000033	PLOT DATE = 4/6/2015	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V			SHEET NO.	OF	SHEETS	STA. 86+25.00	TO STA. 86+25.00	CONTRACT NO. 99519
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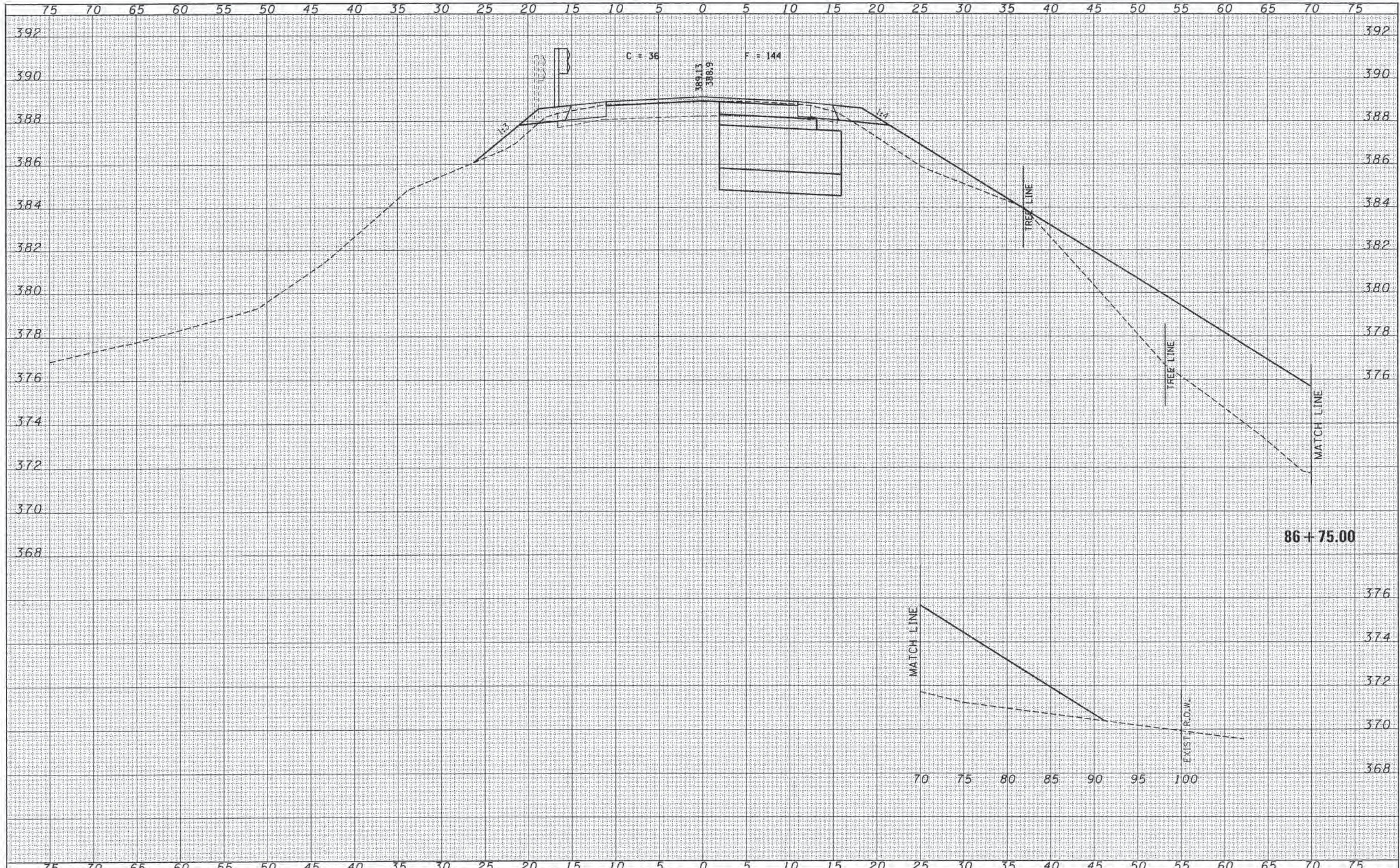
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	59			
3004 STEPHENSON DRIVE, SUITE 207 SPRINGFIELD, ILLINOIS 62767	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V	SHEET NO.	OF	SHEETS	STA. 86+50.00	TO STA. 86+50.00	ILLINOIS FED. AID PROJECT	
ILLINOIS PROFESSIONAL DESIGN FIRM L.S. / P.E. / S.E. / C.E.P.R. / 194308893	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -									

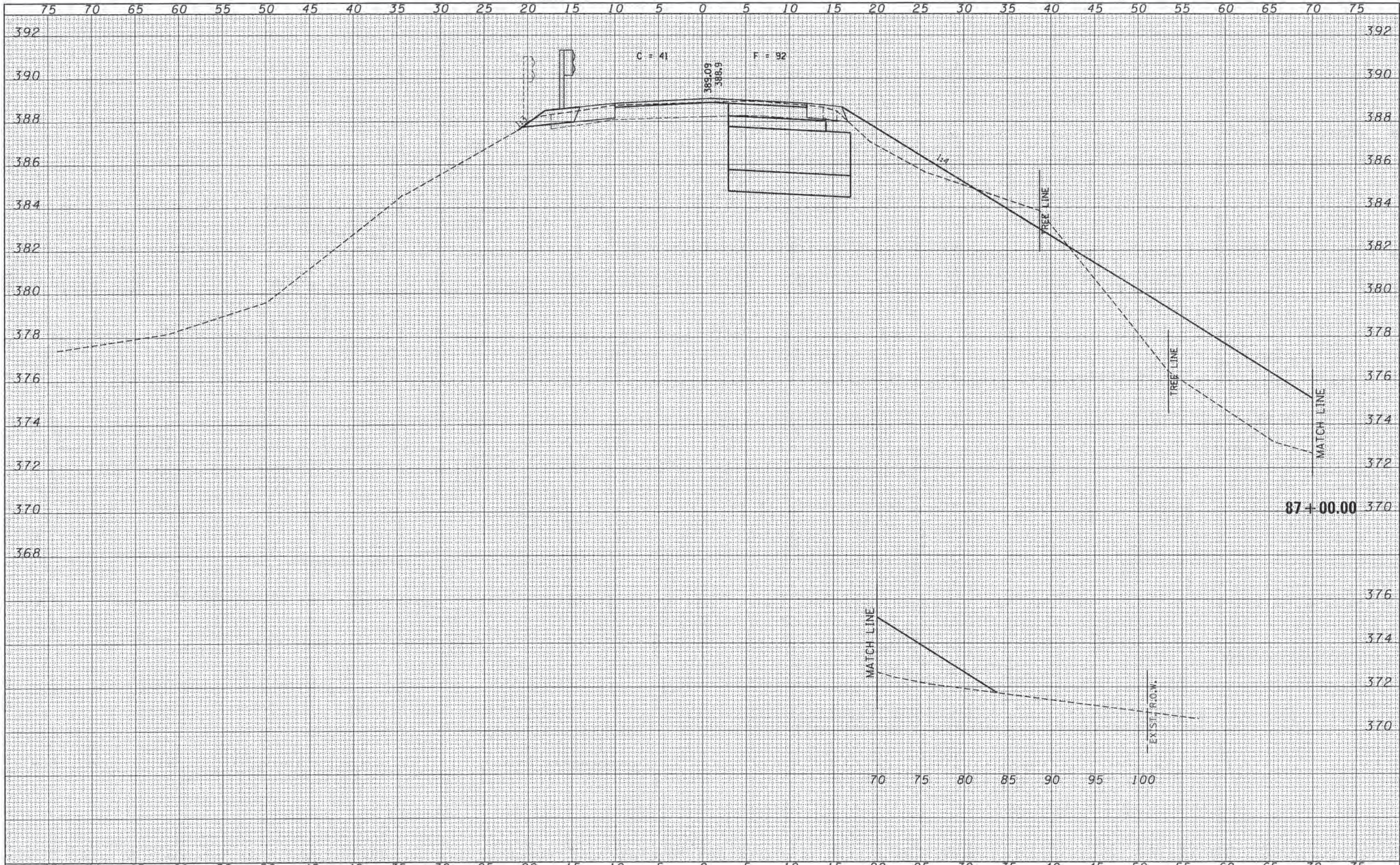
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -	JACKSON COUNTY HIGHWAY DEPARTMENT		C.H. 14 / ROYALTON ROAD		869	10-00163-00-BR	JACKSON	82	60
2015 SITE VERSION DOWNE 3/17/15		CHECKED - J.W.F.	REVISED -									CONTRACT NO. 99519
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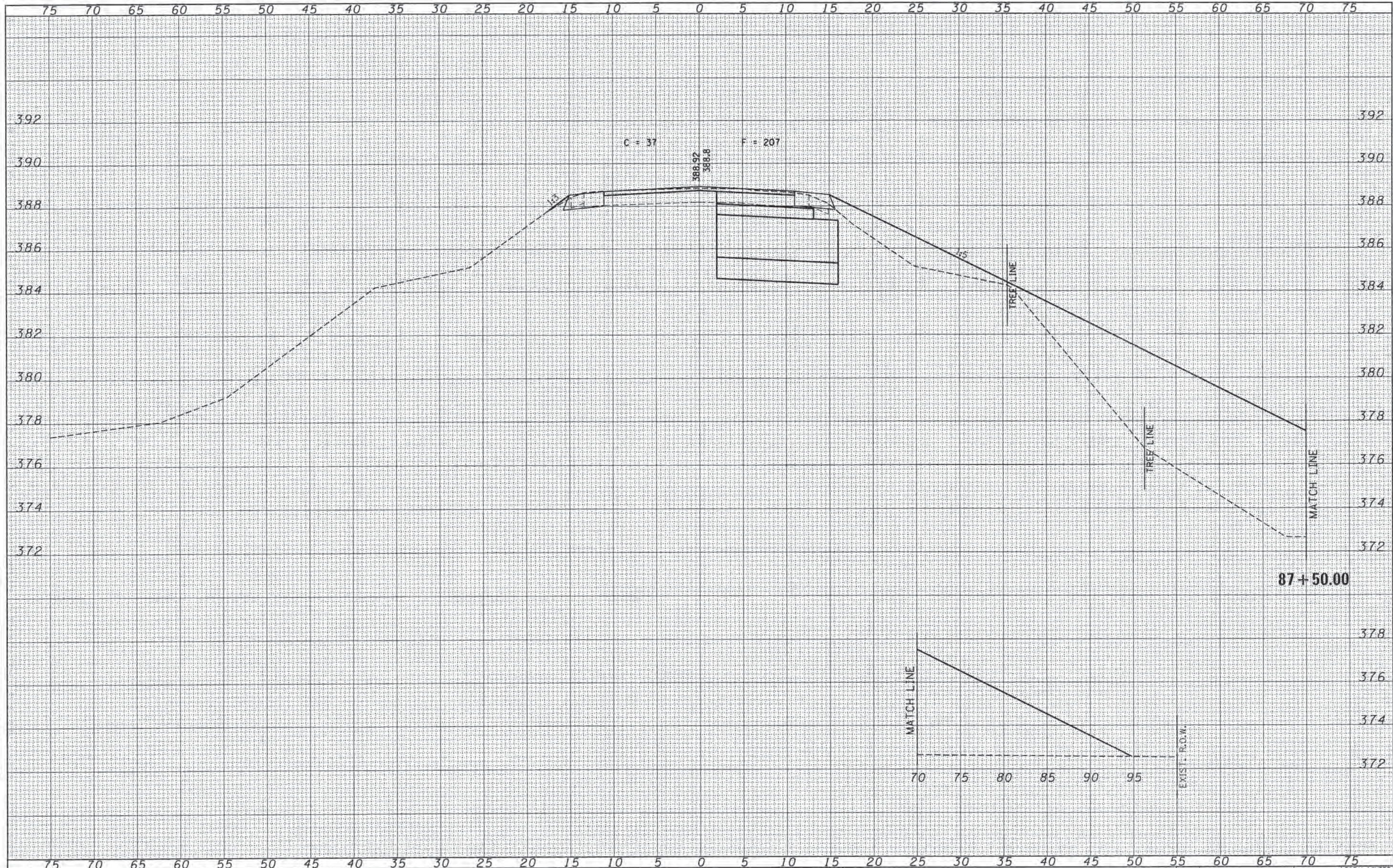
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<b>HAMPTON, LENZINI AND RENWICK, INC.</b>		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	61				
3101 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62781	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519								
ILLINOIS PROFESSIONAL DESIGN FIRM 12/19/02 REG. NO. 0278	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 87+00.00 TO STA. 87+00.00	ILLINOIS FED. AID PROJECT					

