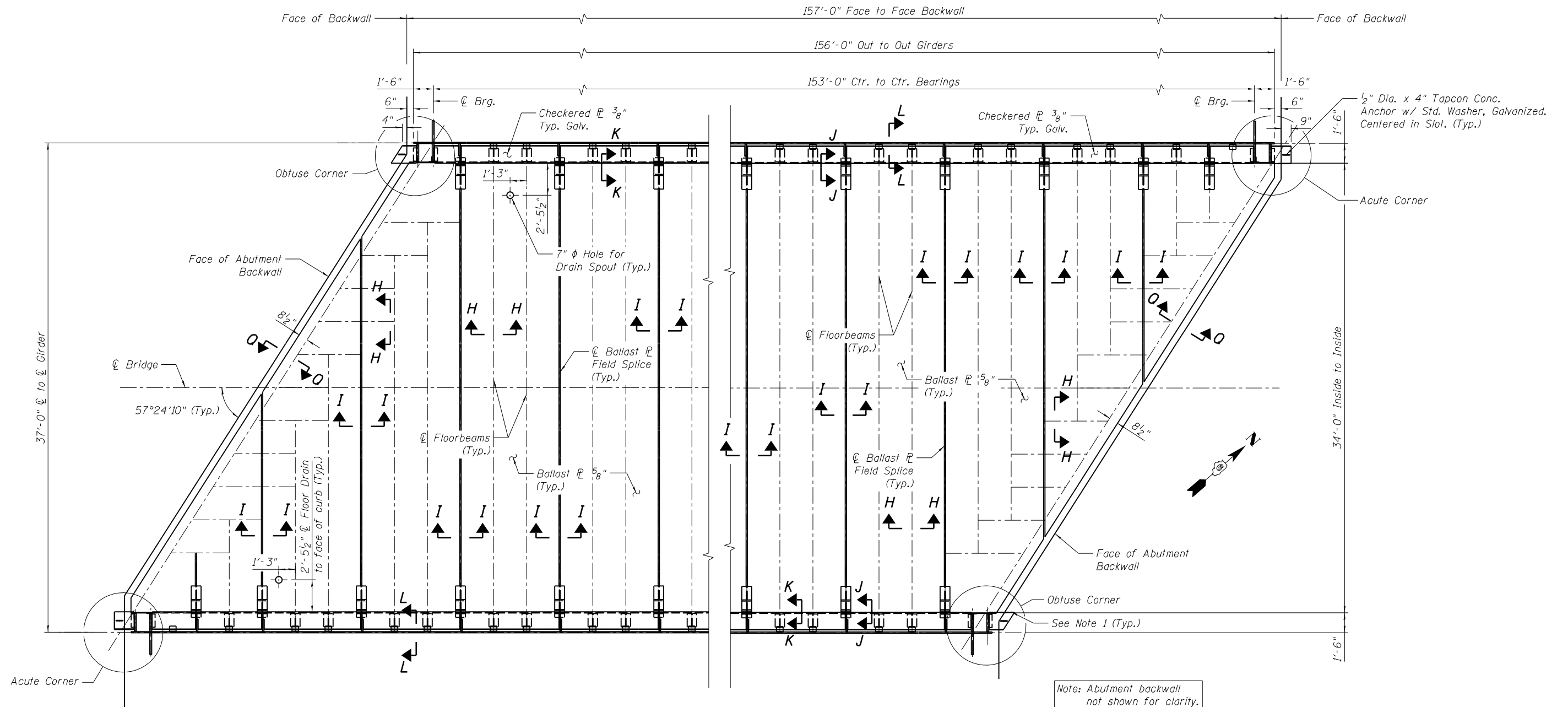


To HANNIBAL, MO
(Timetable West)

To DECATUR, IL
(Timetable East)



CLOSURE PLATE & BALLAST PAN PLAN

See Sheet 15 of 29 for Section H-H, I-I, J-J, K-K, L-L, & O-O.

- Notes:
1. Prior to Setting End Checkered $\#$, Build-up top of Concrete Backwall with Epoxy Grout to Support Checkered $\#$ and Provide Sloped Surface to Eliminate Tripping Hazard. Typical All Four Corners.
 2. Checkered $\#$ Shall be ASTM A786 Gr 36 or ASTM A36. Galvanize after fabrication.

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FILE NAME =
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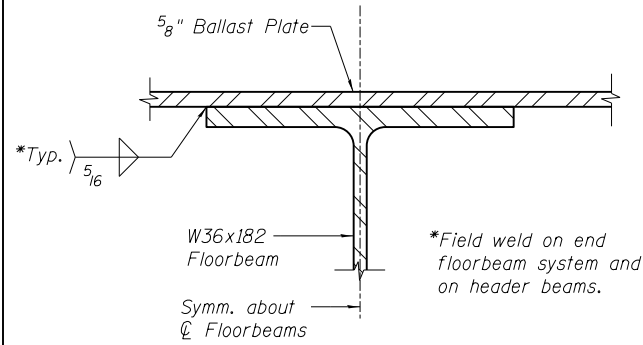
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CLOSURE PLATE AND BALLAST PLATE PLAN
STRUCTURE 084-9961 - 5TH ST NSRR

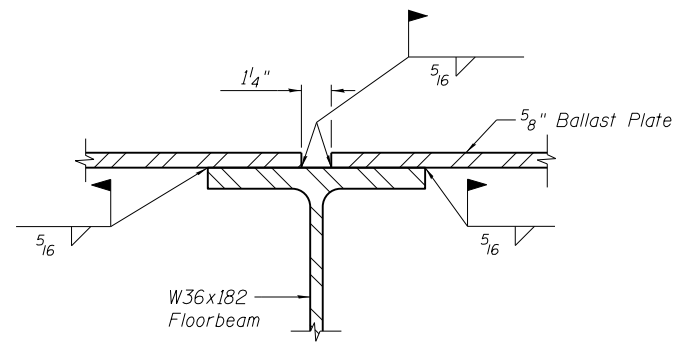
SHEET NO. 14 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO.	93733
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

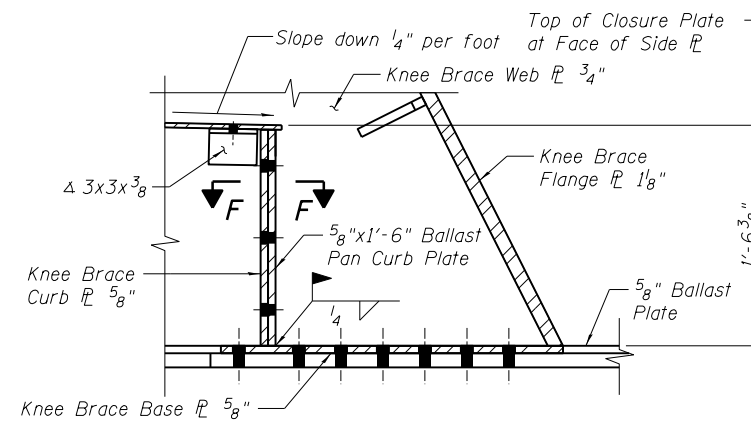
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SECTION H-H BALLAST PLATE TO FLOORBEAM CONNECTION (TYP.)
Similar Detail at Header Beam

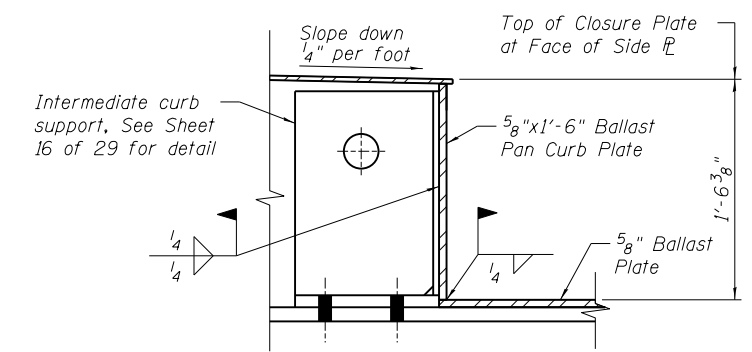


SECTION I-I

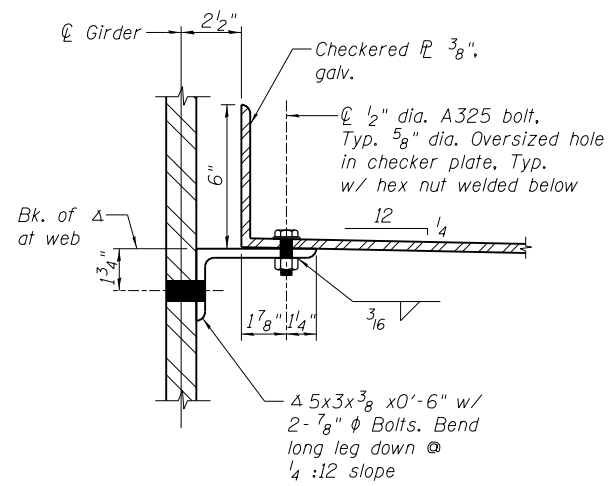


SECTION J-J

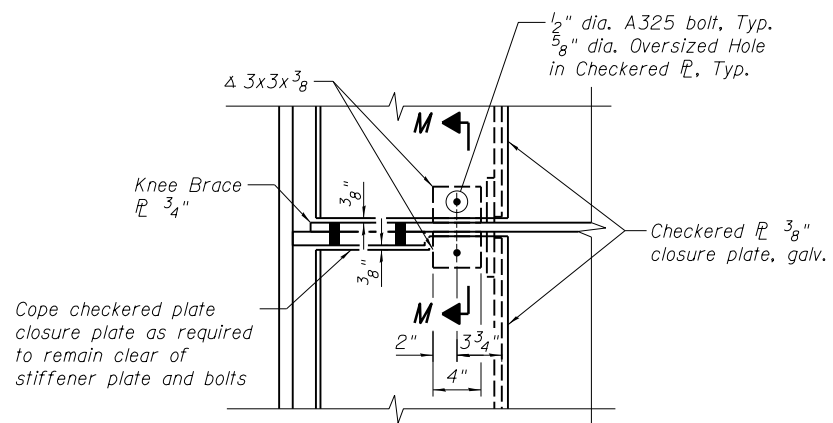
See Sheet 11 of 29 for Section F-F.



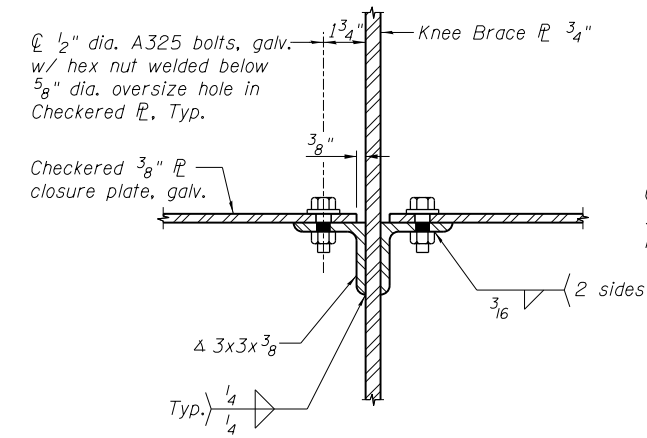
SECTION K-K



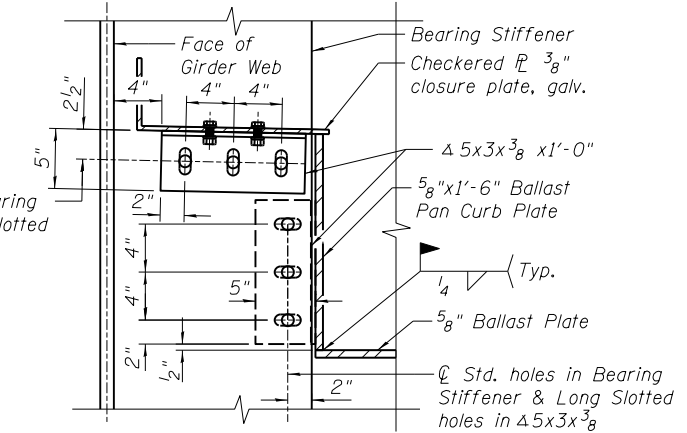
SECTION L-L



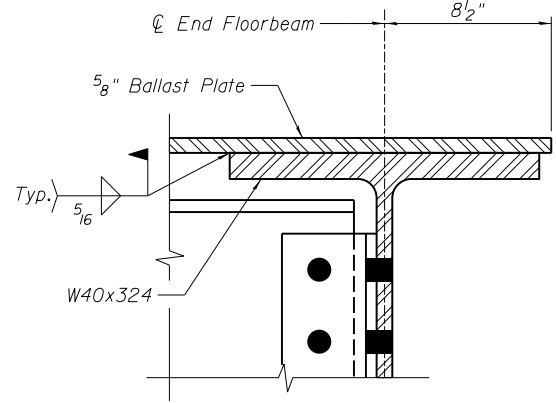
PLAN
Closure Plate at Kneebrace
(Kneebrace Flange Omitted for Clarity)



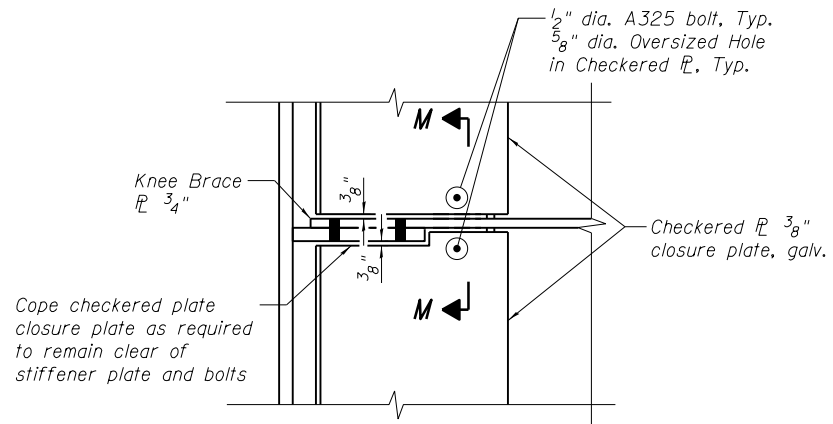
SECTION M-M



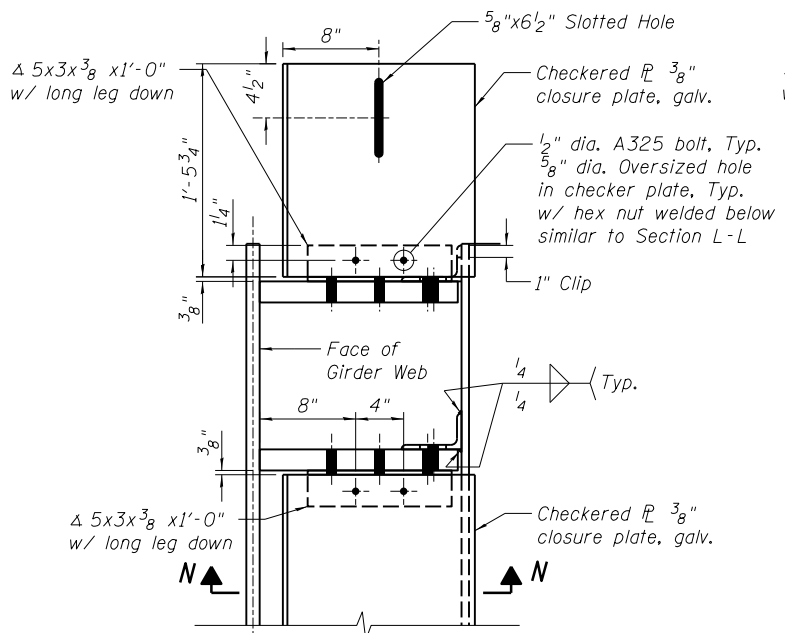
SECTION N-N



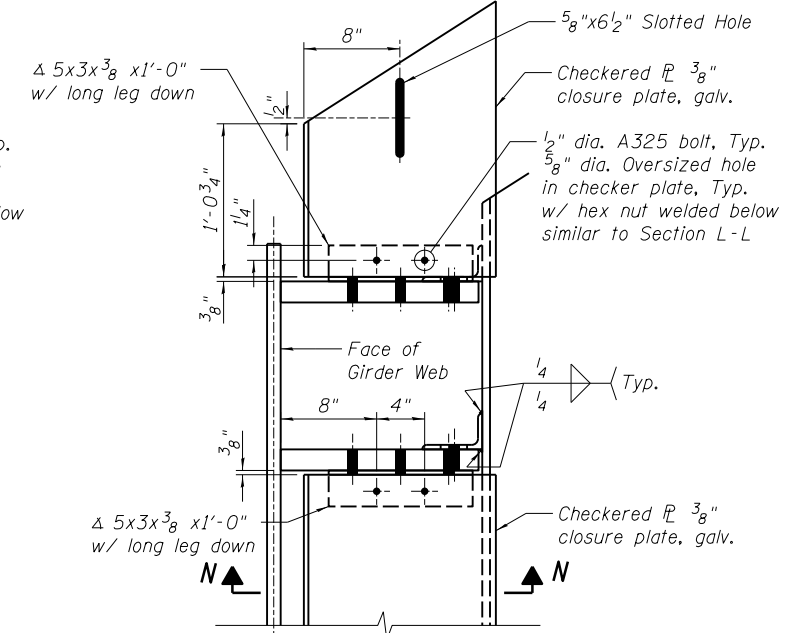
SECTION O-O



PLAN
Closure Plate at Kneebrace
(Kneebrace Flange Omitted for Clarity)

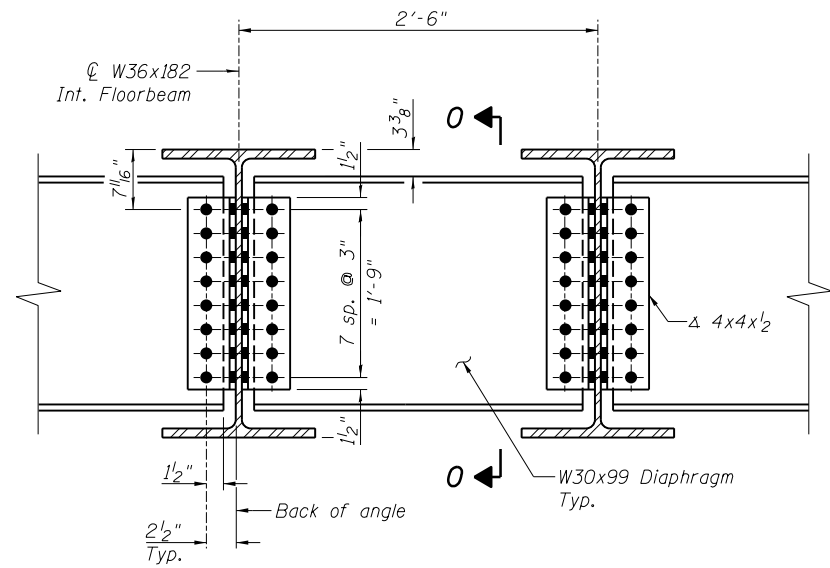


PLAN
Closure Plate at Bearing Stiffener
(Acute Corner)

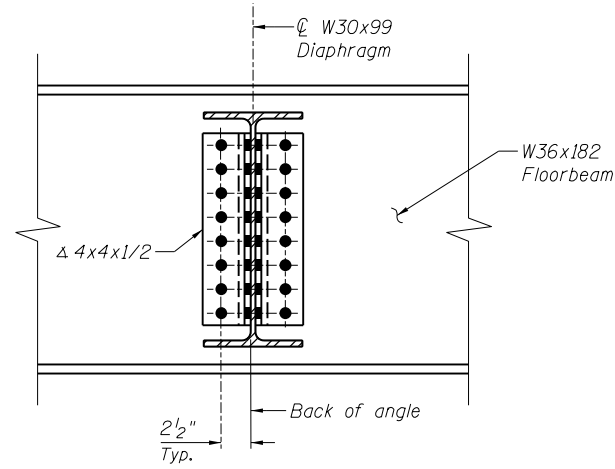


PLAN
Closure Plate at Bearing Stiffener
(Obtuse Corner)

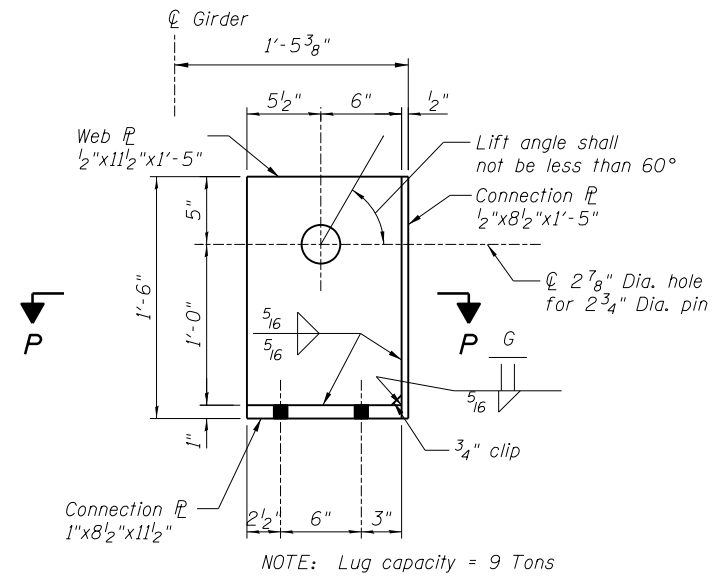
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						*666 & 666 ALT. ILLINOIS FED. AID PROJECT		



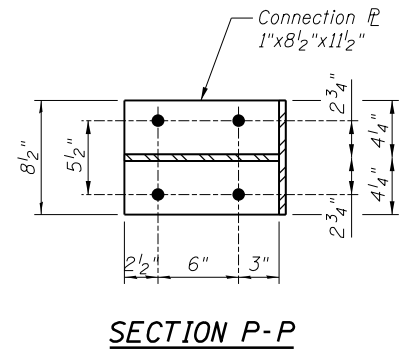
LONGITUDINAL DIAPHRAGM DETAIL



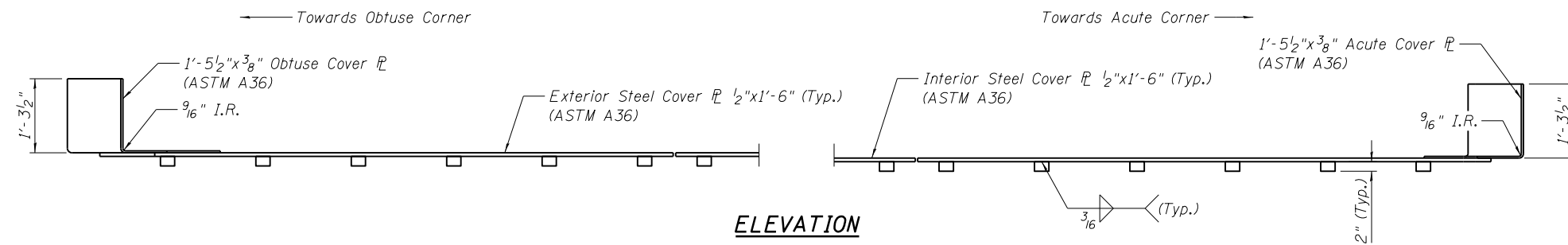
SECTION O-O



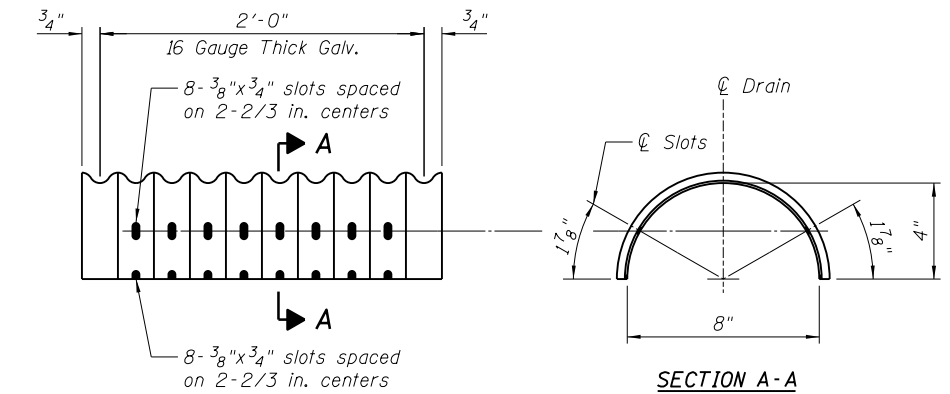
INTERMEDIATE CURB SUPPORT



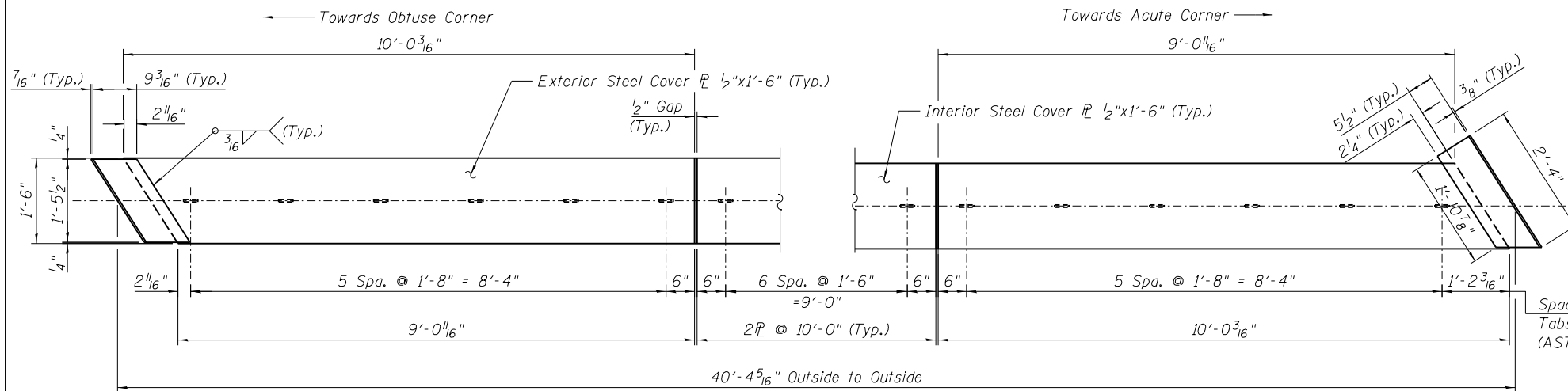
SECTION P-P



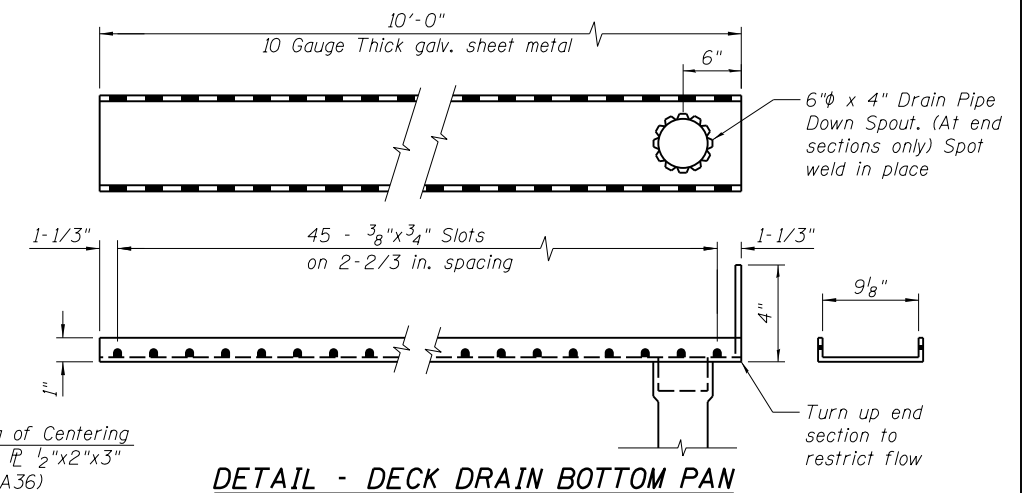
ELEVATION



DETAIL - DECK DRAIN PIPE



**PLAN
COVER PLATES
(Galvanize after Fabrication)**



DETAIL - DECK DRAIN BOTTOM PAN

- Notes:
- Lap Drain Pipe one corrugation at each end.
 - Coordinate outside diameter of drain pipe down spout with 6" ϕ Ductile Iron Pipe.
 - Cost for deck drain pipe and bottom pan shall be included in the cost of "Drainage System".

FILE NAME: P:\spr\svr\306.hanson.dom\hanson_projects\Documents\09Jobs\09L01798\CAD\Struct\5th\Sheet\0849961-09L01798-NSRR-001

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PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**MISCELLANEOUS GIRDER DETAILS - SHEET 1 OF 3
STRUCTURE 084-9961 - 5TH ST NSRR**

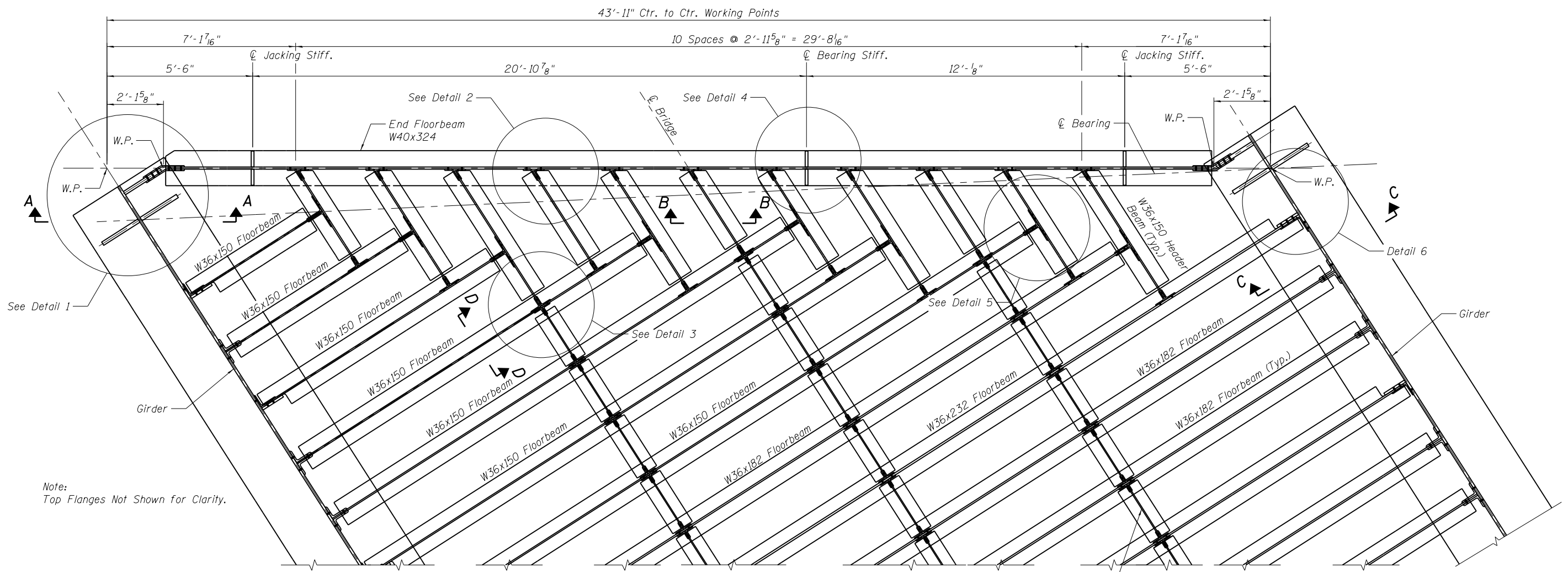
SHEET NO. 16 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO.			93733	
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

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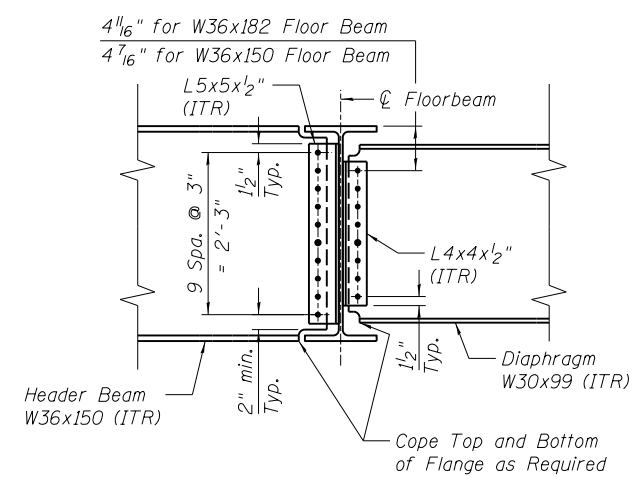
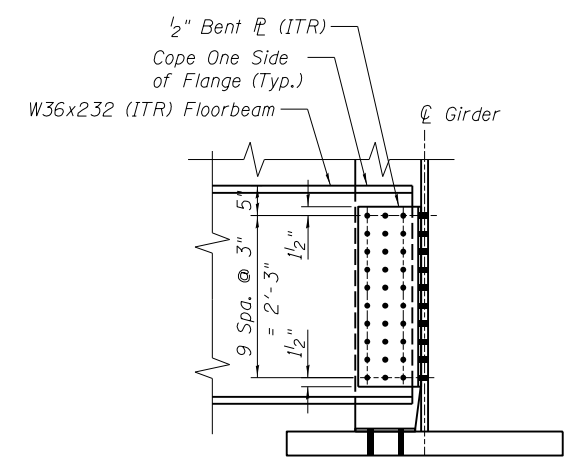
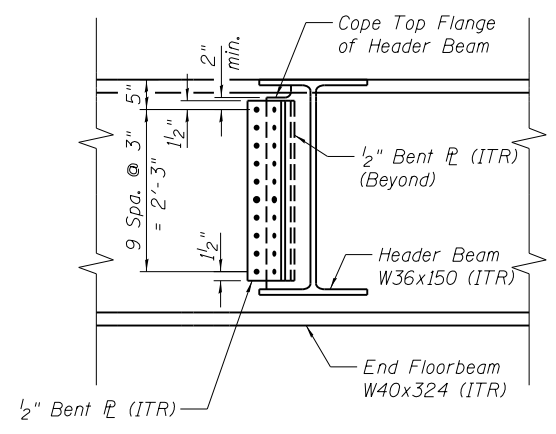
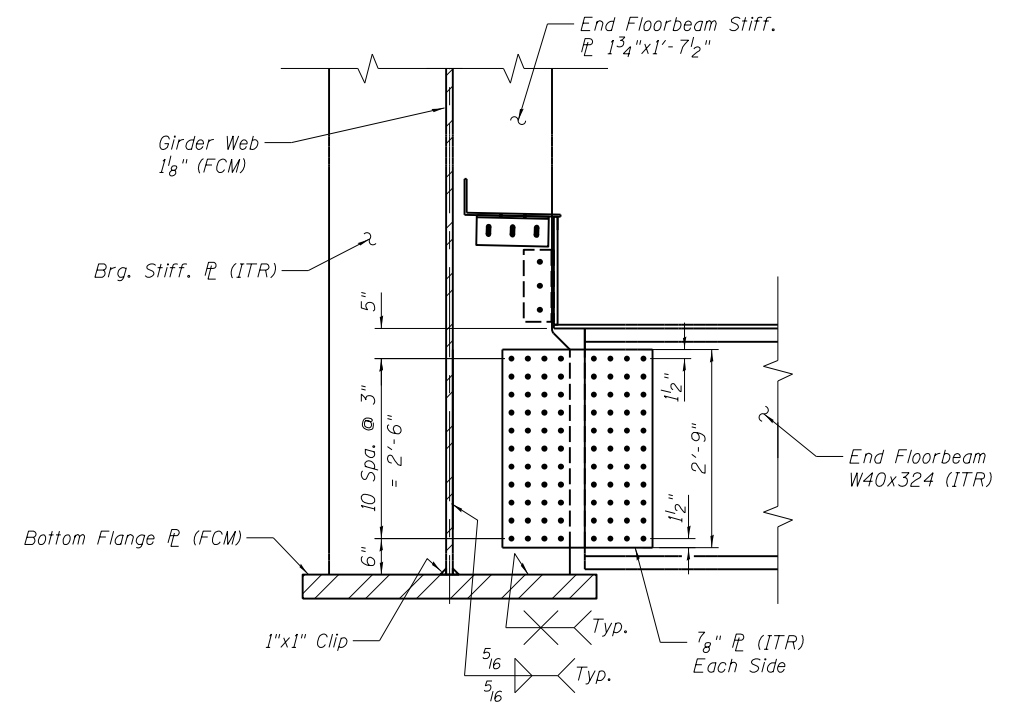


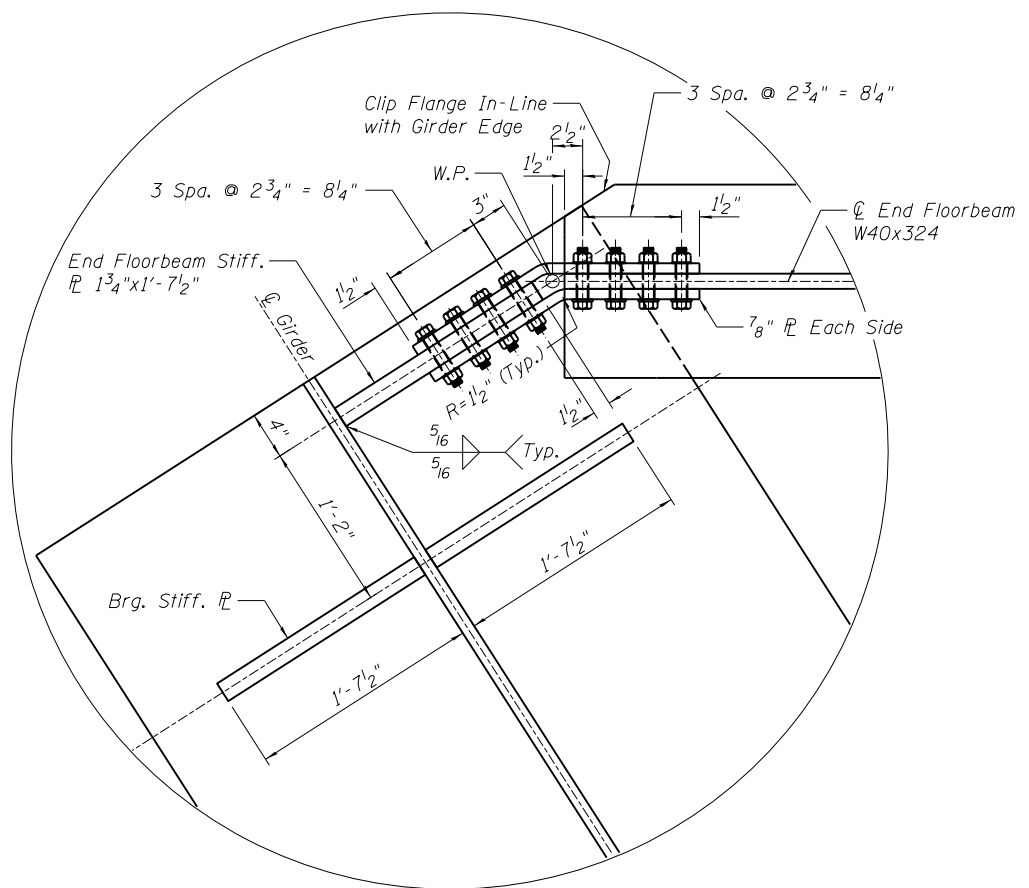
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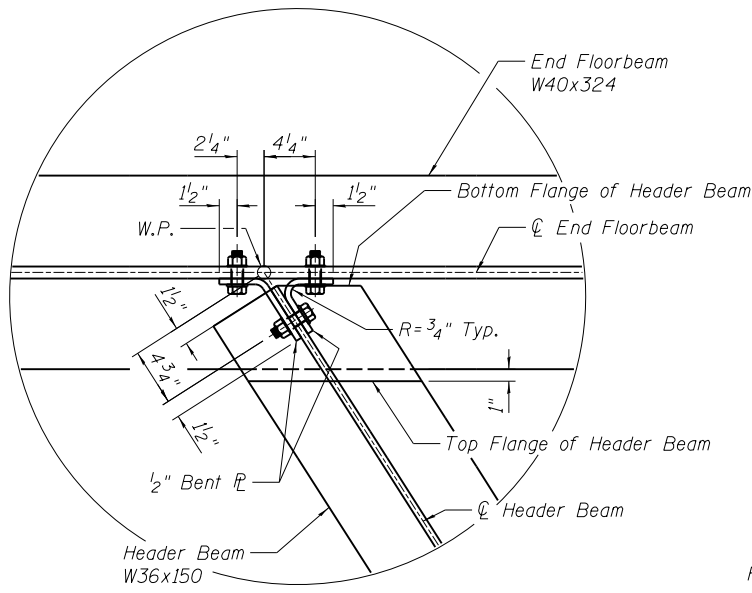
TYPICAL END FLOORBEAM PLAN
See Sheet 18 for Details

Note:
Top Flanges Not Shown for Clarity.

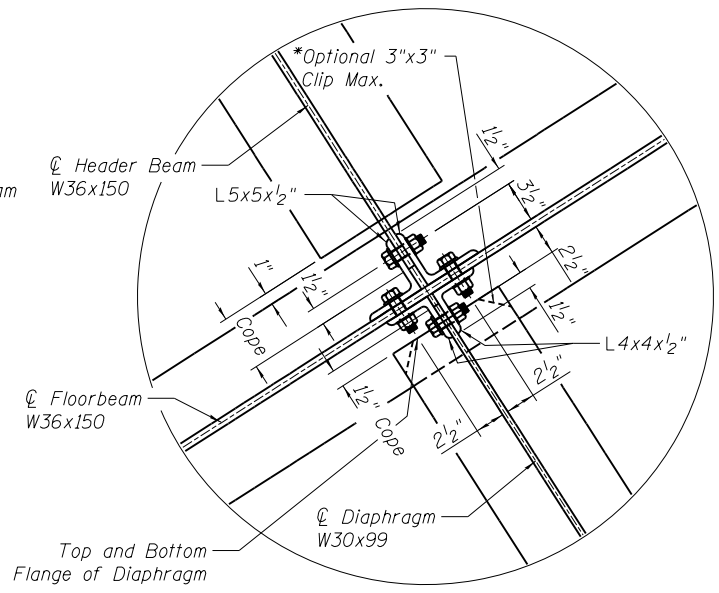




DETAIL 1

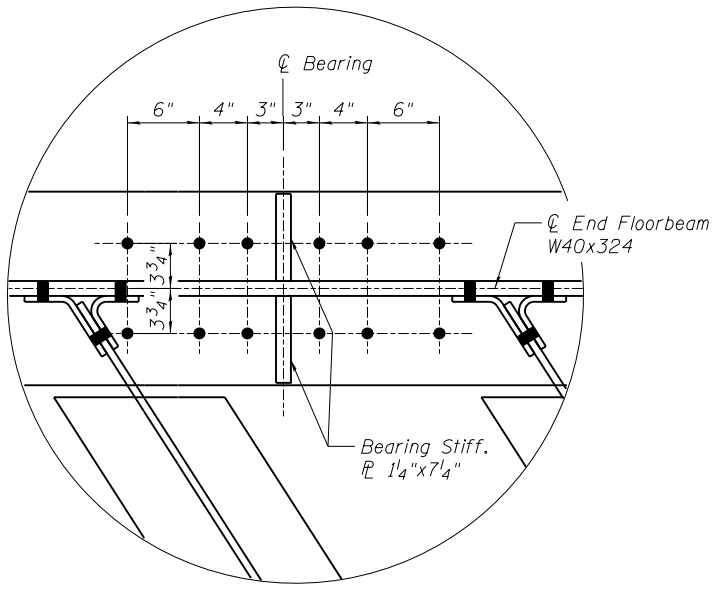


DETAIL 2

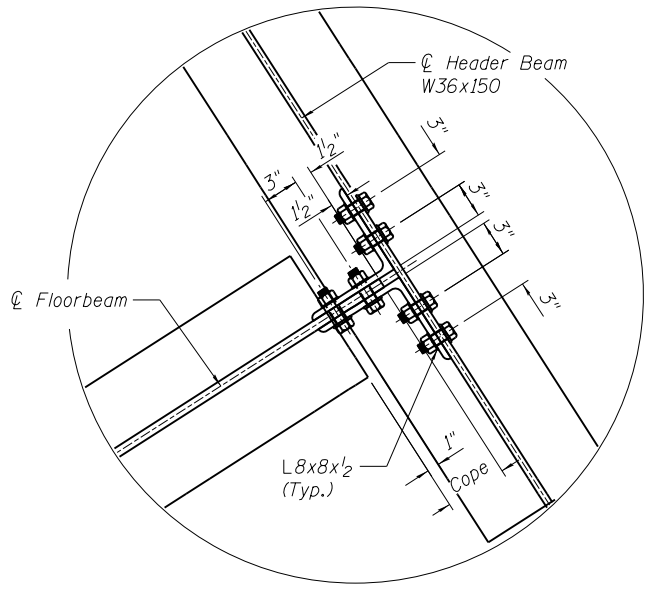


DETAIL 3

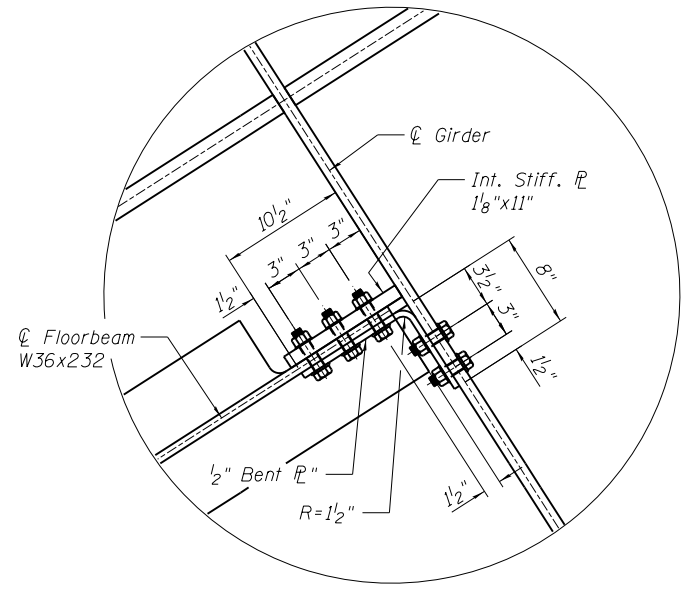
*Clipping diaphragm flanges is permitted to facilitate erection at intermediate and end floor system locations. If clipped it shall be provided at no additional cost to the Department.



DETAIL 4



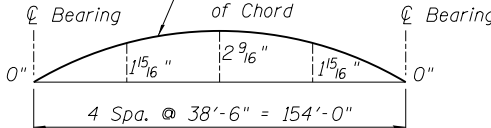
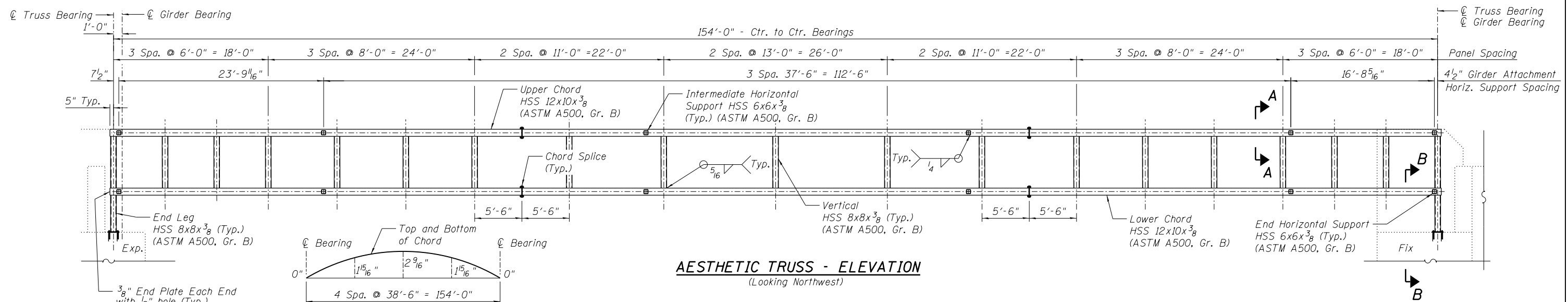
DETAIL 5



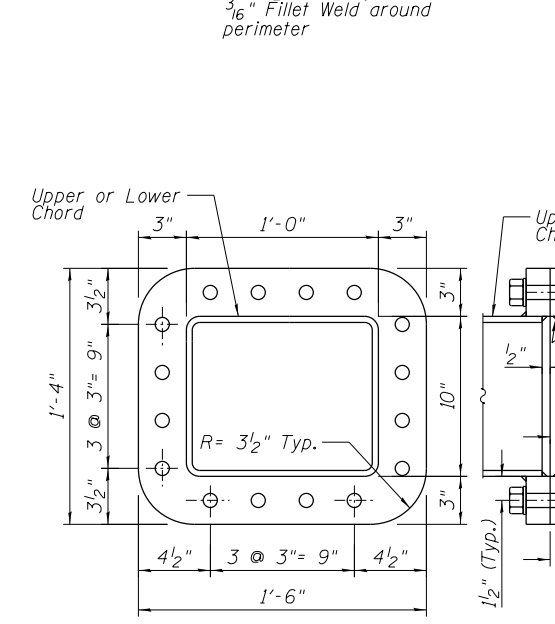
DETAIL 6

To HANNIBAL, MO
(Timetable West)

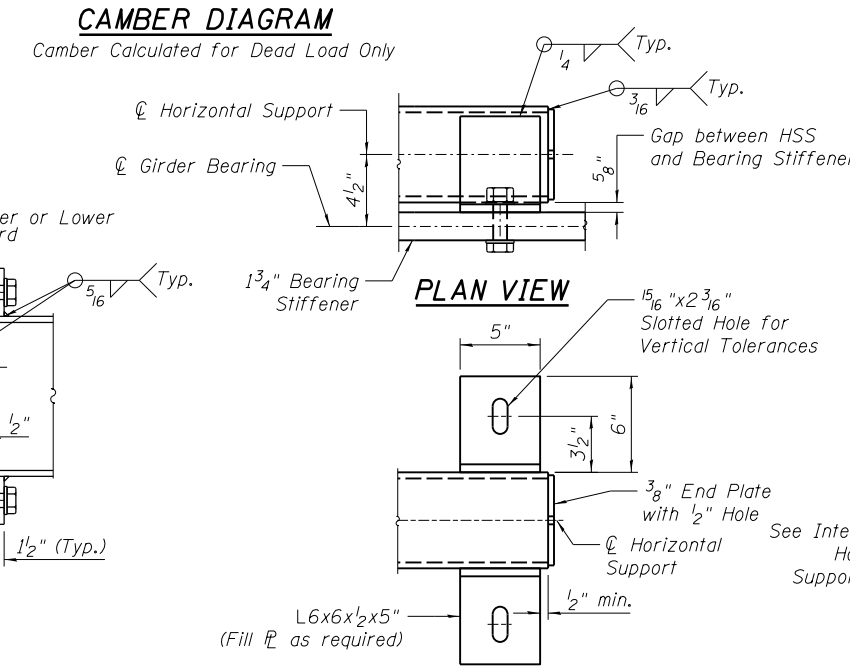
To DECATUR, IL
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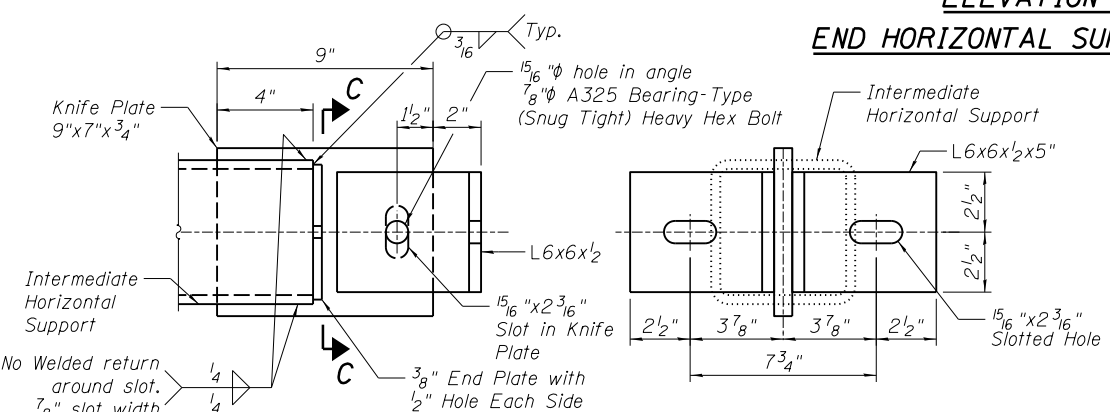
AESTHETIC TRUSS - ELEVATION
(Looking Northwest)



CHORD SPLICE DETAIL

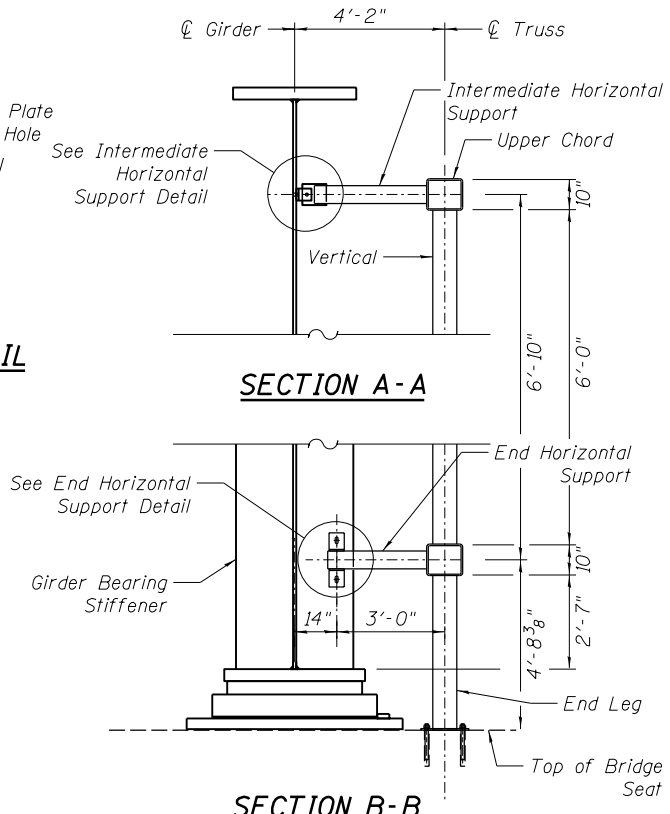


END HORIZONTAL SUPPORT DETAIL



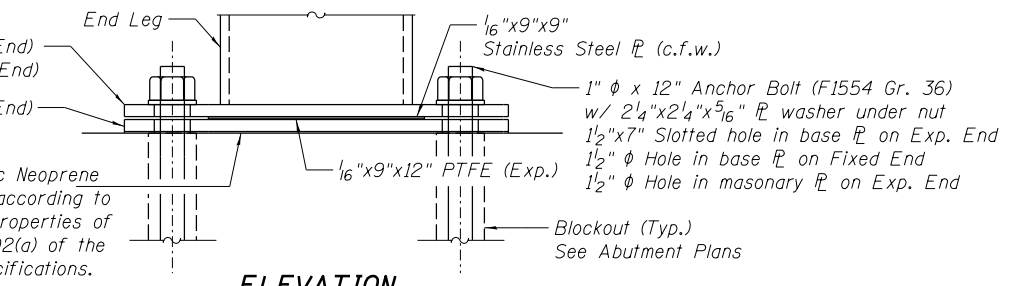
INTERMEDIATE HORIZONTAL SUPPORT DETAIL

SECTION C-C

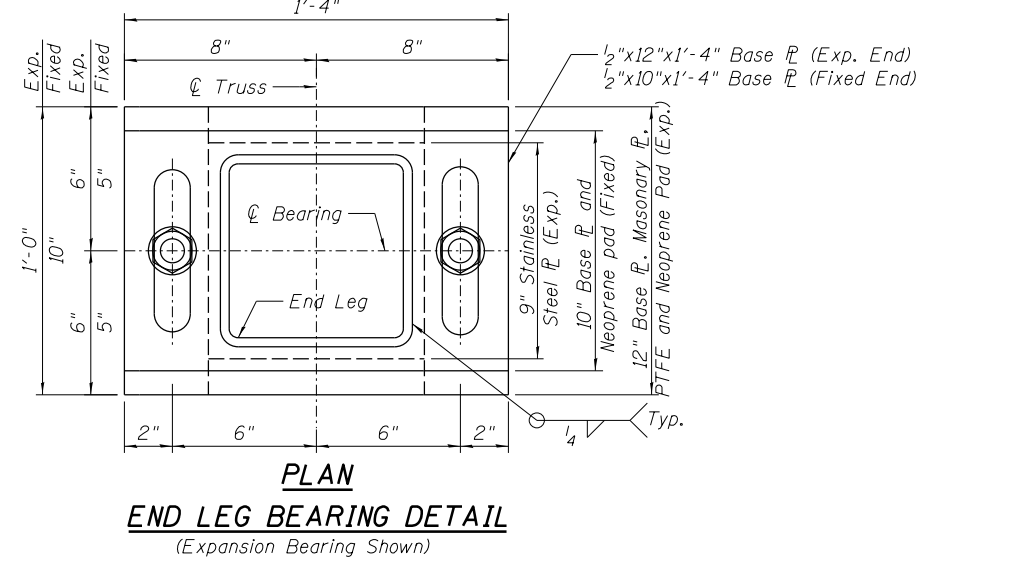


SECTION A-A

SECTION B-B



ELEVATION



END LEG BEARING DETAIL
(Expansion Bearing Shown)

Note:
 Location of Fixed and Expansion bearings shall match the girder.
 Cost for elastomeric neoprene leveling pad, PTFE surface, shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 2."
 Anchor Bolts shall be ASTM F1554 all-thread (or an Engineer approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
 Anchor Bolts shall be installed in blockouts with non-shrink grout meeting the material requirements of Article 1024.02 of the Standard Specifications. Blockouts shall be clean prior to grouting and grout installed according to manufactures recommendations.
 The PTFE shall be bonded directly to the masonry plate according to the manufacturers recommendations.

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PLOT DATE = 4/11/2019			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

AESTHETIC TRUSS
STRUCTURE 084-9961 - 5TH ST NSRR

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO.			93733	

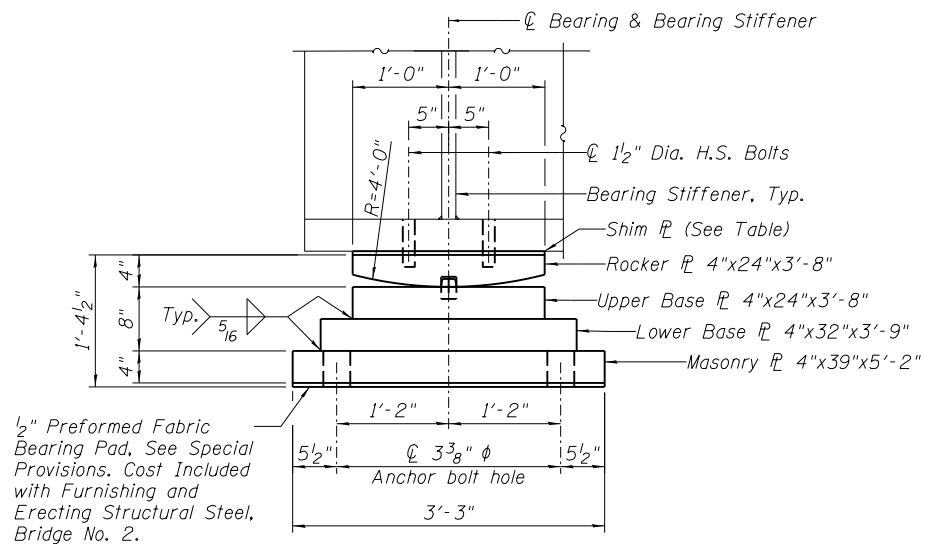
SHEET NO. 19 OF 29 SHEETS

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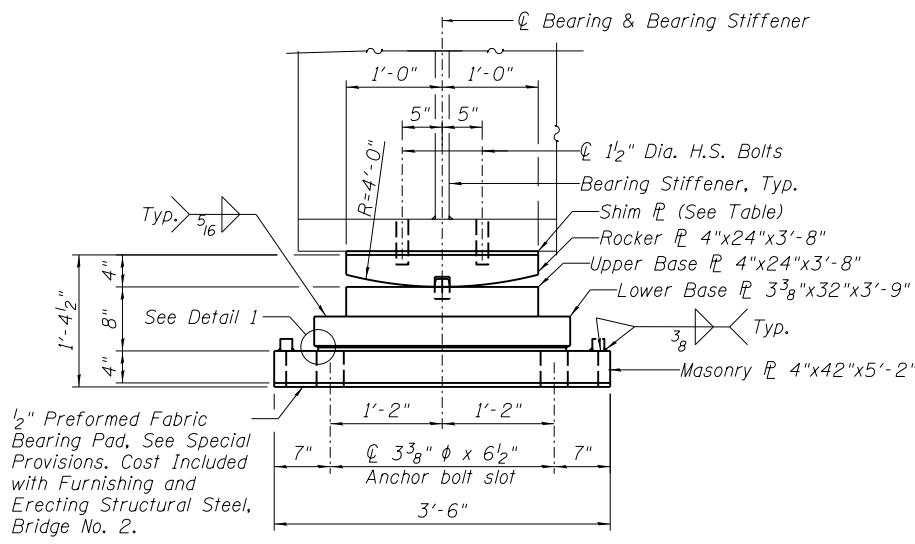


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ELEVATION - FIXED BEARING

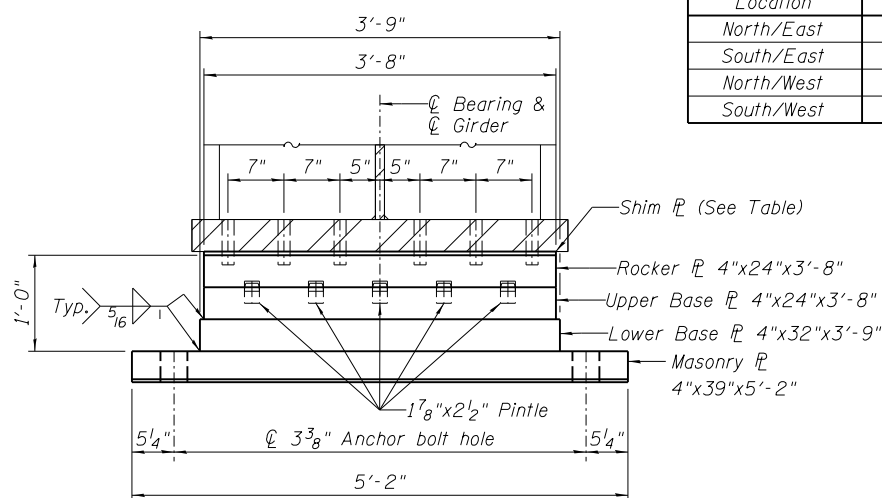
1/2" Preformed Fabric Bearing Pad, See Special Provisions. Cost Included with Furnishing and Erecting Structural Steel, Bridge No. 2.



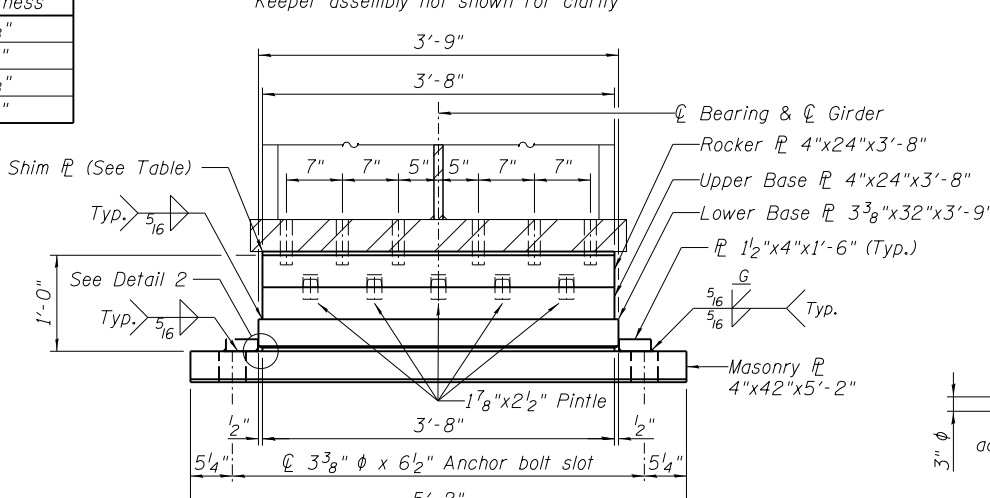
ELEVATION - EXPANSION BEARING

1/2" Preformed Fabric Bearing Pad, See Special Provisions. Cost Included with Furnishing and Erecting Structural Steel, Bridge No. 2.

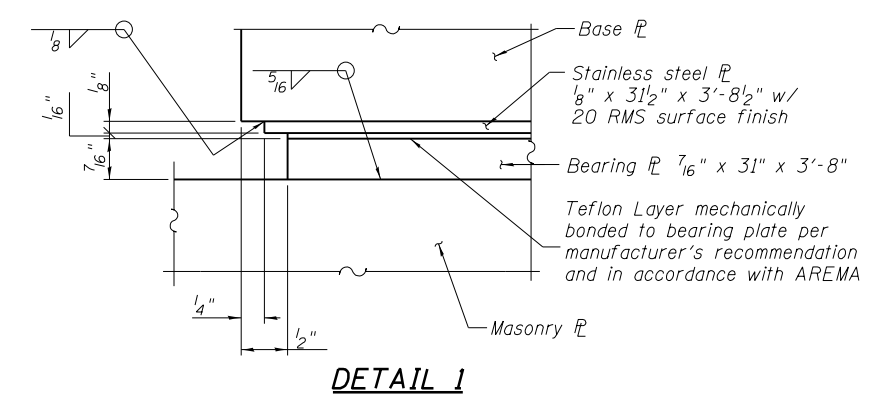
SHIM PLATE THICKNESS	
Location	Thickness
North/East	7/8"
South/East	1/8"
North/West	7/8"
South/West	1/8"



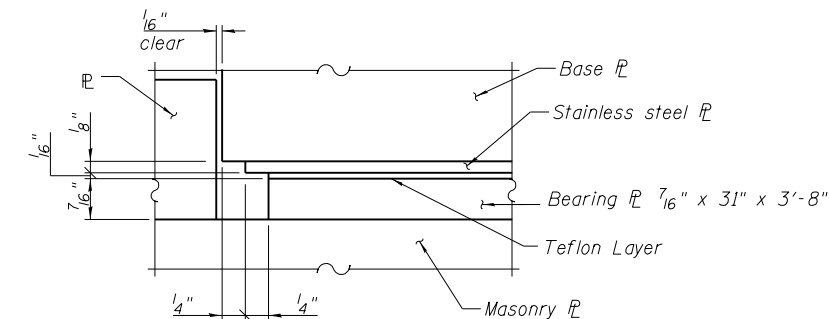
END VIEW - FIXED BEARING



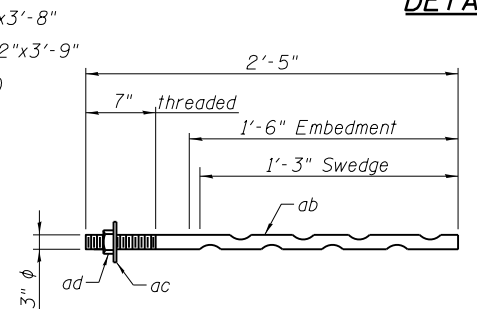
END VIEW - EXPANSION BEARING



DETAIL 1

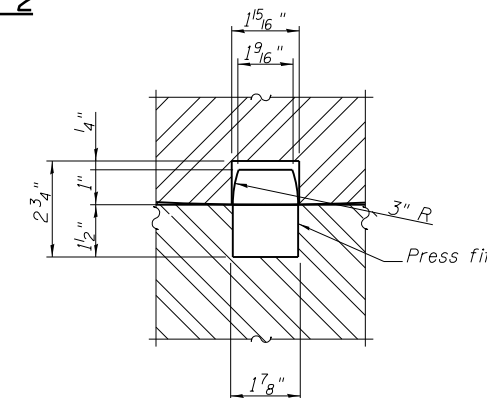


DETAIL 2

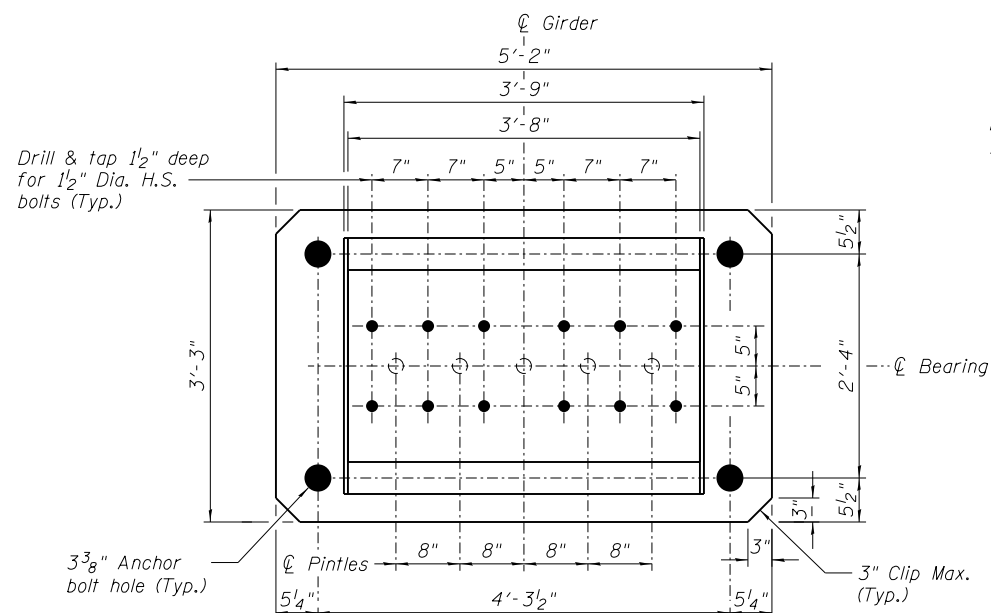


ANCHOR BOLT

1 - Bar 3" Dia. x 2'-5" - ab
 1 - Bar 5/2" Dia. x 1/4" w/ 3/8" Dia. hole at center - ac
 1 - Heavy Hex Nut - ad
 Weight = 69 lbs.
 Galvanize after fabrication
 (16 Required)

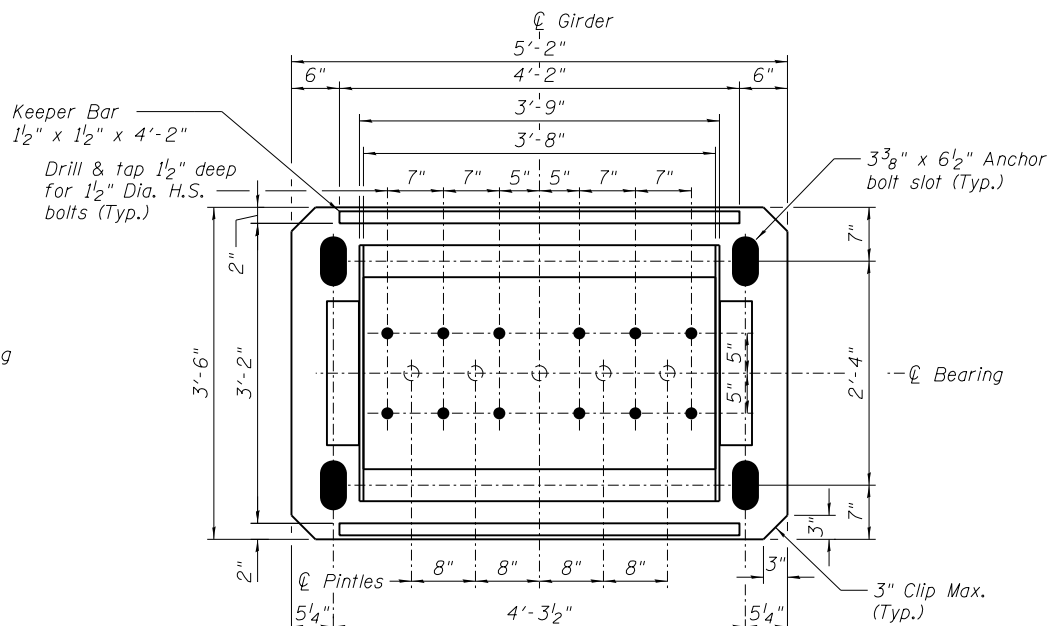


PINTLE DETAIL



PLAN VIEW - FIXED BEARING

(2 Required)



PLAN VIEW - EXPANSION BEARING

(2 Required)

NOTES:

- Steel used for bearing assemblies shall conform to ASTM A709 GR50, unless noted otherwise. Anchor bolts shall conform to ASTM F1554 Gr 105. Cost of bearings and anchor bolts shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 2".
- Stainless steel shall conform to ASTM A480.
- Bearing assembly weldments shall be stressed relieved by heat treating prior to finish machining, per current AWS structural welding codes.
- Teflon Layer shall be composed of virgin unfilled TFE resin, unfilled TFE sheets or unfilled TFE fabric. Filler material, such as milled glass fibers will not be allowed. Teflon layer shall conform to the requirements of AREMA Chapter 15.
- All surfaces in moving contact shall be finished 125/.
- All dimensions shown are final dimensions after machining.
- Bearings to be shop fitted to girders, match marked and assembled in units for shipping.
- Anchor bolts, nuts and plate washers shall be galvanized in accordance with ASTM B695, Class 50.
- Anchor bolt nuts shall be A563 Gr DH Heavy Hex & Washers shall be F436 Type 1.
- Bolt removal and replacements to gain access to properly tighten bearing bolts is incidental to steel erection.

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PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISD -
	CHECKED - MJW	REVISD -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

TPG BEARING DETAILS
 STRUCTURE 084-9961 - 5TH ST NSRR

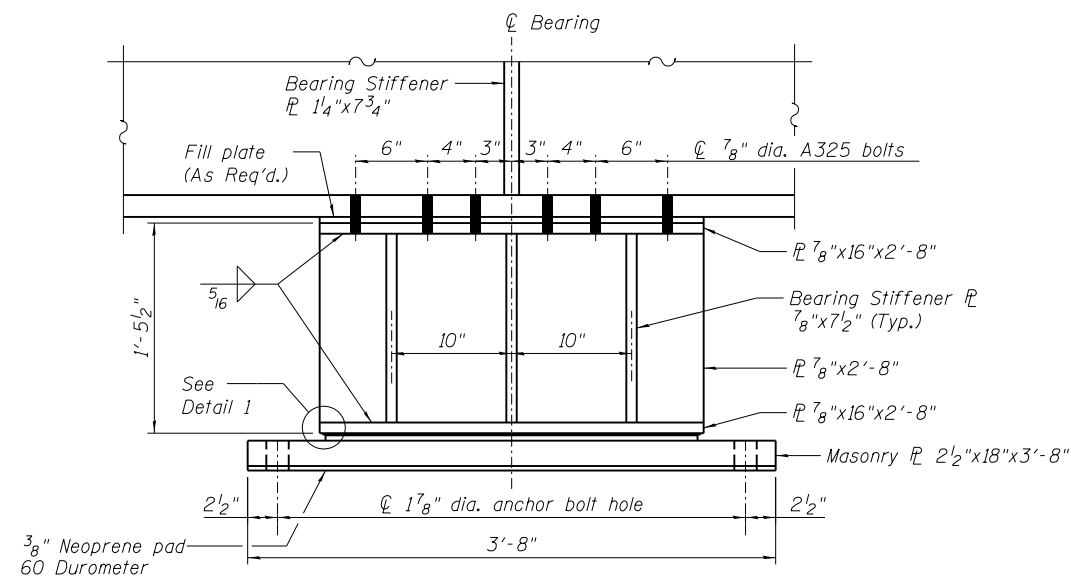
SHEET NO. 20 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO.			93733	
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

FINAL

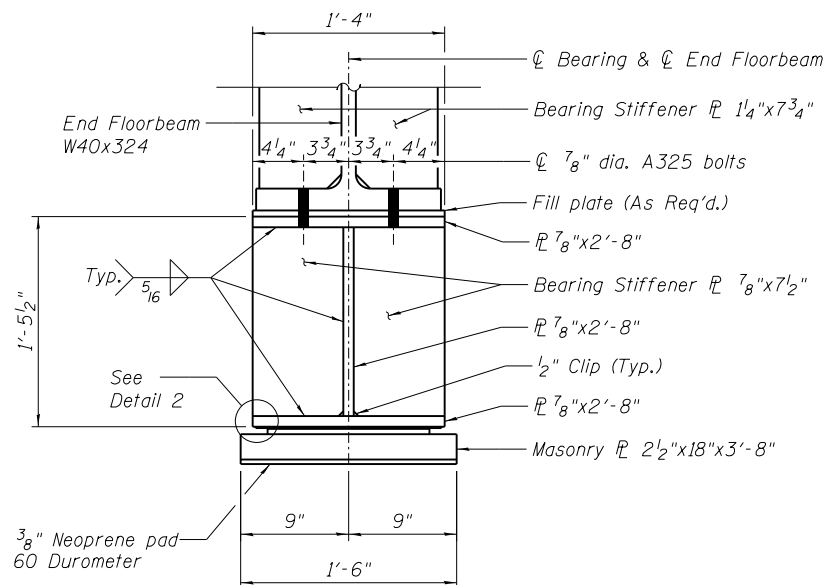


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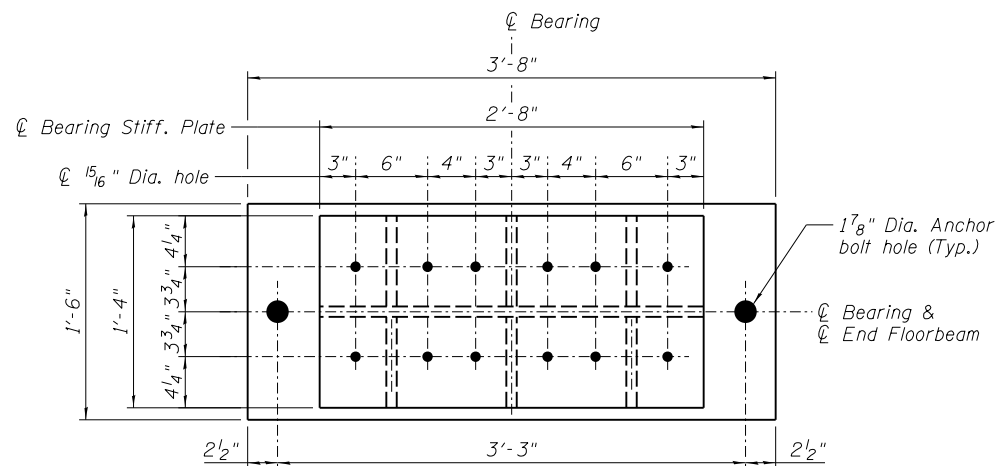
ELEVATION - END FLOORBEAM BEARING

Anchor Bolt not shown for clarity



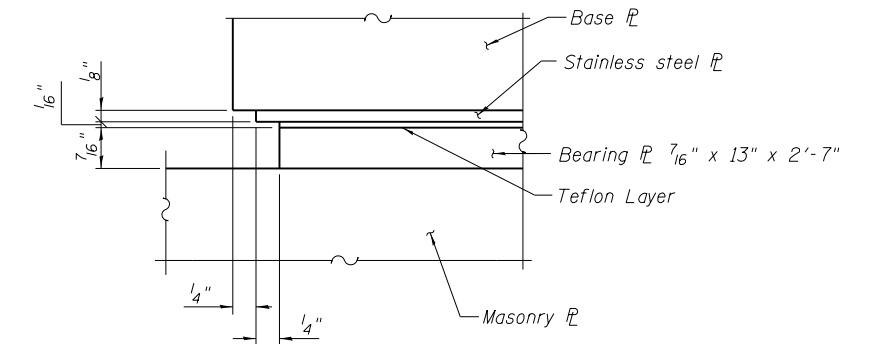
END VIEW - END FLOORBEAM BEARING

Anchor Bolt not shown for clarity

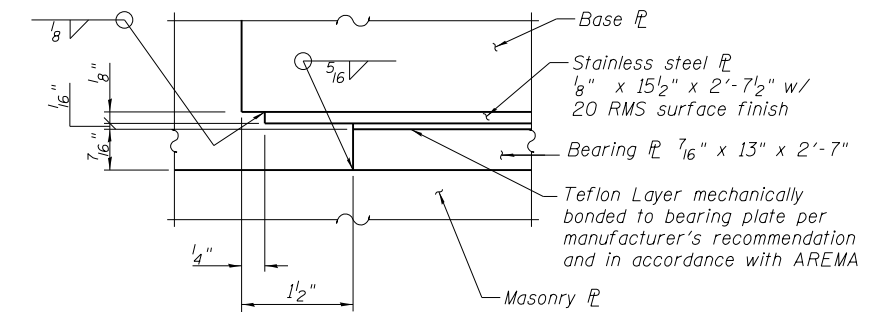


PLAN VIEW - END FLOORBEAM BEARING

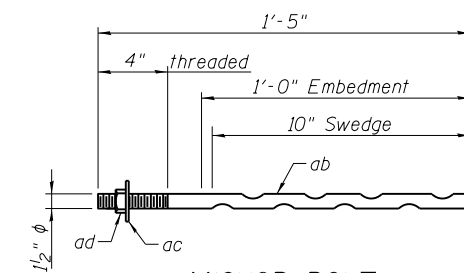
(2 Required)



DETAIL 1



DETAIL 2



ANCHOR BOLT

- 1 - Bar 1 1/2" Dia. x 1'-5" - ab
- 1 - Bar 3" Dia. x 1/4" w/ 1 5/8" Dia. hole at center - ac
- 1 - Heavy Hex Nut - ad
- Weight = 10 lbs.
- Galvanize after fabrication
- (4 Required)

NOTES:

1. Steel used for bearing assemblies shall conform to ASTM A709 GR50, unless noted otherwise. Anchor bolts shall conform to ASTM F1554 Gr 105. Cost of bearings and anchor bolts shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 2".
2. Stainless steel shall conform to ASTM A480.
3. Bearing assembly weldments shall be stress relieved by heat treating prior to finish machining, per current AWS structural welding codes.
4. Teflon Layer shall be composed of virgin unfilled TFE resin, unfilled TFE sheets or unfilled TFE fabric. Filler material, such as milled glass fibers will not be allowed. Teflon layer shall conform to the requirements of AREMA Chapter 15.
5. All surfaces in moving contact shall be finished 125/.
6. All dimensions shown are final dimensions after machining.
7. Bearings to be shop fitted to girders, match marked and assembled in units for shipping.
8. Anchor bolts, nuts and plate washers shall be galvanized in accordance with ASTM B695, Class 50.
9. Anchor bolt nuts shall be A563 Gr DH Heavy Hex & Washers shall be F436 Type 1.
10. Bolt removal and replacements to gain access to properly tighten bearing bolts is incidental to steel erection.

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FILE NAME =	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
	PLOT SCALE = 0:2.0000 ' = 1" / in.	DRAWN - RSJ	REVISED -
	PLOT DATE = 4/11/2019	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**END FLOORBEAM BEARING DETAILS
STRUCTURE 084-9961 - 5TH ST NSRR**

SHEET NO. 21 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	208
			CONTRACT NO. 93733	

•666 & 666 ALT. ILLINOIS FED. AID PROJECT

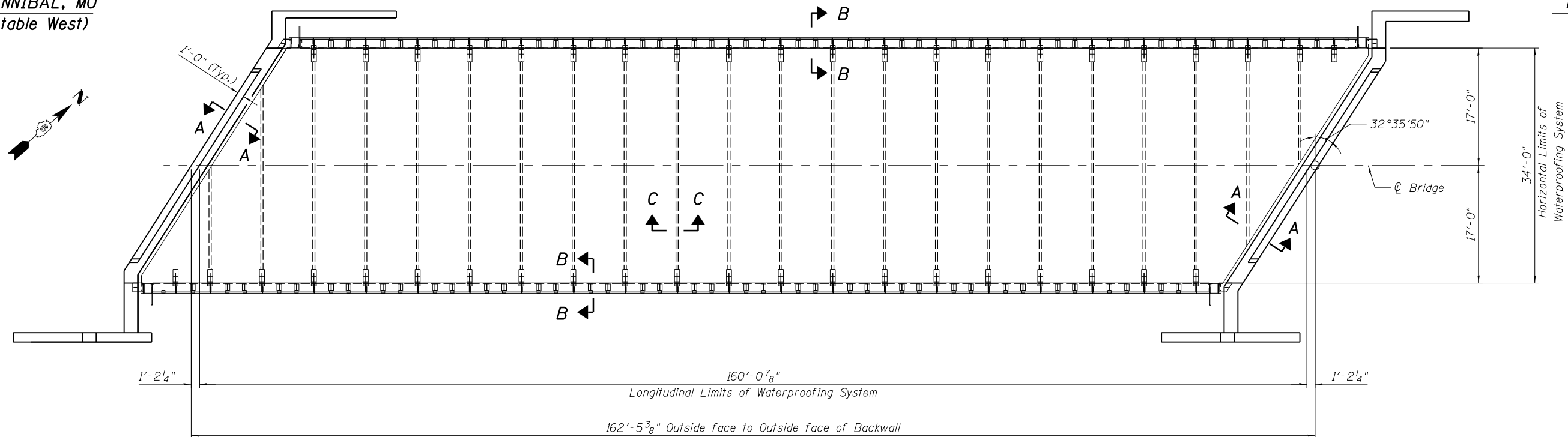
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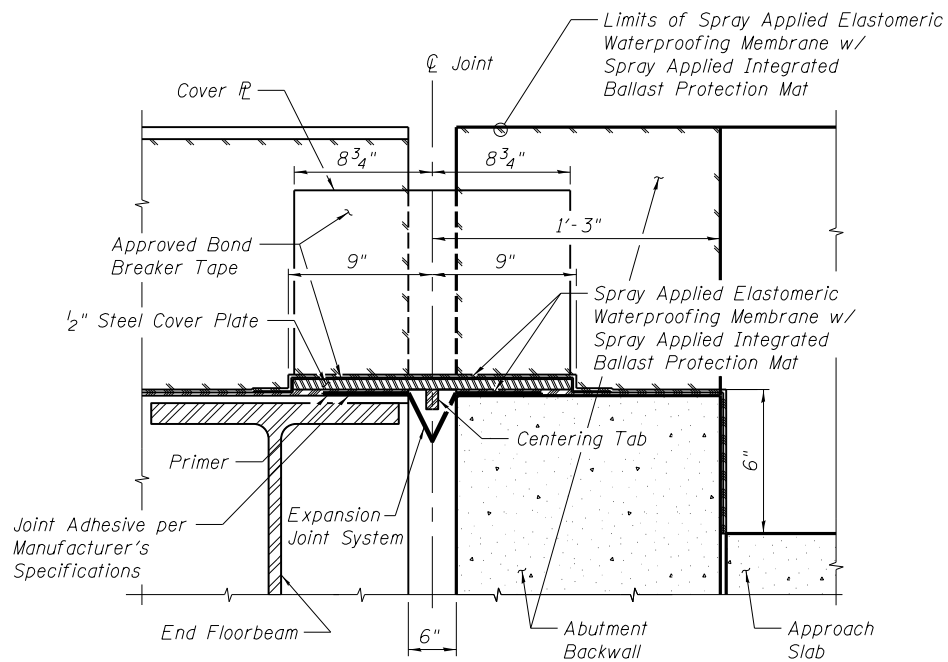
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To HANNIBAL, MO
(Timetable West)

To DECATUR, IL
(Timetable East)

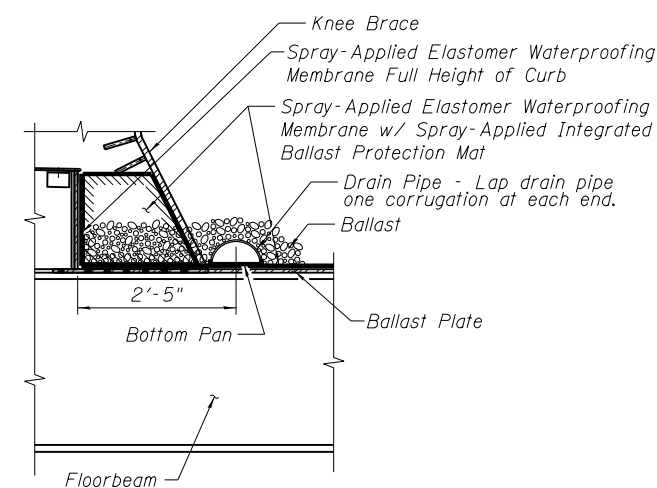


WATERPROOFING LIMITS PLAN

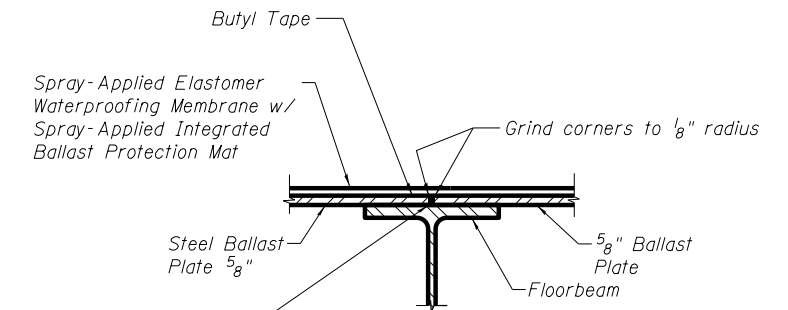


SECTION A-A
(At Rt. 4's to Bk. of Abut.)

- Note:**
1. Bridge deck membrane continuous thru joint.
 2. Typical Joint Detail shown for information only. Waterproofing installer shall determine final details in accordance with the manufacturer's recommendations.



SECTION B-B



Non-staining grey one compound non-sag elastomeric gun grade polyurethane sealant meeting the requirements of ASTM C-920, Type S, Grade NS, Class 25, Cost included with Membrane Waterproofing (Special).

SECTION C-C

- Notes:**
1. Prepare surfaces and apply in accordance with Manufacturer's recommendations.
 2. Structural steel cover plates shall be galvanized.
 3. Cost of joint adhesive and bond breaker tape shall be included in the cost of "Membrane Waterproofing (Special)".
 4. The cover plate is included in the weight of the Structural Steel and will be paid for as "Furnishing and Erecting Structural Steel, Bridge No. 2".
 5. For cover plate details see Sheet 16 of 29.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Membrane Waterproofing (Special)	Sq. Ft.	5,957

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FILE NAME =	USER NAME = Pop00275	DESIGNED - MJW	REVISIONS
		CHECKED - TJH/TDP	REVISIONS
	PLOT SCALE = 0:2.0000 ' = 1" / in.	DRAWN - RSJ	REVISIONS
	PLOT DATE = 4/11/2019	CHECKED - MJW	REVISIONS

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**BRIDGE DECK WATERPROOFING
STRUCTURE 084-9961 - 5TH ST NSRR**

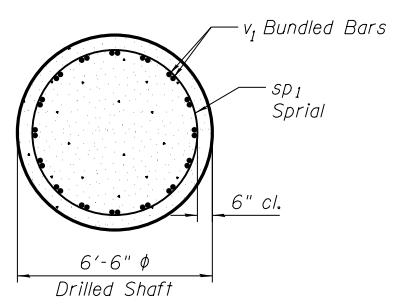
SHEET NO. 22 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO.	93733
*666 & 666 ALT.		ILLINOIS FED. AID PROJECT		

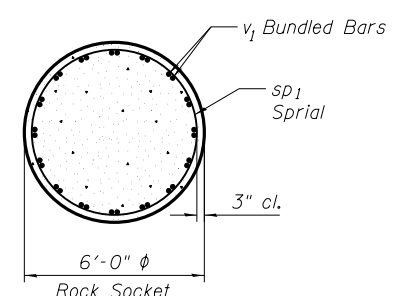
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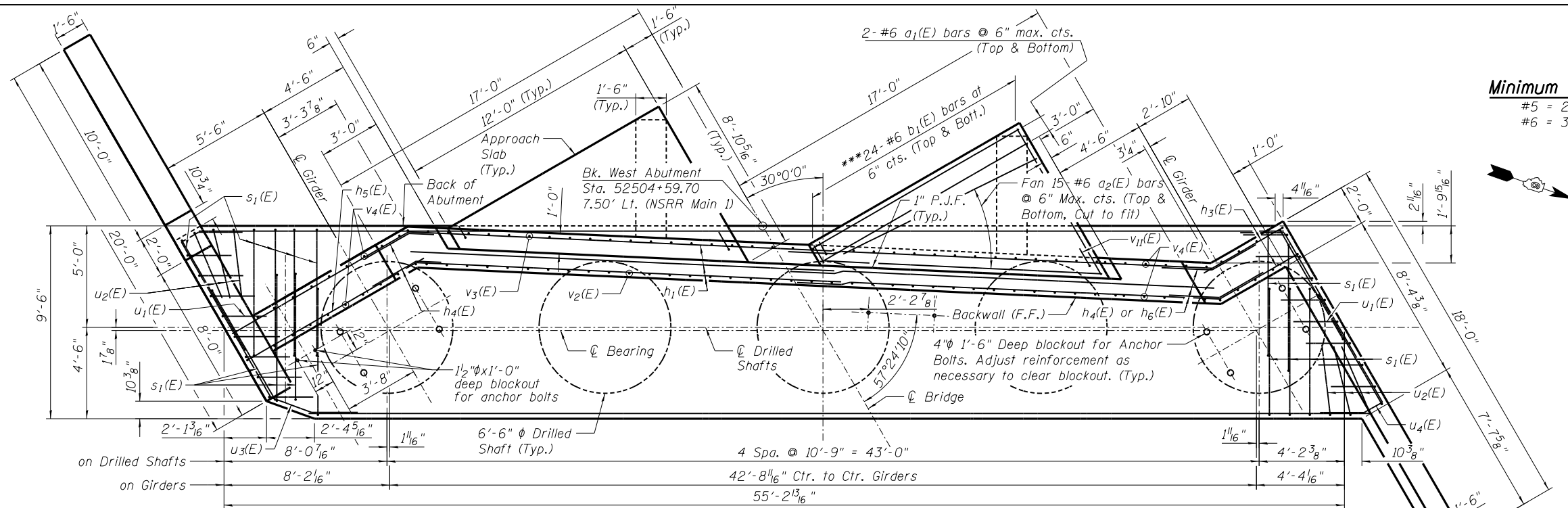
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SECTION B-B

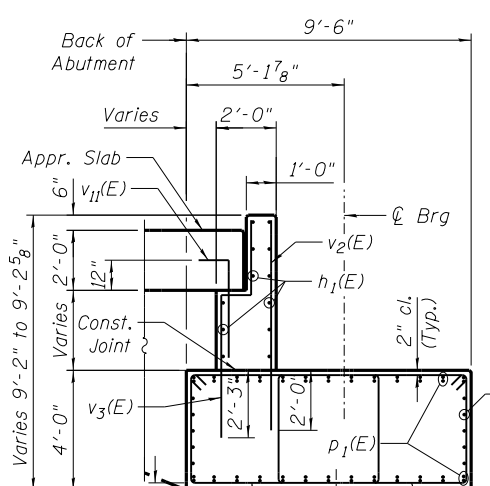


SECTION C-C



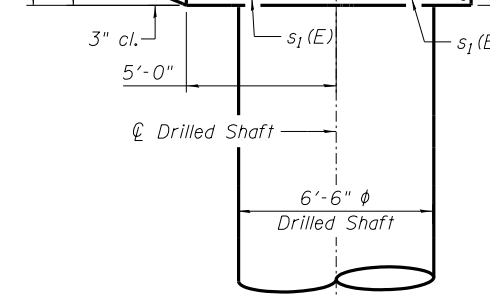
PLAN

Minimum Bar Lap
 #5 = 2'-2"
 #6 = 3'-1"



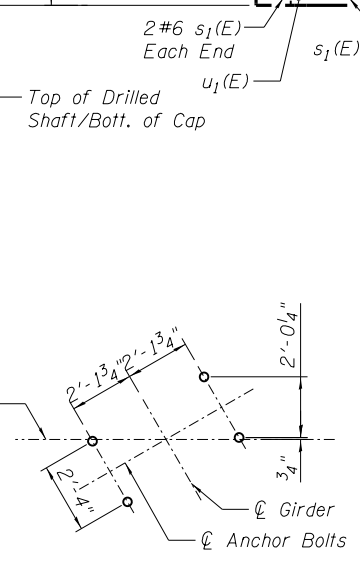
SECTION A-A

(At Rt. L's to Bk. of Abut.)



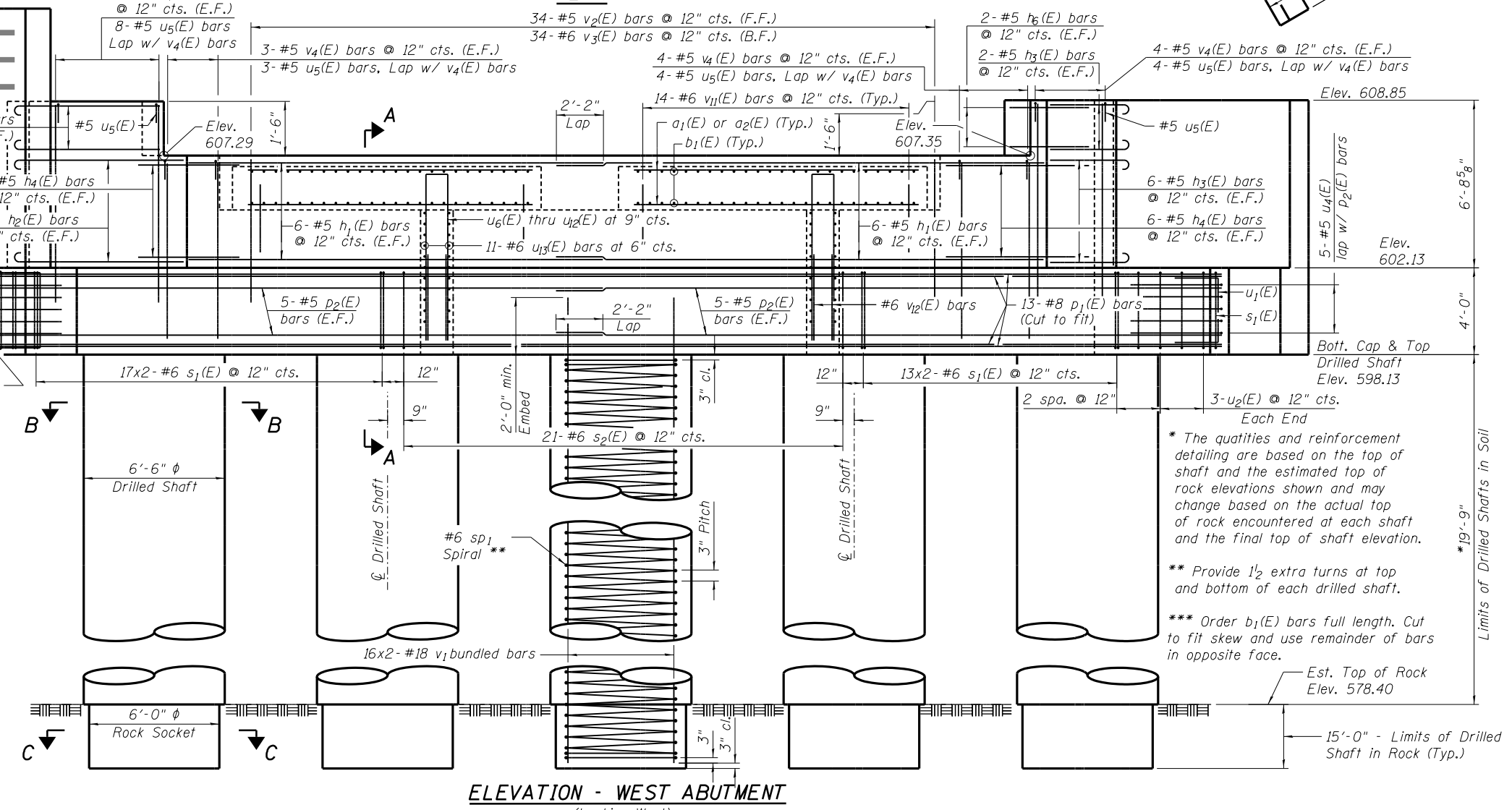
BLOCKOUT LAYOUT

(At End Floorbeam Location)



BLOCKOUT LAYOUT

(At Girders Locations)

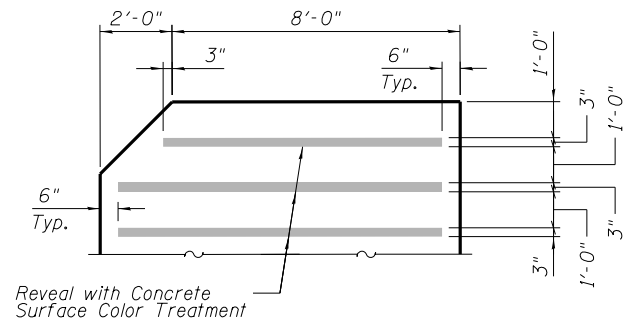


ELEVATION - WEST ABUTMENT

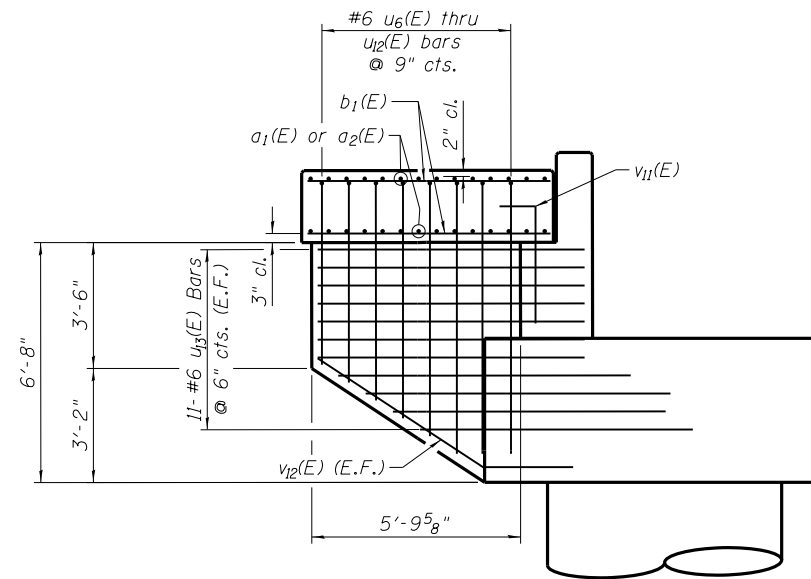
(Looking West)

* The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.
 ** Provide 1/2 extra turns at top and bottom of each drilled shaft.
 *** Order b1(E) bars full length. Cut to fit skew and use remainder of bars in opposite face.

Limits of Drilled Shafts in Soil
 *19'-9"

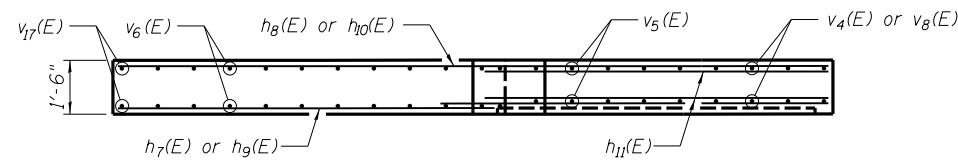


CONCRETE REVEAL DETAIL

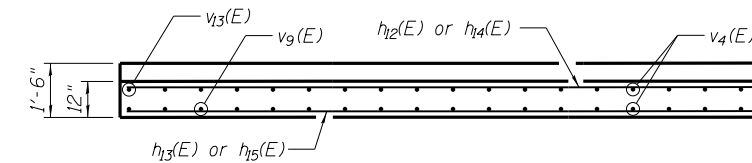


APPROACH SLAB SECTION

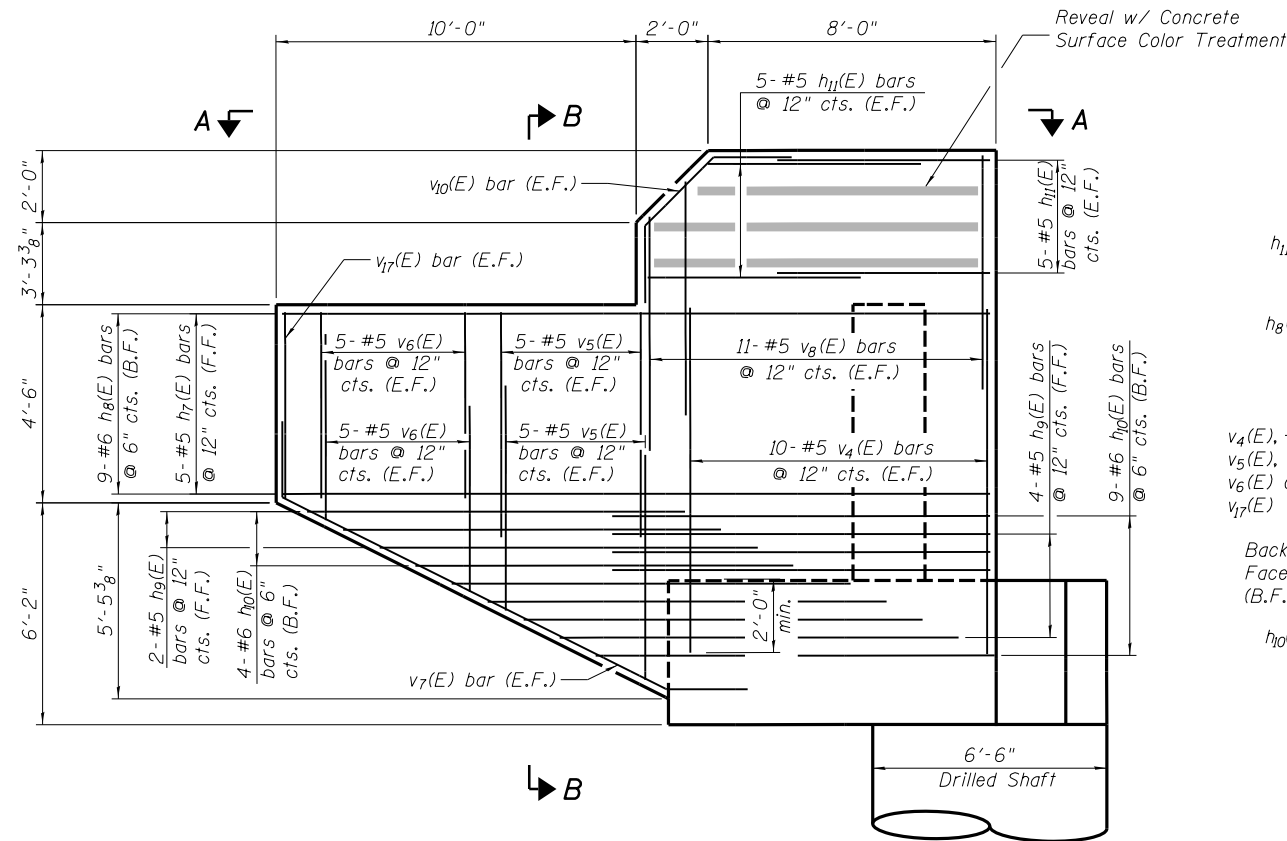
(Horizontal Dimensions at Rt. L's to back of abutment.)



