

| | | |
|---|------------------------------|----------------------------|
| Original Report Date: 09/15/23 | Proposed SN: 096-2013 | Route: FAP Rte. 821 |
| Revised Date: | Existing SN: 096-2000 | Section: (19B2)B-1 |
| Geotechnical Engineer: BBS - Doris D. Gonzalez | | County: Wayne |
| Structural Engineer: Nephtali Rivera-Martinez | | Contract: 74648 |

Indicate the proposed structure type, substructure types, and foundation locations (attach plan and elevation drawing): The project is a structure replacement. The proposed structure is a triple-barrel, cast-in-place (CIP) box culvert. The current TSL shows horizontal cantilever wingwalls at all four ends, with extensions at the Northeast and Southwest wingwalls. The box has a skew of 30 degrees.

Discuss the existing boring data, existing plans foundation information, new subsurface exploration and need for any additional exploration to be provided with SGR Technical Memo (attach all data and subsurface profile plot): The existing structure is a CIP box culvert with horizontal cantilever and L-type wingwalls was constructed in 1957. It should be noted that the original structure was a bridge constructed at the same location in 1922, and parts of the original substructure may be encountered as it may not have been completely removed.

Two borings, designated Boring 1 and Boring 2, were advanced in May 2022. The soil profile consists of interbedded layers of silt, clay, silty loam, and clay loam over clay loam till ending on sandstone. The unconfined compressive strengths range from 0.1 to 1.7 tsf in the silts and loams, and 1.7 to 4.3 tsf in the clay till. Sandstone was encountered in Boring 2 at elevation 380.26 ft.

The existing plans show two borings, which will referred to as B1 (1956) and B2 (1956). The soils shown in these borings are consistent with the soil descriptions on the 2022 borings; however, sandstone was encountered 11 ft higher, at an approximate elevation of 391.3 ft.

Provide the location and maximum height of any new soil fill or magnitude of footing bearing pressure. Estimate the amount and time of the expected settlement. Indicate if further testing, analysis, and/or ground improvement/treatment is necessary: The grade is being maintained, and the proposed culvert's invert is approximately 2 ft below the existing invert. Additional fill may be added behind the new wingwalls; however, the embankment does not appear to be widened significantly. Since the proposed culvert will occupy the footprint of the existing culvert, and the roadway centerline elevation will be increased by less than 6 inches, settlement is not expected to be significant and was not calculated.

Identify any new cuts or fill slope angles and heights. Estimate the factor of safety against slope failure. Indicate if further testing, analysis or ground improvement/treatment is necessary: As mentioned above, the grade is being maintained, also the proposed embankment will have a 2H:1V slope, as the existing. There here was no indication of slope stability issues or movement of the wingwalls in the Bridge Condition Report (BCR) dated January 14, 2021; therefore, slope stability is not a concern and it's expected to have a satisfactory factor of safety.

Indicate at each substructure, the 100-year and 200-year total scour depths in the Hydraulics report, the non-granular scour depth reduction, the proposed ground surface, and the recommended foundation design scour elevations: Since the proposed structure is a box culvert, design scour elevations are not required.

Determining the seismic soil site class, the seismic performance zone, the 0.2 and 1.0 second design spectral accelerations and indicate if that the soils are liquefiable: Since buried structures are not designed for seismic effects, seismic data is not required.

Confirm feasibility of the proposed foundation or wall type and provide design parameters. Attach a pile design table indicating feasible pile types, various nominal required bearings, factored resistances available and corresponding estimated lengths at locations where piles will be used. Provide factored bearing resistance and unit sliding resistance at various elevations and confirm no ground improvement/treatment is necessary where spread footings are proposed. Estimated top of rock elevations as well as preliminary factored unit side and tip resistance values shall be indicated when drilled shafts are proposed: The construction of a precast or cast-in-place concrete box culvert is geotechnically feasible; however, per the Structure Report dated August 30, 2022, a cast-in-place is preferred.

Horizontal cantilever wingwalls are feasible, but are limited to a maximum length of 16 ft. The FGU recommends utilizing horizontal cantilever wingwalls at the NW and SE ends (short wingwalls).

For the NE and SW ends (long wingwalls), the FGU recommends utilizing two-way L-type cantilever wingwalls. Horizontal cantilever wingwalls with extensions are also feasible. Permanent sheet piling extensions were evaluated, but due to the nature of the soil profile and limited embedment due to sandstone being encountered, these would need to be anchored. Soldier pile walls could also be utilized as extensions, but the piles would have to be drilled into rock. In conclusion, based on the encountered soils as well as the proximity to rock, we recommend utilizing two-way L-type cantilever wingwalls at these locations.

Calculate the estimated water surface elevation and determine the need for cofferdams (type 1 or 2), and seal coat: An Estimated Water Surface Elevation equal to 403.55 was provided by the Bridge Planning Unit. The soils under the proposed culvert are mainly composed of silty loams. Since the EWSE is less than 4 ft above the bottom of the box culvert barrel, temporary dewatering of the excavations by stream diversion will be required.

Assess the need for sheeting or soil retention or temporary construction slope and provide recommendation for other construction concerns: The proposed structure will be constructed utilizing stage construction, so temporary earth retention will be required. Based on the soil types, retained height, proximity to rock as well as the potential for encountering obstructions, the Temporary Soil Retention System pay item should be utilized.

Existing Structure: S.N. 096-2000 was originally constructed in 1957 under S.B.I. Route 15, Section 19-BY-1 and 19B-2 at station 296+84. The structure consists of a cast-in-place, triple barrel box culvert, with one barrel (center) at 12'-3" x 7'-4" and two barrels at 9'-8" x 7'-4". The structure has a 25°00'00" left ahead skew, a 42'-0" curb-to-curb bridge clear width, with two 11' striped travel lanes.

No salvage.

D.H.W. Elev. 409.54 ▼

Stone Riprap
Class A5, typ.

