

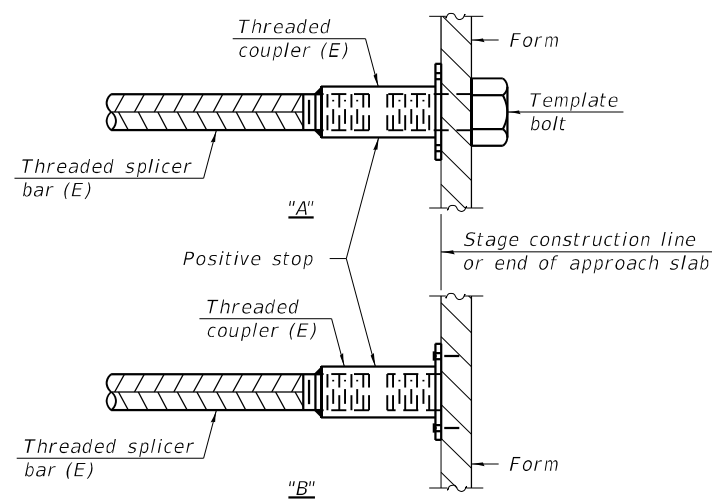
STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length + 1 1/2" + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Minimum lap length
Pier 1 (SB)	#5	40	2'-9"
Pier 2 (SB)	#5	64	2'-9"
Pier 1 (NB)	#5	40	2'-9"
Pier 2 (NB)	#5	64	2'-9"

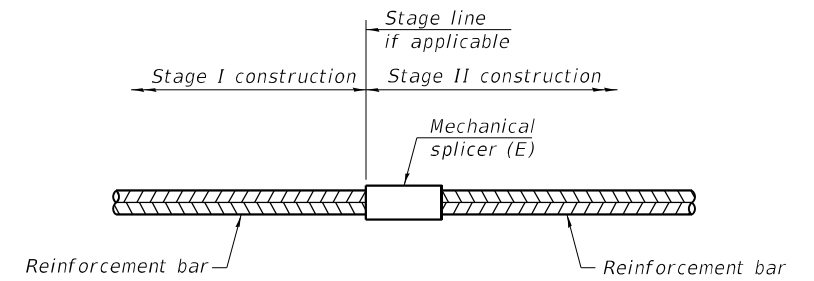


INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.

"B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required
Pier 1 (SB)	#11	102
Pier 2 (SB)	#11	102
Pier 1 (SB)	#11/#8	102
Pier 2 (SB)	#11/#8	102
Pier 1 (NB)	#11	102
Pier 2 (NB)	#11	102
Pier 1 (NB)	#11/#8	102
Pier 2 (NB)	#11/#8	102

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.

MODEL: Default
FILE NAME: \\SERVER18\Projects\54\22057.08 IDOT D3 PTB 204-028 WO 08 1-180 over Bureau Creek\IDGN\Bridges\Final\Plotsheets\006-0193&0194-66K66-06 1-Bar Splicer.dgn

BSD-1

2-1-2023

EFK•Moen
Civil Engineering Design

USER NAME = ABenz	DESIGNED - CMC	REVISED -
	CHECKED - ACB	REVISED -
PLOT SCALE =	DRAWN - CMC	REVISED -
PLOT DATE = 2/12/2024	CHECKED - ACB	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
STRUCTURE NO. 006-0193 (SB) & 006-0194 (NB)

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2B-1)ES	BUREAU	327	201
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

SHEET 61 OF 80 SHEETS



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Jim Zschau
 SECTION 06-2B-1 LOCATION East shoulder, NB lane, south of creek., SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude 41°18'0.17"N, Longitude 89°24'2.38"W
 COUNTY Bureau DRILLING METHOD Mud Rotary (4.0" ID, 8.0" OD) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO.	Station	BORING NO.	Station	Offset	Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T T (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	Hrs.
006-0052 (Exist.)	521+42	NB-SW-1	521+95	16.0 ft East	477.00											
Completely Weathered, Light-gray, Wet SHALE; slightly fissile rock (continued)					394.00											
Completely Weathered, Light-gray, Wet LIMESTONE/SHALE; slightly fissile rock, argillaceous Limestone, calcareous Shale					390.00		50/4		23							
Completely Weathered, Light-gray to Gray, Wet SHALE; slightly fissile to fissile rock							50/2		21							
							50/5		16							
							50/4		16							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Jim Zschau
 SECTION 06-2B-1 LOCATION East shoulder, NB lane, south of creek., SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude 41°18'0.17"N, Longitude 89°24'2.38"W
 COUNTY Bureau DRILLING METHOD Mud Rotary (4.0" ID, 8.0" OD) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO.	Station	BORING NO.	Station	Offset	Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T T (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	Hrs.
006-0052 (Exist.)	521+42	NB-SW-1	521+95	16.0 ft East	477.00											
Completely Weathered, Light-gray to Gray, Wet SHALE; slightly fissile to fissile rock (continued)																
							50/4		22							
							50/4		17							
Bottom of hole @ 128.8 ft End of Boring																

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default
 FILE NAME: \\SERVER18\Projects\54122057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DOT\Bridges\Final\Plotsheets\006-0193-0194-66K66-063-Soil Borings_Log.dgn
 SOIL BORING 107741-026.GPJ IL_DOT.GDT 3/3/23

SOIL BORING 107741-026.GPJ IL_DOT.GDT 3/3/23



USER NAME = ABenz	DESIGNED - CMC	REVISED -
CHECKED - ACB	CHECKED - ACB	REVISED -
PLOT SCALE =	DRAWN - CMC	REVISED -
PLOT DATE = 2/12/2024	CHECKED - ACB	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS
 STRUCTURE NO. 006-0193 (SB) & 006-0194 (NB)

SHEET 63 OF 80 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2B-1)ES	BUREAU	327	203
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Jim Zschau
SECTION 06-2B-1 LOCATION East shoulder, NB lane, north of creek., SEC. 12, TWP. 15N, RNG. 9E, 4th PM,
Latitude 41°18'1.35"N, Longitude 89°24'2.35"W
COUNTY Bureau DRILLING METHOD Mud Rotary (4.0" ID, 8.0" OD) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. Station	D E P T H	B L O W S	U C S Qu	M O I S T %	Surface Water Elev. ft	Stream Bed Elev. ft	Groundwater Elev.: First Encounter	ft	Upon Completion	ft	After Hrs.	ft	D E P T H	B L O W S	U C S Qu	M O I S T %		
																		(ft)
006-0052 (Exist.) 521+42																		
NB-SW-2 520+45 11.0 ft East 476.00																		
Loose Brown, Moist SANDY SILT		4																
		2																
		4		18														
		2												4				
		4												10				
Very Loose Brown, Moist SAND with Gravel; well-graded sand		4																
		1																
		2		11														
Very Loose to Loose Gray, Wet SANDY SILT		1																
		1																
		1		28														
		1												10				
Loose Gray, Wet SAND with Gravel; poorly-graded sand		1																
		1																
		2		33														
		4																
		3		23														
		4																
		4																
		4		21														

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Jim Zschau
SECTION 06-2B-1 LOCATION East shoulder, NB lane, north of creek., SEC. 12, TWP. 15N, RNG. 9E, 4th PM,
Latitude 41°18'1.35"N, Longitude 89°24'2.35"W
COUNTY Bureau DRILLING METHOD Mud Rotary (4.0" ID, 8.0" OD) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. Station	D E P T H	B L O W S	U C S Qu	M O I S T %	Surface Water Elev. ft	Stream Bed Elev. ft	Groundwater Elev.: First Encounter	ft	Upon Completion	ft	After Hrs.	ft	D E P T H	B L O W S	U C S Qu	M O I S T %		
																		(ft)
006-0052 (Exist.) 521+42																		
NB-SW-2 520+45 11.0 ft East 476.00																		
Medium Dense to Dense Gray, Wet SAND with Gravel; poorly-graded sand (continued)																		
			9															
			11															
Dense to Very Dense Gray-brown, Wet SAND with Silt; poorly-graded sand (continued)																		
			12															
Dense Gray, Wet SAND with Silt; poorly-graded sand																		
			17															
Dense to Very Dense Gray-brown, Wet SAND with Silt; poorly-graded sand																		
			17															
Very Dense Gray, Wet GRAVEL with Sand; poorly-graded gravel																		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default
FILE NAME: I:\SERVER18\Projects\54122057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DOT\Bridges\Final\Plotsheets\006-0193&0194-66K66-064-Soil Borings Log.dgn

SOIL BORING 107741-026.GPJ IL_DOT.GDT 3/3/23

EFK Moen
Civil Engineering Design

USER NAME = ABenz	DESIGNED - CMC	REVISED -
CHECKED - ACB	CHECKED - ACB	REVISED -
PLOT SCALE =	DRAWN - CMC	REVISED -
PLOT DATE = 2/12/2024	CHECKED - ACB	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS
STRUCTURE NO. 006-0193 (SB) & 006-0194 (NB)

SHEET 64 OF 80 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2B-1)ES	BUREAU	327	204
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



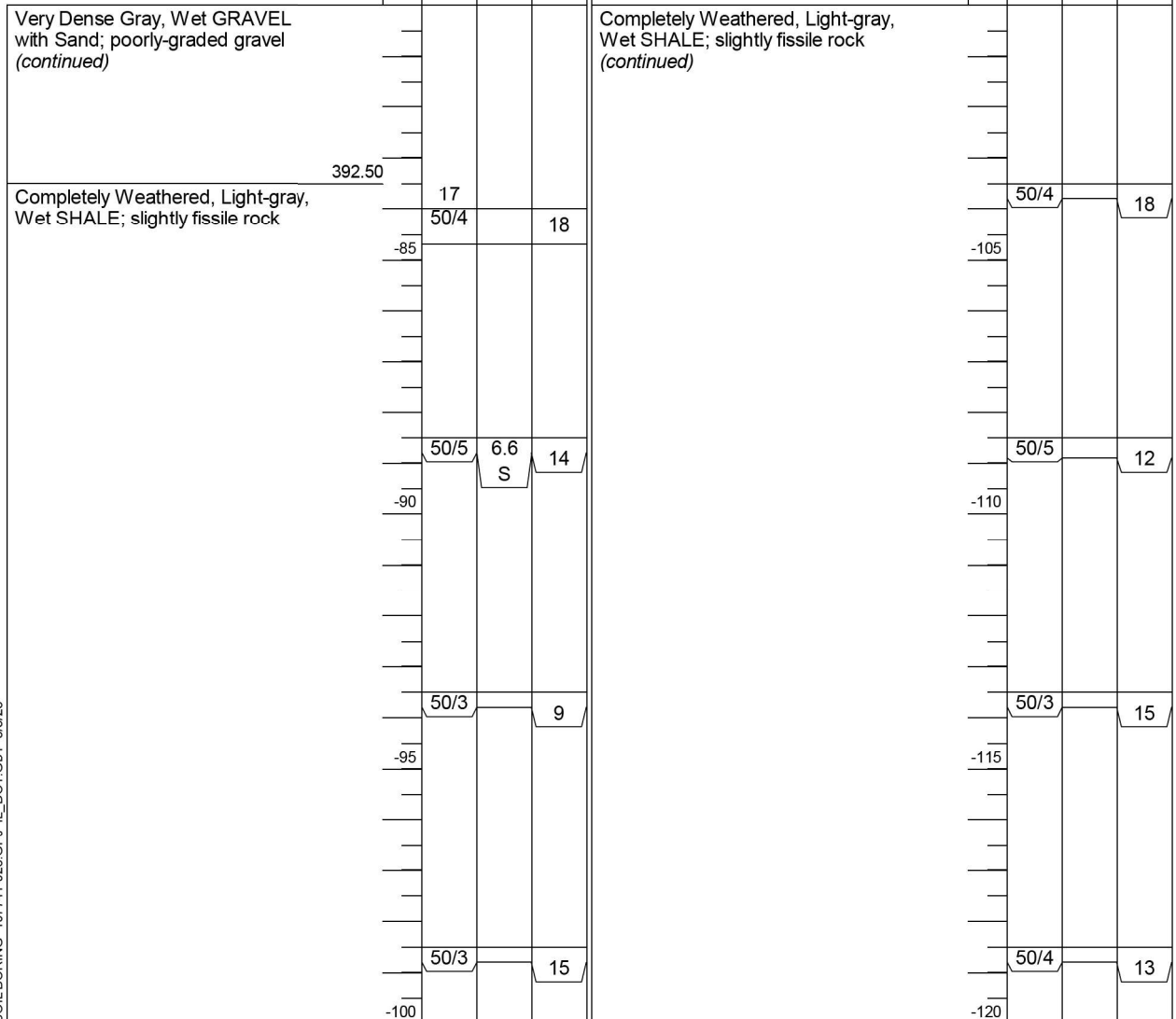
SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Jim Zschau

SECTION 06-2B-1 LOCATION East shoulder, NB lane, north of creek., SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude 41°18'1.35"N, Longitude 89°24'2.35"W

COUNTY Bureau DRILLING METHOD Mud Rotary (4.0" ID, 8.0" OD) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. 006-0052 (Exist.)	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
Station 521+42					Stream Bed Elev. _____ ft
BORING NO. NB-SW-2	(ft)	(/6")	(tsf)	(%)	Groundwater Elev.: _____
Station 520+45					First Encounter 463.5 ft ▼
Offset 11.0 ft East					Upon Completion _____ ft
Ground Surface Elev. 476.00 ft					After _____ Hrs. _____ ft



The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



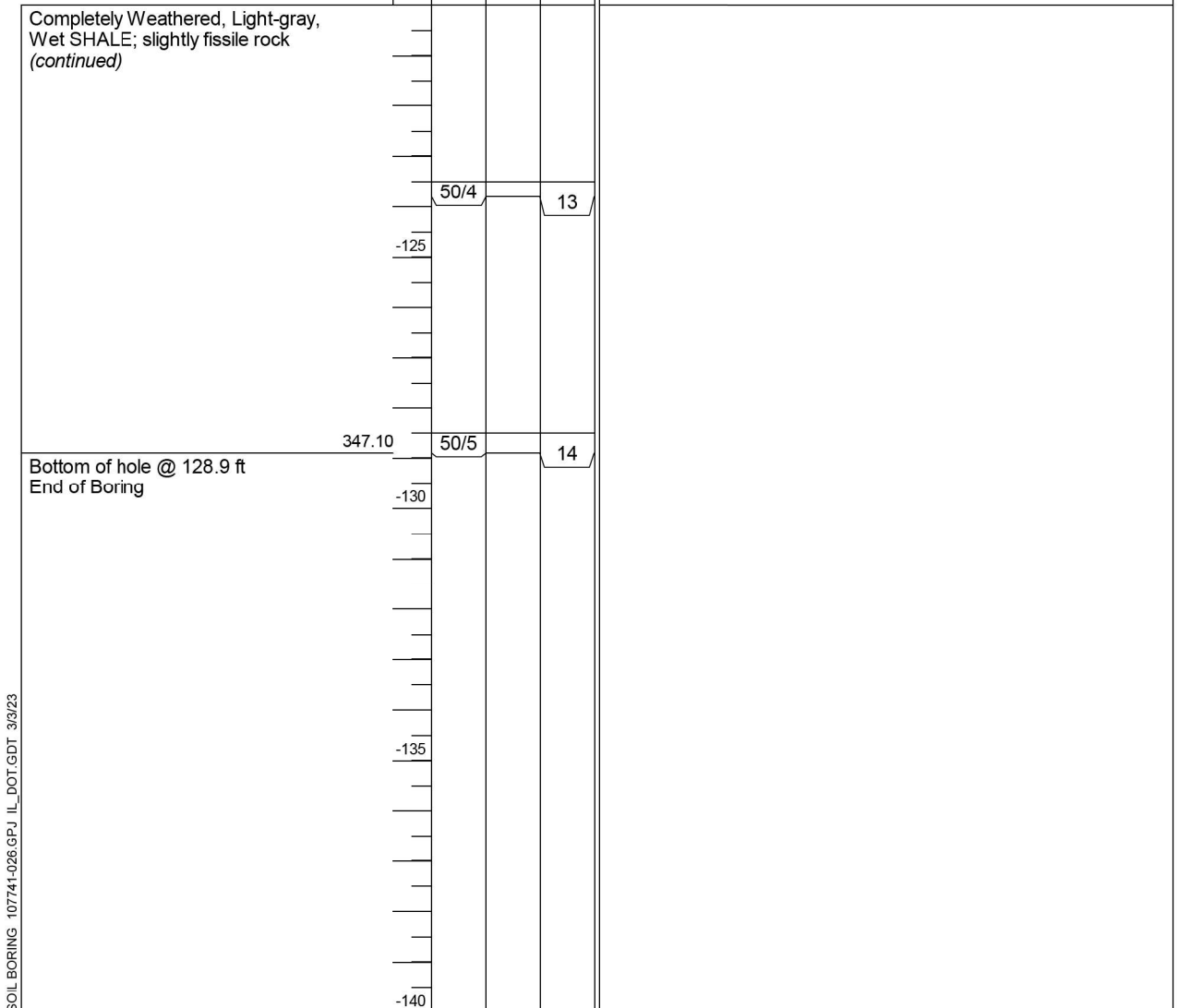
SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Jim Zschau

SECTION 06-2B-1 LOCATION East shoulder, NB lane, north of creek., SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude 41°18'1.35"N, Longitude 89°24'2.35"W

COUNTY Bureau DRILLING METHOD Mud Rotary (4.0" ID, 8.0" OD) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. 006-0052 (Exist.)	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
Station 521+42					Stream Bed Elev. _____ ft
BORING NO. NB-SW-2	(ft)	(/6")	(tsf)	(%)	Groundwater Elev.: _____
Station 520+45					First Encounter 463.5 ft ▼
Offset 11.0 ft East					Upon Completion _____ ft
Ground Surface Elev. 476.00 ft					After _____ Hrs. _____ ft



The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default
FILE NAME: \\SERVER18\Projects\54\22057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DGN\Bridges\Final\plotsheets\006-0193&0194-66K66-065-Soil Borings_Log.dgn

SOIL BORING 107741-026.GPJ IL_DOT.GDT 3/3/23

SOIL BORING 107741-026.GPJ IL_DOT.GDT 3/3/23



USER NAME = ABenz	DESIGNED - CMC	REVISED -
CHECKED - ACB	REVISOR -	
PLOT SCALE =	DRAWN - CMC	REVISED -
PLOT DATE = 2/12/2024	CHECKED - ACB	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS STRUCTURE NO. 006-0193 (SB) & 006-0194 (NB)

SHEET 65 OF 80 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2B-1)ES	BUREAU	327	205
CONTRACT NO. 66K66				



SOIL BORING LOG

Date 1/1/67

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 miles South of IL 26 Junction LOGGED BY _____
 SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM,
 COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE _____

STRUCT. NO. Station	BORING NO. Station	Offset Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.		D E P T H (ft)		B L O W S (/6")		U C S Qu (tsf)		M O I S T (%)		
							ft	ft	ft	ft	ft	ft	ft	ft			
006-0052 (Exist.) 521+42.00	1 520+18	0.0 ft NB CL 470.00															

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Date 1/1/67

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 miles South of IL 26 Junction LOGGED BY _____
 SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM,
 COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE _____

STRUCT. NO. Station	BORING NO. Station	Offset Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.		D E P T H (ft)		B L O W S (/6")		U C S Qu (tsf)		M O I S T (%)		
							ft	ft	ft	ft	ft	ft	ft	ft			
006-0052 (Exist.) 521+42.00	1 520+18	0.0 ft NB CL 470.00															

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)

MODEL: Default
FILE NAME: \\SERVER18\Projects\54\22057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DGN\Bridges\Final\Plotsheets\006-0193&0194-66K66-066-066-Soil Borings Log.dgn



SOIL BORING LOG

Date 1/1/67

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 miles South of IL 26 Junction LOGGED BY
 SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude, Longitude
 COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE

STRUCT. NO.	Station	BORING NO.	Station	Offset	Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After 24 Hrs.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
006-0052 (Exist.)	521+42.00	3	521+46	0.0 ft NB CL	471.00														
Brown Silty Topsoil											Very Tough Gray Clay (continued)								
469.50												450.00							
Soft Dark Brown Silty Clay							2		27			Firm Fine Sand with Trace Small Gravel					24		
▽																			
-5							4		114								25		
445.00												Dense Coarse Sand with Trace Small Gravel					36		
							3		33										
-10							3		27								46		
▽																			
							3		25								29		
456.00																			
-15							4		28								30		
Very Tough Gray Clay																			
								0.7-1.0	31										
							29	P									30		
-20							33	1.1-1.5										27	
								P											

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Date 1/1/67

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 miles South of IL 26 Junction LOGGED BY
 SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude, Longitude
 COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE

STRUCT. NO.	Station	BORING NO.	Station	Offset	Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After 24 Hrs.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
006-0052 (Exist.)	521+42.00	3	521+46	0.0 ft NB CL	471.00														
Dense Coarse Sand with Trace Small Gravel (continued)																			
427.50																			
							46												
Very Dense Coarse Sand, Trace Medium Gravel																			
-45							50												
421.00																			
-50							50												
End of Boring																			
-60																			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)

MODEL: Default
 FILE NAME: \\SERVER18\Projects\54\22057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DOT\Bridges\Final\Plotsheets\006-0193&0194-66K66-06B-06B-Soil Borings Log.dgn
 SOIL BORING 006-0051,0052.GPJ IL_DOT.GDT 2/10/22

SOIL BORING 006-0051,0052.GPJ IL_DOT.GDT 2/10/22



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 miles South of IL 26 Junction LOGGED BY
SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude, Longitude
COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE

Table with columns for STRUCT. NO., BORING NO., Station, Offset, Ground Surface Elev., DPTH, BLOW S, UCS Qu, M O I S T (%), and Soil Description. Includes soil layers like Topsoil, Very Loose Fine Sand, Very Loose Fine Silty Sand, Soft Gray Silty and Sandy Clay, and Firm Medium Sand with Trace of Small Gravel.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 miles South of IL 26 Junction LOGGED BY
SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude, Longitude
COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE

Table with columns for STRUCT. NO., BORING NO., Station, Offset, Ground Surface Elev., DPTH, BLOW S, UCS Qu, M O I S T (%), and Soil Description. Includes soil layers like Dense Medium Sand with Trace of Small Gravel, Very Dense Coarse Sand, and End of Boring.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default
FILE NAME: \\SERVER18\Projects\54\22057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DOT\Bridges\Final\Plotsheets\006-0193&0194-66K66-069-Soil Borings_Log.dgn
SOIL BORING 006-0051.0052.GPJ IL_DOT.GDT 2/10/22



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 miles South of IL 26 Junction LOGGED BY
SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude, Longitude
COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE

Table with columns: STRUCT. NO., Station, BORING NO., Station, Offset, Ground Surface Elev., DEPTH (ft), BLOW (6"), UCS (tsf), MOIST (%), Surface Water Elev., Stream Bed Elev., Groundwater Elev.: First Encounter, Upon Completion, After Hrs., Firm Brown Silty Fine Sand (continued), Dense Fine to Medium Sand, Boulder at 31 Feet, Firm Brown Silty Fine Sand.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 miles South of IL 26 Junction LOGGED BY
SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude, Longitude
COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE

Table with columns: STRUCT. NO., Station, BORING NO., Station, Offset, Ground Surface Elev., DEPTH (ft), BLOW (6"), UCS (tsf), MOIST (%), Surface Water Elev., Stream Bed Elev., Groundwater Elev.: First Encounter, Upon Completion, After Hrs., Dense Fine to Medium Sand (continued), Very Dense Fine to Medium Sand, Wet.

End of Boring

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default FILE NAME: I:\SERVER\18\Projects\54\22057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\Bridges\Final\Plotsheets\006-0193&0194-66K66-070-Soil Borings Log.dgn

SOIL BORING 006-0051.0052.GPJ IL_DOT.GDT 2/10/22

SOIL BORING 006-0051.0052.GPJ IL_DOT.GDT 2/10/22

EFK Moen Civil Engineering Design

Table with columns: USER NAME, DESIGNED, CHECKED, PLOT SCALE, PLOT DATE, REVISIONS (REVISED, CHECKED).

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS STRUCTURE NO. 006-0193 (SB) & 006-0194 (NB)

SHEET 70 OF 80 SHEETS

Table with columns: F.A.I. RTE., SECTION, COUNTY, TOTAL SHEETS, SHEET NO., CONTRACT NO.

ILLINOIS FED. AID PROJECT

SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Larry Myers

SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM,
Latitude 41.3008, Longitude -89.40105

COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. 006-0051 (Exist.)
Station 521+42.00

BORING NO. 01 (NW Quad)
Station 519+16
Offset 61.1 ft Rt.
Ground Surface Elev. 502.55 ft

DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOIST (%)
------------	------------------	-----------	-----------

DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOIST (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOIST (%)
497.55	-5			Augered Bituminous Shoulder, Gray Silty Clay Loam/Silty Loam Till Fill	3			
					3	3.8	15	
					5	B		
				Very Stiff Light Brown Silty Clay Loam Till Fill (continued)	480.55			
					5			
				Medium Brown Fine to Medium Sand - with Minor Sandy Loam Layers - Fill	11		7	
					14			
					-25			
				Very Stiff to Hard Gray Silty Clay Loam Till Fill	4			
		4.5	10		9		3	
		P			10			
					475.55			
				Very Stiff Gray Silty Clay Loam Till - Possibly Fill	4			
		4.5	12		6	3.4	9	
		P			4	B		
					473.05			
				Hard Brown and Gray Silty Clay Loam Till	-30			
					2			
		4.1	13		7	4.1	19	
		B			9	S		
					470.55			
				Hard Gray Silty Clay Loam Grading to Silty Sand	2			
		4.2	15		8	>4.5	8	
		B			16	P		
					468.05			
				Dense to Loose Gray Fine to Coarse Sand with Some Fine to Medium Gravel Pieces	-35			
					8			
		4.1	14		16		5	
		B			16			
					3			
		4.1	11		5		9	
		B			6			
					483.05			
					-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Larry Myers

SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM,
Latitude 41.3008, Longitude -89.40105

COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. 006-0051 (Exist.)
Station 521+42.00

BORING NO. 01 (NW Quad)
Station 519+16
Offset 61.1 ft Rt.
Ground Surface Elev. 502.55 ft

DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOIST (%)
------------	------------------	-----------	-----------

DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOIST (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOIST (%)
				Dense to Loose Gray Fine to Coarse Sand with Some Fine to Medium Gravel Pieces (continued)	2			
					3		16	
					4			
				Free Water at 41'	460.55			
				Loose Gray and Black Silty Fine to Medium Sand with Some Black Organics	2			
					2		26	
					4			
					-45			
					5			
					4		25	
					6			
					455.55			
				Dense to Medium Gray Fine Sand to Coarse Gravel	10			
					14		11	
					18			
					-50			
				*Washed Sample 50.0'-51.5'	6			
					11		12	
					14	*		
					8			
				*Washed Sample 52.5'-54.0'	10		11	
					12	*		
					-55			
				*Washed Sample 55.0'-56.5'	11			
					10		13	
					12	*		
					10			
					12		18	
					15			
					-60			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default
FILE NAME: I:\SERVER\18\Projects\54\22057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DGN\Bridges\Final\Plotsheets\006-0193&0194-66K66-073-Soil Borings Log.dgn
SOIL BORING 006-0051,0052.GPJ IL_DOT.GDT 6/7/22

SOIL BORING 006-0051,0052.GPJ IL_DOT.GDT 6/7/22



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Larry Myers
 SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM,
Latitude 41.29954, Longitude -89.40064
 COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. Station	BORING NO. Station	Offset	Ground Surface Elev.	D		B		U		M	S		Surface Water Elev. ft	D		B		U		M	S		
				PT	H	(ft)	(/6")	(tsf)	(%)		TH	WS		Qu	TS	TH	WS	Qu	TS				
006-0052 (Exist.) 521+42.00	03 (SE Quad) 523+71	62.1 ft Lt.	514.87 ft																				
Augered Bituminous Shoulder. Brown/Gray Silty Clay Loam Till Fill				509.87	-5																		
Hard Gray Silty Clay Loam Till Fill with Some Silty Loam Till Fill Layers						3		6	5.0	11													
						7			S														
						6																	
						8	5.0	10															
						10			S														
						-10																	
						9																	
						13	6.1	8															
						9			S														
						4																	
						5	5.0	10															
						8			S														
						-15																	
						4																	
						4	4.6	10															
						8			S														
						4																	
						5	4.0	12															
						6			B														
						-20																	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Larry Myers
 SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM,
Latitude 41.29954, Longitude -89.40064
 COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. Station	BORING NO. Station	Offset	Ground Surface Elev.	D		B		U		M	S		Surface Water Elev. ft	D		B		U		M	S		
				PT	H	(ft)	(/6")	(tsf)	(%)		TH	WS		Qu	TS	TH	WS	Qu	TS				
006-0052 (Exist.) 521+42.00	03 (SE Quad) 523+71	62.1 ft Lt.	514.87 ft																				
Hard Gray Silty Clay Loam Till Fill with Some Silty Loam Till Fill Layers				470.87		9		10	5.1	10													
						11			S														
						6																	
						6	4.2	10															
						7			S														
						470.87																	
Hard to Very Stiff Black to Brown Silty Clay				448.37		-45																	
						5																	
						5	4.0	20															
						9			B														
						448.37																	
						3																	
						5	3.6	15															
						6			B														
						465.37																	
Medium to Stiff Brown Silty Loam with Layers of Silt and Fine Sand and Fine Gravel Layers						-50																	
						2																	
						2	1.0	25															
						2			P														
						1																	
						2	1.0	24															
						3			P														
						-55																	
						2																	
						2	1.0	19															
						3			P														
						-60																	
						2																	
						2	1.5	25															
						3			P*														
						4																	
						-60																	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default
 FILE NAME: \\SERVER18\Projects\54122057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DGN\Bridge\Final\Plotsheets\006-0193&0194-66K66-075-Soil Borings Log.dgn

SOIL BORING 006-0051.0052.GPJ IL_DOT.GDT 6/7/22

SOIL BORING 006-0051.0052.GPJ IL_DOT.GDT 6/7/22



SOIL BORING LOG

Date 4/27/22

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Larry Myers
 SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude 41.30076, Longitude -89.40074
 COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. Station	BORING NO. Station	Offset	Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)	Surface Water Elev. _____ ft				Stream Bed Elev. _____ ft							
								Groundwater Elev.:				First Encounter _____ ft				Upon Completion _____ ft			
								After _____ Hrs. _____ ft				After _____ Hrs. _____ ft							
Augered Brown to Gray Silty Clay Loam Till Fill								Hard to Very Stiff Gray Silty Clay Loam Till Fill (continued)											
								5				5							
								6		5.0		6							
								11		S		11							
10" Fill Sand at 22.5'																			
								5				5							
								4		3.8		4							
								4		B		4							
497.28 -5								-25											
Hard to Very Stiff Gray Silty Clay Loam Till Fill								Some Silty Clay and Silty Loam Fill Layers after 27'											
								5				4							
								5		4.0		5							
								5		P		5							
								4				4							
								5		4.4		5							
								5		S		5							
-10								-30											
								4				3							
								3		3.2		3							
								3		B		5							
								5				5							
470.28								470.28											
Hard Alternating Layers of Black, Brown, Gray Silty Clay, Silty Loam, Silty Clay Loam Till, Silt - Possibly Fill																			
								7				7							
								4		3.9		11							
								4		B		14							
-15								-35											
								5				8							
								8		4.4		11							
								7		S		13							
465.28								465.28											
Medium to Loose Brown Fine to Coarse Gravel								Medium to Loose Brown Fine to Coarse Gravel											
								5				4							
								7		4.6		11							
								8		S		9							
-20								-40											

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Date 4/27/22

ROUTE FAI 180 (I-180) DESCRIPTION I-180 over Bureau Creek, 1.2 mi South of IL 26 LOGGED BY Larry Myers
 SECTION 06-2B-1 LOCATION NE 1/4, SEC. 12, TWP. 15N, RNG. 9E, 4th PM, Latitude 41.30076, Longitude -89.40074
 COUNTY Bureau DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. Station	BORING NO. Station	Offset	Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)	Surface Water Elev. _____ ft				Stream Bed Elev. _____ ft							
								Groundwater Elev.:				First Encounter _____ ft				Upon Completion _____ ft			
								After _____ Hrs. _____ ft				After _____ Hrs. _____ ft							
Medium to Loose Brown Fine to Coarse Gravel								Dense Gray to Fine Sand to Coarse Gravel - Loamy (continued)											
								2				2							
								4				4							
WH = Weight of Hammer (continued) Free Water at 40'								*Washed Sample 60.0'-61.5'											
								2				2							
								4				4							
								6				6							
437.78								437.78											
Hard Gray Silty Loam Till								*Washed Sample 62.5'-64.0'											
								3				3							
								5				5							
								7				7							
455.28								455.28											
Medium Gray Fine to Coarse Sand with Fine to Coarse Gravel								Hard Gray Silty Loam Till											
								3				3							
								5				5							
								7		*		7							
*Washed Sample 47.5'-49.0'								*Washed Sample 65.0'-66.5'											
								3				3							
								5				5							
								8		*		8							
-50								-70											
								6				6							
								9				9							
								10				10							
448.28								448.28											
Dense Gray to Fine Sand to Coarse Gravel - Loamy								End of Boring											
								13				13							
								20				20							
								13				13							
*Washed Sample 50.0'-51.5'								*Washed Sample 57.5'-59.0'											
								13				13							
								14		*		14							
								15				15							
-60								-80											

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default; FILE NAME: \\SERVER18\Projects\54\22057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DOT\Bridges\Final\Plotsheets\006-0193&0194-66K66-076-Soil Borings_Log.dgn

SOIL BORING 006-0051.0052.GPJ IL_DOT.GDT 6/7/22

SOIL BORING 006-0051.0052.GPJ IL_DOT.GDT 6/7/22



USER NAME =	ABenz
DESIGNED -	CMC
CHECKED -	ACB
REVISOR -	
REVISION -	
PLLOT SCALE =	
DRAWN -	CMC
CHECKED -	ACB
REVISOR -	
REVISION -	
PLLOT DATE =	2/12/2024

DESIGNED -	CMC
CHECKED -	ACB
DRAWN -	CMC
CHECKED -	ACB

REVISED -	
REVISION -	
REVISED -	
REVISION -	

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS STRUCTURE NO. 006-0193 (SB) & 006-0194 (NB)

SHEET 76 OF 80 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2B-1)ES	BUREAU	327	216
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



SOIL BORING LOG

Page 1 of 4
Date 2/13/23

ROUTE F. A. I. - 180 DESCRIPTION I-180 over Bureau Creek LOGGED BY KEG
SECTION 06-2B-1 LOCATION Near Princeton IL, 41.300390333° Latitude and -89.400923361° Longitude
COUNTY Bureau DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO.	Station	BORING NO.	Station	Offset	Ground Surface Elev.	D	B	U	M	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	Hrs.	D	B	U	M	
006-0051	521+42.45	SB-01	520+25.53	19.9 ft RT	468.97	(ft)	(/6")	(tsf)	(%)	ft	ft	ft	ft	ft	ft		(ft)	(/6")	(tsf)	(%)	
SILT LOAM - Grayish Brown, Soft							3											8			
w/ Some Fine Sand + Gravel							3	0.7	20									8	-	10	
13.9% Gravel, 30.2% Sand, 45.3% Silt, 10.5% Clay							2											4			
w/ Some Coarse Sand							2	0.2	28									4	-	13	
SAND - Gray, Loose, w/ Organics							3											10			
SILT LOAM- Light Brown, Soft, w/ some Gray, Fine, Sands							4											9			
SAND - Gray, Loose, Coarse, w/ Some Pebbles							6	0.1	27									8	-	18	
GRAVEL - Medium-dense							4											6			
SILT LOAM - Gray, Medium Stiff to Stiff, w/ Trace Gravel							7	0.2	25									10	-	20	
SAND - Gray, Loose, Coarse, w/ Some Pebbles							6											12			
SILT LOAM - Gray, Medium Stiff to Stiff, w/ Trace Gravel							1		24									11			
SAND - Gray, Loose, Coarse, w/ Some Pebbles							2											14	-	7	
SILT LOAM - Gray, Medium Stiff to Stiff, w/ Trace Gravel							3	0.2	23									12			
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							1											11			
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							3		16									14	-	15	
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							3											12			
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							4											11	1.5	15	
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							4											12	B		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Page 2 of 4
Date 2/13/23

ROUTE F. A. I. - 180 DESCRIPTION I-180 over Bureau Creek LOGGED BY KEG
SECTION 06-2B-1 LOCATION Near Princeton IL, 41.300390333° Latitude and -89.400923361° Longitude
COUNTY Bureau DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO.	Station	BORING NO.	Station	Offset	Ground Surface Elev.	D	B	U	M	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	Hrs.	D	B	U	M	
006-0051	521+42.45	SB-01	520+25.53	19.9 ft RT	468.97	(ft)	(/6")	(tsf)	(%)	ft	ft	ft	ft	ft	ft		(ft)	(/6")	(tsf)	(%)	
SILT LOAM - Gray, Medium Stiff to Stiff, w/ Trace Gravel (continued)																					
w/o Gravel							12														
w/ Gravel							15	1.3	15												
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments (continued)							14	B													
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							13														
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							14	1.5	11												
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							16	P													
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							17														
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							26														
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							28														
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							33														
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							32		10												
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments							39														

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

MODEL: Default
FILE NAME: I:\SERVER18\Projects\54122057.08 IDOT D3 PTB 204-028 WO 08 I-180 over Bureau Creek\DGN\Bridges\Final\Plotsheets\006-0193&0194-66K66-077-Soil Borings Log.dgn



USER NAME = ABenz	DESIGNED - CMC	REVISED -
CHECKED - ACB	REVISIONS -	
PLOT SCALE =	DRAWN - CMC	REVISED -
PLOT DATE = 2/12/2024	CHECKED - ACB	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS
STRUCTURE NO. 006-0193 (SB) & 006-0194 (NB)

SHEET 77 OF 80 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2B-1)ES	BUREAU	327	217
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



SOIL BORING LOG

Page 3 of 4
Date 2/13/23

ROUTE F. A. I. - 180 DESCRIPTION I-180 over Bureau Creek LOGGED BY KEG
SECTION 06-2B-1 LOCATION Near Princeton IL, 41.300390333° Latitude and -89.400923361° Longitude
COUNTY Bureau DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO.	Station	BORING NO.	Station	Offset	Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	H rs.	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)
006-0051	521+42.45	SB-01	520+25.53	19.9 ft RT	468.97															
SAND - Gray, Medium Dense to Very Dense, w/Gravel + Granite Fragments (continued)																				
SHALE - Gray, Moderately Hard						385.5	100/5"		17								100/2.75"		2.8	17
SHALE - Gray, Moderately Hard						-85		5.6 S									-105		P	
Poor Recovery							100/1"		20								100/3.25"		3.2 S	18
Becomes Blueish Gray						-90											-110			
Becomes Blueish Gray							100/3.25"		15								100/2.5"		1.2 S	17
Becomes Soft, Dark Gray						-95		1.5 B									-115			
Becomes Soft, Dark Gray							100/3"		19								100/5.375"		0.7 B	18
Poor Recovery						-100											-120			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Page 4 of 4
Date 2/13/23

ROUTE F. A. I. - 180 DESCRIPTION I-180 over Bureau Creek LOGGED BY KEG
SECTION 06-2B-1 LOCATION Near Princeton IL, 41.300390333° Latitude and -89.400923361° Longitude
COUNTY Bureau DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO.	Station	BORING NO.	Station	Offset	Ground Surface Elev.	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	H rs.	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)
006-0051	521+42.45	SB-01	520+25.53	19.9 ft RT	468.97															
SHALE - Gray, Moderately Hard																				
SHALE - Gray, Moderately Hard																				
Becomes Moderately Hard to Hard							100/5.75"		19											
Becomes Moderately Hard to Hard						-125		4.2 B												
Poor Recovery							100/5"		21											
Becomes Blueish Gray						-130		4.8 B												
Becomes Blueish Gray							100/0.8"													
No Recovery																				
End of Boring						334.0														
End of Boring						-135														

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

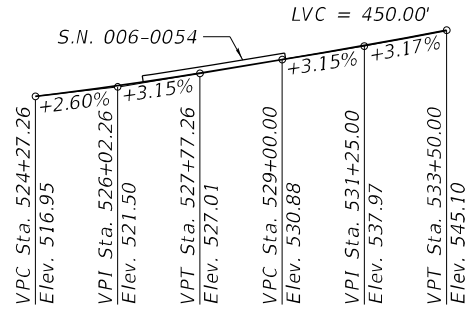
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	USER NAME = ABenz	DESIGNED - CMC	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SOIL BORING LOGS STRUCTURE NO. 006-0193 (SB) & 006-0194 (NB)	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE =	DRAWN - CMC	REVISED -			180	(06-2B-1)ES	BUREAU	327	218
	PLOT DATE = 2/12/2024	CHECKED - ACB	REVISED -			SHEET 78 OF 80 SHEETS		CONTRACT NO. 66K66		
								ILLINOIS FED. AID PROJECT		

Benchmark:
Square cut on wall at southwest corner of NB I-180 over Bottom Road in concrete. Elev. 532.52.