SECTION A-A - PLAN VIEW

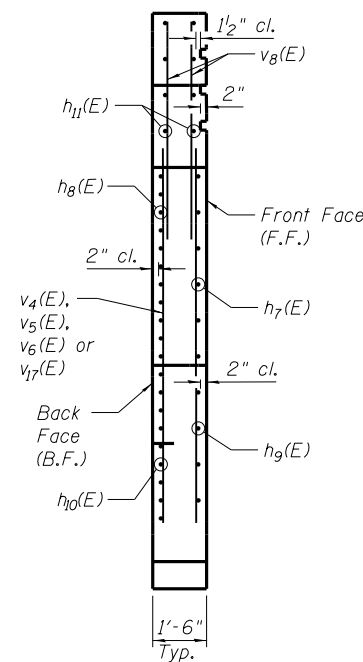


SECTION C-C - PLAN VIEW

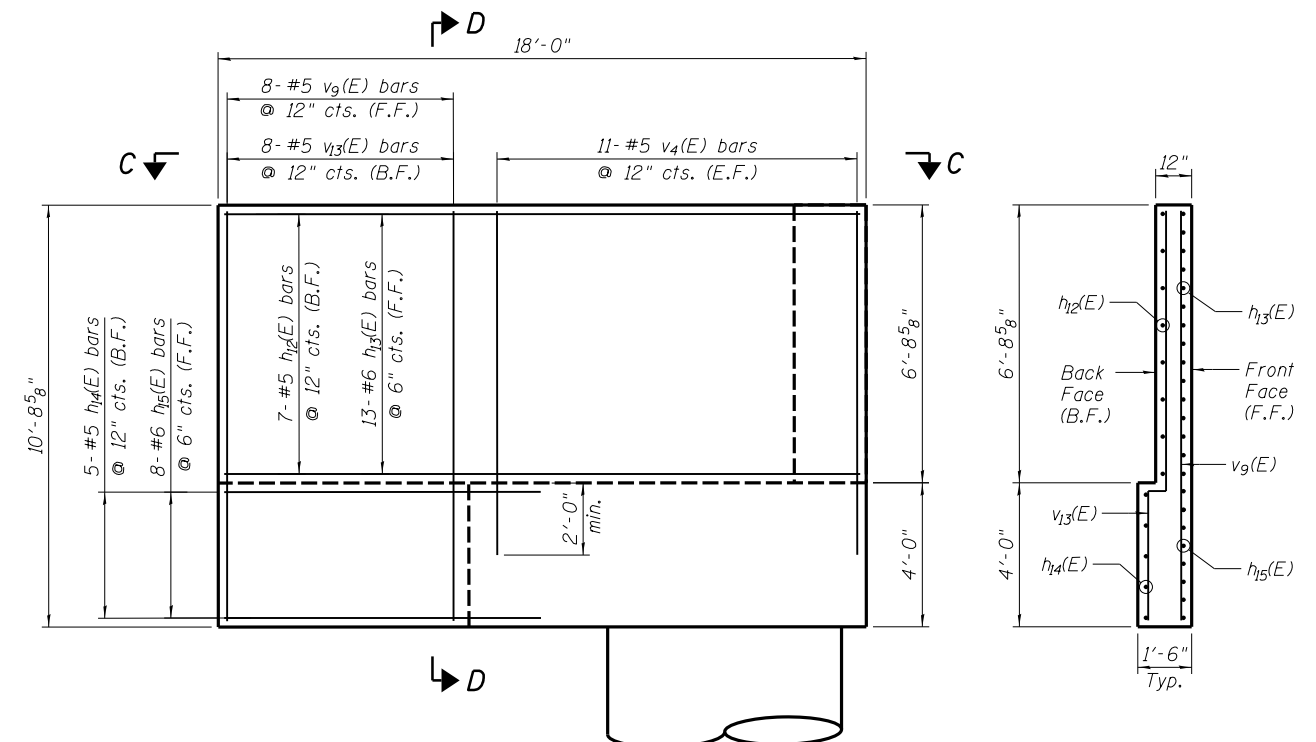


ELEVATION - SOUTH WING END VIEW

(Looking North)



WINGWALL SECTION B-B



ELEVATION - NORTH CHEEK END VIEW

(Looking South)

CHEEK WALL SECTION D-D

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FILE NAME =	USER NAME = Pop02275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
		DRAWN - RSJ	REVISED -
		CHECKED - MJW	REVISED -
PLOT SCALE = 0:2.0000' = 1" / in.			
PLOT DATE = 4/11/2019			

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**WEST ABUTMENT DETAILS
STRUCTURE 084-9961 - 5TH ST NSRR**

SHEET NO. 24 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	211
CONTRACT NO. 93733				

•666 & 666 ALT. | ILLINOIS FED. AID PROJECT

FINAL



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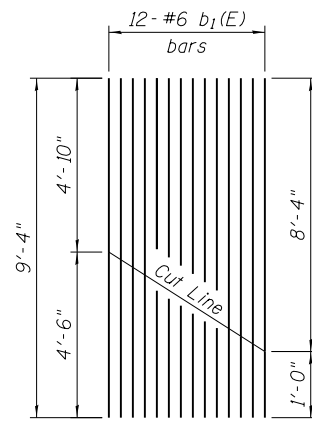
**BILL OF MATERIAL
WEST ABUTMENT**

Bar	No.	Size	Length	Shape
a ₁ (E)	8	#6	11'-8"	—
a ₂ (E)	60	#6	13'-8"	—
b ₁ (E)	48	#6	9'-4"	—
h ₁ (E)	24	#5	20'-11"	—
h ₂ (E)	12	#5	10'-8"	—
h ₃ (E)	16	#5	5'-1"	—
h ₄ (E)	24	#5	5'-0"	—
h ₅ (E)	4	#5	8'-9"	—
h ₆ (E)	4	#5	3'-7"	—
h ₇ (E)	5	#5	19'-8"	—
h ₈ (E)	9	#6	19'-8"	—
h ₉ (E)	6	#5	10'-1"	—
h ₁₀ (E)	13	#6	11'-1"	—
h ₁₁ (E)	20	#5	5'-11"	—
h ₁₂ (E)	7	#5	17'-8"	—
h ₁₃ (E)	13	#6	17'-8"	—
h ₁₄ (E)	5	#5	8'-11"	—
h ₁₅ (E)	8	#6	9'-2"	—
p ₁ (E)	52	#8	54'-9"	—
p ₂ (E)	20	#5	28'-6"	—
s ₁ (E)	66	#6	21'-0"	□
s ₂ (E)	21	#6	26'-10"	□
sp ₁	5	#6	*34'-0"	⋈
u ₁ (E)	16	#5	7'-9"	—
u ₂ (E)	6	#5	10'-7"	—
u ₃ (E)	5	#5	16'-6"	—
u ₄ (E)	5	#5	15'-2"	—
u ₅ (E)	19	#5	3'-4"	—
u ₆ (E)	2	#6	11'-0"	—
u ₇ (E)	2	#6	11'-10"	—
u ₈ (E)	2	#6	12'-10"	—
u ₉ (E)	2	#6	13'-10"	—
u ₁₀ (E)	2	#6	14'-10"	—
u ₁₁ (E)	2	#6	15'-10"	—
u ₁₂ (E)	4	#6	16'-10"	—
u ₁₃ (E)	44	#6	7'-5"	—
v ₁	160	#18	36'-11"	—
v ₂ (E)	34	#5	7'-1"	—
v ₃ (E)	34	#6	8'-4"	—
v ₄ (E)	80	#5	8'-7"	—
v ₅ (E)	20	#5	5'-9"	—
v ₆ (E)	20	#5	4'-8"	—
v ₇ (E)	2	#5	16'-6"	—
v ₈ (E)	22	#5	7'-6"	—
v ₉ (E)	8	#5	10'-3"	—
v ₁₀ (E)	2	#5	7'-0"	—
v ₁₁ (E)	28	#6	4'-3"	—
v ₁₂ (E)	4	#6	11'-1"	—
v ₁₃ (E)	8	#5	10'-9"	—
v ₁₇ (E)	2	#5	4'-3"	—
Structure Excavation		Cu. Yds.	114	
Concrete Structures		Cu. Yds.	127.7	
Drilled Shaft in Soil		Cu. Yds.	121.2	
Drilled Shaft in Rock		Cu. Yds.	78.5	
Reinforcement Bars		Pound	98,360	
Reinforcement Bars, Epoxy Coated		Pound	19,010	

* Length is height of spiral.

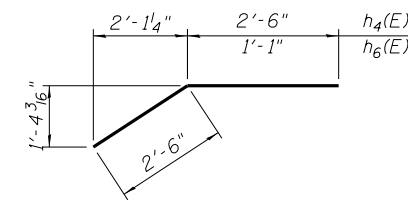
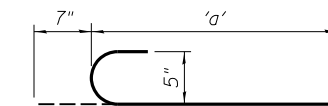
MIN. BAR LAPS FOR SPIRALS

#6 Bars = 2'-7"

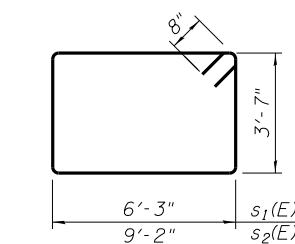


BARS h₂(E), h₃(E) & h₅(E)

Bar	'a'
h ₂ (E)	10'-1"
h ₃ (E)	4'-6"
h ₅ (E)	8'-2"



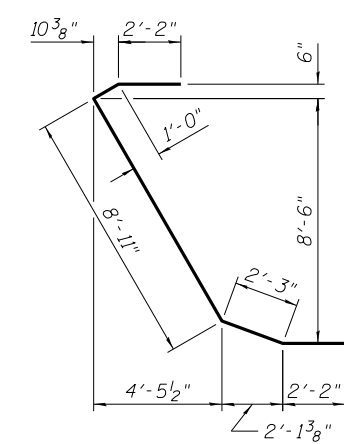
BARS h₄(E) & h₆(E)



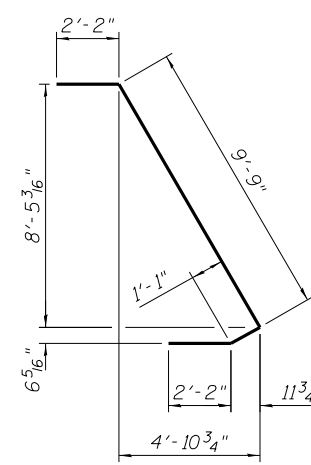
BAR s₁(E) & s₂(E)

BAR CUTTING DIAGRAM FOR b₁(E)

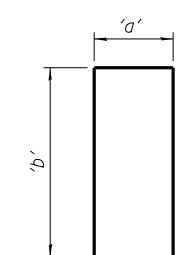
Order b₁(E) full length. Cut as shown and use remainder of bars in opposite face.



BAR u₃(E)

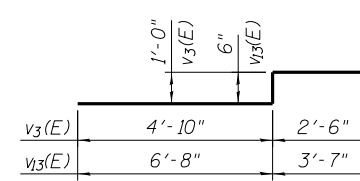


BAR u₄(E)

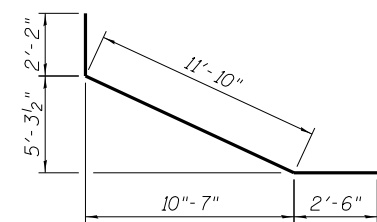


Bar	'a'	'b'
u ₁ (E)	3'-5"	2'-2"
u ₂ (E)	3'-7"	3'-6"
u ₅ (E)	1'-8"	0'-10"
u ₆ (E)	1'-0"	5'-0"
u ₇ (E)	1'-0"	5'-5"
u ₈ (E)	1'-0"	5'-11"
u ₉ (E)	1'-0"	6'-5"
u ₁₀ (E)	1'-0"	6'-11"
u ₁₁ (E)	1'-0"	7'-5"
u ₁₂ (E)	1'-0"	7'-11"

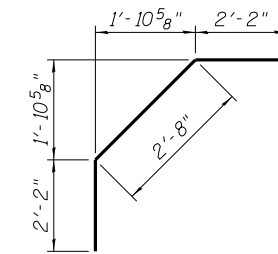
**BARS u₁(E), u₂(E), u₅(E), u₆(E)
u₇(E), u₈(E), u₉(E), u₁₀(E), u₁₁(E), u₁₂(E)**



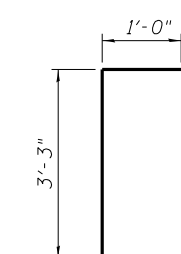
BARS v₃(E) & v₁₃(E)



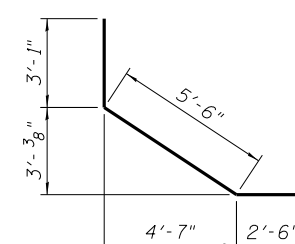
BAR v₇(E)



BARS v₁₀(E)



BAR v₁₁(E)

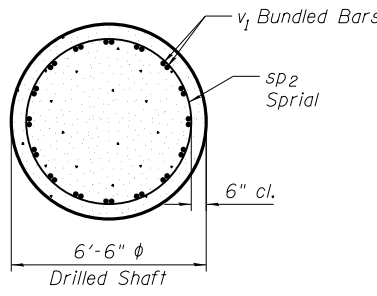


BARS v₁₂(E)

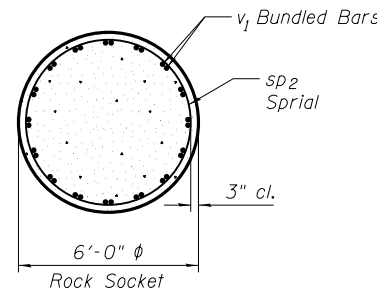
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PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	212
CONTRACT NO.			93733	
*666 & 666 ALT.		ILLINOIS FED. AID PROJECT		

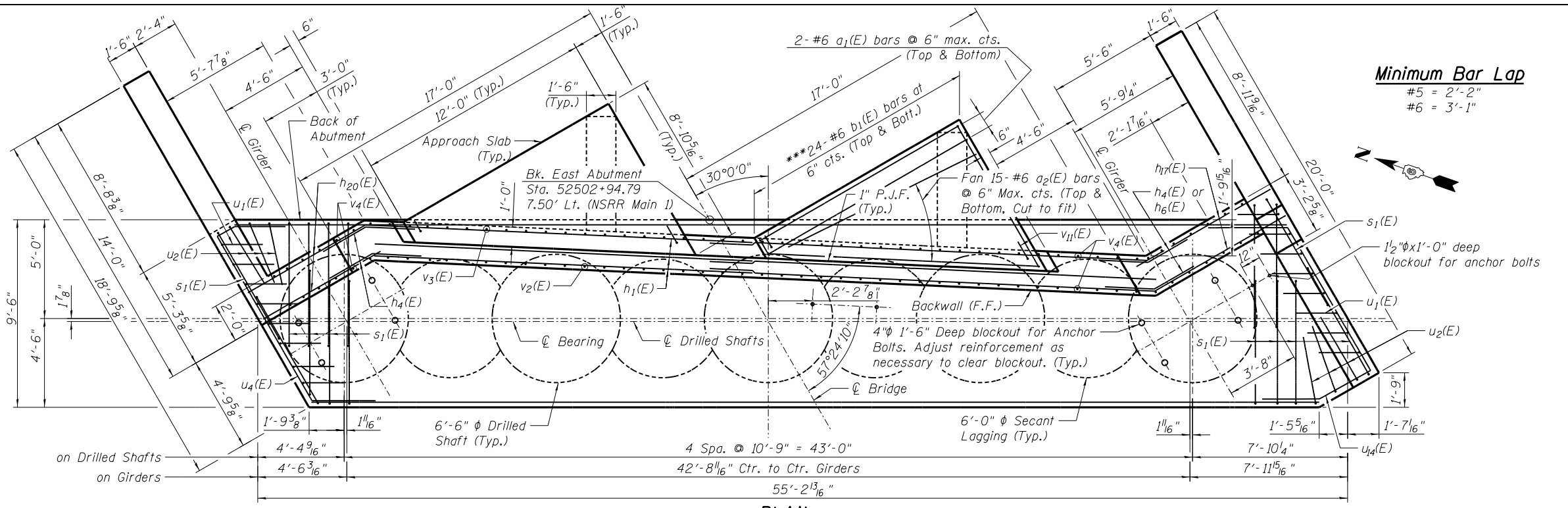




SECTION B-B

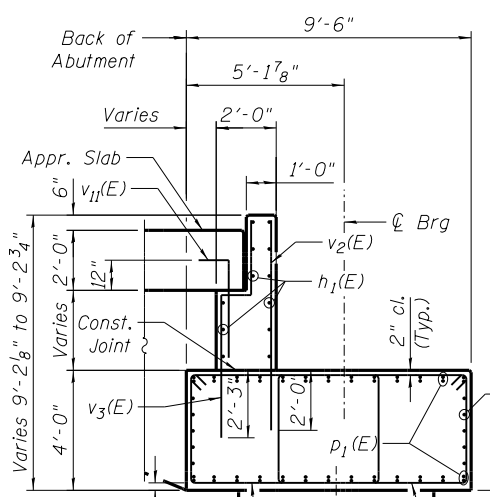


SECTION C-C



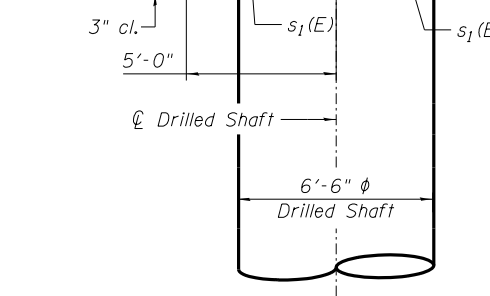
Minimum Bar Lap
 #5 = 2'-2"
 #6 = 3'-1"

PLAN



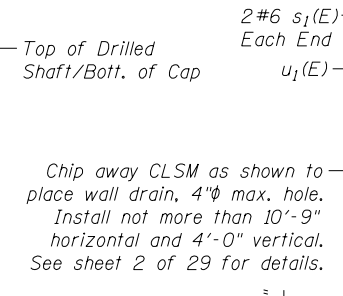
SECTION A-A

(At Rt. L's to Bk. of Abut.)



BLOCKOUT LAYOUT

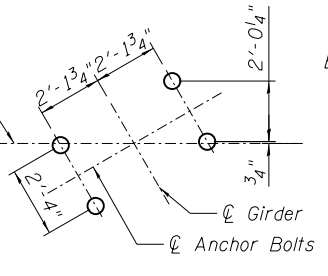
(At Floorbeam Location)



BLOCKOUT LAYOUT

(At Girders Locations)

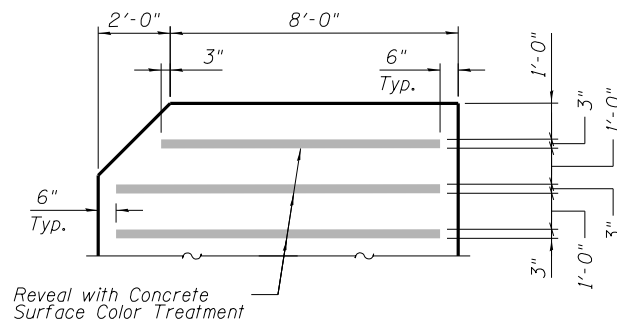
Chip away CLSM as shown to place wall drain, 4" max. hole. Install not more than 10'-9" horizontal and 4'-0" vertical. See sheet 2 of 29 for details.



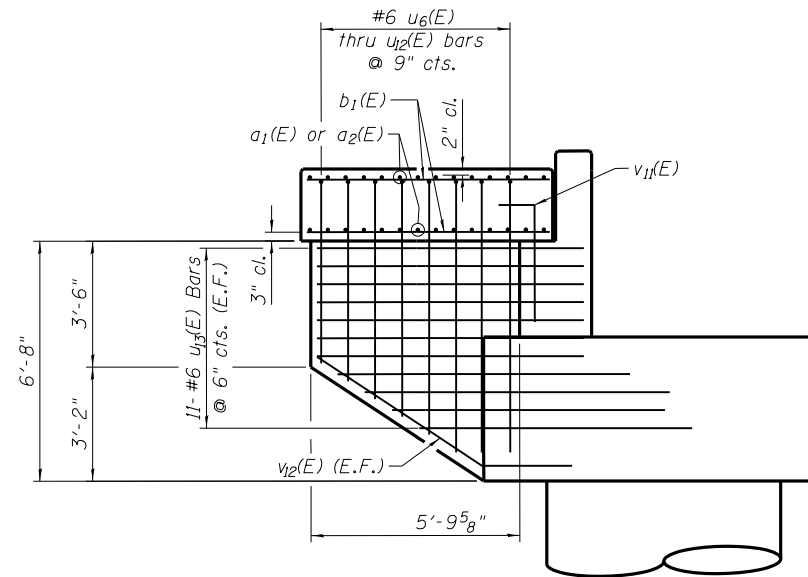
ELEVATION - EAST ABUTMENT
(Looking East)

* The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.
 ** Provide 1/2 extra turns at top and bottom of each drilled shaft.
 *** Order 1 b (E) bars full length. Cut to fit skew and use remainder of bars in opposite face.

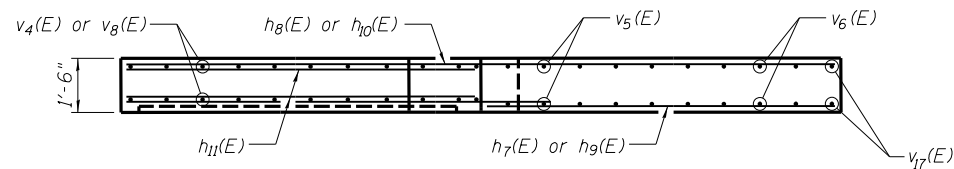
FINAL Copyright Hanson Professional Services Inc, 2019	USER NAME = Pop00275 PLOT SCALE = @2.0000 "/>	DESIGNED - MJW CHECKED - TJH/TDP DRAWN - RSJ CHECKED - MJW	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	EAST ABUTMENT STRUCTURE 084-9961 - 5TH ST NSRR	F.A.P. R.T.E. = SECTION = (109) VB,(110) VB-5 COUNTY = SANGAMON CONTRACT NO. = 93733	TOTAL SHEETS = 382 SHEET NO. = 213
	SHEET NO. 26 OF 29 SHEETS		ILLINOIS FED. AID PROJECT				
	SHEET NO. 26 OF 29 SHEETS		ILLINOIS FED. AID PROJECT				
	SHEET NO. 26 OF 29 SHEETS		ILLINOIS FED. AID PROJECT				



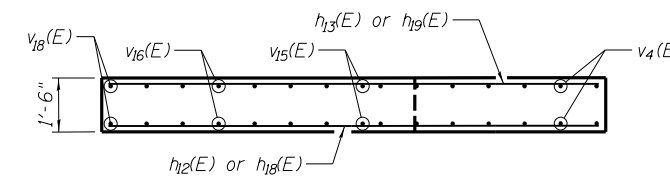
CONCRETE REVEAL DETAIL



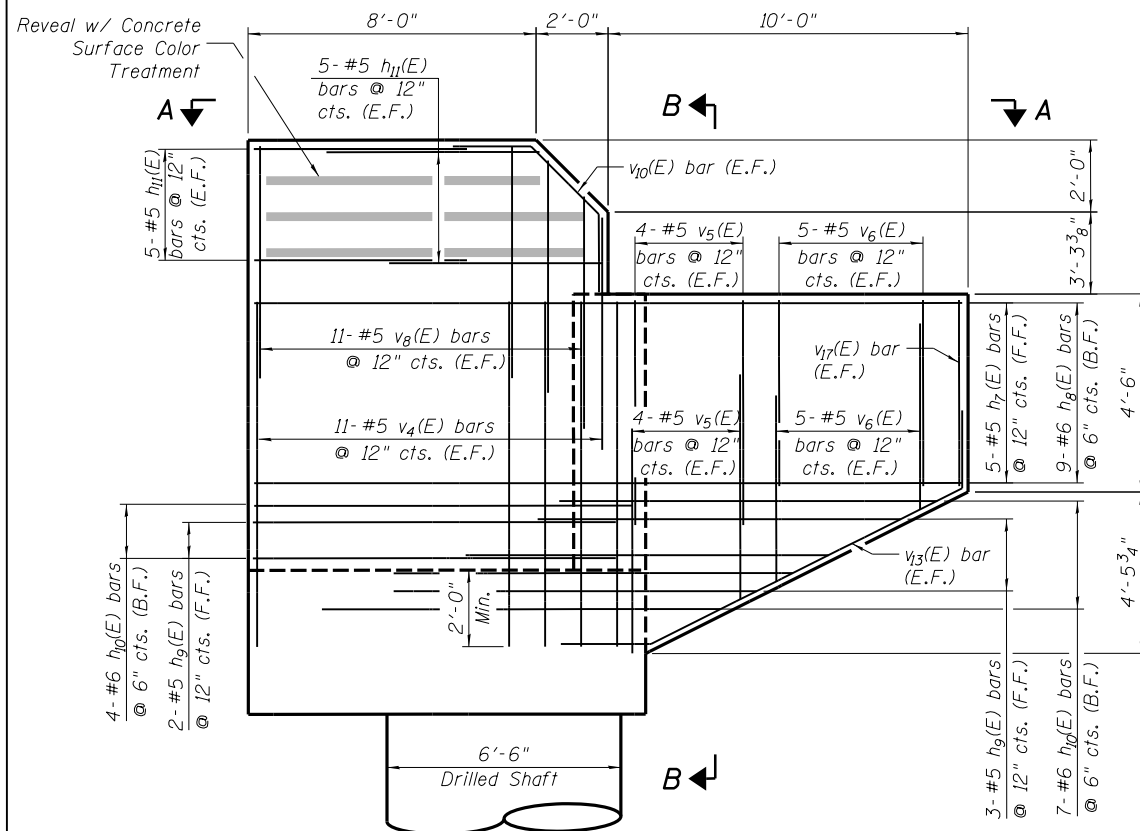
APPROACH SLAB SECTION
(Horizontal Dimensions at Rt. L's to back of abutment.)



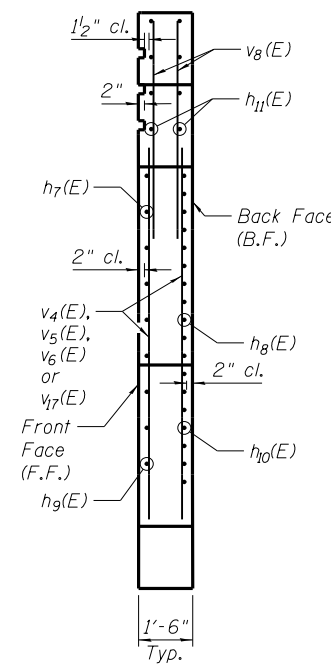
SECTION A-A - PLAN VIEW



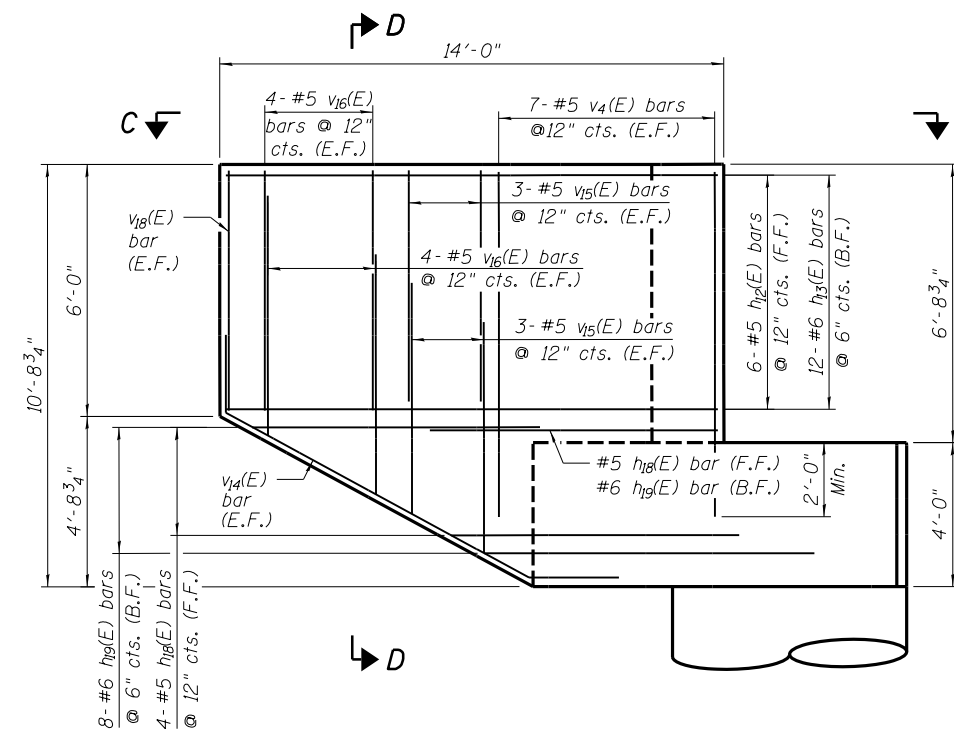
SECTION C-C - PLAN VIEW



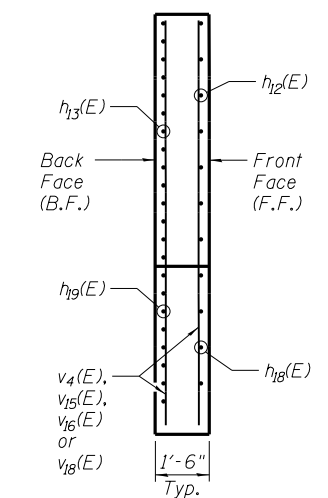
ELEVATION - SOUTH WING END VIEW
(Looking North)



WINGWALL SECTION B-B



ELEVATION - NORTH WING END VIEW
(Looking South)



WINGWALL SECTION D-D

\\sp1\svr\306\hanson\domit\hanson_projects\Documents\09\Jobs\09101798\CAD\Struct\5th\Sheet\0849961-09101798-NSRR-001

FILE NAME =	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
		DRAWN - RSJ	REVISED -
		CHECKED - MJW	REVISED -
PLOT SCALE = 0:2.0000 '1" =			
PLOT DATE = 4/11/2019			

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**EAST ABUTMENT DETAILS
STRUCTURE 084-9961 - 5TH ST NSRR**

SHEET NO. 27 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	214
			CONTRACT NO. 93733	

•666 & 666 ALT. ILLINOIS FED. AID PROJECT

FINAL



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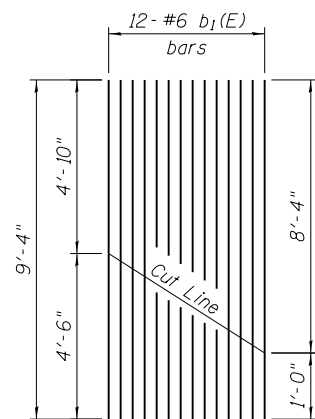
**BILL OF MATERIAL
EAST ABUTMENT**

Bar	No.	Size	Length	Shape
a ₁ (E)	8	#6	11'-8"	—
a ₂ (E)	60	#6	13'-8"	—
b ₁ (E)	48	#6	9'-4"	—
h ₁ (E)	24	#5	20'-11"	—
h ₄ (E)	24	#5	5'-0"	—
h ₆ (E)	4	#5	3'-7"	—
h ₇ (E)	5	#5	19'-8"	—
h ₈ (E)	9	#6	19'-8"	—
h ₉ (E)	5	#5	10'-1"	—
h ₁₀ (E)	11	#6	11'-1"	—
h ₁₁ (E)	20	#5	5'-11"	—
h ₁₂ (E)	6	#5	13'-8"	—
h ₁₃ (E)	12	#6	13'-8"	—
h ₁₆ (E)	12	#5	7'-6"	—
h ₁₇ (E)	16	#5	8'-2"	—
h ₁₈ (E)	5	#5	8'-0"	—
h ₁₉ (E)	9	#6	9'-2"	—
h ₂₀ (E)	4	#5	5'-5"	—
p ₁ (E)	52	#8	54'-9"	—
p ₂ (E)	20	#5	28'-6"	—
s ₁ (E)	66	#6	21'-0"	□
s ₂ (E)	21	#6	26'-10"	□
SD ₂	5	#6	*34'-5"	⋈
u ₁ (E)	16	#5	7'-9"	—
u ₂ (E)	6	#5	10'-7"	—
u ₄ (E)	5	#5	15'-2"	—
u ₅ (E)	19	#5	3'-4"	—
u ₆ (E)	2	#6	11'-0"	—
u ₇ (E)	2	#6	11'-10"	—
u ₈ (E)	2	#6	12'-10"	—
u ₉ (E)	2	#6	13'-10"	—
u ₁₀ (E)	2	#6	14'-10"	—
u ₁₁ (E)	2	#6	15'-10"	—
u ₁₂ (E)	4	#6	16'-10"	—
u ₁₃ (E)	44	#6	7'-5"	—
u ₁₄ (E)	5	#5	21'-1"	—
v ₁	160	#18	36'-11"	—
v ₂ (E)	34	#5	7'-1"	—
v ₃ (E)	34	#6	8'-4"	—
v ₄ (E)	74	#5	8'-7"	—
v ₅ (E)	16	#5	5'-9"	—
v ₆ (E)	18	#5	4'-8"	—
v ₈ (E)	22	#5	7'-6"	—
v ₁₀ (E)	2	#5	7'-0"	—
v ₁₁ (E)	28	#6	4'-3"	—
v ₁₂ (E)	4	#6	11'-1"	—
v ₁₃ (E)	2	#5	14'-4"	—
v ₁₄ (E)	2	#5	14'-3"	—
v ₁₅ (E)	12	#5	5'-11"	—
v ₁₆ (E)	18	#5	6'-2"	—
v ₁₇ (E)	2	#5	4'-3"	—
v ₁₈ (E)	2	#5	5'-9"	—
Structure Excavation		Cu. Yds.	114	
Concrete Structures		Cu. Yds.	127.2	
Drilled Shaft in Soil		Cu. Yds.	124.1	
Drilled Shaft in Rock		Cu. Yds.	78.5	
Secant Lagging		Cu. Ft.	2,283	
Reinforcement Bars		Pound	98,600	
Reinforcement Bars, Epoxy Coated		Pound	18,820	

* Length is height of spiral.

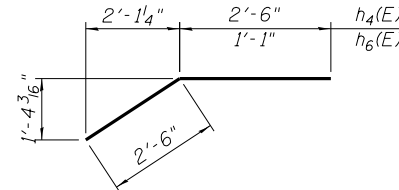
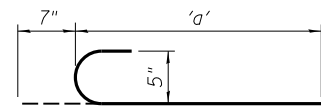
MIN. BAR LAPS FOR SPIRALS

#6 Bars = 2'-7"

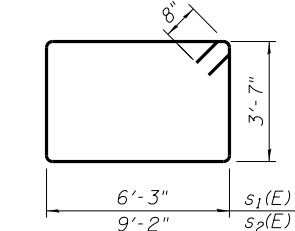


BARS h₁₆(E), h₁₇(E) & h₂₀(E)

Bar	'a'
h ₁₆ (E)	6'-11"
h ₁₇ (E)	7'-7"
h ₂₀ (E)	4'-10"



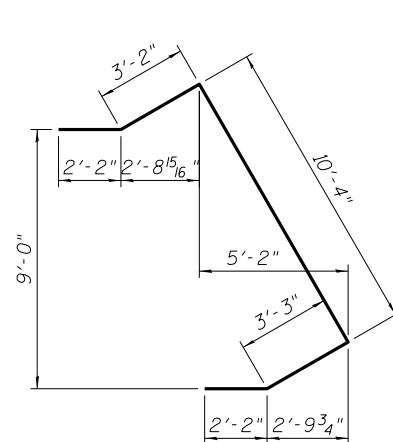
BARS h₄(E) & h₆(E)



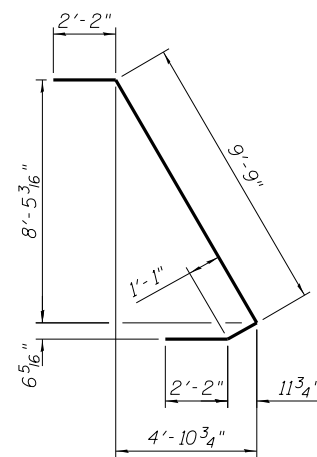
BAR s₁(E) & s₂(E)

BAR CUTTING DIAGRAM FOR b₁(E)

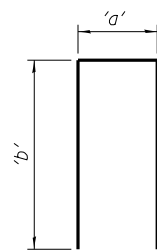
Order b₁(E) full length. Cut as shown and use remainder of bars in opposite face.



BAR u₄(E)

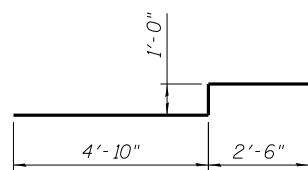


BAR u₄(E)

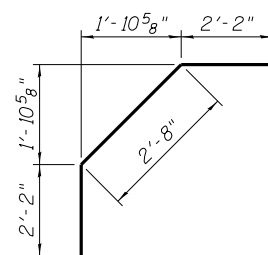


BARS u₁(E), u₂(E), u₅(E), u₆(E), u₇(E), u₈(E), u₉(E), u₁₀(E), u₁₁(E), u₁₂(E)

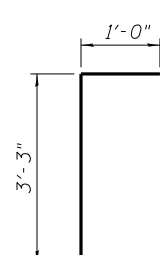
Bar	'a'	'b'
u ₁ (E)	3'-5"	2'-2"
u ₂ (E)	3'-7"	3'-6"
u ₅ (E)	1'-8"	0'-10"
u ₆ (E)	1'-0"	5'-0"
u ₇ (E)	1'-0"	5'-5"
u ₈ (E)	1'-0"	5'-11"
u ₉ (E)	1'-0"	6'-5"
u ₁₀ (E)	1'-0"	6'-11"
u ₁₁ (E)	1'-0"	7'-5"
u ₁₂ (E)	1'-0"	7'-11"



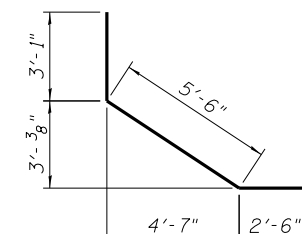
BAR v₃(E)



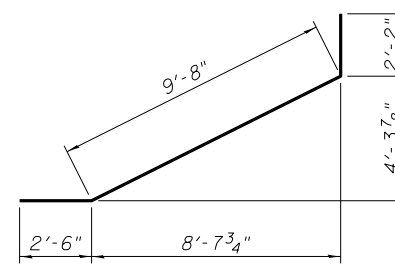
BARS v₁₀(E)



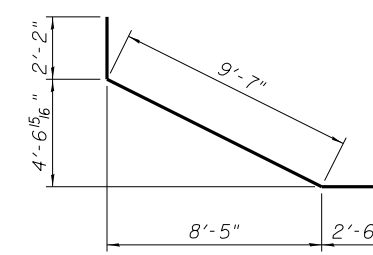
BAR v₁₁(E)



BARS v₁₂(E)

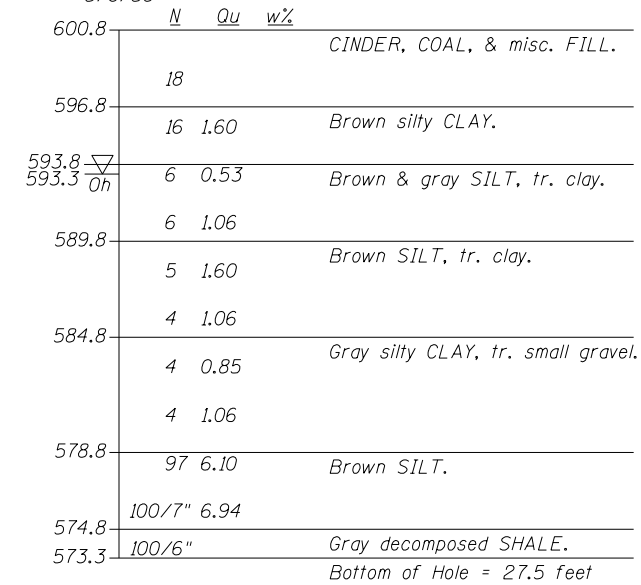


BAR v₁₃(E)

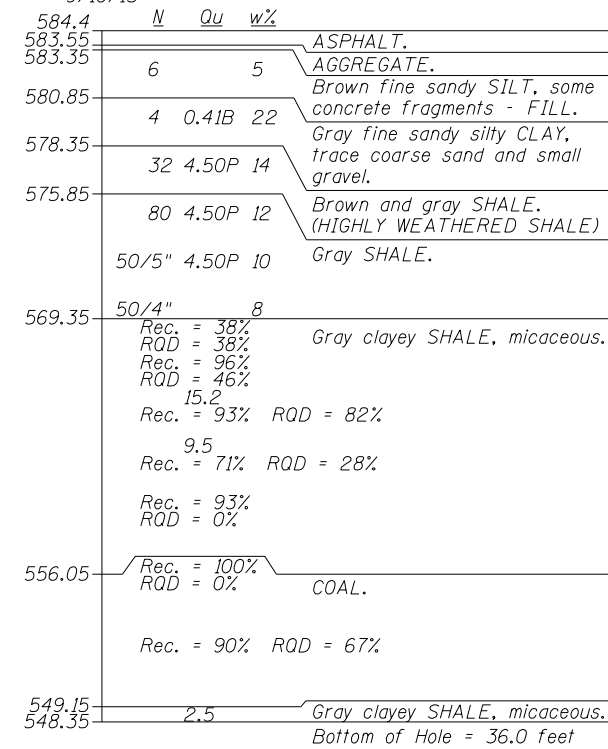


BAR v₁₄(E)

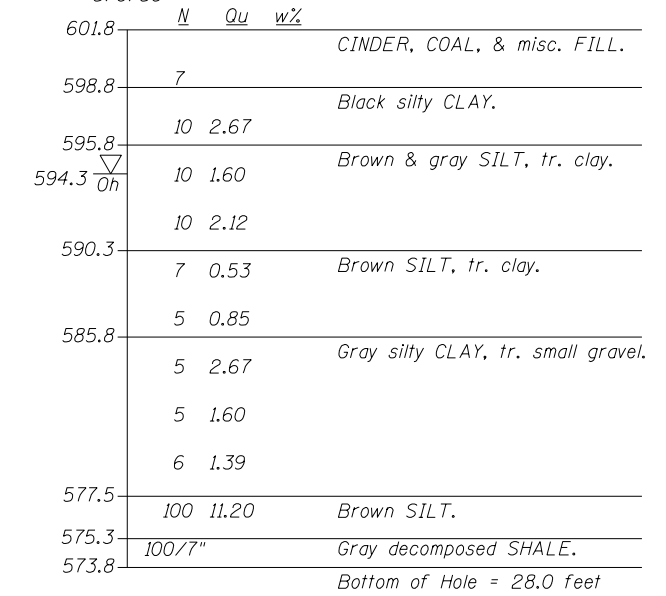
B-3
Sta. 999+30, 27' LT
5/6/58



B-147
Sta. 100+21, 20' LT
9/10/13



B-1
Sta. 1000+06, 27' RT
5/6/58



LEGEND

N Standard Penetration Test N (blows/ft)
Qu Unconfined Strength (tsf)
w% Natural Moisture Content (%)

DD Water Surface Elevation Encountered in Boring
558.10 DD = during drilling
Oh = at completion
24h = 24 hours after completion

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FILE NAME =	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
		DRAWN - RSJ	REVISED -
		CHECKED - MJW	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUBSURFACE DATA PROFILE
STRUCTURE 084-9961 - 5TH ST NSRR

SHEET NO. 29 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	216
CONTRACT NO. 93733				

*666 & 666 ALT. ILLINOIS FED. AID PROJECT

FINAL

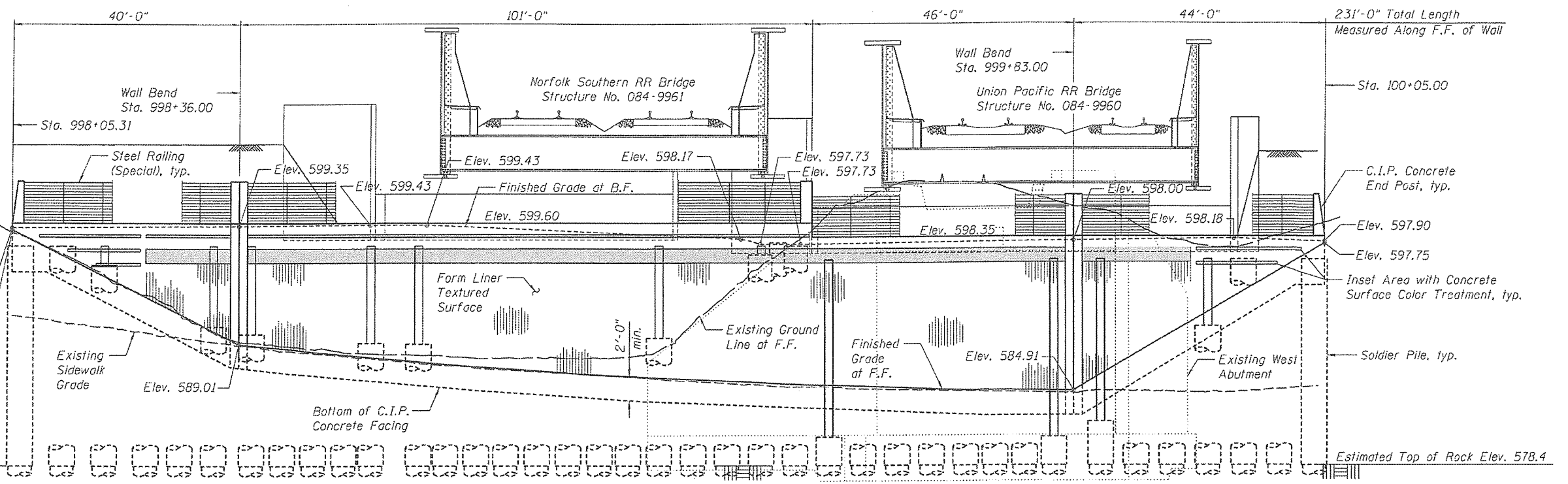


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Benchmark:
 BM# D2218-07 - Chiseled 'X' on West Bolt of fire hydrant - SE Quad 6th Street and Wellesly Avenue.
 Elevation = 598.884

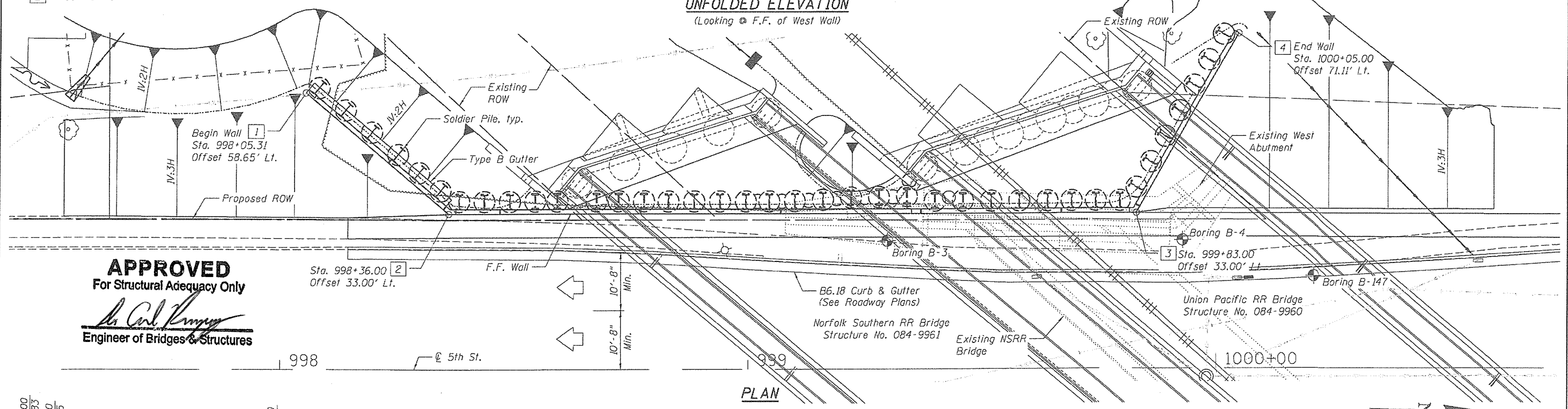
Existing Structure: SM 084-0032 - Built in 1961 under 109-15B. Steel through plate girder structure supported on closed abutments. Bk. to Bk. Abutment length is 95'-3" and ctr. to ctr. through girder width of 18'-6". Structure to be Removed and Replaced.

Construction Sequence: See Sheet 3 of 17
 Traffic Control: Temporary Lane Closures
 Salvage: None

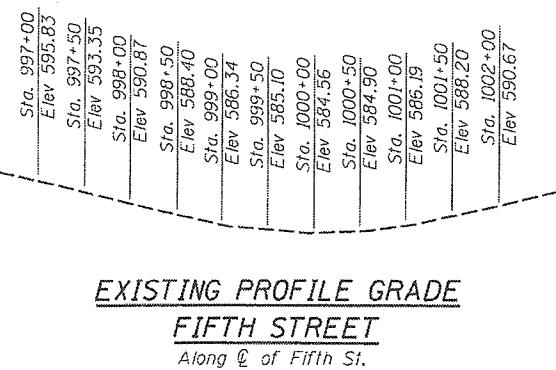


Note: Wall offsets are measured from @ 5th Street to the front face of C.I.P. Facing.

F.F. - Front Face
 B.F. - Back Face
 [2] - Control Point



APPROVED
 For Structural Adequacy Only
As Carl Krueger
 Engineer of Bridges & Structures

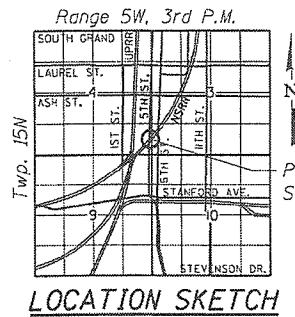


ROBERT CHANTONE
 LICENSED STRUCTURAL ENGINEER
 STATE OF ILLINOIS
 081-000949
Robert Chantone
 Seal 10, 2019
 Expires November 30, 2020

I certify that to the best of my knowledge, information and belief, this retaining wall design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AREMA Specifications.

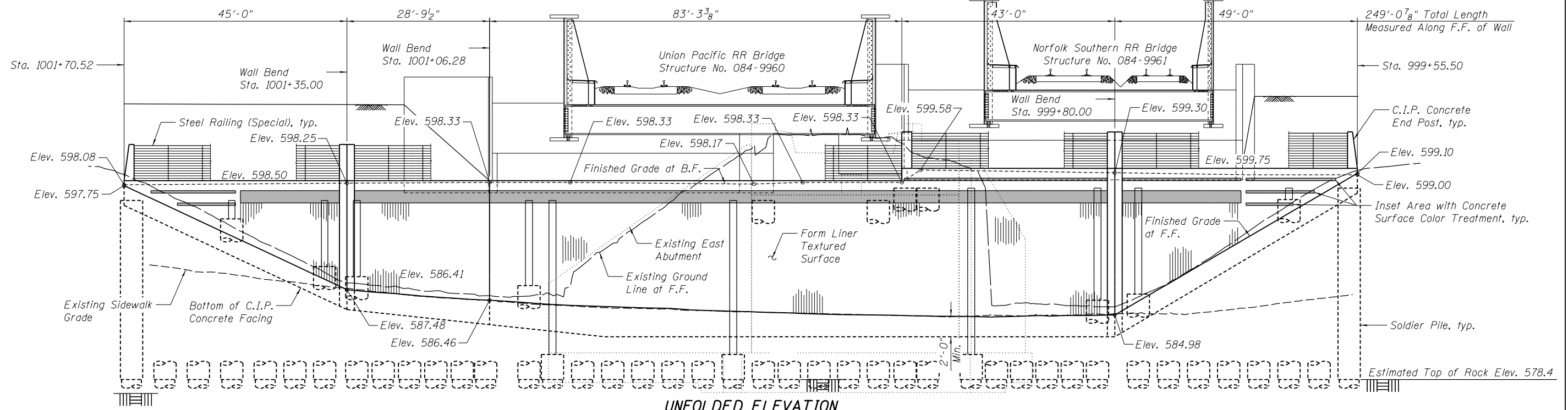
DESIGN SPECIFICATIONS
 2017 AREMA Specifications

DESIGN STRESSES
 FIELD UNITS
 $f'_c = 4,000$ psi
 $f_y = 60,000$ psi (Reinforcement)
 $f_y = 50,000$ psi (M270 Grade 50)

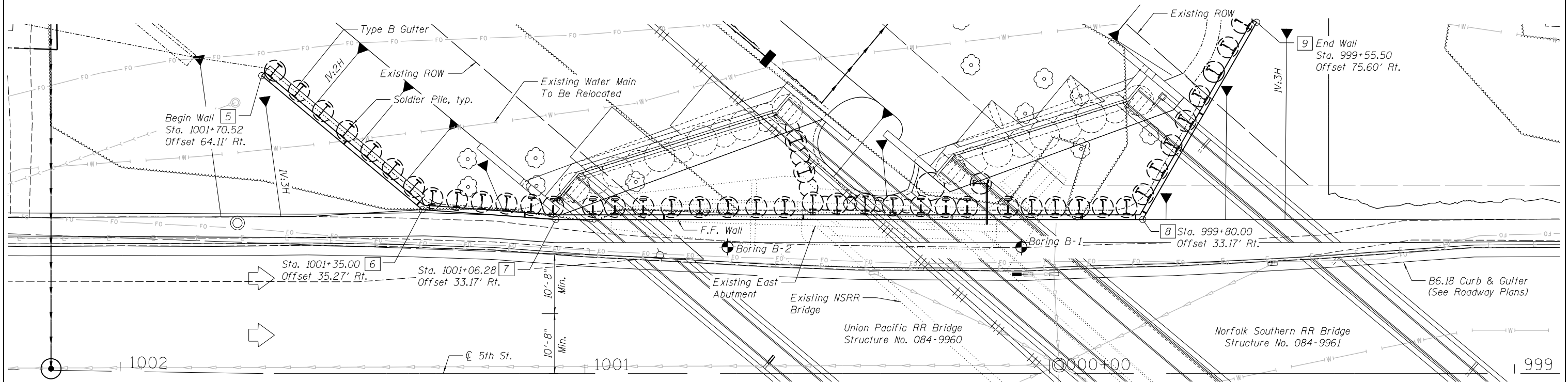


WEST WALL GENERAL PLAN & ELEVATION
 5TH ST. RETAINING WALLS
 F.A.P. 666 ALT.-SECTION (109)VB, (110)VB-5
 SANGAMON COUNTY
 STATION 998+05.31 TO 1001+70.52

HANSON USER NAME = Pop0275 PLOT SCALE = 0.1667' / 1" = PLOT DATE = 4/18/2019	DESIGNED - RGC	REVISIONS	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL PLAN & ELEVATION - WEST WALL RETAINING WALLS - 5TH STREET SHEET NO. 1 OF 17 SHEETS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	CHECKED - KMS	REVISIONS			109) VB, (110) VB-5	SANGAMON	382	217	
	DRAWN - EJM	REVISIONS			CONTRACT NO. 93733				
	CHECKED - RGC	REVISIONS			ILLINOIS FED. AID PROJECT				



UNFOLDED ELEVATION
(Looking @ F.F. of East Wall)



PLAN

Note: Wall offsets are measured from \odot 5th Street to the front face of C.I.P. Facing.

F.F. - Front Face
B.F. - Back Face

\square - Control Point

EAST WALL GENERAL PLAN & ELEVATION
5TH ST. RETAINING WALLS
F.A.P. 666 ALT.-SECTION (109)VB, (110)VB-5
SANGAMON COUNTY
STATION 998+05.31 TO 1001+70.52

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USER NAME = Pop02275	DESIGNED - RGC	REVISD -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISD -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISD -
	CHECKED - RGC	REVISD -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL PLAN & ELEVATION - EAST WALL
RETAINING WALLS - 5TH STREET

SHEET NO. 2 OF 17 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB, (110) VB-5	SANGAMON	382	218
			CONTRACT NO. 93733	
ILLINOIS FED. AID PROJECT				

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WALL CONTROL POINTS

Control Point	Station	Offset
1	998+05.31	58.65' LT
2	998+36.00	33.00' LT
3	999+83.00	33.00' LT
4	1000+05.00	71.11' LT
5	1001+70.52	64.11' RT
6	1001+35.00	35.27' RT
7	1001+06.28	33.17' RT
8	999+80.00	33.17' RT
9	999+55.50	75.60' RT

Control Points are to Front Face of C.I.P. Facing.

GENERAL NOTES

1. Reinforcement bars designated (E) shall be epoxy coated.
2. All substructure concrete shall have a compressive strength of 4,000 psi at 14 days.
3. The Contractor is responsible for the design and performance of the Untreated Timber Lagging using no less than a 3 in. nominal rough-sawn thickness and timber with a minimum allowable bending stress of 1000 psi.

INDEX OF SHEETS

1. General Plan & Elevation - West Wall
2. General Plan & Elevation - East Wall
3. General Data
4. Typical Sections
5. Typical Sections
6. Soldier Piles - West Wall
7. Soldier Piles - East Wall
8. Concrete Facing - West Wall
9. Concrete Facing - West Wall
10. Concrete Facing - East Wall
11. Concrete Facing - East Wall
12. Concrete Facing Details
13. Concrete Facing Details
14. Railing Details
15. Railing Details
16. Slope Wall Details
17. Subsurface Data Profile

CONSTRUCTION SEQUENCE

Stage 1: Maintain rail traffic on existing track.

- Item 4: NSRR Bridge and south ends of retaining walls
- a. Drill and place the Secant Lagging to existing ground surface at East Abutment and West Retaining Wall, south of Soldier Pile 24.
 - b. Install drilled shafts for the East Abutment, forming above existing ground as required.
 - c. Drill and set Temporary Soldier Pile C in front of new East Abutment.
 - d. Install timber lagging between Temporary Soldier Pile C and back of Existing East Abutment while excavating south wingwall. Use abutment drilled shafts and secant lagging to retain RR embankment.
 - e. Remove conflicting portions of the existing East Abutment's south wingwall stem.
 - f. Drill and set Soldier Piles 29-42 of the East Retaining Wall and Soldier Piles 1-23 of the West Retaining Wall. Drill through footings of existing wingwalls as required.
 - g. Install timber lagging between Temporary Soldier Pile C and Soldier Pile 29, Soldier Piles 29-42 of the West Retaining Wall, and Soldier Piles 1-18 of the West Retaining Wall while filling behind retaining walls to bottom of new abutments.
 - h. Install drilled shafts for the West Abutment.
 - i. Construct cast-in-place concrete abutments.
 - j. Install timber lagging while excavating in front of wall to bottom of facing.
 - k. Install pipe underdrain and cast-in-place concrete facing panels W1-W5 and E9-E10.
 - l. Place fill behind new abutments and between new abutments and retaining walls.
 - m. Set bridge superstructure during weekend closure of 5th Street.
 - n. Complete bridge construction, including roadway luminaires. Complete NSRR embankment and subballast placement.
 - o. NSRR places ballast and shifts tracks to Temporary NSRR Main 1 (outside position on new bridge).

Stage 4A: Maintain Rail traffic on Temporary NSRR Main 1.

- Item 5: Remove Existing NSRR Bridge and construct UPRR Bridge and north ends of retaining walls
- a. Remove existing bridge superstructure during weekend closure of 5th Street.
 - b. Drill and place the Secant Lagging to existing ground surface at both abutments and East Retaining Wall, north of Soldier Pile 26.
 - c. Drill and set Temporary Soldier Piles A and B, Soldier Piles 22-26 of the East Retaining Wall and Soldier Pile 24 of the West Retaining Wall. Drill through footings of existing abutments as required.
 - d. Install drilled shafts for the West and East Abutments, forming above existing ground as required.
 - e. Drill and set Soldier Piles 1-13 of the East Wall.
 - f. Remove conflicting portions of the existing bridge abutments. Use soldier piles, temporary soldier piles, abutment drilled shafts and secant lagging to retain RR embankment.
 - g. Drill and set Soldier Piles 14-21 and 27-28 of the East Wall and Soldier Piles 25-39 of the West Wall.
 - h. Install timber lagging while filling behind retaining walls to bottom of abutments. Abandon temporary soldier piles.
 - i. Construct cast-in-place concrete abutments.
 - j. Install timber lagging while excavating in front of wall to bottom of facing.
 - k. Install remainder of pipe underdrain and cast-in-place concrete facing.
 - l. Place fill behind new abutments and between new abutments and retaining walls.
 - m. Set bridge superstructure during weekend closure of 5th Street.
 - n. Complete bridge construction. Complete UPRR embankment and subballast placement.
 - o. NSRR installs tracks on NSRR Main 1 (inside position on new bridge).

Note: See Railroad Plans for stages and items not affecting these structures. See Roadway Plans and Special Provisions for 5th Street traffic control restrictions.

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Porous Granular Embankment	Cu. Yd.	1795
Structure Excavation	Cu. Yd.	477
Form Liner Textured Surface	Sq. Ft.	4364
Stud Shear Connectors	Each	739
Reinforcement Bars, Epoxy Coated	Pound	40110
Slope Wall 4 Inch	Sq. Yd.	300
Furnishing Soldier Piles (W-Section)	Foot	2923
Drilling and Setting Soldier Piles (in Soil)	Cu. Ft.	16274.9
Drilling and Setting Soldier Piles (in Rock)	Cu. Ft.	17041.0
Untreated Timber Lagging	Sq. Ft.	3951
Secant Lagging	Cu. Ft.	2219
Concrete Structures (Retaining Wall)	Cu. Yd.	268.3
Concrete Sealer	Sq. Ft.	6046
Geocomposite Wall Drain	Sq. Yd.	300
Concrete Gutter, Type B	Foot	82
Concrete Surface Color Treatment	Sq. Ft.	548
Steel Railing (Special)	Foot	456
Pipe Underdrains for Structures 4"	Foot	623

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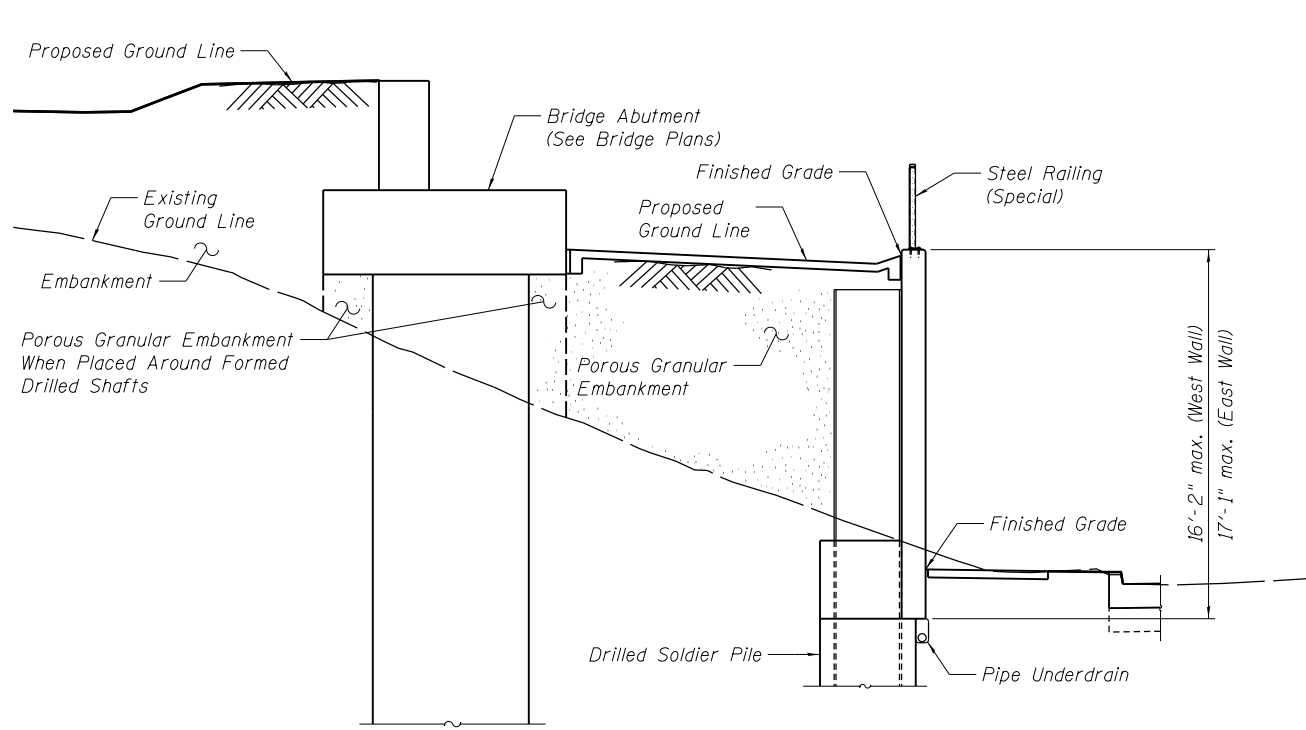
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	CHECKED - KMS	REVISED -
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PLOT DATE = 4/11/2019	CHECKED - RGC	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

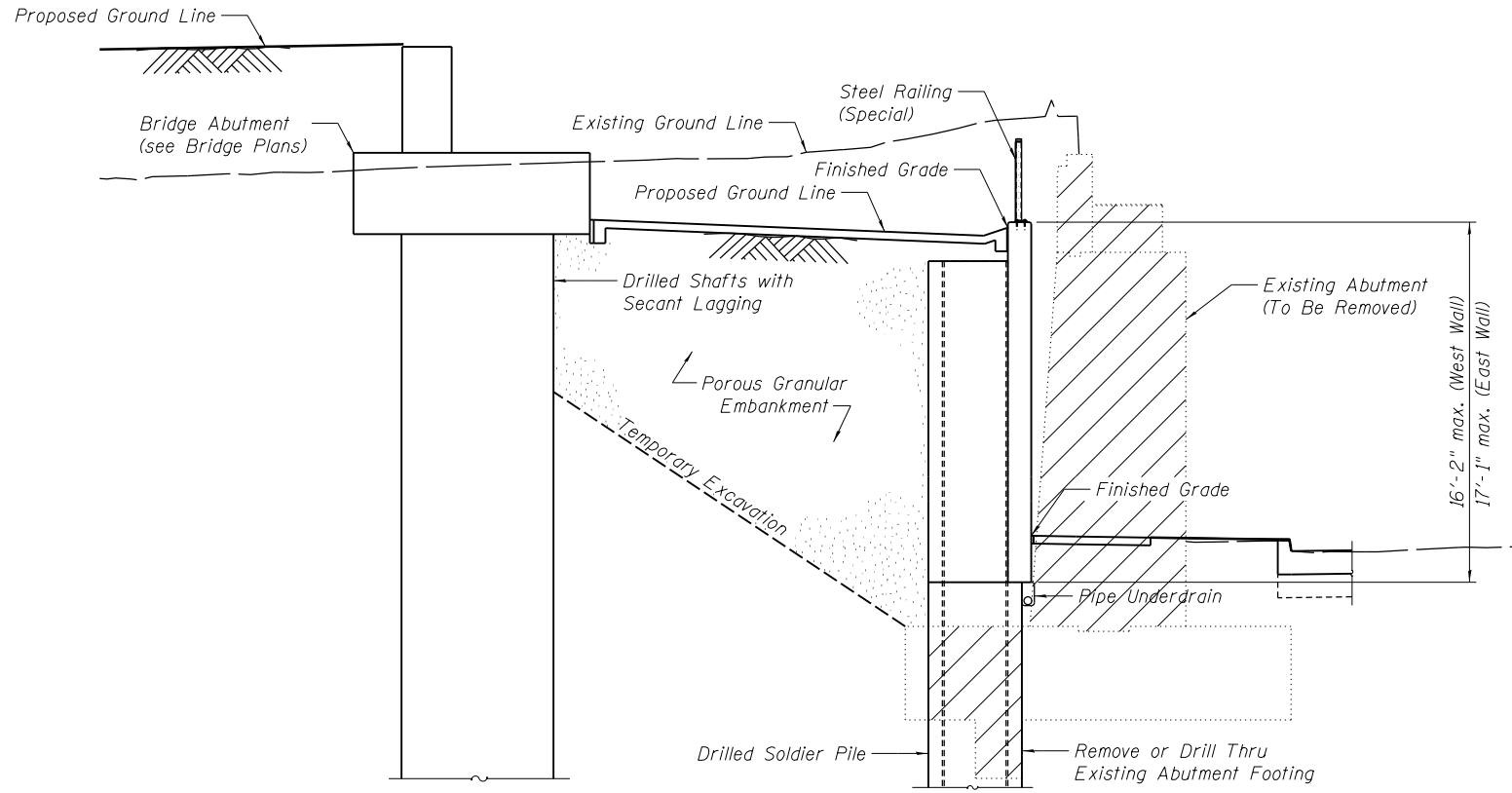
**GENERAL DATA
RETAINING WALLS - 5TH STREET**

SHEET NO. 3 OF 17 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	219
CONTRACT NO.			93733	
ILLINOIS FED. AID PROJECT				

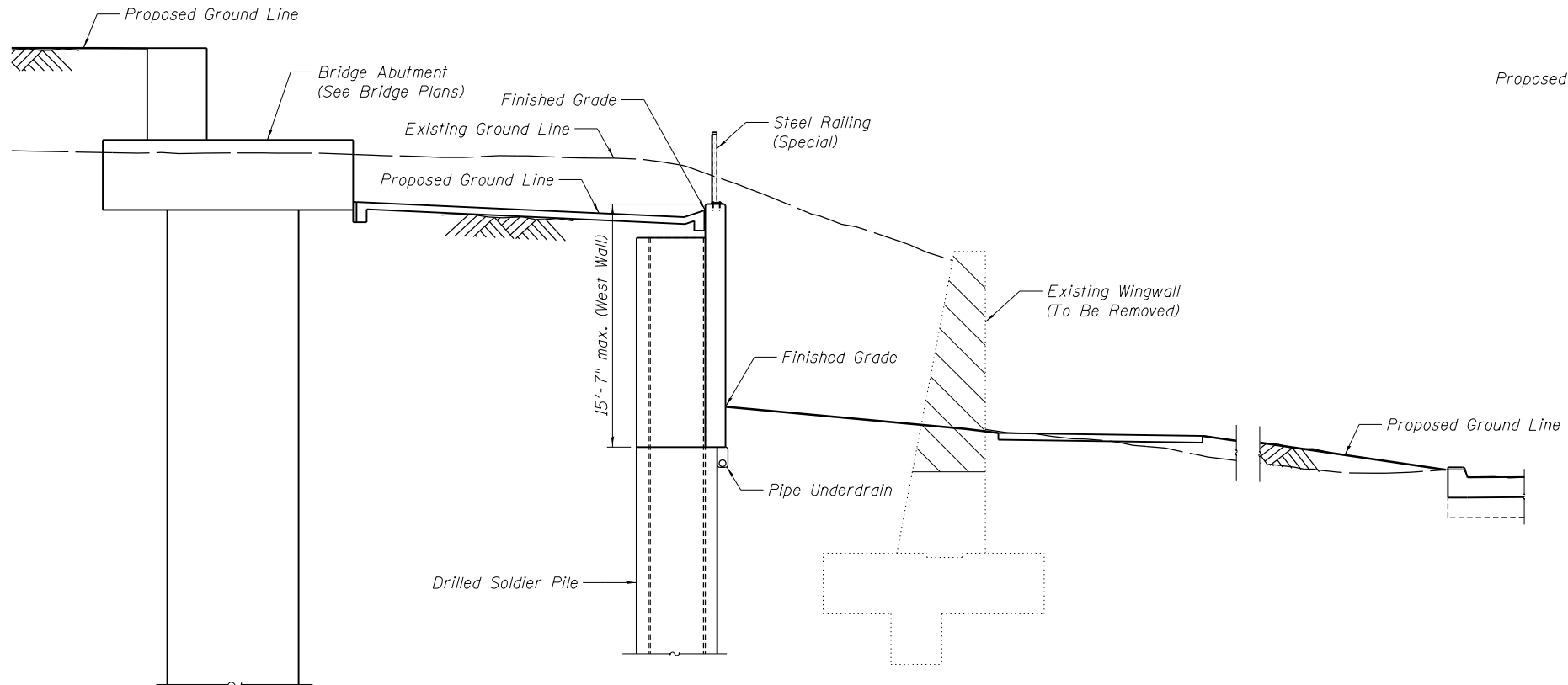


TYPICAL WALL SECTION



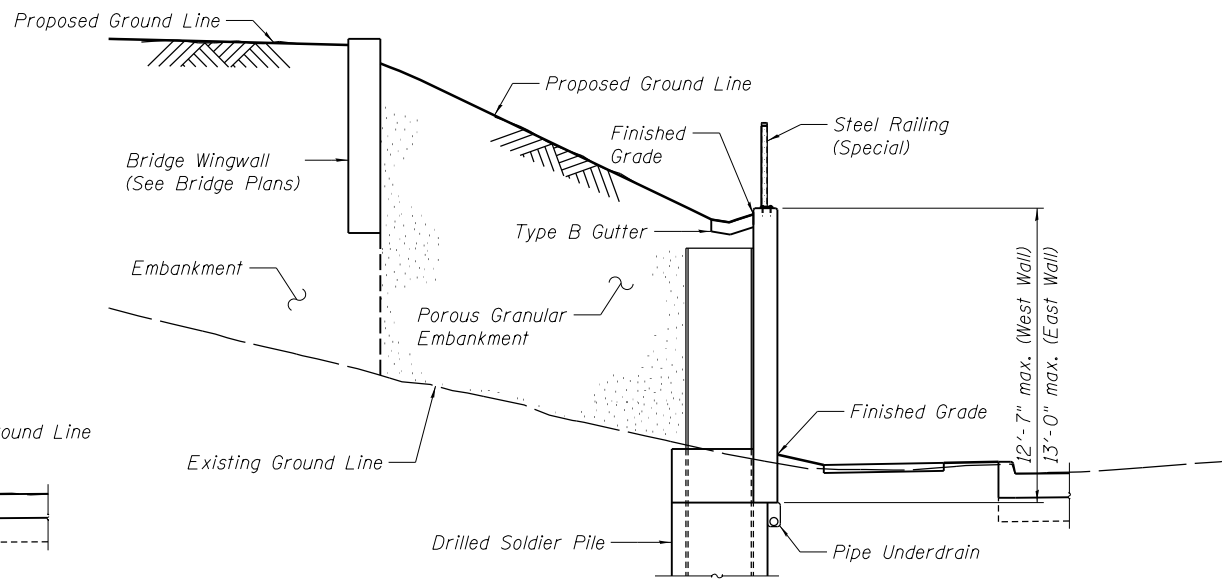
WALL SECTION WITH TEMPORARY EXCAVATION

W. Wall Sta. 999+37 to 999+83±
E. Wall Sta. 999+99 to 1001+15± and Sta. 1000+53 to 1000+94±



WALL SECTION BEHIND EXISTING WINGWALL

West Wall Sta. 999+83 to 999+93±



WALL SECTION PARALLEL TO RAILROAD

West Wall Sta. 998+05.31 to 998+36.00
East Wall Sta. 1001+35.00 to 1001+70.52

**WALL SECTIONS
5TH ST. RETAINING WALLS
F.A.P. 666 ALT.-SECTION (109)VB, (110)VB-5
SANGAMON COUNTY
STATION 998+05.31 TO 1001+70.52**

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PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TYPICAL SECTIONS
RETAINING WALLS - 5TH STREET**

SHEET NO. 4 OF 17 SHEETS

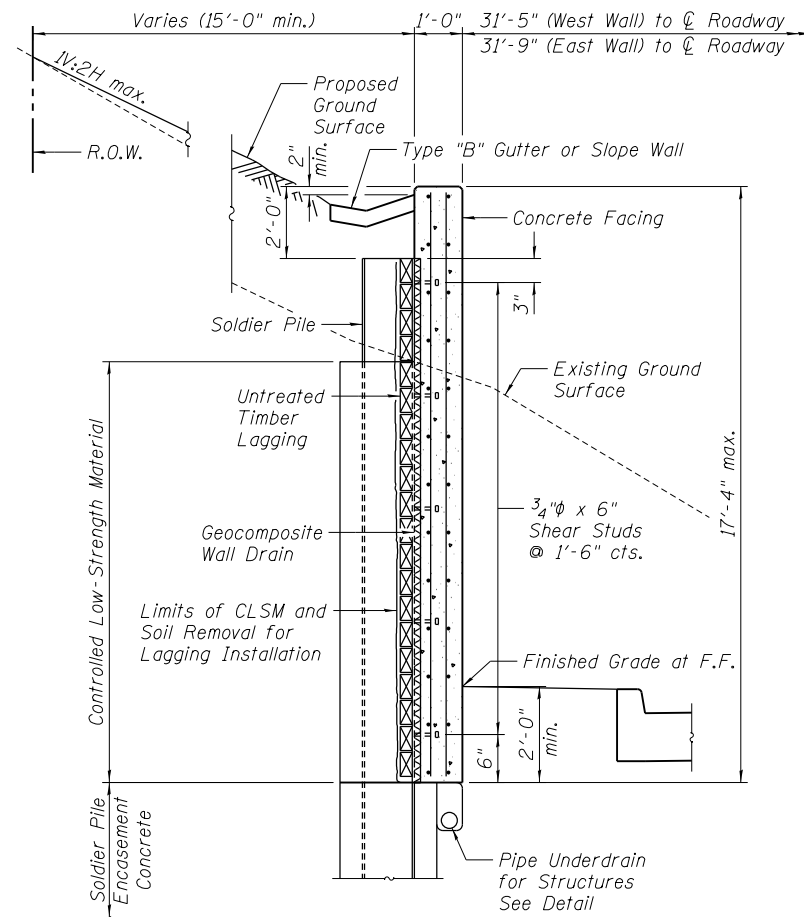
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	220
			CONTRACT NO. 93733	

ILLINOIS FED. AID PROJECT

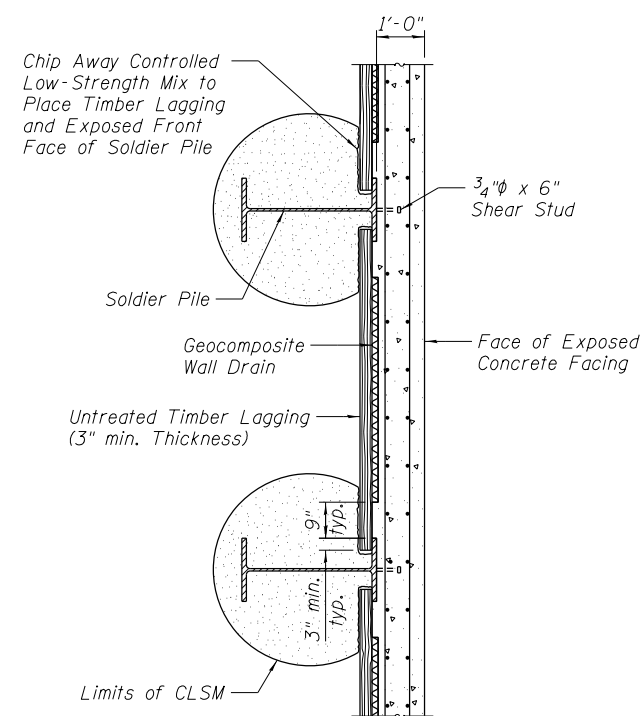
FINAL



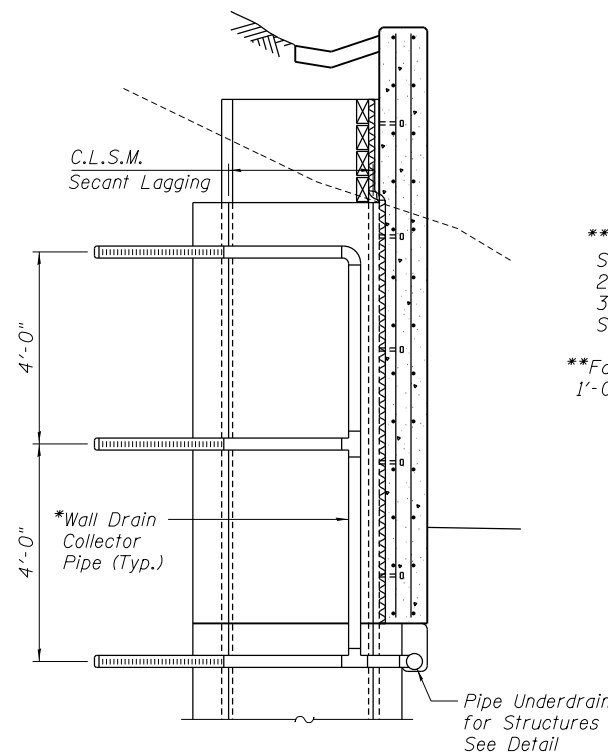
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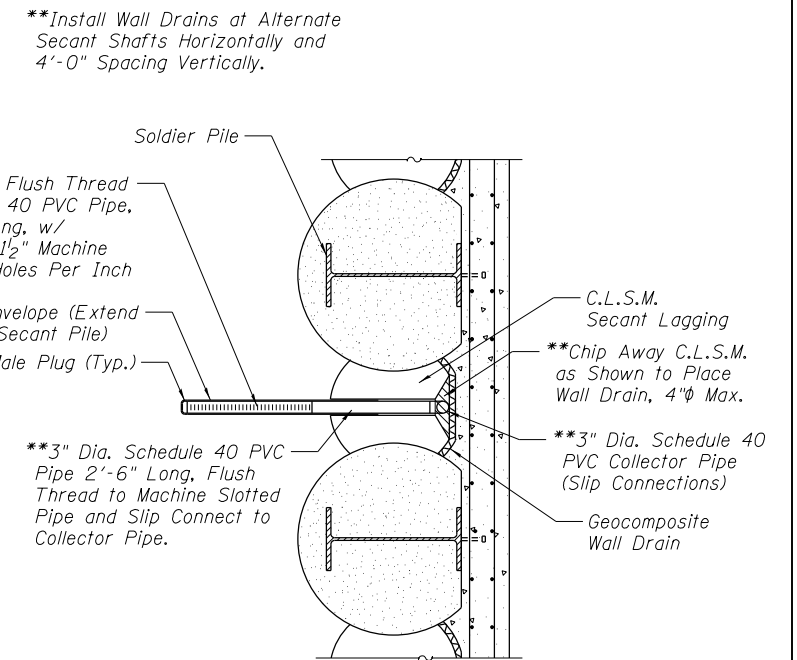
SECTION THRU DRILLED SOLDIER PILE WALL WITH ENCASMENT AND C.I.P. FACING



SECTION THRU DRILLED SOLDIER PILE WALL

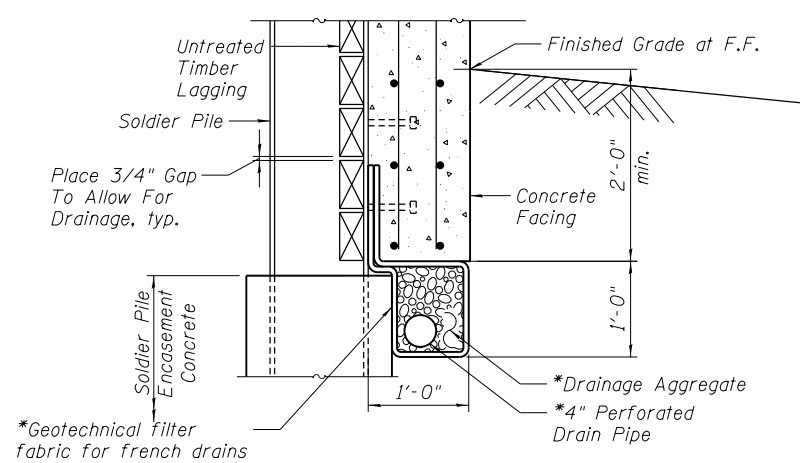


SECTION THRU DRILLED SOLDIER PILE WALL WITH SECANT LAGGING

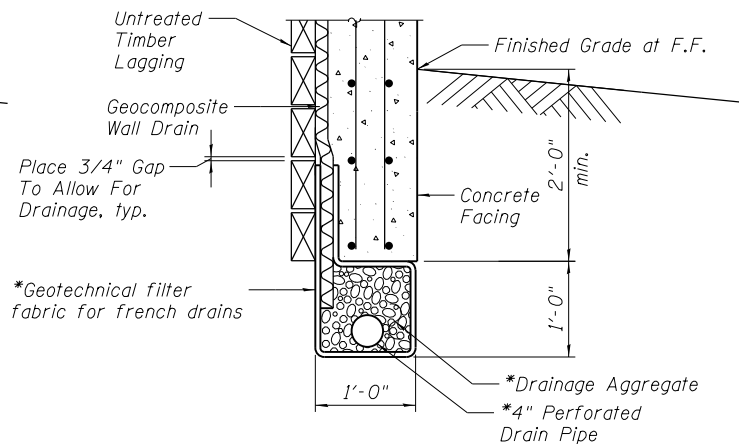


SECTION THRU SECANT LAGGING

** Included In The Cost of Secant Lagging.



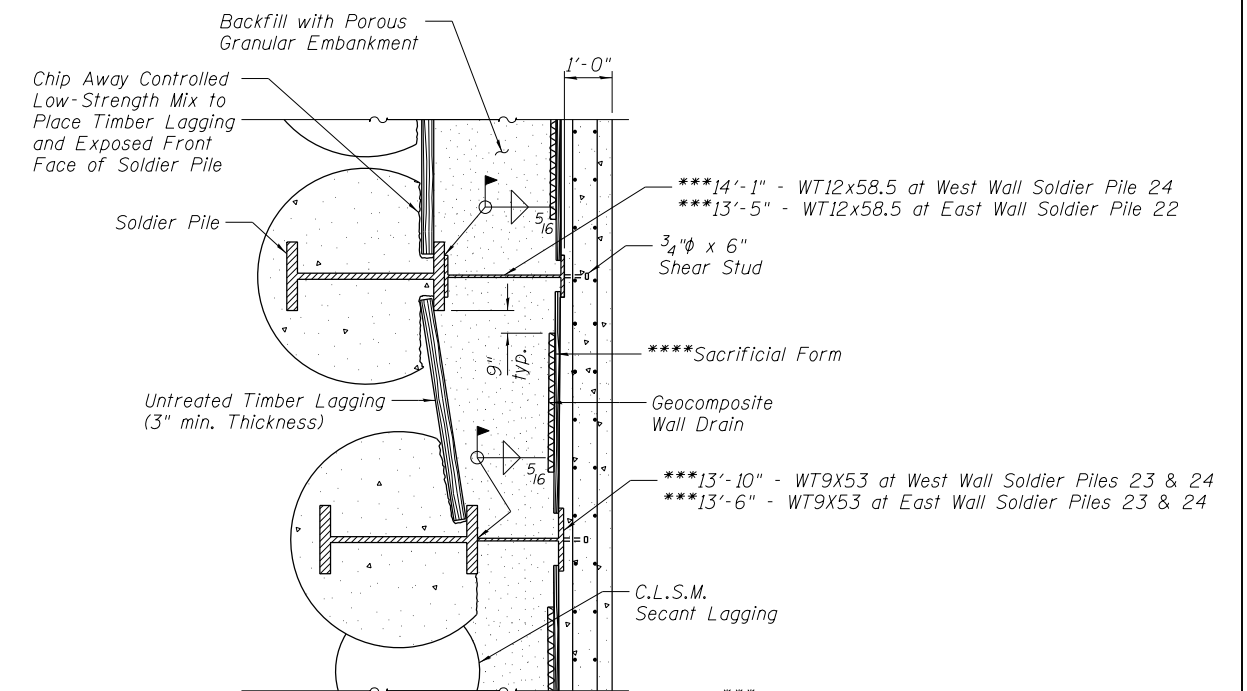
AT SOLDIER PILES



BETWEEN SOLDIER PILES

UNDERDRAIN DETAIL FOR SOLDIER PILE WALLS

*Included in the Cost of Pipe Underdrains for Structures, 4".



SECTION AT OFFSET FACING

***Included in the Cost of Furnishing Soldier Piles (W Section).
 ****Included in the Cost of Concrete Structures (Retaining Wall).

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USER NAME = Pop00275	DESIGNED - RGC	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TYPICAL SECTIONS
RETAINING WALLS - 5TH STREET**

SHEET NO. 5 OF 17 SHEETS

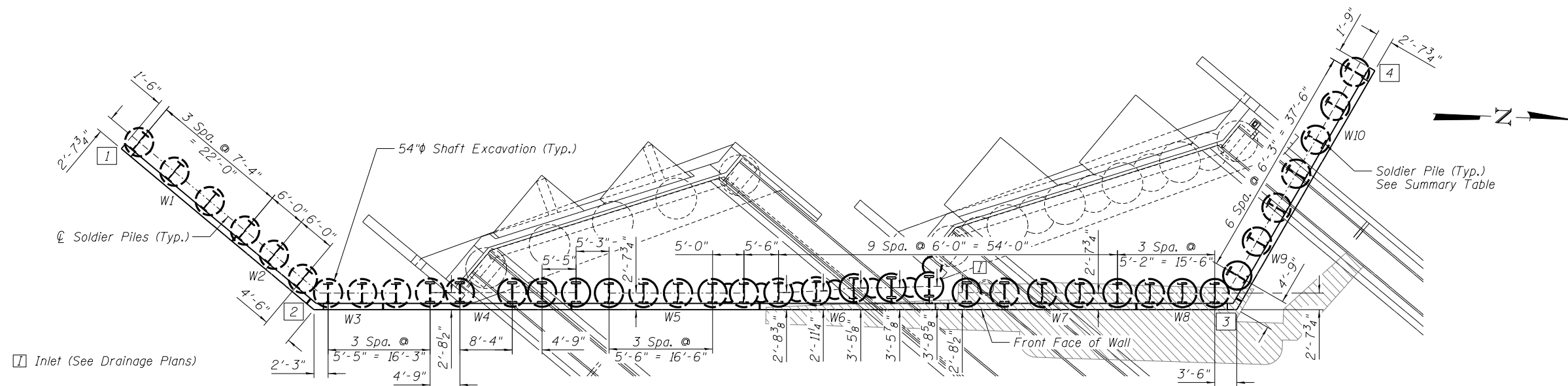
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	221
			CONTRACT NO. 93733	

ILLINOIS FED. AID PROJECT

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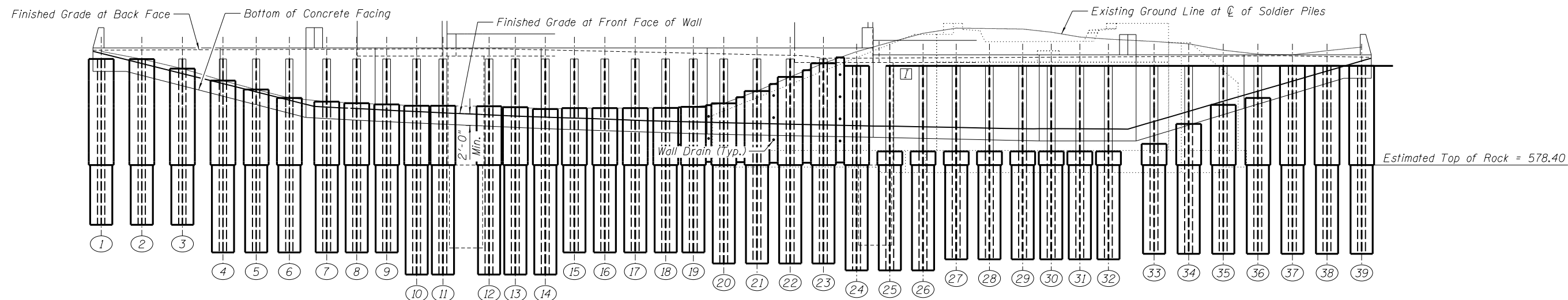


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PLAN

Note: All Dimensions are Measured Along Front Face of Wall



ELEVATION

Unfolded Along Face of Wall

**WEST WALL
STUD SHEAR CONNECTORS REQUIRED**

Pile No.	Number Required on Each Pile
1	3
2	4
3	5
4	6
5	7
6	8
7-9	9
10-16	10
17-23	11
24-25	10
26-32	11
33	10
34	9
35	8
36	6
37	5
38	4
39	3

2 = Control Point

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Stud Shear Connectors	Each	349
Furnishing Soldier Piles (W Section)	Foot	1377
Drilling and Setting Soldier Piles (in Soil)	Cu. Ft.	6873.7
Drilling and Setting Soldier Piles (in Rock)	Cu. Ft.	8145.6
Untreated Timber Lagging	Sq. Ft.	1882
Secant Lagging	Cu. Ft.	622

SOLDIER PILE SUMMARY

PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.	PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.
1	W40x249	30'-0"	567.60	597.60	14	W36x652	39'-0"	558.60	597.60
2	W40x249	30'-0"	567.60	597.60	15	W36x487	35'-0"	562.60	597.60
3	W40x249	30'-0"	567.60	597.60	16	W36x487	35'-0"	562.60	597.60
4	W40x249	35'-0"	562.60	597.60	17	W36x487	35'-0"	562.60	597.60
5	W40x249	35'-0"	562.60	597.60	18	W36x487	35'-0"	562.60	597.60
6	W40x249	35'-0"	562.60	597.60	19	W36x487	35'-0"	562.60	597.60
7	W36x487	35'-0"	562.60	597.60	20	W36x487	37'-0"	560.60	597.60
8	W36x487	35'-0"	562.60	597.60	21	W36x487	37'-0"	560.60	597.60
9	W36x487	35'-0"	562.60	597.60	22	W36x487	37'-0"	560.60	597.60
10	W36x652	39'-0"	558.60	597.60	23	W36x652	37'-0"	560.60	597.60
11	W36x652	39'-0"	558.60	597.60	24	W36x652	37'-0"	559.35	596.35
12	W36x652	39'-0"	558.60	597.60	25	W36x652	37'-0"	559.35	596.35
13	W36x652	39'-0"	558.60	597.60	26	W36x652	37'-0"	559.35	596.35
					27	W36x487	35'-0"	561.35	596.35
					28	W36x487	35'-0"	561.35	596.35
					29	W36x487	35'-0"	561.35	596.35
					30	W36x487	35'-0"	561.35	596.35
					31	W36x487	35'-0"	561.35	596.35
					32	W36x487	35'-0"	561.35	596.35
					33	W36x487	34'-0"	562.35	596.35
					34	W36x487	34'-0"	562.35	596.35
					35	W36x487	34'-0"	562.35	596.35
					36	W36x487	34'-0"	562.35	596.35
					37	W36x487	34'-0"	562.35	596.35
					38	W36x487	34'-0"	562.35	596.35
					39	W36x487	34'-0"	562.35	596.35

SECANT LAGGING SUMMARY

BETWEEN PILES NO.	DIAMETER	LENGTH	BOTTOM ELEV.	TOP ELEV.
18-19	36"	7'-11"	*580.89	588.84
19-20	36"	8'-4"	*580.89	589.25
20-21	36"	9'-9"	*580.89	590.68
21-22	36"	12'-2"	*580.89	593.07
22-23	36"	14'-8"	*580.89	595.58
23-24	36"	16'-0"	*580.89	597.87
24-BR	36"	18'-0"	578.96	596.96

* Top of existing footing

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FINAL



USER NAME = Pop00275	DESIGNED - RGC	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

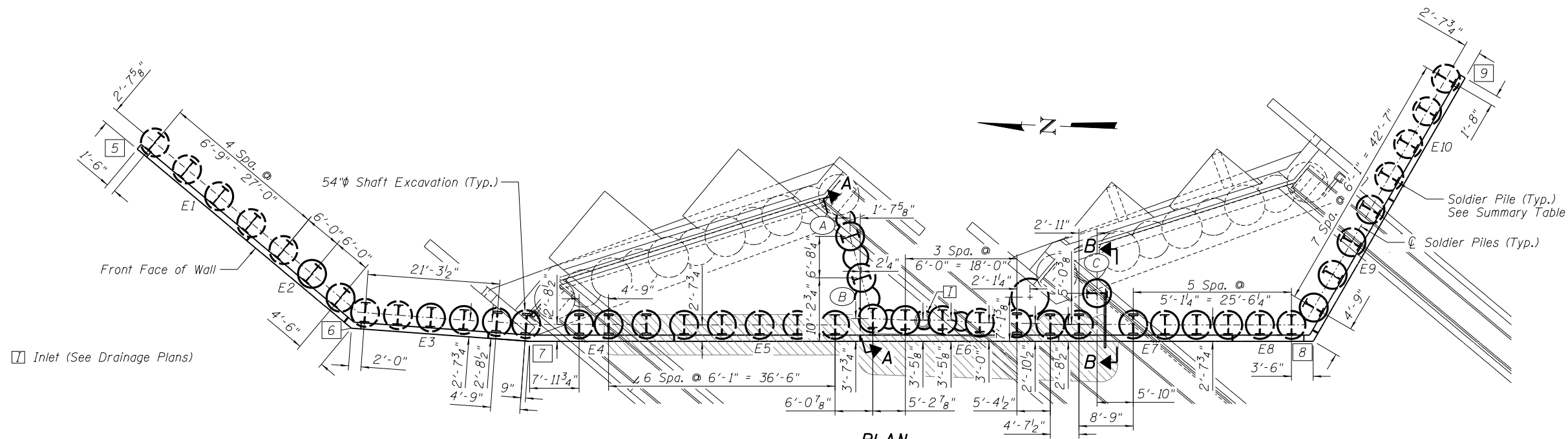
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOLDIER PILES - WEST WALL
RETAINING WALLS - 5TH STREET

SHEET NO. 6 OF 17 SHEETS

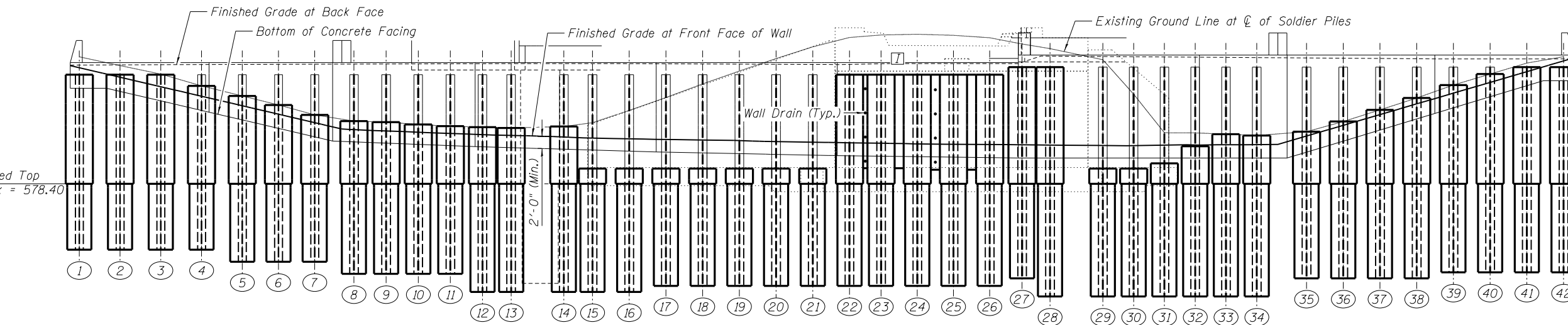
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	222
CONTRACT NO.			93733	

ILLINOIS FED. AID PROJECT



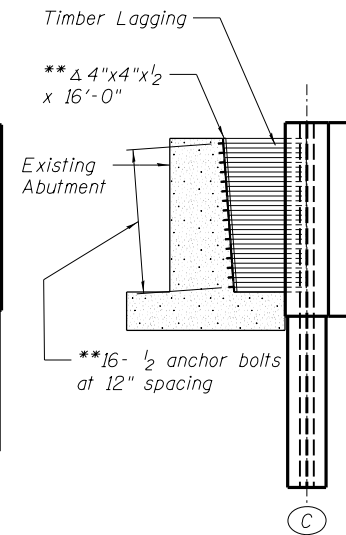
PLAN

Note: All Dimensions are Measured Along Front Face of Wall



ELEVATION

Unfolded Along Face of Wall



SECTION B-B

**Included in the Cost of Untreated Timber Lagging.

SOLDIER PILE SUMMARY

PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.	PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.	PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.
1	W40x249	29'-0"	567.50	596.50	16	W36x652	36'-0"	560.50	596.50	31	W36x487	38'-0"	559.75	597.75
2	W40x249	29'-0"	567.50	596.50	17	W36x487	35'-0"	561.50	596.50	32	W36x487	38'-0"	559.75	597.75
3	W40x249	29'-0"	567.50	596.50	18	W36x487	35'-0"	561.50	596.50	33	W36x487	38'-0"	559.75	597.75
4	W40x249	29'-0"	567.50	596.50	19	W36x487	35'-0"	561.50	596.50	34	W36x487	38'-0"	559.75	597.75
5	W40x249	31'-0"	565.50	596.50	20	W36x487	35'-0"	561.50	596.50	35	W36x487	35'-0"	562.75	597.75
6	W40x249	31'-0"	565.50	596.50	21	W36x487	35'-0"	561.50	596.50	36	W36x487	35'-0"	562.75	597.75
7	W40x249	31'-0"	565.50	596.50	22	W36x487	35'-0"	561.50	596.50	37	W36x487	35'-0"	562.75	597.75
8	W36x487	33'-0"	563.50	596.50	23	W36x487	35'-0"	561.50	596.50	38	W36x487	35'-0"	562.75	597.75
9	W36x487	33'-0"	563.50	596.50	24	W36x487	35'-0"	561.50	596.50	39	W36x487	34'-0"	563.75	597.75
10	W36x487	33'-0"	563.50	596.50	25	W36x487	35'-0"	561.50	596.50	40	W36x487	34'-0"	563.75	597.75
11	W36x487	33'-0"	563.50	596.50	26	W36x652	35'-0"	561.50	596.50	41	W36x487	34'-0"	563.75	597.75
12	W36x652	36'-0"	560.50	596.50	27	W36x652	35'-0"	562.75	597.75	42	W36x487	34'-0"	563.75	597.75
13	W36x652	36'-0"	560.50	596.50	28	W36x652	38'-0"	559.75	597.75	A	W40x249	30'-0"	567.27	597.27
14	W36x652	36'-0"	560.50	596.50	29	W36x652	38'-0"	559.75	597.75	B	W40x249	30'-0"	567.27	597.27
15	W36x652	36'-0"	560.50	596.50	30	W36x487	38'-0"	559.75	597.75	C	W36x487	38'-0"	560.59	598.59

SECANT LAGGING SUMMARY

BETWEEN PILES NO.	DIAMETER	LENGTH	BOTTOM ELEV.	TOP ELEV.
22-23	36"	15'-7"	*580.94	596.50
23-24	36"	15'-7"	*580.94	596.50
24-25	36"	15'-7"	*580.94	596.50
25-26	36"	15'-7"	*580.94	596.50
BR-A	36"	18'-4"	578.94	597.27
A-B	48"	18'-4"	578.94	597.27
B-22	48"	16'-4"	*580.94	597.27
26-BR	72"	20'-11"	*580.94	601.84

* Top of existing footing

**EAST WALL
STUD SHEAR CONNECTORS REQUIRED**

PILE NO.	Number Required on Each Pile
1	3
2	4
3	5
4	6
5	7
6	8
7-9	9
10-18	10
19-26	11
27-34	12
35	11
36	10
37	9
38	8
39	6
40	5
41	4
42	3

[6] = Control Point

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Stud Shear Connectors	Each	390
Furnishing Soldier Piles (W Section)	Foot	1546
Drilling and Setting Soldier Piles (in Soil)	Cu. Ft.	9401.2
Drilling and Setting Soldier Piles (in Rock)	Cu. Ft.	8895.4
Untreated Timber Lagging	Sq. Ft.	2069
Secant Lagging	Cu. Ft.	1597

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USER NAME = Pop00275	DESIGNED - RGC	REVISD -
	CHECKED - KMS	REVISD -
PLOT SCALE = 0.1667' / in.	DRAWN - EJM	REVISD -
PLOT DATE = 4/11/2019	CHECKED - RGC	REVISD -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**SOLDIER PILES - EAST WALL
RETAINING WALLS - 5TH STREET**

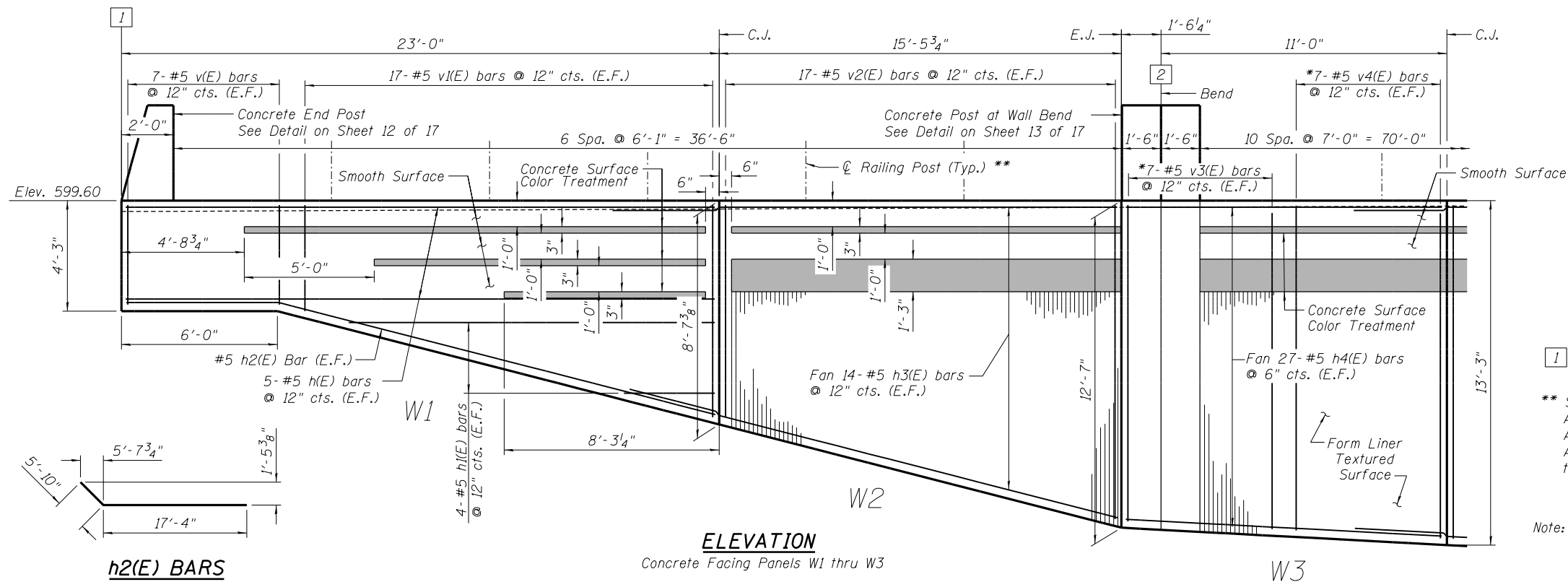
SHEET NO. 7 OF 17 SHEETS

F.A.U. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	223
			CONTRACT NO. 93733	
ILLINOIS FED. AID PROJECT				

FINAL

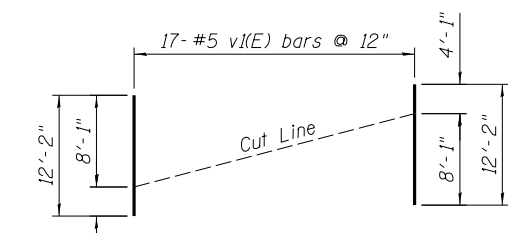


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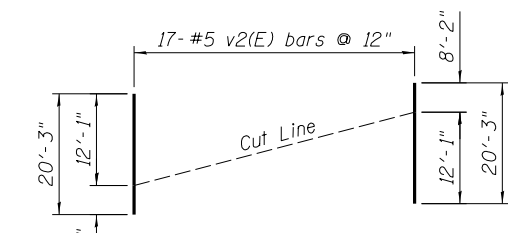


ELEVATION
Concrete Facing Panels W1 thru W3

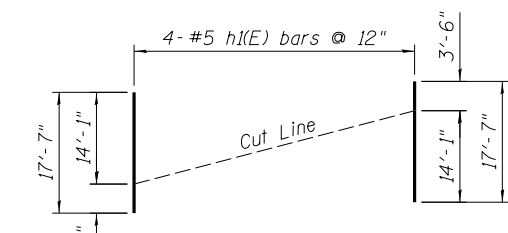
1 = Control Point
 ** Steel Railing (Special)
 All Measurements are Along Top of Wall.
 Adjust as Necessary to Avoid C.J.'s & E.J.'s.
 Note: E.J. = Expansion Joint
 C.J. = Construction Joint
 E.F. = Each Face
 * = Stagger Bars



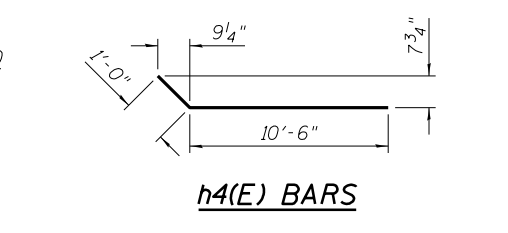
v1(E) BARS
Cut Bars to be Placed E.F.



v2(E) BARS
Cut Bars to be Placed E.F.



h1(E) BARS
Cut Bars to be Placed E.F.