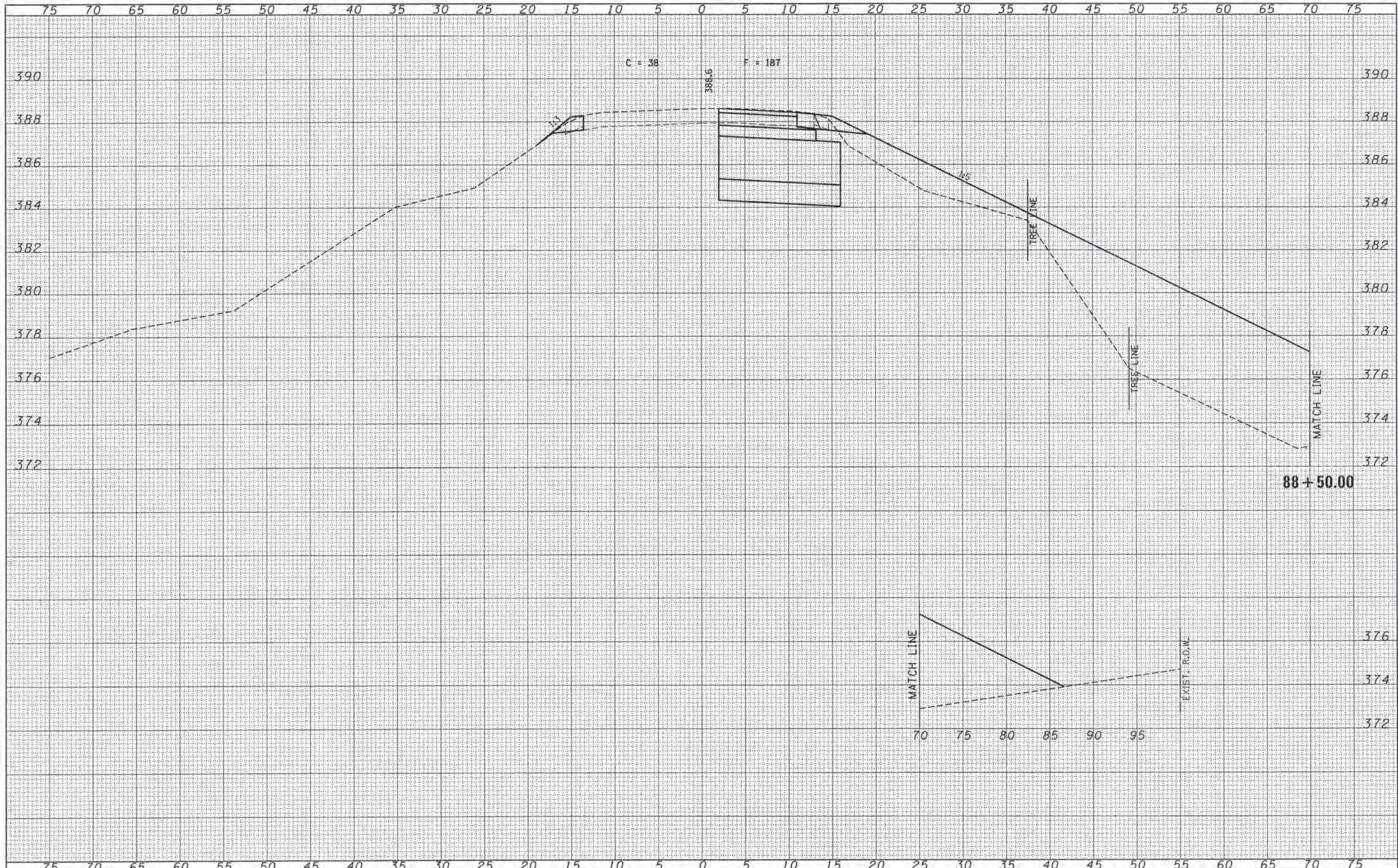
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	62		
300 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761 ILLINOIS PROFESSIONAL ENGINEERING FIRM L.L. / R.E. / M.C. CORP. 134-000009	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 87+50.00 TO STA. 87+50.00	CONTRACT NO. 99519			
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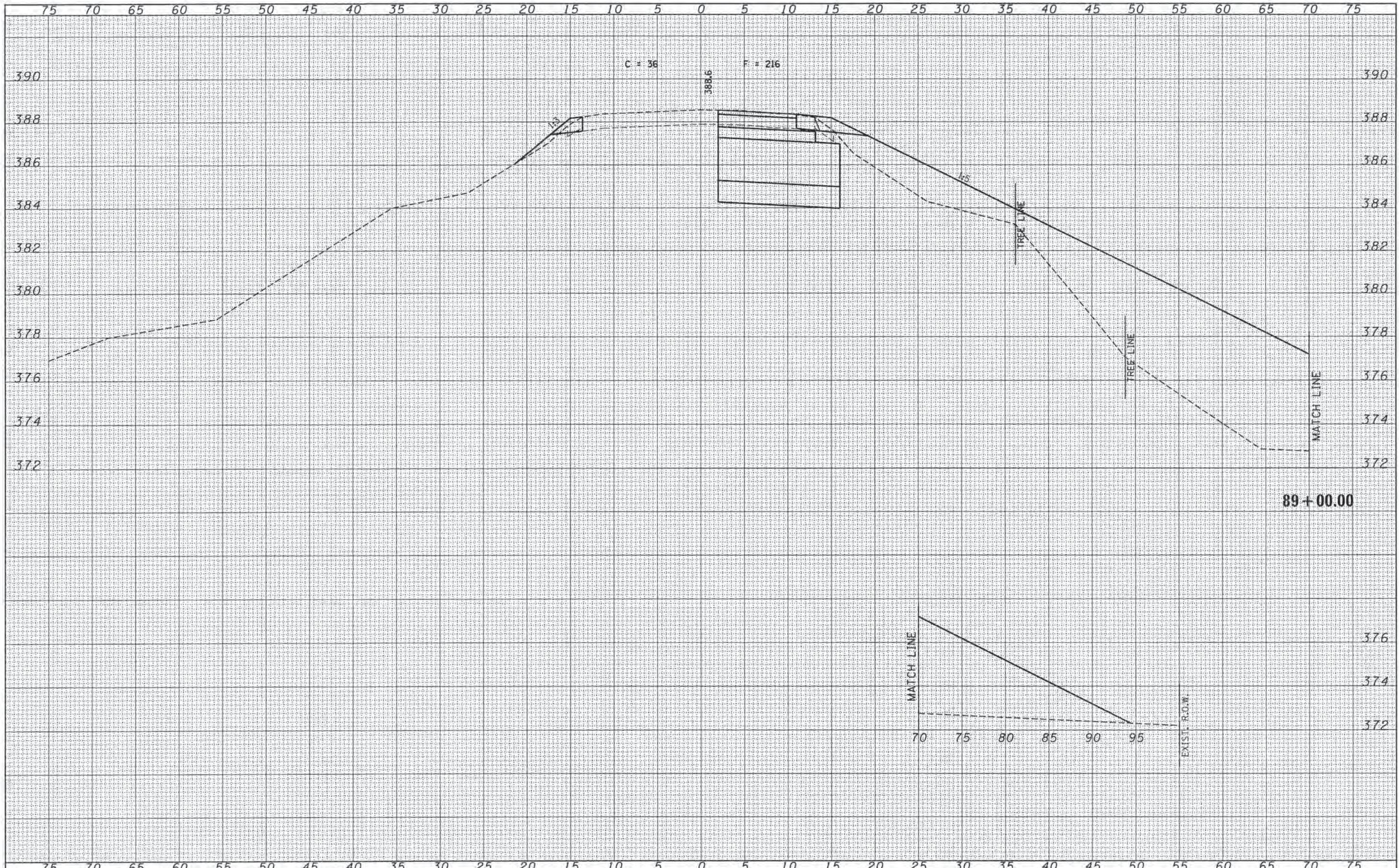
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HAMPTON, LENZINI AND RENWICK, INC. 398 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62708	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		869	64	10-00163-00-BR	JACKSON	82	64			
ILLINOIS PROFESSIONAL DESIGN FIRM L3 (FR) 7 88 CORP. 04/2009	PLOT DATE = 4/6/2015	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V    SHEET NO. OF SHEETS    STA. 88+50.00 TO STA. 88+50.00				CONTRACT NO. 99519				
		DATE - 02/23/15	REVISED -		ILLINOIS FED. AID PROJECT								