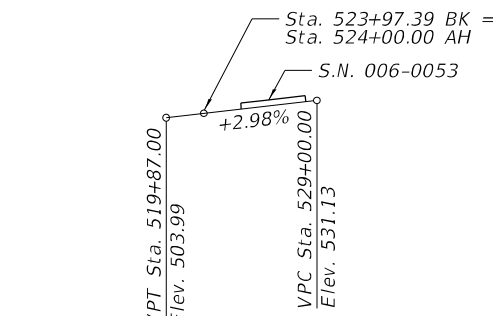
Existing Structure:
SN 006-0053 and SN 006-0054 were originally built in 1967 as Section 06-2-HB-1 to carry I-180 over C.H. 23. Each superstructure consists a 7½" normal crowned reinforced concrete slab supported on three simple spans of 42" concrete I-beams. Substructure elements consist of concrete stub abutments and three column piers, on concrete pile supported footings. In 2000, concrete repairs were made to the deck and beams, expansion joint and abutment bearing were replaced and a micro-silica overlay was placed. Bridges measure 176'-3" bk.-to-bk. of abutments and 42'-6" wide o.-to-o. The structures are skewed 23°46' left-forward. The existing superstructure will be removed and replaced. Traffic will be maintained utilizing cross-overs.

No salvage.



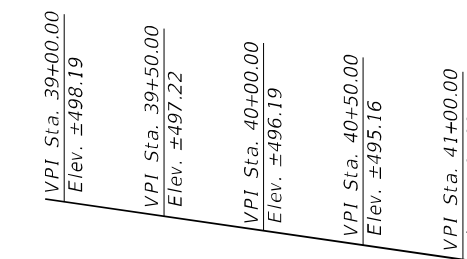
PROFILE GRADE - I-180 (NB)

(Along inside Edge of Pavement)
Up to ¼ inch to be ground off the bridge deck and the bridge approach slabs. The Profile Grade shows the final grade after grinding.

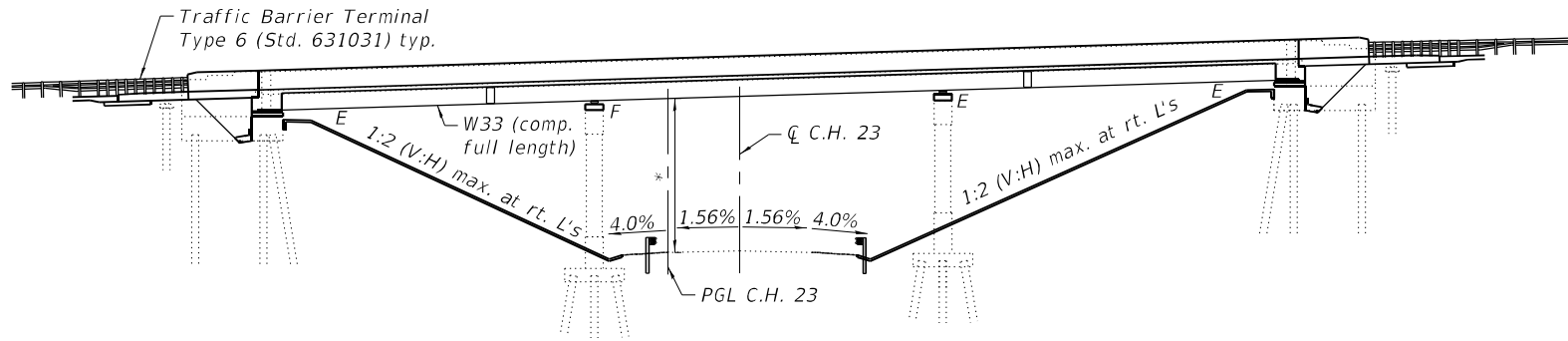


PROFILE GRADE - I-180 (SB)

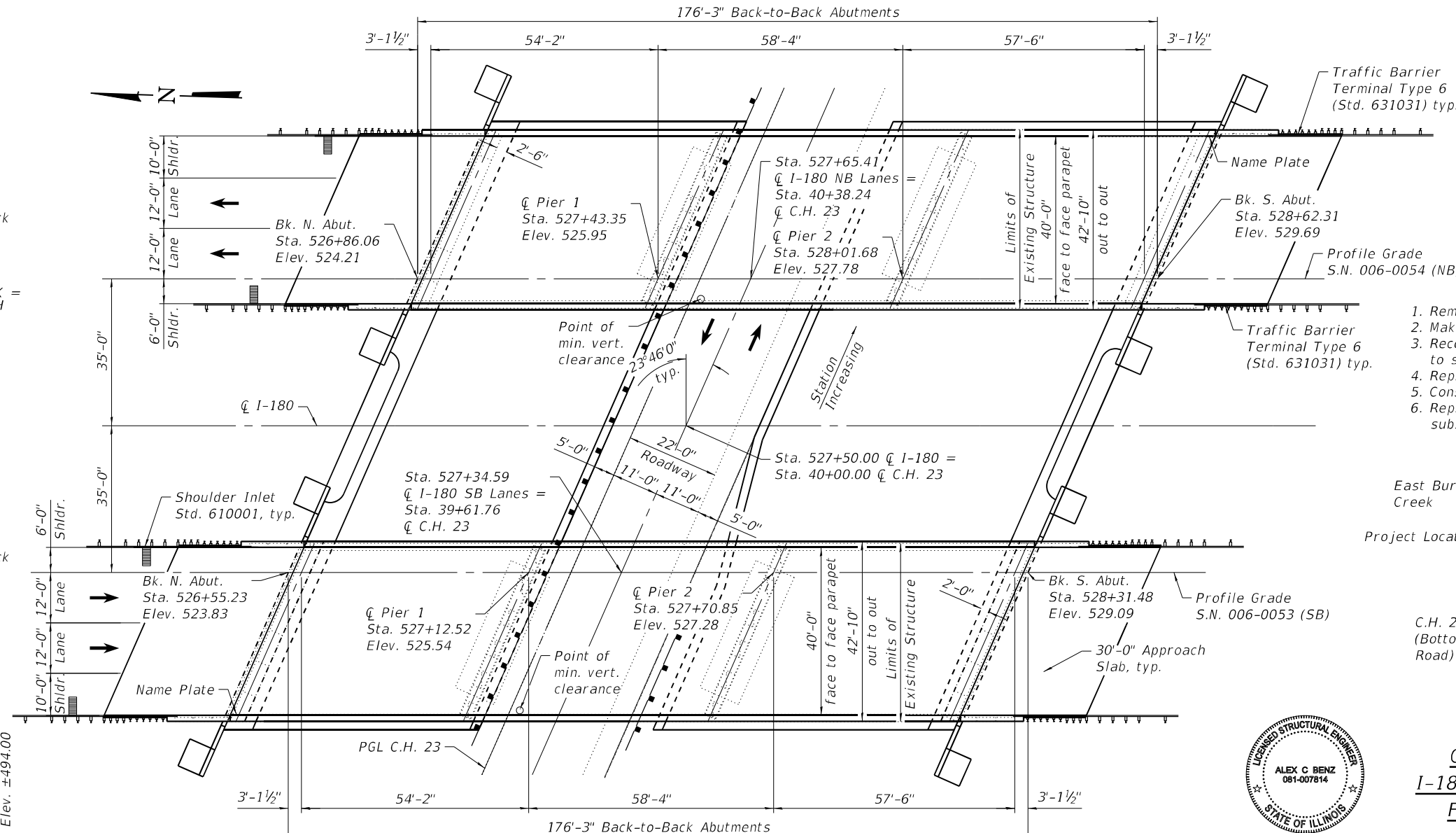
(Along inside Edge of Pavement)
Up to ¼ inch to be ground off the bridge deck and the bridge approach slabs. The Profile Grade shows the final grade after grinding.



PROFILE GRADE - COUNTY HWY 23



ELEVATION
* 27'-0" N.B. Min. Cl.
24'-0" S.B. Min. Cl.



PLAN

LOADING HL-93
Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS
2020 AASHTO LRFD Bridge Design Specifications, 9th Edition

DESIGN STRESSES

FIELD UNITS
NEW CONSTRUCTION
f'c = 3,500 psi
f'c = 4,000 psi (Superstructure)
fy = 60,000 psi (Reinforcement)
fy = 50,000 psi (M270 Grade 50)

EXISTING CONSTRUCTION
f'c = 1,400 psi (1967 Construction)
fs = 20,000 psi (Reinforcement)

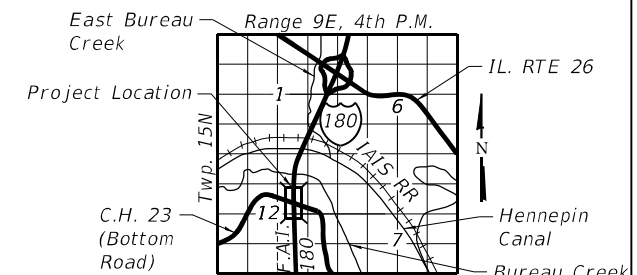
SEISMIC DATA

Seismic Performance Category (SPC) = A
Acceleration Coefficient (A) = 0.04g
Soil Coefficient (S) = 1.5

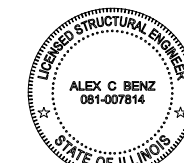


SCOPE OF WORK

1. Remove and replace existing superstructure.
2. Make new deck composite full length.
3. Reconfigure existing abutments and wingwalls to semi-integral configuration.
4. Replace bearings utilizing extensions.
5. Construct approach slabs.
6. Replace north and south slopewalls and repair substructures.



LOCATION SKETCH



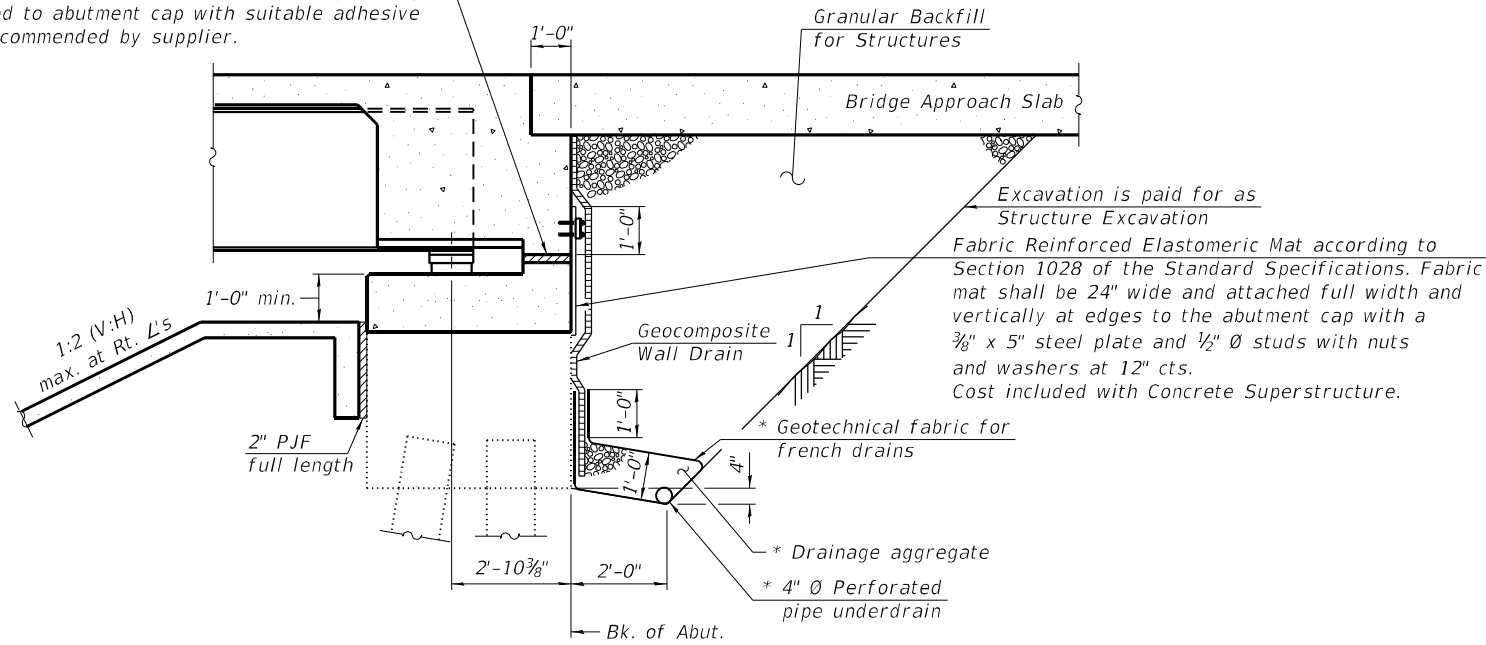
Signed: *Alex C. Benz*
Date: 1/11/2024
License Expires: 11/30/2024

GENERAL PLAN & ELEVATION
I-180 OVER C.H. 23 (BOTTOM ROAD)
F.A.I. 180-SEC. (06-2HB-1)ES
BUREAU COUNTY
STATION 527+50.00
STRUCTURE NO. 006-0053 (SB)
STRUCTURE NO. 006-0054 (NB)

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	PLOT SCALE =	DRAWN - ACB	REVISED -			SHEET 1 OF 42 SHEETS	CONTRACT NO. 66K66		ILLINOIS FED. AID PROJECT	
	PLOT DATE = 1/11/2024	CHECKED - CDL	REVISED -							

2" PJF (per Article 1051.09 of the Standard Specifications) full width and vertically at edges bonded to abutment cap with suitable adhesive as recommended by supplier.

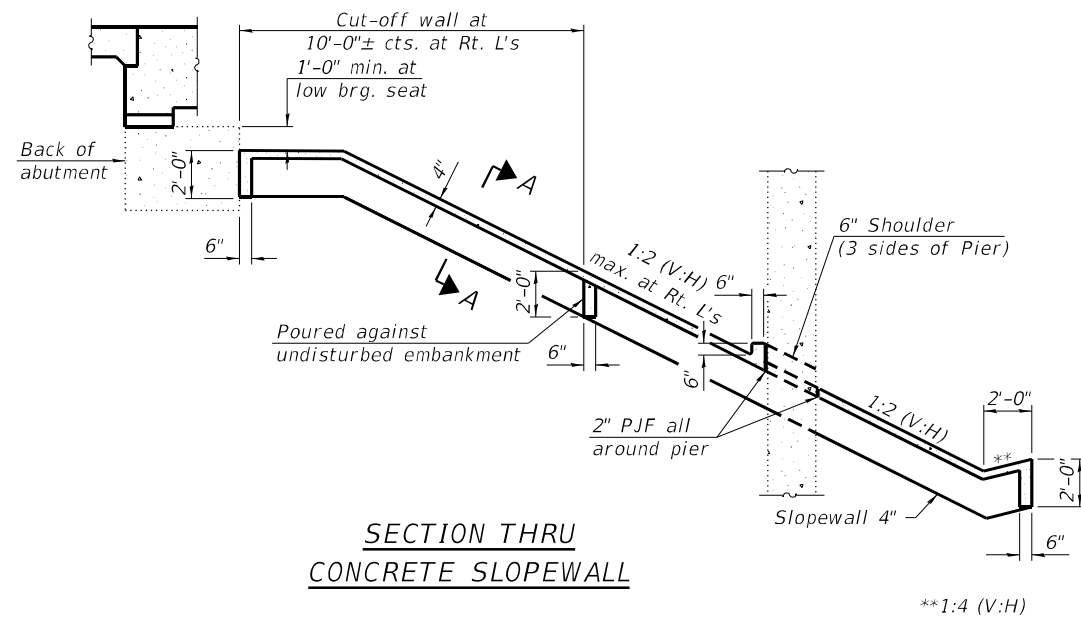


SECTION THRU SEMI-INTEGRAL ABUTMENT
(Horiz. dim. at Rt. L's)

*Included in the cost of Pipe Underdrains for Structures.

Note:

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).



SECTION THRU CONCRETE SLOPEWALL

**1:4 (V:H)

STA. 527+51.27
RE-BUILT BY
STATE OF ILLINOIS
F.A.I. Rte. 180 Sec. (06-2HB-1)BR
LOADING HL-93
STR. NO. 006-0053

STA. 527+51.27
RE-BUILT BY
STATE OF ILLINOIS
F.A.I. Rte. 180 Sec. (06-2HB-1)BR
LOADING HL-93
STR. NO. 006-0054

NAME PLATE
See Std. 515001

Existing Name Plate shall be cleaned and relocated next to new Name Plate. Cost included with Name Plates.

GENERAL NOTES

Fasteners shall be ASTM F 3125 Grade A325 Type 1, mechanically galvanized bolts in painted areas. Bolts 7/8 in. diameter, holes 15/16 in. diameter, unless otherwise noted.

Calculated weight of Structural Steel = 158,460 lb (Grade 50)
34,210 lb (Grade 36)

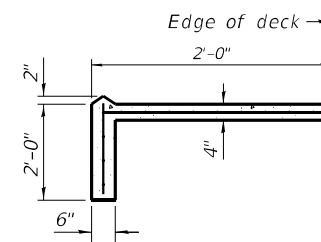
No field welding is permitted except as specified in the contract documents. Reinforcement bars designated (E) shall be epoxy coated.

Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8 in. (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.

Plan dimensions and details relative to the existing structure have been taken from existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

The Inorganic Zinc Rich Primer/Acrylic/Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Interstate Green, Munsell No. 7.5G 4/8.

Deck slab repairs may be required on the Stage 1 Traffic existing bridge (SN 006-0054) before shifting traffic to it. See Roadway Plans for more information and quantities.



SECTION A-A

Slope wall shall be reinforced with welded wire fabric, 6 in. x 6 in. - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Removal of Existing Superstructures	Each	2		2
Concrete Removal	Cu. Yd.		75.6	75.6
Slope Wall Removal	Sq. Yd.		2,336	2,336
Structure Excavation	Cu. Yd.		378	378
Concrete Structures	Cu. Yd.		145.8	145.8
Concrete Superstructure	Cu. Yd.	588.6		588.6
Protective Coat	Sq. Yd.	2,464		2,464
Concrete Superstructure (Approach Slab)	Cu. Yd.	237.6		237.6
Furnishing and Erecting Structural Steel	L. Sum	0.5		0.5
Stud Shear Connectors	Each	7,092		7,092
Reinforcement Bars, Epoxy Coated	Pound	232,100	20,950	253,050
Slope Wall 4 Inch	Sq. Yd.		2,322	2,322
Name Plates	Each	2		2
Elastomeric Bearing Assembly, Type I	Each		36	36
Anchor Bolts, 3/4"	Each		120	120
Granular Backfill For Structures	Cu. Yd.		349	349
Geocomposite Wall Drain	Sq. Yd.		193	193
Pipe Underdrains For Structures 4"	Foot		345	345
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	1,248		1,248
Bar Terminator	Each	500		500
Structural Repair of Concrete (Depth Equal to or Less than 5 Inches)	Sq. Ft.		75	75
Structural Repair of Concrete (Depth Greater than 5 Inches)	Sq. Ft.		40	40
Diamond Grinding (Bridge Section)	Sq. Yd.	2,252		2,252

INDEX OF SHEETS

- 1 General Plan and Elevation
- 2 General Data
- 3 Temporary Concrete Barrier
- 4 Top of Slab Elevations (SB)
- 5 Top of Slab Elevations (SB)
- 6 Top of Slab Elevations (SB)
- 7 Top of Slab Elevations (NB)
- 8 Top of Slab Elevations (NB)
- 9 Top of Slab Elevations (NB)
- 10 Top of North Approach Slab Elevations (SB)
- 11 Top of South Approach Slab Elevations (SB)
- 12 Top of North Approach Slab Elevations (NB)
- 13 Top of South Approach Slab Elevations (NB)
- 14 Superstructure (SB)
- 15 Superstructure Details (SB)
- 16 Diaphragm Details (SB)
- 17 Superstructure (NB)
- 18 Superstructure Details (NB)
- 19 Diaphragm Details (NB)
- 20 Bridge Approach Slab Details (SB)
- 21 Bridge Approach Slab Details (SB)
- 22 Bridge Approach Slab Details (NB)
- 23 Bridge Approach Slab Details (NB)
- 24 Framing Plan (SB)
- 25 Framing Plan (NB)
- 26 Structural Steel Details
- 27 Abutment Bearing Details (SB)
- 28 Abutment Bearing Details (NB)
- 29 Pier Bearing Details
- 30 Abutment Concrete Removal
- 31 North Abutment (SB)
- 32 South Abutment (SB)
- 33 Wingwall Extension Details (SB)
- 34 North Abutment (NB)
- 35 South Abutment (NB)
- 36 Wingwall Extension Details (NB)
- 37 Pier Removal and Repairs
- 38 Pier 1 Details (SB)
- 39 Pier 1 Details (SB)
- 40 Pier 1 Details (NB)
- 41 Pier 2 Details (NB)
- 42 Concrete Parapet Slipforming Option

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EFK Moen
Civil Engineering Design

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CHECKED - CDL
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DRAWN - ACB
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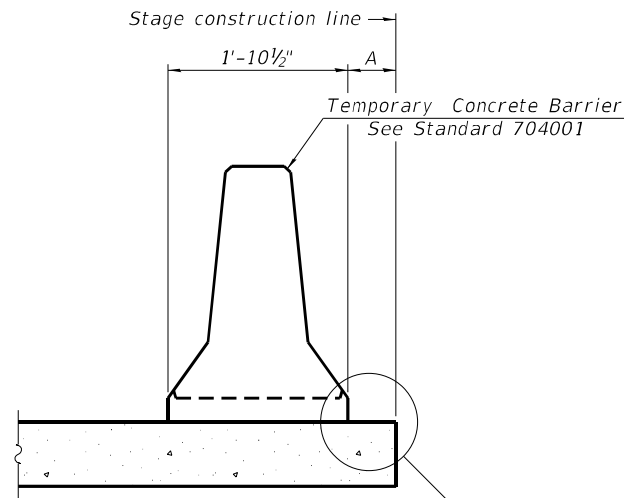
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL DATA
STRUCTURE NO. 006-0053 (SB) & 006-0054 (NB)

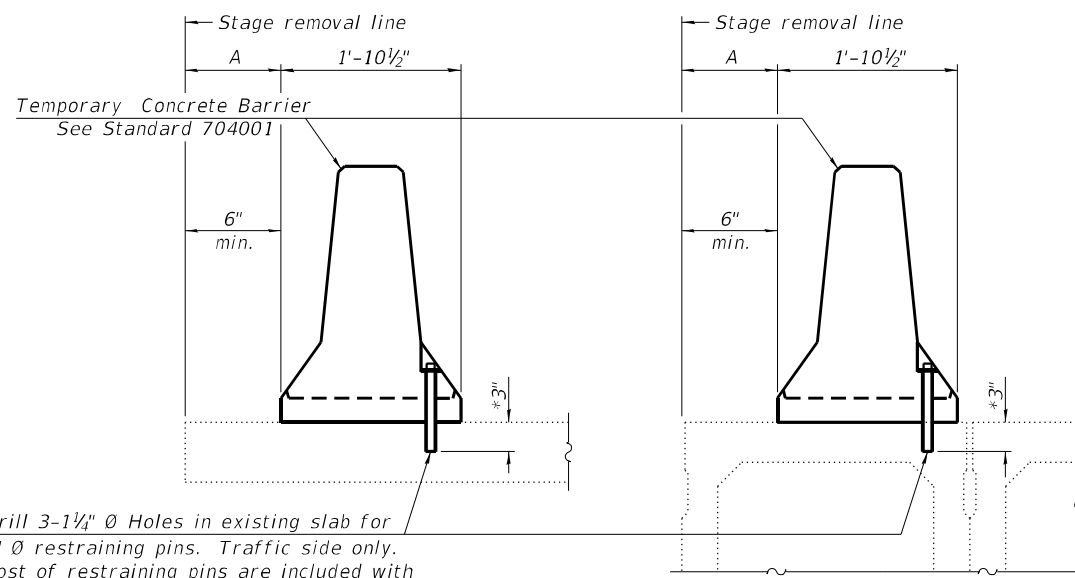
SHEET 2 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	222
CONTRACT NO. 66K66				
		ILLINOIS	FED. AID PROJECT	



When "A" is 3'-1" or less, the temporary concrete barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

NEW SLAB OR NEW DECK BEAM



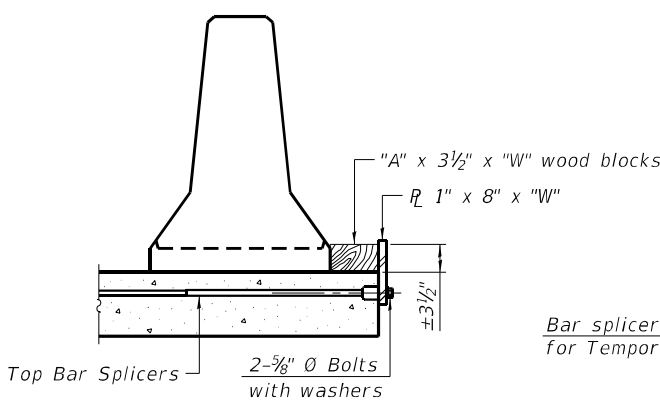
Drill 3-1/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint is required when "A" is greater than 3'-1".

EXISTING SLAB

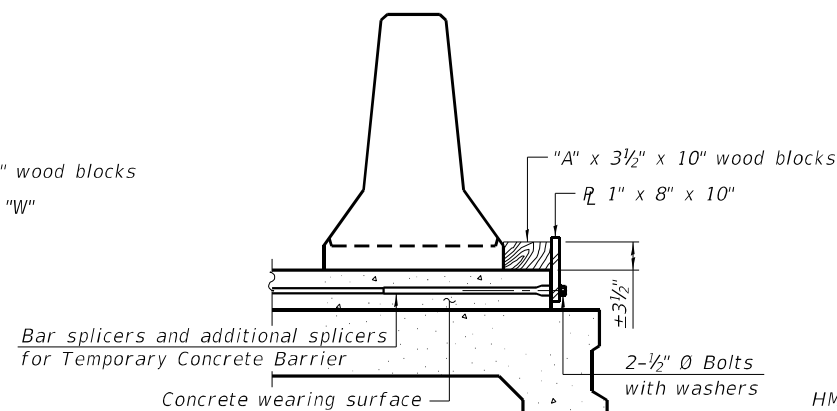
EXISTING DECK BEAM

* When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

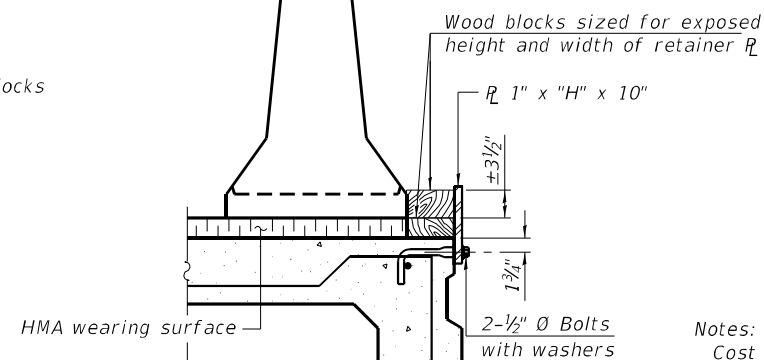
SECTIONS THRU SLAB OR DECK BEAM



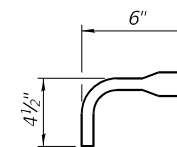
DETAIL I



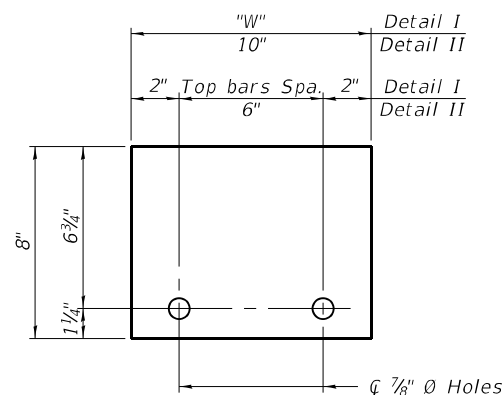
DETAIL II



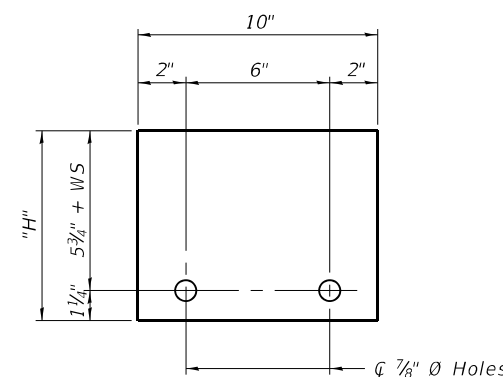
DETAIL III



BAR SPLICER FOR #4 BAR - DETAIL III



STEEL RETAINER 1" x 8" x "W" (Detail I and II)



STEEL RETAINER 1" x "H" x 10" (Detail III)

Notes:

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate center of each temporary concrete barrier.

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than 1 1/2', the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6' to accommodate the shear key clamping device.

Detail I - Installation for a new bridge deck or bridge slab.

Detail II - Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

RAILING CRITERIA

NCHRP 350 Test Level	3
Railing Weight (plf)	440

R-27 10-12-2021

EFK•Moen
Civil Engineering Design

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DESIGNED -	ACB
CHECKED -	CDL
PLOT SCALE =	
DRAWN -	ACB
PLOT DATE =	1/11/2024
CHECKED -	CDL
REVISED -	

DESIGNED -	ACB
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

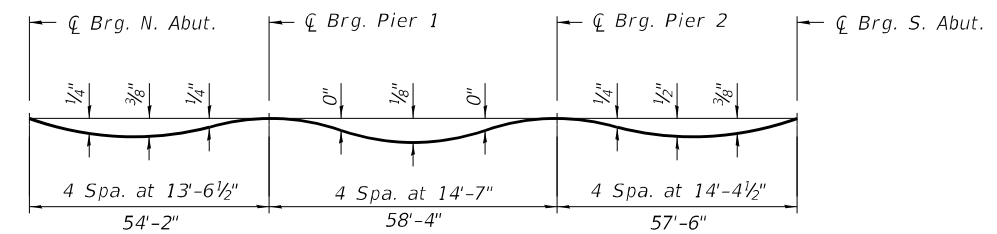
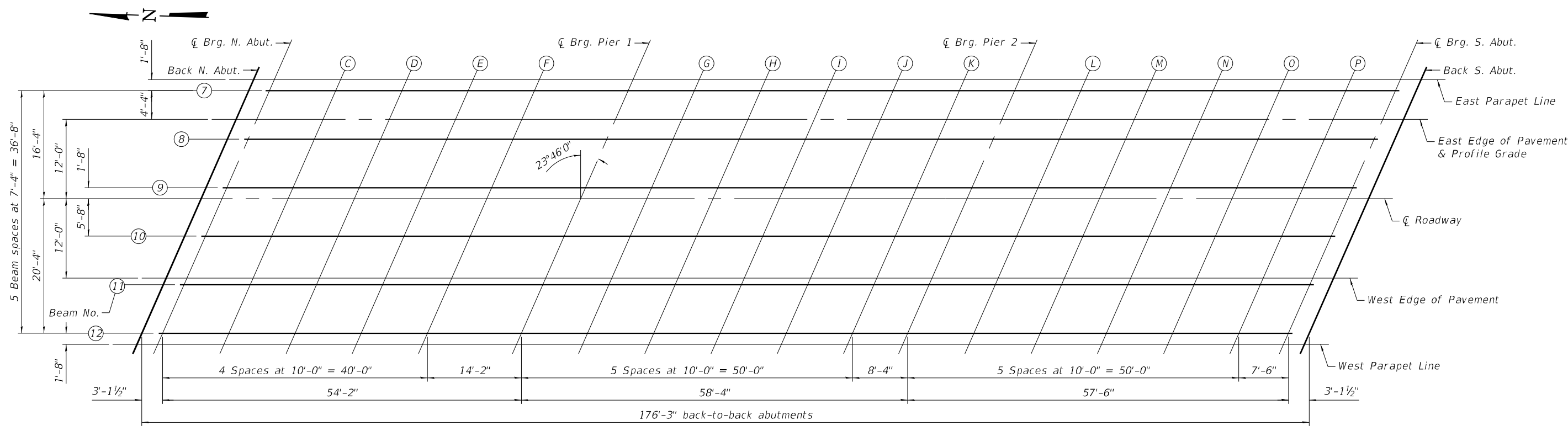
TEMPORARY CONCRETE BARRIER
STRUCTURE NO. 006-0053 (SB) & 006-0054 (NB)

SHEET 3 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	223
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

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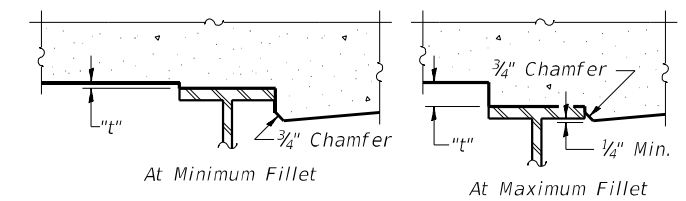
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DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

Note:
 The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections and grinding as shown on sheets 5 and 6 of 42.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown above. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection and Grinding" shown on sheets 5 and 6 of 42, minus the initial slab thickness prior to grinding, equals the fillet heights "t" above top flange of beams.
 The slab is to be ground after curing to achieve smoothness, but the slab is not to be ground to elevations below the "Theoretical Grade Elevations" shown on sheets 5 and 6 of 42. For grinding the deck, see Special Provisions.