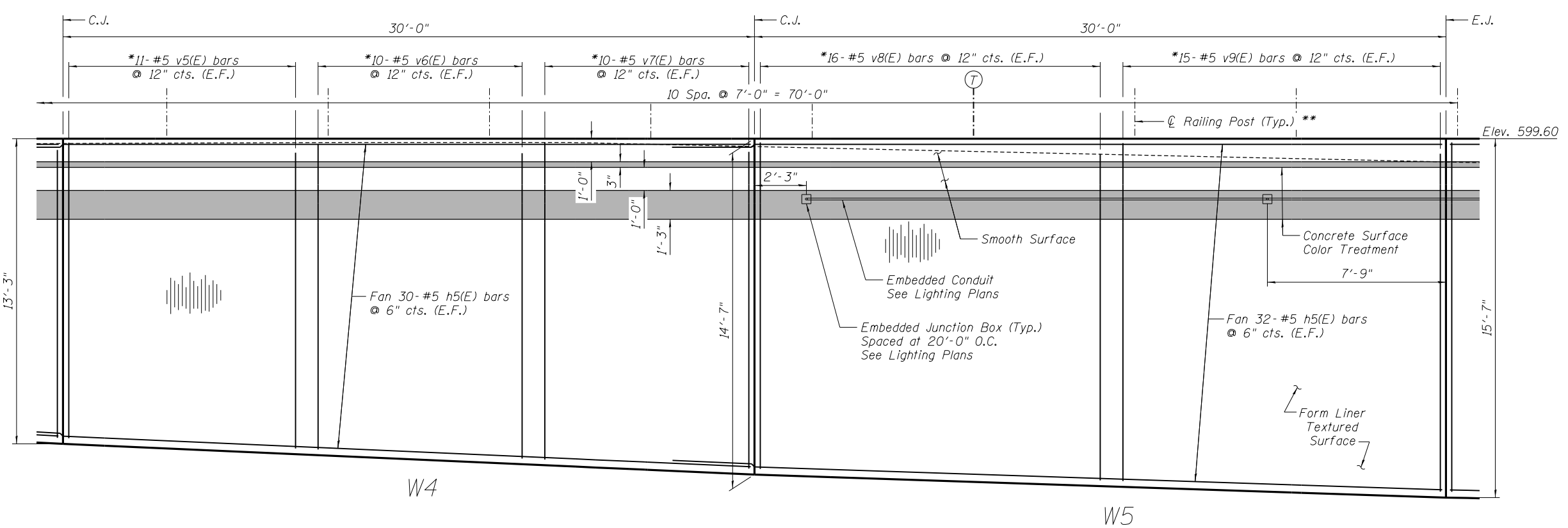
h4(E) BARS

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	10	#5	22'-8"	—
h1(E)	4	#5	17'-7"	—
h2(E)	2	#5	23'-2"	—
h3(E)	28	#5	19'-5"	—
h4(E)	54	#5	11'-6"	—
h5(E)	124	#5	33'-5"	—
v(E)	14	#5	3'-10"	—
v1(E)	17	#5	12'-2"	—
v2(E)	17	#5	20'-3"	—
v3(E)	14	#5	12'-2"	—
v4(E)	14	#5	12'-5"	—
v5(E)	22	#5	12'-10"	—
v6(E)	20	#5	13'-3"	—
v7(E)	20	#5	13'-9"	—
v8(E)	32	#5	14'-2"	—
v9(E)	30	#5	14'-8"	—
Reinforcement Bars Epoxy Coated		Pound	8670	
Concrete Structures		Cu. Yd.	56.9	

MIN. BAR LAPS
#5 Bars = 3'-4"

Ⓣ = Intermediate Tensioning Posts



ELEVATION
Concrete Facing Panels W4 & W5

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USER NAME = Pop00275	DESIGNED - RGC	REVISD -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISD -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISD -
	CHECKED - RGC	REVISD -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CONCRETE FACING - WEST WALL
RETAINING WALLS - 5TH STREET

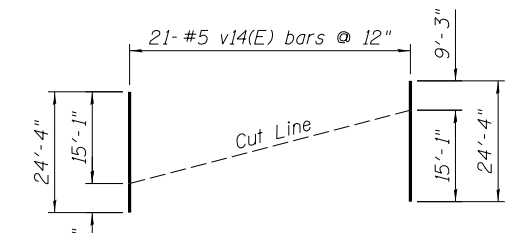
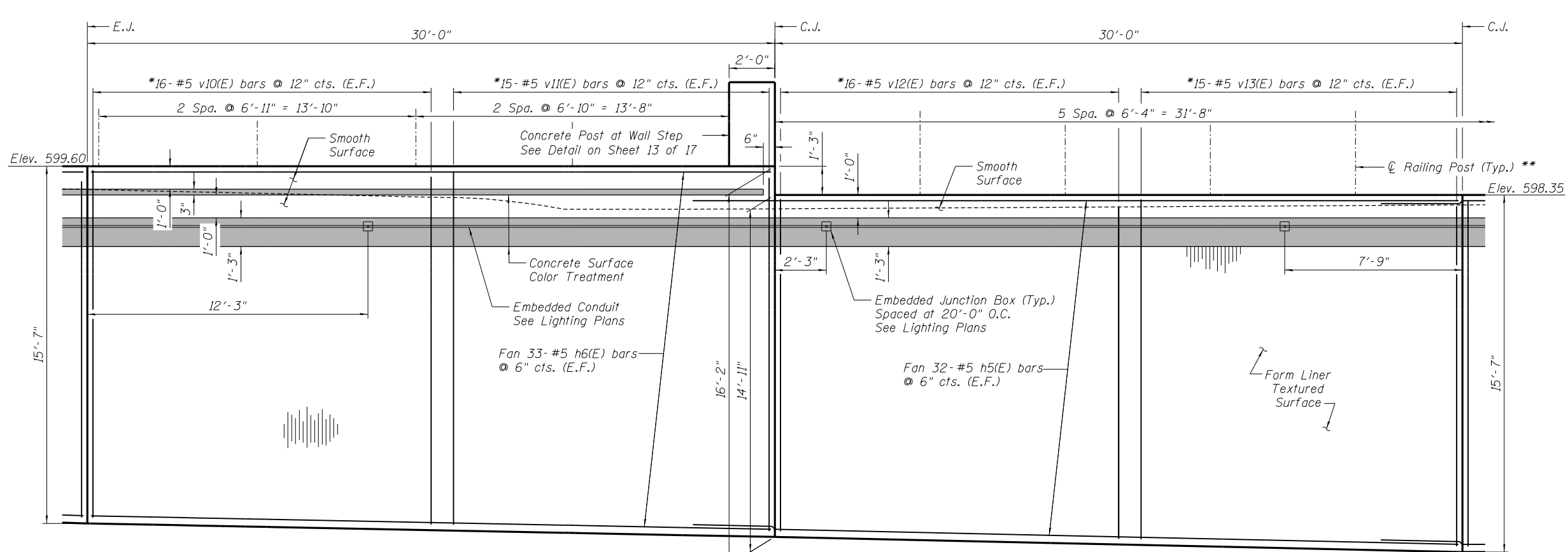
SHEET NO. 8 OF 17 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 93733			ILLINOIS FED. AID PROJECT	

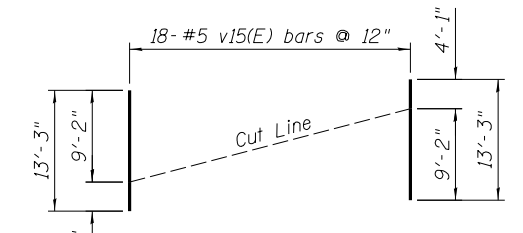
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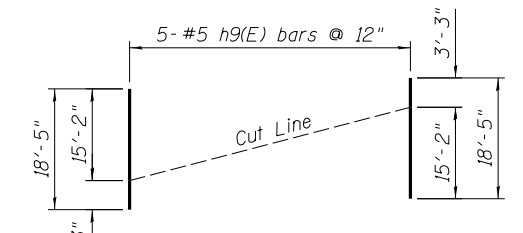
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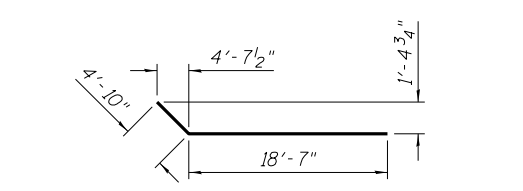
v14(E) BARS
Cut Bars to be Placed E.F.



v15(E) BARS
Cut Bars to be Placed E.F.



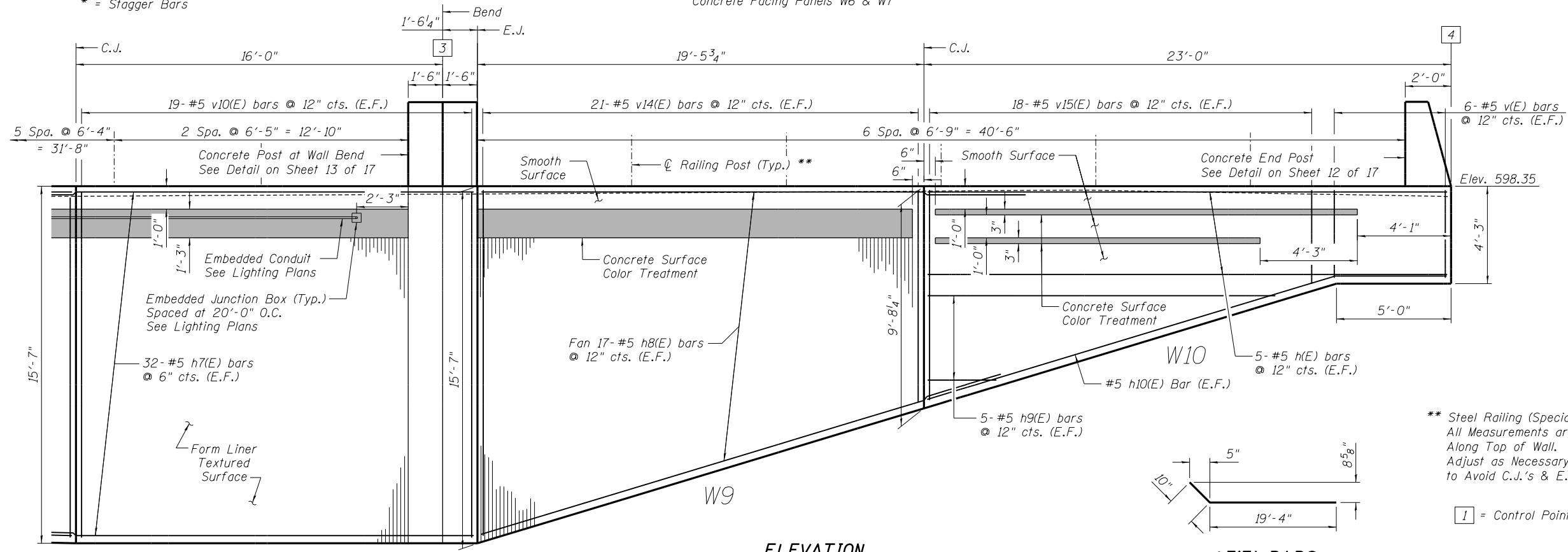
h9(E) BARS
Cut Bars to be Placed E.F.



h10(E) BARS

Note: E.J. = Expansion Joint
C.J. = Construction Joint
E.F. = Each Face
* = Stagger Bars

ELEVATION
Concrete Facing Panels W6 & W7



BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	10	#5	22'-8"	—
h5(E)	64	#5	33'-5"	—
h6(E)	66	#5	29'-8"	—
h7(E)	64	#5	20'-2"	—
h8(E)	34	#5	23'-9"	—
h9(E)	5	#5	18'-5"	—
h10(E)	2	#5	23'-5"	—
v(E)	12	#5	3'-10"	—
v10(E)	70	#5	15'-2"	—
v11(E)	30	#5	15'-5"	—
v12(E)	32	#5	14'-6"	—
v13(E)	30	#5	14'-10"	—
v14(E)	21	#5	24'-4"	—
v15(E)	18	#5	13'-3"	—
Reinforcement Bars Epoxy Coated		Pound	10210	
Concrete Structures (Retaining Wall)		Cu. Yd.	69.0	

** Steel Railing (Special)
All Measurements are Along Top of Wall.
Adjust as Necessary to Avoid C.J.'s & E.J.'s.

1 = Control Point

MIN. BAR LAPS
#5 Bars = 3'-4"

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USER NAME = Pop00275	DESIGNED - RGC	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**CONCRETE FACING - WEST WALL
RETAINING WALLS - 5TH STREET**

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	225
CONTRACT NO.			93733	

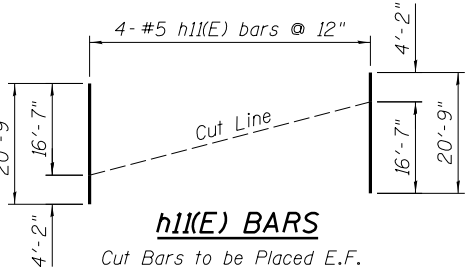
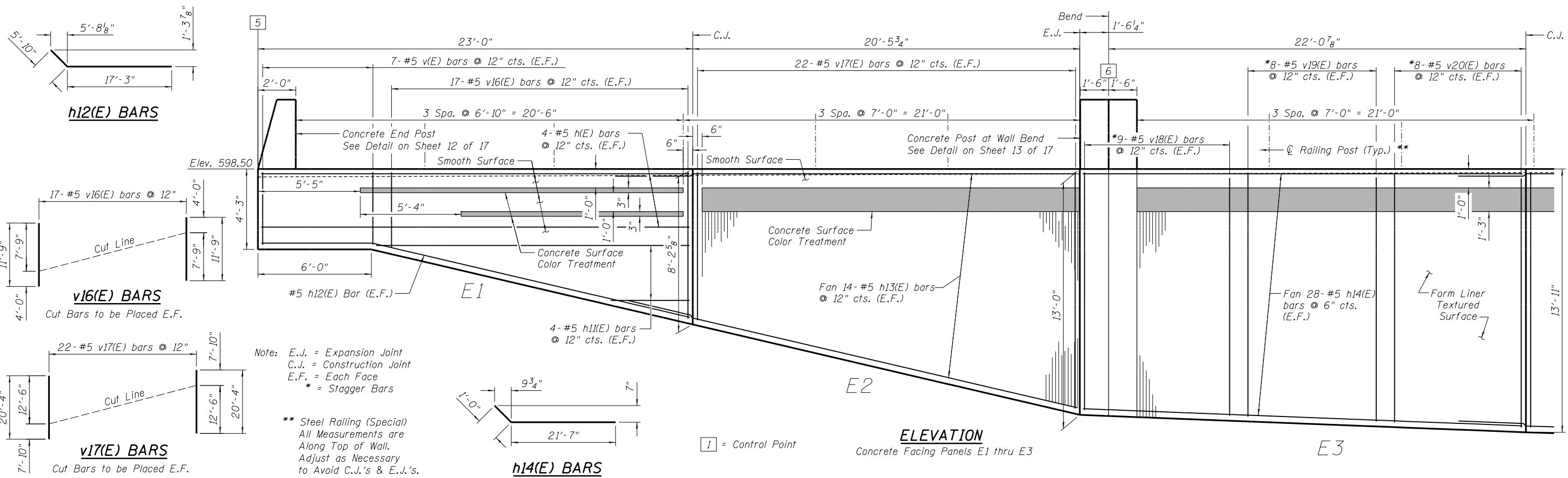
SHEET NO. 9 OF 17 SHEETS

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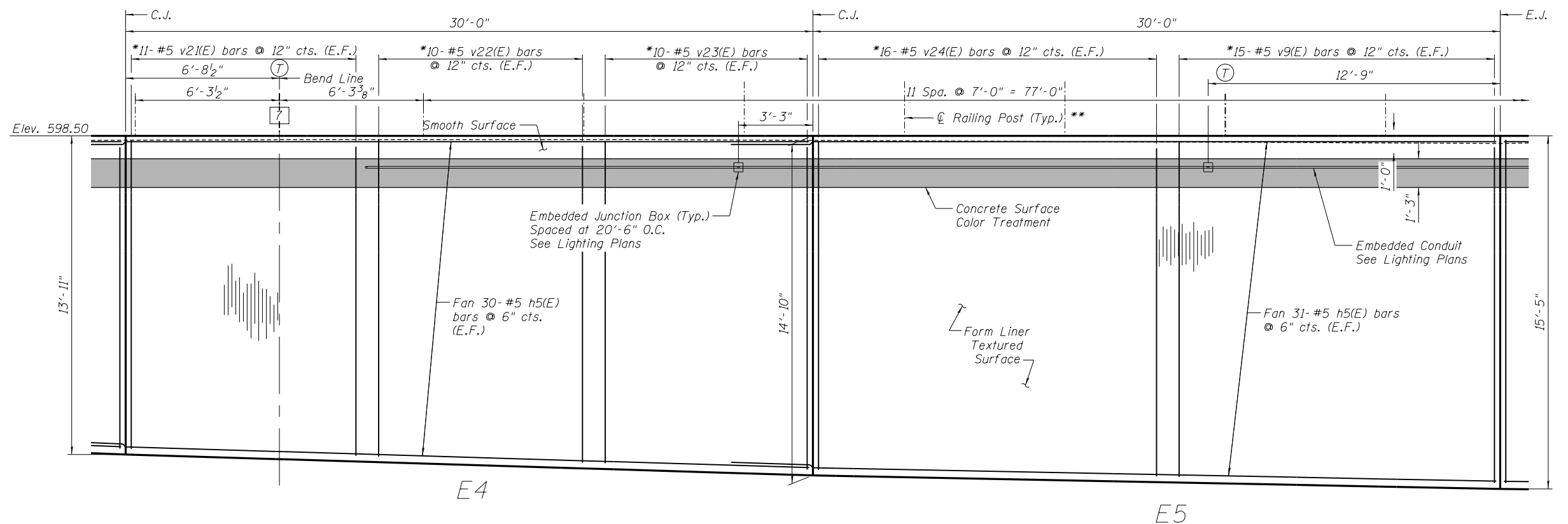


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Note: E.J. = Expansion Joint
 C.J. = Construction Joint
 E.F. = Each Face
 * = Stagger Bars

** Steel Railing (Special)
 All Measurements are
 Along Top of Wall.
 Adjust as Necessary
 to Avoid C.J.'s & E.J.'s.



MIN. BAR LAPS
 #5 Bars = 3'-4" (T) = Intermediate Tensioning Posts

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	8	#5	22'-8"	---
h5(E)	122	#5	33'-5"	---
h11(E)	4	#5	20'-9"	---
h12(E)	2	#5	23'-1"	---
h13(E)	28	#5	24'-5"	---
h14(E)	56	#5	22'-7"	---
v(E)	14	#5	3'-10"	---
v9(E)	30	#5	14'-8"	---
v16(E)	17	#5	11'-9"	---
v17(E)	22	#5	20'-4"	---
v18(E)	18	#5	12'-7"	---
v19(E)	16	#5	12'-11"	---
v20(E)	16	#5	13'-2"	---
v21(E)	22	#5	13'-6"	---
v22(E)	20	#5	13'-10"	---
v23(E)	20	#5	14'-1"	---
v24(E)	32	#5	14'-4"	---
Reinforcement Bars Epoxy Coated		Pound	9840	
Concrete Structures (Retaining Wall)		Cu. Yd.	65.9	

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**CONCRETE FACING - EAST WALL
 RETAINING WALLS - 5TH STREET**

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	226
CONTRACT NO.			93733	

SHEET NO. 10 OF 17 SHEETS

ILLINOIS FED. AID PROJECT

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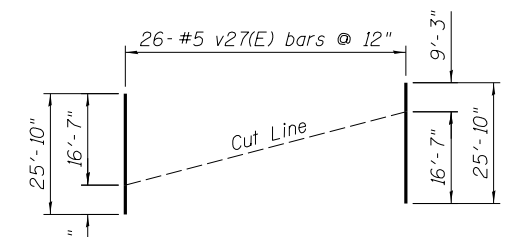
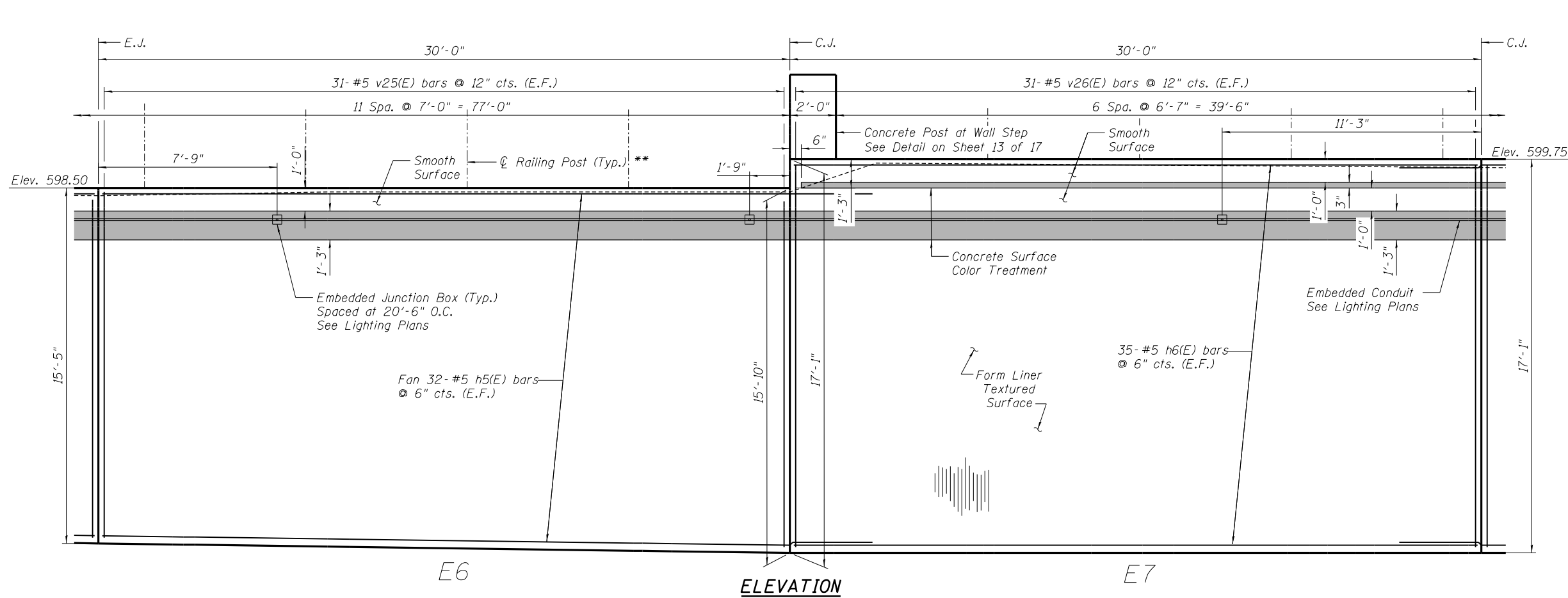
USER NAME = Pop00275
 DESIGNED - RGC
 CHECKED - KMS
 PLOT SCALE = 0.1667' / in.
 DRAWN - EJM
 PLOT DATE = 4/11/2019
 CHECKED - RGC

DESIGNED - RGC
 CHECKED - KMS
 DRAWN - EJM
 CHECKED - RGC

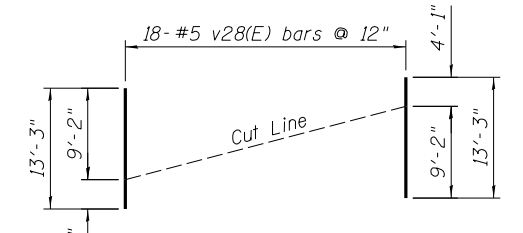
REVISED -
 REVISED -
 REVISED -
 REVISED -

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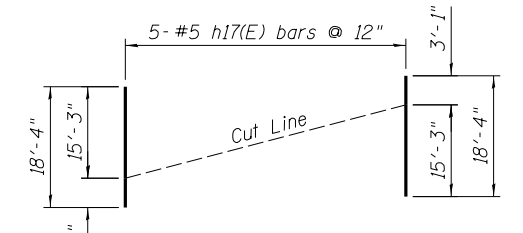
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v27(E) BARS
Cut Bars to be Placed E.F.

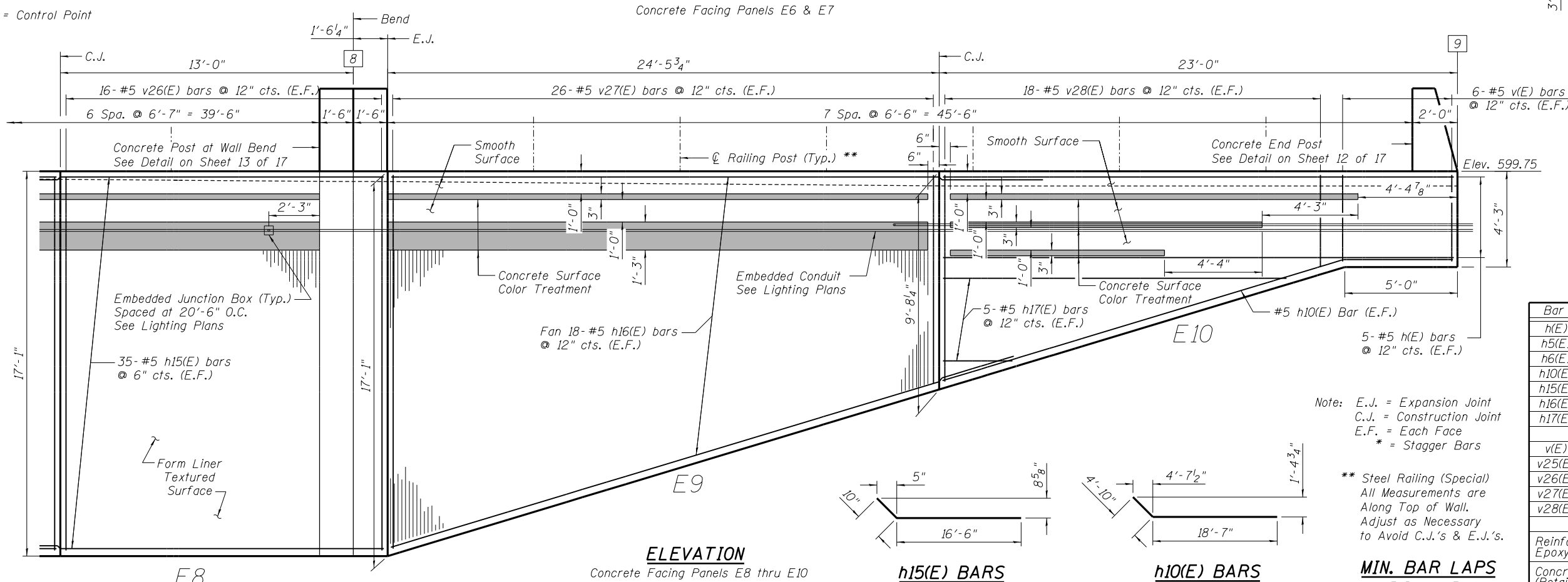


v28(E) BARS
Cut Bars to be Placed E.F.



h17(E) BARS
Cut Bars to be Placed E.F.

1 = Control Point



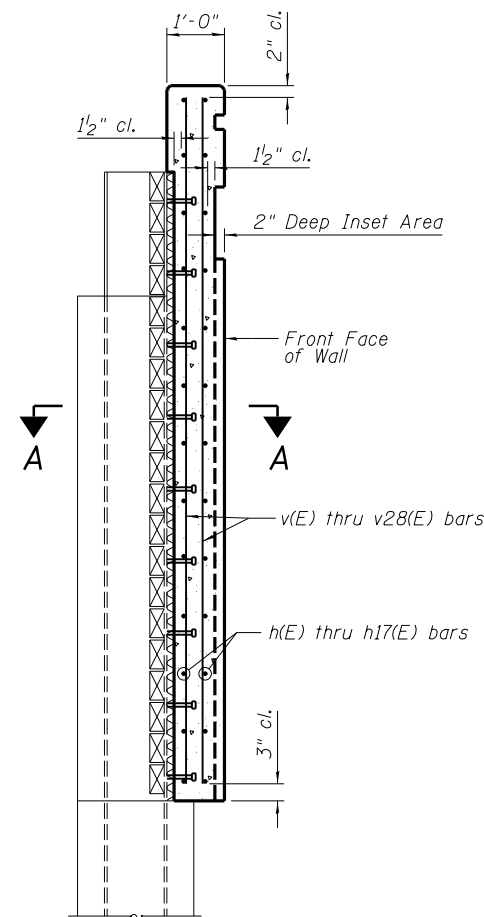
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	10	#5	22'-8"	—
h5(E)	64	#5	33'-5"	—
h6(E)	70	#5	29'-8"	—
h10(E)	2	#5	23'-5"	—
h15(E)	70	#5	17'-4"	—
h16(E)	36	#5	28'-11"	—
h17(E)	5	#5	18'-4"	—
v(E)	12	#5	3'-10"	—
v25(E)	62	#5	15'-0"	—
v26(E)	94	#5	16'-7"	—
v27(E)	26	#5	25'-10"	—
v28(E)	18	#5	13'-3"	—
Reinforcement Bars Epoxy Coated		Pound	10720	
Concrete Structures (Retaining Wall)		Cu. Yd.	73.7	

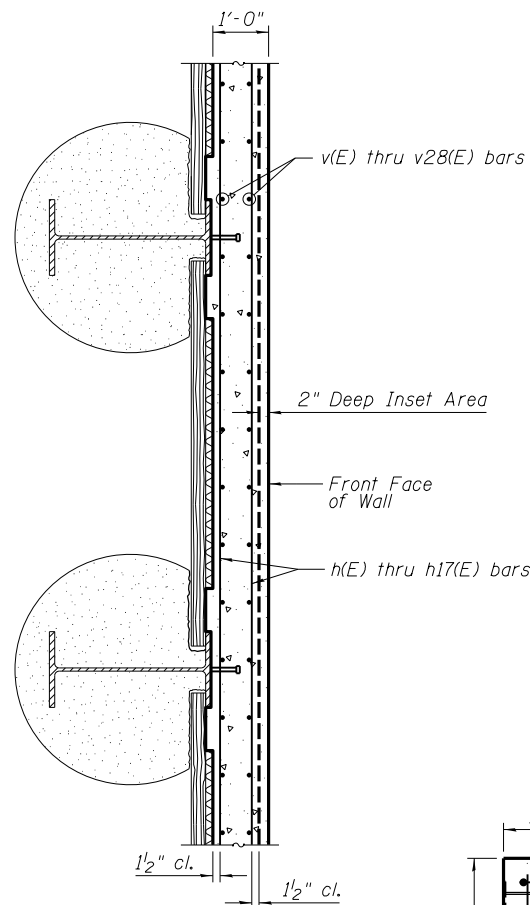
Note: E.J. = Expansion Joint
C.J. = Construction Joint
E.F. = Each Face
* = Stagger Bars

** Steel Railing (Special)
All Measurements are
Along Top of Wall.
Adjust as Necessary
to Avoid C.J.'s & E.J.'s.

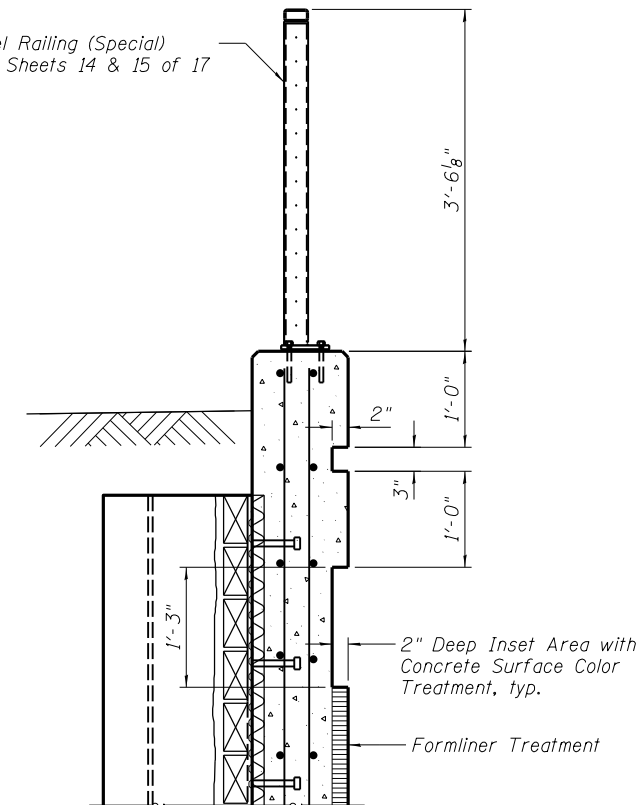
MIN. BAR LAPS
#5 Bars = 3'-4"



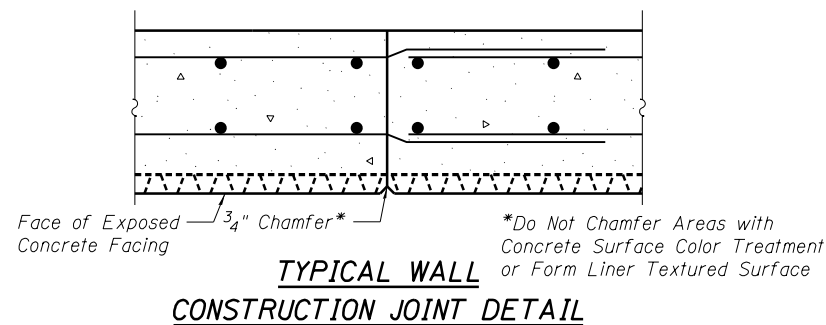
TYPICAL SECTION THRU CONCRETE FACING



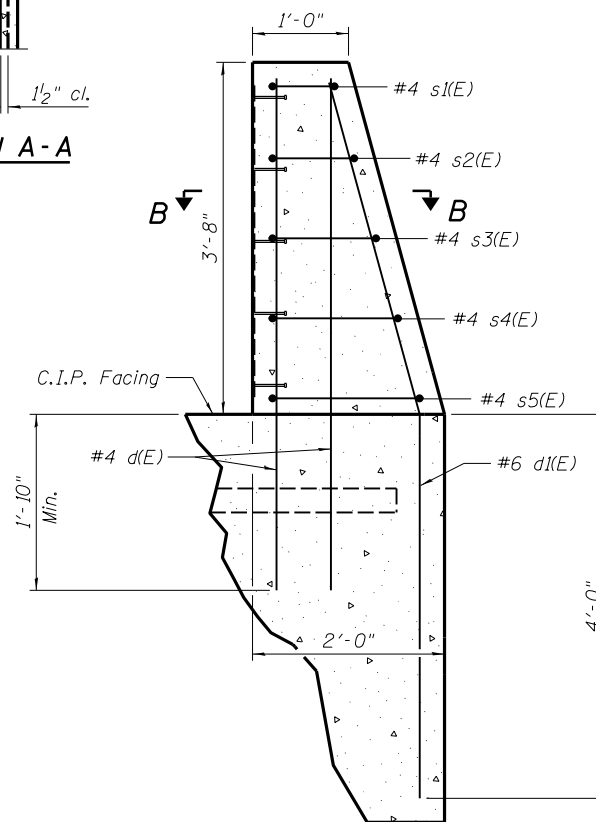
SECTION A-A



RAILING & RUSTICATION DETAIL

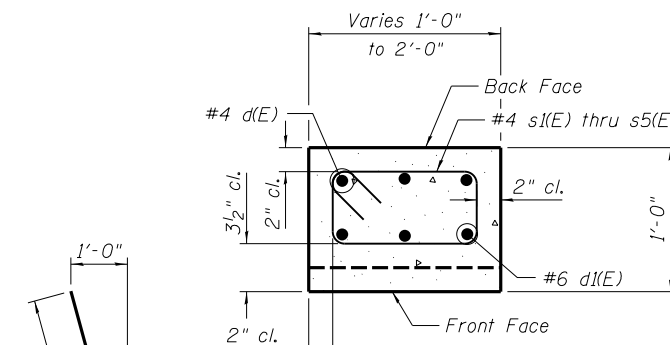


TYPICAL WALL CONSTRUCTION JOINT DETAIL

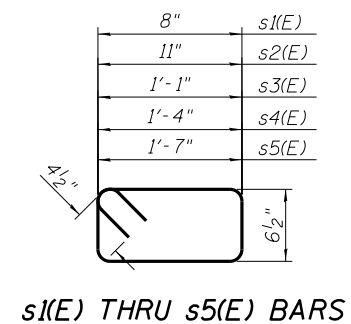


CABLE ANCHORAGE CONCRETE END POST DETAIL

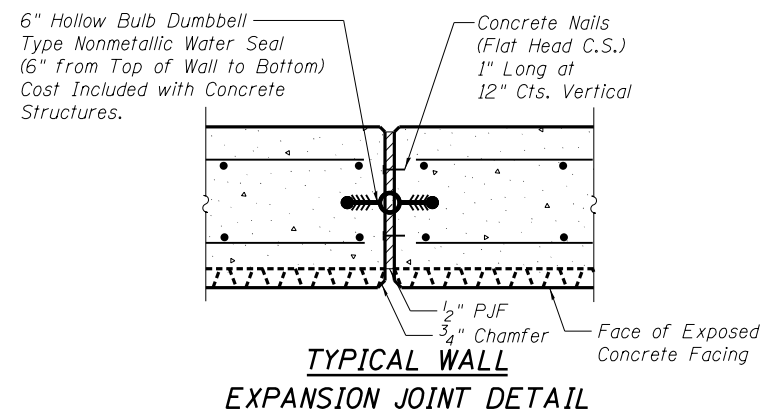
Typ. of 4 Posts on Concrete Facing
See Sheet 15 of 17 for Railing Connection Details



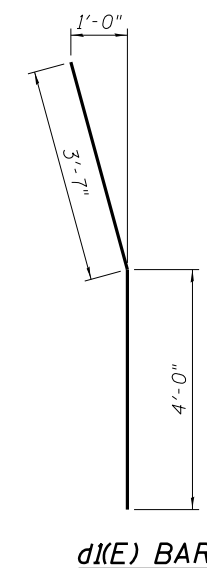
SECTION B-B



s1(E) THRU s5(E) BARS



TYPICAL WALL EXPANSION JOINT DETAIL



d1(E) BAR

BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
d(E)	16	#4	5'-5"	—	
d1(E)	8	#6	7'-7"	—	
s1(E)	4	#4	3'-2"	□	
s2(E)	4	#4	3'-8"	□	
s3(E)	4	#4	4'-0"	□	
s4(E)	4	#4	4'-6"	□	
s5(E)	4	#4	5'-0"	□	
Reinforcement Bars Epoxy Coated				Pound	200
Concrete Structures (Retaining Wall)				Cu. Yd.	0.8

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USER NAME = Pop00275	DESIGNED - RGC	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**CONCRETE FACING DETAILS
RETAINING WALLS - 5TH STREET**

SHEET NO. 12 OF 17 SHEETS

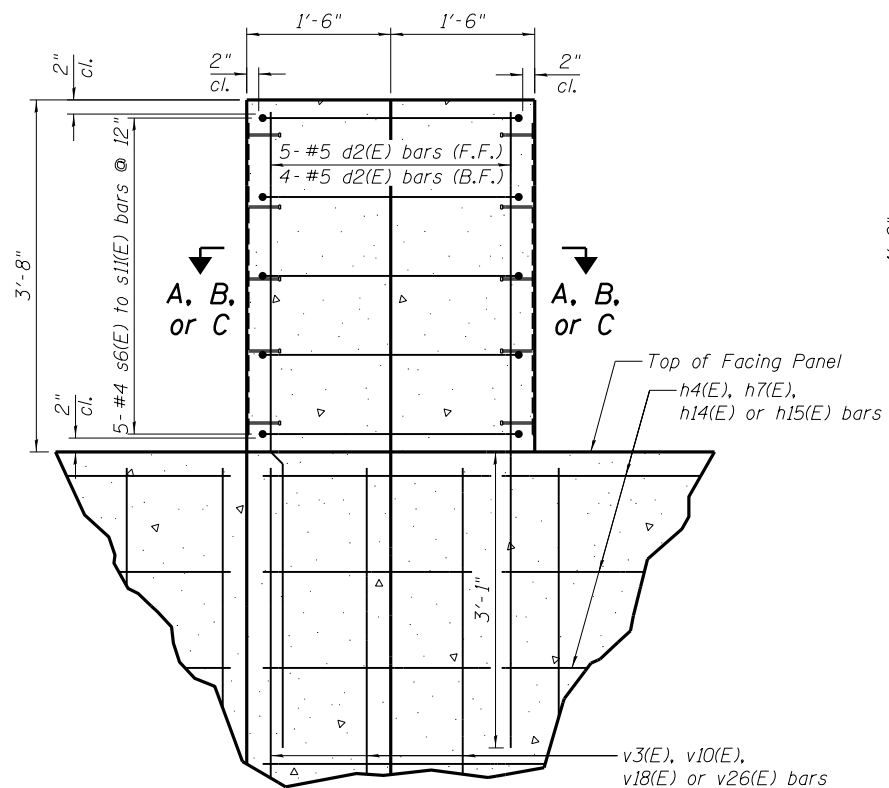
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	228
			CONTRACT NO. 93733	

ILLINOIS FED. AID PROJECT

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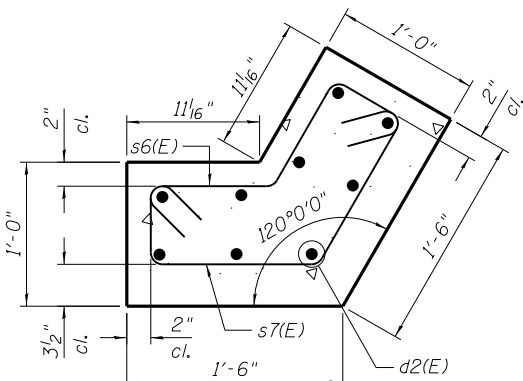


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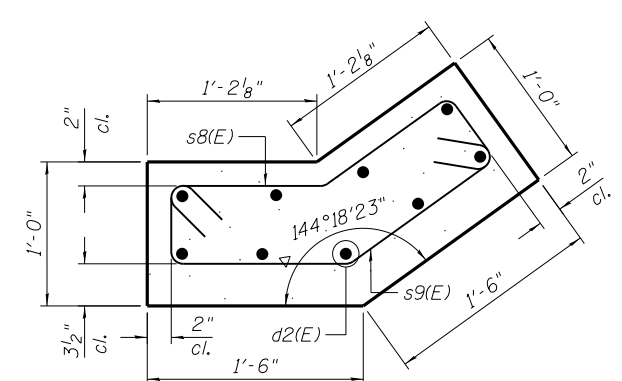
CABLE ANCHORAGE CONCRETE POST AT WALL BENDS - UNFOLDED VIEW

Typ. of 4 Posts on Concrete Facing
See Sheet 15 of 17 for Railing Connection Details



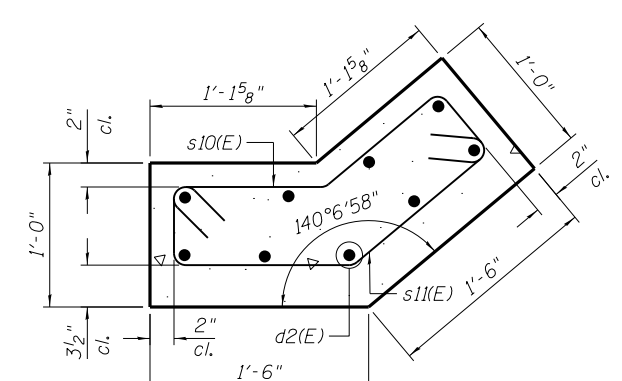
SECTION A-A

West Wall - North Bend & East Wall - South Bend



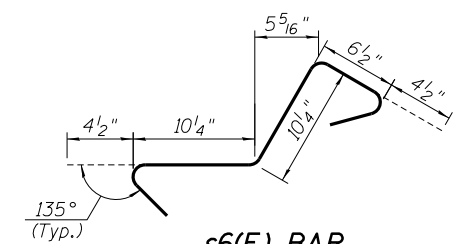
SECTION B-B

East Wall - North Bend

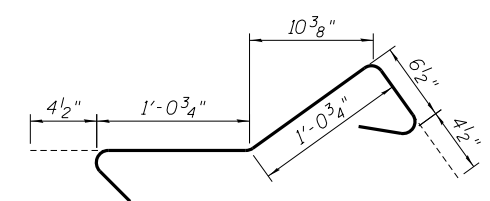


SECTION C-C

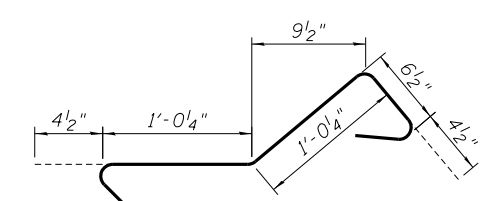
West Wall - South Bend



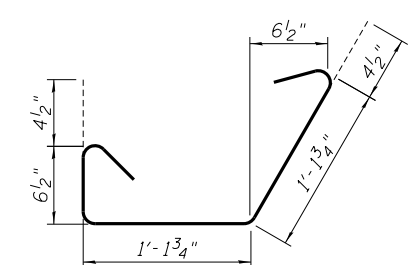
s6(E) BAR



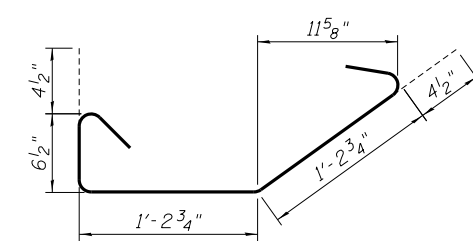
s8(E) BAR



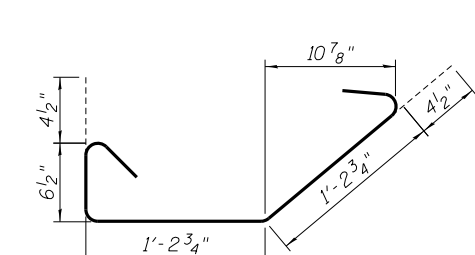
s10(E) BAR



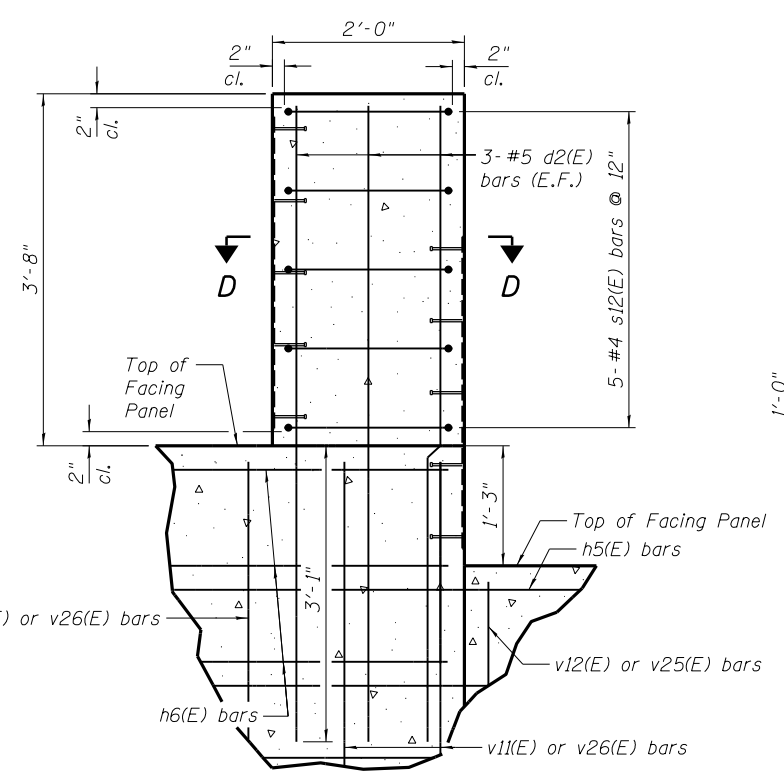
s7(E) BAR



s9(E) BAR

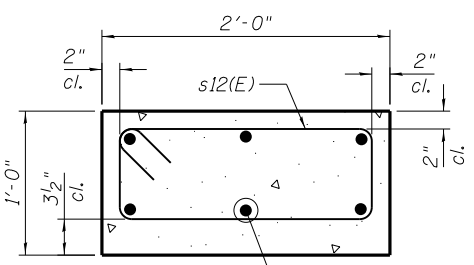


s11(E) BAR

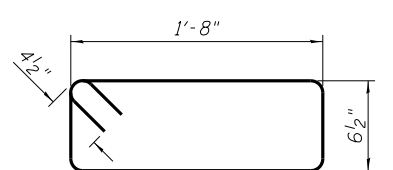


CABLE ANCHORAGE CONCRETE POST AT STEP IN WALL

Typ. of 2 Posts on Concrete Facing
See Sheet 15 of 17 for Railing Connection Details



SECTION D-D



s12(E) BAR

BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
d2(E)	48	#5	6'-10"	—	
s6(E)	10	#4	3'-0"	⌋	
s7(E)	10	#4	3'-9"	⌋	
s8(E)	5	#4	3'-5"	⌋	
s9(E)	5	#4	3'-9"	⌋	
s10(E)	5	#4	3'-4"	⌋	
s11(E)	5	#4	3'-9"	⌋	
s12(E)	10	#4	5'-2"	⌋	
Reinforcement Bars Epoxy Coated				Pound	470
Concrete Structures (Retaining Wall)				Cu. Yd.	2.0

Notes:

Railing posts shall be vertical.

Anchor rods shall be ASTM F1554, Gr. 55, galvanized steel all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor rods may be used in lieu of ASTM F1554. The anchor rods shall be hot-dipped galvanized according to AASHTO M232, Class C.

Tube segments shall have all corners ground to remove burrs or sharp projections.

All bolts, eyebolts, nuts and washers must satisfy the requirements of ASTM A307 Gr. A unless noted otherwise.

The anchor rods shall be installed according to Article 509.06 of the Standard Specifications. Embedment shall be 4" min. or according to the manufactures specifications whatever is greater.

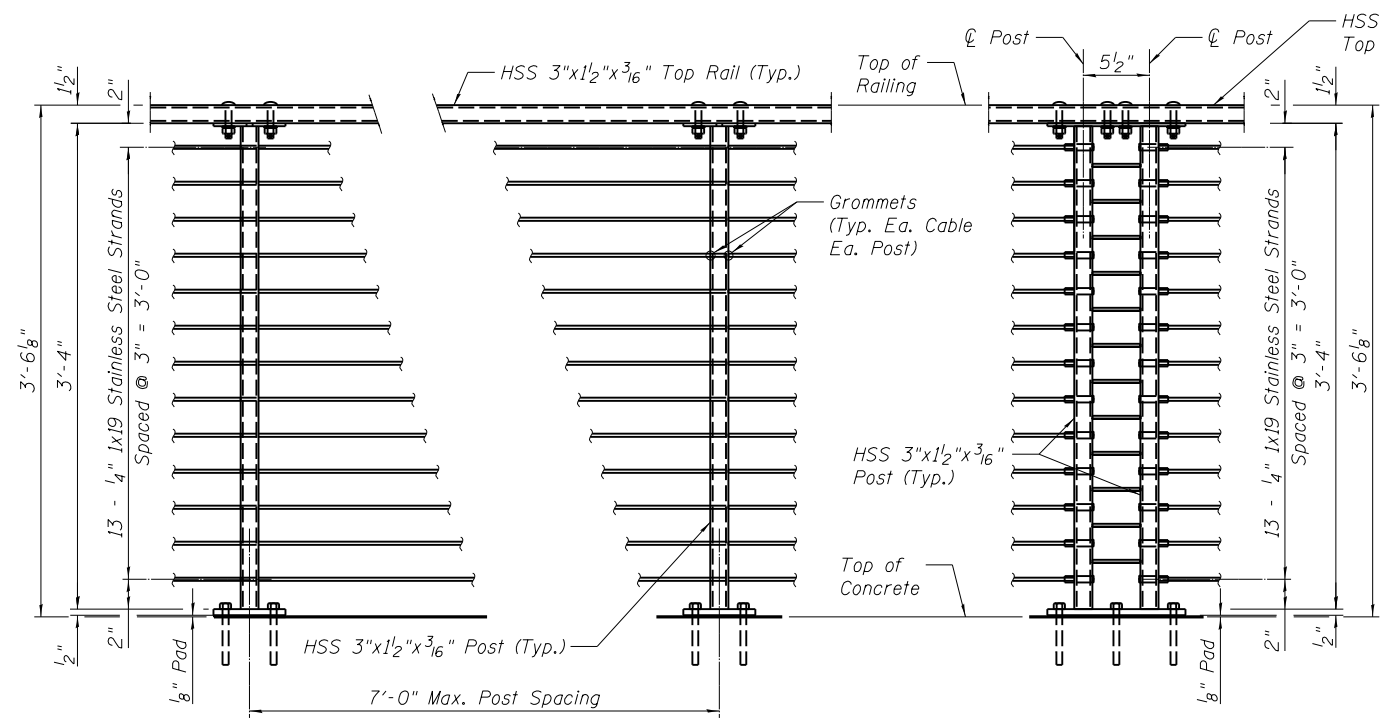
Structural steel plates and bars of the Steel Railing shall conform to the requirements of ASTM A36/36M.

Tubular steel posts shall be according to the requirements of ASTM A500, Grade B.

All steel rail members, with the exception of the stainless steel strand and fittings, shall be hot dipped galvanized according to 509.05 of the Standard Specifications.

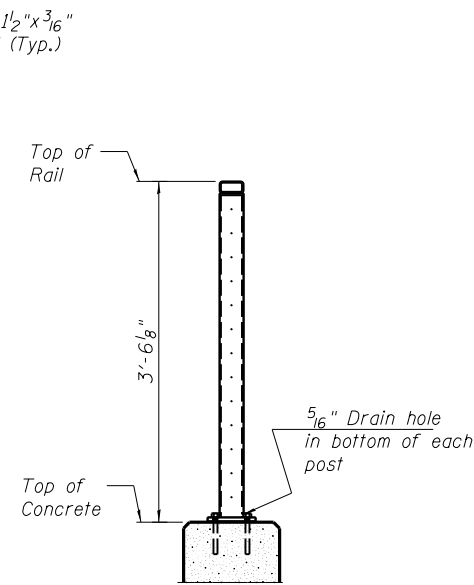
All studs shall be 1/2"φx4" granular or solid flux filled headed studs automatically end welded to plates.

See Sheets 8 thru 11 of 17 for rail post spacing.

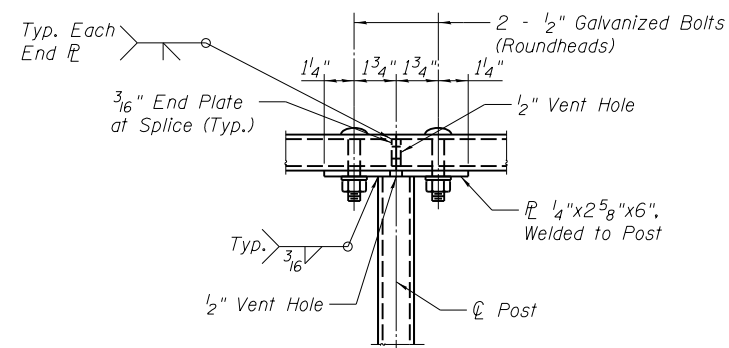


TYPICAL RAILING PANEL

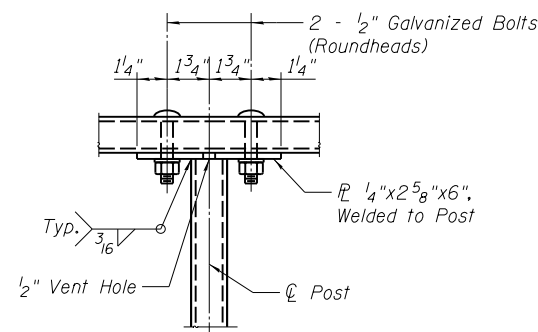
INTERMEDIATE TENSIONING POSTS



POST DETAIL



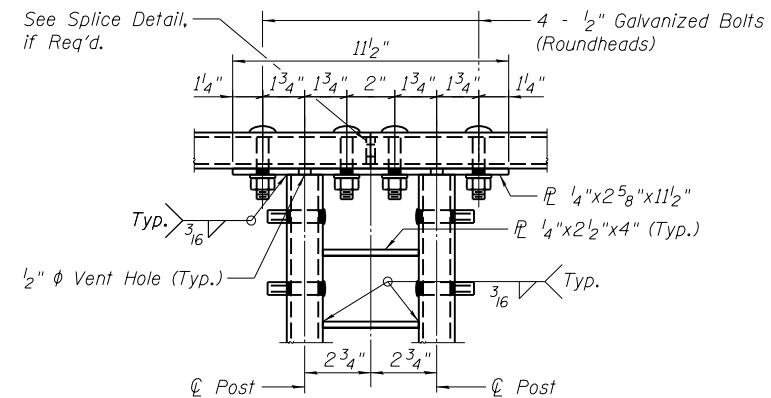
TOP RAIL - WITH SPLICE



TOP RAIL - NO SPLICE

TYPICAL RAIL/POST CONNECTION

(Strands not shown for clarity.)



TOP PLATE
INTERMEDIATE TENSIONING POSTS

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Steel Railing (Special)	Foot	456

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FINAL



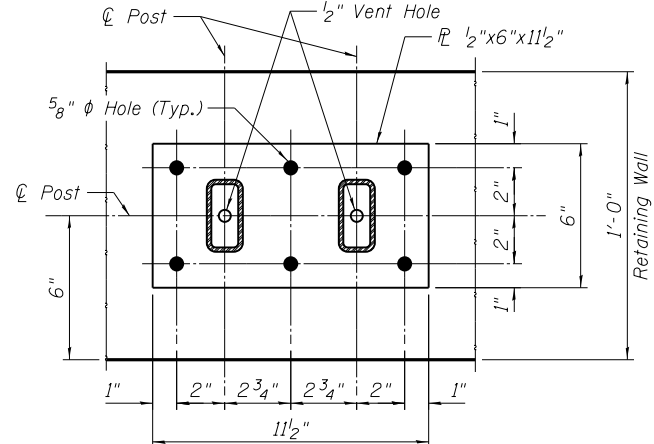
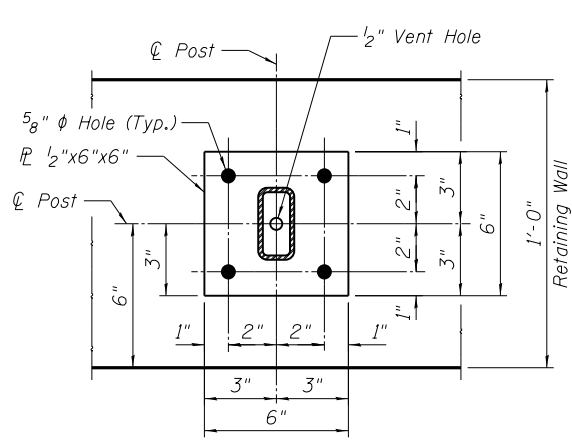
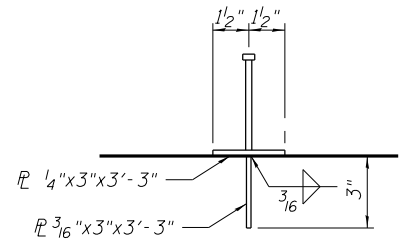
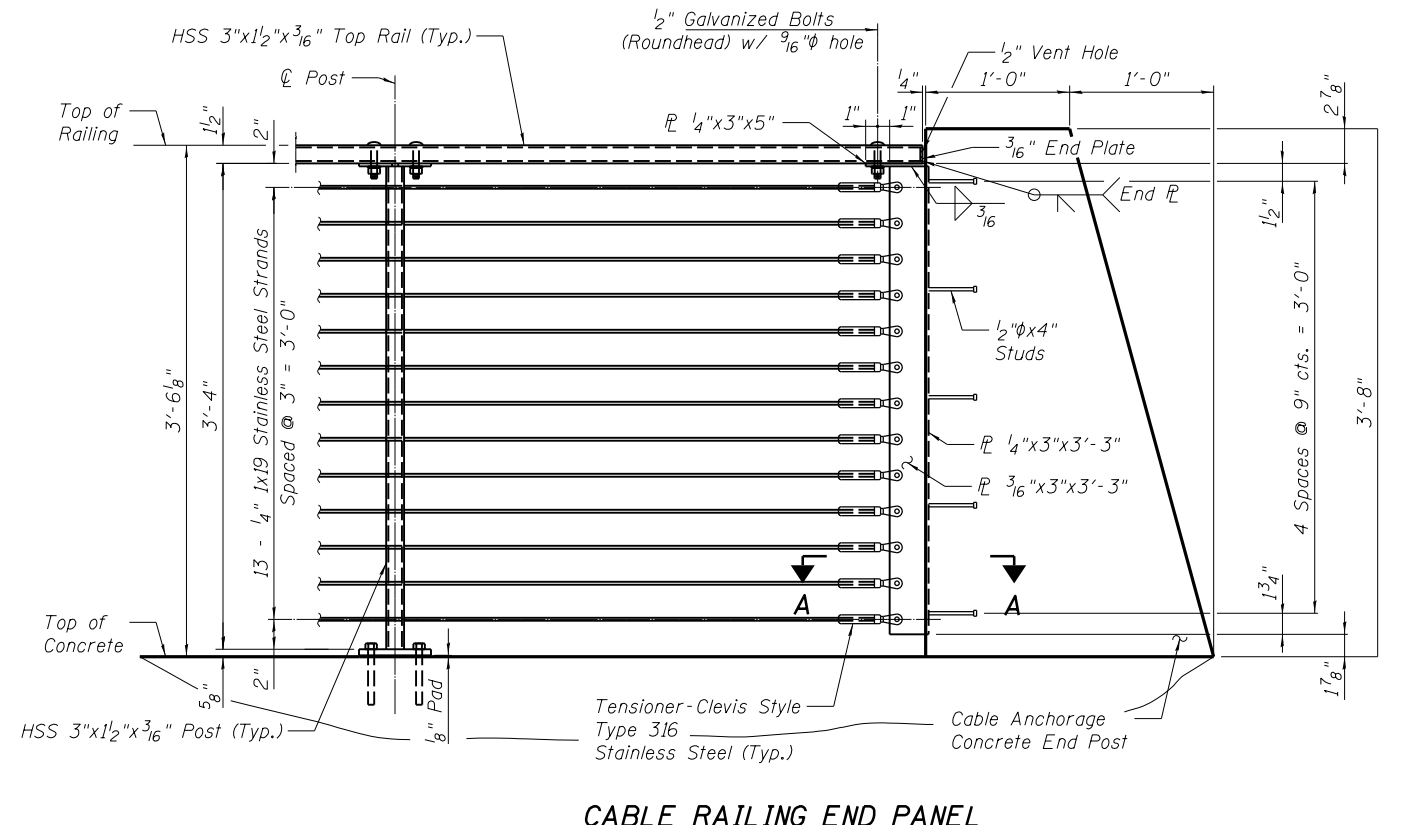
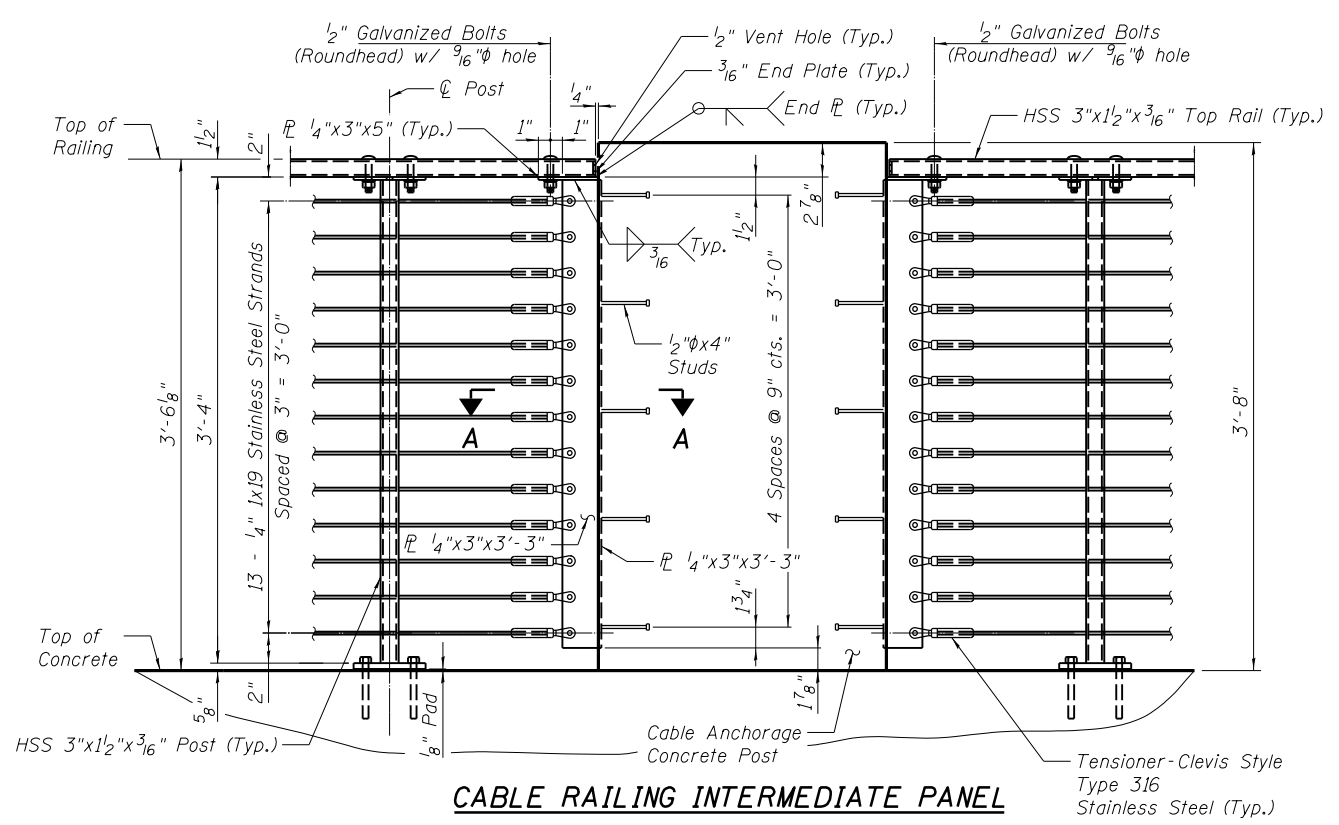
USER NAME = Pop00275	DESIGNED - RGC	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

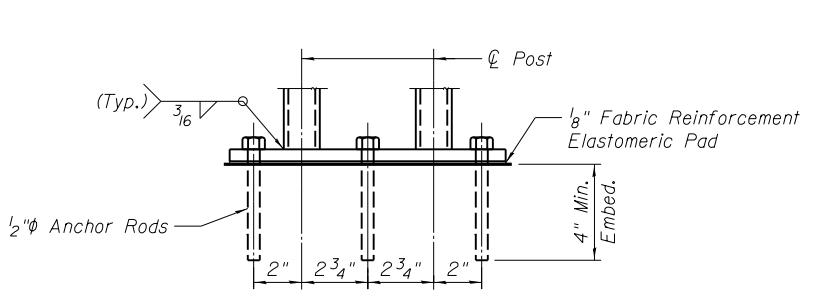
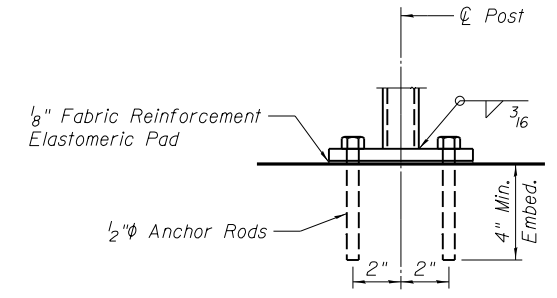
RAILING DETAILS
RETAINING WALLS - 5TH STREET

SHEET NO. 14 OF 17 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	230
			CONTRACT NO.	93733
ILLINOIS FED. AID PROJECT				



SECTION A-A



TYPICAL ANCHOR ROD DETAILS

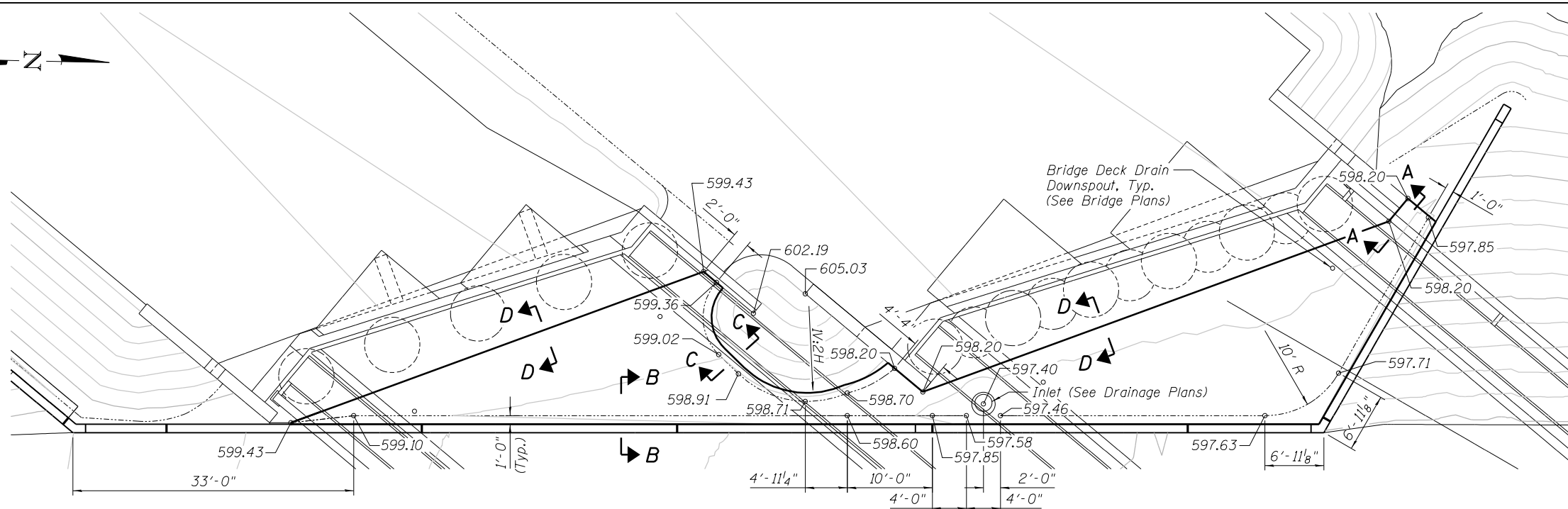
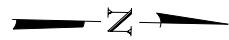
BASE PLATE DETAILS

USER NAME = Pop00275	DESIGNED - RGC	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

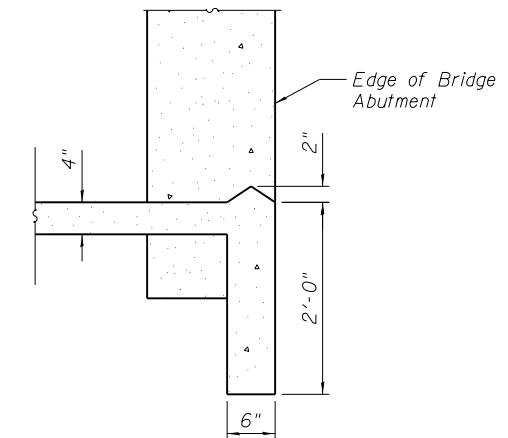
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
(109) VB,(110) VB-5		SANGAMON	382	231
			CONTRACT NO. 93733	

FINAL

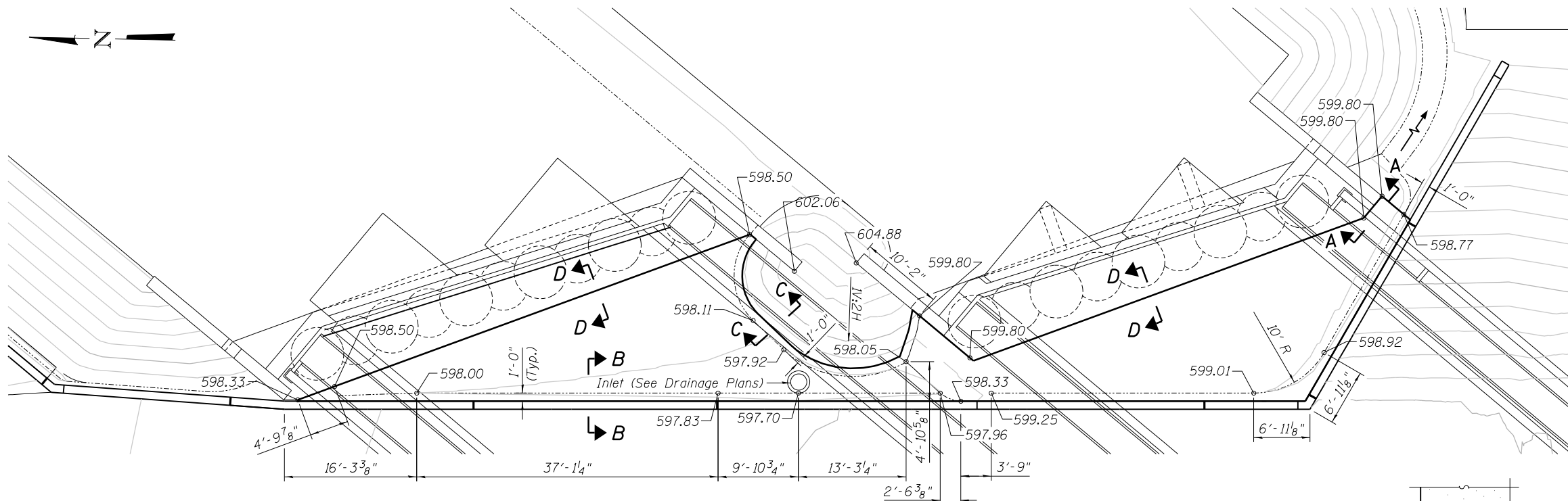
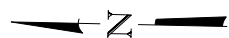




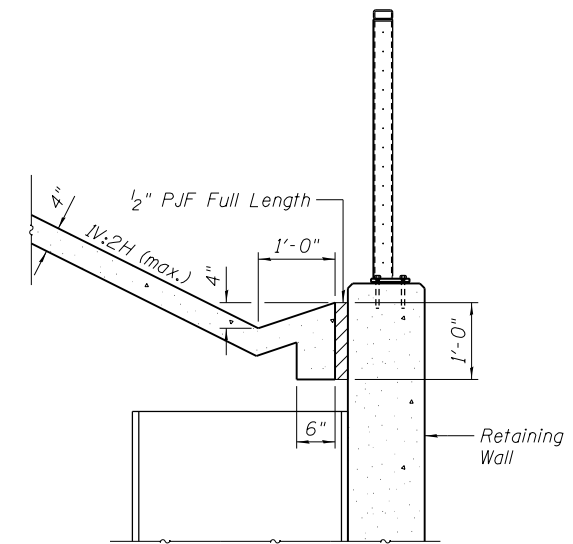
PLAN - WEST WALL



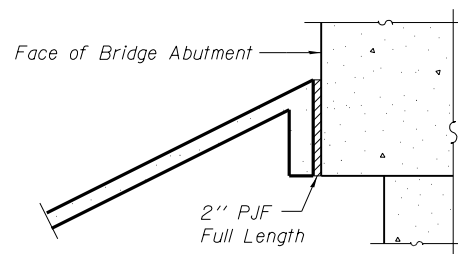
SECTION A-A



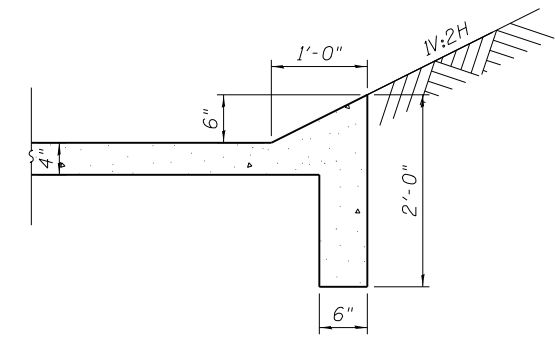
PLAN - EAST WALL



SECTION B-B



SECTION D-D



SECTION C-C

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Slope Wall 4 Inch	Sq. Yd.	300

Note:
Sloped wall shall be reinforced with welded wire fabric,
6" x 6" - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.

FINAL



USER NAME = Pop00275	DESIGNED - RGC	REVISIONS -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISIONS -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISIONS -
	CHECKED - RGC	REVISIONS -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SLOPE WALL DETAILS
RETAINING WALLS - 5TH STREET

SHEET NO. 16 OF 17 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	232
CONTRACT NO.			93733	
ILLINOIS FED. AID PROJECT				

B-3
Sta. 999+30, 27' LT
5/6/58

	N	Qu	w%	
600.8				CINDER, COAL, & misc. FILL.
	18			
596.8	16	1.60		Brown silty CLAY.
593.8	6	0.53		Brown & gray SILT, tr. clay.
593.3 Oh	6	1.06		
589.8	5	1.60		Brown SILT, tr. clay.
	4	1.06		
584.8	4	0.85		Gray silty CLAY, tr. small gravel.
	4	1.06		
578.8	97	6.10		Brown SILT.
	100/7"	6.94		
574.8				Gray decomposed SHALE.
573.3				100/6"
				Bottom of Hole = 27.5 feet

B-4
Sta. 999+93, 27' LT
5/6/58

	N	Qu	w%	
601.4				CINDER, COAL, & misc. FILL.
	7			
597.4	14	2.67		Brown silty CLAY.
594.4	9	0.53		Brown & gray SILT, tr. clay.
593.9 Oh	9	0.85		
589.9	7	0.53		Brown SILT, tr. clay.
	6	0.32		
585.4	4	0.53		Gray silty CLAY.
582.4	7	1.60		No Description.
579.4	39	8.54		Brown SILT.
576.4				100/7" 10.15
				Gray decomposed SHALE.
573.5				100/5"
				Bottom of Hole = 27.9 feet

B-147
Sta. 1000+21, 20' LT
9/10/13

	N	Qu	w%	
584.4				ASPHALT.
583.55	6	5		AGGREGATE.
583.35				Brown fine sandy SILT, some concrete fragments - FILL.
580.85	4	0.41B	22	Gray fine sandy silty CLAY, trace coarse sand and small gravel.
578.35	32	4.50P	14	Brown and gray SHALE. (HIGHLY WEATHERED SHALE)
575.85	80	4.50P	12	Gray SHALE.
	50/5"	4.50P	10	
569.35	50/4"		8	Gray clayey SHALE, micaceous.
				Rec. = 38% RQD = 38%
				Rec. = 96% RQD = 46%
				15.2
				Rec. = 93% RQD = 82%
				9.5
				Rec. = 71% RQD = 28%
				Rec. = 93% RQD = 0%
556.05				Rec. = 100% RQD = 0% COAL.
				Rec. = 90% RQD = 67%
549.15				2.5
548.35				Gray clayey SHALE, micaceous.
				Bottom of Hole = 36.0 feet

B-2
Sta. 1000+69, 27' RT
5/6/58

	N	Qu	w%	
601.4				Black CLAY FILL.
600.4				CINDER, COAL, & misc. FILL.
	14			
597.9	17	1.60		Brown & gray silty CLAY.
594.4	11			Brown & gray SILT, tr. clay. Became soft at 592.9.
593.9 Oh	3	0.53		
589.9	5	0.85		Brown SILT, tr. clay.
587.9	4	0.53		Brown & gray silty CLAY.
585.4	4	1.06		Gray silty CLAY.
	5	1.06		
579.4	34	10.15		Brown SILT, tr. clay.
	100/10"	8.54		
575.4				Gray decomposed SHALE.
573.2				100/6" 8.54
				Bottom of Hole = 28.2 feet

B-1
Sta. 1000+06, 27' RT
5/6/58

	N	Qu	w%	
601.8				CINDER, COAL, & misc. FILL.
	7			
598.8	10	2.67		Black silty CLAY.
595.8	10	1.60		Brown & gray SILT, tr. clay.
594.3 Oh	10	2.12		
590.3	7	0.53		Brown SILT, tr. clay.
585.8	5	0.85		Gray silty CLAY, tr. small gravel.
	5	2.67		
	5	1.60		
	6	1.39		
577.5				100 11.20
				Brown SILT.
575.3				100/7"
				Gray decomposed SHALE.
573.8				
				Bottom of Hole = 28.0 feet

LEGEND

N Standard Penetration Test N (blows/ft)
 Qu Unconfined Strength (tsf)
 w% Natural Moisture Content (%)

DD Water Surface Elevation Encountered in Boring
 DD during drilling
 Oh = at completion
 24h = 24 hours after completion

pw:\spsvr\306.hanson.dom\hanson_projects\Documents\09\Jobs\09L0179B\CAD\Struct\5th\Sheet\09L0179B-5thRetainingWallPlans

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	CHECKED - KMS	REVISED -
PLOT SCALE = 0.1667' / in.	DRAWN - EJM	REVISED -
PLOT DATE = 4/11/2019	CHECKED - RGC	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUBSURFACE DATA PROFILE
RETAINING WALLS - 5TH STREET

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	233
CONTRACT NO.			93733	

SHEET NO. 17 OF 17 SHEETS

ILLINOIS FED. AID PROJECT

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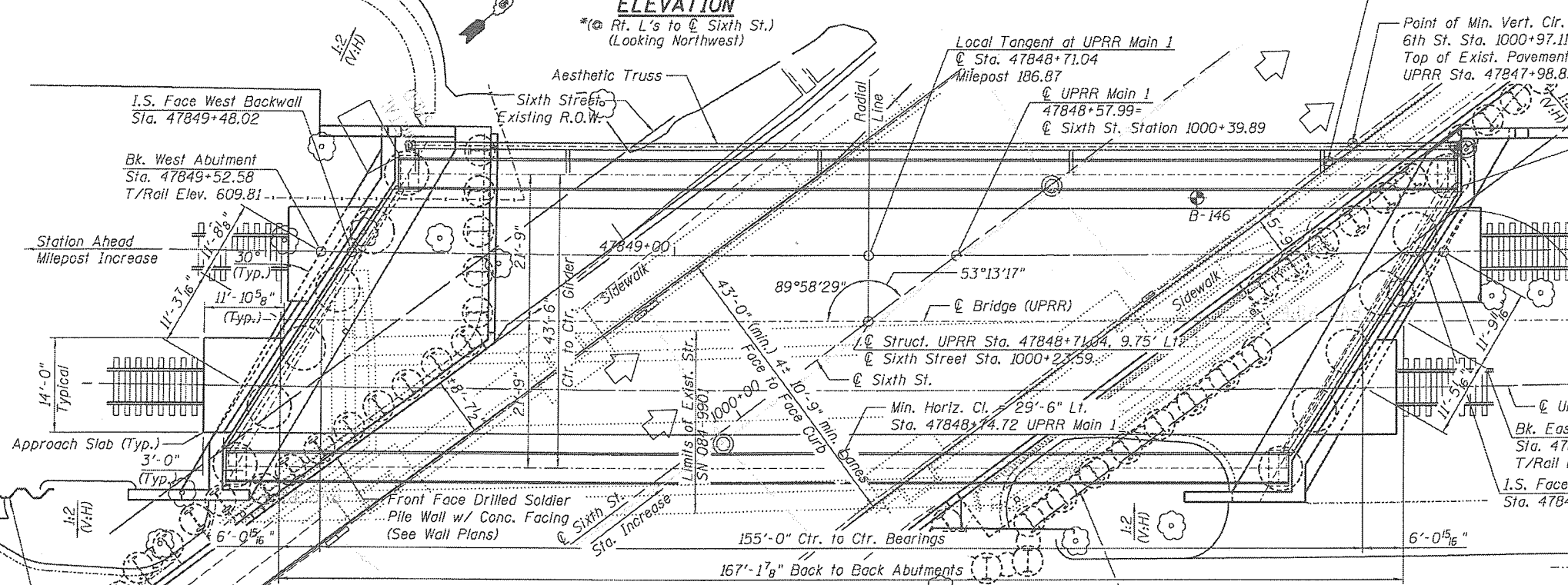
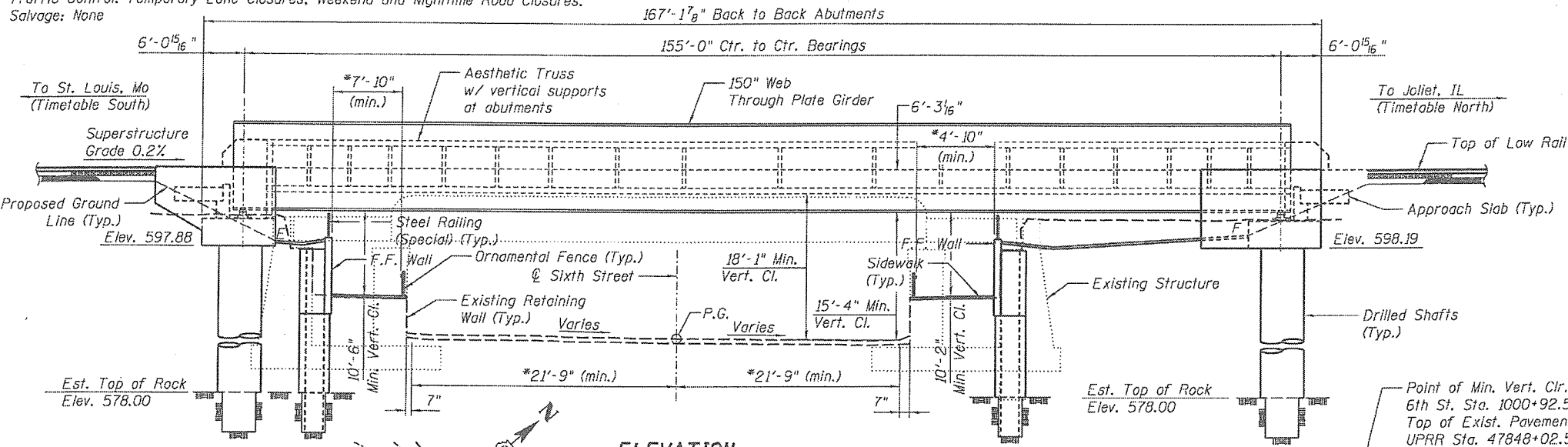
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Benchmark: BM# D2218-07 - Chiseled 'X' on West Bolt of fire hydrant - SE Quad 6th Street and Wellesly Avenue. Elevation = 598.884

Existing Structure: SN 084-9901 - Built in 1934 under 109-S-NRM. Three Span Steel through plate girder structure supported on closed abutments. Bk. to Bk. Abutment length is 116'-4" and ctr. to ctr. through girder width of 20'-0". Structure to be removed and replaced.

Construction Sequence: For Sequence and Details. See retaining wall General Data sheet. Traffic Control: Temporary Lane Closures, Weekend and Nighttime Road Closures. Salvage: None

Railroad utilities may exist within existing NSRR right-of-way. Prior to the start of any construction or excavation, utility relocations will have to be coordinated with the NSRR.



DESIGN SPECIFICATIONS
2017 AREMA Specifications
Live Load Deflection: L/640
Composite Design for Floorbeam Defl. Req.
Design Speed: 50 m.p.h.

DESIGN STRESSES
FIELD UNITS
f'c = 4,000 psi
fy = 60,000 psi (Reinforcement)
fy = 50,000 psi (ASTM A709 Grade 50)

LOADING COOPER E-80
Impact: Diesel Impact
Allow 30" of Ballast Dead Load

PLAN

Indicates Boring Location

SEISMIC DATA
AREMA

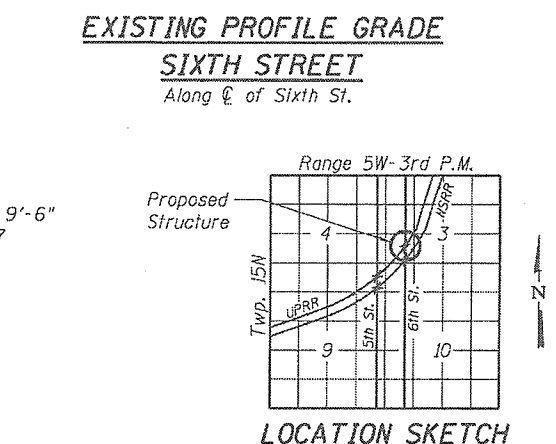
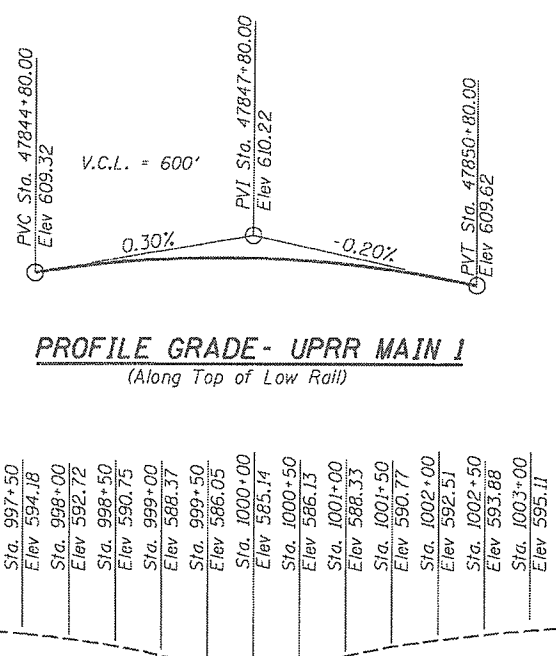
Ground Motion Level	PGA	S _g	S _i
Level 1 (100 Year)	0.010	0.025	0.005
Level 2 (475 Year)	0.040	0.090	0.035
Level 3 (2475 Year)	0.10	0.22	0.10

Soil Site Class = C

CURVE DATA
(UPRR Main 1)

P.I. Sta. 47824+35.33
Δ = 37°24'41" (Rt.)
D = 0°41'30"
T = 2,804.81'
L = 5,408.86'
R = 8,283.78
E = 461.96'
Long Chord = 5,313.32'
Mid. Ord. = 437.56
S.E. = 3/4"

S.C. Sta. = 47796+30.51
C.S. Sta. = 47850+39.38



I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AREMA Specifications.

MATTHEW J. WILLEY
REGISTERED STRUCTURAL ENGINEER
STATE OF ILLINOIS
No. 081-006588
4-12-2019
LIC. EXP. DATE: 11-30-2020

APPROVED
For Structural Adequacy Only

GENERAL PLAN & ELEVATION
UPRR (MP 186.87) OVER BUSINESS 55 (6TH ST.)
F.A.P. 666 - SECTION (109)VB, (110)VB-5
SANGAMON COUNTY
UPRR SUBDIVISION-SPRINGFIELD
STATION 47848+71.04
STRUCTURE NO. 084-9962

Matthew J. Willey
Engineer of Bridges & Structures

GENERAL NOTES

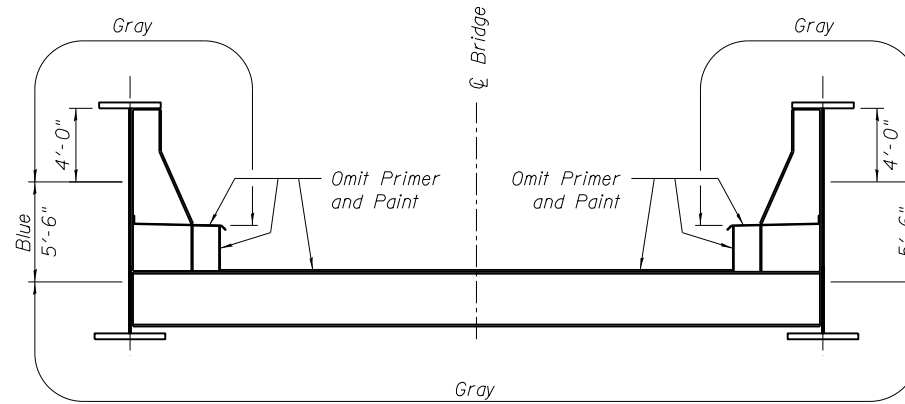
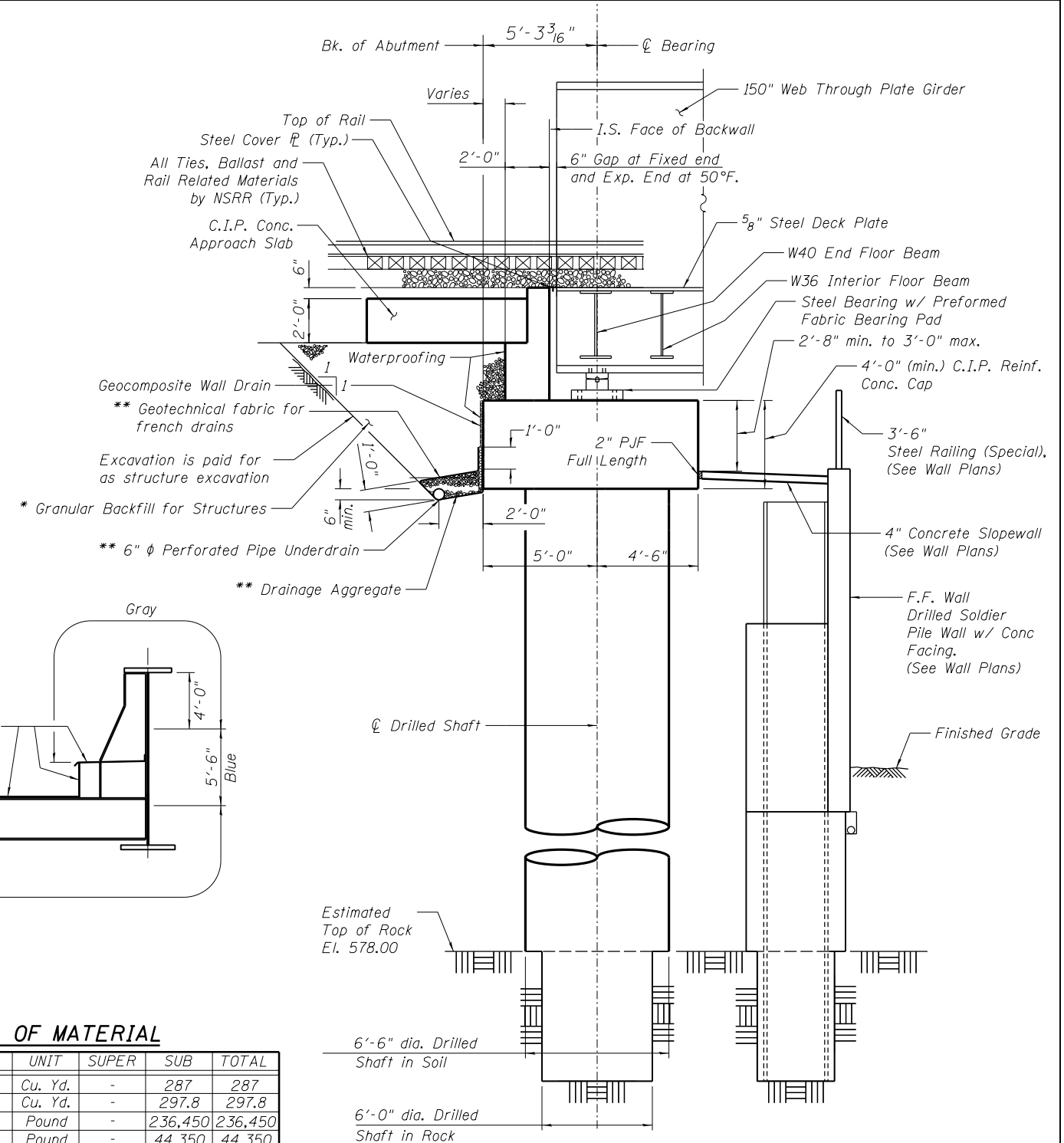
- Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts 1/2 in. φ, holes 5/16 in. φ, unless otherwise noted.
- Calculated weight of Structural Steel, ASTM A709, Gr. 50 = 1,681,926 lbs.
 ASTM A36, Gr. 36 = 31,558 lbs.
 ASTM A500, Gr. 46 = 22,194 lbs.
- All structural steel shall be ASTM A709 Grade 50 unless otherwise noted on the plans.
- All substructure concrete shall have a compressive strength of 4,000 psi at 14 days.
- No field welding is permitted except as specified in the contract documents.
- Reinforcement bars designated (E) shall be epoxy coated.
- Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8 inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- Concrete Sealer shall be applied to the following surfaces:
 Abutments - inside face of backwall, inside face of cheekwall, top of cap, (except surfaces coated with surface color treatment).
 Concrete Surface Color Treatment shall be applied to the following surfaces:
 Abutments - concrete facing, wingwall and cheekwall surfaces coated with concrete surface color treatment.
- The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. All coatings on faying surfaces shall satisfy RSCS requirements for Class B slip coefficient. The color of the final finish coat for girder flanges, all interior steel surfaces, bottom of deck plate, and aesthetic truss shall be gray, Munsell No. 5B 7/1. The color of the final finish coat for a 5.5 foot tall strip on the exterior face of girder web starting 4 foot down from the top flange shall be blue, Munsell No. 10B 3/6. See painting diagram for more information.
- Waterproofing shall be applied to the backside of the abutment cap and backside of wingwalls for surfaces below ground. This shall be according to Article 503.18 of the Std. Spec. Cost included with Concrete Structures.

Drilled shaft cross-hole sonic log (CSL) testing:

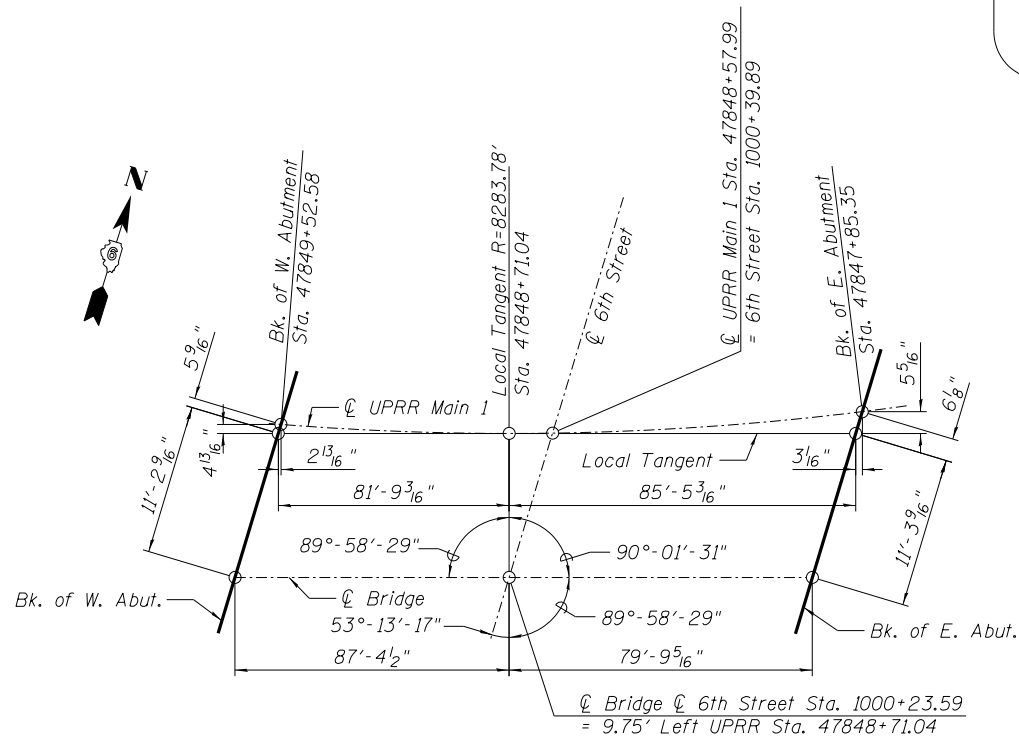
- Drilled shafts shall be evaluated by cross-hole sonic log testing. Testing pipes shall be installed in each drilled shaft to facilitate the logging process, which will follow completion of each shaft.
- Furnish and install six standard 2 inch nominal diameter steel pipes (ASTM A53, Grade B) for use in CSL testing of each drilled shaft. Pipes shall be equally spaced around the interior of the reinforcing steel cage.
- Pipes shall be fitted with a screw-on watertight shoe and cap and shall be securely fixed to the interior of the reinforcing steel cage. Watertight joints shall be used to achieve the required length. The pipes shall be filled with water and plugged or capped before concrete placement. The upper end of the pipe shall not be left open during or after concrete placement. The pipes shall extend at least 2'-6" above the top of the drilled shaft concrete.
- CSL testing will be completed by the Engineer at no cost to the Contractor. If CSL test results are unsatisfactory according to the Engineer, the Contractor shall propose a method of correction including designs if required to the Engineer for approval. The correction shall be at the expense of the Contractor.

INDEX OF SHEETS

- General Plan & Elevation
- General Data
- Foundation Layout
- Sheet Piling
- Typical Section
- Framing Plan
- Outside Elevation of Girder (1 of 2)
- Outside Elevation of Girder (2 of 2)
- Inside Elevation of Girder (1 of 2)
- Inside Elevation of Girder (2 of 2)
- Typical Sections
- Girder Sections & Details
- Girder Splice Details
- Walkway and Ballast Plate Plan
- Walkway and Ballast Plate Details
- Miscellaneous Girder Details (1 of 3)
- Miscellaneous Girder Details (2 of 3)
- Miscellaneous Girder Details (3 of 3)
- Aesthetic Truss
- TPG Bearing Details
- End Floorbeam Bearing Details
- Bridge Deck Waterproofing
- West Abutment
- West Abutment Details
- West Abutment Bill of Material
- East Abutment
- East Abutment Details
- East Abutment Bill of Material
- Subsurface Data Profile



PAINTING DIAGRAM



OFFSET SKETCH

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Structure Excavation	Cu. Yd.	-	287	287
Concrete Structures	Cu. Yd.	-	297.8	297.8
Reinforcement Bars	Pound	-	236,450	236,450
Reinforcement Bars, Epoxy Coated	Pound	-	44,350	44,350
Name Plates	Each	-	1	1
Drilled Shaft in Soil	Cu. Yd.	-	295.5	295.5
Drilled Shaft in Rock	Cu. Yd.	-	188.4	188.4
Membrane Waterproofing (Special)	Sq. Ft.	6,293	-	6,293
Concrete Sealer	Sq. Ft.	-	1,807	1,807
Geocomposite Wall Drain	Sq. Yd.	-	54	54
Drainage System, No. 3	Each	1	-	1
Crosshole Sonic Logging Access Ducts	Foot	-	2,703	2,703
Concrete Surface Color Treatment	Sq. Ft.	-	12	12
Granular Backfill for Structures	Cu. Yd.	-	188	188
Furnishing and Erecting Structural Steel, Bridge No. 3	L. Sum	1	-	1
Permanent Sheet Piling	Sq. Ft.	-	566	566
Pipe Underdrains for Structures, 6"	Foot	-	175	175

ABUTMENT SECTION
(At Rt. L's to Back of Abutment)

Notes:
West Abutment Section is Shown, East Similar.

* Granular Backfill for Structures Shall Be Placed and Compacted According to Section 502.10 of the Standard Specifications.

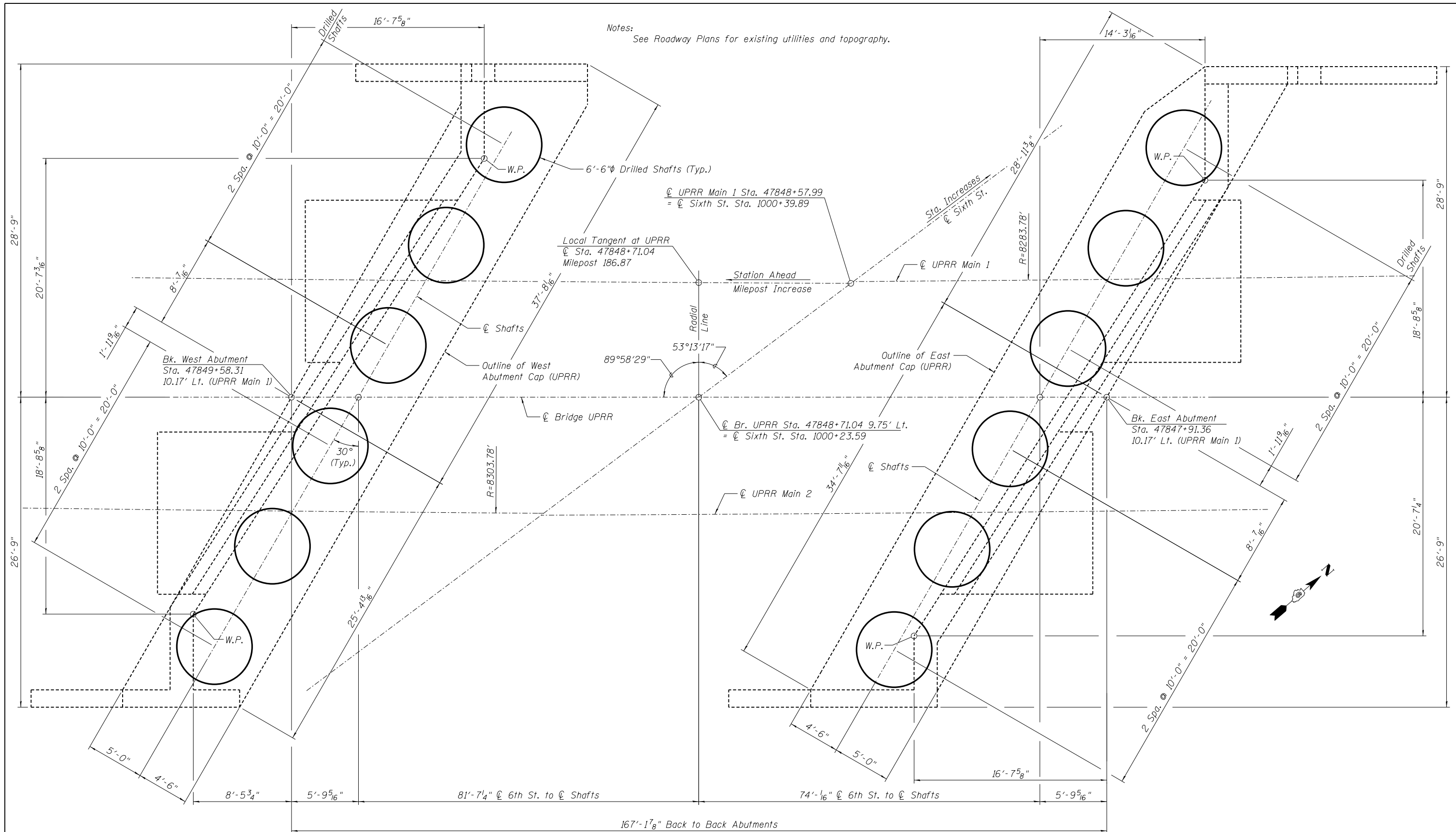
** Included in the cost of "Pipe Underdrains for Structures, 6". For additional drainage details see Railway Plans.

UNION PACIFIC RAILROAD
S.N. 084-9962 BUILT 20__ BY
CITY OF SPRINGFIELD
SEC. (109)VB, (110)VB-5
STATION 47848+71.04
MILE POST 186.87
LOADING COOPER E-80

NAME PLATE
See Std. 515001

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PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(109) VB,(110) VB-5	SANGAMON	382	235
			CONTRACT NO.	93733



Notes:
See Roadway Plans for existing utilities and topography.