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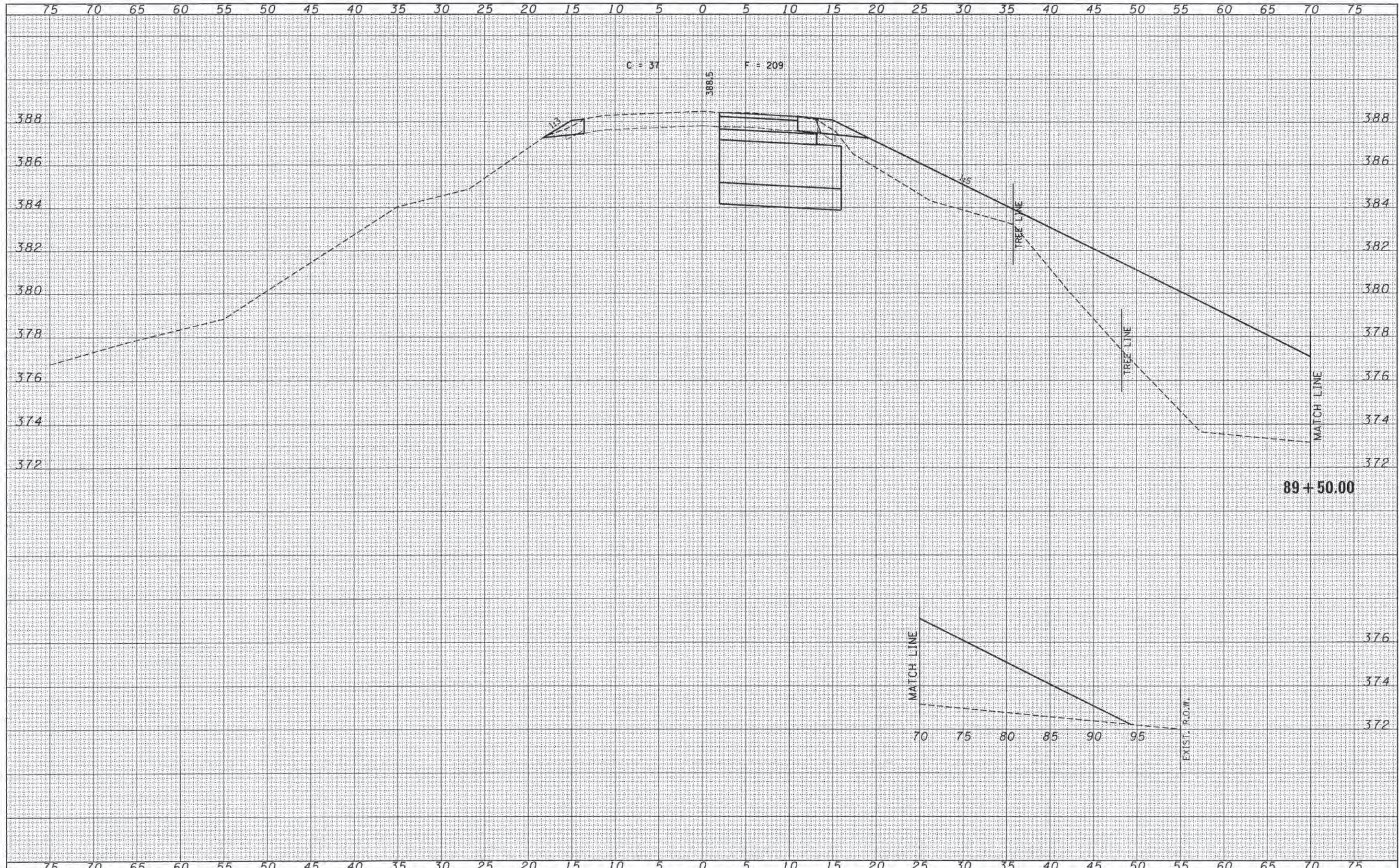
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HAMPTON, LENZINI AND RENWICK, INC. 3015 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761		DRAWN - T.W.K.	REVISED -	B69	10-00163-00-BR	JACKSON	82	65
<b>HLR</b> ILLINOIS PROFESSIONAL DESIGN FIRM 131 W. 1 <sup>ST</sup> ST. CO. BLDG. 184-00083	PLOT SCALE =	CHECKED - J.W.F.	REVISED -	CONTRACT NO. 99519				
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				SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 89+00.00 TO STA. 89+00.00		

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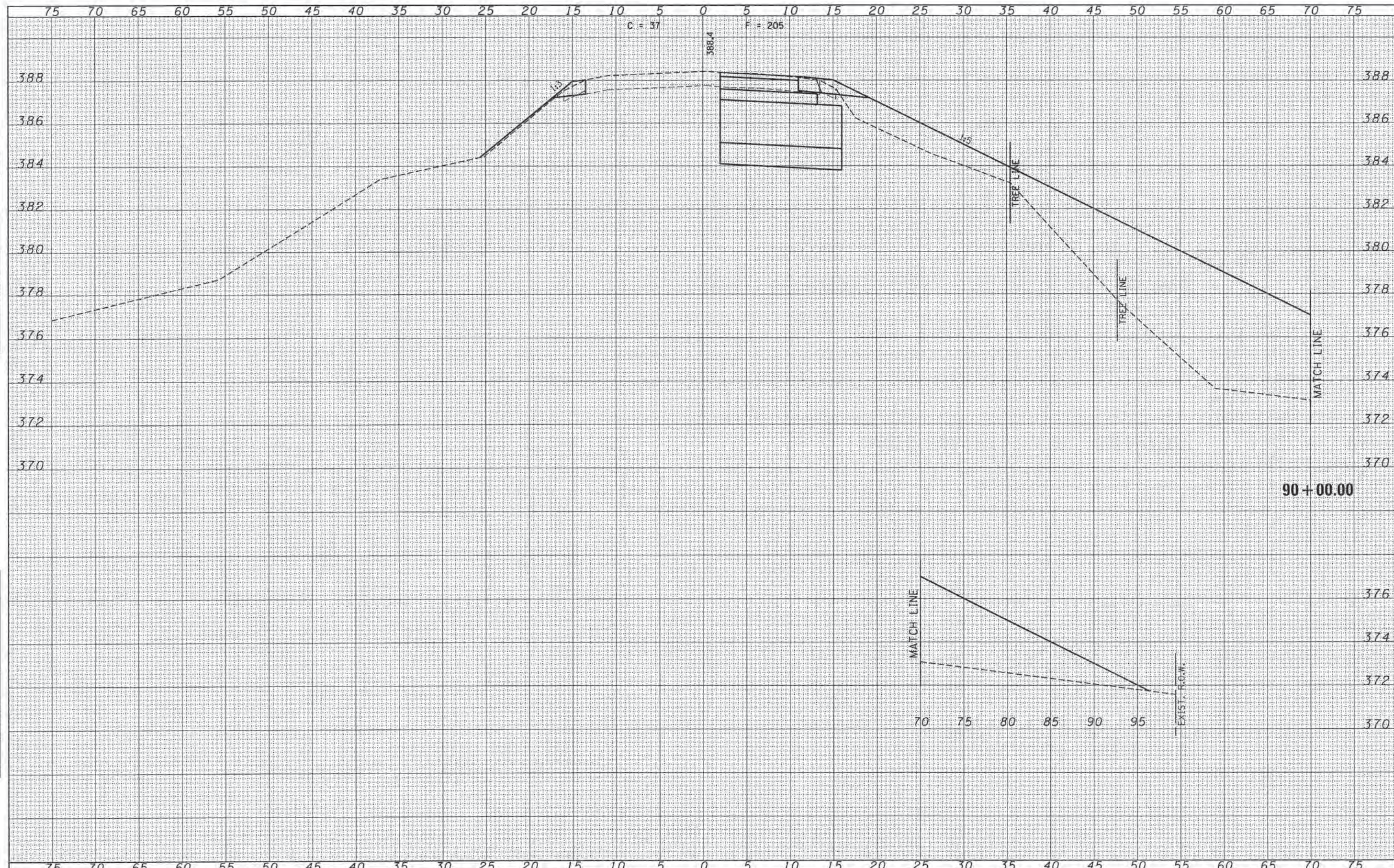
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	66	CONTRACT NO. 99519	
300 E. VANDERBILT DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V	SHEET NO.	OF SHEETS	STA. 89+50.00	TO STA. 89+50.00	ILLINOIS FED. AID PROJECT	
ILLINOIS PROFESSIONAL DESIGN FIRM 18 FIVE FIVE CORP. 194 200018	PLOT DATE = 4/6/2015	DATE = 02/23/15	REVISED -								