FILLET HEIGHTS

E-S1 1-14-2019

EFK Moen
 Civil Engineering Design

USER NAME = ABenz	DESIGNED - ACB	REVISED -
PLOT SCALE =	CHECKED - CDL	REVISED -
PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
	CHECKED - CDL	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 006-0053 (SB)

SHEET 4 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	224
CONTRACT NO. 66K66				

ILLINOIS FED. AID PROJECT

EAST PARAPET LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+57.87	-6.00	523.79	523.81
☉ Brg. N. Abut.	526+61.00	-6.00	523.88	523.91
C	526+71.00	-6.00	524.18	524.22
D	526+81.00	-6.00	524.48	524.53
E	526+91.00	-6.00	524.78	524.83
F	527+01.00	-6.00	525.08	525.12
☉ Brg. Pier 1	527+15.16	-6.00	525.50	525.52
G	527+25.16	-6.00	525.80	525.82
H	527+35.16	-6.00	526.10	526.12
I	527+45.16	-6.00	526.39	526.42
J	527+55.16	-6.00	526.69	526.72
K	527+65.16	-6.00	526.99	527.01
☉ Brg. Pier 2	527+73.49	-6.00	527.24	527.26
L	527+83.49	-6.00	527.54	527.57
M	527+93.49	-6.00	527.83	527.89
N	528+03.49	-6.00	528.13	528.19
O	528+13.49	-6.00	528.43	528.49
P	528+23.49	-6.00	528.73	528.77
☉ Brg. S. Abut.	528+30.99	-6.00	528.95	528.97
Back S. Abut.	528+34.12	-6.00	529.05	529.07

BEAM 7

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+57.14	-4.33	523.80	523.82
☉ Brg. N. Abut.	526+60.26	-4.33	523.90	523.92
C	526+70.26	-4.33	524.19	524.25
D	526+80.26	-4.33	524.49	524.53
E	526+90.26	-4.33	524.79	524.81
F	527+00.26	-4.33	525.09	525.11
☉ Brg. Pier 1	527+14.43	-4.33	525.51	525.53
G	527+24.43	-4.33	525.81	525.83
H	527+34.43	-4.33	526.11	526.13
I	527+44.43	-4.33	526.41	526.43
J	527+54.43	-4.33	526.70	526.72
K	527+64.43	-4.33	527.00	527.04
☉ Brg. Pier 2	527+72.76	-4.33	527.25	527.30
L	527+82.76	-4.33	527.55	527.57
M	527+92.76	-4.33	527.85	527.87
N	528+02.76	-4.33	528.14	528.17
O	528+12.76	-4.33	528.44	528.47
P	528+22.76	-4.33	528.74	528.76
☉ Brg. S. Abut.	528+30.26	-4.33	528.96	528.99
Back S. Abut.	528+33.39	-4.33	529.06	529.08

EAST EDGE OF PAVEMENT & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+55.23	0.00	523.83	523.85
☉ Brg. N. Abut.	526+58.35	0.00	523.93	523.95
C	526+68.35	0.00	524.22	524.26
D	526+78.35	0.00	524.52	524.57
E	526+88.35	0.00	524.82	524.87
F	526+98.35	0.00	525.12	525.16
☉ Brg. Pier 1	527+12.52	0.00	525.54	525.56
G	527+22.52	0.00	525.84	525.86
H	527+32.52	0.00	526.14	526.16
I	527+42.52	0.00	526.44	526.46
J	527+52.52	0.00	526.73	526.76
K	527+62.52	0.00	527.03	527.05
☉ Brg. Pier 2	527+70.85	0.00	527.28	527.30
L	527+80.85	0.00	527.58	527.61
M	527+90.85	0.00	527.88	527.93
N	528+00.85	0.00	528.17	528.24
O	528+10.85	0.00	528.47	528.53
P	528+20.85	0.00	528.77	528.81
☉ Brg. S. Abut.	528+28.35	0.00	528.99	529.01
Back S. Abut.	528+31.48	0.00	529.09	529.11

BEAM 8

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+53.91	3.00	523.84	523.86
☉ Brg. N. Abut.	526+57.03	3.00	523.93	523.95
C	526+67.03	3.00	524.23	524.27
D	526+77.03	3.00	524.53	524.58
E	526+87.03	3.00	524.83	524.88
F	526+97.03	3.00	525.12	525.16
☉ Brg. Pier 1	527+11.20	3.00	525.55	525.57
G	527+21.20	3.00	525.84	525.87
H	527+31.20	3.00	526.14	526.17
I	527+41.20	3.00	526.44	526.47
J	527+51.20	3.00	526.74	526.76
K	527+61.20	3.00	527.04	527.06
☉ Brg. Pier 2	527+69.53	3.00	527.29	527.31
L	527+79.53	3.00	527.58	527.62
M	527+89.53	3.00	527.88	527.93
N	527+99.53	3.00	528.18	528.24
O	528+09.53	3.00	528.48	528.54
P	528+19.53	3.00	528.78	528.82
☉ Brg. S. Abut.	528+27.03	3.00	529.00	529.02
Back S. Abut.	528+30.16	3.00	529.09	529.11

BEAM 9

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+50.68	10.33	523.85	523.87
☉ Brg. N. Abut.	526+53.80	10.33	523.95	523.97
C	526+63.80	10.33	524.24	524.28
D	526+73.80	10.33	524.54	524.59
E	526+83.80	10.33	524.84	524.89
F	526+93.80	10.33	525.14	525.18
☉ Brg. Pier 1	527+07.97	10.33	525.56	525.58
G	527+17.97	10.33	525.86	525.88
H	527+27.97	10.33	526.16	526.18
I	527+37.97	10.33	526.45	526.48
J	527+47.97	10.33	526.75	526.78
K	527+57.97	10.33	527.05	527.07
☉ Brg. Pier 2	527+66.30	10.33	527.30	527.32
L	527+76.30	10.33	527.60	527.63
M	527+86.30	10.33	527.90	527.95
N	527+96.30	10.33	528.19	528.26
O	528+06.30	10.33	528.49	528.55
P	528+16.30	10.33	528.79	528.83
☉ Brg. S. Abut.	528+23.80	10.33	529.01	529.03
Back S. Abut.	528+26.93	10.33	529.11	529.13

☉ ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+49.95	12.00	523.86	523.88
☉ Brg. N. Abut.	526+53.07	12.00	523.95	523.97
C	526+63.07	12.00	524.25	524.29
D	526+73.07	12.00	524.54	524.60
E	526+83.07	12.00	524.84	524.89
F	526+93.07	12.00	525.14	525.18
☉ Brg. Pier 1	527+07.24	12.00	525.56	525.58
G	527+17.24	12.00	525.86	525.88
H	527+27.24	12.00	526.16	526.19
I	527+37.24	12.00	526.46	526.49
J	527+47.24	12.00	526.76	526.78
K	527+57.24	12.00	527.05	527.07
☉ Brg. Pier 2	527+65.57	12.00	527.30	527.32
L	527+75.57	12.00	527.60	527.64
M	527+85.57	12.00	527.90	527.95
N	527+95.57	12.00	528.20	528.26
O	528+05.57	12.00	528.49	528.55
P	528+15.57	12.00	528.79	528.83
☉ Brg. S. Abut.	528+23.07	12.00	529.02	529.04
Back S. Abut.	528+26.20	12.00	529.11	529.13

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MODEL: Default
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PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
	CHECKED - CDL	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 006-0053 (SB)

SHEET 5 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	225
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

BEAM 10

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+47.45	17.67	523.70	523.72
☉ Brg. N. Abut.	526+50.57	17.67	523.79	523.81
C	526+60.57	17.67	524.09	524.13
D	526+70.57	17.67	524.39	524.44
E	526+80.57	17.67	524.68	524.73
F	526+90.57	17.67	524.98	525.02
☉ Brg. Pier 1	527+04.74	17.67	525.40	525.42
G	527+14.74	17.67	525.70	525.72
H	527+24.74	17.67	526.00	526.03
I	527+34.74	17.67	526.30	526.33
J	527+44.74	17.67	526.60	526.62
K	527+54.74	17.67	526.89	526.91
☉ Brg. Pier 2	527+63.07	17.67	527.14	527.16
L	527+73.07	17.67	527.44	527.48
M	527+83.07	17.67	527.74	527.79
N	527+93.07	17.67	528.04	528.10
O	528+03.07	17.67	528.34	528.39
P	528+13.07	17.67	528.63	528.67
☉ Brg. S. Abut.	528+20.57	17.67	528.86	528.88
Back S. Abut.	528+23.70	17.67	528.95	528.97

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+44.66	24.00	523.52	523.54
☉ Brg. N. Abut.	526+47.78	24.00	523.61	523.63
C	526+57.78	24.00	523.91	523.95
D	526+67.78	24.00	524.21	524.26
E	526+77.78	24.00	524.51	524.56
F	526+87.78	24.00	524.80	524.84
☉ Brg. Pier 1	527+01.95	24.00	525.23	525.25
G	527+11.95	24.00	525.52	525.55
H	527+21.95	24.00	525.82	525.85
I	527+31.95	24.00	526.12	526.15
J	527+41.95	24.00	526.42	526.44
K	527+51.95	24.00	526.72	526.74
☉ Brg. Pier 2	527+60.28	24.00	526.96	526.99
L	527+70.28	24.00	527.26	527.30
M	527+80.28	24.00	527.56	527.61
N	527+90.28	24.00	527.86	527.92
O	528+00.28	24.00	528.16	528.21
P	528+10.28	24.00	528.46	528.49
☉ Brg. S. Abut.	528+17.78	24.00	528.68	528.70
Back S. Abut.	528+20.91	24.00	528.77	528.79

BEAM 11

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+44.22	25.00	523.48	523.51
☉ Brg. N. Abut.	526+47.34	25.00	523.58	523.60
C	526+57.34	25.00	523.88	523.92
D	526+67.34	25.00	524.17	524.23
E	526+77.34	25.00	524.47	524.52
F	526+87.34	25.00	524.77	524.81
☉ Brg. Pier 1	527+01.51	25.00	525.19	525.21
G	527+11.51	25.00	525.49	525.51
H	527+21.51	25.00	525.79	525.82
I	527+31.51	25.00	526.09	526.12
J	527+41.51	25.00	526.39	526.41
K	527+51.51	25.00	526.68	526.70
☉ Brg. Pier 2	527+59.84	25.00	526.93	526.95
L	527+69.84	25.00	527.23	527.27
M	527+79.84	25.00	527.53	527.58
N	527+89.84	25.00	527.83	527.89
O	527+99.84	25.00	528.12	528.18
P	528+09.84	25.00	528.42	528.46
☉ Brg. S. Abut.	528+17.34	25.00	528.65	528.67
Back S. Abut.	528+20.47	25.00	528.74	528.76

BEAM 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+40.99	32.33	523.24	523.26
☉ Brg. N. Abut.	526+44.12	32.33	523.34	523.36
C	526+54.12	32.33	523.63	523.67
D	526+64.12	32.33	523.93	523.98
E	526+74.12	32.33	524.23	524.28
F	526+84.12	32.33	524.53	524.57
☉ Brg. Pier 1	526+98.28	32.33	524.95	524.97
G	527+08.28	32.33	525.25	525.27
H	527+18.28	32.33	525.55	525.57
I	527+28.28	32.33	525.84	525.87
J	527+38.28	32.33	526.14	526.17
K	527+48.28	32.33	526.44	526.46
☉ Brg. Pier 2	527+56.61	32.33	526.69	526.71
L	527+66.61	32.33	526.99	527.02
M	527+76.61	32.33	527.28	527.34
N	527+86.61	32.33	527.58	527.64
O	527+96.61	32.33	527.88	527.94
P	528+06.61	32.33	528.18	528.22
☉ Brg. S. Abut.	528+14.11	32.33	528.40	528.42
Back S. Abut.	528+17.24	32.33	528.50	528.52

WEST PARAPET LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+40.26	34.00	523.19	523.21
☉ Brg. N. Abut.	526+43.38	34.00	523.28	523.30
C	526+53.38	34.00	523.58	523.62
D	526+63.38	34.00	523.88	523.93
E	526+73.38	34.00	524.17	524.22
F	526+83.38	34.00	524.47	524.51
☉ Brg. Pier 1	526+97.55	34.00	524.89	524.92
G	527+07.55	34.00	525.19	525.21
H	527+17.55	34.00	525.49	525.52
I	527+27.55	34.00	525.79	525.82
J	527+37.55	34.00	526.09	526.11
K	527+47.55	34.00	526.39	526.41
☉ Brg. Pier 2	527+55.88	34.00	526.63	526.65
L	527+65.88	34.00	526.93	526.97
M	527+75.88	34.00	527.23	527.28
N	527+85.88	34.00	527.53	527.59
O	527+95.88	34.00	527.83	527.88
P	528+05.88	34.00	528.12	528.16
☉ Brg. S. Abut.	528+13.38	34.00	528.35	528.37
Back S. Abut.	528+16.51	34.00	528.44	528.46

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PLOT SCALE =		DRAWN -	ACB	REVISED -	
PLOT DATE =	1/11/2024	CHECKED -	CDL	REVISED -	

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

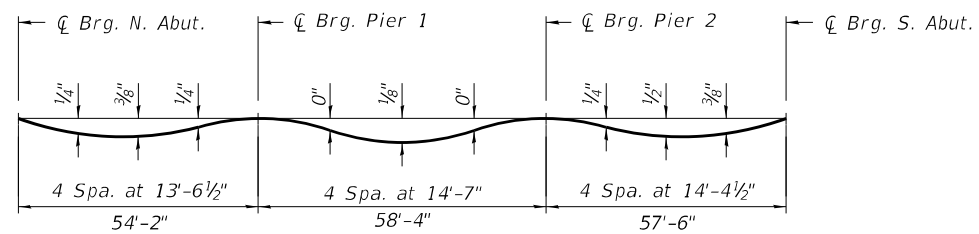
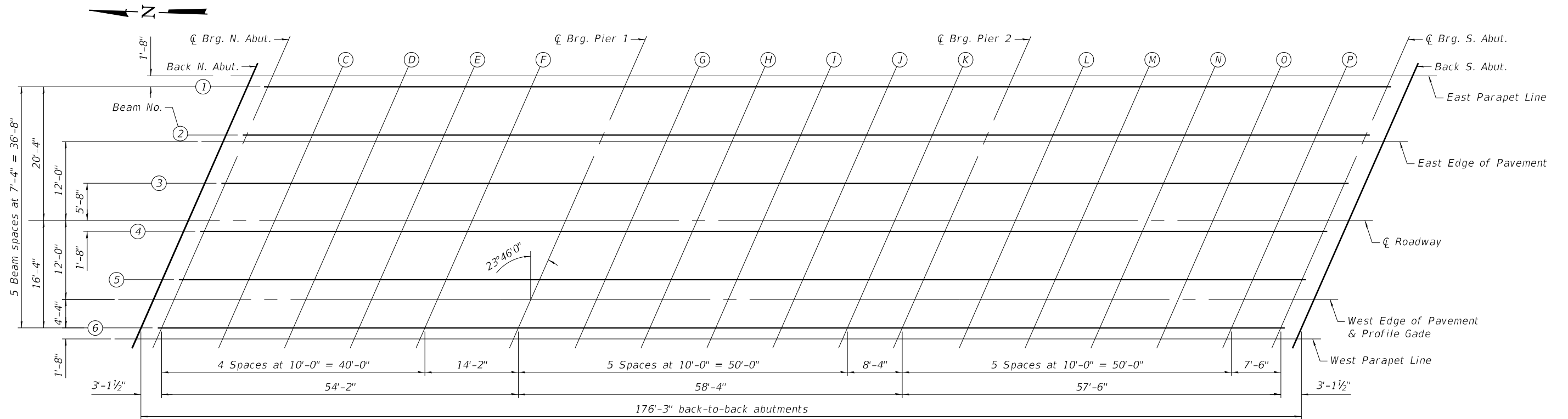
**TOP OF SLAB ELEVATIONS
STRUCTURE NO. 006-0053 (SB)**

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	226
CONTRACT NO. 66K66				

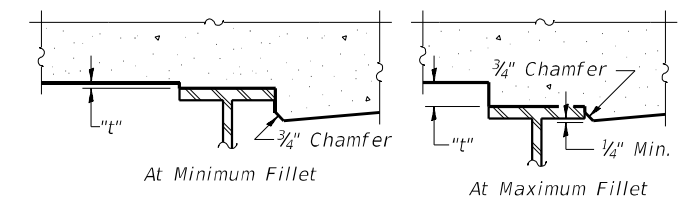
SHEET 6 OF 42 SHEETS

ILLINOIS FED. AID PROJECT

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Note:
 The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections and grinding as shown on sheets 8 and 9 of 42.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown above. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection and Grinding" shown on sheets 8 and 9 of 42, minus the initial slab thickness prior to grinding, equals the fillet heights "t" above top flange of beams.
 The slab is to be ground after curing to achieve smoothness, but the slab is not to be ground to elevations below the "Theoretical Grade Elevations" shown on sheets 8 and 9 of 42. For grinding the deck, see Special Provisions.

FILLET HEIGHTS

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STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
 STRUCTURE NO. 006-0054 (NB)

SHEET 7 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	227
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

EAST PARAPET LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	527+01.03	-34.00	524.46	524.48
☉ Brg. N. Abut.	527+04.16	-34.00	524.55	524.57
C	527+14.16	-34.00	524.86	524.90
D	527+24.16	-34.00	525.16	525.21
E	527+34.16	-34.00	525.47	525.52
F	527+44.16	-34.00	525.78	525.82
☉ Brg. Pier 1	527+58.32	-34.00	526.22	526.24
G	527+68.32	-34.00	526.53	526.55
H	527+78.32	-34.00	526.85	526.87
I	527+88.32	-34.00	527.16	527.19
J	527+98.32	-34.00	527.48	527.50
K	528+08.32	-34.00	527.79	527.81
☉ Brg. Pier 2	528+16.65	-34.00	528.05	528.07
L	528+26.65	-34.00	528.37	528.40
M	528+36.65	-34.00	528.68	528.74
N	528+46.65	-34.00	529.00	529.06
O	528+56.65	-34.00	529.31	529.37
P	528+66.65	-34.00	529.63	529.67
☉ Brg. S. Abut.	528+74.15	-34.00	529.86	529.89
Back S. Abut.	528+77.28	-34.00	529.96	529.98

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	527+00.30	-32.33	524.47	524.49
☉ Brg. N. Abut.	527+03.42	-32.33	524.56	524.58
C	527+13.42	-32.33	524.87	524.91
D	527+23.42	-32.33	525.17	525.22
E	527+33.42	-32.33	525.48	525.53
F	527+43.42	-32.33	525.79	525.83
☉ Brg. Pier 1	527+57.59	-32.33	526.23	526.25
G	527+67.59	-32.33	526.54	526.56
H	527+77.59	-32.33	526.86	526.88
I	527+87.59	-32.33	527.17	527.20
J	527+97.59	-32.33	527.49	527.51
K	528+07.59	-32.33	527.80	527.82
☉ Brg. Pier 2	528+15.92	-32.33	528.06	528.08
L	528+25.92	-32.33	528.38	528.41
M	528+35.92	-32.33	528.69	528.75
N	528+45.92	-32.33	529.01	529.07
O	528+55.92	-32.33	529.32	529.38
P	528+65.92	-32.33	529.64	529.68
☉ Brg. S. Abut.	528+73.42	-32.33	529.87	529.90
Back S. Abut.	528+76.55	-32.33	529.97	529.99

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+97.07	-25.00	524.52	524.54
☉ Brg. N. Abut.	527+00.19	-25.00	524.61	524.63
C	527+10.19	-25.00	524.92	524.96
D	527+20.19	-25.00	525.22	525.27
E	527+30.19	-25.00	525.53	525.58
F	527+40.19	-25.00	525.84	525.87
☉ Brg. Pier 1	527+54.36	-25.00	526.28	526.30
G	527+64.36	-25.00	526.59	526.61
H	527+74.36	-25.00	526.90	526.93
I	527+84.36	-25.00	527.22	527.24
J	527+94.36	-25.00	527.53	527.56
K	528+04.36	-25.00	527.85	527.87
☉ Brg. Pier 2	528+12.69	-25.00	528.11	528.13
L	528+22.69	-25.00	528.42	528.46
M	528+32.69	-25.00	528.74	528.79
N	528+42.69	-25.00	529.05	529.12
O	528+52.69	-25.00	529.37	529.43
P	528+62.69	-25.00	529.68	529.72
☉ Brg. S. Abut.	528+70.19	-25.00	529.92	529.94
Back S. Abut.	528+73.32	-25.00	530.02	530.04

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+96.63	-24.00	524.52	524.54
☉ Brg. N. Abut.	526+99.75	-24.00	524.62	524.64
C	527+09.75	-24.00	524.92	524.96
D	527+19.75	-24.00	525.23	525.28
E	527+29.75	-24.00	525.53	525.58
F	527+39.75	-24.00	525.84	525.88
☉ Brg. Pier 1	527+53.92	-24.00	526.28	526.30
G	527+63.92	-24.00	526.59	526.62
H	527+73.92	-24.00	526.91	526.93
I	527+83.92	-24.00	527.22	527.25
J	527+93.92	-24.00	527.54	527.56
K	528+03.92	-24.00	527.85	527.87
☉ Brg. Pier 2	528+12.25	-24.00	528.11	528.14
L	528+22.25	-24.00	528.43	528.47
M	528+32.25	-24.00	528.74	528.80
N	528+42.25	-24.00	529.06	529.12
O	528+52.25	-24.00	529.37	529.43
P	528+62.25	-24.00	529.69	529.73
☉ Brg. S. Abut.	528+69.75	-24.00	529.93	529.95
Back S. Abut.	528+72.88	-24.00	530.02	530.05

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+93.84	-17.67	524.53	524.56
☉ Brg. N. Abut.	526+96.96	-17.67	524.63	524.65
C	527+06.96	-17.67	524.93	524.97
D	527+16.96	-17.67	525.24	525.29
E	527+26.96	-17.67	525.54	525.59
F	527+36.96	-17.67	525.85	525.89
☉ Brg. Pier 1	527+51.13	-17.67	526.29	526.31
G	527+61.13	-17.67	526.60	526.62
H	527+71.13	-17.67	526.91	526.94
I	527+81.13	-17.67	527.23	527.26
J	527+91.13	-17.67	527.54	527.57
K	528+01.13	-17.67	527.86	527.88
☉ Brg. Pier 2	528+09.46	-17.67	528.12	528.14
L	528+19.46	-17.67	528.44	528.47
M	528+29.46	-17.67	528.75	528.80
N	528+39.46	-17.67	529.07	529.13
O	528+49.46	-17.67	529.38	529.44
P	528+59.46	-17.67	529.70	529.74
☉ Brg. S. Abut.	528+66.96	-17.67	529.93	529.95
Back S. Abut.	528+70.09	-17.67	530.03	530.05

☉ ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+91.34	-12.00	524.54	524.56
☉ Brg. N. Abut.	526+94.47	-12.00	524.64	524.66
C	527+04.47	-12.00	524.94	524.98
D	527+14.47	-12.00	525.25	525.30
E	527+24.47	-12.00	525.55	525.60
F	527+34.47	-12.00	525.86	525.90
☉ Brg. Pier 1	527+48.63	-12.00	526.30	526.32
G	527+58.63	-12.00	526.61	526.63
H	527+68.63	-12.00	526.92	526.95
I	527+78.63	-12.00	527.24	527.26
J	527+88.63	-12.00	527.55	527.58
K	527+98.63	-12.00	527.87	527.89
☉ Brg. Pier 2	528+06.96	-12.00	528.13	528.15
L	528+16.96	-12.00	528.44	528.48
M	528+26.96	-12.00	528.76	528.81
N	528+36.96	-12.00	529.07	529.13
O	528+46.96	-12.00	529.39	529.45
P	528+56.96	-12.00	529.70	529.74
☉ Brg. S. Abut.	528+64.46	-12.00	529.94	529.96
Back S. Abut.	528+67.59	-12.00	530.04	530.06

E-S1 1-14-2019



USER NAME = ABenz	DESIGNED - ACB	REVISED -
PLOT SCALE =	CHECKED - CDL	REVISED -
PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
	CHECKED - CDL	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 006-0054 (NB)

SHEET 8 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	228
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

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BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+90.61	-10.33	524.50	524.52
☉ Brg. N. Abut.	526+93.73	-10.33	524.59	524.61
C	527+03.73	-10.33	524.89	524.93
D	527+13.73	-10.33	525.20	525.25
E	527+23.73	-10.33	525.50	525.55
F	527+33.73	-10.33	525.81	525.85
☉ Brg. Pier 1	527+47.90	-10.33	526.25	526.27
G	527+57.90	-10.33	526.56	526.58
H	527+67.90	-10.33	526.87	526.90
I	527+77.90	-10.33	527.19	527.22
J	527+87.90	-10.33	527.50	527.53
K	527+97.90	-10.33	527.82	527.84
☉ Brg. Pier 2	528+06.23	-10.33	528.08	528.10
L	528+16.23	-10.33	528.40	528.43
M	528+26.23	-10.33	528.71	528.76
N	528+36.23	-10.33	529.03	529.09
O	528+46.23	-10.33	529.34	529.40
P	528+56.23	-10.33	529.66	529.69
☉ Brg. S. Abut.	528+63.73	-10.33	529.89	529.91
Back S. Abut.	528+66.86	-10.33	529.99	530.01

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+87.38	-3.00	524.29	524.31
☉ Brg. N. Abut.	526+90.50	-3.00	524.38	524.40
C	527+00.50	-3.00	524.69	524.73
D	527+10.50	-3.00	524.99	525.04
E	527+20.50	-3.00	525.30	525.35
F	527+30.50	-3.00	525.60	525.64
☉ Brg. Pier 1	527+44.67	-3.00	526.04	526.06
G	527+54.67	-3.00	526.35	526.37
H	527+64.67	-3.00	526.66	526.69
I	527+74.67	-3.00	526.98	527.00
J	527+84.67	-3.00	527.29	527.32
K	527+94.67	-3.00	527.61	527.63
☉ Brg. Pier 2	528+03.00	-3.00	527.87	527.89
L	528+13.00	-3.00	528.18	528.22
M	528+23.00	-3.00	528.50	528.55
N	528+33.00	-3.00	528.81	528.87
O	528+43.00	-3.00	529.13	529.19
P	528+53.00	-3.00	529.44	529.48
☉ Brg. S. Abut.	528+60.50	-3.00	529.68	529.70
Back S. Abut.	528+63.63	-3.00	529.78	529.80

WEST EDGE OF PAVEMENT & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+86.06	0.00	524.21	524.23
☉ Brg. N. Abut.	526+89.18	0.00	524.30	524.32
C	526+99.18	0.00	524.60	524.64
D	527+09.18	0.00	524.90	524.96
E	527+19.18	0.00	525.21	525.26
F	527+29.18	0.00	525.52	525.56
☉ Brg. Pier 1	527+43.35	0.00	525.95	525.97
G	527+53.35	0.00	526.26	526.29
H	527+63.35	0.00	526.58	526.60
I	527+73.35	0.00	526.89	526.92
J	527+83.35	0.00	527.20	527.23
K	527+93.35	0.00	527.52	527.54
☉ Brg. Pier 2	528+01.68	0.00	527.78	527.80
L	528+11.68	0.00	528.10	528.13
M	528+21.68	0.00	528.41	528.46
N	528+31.68	0.00	528.73	528.79
O	528+41.68	0.00	529.04	529.10
P	528+51.68	0.00	529.36	529.40
☉ Brg. S. Abut.	528+59.18	0.00	529.59	529.61
Back S. Abut.	528+62.31	0.00	529.69	529.71

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+84.15	4.33	524.06	524.08
☉ Brg. N. Abut.	526+87.28	4.33	524.15	524.18
C	526+97.28	4.33	524.46	524.50
D	527+07.28	4.33	524.76	524.81
E	527+17.28	4.33	525.06	525.11
F	527+27.28	4.33	525.37	525.41
☉ Brg. Pier 1	527+41.44	4.33	525.81	525.83
G	527+51.44	4.33	526.12	526.14
H	527+61.44	4.33	526.43	526.46
I	527+71.44	4.33	526.74	526.77
J	527+81.44	4.33	527.06	527.08
K	527+91.44	4.33	527.37	527.39
☉ Brg. Pier 2	527+99.77	4.33	527.63	527.66
L	528+09.77	4.33	527.95	527.99
M	528+19.77	4.33	528.26	528.32
N	528+29.77	4.33	528.58	528.64
O	528+39.77	4.33	528.89	528.95
P	528+49.77	4.33	529.21	529.25
☉ Brg. S. Abut.	528+57.27	4.33	529.45	529.47
Back S. Abut.	528+60.40	4.33	529.54	529.57

WEST PARAPET LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Back N. Abut.	526+83.42	6.00	524.01	524.03
☉ Brg. N. Abut.	526+86.54	6.00	524.10	524.12
C	526+96.54	6.00	524.40	524.44
D	527+06.54	6.00	524.70	524.76
E	527+16.54	6.00	525.01	525.06
F	527+26.54	6.00	525.32	525.35
☉ Brg. Pier 1	527+40.71	6.00	525.75	525.77
G	527+50.71	6.00	526.06	526.08
H	527+60.71	6.00	526.37	526.40
I	527+70.71	6.00	526.69	526.71
J	527+80.71	6.00	527.00	527.03
K	527+90.71	6.00	527.32	527.34
☉ Brg. Pier 2	527+99.04	6.00	527.58	527.60
L	528+09.04	6.00	527.89	527.93
M	528+19.04	6.00	528.21	528.26
N	528+29.04	6.00	528.52	528.59
O	528+39.04	6.00	528.84	528.90
P	528+49.04	6.00	529.15	529.19
☉ Brg. S. Abut.	528+56.54	6.00	529.39	529.41
Back S. Abut.	528+59.67	6.00	529.49	529.51

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CHECKED -	CDL	REVISOR -			
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PLOT DATE =	1/11/2024	CHECKED -	CDL	REVISED -	

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS
STRUCTURE NO. 006-0054 (NB)**

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	229
CONTRACT NO. 66K66				
SHEET 9 OF 42 SHEETS		ILLINOIS FED. AID PROJECT		

EAST EDGE OF SHOULDER

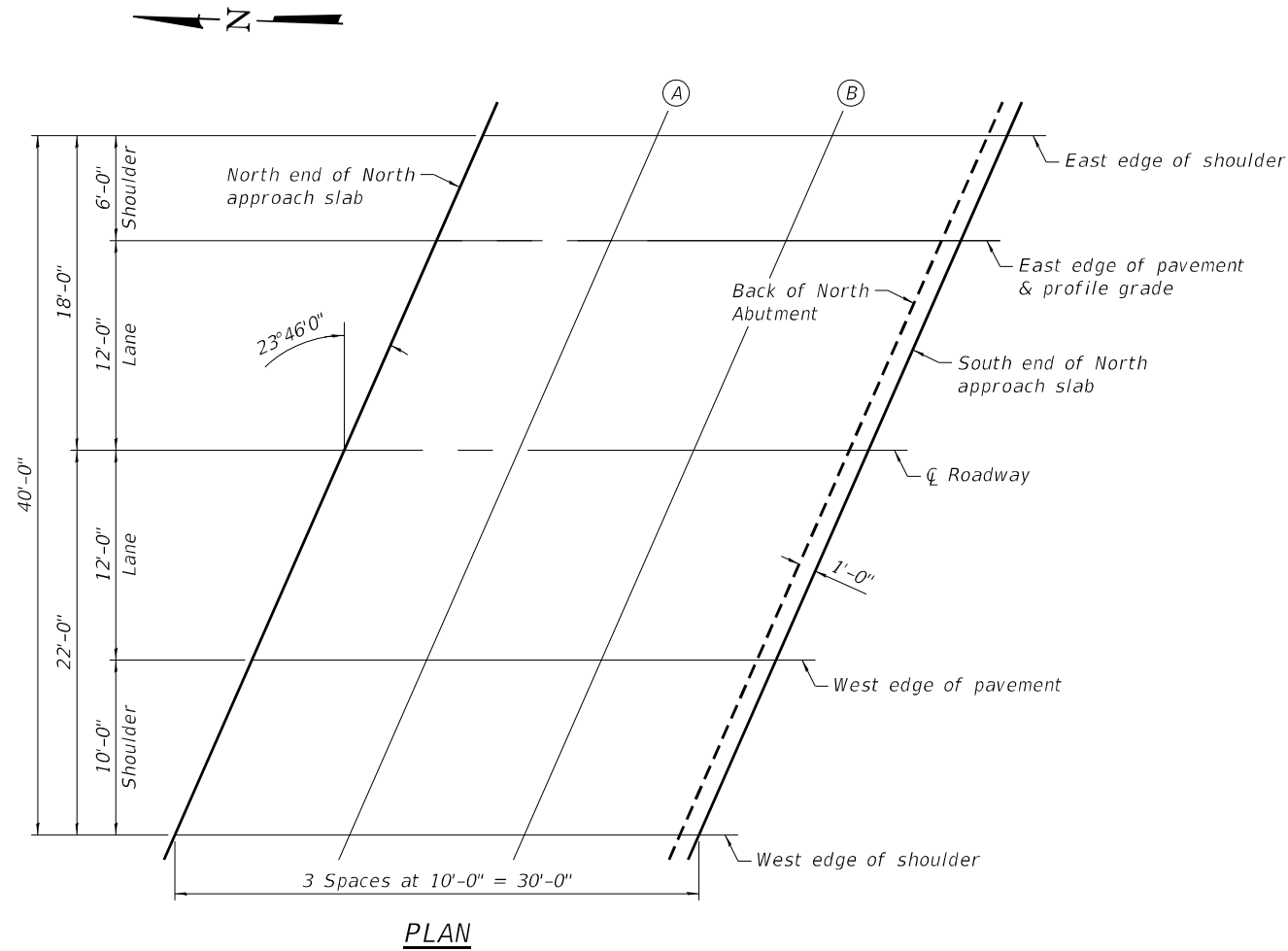
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+28.96	-6.00	522.93	522.95
A	526+38.96	-6.00	523.23	523.25
B	526+48.96	-6.00	523.53	523.55
S. End of N. Appr Slab	526+58.96	-6.00	523.82	523.85

EAST EDGE OF PAVEMENT & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+26.32	0.00	522.97	522.99
A	526+36.32	0.00	523.27	523.29
B	526+46.32	0.00	523.57	523.59
S. End of N. Appr Slab	526+56.32	0.00	523.87	523.89

CL ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+21.04	12.00	522.99	523.01
A	526+31.04	12.00	523.29	523.31
B	526+41.04	12.00	523.59	523.61
S. End of N. Appr Slab	526+51.04	12.00	523.89	523.91



WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+15.75	24.00	522.66	522.68
A	526+25.75	24.00	522.95	522.97
B	526+35.75	24.00	523.25	523.27
S. End of N. Appr Slab	526+45.75	24.00	523.55	523.57

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+11.35	34.00	522.32	522.35
A	526+21.35	34.00	522.62	522.64
B	526+31.35	34.00	522.92	522.94
S. End of N. Appr Slab	526+41.35	34.00	523.22	523.24

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2-17-2017

EFK Moen
Civil Engineering Design

USER NAME =	ACB	DESIGNED -	ACB	REVISED -	
CHECKED -	CDL	CHECKED -	CDL	REVISED -	
PLOT SCALE =		DRAWN -	ACB	REVISED -	
PLOT DATE =	1/11/2024	CHECKED -	CDL	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF NORTH APPROACH SLAB ELEVATIONS
STRUCTURE NO. 006-0053 (SB)

SHEET 10 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	230
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+33.04	-6.00	529.01	529.03
Q	528+43.04	-6.00	529.31	529.33
R	528+53.04	-6.00	529.61	529.63
S. End of S. Appr Slab	528+63.04	-6.00	529.91	529.93

EAST EDGE OF PAVEMENT & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+30.40	0.00	529.06	529.08
Q	528+40.40	0.00	529.35	529.37
R	528+50.40	0.00	529.65	529.67
S. End of S. Appr Slab	528+60.40	0.00	529.95	529.97

CL ROADWAY

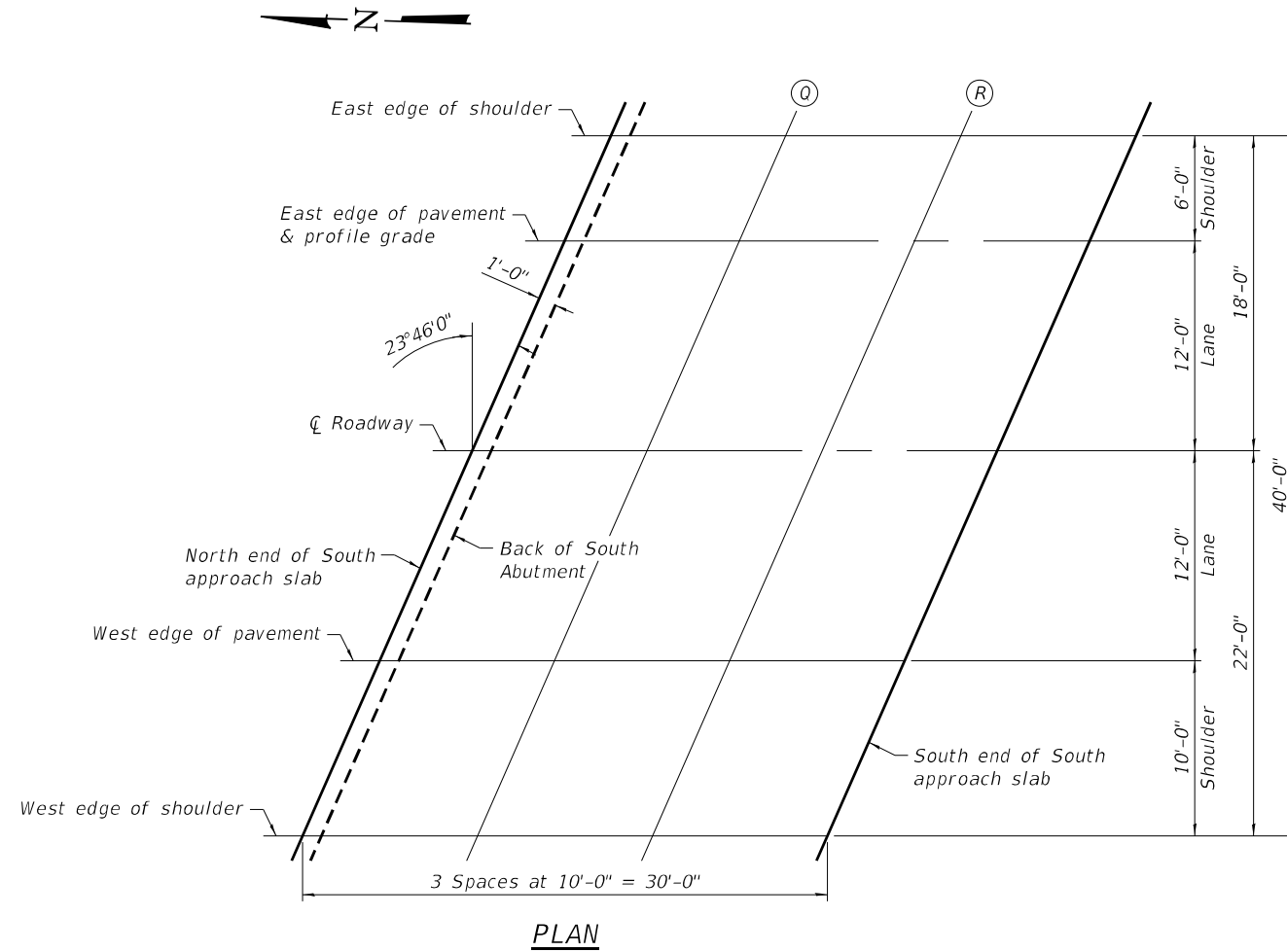
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+25.11	12.00	529.08	529.10
Q	528+35.11	12.00	529.38	529.40
R	528+45.11	12.00	529.67	529.69
S. End of S. Appr Slab	528+55.11	12.00	529.97	529.99

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+19.83	24.00	528.74	528.76
Q	528+29.83	24.00	529.04	529.06
R	528+39.83	24.00	529.34	529.36
S. End of S. Appr Slab	528+49.83	24.00	529.63	529.66