FOUNDATION LAYOUT PLAN

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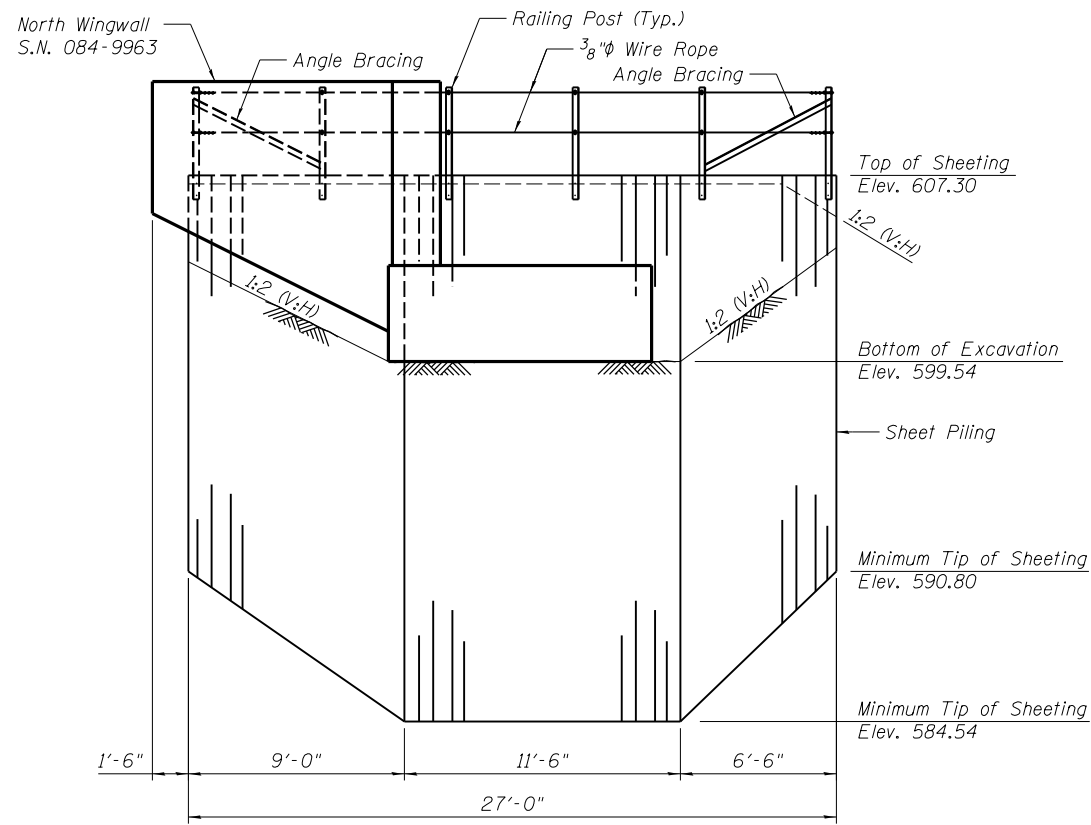
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FOUNDATION LAYOUT
STRUCTURE 084-9962 - 6TH ST UPRR**

SHEET NO. 3 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	236
			CONTRACT NO. 93733	

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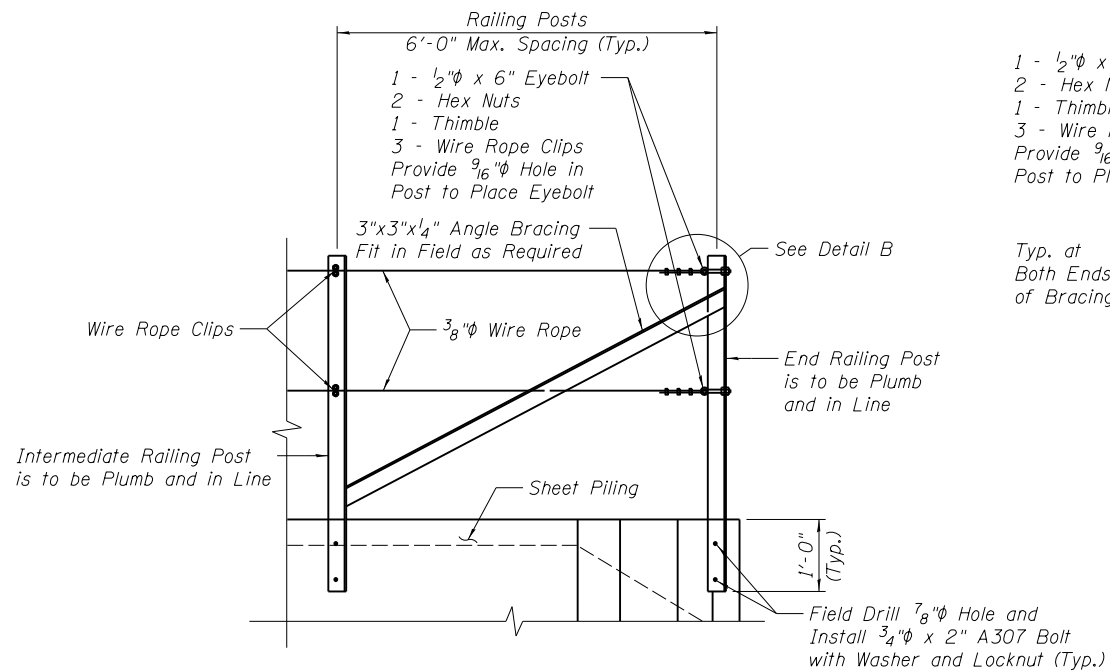


Minimum Section Modulus = 36.0 in³/ft
 Minimum Moment of Inertia = 220.00 in⁴/ft

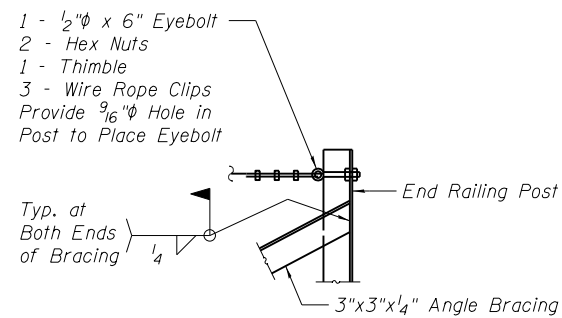
SHEET PILING

(Looking North)

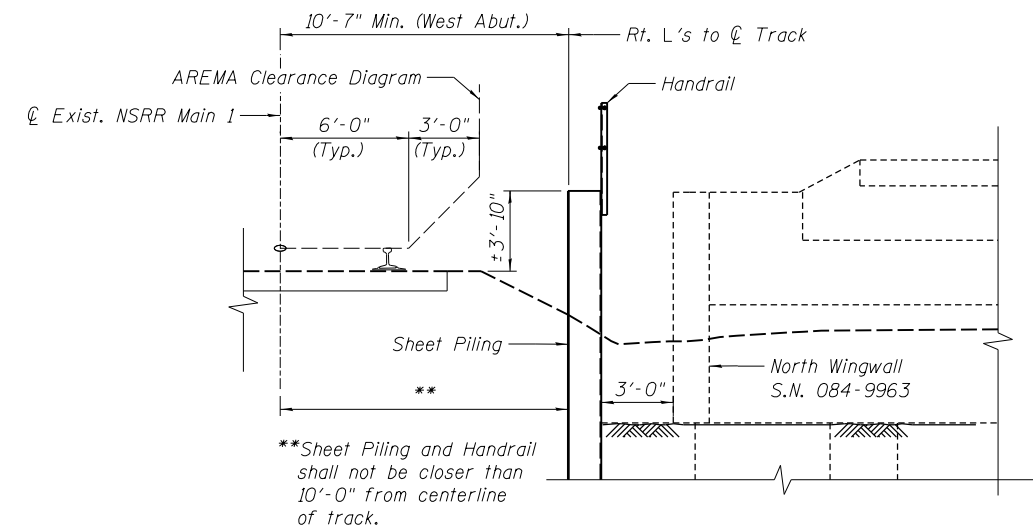
See S.N. 084-9963 Sheet 3 of 29 for Plan View



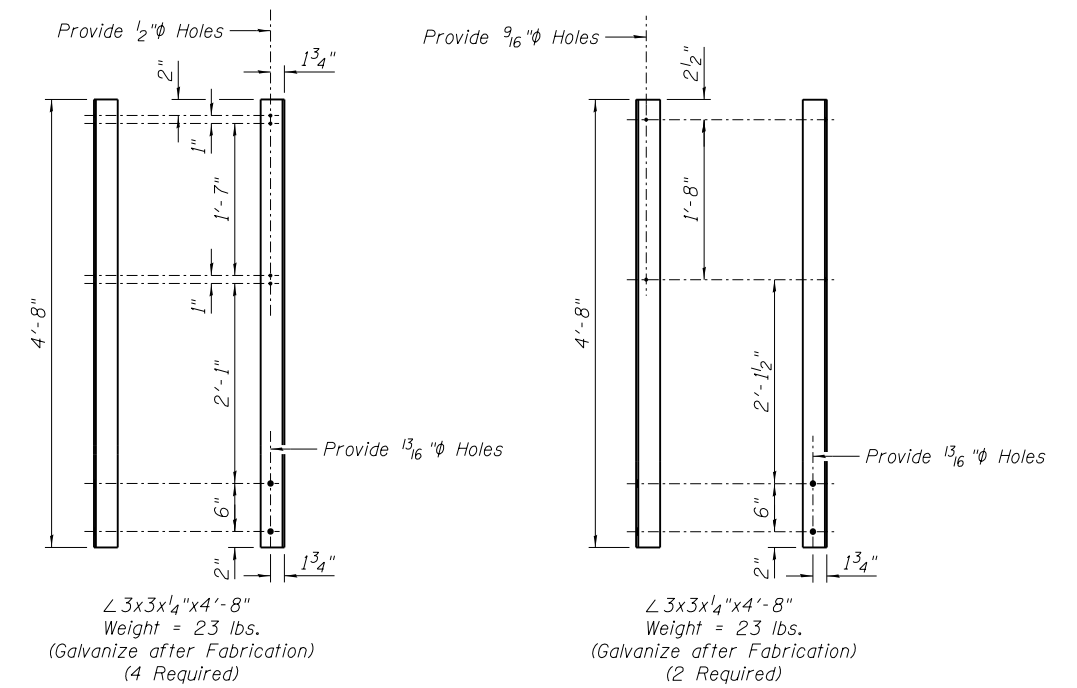
DETAIL OF RAILING POST BRACE AT SHEET PILE WALL ENDS



DETAIL B



HANDRAIL DETAIL
(Looking East)



INTERMEDIATE RAILING POST

END RAILING POST

Notes:
 All Handrail components and hardware shall be galvanized.

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SHEET PILING
 STRUCTURE 084-9962 - 6TH ST UPRR

SHEET NO. 4 OF 29 SHEETS

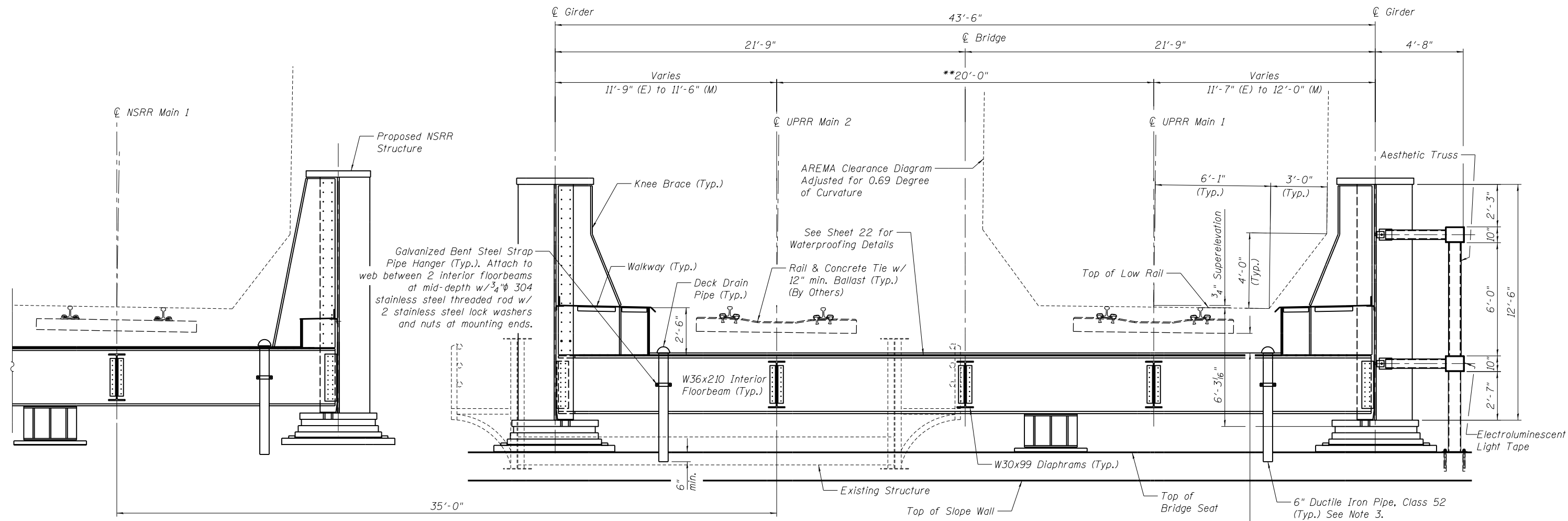
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(109) VB,(110) VB-5	SANGAMON	382	237
			CONTRACT NO.	93733

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TYPICAL SECTION - 6TH ST. (UPRR)
(Looking West)

**Dimensions are at Rt L's to \varnothing Track

Legend:
(E) End of Bridge
(M) Middle of Bridge

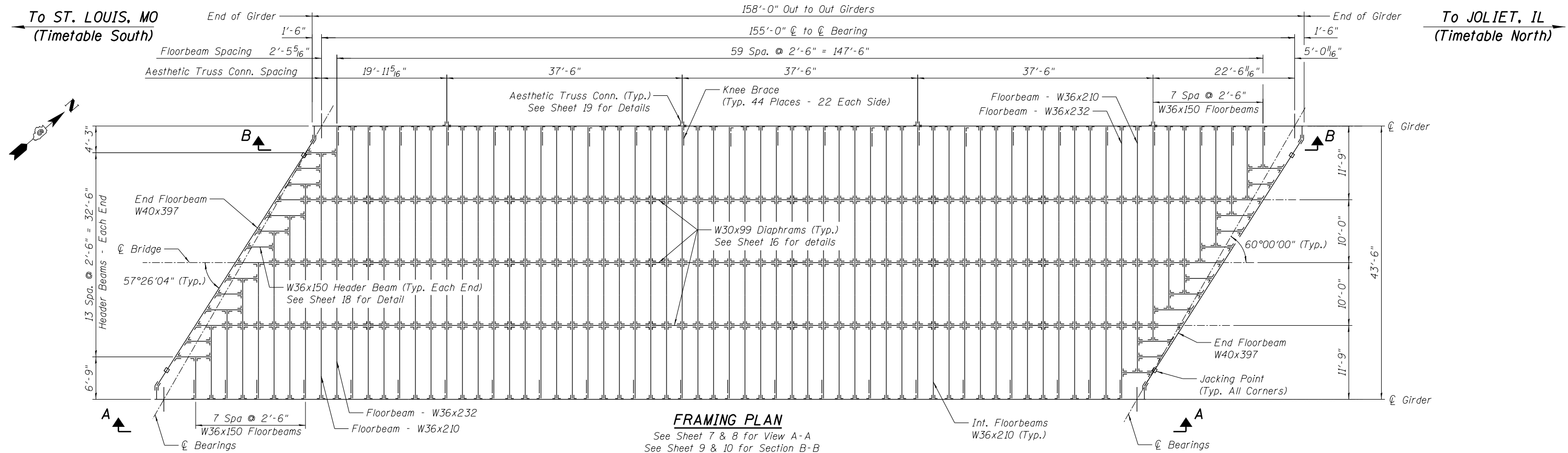
- Notes:
1. Retaining Wall and Steel Railing not shown for clarity.
 2. Drain pipe on west end only near low end of bridge deck.
 3. With the ductile iron pipe fitted to the bottom of the deck drain bottom pan downspout, drill 4 holes through ductile iron pipe and downspout. Holes shall be aligned with the 4 quadrants of the pipe. Attach ductile iron pipe to downspout with 4 stainless steel carriage bolts. Rounded heads of carriage bolts shall be oriented towards the center of the pipe.
 4. Cost of deck drain pipe, bottom pan, downspout, brackets and other hardware shall be included in the cost of Drainage System.

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CHECKED - TJH/TDP	REVISED -
DRAWN - RSJ	REVISED -
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TYPICAL SECTION
STRUCTURE 084-9962 - 6TH ST UPRR**
SHEET NO. 5 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	238
CONTRACT NO. 93733				
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				



STEEL NOTES

GENERAL: All materials, fabrication, and erection shall be in accordance with chapter 15 of the current AREMA Manual for Railway Engineering.

Dead Load: (assumed)	
Rail	400
Ballast (Incl. Tie)	7,800
Waterproofing	190
Future Ballast	2,510
Steel	11,000
Total	21,900 lbs. per lin ft. of track

MATERIAL: Zone 2 Conditions control for Charpy V-Notch testing.

Fracture Critical Members (FCM) shall be Charpy V-Notch tested. According to AREMA Table 15-9-3, Zone 2, P frequency in accordance with ASTM A673.

Impact Test Required (ITR) members shall be Charpy V-Notch (CVN) tested, according to AREMA Table 15-9-2, Zone 2, H frequency in accordance with ASTM A673.

FABRICATION: The top surface of beams shall be adjusted to form a straight line at any transverse section throughout the span. Tolerance is plus or minus 1/8\".

SPLICE NOTES:

- No two parts or members shall be spliced by shop welding at the same location, or within the length of a bolted field splice.
- Web splices by shop welding shall be located a minimum of 36\" away from any flange splice.
- Splices of the web or flanges shall not be permitted within the central 30'-0\" or the girder span length. This requirement may be waived only by the approval of the Engineer.

MOMENT & SHEAR TABLE FOR STEEL THRU PLATE GIRDER

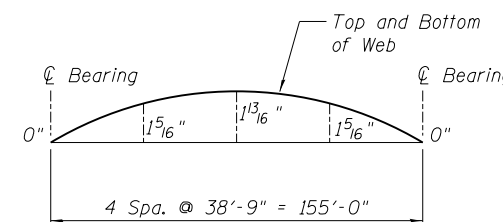
DESCRIPTION	MOMENT	SHEAR
Dead Load	32,884 ft. -k	849 k
Live Load	31,109 ft. -k	881 k
Impact	7,111 ft. -k	201 k
Total	71,104 ft. -k	1,931 k
Section	See Sheet 12 of 29	
Steel	A.S.T.M. A709, Gr. 50	
Net I	2,550,926 in ⁴	
Net S (Bot.)	31,375 in ³	
f _{st} (Bot.)	27.3 ksi	
Gross I	2,777,585 in ⁴	
Gross S (Top)	33,807 in ³	
f _{sc} (Top)	25.3 ksi	

I- Moment of Inertia of the Section
 S- Section Modulus
 fs- Max. Unfactored Stress in the Section Due to D.L. + L.L. + Impact

MOMENT & SHEAR TABLE FOR STEEL FLOORBEAMS

DESCRIPTION	MOMENT	SHEAR	MOMENT *	SHEAR *
Dead Load	255 ft. -k	21.3 k	4,667 ft. -k	849 k
Live Load	230 ft. -k	20.4 k		
Impact	681 ft. -k	60.7 k		
Total	1,166 ft. -k	102.4 k	4,667 ft. -k	849 k
Section	W36x210		W40x397	
Steel	A.S.T.M. A709, Gr. 50		A.S.T.M. A709, Gr. 50	
Net I	12,886 in ⁴		28,366 in ⁴	
Net S	702 in ³		1384 in ³	
fs	19.9 ksi		40.5 ksi	

*Jacking Conditions Control 50% Allowable Stress Increase is Permitted

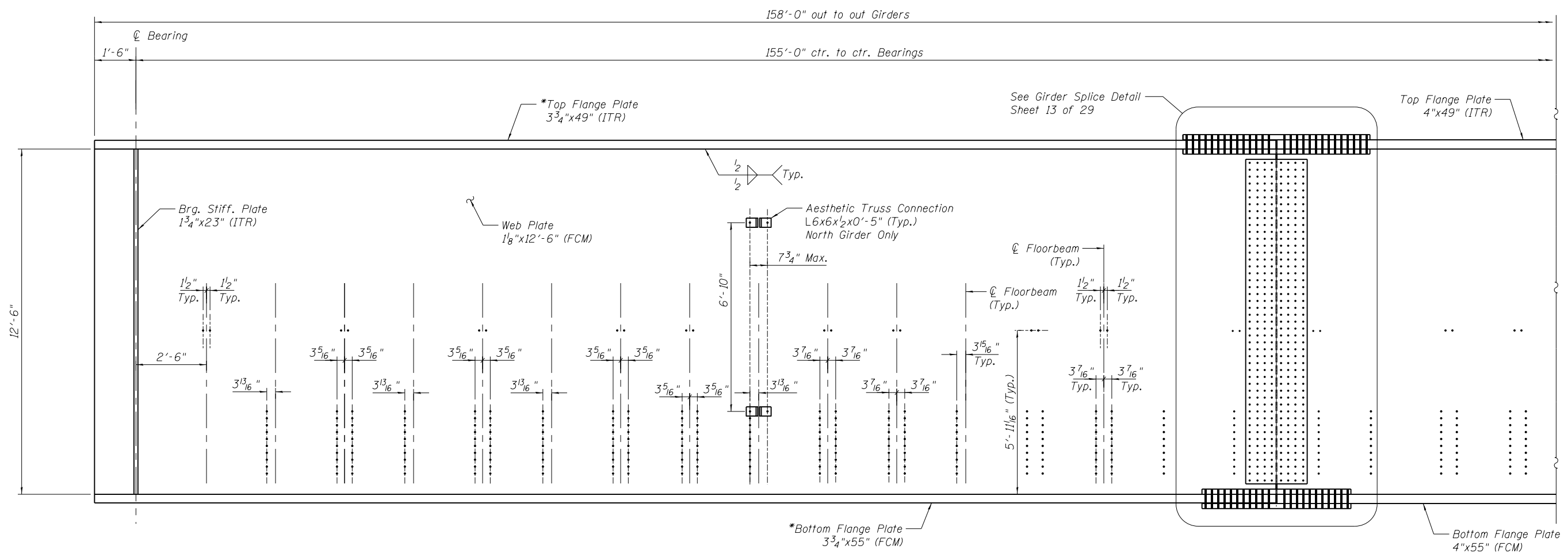


CAMBER DIAGRAM

Camber Calculated for Dead Load Only

To ST. LOUIS, MO
(Timetable South)

To JOLIET, IL
(Timetable North)



VIEW A-A - OUTSIDE ELEVATION OF GIRDER

Note:
1. FCM - Fracture Critical Member
2. ITR - Impact Test Required

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

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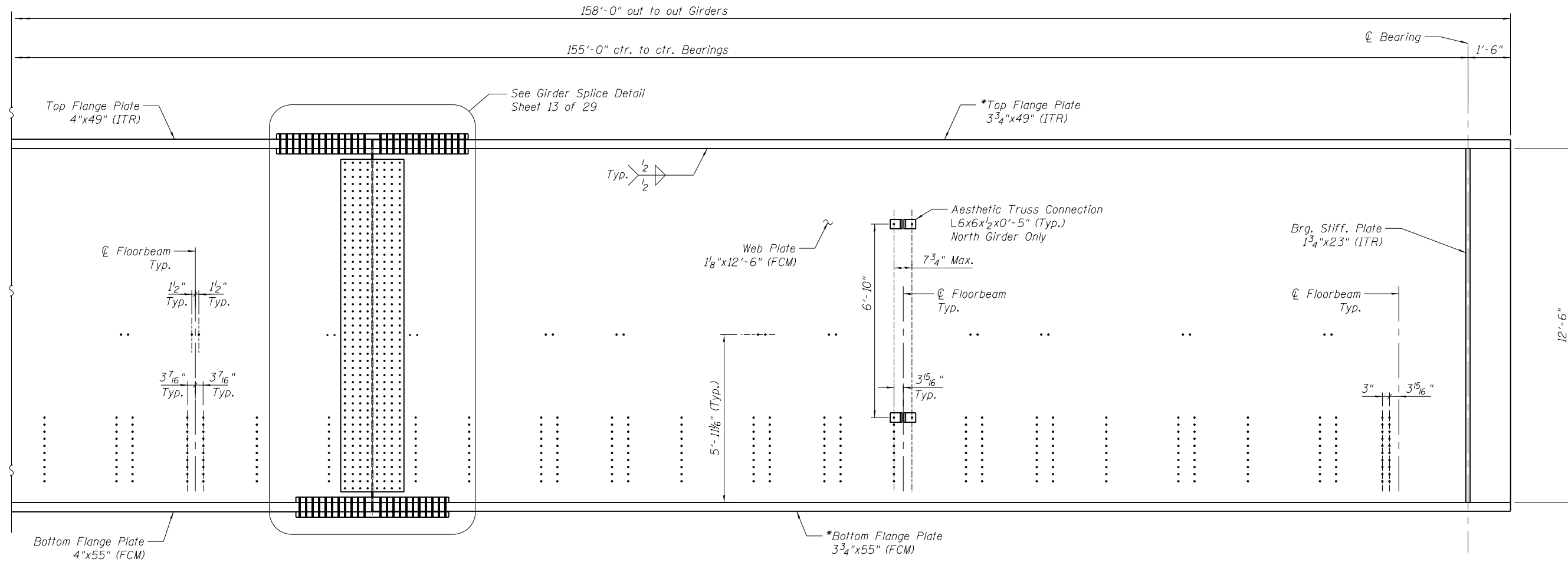
OUTSIDE ELEVATION OF GIRDER - SHEET 1 OF 2
STRUCTURE 084-9962 - 6TH ST UPRR

SHEET NO. 7 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(109) VB,(110) VB-5	SANGAMON	382	240
			CONTRACT NO. 93733	
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

To ST. LOUIS, MO
(Timetable South)

To JOLIET, IL
(Timetable North)



VIEW A-A - OUTSIDE ELEVATION OF GIRDER

Note:
1. FCM - Fracture Critical Member
2. ITR - Impact Test Required

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

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OUTSIDE ELEVATION OF GIRDER - SHEET 2 OF 2
STRUCTURE 084-9962 - 6TH ST UPRR

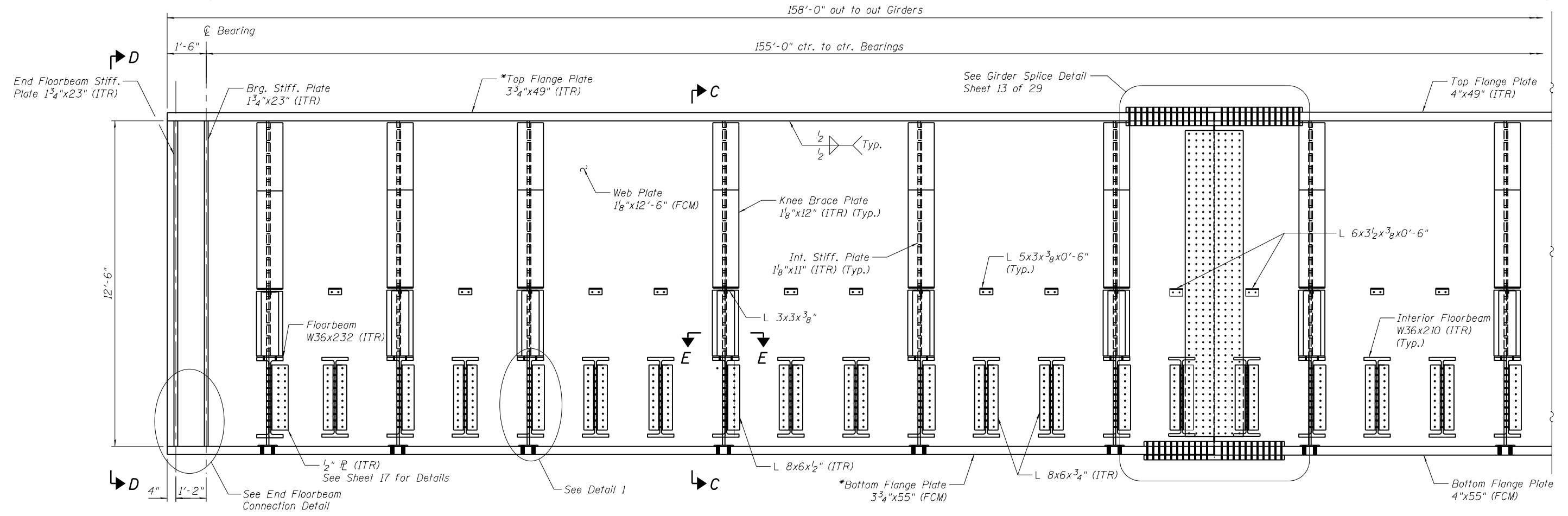
SHEET NO. 8 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO.			93733	
*666 & 666 ALT.		ILLINOIS FED. AID PROJECT		

FINAL

To ST. LOUIS, MO
(Timetable South)

To JOLIET, IL
(Timetable North)

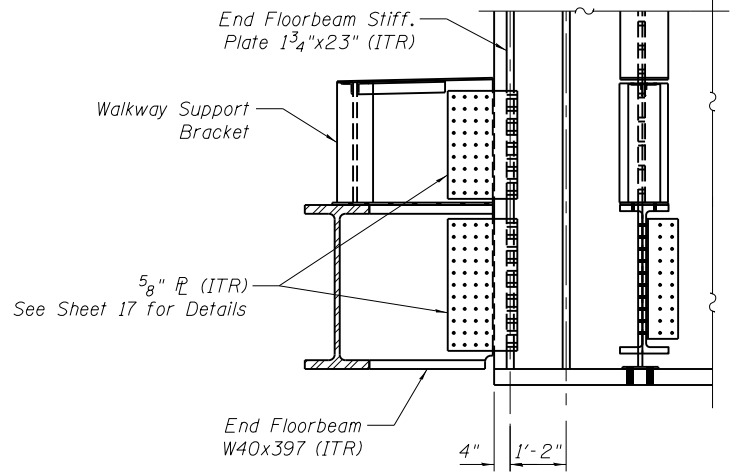


SECTION B-B - INSIDE ELEVATION OF GIRDER

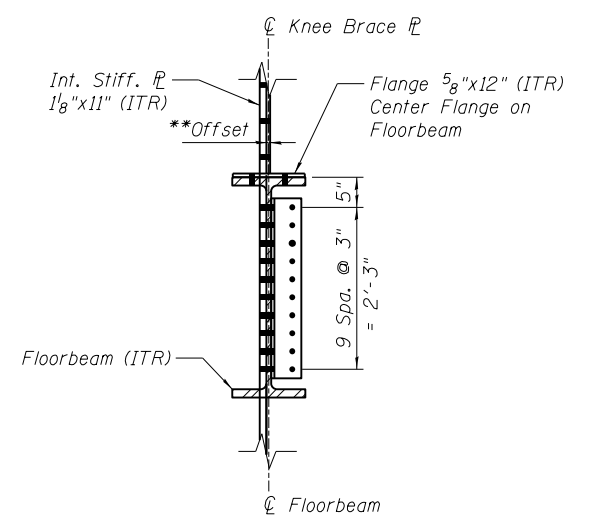
See Sheet 11 of 29 for Section C-C & D-D.

- Note:
1. FCM - Fracture Critical Member
2. ITR - Impact Test Required

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

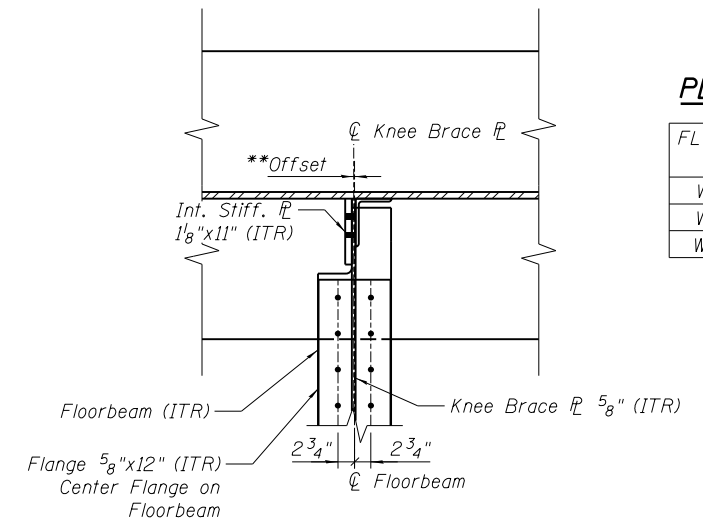


END FLOORBEAM CONNECTION



DETAIL 1

**See Table for Offset Dimension



SECTION E-E

**See Table for Offset Dimension

KNEE BRACE PLATE OFFSETS

FLOORBEAM SHAPE	OFFSET
W36x150	0"
W36x210	1/8"
W36x232	1/8"

\\spr-svr\306.hanson.dom\hanson_projects\Documents\09Jobs\09L0179B\CAD\Struct\6th\Sheet\0849962-09L0179B-UPRR-001

FILE NAME :	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
	PLOT SCALE = 0:2.0000 '1' / in.	DRAWN - RSJ	REVISED -
	PLOT DATE = 4/11/2019	CHECKED - MJW	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

INSIDE ELEVATION OF GIRDER - SHEET 1 OF 2
STRUCTURE 084-9962 - 6TH ST UPRR

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(109) VB,(110) VB-5	SANGAMON	382	242
			CONTRACT NO.	93733
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

SHEET NO. 9 OF 29 SHEETS

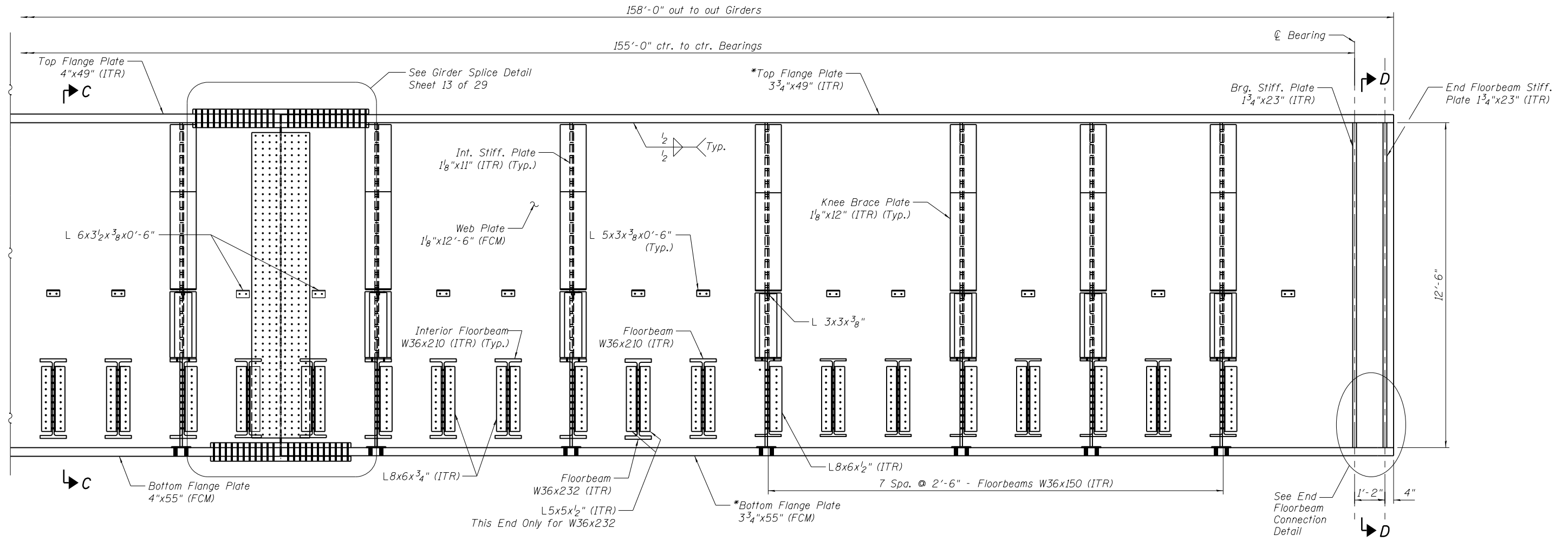
FINAL



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To ST. LOUIS, MO
(Timetable South)

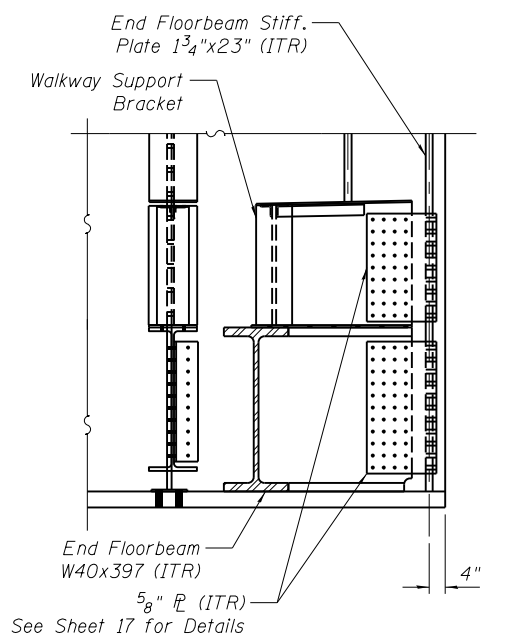
To JOLIET, IL
(Timetable North)



Note:
1. FCM - Fracture Critical Member
2. ITR - Impact Test Required

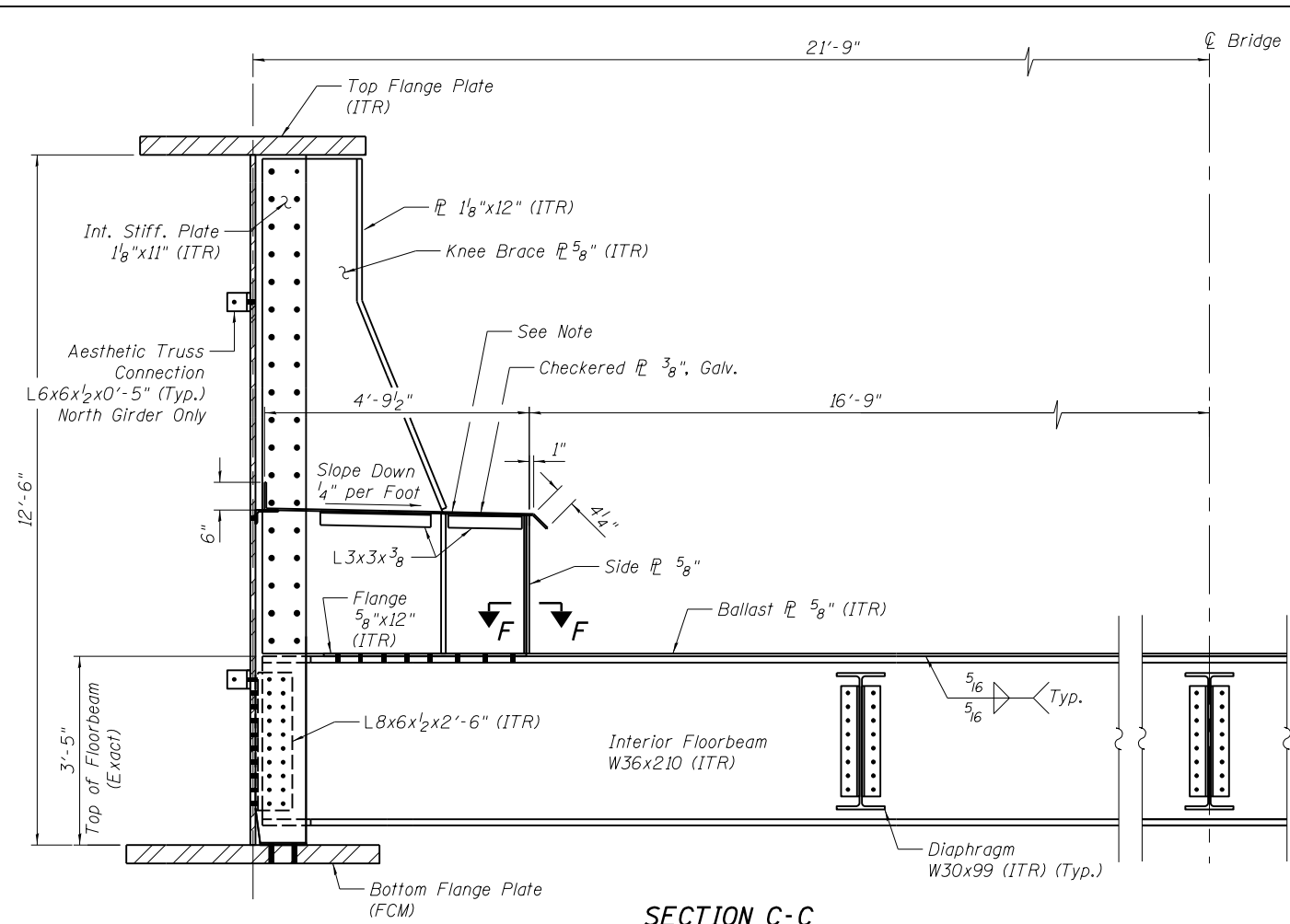
SECTION B-B - INSIDE ELEVATION OF GIRDER
See Sheet 11 of 29 for Section C-C & D-D.

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

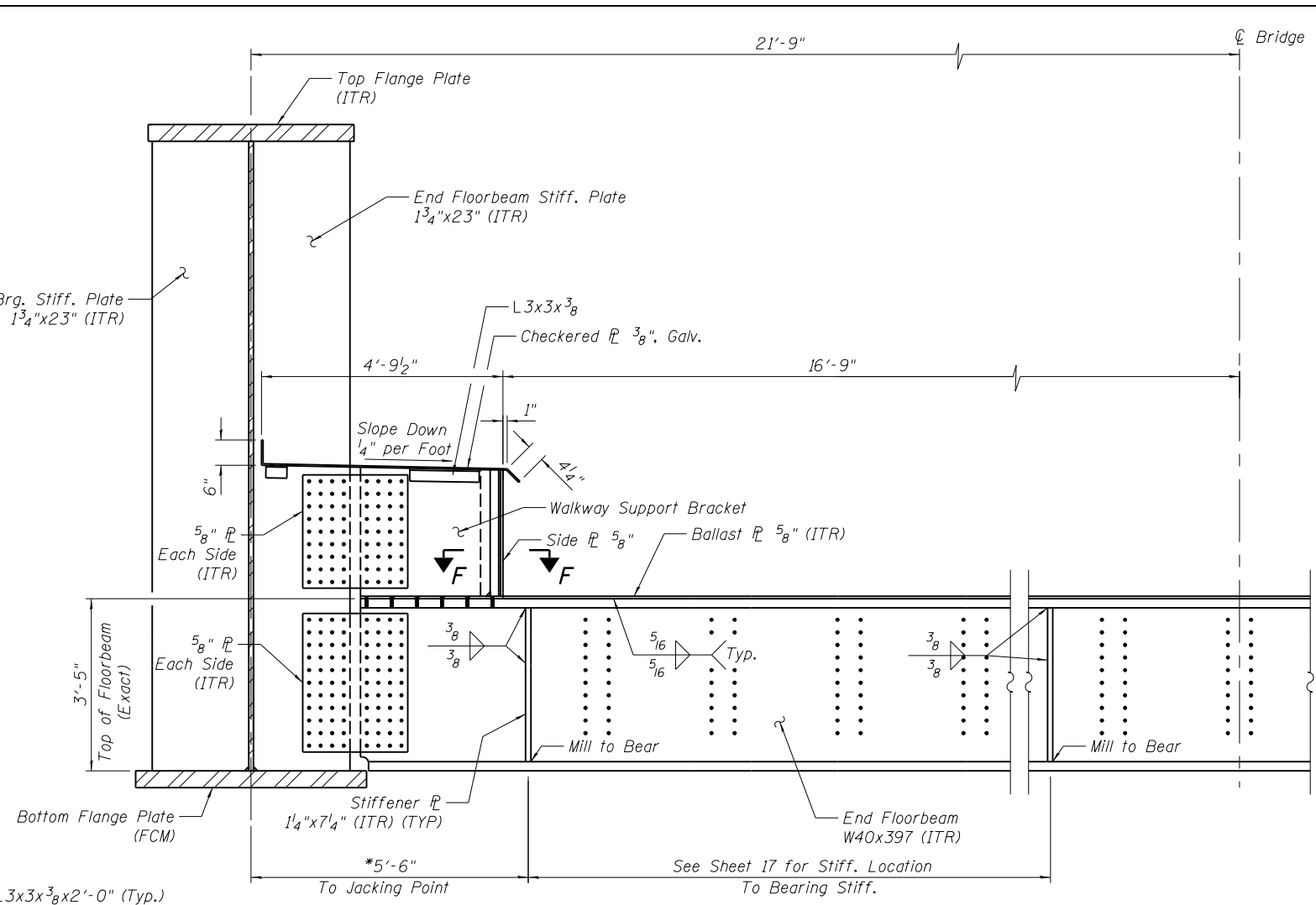


END FLOORBEAM CONNECTION DETAIL

<p>FINAL</p> <p>© Copyright Hanson Professional Services Inc., 2019</p>	<p>FILE NAME = D:\spr-svr\306.hanson.dom\hanson_projects\Documents\09Jobs\09L0179B\CAD\Struct\6th\Sheet\0849962-09L0179B-UPRR-001</p>	<p>USER NAME = Pop00275</p>	<p>DESIGNED - MJW</p>	<p>REVISÉ -</p>	<p>STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	<p>INSIDE ELEVATION OF GIRDER - SHEET 2 OF 2 STRUCTURE 084-9962 - 6TH ST UPRR</p>	<p>F.A.P. RTE. *</p>	<p>SECTION (109) VB,(110) VB-5</p>	<p>COUNTY SANGAMON</p>	<p>TOTAL SHEETS 382</p>	<p>SHEET NO. 243</p>
	<p>PLOT SCALE = 0:2.0000 '"/ in.</p>	<p>DRAWN - RSJ</p>	<p>REVISÉ -</p>	<p>CONTRACT NO. 93733</p>							
	<p>PLOT DATE = 4/11/2019</p>	<p>CHECKED - MJW</p>	<p>REVISÉ -</p>	<p>ILLINOIS FED. AID PROJECT</p>							
	<p>SHEET NO. 10 OF 29 SHEETS</p>						<p>666 & 666 ALT.</p>				

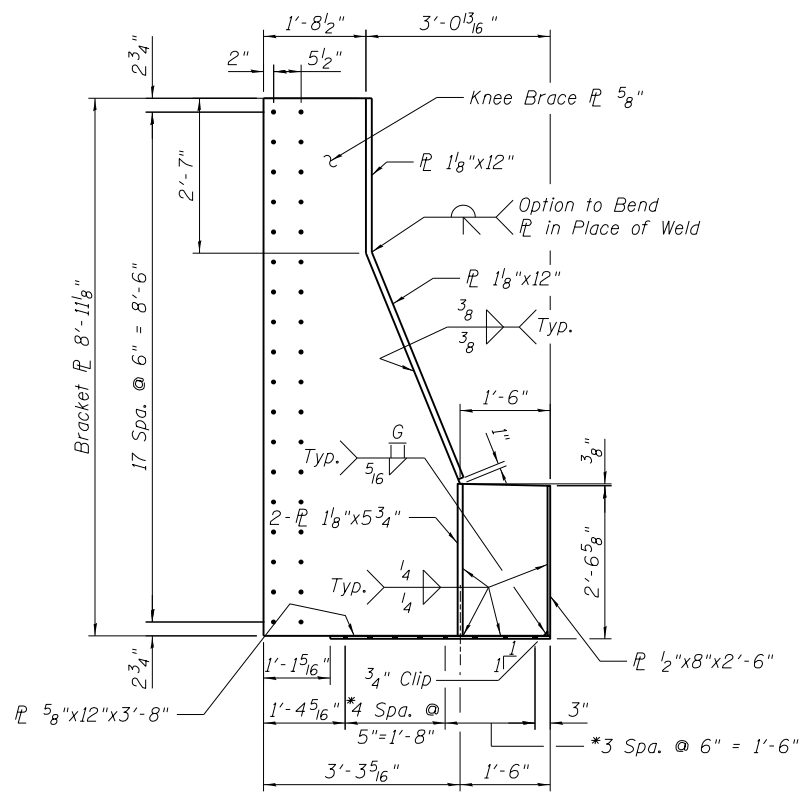


SECTION C-C



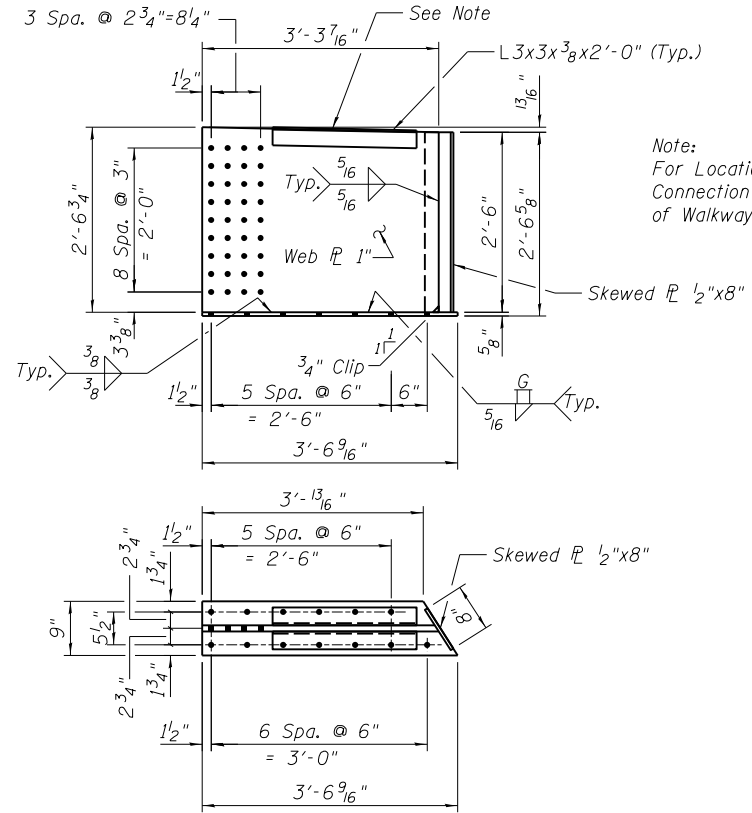
SECTION D-D
*(Along End Floorbeam \mathcal{C})

Note:
For Location of L3x3x3/8, Welding and Bolted Connection to Walkway Checker Plate see Plan View of Walkway at Knee Brace on Sheet 15.



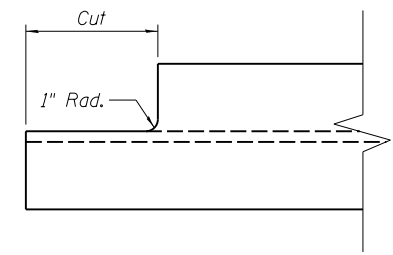
KNEE BRACE

*See Detail 1 on Sheet 9 for Hole Locations.

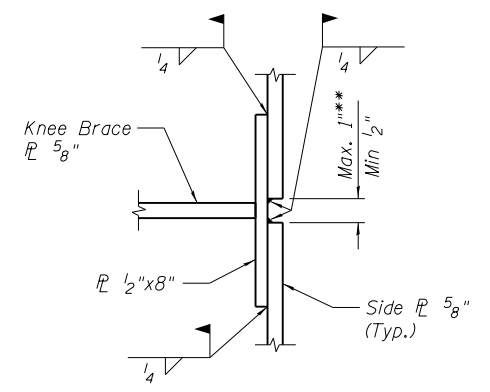


WALKWAY SUPPORT BRACKET

See Sheet 14 for Skew Angle



FLOORBEAM COPE AT INTERIOR STIFFENER



SECTION F-F

**No Gap at Walkway Support Bracket Provide Continuous Side \mathcal{C}

FILE NAME: ...

USER NAME = Pop0275	DESIGNED - MJW	REVISED -
PLOT SCALE = 0:2.0000' = 1" / in.	CHECKED - TJH/TDP	REVISED -
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TYPICAL SECTIONS
STRUCTURE 084-9962 - 6TH ST UPRR**

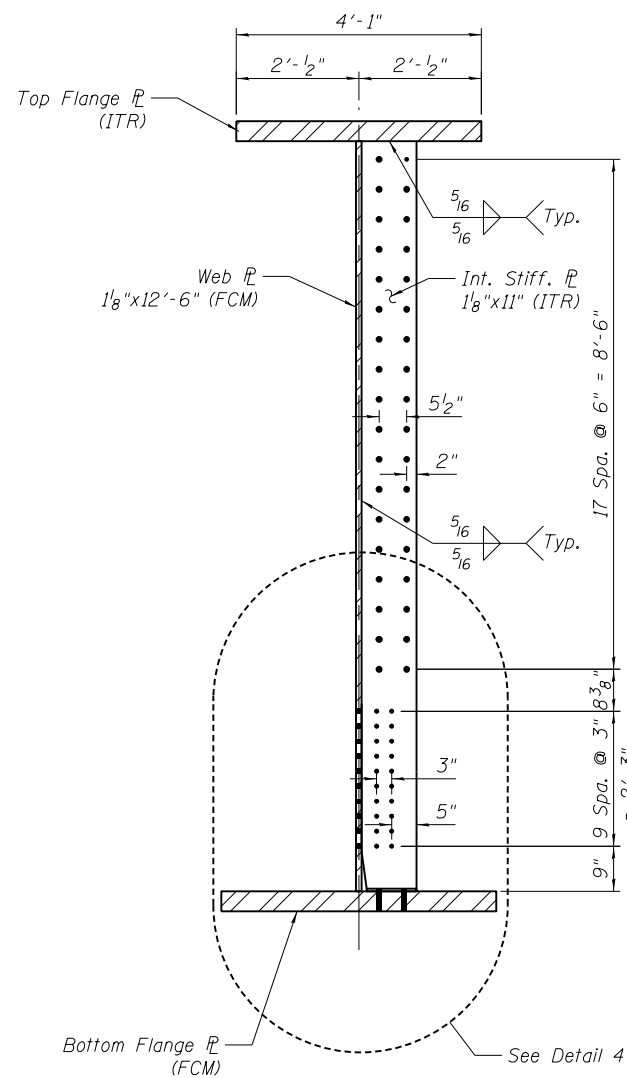
SHEET NO. 11 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO.	93733
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

FINAL

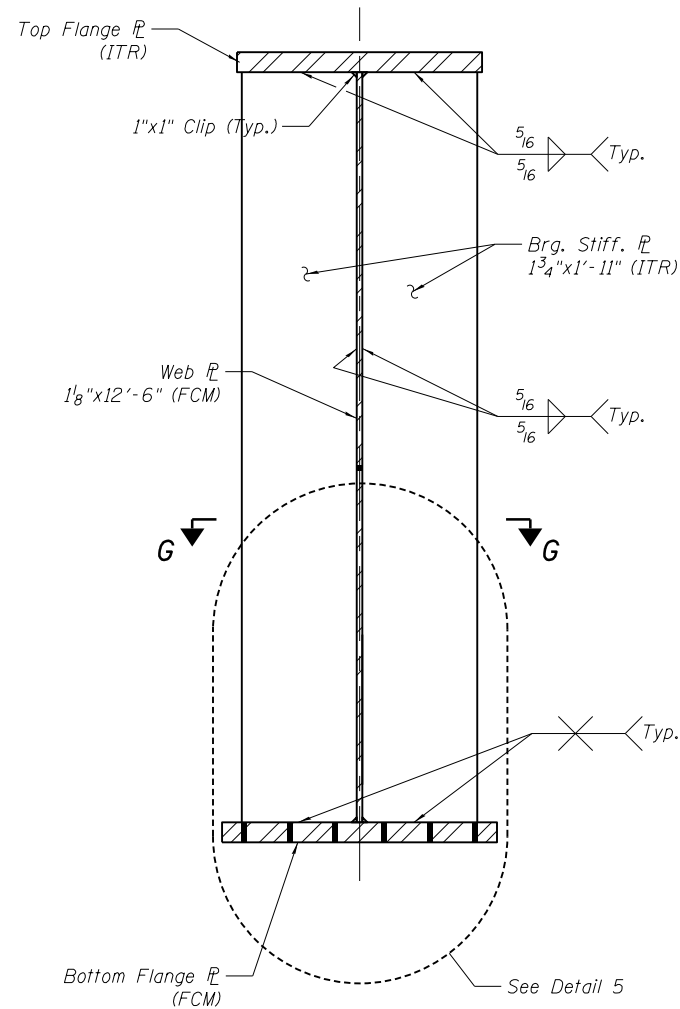


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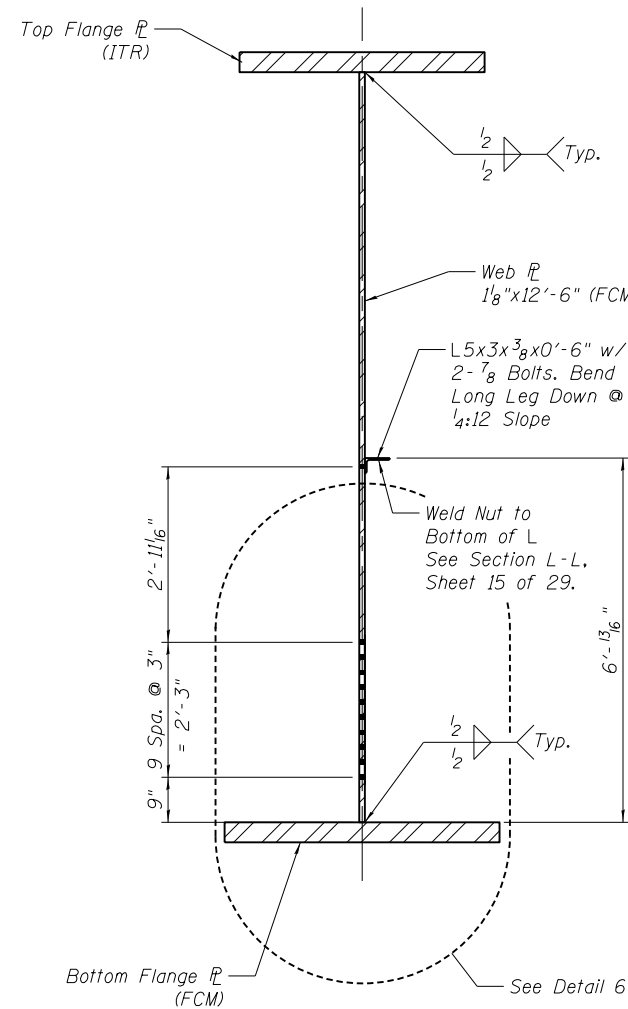


TYPICAL SECTION AT INT. STIFFENER AND KNEE BRACE

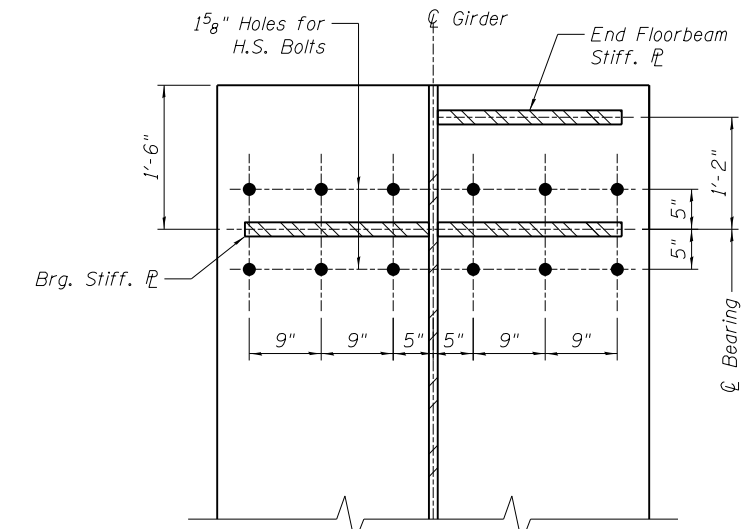
(Knee Brace Omitted for Clarity)



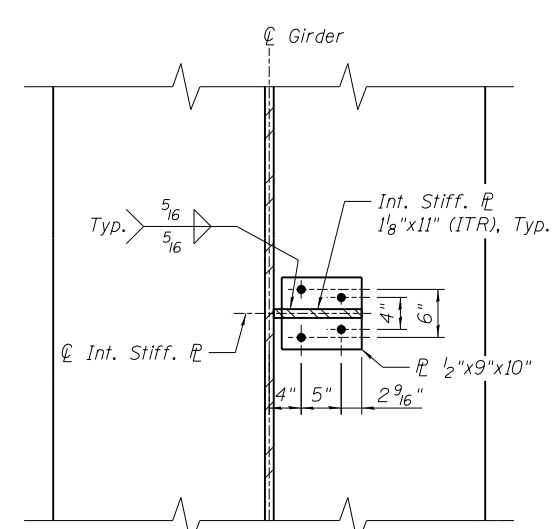
TYPICAL SECTION AT BEARING STIFFENER



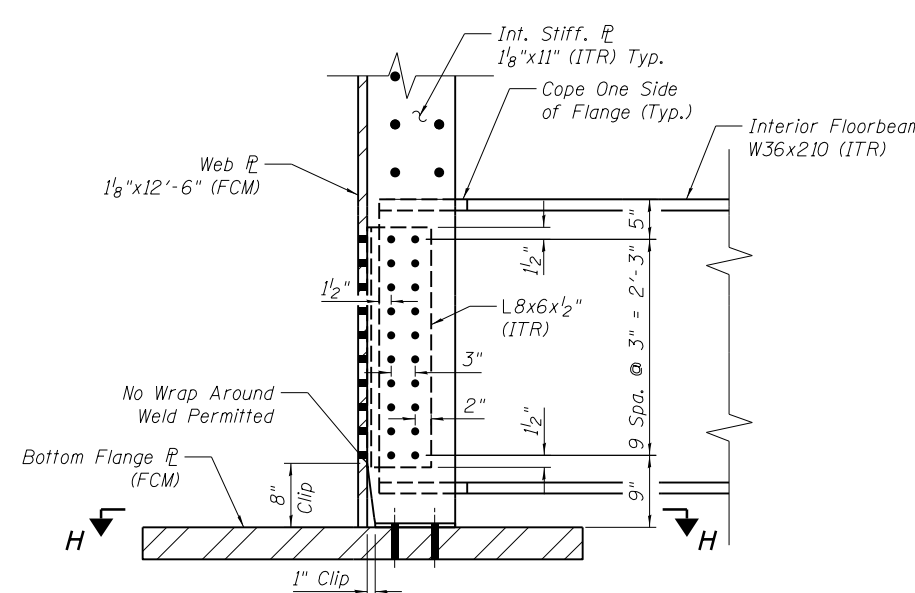
TYPICAL SECTION AT CHECKERED PLATE SUPPORT



SECTION G-G

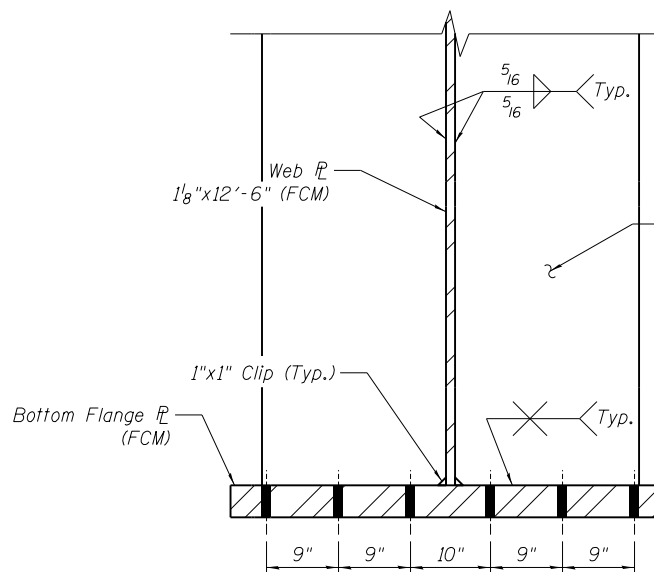


SECTION H-H



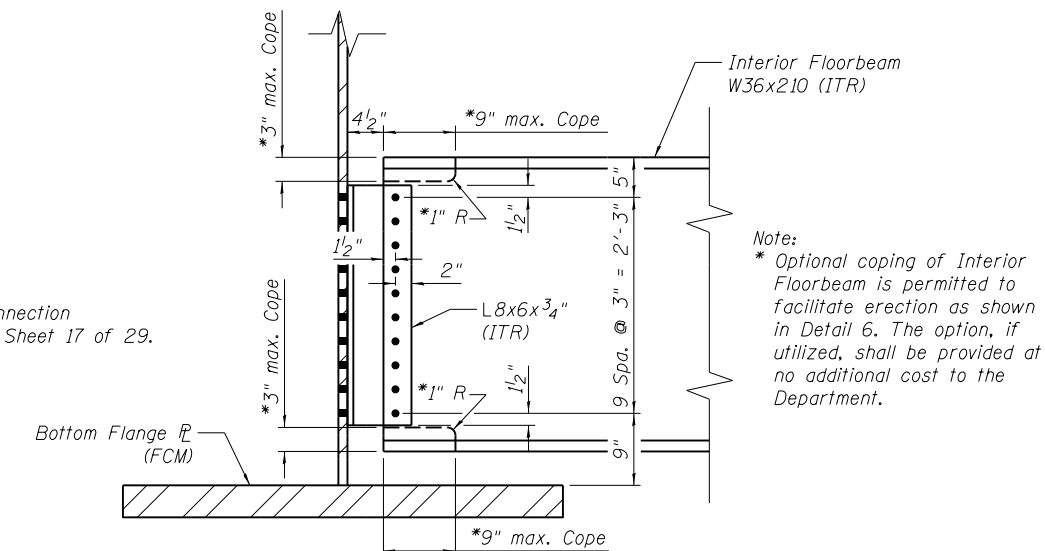
DETAIL 4

Typical for Interior Stiffener Unless Otherwise Noted (Knee Brace Omitted for Clarity)



DETAIL 5

Typical at Bearing Stiffener



DETAIL 6

Typical Floorbeam Connection Between Interior Stiffeners

Note:
* Optional coping of Interior Floorbeam is permitted to facilitate erection as shown in Detail 6. The option, if utilized, shall be provided at no additional cost to the Department.

FILE NAME = ... USER NAME = Pop00275

DESIGNED - MJW	REVISIONS -
CHECKED - TJH/TDP	REVISIONS -
DRAWN - RSJ	REVISIONS -
CHECKED - MJW	REVISIONS -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

GIRDER SECTIONS & DETAILS STRUCTURE 084-9962 - 6TH ST UPRR

SHEET NO. 12 OF 29 SHEETS

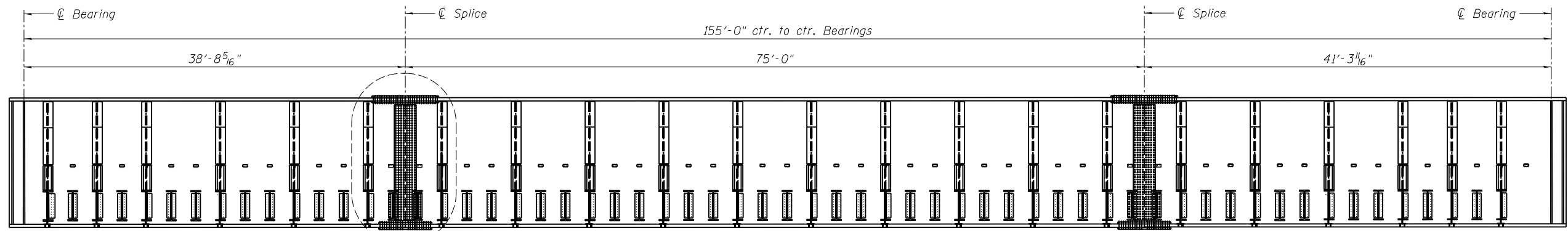
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO.			93733	

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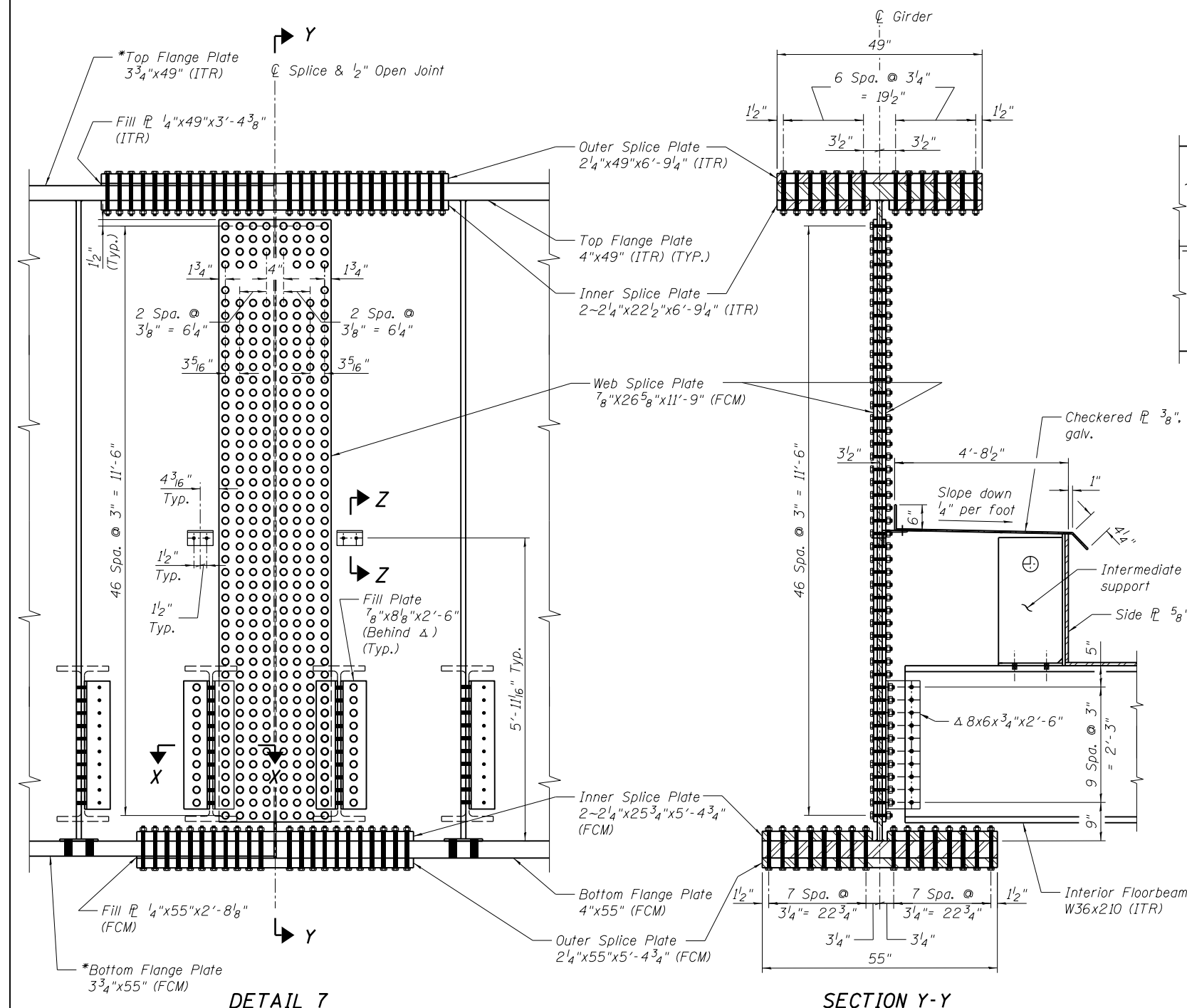
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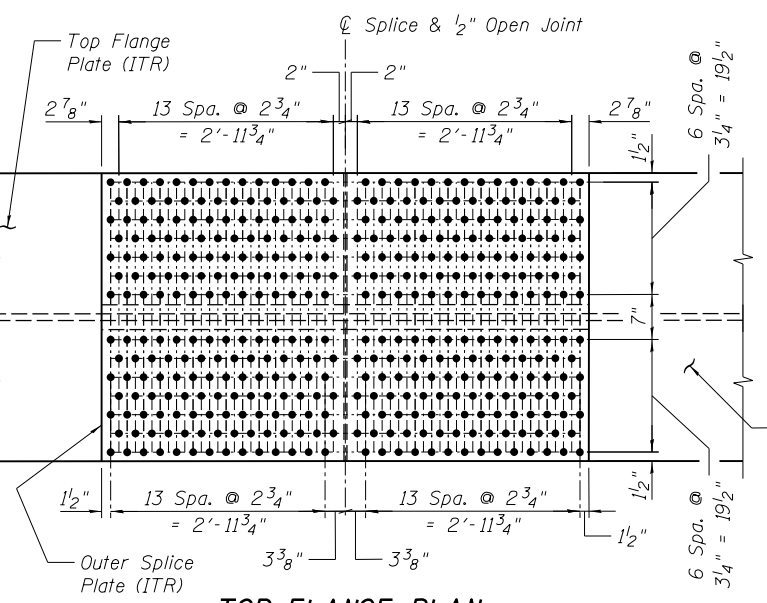


GIRDER ELEVATION (INSIDE FACE SHOWN)

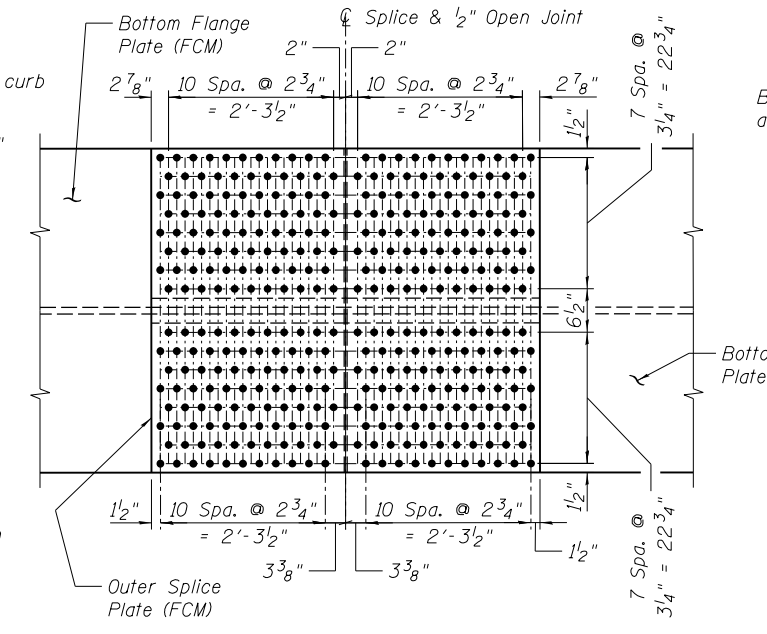


DETAIL 7

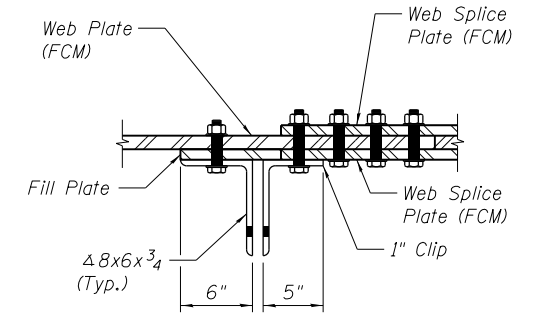
SECTION Y-Y



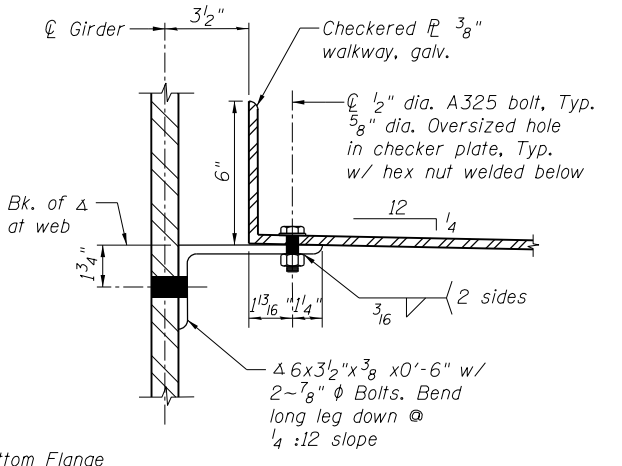
TOP FLANGE PLAN (Looking Down)



BOTTOM FLANGE PLAN (Looking Up)



SECTION X-X



SECTION Z-Z

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

PROJECT: 084-9962-09L0179B-UPRR-001

FILE NAME: 084-9962-09L0179B-UPRR-001	USER NAME: Pop00275	DESIGNED: MJW	REVISED: -
		CHECKED: TJH/TDP	REVISED: -
	PLOT SCALE: 0.25000" = 1'-0"	DRAWN: RSJ	REVISED: -
	PLOT DATE: 4/11/2019	CHECKED: MJW	REVISED: -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**GIRDER SPLICE DETAILS
STRUCTURE 084-9962 - 6TH ST UPRR**

SHEET NO. 13 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 93733	

•666 & 666 ALT. ILLINOIS FED. AID PROJECT

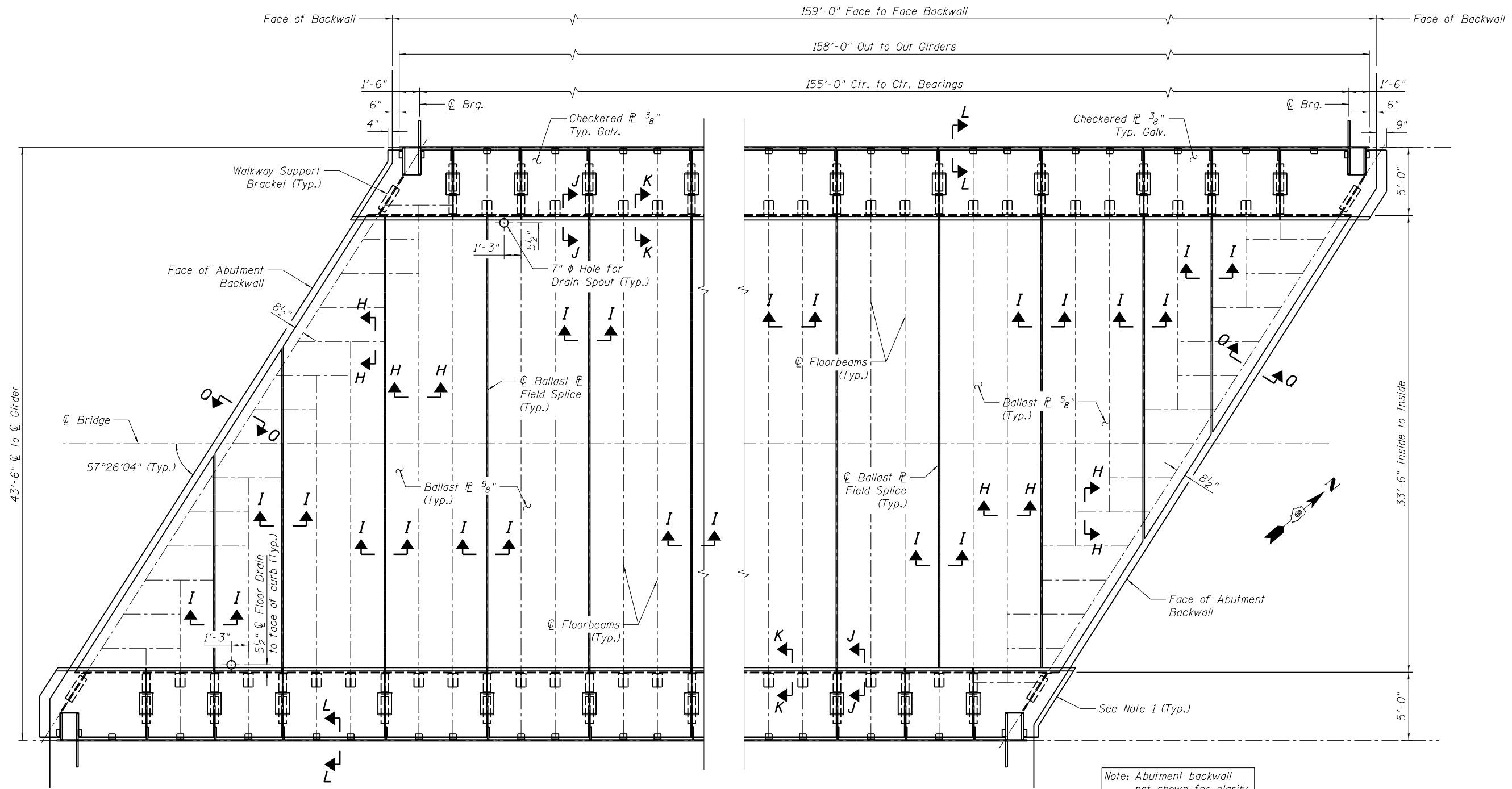
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To ST. LOUIS, MO
(Timetable South)

To JOLIET, IL
(Timetable North)



WALK & BALLAST PAN PLAN

See Sheet 15 of 29 for Section H-H, I-I, J-J, K-K, L-L, & O-O.

Note: Abutment backwall
not shown for clarity.

- Notes:
1. Prior to Setting End Checkered \bar{P} , Build-up top of Concrete Backwall with Epoxy Grout to Support Checkered \bar{P} and Provide Sloped Surface to Eliminate Tripping Hazard. Typical All Four Corners.
 2. Checkered \bar{P} Shall be ASTM A786 Gr 36 or ASTM A36. Galvanize after fabrication.

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FILE NAME =
 HANSON
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USER NAME = Pop00275	DESIGNED - MJW	REVISIONS
PLOT SCALE = 0:2.0000 '1" / in.	CHECKED - TJH/TDP	REVISIONS
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISIONS
	CHECKED - MJW	REVISIONS

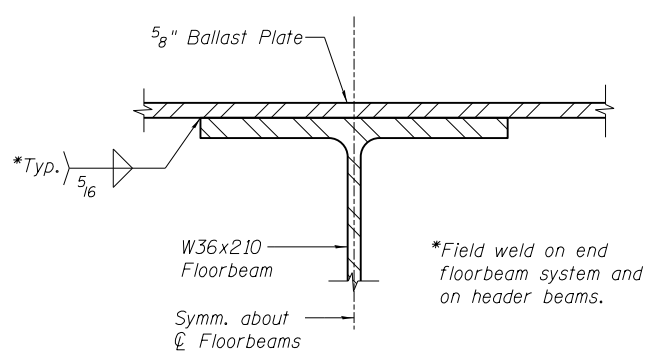
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**WALKWAY AND BALLAST PLATE PLAN
STRUCTURE 084-9962 - 6TH ST UPRR**

SHEET NO. 14 OF 29 SHEETS

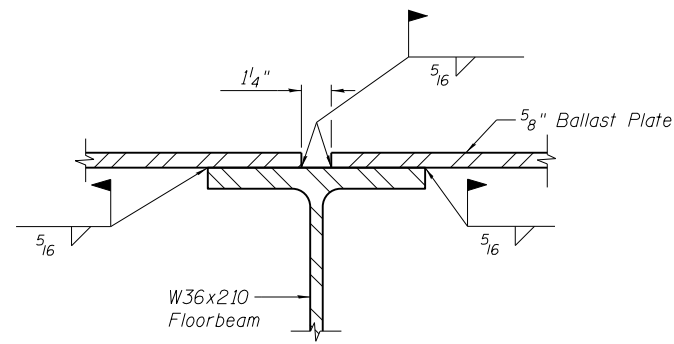
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	247
			CONTRACT NO.	93733
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

FINAL

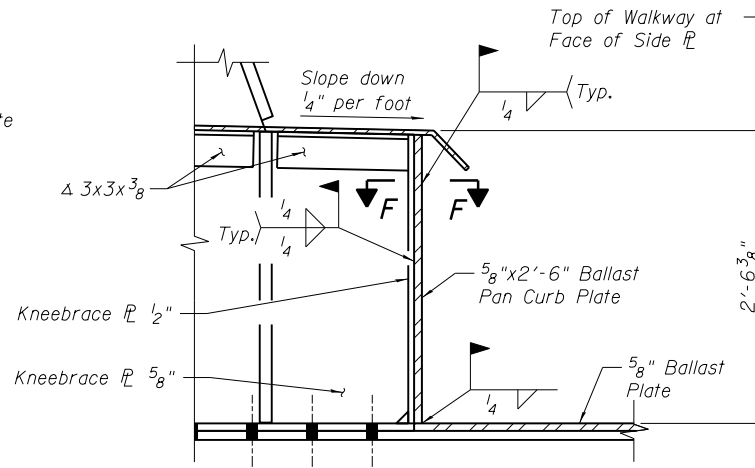


SECTION H-H BALLAST PLATE TO FLOORBEAM CONNECTION (TYP.)

Similar Detail at Header Beam

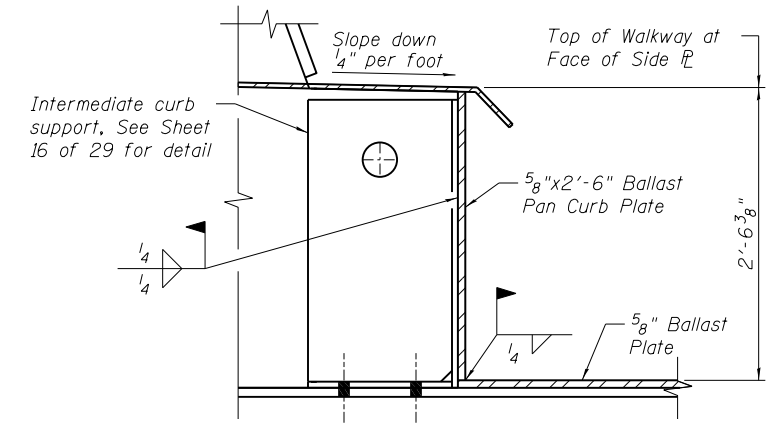


SECTION I-I

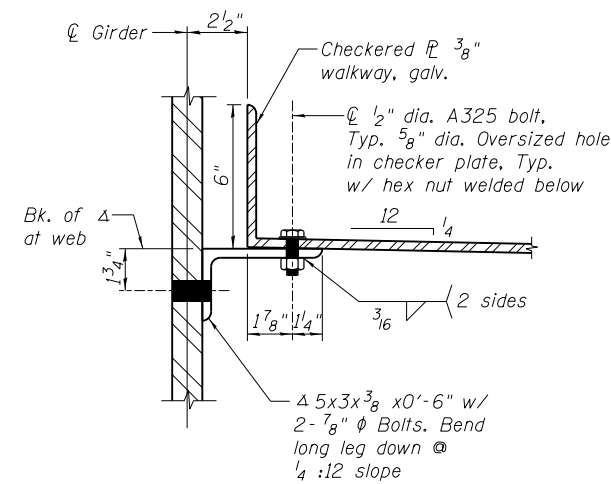


SECTION J-J

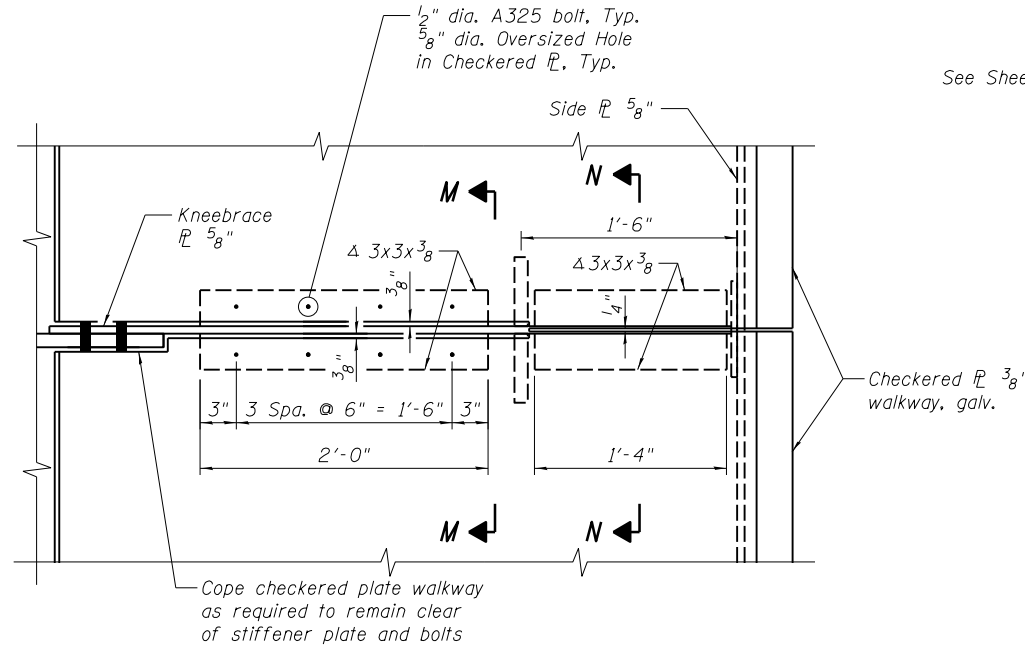
See Sheet 11 of 29 for Section F-F.



SECTION K-K

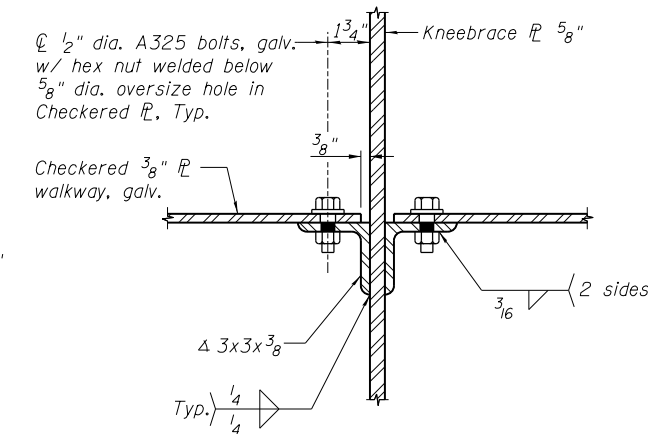


SECTION L-L

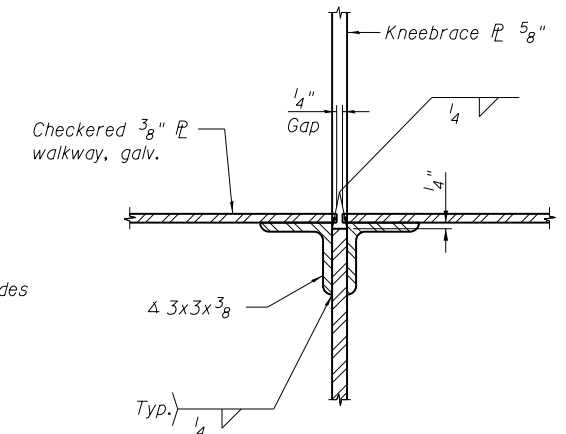


PLAN

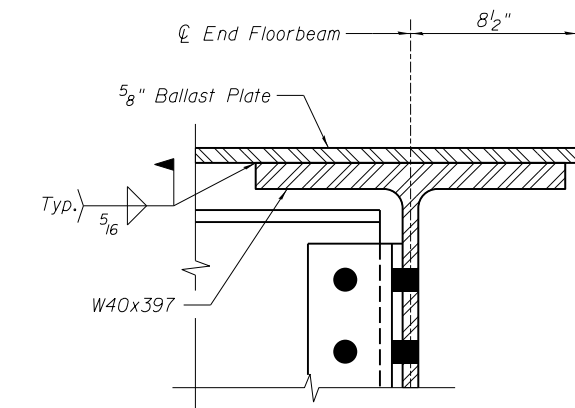
Walkway at Kneebrace



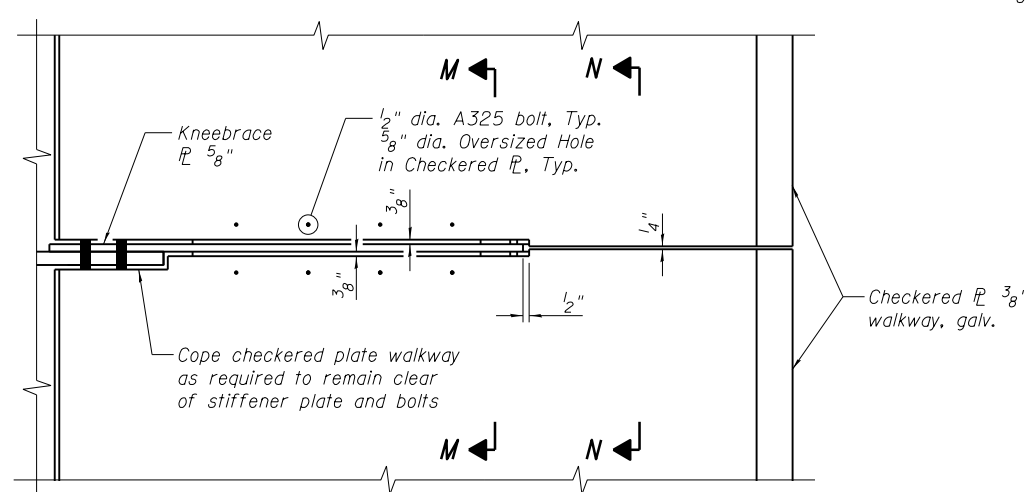
SECTION M-M



SECTION N-N

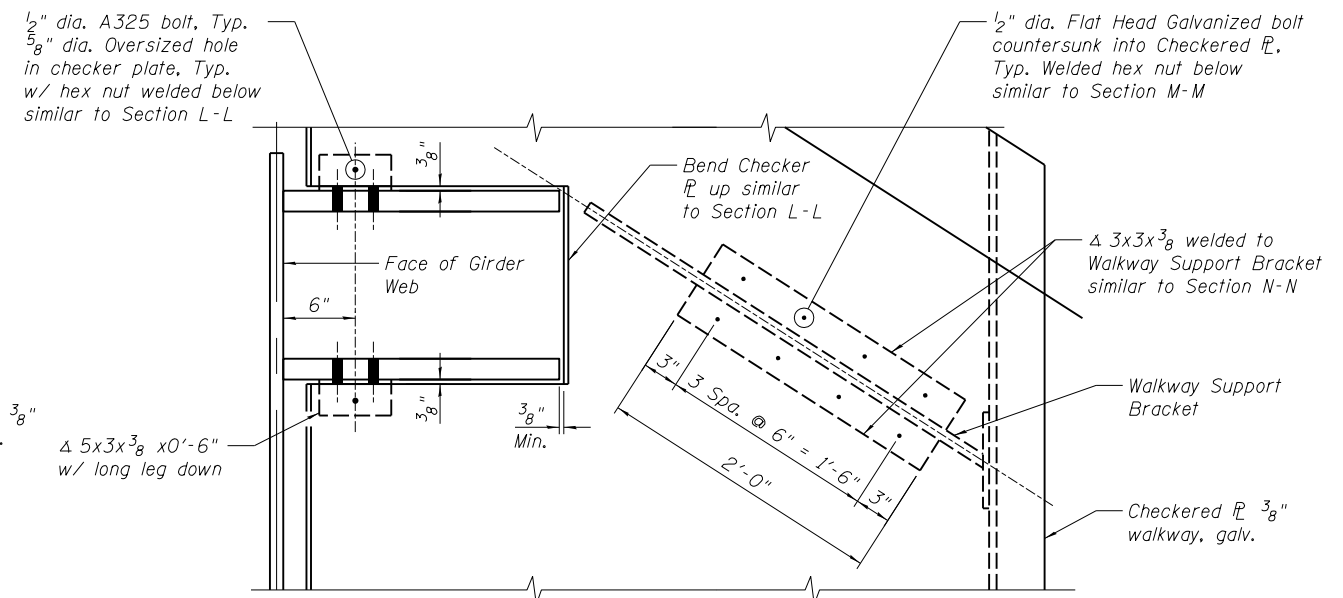


SECTION O-O



PLAN

Walkway at Kneebrace



PLAN

Walkway at Bearing Stiffener

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FILE NAME =
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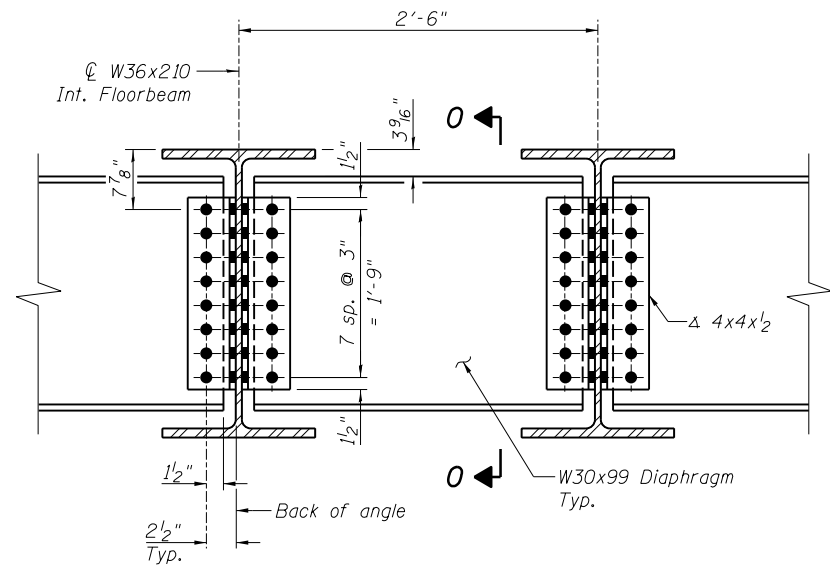
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PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISD -
	CHECKED - MJW	REVISD -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

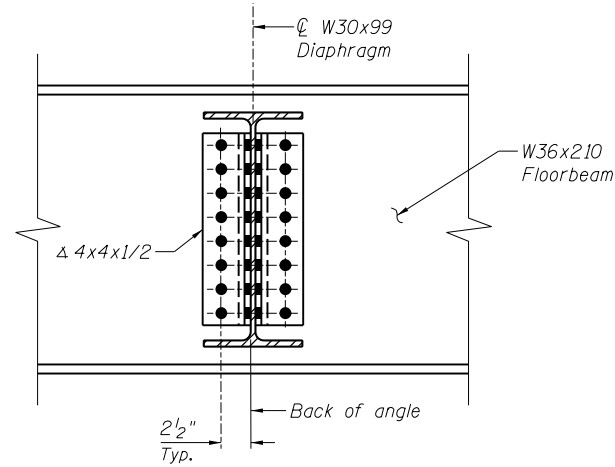
**WALKWAY AND BALLAST PLATE DETAILS
STRUCTURE 084-9962 - 6TH ST UPRR**

SHEET NO. 15 OF 29 SHEETS

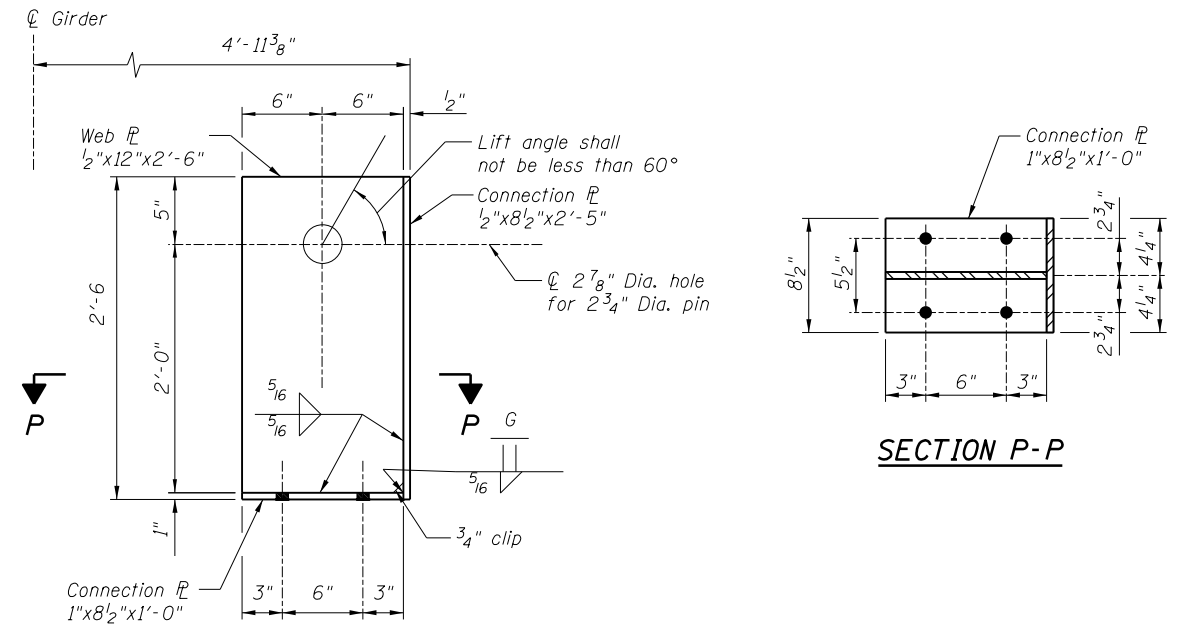
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	248
			CONTRACT NO. 93733	
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				



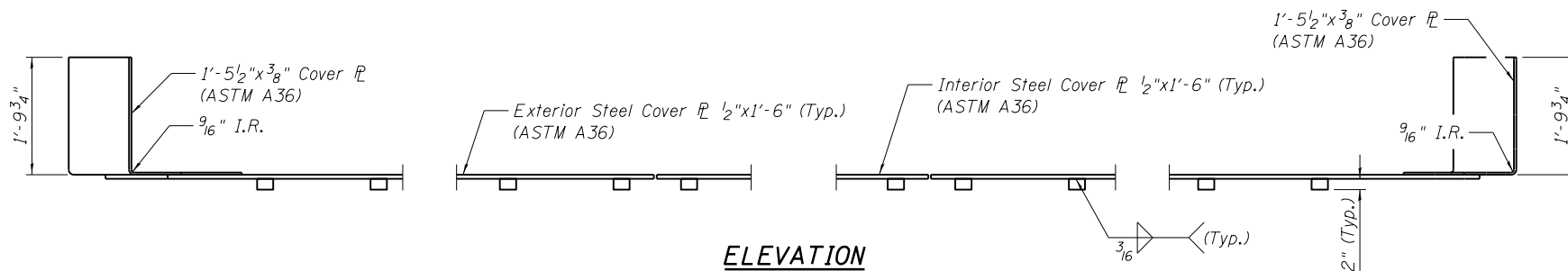
LONGITUDINAL DIAPHRAGM DETAIL



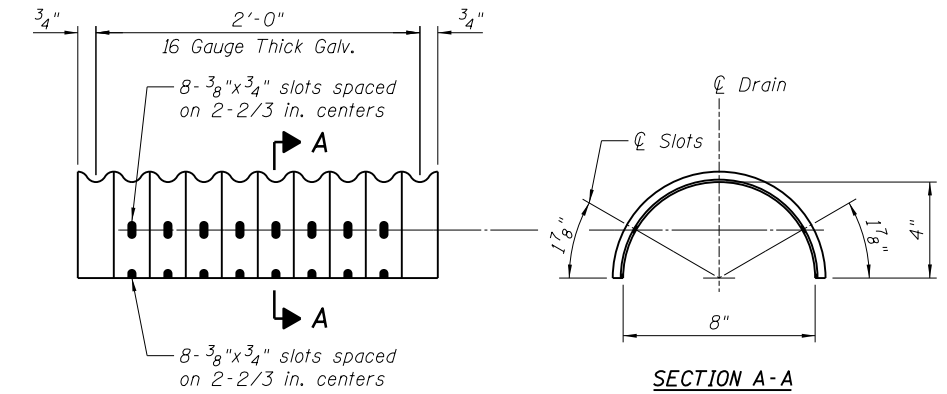
SECTION O-O



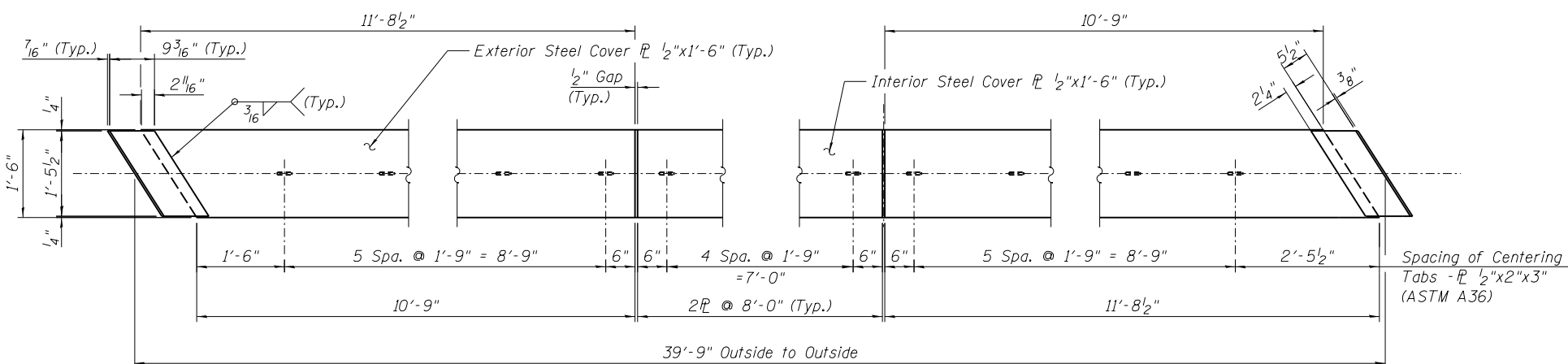
INTERMEDIATE CURB SUPPORT



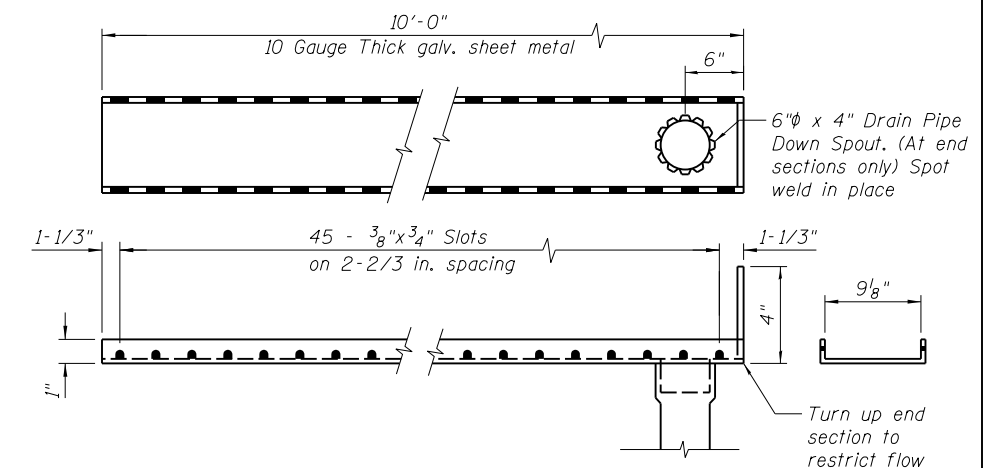
ELEVATION



DETAIL - DECK DRAIN PIPE



**PLAN
COVER PLATES
(Galvanize after Fabrication)**



DETAIL - DECK DRAIN BOTTOM PAN

- Notes:
- Lap Drain Pipe one corrugation at each end.
 - Coordinate outside diameter of drain pipe down spout with 6" ϕ Ductile Iron Pipe.
 - Cost for deck drain pipe and bottom pan shall be included in the cost of "Drainage System".

FILE NAME = ...

USER NAME = Pop00275	DESIGNED - MJW	REVISED -
PLOT SCALE = 0:2.0000' = 1" / in.	CHECKED - TJH/TDP	REVISED -
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**MISCELLANEOUS GIRDER DETAILS - SHEET 1 OF 3
STRUCTURE 084-9962 - 6TH ST UPRR**

SHEET NO. 16 OF 29 SHEETS

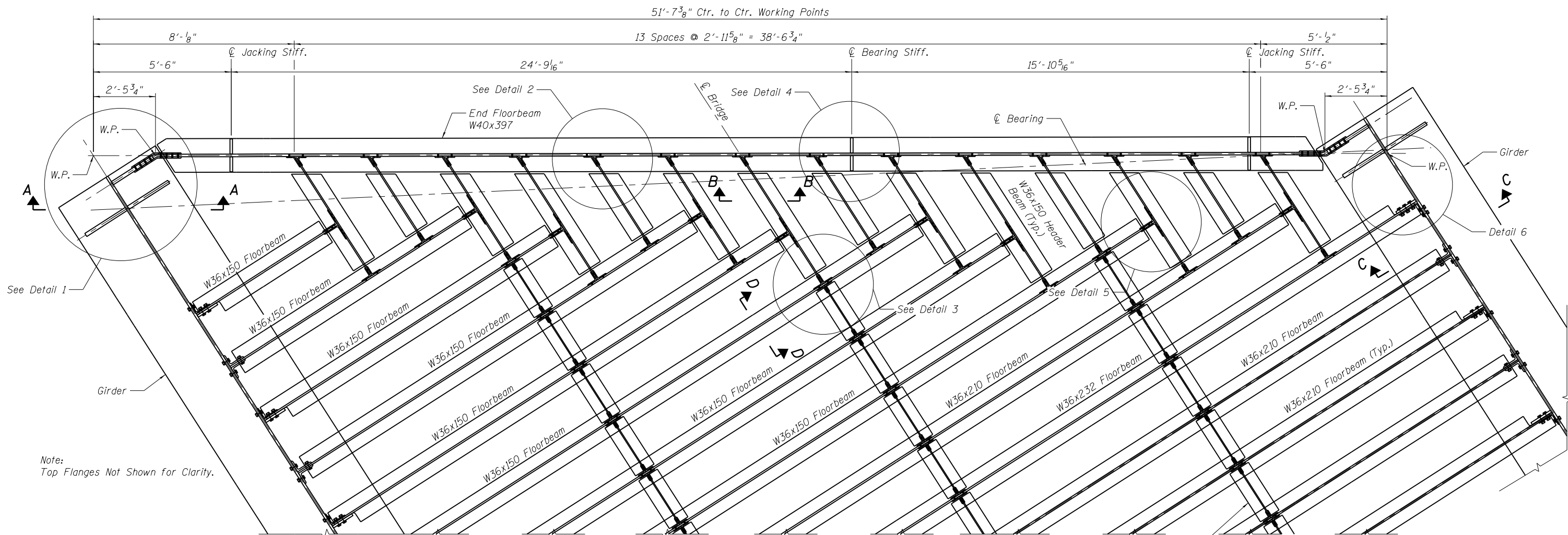
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO.			93733	

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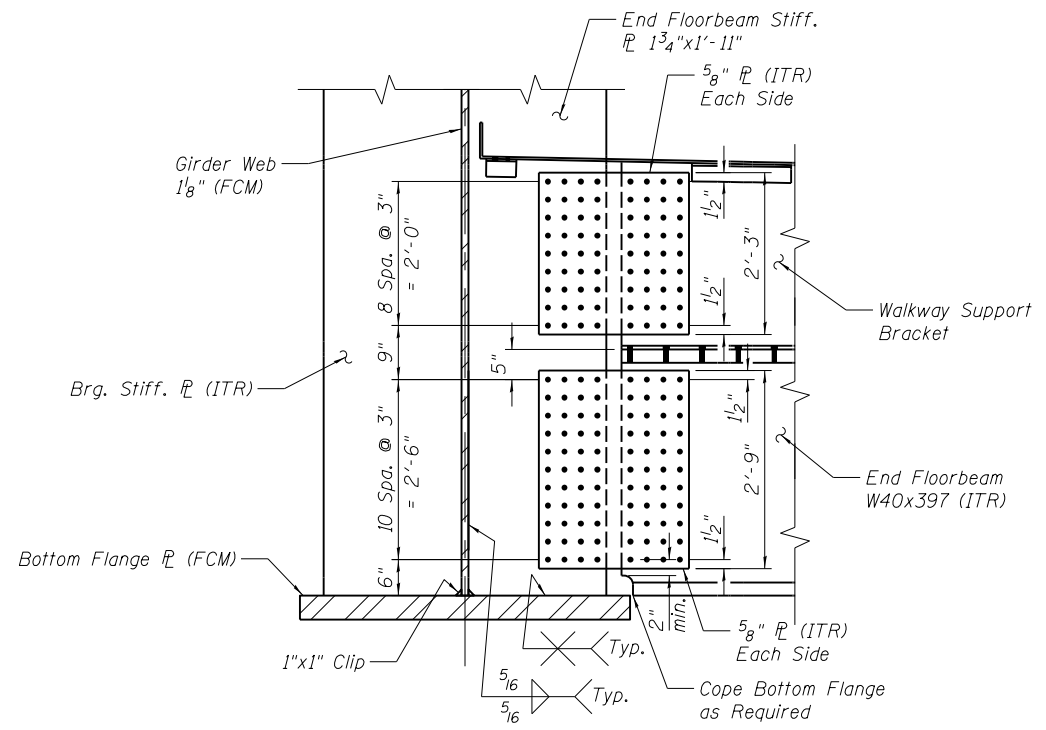
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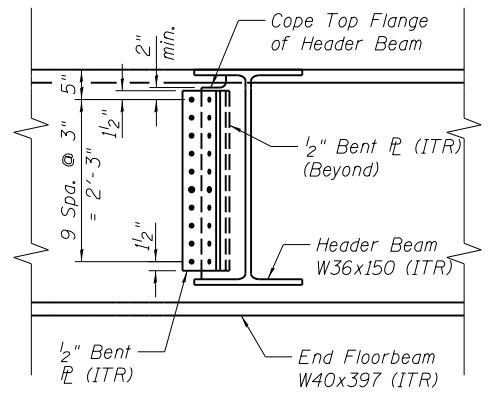
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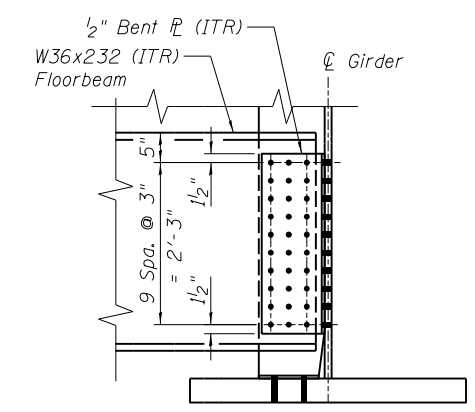
TYPICAL END FLOORBEAM PLAN
See Sheet 18 for Details



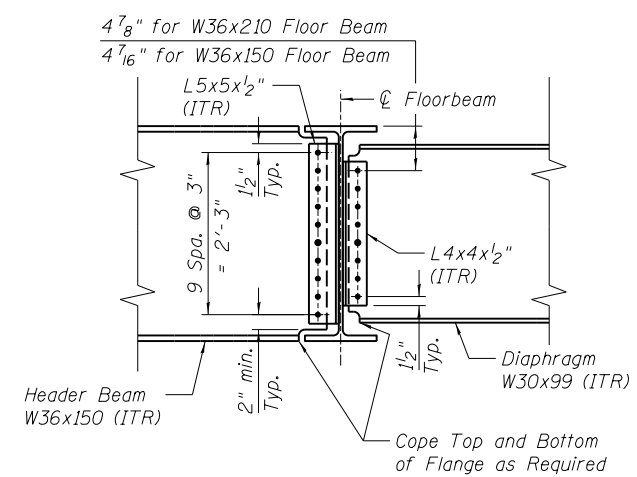
SECTION A-A
See Detail 1 on Sheet 18 for Horizontal Bolt Spacing.