Downstream
Elev. 401.60

L-Type Two-Way Cantilever Wingwall, typ.
(At N.E. & S.W. corners)

The diagram illustrates the cross-section of a bridge deck with the following details:

- Overall Dimensions:** Total width is 50'-0" Out-to-Out Headwalls. The deck is divided into two 25'-0" sections.
- Lane and Shoulder Widths:** Each side has an 11'-0" Lane and an 8'-0" Shoulder. Total shoulder width is 16'-0".
- Clearance and Slopes:** The centerline (CL) Rdwy. is 25'-0" from the headwall. Slopes are 4.0% on the shoulders and 1.5% on the lanes. A 0.5% slope is indicated for the Stage II Construction area.
- Invert Elevations:**
 - Downstream Invert Elev. 400.60
 - Upstream Invert Elev. 400.92
 - Upstream F.L. Elev. 401.92
- Construction Stages:**
 - Stage II Construction:** The area from the headwall to the Stage II Removal line.
 - Stage I Construction:** The area from the Stage II Removal line to the Stage I Removal line.
 - Stage II Removal:** The area from the headwall to the Stage II Removal line.
 - Stage I Removal:** The area from the Stage II Removal line to the Stage I Removal line.
- Other Features:**
 - PG (Proposed Grade) line is shown at 1'-1" from the centerline.
 - Stage Construction Line is shown at 1'-2" from the centerline.
 - Typical dimensions of 9" and 8" are noted for specific components.

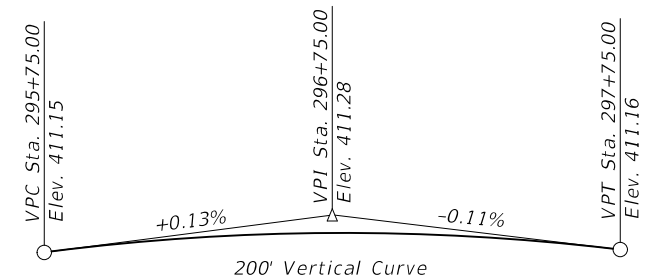
LONGITUDINAL SECTION

(All horizontal perpendicular to C Rdwy - Looking West.)

Horizontal Cantilever Wingwall, typ.
(At N.W. & S.E. corners)

Note: Existing 1922 abutments portions to be removed, if required.

**Subject to refinement during final design*



PROFILE GRADE

(along ζ IL 15)

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications,
9th Edition

LOADING HL-93

Allow 50#/sq.ft. for Future Wearing Surface

DESIGN STRESSES

FIELD UNITS

$$f'_c = 3,500 \text{ psi}$$
 $f_y = 60,000 \text{ psi (Reinforcement)}$

HIGHWAY CLASSIFICATION

F.A.P. Rte. 821 (IL Rte. 15)

Functional Class: Other Principal Arterial

ADT: 3400 (2025); 3900 (2045)

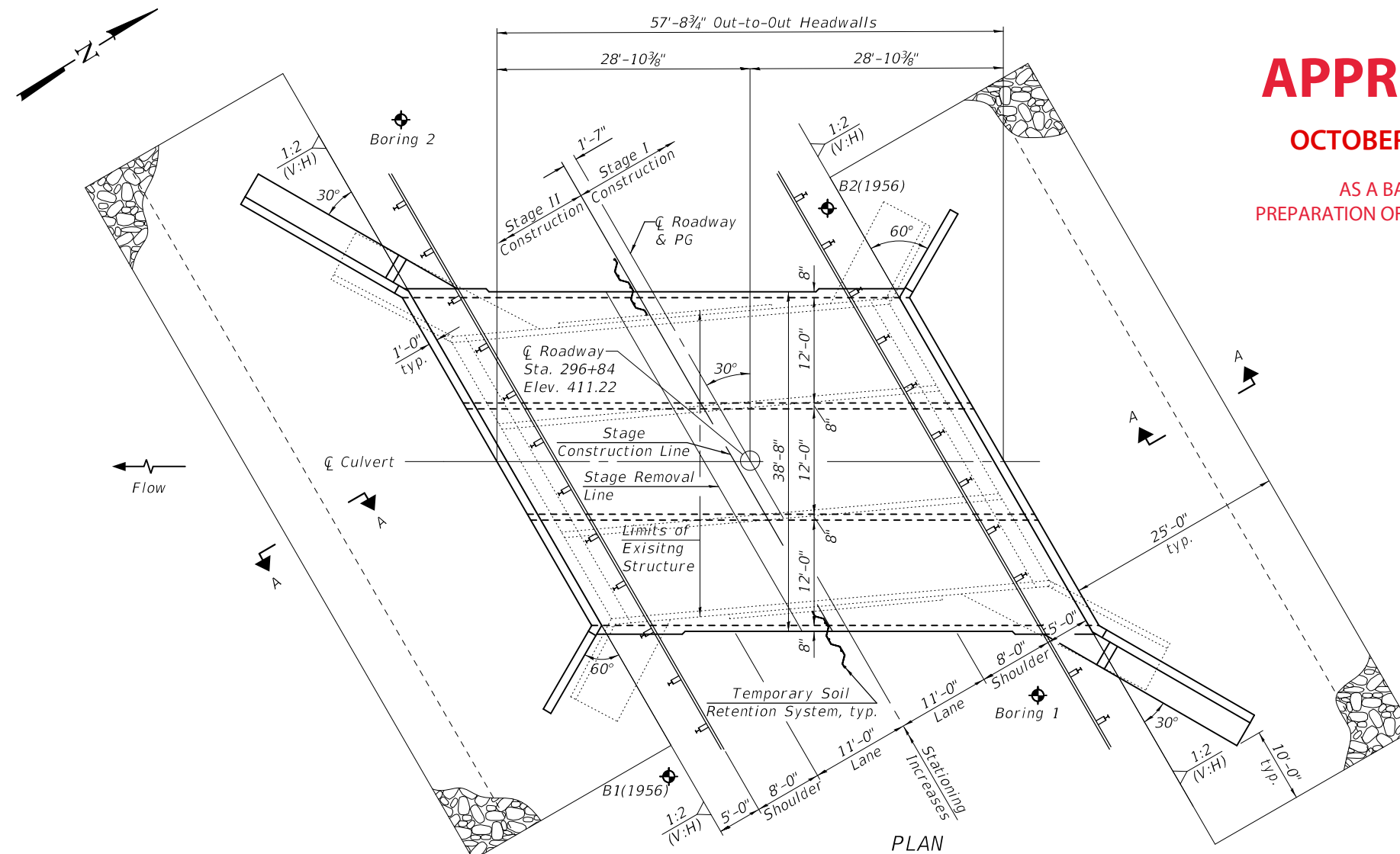
ADTT: 299 (2025); 343 (2045)

DHV: 413 (2045)

Design Speed: 60 m.p.h.