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+15.43	34.00	528.41	528.43
Q	528+25.43	34.00	528.71	528.73
R	528+35.43	34.00	529.00	529.03
S. End of S. Appr Slab	528+45.43	34.00	529.30	529.32



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2-17-2017

EFK Moen
Civil Engineering Design

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PLOT SCALE =		DRAWN -	ACB	REVISED -	
PLOT DATE =	1/11/2024	CHECKED -	CDL	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SOUTH APPROACH SLAB ELEVATIONS
STRUCTURE NO. 006-0053 (SB)

SHEET 11 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	231
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

EAST EDGE OF SHOULDER

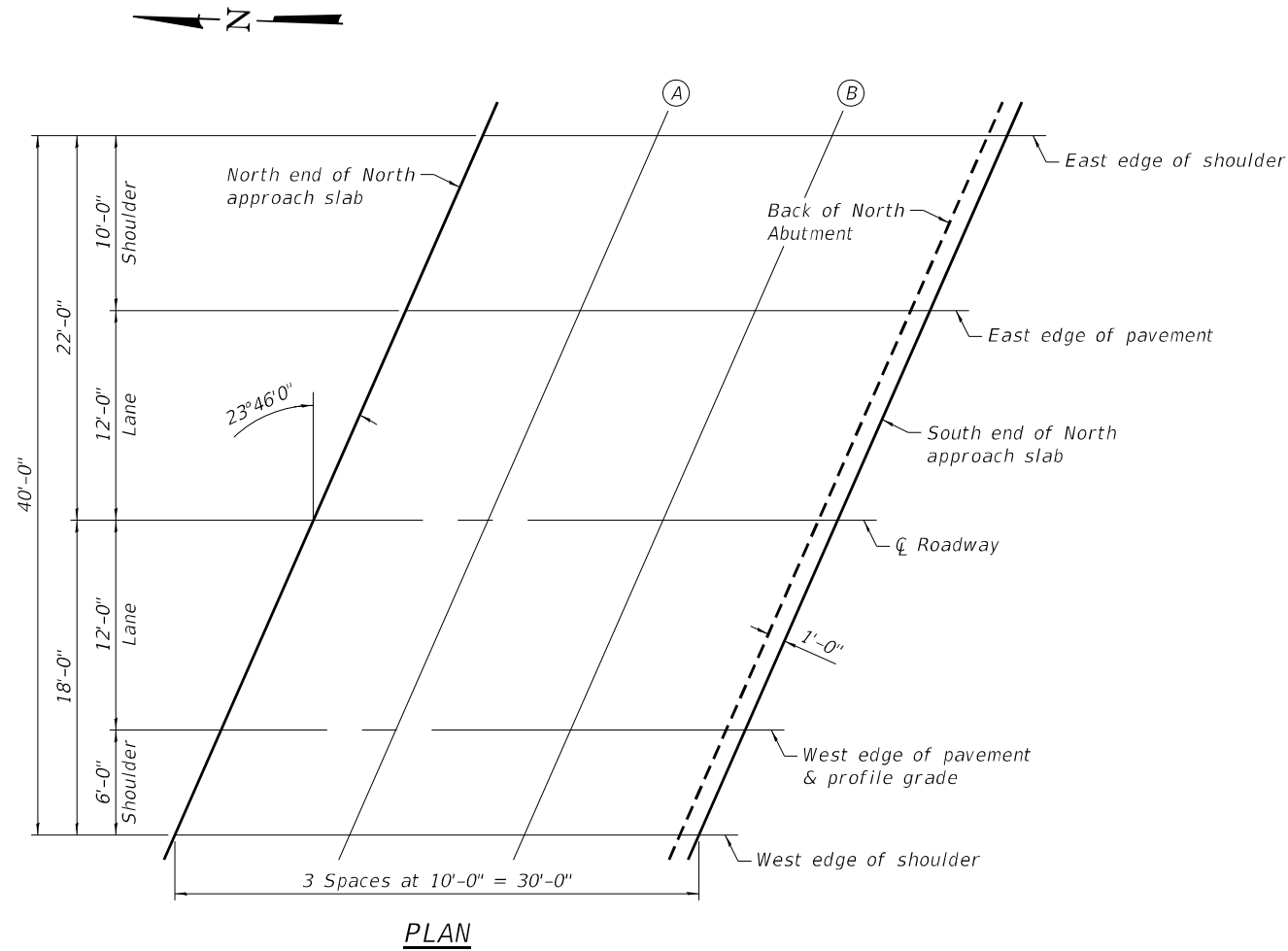
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+72.12	-34.00	523.59	523.61
A	526+82.12	-34.00	523.89	523.91
B	526+92.12	-34.00	524.19	524.21
S. End of N. Appr Slab	527+02.12	-34.00	524.49	524.51

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+67.72	-24.00	523.66	523.68
A	526+77.72	-24.00	523.95	523.97
B	526+87.72	-24.00	524.26	524.28
S. End of N. Appr Slab	526+97.72	-24.00	524.56	524.58

☐ ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+62.44	-12.00	523.68	523.70
A	526+72.44	-12.00	523.98	524.00
B	526+82.44	-12.00	524.28	524.30
S. End of N. Appr Slab	526+92.44	-12.00	524.58	524.60



WEST EDGE OF PAVEMENT & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+57.15	0.00	523.34	523.36
A	526+67.15	0.00	523.64	523.66
B	526+77.15	0.00	523.94	523.96
S. End of N. Appr Slab	526+87.15	0.00	524.24	524.26

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of N. Appr Slab	526+54.51	6.00	523.14	523.16
A	526+64.51	6.00	523.44	523.46
B	526+74.51	6.00	523.74	523.76
S. End of N. Appr Slab	526+84.51	6.00	524.04	524.06

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2-17-2017

EFK Moen
Civil Engineering Design

USER NAME = ABenz	DESIGNED - ACB	REVISED -
PLOT SCALE =	CHECKED - CDL	REVISED -
PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
	CHECKED - CDL	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF NORTH APPROACH SLAB ELEVATIONS
STRUCTURE NO. 006-0054 (NB)

SHEET 12 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	232
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+76.19	-34.00	529.93	529.95
Q	528+86.19	-34.00	530.24	530.26
R	528+96.19	-34.00	530.56	530.58
S. End of S. Appr Slab	529+06.19	-34.00	530.87	530.89

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+71.79	-24.00	529.99	530.01
Q	528+81.79	-24.00	530.31	530.33
R	528+91.79	-24.00	530.62	530.64
S. End of S. Appr Slab	529+01.79	-24.00	530.94	530.96

CL ROADWAY

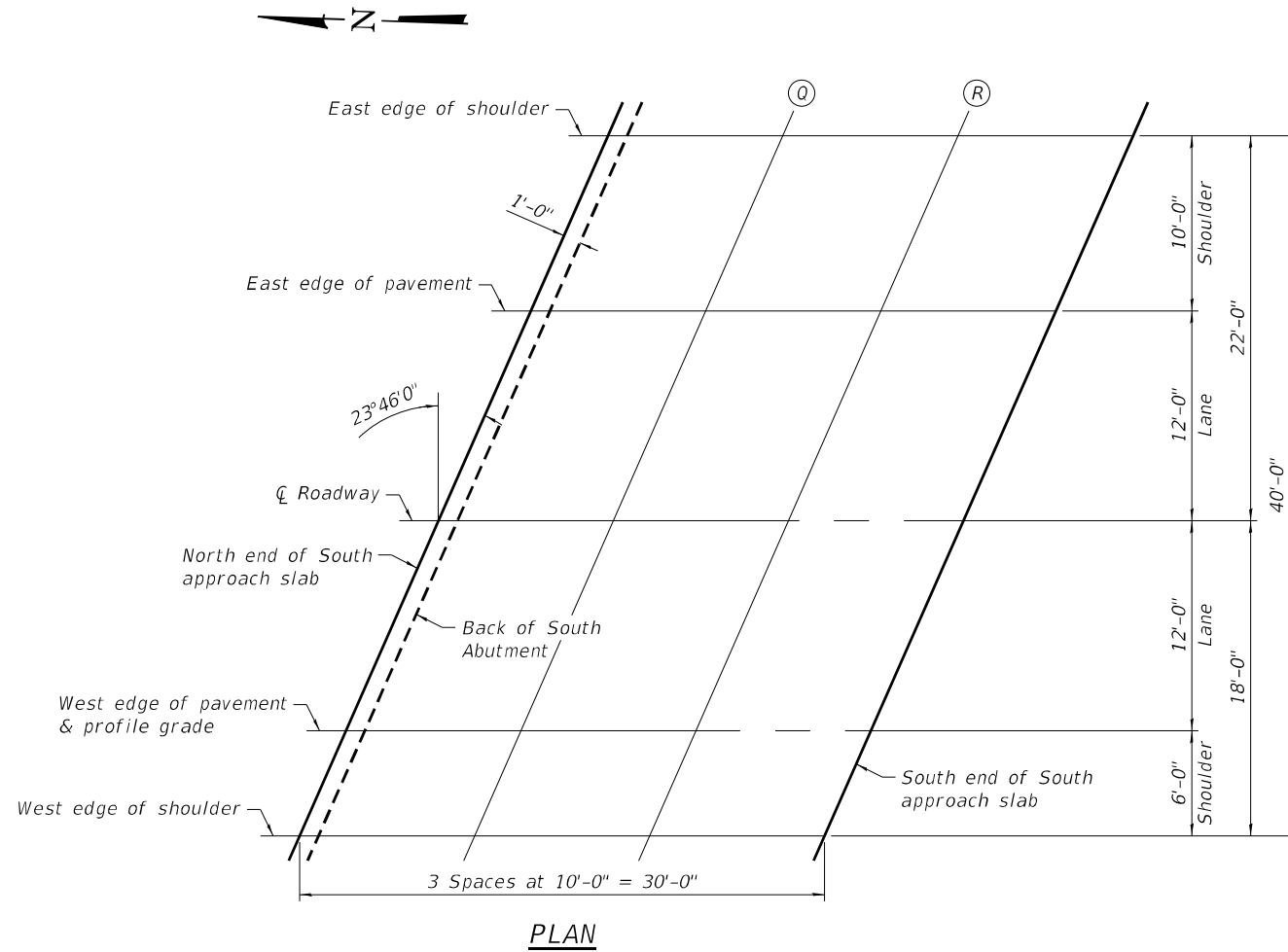
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+66.50	-12.00	530.00	530.02
Q	528+76.50	-12.00	530.32	530.34
R	528+86.50	-12.00	530.63	530.65
S. End of S. Appr Slab	528+96.50	-12.00	530.95	530.97

WEST EDGE OF PAVEMENT & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+61.22	0.00	529.66	529.68
Q	528+71.22	0.00	529.97	529.99
R	528+81.22	0.00	530.29	530.31
S. End of S. Appr Slab	528+91.22	0.00	530.60	530.62

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End of S. Appr Slab	528+58.58	6.00	529.45	529.47
Q	528+68.58	6.00	529.77	529.79
R	528+78.58	6.00	530.08	530.10
S. End of S. Appr Slab	528+88.58	6.00	530.40	530.42



MODEL: Default
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E-AS1

2-17-2017

EFK Moen
Civil Engineering Design

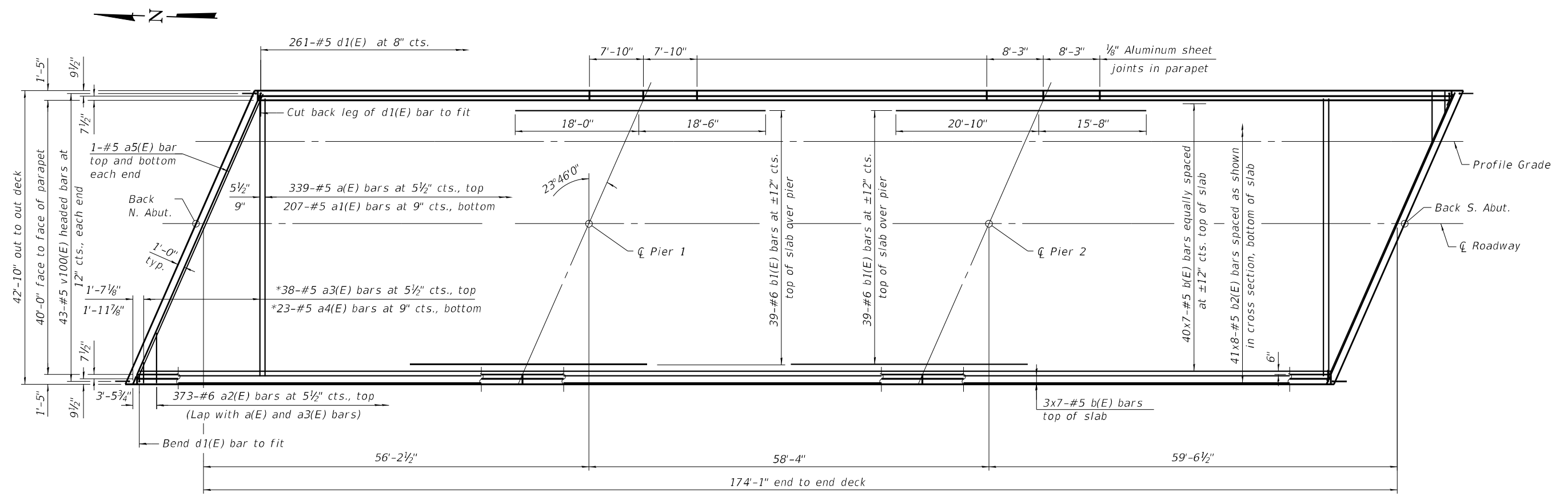
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		CHECKED -	CDL	REVISED -	
PLOT SCALE =		DRAWN -	ACB	REVISED -	
PLOT DATE =	1/11/2024	CHECKED -	CDL	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SOUTH APPROACH SLAB ELEVATIONS
STRUCTURE NO. 006-0054 (NB)

SHEET 13 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	233
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



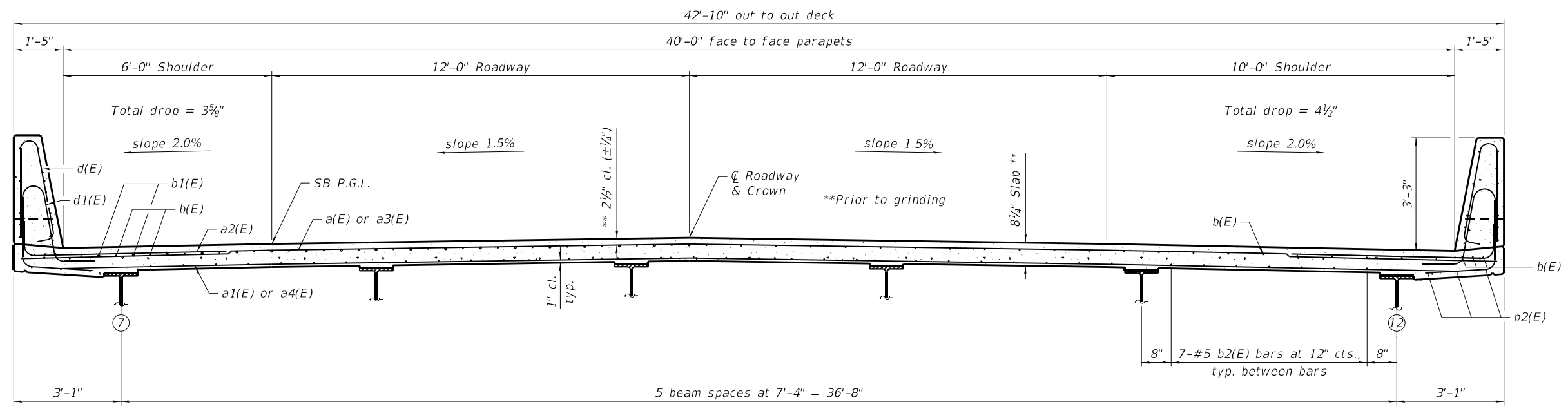
PLAN

MINIMUM BAR LAP

#5 bar = 3'-6"

* See Field Cutting Diagram on sheet 15 of 42.

Notes:
See sheet 15 of 42 for superstructure details and Bill of Material.
Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.



CROSS SECTION
(Looking South)

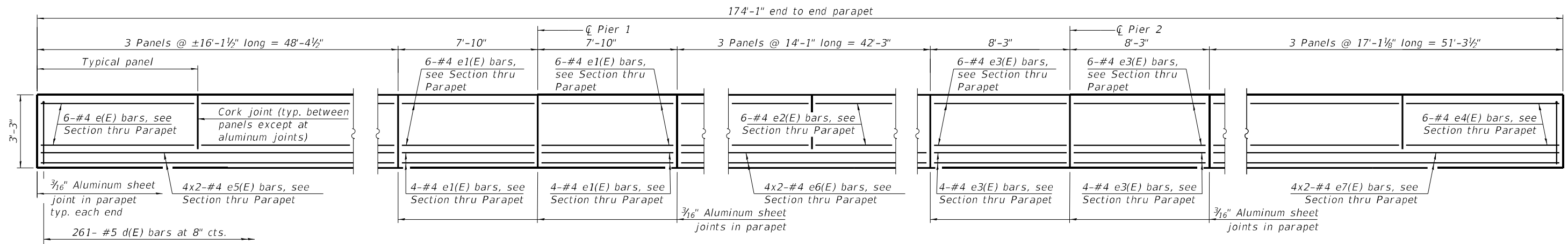
NEAR PIER

NEAR MIDSPAN

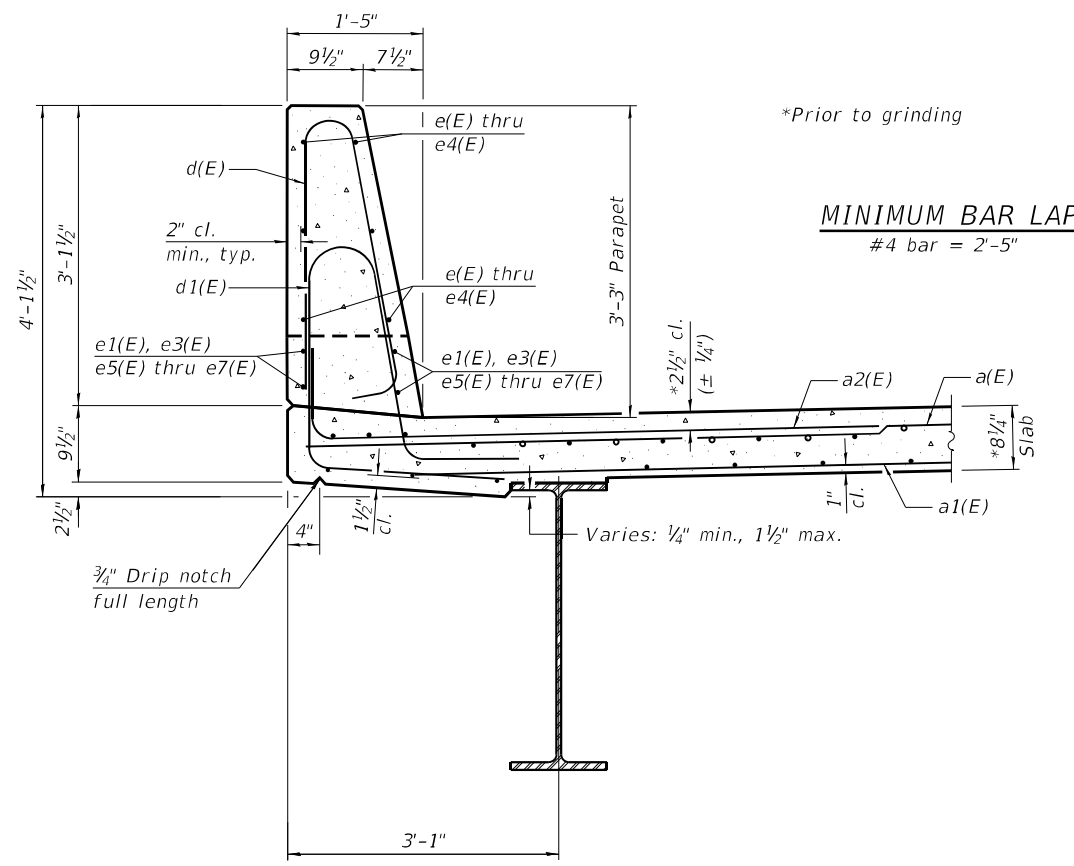
SI-SB-2-L(≤30°) 6-15-2019

EFK•Moen Civil Engineering Design	USER NAME = ABenz	DESIGNED - ACB	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE STRUCTURE NO. 006-0053 (SB)	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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	PLOT DATE = 1/11/2024	CHECKED - CDL	REVISED -			CONTRACT NO. 66K66				
	SHEET 14 OF 42 SHEETS					ILLINOIS	FED. AID PROJECT			

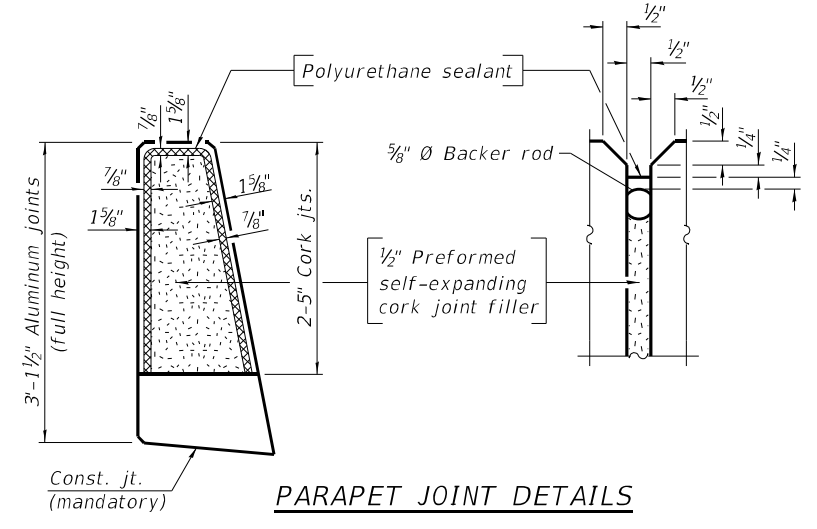
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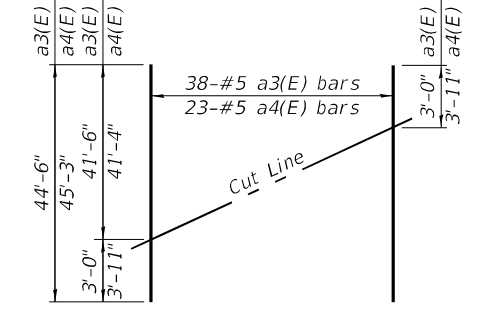
INSIDE ELEVATION OF PARAPET



SECTION THRU PARAPET



PARAPET JOINT DETAILS



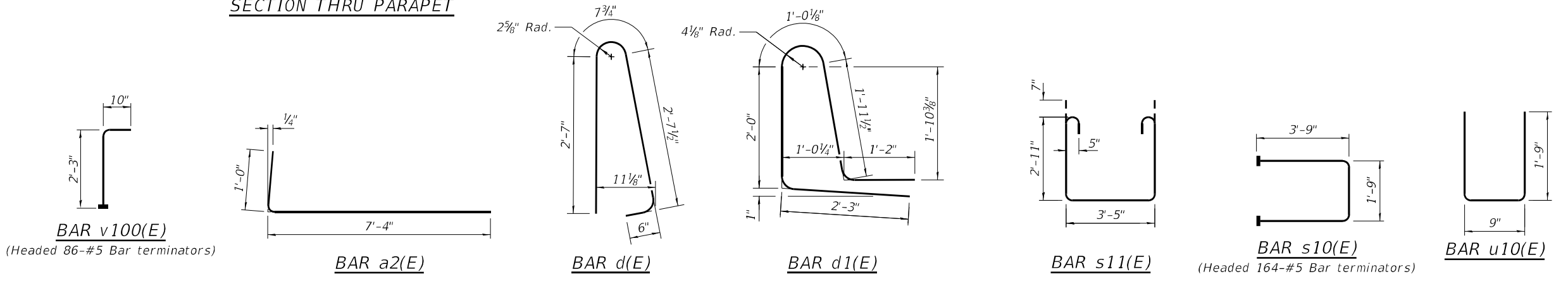
FIELD CUTTING DIAGRAM

SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	339	#5	42'-6"	—
a1(E)	207	#5	40'-6"	—
a2(E)	746	#6	8'-4"	—
a3(E)	38	#5	44'-6"	—
a4(E)	23	#5	45'-3"	—
a5(E)	4	#5	46'-5"	—
b(E)	322	#5	27'-10"	—
b1(E)	78	#6	36'-6"	—
b2(E)	328	#5	24'-10"	—
d(E)	522	#5	6'-5"	—
d1(E)	522	#5	8'-5"	—
e(E)	36	#4	15'-9"	—
e1(E)	40	#4	7'-6"	—
e2(E)	36	#4	13'-9"	—
e3(E)	40	#4	7'-11"	—
e4(E)	36	#4	16'-9"	—
e5(E)	16	#4	25'-3"	—
e6(E)	16	#4	22'-3"	—
e7(E)	16	#4	26'-9"	—
m10(E)	12	#6	46'-6"	—
m11(E)	30	#6	7'-8"	—
m12(E)	20	#6	7'-8"	—
m13(E)	8	#4	24'-11"	—
m14(E)	12	#6	2'-11"	—
m15(E)	8	#6	2'-11"	—
s10(E)	82	#5	9'-3"	—
s11(E)	82	#5	10'-5"	—
u10(E)	82	#4	4'-3"	—
v100(E)	86	#5	3'-1"	—
Reinforcement Bars, Epoxy Coated		Lbs.		72,490
Concrete Superstructure		Cu. Yds.		286.7

Bars indicated thus 1 x 2-#4 etc. indicates 1 line of bars with 2 lengths per line.

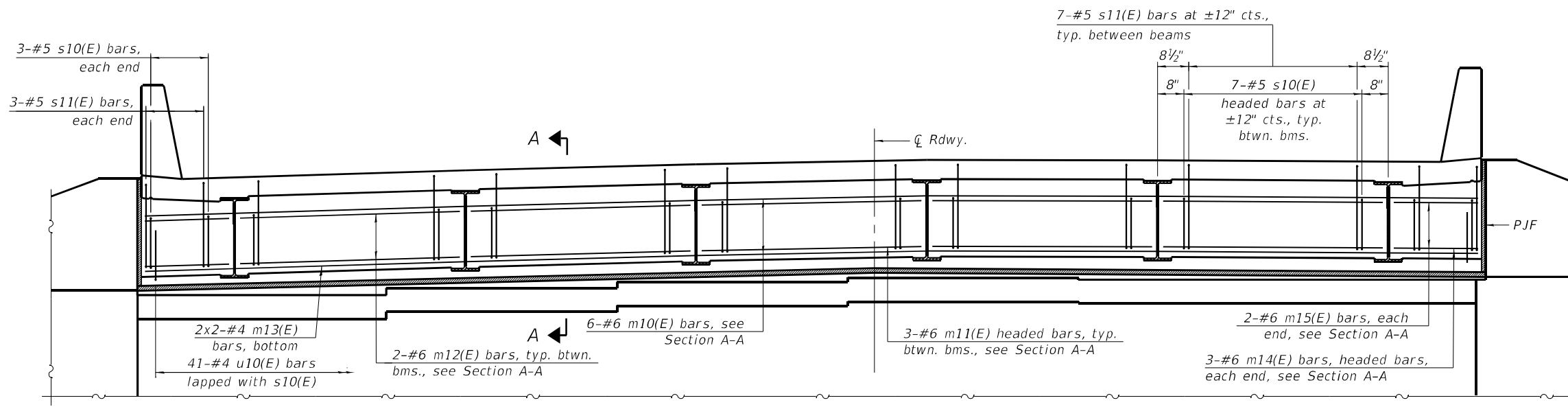
Notes:
 The 3/16" min. aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated with 5 mils of either bitumen paint or epoxy paint to minimize reaction with wet concrete. Cost included with Concrete Superstructure.
 The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.
 Bar terminators, paid for separately. See Total Bill of Material.



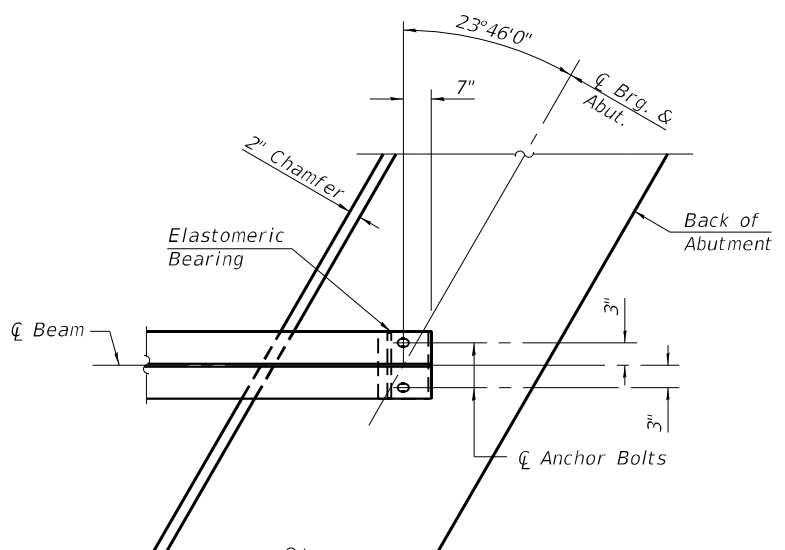
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EFK Moen Civil Engineering Design	USER NAME = ABenz PLOT SCALE = PLOT DATE = 1/11/2024	DESIGNED - ACB CHECKED - CDL DRAWN - ACB CHECKED - CDL	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE DETAILS STRUCTURE NO. 006-0053 (SB)	F.A.I. RTE. = 180 SECTION = (06-2HB-1)ES COUNTY = BUREAU TOTAL SHEETS = 327 SHEET NO. = 235 CONTRACT NO. 66K66
	SHEET 15 OF 42 SHEETS					ILLINOIS FED. AID PROJECT

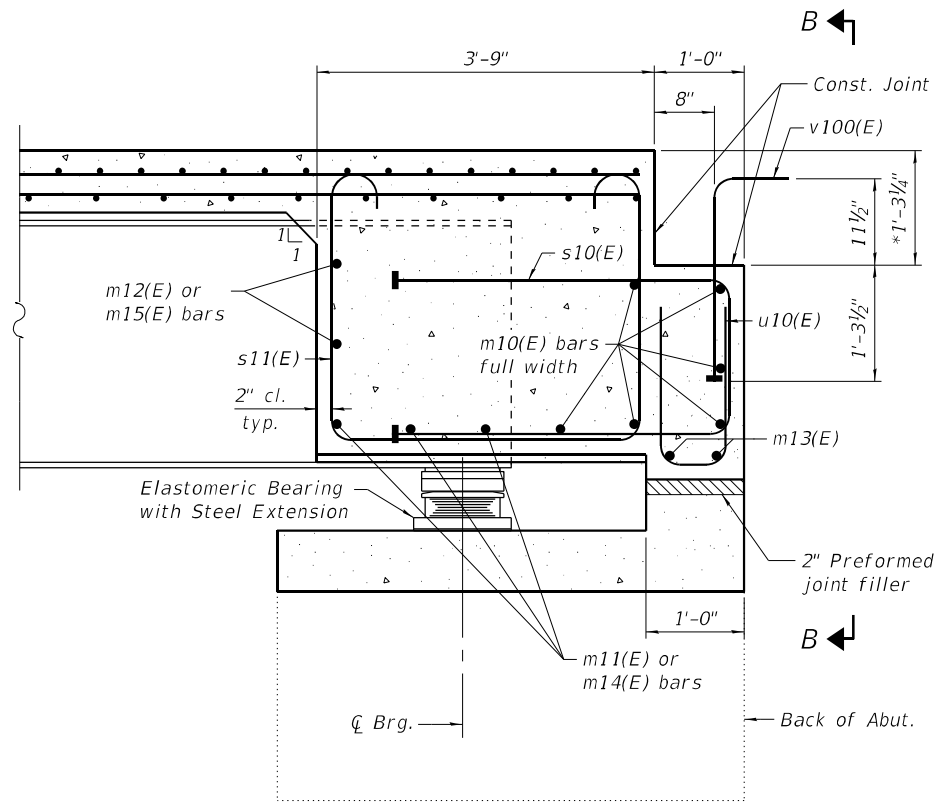
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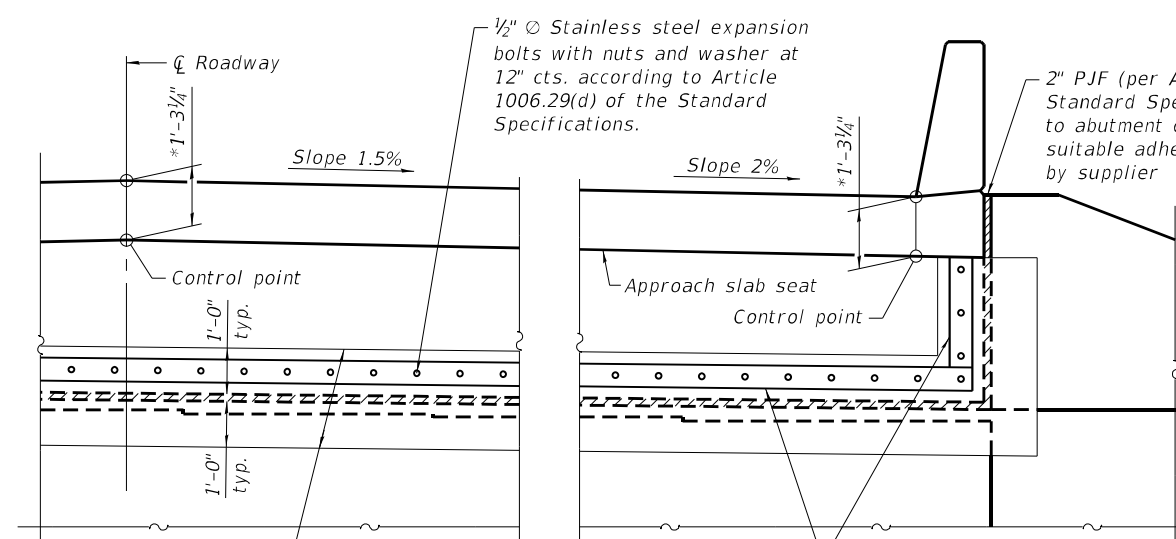
DIAPHRAGM AT ABUTMENT
 (Horizontal dimensions are at right angles)



PLAN AT ABUTMENT
 (Showing bottom flange of beam)



SECTION A-A
 (Horizontal dimensions are at right angles)



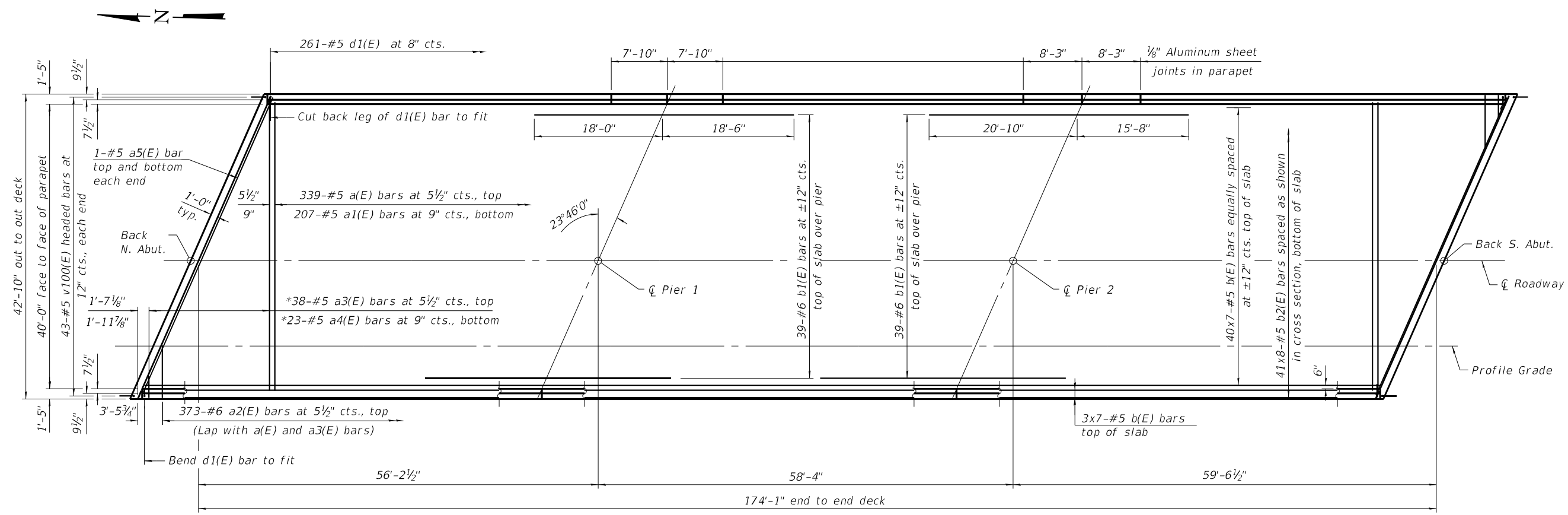
VIEW B-B

Limits of fabric reinforced elastomeric mat according to Section 1028 of the Standard Specifications and installed according to applicable requirements of Article 520.09 of the Standard Specifications.

Notes:
 See sheet 15 of 42 for superstructure details and Bill of Material.
 The s10(E), s11(E), and u(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.
 The approach slab seat shall have a constant slope determined from the control points shown.
 Cost of Fabric Reinforced Elastomeric Mat, galvanized plate, stainless steel expansion bolts with nuts and washers and installation are included in the cost of Concrete Superstructure.