SECTION B-B
See Detail 2 on Sheet 18 for Horizontal Bolt Spacing.

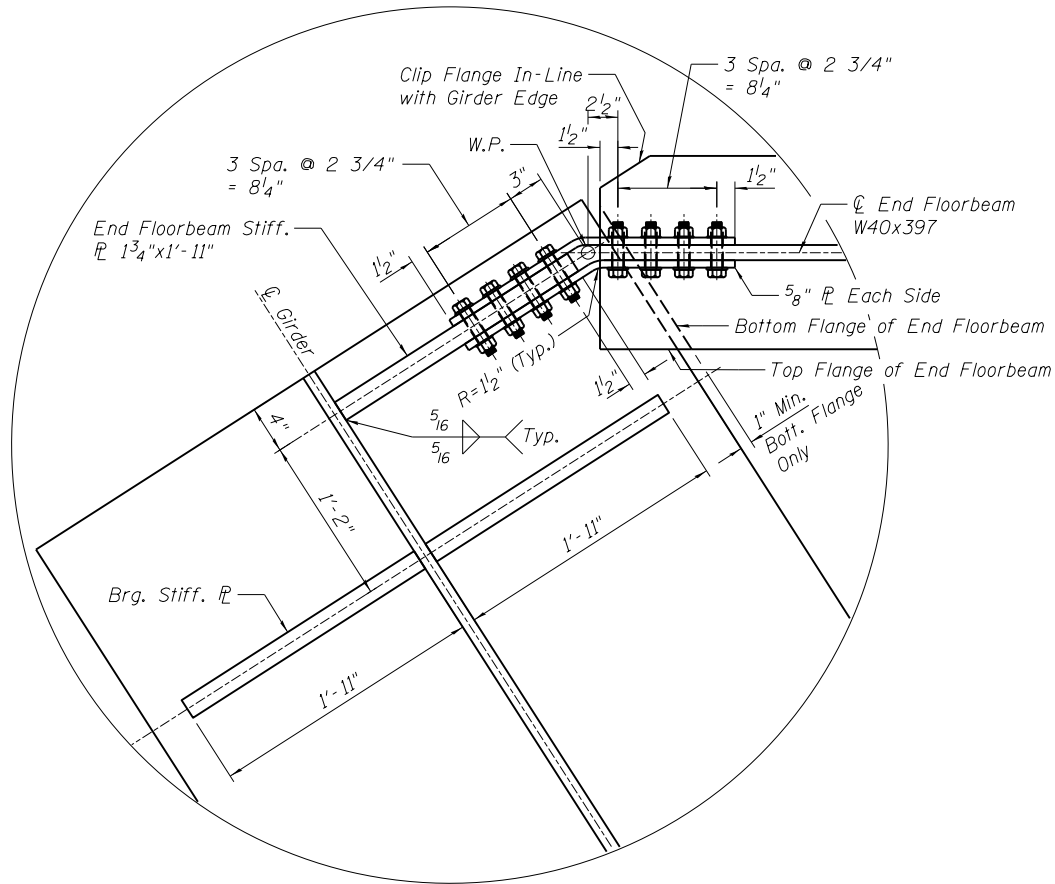


SECTION C-C
See Detail 6 on Sheet 18 for Horizontal Bolt Spacing.

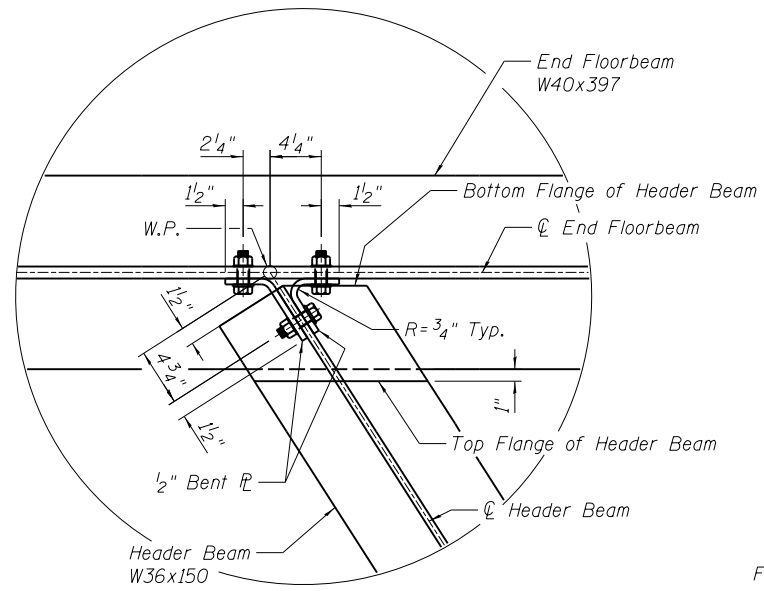


SECTION D-D
See Detail 3 on Sheet 18 for Horizontal Bolt Spacing.

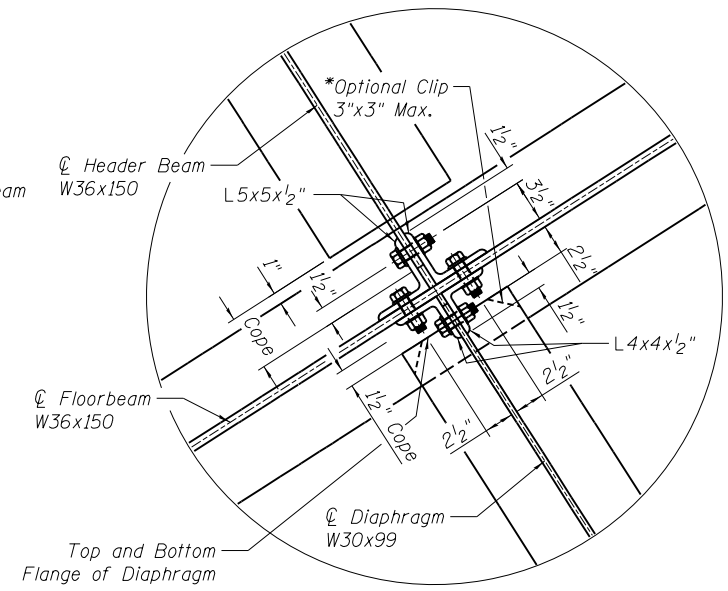
<p>FINAL</p> <p>© Copyright Hanson Professional Services Inc., 2019</p>	<p>FILE NAME =</p> <p>USER NAME = Pop00275</p> <p>PLOT SCALE = 0:2.0000 ' = 1/8" / in.</p> <p>PLOT DATE = 4/11/2019</p>	<p>DESIGNED - MJW</p> <p>CHECKED - TJH/TDP</p> <p>DRAWN - RSJ</p> <p>CHECKED - MJW</p>	<p>REVISED -</p> <p>REVISED -</p> <p>REVISED -</p> <p>REVISED -</p>	<p>STATE OF ILLINOIS</p> <p>DEPARTMENT OF TRANSPORTATION</p>	<p>MISCELLANEOUS GIRDER DETAILS - SHEET 2 OF 3</p> <p>STRUCTURE 084-9962 - 6TH ST UPRR</p> <p>SHEET NO. 17 OF 29 SHEETS</p>	<p>F.A.P. RTE.</p> <p>SECTION (109) VB,(110) VB-5</p> <p>COUNTY SANGAMON</p> <p>TOTAL SHEETS 382</p> <p>SHEET NO. 250</p> <p>CONTRACT NO. 93733</p> <p>•666 & 666 ALT. ILLINOIS FED. AID PROJECT</p>
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DETAIL 1

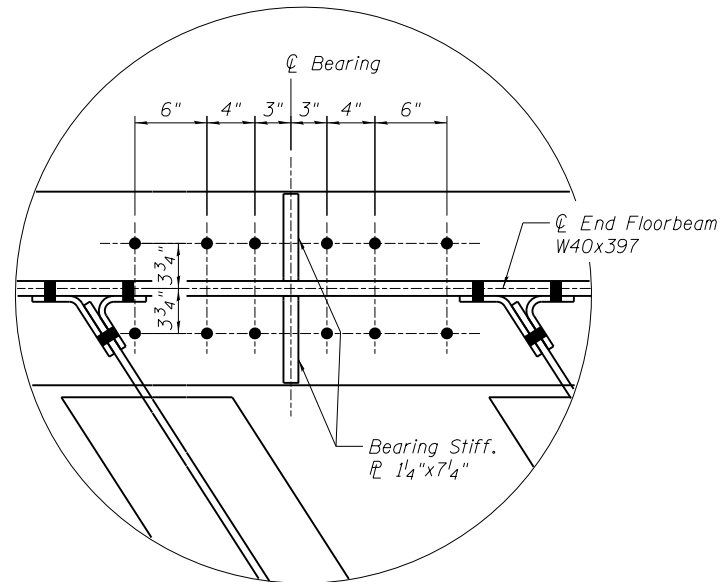


DETAIL 2

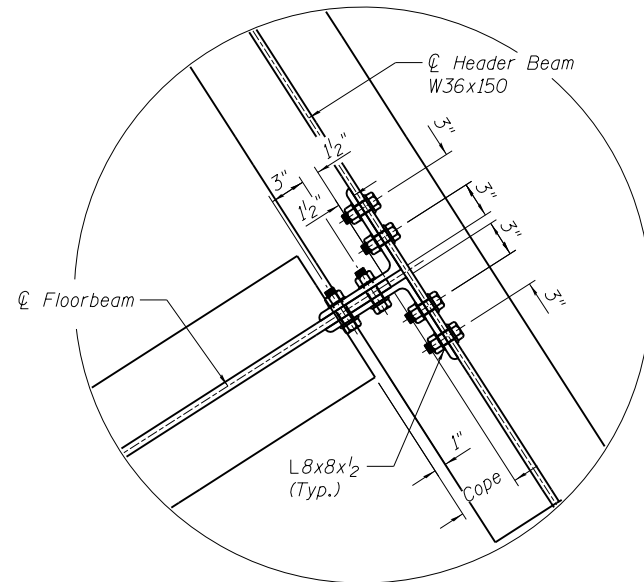


DETAIL 3

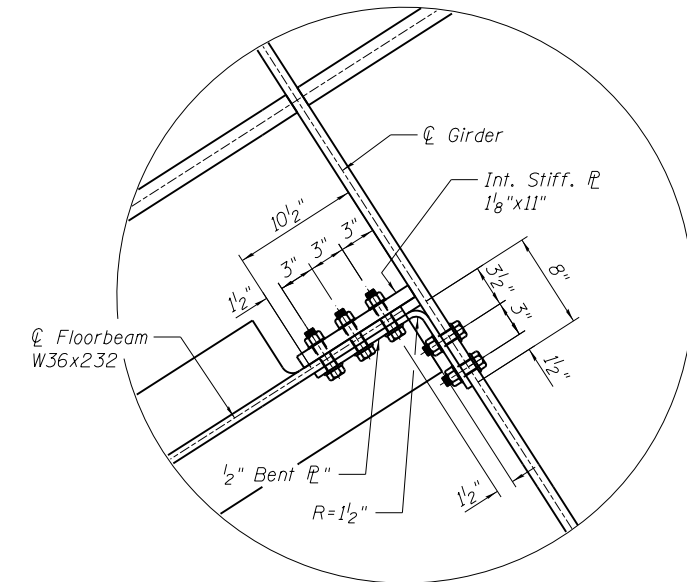
*Clipping diaphragm flanges is permitted to facilitate erection at intermediate and end floor system locations. If Clipped it shall be provided at no additional cost to the Department.



DETAIL 4



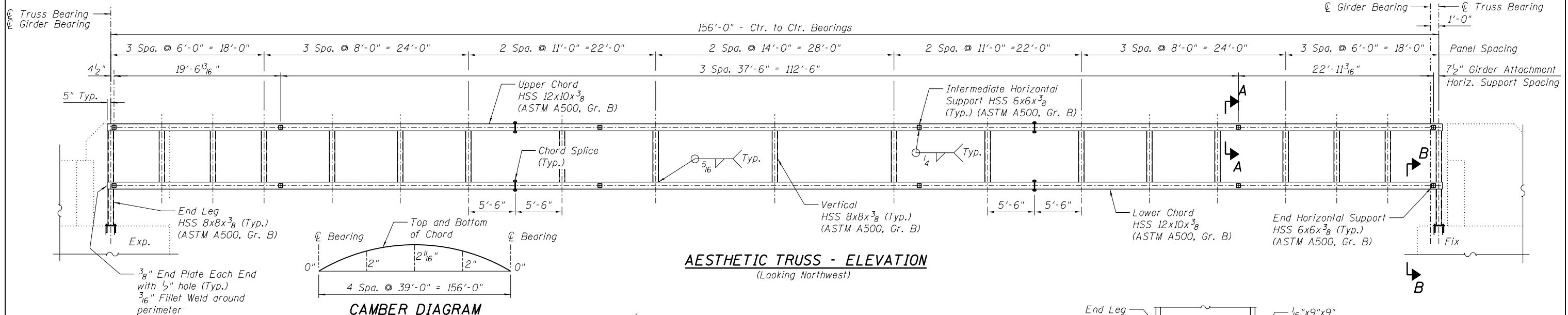
DETAIL 5



DETAIL 6

To ST. LOUIS, MO
(Timetable South)

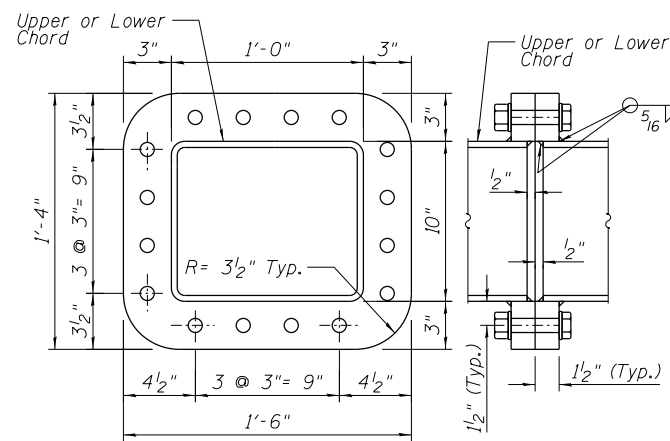
To JOLIET, IL
(Timetable North)



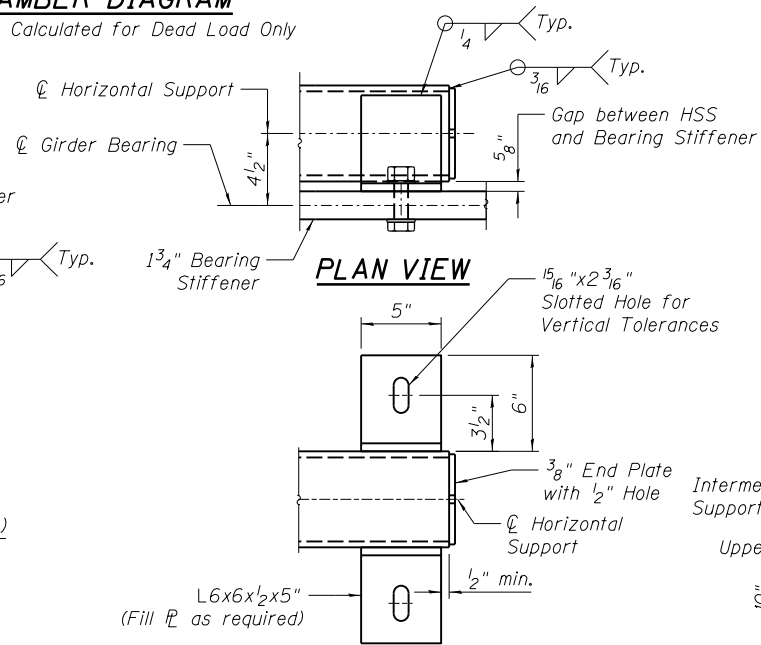
AESTHETIC TRUSS - ELEVATION
(Looking Northwest)

CAMBER DIAGRAM

Camber Calculated for Dead Load Only



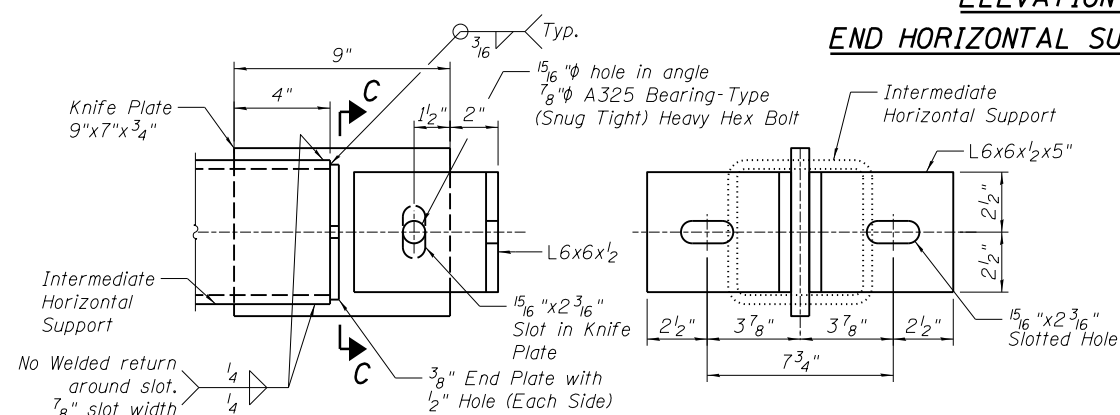
CHORD SPLICE DETAIL



PLAN VIEW

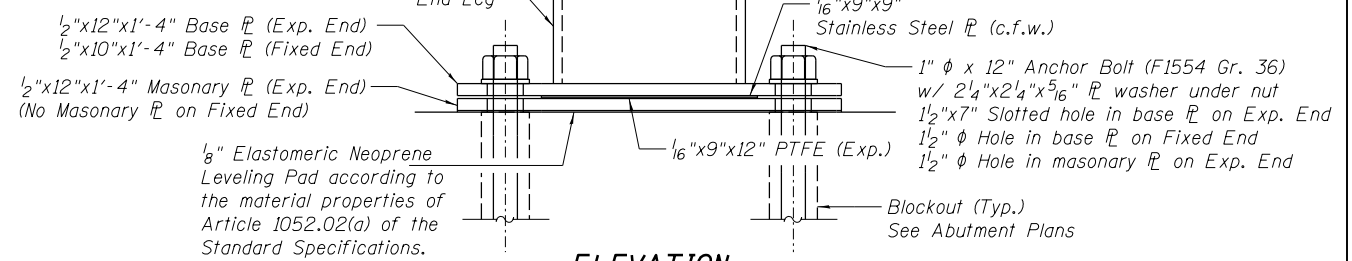
ELEVATION VIEW

END HORIZONTAL SUPPORT DETAIL

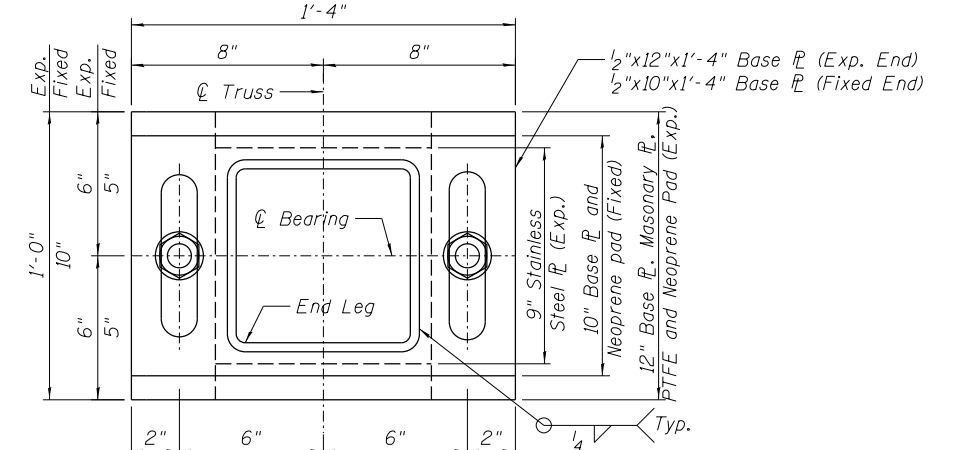


INTERMEDIATE HORIZONTAL SUPPORT DETAIL

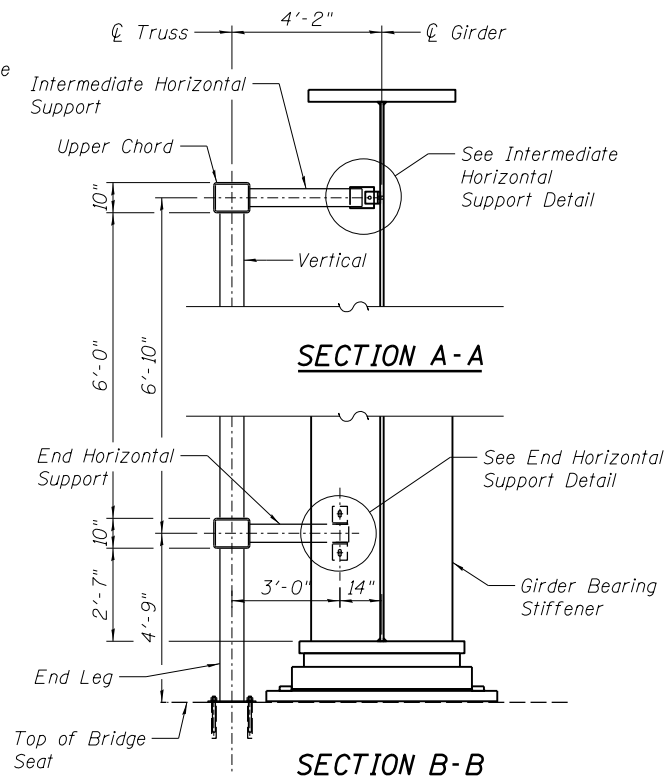
SECTION C-C



ELEVATION



PLAN
END LEG BEARING DETAIL
(Expansion Bearing Shown)



SECTION A-A

SECTION B-B

Note:
Location of Fixed and Expansion bearings shall match the girder.
Cost for elastomeric neoprene leveling pad, PTFE surface, shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 3."
Anchor Bolts shall be ASTM F1554 all-thread (or an Engineer approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Anchor Bolts shall be installed in blockouts with non-shrink grout meeting the material requirements of Article 1024.02 of the Standard Specifications. Blockouts shall be clean prior to grouting and grout installed according to manufacturer's recommendations.
The PTFE shall be bonded directly to the masonry plate according to the manufacturer's recommendations.

PROJECT: 0849962-09L0179B-UPRR-001

FILE NAME: 0849962-09L0179B-UPRR-001	USER NAME: Pop00275	DESIGNED: MJW	REVISED: -
		CHECKED: TJH/TDP	REVISED: -
		DRAWN: RSJ	REVISED: -
		CHECKED: MJW	REVISED: -
			REVISED: -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

AESTHETIC TRUSS
STRUCTURE 084-9962 - 6TH ST UPRR

SHEET NO. 19 OF 29 SHEETS

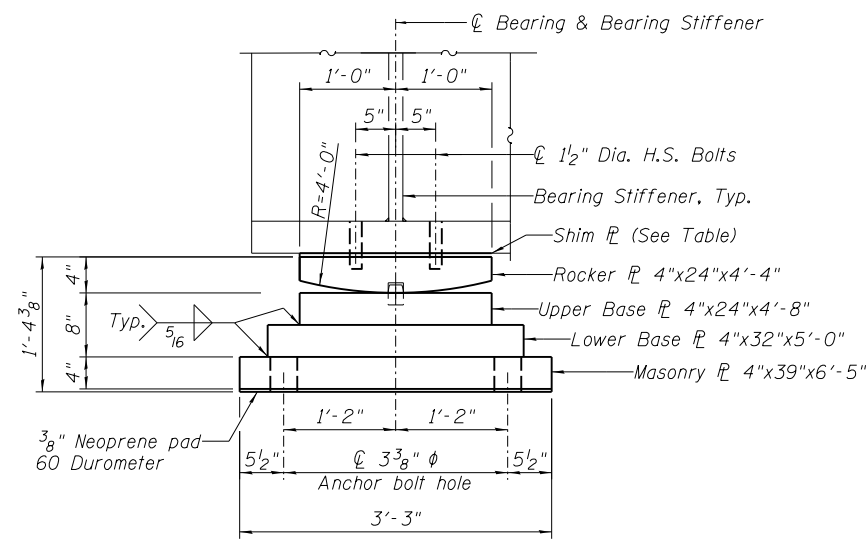
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•	(109) VB,(110) VB-5	SANGAMON	382	252
			CONTRACT NO. 93733	

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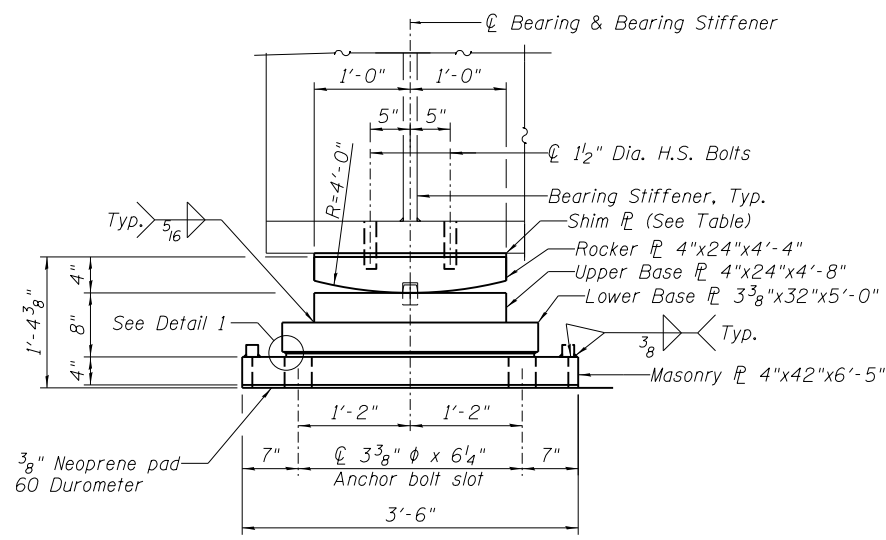
FINAL



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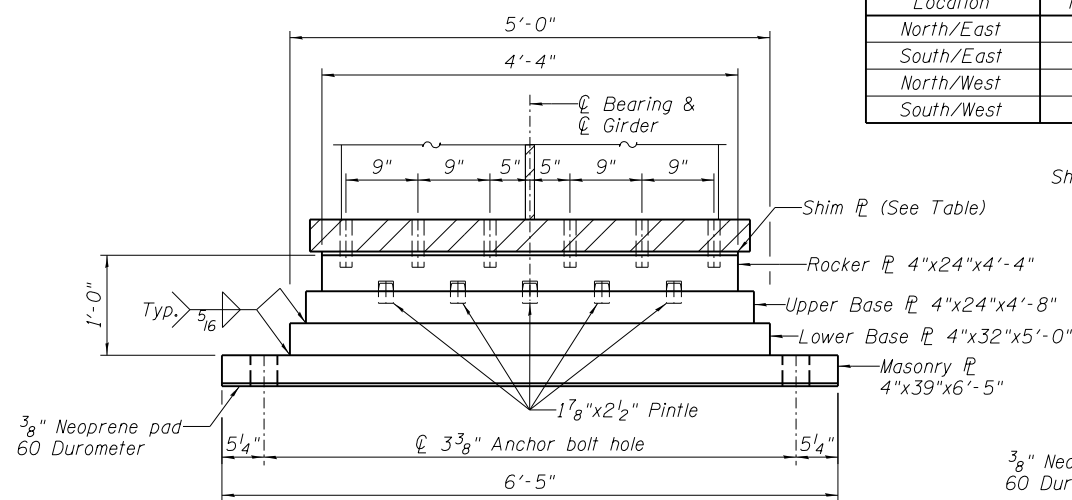


ELEVATION - FIXED BEARING

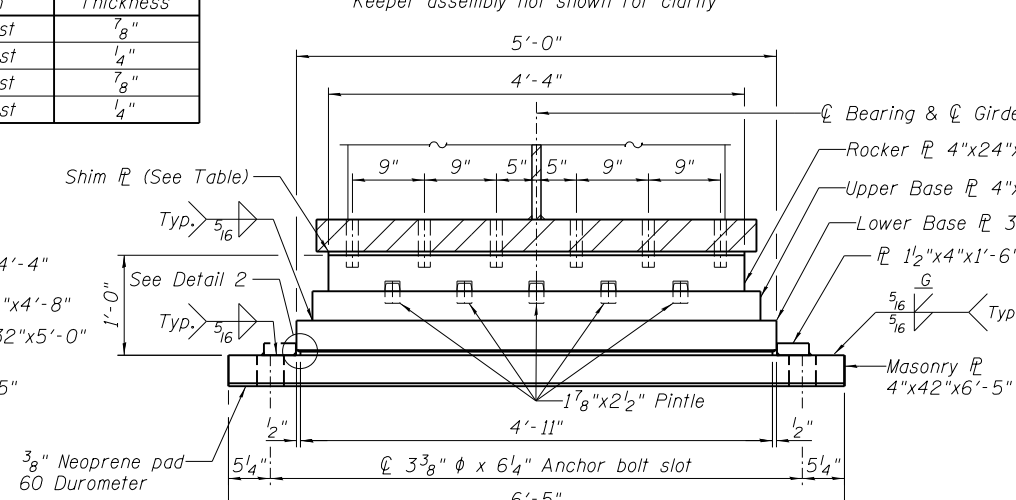


ELEVATION - EXPANSION BEARING

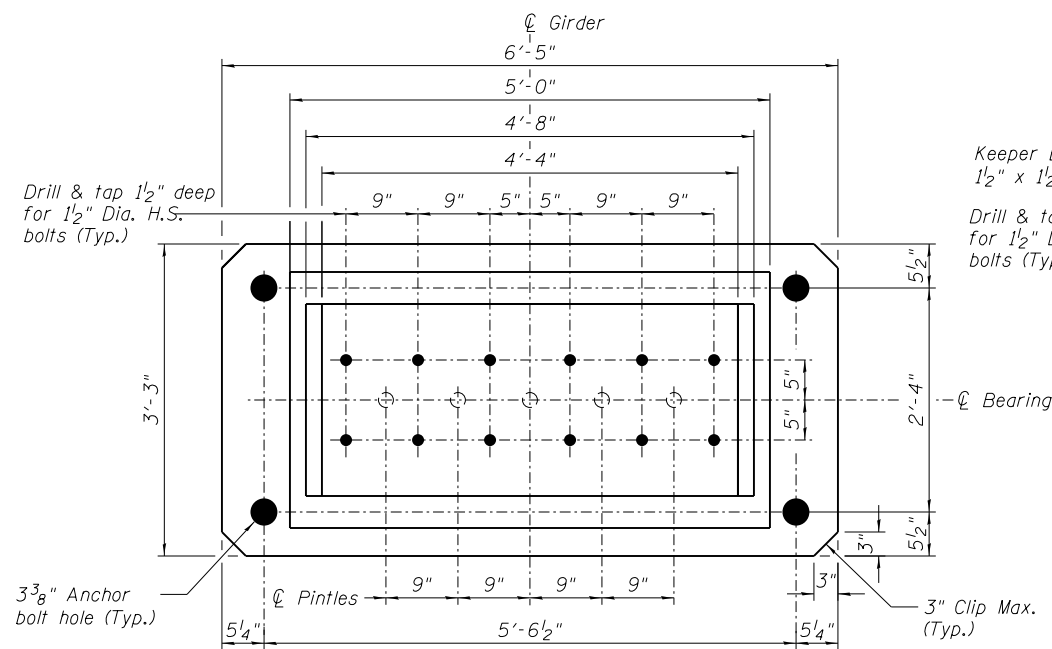
SHIM PLATE THICKNESS	
Location	Thickness
North/East	7/8"
South/East	1/4"
North/West	7/8"
South/West	1/4"



END VIEW - FIXED BEARING

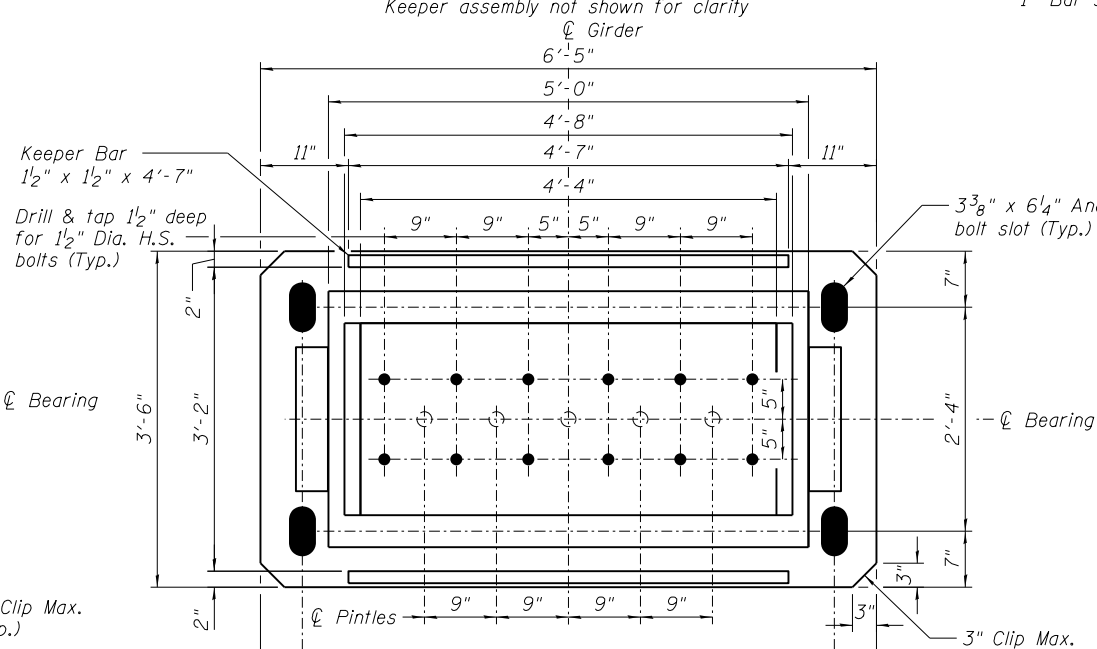


END VIEW - EXPANSION BEARING



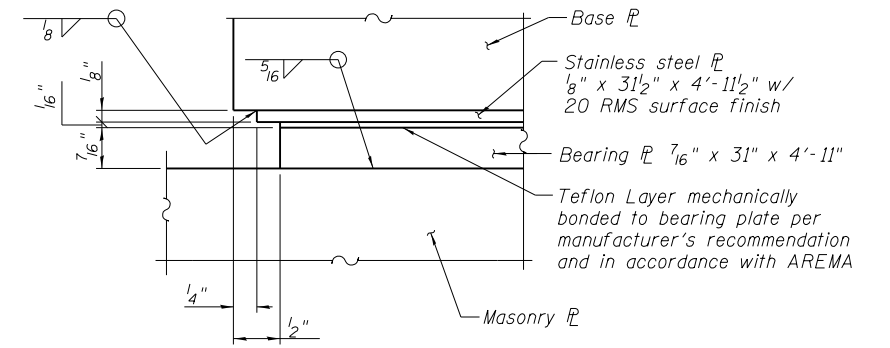
PLAN VIEW - FIXED BEARING

(2 Required)

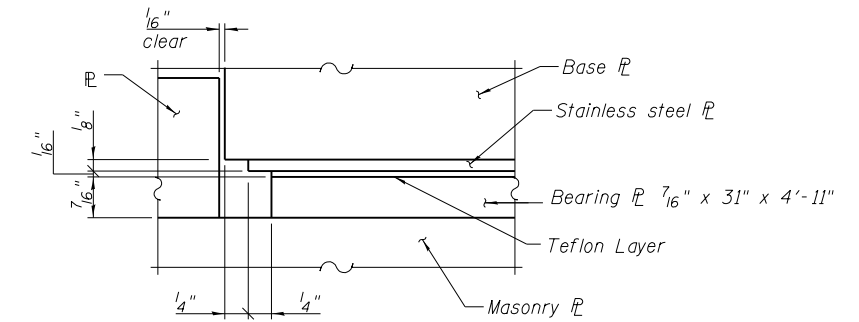


PLAN VIEW - EXPANSION BEARING

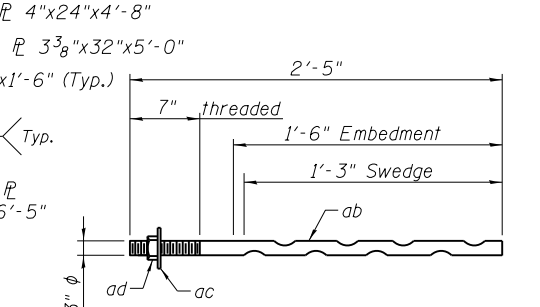
(2 Required)



DETAIL 1

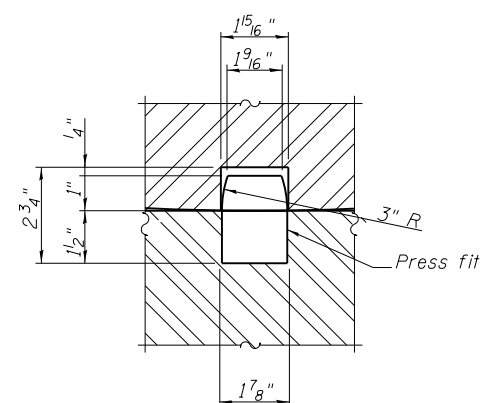


DETAIL 2



ANCHOR BOLT

- 1 - Bar 3" Dia. x 2'-5" - ab
- 1 - Bar 5 1/2" Dia. x 1/4" w/ 3/8" Dia. hole at center - ac
- 1 - Heavy Hex Nut - ad
- Weight = 69 lbs.
- Galvanize after fabrication
- (16 Required)



PINTLE DETAIL

NOTES:

- Steel used for bearing assemblies shall conform to ASTM A709 GR50, unless noted otherwise. Anchor bolts shall conform to ASTM F1554 Gr 105. Cost of bearings and anchor bolts shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 3".
- Stainless steel shall conform to ASTM A480.
- Bearing assembly weldments shall be stress relieved by heat treating prior to finish machining, per current AWS structural welding codes.
- Teflon Layer shall be composed of virgin unfilled TFE resin, unfilled TFE sheets or unfilled TFE fabric. Filler material, such as milled glass fibers will not be allowed. Teflon layer shall conform to the requirements of AREMA Chapter 15.
- All surfaces in moving contact shall be finished 125/.
- All dimensions shown are final dimensions after machining.
- Bearings to be shop fitted to girders, match marked and assembled in units for shipping.
- Anchor bolts, nuts and plate washers shall be galvanized in accordance with ASTM B695, Class 50.
- Anchor bolt nuts shall be A563 Gr DH Heavy Hex & Washers shall be F436 Type 1.
- Bolt removal and replacements to gain access to properly tighten bearing bolts is incidental to steel erection.

FILE NAME = ... PROJECTS\DOCUMENTS\09\JOBS\09L0179B\CAD\STRUCT\6TH\Sheet\0849962-09L0179B-UPRR-001

USER NAME = Pop00275	DESIGNED - MJW	REVISD -
PLOT SCALE = 0:2.0000 " = 1"	CHECKED - TJH/TDP	REVISD -
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISD -
	CHECKED - MJW	REVISD -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TPG BEARING DETAILS
STRUCTURE 084-9962 - 6TH ST UPRR**

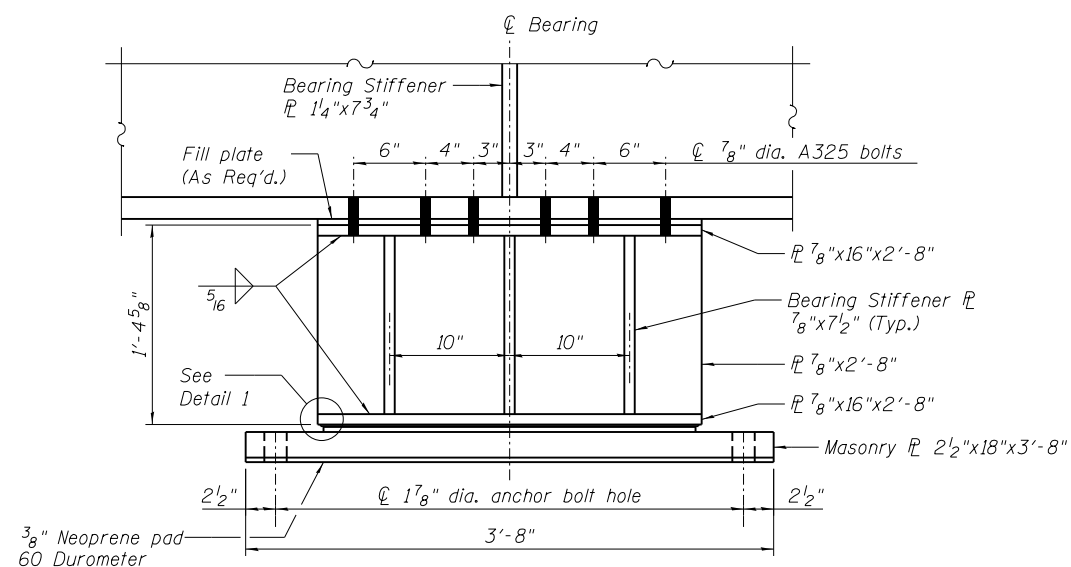
SHEET NO. 20 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 93733	
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

FINAL

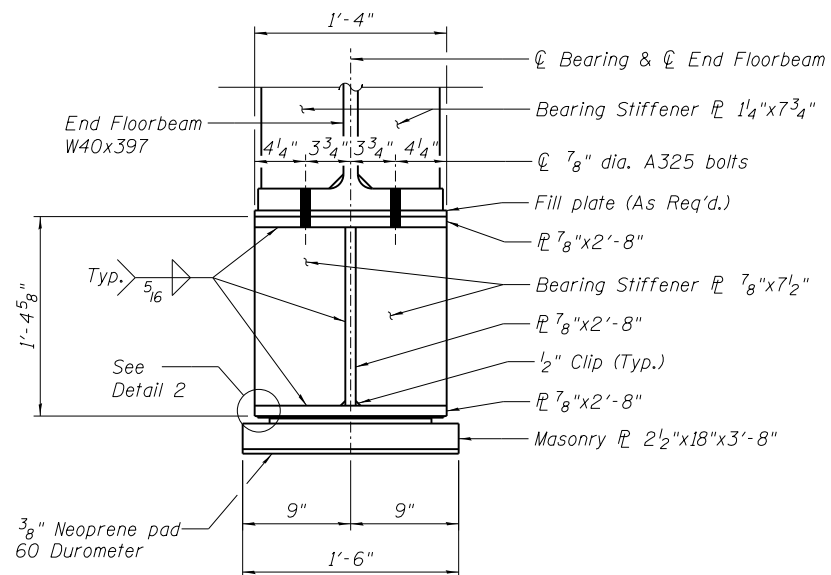


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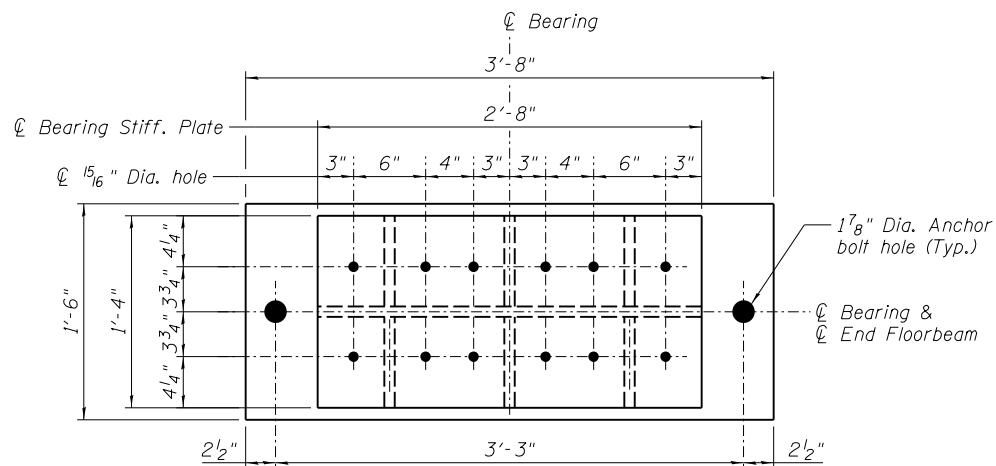
ELEVATION - END FLOORBEAM BEARING

Anchor Bolt not shown for clarity



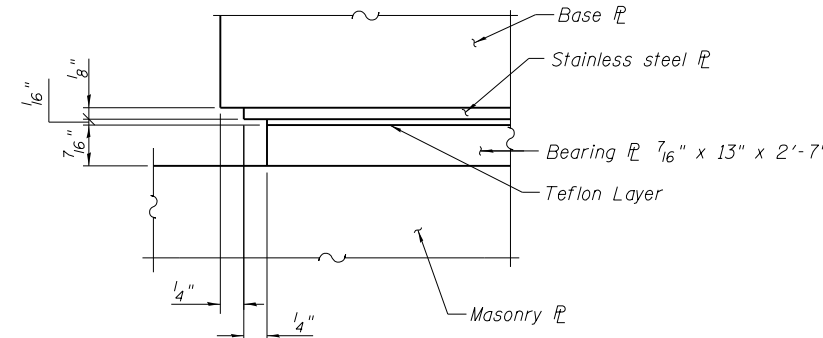
END VIEW - END FLOORBEAM BEARING

Anchor Bolt not shown for clarity

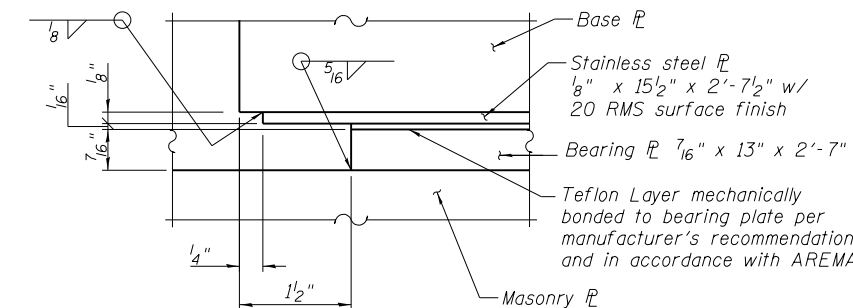


PLAN VIEW - END FLOORBEAM BEARING

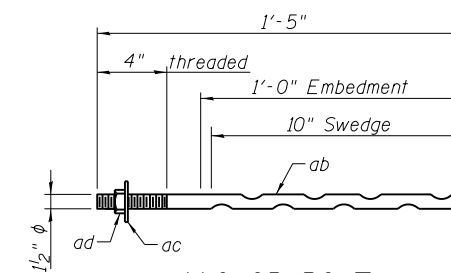
(2 Required)



DETAIL 1



DETAIL 2



ANCHOR BOLT

- 1 - Bar 1/2" Dia. x 1'-5" - ab
- 1 - Bar 3" Dia. x 4" w/ 1 5/8" Dia. hole at center - ac
- 1 - Heavy Hex Nut - ad
- Weight = 10 lbs.
- Galvanize after fabrication
- (4 Required)

NOTES:

1. Steel used for bearing assemblies shall conform to ASTM A709 GR50, unless noted otherwise. Anchor bolts shall conform to ASTM F1554 Gr 105. Cost of bearings and anchor bolts shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 3".
2. Stainless steel shall conform to ASTM A480.
3. Bearing assembly weldments shall be stressed relieved by heat treating prior to finish machining, per current AWS structural welding codes.
4. Teflon Layer shall be composed of virgin unfilled TFE resin, unfilled TFE sheets or unfilled TFE fabric. Filler material, such as milled glass fibers will not be allowed. Teflon layer shall conform to the requirements of AREMA Chapter 15.
5. All surfaces in moving contact shall be finished 125/.
6. All dimensions shown are final dimensions after machining.
7. Bearings to be shop fitted to girders, match marked and assembled in units for shipping.
8. Anchor bolts, nuts and plate washers shall be galvanized in accordance with ASTM B695, Class 50.
9. Anchor bolt nuts shall be A563 Gr DH Heavy Hex & Washers shall be F436 Type 1.
10. Bolt removal and replacements to gain access to properly tighten bearing bolts is incidental to steel erection.

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FILE NAME =	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
	PLOT SCALE = 0:2.0000 ' = 1" / in.	DRAWN - RSJ	REVISED -
	PLOT DATE = 4/11/2019	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**END FLOORBEAM BEARING DETAILS
STRUCTURE 084-9962 - 6TH ST UPRR**

SHEET NO. 21 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO.			93733	

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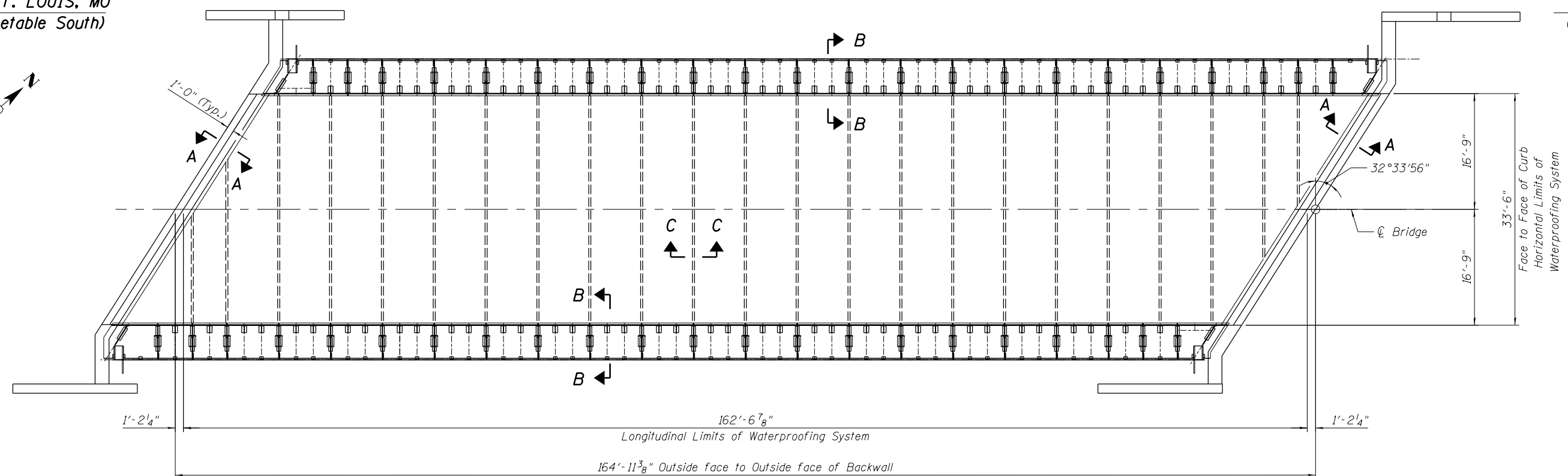
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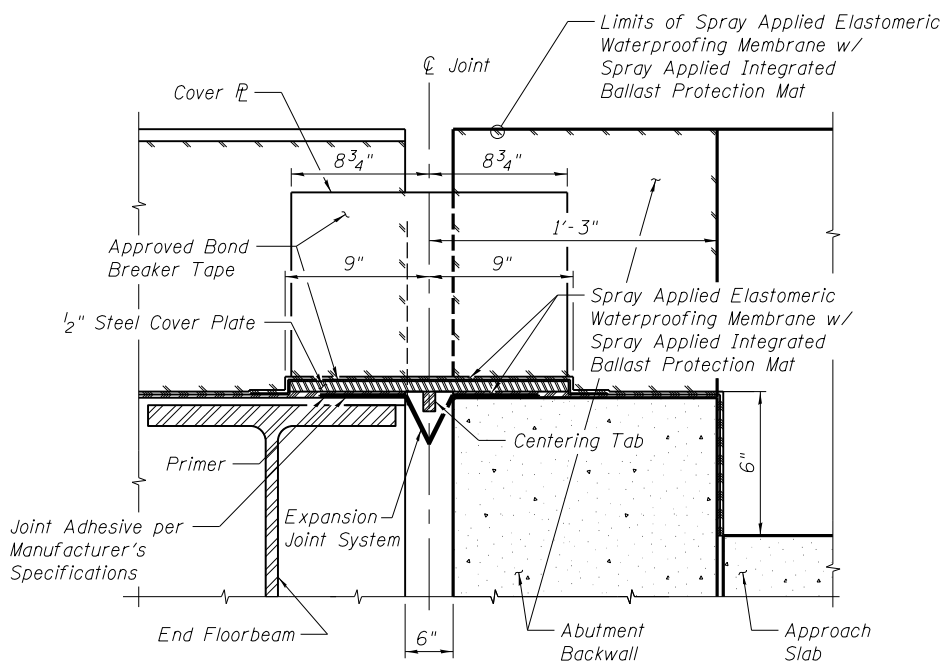
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To ST. LOUIS, MO
(Timetable South)

To JOLIET, IL
(Timetable North)

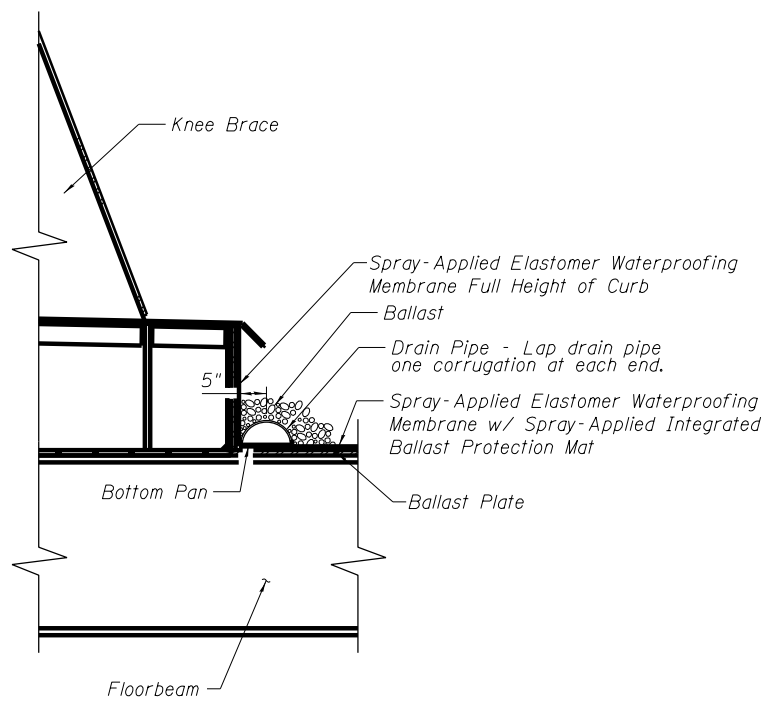


WATERPROOFING LIMITS PLAN

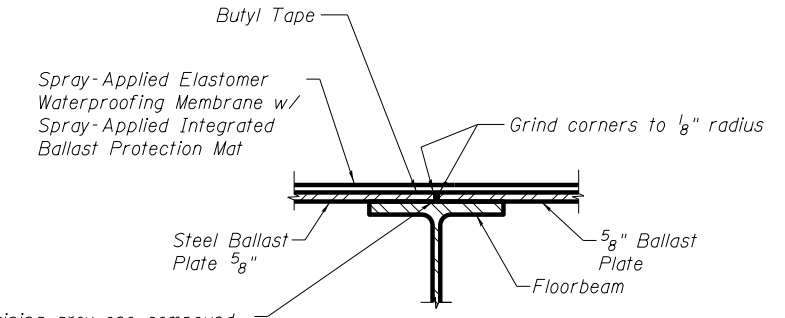


- Note:**
1. Bridge deck membrane continuous thru joint.
 2. Typical Joint Detail shown for information only. Waterproofing installer shall determine final details in accordance with the manufacturer's recommendations.

SECTION A-A
(At Rt. 4's to Bk. of Abut.)



SECTION B-B



Non-staining grey one compound non-sag elastomeric gun grade polyurethane sealant meeting the requirements of ASTM C-920, Type S, Grade NS, Class 25. Cost included with Membrane Waterproofing (Special).

SECTION C-C

- Notes:**
1. Prepare surfaces and apply in accordance with Manufacturer's recommendations.
 2. Structural steel cover plates shall be galvanized.
 3. Cost of joint adhesive and bond breaker tape shall be included in the cost of "Membrane Waterproofing (Special)".
 4. The cover plate is included in the weight of the Structural Steel and will be paid for as "Furnishing and Erecting Structural Steel, Bridge No. 3".
 5. For cover plate details see Sheet 16 of 29.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Membrane Waterproofing (Special)	Sq. Ft.	6,293

Path: \\spr-svr-306.hanson.dom\hanson_projects\Documents\09Jobs\09L0179B\CAD\Struct\6th\Sheet\0849962-09L0179B-UPRR-001



FILE NAME :	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
	PLOT SCALE = 0:2.0000 '1' / in.	DRAWN - RSJ	REVISED -
	PLOT DATE = 4/11/2019	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

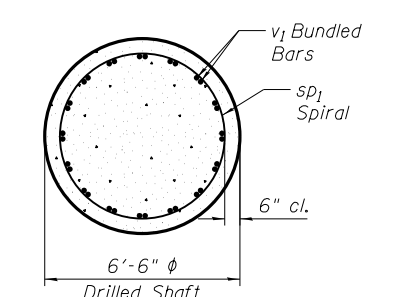
**BRIDGE DECK WATERPROOFING
STRUCTURE 084-9962 - 6TH ST UPRR**

SHEET NO. 22 OF 29 SHEETS

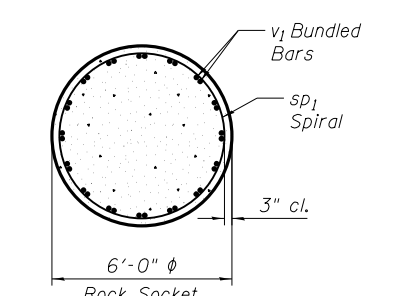
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 93733	
•666 & 666 ALT.		ILLINOIS FED. AID PROJECT		

FINAL

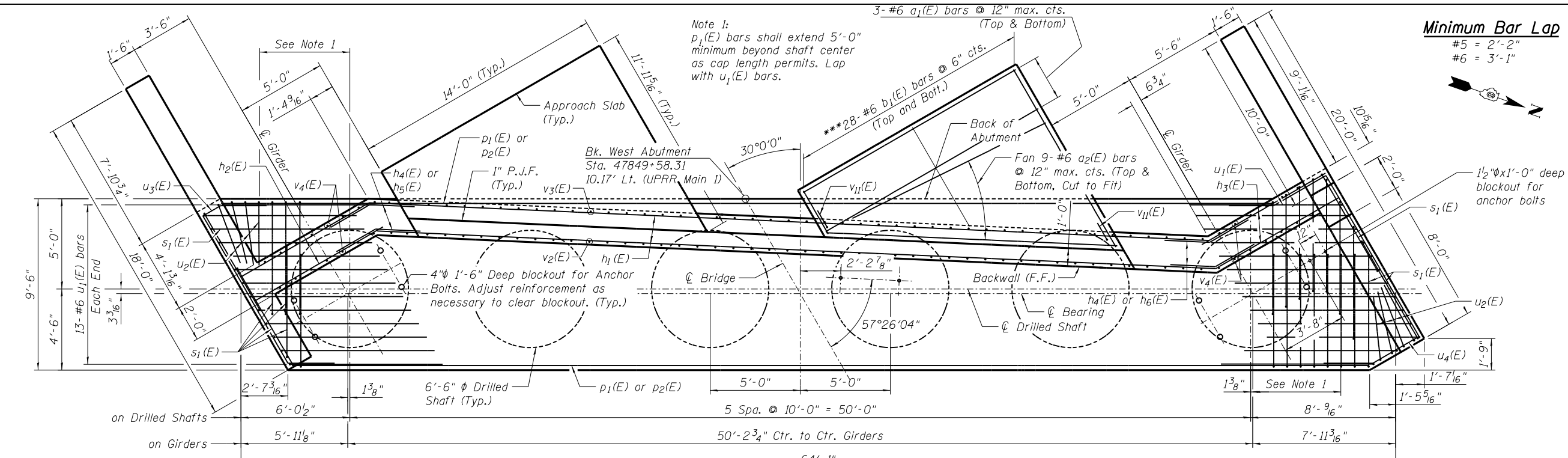
Minimum Bar Lap
 #5 = 2'-2"
 #6 = 3'-1"



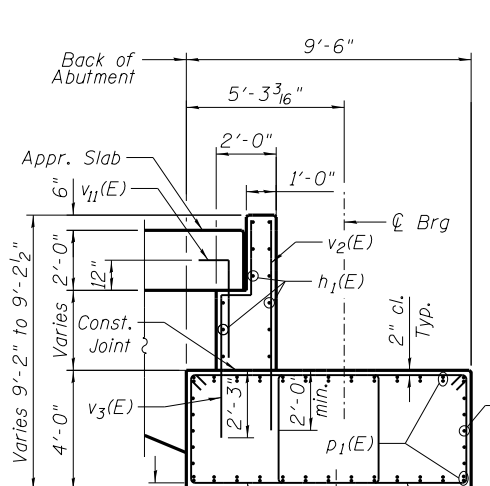
SECTION B-B



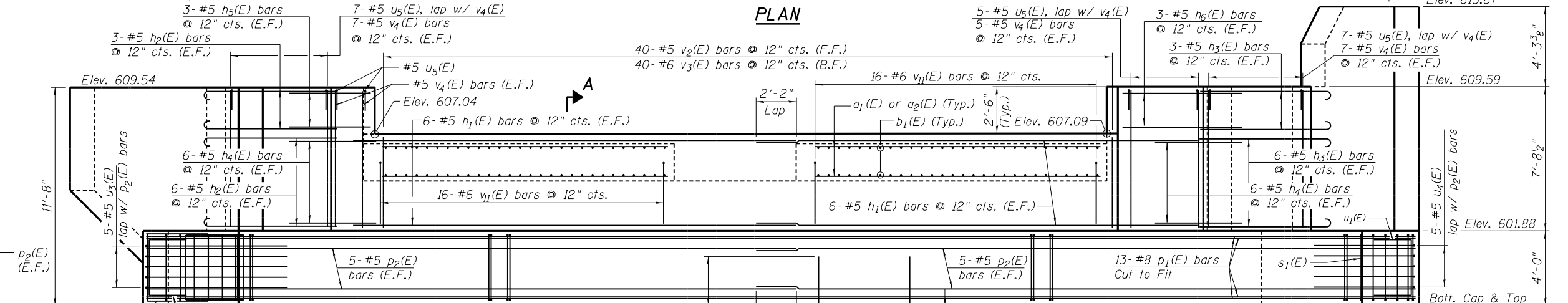
SECTION C-C



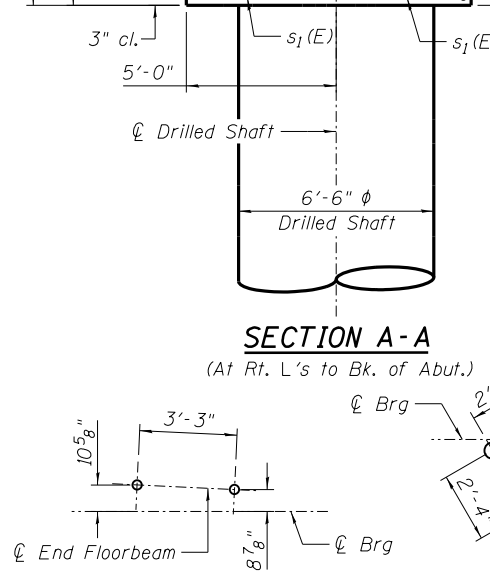
PLAN



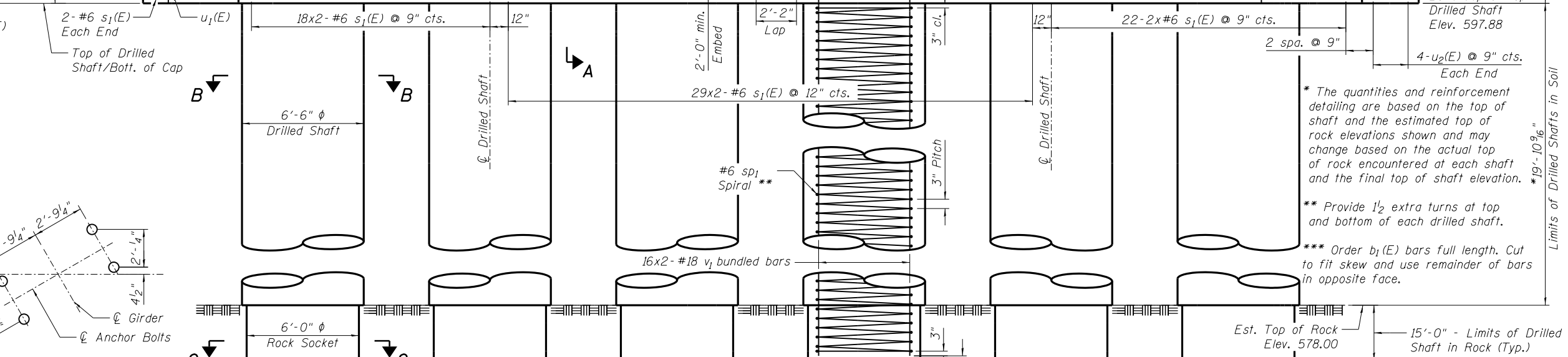
SECTION A-A



ELEVATION - WEST ABUTMENT

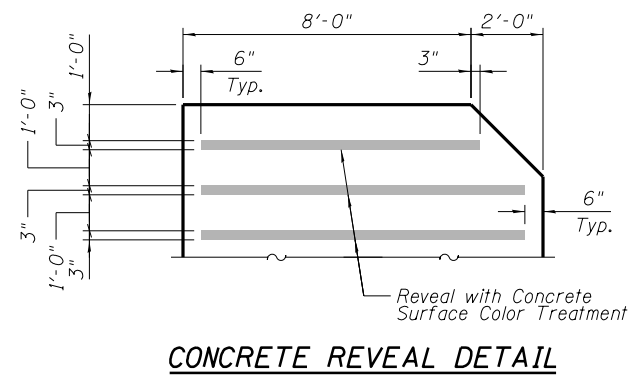


BLOCKOUT LAYOUT

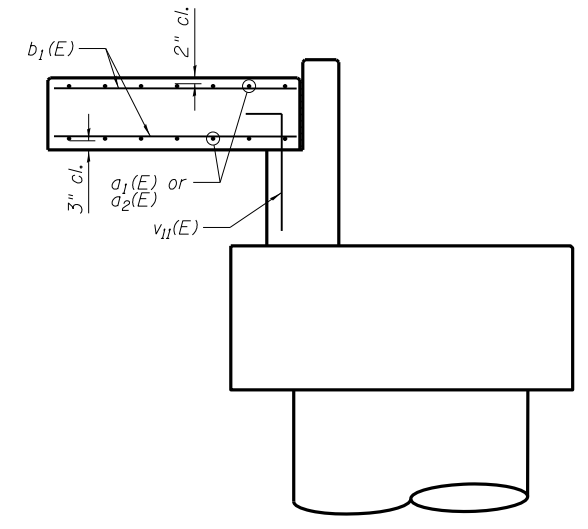


BLOCKOUT LAYOUT

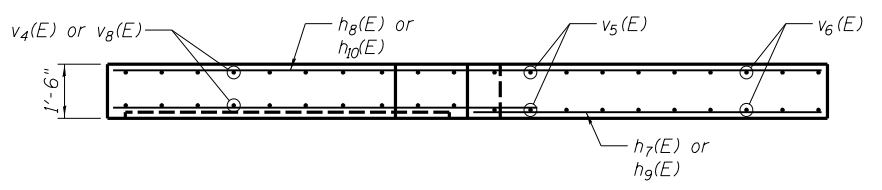
* The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.
 ** Provide 1/2 extra turns at top and bottom of each drilled shaft.
 *** Order b1(E) bars full length. Cut to fit skew and use remainder of bars in opposite face.



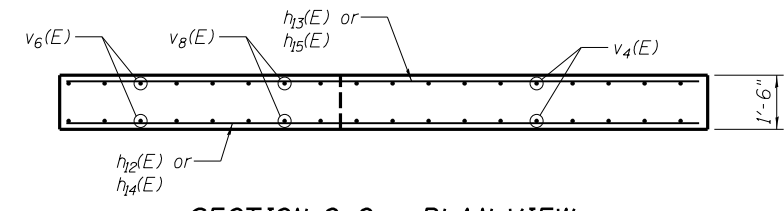
CONCRETE REVEAL DETAIL



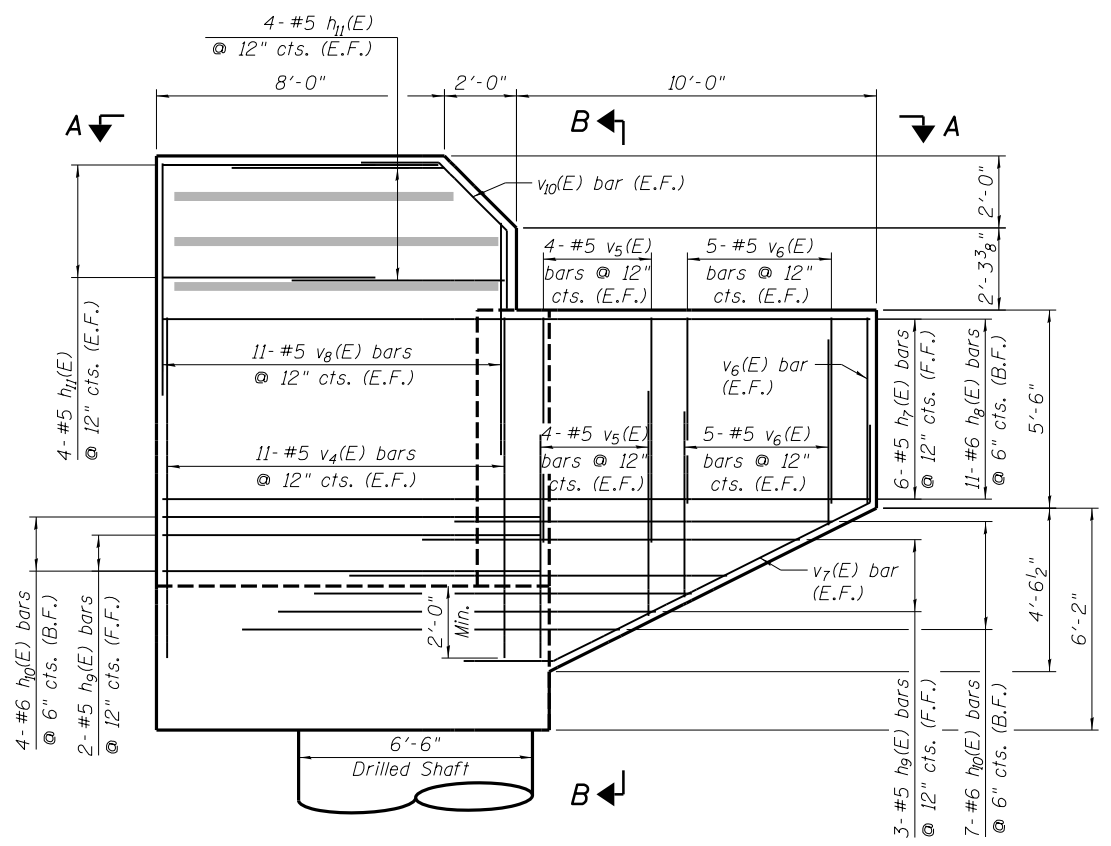
APPROACH SLAB SECTION



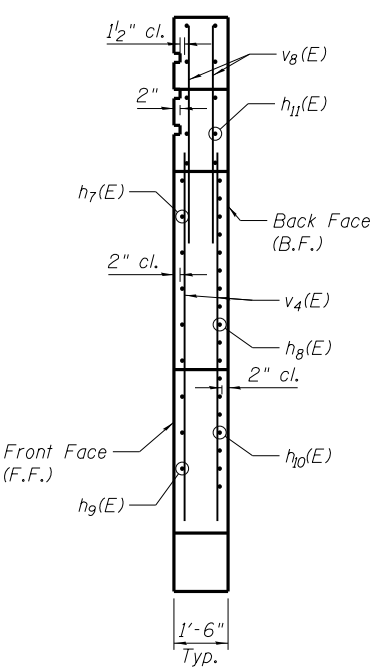
SECTION A-A - PLAN VIEW



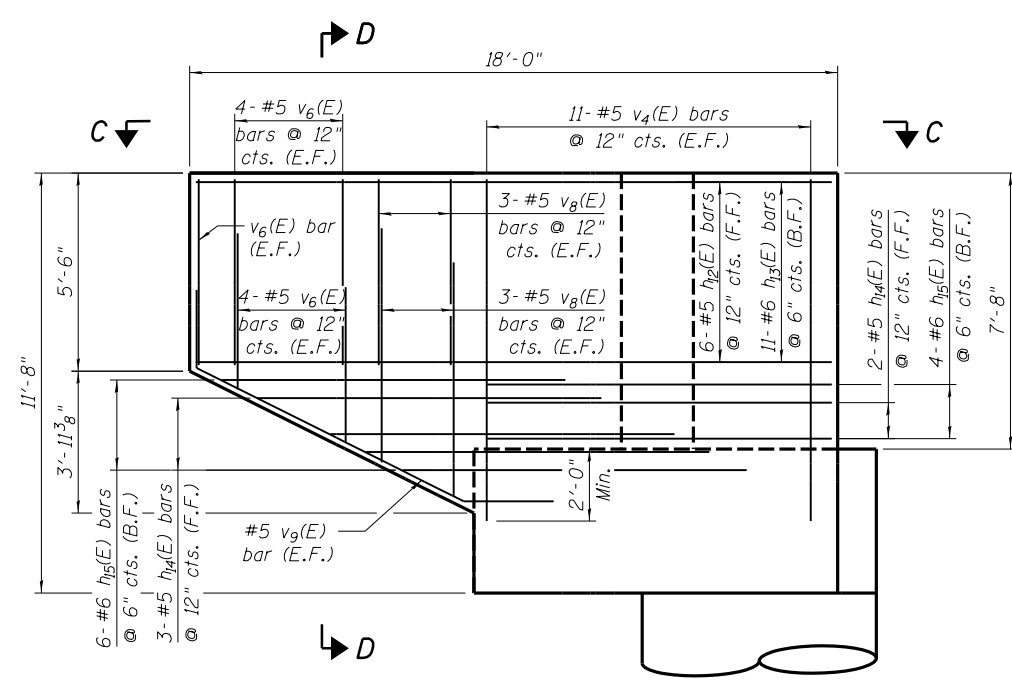
SECTION C-C - PLAN VIEW



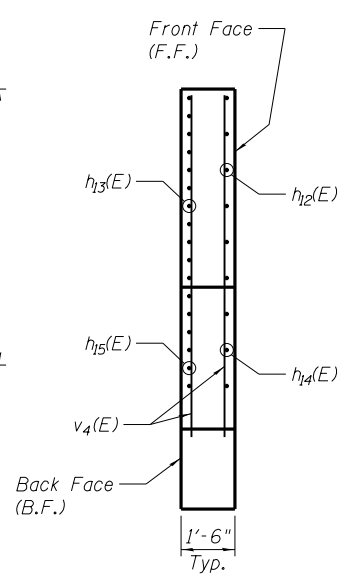
ELEVATION - NORTH WING END VIEW
(Looking South)



WINGWALL SECTION B-B



ELEVATION - SOUTH WING END VIEW
(Looking North)



WINGWALL SECTION D-D

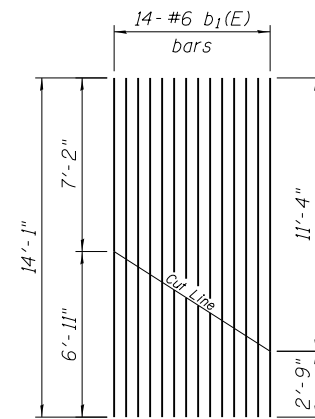
**BILL OF MATERIAL
WEST ABUTMENT**

Bar	No.	Size	Length	Shape
a ₁ (E)	12	#6	13'-8"	—
a ₂ (E)	36	#6	16'-2"	—
b ₁ (E)	56	#6	14'-1"	—
h ₁ (E)	24	#5	24'-5"	—
h ₂ (E)	18	#5	8'-9"	U
h ₃ (E)	18	#5	8'-6"	U
h ₄ (E)	24	#5	5'-0"	/
h ₅ (E)	6	#5	4'-0"	/
h ₆ (E)	6	#5	6'-11"	/
h ₇ (E)	6	#5	19'-8"	—
h ₈ (E)	11	#6	19'-8"	—
h ₉ (E)	5	#5	9'-11"	—
h ₁₀ (E)	11	#6	10'-11"	—
h ₁₁ (E)	16	#5	5'-11"	—
h ₁₂ (E)	6	#5	17'-8"	—
h ₁₃ (E)	11	#6	17'-8"	—
h ₁₄ (E)	5	#5	9'-1"	—
h ₁₅ (E)	10	#6	10'-1"	—
p ₁ (E)	52	#8	60'-0"	—
p ₂ (E)	20	#5	32'-2"	—
s ₁ (E)	146	#6	21'-0"	□
sp ₁	6	#6	*34'-2"	WWW
u ₁ (E)	26	#6	20'-5"	J
u ₂ (E)	8	#5	10'-7"	J
u ₃ (E)	5	#5	15'-3"	/
u ₄ (E)	5	#5	21'-3"	/
u ₅ (E)	21	#5	3'-4"	J
v ₁	192	#18	36'-11"	—
v ₂ (E)	40	#5	7'-1"	—
v ₃ (E)	40	#6	8'-4"	—
v ₄ (E)	86	#5	9'-7"	—
v ₅ (E)	16	#5	6'-3"	—
v ₆ (E)	40	#5	5'-2"	—
v ₇ (E)	2	#5	14'-6"	/
v ₈ (E)	34	#5	6'-6"	—
v ₉ (E)	2	#5	13'-0"	/
v ₁₀ (E)	2	#5	7'-0"	/
v ₁₁ (E)	32	#6	4'-3"	J
Structure Excavation		Cu. Yds.	179	
Concrete Structures		Cu. Yds.	147.1	
Drilled Shaft in Soil		Cu. Yds.	146.6	
Drilled Shaft in Rock		Cu. Yds.	94.2	
Reinforcement Bars		Pound	118,130	
Reinforcement Bars, Epoxy Coated		Pound	22,060	
Crosshole Sonic Logging Access Ducts		Foot	1,346	

* Length is height of spiral

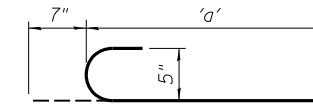
MIN. BAR LAPS FOR SPIRAL

#6 bars = 2'-7"



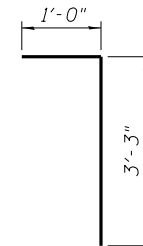
BAR CUTTING DIAGRAM FOR b₁(E)

Order b₁(E) full length. Cut as shown and use remainder of bars in opposite face.

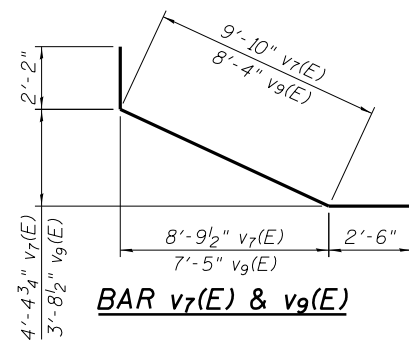


BARS h₂(E), h₃(E)

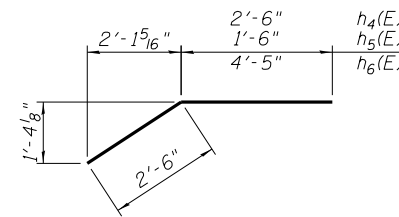
Bar	'a'
h ₂ (E)	8'-2"
h ₃ (E)	7'-11"



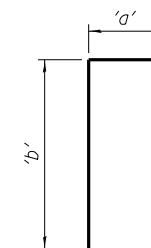
BAR v₁₁(E)



BAR v₇(E) & v₉(E)

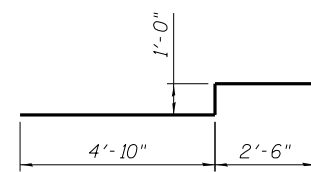


BARS h₄(E) & h₅(E) & h₆(E)

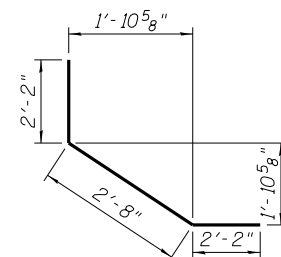


BARS u₁(E), u₂(E), u₅(E)

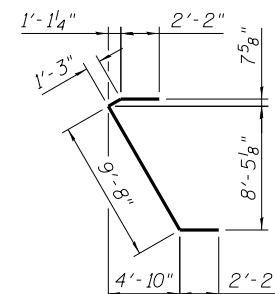
Bar	'a'	'b'
u ₁ (E)	3'-5"	8'-6"
u ₂ (E)	3'-7"	3'-6"
u ₅ (E)	1'-8"	0'-10"



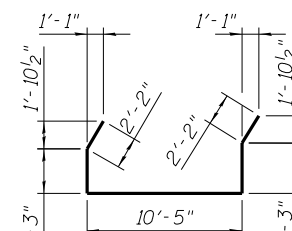
BAR v₃(E)



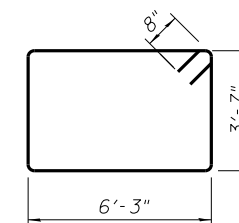
BARS v₁₀(E)



BAR u₃(E)

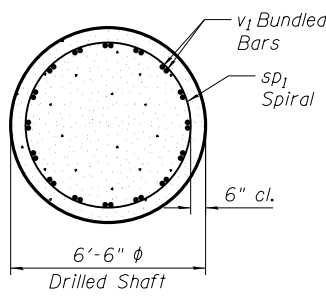


BAR u₄(E)

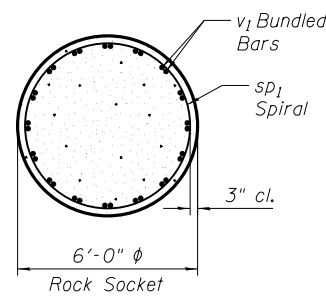


BAR s₁(E)

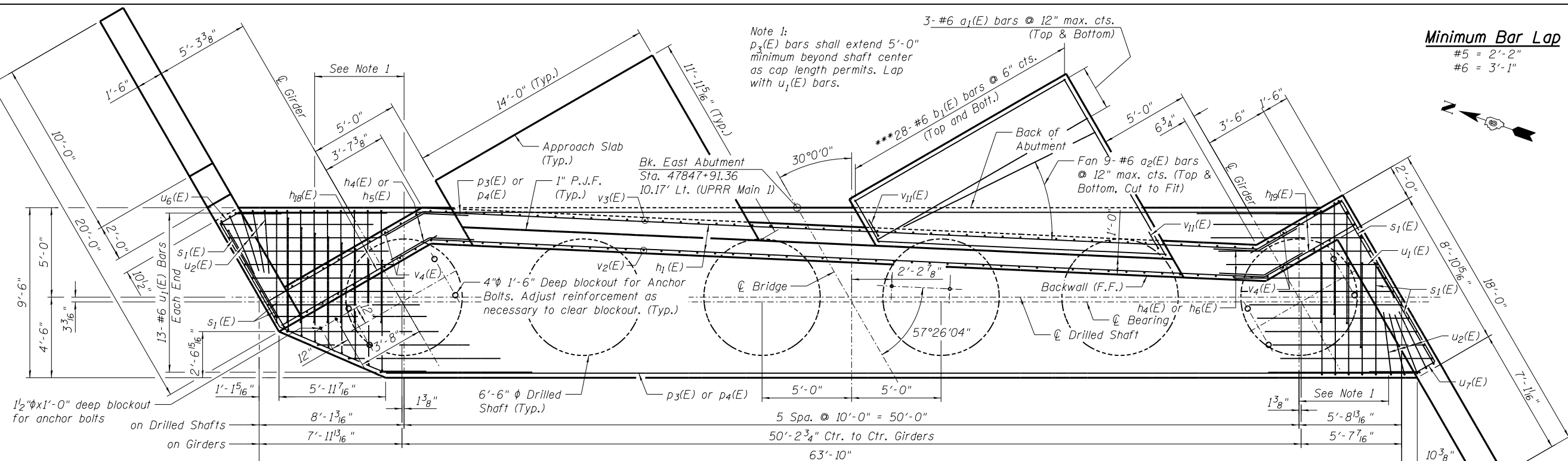
Minimum Bar Lap
 #5 = 2'-2"
 #6 = 3'-1"



SECTION B-B

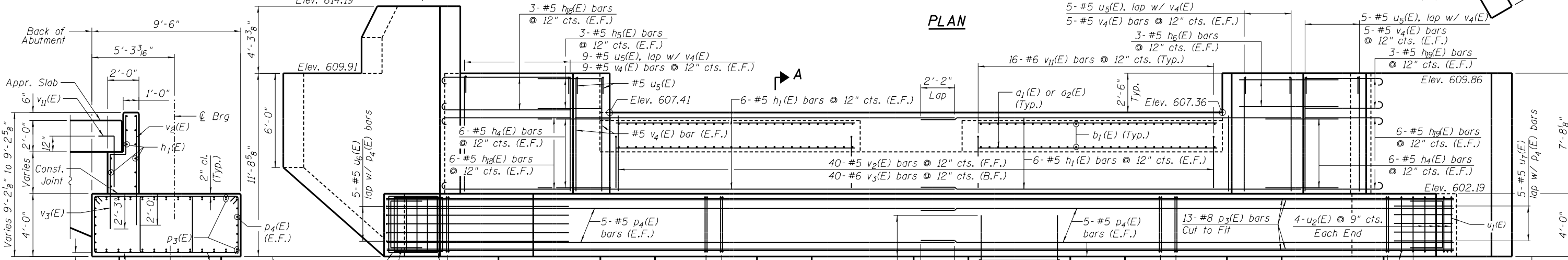


SECTION C-C

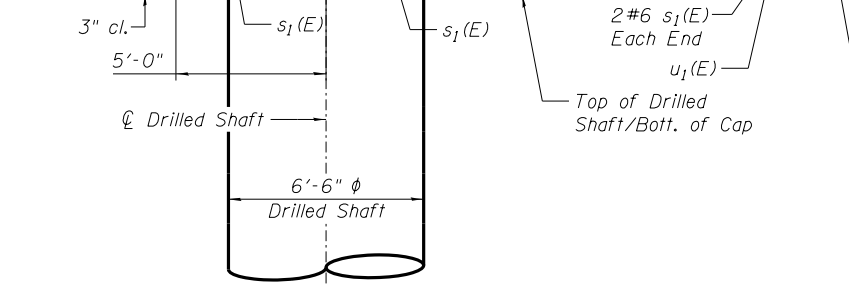


Note 1:
 p3(E) bars shall extend 5'-0"
 minimum beyond shaft center
 as cap length permits. Lap
 with u1(E) bars.

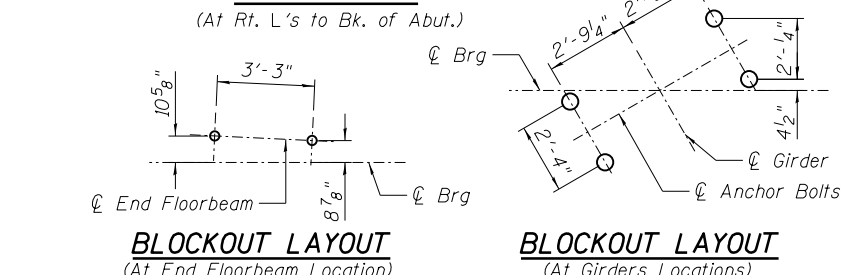
PLAN



ELEVATION - EAST ABUTMENT
 (Looking East)



SECTION A-A
 (At Rt. L's to Bk. of Abut.)



BLOCKOUT LAYOUT
 (At End Floorbeam Location)

BLOCKOUT LAYOUT
 (At Girders Locations)

* The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.
 ** Provide 1/2 extra turns at top and bottom of each drilled shaft.
 *** Order b1(E) bars full length. Cut to fit skew and use remainder of bars in opposite face.

Project Name: STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION - EAST ABUTMENT STRUCTURE 084-9962 - 6TH ST UPRR

FILE NAME	USER NAME	DESIGNED	REVISIONS
PROJECT	POP00275	MJW	-
		TJH/TDP	-
		RSJ	-
		MJW	-

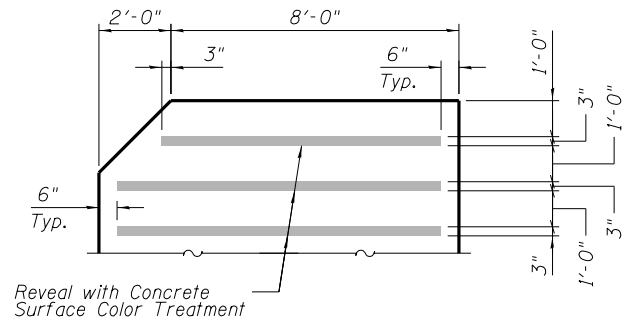
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**EAST ABUTMENT
 STRUCTURE 084-9962 - 6TH ST UPRR**

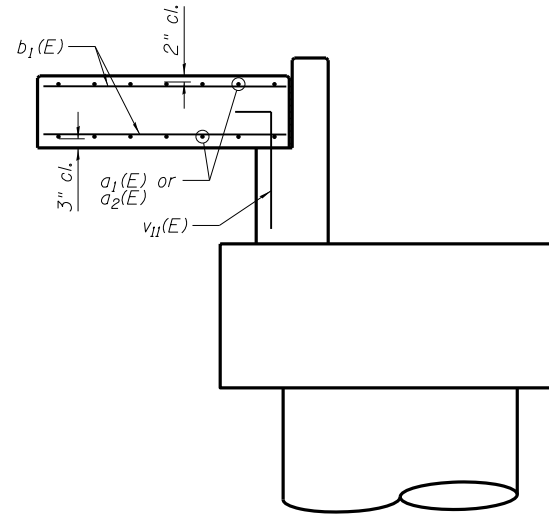
SHEET NO. 26 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
666 & 666 ALT.	(109) VB,(110) VB-5	SANGAMON	382	259
CONTRACT NO. 93733			ILLINOIS FED. AID PROJECT	

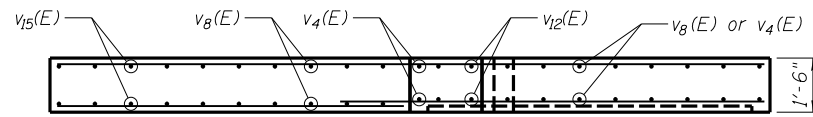
FINAL



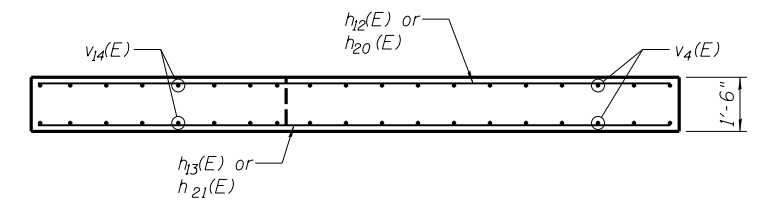
CONCRETE REVEAL DETAIL



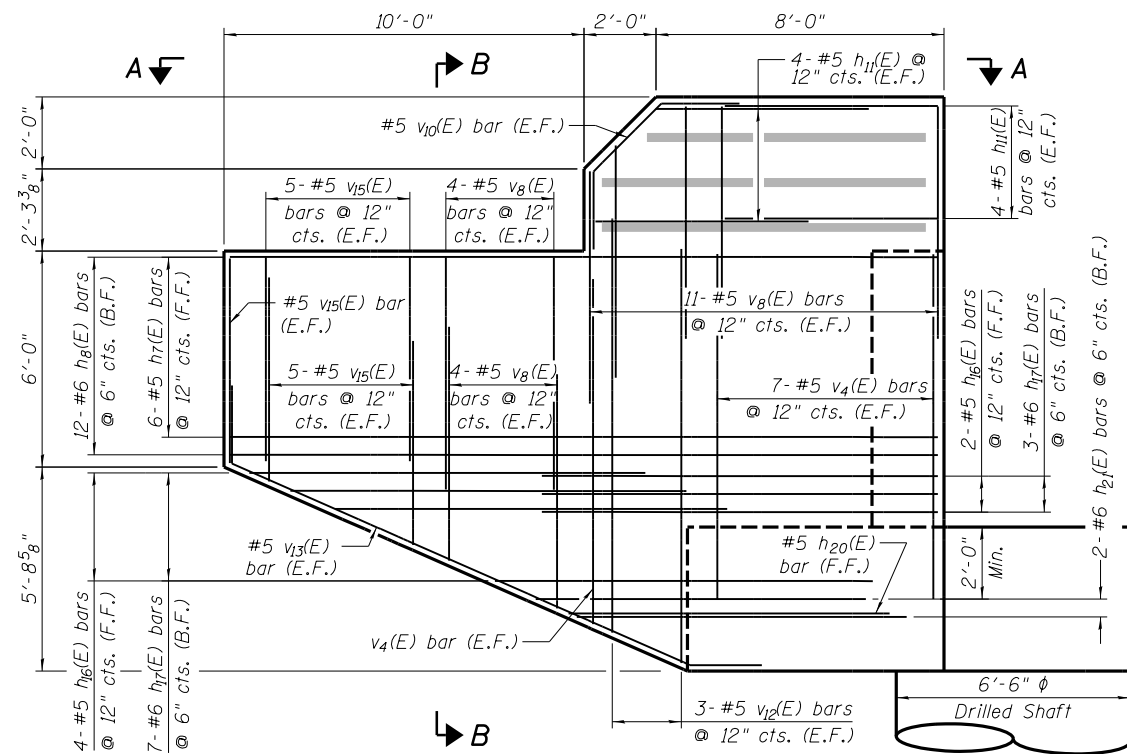
APPROACH SLAB SECTION



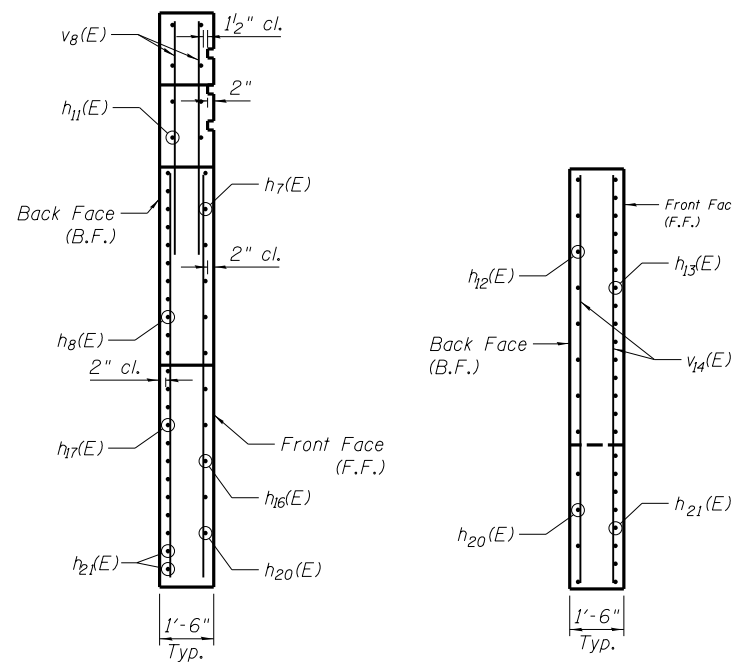
SECTION A-A - PLAN VIEW



SECTION C-C - PLAN VIEW

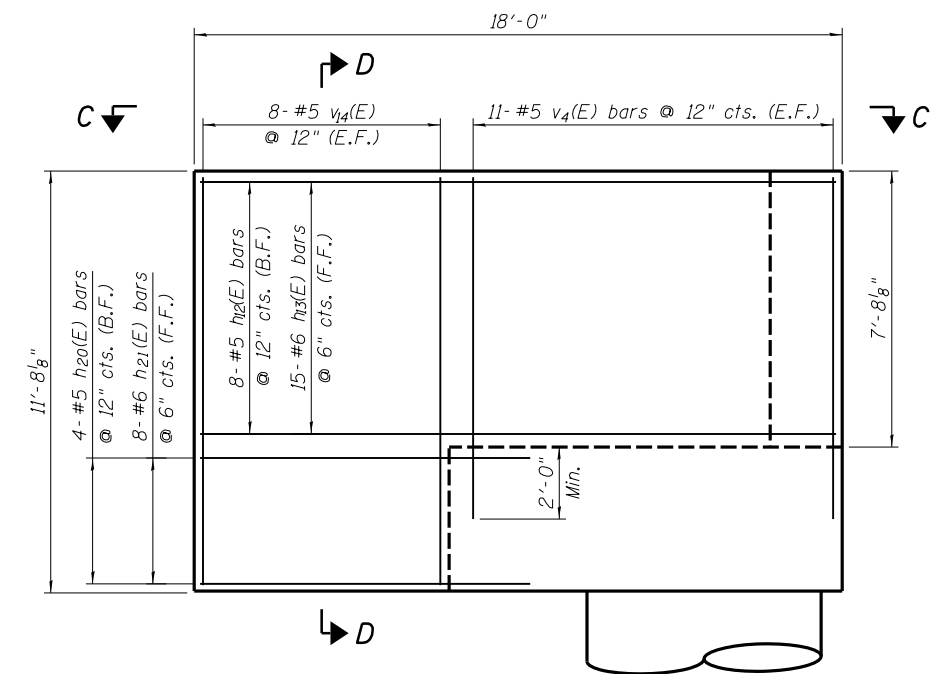


ELEVATION - NORTH WING END VIEW
(Looking South)



WINGWALL SECTION B-B

CHEEK WALL SECTION D-D



ELEVATION - SOUTH CHEEK END VIEW
(Looking North)

\\spr-svr306.hanson.dom\hanson_projects\Documents\09Jobs\09L0179B\CAD\Struct\6th\Sheet\0849962-09L0179B-UPRR-001

FILE NAME =
HANSON
© Copyright Hanson Professional Services Inc, 2019

USER NAME = Pop00275
PLOT SCALE = 0:2.0000 ' = 1/8" in.
PLOT DATE = 4/11/2019

DESIGNED - MJW
CHECKED - TJH/TDP
DRAWN - RSJ
CHECKED - MJW

REVISED -
REVISED -
REVISED -
REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EAST ABUTMENT DETAILS
STRUCTURE 084-9962 - 6TH ST UPRR

SHEET NO. 27 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	260
			CONTRACT NO.	93733
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

FINAL

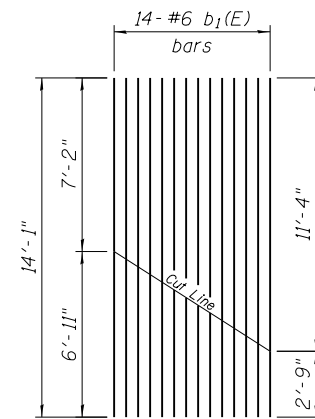
**BILL OF MATERIAL
EAST ABUTMENT**

Bar	No.	Size	Length	Shape
a ₁ (E)	12	#6	13'-8"	—
a ₂ (E)	36	#6	16'-2"	—
b ₁ (E)	56	#6	14'-1"	—
h ₁ (E)	24	#5	24'-5"	—
h ₄ (E)	24	#5	5'-0"	—
h ₅ (E)	6	#5	4'-0"	—
h ₆ (E)	6	#5	6'-11"	—
h ₇ (E)	6	#5	19'-8"	—
h ₈ (E)	12	#6	19'-8"	—
h ₁₁ (E)	16	#5	5'-11"	—
h ₁₂ (E)	8	#5	17'-8"	—
h ₁₃ (E)	15	#6	17'-8"	—
h ₁₆ (E)	6	#5	10'-6"	—
h ₁₇ (E)	12	#6	11'-0"	—
h ₁₈ (E)	18	#5	10'-8"	—
h ₁₉ (E)	18	#5	6'-6"	—
h ₂₀ (E)	5	#5	8'-11"	—
h ₂₁ (E)	10	#6	9'-2"	—
p ₃ (E)	52	#8	60'-0"	—
p ₄ (E)	20	#5	32'-0"	—
s ₁ (E)	148	#6	21'-0"	□
sp ₂	6	#6	*34'-5"	WWW
u ₁ (E)	26	#6	20'-5"	┌
u ₂ (E)	8	#5	10'-7"	┌
u ₅ (E)	21	#5	3'-4"	┌
u ₆ (E)	5	#5	18'-10"	└
u ₇ (E)	5	#5	17'-4"	└
v ₁	192	#18	36'-11"	—
v ₂ (E)	40	#5	7'-1"	—
v ₃ (E)	40	#6	8'-4"	—
v ₄ (E)	80	#5	9'-7"	—
v ₈ (E)	38	#5	6'-6"	—
v ₁₀ (E)	2	#5	7'-0"	—
v ₁₁ (E)	32	#6	4'-3"	—
v ₁₂ (E)	6	#5	11'-6"	—
v ₁₃ (E)	2	#5	18'-6"	—
v ₁₄ (E)	16	#5	11'-4"	—
v ₁₅ (E)	22	#5	5'-8"	—
Structure Excavation		Cu. Yds.	108	
Concrete Structures		Cu. Yds.	150.7	
Drilled Shaft in Soil		Cu. Yds.	148.9	
Drilled Shaft in Rock		Cu. Yds.	94.2	
Reinforcement Bars		Pound	118,320	
Reinforcement Bars, Epoxy Coated		Pound	22,290	
Crosshole Sonic Logging Access Ducts		Foot	1,357	

* Length is height of spiral

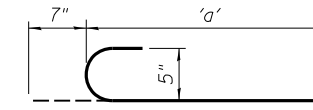
MIN. BAR LAPS FOR SPIRAL

#6 bars = 2'-7"



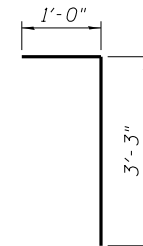
BAR CUTTING DIAGRAM FOR b₁(E)

Order b₁(E) full length. Cut as shown and use remainder of bars in opposite face.