Posted Speed: 55 m.p.h.

Two-Way Traffic; Directional Distribution: 51:49



Range 7E, 3rd P.M.

Proposed Structure

Twp. 25

Boyleston

25

LOCATION SKETCH

GENERAL PLAN & ELEVATION
IL. 15 OVER UNNAMED CREEK
F.A.P. RTE. 821 - SEC. (19B2)B-1
WAYNE COUNTY
STATION 296+84.00
STRUCTURE NO. 096-2013

| | |
|------------|--------------------------|
| DESIGNED - | NEPHTALI RIVERA-MARTINEZ |
| CHECKED - | MICHAEL A. PAULIONIS |
| DRAWN - | ALAN JOHNSTONE |
| CHECKED - | N.R.M. / M.A.P. / P.D.C. |

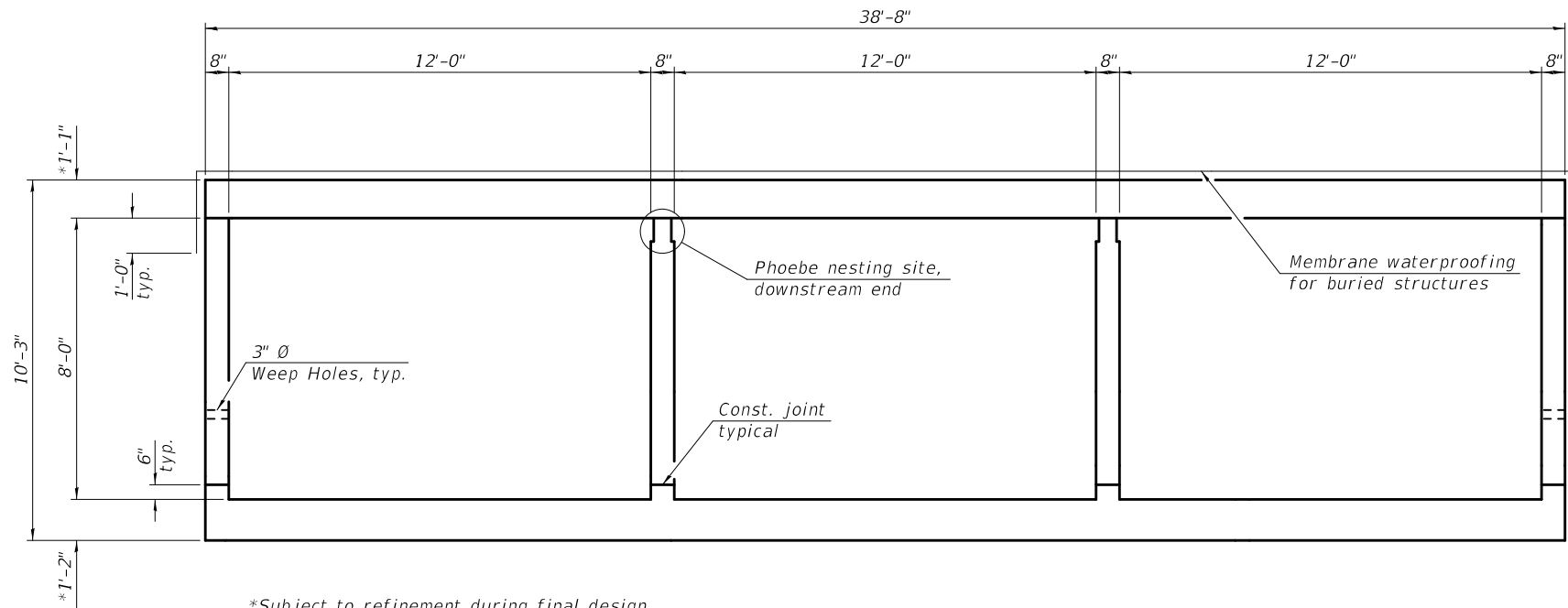
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

| | | | | |
|--------------------|-----------|------------------|-----------------|--------------|
| F.A.P. RTE. | SECTION | COUNTY | TOTAL SHEETS | SHEET NO. |
| 821 | (19B2)E-1 | WAYNE | 2 | 1 |
| CONTRACT NO. 74648 | | | | |
| ILLINOIS | | FED. AID PROJECT | | |