USER NAME = ABenz	DESIGNED - ACB	REVISED -
CHECKED - CDL	REVISIONS -	
PLOT SCALE =	DRAWN - ACB	REVISED -
PLOT DATE = 1/11/2024	CHECKED - CDL	REVISED -

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	236
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

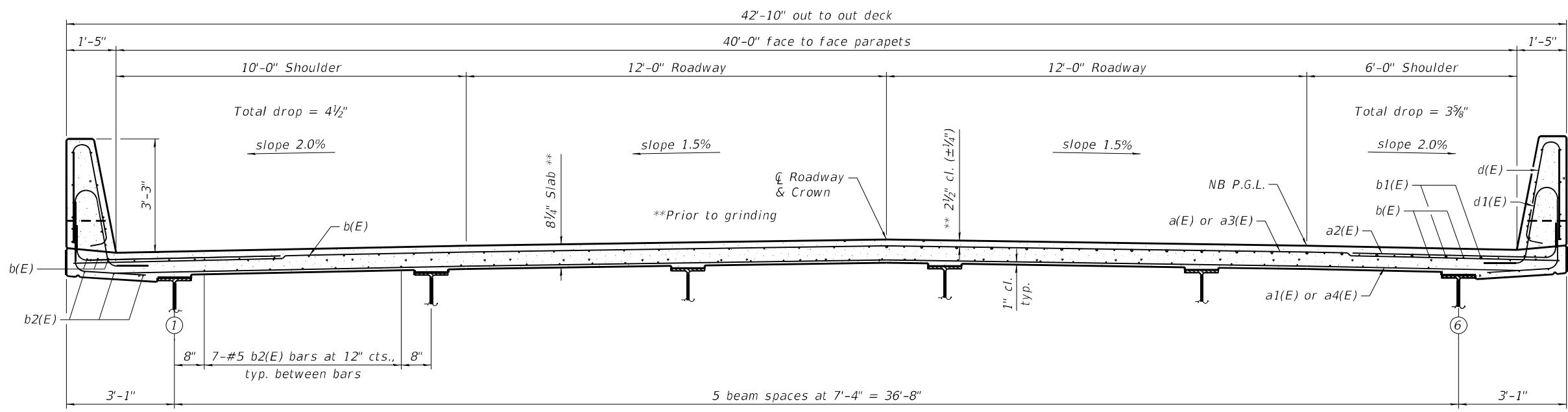


PLAN

MINIMUM BAR LAP

#5 bar = 3'-6"
 * See Field Cutting Diagram on sheet 18 of 42.

Notes:
 See sheet 18 of 42 for superstructure details and Bill of Material.
 Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.



CROSS SECTION
 (Looking South)

SI-SB-2-L(≤30°) 6-15-2019

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EFK Moen
 Civil Engineering Design

USER NAME = ABenz	DESIGNED - ACB	REVISED -
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PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
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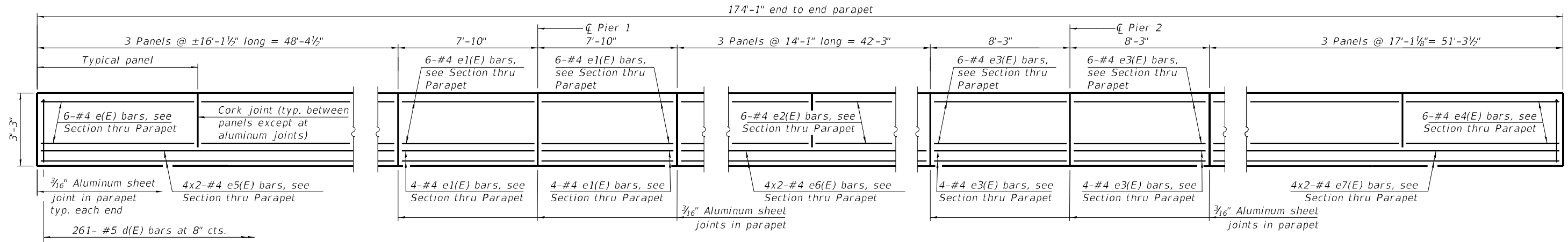
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE
STRUCTURE NO. 006-0054 (NB)

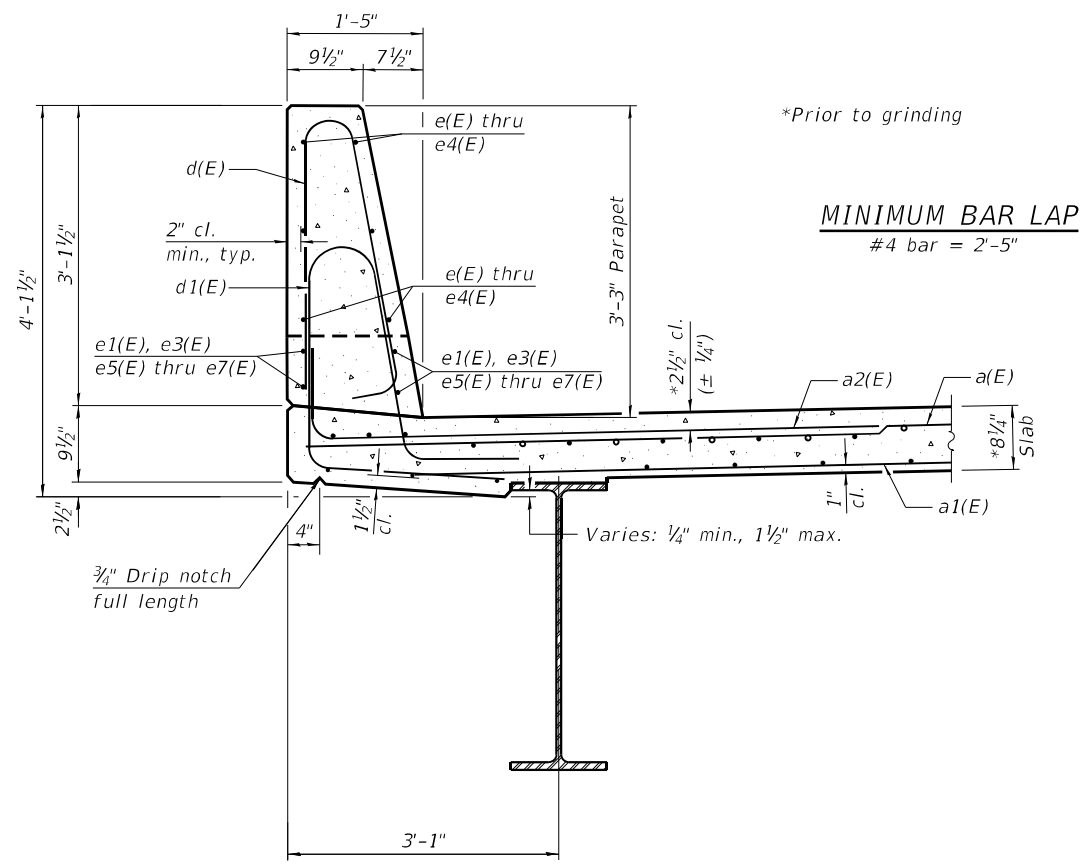
SHEET 17 OF 42 SHEETS

F.A.I. RTE. 180	SECTION (06-2HB-1)ES	COUNTY BUREAU	TOTAL SHEETS 327	SHEET NO. 237
CONTRACT NO. 66K66				

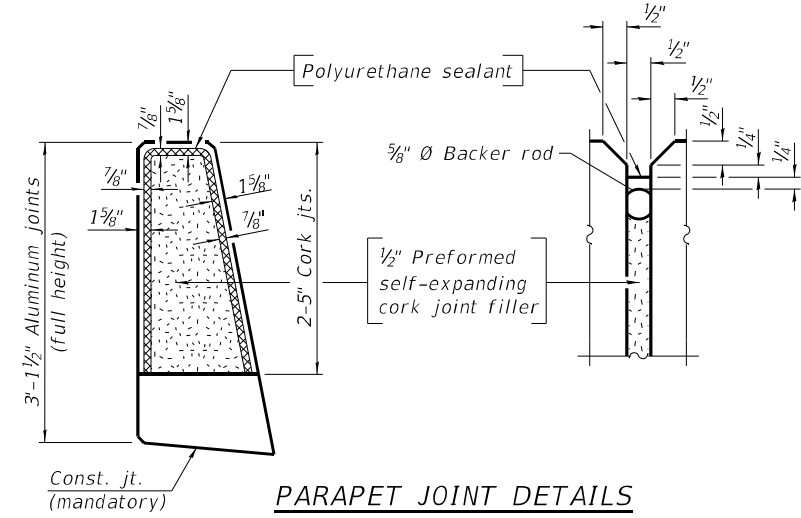
ILLINOIS FED. AID PROJECT



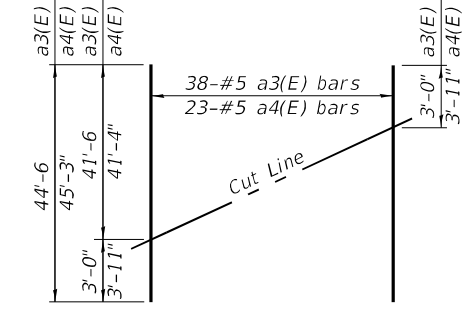
INSIDE ELEVATION OF PARAPET



SECTION THRU PARAPET



PARAPET JOINT DETAILS



FIELD CUTTING DIAGRAM

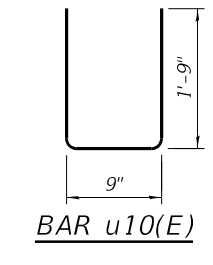
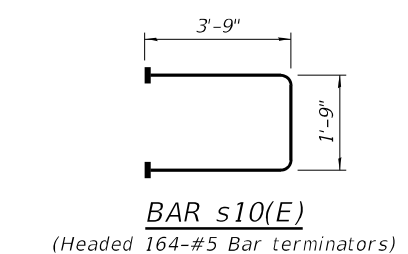
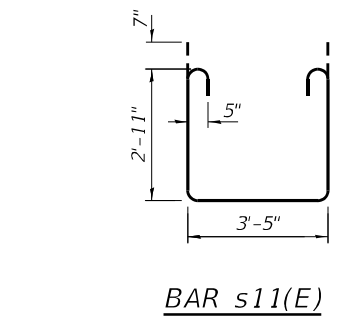
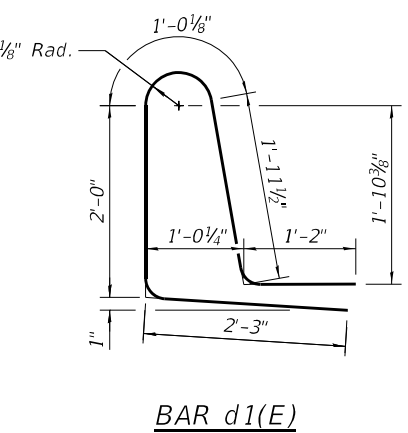
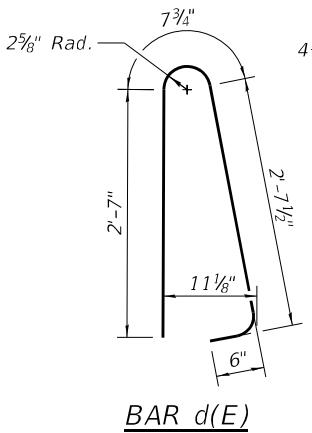
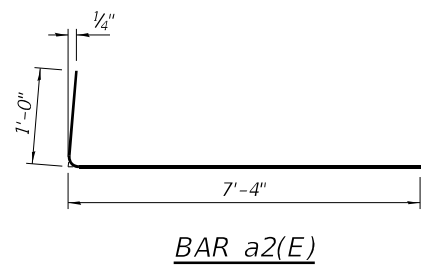
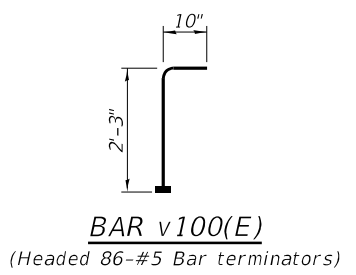
Order a3(E) and a4(E) bars full length. Cut as shown and use remainder of bars in opposite end of deck.

SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	339	#5	42'-6"	—
a1(E)	207	#5	40'-6"	—
a2(E)	746	#6	8'-4"	—
a3(E)	38	#5	44'-6"	—
a4(E)	23	#5	45'-3"	—
a5(E)	4	#5	46'-5"	—
b(E)	322	#5	27'-10"	—
b1(E)	78	#6	36'-6"	—
b2(E)	328	#5	24'-10"	—
d(E)	522	#5	6'-5"	—
d1(E)	522	#5	8'-5"	—
e(E)	36	#4	15'-9"	—
e1(E)	40	#4	7'-6"	—
e2(E)	36	#4	13'-9"	—
e3(E)	40	#4	7'-11"	—
e4(E)	36	#4	16'-9"	—
e5(E)	16	#4	25'-3"	—
e6(E)	16	#4	22'-3"	—
e7(E)	16	#4	26'-9"	—
m10(E)	12	#6	46'-6"	—
m11(E)	30	#6	7'-8"	—
m12(E)	20	#6	7'-8"	—
m13(E)	8	#4	24'-11"	—
m14(E)	12	#6	2'-11"	—
m15(E)	8	#6	2'-11"	—
s10(E)	82	#5	9'-3"	—
s11(E)	82	#5	10'-5"	—
u10(E)	82	#4	4'-3"	—
v100(E)	86	#5	3'-1"	—
Reinforcement Bars, Epoxy Coated		Lbs.		72,490
Concrete Superstructure		Cu. Yds.		286.7

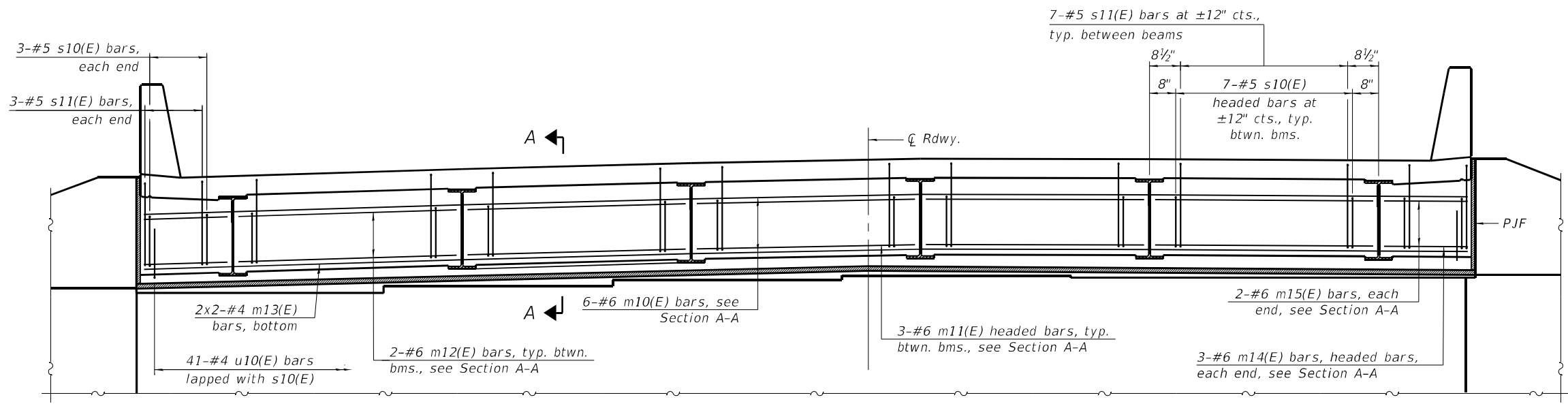
Bars indicated thus 1 x 2-#4 etc. indicates 1 line of bars with 2 lengths per line.

Notes:
The ¾" min. aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated with 5 mils of either bitumen paint or epoxy paint to minimize reaction with wet concrete. Cost included with Concrete Superstructure.
The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.
Bar terminators, paid for separately. See Total Bill of Material.

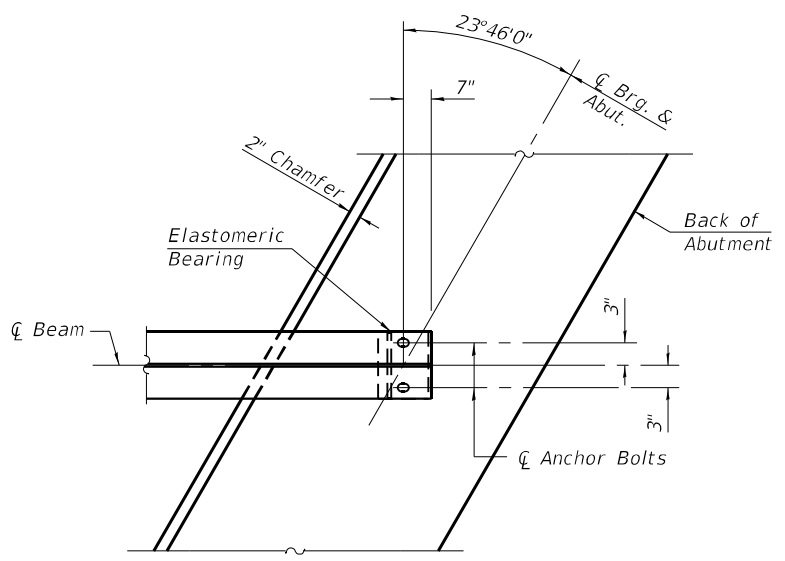


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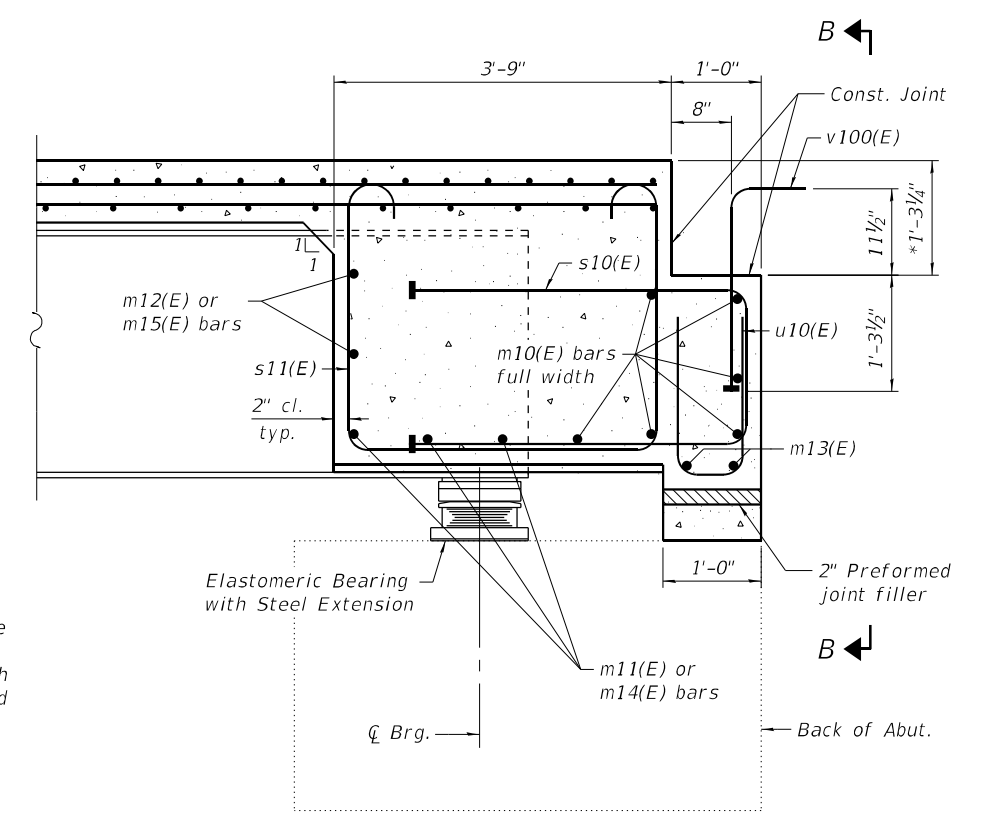
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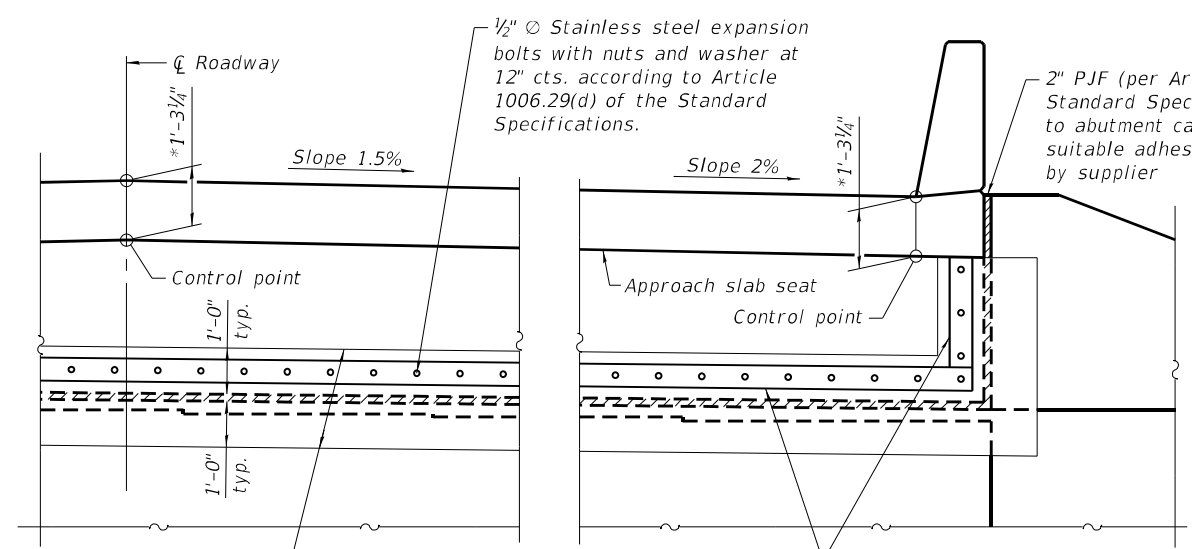
DIAPHRAGM AT ABUTMENT
 (Horizontal dimensions are at right angles)



PLAN AT ABUTMENT
 (Showing bottom flange of beam)



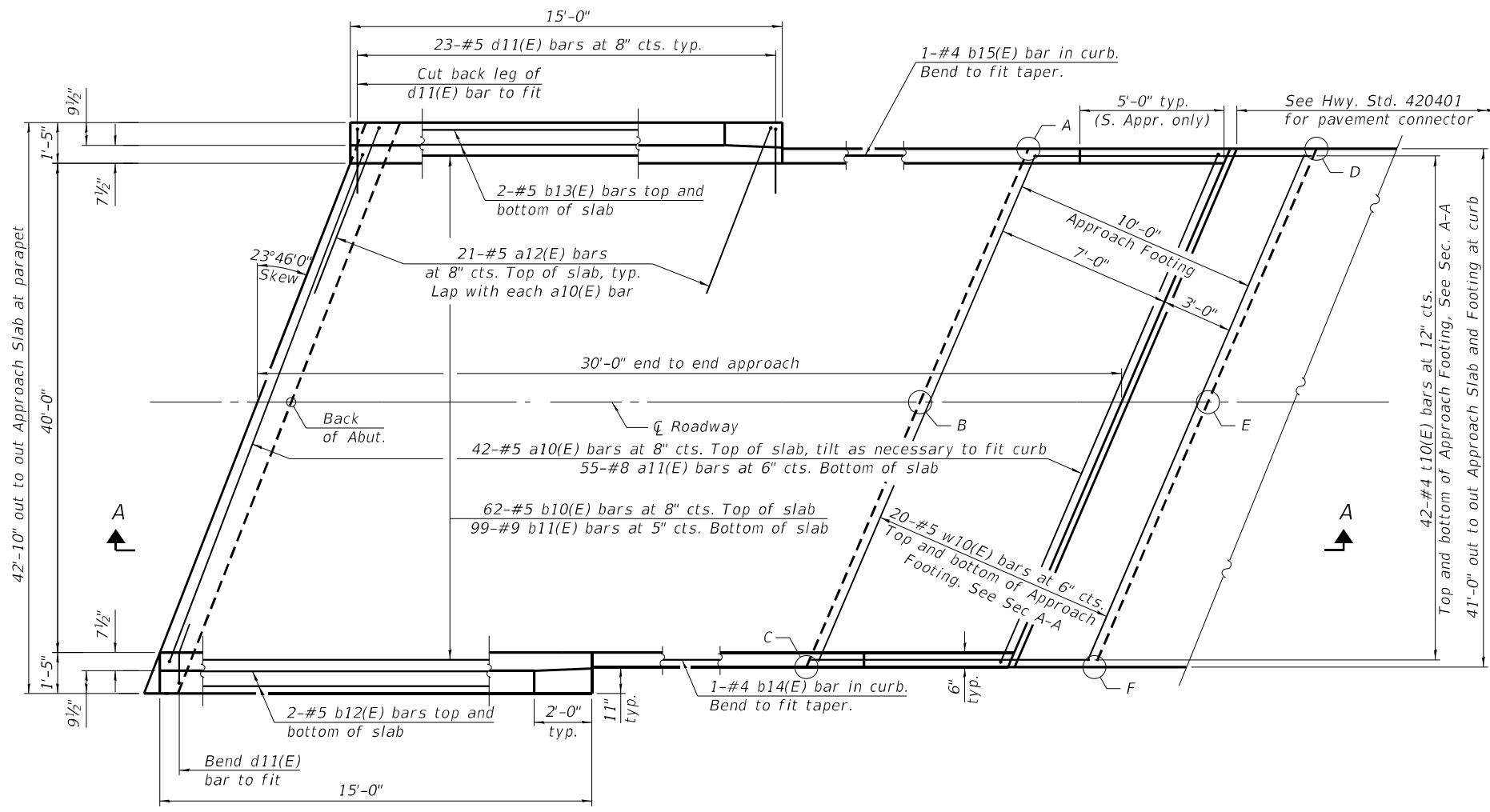
SECTION A-A
 (Horizontal dimensions are at right angles)



VIEW B-B

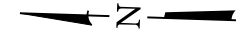
Limits of fabric reinforced elastomeric mat according to Section 1028 of the Standard Specifications and installed according to applicable requirements of Article 520.09 of the Standard Specifications.

Notes:
 See sheet 18 of 42 for superstructure details and Bill of Material.
 The s10(E), s11(E), and u(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.
 The approach slab seat shall have a constant slope determined from the control points shown.
 Cost of Fabric Reinforced Elastomeric Mat, galvanized plate, stainless steel expansion bolts with nuts and washers and installation are included in the cost of Concrete Superstructure.



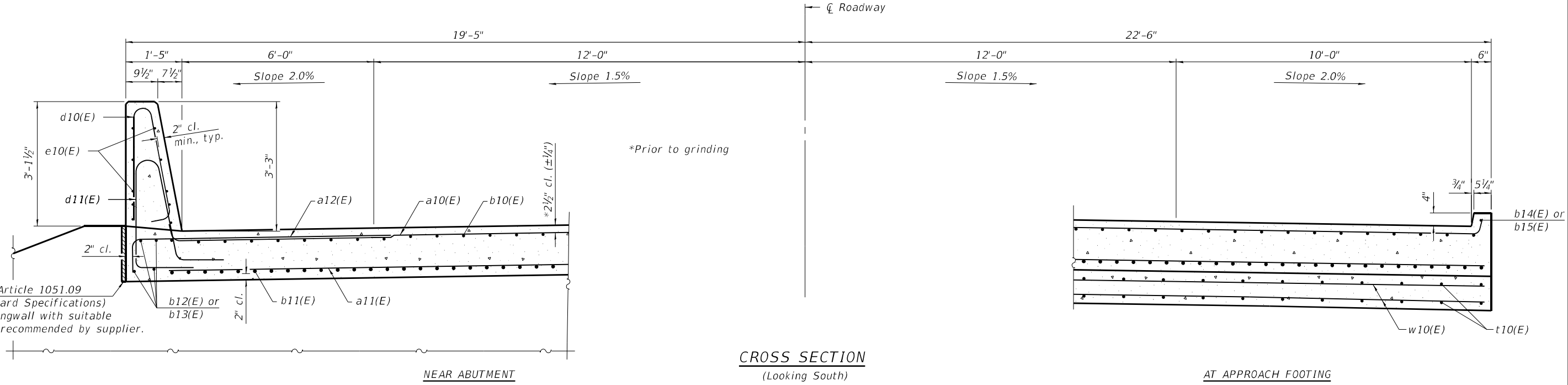
PLAN

(South approach slab shown; North approach slab similar by 180° rotation)



TOP AND BOTTOM ELEVATIONS FOR APPROACH FOOTING

Point/Location	North Approach		South Approach		
	Top	Bottom	Top	Bottom	
A - S.W.	521.29	520.45	A - N.E.	528.43	527.59
B - S. C	521.97	521.14	B - N. C	528.49	527.66
C - S.E.	521.90	521.07	C - N.W.	527.81	526.98
D - N.W.	521.86	521.03	D - S.E.	528.75	527.92
E - N. C	521.65	520.81	E - S. C	528.82	527.99
F - N.E.	522.32	521.49	F - S.W.	528.13	527.30



CROSS SECTION

(Looking South)

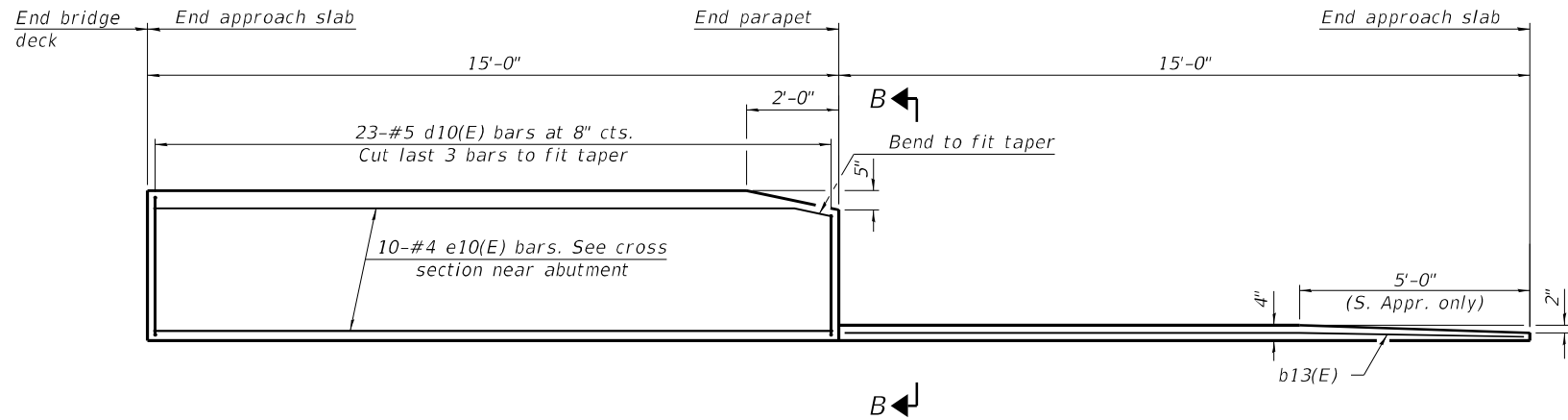
AT APPROACH FOOTING

BAIA-CIP-39CS-L($\leq 30^\circ$) 10-27-2023

(Sheet 1 of 2)

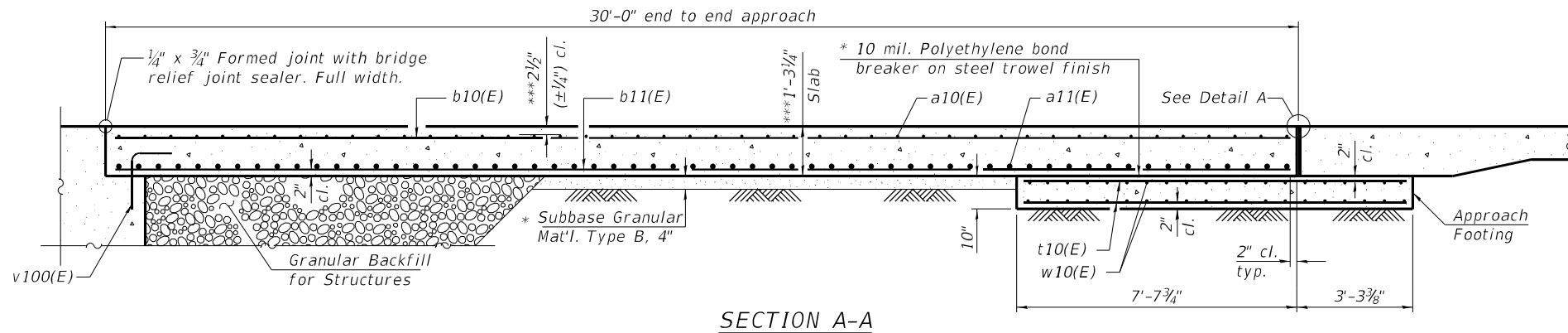
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	PLOT SCALE =	DRAWN - ACB	REVISED -			CONTRACT NO. 66K66					
	PLOT DATE = 1/11/2024	CHECKED - CDL	REVISED -			ILLINOIS FED. AID PROJECT					

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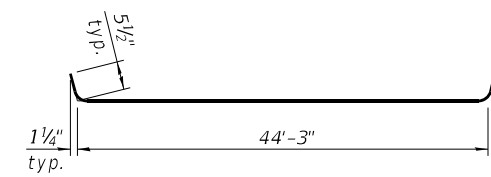
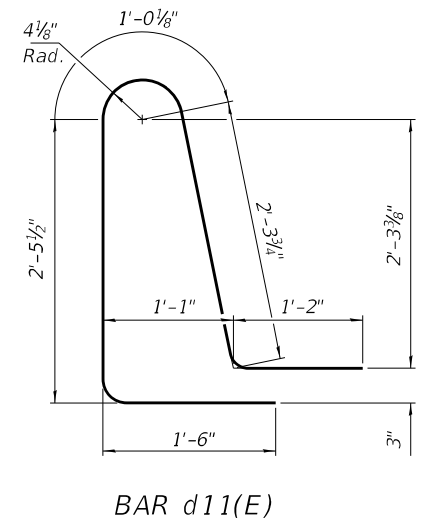
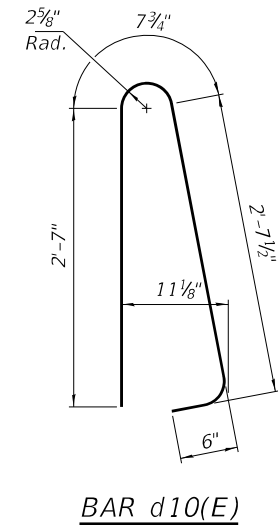


INSIDE ELEVATION OF PARAPET AND CURB

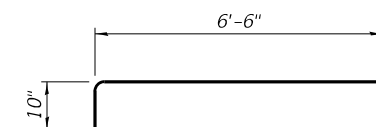
Notes:
 The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.
 Parapet concrete shall be paid for as Concrete Superstructure.
 Approach slab shall be paid for as Concrete Superstructure (Approach Slab).
 Approach footing concrete shall be paid for as Concrete Structures.
 The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
 Cost of excavation for approach footing included with Concrete Structures.
 For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 42.