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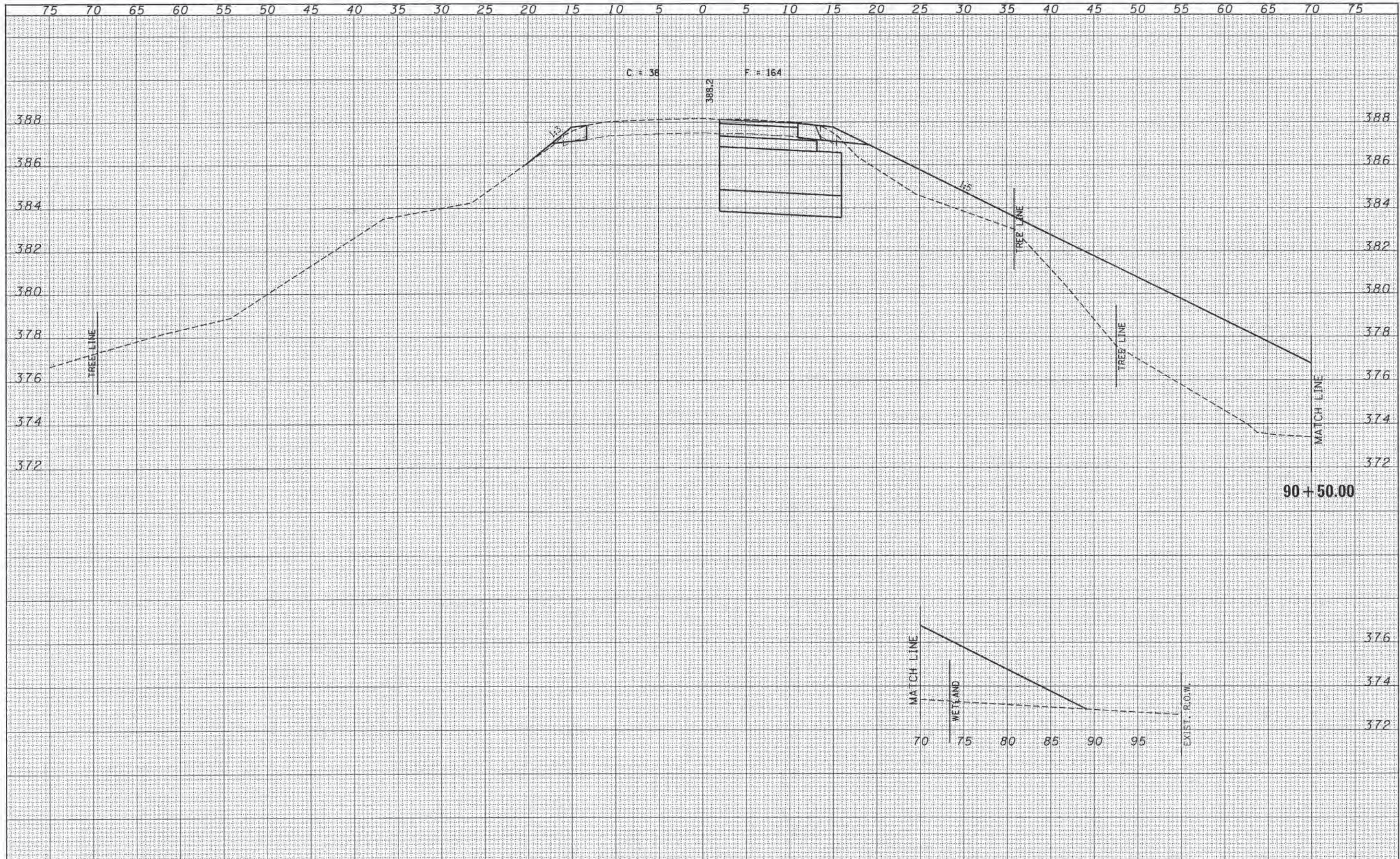
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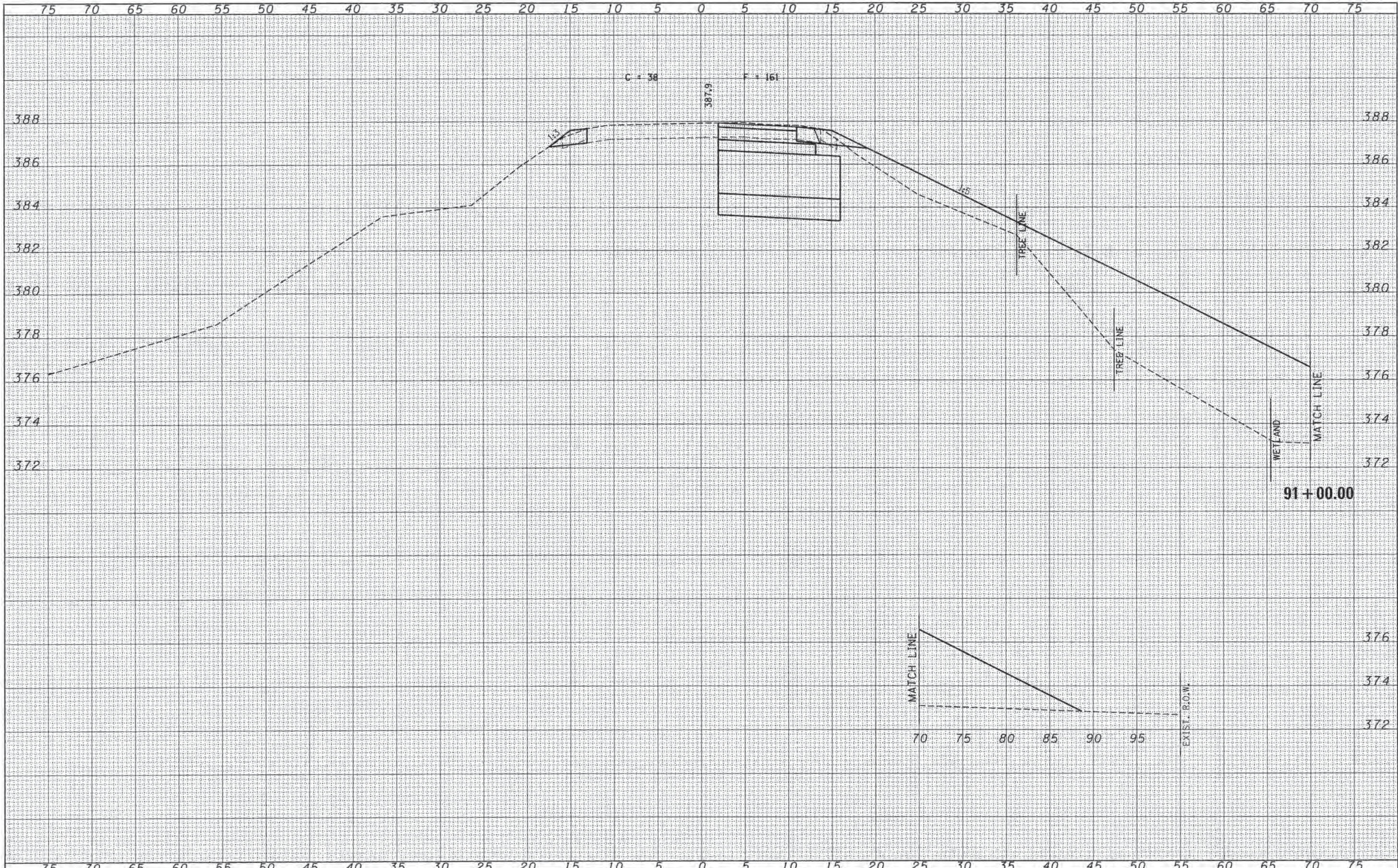
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	67				
300 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62781		CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V			SHEET NO.	OF	SHEETS	STA. 90+00.00 TO STA. 90+00.00	CONTRACT NO. 99519	
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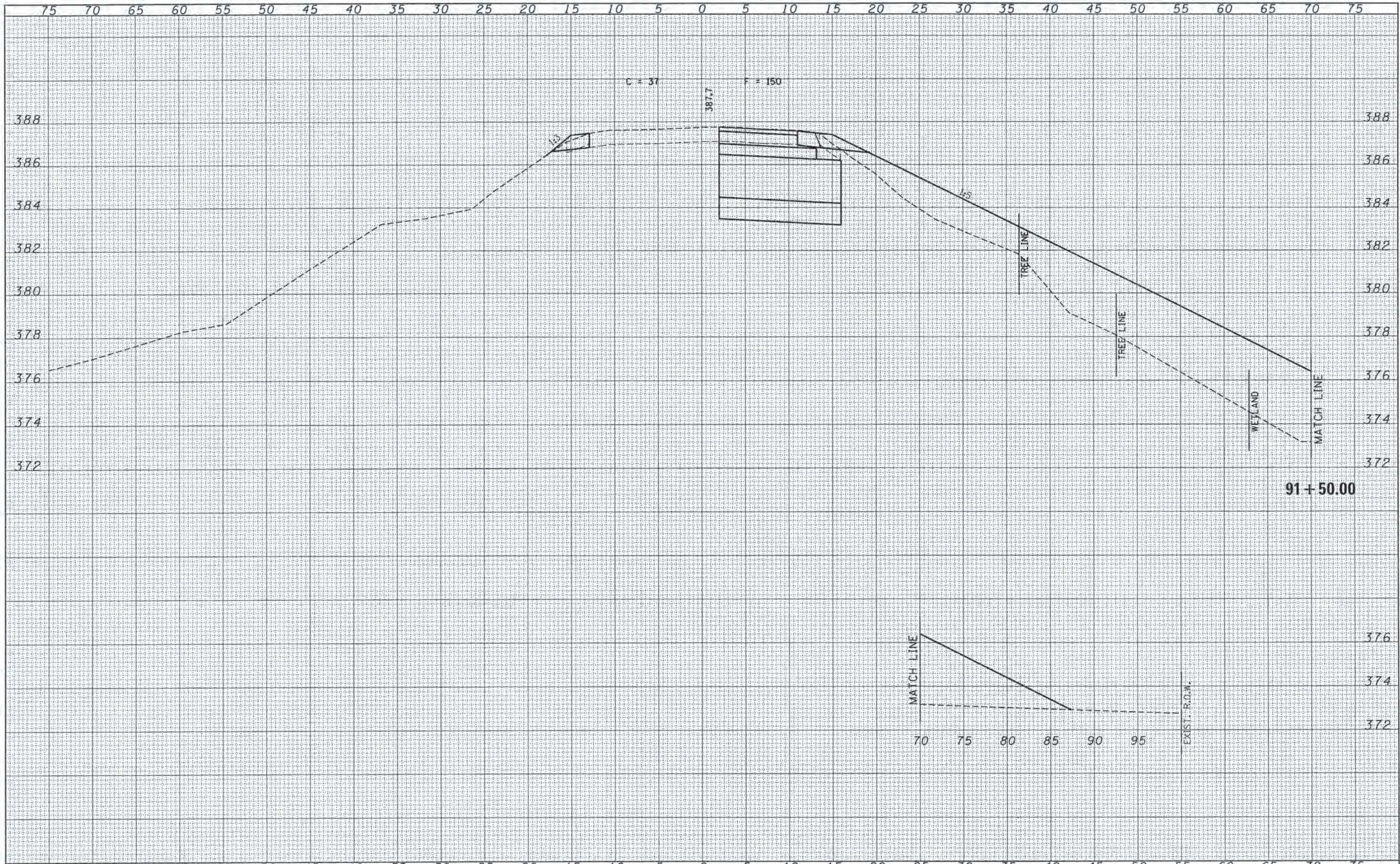
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	68		
300 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519						
ILLINOIS PROFESSIONAL DESIGN FIRM L3 (F.E.) 116 CORP. 194-000000	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 90+50.00 TO STA. 90+50.00	[ILLINOIS] FED. AID PROJECT			



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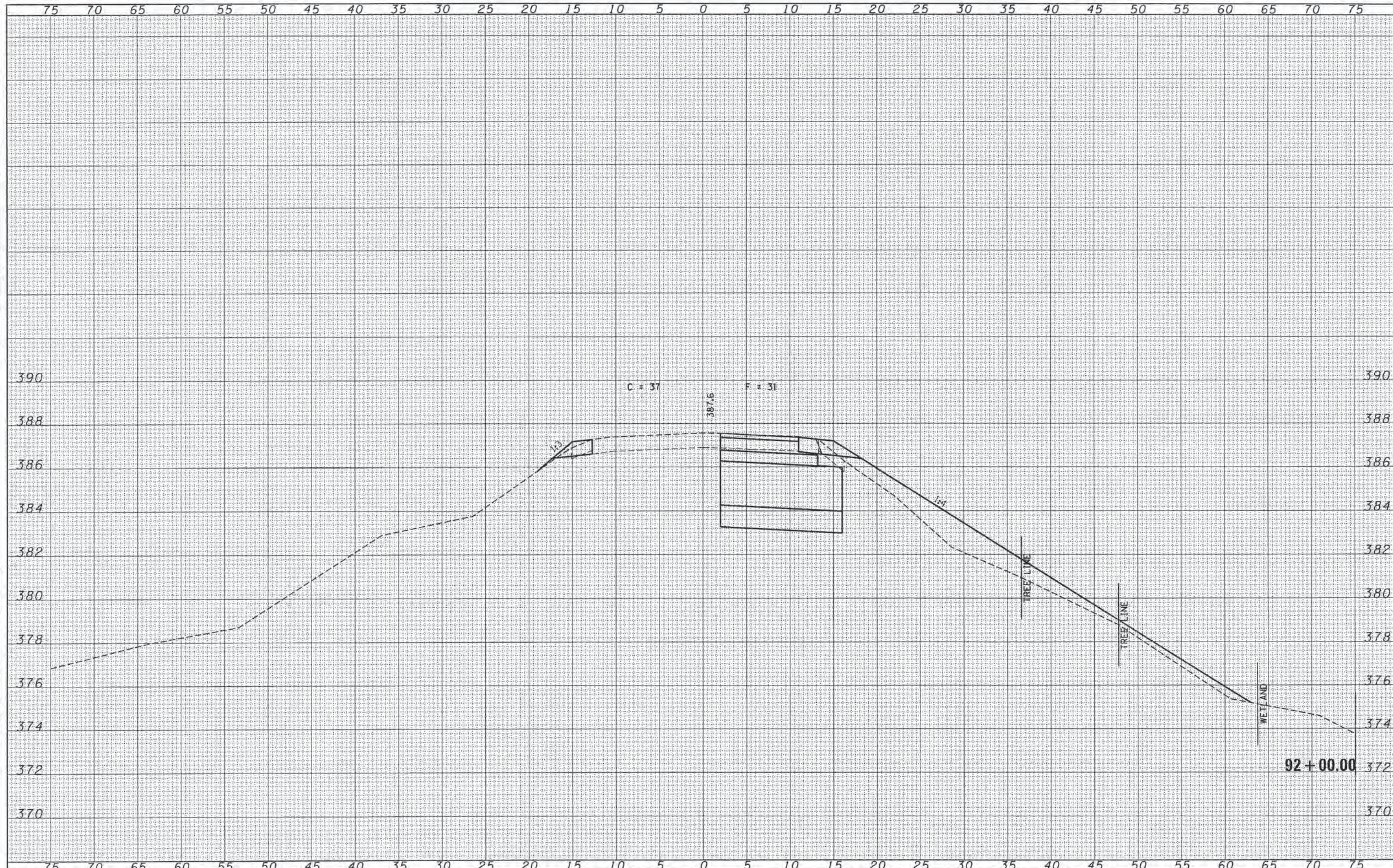
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -	869	10-00163-00-BR	JACKSON	82	69
2001 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62701		CHECKED - J.W.F.	REVISED -					CONTRACT NO. 99519
ILLINOIS PROFESSIONAL DESIGN FIRM 161 P.E. / 148 C.D.P.		DATE - 02/23/15	REVISED -	SCALE: 5/8"=2V				ILLINOIS FED. AID PROJECT
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				STATION CROSS SECTIONS C.H. 14 / ROYALTON ROAD				