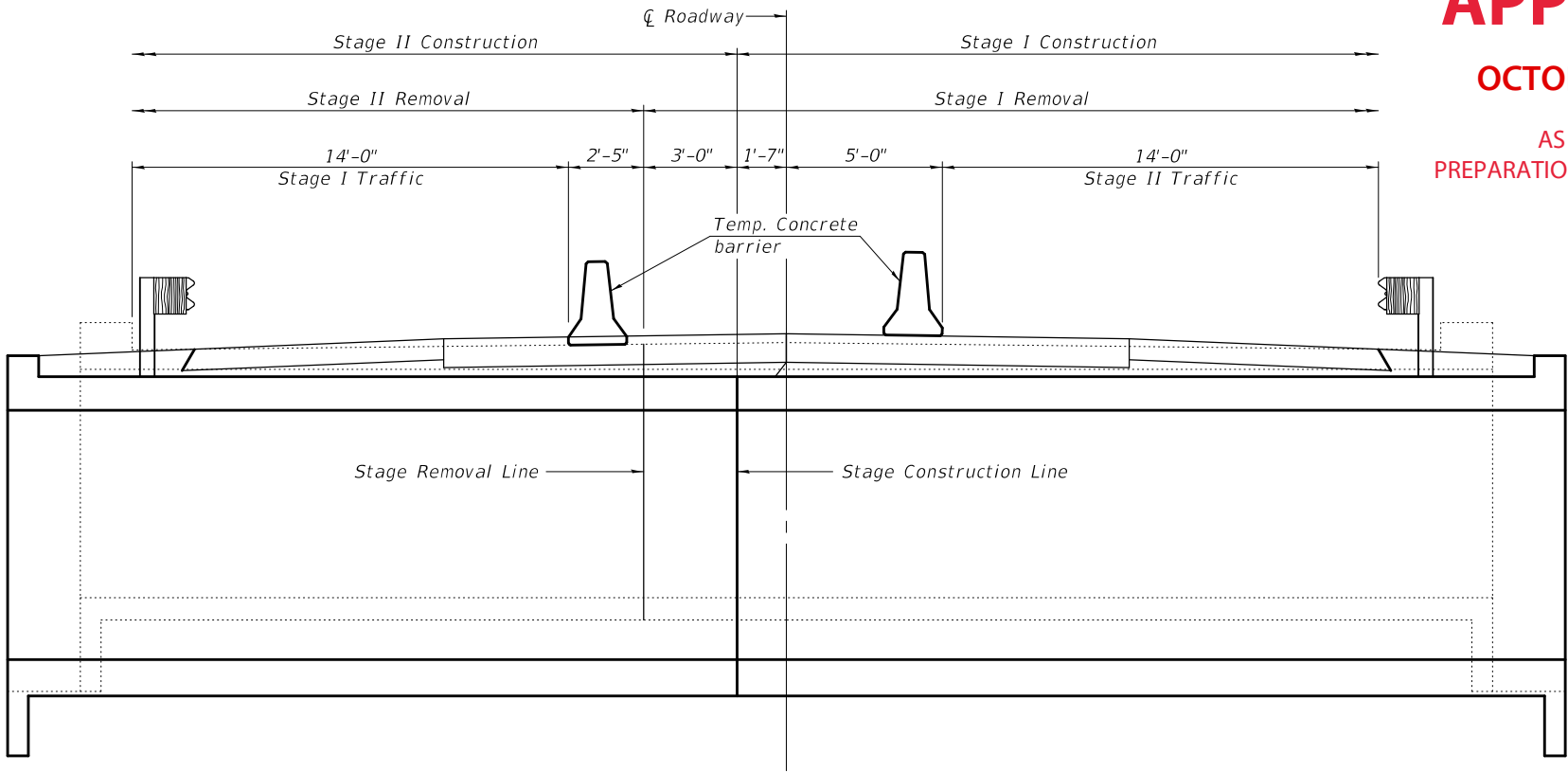
SHEET 1 OF 2 SHEETS

MODEL: 0962013-74648-TSL-002
FILE NAME: pw:\idot-pw\entley.com\FW\DOT\Documents\IDOT Offices\Bureau of Bridges and Structures\Projects\0962013\CADD Plans\0962013-74648.dgn



*Subject to refinement during final design.

SECTION THRU BAREL

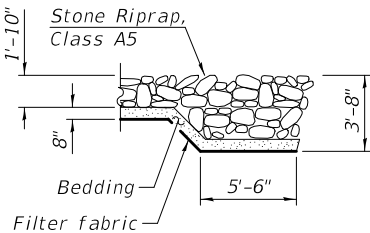


LONGITUDINAL SECTION - STAGE CONSTRUCTION
(Horizontal dimensions are perpendicular to ϕ roadway-Looking West)

APPROVED

OCTOBER 30, 2023

AS A BASIS FOR
PREPARATION OF DETAILED PLANS



SECTION A-A

DETAILS
IL. 15 OVER UNNAMED CREEK
F.A.P. RTE. 821 - SEC. (19B2)B-1
WAYNE COUNTY
STATION 296+84.00
STRUCTURE NO. 096-2013

| | | |
|----------|---|--------------------------|
| DESIGNED | - | NEPHTALI RIVERA-MARTINEZ |
| CHECKED | - | MICHAEL A. PAULIONIS |
| DRAWN | - | ALAN JOHNSTONE |
| CHECKED | - | N.R.M. / M.A.P. / P.D.C. |

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SHEET 2 OF 2 SHEETS

| F.A.P. RTE. | SECTION | COUNTY | TOTAL SHEETS | SHEET NO. |
|---------------------------|-----------|--------|--------------|-----------|
| 821 | (19B2)B-1 | WAYNE | 2 | 2 |
| CONTRACT NO. 74648 | | | | |
| ILLINOIS FED. AID PROJECT | | | | |



Illinois Department of Transportation

Division of Highways
IDOT D7

SOIL BORING LOG

Page 1 of 1

Date 5/9/22

ROUTE FAP 821 (IL 15) DESCRIPTION IL 15 over Unnamed Stream LOGGED BY Sandschafer

SECTION (19B2)B-1 LOCATION Northeast Corner, SEC. 7, TWP. 2E, RNG. 7S, 3rd PM,
Latitude N 38.370146, Longitude W 88.465384

COUNTY Wayne DRILLING METHOD Hollow stem auger & split spoon HAMMER Auto ETR = 91.8% @ 57.4