SECTION A-A



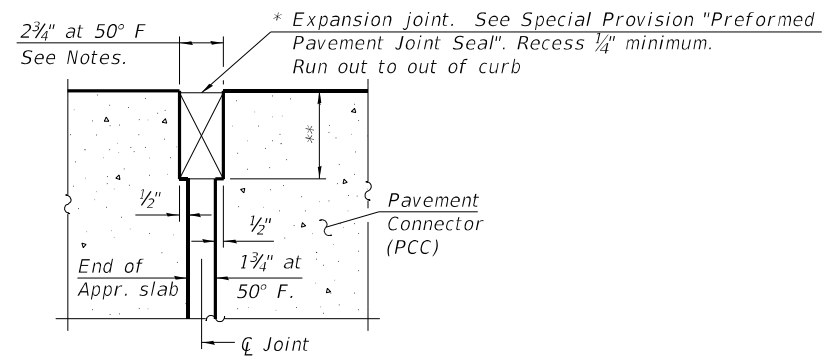
BAR a10(E)



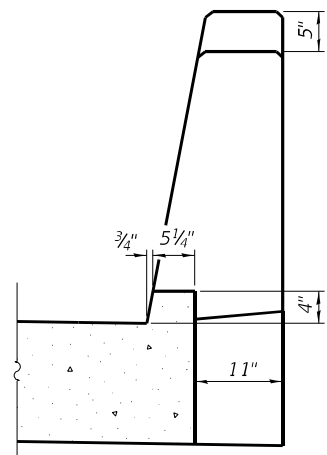
BAR a12(E)

TWO APPROACHES
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a10(E)	84	#5	45'-2"	
a11(E)	110	#8	44'-5"	
a12(E)	84	#5	7'-4"	
b10(E)	124	#5	29'-8"	
b11(E)	198	#9	29'-8"	
b12(E)	8	#5	14'-8"	
b13(E)	8	#5	14'-8"	
b14(E)	2	#4	14'-6"	
b15(E)	2	#4	14'-10"	
d10(E)	92	#5	6'-5"	
d11(E)	92	#5	8'-6"	
e10(E)	40	#4	14'-8"	
t10(E)	84	#4	10'-6"	
w10(E)	80	#5	44'-5"	
Concrete Superstructure		Cu. Yd.	7.6	
Concrete Superstructure (Approach Slab)		Cu. Yd.	118.8	
Concrete Structures		Cu. Yd.	27.6	
Reinforcement Bars, Epoxy Coated		Pound	47,860	



DETAIL A
(at Rt. L's)



VIEW B-B

* Cost included with Concrete Superstructure (Approach Slab).
 ** Per manufacturer recommendations
 *** Prior to grinding

BAIA-CIP-39CS-L(≤30°) 10-27-2023

(Sheet 2 of 2)

MODEL: Default
FILE NAME: \\SERVER18\Projects\5422057.03 IDOT D3 PTB 204-028 WO 03 1-180 over Bottom Road\DGM\Bridges\Final\Plotsheets\006-0053&0054+66K66-021-Approach (SB).dgn

EFK Moen
Civil Engineering Design

USER NAME =	ABenz
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PLOT DATE =	1/11/2024

DESIGNED -	ACB
CHECKED -	CDL
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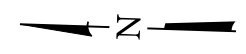
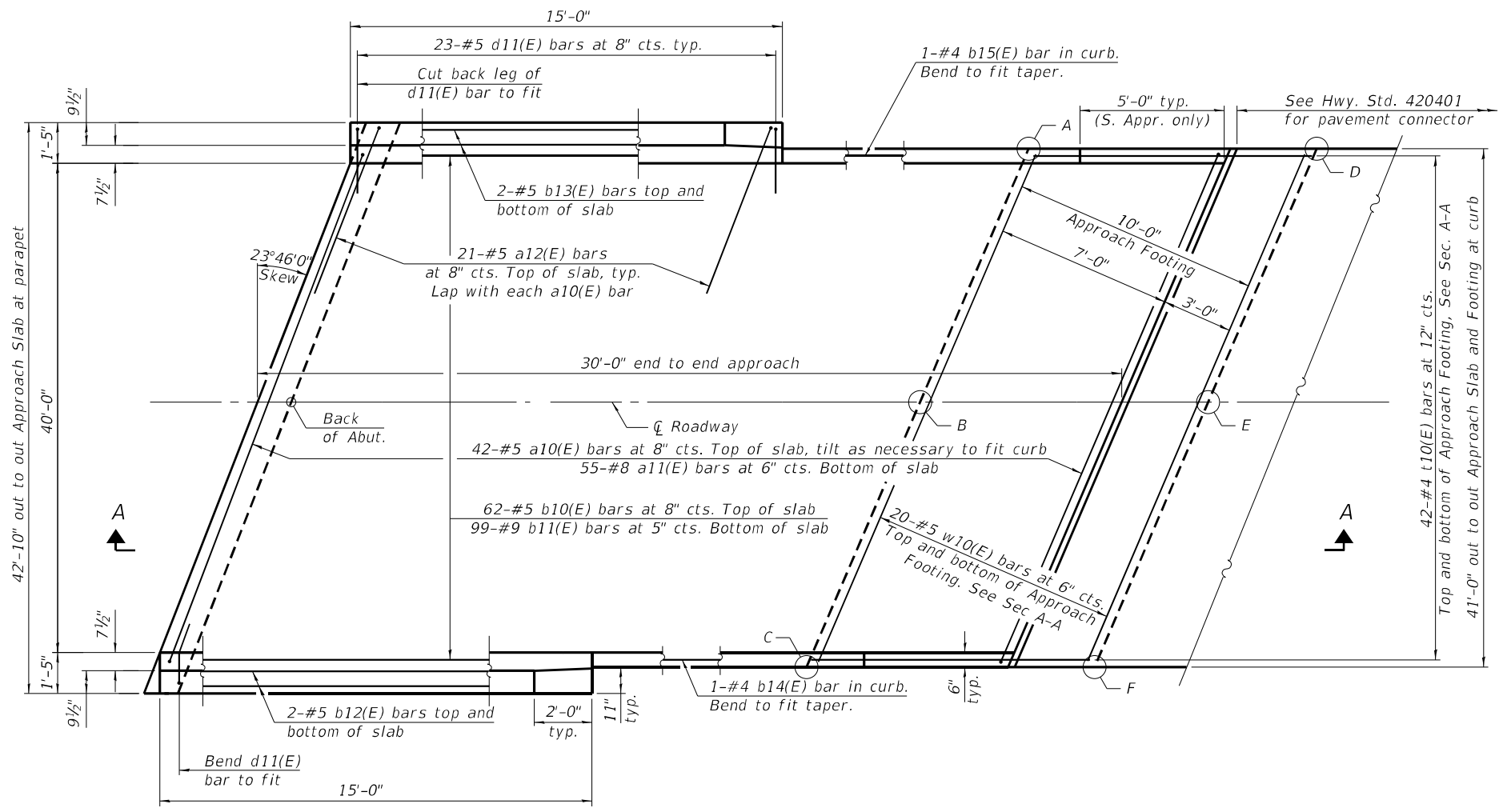
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 006-0053 (SB)

SHEET 21 OF 42 SHEETS

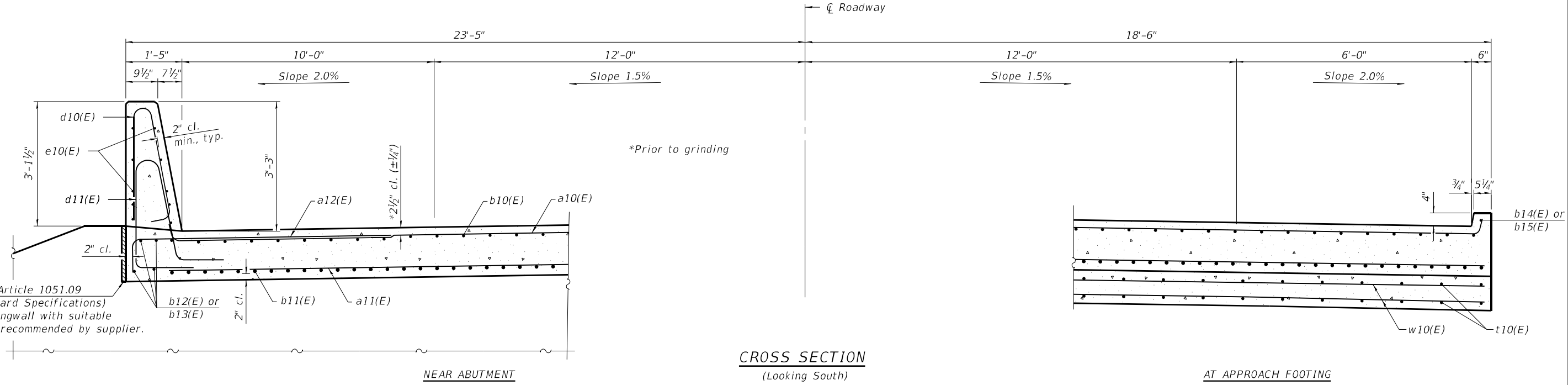
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	241
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



**TOP AND BOTTOM ELEVATIONS
FOR APPROACH FOOTING**

North Approach			South Approach		
Point/Location	Top	Bottom	Point/Location	Top	Bottom
A - S.W.	522.10	521.27	A - N.E.	529.38	528.55
B - S. ζ	522.30	521.46	B - N. ζ	529.10	528.26
C - S.E.	522.56	521.73	C - N.W.	528.89	528.06
D - N.W.	521.78	520.95	D - S.E.	529.72	528.89
E - N. ζ	521.97	521.14	E - S. ζ	529.44	528.61
F - N.E.	522.24	521.40	F - S.W.	529.24	528.40

PLAN
(South approach slab shown; North approach slab similar by 180° rotation)



CROSS SECTION
(Looking South)

AT APPROACH FOOTING

BAIA-CIP-39CS-L($\leq 30^\circ$) 10-27-2023

(Sheet 1 of 2)

MODEL: Default
FILE NAME: I:\SERVER18\Projects\5422057.03 IDOT D3 PTB 204-028 WO 03 1-180 over Bottom Road\DWG\Bridges\Final\Plotsheets\006-0054+66K66-022-Approach (NB).dgn

EFK Moen
Civil Engineering Design

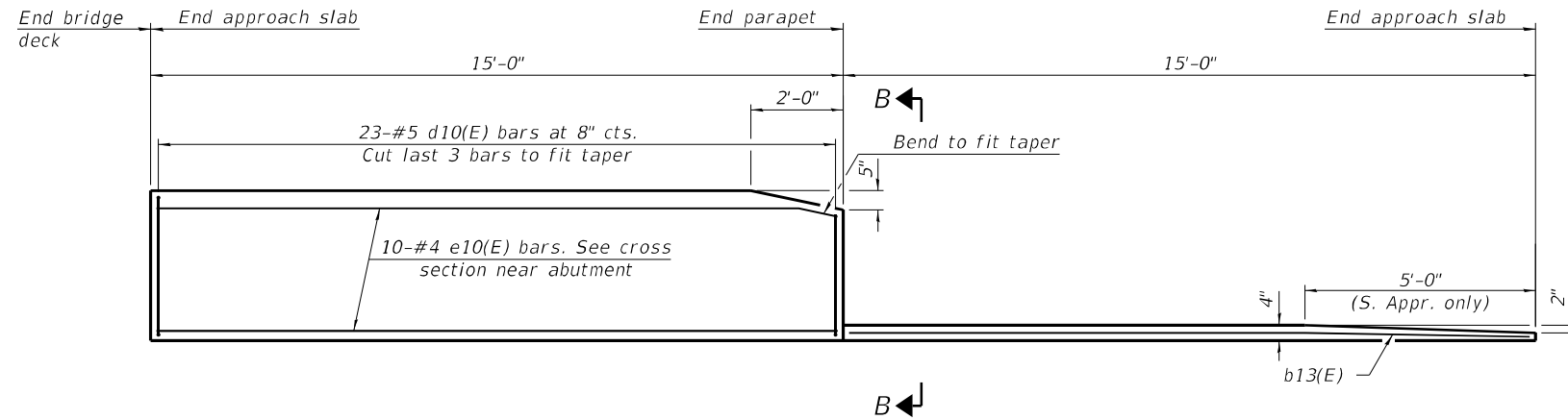
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PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 006-0054 (NB)**

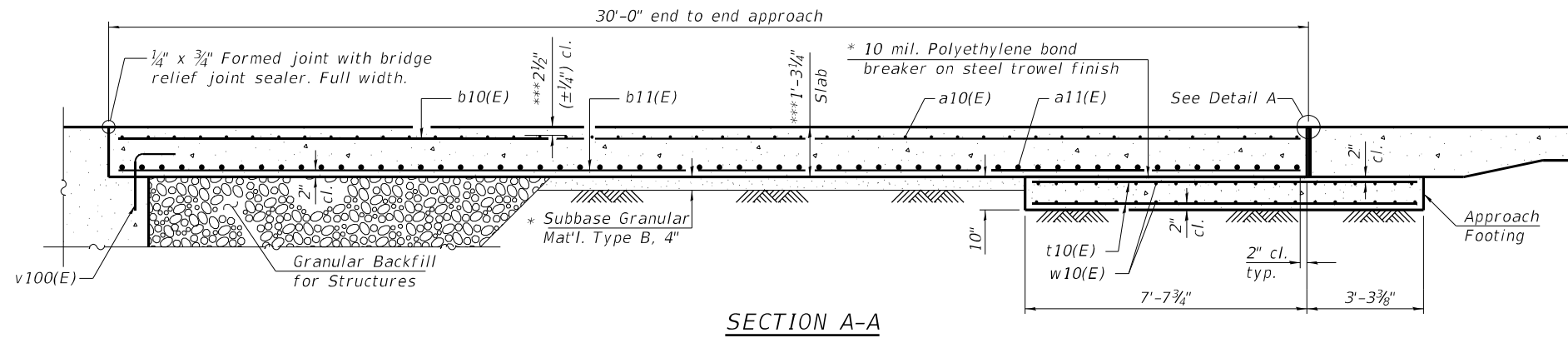
SHEET 22 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	242
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

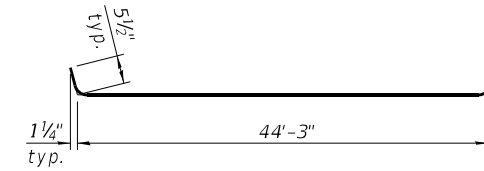
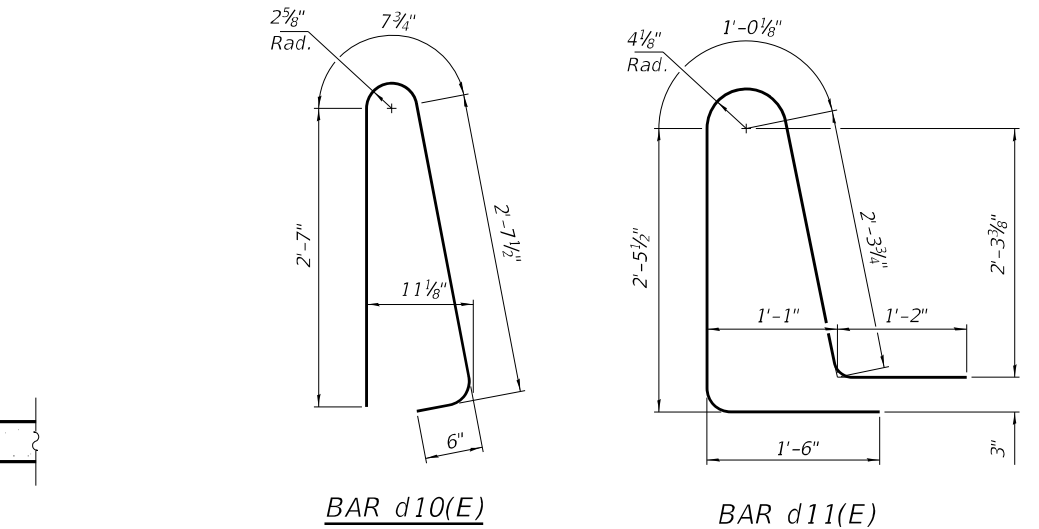


INSIDE ELEVATION OF PARAPET AND CURB

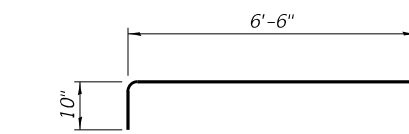
Notes:
 The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.
 Parapet concrete shall be paid for as Concrete Superstructure.
 Approach slab shall be paid for as Concrete Superstructure (Approach Slab).
 Approach footing concrete shall be paid for as Concrete Structures.
 The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
 Cost of excavation for approach footing included with Concrete Structures.
 For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 42.



SECTION A-A



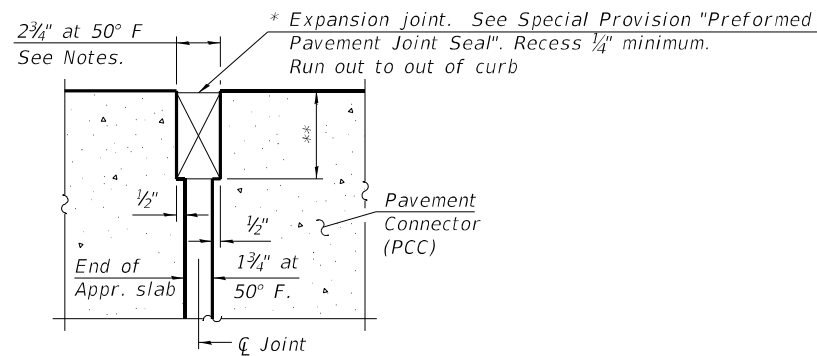
BAR a10(E)



BAR a12(E)

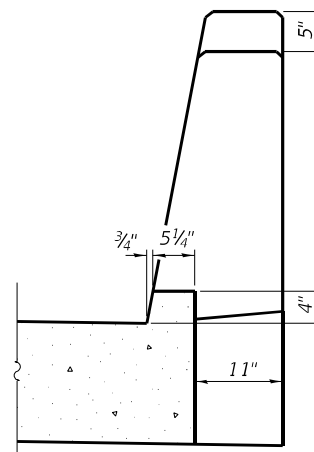
TWO APPROACHES
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a10(E)	84	#5	45'-2"	
a11(E)	110	#8	44'-5"	
a12(E)	84	#5	7'-4"	
b10(E)	124	#5	29'-8"	
b11(E)	198	#9	29'-8"	
b12(E)	8	#5	14'-8"	
b13(E)	8	#5	14'-8"	
b14(E)	2	#4	14'-6"	
b15(E)	2	#4	14'-10"	
d10(E)	92	#5	6'-5"	
d11(E)	92	#5	8'-6"	
e10(E)	40	#4	14'-8"	
t10(E)	84	#4	10'-6"	
w10(E)	80	#5	44'-5"	
Concrete Superstructure		Cu. Yd.	7.6	
Concrete Superstructure (Approach Slab)		Cu. Yd.	118.8	
Concrete Structures		Cu. Yd.	27.6	
Reinforcement Bars, Epoxy Coated		Pound	47,860	



DETAIL A
(at Rt. L's)

* Cost included with Concrete Superstructure (Approach Slab).
 ** Per manufacturer recommendations
 *** Prior to grinding



VIEW B-B

(Sheet 2 of 2)

MODEL: Default
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BAIA-CIP-39CS-L(≤30°) 10-27-2023

EFK Moen
Civil Engineering Design

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PLOT SCALE =	
PLOT DATE =	1/11/2024

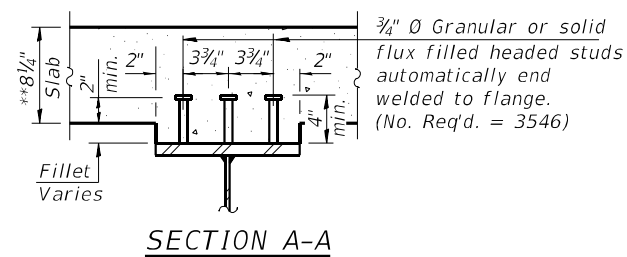
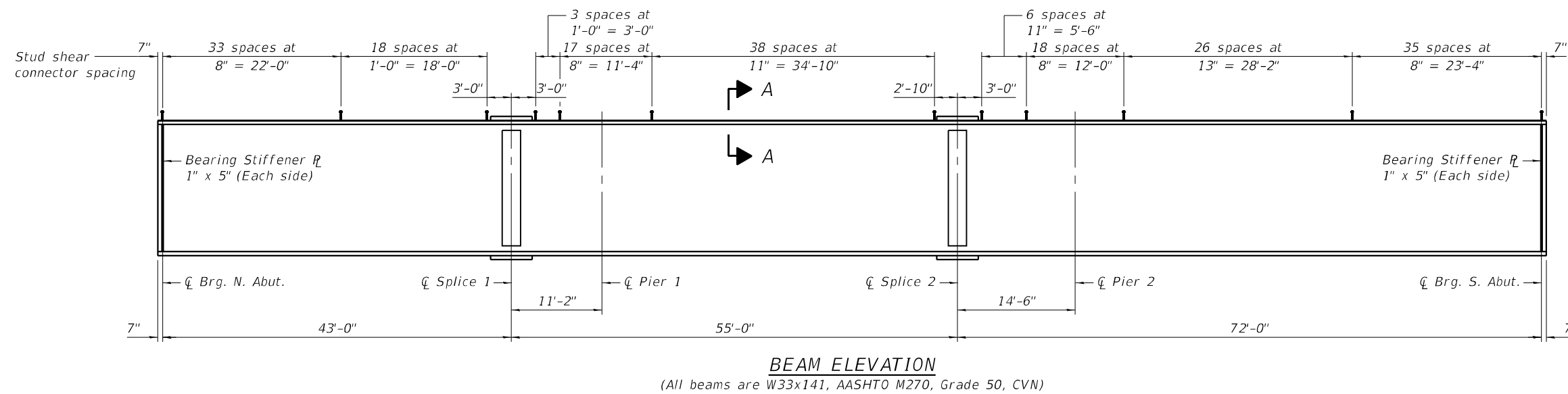
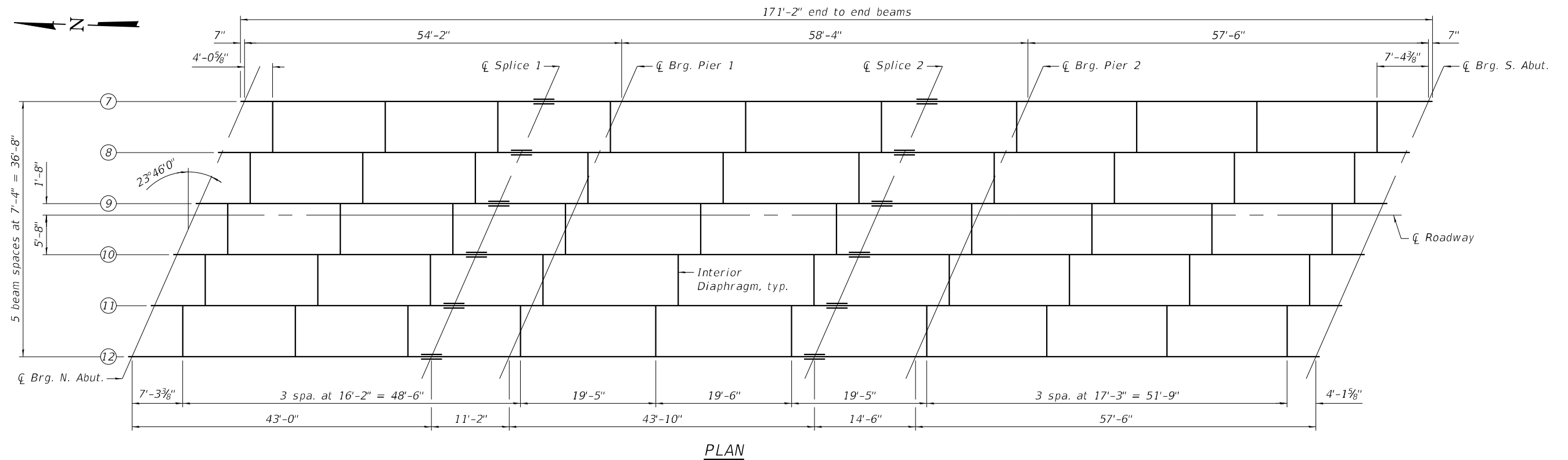
DESIGNED -	ACB	REVISED -	
CHECKED -	CDL	REVISED -	
DRAWN -	ACB	REVISED -	
CHECKED -	CDL	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 006-0054 (NB)

SHEET 23 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	243
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



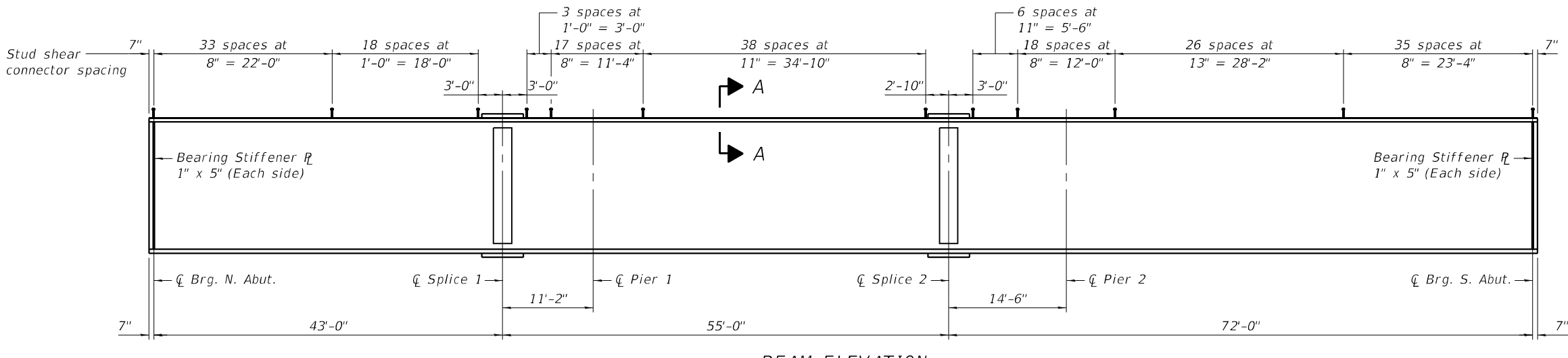
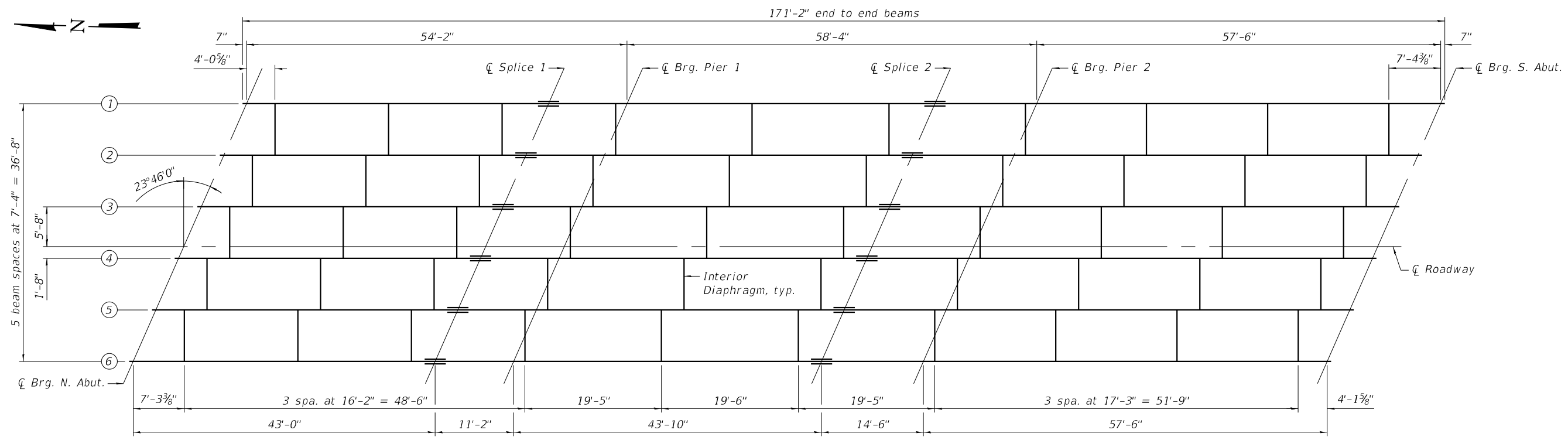
*TOP OF BEAM ELEVATIONS

Location	℄ Bearing N. Abut.	℄ Splice 1	℄ Bearing Pier 1	℄ Splice 2	℄ Bearing Pier 2	℄ Bearing S. Abut.
Beam 7	523.19	524.46	524.72	526.11	526.43	528.26
Beam 8	523.22	524.51	524.75	526.14	526.49	528.29
Beam 9	523.24	524.53	524.77	526.15	526.51	528.31
Beam 10	523.08	524.37	524.61	526.00	526.35	528.15
Beam 11	522.87	524.16	524.40	525.78	526.14	527.94
Beam 12	522.63	523.91	524.16	525.54	525.90	527.69

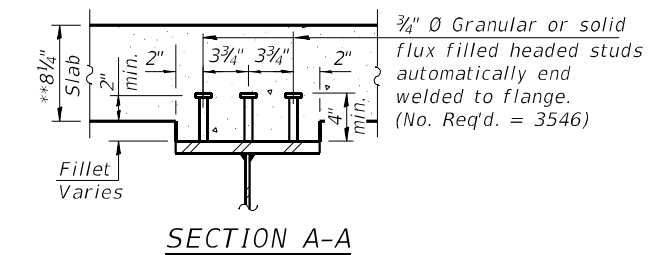
*For fabrication use only.

Notes:
Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
See sheet 26 of 42 for additional structural steel details.

MODEL: Default
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(All beams are W33x141, AASHTO M270, Grade 50, CVN)



*TOP OF BEAM ELEVATIONS

Location	℄ Bearing N. Abut.	℄ Splice 1	℄ Bearing Pier 1	℄ Splice 2	℄ Bearing Pier 2	℄ Bearing S. Abut.
Beam 1	523.85	525.18	525.44	526.89	527.27	529.17
Beam 2	523.90	525.22	525.48	526.94	527.32	529.21
Beam 3	523.92	525.24	525.50	526.95	527.33	529.22
Beam 4	523.88	525.20	525.46	526.91	527.29	529.18
Beam 5	523.68	524.99	525.25	526.70	527.08	528.97
Beam 6	523.45	524.76	525.02	526.46	526.84	528.74

*For fabrication use only.

Notes:
 Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
 See sheet 26 of 42 for additional structural steel details.

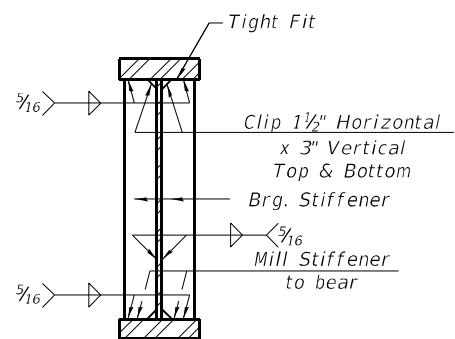
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EFK Moen Civil Engineering Design	USER NAME = ABenz	DESIGNED - ACB	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	FRAMING PLAN STRUCTURE NO. 006-0054 (NB)	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE =	DRAWN - ACB	REVISED -			180	(06-2HB-1)ES	BUREAU	327	245
PLOT DATE = 1/11/2024	CHECKED - CDL	REVISED -		SHEET 25 OF 42 SHEETS			CONTRACT NO. 66K66			
							ILLINOIS FED. AID PROJECT			

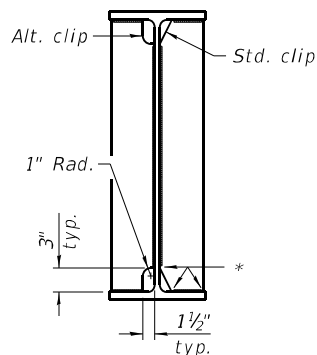
INTERIOR BEAM MOMENT TABLE						
		0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.6 Sp. 3
I_s	(in ⁴)	7450	7450	7450	7450	7450
$I_c(n)$	(in ⁴)	20959	20959	20959	20959	20959
$I_c(3n)$	(in ⁴)	15741	15741	15741	15741	15741
$I_c(cr)$	(in ⁴)	-	10335	-	10335	-
S_s	(in ³)	447.4	447.4	447.4	447.4	447.4
$S_c(n)$	(in ³)	666.1	666.1	666.1	666.1	666.1
$S_c(3n)$	(in ³)	607.1	607.1	607.1	607.1	607.1
$S_c(cr)$	(in ³)	-	512.7	-	512.7	-
S_x	(in ³)	635.5	-	656.0	-	630.6
DC1	(k/')	0.913	0.913	0.913	0.913	0.913
M _{DC1}	(k)	204.6	274.4	88.9	303.5	232.4
DC2	(k/')	0.175	0.175	0.175	0.175	0.175
M _{DC2}	(k)	40.1	53.8	17.8	59.4	45.7
DW	(k/')	0.333	0.333	0.333	0.333	0.333
M _{DW}	(k)	76.3	102.5	34.0	113.2	87.0
LLDF		0.62	0.62	0.62	0.62	0.62
M _{ℓ + IM}	(k)	559.1	546.2	497.1	588.4	628.1
f _r (Strength I)	(ksi)	0.0	0.0	0.0	0.0	0.0
M _u + 1/3 f _r S _x	(k)	1399	1520	1054	1653	1577
φ _r M _n	(k)	3422	-	3531	-	3395
f _s DC1	(ksi)	5.5	7.4	2.4	8.1	6.2
f _s DC2	(ksi)	0.8	1.3	0.4	1.4	0.9
f _s DW	(ksi)	1.5	2.4	0.7	2.6	1.7
f _s (ℓ+IM)	(ksi)	10.1	12.8	9.0	13.8	11.3
f _r (Service II)	(ksi)	0.0	0.0	0.0	0.0	0.0
f _s + 1/2 (Service II)	(ksi)	20.9	27.6	15.0	30.1	23.6
Service II Resistance	(ksi)	47.5	47.5	47.5	47.5	47.5
f _s + 1/3 (Strength I)	(ksi)	-	-	-	-	-
φ _r F _n	(ksi)	-	50.0	-	50.0	-
V _r	(k)	26.0	34.5	24.0	38.0	23.0

BEAM REACTION TABLE					
		N. Abut.	Pier 1	Pier 2	S. Abut.
LLDF		0.84	0.77	0.77	0.84
OCF		1.10	-	-	1.10
R _{DC1}	(k)	*39.2	54.7	57.3	*40.5
R _{DC2}	(k)	3.7	10.7	11.3	4.0
R _{DW}	(k)	7.1	20.5	21.5	7.6
R _ℓ	(k)	61.1	111.1	114.1	63.1
R _{IM}	(k)	15.9	24.8	25.2	16.1
R _{Total (Strength I)(Impact)}	(k)	199.1	350.2	361.6	205.6
R _{Total (Strength I)(No Impact)}	(k)	171.3	306.8	317.6	177.4

*R_{DC1} includes service reaction due to weight of approach slab and parapet on approach slab.

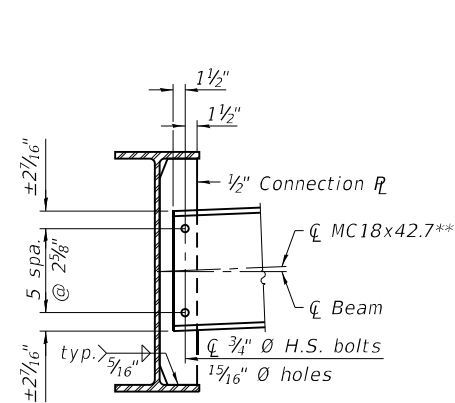


**BEARING STIFFENER
AT ABUTMENT**



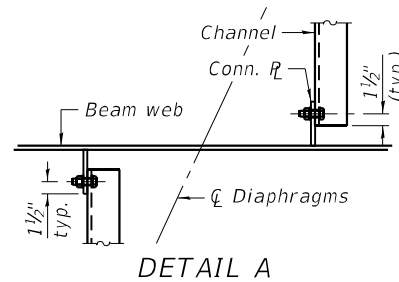
WELD LIMITS AND CLIP DETAILS

* Stop welds 1/4" (±1/8") from edges as shown. Typical.

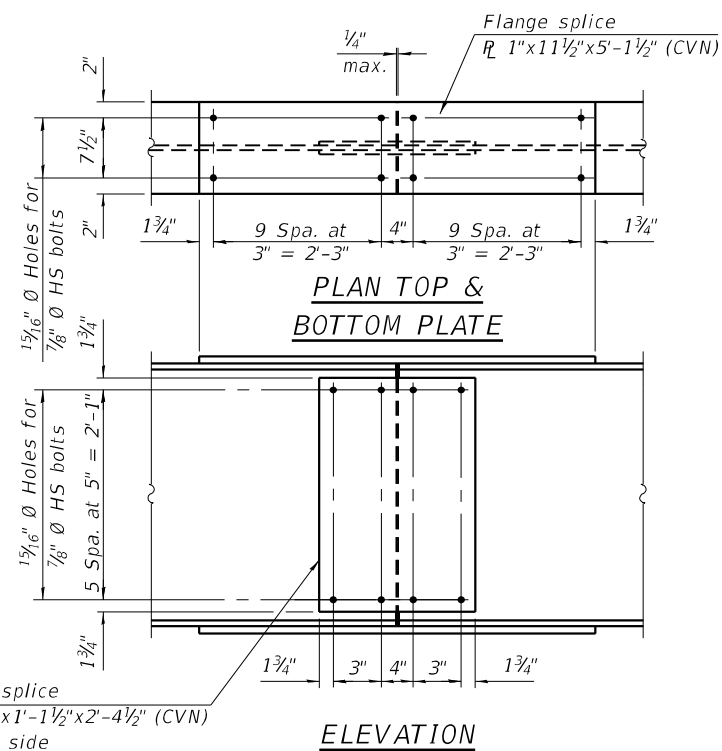


**INTERIOR DIAPHRAGM
(100 Required)**

** Alternate channels of equal depth and larger weight are permitted to facilitate material acquisition. Alternate channels, if utilized, shall be provided at no additional cost to the department.



Notes:
Load carrying components designed "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
Two hardened washers required for each set of oversized holes.
See Detail A for connection plate orientation.
All diaphragms shall be installed as steel is erected and secured with erection pins and bolts. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor bolts.



**SPlice DETAIL
(24 Required)**

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in.⁴ and in.³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.⁴ and in.³).

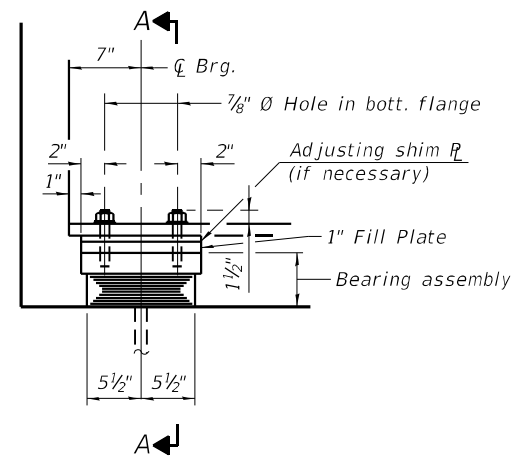
$I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.⁴ and in.³).

S_x : Section modulus about the major axis of a section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in.³).

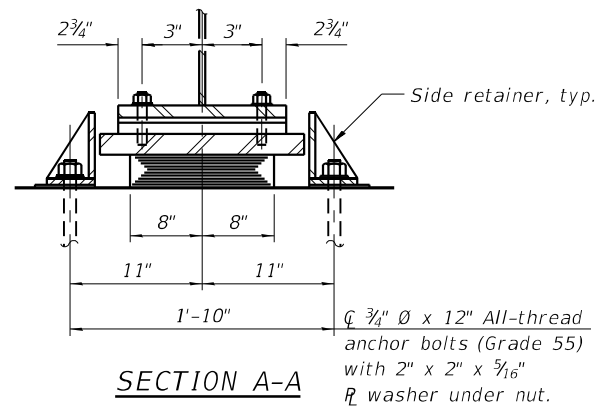
DC1: Un-factored non-composite dead load (kips/ft.).
DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).
M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
LLDF: Live Load Distribution Factor for moment and shear computed according to Article 4.6.2.2 and further IDOT provisions.
M_{ℓ + IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
M_u : Strength I load combination of factored design moments (kip-ft.).
1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{ℓ + IM}
f_ℓ : Factored calculated flange lateral bending stress as calculated using Article 6.10.1.6 and as further simplified by IDOT provisions (ksi).
φ_r M_n: Factored nominal flexural resistance of the section determined as specified in Article 6.10.7.1 or A6 as applicable (kip-ft.).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
M_{DC1} / S_s
 f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
M_{DC2} / S_{c(3n)} or M_{DC2} / S_{c(cr)} as applicable.
 f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
M_{DW} / S_{c(3n)} or M_{DW} / S_{c(cr)} as applicable.
 f_s (ℓ + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
M_{ℓ + IM} / S_{c(n)} or M_{ℓ + IM} / S_{c(cr)} as applicable.
 $f_s + f_i / 2$ (Service II): Sum of stresses as computed below (ksi).
f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (ℓ + IM) + f_i / 2
Service II Resistance: Composite (0.95R_nF_{yr}) or noncomposite (0.80R_nF_{yr}) stress capacity according to Article 6.10.4.2 (ksi).
 $f_s + f_i / 3$ (Strength I): Sum of stresses as computed below on non-compact sections (ksi).
1.25 (f_s DC1 + f_s DC2) + 1.5 f_s DW + 1.75 f_s (ℓ + IM) + f_i / 3
φ_r F_n : Factored nominal flexural resistance of the section as specified in Article 6.10.7.2 or 6.10.8 as applicable (ksi).
V_r: Maximum factored shear range in span computed according to Article 6.10.10.
OCF: Obtuse Correction Factor according to Article 4.6.2.2.3c or as further simplified by IDOT provisions.
R_{DC1} : Un-factored reaction due to non-composite dead load (kip).
R_{DC2} : Un-factored reaction due to long-term composite (superimposed excluding future wearing surface) dead load (kip).
R_{DW} : Un-factored reaction due to long-term composite (superimposed future wearing surface only) dead load (kip).
R_ℓ : Un-factored live load reaction (kip).
R_{IM} : Un-factored dynamic load allowance (impact) (kip).
R_{Total (Strength I)(Impact)}: Strength I load combination of factored design reactions (kip).
1.25 (R_{DC1} + R_{DC2}) + 1.5R_{DW} + 1.75 (R_ℓ + R_{IM})
R_{Total (Strength I)(No Impact)}: Strength I load combination of factored design reactions, not including dynamic load allowance (Impact) (kip).
1.25 (R_{DC1} + R_{DC2}) + 1.5R_{DW} + 1.75 (R_ℓ)

MODEL: Default
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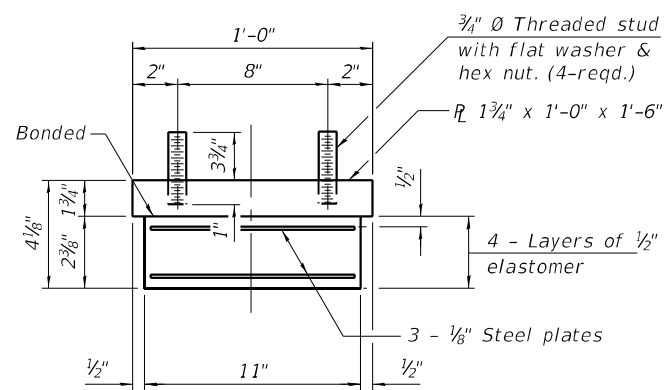
ELEVATION AT ABUT.