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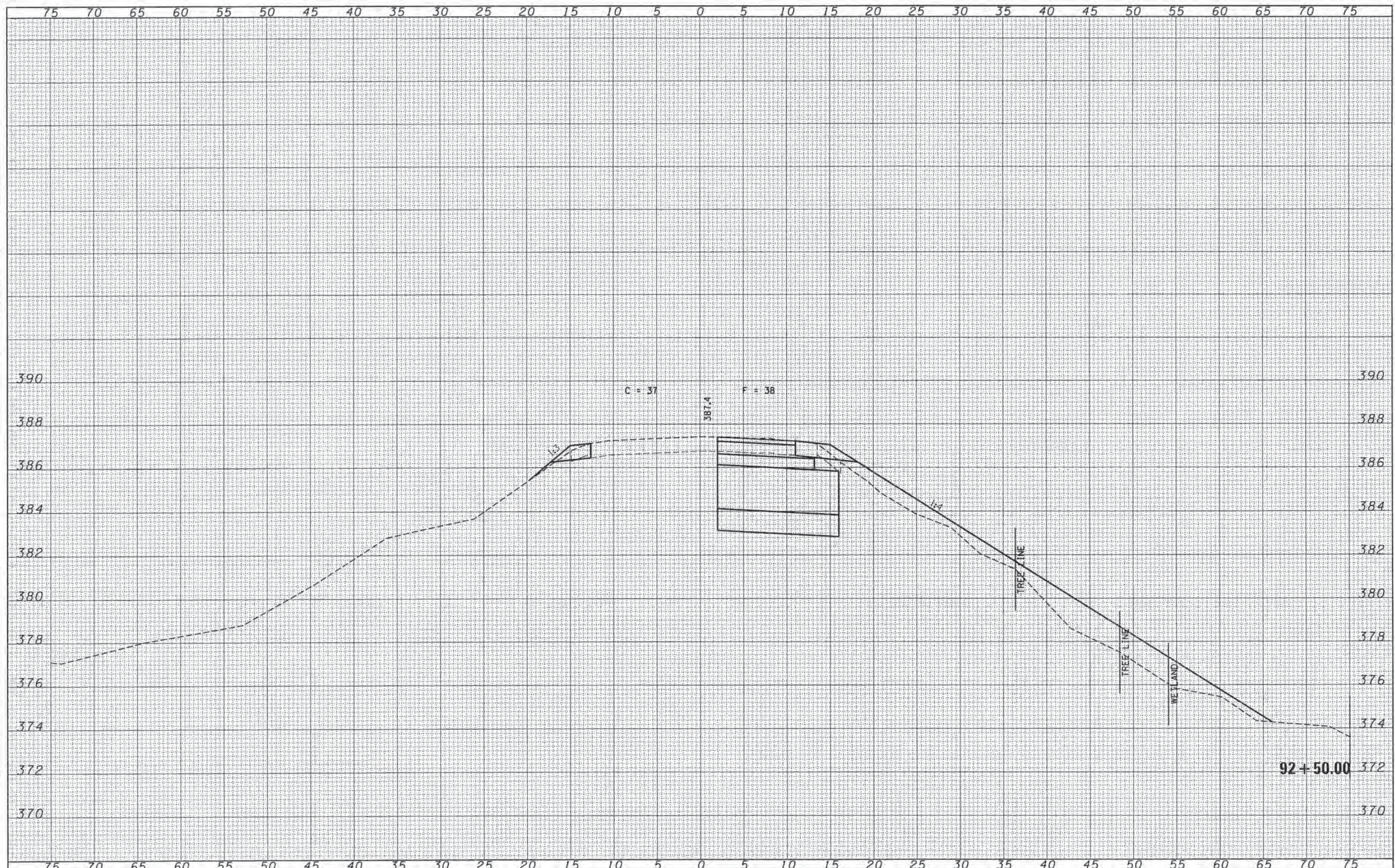
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	70				
300 STEVENSON DRIVE, SUITE 207 SPRINGFIELD, ILLINOIS 62761		CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V				SHEET NO. OF SHEETS	STA. 91+50.00 TO STA. 91+50.00	ILLINOIS FED. AID PROJECT		
ILLINOIS PROFESSIONAL DESIGN FIRM L.S. # 151-022977		DATE - 02/23/15	REVISED -						CONTRACT NO. 99519				



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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	71		
3041 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62764	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V		SHEET NO. OF SHEETS		STA. 92+00.00 TO STA. 92+00.00	CONTRACT NO. 99519	
ILLINOIS PROFESSIONAL DESIGN FIRM L.P. 118166 CORP. 184.00055	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		ILLINOIS FED. AID PROJECT						

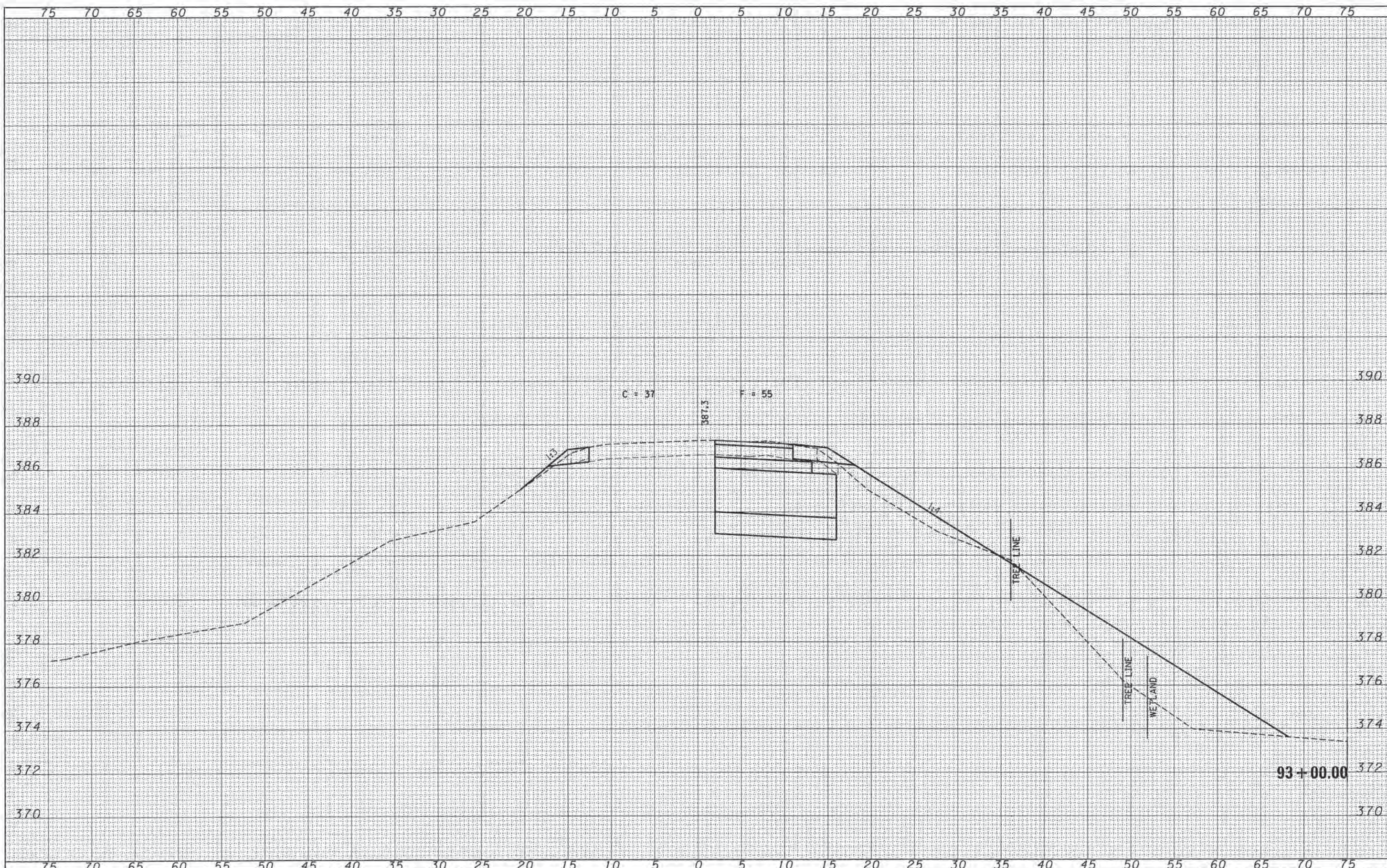


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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	72			
3915 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62765	PLOT SCALE :	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V		SHEET NO. OF SHEETS		STA. 92+50.00 TO STA. 92+50.00		CONTRACT NO. 99519	
ILLINOIS PROFESSIONAL DESIGN FIRM C/L P/E: BR COFF. 184/00069	PLOT DATE : 4/6/2015	DATE - 02/23/15	REVISED -		ILLINOIS FED. AID PROJECT							