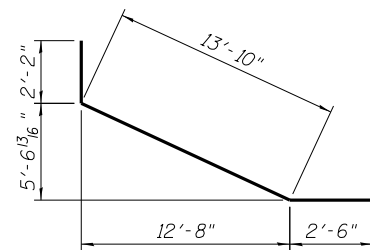


BARS h₁₈(E), h₁₉(E)

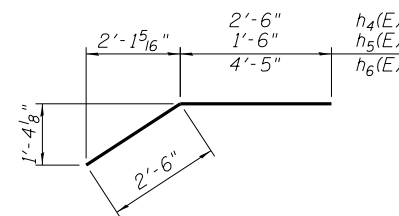
Bar	'a'
h ₁₈ (E)	10'-7"
h ₁₉ (E)	5'-11"



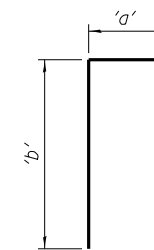
BAR v₁₁(E)



BAR v₁₃(E)

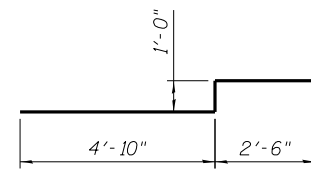


BARS h₄(E) & h₅(E) & h₆(E)

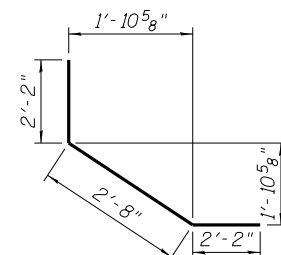


BARS u₁(E), u₂(E), u₅(E)

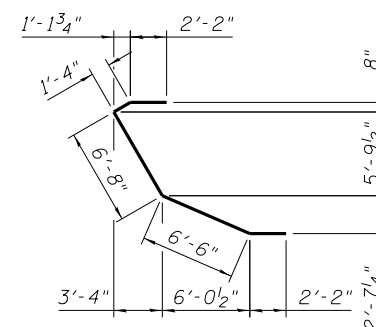
Bar	'a'	'b'
u ₁ (E)	3'-5"	8'-6"
u ₂ (E)	3'-7"	3'-6"
u ₅ (E)	1'-8"	0'-10"



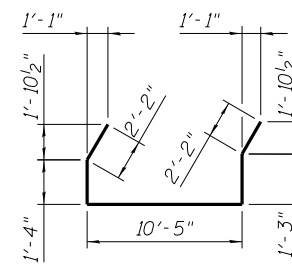
BAR v₃(E)



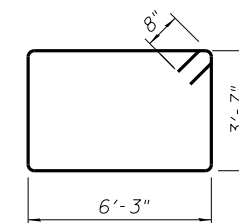
BARS v₁₀(E)



BAR u₆(E)



BAR u₇(E)



BAR s₁(E)

B-145
Sta. 998+21.66' LT
9/5/13

	N	Qu	w%	
601.0				TOPSOIL
600.04	8	4.50P	15	Brown very fine sandy clayey SILT, some brick and rock fragments - FILL.
	12	4.50P	16	
595.04	12	3.00P	21	Brown and gray very fine sandy SILT.
	8	1.44B	23	
590.04	7	3.00P	24	Brown very fine sandy SILT, some clay.
587.54	5	0.58B	26	Dark gray very fine sandy silty CLAY.
585.04	5	1.03B	24	Gray very fine sandy silty CLAY, trace small gravel.
	5	0.70B	22	
577.54	63	4.50P	16	Brown and gray SHALE. (HIGHLY WEATHERED SHALE)
572.54	50/4"		9	Gray SHALE.
	50/5"		8	
566.04	Rec. = 77% RQD = 73% Rec. = 90% RQD = 56%			Gray sandy SHALE, micaceous.
562.54	11.3 Rec. = 90% Rec. = 99% RQD = 68%	RQD = 48%		Gray clayey SHALE.
558.04				Gray sandy SHALE, micaceous.
556.04	Rec. = 100% RQD = 46% Rec. = 67% RQD = 0%			COAL.
551.54				Bottom of Hole = 49.5 feet

B-146
Sta. 1000+74.15' RT
9/11/13

	N	Qu	w%	
587.0				ASPHALT.
586.61				CONCRETE.
585.86	4		24	Dark gray very fine sandy silty CLAY.
583.53	4	0.66B	25	Blue-gray very fine to fine sandy silty CLAY.
	6	2.47S	19	
578.53	57	4.50P	14	Brown and gray SHALE. (HIGHLY WEATHERED SHALE)
576.03	50	4.50P	11	Gray SHALE.
572.03	50/5"		11	Gray clayey SHALE, micaceous.
	Rec. = 81% RQD = 19% Rec. = 88% RQD = 71%			
	12.7 Rec. = 75% Rec. = 85% RQD = 51%	RQD = 44%		
	21.9 Rec. = 91% RQD = 78%			
556.5	Rec. = 100% RQD = 78%			Stiff to very stiff gray shaley CLAY.
553.5				Gray sandy SHALE, micaceous.
553.0				COAL.
552.0				Bottom of Hole = 35.0 feet

LEGEND

- N Standard Penetration Test N (blows/ft)
- Qu Unconfined Strength (tsf)
- w% Natural Moisture Content (%)

DD ∇ Water Surface Elevation Encountered in Boring
 558.10 ∇ DD = during drilling
 Oh = at completion
 24h = 24 hours after completion

D:\s\pr\svr\306\hanson\dom\hanson_projects\Documents\09\Jobs\09L0179B\CAD\Struct\6th\Sheet\0849962-09L0179B-UPRR-001

FILE NAME :	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
	PLOT SCALE = 0:2.0000 ' = 1"	DRAWN - RSJ	REVISED -
	PLOT DATE = 4/11/2019	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**SUBSURFACE DATA PROFILE
STRUCTURE 084-9962 - 6TH ST UPRR**

SHEET NO. 29 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(109) VB,(110) VB-5	SANGAMON	382	262
CONTRACT NO.			93733	

*666 & 666 ALT. ILLINOIS FED. AID PROJECT

FINAL



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Benchmark:
 BM# D2218-07 - Chiseled 'X' on West Bolt of fire hydrant - SE Quad
 6th Street and Wellesly Avenue.
 Elevation = 598.884

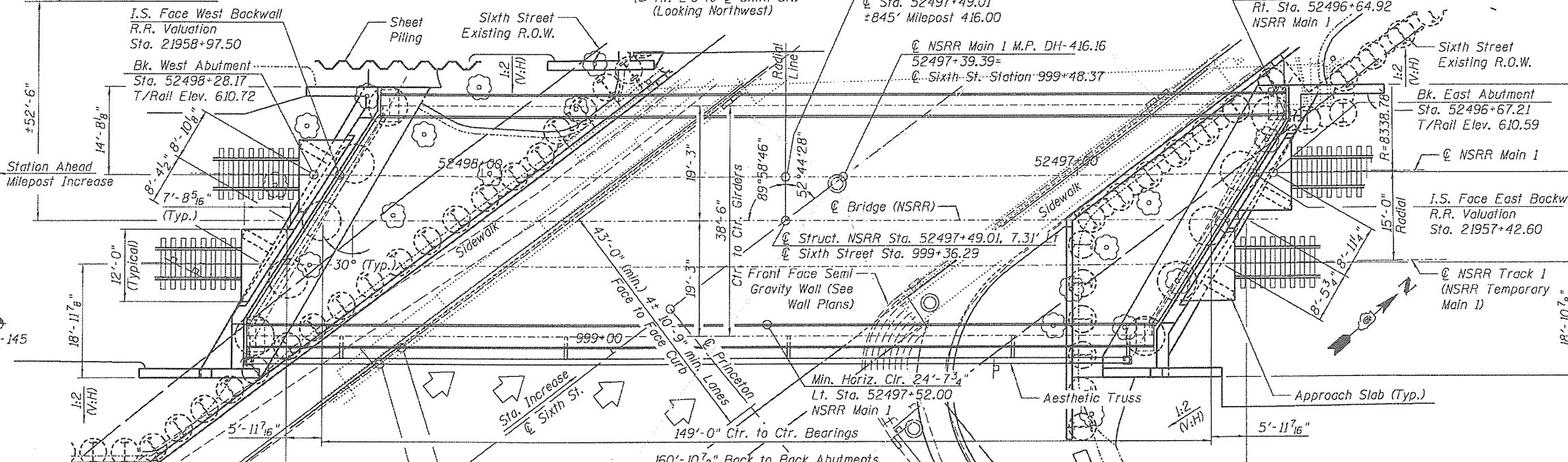
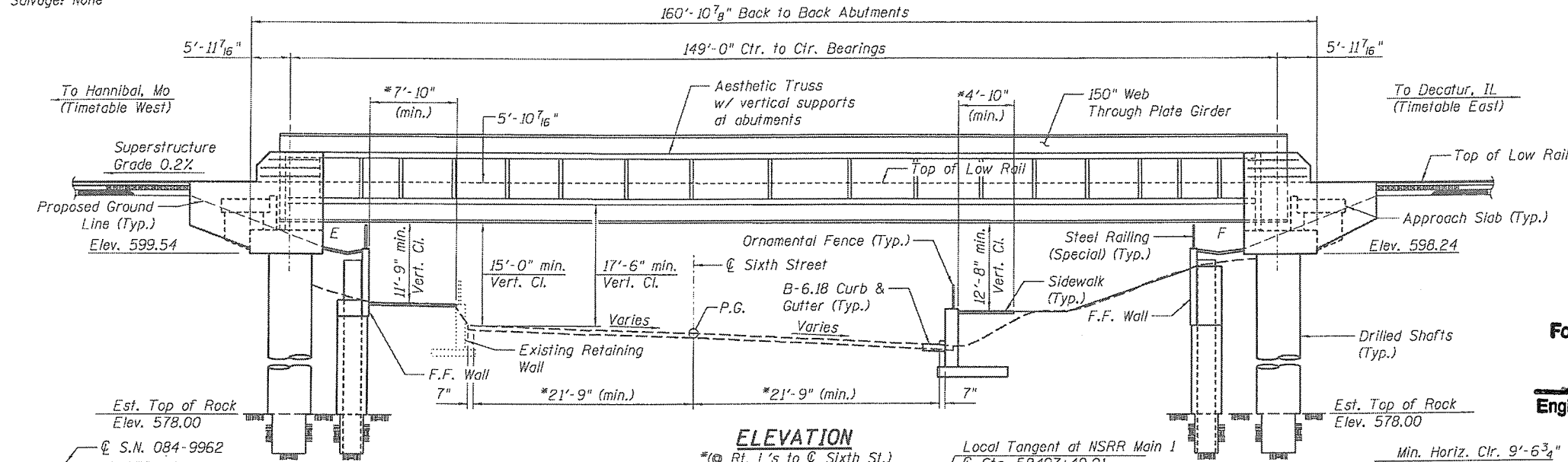
Existing Structure: SN 084-9901 - Built in 1934 under 109-S-NRM. Three Span Steel through plate girder structure supported on closed abutments. Bk. to Bk. Abutment length is 116'-4" and ctr. to ctr. through girder width of 20'-0". Structure to be removed and replaced.

Construction Sequence: For Sequence and Details, See retaining wall General Data sheet.

Traffic Control: Temporary Lane Closures, Weekend and Nighttime Road Closures.

Salvage: None

Railroad utilities may exist within existing NSRR right-of-way. Prior to the start of any construction or excavation, utility relocations will have to be coordinated with the NSRR.



Indicates Boring Location
 Point of Min. Vert. Clr. to Aesthetic Truss 6th St. Sta. 999+67.75, 22.11' Lt. Top of Exist. Pavement El. 589.79 NSRR Sta. 52498+16.69, 31.48' Lt.
 Point of Min. Vert. Clr. to Bridge 6th St. Sta. 998+72.33, 22.08' Lt. Top of Exist. Pavement El. 589.57 NSRR Sta. 52498+13.07, 28.70' Lt.

SEISMIC DATA

AREMA

Ground Motion Level	PGA	S _s	S ₁
Level 1 (100 Year)	0.010	0.025	0.005
Level 2 (475 Year)	0.040	0.090	0.035
Level 3 (2475 Year)	0.10	0.22	0.10

Soil Site Class = C

LOADING COOPER E-80
 Impact: Diesel Impact
 Allow 6" of Future Ballast Dead Load

DESIGN SPECIFICATIONS

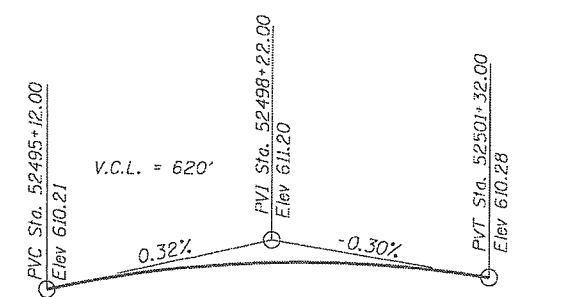
2017 AREMA Specifications
 Live Load Deflection: L/640
 Composite Design for Floorbeam Defl. Req.
 Design Speed: 50 m.p.h.

DESIGN STRESSES

FIELD UNITS
 f'_c = 4,000 psi
 f_y = 60,000 psi (Reinforcement)
 f_y = 50,000 psi (ASTM A709 Grade 50)

CURVE DATA

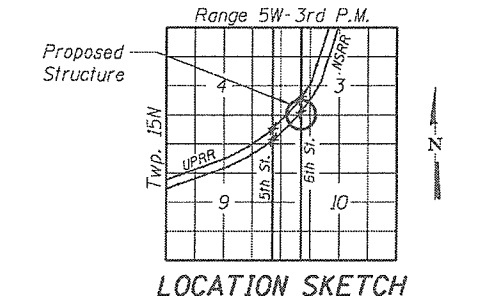
(NSRR Main 1)
 P.I. Sta. 52472+27.28
 Δ = 37°24'51" (Rt.)
 D = 0°41'14"
 T = 2,823.67'
 L = 5,445.19'
 R = 8,338.78'
 E = 465.10'
 Long Chord = 5,348.99'
 Mid. Ord. = 440.53'
 S.E. = 1"
 S.C. Sta. = 52444+03.61
 C.S. Sta. = 52498+48.80



EXISTING PROFILE GRADE SIXTH STREET
 Along @ of Sixth St.

Sta.	Elev.
997+00	595.17
997+50	594.18
998+00	594.00
998+50	592.72
999+00	590.75
999+50	588.37
1000+00	586.05
1000+50	585.14
1001+00	586.13
1001+50	588.33
1002+00	590.77
1002+50	592.51
1003+00	593.88
1003+50	595.11

APPROVED
 For Structural Adequacy Only
 Matthew J. Willey
 Engineer of Bridges & Structures



I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AREMA Specifications.

STATE OF ILLINOIS
 LICENSED STRUCTURAL ENGINEER
 MATTHEW J. WILLEY
 081-006588
 Matthew J. Willey
 SIGNATURE
 4-12-2019
 DATE
 I.C. EXP. DATE: 11-30-2020

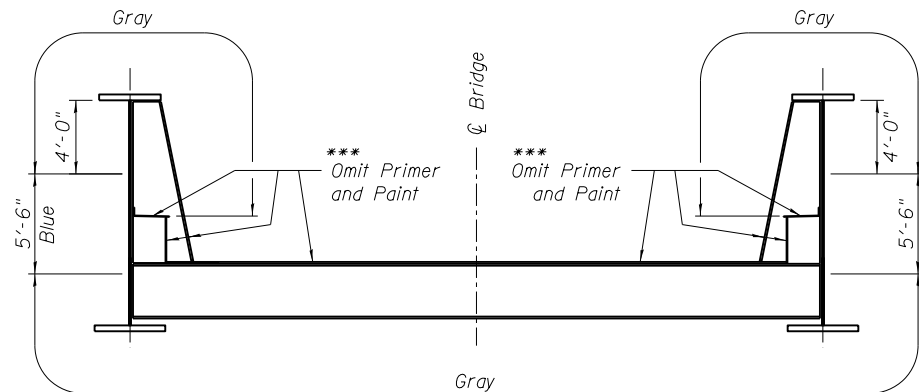
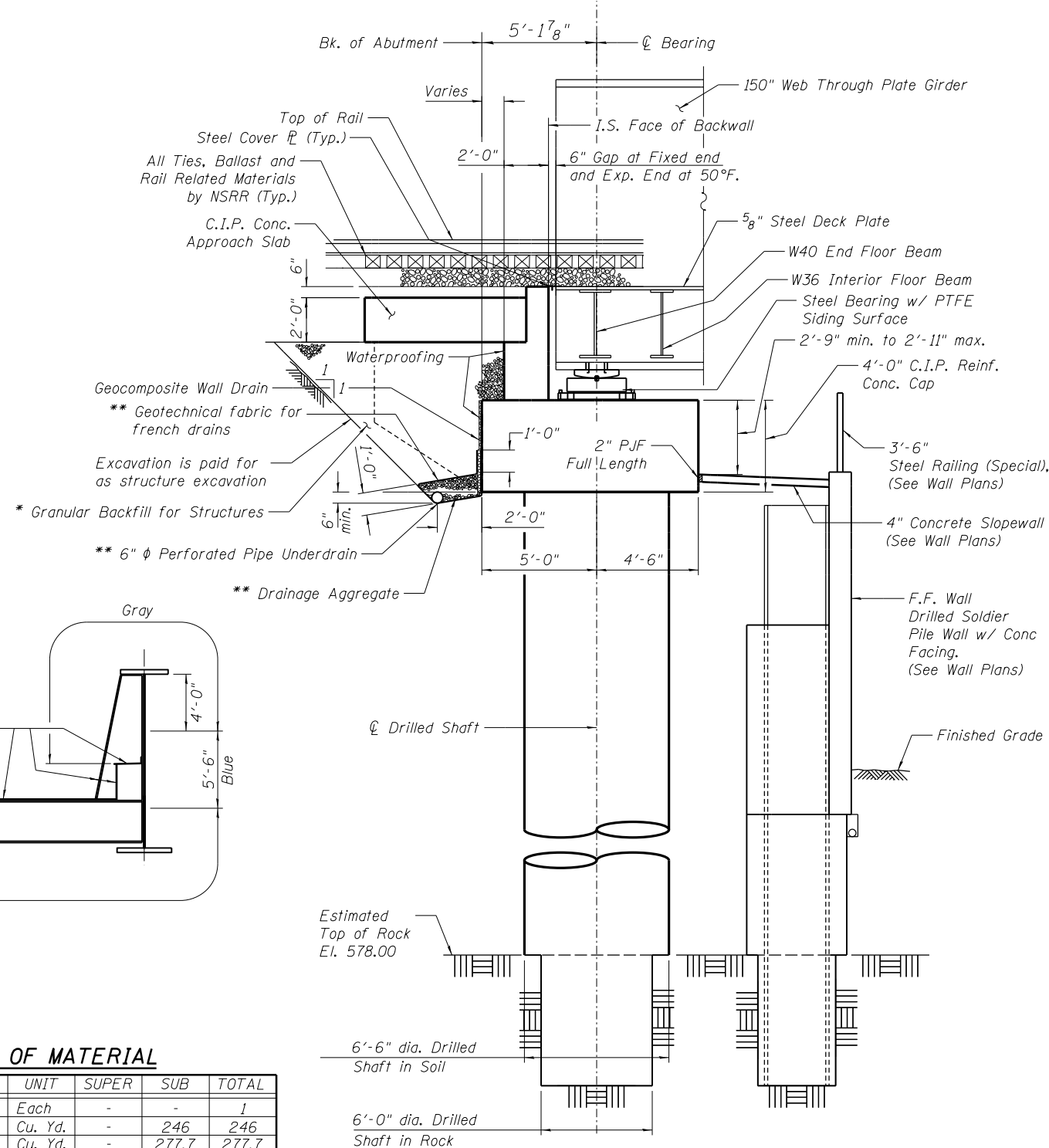
GENERAL PLAN & ELEVATION
NSRR (MP DH-416.16) OVER BUSINESS 55 (6TH ST.)
F.A.P. 666-SECTION (109)VB, (110)VB-5
SANGAMON COUNTY
STATION 52497+49.01
STRUCTURE NO. 084-9963

GENERAL NOTES

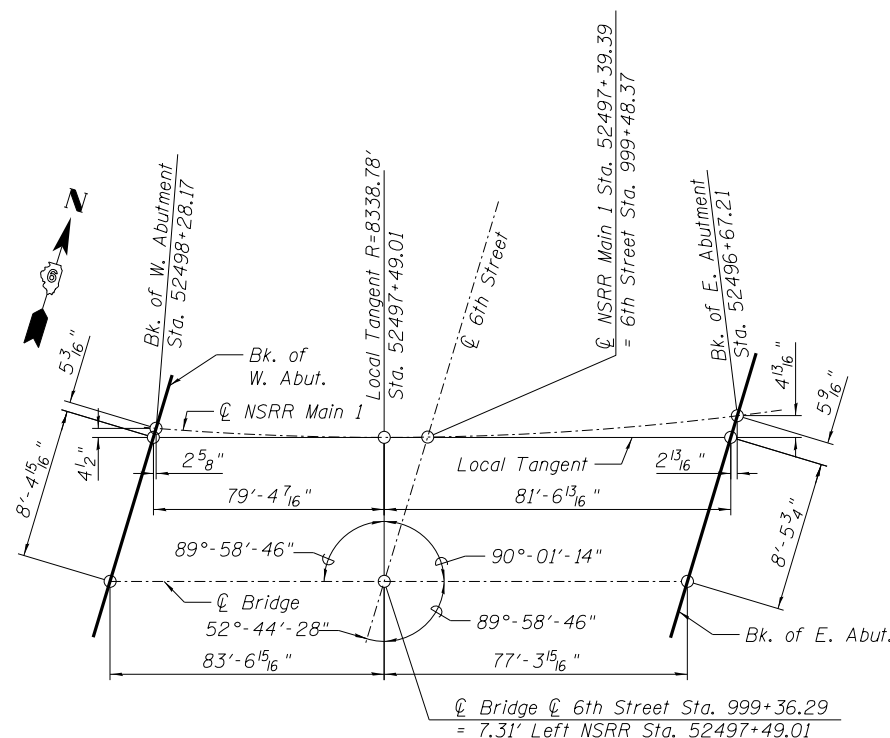
- Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts $\frac{1}{2}$ in. ϕ , holes $\frac{1}{16}$ in. ϕ , unless otherwise noted.
- Calculated weight of Structural Steel, ASTM A709, Gr. 50 = 1,398,349 lbs.
 ASTM A36, Gr. 36 = 14,109 lbs.
 ASTM A500, Gr. 46 = 21,557 lbs.
- All structural steel shall be ASTM A709 Grade 50 unless otherwise noted on the plans.
- All substructure concrete shall have a compressive strength of 4,000 psi at 14 days.
- No field welding is permitted except as specified in the contract documents.
- Reinforcement bars designated (E) shall be epoxy coated.
- Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of $\frac{1}{8}$ inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- Concrete Sealer shall be applied to the following surfaces:
 Abutments - inside face of backwall, inside face of cheekwall and top of cap (except surfaces coated with surface color treatment).
 Concrete Surface Color Treatment shall be applied to the following surfaces:
 Abutments - concrete facing, wingwall and cheekwall surfaces coated with concrete surface color treatment.
- The Inorganic Zinc Rich Primer / Acrylic / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. All coatings on faying surfaces shall satisfy RCSC requirements for Class B slip coefficient. The color of the final finish coat for girder flanges, all interior steel surfaces, bottom of deck plate, and aesthetic truss shall be gray, Munsell No. 5B 7/1. The color of the final finish coat for a 5.5 foot tall strip on the exterior face of girder web starting 4 foot down from the top flange shall be blue, Munsell No. 10B 3/6. See painting diagram for more information.
- Waterproofing shall be applied to the backside of the abutment cap and backwall and backside of wingwalls for surfaces below ground. This shall be according to Article 503.18 of the Std. Spec. Cost included with Concrete Structures.
- The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

INDEX OF SHEETS

- General Plan & Elevation
- General Data
- Foundation Layout
- Structural Removal
- Typical Section
- Framing Plan
- Outside Elevation of Girder (1 of 2)
- Outside Elevation of Girder (2 of 2)
- Inside Elevation of Girder (1 of 2)
- Inside Elevation of Girder (2 of 2)
- Typical Sections
- Girder Sections & Details
- Girder Splice Details
- Closure Plate and Ballast Plate Plan
- Closure Plate and Ballast Plate Details
- Miscellaneous Girder Details (1 of 3)
- Miscellaneous Girder Details (2 of 3)
- Miscellaneous Girder Details (3 of 3)
- Aesthetic Truss
- TPG Bearing Details
- End Floorbeam Bearing Details
- Bridge Deck Waterproofing
- West Abutment
- West Abutment Details
- West Abutment Bill of Material
- East Abutment
- East Abutment Details
- East Abutment Bill of Material
- Subsurface Data Profile



PAINTING DIAGRAM



OFFSET SKETCH

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Removal of Existing Structures No. 4	Each	-	-	1
Structure Excavation	Cu. Yd.	-	246	246
Concrete Structures	Cu. Yd.	-	277.7	277.7
Reinforcement Bars	Pound	-	206,790	206,790
Reinforcement Bars, Epoxy Coated	Pound	-	44,530	44,530
Name Plates	Each	-	1	1
Drilled Shaft in Soil	Cu. Yd.	-	256.8	256.8
Drilled Shaft in Rock	Cu. Yd.	-	162.4	162.4
Membrane Waterproofing (Special)	Sq. Ft.	5,906	-	5,906
Concrete Sealer	Sq. Ft.	-	1,515	1,515
Geocomposite Wall Drain	Sq. Yd.	-	52	52
Drainage System, No. 4	Each	1	-	1
Concrete Surface Color Treatment	Sq. Ft.	-	12	12
Granular Backfill for Structures	Cu. Yd.	-	182	182
Furnishing and Erecting Structural Steel, Bridge No. 4	L. Sum	1	-	1
Pipe Underdrains for Structures, 6"	Foot	-	161	161

ABUTMENT SECTION
(At Rt. L's to Back of Abutment)

Notes:
West Abutment Section is Shown, East Similar with the Exception of a 5'-0" Deep Abutment Cap.

* Granular Backfill for Structures Shall Be Placed and Compacted According to Section 502.10 of the Standard Specifications.

** Included in the Cost of "Pipe Underdrains for Structures, 6". For Additional Drainage Details See Railway Plans.

*** Omit Primer and Paint only on portion of Structural Steel to receive Membrane Waterproofing

NORFOLK SOUTHERN RAILWAY
S.N. 084-9963 BUILT 20__ BY
CITY OF SPRINGFIELD
SEC. (109)VB, (110)VB-5
STATION 52497+49.01
MILE POST DH-416.16
LOADING COOPER E-80

NAME PLATE
See Std. 515001

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL DATA
STRUCTURE 084-9963 - 6TH ST NSRR

SHEET NO. 2 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO.	93733
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

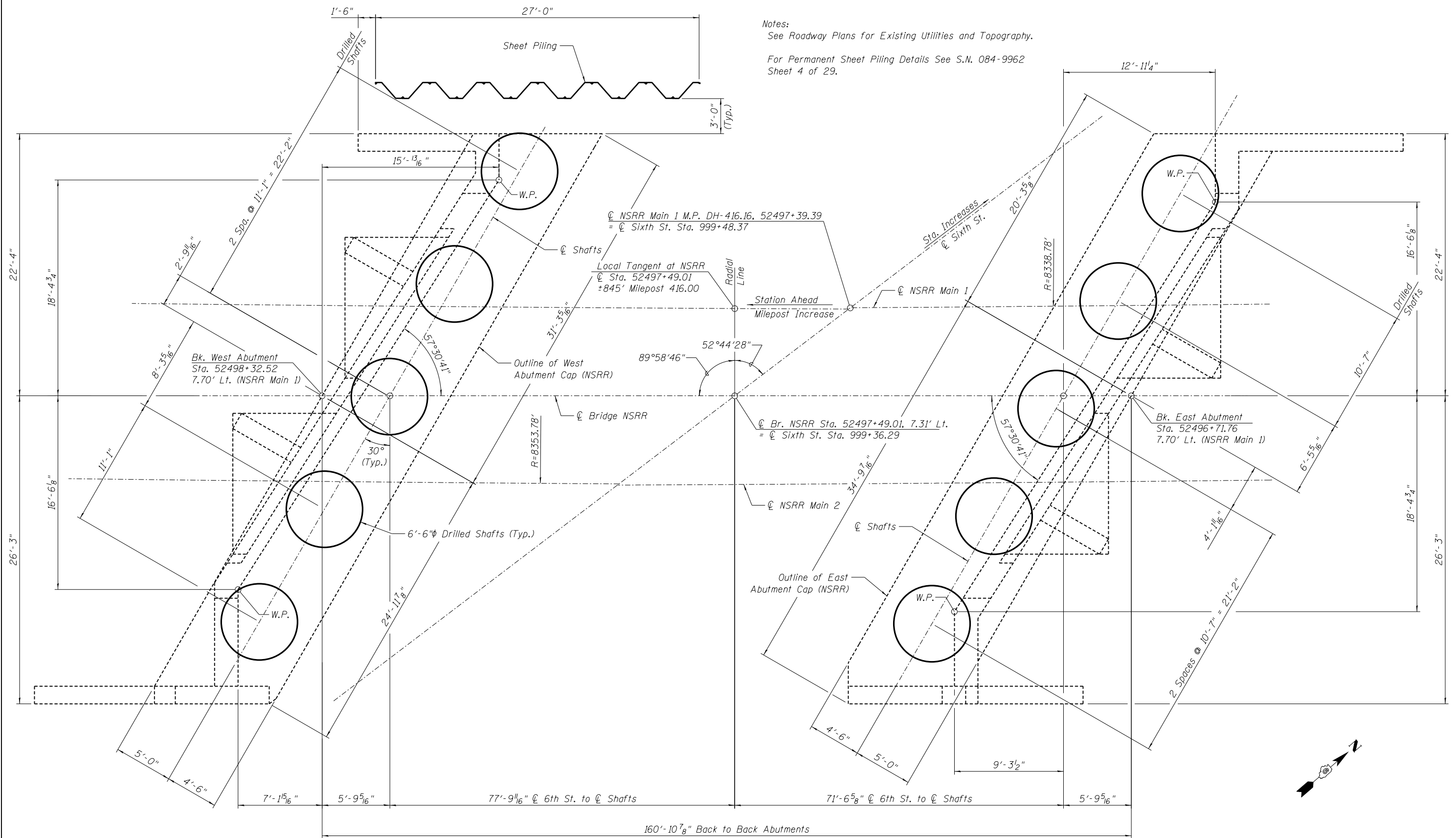
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Notes:
See Roadway Plans for Existing Utilities and Topography.

For Permanent Sheet Piling Details See S.N. 084-9962
Sheet 4 of 29.



FOUNDATION LAYOUT PLAN

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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FOUNDATION LAYOUT
STRUCTURE 084-9963 - 6TH ST NSRR**

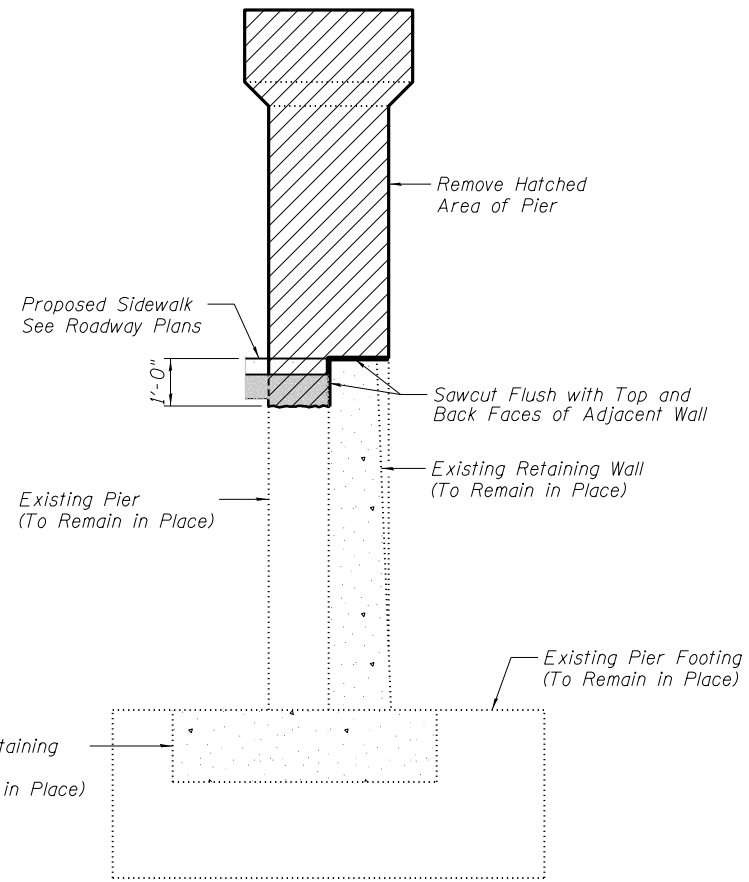
SHEET NO. 3 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 93733	
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

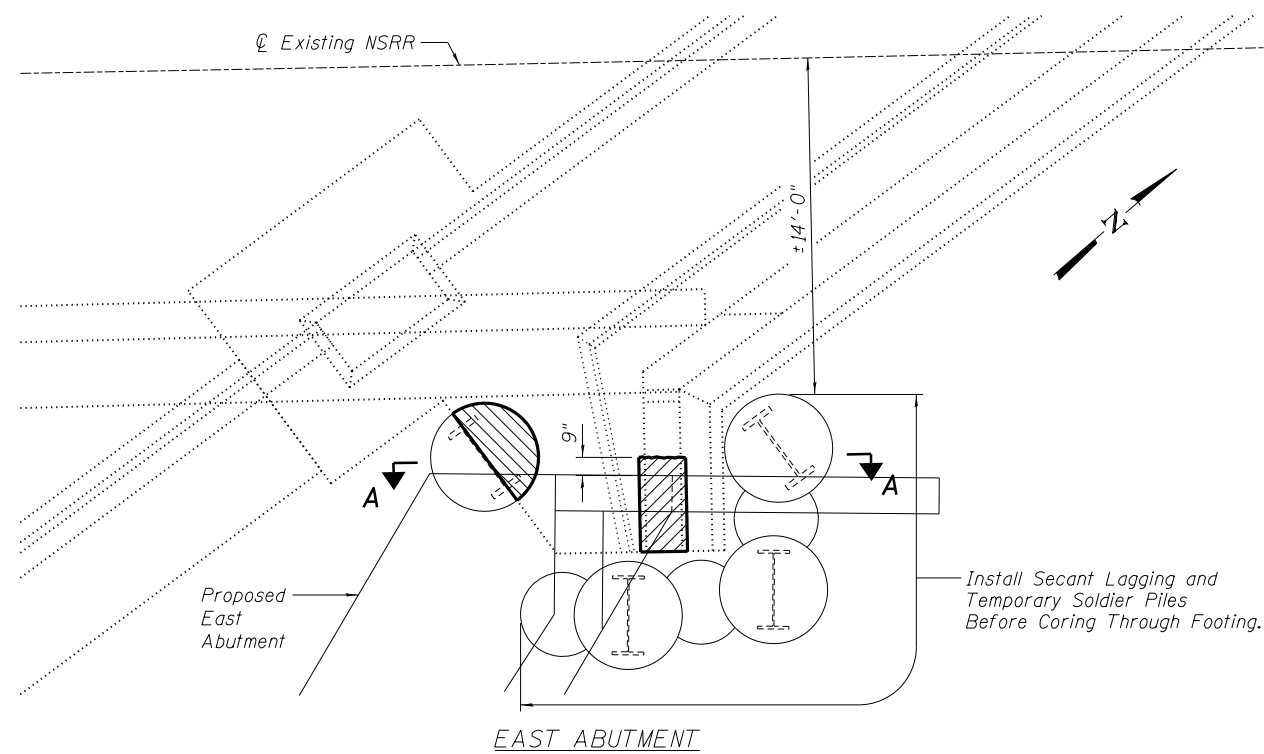
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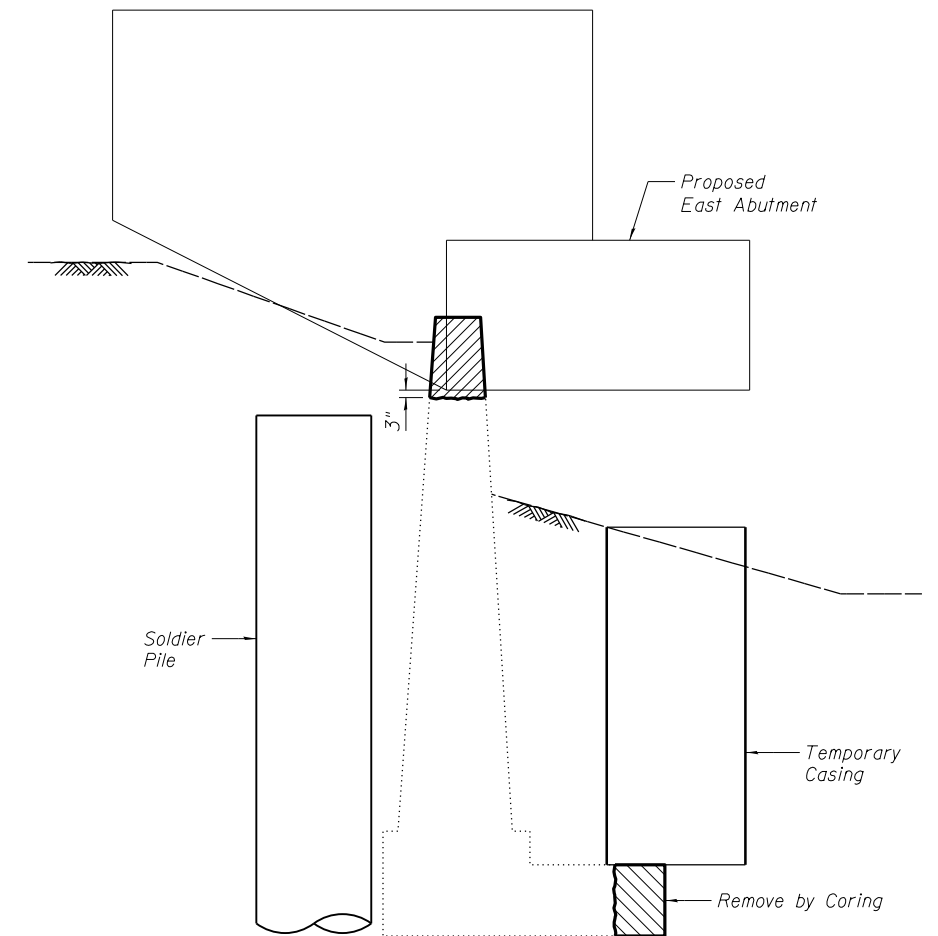
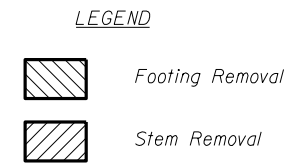
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EXISTING STRUCTURE REMOVAL AT PIER
(Typical of 4 Locations)



STAGE 1 REMOVAL OF EXISTING STRUCTURES



SECTION A-A

Existing Structure Removal Notes:

See Retaining Wall Plans for construction staging notes and details of temporary and permanent soldier piles, and secant lagging.

During Stage 1, removal shall be limited to the areas shown. Coring of footing shall be no more than 3 inches beyond the neat perimeter of the soldier pile excavation.

The portion of existing wingwall stem that conflicts with the proposed abutment shall be removed to the limits shown in accordance with Article 501.05 of the Standard Specifications.

Removal of the remainder of existing structure shall be completed after active track has been shifted to new bridge.

Cost of staged removal shall be included with Removal of Existing Structures No. 4.

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**STRUCTURAL REMOVAL
STRUCTURE 084-9963 - 6TH ST NSRR**

SHEET NO. 4 OF 29 SHEETS

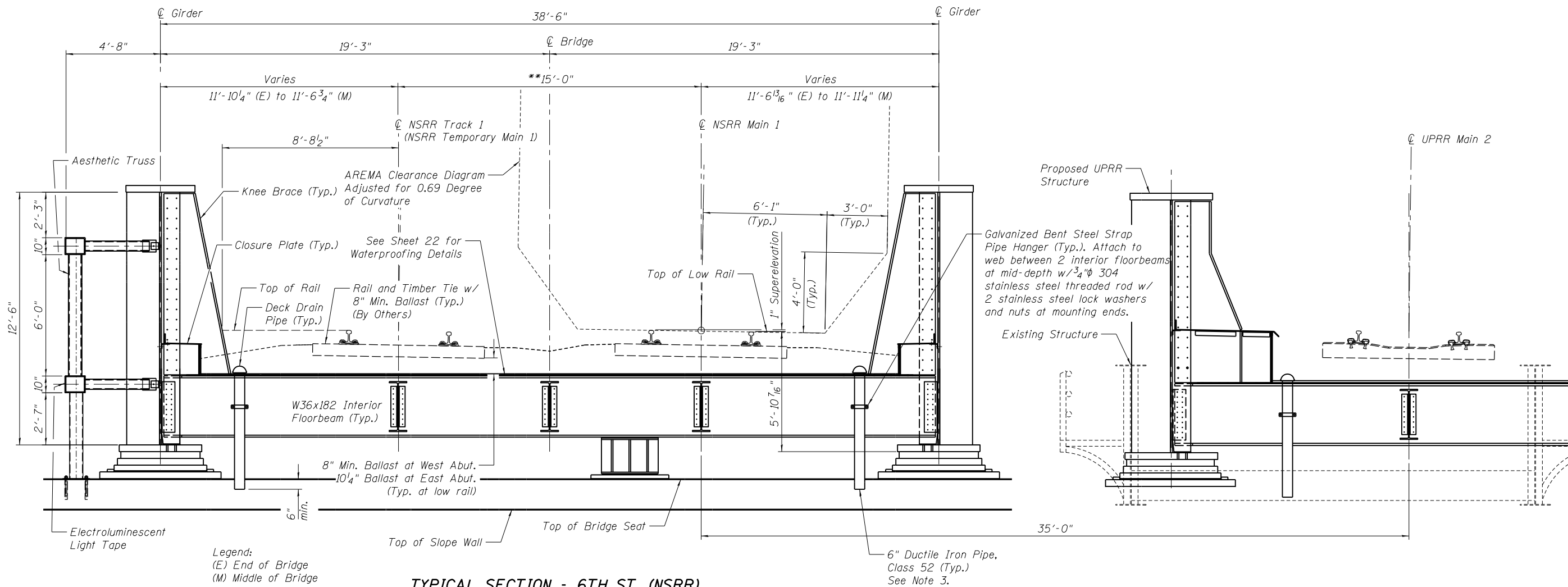
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*	(109) VB,(110) VB-5	SANGAMON	382	266
			CONTRACT NO. 93733	

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TYPICAL SECTION - 6TH ST. (NSRR)
 (Looking West)
 **Dimensions are at Rt L's to \varnothing Track.

Legend:
 (E) End of Bridge
 (M) Middle of Bridge

- Notes:
1. Retaining Wall and Steel Railing not shown for clarity.
 2. Drain pipe on east end only near low end of bridge deck.
 3. With the ductile iron pipe fitted to the bottom of the deck drain bottom pan downspout, drill 4 holes through ductile iron pipe and downspout. Holes shall be aligned with the 4 quadrants of the pipe. Attach ductile iron pipe to downspout with 4 stainless steel carriage bolts. Rounded heads of carriage bolts shall be oriented towards the center of the pipe.
 4. Cost of deck drain pipe, bottom pan, downspout, brackets and other hardware shall be included in the cost of Drainage System.

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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TYPICAL SECTION
 STRUCTURE 084-9963 - 6TH ST NSRR**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 93733	

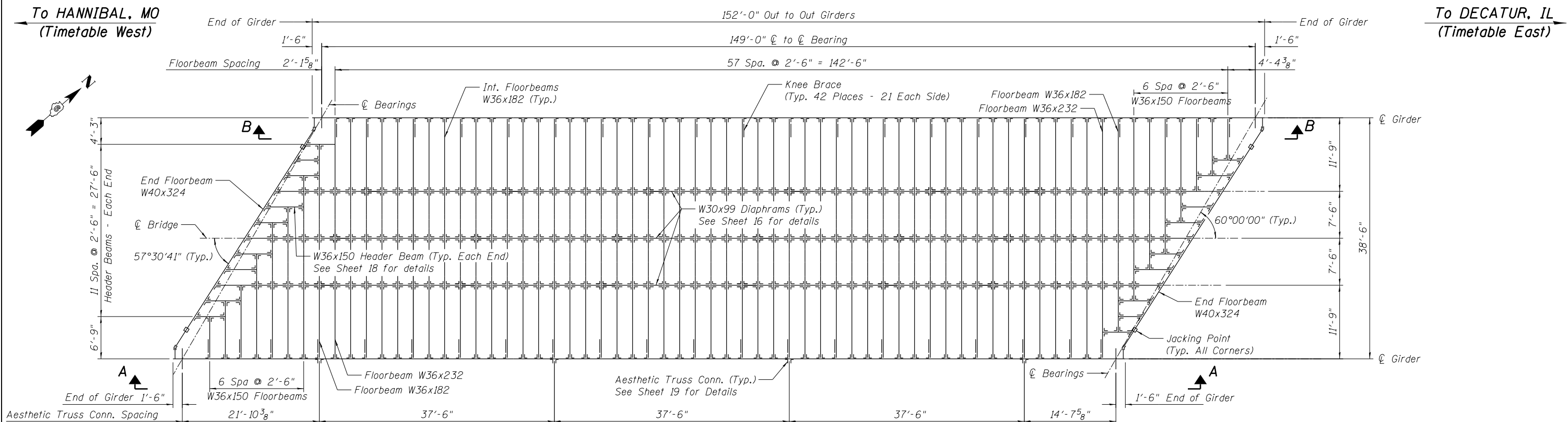
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FRAMING PLAN

See Sheet 7 & 8 for View A-A
See Sheet 9 & 10 for Section B-B

STEEL NOTES

GENERAL: All materials, fabrication, and erection shall be in accordance with chapter 15 of the current AREMA Manual for Railway Engineering.

Dead Load: (assumed)	
Rail	400
Ballast (Incl. Tie)	4,760
Waterproofing	200
Future Ballast	2,590
Steel	9,450
Total	17,400 lbs. per lin ft. of track

MATERIAL: Zone 2 Conditions control for Charpy V-Notch testing.

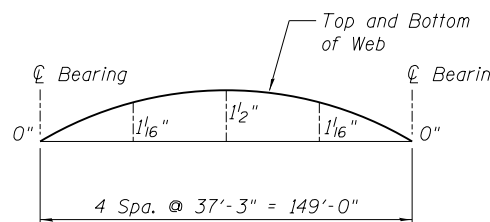
Fracture Critical Members (FCM) shall be Charpy V-Notch tested, according to AREMA Table 15-9-3, Zone 2, P frequency in accordance with ASTM A673.

Impact Test Required (ITR) members shall be Charpy V-Notch (CVN) tested, according to AREMA Table 15-9-2, Zone 2, H frequency in accordance with ASTM A673.

FABRICATION: The top surface of beams shall be adjusted to form a straight line at any transverse section throughout the span. Tolerance is plus or minus 1/8".

SPLICE NOTES:

- No two parts or members shall be spliced by shop welding at the same location, or within the length of a bolted field splice.
- Web splices by shop welding shall be located a minimum of 36" away from any flange splice.
- Splices of the web or flanges shall not be permitted within the central 30'-0" of the girder span length. This requirement may be waived only by the approval of the Engineer.



CAMBER DIAGRAM
Camber Calculated for Dead Load Only

MOMENT & SHEAR TABLE FOR STEEL THRU PLATE GIRDER

DESCRIPTION	MOMENT	SHEAR
Dead Load	24,144 ft. -k	648 k
Live Load	28,965 ft. -k	854 k
Impact	6,839 ft. -k	202 k
Total	59,948 ft. -k	1,704 k
Section	See Sheet 12 of 29	
Steel	A.S.T.M. A709, Gr. 50	
Net I	2,200,778 in ⁴	
Net S (Bot.)	27,222 in ³	
fst (Bot.)	26.5 ksi	
Gross I	2,397,326 in ⁴	
Gross S (Top)	29,041 in ³	
fsc (Top.)	24.8 ksi	

I- Moment of Inertia of the Section
S- Section Modulus
fs- Max. Unfactored Stress in the Section Due to D.L. + L.L. + Impact

MOMENT & SHEAR TABLE FOR STEEL FLOORBEAMS

DESCRIPTION	MOMENT	SHEAR	MOMENT *	SHEAR *
Dead Load	163 ft. -k	16.1 k	3,565 ft. -k	648 k
Live Load	240 ft. -k	20.5 k		
Impact	712 ft. -k	60.8 k		
Total	1,115 ft. -k	97.4 k	3,565 ft. -k	648 k
Section	W36x182		W40x324	
Steel	A.S.T.M. A709, Gr. 50		A.S.T.M. A709, Gr. 50	
Net I	11,026 in ⁴		22,636 in ⁴	
Net S	607 in ³		1,126 in ³	
fs	22.0 ksi		38.0 ksi	

* Jacking Conditions Control 50% Allowable Stress Increase is Permitted

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN
STRUCTURE 084-9963 - 6TH ST NSRR

SHEET NO. 6 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO.	93733
*666 & 666 ALT.		ILLINOIS FED. AID PROJECT		

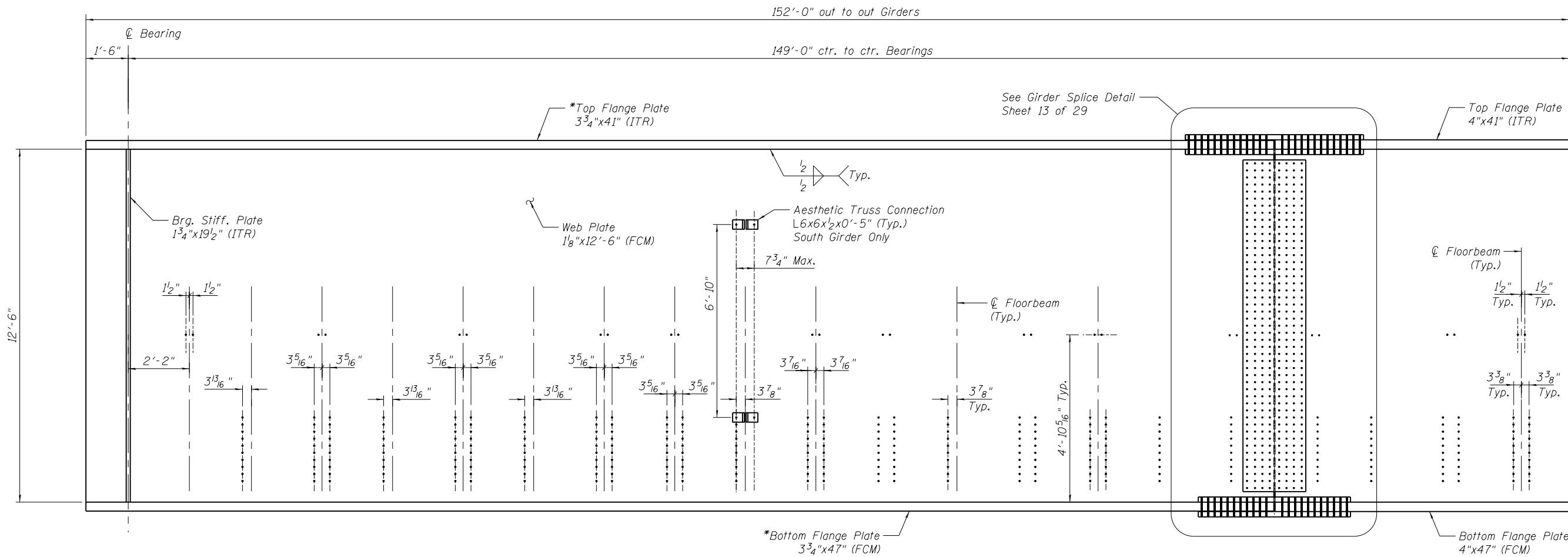
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To HANNIBAL, MO
(Timetable West)

To DECATUR, IL
(Timetable East)



VIEW A-A - OUTSIDE ELEVATION OF GIRDER

Note:
1. FCM - Fracture Critical Member
2. ITR - Impact Test Required

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

OUTSIDE ELEVATION OF GIRDER - SHEET 1 OF 2
STRUCTURE 084-9963 - 6TH ST NSRR

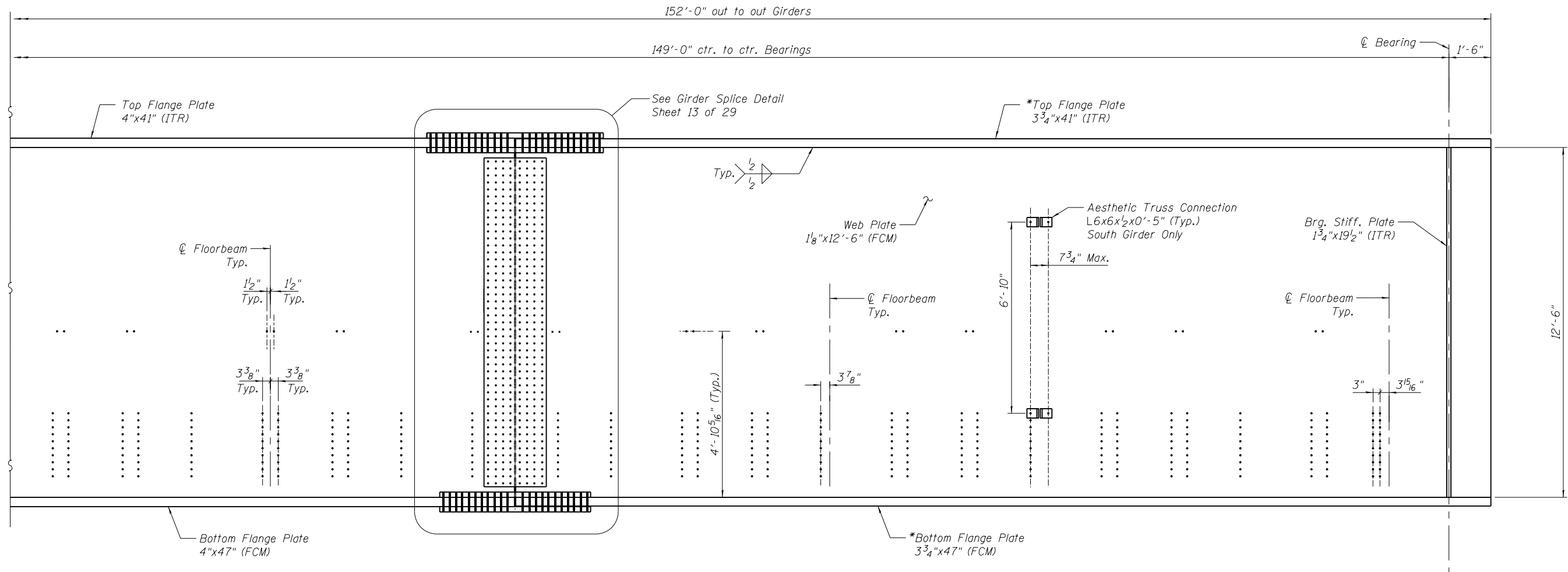
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CONTRACT NO.			93733	
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

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To HANNIBAL, MO
(Timetable West)

To DECATUR, IL
(Timetable East)



VIEW A-A - OUTSIDE ELEVATION OF GIRDER

Note:
1. FCM - Fracture Critical Member
2. ITR - Impact Test Required

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

OUTSIDE ELEVATION OF GIRDER - SHEET 2 OF 2
STRUCTURE 084-9963 - 6TH ST NSRR

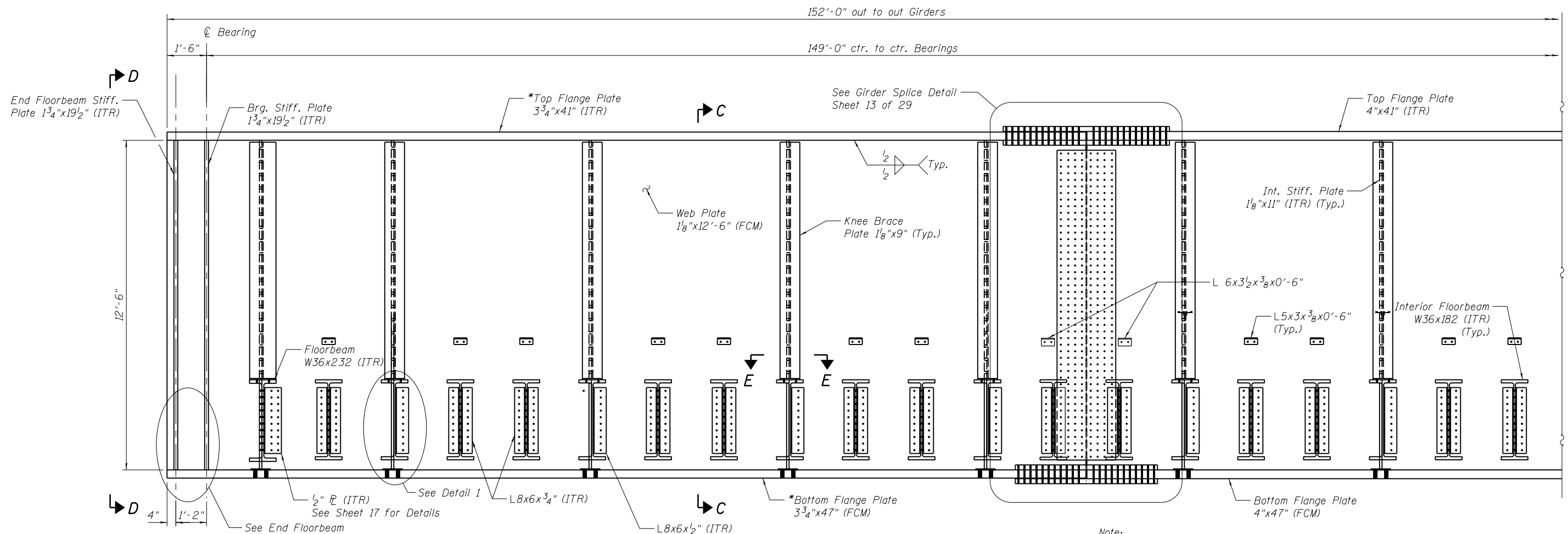
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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 93733	
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

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To HANNIBAL, MO
(Timetable West)

To DECATUR, IL
(Timetable East)

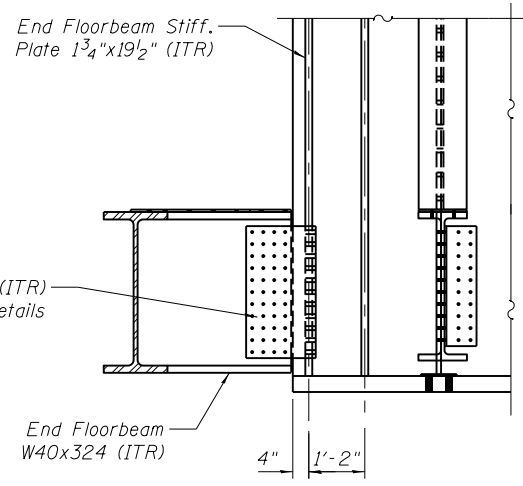


SECTION B-B - INSIDE ELEVATION OF GIRDER

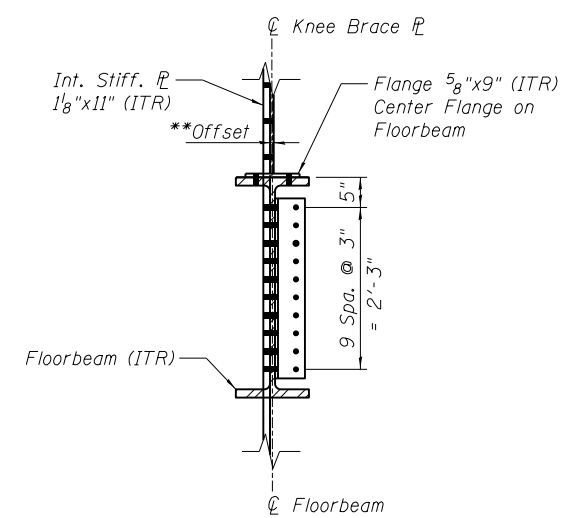
See Sheet 11 of 29 for Section C-C & D-D.

- Note:
1. FCM - Fracture Critical Member
2. ITR - Impact Test Required

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

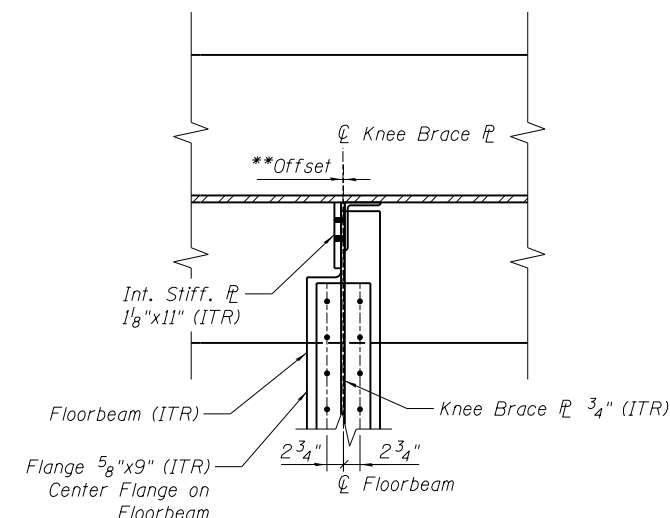


END FLOORBEAM CONNECTION



DETAIL 1

**See Table for Offset Dimension



SECTION E-E

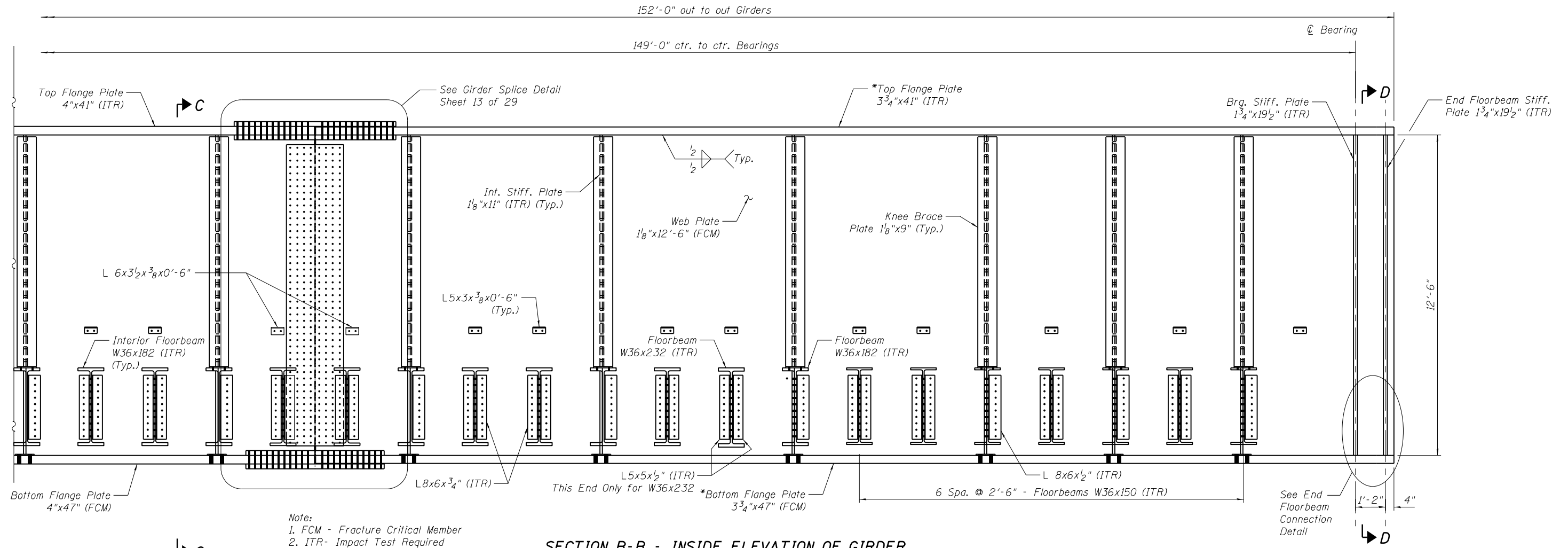
**See Table for Offset Dimension

KNEE BRACE PLATE OFFSETS

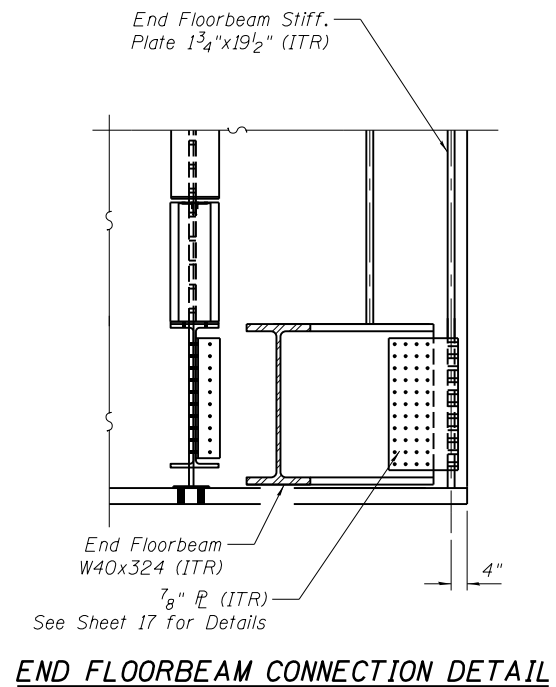
FLOORBEAM SHAPE	OFFSET
W36x150	-1/16"
W36x182	0"
W36x232	1/16"

To HANNIBAL, MO
(Timetable West)

To DECATUR, IL
(Timetable East)



SECTION B-B - INSIDE ELEVATION OF GIRDER
See Sheet 11 of 29 for Section C-C & D-D.



END FLOORBEAM CONNECTION DETAIL

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**INSIDE ELEVATION OF GIRDER - SHEET 2 OF 2
STRUCTURE 084-9963 - 6TH ST NSRR**

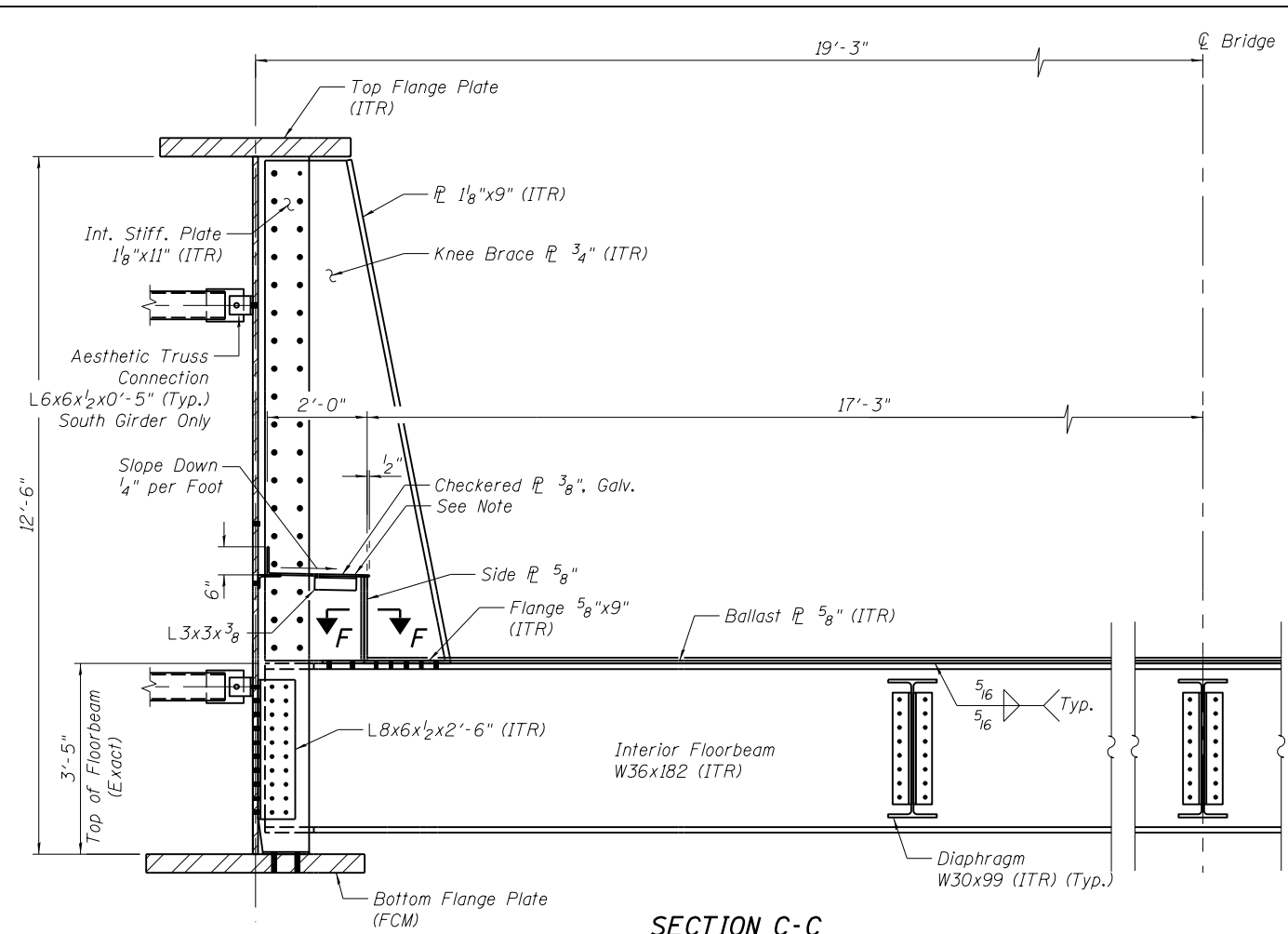
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CONTRACT NO. 93733				
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

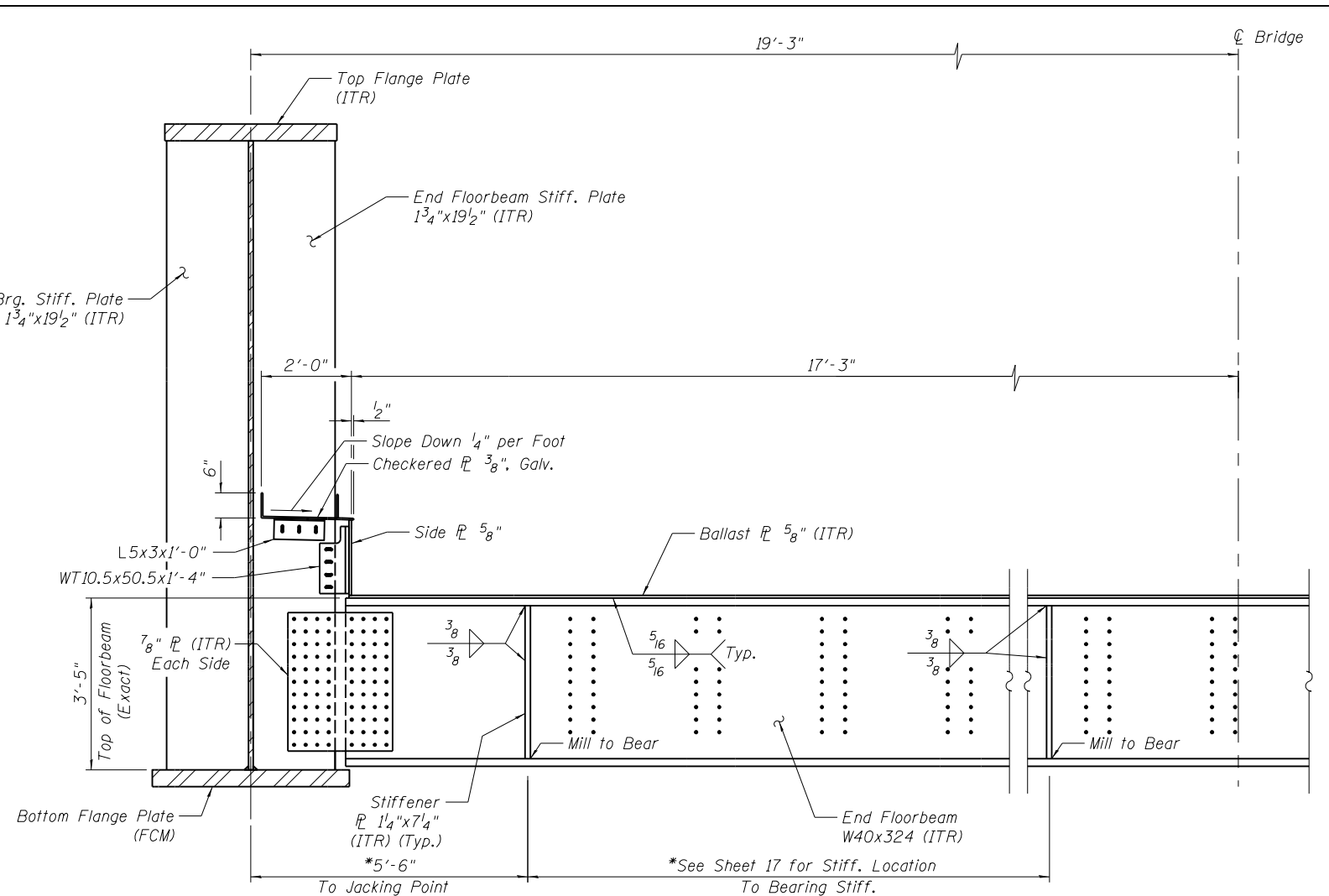
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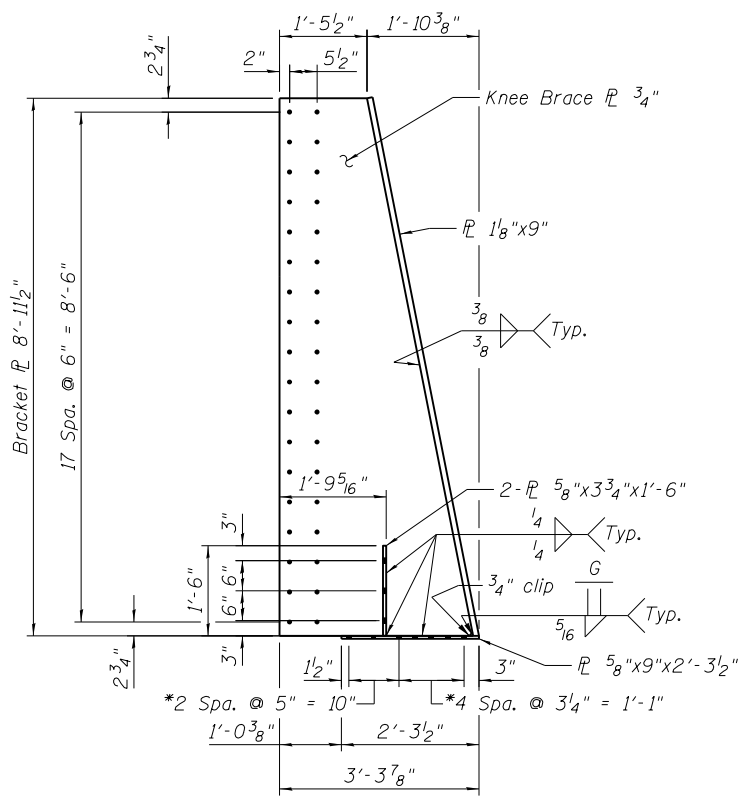
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SECTION C-C



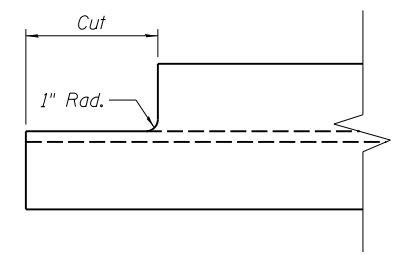
SECTION D-D
*(Along End Floorbeam C)



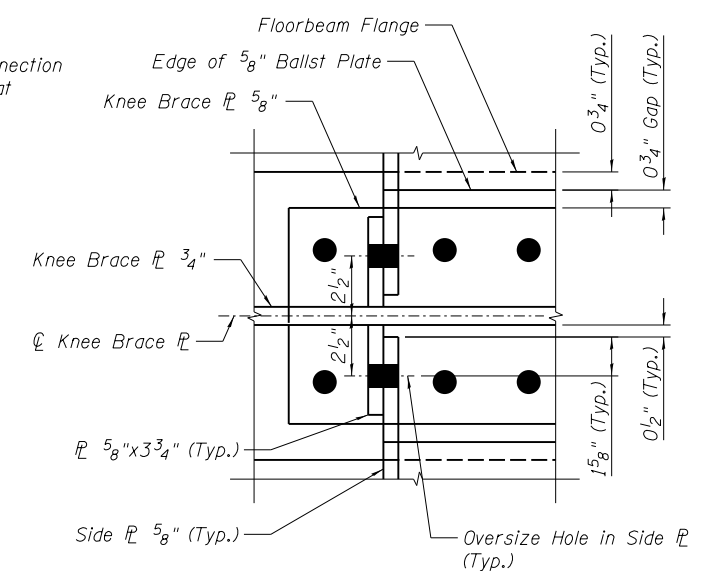
KNEE BRACE

*See Detail 1 on Sheet 9 for Hole Locations.

Note:
For Location of L3x3x3/8, Welding and Bolted Connection to Closure Plate See Plan View of Closure Plate at Knee Brace on Sheet 15.



FLOORBEAM COPE AT INTERIOR STIFFENER



SECTION F-F

Weld Ballast Plate to Floorbeam as shown in Section I-I on Sheet 15

FILE NAME: ... PROJECTS\DOCUMENTS\09\JOBS\09L0179B\CAD\STRUCT\6th\Sheet\0849963-09L0179B-NSRR-001

USER NAME = Pop00275	DESIGNED - MJW	REVISED -
PLOT SCALE = 0:2.0000 ' = 1" IN.	CHECKED - TJH/TDP	REVISED -
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TYPICAL SECTIONS
STRUCTURE 084-9963 - 6TH ST NSRR**

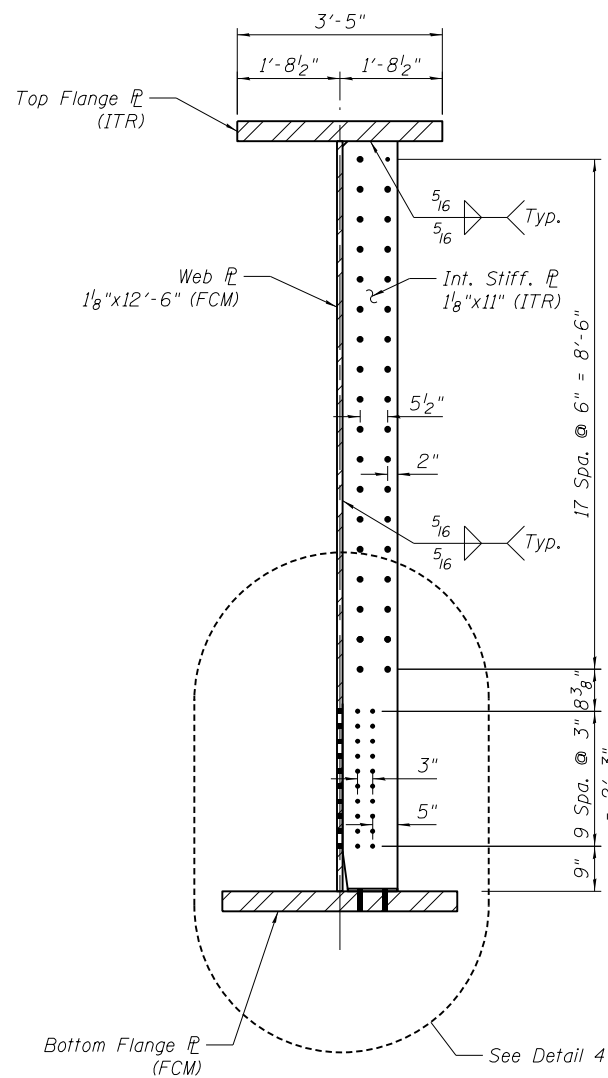
SHEET NO. 11 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	273
			CONTRACT NO. 93733	
•666 & 666 ALT.		ILLINOIS FED. AID PROJECT		

FINAL

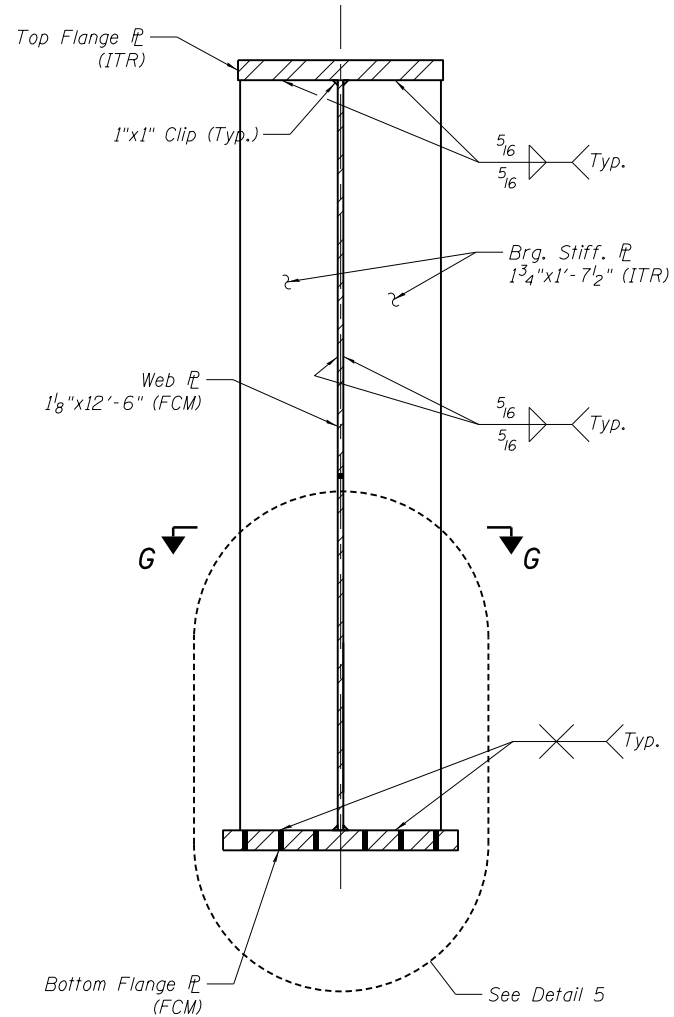


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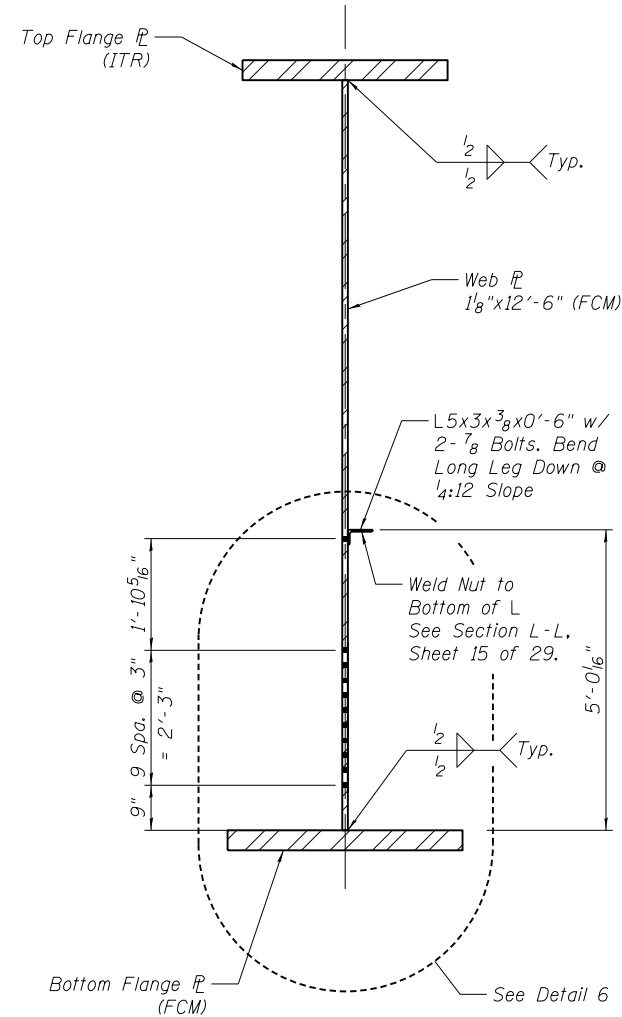


TYPICAL SECTION AT INT. STIFFENER AND KNEE BRACE

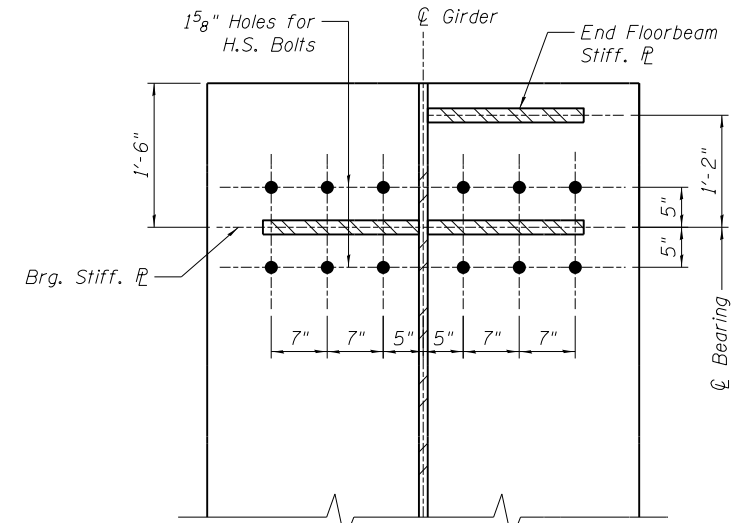
(Knee Brace Omitted for Clarity)



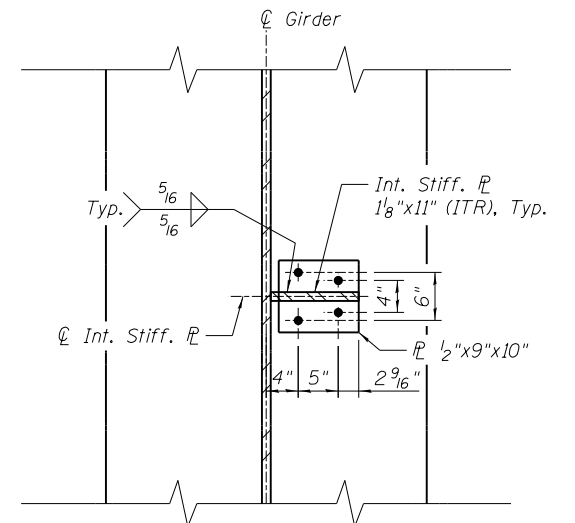
TYPICAL SECTION AT BEARING STIFFENER



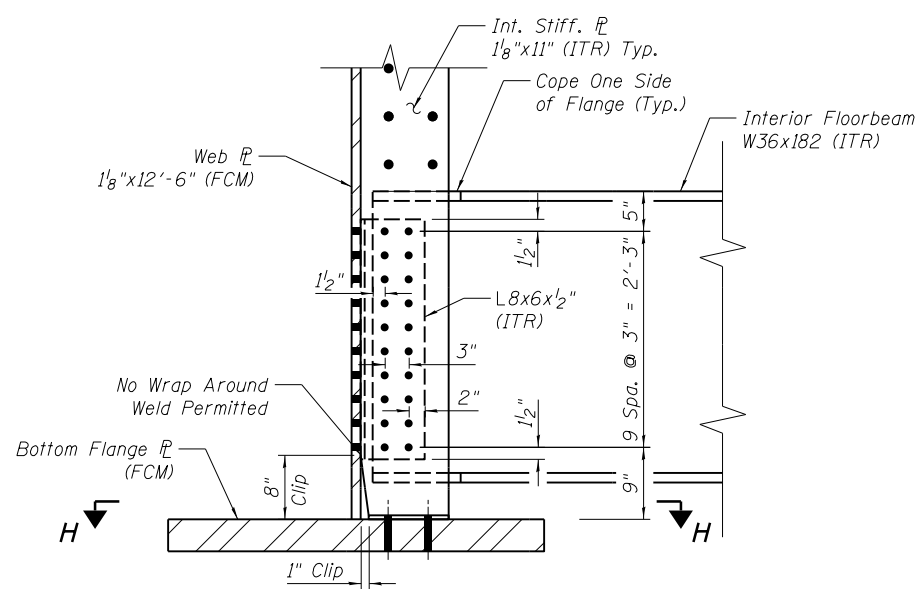
TYPICAL SECTION AT CHECKERED PLATE SUPPORT



SECTION G-G

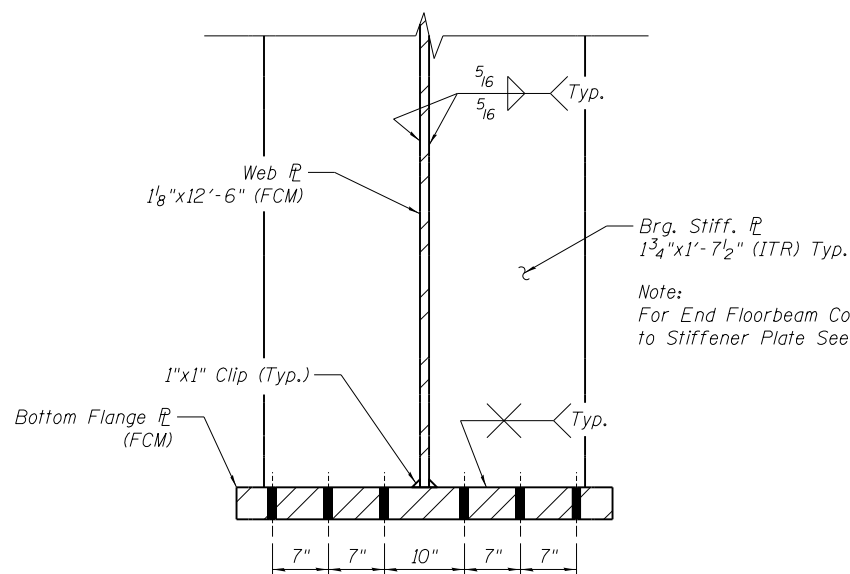


SECTION H-H



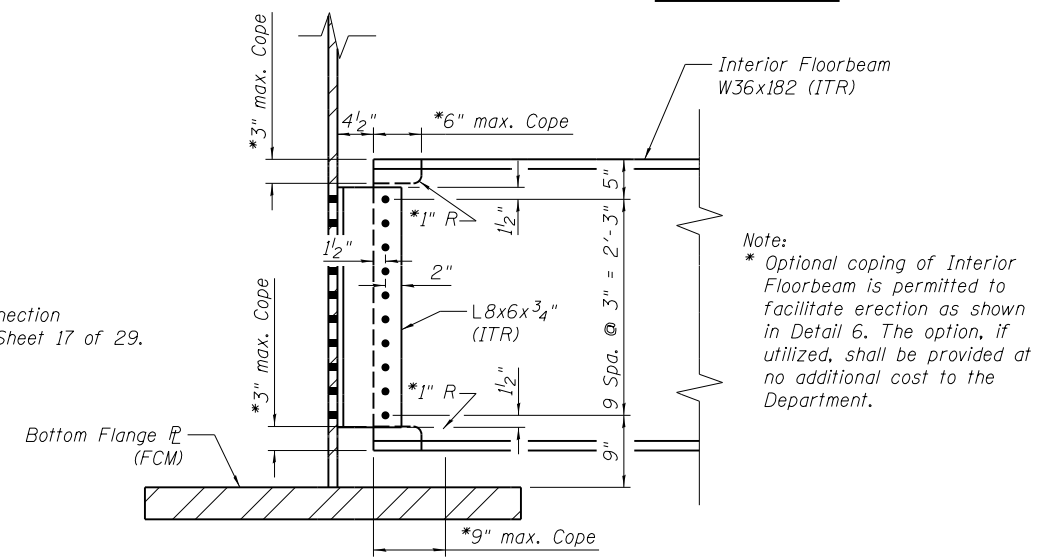
DETAIL 4

Typical for Interior Stiffener Unless Otherwise Noted
(Knee Brace Omitted for Clarity)



DETAIL 5

Typical at Bearing Stiffener



DETAIL 6

Typical Floorbeam Connection Between Interior Stiffeners

FILE NAME = ... PROJECTS\DOCUMENTS\09\JOBS\09L0179B\CAD\STRUCT\6TH SHEET\0849963-09L0179B-NSRR-001

USER NAME = Pop00275	DESIGNED - MJW	REVISD -
PLOT SCALE = 0:2.0000 '1' / in.	CHECKED - TJH/TDP	REVISD -
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISD -
	CHECKED - MJW	REVISD -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**GIRDER SECTIONS & DETAILS
STRUCTURE 084-9963 - 6TH ST NSRR**

SHEET NO. 12 OF 29 SHEETS

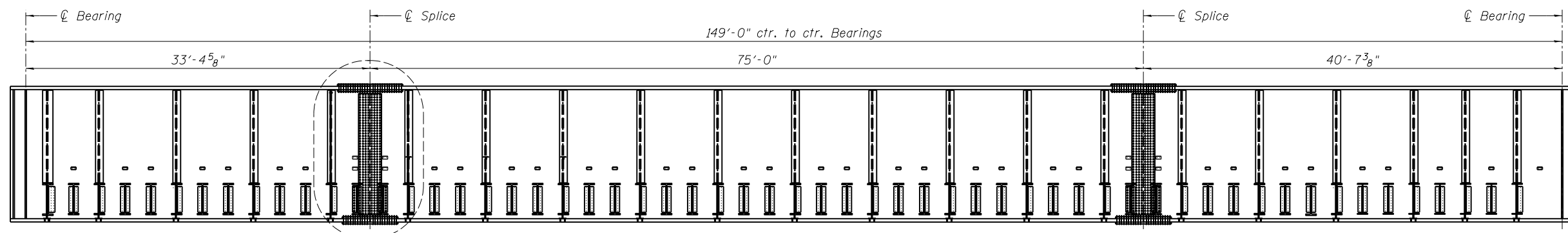
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	274
			CONTRACT NO. 93733	

•666 & 666 ALT. ILLINOIS FED. AID PROJECT

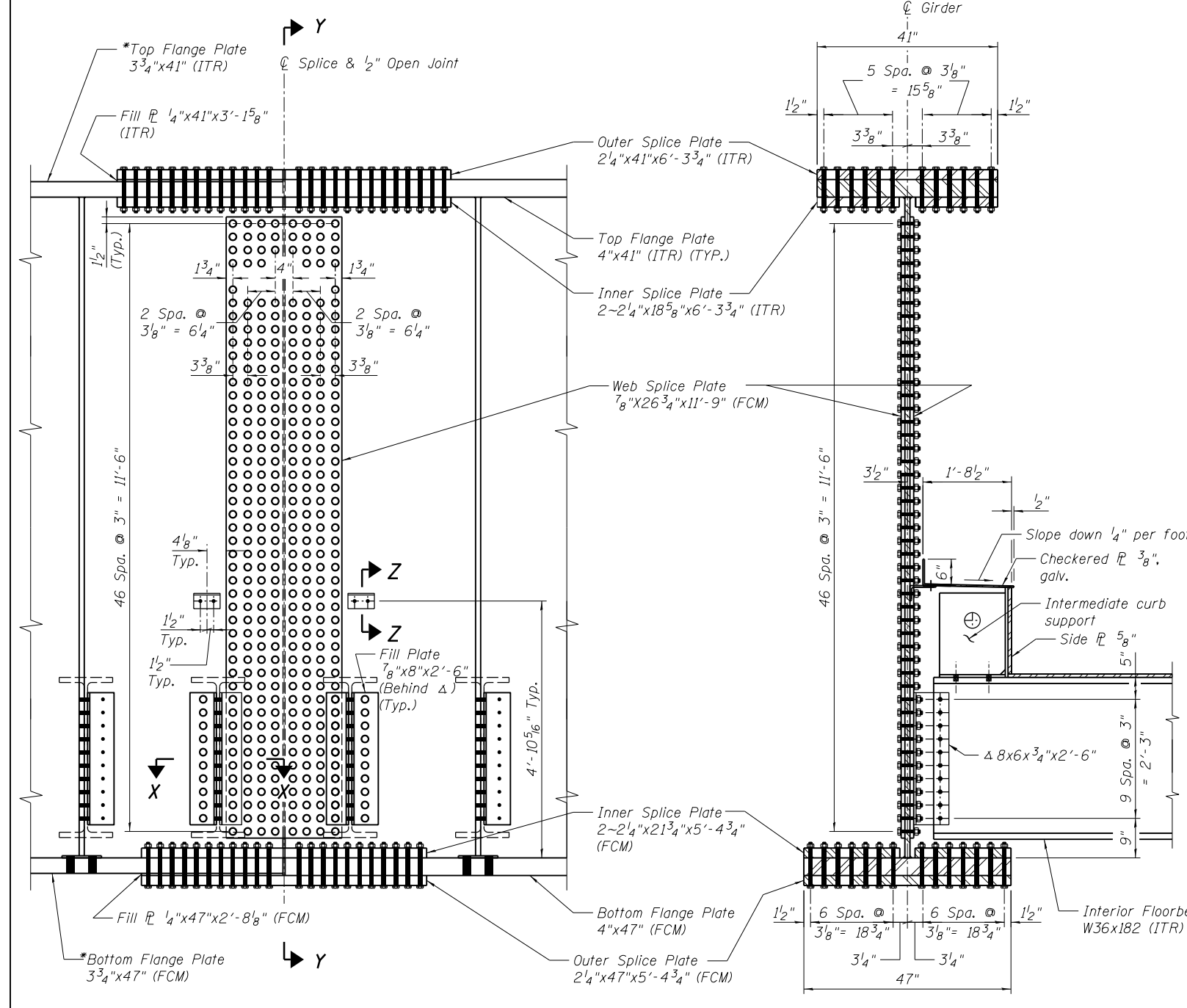
FINAL



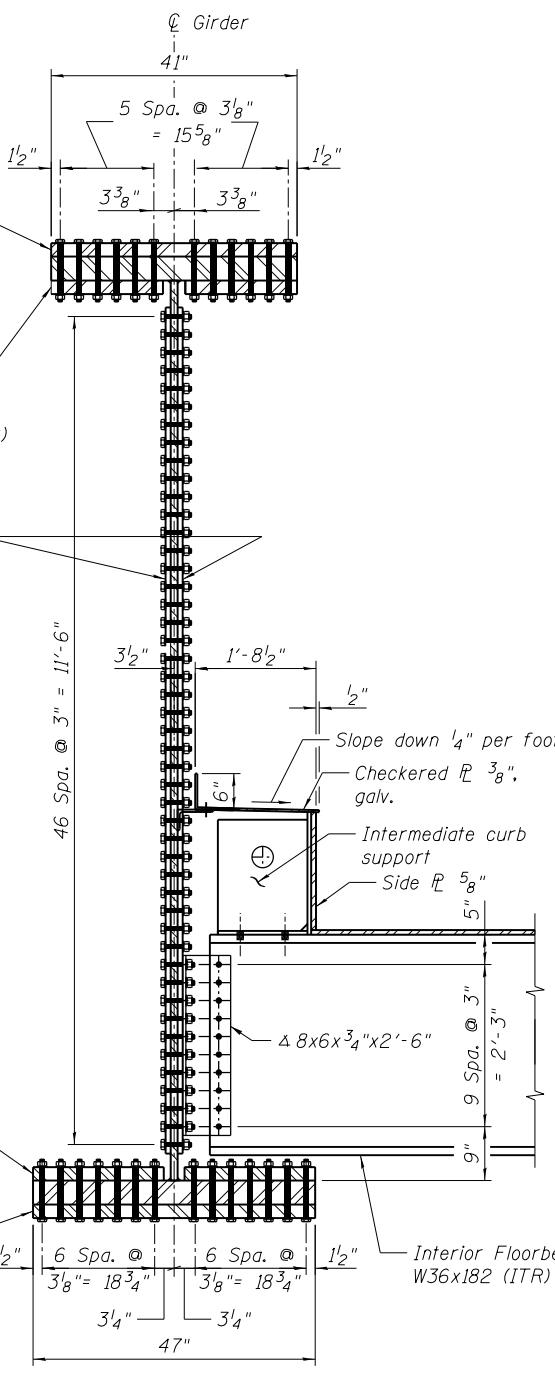
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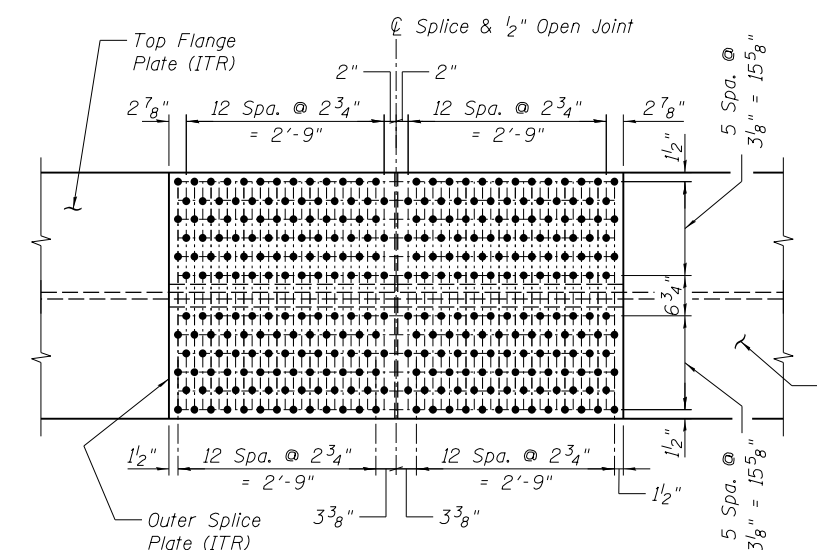
GIRDER ELEVATION (INSIDE FACE SHOWN)



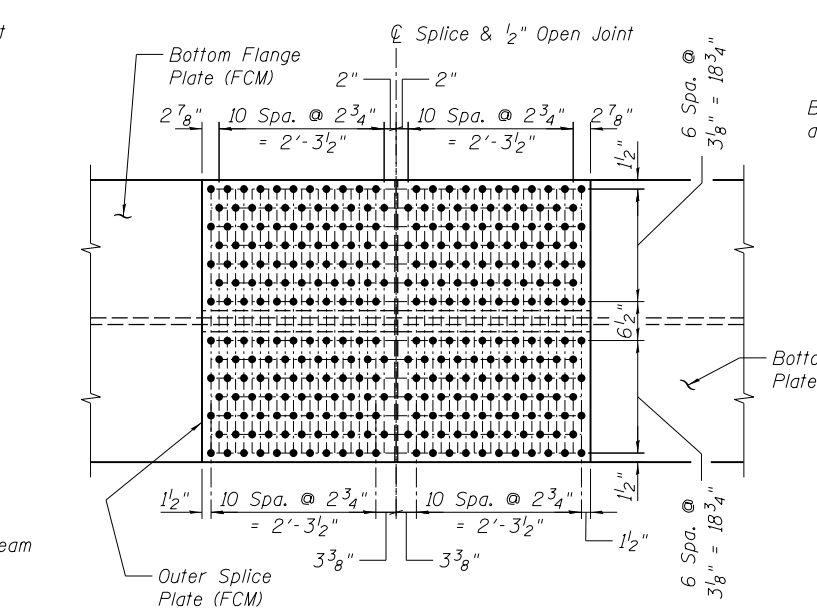
DETAIL 7



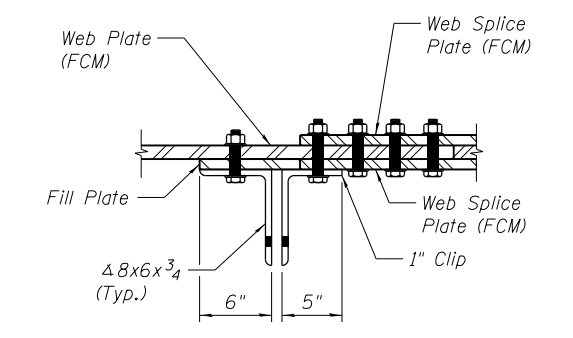
SECTION Y-Y



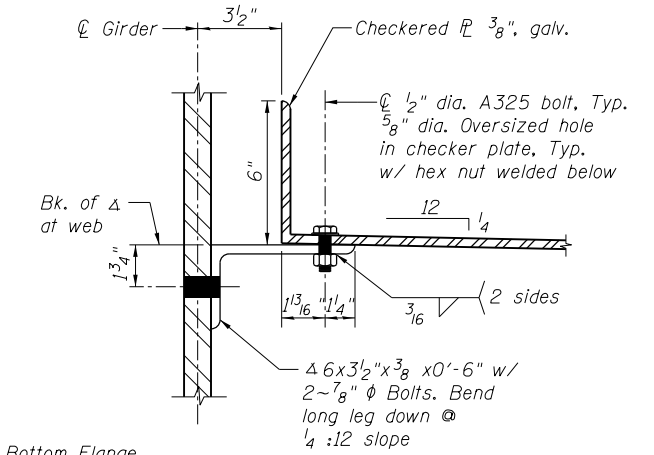
TOP FLANGE PLAN (Looking Down)



BOTTOM FLANGE PLAN (Looking Up)



SECTION X-X



SECTION Z-Z

* Alternate larger 4" flange thickness is permitted throughout to facilitate material acquisition. Calculated weight of structural steel is based on lighter section. Calculated girder stresses are based on heavier section. The alternate, if utilized, shall be provided at no additional cost to the Department.

FILE NAME: ... PROJECTS\DOCUMENTS\... SHEET 0849963-09L0179B-NSRR-001

USER NAME = Pop00275	DESIGNED - MJW	REVISED -
PLOT SCALE = 0.2,0000 ' / in.	CHECKED - TJH/TDP	REVISED -
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

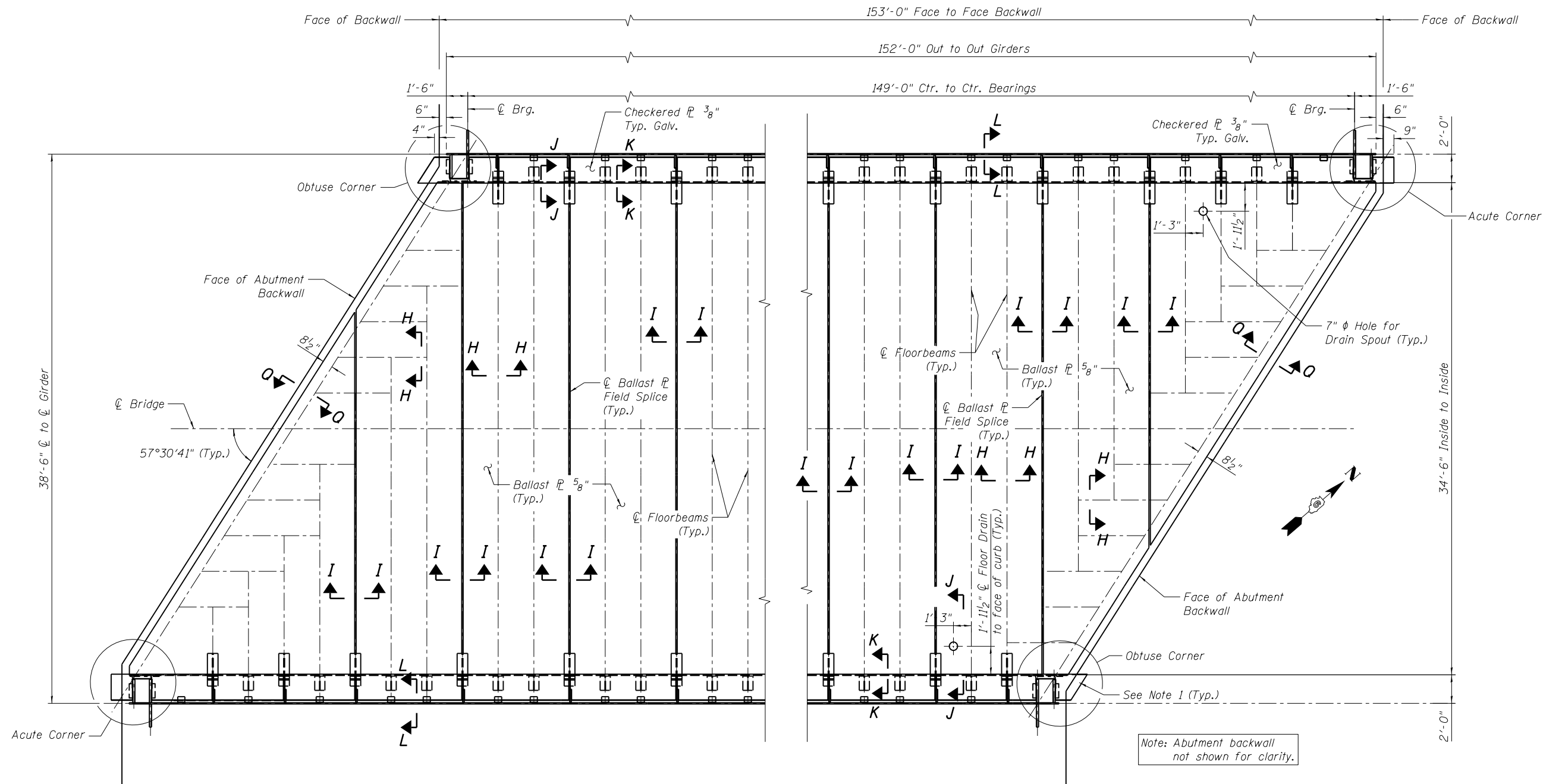
**GIRDER SPlice DETAILS
STRUCTURE 084-9963 - 6TH ST NSRR**

SHEET NO. 13 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(109) VB,(110) VB-5	SANGAMON	382	275
			CONTRACT NO. 93733	
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

To HANNIBAL, MO
(Timetable West)

To DECATUR, IL
(Timetable East)



CLOSURE PLATE & BALLAST PAN PLAN

See Sheet 15 of 29 for Section H-H, I-I, J-J, K-K, L-L, & O-O.

Notes:

1. Prior to Setting End Checked $\#$, Build-up top of Concrete Backwall with Epoxy Grout to Support Checked $\#$ and Provide Sloped Surface to Eliminate Tripping Hazard. Typical All Four Corners.
2. Checked $\#$ Shall be ASTM A786 Gr 36 or ASTM A36. Galvanize after fabrication.

\\spr-svr306.hanson.dom\hanson_projects\Documents\09Jobs\09L01798\CAD\Struct\6th\Sheet\0849963-09L01798-NSRR-001

FILE NAME =
 HANSON
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USER NAME = Pop00275	DESIGNED - MJW	REVISD -
	CHECKED - TJH/TDP	REVISD -
PLOT SCALE = 0:2.0000 '1' / in.	DRAWN - RSJ	REVISD -
PLOT DATE = 4/11/2019	CHECKED - MJW	REVISD -

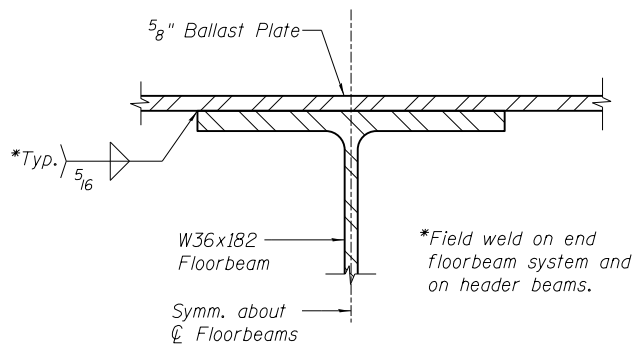
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**CLOSURE PLATE AND BALLAST PLATE PLAN
STRUCTURE 084-9963 - 6TH ST NSRR**

SHEET NO. 14 OF 29 SHEETS

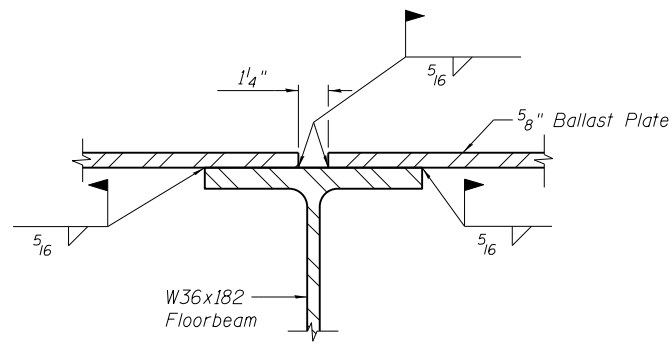
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	276
			CONTRACT NO. 93733	
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

FINAL

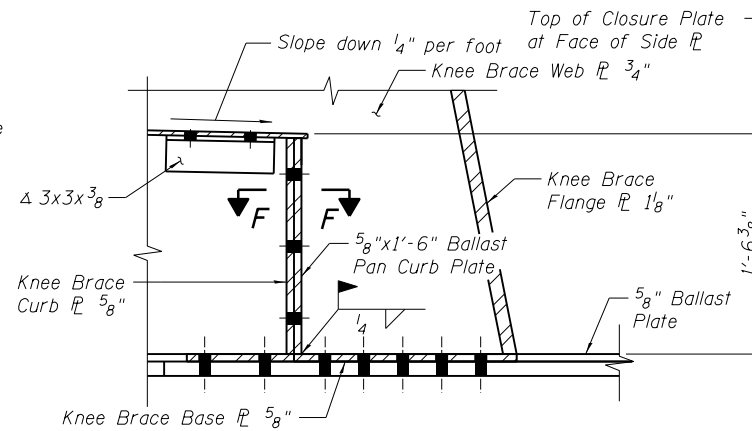


SECTION H-H BALLAST PLATE TO FLOORBEAM CONNECTION (TYP.)