SECTION A-A

Side retainer, typ.
 1'-10" \varnothing 3/4" \times 12" All-thread anchor bolts (Grade 55) with 2" \times 2" \times 5/16" R washer under nut.

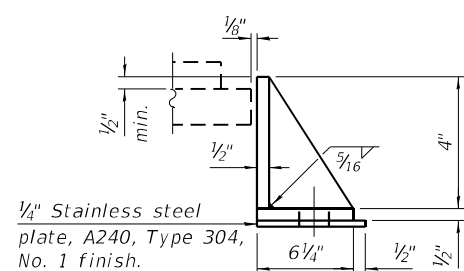
TYPE I ELASTOMERIC EXP. BRG.



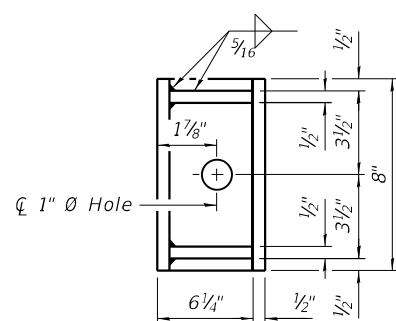
BEARING ASSEMBLY

Note:
 Shim plates shall not be placed under bearing assembly.

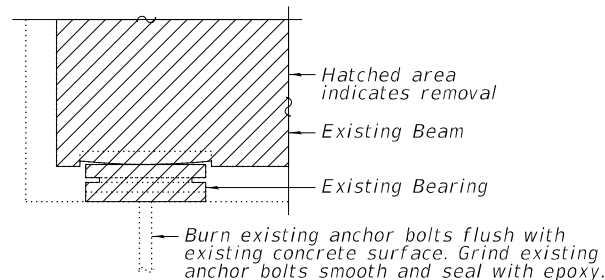
Notes:
 Side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.
 Fill plates, shim plates, and connection bolts are included with Furnishing and Erecting Structural Steel.
 Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
 The fill plates and structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M270 Grade 50.
 Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed shown on bearing details.



SIDE RETAINER
 Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



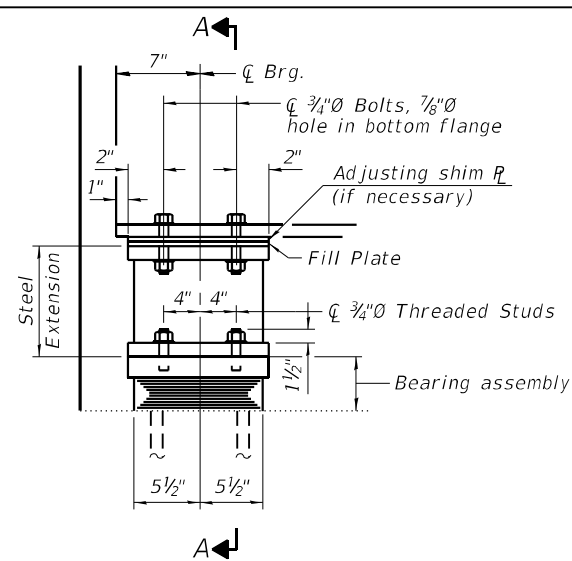
EXISTING BEARING REMOVAL DETAIL
 (Cost included with Removal of Existing Superstructure)



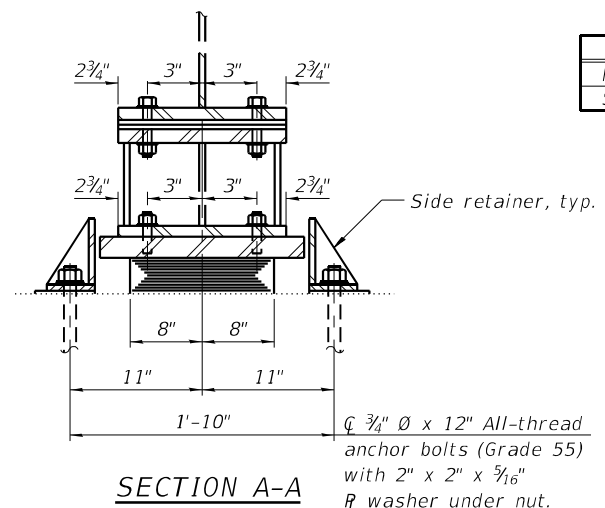
BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type I	Each	12
Anchor Bolts, 3/4"	Each	24

MODEL: Default
 FILE NAME: \\SERVER18\Projects\554\22057.03 IDOT D3 PTB 204-028 WO 03 1-180 over Bottom Road\DGM\Bridges\Final\Plotsheets\006-0053&0054+66K66-027-Abutment Bearings (SB).dgn
 1/11/2024 8:51:59 AM



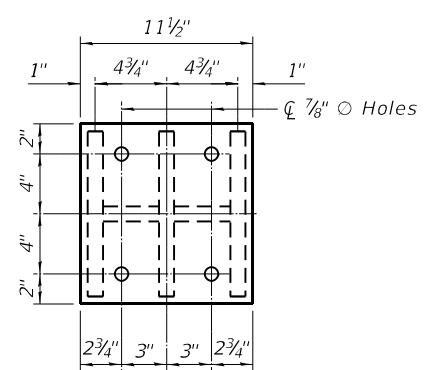
ELEVATION AT ABUT.



SECTION A-A

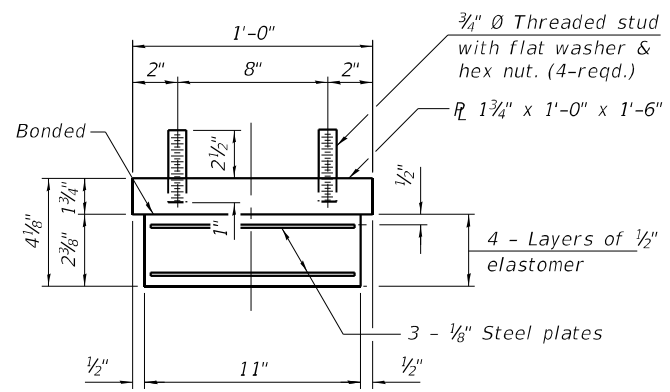
TABLE OF FILL PLATE THICKNESSES

	Beam 1	Beam 2	Beam 3	Beam 4	Beam 5	Beam 6
N. Abut.	3/4"	7/8"	1"	5/8"	3/4"	7/8"
S. Abut.	1 1/8"	1 1/8"	1 1/4"	7/8"	1/2"	3/4"



PLAN - TOP EXTENSION PLATE

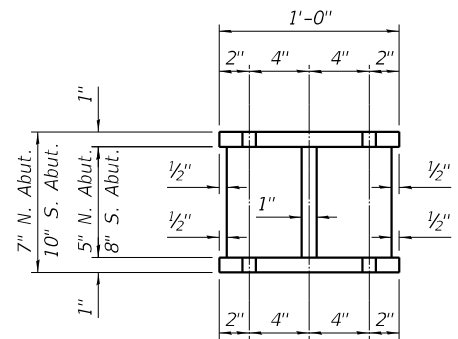
TYPE I ELASTOMERIC EXP. BRG.



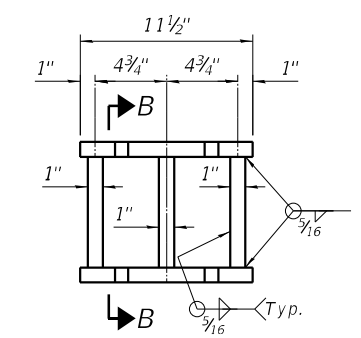
BEARING ASSEMBLY

Note:
Shim plates shall not be placed under bearing assembly.

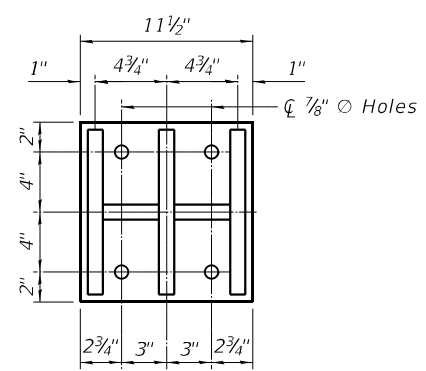
Notes:
Side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.
Steel Extensions, fill plates, shim plates, and connection bolts are included with Furnishing and Erecting Structural Steel.
Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
The steel extensions, fill plates, and structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M270 Grade 50.
Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed shown on bearing details.
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.



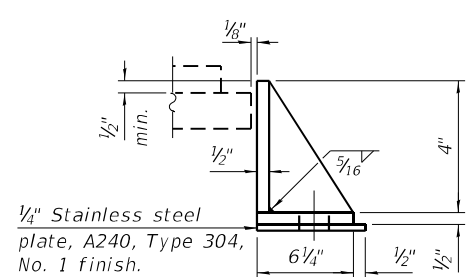
SECTION B-B



END VIEW

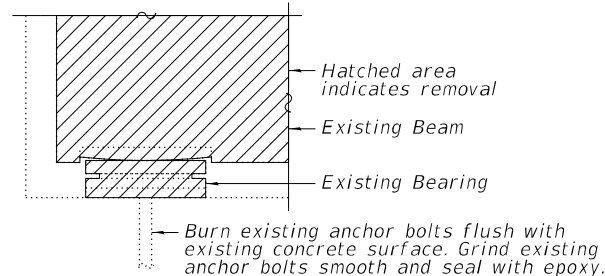


PLAN - BOTTOM EXTENSION PLATE



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



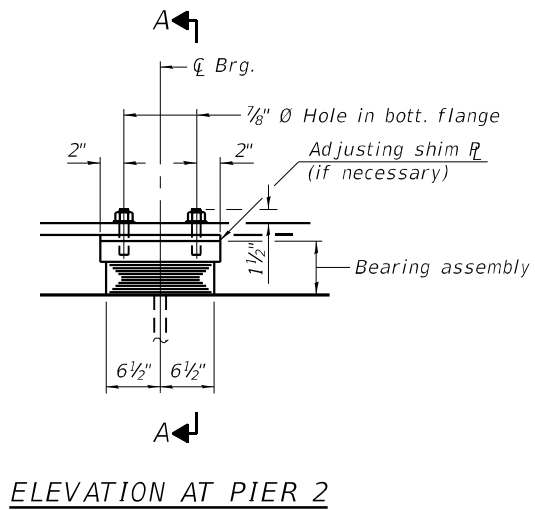
EXISTING BEARING REMOVAL DETAIL

(Cost included with Removal of Existing Superstructure)

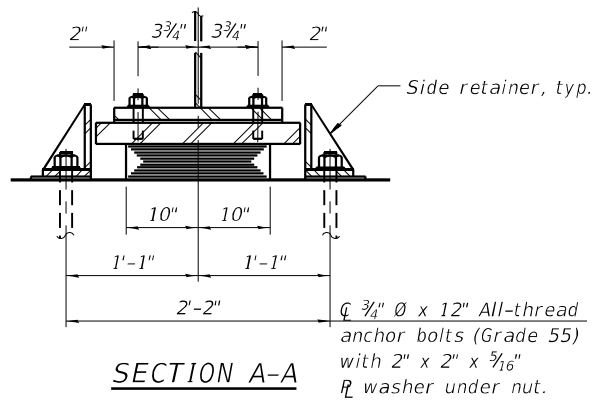
BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type I	Each	12
Anchor Bolts, 3/4"	Each	48

MODEL: Default
FILE NAME: \\SERVER18\Projects\5422057.03 IDOT D3 PTB 204-028 WO 03 1-180 over Bottom Road\DGM\Bridges\Final\Plotsheets\006-0054-66K66-028-Abutment Bearings (NB).dgn
2/6/2024 11:19:29 AM

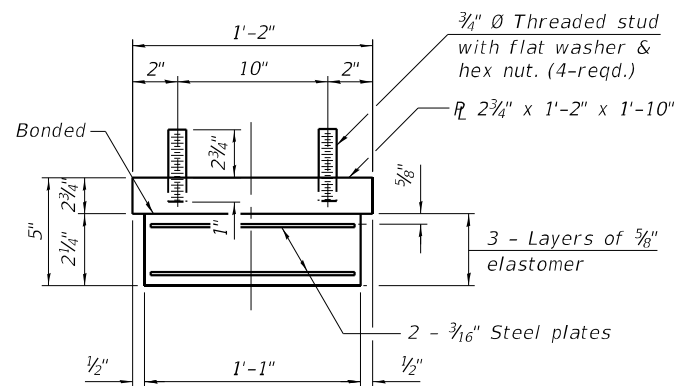


ELEVATION AT PIER 2



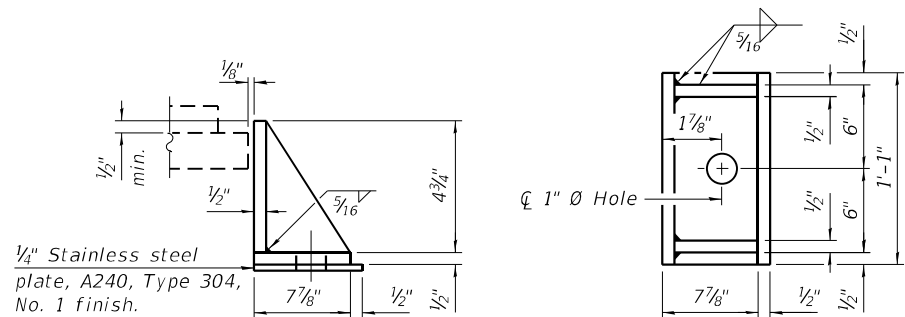
SECTION A-A

TYPE I ELASTOMERIC EXP. BRG.



BEARING ASSEMBLY

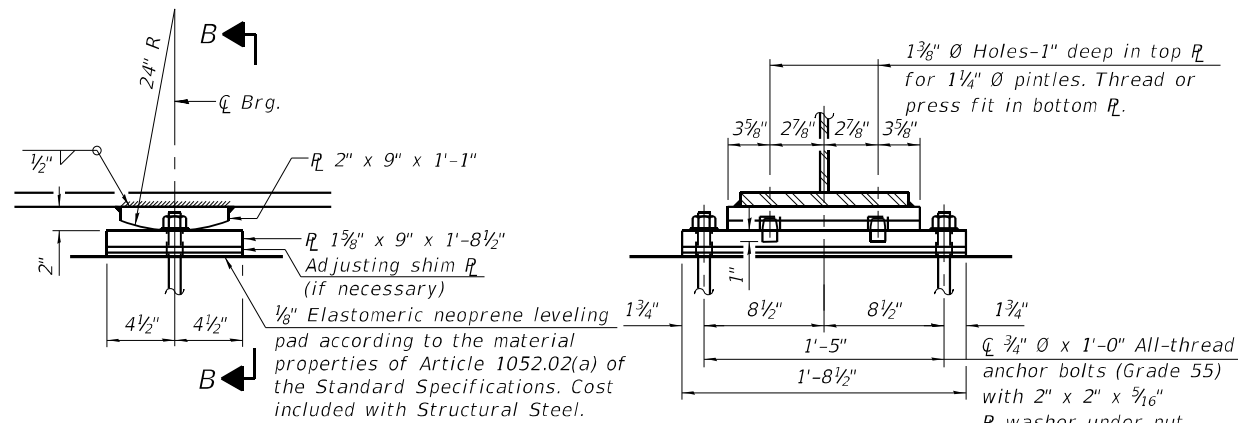
Note:
Shim plates shall not be placed under bearing assembly.



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

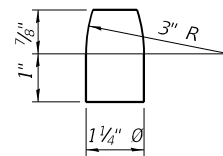
Notes:
Side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.
Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M270 Grade 50.
The structural steel plates and pintles shall conform to the requirements of AASHTO M270 Grade 50.
Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed shown on bearing details.



ELEVATION AT PIER 1

SECTION B-B

FIXED BEARING



PINTLE

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type I	Each	12
Anchor Bolts, 3/4"	Each	48

I-2E-1

6-15-2019

EFK Moen
Civil Engineering Design

USER NAME =	ACB
DESIGNED -	CDL
CHECKED -	ACB
PLOT SCALE =	
DRAWN -	CDL
PLOT DATE =	1/11/2024

DESIGNED -	ACB
CHECKED -	CDL
REVISOR -	
REVISIONS -	
REVISIONS -	
REVISIONS -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

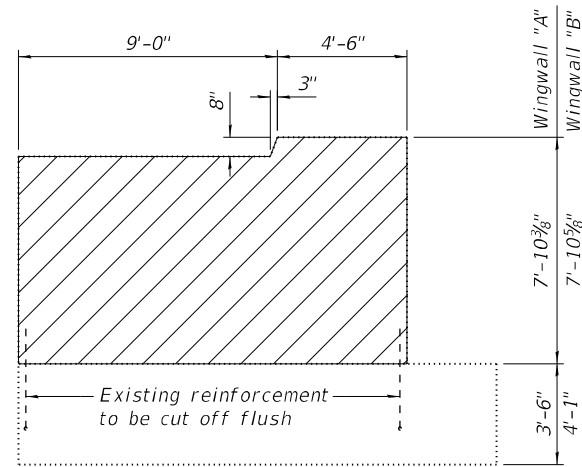
PIER BEARING DETAILS
STRUCTURE NO. 006-0053 (SB) & 006-0054 (NB)

SHEET 29 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	249
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

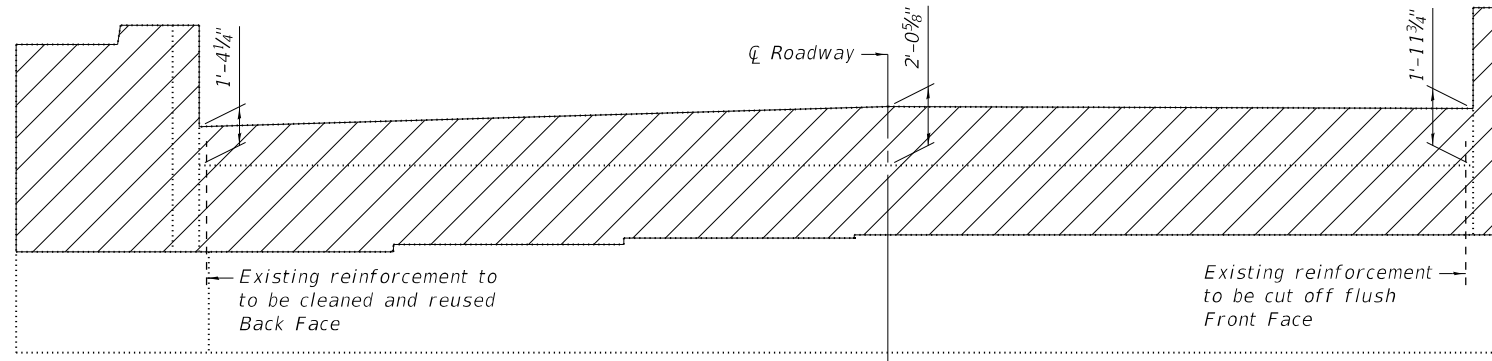
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1/11/2024 8:52:07 AM

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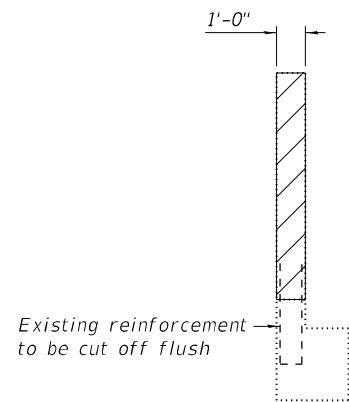
WINGWALL ELEVATION

(S.B. North Abutment West Wingwall shown, other wingwalls similar)

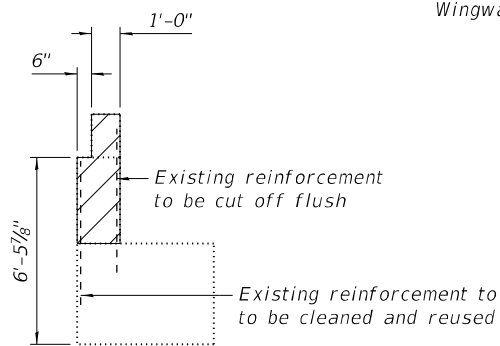


ELEVATION

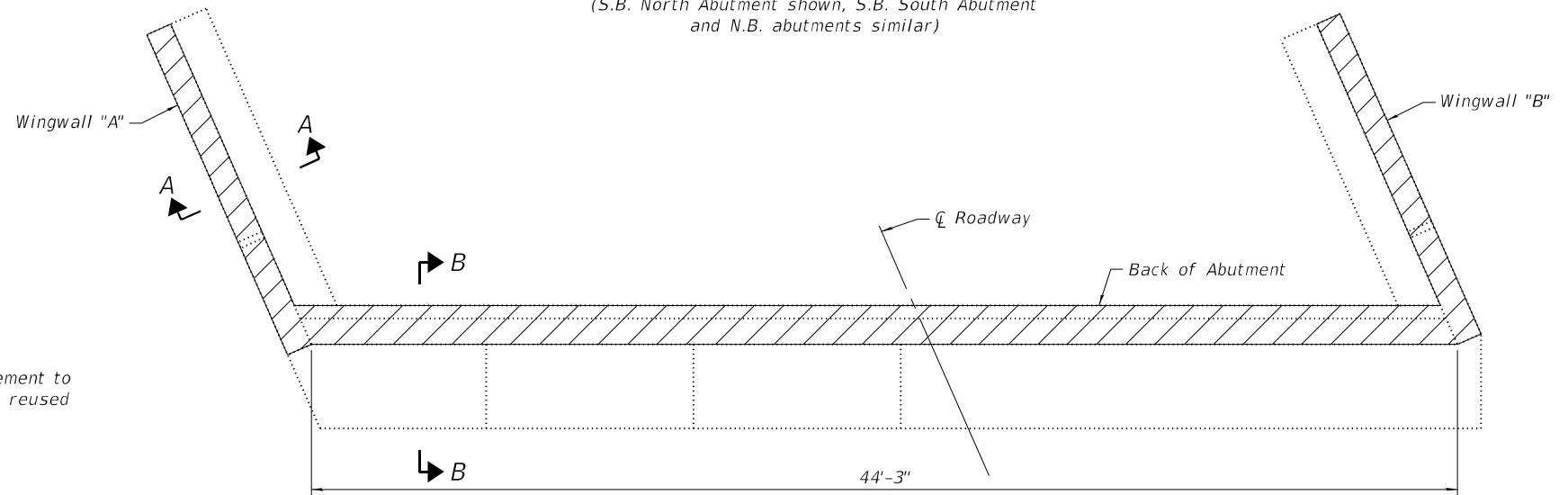
(S.B. North Abutment shown, S.B. South Abutment and N.B. abutments similar)



SECTION A-A



SECTION B-B



PLAN

(S.B. North Abutment shown, S.B. South Abutment and N.B. abutments similar)

**STRUCTURE NO. 006-0053
BILL OF MATERIAL - 2 ABUTMENTS**

Item	Unit	Total
Concrete Removal	Cu. Yd.	33.8

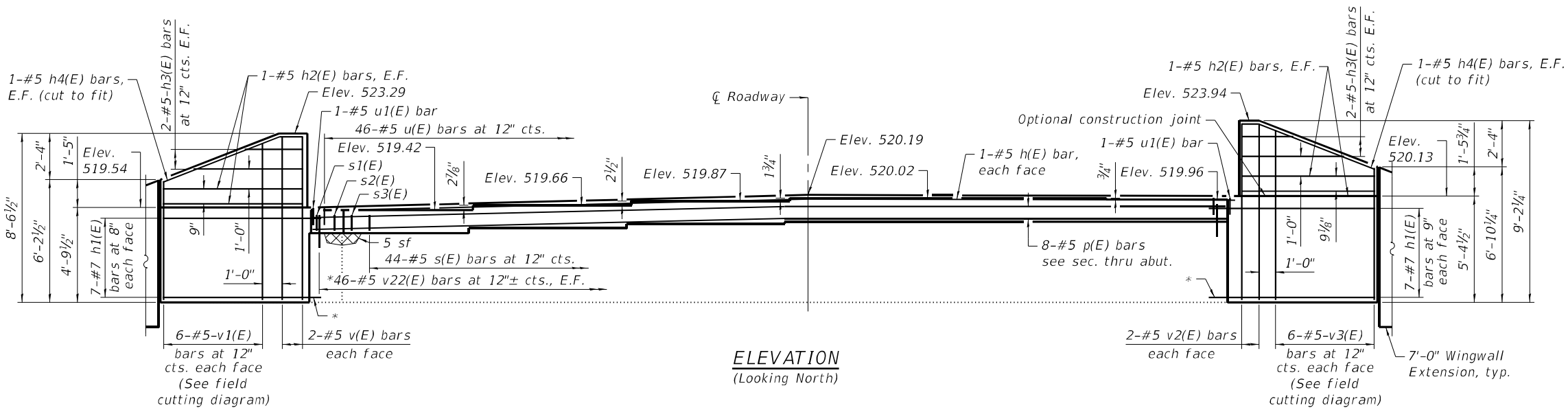
**STRUCTURE NO. 006-0054
BILL OF MATERIAL - 2 ABUTMENTS**

Item	Unit	Total
Concrete Removal	Cu. Yd.	33.8

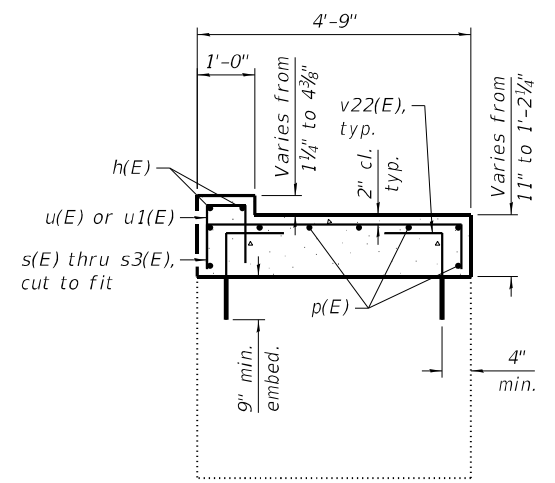
Notes:
 Hatched areas indicate Concrete Removal.
 Existing reinforcement extending into new construction shall be cleaned, straightened, and incorporated into the new construction. Cost included with Concrete Removal.
 Existing reinforcement not extending into new construction shall be cut off flush and sealed with epoxy. Cost included with Concrete Removal.
 Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost included with Concrete Removal.

USER NAME = ABenz	DESIGNED - ACB	REVISED -
PLOT SCALE =	CHECKED - CDL	REVISED -
PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
	CHECKED - CDL	REVISED -

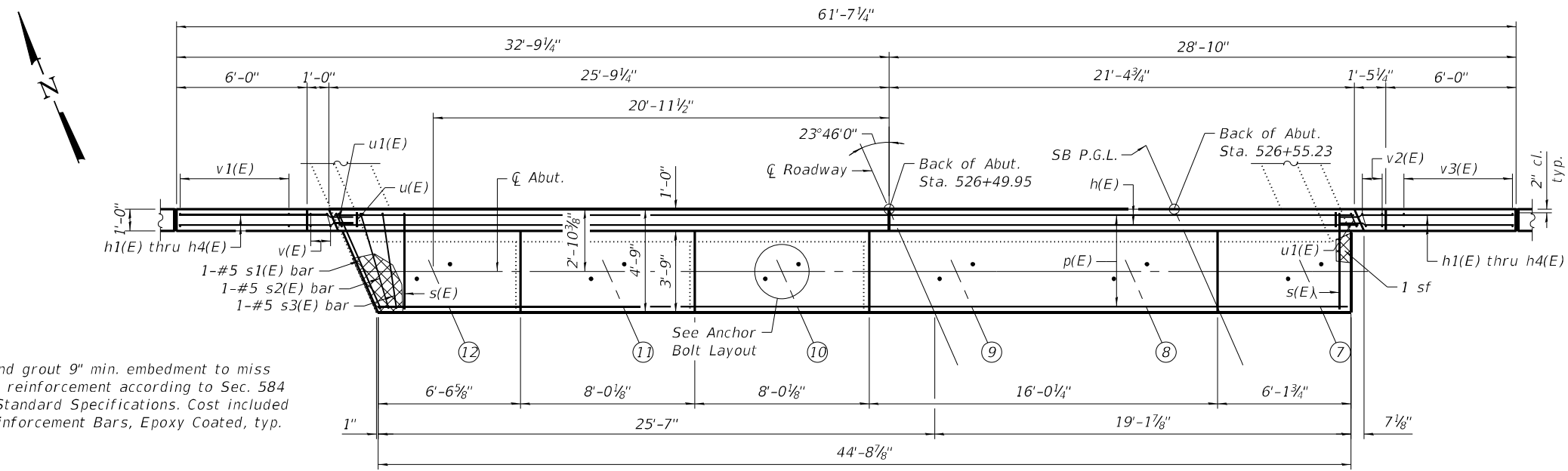
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	250
CONTRACT NO. 66K66				



ELEVATION
(Looking North)



SECTION THRU ABUTMENT



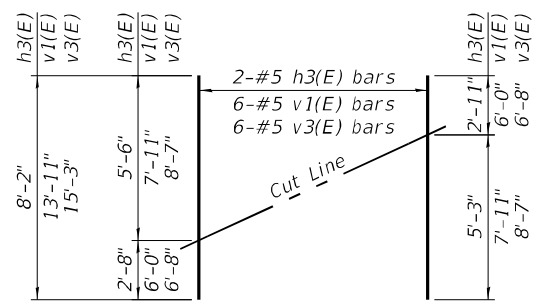
PLAN

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	2	#5	46'-10"	—
h1(E)	28	#7	8'-7"	—
h2(E)	8	#5	7'-1"	—
h3(E)	4	#5	8'-2"	—
h4(E)	4	#5	7'-6"	—
h5(E)	32	#4	6'-8"	—
n(E)	32	#4	4'-6"	└
p(E)	8	#5	46'-10"	—
s(E)	44	#5	6'-3"	┌
s1(E)	1	#5	6'-8"	┌
s2(E)	1	#5	6'-5"	┌
s3(E)	1	#5	6'-4"	┌
t(E)	18	#5	6'-10"	└
t1(E)	18	#5	6'-2"	—
u(E)	46	#5	2'-4"	┌
u1(E)	2	#5	2'-5"	┌
v(E)	4	#5	8'-2"	—
v1(E)	6	#5	13'-11"	—
v2(E)	4	#5	8'-10"	—
v3(E)	6	#5	15'-3"	—
v8(E)	8	#4	9'-1"	—
v9(E)	8	#4	10'-5"	—
v22(E)	92	#5	2'-5"	┌
w(E)	28	#4	6'-8"	—
Structure Excavation	Cu. Yd.		91	
Concrete Structures	Cu. Yd.		20.2	
Reinforcement Bars, Epoxy Coated	Pound		2,730	
Structural Repair of Concrete (Depth Greater than 5 Inches)	Sq. Ft.		6	

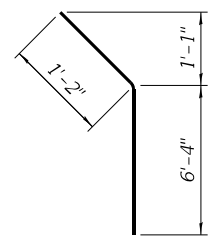
*Drill and grout 9" min. embedment to miss existing reinforcement according to Sec. 584 of the Standard Specifications. Cost included with Reinforcement Bars, Epoxy Coated, typ.

Structural Repair of Concrete (Depth Greater than 5 Inches)

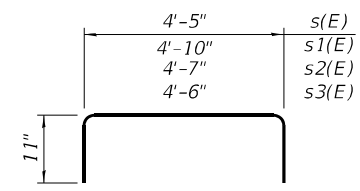


FIELD CUTTING DIAGRAM

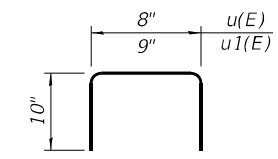
Order h3(E), v1(E), and v3(E) full length. Cut as shown and use remainder of bars in opposite face.



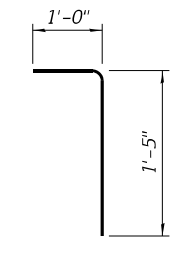
BAR h4(E)



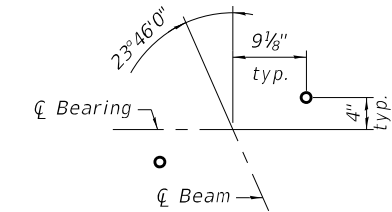
BAR s(E) thru s3(E)



BAR u(E) & u1(E)



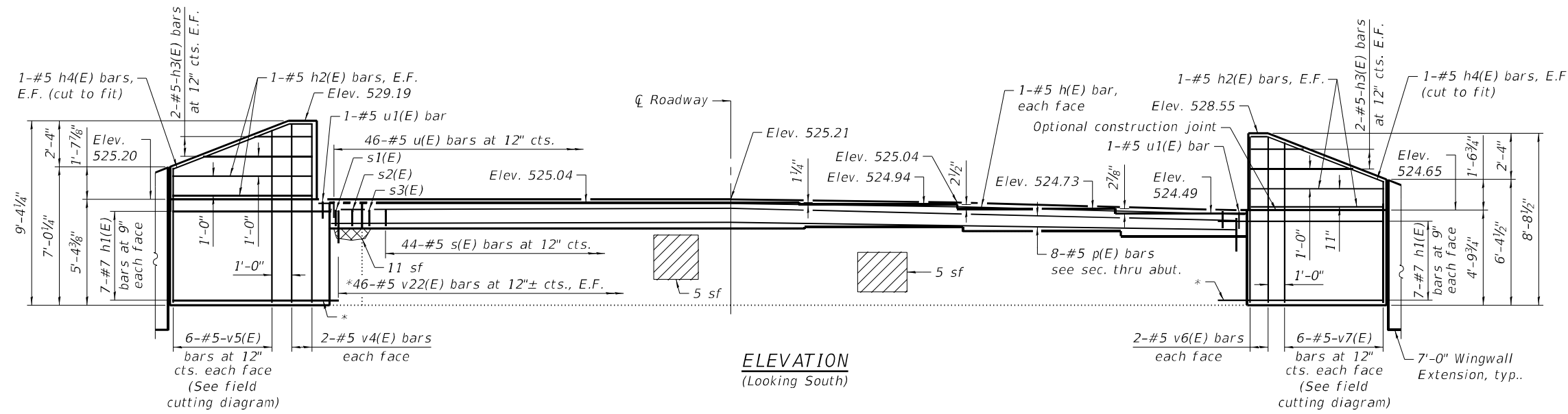
BAR v22(E)



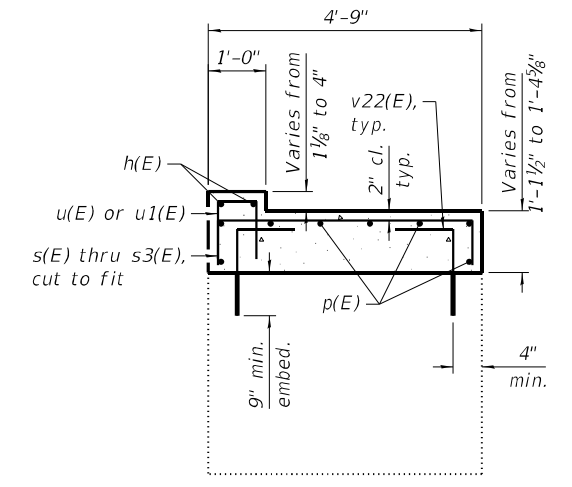
ANCHOR BOLT LAYOUT

Notes:
Space reinforcement in cap to miss anchor bolts.
Pour steps monolithically with cap.
E.F. = each face

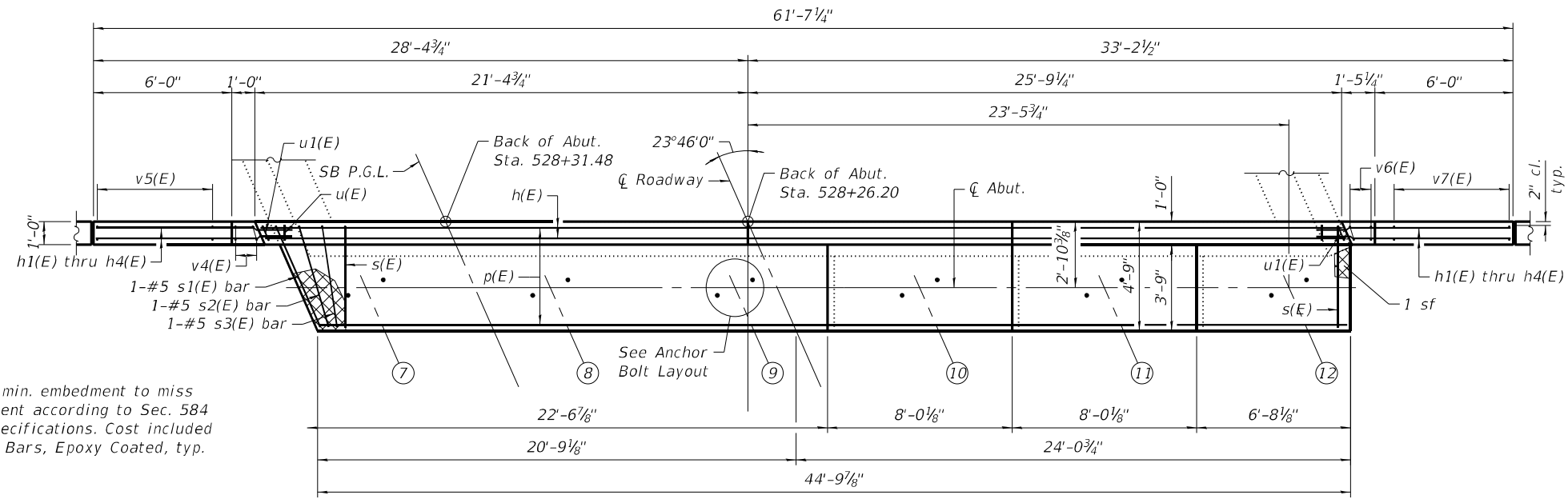
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2/6/2024 11:19:39 AM



ELEVATION
(Looking South)



SECTION THRU ABUTMENT



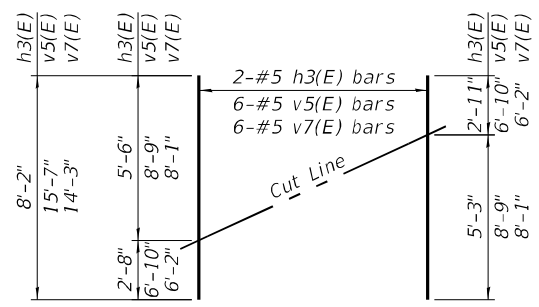
PLAN

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	2	#5	46'-10"	—
h1(E)	28	#7	8'-7"	—
h2(E)	8	#5	7'-1"	—
h3(E)	4	#5	8'-2"	—
h4(E)	4	#5	7'-6"	—
h5(E)	32	#4	6'-8"	—
n(E)	32	#4	4'-6"	└
p(E)	8	#5	46'-10"	—
s(E)	44	#5	6'-3"	┌
s1(E)	1	#5	6'-8"	┌
s2(E)	1	#5	6'-5"	┌
s3(E)	1	#5	6'-4"	┌
t(E)	18	#5	6'-10"	└
t1(E)	18	#5	6'-2"	—
u(E)	46	#5	2'-4"	┌
u1(E)	2	#5	2'-5"	┌
v4(E)	4	#5	9'-0"	—
v5(E)	6	#5	15'-7"	—
v6(E)	4	#5	8'-4"	—
v7(E)	6	#5	14'-3"	—
v8(E)	8	#4	9'-6"	—
v9(E)	8	#4	10'-5"	—
v22(E)	92	#5	2'-5"	└
w(E)	28	#4	6'-8"	—
Structure Excavation	Cu. Yd.		103	
Concrete Structures	Cu. Yd.		21.1	
Reinforcement Bars, Epoxy Coated	Pound		2,730	
Structural Repair of Concrete (Depth Greater than 5 Inches)	Sq. Ft.		12	
Structural Repair of Concrete (Depth Equal to or Less than 5 Inches)	Sq. Ft.		10	

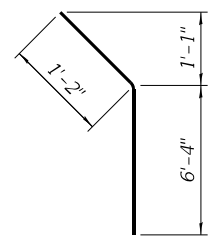
*Drill and grout 9" min. embedment to miss existing reinforcement according to Sec. 584 of the Standard Specifications. Cost included with Reinforcement Bars, Epoxy Coated, typ.