STRUCT. NO. 096-2000 (E)
096-2013 (P)
Station 296+84

BORING NO. 1 East Abutment
Station 296+45
Offset 15.0 ft RT
Ground Surface Elev. 410.95 ft

| DEPTH TH (ft) | BLOW S (/6") | UCS Qu (tsf) | MOIST T (%) | Surface Water Elev. <u>402.90</u> ft | Stream Bed Elev. <u>402.60</u> ft | Groundwater Elev.: | DEPTH TH (ft) | BLOW S (/6") | UCS Qu (tsf) | MOIST T (%) |
|--|--------------------|--------------------|-------------------|--|-----------------------------------|---------------------------------------|---------------------|--------------------|--------------------|-------------------|
| | | | | | | First Encounter <u>394.0</u> ft | | | | |
| | | | | | | Upon Completion <u>403.0</u> Caved ft | | | | |
| | | | | | | After 24 Hrs. <u>404.5</u> ft | | | | |
| Aggregate Shoulder Mixed with Clay <u>409.95</u> | | | | Stiff, moist, brown, CLAY LOAM | | | 3 | 1.2 | 18 | |
| Brown, CLAY | | | | | | | 3 | B | | |
| | 2 | | | | | 388.95 | | | | |
| Stiff, moist | 2 | 1.5 | 18 | Brown, fine-grained, SAND | 388.45 | | 2 | | | |
| | 1 | P | | Hard, moist, grey, CLAY LOAM Till | | | 5 | 4.3 | 13 | |
| | | | | | | | 10 | B | | |
| <u>406.45</u> | | | | | | | | | | |
| Very soft, wet, brown, SILT | -5 | 1 | | Stiff | | | -25 | 3 | | |
| | 1 | 0.2 | 32 | | | | | 3 | 1.7 | 18 |
| | 1 | B | | | | | | 6 | B | |
| | | | | | | | | | | |
| Grey | 1 | | | Hard | | | | 2 | | |
| | 1 | 0.1 | 27 | | | | | 4 | 4.1 | 15 |
| | 1 | B | | | | | | 8 | B | |
| <u>401.45</u> | | | | | | | | | | |
| Stiff, moist, grey, SILTY LOAM | -10 | 1 | | Very stiff | | | -30 | 2 | | |
| | 3 | 1.0 | 20 | | | | | 5 | 2.1 | 15 |
| | 3 | B | | | | | | 7 | B | |
| | | | | | | 379.95 | | | | |
| Medium, grey and brown marbled | 1 | | | Benchmark: BM 475 - Cut Square in Southwest Corner of Bridge | | | | | | |
| | 6 | 0.4 | 19 | Curb SN 096-2000, Sta. 297+10, | | | | | | |
| | 3 | B | | 21 feet LT; Elevation = 411.563 feet. | | | | | | |
| | | | | End of Boring | | | | | | |
| <u>395.95</u> | -15 | 1 | | | | | -35 | | | |
| Stiff, moist, grey, CLAY | | 2 | 1.2 | | | | | | | |
| | | 3 | B | | | | | | | |
| <u>393.95</u> | | | | | | | | | | |
| Soft, moist, grey, SILTY LOAM | | 2 | | | | | | | | |
| | | 2 | 0.3 | | | | | | | |
| | | 2 | B | | | | | | | |
| <u>391.45</u> | | | | | | | | | | |
| Stiff, moist, brown, CLAY LOAM <u>390.95</u> | -20 | 1 | | | | | -40 | | | |

SOIL BORING 096-2000(E), -2013(P) SOIL 2022.GPJ IL_DOT.GDT 5/25/22

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), WH-Weight of Hammer, NT-Not Tested.