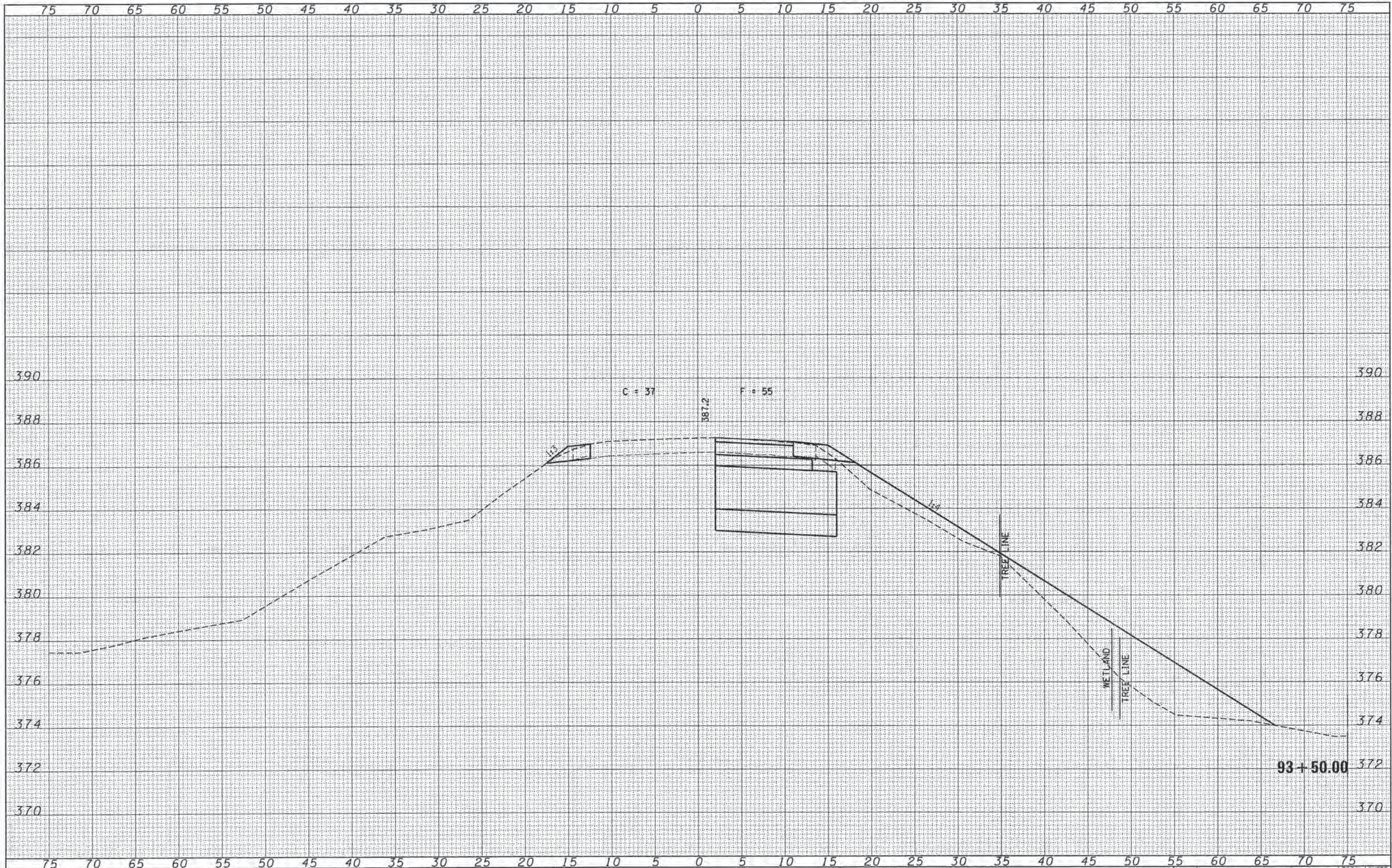




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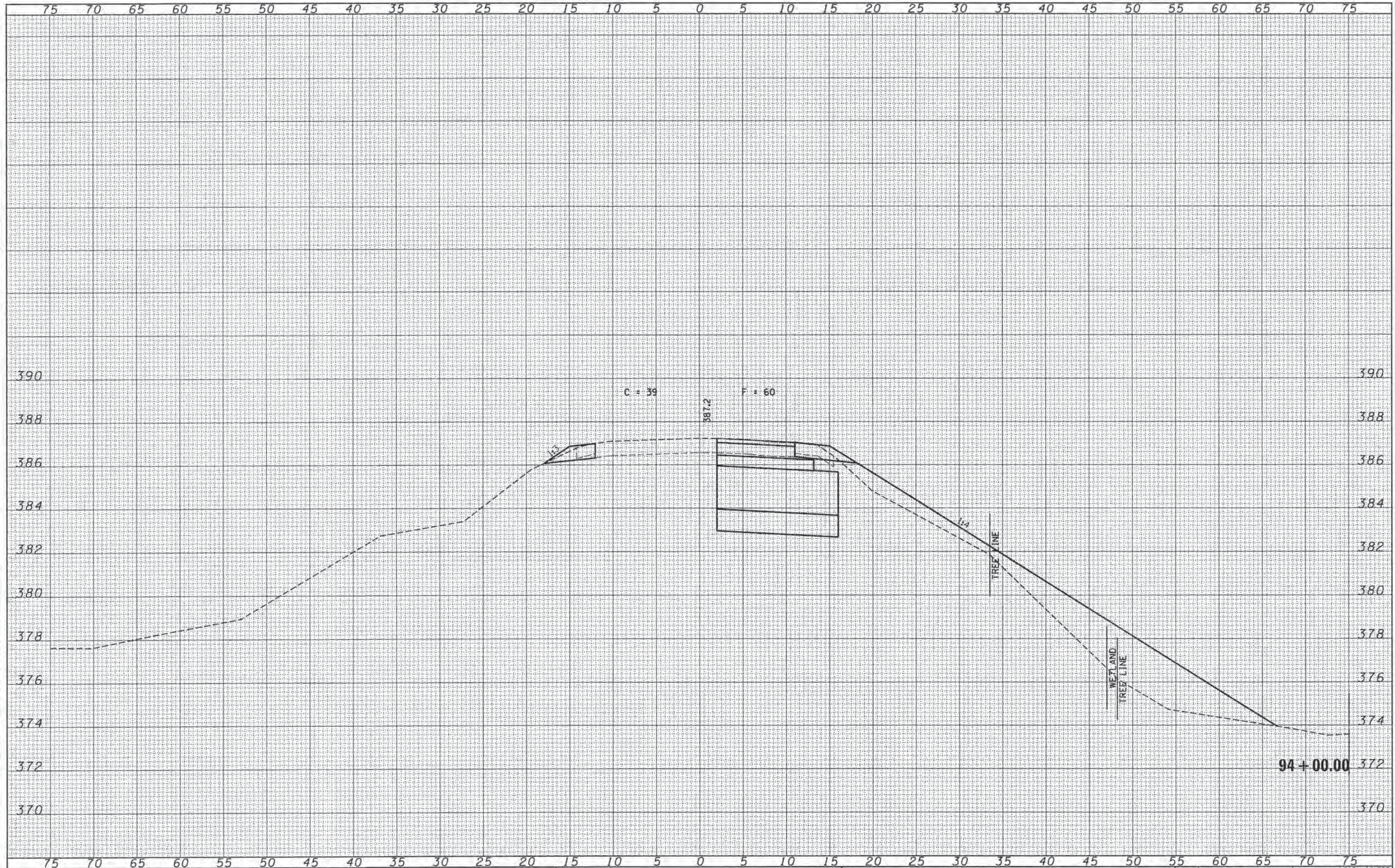
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HAMPTON, LENZINI AND RENWICK, INC. 3803 IREVERSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62779	PLLOT SCALE =	DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	73	CONTRACT NO. 99519	
ILLINOIS PROFESSIONAL DESIGN FIRM L.P. - P.E. / S.E. COMP. 19830009	PLLOT DATE = 4/6/2015	CHECKED - J.W.F.	REVISED -		SCALE: 5Hx2V	SHEET NO.	OF SHEETS	STA. 93+00.00	TO STA. 93+00.00	[ILLINOIS] FED. AID PROJECT	
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	74			
3800 STEVENSON DRIVE, SUITE 200 SPRINGFIELD, ILLINOIS 62767	PLLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V				SHEET NO. OF SHEETS	STA. 93+50.00 TO STA. 93+50.00	ILLINOIS FED. AID PROJECT	
ILLINOIS PROFESSIONAL ENGINEERING FIRM L3 / P.E. I.S.E. CORP. 18430893	PLLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		CONTRACT NO. 99519							



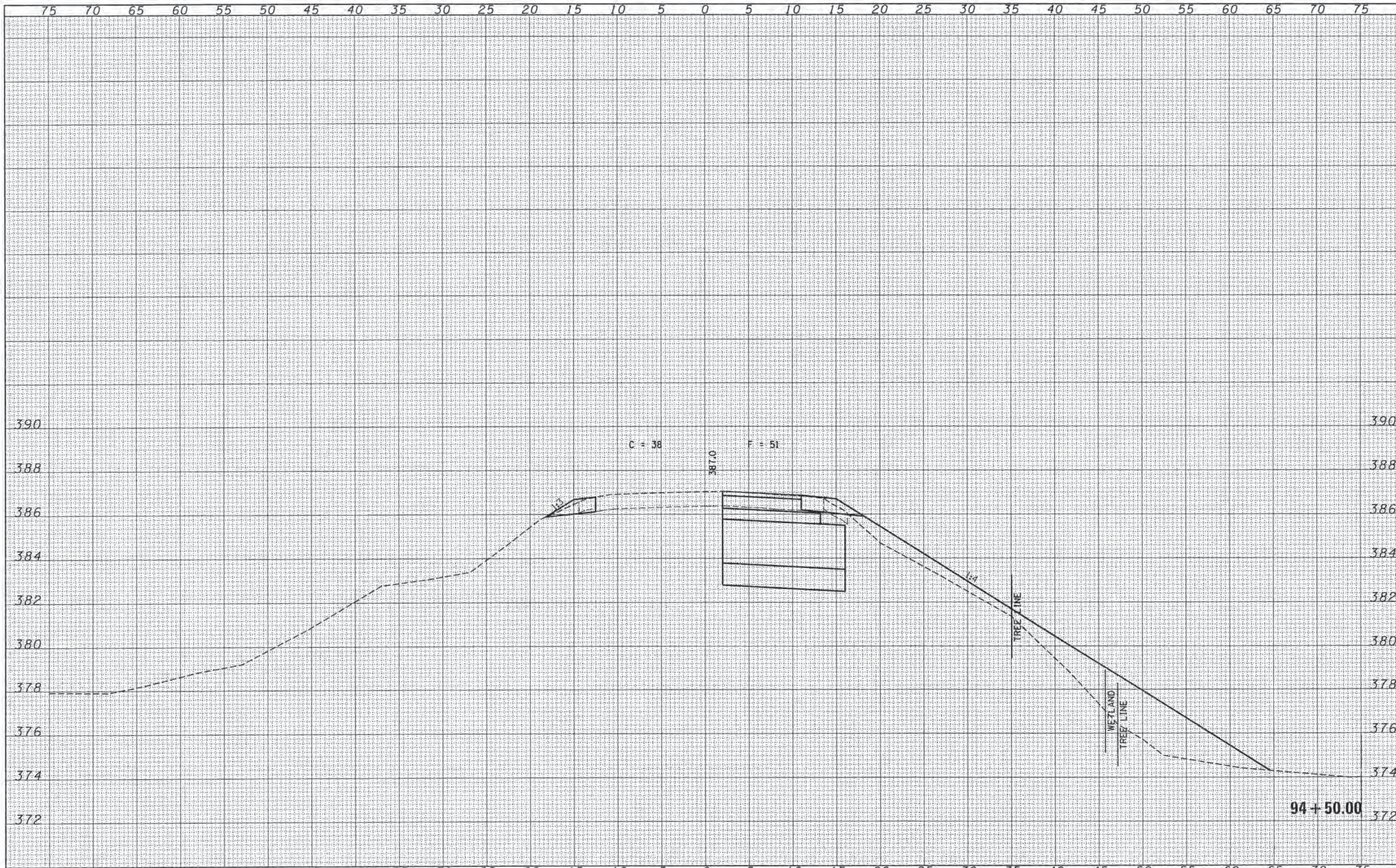
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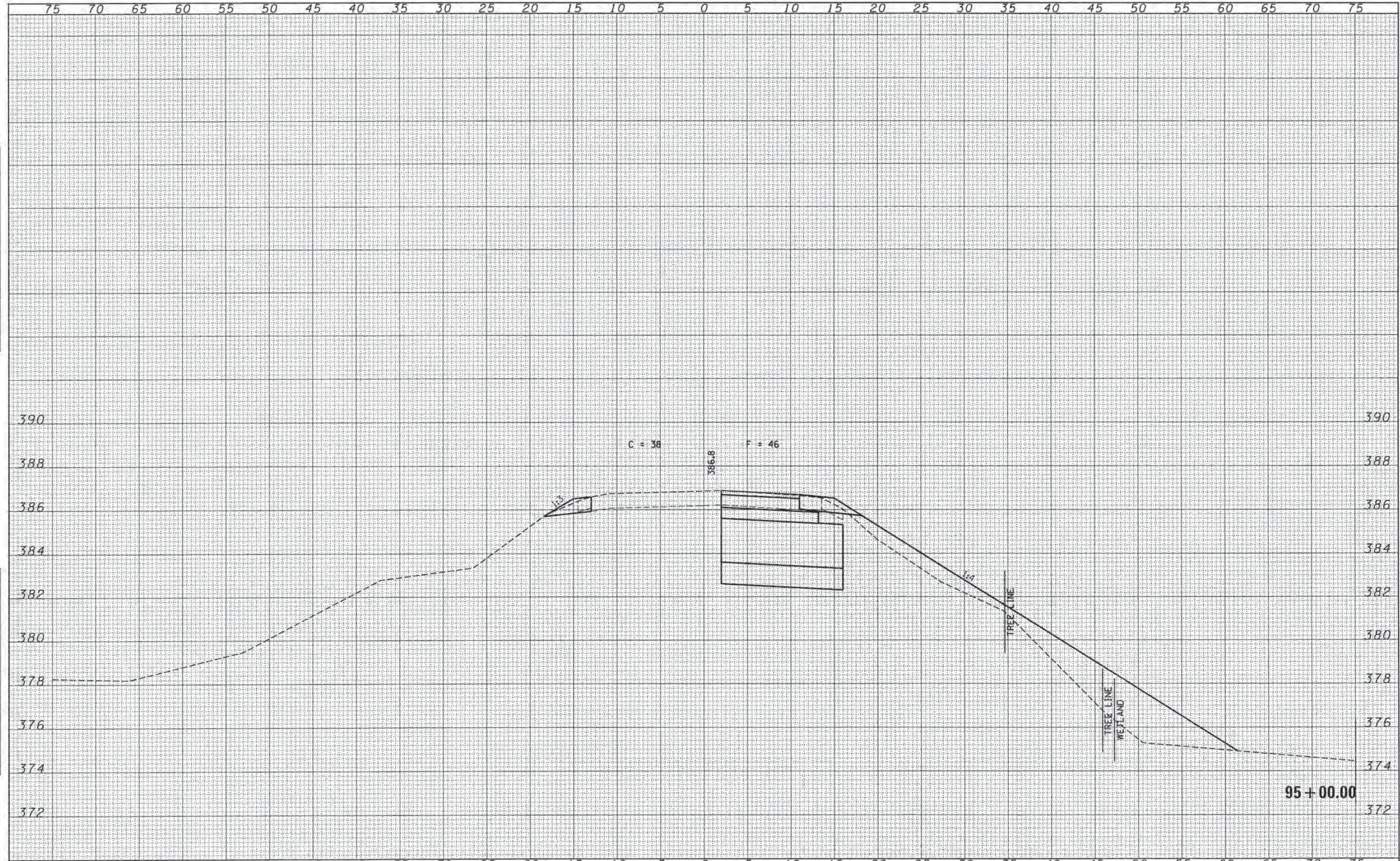
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	75			
2303 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62761	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V			SHEET NO.	OF SHEETS	STA. 94+00.00 TO STA. 94+00.00	CONTRACT NO. 99519	
ILLINOIS PROFESSIONAL DESIGN FIRM L.S./R.E./S.E. CORP. 184.00000	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		ILLINOIS FED. AID PROJECT							