Similar Detail at Header Beam

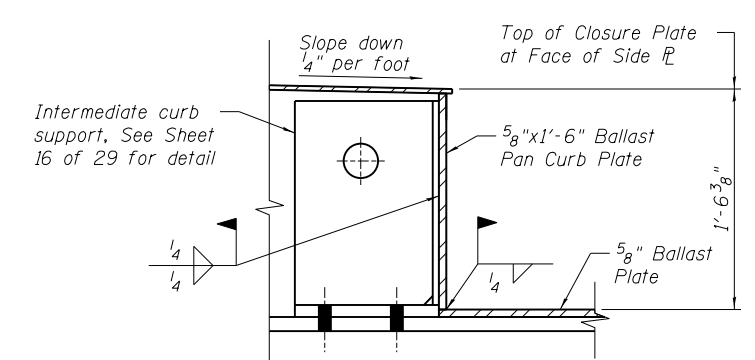


SECTION I-I

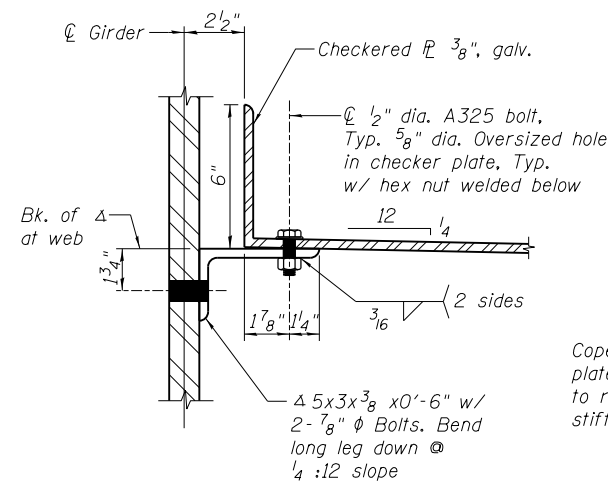


SECTION J-J

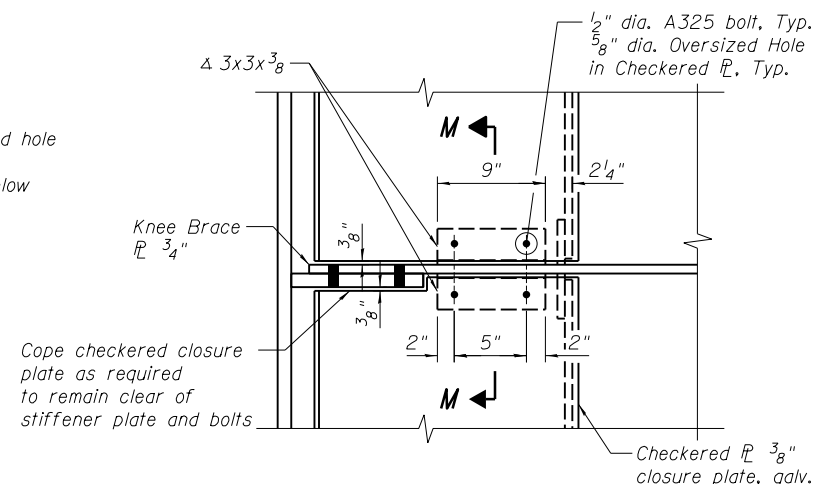
See Sheet 11 of 29 for Section F-F.



SECTION K-K

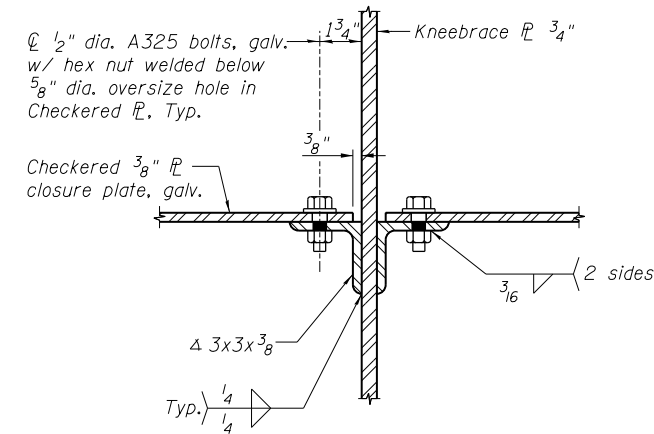


SECTION L-L

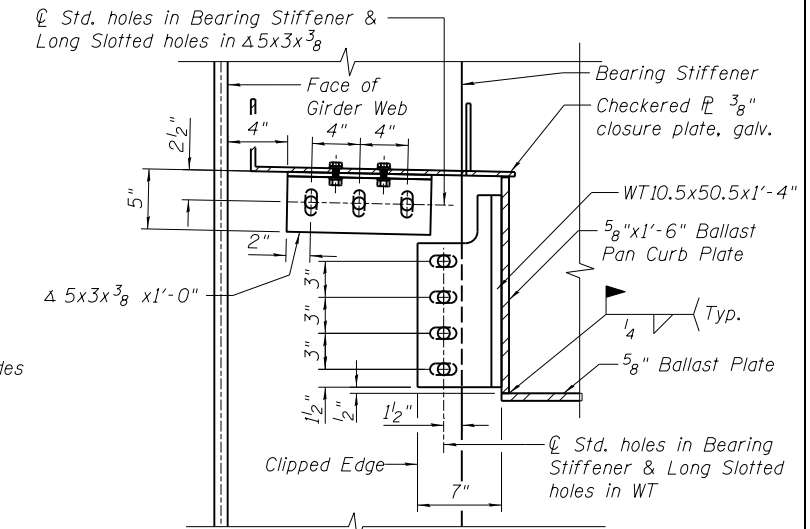


PLAN

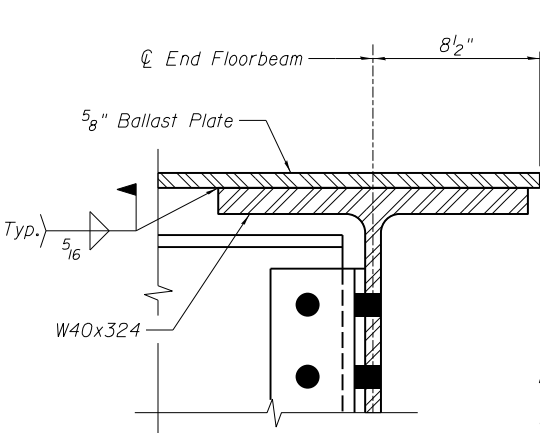
Closure Plate at Kneebrace (Kneebrace Flange Omitted for Clarity)



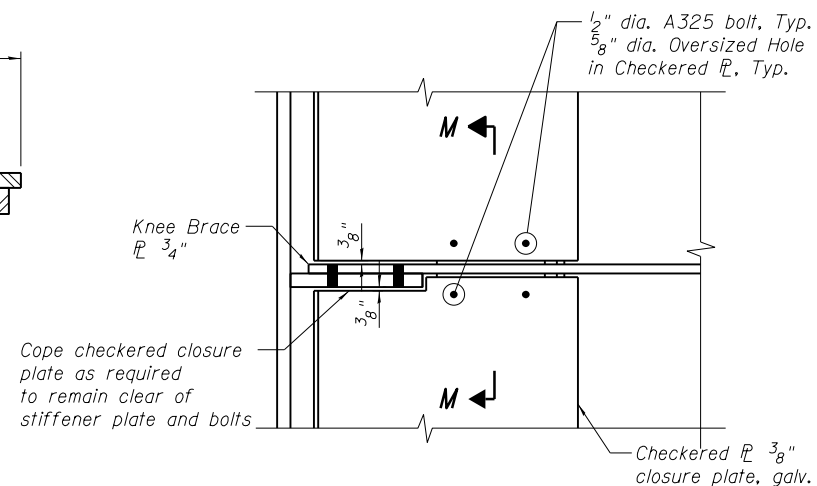
SECTION M-M



SECTION N-N

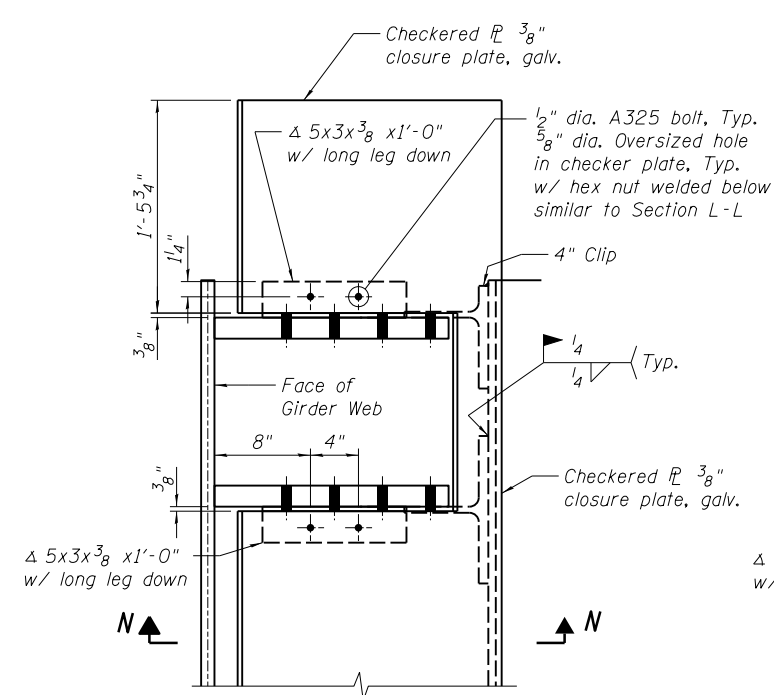


SECTION Q-Q



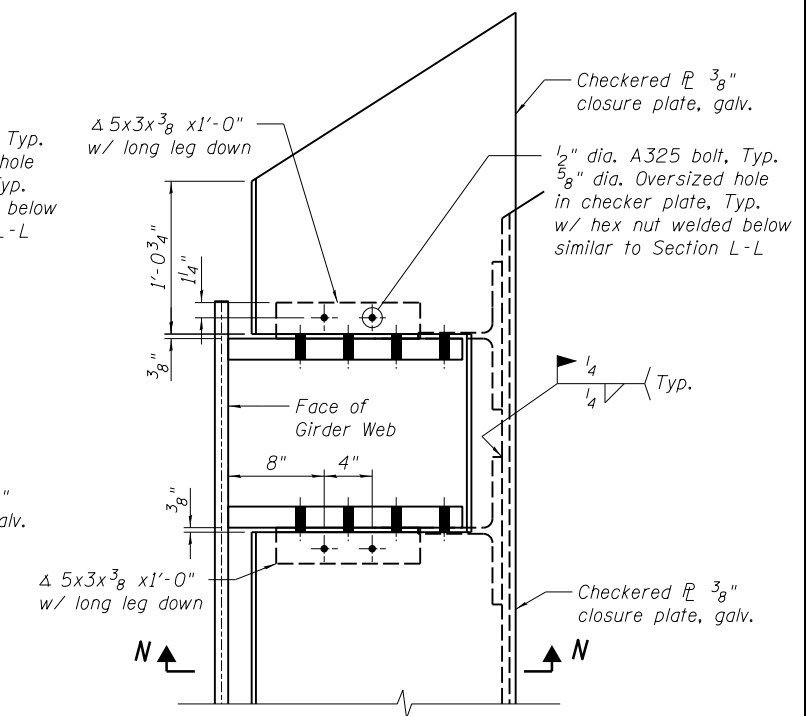
PLAN

Closure Plate at Kneebrace (Kneebrace Flange Omitted for Clarity)



PLAN

Closure Plate at Bearing Stiffener (Acute Corner)



PLAN

Closure Plate at Bearing Stiffener (Obtuse Corner)

FINAL
DESIGNED MW
DRAWN RSJ
REVIEWED MM
DATE

\\sp1\svr\306\hanson.dom\hanson_projects\Documents\09\Jobs\09L01798\CAD\Struct\6th\Sheet\0849963-09L01798-NSRR-001

FILE NAME =
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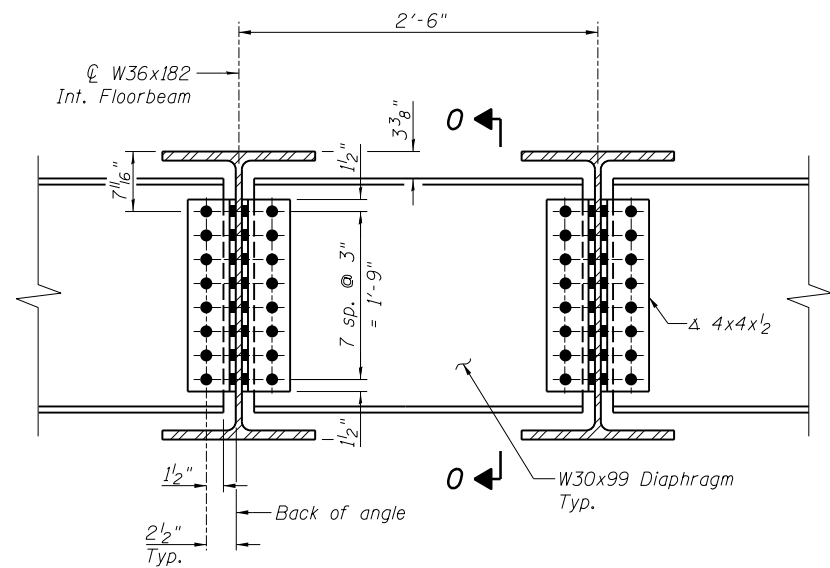
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	CHECKED - MJW	REVISIONS

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

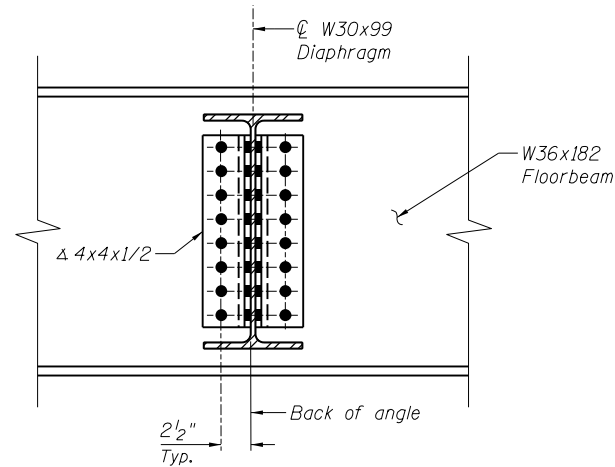
**CLOSURE PLATE AND BALLAST PLATE DETAILS
STRUCTURE 084-9963 - 6TH ST NSRR**

SHEET NO. 15 OF 29 SHEETS

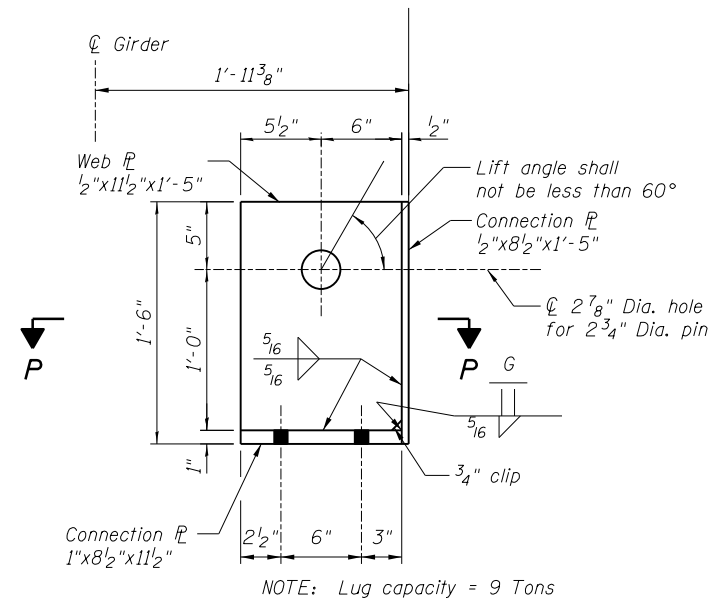
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	277
•666 & 666 ALT. ILLINOIS FED. AID PROJECT			CONTRACT NO. 93733	



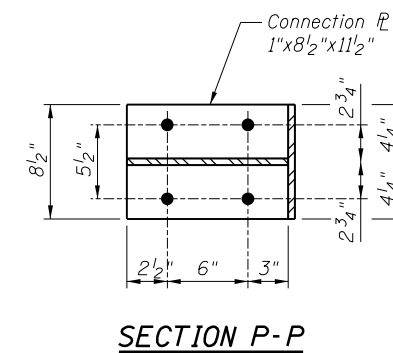
LONGITUDINAL DIAPHRAGM DETAIL



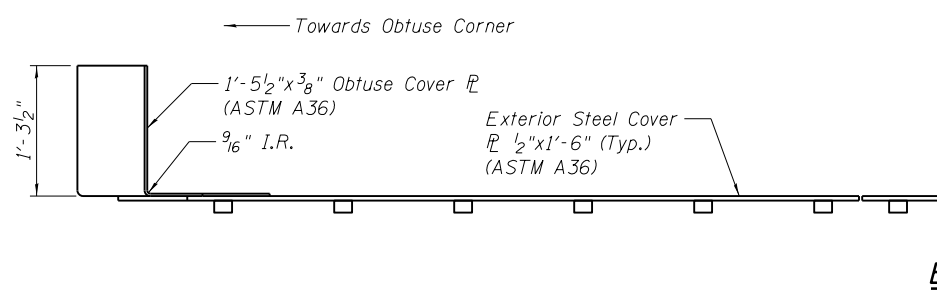
SECTION O-O



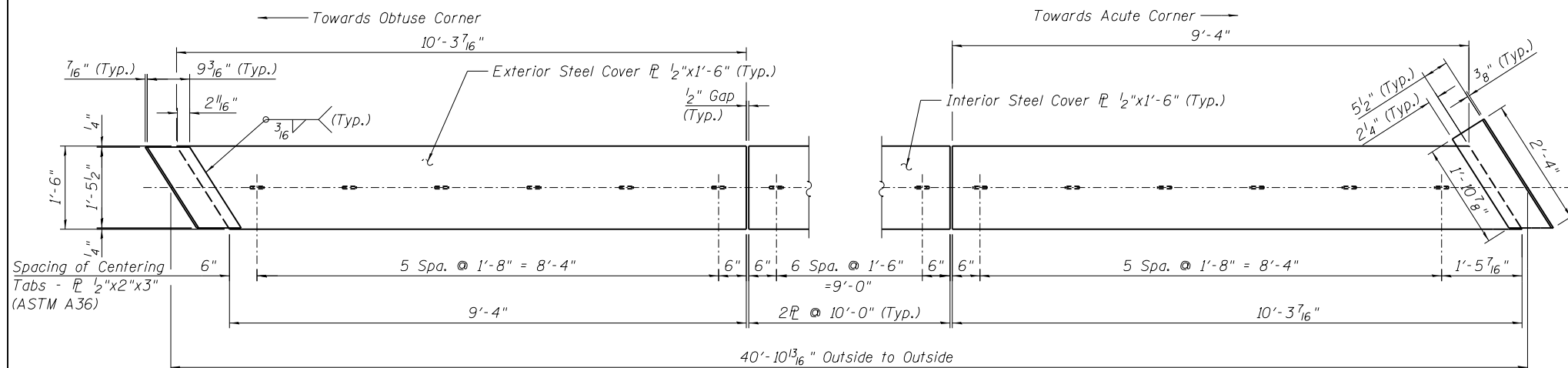
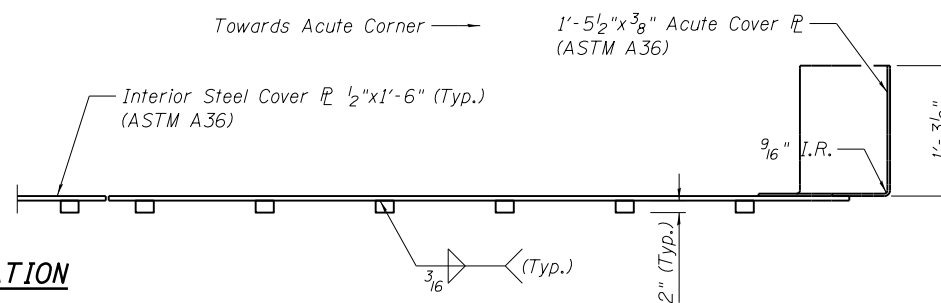
INTERMEDIATE CURB SUPPORT



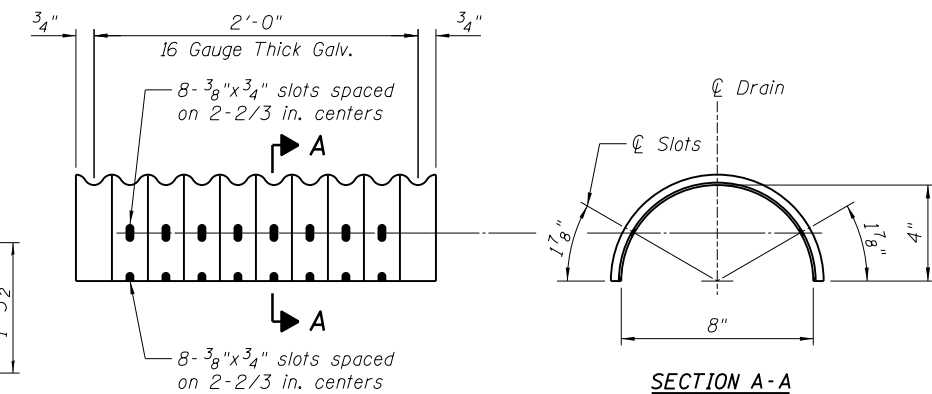
SECTION P-P



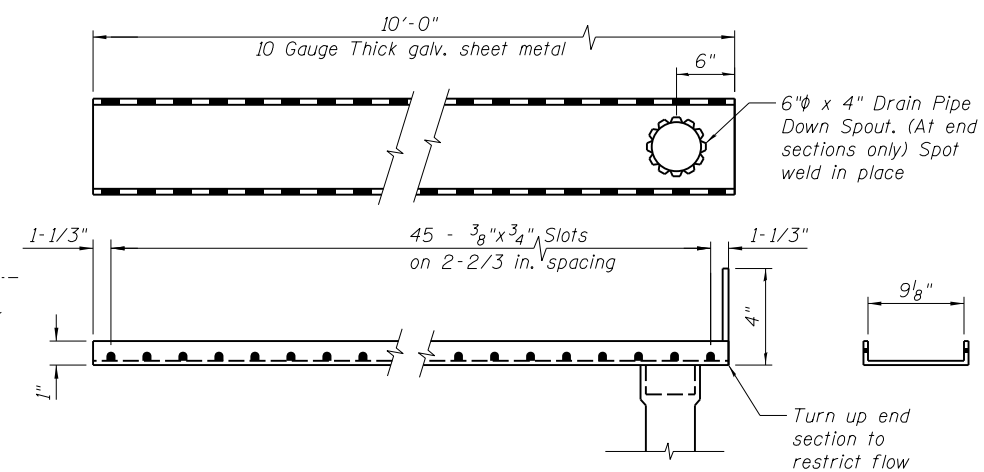
ELEVATION



COVER PLATES
(Galvanize after Fabrication)

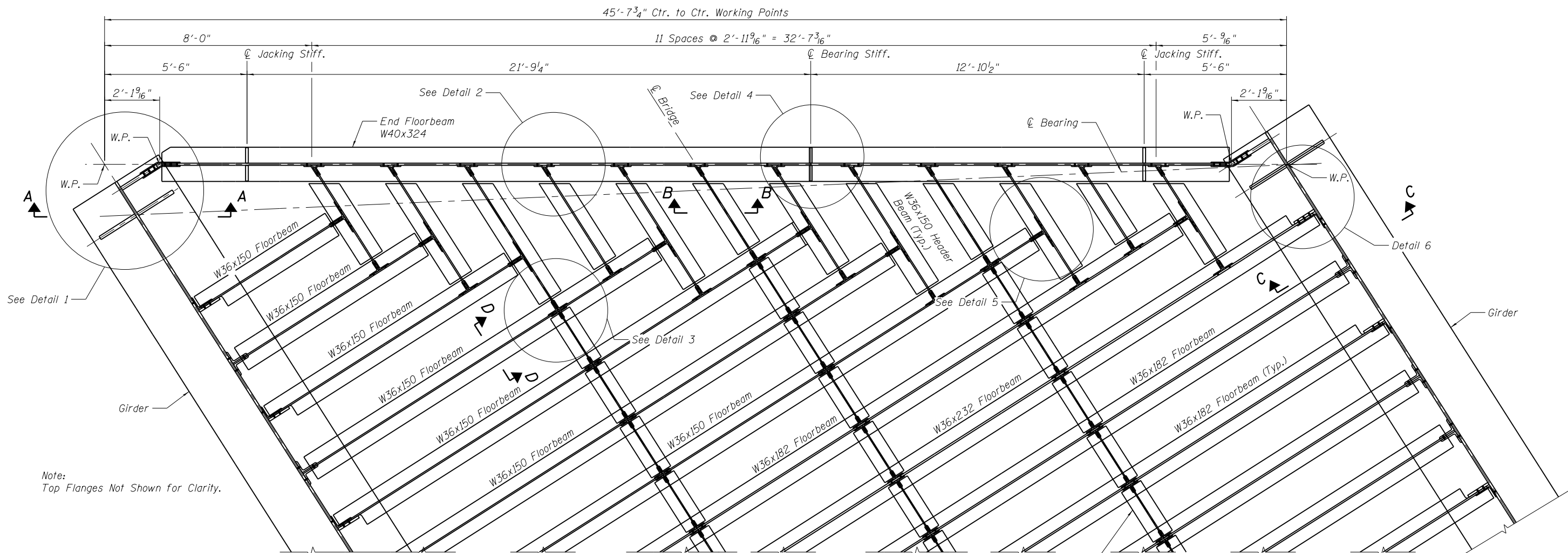


DETAIL - DECK DRAIN PIPE



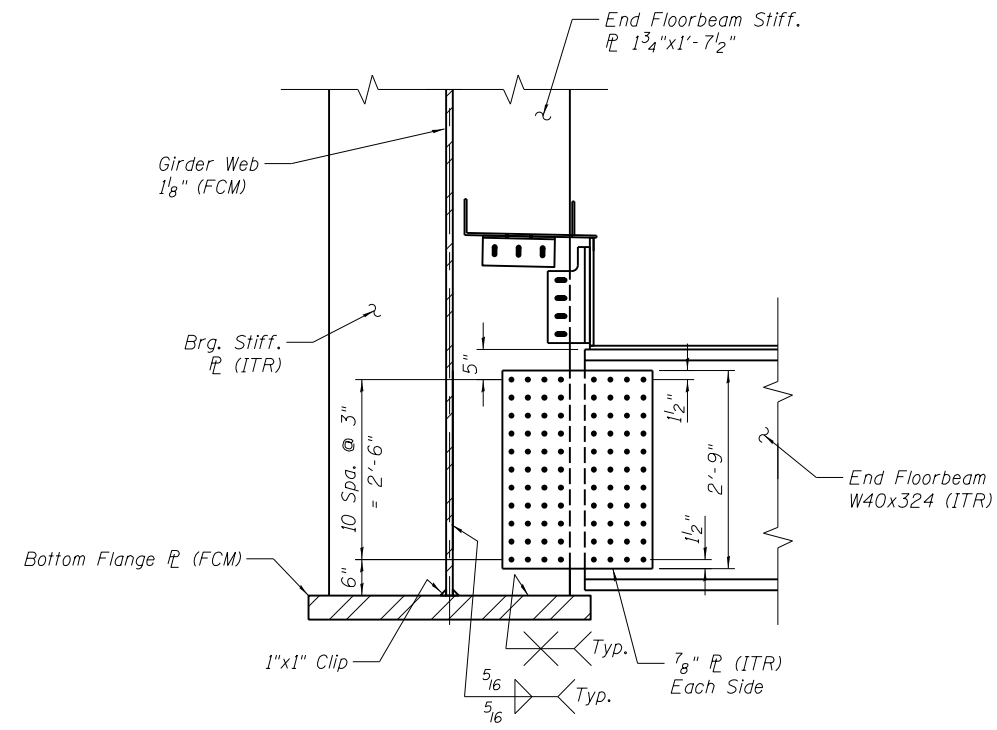
DETAIL - DECK DRAIN BOTTOM PAN

- Notes:
- Lap Drain Pipe one corrugation at each end.
 - Coordinate outside diameter of drain pipe down spout with 6" ϕ Ductile Iron Pipe.
 - Cost for deck drain pipe and bottom pan shall be included in the cost for "Drainage System".

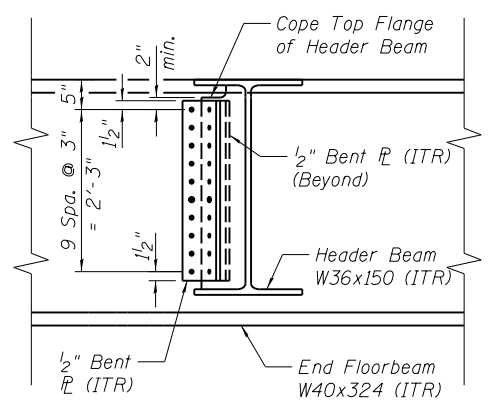


TYPICAL END FLOORBEAM PLAN
See Sheet 18 for Details

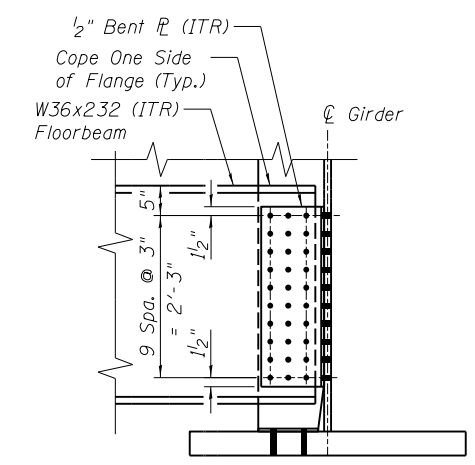
Note:
Top Flanges Not Shown for Clarity.



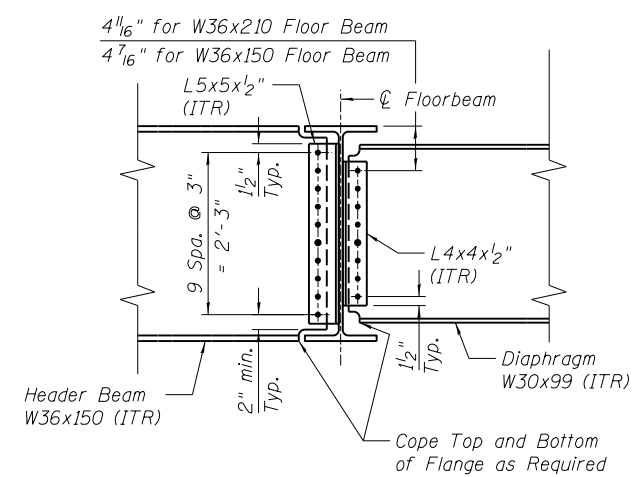
SECTION A-A
See Detail 1 on Sheet 18 for Horizontal Bolt Spacing.



SECTION B-B
See Detail 2 on Sheet 18 for Horizontal Bolt Spacing.

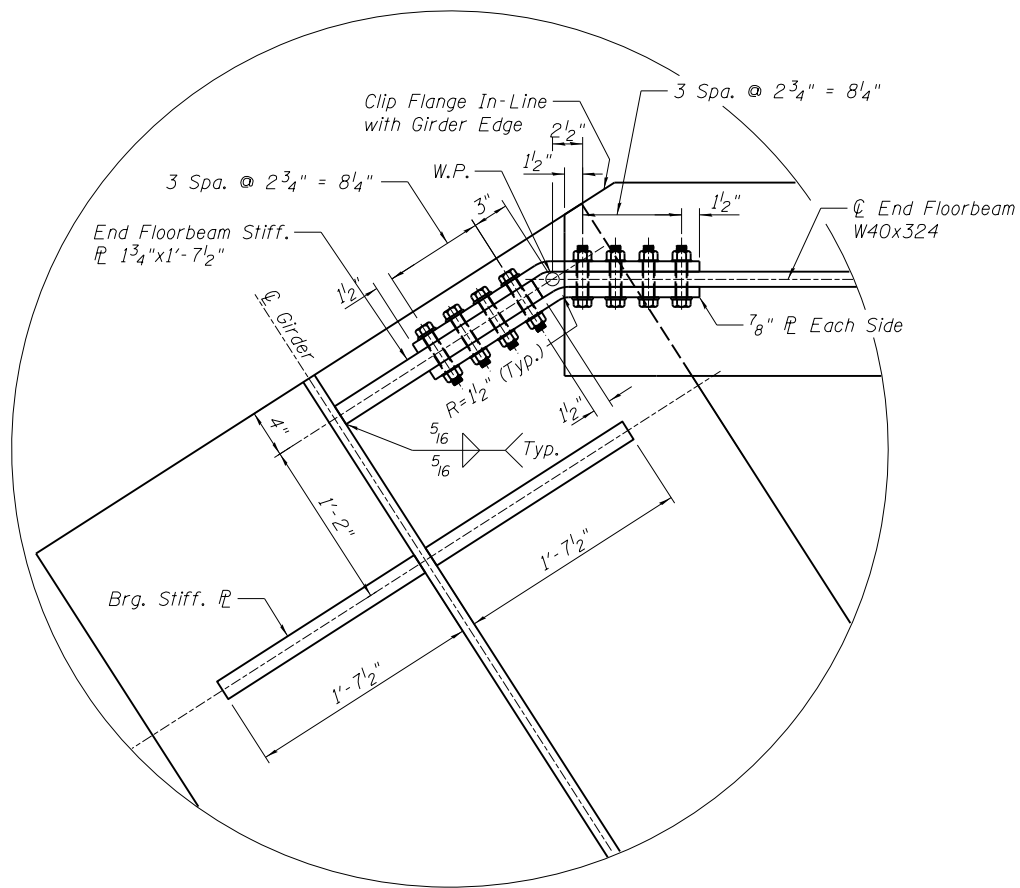


SECTION C-C
See Detail 6 on Sheet 18 for Horizontal Bolt Spacing.

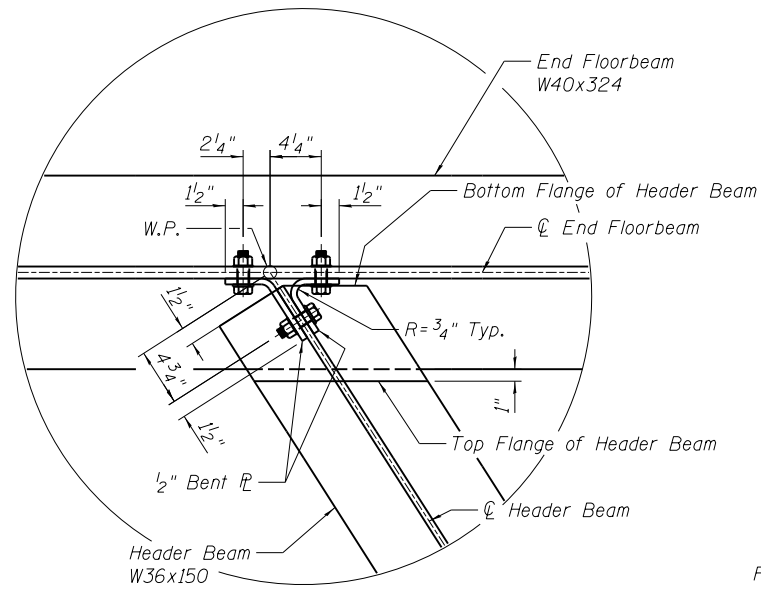


SECTION D-D
See Detail 3 on Sheet 18 for Horizontal Bolt Spacing.

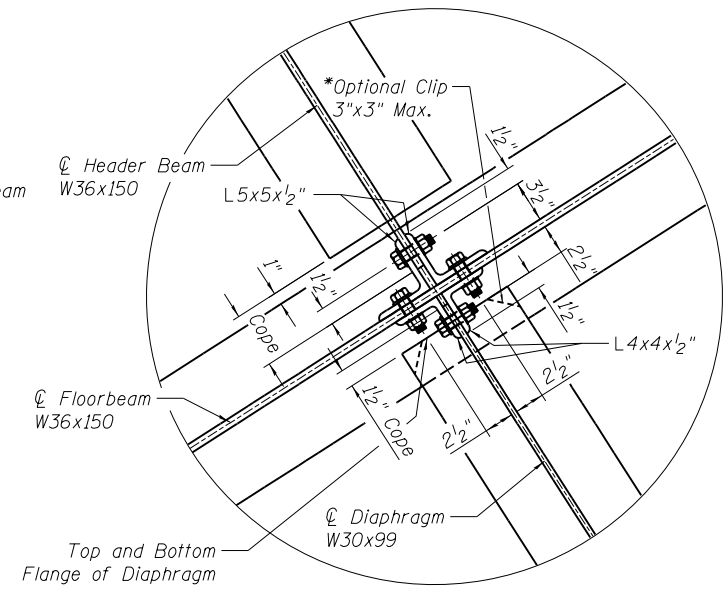
FINAL <small>© Copyright Hanson Professional Services Inc., 2019</small>	FILE NAME = <small>...Projects\Documents\09Jobs\09L0179B\CAD\Structure\6th\Sheet\0849963-09L0179B-NSRR-001</small> USER NAME = Pop00275 PLOT SCALE = 0:2.0000 " = 1" / in. PLOT DATE = 4/11/2019	DESIGNED - MJW CHECKED - TJH/TDP DRAWN - RSJ CHECKED - MJW	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	MISCELLANEOUS GIRDER DETAILS - SHEET 2 OF 3 STRUCTURE 084-9963 - 6TH ST NSRR	F.A.P. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO. (109) VB,(110) VB-5 SANGAMON 382 279 CONTRACT NO. 93733
	SHEET NO. 17 OF 29 SHEETS					666 & 666 ALT. ILLINOIS FED. AID PROJECT



DETAIL 1

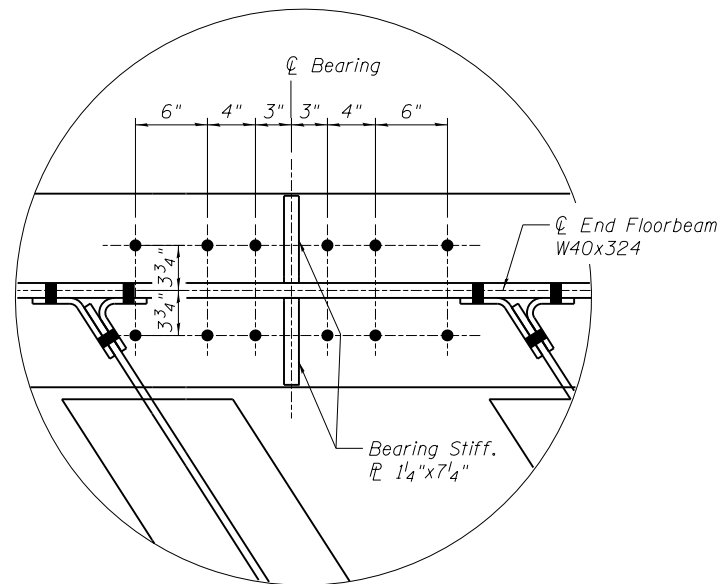


DETAIL 2

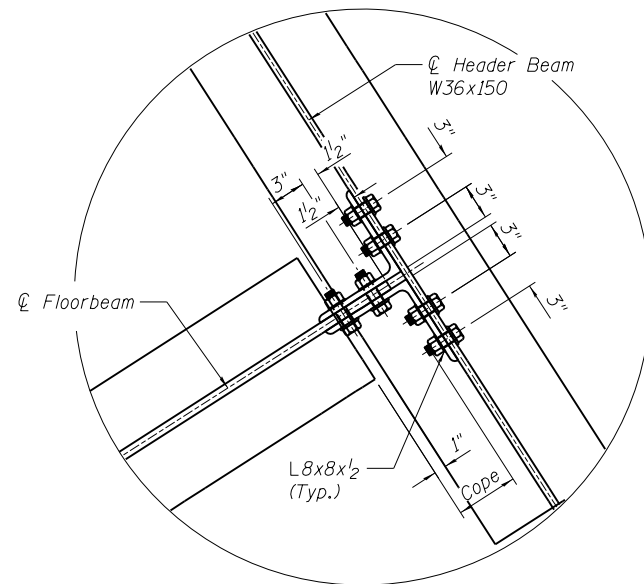


DETAIL 3

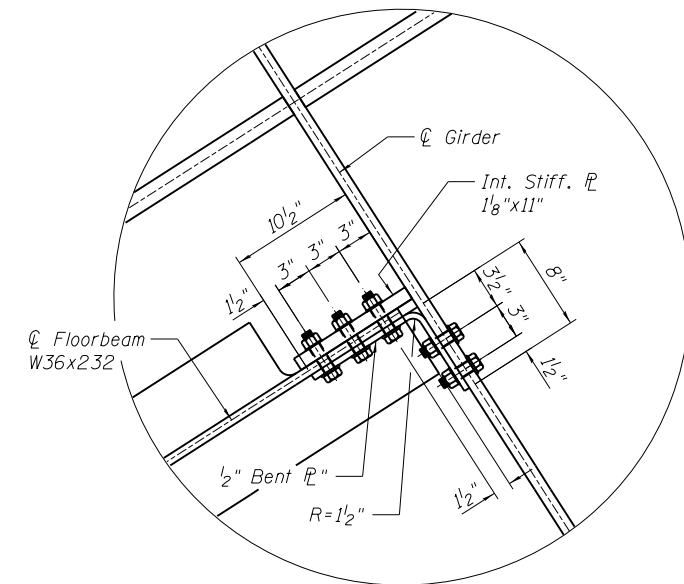
*Clipping diaphragm flanges is permitted to facilitate erection at intermediate and end floor system locations. If clipped it shall be provided at no additional cost to the Department.



DETAIL 4



DETAIL 5



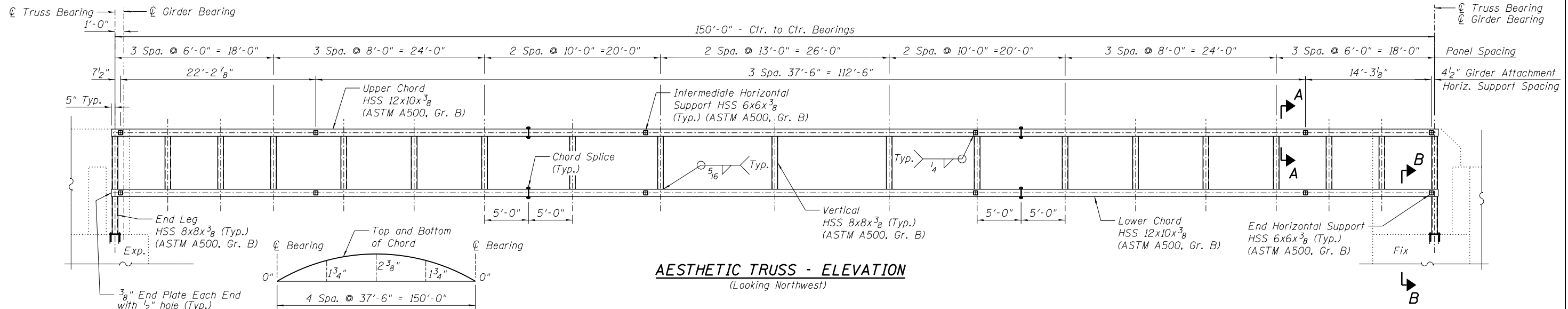
DETAIL 6

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PLOT SCALE = 0:2.0000 ' = 1" / in.	CHECKED - TJH/TDP	REVISED -
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(109) VB,(110) VB-5	SANGAMON	382	280
			CONTRACT NO. 93733	
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				

To HANNIBAL, MO
(Timetable West)

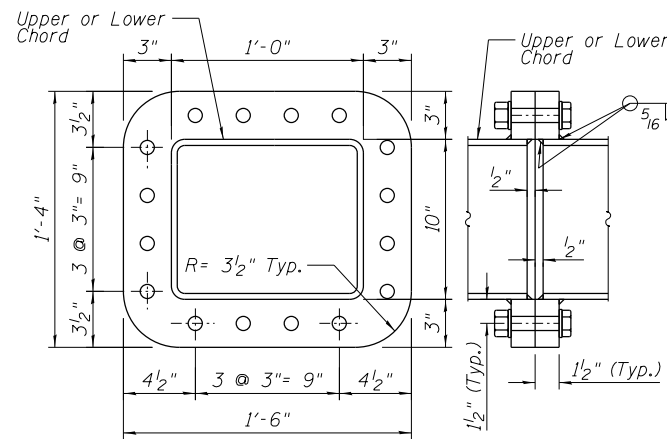
To DECATUR, IL
(Timetable East)



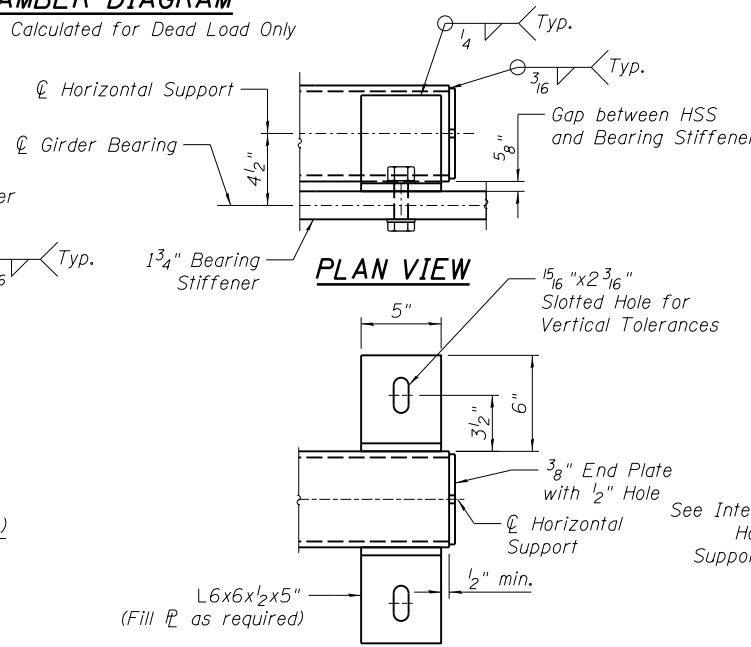
AESTHETIC TRUSS - ELEVATION
(Looking Northwest)

CAMBER DIAGRAM

Camber Calculated for Dead Load Only



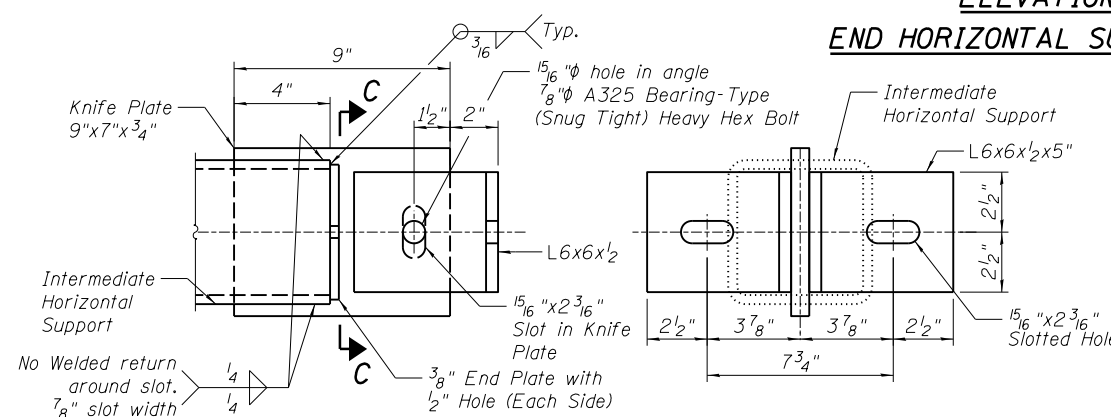
CHORD SPLICE DETAIL



PLAN VIEW

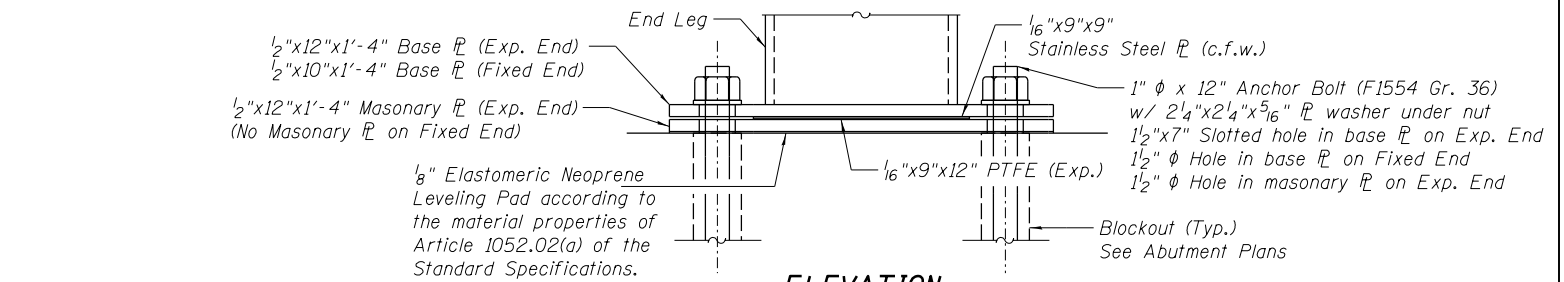
ELEVATION VIEW

END HORIZONTAL SUPPORT DETAIL

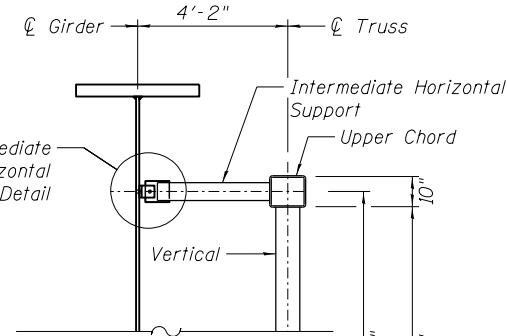


INTERMEDIATE HORIZONTAL SUPPORT DETAIL

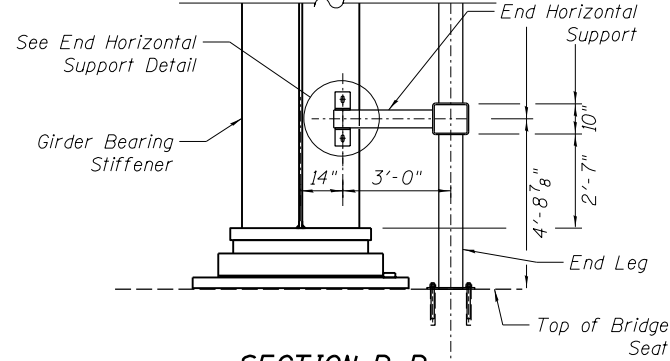
SECTION C-C



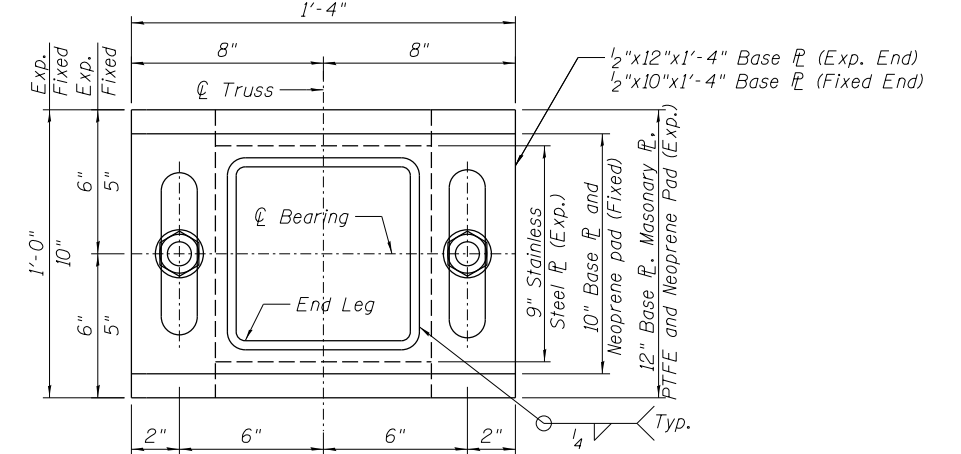
ELEVATION



SECTION A-A



SECTION B-B



PLAN
END LEG BEARING DETAIL
(Expansion Bearing Shown)

Note:
Location of Fixed and Expansion bearings shall match the girder.
Cost for elastomeric neoprene leveling pad, PTFE surface, shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 4."
Anchor Bolts shall be ASTM F1554 all-thread (or an Engineer approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Anchor Bolts shall be installed in blockouts with non-shrink grout meeting the material requirements of Article 1024.02 of the Standard Specifications. Blockouts shall be clean prior to grouting and grout installed according to manufactures recommendations.
The PTFE shall be bonded directly to the masonry plate according to the manufacturers recommendations.

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FILE NAME :	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
	PLOT SCALE = 0:2.0000 " = 1' in.	DRAWN - RSJ	REVISED -
	PLOT DATE = 4/11/2019	CHECKED - MJW	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

AESTHETIC TRUSS
STRUCTURE 084-9963 - 6TH ST NSRR

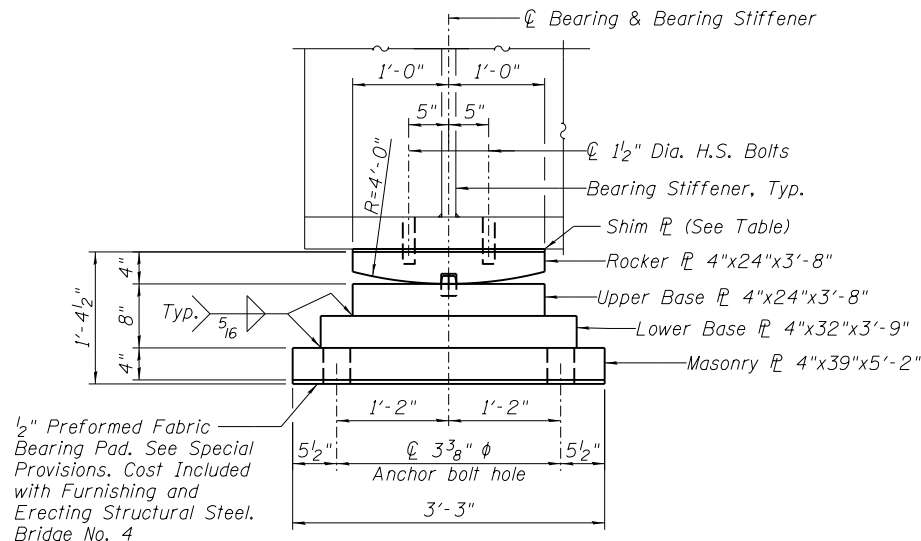
SHEET NO. 19 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	281
			CONTRACT NO. 93733	
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

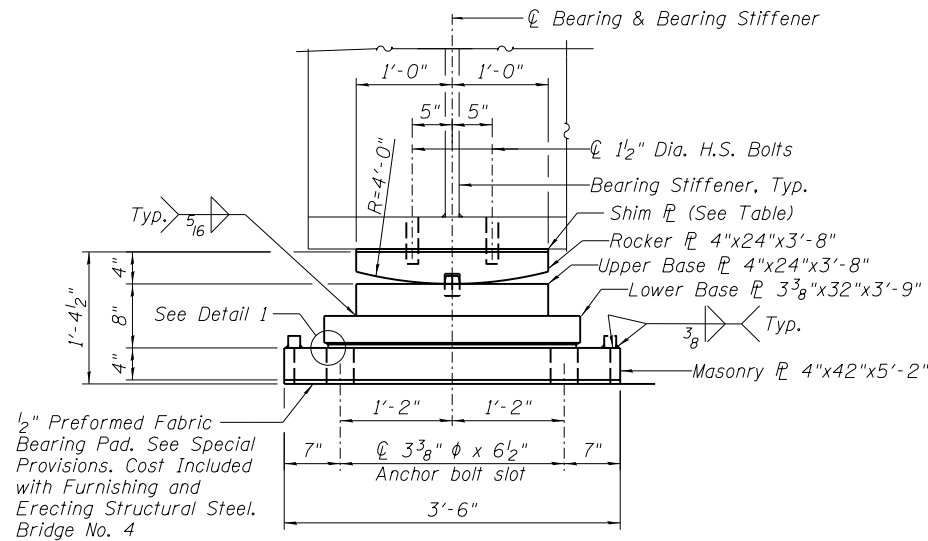
FINAL



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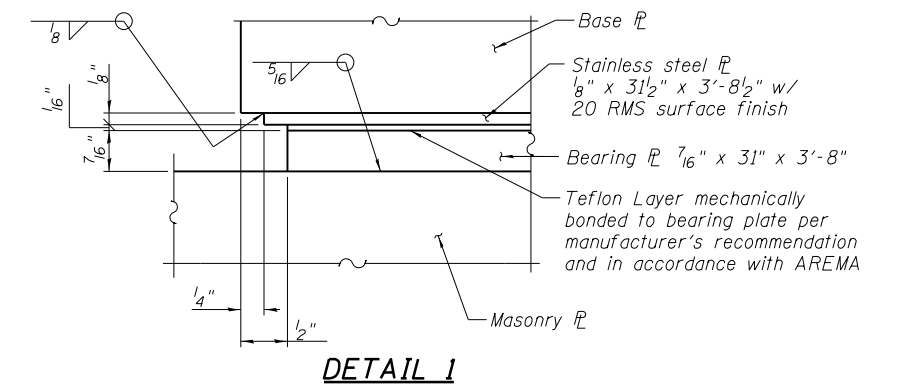


ELEVATION - FIXED BEARING

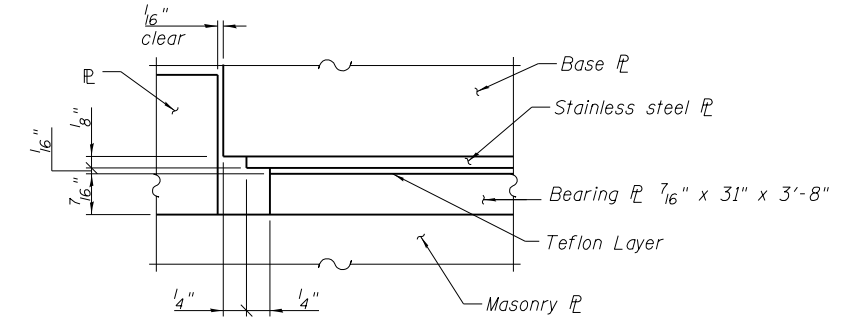


ELEVATION - EXPANSION BEARING

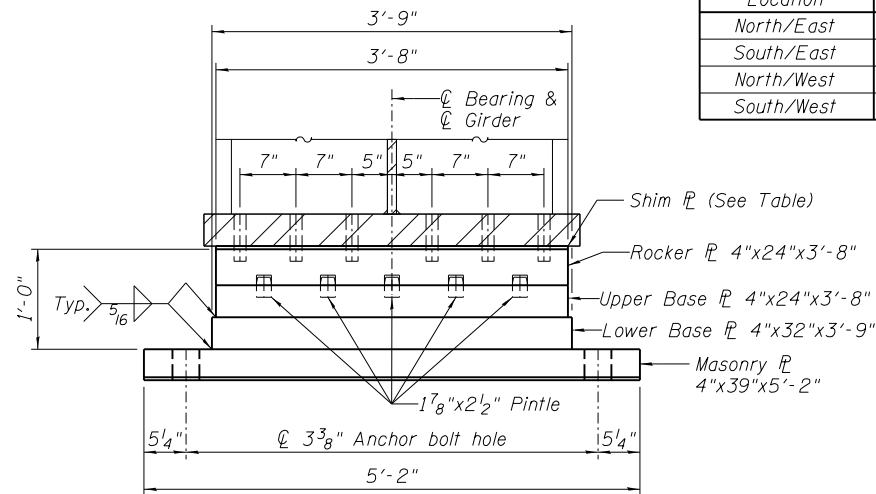
SHIM PLATE THICKNESS	
Location	Thickness
North/East	1/8"
South/East	5/8"
North/West	1/8"
South/West	5/8"



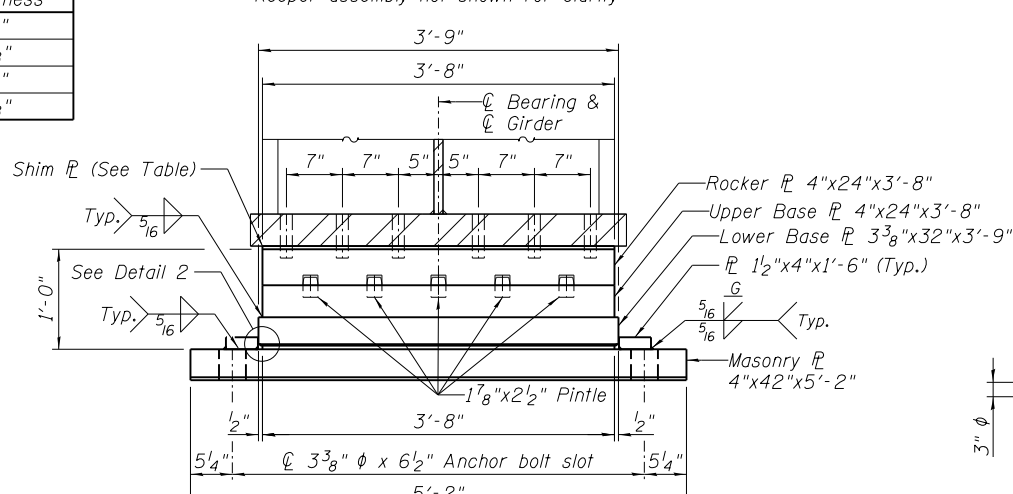
DETAIL 1



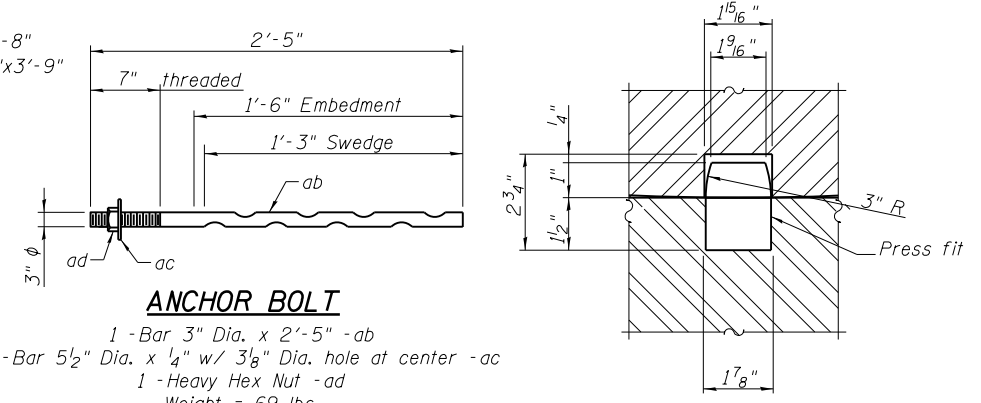
DETAIL 2



END VIEW - FIXED BEARING

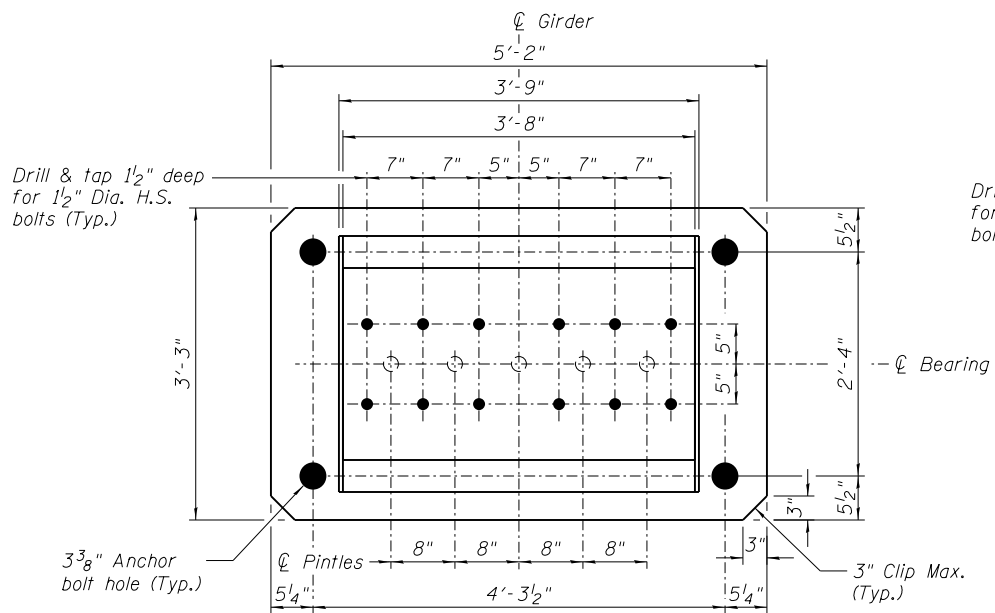


END VIEW - EXPANSION BEARING



ANCHOR BOLT

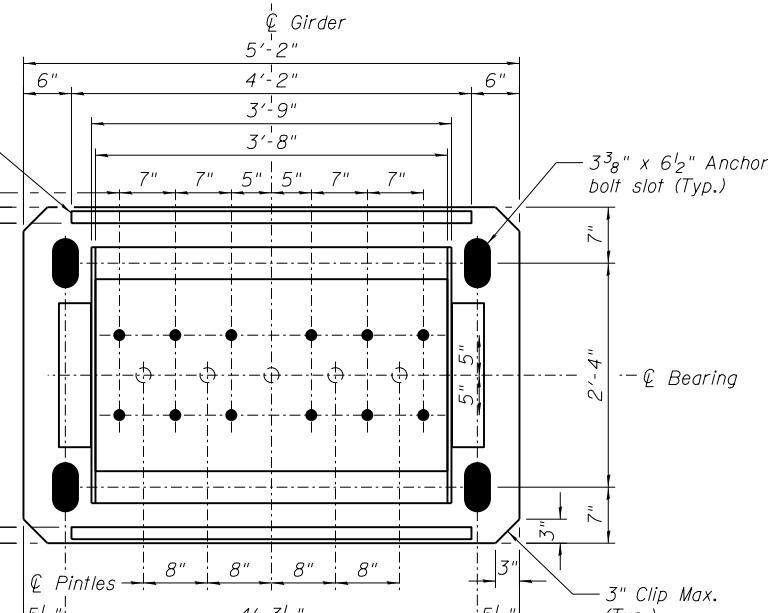
PINTLE DETAIL



PLAN VIEW - FIXED BEARING

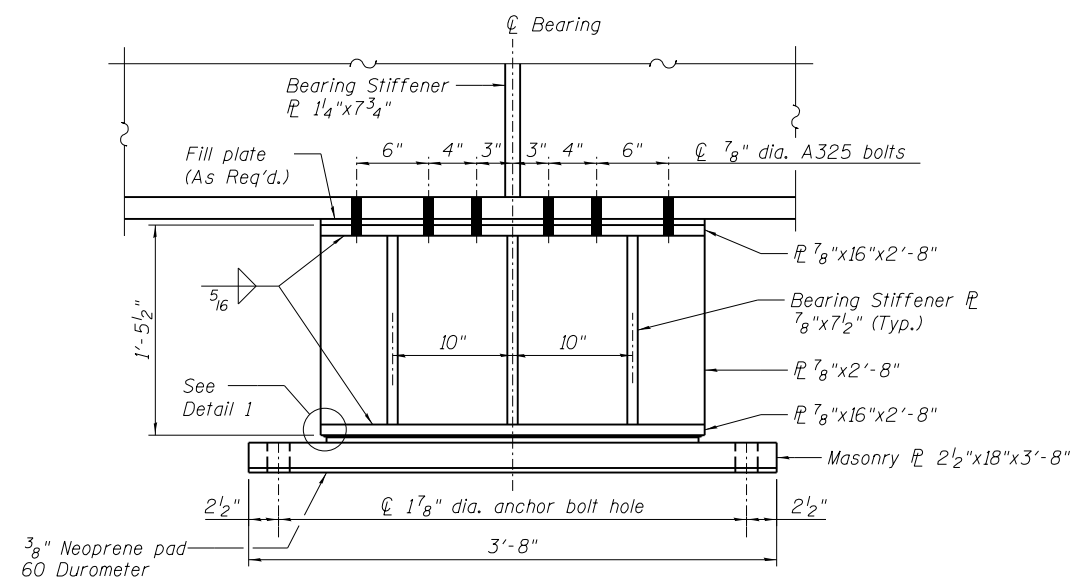
Keeper Bar
1 1/2" x 1 1/2" x 4'-2"

Drill & tap 1/2" deep
for 1/2" Dia. H.S.
bolts (Typ.)



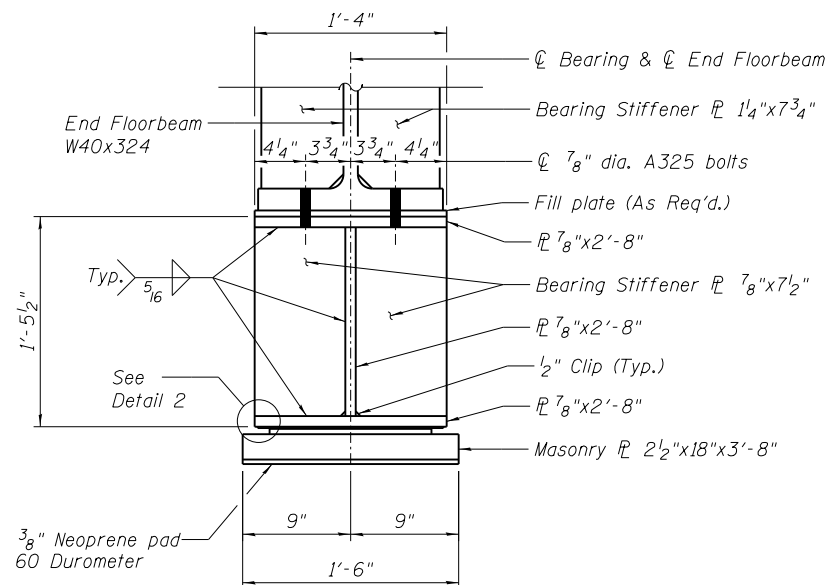
PLAN VIEW - EXPANSION BEARING

- NOTES:**
- Steel used for bearing assemblies shall conform to ASTM A709 GR50, unless noted otherwise. Anchor bolts shall conform to ASTM F1554 Gr 105. Cost of bearings and anchor bolts shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 4".
 - Stainless steel shall conform to ASTM A480.
 - Bearing assembly weldments shall be stressed relieved by heat treating prior to finish machining, per current AWS structural welding codes.
 - Teflon Layer shall be composed of virgin unfilled TFE resin, unfilled TFE sheets or unfilled TFE fabric. Filler material, such as milled glass fibers will not be allowed. Teflon layer shall conform to the requirements of AREMA Chapter 15.
 - All surfaces in moving contact shall be finished 125/.
 - All dimensions shown are final dimensions after machining.
 - Bearings to be shop fitted to girders, match marked and assembled in units for shipping.
 - Anchor bolts, nuts and plate washers shall be galvanized in accordance with ASTM B695, Class 50.
 - Anchor bolt nuts shall be A563 Gr DH Heavy Hex & Washers shall be F436 Type 1.
 - Bolt removal and replacements to gain access to properly tighten bearing bolts is incidental to steel erection.



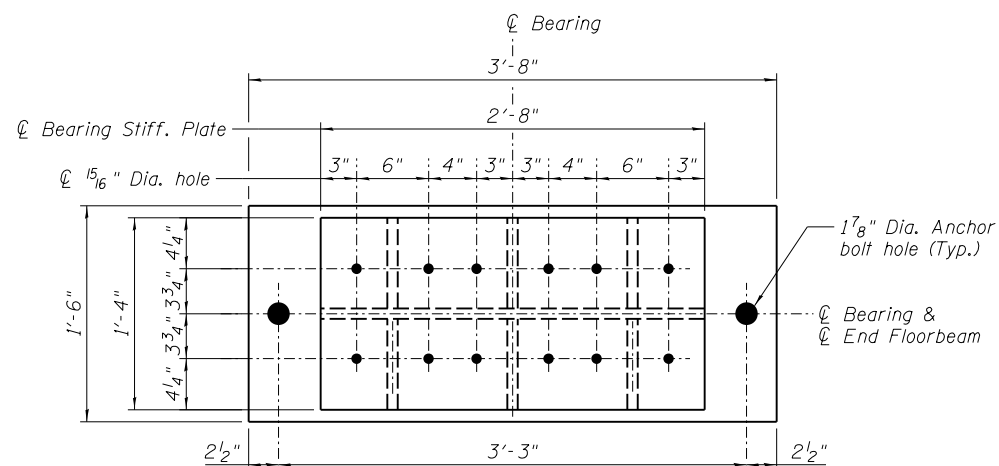
ELEVATION - END FLOORBEAM BEARING

Anchor Bolt not shown for clarity



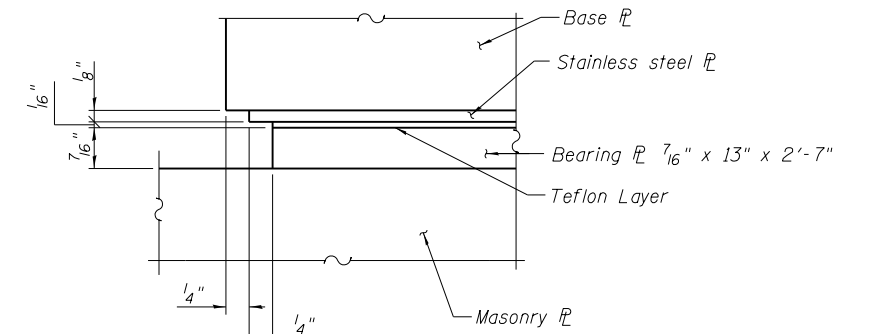
END VIEW - END FLOORBEAM BEARING

Anchor Bolt not shown for clarity

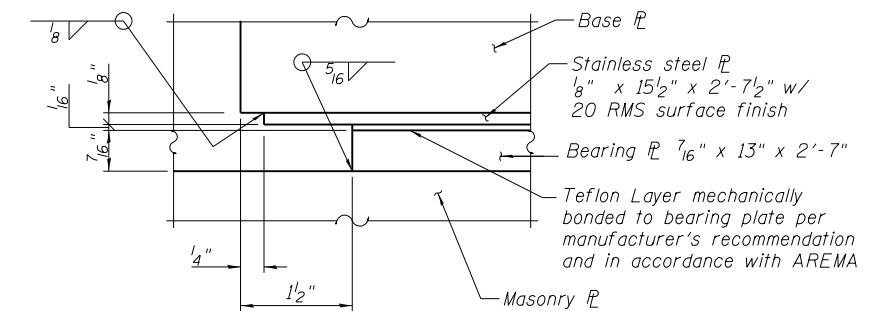


PLAN VIEW - END FLOORBEAM BEARING

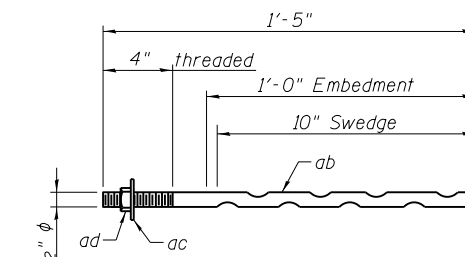
(2 Required)



DETAIL 1



DETAIL 2



ANCHOR BOLT

- 1 - Bar 1 1/2" Dia. x 1'-5" - ab
- 1 - Bar 3" Dia. x 1/4" w/ 1 5/8" Dia. hole at center - ac
- 1 - Heavy Hex Nut - ad
- Weight = 10 lbs.
- Galvanize after fabrication
- (4 Required)

NOTES:

1. Steel used for bearing assemblies shall conform to ASTM A709 GR50, unless noted otherwise. Anchor bolts shall conform to ASTM F1554 Gr 105. Cost of bearings and anchor bolts shall be included in the cost of "Furnishing and Erecting Structural Steel, Bridge No. 4".
2. Stainless steel shall conform to ASTM A480.
3. Bearing assembly weldments shall be stress relieved by heat treating prior to finish machining, per current AWS structural welding codes.
4. Teflon Layer shall be composed of virgin unfilled TFE resin, unfilled TFE sheets or unfilled TFE fabric. Filler material, such as milled glass fibers will not be allowed. Teflon layer shall conform to the requirements of AREMA Chapter 15.
5. All surfaces in moving contact shall be finished 125/.
6. All dimensions shown are final dimensions after machining.
7. Bearings to be shop fitted to girders, match marked and assembled in units for shipping.
8. Anchor bolts, nuts and plate washers shall be galvanized in accordance with ASTM B695, Class 50.
9. Anchor bolt nuts shall be A563 Gr DH Heavy Hex & Washers shall be F436 Type 1.
10. Bolt removal and replacements to gain access to properly tighten bearing bolts is incidental to steel erection.

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FILE NAME =	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
	PLOT SCALE = 0:2.0000 " = 1" / in.	DRAWN - RSJ	REVISED -
	PLOT DATE = 4/11/2019	CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**END FLOORBEAM BEARING DETAILS
STRUCTURE 084-9963 - 6TH ST NSRR**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	283
			CONTRACT NO. 93733	

SHEET NO. 21 OF 29 SHEETS

•666 & 666 ALT. ILLINOIS FED. AID PROJECT

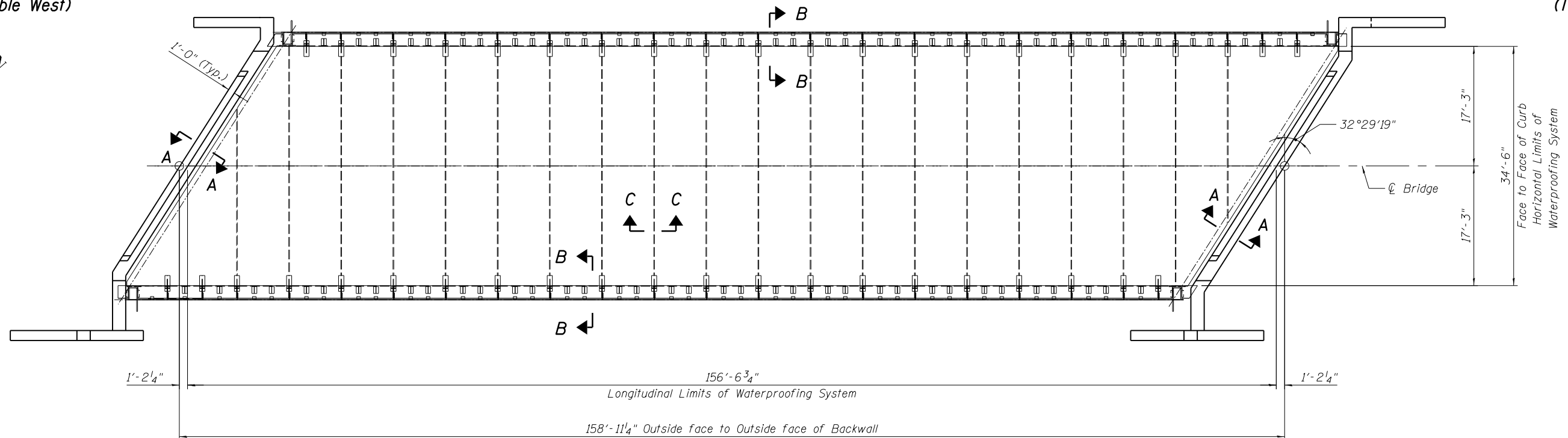
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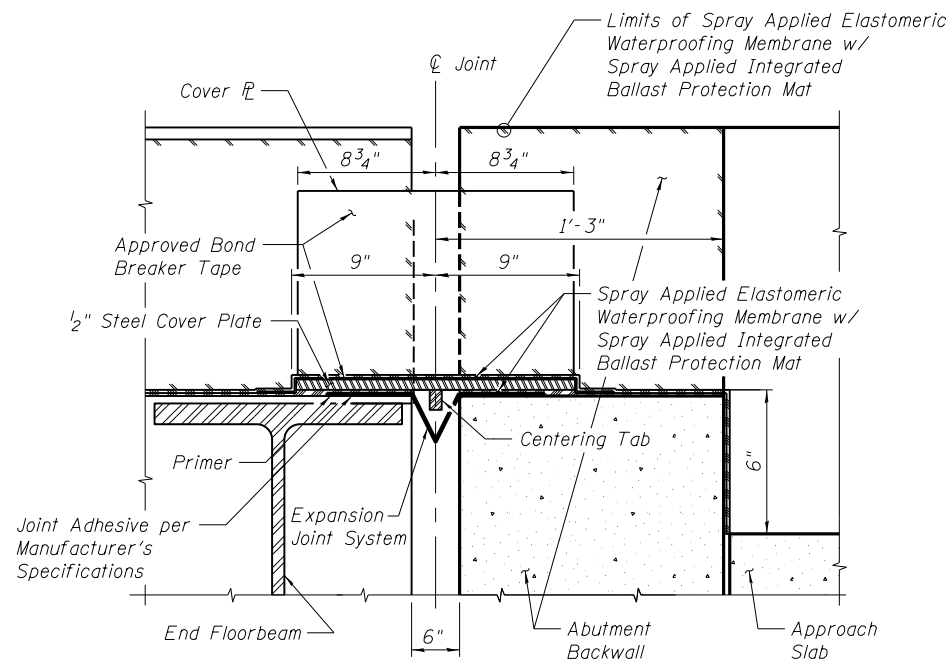
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To HANNIBAL, MO
(Timetable West)

To DECATUR, IL
(Timetable East)



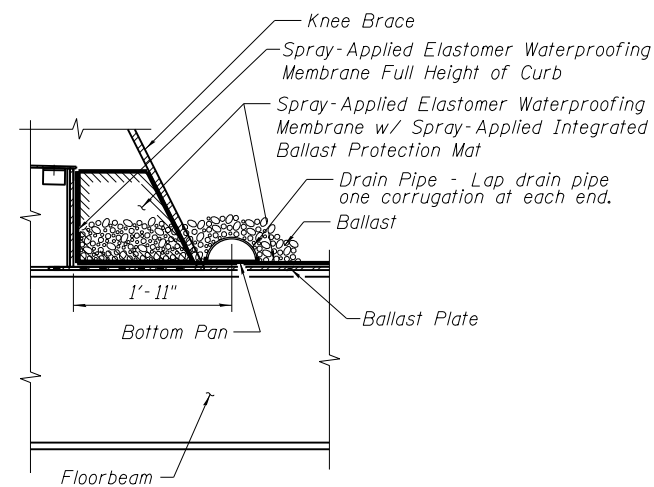
WATERPROOFING LIMITS PLAN



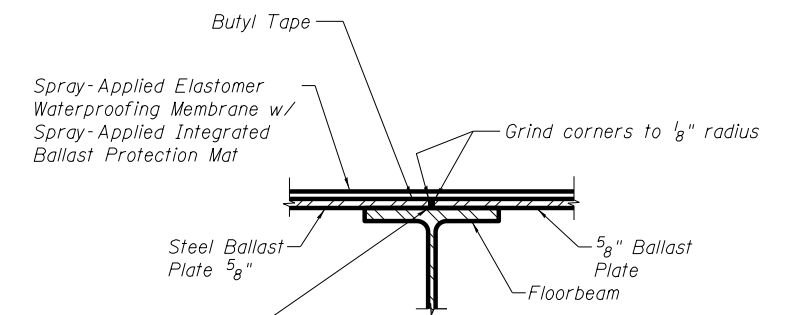
- Note:**
1. Bridge deck membrane continuous thru joint.
 2. Typical Joint Detail shown for information only. Waterproofing installer shall determine final details in accordance with the manufacturer's recommendations.

SECTION A-A

(At Rt. ∅'s to Bk. of Abut.)



SECTION B-B



Non-staining grey one compound non-sag elastomeric gun grade polyurethane sealant meeting the requirements of ASTM C-920, Type S, Grade NS, Class 25. Cost included with Membrane Waterproofing (Special).

SECTION C-C

Notes:

1. Prepare surfaces and apply in accordance with Manufacturer's recommendations.
2. Structural steel cover plates shall be galvanized.
3. Cost of joint adhesive and bond breaker tape shall be included in the cost of "Membrane Waterproofing (Special)".
4. The cover plate is included in the weight of the Structural Steel and will be paid for as "Furnishing and Erecting Structural Steel, Bridge No. 4".
5. For cover plate details see Sheet 16 of 29.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Membrane Waterproofing (Special)	Sq. Ft.	5,906

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FILE NAME =

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USER NAME = Pop00275	DESIGNED - MJW	REVISIONS
PLOT SCALE = 0:2.0000 '1" / in.	CHECKED - TJH/TDP	REVISIONS
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISIONS
	CHECKED - MJW	REVISIONS

STATE OF ILLINOIS
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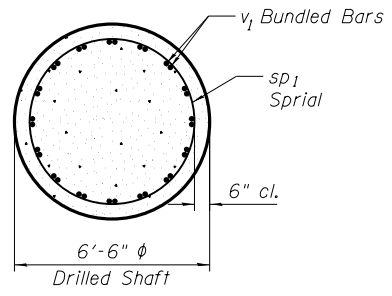
BRIDGE DECK WATERPROOFING
STRUCTURE 084-9963 - 6TH ST NSRR

SHEET NO. 22 OF 29 SHEETS

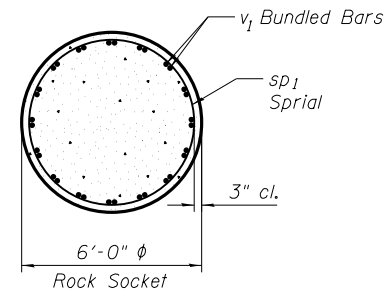
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	284
			CONTRACT NO.	93733
•666 & 666 ALT. ILLINOIS FED. AID PROJECT				

FINAL

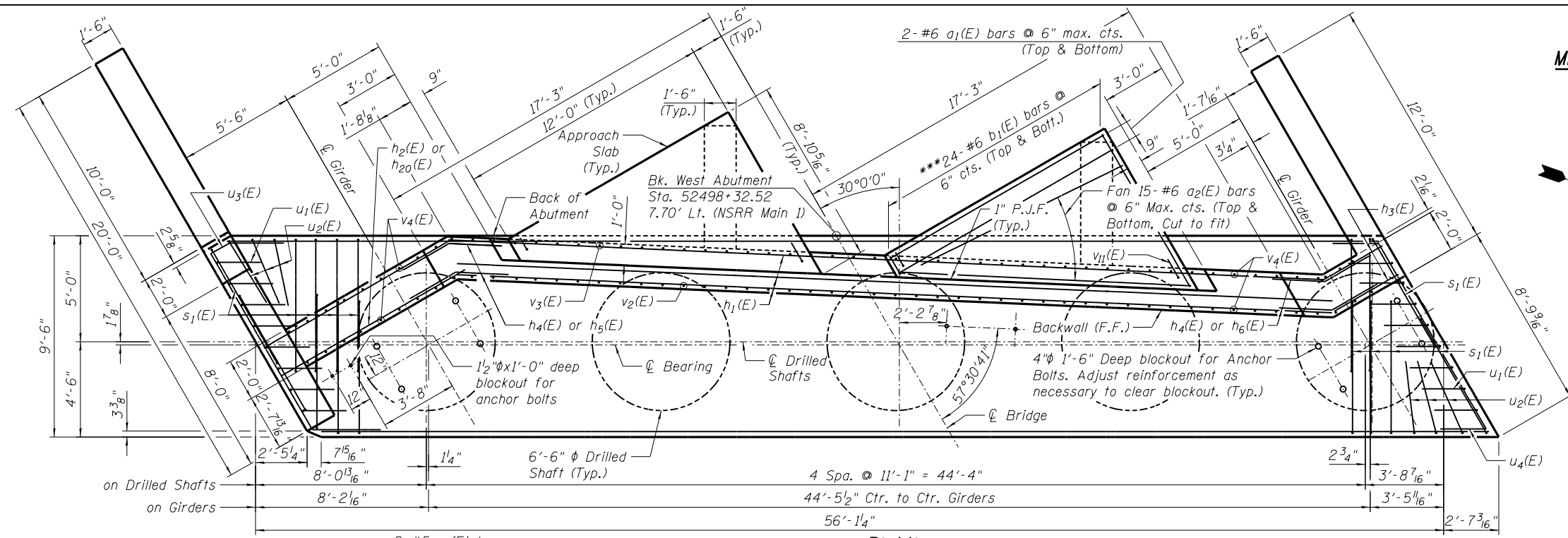
Minimum Bar Lap
 #5 = 2'-2"
 #6 = 3'-1"



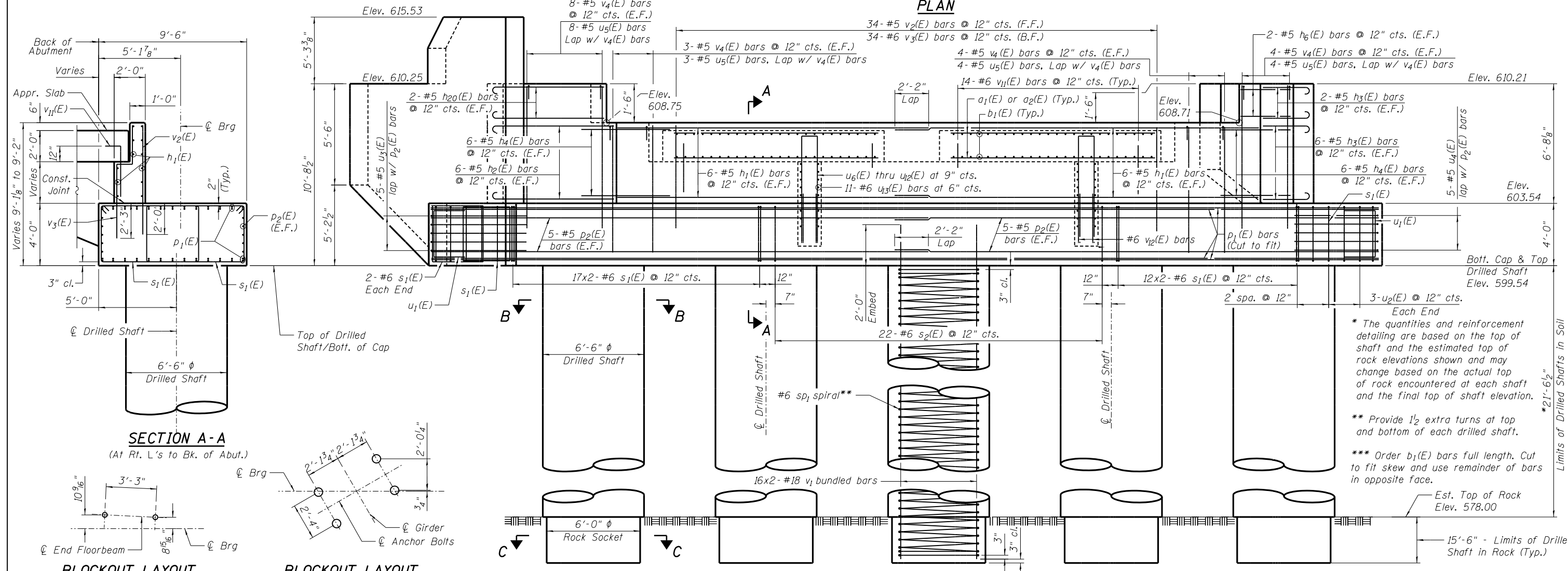
SECTION B-B
 Drilled Shaft



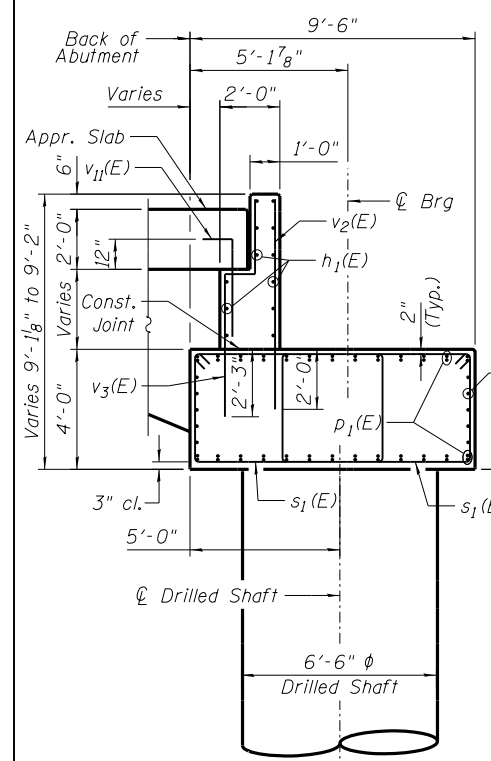
SECTION C-C
 Rock Socket



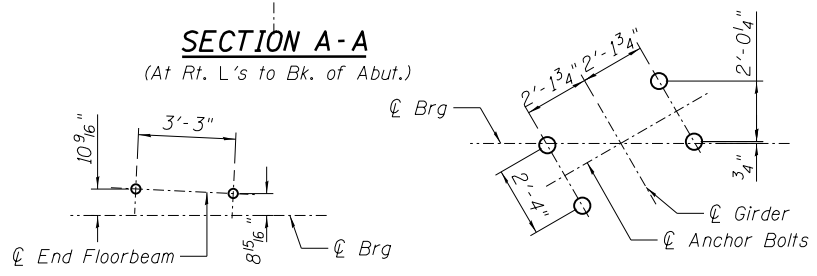
PLAN



ELEVATION - WEST ABUTMENT
 (Looking West)



SECTION A-A
 (At Rt. L's to Bk. of Abut.)



BLOCKOUT LAYOUT
 (At Floorbeam Location)

BLOCKOUT LAYOUT
 (At Girders Locations)

* The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.

** Provide 1/2 extra turns at top and bottom of each drilled shaft.

*** Order b1(E) bars full length. Cut to fit skew and use remainder of bars in opposite face.

Est. Top of Rock Elev. 578.00

15'-6" - Limits of Drilled Shaft in Rock (Typ.)

Limits of Drilled Shafts in Soil

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		CHECKED - MJW	REVISED -
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WEST ABUTMENT
STRUCTURE 084-9963 - 6TH ST NSRR

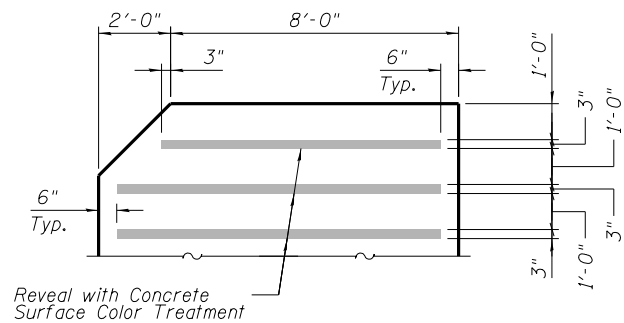
SHEET NO. 23 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	285
CONTRACT NO. 93733				

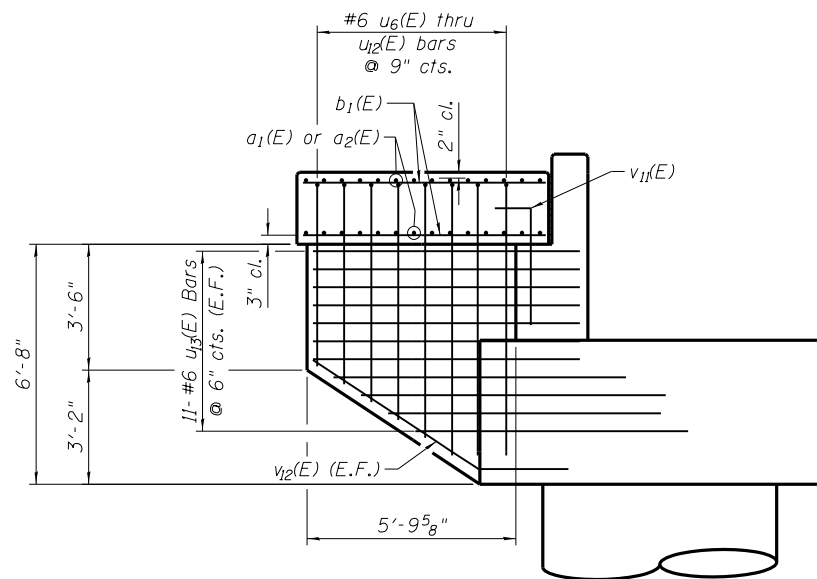
FINAL



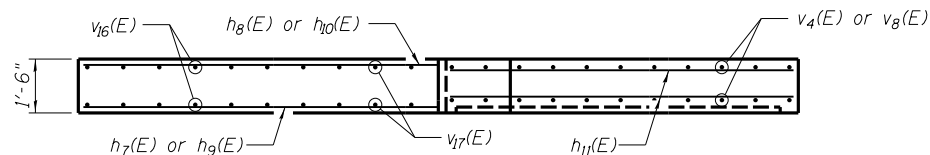
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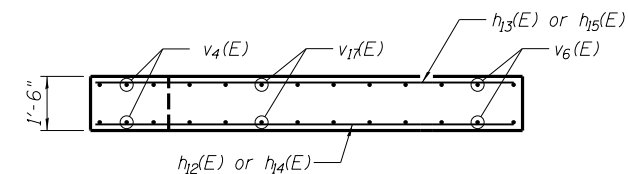
CONCRETE REVEAL DETAIL



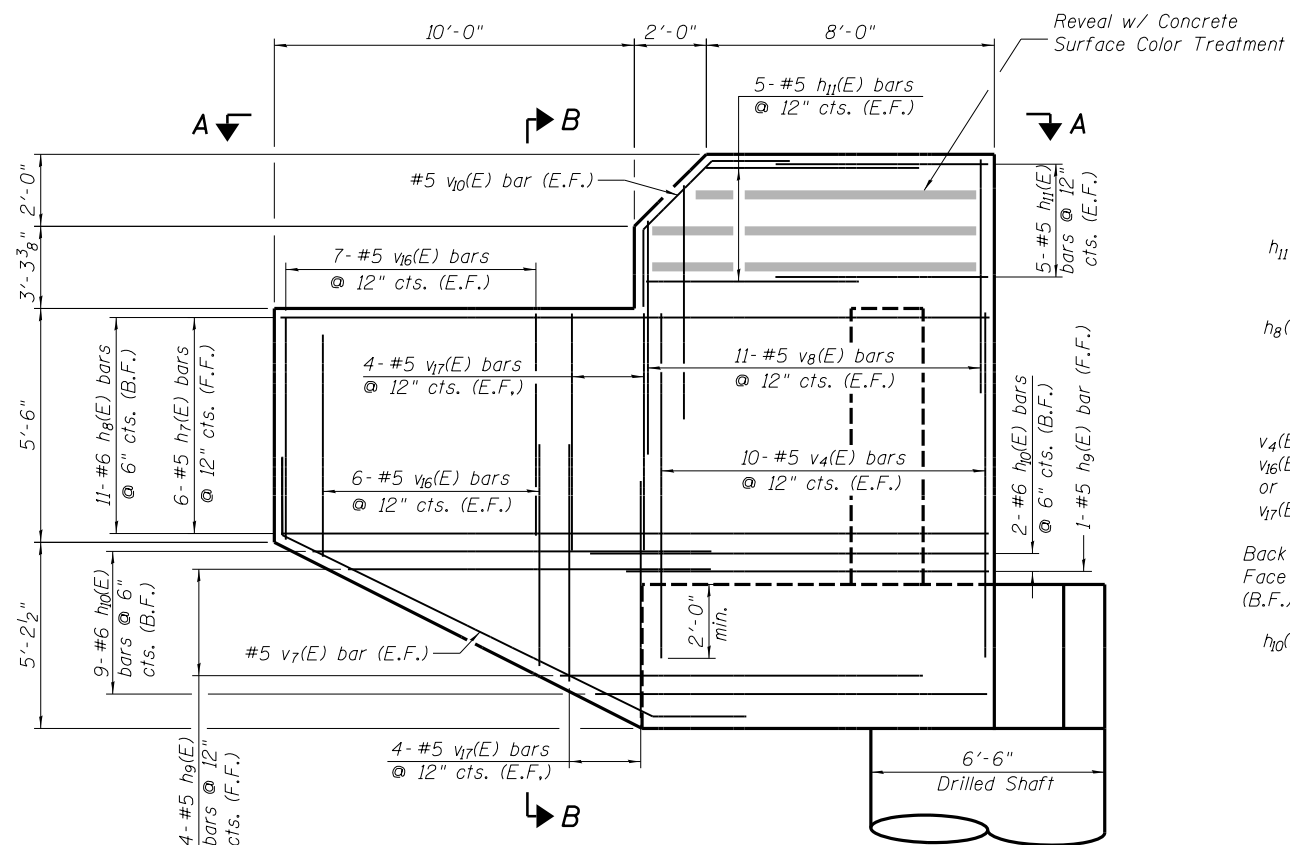
APPROACH SLAB SECTION
(Horizontal Dimensions at Rt. L's to back of abutment.)



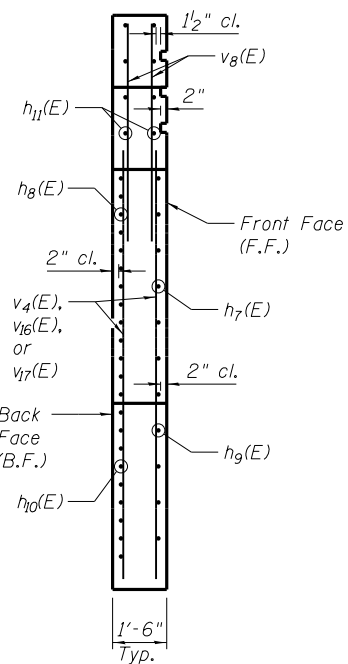
SECTION A-A - PLAN VIEW



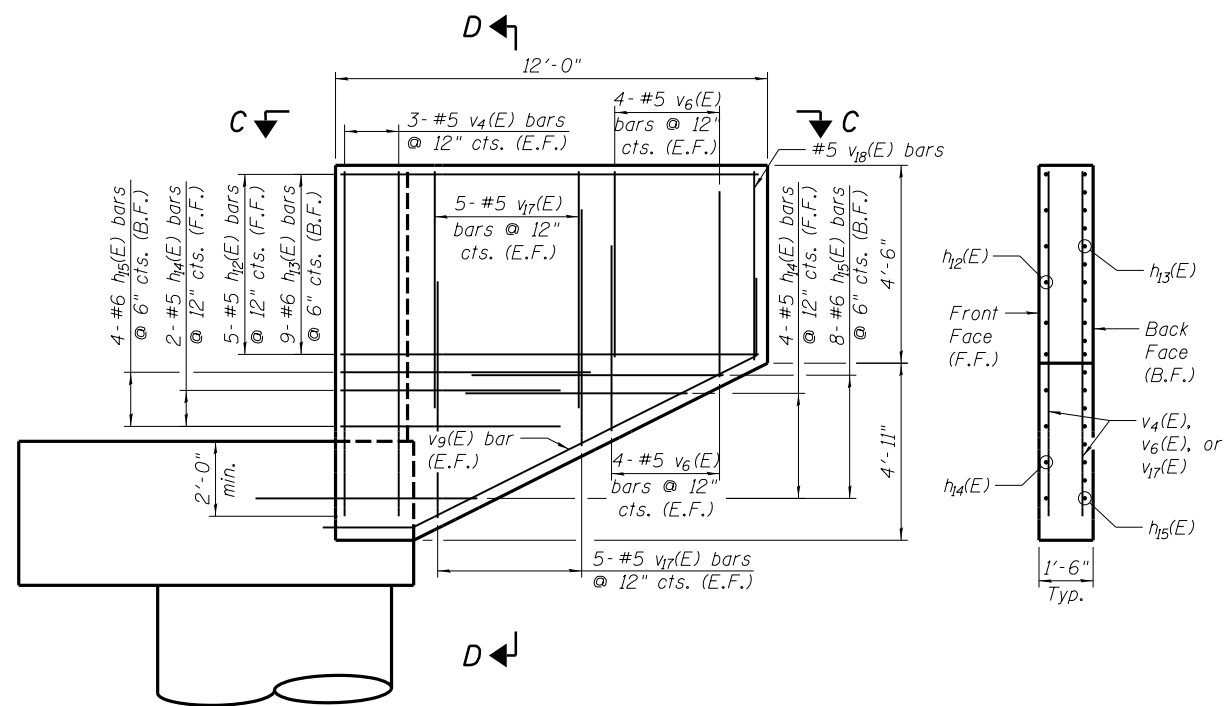
SECTION C-C - PLAN VIEW



ELEVATION - SOUTH WING END VIEW
(Looking North)



WINGWALL SECTION B-B



ELEVATION - NORTH WING END VIEW
(Looking South)

WINGWALL SECTION D-D

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FILE NAME =
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 PLOT DATE = 4/11/2019

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 CHECKED - TJH/TDP
 DRAWN - RSJ
 CHECKED - MJW

REVISED -
 REVISED -
 REVISED -
 REVISED -

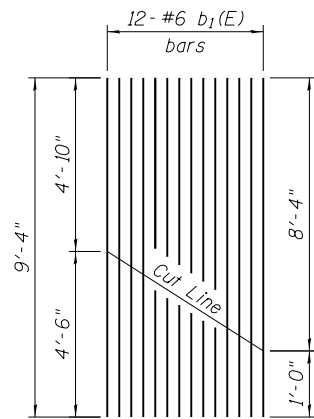
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**WEST ABUTMENT DETAILS
 STRUCTURE 084-9963 - 6TH ST NSRR**

SHEET NO. 24 OF 29 SHEETS

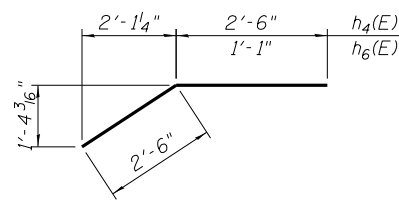
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 666 & 666 ALT.			93733	
ILLINOIS FED. AID PROJECT				

FINAL

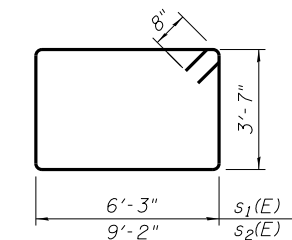


BARS $h_2(E), h_3(E)$ & $h_{20}(E)$

Bar	'a'
$h_2(E)$	10'-1"
$h_3(E)$	3'-8"
$h_{20}(E)$	8'-6"



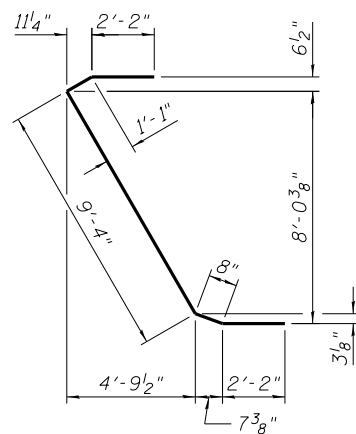
BARS $h_4(E)$ & $h_6(E)$



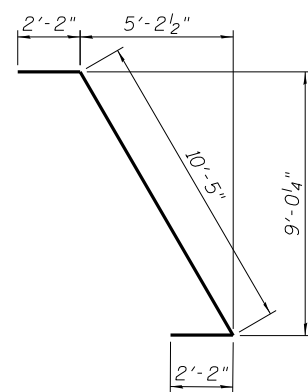
BAR $s_1(E)$ & $s_2(E)$

BAR CUTTING DIAGRAM FOR $b_1(E)$

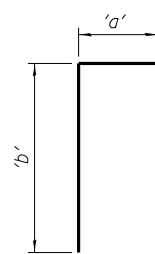
Order $b_1(E)$ full length. Cut as shown and use remainder of bars in opposite face.