Division of Highways
IDOT D7

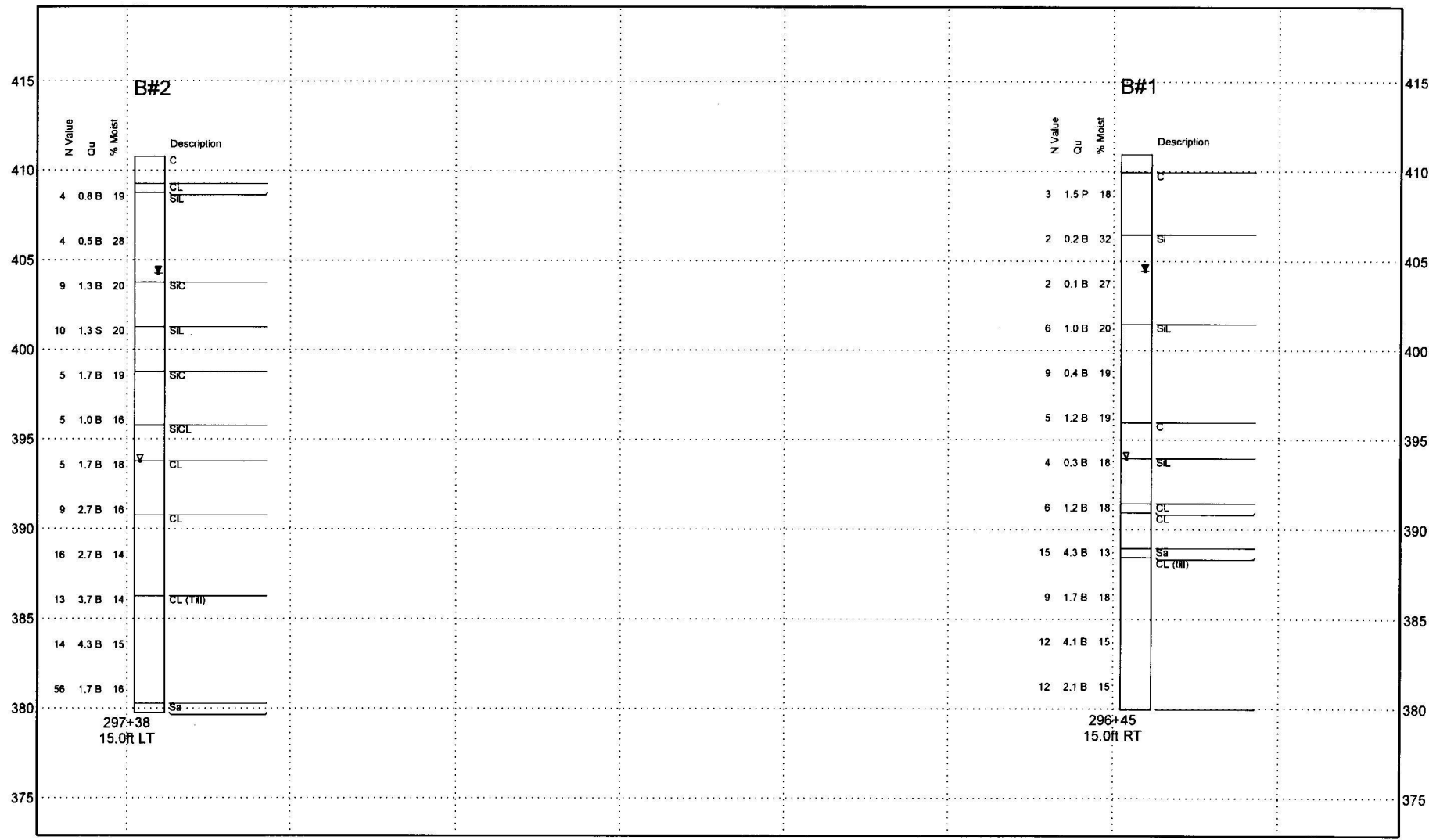
Page 1 of 1

Date 5/9/22

| | | | | |
|--|--------|---------|----------|----|
| Very stiff, moist, brown, CLAY LOAM | | 4 5 | 2.7 B | 16 |
| | | | | |
| | | 2 | | |
| | | 6 10 | 2.7 B | 14 |
| | | | | |
| | 386.26 | | | |
| Very stiff, moist, grey, CLAY LOAM Till | -25 | 2 | | |
| | | 6 7 | 3.7 B | 14 |
| | | | | |
| | | 2 | | |
| <i>Hard</i> | | 5 9 | 4.3 B | 15 |
| | | | | |
| | -30 | 2 | | |
| | 380.26 | 40 | 1.7 | 16 |
| Soft, brown, SANDSTONE | 379.76 | 16 | B | |
| Benchmark: BM 475 - Cut Square in Southwest Corner of Bridge Curb SN 096-2000, Sta. 297+10, 21 feet LT; Elevation = 411.563 feet. End of Boring | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | -35 | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | -40 | | | |

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated). The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), WH-Weight of Hammer, NT-Not Tested.

Structure Number 096-2000 (E) 096-2013 (P) IL 15 over Unnamed Stream
Located in the Northeast Corner of Section 7, Township 2E, Range 7S of the 3 P.M.



NOT TO HORIZONTAL SCALE

VARIATIONS IN SUBSURFACE
CONDITIONS MAY EXIST
BETWEEN BORINGS

Groundwater
First Encounter
Completion
after (refer to log) hours

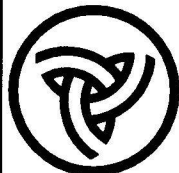
Abbreviations
WH - Sampler Advanced by Weight
of Hammer, WOP - Weight of Pipe
B.S. - Before Sealing
NT - Not Tested

SUBSURFACE DATA PROFILE

Route: FAP 821 (IL 15)

Section: (19B2)B-1

County: Wayne

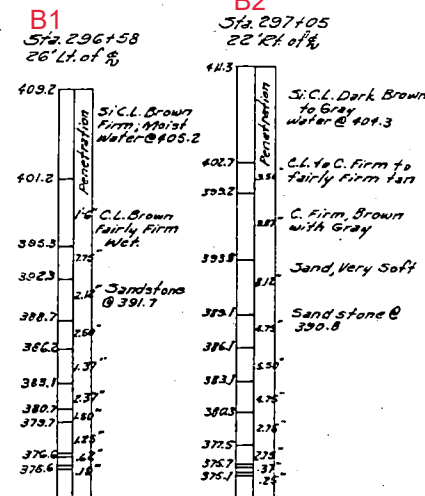
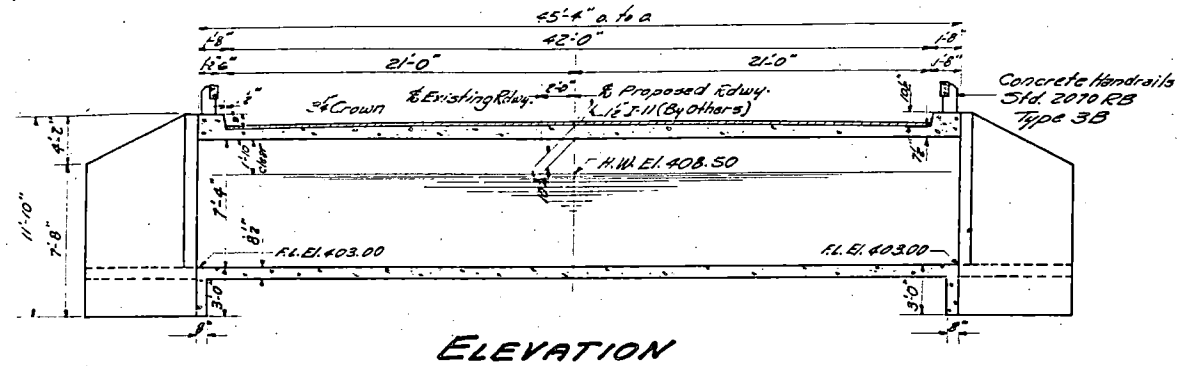


Illinois Department
of Transportation

Division of Highways
IDOT D7

STATE OF ILLINOIS
DEPARTMENT OF PUBLIC WORKS & BUILDINGS
DIVISION OF HIGHWAYS

3M. Chiseled on S.W. Wingwall of Existing Bridge
Sta. 296+76.5 Elev. 410.01 (USGS Datum)
Existing Structures: R.C. Thru Girder, 1 Span @ 35' clear,
20' Roadway, Built 1922, R.C. Abutments.
Removal: Bridge Contractor to remove existing Superstructure
and necessary portion of existing Abutments at time
of construction.
Salvage: None.