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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	76
300 STEVENSON DRIVE, SUITE 201 VERMILION, ILL. 60440-1073	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519				
ILLINOIS PROFESSIONAL DESIGN FIRM ID / NO. - 36 CORP. 18-00899	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		ILLINOIS FED. AID PROJECT				



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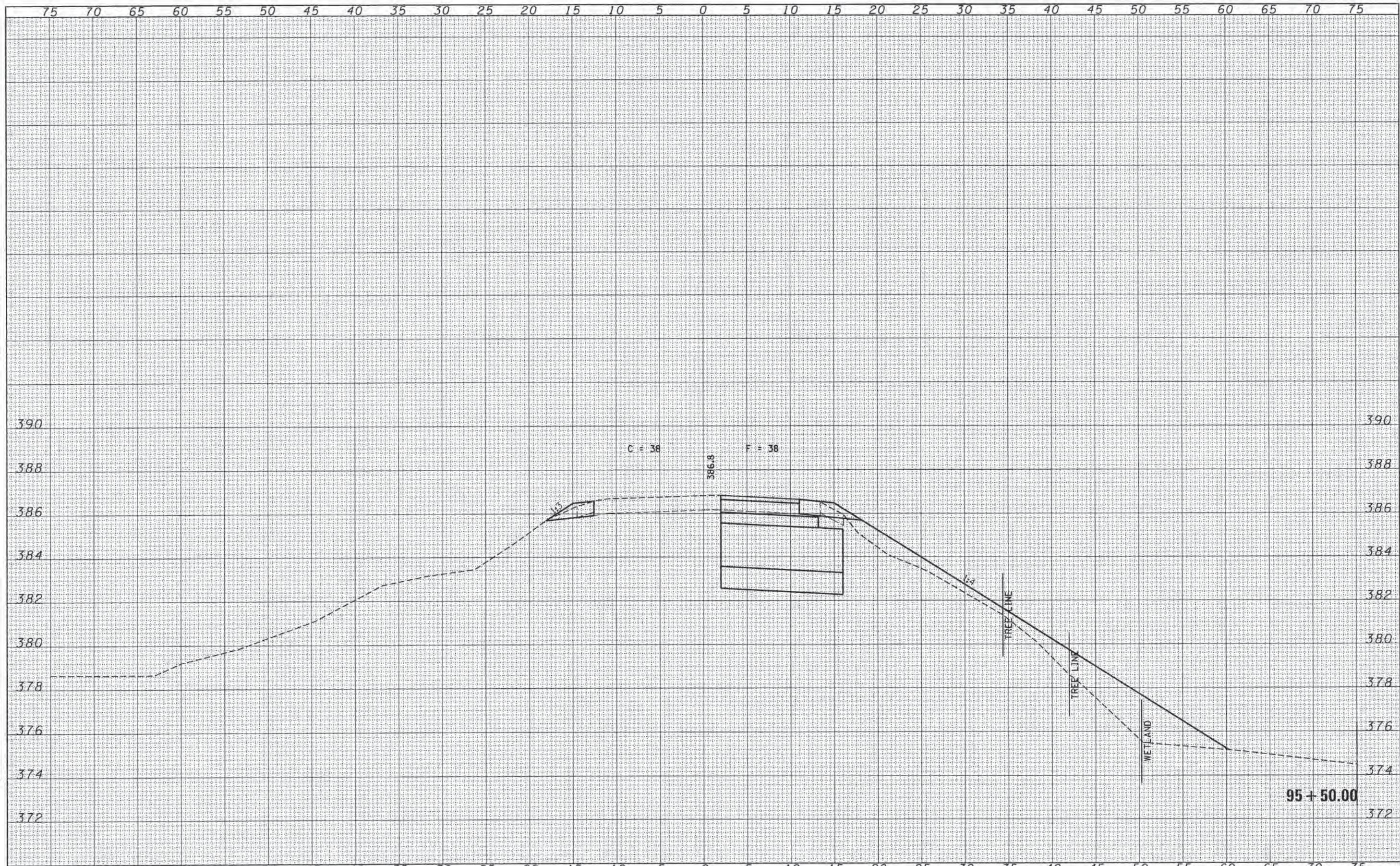
STATE OF ILLINOIS  
JACKSON COUNTY HIGHWAY DEPARTMENT

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F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
869	10-00163-00-BR	JACKSON	82	77
CONTRACT NO. 99519				
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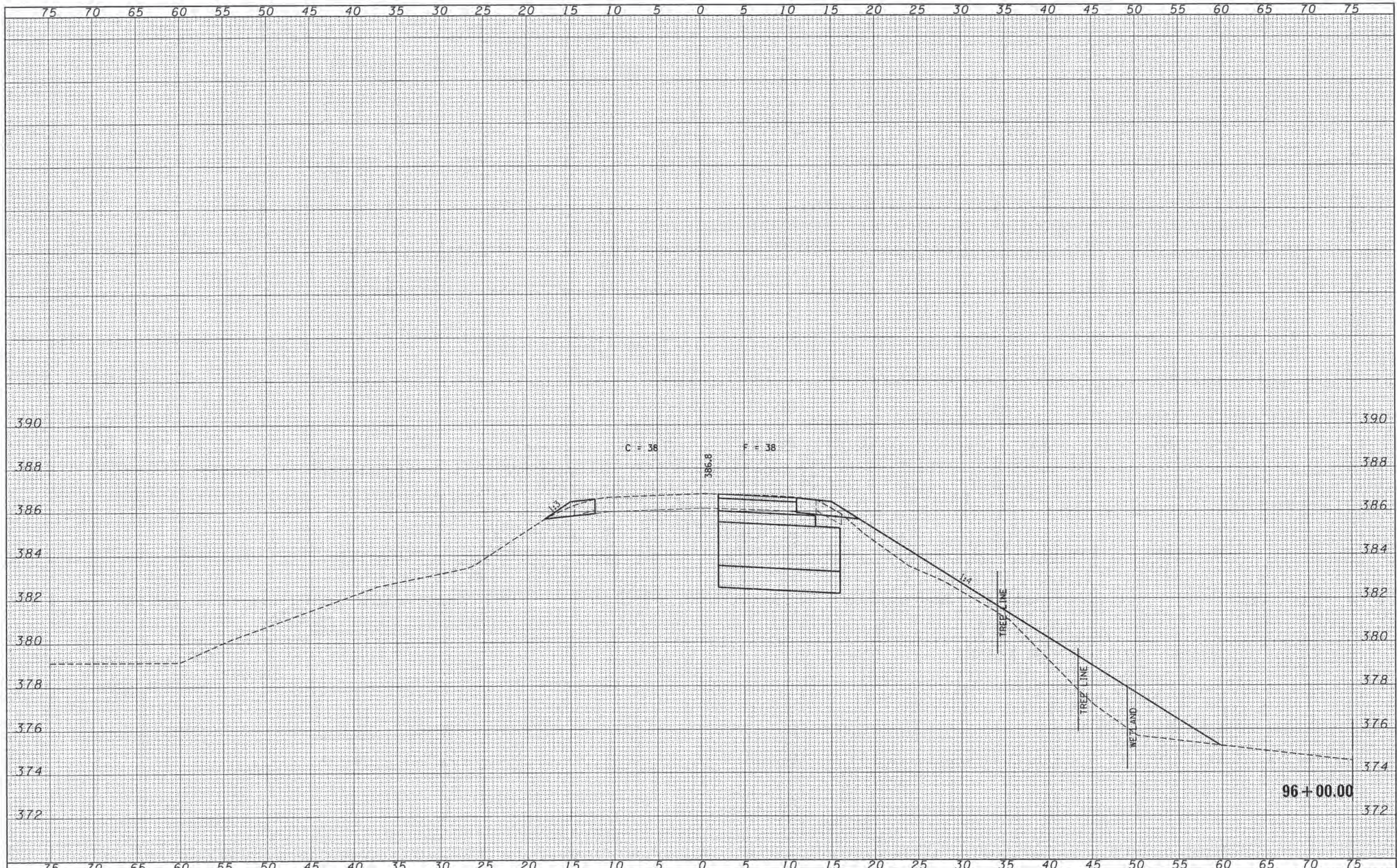
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	78		
305 STEVENSON DRIVE SUITE 201 SPRINGFIELD, ILLINOIS 62761	PLOT SCALE =	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 95+50.00 TO STA. 95+50.00	ILLINOIS FED. AID PROJECT			
ILLINOIS PROFESSIONAL DESIGN FIRM L3   P E   S E   C O R P .   194.000001	PLOT DATE = 4/6/2015	DATE - 02/23/15	REVISED -		CONTRACT NO. 99519						