BAR $u_3(E)$

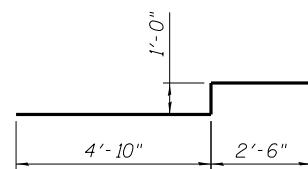


BAR $u_4(E)$

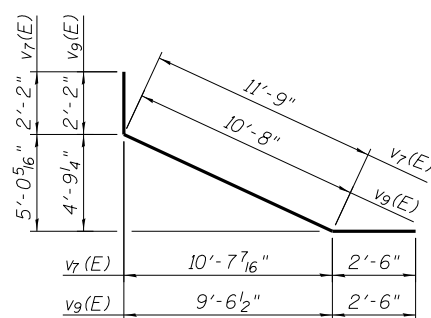


Bar	'a'	'b'
$u_1(E)$	3'-5"	2'-2"
$u_2(E)$	3'-7"	3'-6"
$u_5(E)$	1'-8"	0'-10"
$u_6(E)$	1'-0"	5'-0"
$u_7(E)$	1'-0"	5'-5"
$u_8(E)$	1'-0"	5'-11"
$u_9(E)$	1'-0"	6'-5"
$u_{10}(E)$	1'-0"	6'-11"
$u_{11}(E)$	1'-0"	7'-5"
$u_{12}(E)$	1'-0"	7'-11"

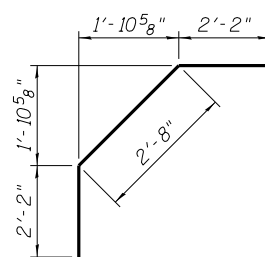
BARS $u_1(E), u_2(E), u_5(E), u_6(E), u_7(E), u_8(E), u_9(E), u_{10}(E), u_{11}(E), u_{12}(E)$



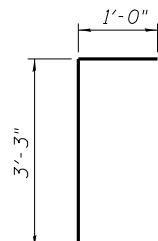
BAR $v_3(E)$



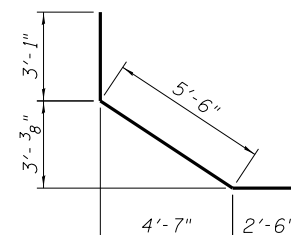
BARS $v_7(E)$ & $v_9(E)$



BARS $v_{10}(E)$



BAR $v_{11}(E)$



BARS $v_{12}(E)$

**BILL OF MATERIAL
WEST ABUTMENT**

Bar	No.	Size	Length	Shape
$a_1(E)$	8	#6	11'-8"	—
$a_2(E)$	60	#6	13'-8"	—
$b_1(E)$	48	#6	9'-4"	—
$h_1(E)$	24	#5	21'-10"	—
$h_2(E)$	18	#5	10'-8"	—
$h_3(E)$	16	#5	4'-3"	—
$h_4(E)$	24	#5	5'-0"	—
$h_6(E)$	4	#5	3'-7"	—
$h_7(E)$	6	#5	19'-8"	—
$h_8(E)$	11	#6	19'-8"	—
$h_9(E)$	5	#5	10'-1"	—
$h_{10}(E)$	11	#6	11'-1"	—
$h_{11}(E)$	20	#5	5'-11"	—
$h_{12}(E)$	5	#5	11'-8"	—
$h_{13}(E)$	9	#6	11'-8"	—
$h_{14}(E)$	6	#5	6'-2"	—
$h_{15}(E)$	12	#6	7'-0"	—
$h_{20}(E)$	4	#5	9'-1"	—
$p_1(E)$	52	#8	55'-8"	—
$p_2(E)$	20	#5	28'-11"	—
$s_1(E)$	64	#6	21'-0"	□
$s_2(E)$	22	#6	26'-10"	□
sp_1	5	#6	*36'-3"	AAA
$u_1(E)$	16	#5	7'-9"	—
$u_2(E)$	6	#5	10'-7"	—
$u_3(E)$	5	#5	15'-5"	—
$u_4(E)$	5	#5	14'-9"	—
$u_5(E)$	19	#5	3'-4"	—
$u_6(E)$	2	#6	11'-0"	—
$u_7(E)$	2	#6	11'-10"	—
$u_8(E)$	2	#6	12'-10"	—
$u_9(E)$	2	#6	13'-10"	—
$u_{10}(E)$	2	#6	14'-10"	—
$u_{11}(E)$	2	#6	15'-10"	—
$u_{12}(E)$	4	#6	16'-10"	—
$u_{13}(E)$	44	#6	7'-5"	—
v_1	160	#18	38'-10"	—
$v_2(E)$	34	#5	7'-1"	—
$v_3(E)$	34	#6	8'-4"	—
$v_4(E)$	64	#5	8'-7"	—
$v_6(E)$	16	#5	4'-8"	—
$v_7(E)$	2	#5	16'-5"	—
$v_8(E)$	22	#5	7'-6"	—
$v_9(E)$	2	#5	15'-4"	—
$v_{10}(E)$	2	#5	7'-0"	—
$v_{11}(E)$	28	#6	4'-3"	—
$v_{12}(E)$	4	#6	11'-1"	—
$v_{16}(E)$	26	#5	5'-2"	—
$v_{17}(E)$	36	#5	6'-2"	—
$v_{18}(E)$	2	#5	4'-3"	—
Structure Excavation		Cu. Yds.	116	
Concrete Structures		Cu. Yds.	128.0	
Drilled Shaft in Soil		Cu. Yds.	132.4	
Drilled Shaft in Rock		Cu. Yds.	81.2	
Reinforcement Bars		Pound	103,730	
Reinforcement Bars, Epoxy Coated		Pound	18,920	

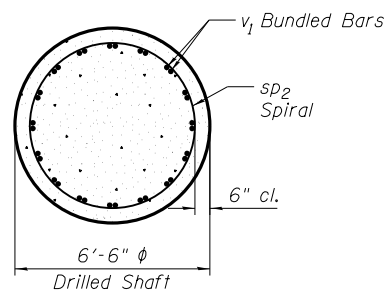
* Length is height of spiral.

MIN. BAR LAPS FOR SPIRALS

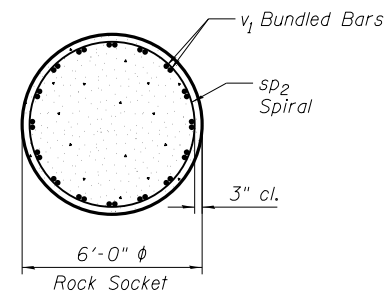
#6 Bars = 2'-7"

USER NAME = Pop00275	DESIGNED - MJW	REVISED -
PLOT SCALE = 0:2.0000 ' = 1"	CHECKED - TJH/TDP	REVISED -
PLOT DATE = 4/11/2019	DRAWN - RSJ	REVISED -
	CHECKED - MJW	REVISED -

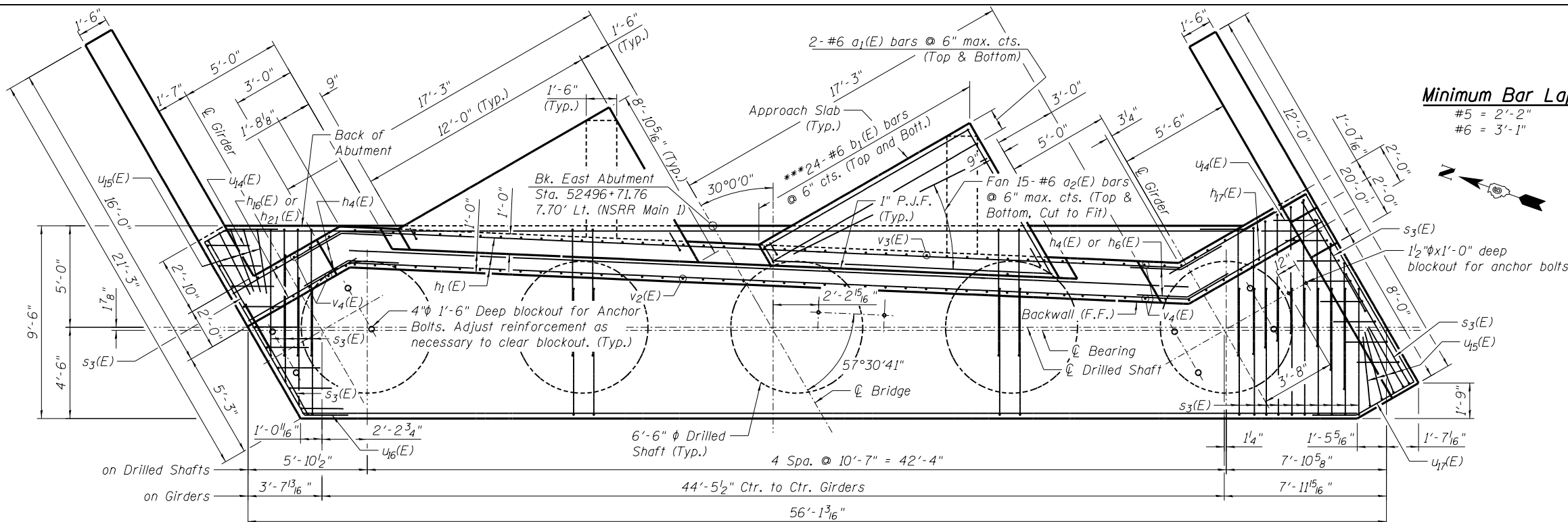
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 93733	
*666 & 666 ALT. ILLINOIS FED. AID PROJECT				



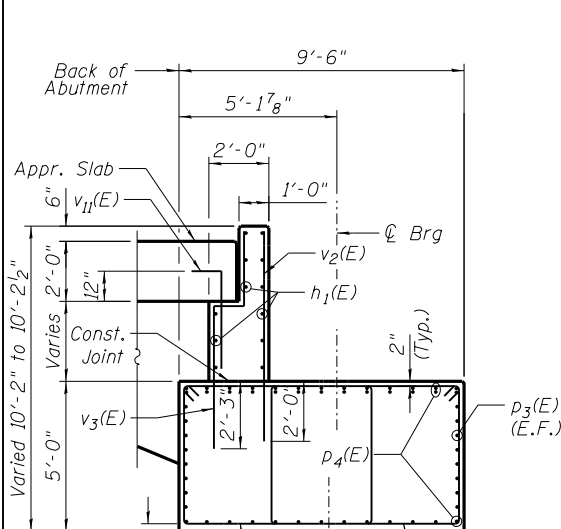
SECTION B-B



SECTION C-C

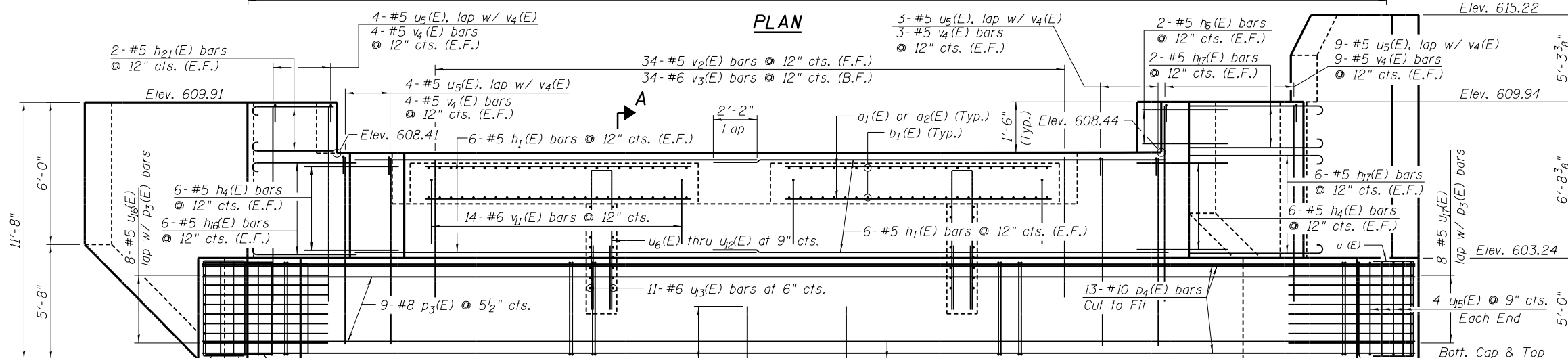


Minimum Bar Lap
 #5 = 2'-2"
 #6 = 3'-1"



SECTION A-A

(At Rt. L's to Bk. of Abut.)



ELEVATION - EAST ABUTMENT

(Looking East)

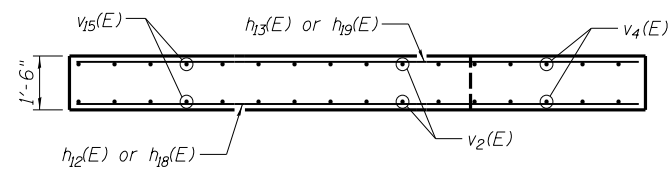
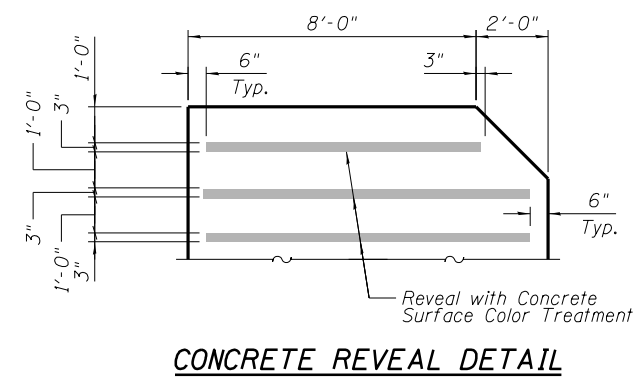
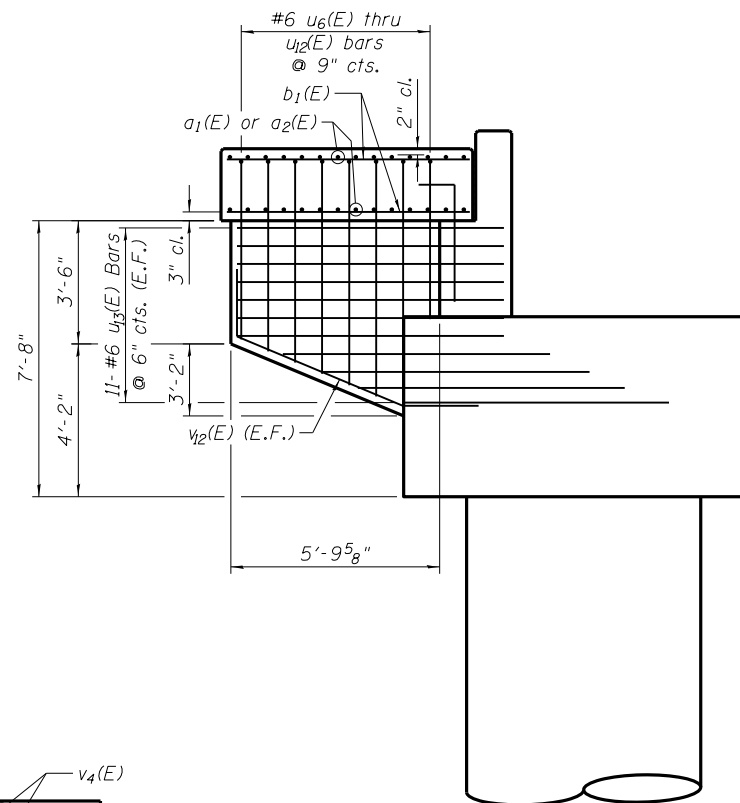
B
 * The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.
 ** Provide 1/2 extra turns at top and bottom of each drilled shaft.
 *** Order b1(E) bars full length. Cut to fit skew and use remainder of bars in opposite face.

BLOCKOUT LAYOUT

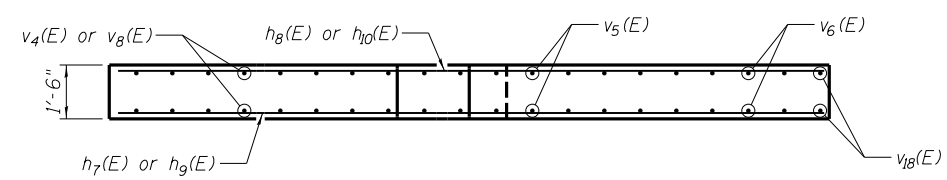
(At End Floorbeam Location)

BLOCKOUT LAYOUT

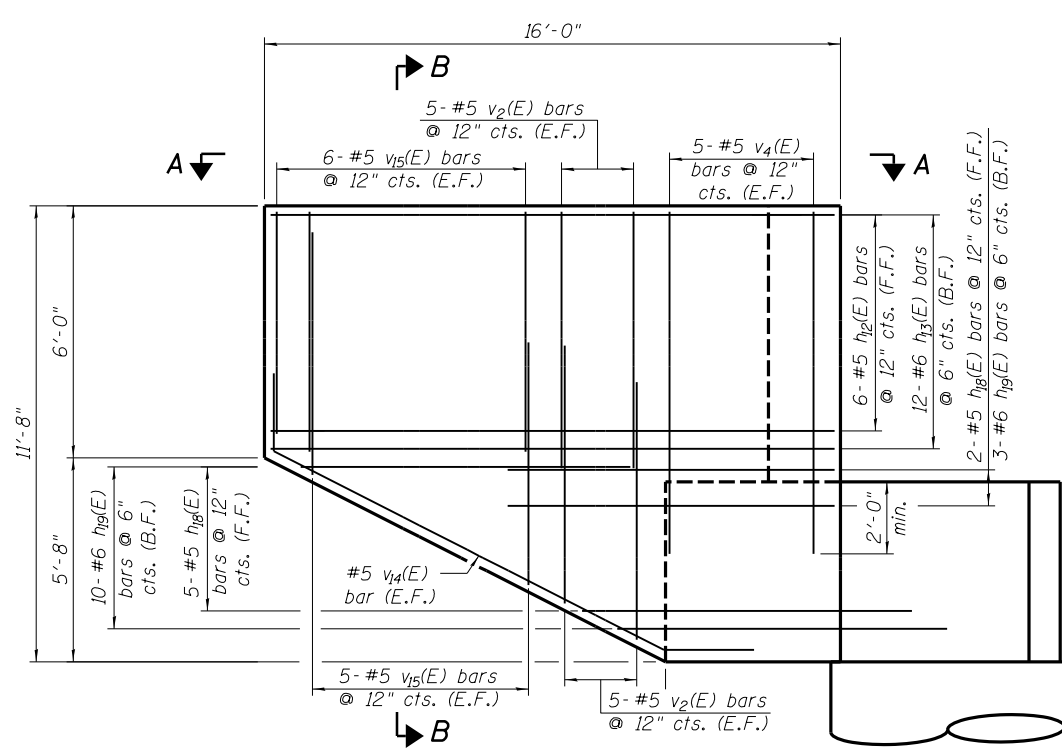
(At Girders Locations)



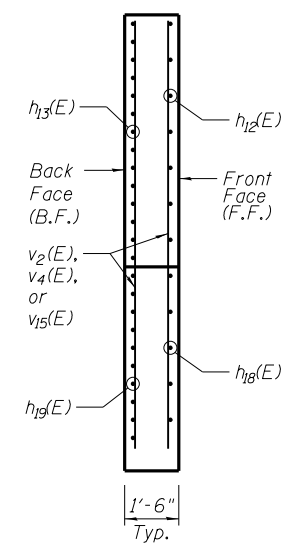
APPROACH SLAB SECTION
(Horizontal Dimensions at Rt. L's to back of abutment.)



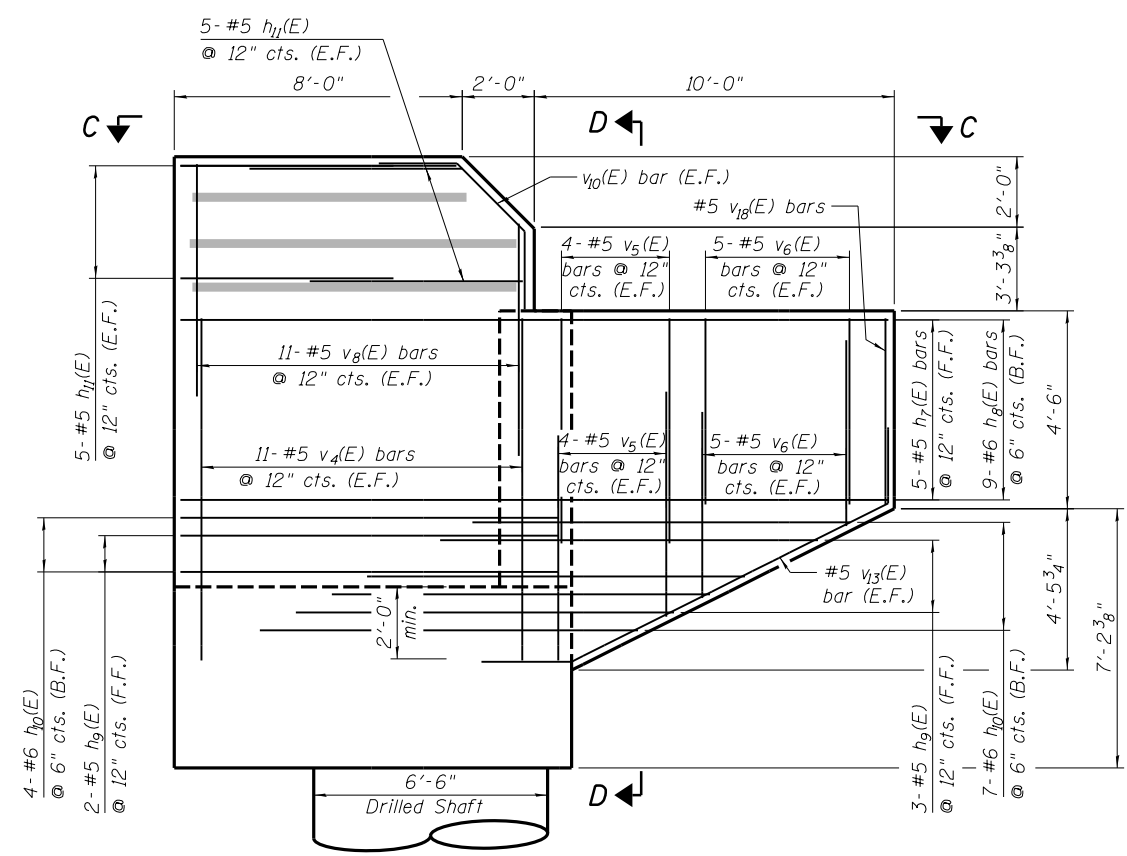
SECTION C-C - PLAN VIEW



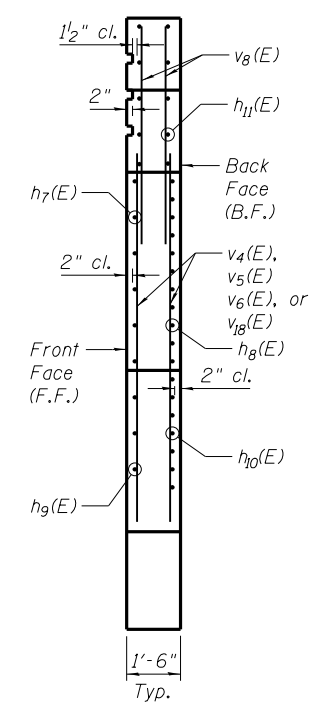
ELEVATION - NORTH WING END VIEW
(Looking South)



WINGWALL SECTION B-B



ELEVATION - SOUTH WING END VIEW
(Looking North)



WINGWALL SECTION D-D

dw:\sps\svr\306.hanson.dom\hanson_projects\documents\09jobs\09L01798\CAD\Structure\6th\Sheet\0849963-09L01798-NSRR-001

FILE NAME =	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
		DRAWN - RSJ	REVISED -
		CHECKED - MJW	REVISED -
PLOT SCALE = @2.0000 1' / in.			
PLOT DATE = 4/11/2019			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EAST ABUTMENT DETAILS
STRUCTURE 084-9963 - 6TH ST NSRR

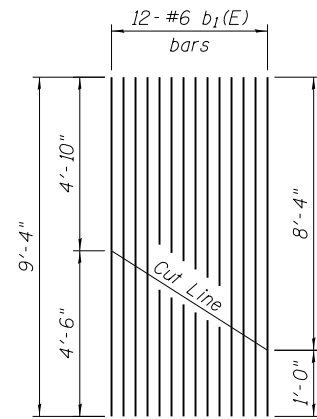
SHEET NO. 27 OF 29 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	289
CONTRACT NO. 666 & 666 ALT.			93733	
ILLINOIS FED. AID PROJECT				

FINAL

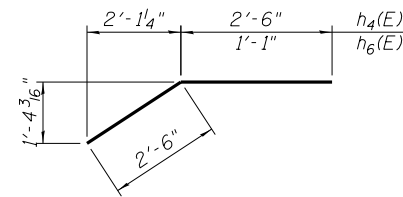


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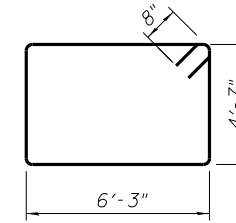


BARS $h_{16}(E)$, $h_{17}(E)$ & $h_{21}(E)$

Bar	'a'
$h_{16}(E)$	6'-2"
$h_{17}(E)$	7'-8"
$h_{21}(E)$	4'-7"



BARS $h_4(E)$ & $h_6(E)$



BAR $s_3(E)$

**BILL OF MATERIAL
EAST ABUTMENT**

Bar	No.	Size	Length	Shape
$a_1(E)$	8	#6	11'-8"	—
$a_2(E)$	60	#6	13'-8"	—
$b_1(E)$	48	#6	9'-4"	—
$h_1(E)$	24	#5	21'-10"	—
$h_4(E)$	24	#5	5'-0"	—
$h_6(E)$	4	#5	3'-7"	—
$h_7(E)$	5	#5	19'-8"	—
$h_8(E)$	9	#6	19'-8"	—
$h_9(E)$	5	#5	10'-1"	—
$h_{10}(E)$	11	#6	11'-1"	—
$h_{11}(E)$	20	#5	5'-11"	—
$h_{12}(E)$	6	#5	15'-8"	—
$h_{13}(E)$	12	#6	15'-8"	—
$h_{16}(E)$	12	#5	6'-9"	—
$h_{17}(E)$	16	#5	8'-3"	—
$h_{18}(E)$	7	#5	8'-8"	—
$h_{19}(E)$	13	#6	9'-1"	—
$h_{21}(E)$	4	#5	5'-2"	—
$p_3(E)$	18	#8	55'-8"	—
$p_4(E)$	39	#10	55'-8"	—
$s_3(E)$	142	#6	23'-0"	□
sp_2	5	#6	*35'-0"	⋈
$u_5(E)$	20	#5	3'-4"	┌
$u_6(E)$	2	#6	11'-0"	┌
$u_7(E)$	2	#6	11'-10"	┌
$u_8(E)$	2	#6	12'-10"	┌
$u_9(E)$	2	#6	13'-10"	┌
$u_{10}(E)$	2	#6	14'-10"	┌
$u_{11}(E)$	2	#6	15'-10"	┌
$u_{12}(E)$	4	#6	16'-10"	┌
$u_{13}(E)$	44	#6	7'-5"	┌
$u_{14}(E)$	18	#5	8'-9"	┌
$u_{15}(E)$	8	#5	11'-7"	┌
$u_{16}(E)$	8	#5	15'-3"	┌
$u_{17}(E)$	8	#5	21'-9"	┌
v_1	160	#18	38'-10"	—
$v_2(E)$	54	#5	7'-1"	—
$v_3(E)$	34	#6	8'-4"	—
$v_4(E)$	72	#5	8'-7"	—
$v_5(E)$	16	#5	5'-9"	—
$v_6(E)$	20	#5	4'-8"	—
$v_8(E)$	22	#5	7'-6"	—
$v_{10}(E)$	2	#5	7'-0"	—
$v_{11}(E)$	28	#6	4'-3"	—
$v_{12}(E)$	4	#6	11'-1"	—
$v_{13}(E)$	2	#5	14'-6"	—
$v_{14}(E)$	2	#5	16'-10"	—
$v_{15}(E)$	22	#5	5'-8"	—
$v_{18}(E)$	2	#5	4'-3"	—
Structure Excavation		Cu. Yds.	130	
Concrete Structures		Cu. Yds.	149.7	
Drilled Shaft in Soil		Cu. Yds.	124.4	
Drilled Shaft in Rock		Cu. Yds.	81.2	
Reinforcement Bars		Pound	103,060	
Reinforcement Bars, Epoxy Coated		Pound	25,610	

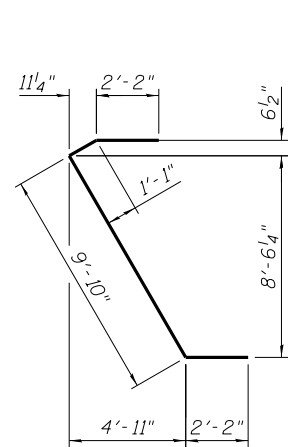
* Length is height of spiral.

MIN. BAR LAPS FOR SPIRALS

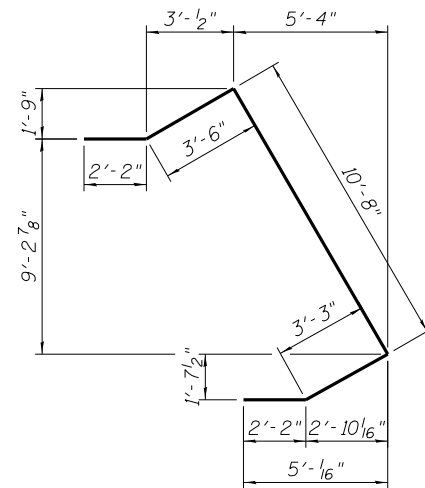
#6 Bars = 2'-7"

BAR CUTTING DIAGRAM FOR $b_1(E)$

Order $b_1(E)$ full length. Cut as shown and use remainder of bars in opposite face.



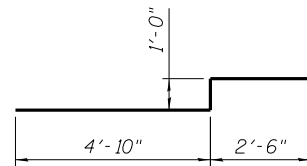
BAR $u_{16}(E)$



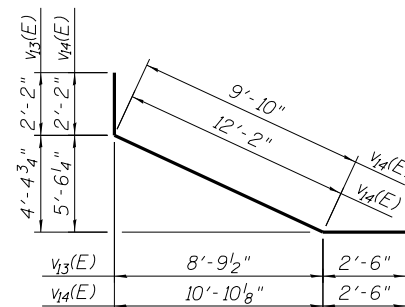
BAR $u_{17}(E)$

Bar	'a'	'b'
$u_5(E)$	1'-8"	0'-10"
$u_6(E)$	1'-0"	5'-0"
$u_7(E)$	1'-0"	5'-5"
$u_8(E)$	1'-0"	5'-11"
$u_9(E)$	1'-0"	6'-5"
$u_{10}(E)$	1'-0"	6'-11"
$u_{11}(E)$	1'-0"	7'-5"
$u_{12}(E)$	1'-0"	7'-11"
$u_{14}(E)$	4'-5"	2'-2"
$u_{15}(E)$	4'-7"	3'-6"

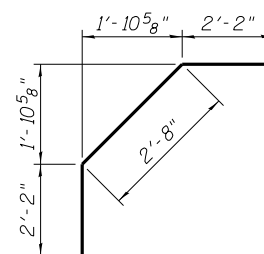
BARS $u_5(E)$, $u_6(E)$, $u_7(E)$, $u_8(E)$, $u_9(E)$, $u_{10}(E)$, $u_{11}(E)$, $u_{12}(E)$, $u_{14}(E)$, $u_{15}(E)$



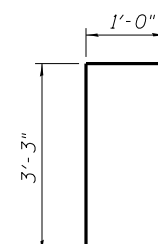
BAR $v_3(E)$



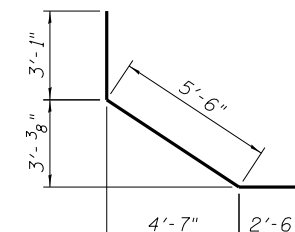
BAR $v_{13}(E)$ & $v_{14}(E)$



BARS $v_{10}(E)$



BAR $v_{11}(E)$



BARS $v_{12}(E)$

B-145
Sta. 998+21.66' LT
9/5/13

Elevation	N	Qu	w%	Description
601.0				TOPSOIL
600.04	8	4.50P	15	Brown very fine sandy clayey SILT, some brick and rock fragments - FILL.
	12	4.50P	16	
595.04	12	3.00P	21	Brown and gray very fine sandy SILT.
	8	1.44B	23	
590.04	7	3.00P	24	Brown very fine sandy SILT, some clay.
587.54	5	0.58B	26	Dark gray very fine sandy silty CLAY.
585.04	5	1.03B	24	Gray very fine sandy silty CLAY, trace small gravel.
	5	0.70B	22	
577.54	63	4.50P	16	Brown and gray SHALE. (HIGHLY WEATHERED SHALE)
572.54	50/4"		9	Gray SHALE.
	50/5"		8	
566.04	Rec. = 77% RQD = 73% Rec. = 90% RQD = 56%			Gray sandy SHALE, micaceous.
562.54	11.3 Rec. = 90% Rec. = 99% RQD = 68%	RQD = 48%		Gray clayey SHALE.
558.04				Gray sandy SHALE, micaceous.
556.04	Rec. = 100% RQD = 46% Rec. = 67% RQD = 0%			COAL.
551.54				Bottom of Hole = 49.5 feet

B-146
Sta. 1000+74.15' RT
9/11/13

Elevation	N	Qu	w%	Description
587.0				ASPHALT.
586.61				CONCRETE.
585.86	4		24	Dark gray very fine sandy silty CLAY.
583.53	4	0.66B	25	Blue-gray very fine to fine sandy silty CLAY.
	6	2.47S	19	
578.53	57	4.50P	14	Brown and gray SHALE. (HIGHLY WEATHERED SHALE)
576.03	50	4.50P	11	Gray SHALE.
572.03	50/5"		11	Gray clayey SHALE, micaceous.
	Rec. = 81% RQD = 19% Rec. = 88% RQD = 71%			
	12.7 Rec. = 75% Rec. = 85% RQD = 51%	RQD = 44%		
	21.9 Rec. = 91% RQD = 78%			
556.5	Rec. = 100% RQD = 78%			Stiff to very stiff gray shaley CLAY.
553.5				Gray sandy SHALE, micaceous.
553.0				COAL.
552.0				Bottom of Hole = 35.0 feet

LEGEND

- N Standard Penetration Test N (blows/ft)
- Qu Unconfined Strength (tsf)
- w% Natural Moisture Content (%)

DD Water Surface Elevation Encountered in Boring
 558.10 DD = during drilling
 Oh = at completion
 24h = 24 hours after completion

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FILE NAME =	USER NAME = Pop00275	DESIGNED - MJW	REVISED -
		CHECKED - TJH/TDP	REVISED -
		DRAWN - RSJ	REVISED -
		CHECKED - MJW	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**SUBSURFACE DATA PROFILE
STRUCTURE 084-9963 - 6TH ST NSRR**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	291
			CONTRACT NO. 93733	

SHEET NO. 29 OF 29 SHEETS

•666 & 666 ALT. ILLINOIS FED. AID PROJECT

FINAL

HANSON
© Copyright Hanson Professional Services Inc., 2019

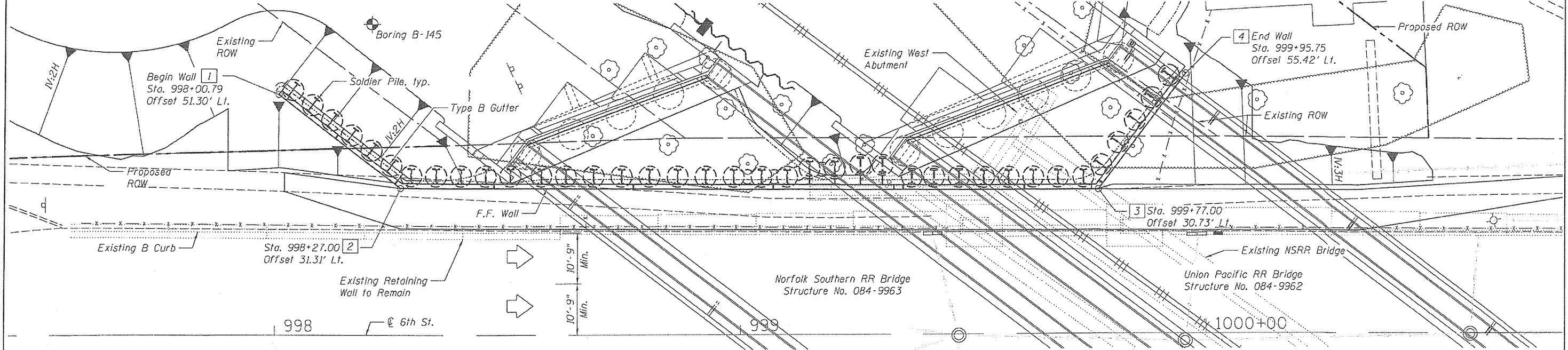
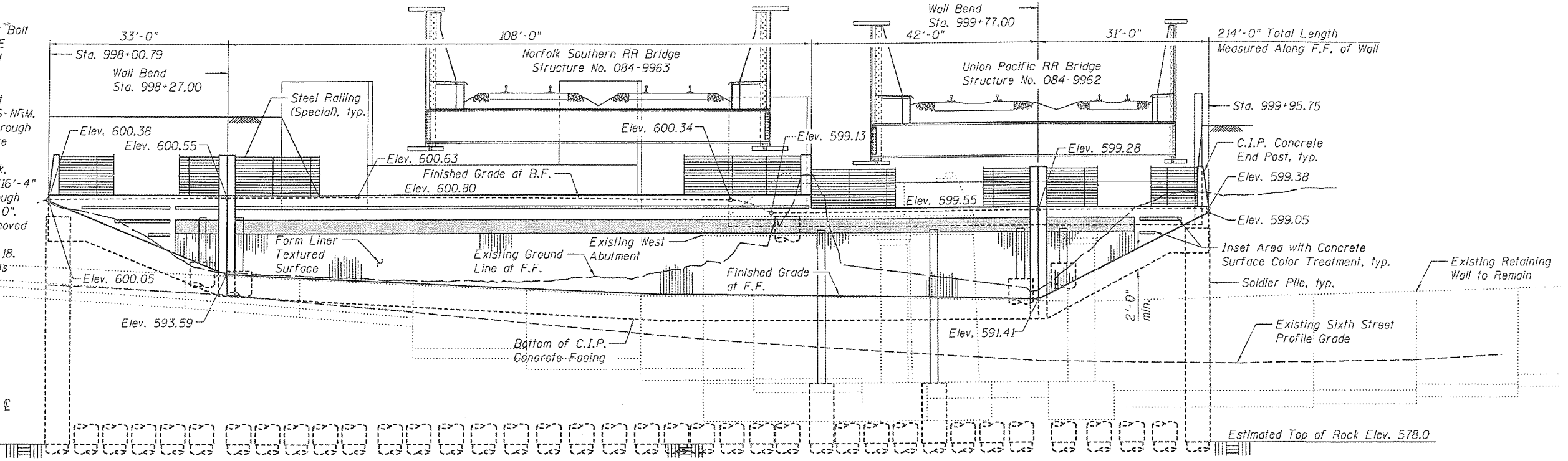
Benchmark:
BM# D2218-07 - Chiseled 'X' on West Bolt of fire hydrant - SE Quad 6th Street and Wellesly Avenue. Elevation = 598.884

Existing Structure: SN 084-9901 - Built in 1934 under 109-S-NRM. Three Span Steel through plate girder structure supported on closed abutments. Bk. to Bk. Abutment length is 116'-4" and ctr. to ctr. through girder width of 20'-0". Structure to be Removed and Replaced.

Construction Sequence: See Sheet 3 of 18.
Traffic Control: Temporary Lane Closures
Salvage: None

Note: Wall offsets are measured from @ 6th Street to the front face of C.I.P. Facing.

F.F. - Front Face
B.F. - Back Face
[2] - Control Point



EXISTING PROFILE GRADE SIXTH STREET Along @ of Sixth St.

Sta. 997+00	Elev. 595.17
Sta. 997+50	Elev. 594.18
Sta. 998+00	Elev. 592.72
Sta. 998+50	Elev. 590.75
Sta. 999+00	Elev. 588.37
Sta. 999+50	Elev. 586.05
Sta. 1000+00	Elev. 585.14
Sta. 1000+50	Elev. 586.13
Sta. 1001+00	Elev. 588.33
Sta. 1001+50	Elev. 590.77
Sta. 1002+00	Elev. 592.57
Sta. 1002+50	Elev. 593.88
Sta. 1003+00	Elev. 595.17

APPROVED
For Structural Adequacy Only

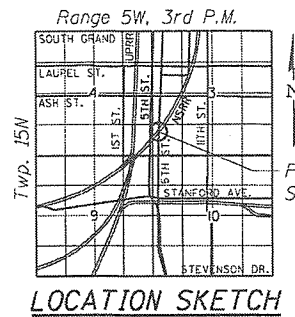
Robert Chantone
Engineer of Bridges & Structures

ROBERT CHANTONE
081-00048
STATE OF ILLINOIS
Expires November 30, 2020

I certify that to the best of my knowledge, information and belief, this retaining wall design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AREMA Specifications.

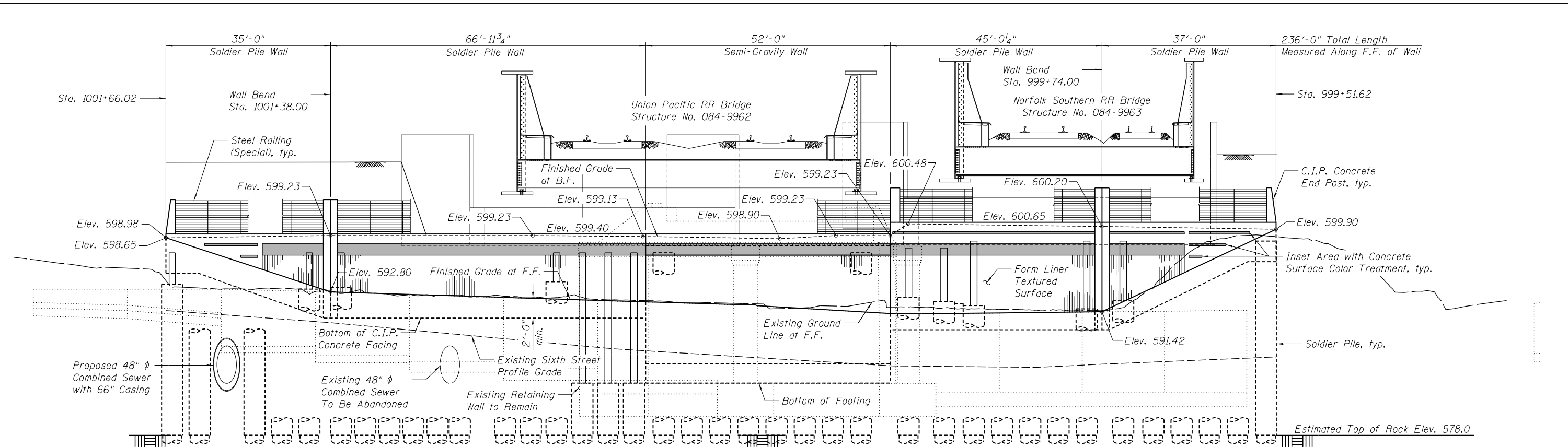
DESIGN SPECIFICATIONS
2017 AREMA Specifications

DESIGN STRESSES
FIELD UNITS
f'c = 4,000 psi
fy = 60,000 psi (Reinforcement)
fy = 50,000 psi (M270 Grade 50)

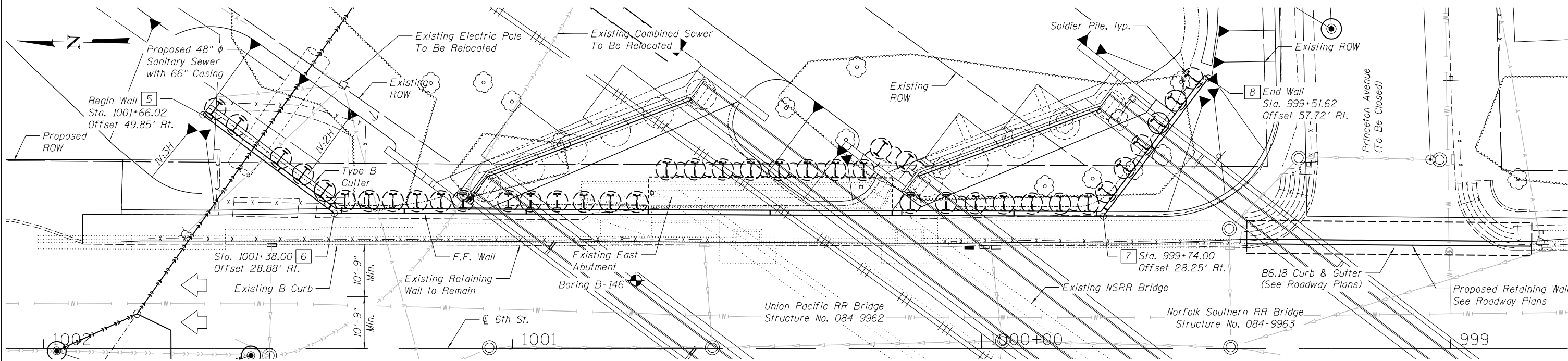


WEST WALL GENERAL PLAN & ELEVATION
6TH ST. RETAINING WALLS
F.A.P. 666-SECTION (109)VB, (110)VB-5
SANGAMON COUNTY
STATION 998+00.79 TO 1001+66.02

HANSON USER NAME = Pop82275 PLOT SCALE = 0.1667' / 1" = PLOT DATE = 4/18/2019	DESIGNED - RGC CHECKED - KMS DRAWN - EJM CHECKED - RGC	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL PLAN & ELEVATION - WEST WALL RETAINING WALLS - 6TH STREET SHEET NO. 1 OF 18 SHEETS	F.A.P. SECTION COUNTY TOTAL SHEETS SHEET NO. (109) VB, (110) VB-5 SANGAMON 382 292 CONTRACT NO. 93733 ILLINOIS FED. AID PROJECT
---	---	--	---	--	--



UNFOLDED ELEVATION
 (Looking @ F.F. of East Wall)



PLAN

Note: Wall offsets are measured from ϕ 6th Street to the front face of C.I.P. Facing.

F.F. - Front Face
 B.F. - Back Face
 [6] - Control Point

EAST WALL GENERAL PLAN & ELEVATION
6TH ST. RETAINING WALLS
F.A.P. 666-SECTION (109)VB, (110)VB-5
SANGAMON COUNTY
STATION 998+00.79 TO 1001+66.02

px:\sps\svr\306.hanson.dom\hanson_projects\Documents\09\Jobs\09L01798\CAD\Struct\6th\Sheet\09L01798-6thRetainingWallPlans

USER NAME = Pop00275	DESIGNED - RGC	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL PLAN & ELEVATION - EAST WALL
RETAINING WALLS - 6TH STREET

SHEET NO. 2 OF 18 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	293
			CONTRACT NO. 93733	

ILLINOIS FED. AID PROJECT

FINAL



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WALL CONTROL POINTS

Control Point	Station	Offset
1	998+00.79	51.30' LT
2	998+27.00	31.31' LT
3	999+77.00	30.73' LT
4	999+95.75	55.42' LT
5	1001+66.02	49.85' RT
6	1001+38.00	28.88' RT
7	999+74.00	28.25' RT
8	999+51.62	57.72' RT

Control Points are to Front Face of C.I.P. Facing.

GENERAL NOTES

- Reinforcement bars designated (E) shall be epoxy coated.
- All substructure concrete shall have a compressive strength of 4,000 psi at 14 days.
- The Contractor is responsible for the design and performance of the Untreated Timber Lagging using no less than a 3 in. nominal rough-sawn thickness and timber with a minimum allowable bending stress of 1000 psi.

INDEX OF SHEETS

1. General Plan & Elevation - West Wall
2. General Plan & Elevation - East Wall
3. General Data
4. Typical Sections
5. Typical Sections
6. Soldier Piles - West Wall
7. Soldier Piles - East Wall
8. Concrete Facing - West Wall
9. Concrete Facing - West Wall
10. Concrete Facing - East Wall
11. Concrete Facing - East Wall
12. Concrete Facing - East Wall
13. Concrete Facing Details
14. Concrete Facing Details
15. Railing Details
16. Railing Details
17. Slope Wall Details
18. Subsurface Data Profile

CONSTRUCTION SEQUENCE

Stage 1: Maintain rail traffic on existing track.

Item 4: NSRR Bridge and south ends of retaining walls

- Drill and set Soldier Piles 1-5 of the East Retaining Wall, in location of Jacked-In-Place Sanitary Sewer. Install Sanitary Sewer.
- Drill and place the Secant Lagging to existing ground surface for the West Retaining Wall between Soldier Piles 19-23 and for the East Retaining Wall between Soldier Piles 25-32.
- Drill and set Soldier Pile 25 and Temporary Soldier Piles A & B of the East Retaining Wall.
- Drill and set Soldier Piles 1-24 of the West Retaining Wall and Soldier Piles 26-38 of the East Retaining Wall. Drill through footing of existing East Abutment wingwall as required.
- Install timber lagging while excavating in front of soldier piles to bottom of facing and filling behind soldier piles to bottom of new abutments.
- Install drilled shafts for the West and East Abutments.
- Remove conflicting portion of existing East Abutment wingwall.
- Construct cast-in-place concrete abutments.
- Install pipe underdrain and cast-in-place concrete facing panels W1-W5 and E10-E11.
- Place fill behind new abutments and between new abutments and retaining walls.
- Set bridge superstructure during weekend closure of 6th Street.
- Complete bridge superstructure, including roadway luminaires. Complete Stage 1 railroad embankment and subballast placement.
- NSRR places ballast and shifts tracks to Temporary NSRR Main 1 (outside position on new bridge).

Stage 4A: Maintain Rail traffic on Temporary NSRR Main 1.

Item 5: UPRR Bridge and north ends of retaining walls

- Remove existing bridge superstructure during weekend closure of 6th Street.
- Drill and place the Secant Lagging to existing ground surface for the East Retaining Wall between Soldier Piles 18-25.
- Drill and set Soldier Pile 25 of the West Retaining Wall and Soldier Piles 18-24 of the East Retaining Wall.
- Excavate around existing abutments using previously installed soldier piles to retain railroad embankment near active track.
- Remove existing abutment and wingwall stems to top of existing footing. Install timber lagging between Soldier Piles 23-25 of the West Retaining Wall to retain embankment while removing south end of existing West Abutment. Remove existing footings only where they conflict with new soldier piles or drilled shafts.
- Drill and set Soldier Piles 26-38 of the West Retaining Wall and Soldier Piles 6-17 of the East Retaining Wall.
- Construct semi-gravity wall panels E6-E7.
- Install timber lagging while excavating in front of soldier piles to bottom of facing and filling behind soldier piles to bottom of abutments.
- Install drilled shafts for the new abutments. Construct cast-in-place concrete abutments.
- Install pipe underdrain and cast-in-place concrete facing panels W6-W9, E1-E5, and E8-E9.
- Place fill behind new abutments and between new abutments and retaining walls.
- Set bridge superstructure during weekend closure of 6th Street.
- Complete bridge superstructure. Complete Stage 4A railroad embankment and subballast placement.
- NSRR installs tracks on NSRR Main 1 (inside position on new bridge).

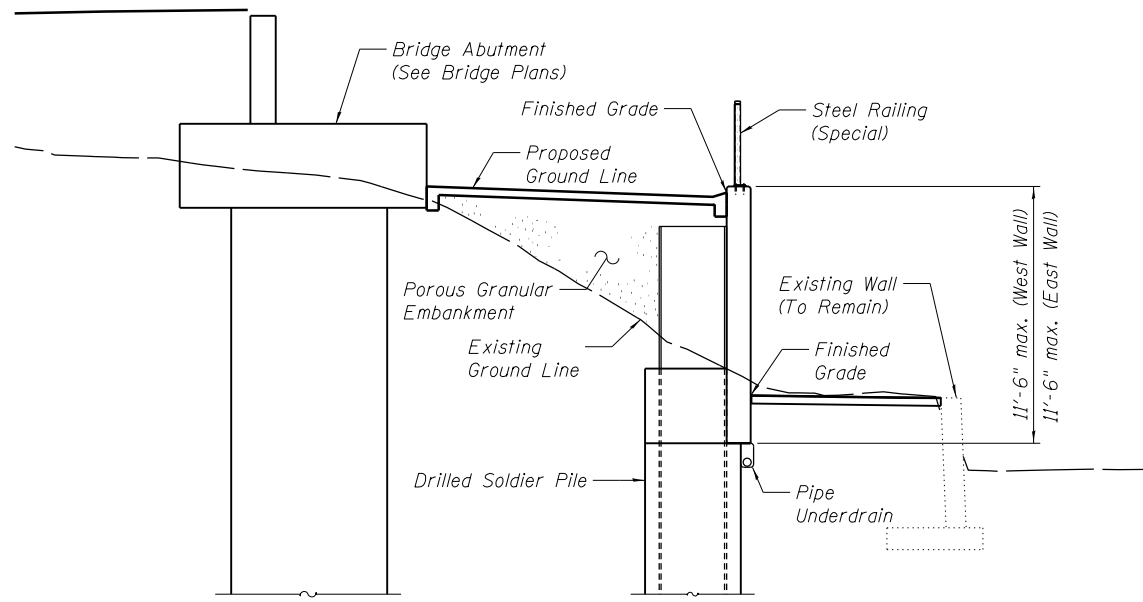
TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Porous Granular Embankment	Cu. Yd.	1267
Structure Excavation	Cu. Yd.	395
Form Liner Textured Surface	Sq. Ft.	2785
Stud Shear Connectors	Each	399
Reinforcement Bars, Epoxy Coated	Pound	28560
Slope Wall 4 Inch	Sq. Yd.	301
Furnishing Soldier Piles (W-Section)	Foot	2943
Drilling and Setting Soldier Piles (in Soil)	Cu. Ft.	21193.0
Drilling and Setting Soldier Piles (in Rock)	Cu. Ft.	17326.8
Untreated Timber Lagging	Sq. Ft.	2017
Secant Lagging	Cu. Ft.	1945
Concrete Structures (Retaining Wall)	Cu. Yd.	217.7
Concrete Sealer	Sq. Ft.	3959
Geocomposite Wall Drain	Sq. Yd.	160
Concrete Gutter, Type B	Foot	65
Concrete Surface Color Treatment	Sq. Ft.	514
Steel Railing (Special)	Foot	426
Pipe Underdrains for Structures 4"	Foot	597

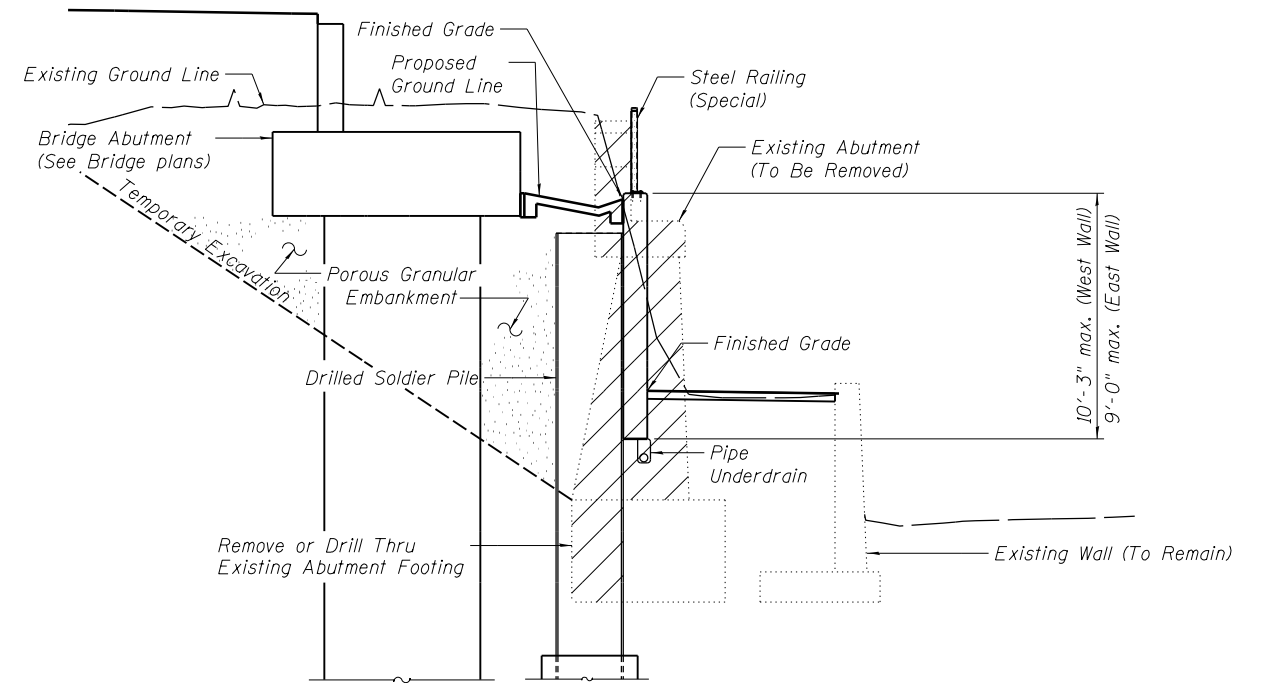


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PLOT SCALE = 0.1667' / in.	DRAWN - EJM	REVISED -
PLOT DATE = 4/11/2019	CHECKED - RGC	REVISED -

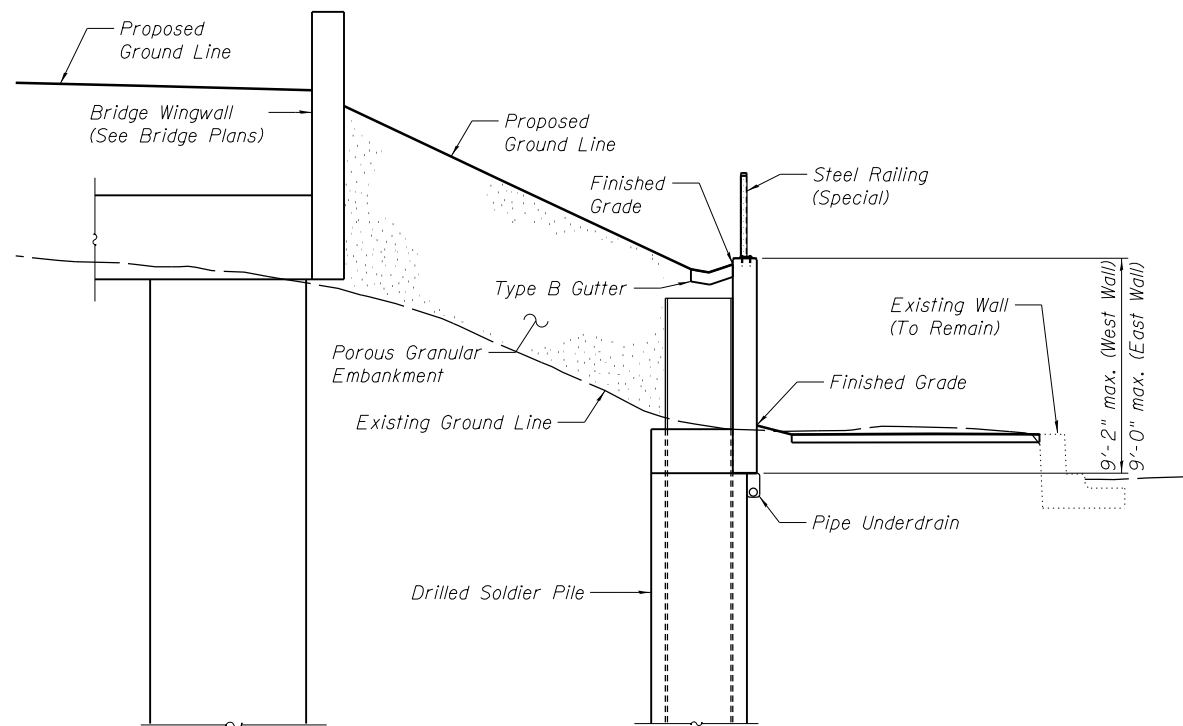
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
•	(109) VB,(110) VB-5	SANGAMON	382	294
			CONTRACT NO.	93733
ILLINOIS FED. AID PROJECT				



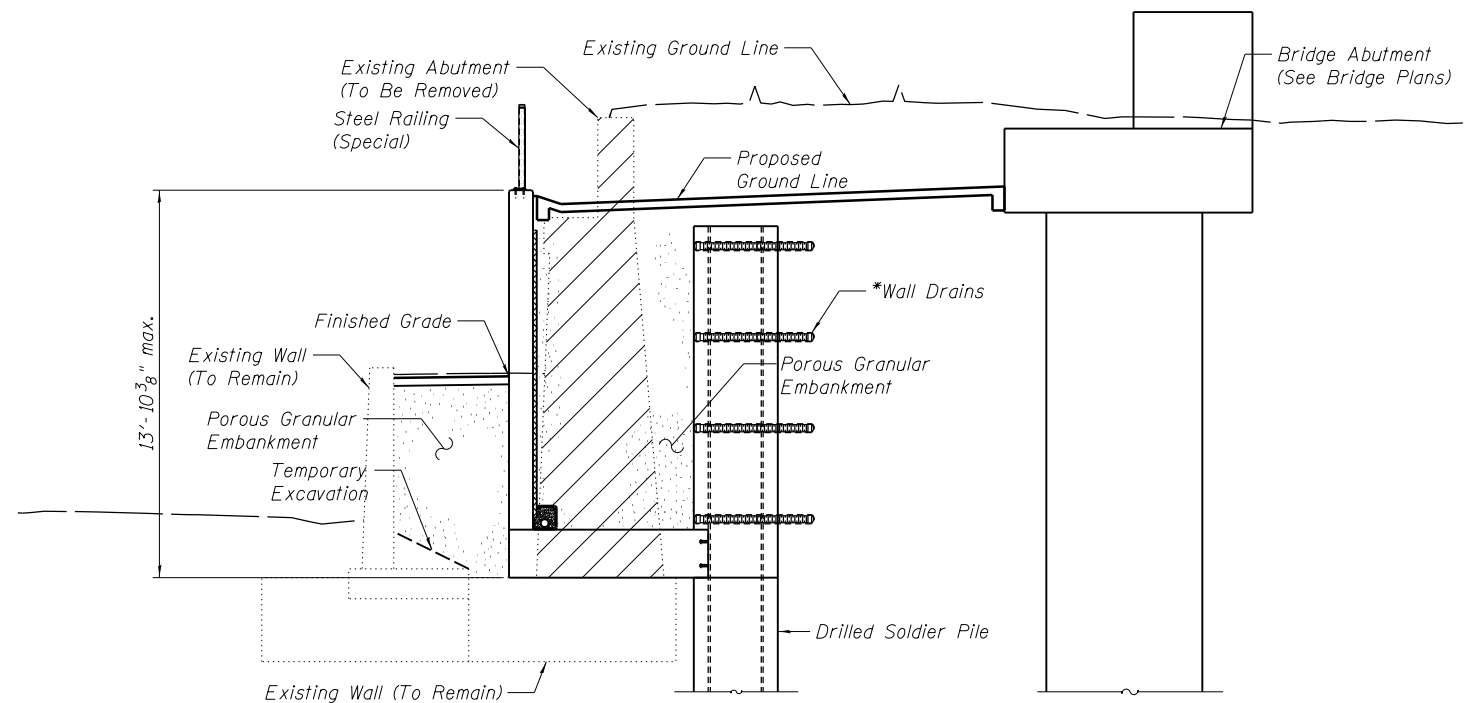
TYPICAL WALL SECTION
 Except East Wall Sta. 1000+19.00 to 1000+71.00



WALL SECTION WITH TEMPORARY EXCAVATION
 West Wall Sta. 999+35 to 999+60±
 East Wall Sta. 1000+71 to 1000+85±



WALL SECTION PARALLEL TO RAILROAD
 West Wall Sta. 998+00.79 to 998+27.00
 East Wall Sta. 1001+38.00 to 1001+66.02



SEMI-GRAVITY WALL SECTION
 East Wall Sta. 1000+19.00 to 1000+71.00
 * Included In The Cost of Secant Lagging.

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PLOT SCALE = 0.1667' / 1"	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
	CHECKED - RGC	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

TYPICAL SECTIONS
 RETAINING WALLS - 6TH STREET

SHEET NO. 4 OF 18 SHEETS

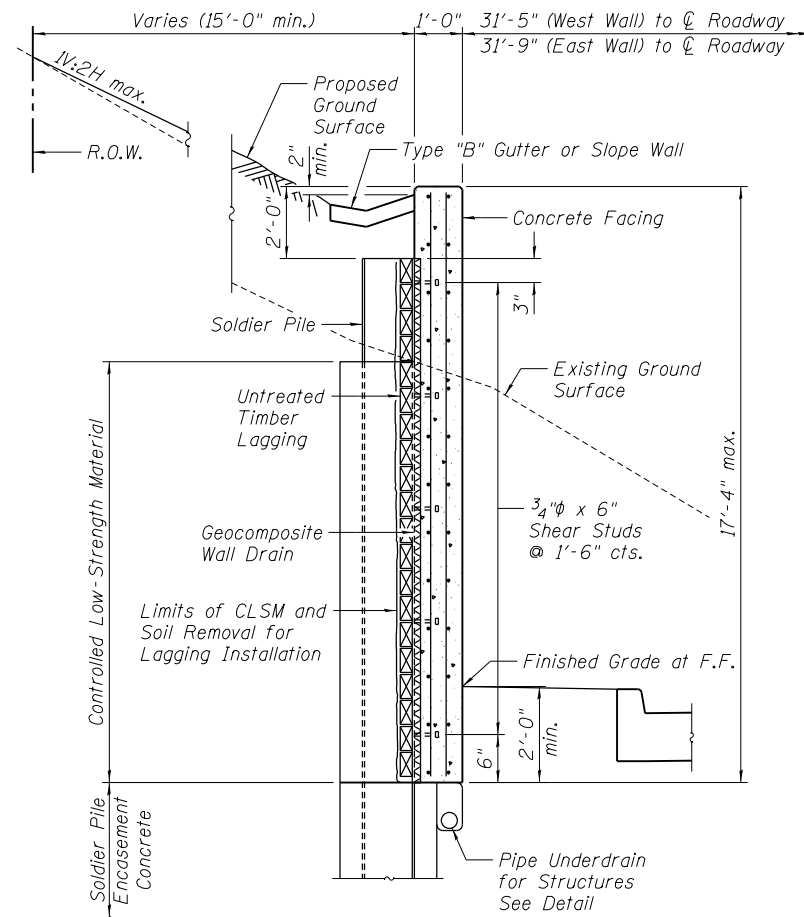
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			CONTRACT NO.	93733

ILLINOIS FED. AID PROJECT

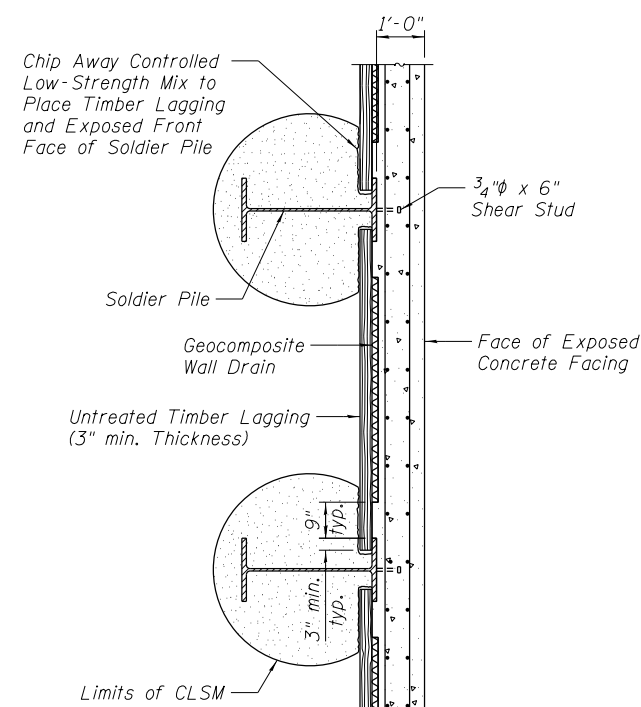
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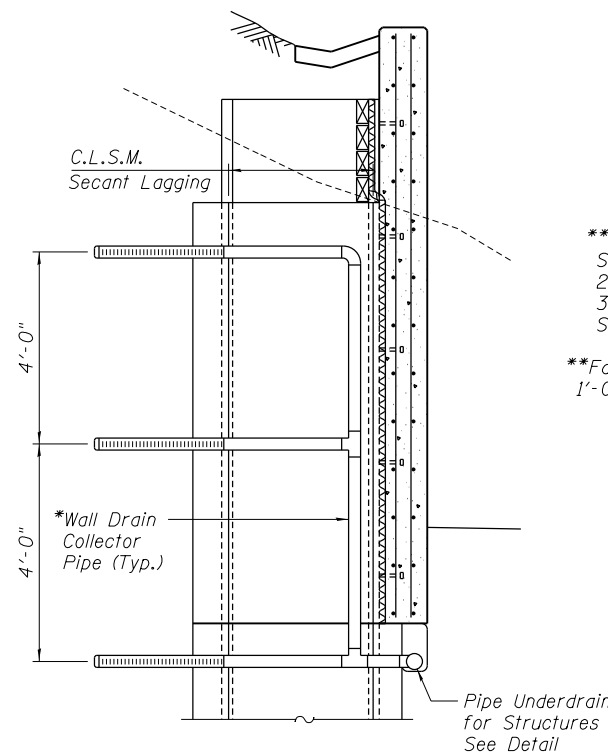
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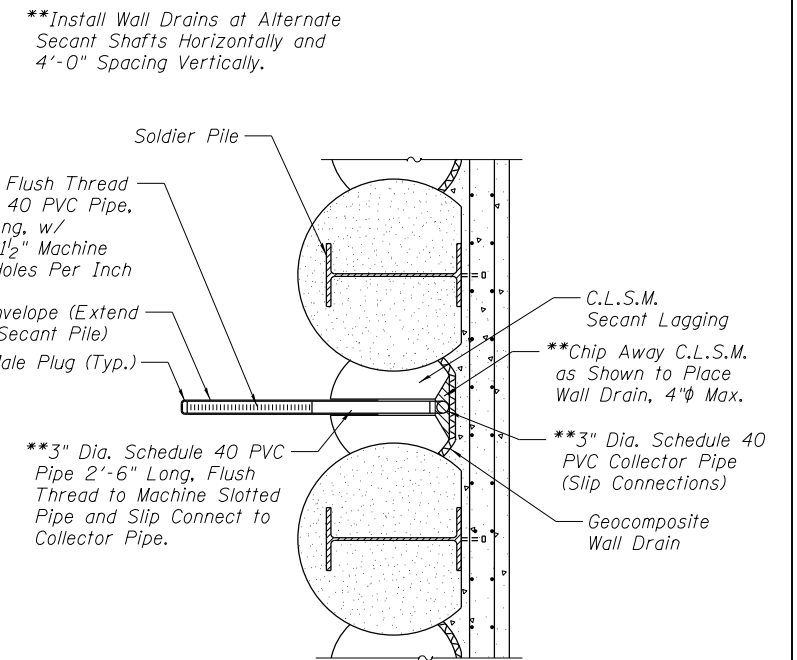
SECTION THRU DRILLED SOLDIER PILE WALL WITH ENCASMENT AND C.I.P. FACING



SECTION THRU DRILLED SOLDIER PILE WALL

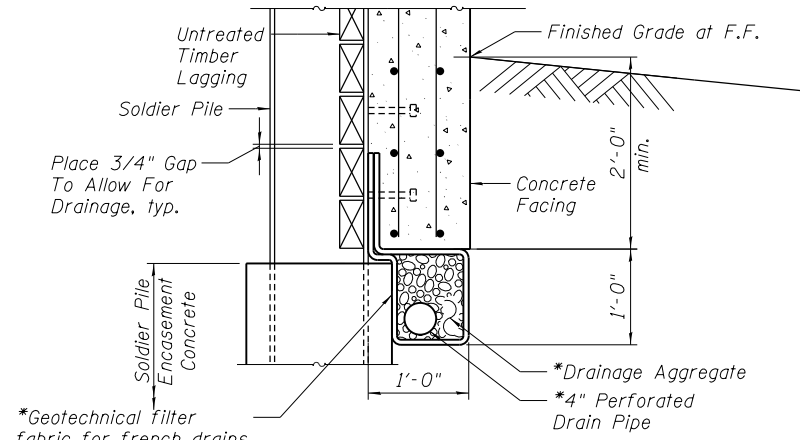


SECTION THRU DRILLED SOLDIER PILE WALL WITH SECANT LAGGING

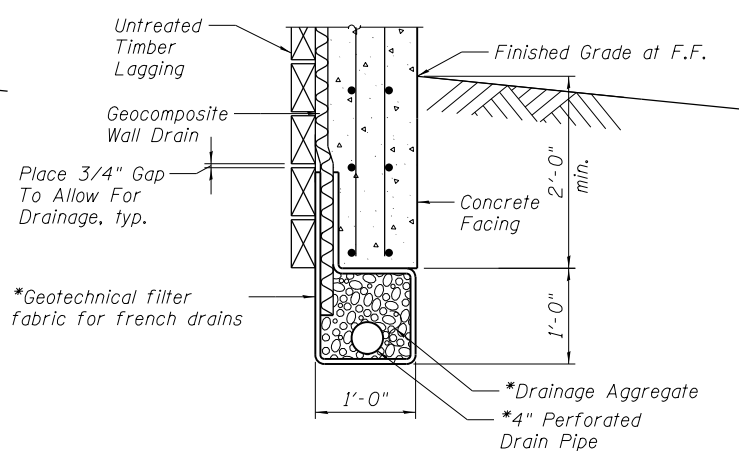


SECTION THRU SECANT LAGGING

** Included In The Cost of Secant Lagging.



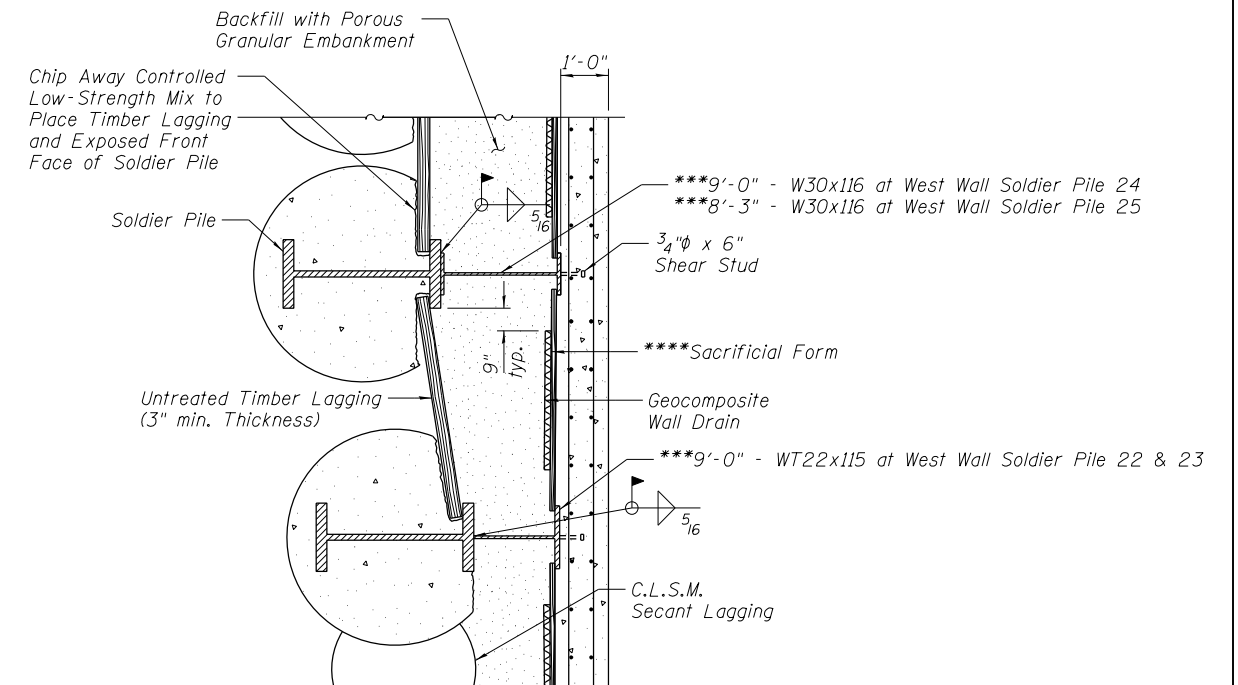
AT SOLDIER PILES



BETWEEN SOLDIER PILES

UNDERDRAIN DETAIL FOR SOLDIER PILE WALLS

*Included in the Cost of Pipe Underdrains for Structures, 4".



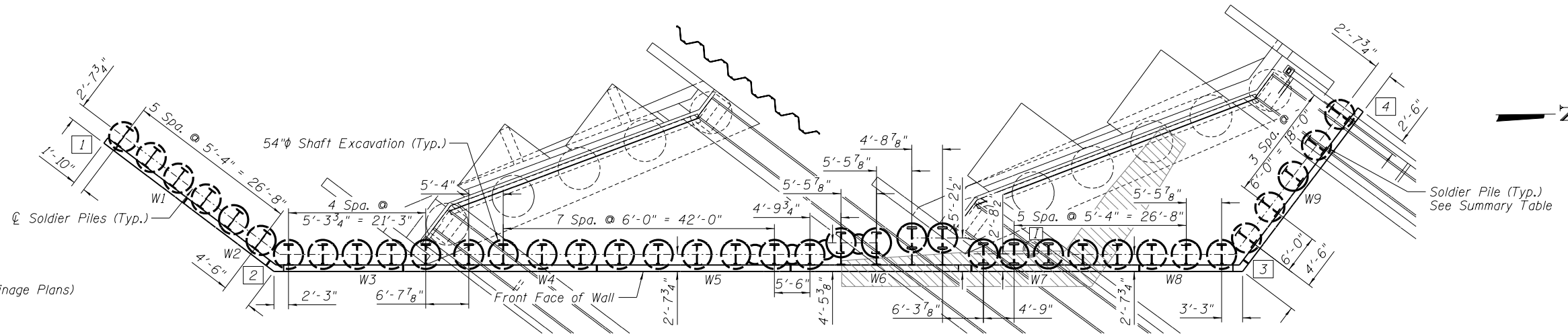
SECTION AT OFFSET FACING

***Included in the Cost of Furnishing Soldier Piles (W Section).
****Included in the Cost of Concrete Structures (Retaining Wall).

USER NAME = Pop00275	DESIGNED - RGC	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISED -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISED -
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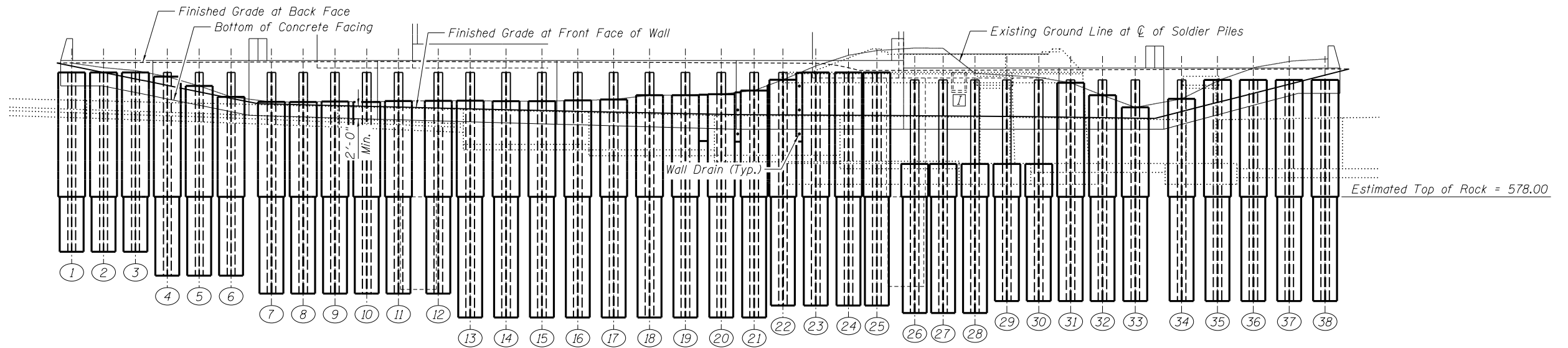
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*	(109) VB,(110) VB-5	SANGAMON	382	296
			CONTRACT NO. 93733	
ILLINOIS FED. AID PROJECT				

1 Inlet (See Drainage Plans)



PLAN

Note: All Dimensions are Measured Along Front Face of Wall



ELEVATION

Unfolded Along Face of Wall

SOLDIER PILE SUMMARY

PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.	PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.
1	W40x249	30'-0"	568.80	598.80	14	W36x487	41'-0"	557.80	598.80
2	W40x249	30'-0"	568.80	598.80	15	W36x487	41'-0"	557.80	598.80
3	W40x249	30'-0"	568.80	598.80	16	W36x487	41'-0"	557.80	598.80
4	W40x249	34'-0"	564.80	598.80	17	W36x487	41'-0"	557.80	598.80
5	W40x249	34'-0"	564.80	598.80	18	W36x487	41'-0"	557.80	598.80
6	W40x249	34'-0"	564.80	598.80	19	W36x487	41'-0"	557.80	598.80
7	W36x487	37'-0"	561.80	598.80	20	W36x487	41'-0"	557.80	598.80
8	W36x487	37'-0"	561.80	598.80	21	W36x487	41'-0"	557.80	598.80
9	W36x487	37'-0"	561.80	598.80	22	W36x487	39'-0"	559.80	598.80
10	W36x487	37'-0"	561.80	598.80	23	W36x487	39'-0"	559.80	598.80
11	W36x487	37'-0"	561.80	598.80	24	W36x652	39'-0"	559.80	598.80
12	W36x487	37'-0"	561.80	598.80	25	W36x652	39'-0"	558.55	597.55
13	W36x487	41'-0"	557.80	598.80	26	W36x652	39'-0"	558.55	597.55
					27	W36x652	39'-0"	558.55	597.55
					28	W36x652	39'-0"	558.55	597.55
					29	W36x487	37'-0"	560.55	597.55
					30	W36x487	37'-0"	560.55	597.55
					31	W36x487	37'-0"	560.55	597.55
					32	W36x487	37'-0"	560.55	597.55
					33	W36x487	37'-0"	560.55	597.55
					34	W36x487	39'-0"	558.55	597.55
					35	W36x487	39'-0"	558.55	597.55
					36	W36x487	39'-0"	558.55	597.55
					37	W36x487	39'-0"	558.55	597.55
					38	W36x487	39'-0"	558.55	597.55

SECANT LAGGING SUMMARY

BETWEEN PILES NO.	DIAMETER	LENGTH	BOTTOM ELEV.	TOP ELEV.
19-20	36"	7'-9"	587.32	595.07
20-21	36"	8'-3"	587.22	595.47
21-22	36"	9'-3"	587.42	596.67
22-23	36"	11'-6"	587.23	598.73

**WEST WALL
STUD SHEAR CONNECTORS REQUIRED**

Pile No.	Number Required on Each Pile
1	2
2-3	3
4	4
5-6	5
7-12	6
13-24	7
25-34	6
35	5
36	4
37	3
38	2

2 = Control Point

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Stud Shear Connectors	Each	216
Furnishing Soldier Piles (W Section)	Foot	1426
Drilling and Setting Soldier Piles (in Soil)	Cu. Ft.	10023.2
Drilling and Setting Soldier Piles (in Rock)	Cu. Ft.	8207.1
Untreated Timber Lagging	Sq. Ft.	1104
Secant Lagging	Cu. Ft.	260

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USER NAME = Pop00275	DESIGNED - RGC	REVISED -
	CHECKED - KMS	REVISED -
PLOT SCALE = 0.1667' / in.	DRAWN - EJM	REVISED -
PLOT DATE = 4/11/2019	CHECKED - RGC	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**SOLDIER PILES - WEST WALL
RETAINING WALLS - 6TH STREET**

SHEET NO. 6 OF 18 SHEETS

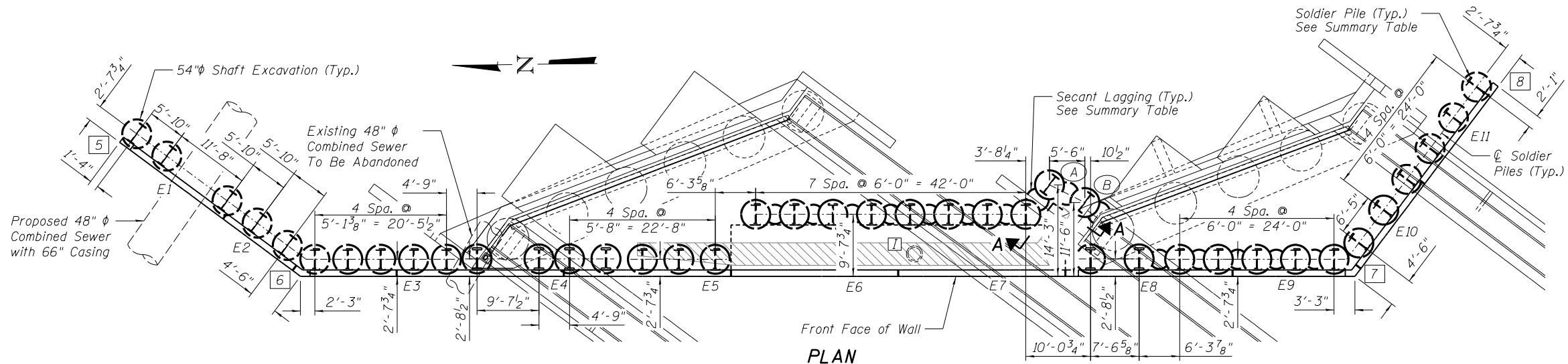
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	297
			CONTRACT NO. 93733	

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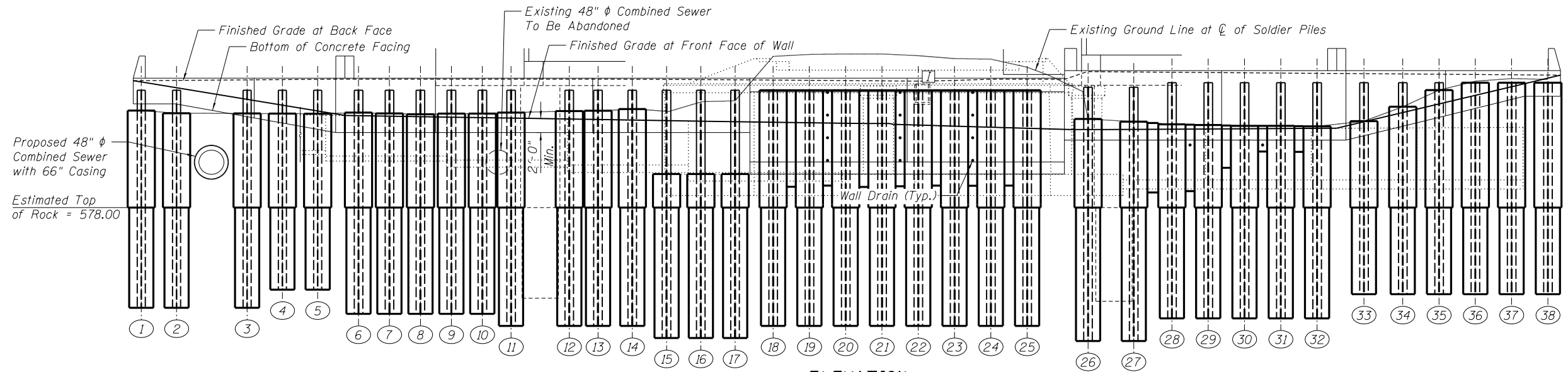
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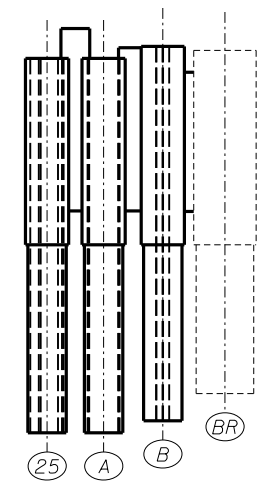
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PLAN
Note: All Dimensions are Measured Along Front Face of Wall



ELEVATION
Unfolded Along Face of Wall



SECTION A-A
Unfolded View

**EAST WALL
STUD SHEAR CONNECTORS REQUIRED**

Pile No.	Number Required on Each Pile
1	2
2	3
3	4
4-5	5
6-17	6
18-25	2
26-33	7
34	6
35	5
36	4
37	3
38	2

[6] = Control Point

SOLDIER PILE SUMMARY

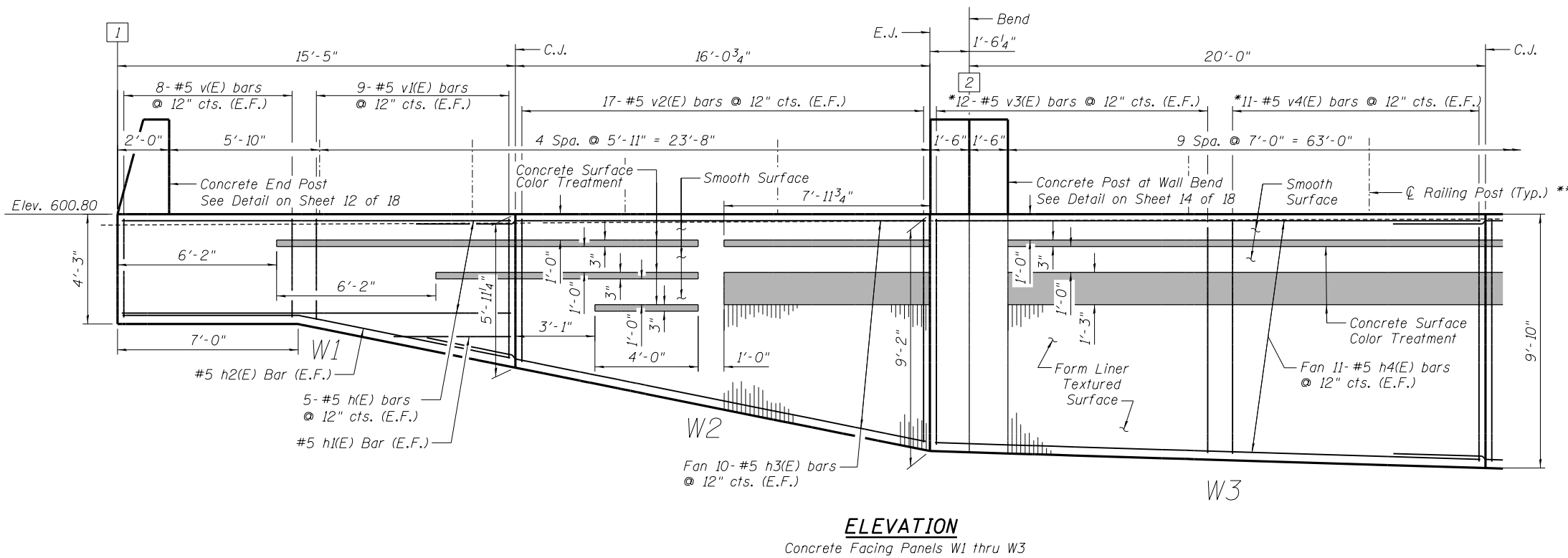
PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.	PILE NO.	PILE SIZE	LENGTH	BOTTOM ELEV.	TOP ELEV.
1	W36x487	36'-0"	561.40	597.40	15	W36x487	41'-0"	556.40	597.40
2	W36x487	36'-0"	561.40	597.40	16	W36x487	41'-0"	556.40	597.40
3	W36x487	36'-0"	561.40	597.40	17	W36x487	41'-0"	556.40	597.40
4	W36x487	33'-0"	564.40	597.40	18	W36x487	39'-0"	558.40	597.40
5	W36x487	33'-0"	564.40	597.40	19	W36x487	39'-0"	558.40	597.40
6	W36x487	37'-0"	560.40	597.40	20	W36x487	39'-0"	558.40	597.40
7	W36x487	37'-0"	560.40	597.40	21	W36x487	39'-0"	558.40	597.40
8	W36x487	37'-0"	560.40	597.40	22	W36x487	39'-0"	558.40	597.40
9	W36x487	37'-0"	560.40	597.40	23	W36x487	39'-0"	558.40	597.40
10	W36x487	37'-0"	560.40	597.40	24	W36x487	39'-0"	558.40	597.40
11	W36x652	39'-0"	558.40	597.40	25	W36x487	39'-0"	558.40	597.40
12	W36x652	39'-0"	558.40	597.40	26	W36x487	42'-0"	555.91	597.91
13	W36x652	39'-0"	558.40	597.40	27	W36x487	42'-0"	555.91	597.91
14	W36x652	39'-0"	558.40	597.40	28	W36x487	39'-0"	559.65	598.65
					29	W36x487	39'-0"	559.65	598.65
					30	W36x487	39'-0"	559.65	598.65
					31	W36x487	39'-0"	559.65	598.65
					32	W36x487	39'-0"	559.65	598.65
					33	W36x487	35'-0"	563.65	598.65
					34	W36x487	35'-0"	563.65	598.65
					35	W36x487	35'-0"	563.65	598.65
					36	W36x487	35'-0"	563.65	598.65
					37	W36x487	35'-0"	563.65	598.65
					38	W36x487	35'-0"	563.65	598.65
					Temp A	W36x487	39'-0"	558.40	597.40
					Temp B	W36x487	39'-0"	559.91	597.91

SECANT LAGGING SUMMARY

BETWEEN PILES NO.	DIAMETER	LENGTH	BOTTOM ELEV.	TOP ELEV.
18-19	36"	21'-9"	581.43	603.18
19-20	36"	21'-9"	581.63	603.38
20-21	36"	22'-0"	581.42	603.42
21-22	36"	21'-9"	581.53	603.28
22-23	36"	21'-9"	581.64	602.89
23-24	36"	21'-0"	581.61	602.61
24-25	36"	20'-3"	581.64	601.89
25-A	36"	19'-0"	581.53	600.53
A-B	36"	17'-0"	581.52	598.52
B-BR	36"	14'-6"	581.47	595.97
27-28	36"	11'-6"	580.50	592.00
28-29	36"	11'-0"	580.72	591.72
29-30	36"	7'-0"	584.59	591.59
30-31	36"	4'-3"	587.27	591.52
31-32	36"	4'-3"	587.22	591.47

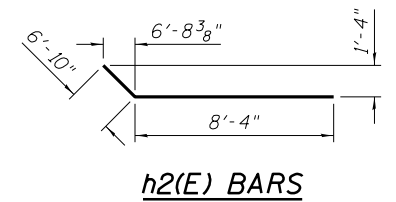
BILL OF MATERIAL

ITEM	UNIT	TOTAL
Stud Shear Connectors	Each	183
Furnishing Soldier Piles (W Section)	Foot	1517
Drilling and Setting Soldier Piles (in Soil)	Cu. Ft.	11169.8
Drilling and Setting Soldier Piles (in Rock)	Cu. Ft.	9119.7
Untreated Timber Lagging	Sq. Ft.	913
Secant Lagging	Cu. Ft.	1685

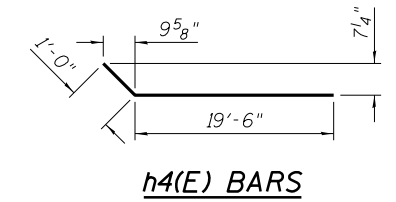


ELEVATION

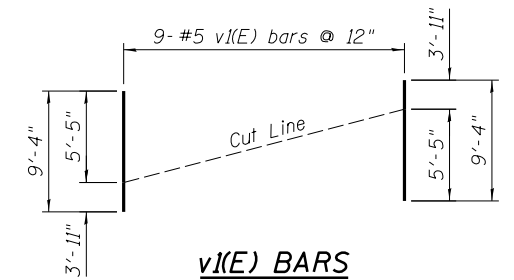
Concrete Facing Panels W1 thru W3



h2(E) BARS

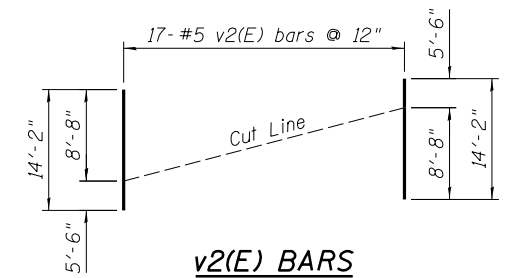


h4(E) BARS



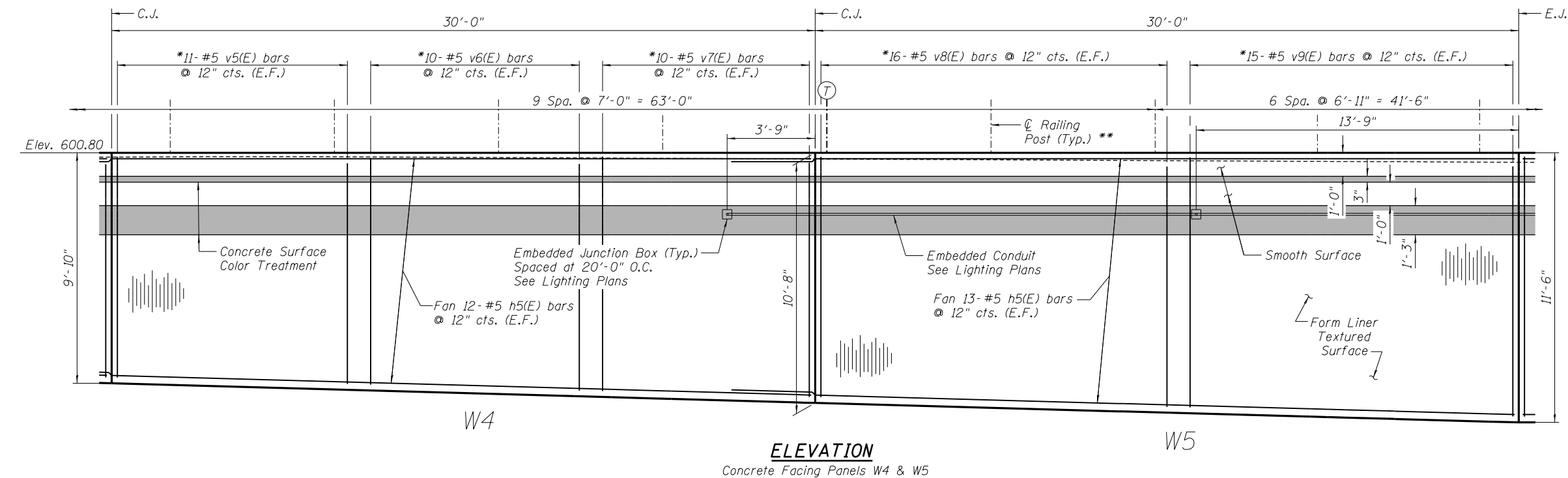
v(E) BARS

Cut Bars to be Placed E.F.



v2(E) BARS

Cut Bars to be Placed E.F.



ELEVATION

Concrete Facing Panels W4 & W5

BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
h(E)	10	#5	15'-1"	---	
h1(E)	2	#5	4'-6"	---	
h2(E)	2	#5	15'-2"	✓	
h3(E)	20	#5	19'-9"	---	
h4(E)	22	#5	20'-6"	✓	
h5(E)	50	#5	33'-5"	---	
v(E)	16	#5	3'-10"	---	
v1(E)	9	#5	9'-4"	---	
v2(E)	17	#5	14'-2"	---	
v3(E)	24	#5	8'-9"	---	
v4(E)	22	#5	9'-1"	---	
v5(E)	22	#5	9'-5"	---	
v6(E)	20	#5	9'-8"	---	
v7(E)	20	#5	10'-0"	---	
v8(E)	32	#5	10'-3"	---	
v9(E)	30	#5	10'-8"	---	
Reinforcement Bars Epoxy Coated				Pound	4960
Concrete Structures (Retaining Wall)				Cu. Yd.	42.9

(T) = Intermediate Tensioning Posts

** Steel Railing (Special)
All Measurements are
Along Top of Wall.
Adjust as Necessary
to Avoid C.J.'s & E.J.'s.

Note: E.J. = Expansion Joint
C.J. = Construction Joint
E.F. = Each Face
* = Stagger Bars

1 = Control Point

MIN. BAR LAPS
#5 Bars = 3'-4"

FINAL



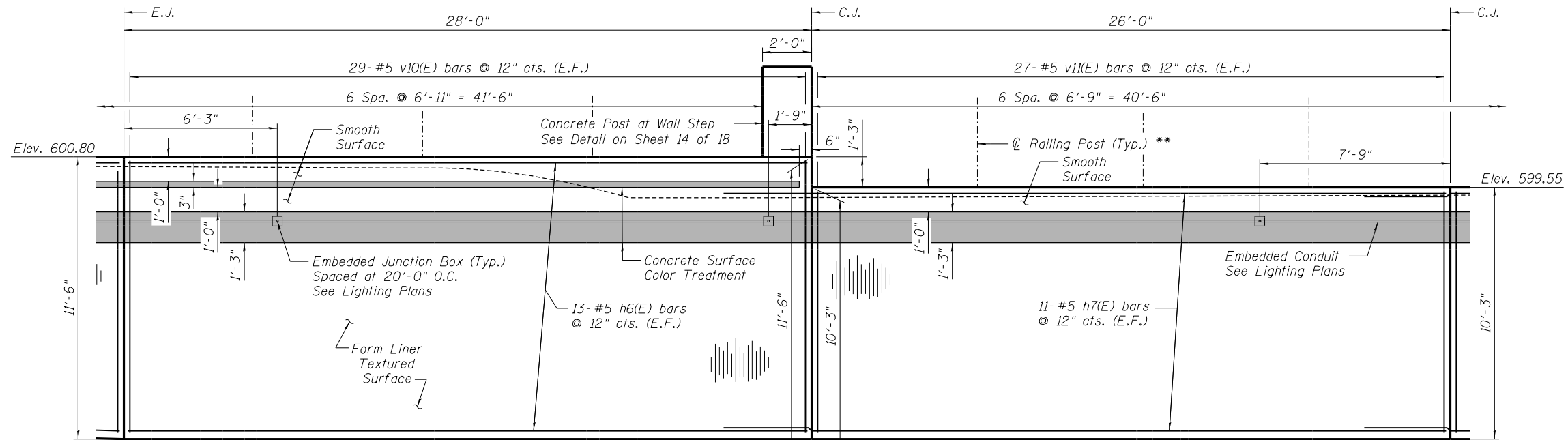
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PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISD -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISD -
	CHECKED - RGC	REVISD -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

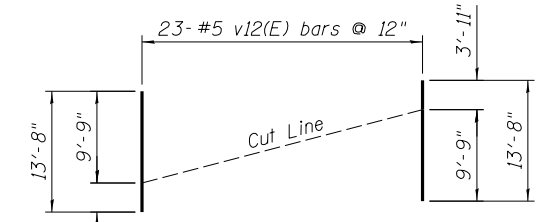
CONCRETE FACING - WEST WALL
RETAINING WALLS - 6TH STREET

SHEET NO. 8 OF 18 SHEETS

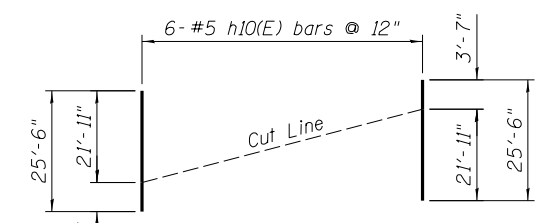
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	299
CONTRACT NO.			93733	
ILLINOIS FED. AID PROJECT				



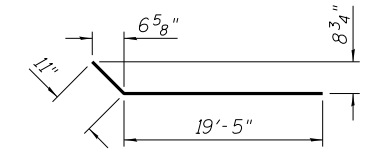
ELEVATION
Concrete Facing Panels W6 & W7



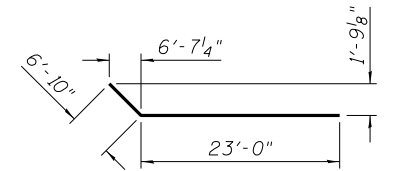
v12(E) BARS
Cut Bars to be Placed E.F.



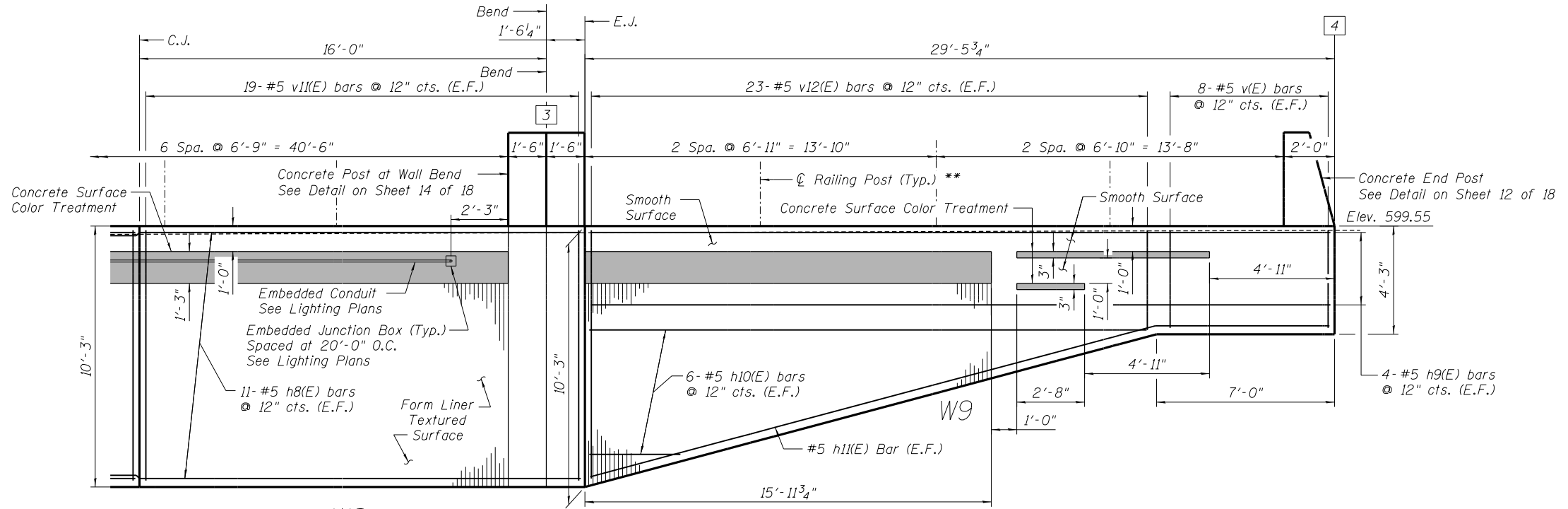
h10(E) BARS
Cut Bars to be Placed E.F.



h8(E) BARS



h11(E) BARS



ELEVATION
Concrete Facing Panels W8 & W9

BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
h6(E)	26	#5	27'-8"	—	
h7(E)	22	#5	29'-5"	—	
h8(E)	22	#5	20'-4"	↘	
h9(E)	8	#5	29'-1"	—	
h10(E)	6	#5	25'-6"	—	
h11(E)	2	#5	29'-10"	↘	
v(E)	16	#5	3'-10"	—	
v10(E)	58	#5	11'-1"	—	
v11(E)	92	#5	9'-10"	—	
v12(E)	23	#5	13'-8"	—	
Reinforcement Bars Epoxy Coated				Pound	4350
Concrete Structures (Retaining Wall)				Cu. Yd.	40.4

Note: E.J. = Expansion Joint
C.J. = Construction Joint
E.F. = Each Face
* = Stagger Bars

1 = Control Point

** Steel Railing (Special)
All Measurements are Along Top of Wall.
Adjust as Necessary to Avoid C.J.'s & E.J.'s.

MIN. BAR LAPS
#5 Bars = 3'-4"

pw:\spsvr\306.hanson.dom\hanson Projects\Documents\09Jobs\09L0179B\CAD\Struct\6th\Sheet\09L0179B-6thRetainingWallPlans

USER NAME = Pop00275	DESIGNED - RGC	REVISIONS -
PLOT SCALE = 0.1667' / in.	CHECKED - KMS	REVISIONS -
PLOT DATE = 4/11/2019	DRAWN - EJM	REVISIONS -
	CHECKED - RGC	REVISIONS -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**CONCRETE FACING - WEST WALL
RETAINING WALLS - 6TH STREET**

SHEET NO. 9 OF 18 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	(109) VB,(110) VB-5	SANGAMON	382	300
CONTRACT NO.			93733	
ILLINOIS FED. AID PROJECT				

FINAL



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