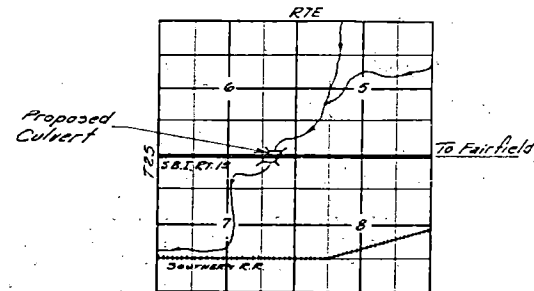


All penetrations with 190*
hammer dropping 19.5" for 10 blows.

BORING DATA

WATERWAY INFORMATION

Drainage Area ----- 1400 Acres
Character ----- Level Rolling Wooded & Cultivated
Required Opening ----- 173 Sq. Ft. (30 yr. flood)
Present Opening ----- 95 Sq. Ft.
Proposed Opening ----- 173 Sq. Ft.



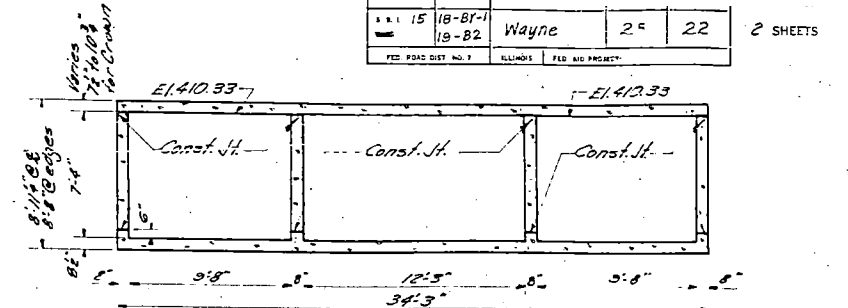
STRESSES

$f_c = 800$ psi (Wings & Footings)
 $f_c = 1400$ psi (Barrel)
 $f_s = 20,000$ psi (Reinf.)
 $n = 10$

STATION 296+84
BUILT 195 BY
STATE OF ILLINOIS
S.B.I. Rt. 15, Sec. 19B-2
P.A. PROJECT P-16(7)
LOADING H20-S16

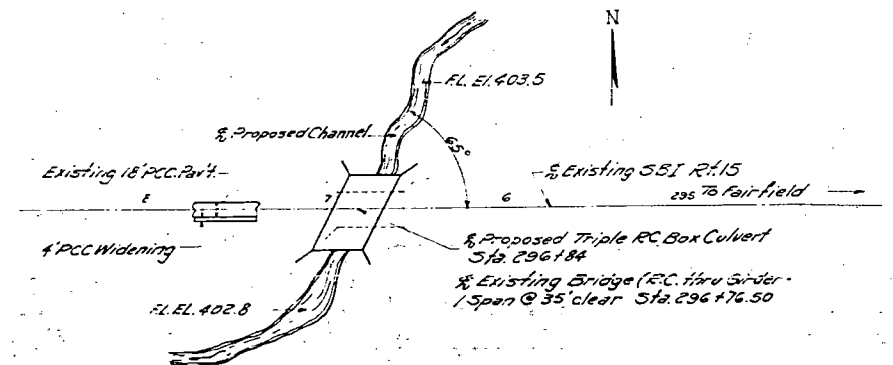
LETTERING FOR NAME R.

See Std 3113



GENERAL NOTES

Class A Concrete shall be used throughout except in handrails.
Handrail Concrete shall be used in handrails.
The handrail concrete in the rail posts and railing shall be
poured in separate operations.
Bar lap is 20 diameters minimum unless otherwise shown.
The following surface shall be waterproofed, the backs
of wingwalls from top of footing to top of wing; the top &
sides of culvert to lower construction joint. Waterproofing
shall be done in accordance with Article 51.20 of the Standard
Specifications.
Exposed edges shall be beveled.
For back filling and embankment, see Standard Specification.



LOCATION SKETCH

TOTAL BILL OF MATERIAL

| Item | Unit | Total |
|--------------------------------|----------|--------|
| Class A Concrete | Cu. Yds. | 165.0 |
| Handrail Concrete | Cu. Yds. | 1.9 |
| Reinforcement Bars | Lbs. | 30,400 |
| Name Plates | Each | One |
| Removal of Existing Structures | Each | One |

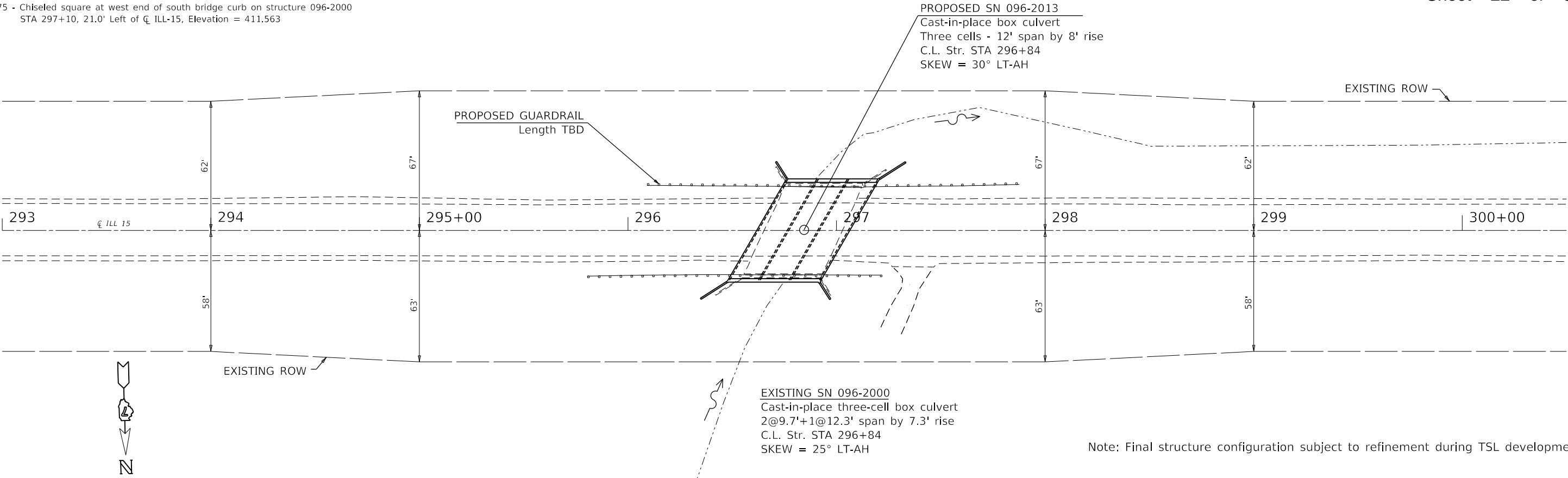
GENERAL PLAN & ELEVATION
S.B.I. RT. 15 SEC. 19B-2
WAYNE COUNTY
STATION 296+84