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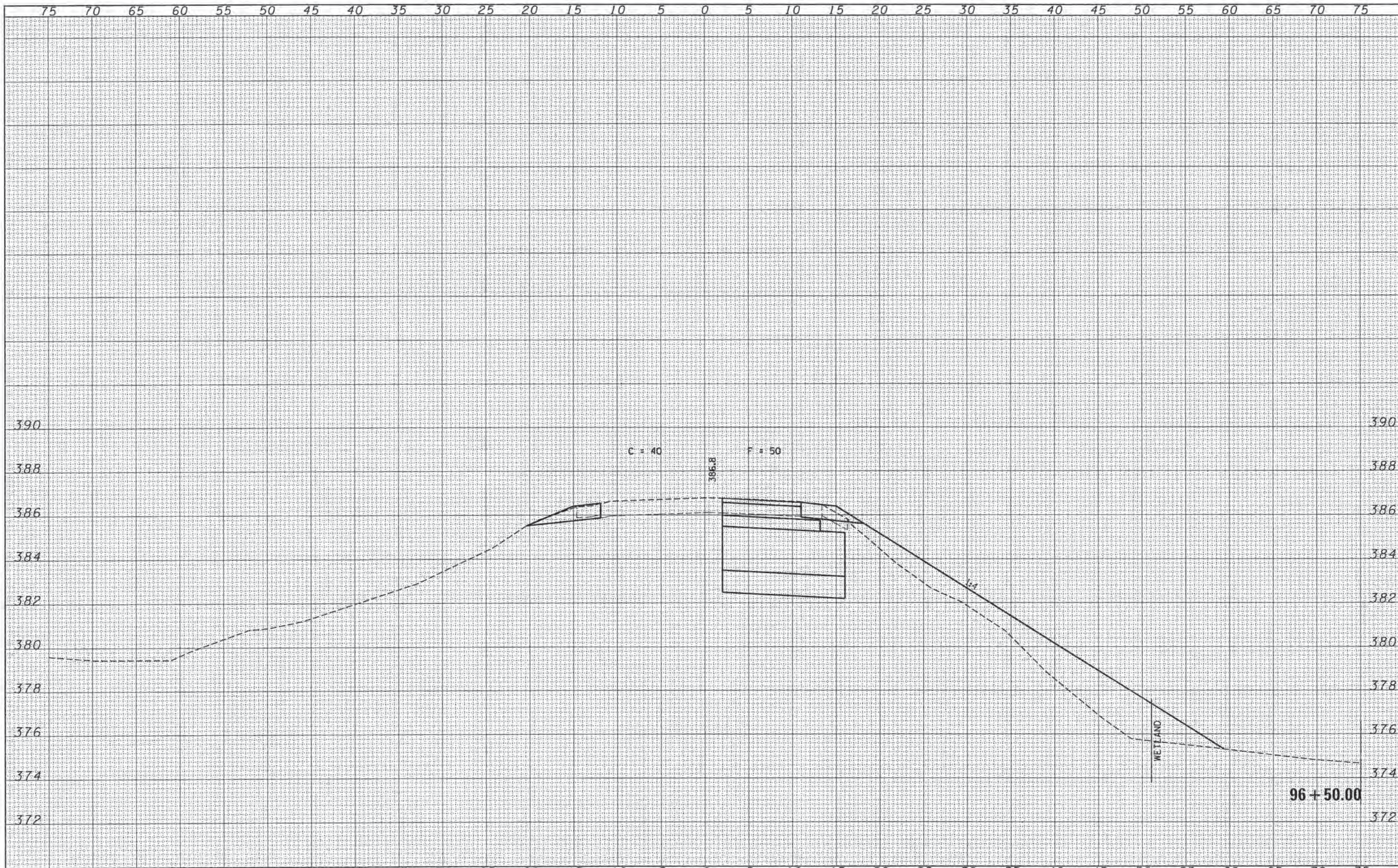
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HAMPTON, LENZINI AND RENWICK, INC. 2000 AFTERVIEW DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62702	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	79	CONTRACT NO. 99519	
ILLINOIS PROFESSIONAL ENGINEER #1194 15 / PE / DE CDMS 104 00000	PLOT DATE = 4/6/2015	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 96+00.00 TO STA. 96+00.00	ILLINOIS FED. AID PROJECT			
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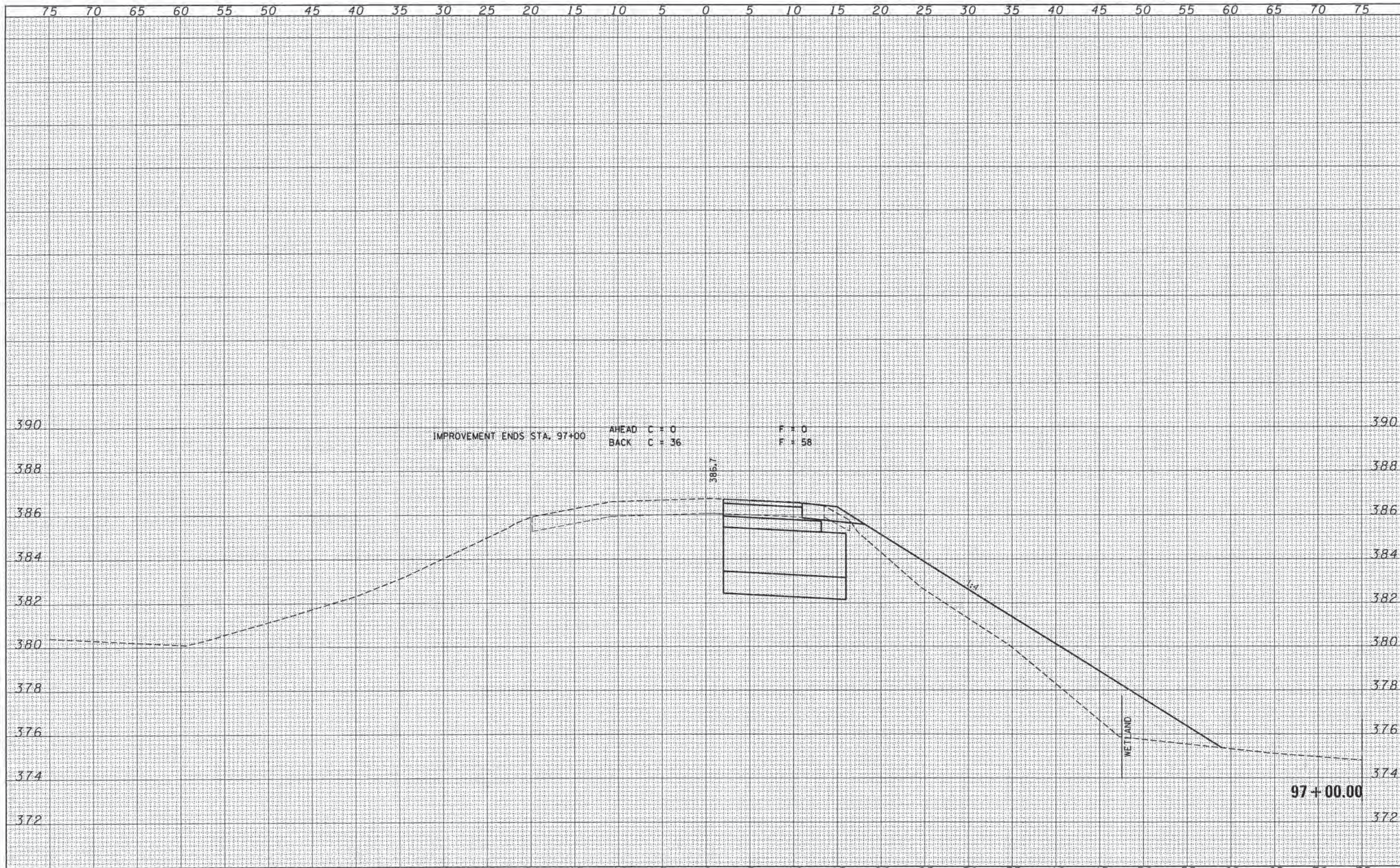


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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	B2	80			
300 STEVENSON DRIVE, SUITE 200 SPRINGFIELD, ILLINOIS 62761	PLLOT SCALE =	CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519							
ILLINOIS PROFESSIONAL SURVEYORS 151 W. PINE ST. CHICAGO, ILL. 60601	PLLOT DATE = 4/8/2015	DATE - 02/23/15	REVISED -		SCALE: 5H:2V	SHEET NO. OF SHEETS	STA. 96+50.00 TO STA. 96+50.00	ILLINOIS FED. AID PROJECT				

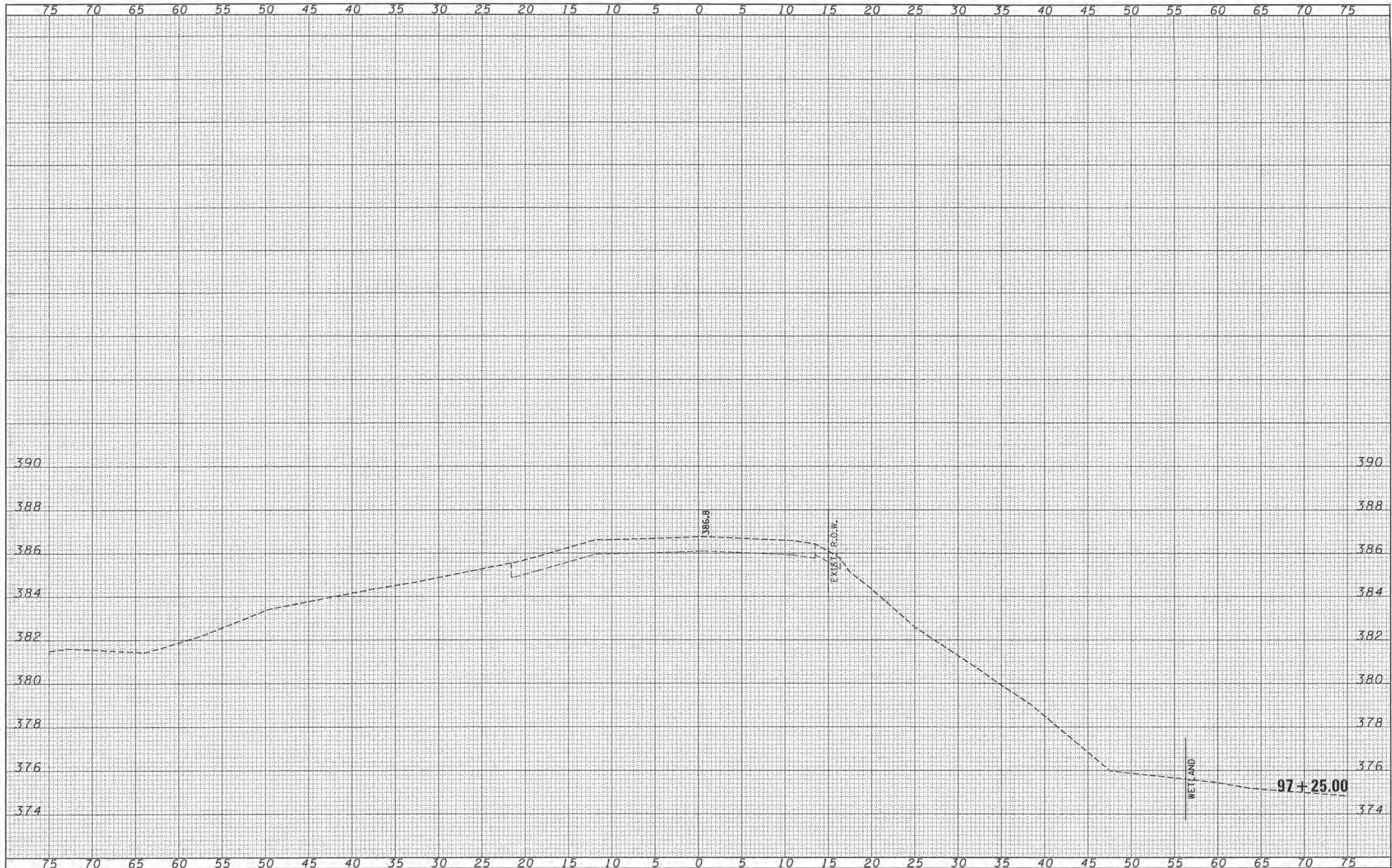


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HAMPTON, LENZINI AND RENWICK, INC. 298 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L3 / PE / SE CORP. 184-000843	PLOT SCALE =	DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	81	CONTRACT NO. 99519		
PLOT DATE = 4/6/2015	DATE - 02/23/15	CHECKED - J.W.F.	REVISED -		SCALE: 5H:2V	SHEET NO.	OF	SHEETS	STA. 97+00.00	TO STA. 97+00.00	ILLINOIS FED. AID PROJECT	
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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		869	10-00163-00-BR	JACKSON	82	82			
2301 STEVENSON DRIVE, SUITE 201 BETHLEHEM, ILLINOIS 61516		CHECKED - J.W.F.	REVISED -		CONTRACT NO. 99519							
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