LOADING H20-S16-44

DESIGNED: Harry P. Shahan
CHECKED: [Signature]
DRAWN: [Signature] E. RUSH
CHECKED: [Signature]
MAY 10 1956
EXAMINED: [Signature]
PASSED: [Signature]
APPROVED: [Signature]
CHIEF ENGINEER

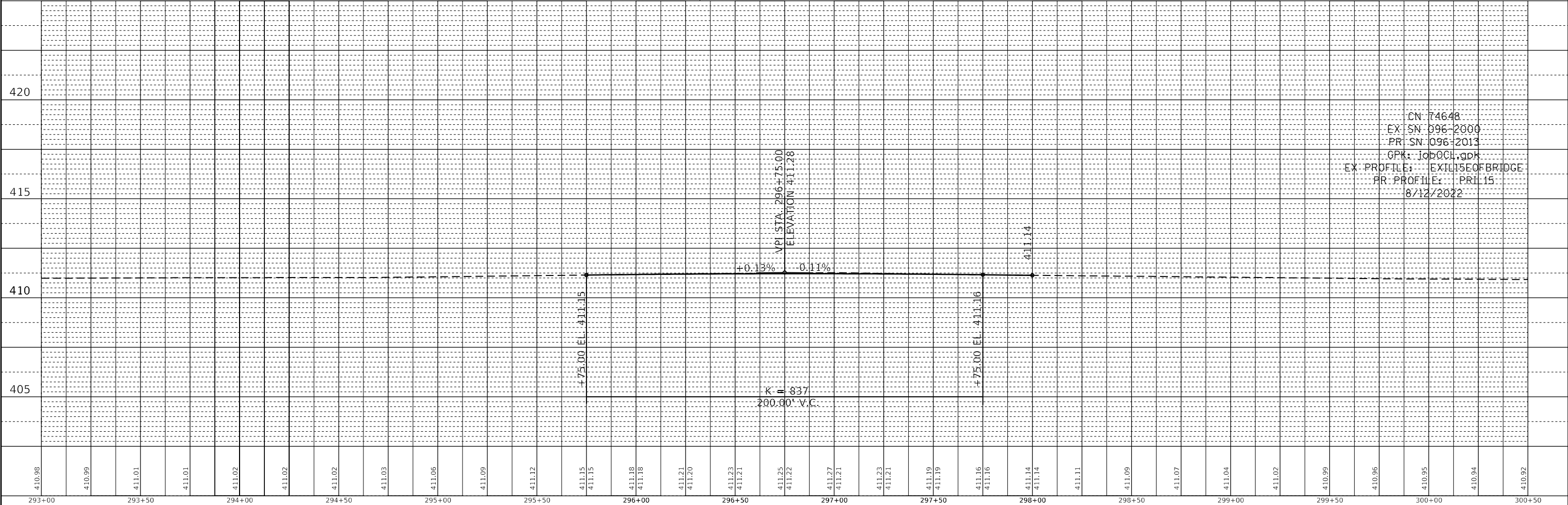
BENCHMARK:
BM 475 - Chiseled square at west end of south bridge curb on structure 096-2000
STA 297+10, 21.0' Left of C ILL-15, Elevation = 411.563

| PLAN | NO. | NOTE BOOK | SURVEYED | PLOTTED | ALIGNMENT CHECKED | CADD FILE NAME | BY | DATE |
|------|-----|-----------|----------|---------|-------------------|----------------|----|------|
| | | | | | | | | |



Note: Final structure configuration subject to refinement during TSL development.

| PROFILE | NO. | NOTE BOOK | SURVEYED | PLOTTED | GRADES CHECKED | STRUCTURE NOTATION CHNG | BY | DATE |
|---------|-----|-----------|----------|---------|----------------|-------------------------|----|------|
| | | | | | | | | |



CN 74648
EX SN 096-2000
PR SN 096-2013
GPK: 106001.gpk
EX PROFILE: EXISTING OF BRIDGE
PR PROFILE: PRI 15
8/12/2022

MODEL: \$MODELNAME\$
FILE NAME: \$FILES\$

| | | |
|-------------------------|--------------------|-----------|
| USER NAME = \$USERS\$ | DESIGNED - | REVISED - |
| | DRAWN - D. Macklin | REVISED - |
| PLOT SCALE = \$SCALES\$ | CHECKED - | REVISED - |
| PLOT DATE = \$DATES\$ | DATE - August 2022 | REVISED - |

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

| PLAN & PROFILE VIEW | | | |
|---|---------------------|-------------|----------------|
| STRUCTURE 096-2000 EXISTING / 096-2013 PROPOSED | | | |
| SCALE: _____ | SHEET 1 OF 1 SHEETS | STA. ###-## | TO STA. ###-## |

| F.A.P. RTE. | SECTION | COUNTY | TOTAL SHEETS | SHEET NO. |
|---------------------------|-----------|--------|--------------|-----------|
| 821 | (19B2)B-1 | WAYNE | | |
| CONTRACT NO. 74648 | | | | |
| ILLINOIS FED. AID PROJECT | | | | |