Structural Repair of Concrete (Depth Greater than 5 Inches)
 Structural Repair of Concrete (Depth Equal to or Less than 5 Inches)

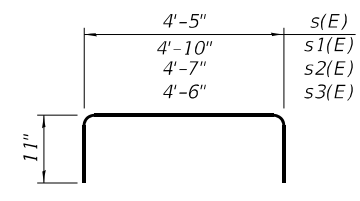


FIELD CUTTING DIAGRAM

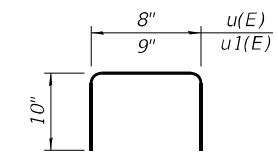
Order h3(E), v5(E), and v7(E) full length. Cut as shown and use remainder of bars in opposite face.



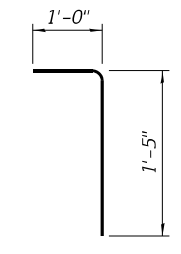
BAR h4(E)



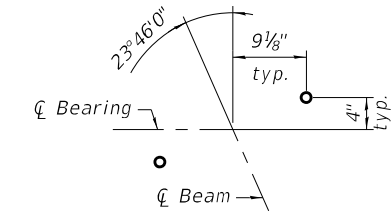
BAR s(E) thru s3(E)



BAR u(E) & u1(E)



BAR v22(E)

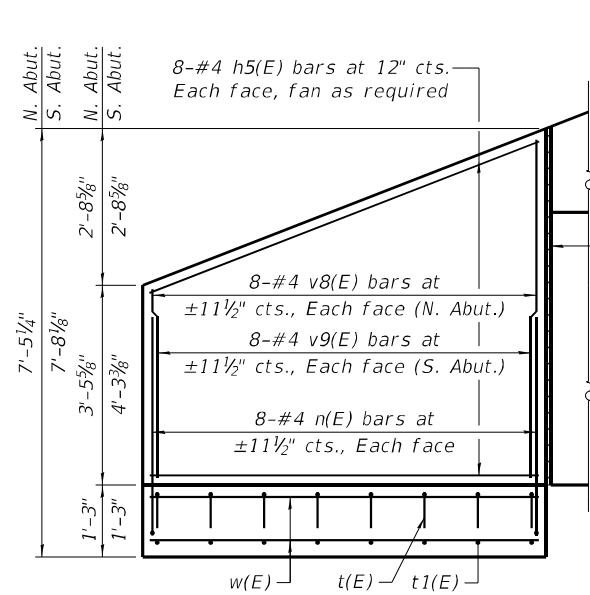
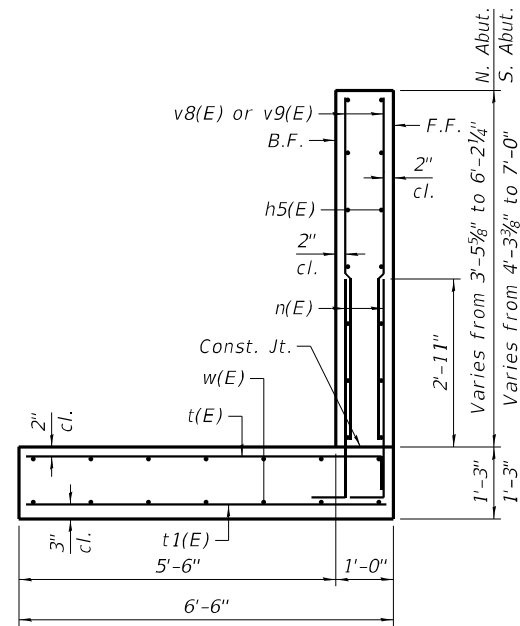


ANCHOR BOLT LAYOUT

Notes:
 Space reinforcement in cap to miss anchor bolts.
 Pour steps monolithically with cap.
 E.F. = each face

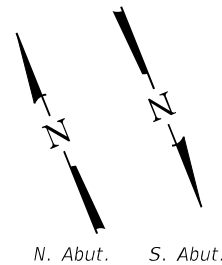
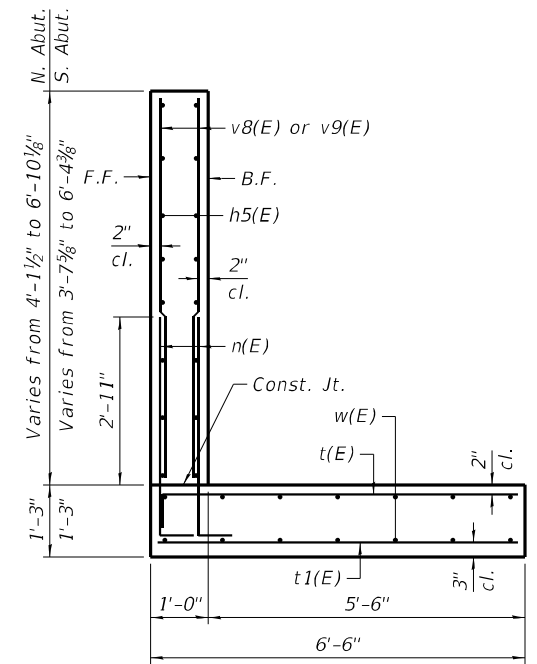
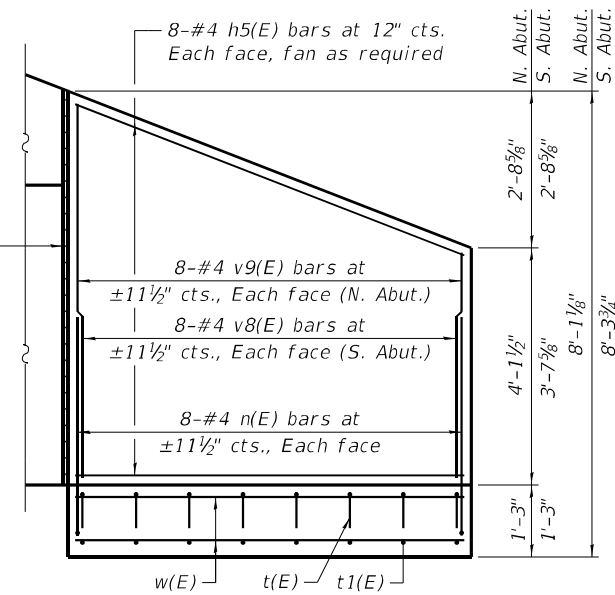
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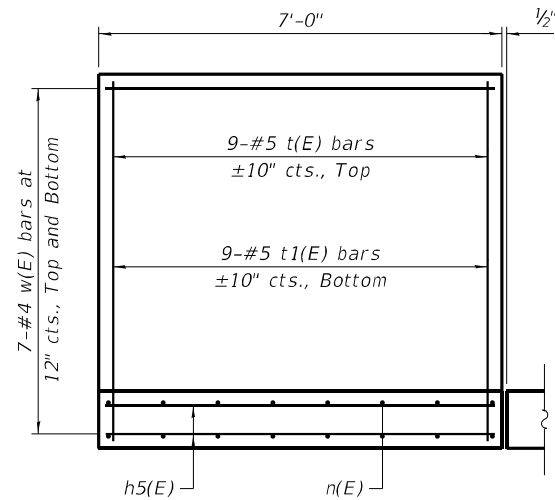


ELEVATION

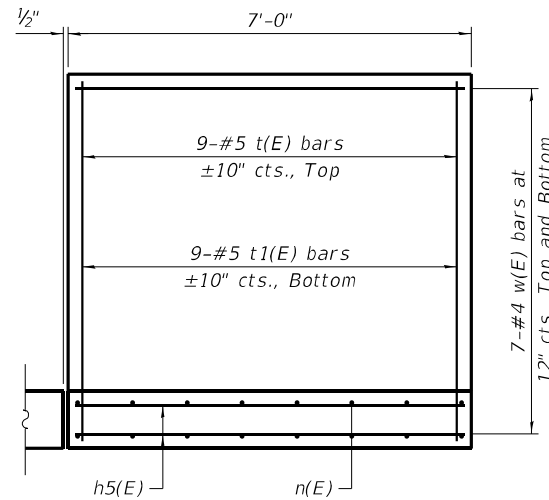
Maximum applied service bearing pressure, $Q_{max} = 1080$ psf.



N. Abut. S. Abut.

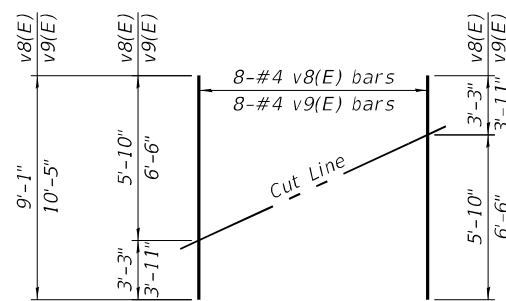


FOOTING PLAN



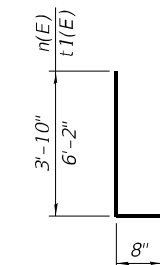
MINIMUM BAR LAP

#4 bar = 2'-7"

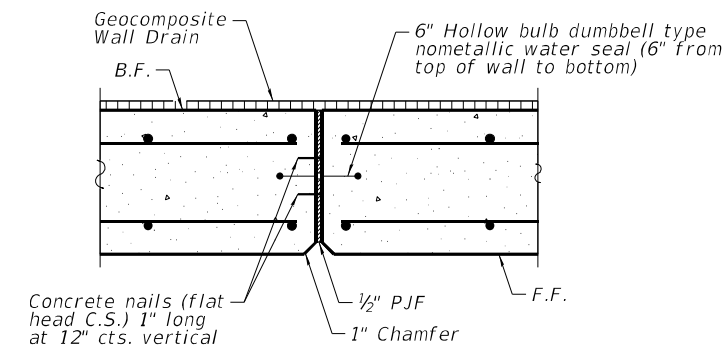


FIELD CUTTING DIAGRAM

Order v8(E) and v9(E) full length. Cut as shown and use remainder of bars in opposite face.



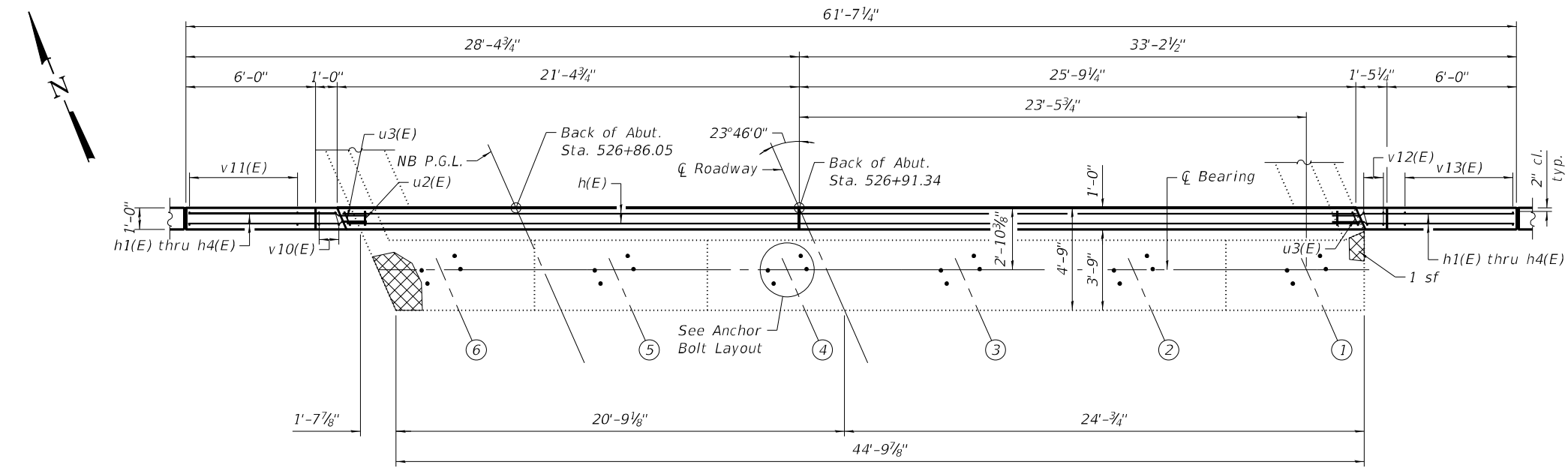
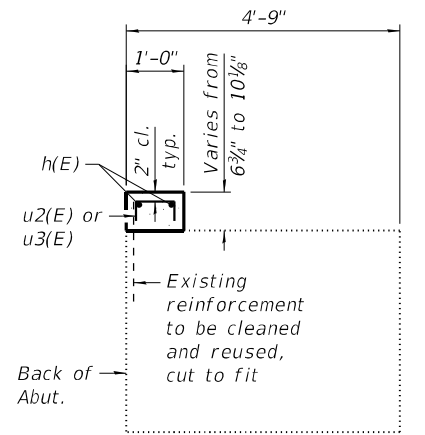
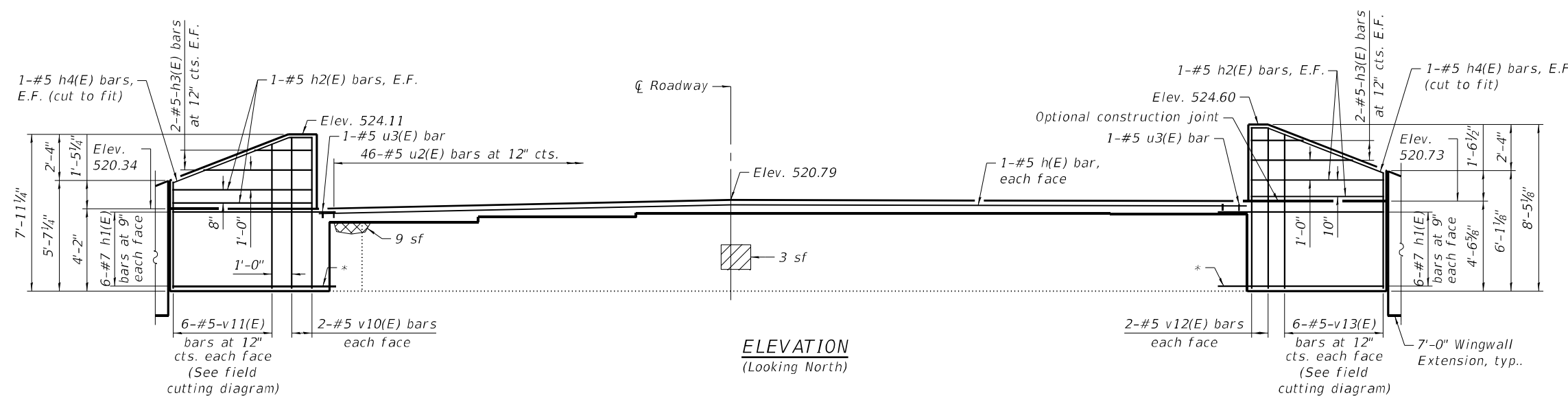
BARS n(E) & t1(E)



EXPANSION JOINT

Notes:
 For Bill of Material and wingwall details, see sheets 31 and 32 of 42.
 Cost of 6" dumbbell type nonmetallic water seal, PJF, concrete nails included with Concrete Structures.
 6" Dumbbell type nonmetallic water seal shall be in accordance with Article 503.12 and Section 1054 of the Standard Specifications.
 B.F. = back face
 F.F. = front face

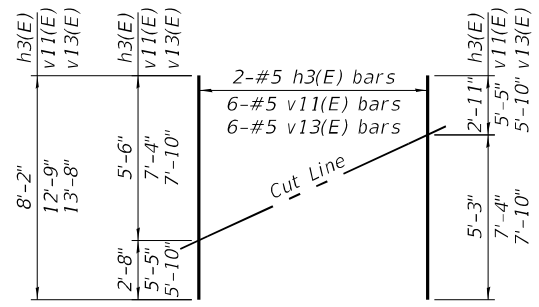
MODEL: Default
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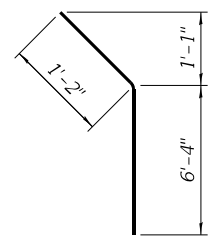
*Drill and grout 9" min. embedment to miss existing reinforcement according to Sec. 584 of the Standard Specifications. Cost included with Reinforcement Bars, Epoxy Coated, typ.

BILL OF MATERIAL

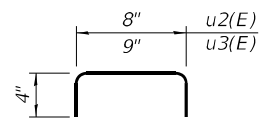
Bar	No.	Size	Length	Shape
h(E)	2	#5	46'-10"	—
h1(E)	24	#7	8'-7"	—
h2(E)	8	#5	7'-1"	—
h3(E)	4	#5	8'-2"	—
h4(E)	4	#5	7'-6"	—
h5(E)	32	#4	6'-8"	—
n1(E)	32	#4	4'-4"	└
t(E)	18	#5	6'-10"	└
t1(E)	18	#5	6'-2"	—
u2(E)	46	#5	1'-4"	└
u3(E)	2	#5	1'-5"	└
v10(E)	4	#5	7'-7"	—
v11(E)	6	#5	12'-9"	—
v12(E)	4	#5	8'-1"	—
v13(E)	6	#5	13'-8"	—
v18(E)	8	#4	7'-11"	—
v19(E)	8	#4	9'-9"	—
w(E)	28	#4	6'-8"	—
Structure Excavation	Cu. Yd.		86	
Concrete Structures	Cu. Yd.		12.3	
Reinforcement Bars, Epoxy Coated	Pound		1,640	
Structural Repair of Concrete (Depth Greater than 5 Inches)	Sq. Ft.		10	
Structural Repair of Concrete (Depth Equal to or Less than 5 Inches)	Sq. Ft.		3	



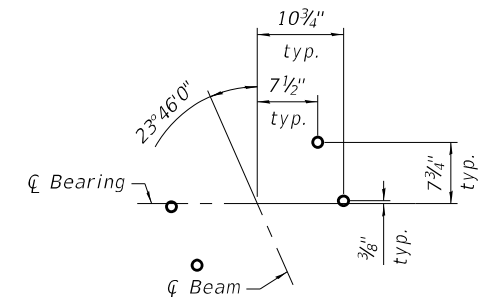
FIELD CUTTING DIAGRAM
 Order h3(E), v11(E), and v13(E) full length. Cut as shown and use remainder of bars in opposite face.



BAR h4(E)



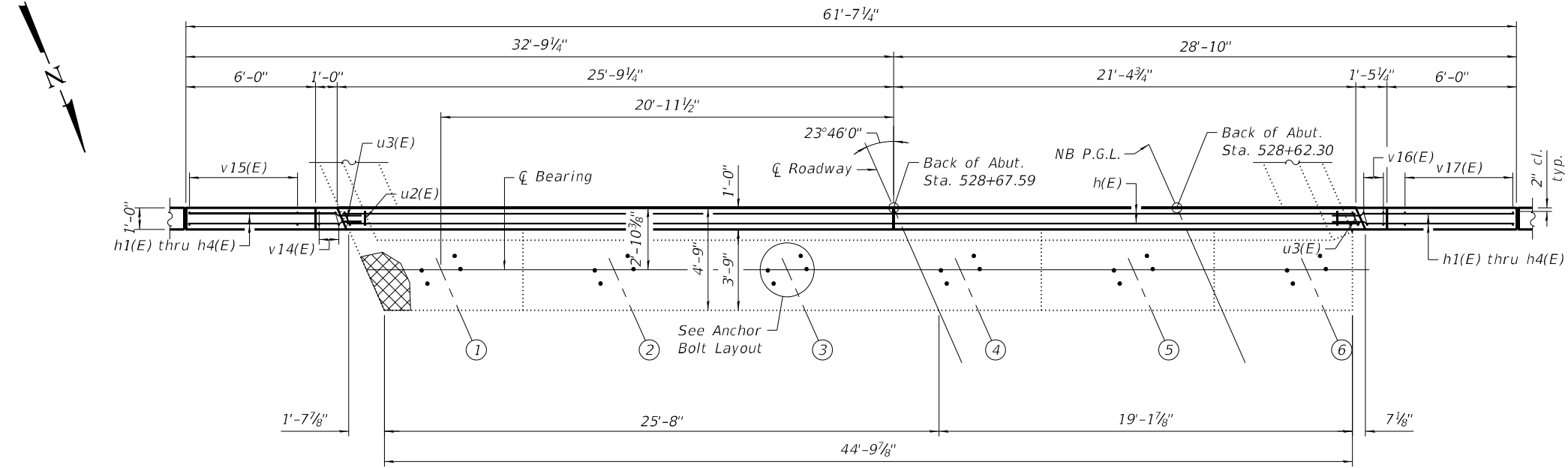
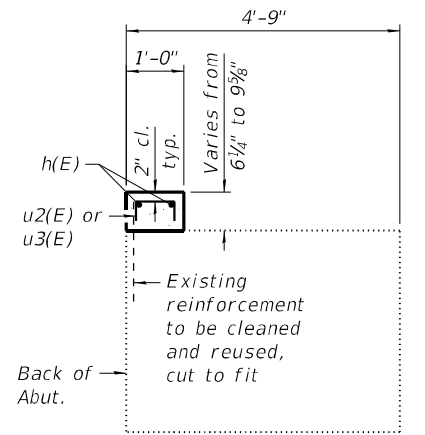
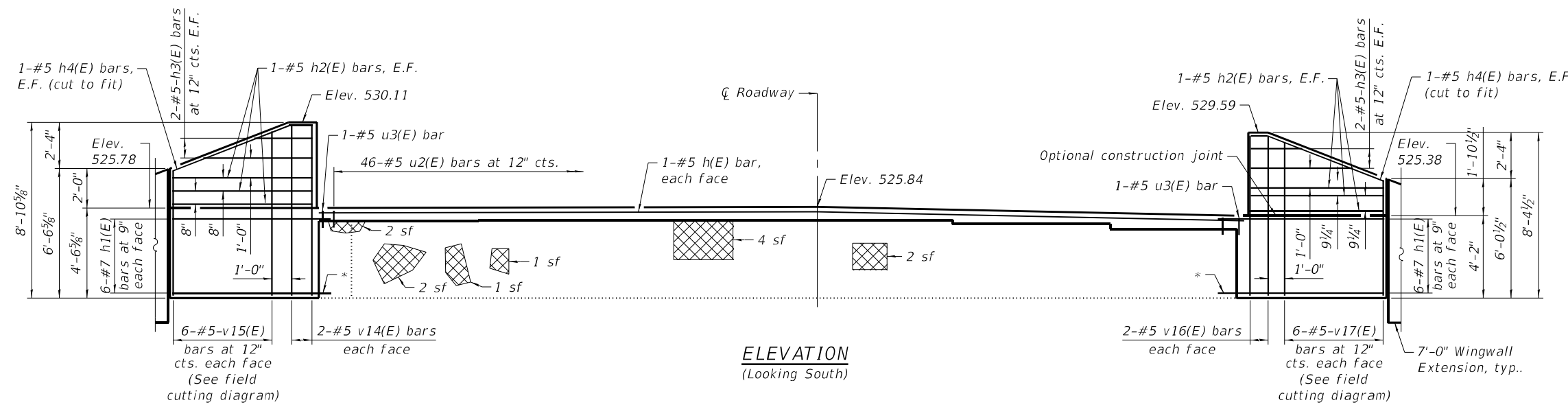
BAR u2(E) & u3(E)



ANCHOR BOLT LAYOUT

Notes:
 E.F. = each face

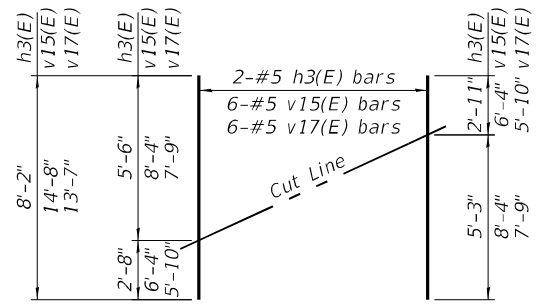
MODEL: Default
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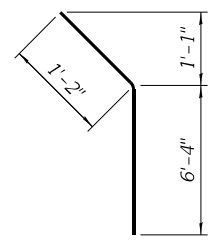
*Drill and grout 9" min. embedment to miss existing reinforcement according to Sec. 584 of the Standard Specifications. Cost included with Reinforcement Bars, Epoxy Coated, typ.

BILL OF MATERIAL

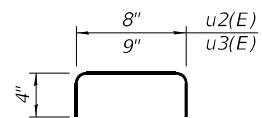
Bar	No.	Size	Length	Shape
h(E)	2	#5	46'-10"	—
h1(E)	24	#7	8'-7"	—
h2(E)	12	#5	7'-1"	—
h3(E)	4	#5	8'-2"	—
h4(E)	4	#5	7'-6"	—
h5(E)	32	#4	6'-8"	—
n1(E)	32	#4	4'-4"	└
t(E)	18	#5	6'-10"	└
t1(E)	18	#5	6'-2"	—
u2(E)	46	#5	1'-4"	┌
u3(E)	2	#5	1'-5"	┌
v14(E)	4	#5	8'-6"	—
v15(E)	6	#5	14'-8"	—
v16(E)	4	#5	8'-0"	—
v17(E)	9	#5	13'-7"	—
v20(E)	8	#4	8'-11"	—
v21(E)	8	#4	8'-9"	—
w(E)	28	#4	6'-8"	—
Structure Excavation	Cu. Yd.		98	
Concrete Structures	Cu. Yd.		12.5	
Reinforcement Bars, Epoxy Coated	Pound		1,730	
Structural Repair of Concrete (Depth Greater than 5 Inches)	Sq. Ft.		12	



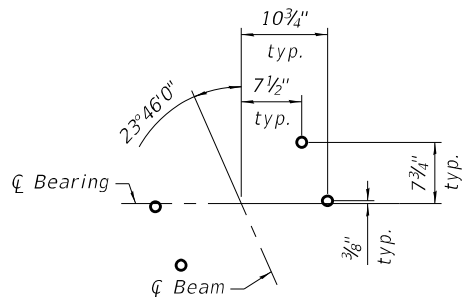
FIELD CUTTING DIAGRAM
 Order h3(E), v15(E), and v17(E) full length. Cut as shown and use remainder of bars in opposite face.



BAR h4(E)



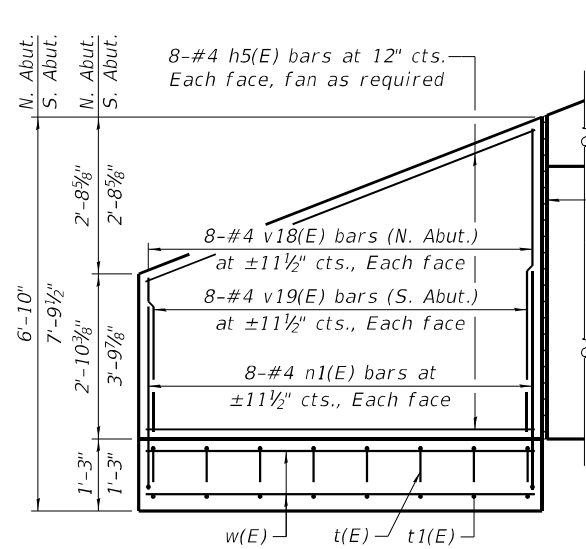
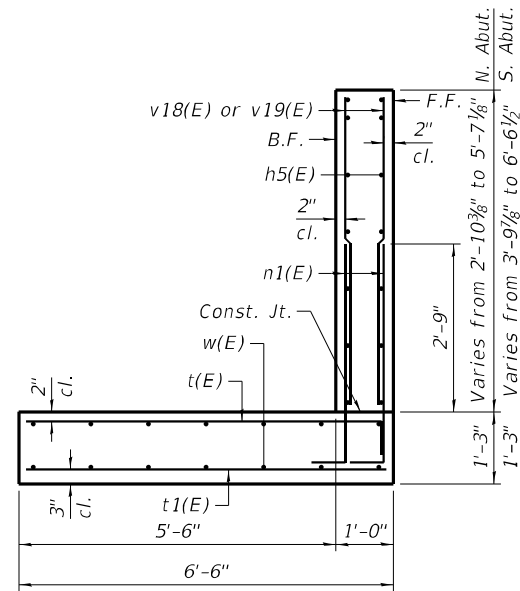
BAR u2(E) & u3(E)



ANCHOR BOLT LAYOUT

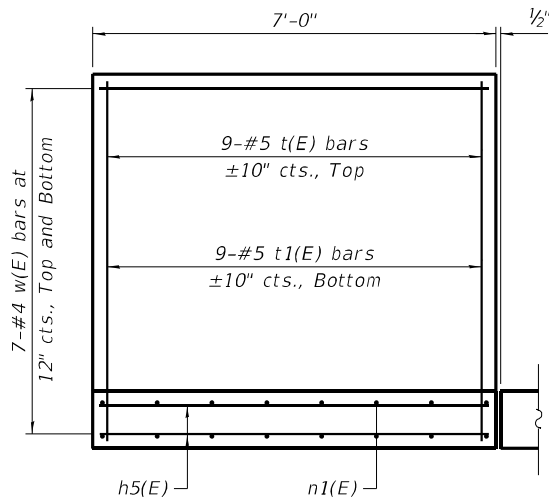
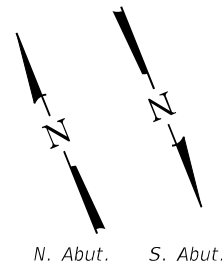
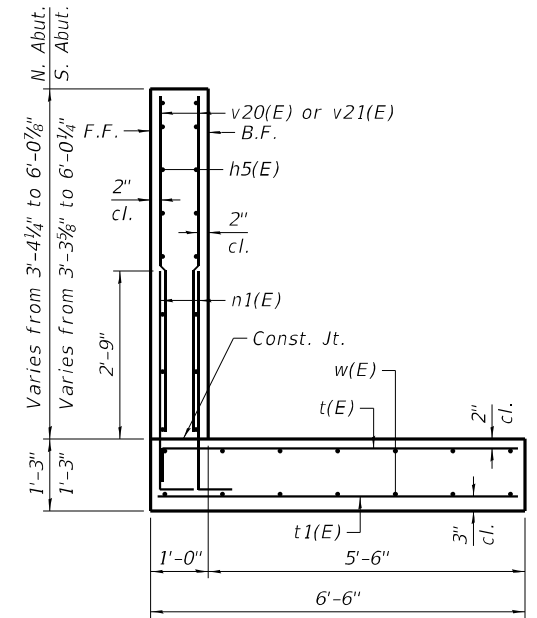
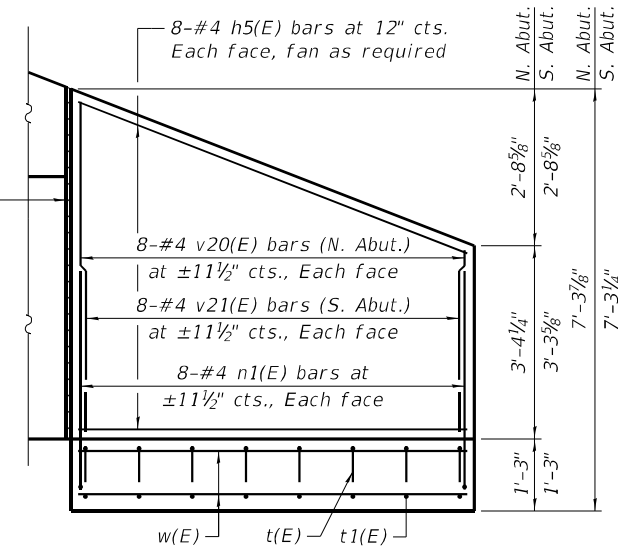
Notes:
 Pour steps monolithically with cap.
 E.F. = each face

MODEL: Default
 FILE NAME: \\SERVER18\Projects\5422057.03_IDOT.D3_PTB_204-028_WO_03_1-180_over_Bottom_Road\DWG\Bridges\Final\Plotsheets\006-0054-66K66-036-Wingwall_Extension_Details_(NB).dgn

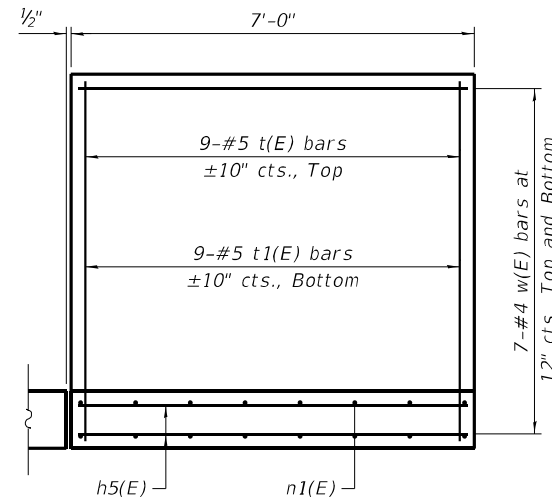


ELEVATION

Maximum applied service bearing pressure, $Q_{max} = 1080$ psf.

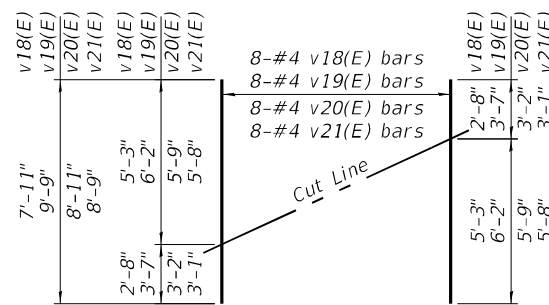


FOOTING PLAN



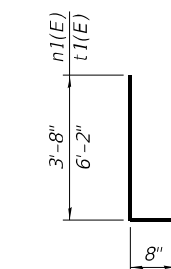
MINIMUM BAR LAP

#4 bar = 2'-7"

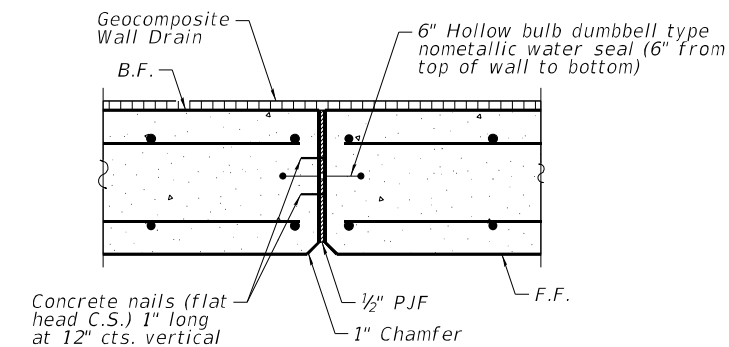


FIELD CUTTING DIAGRAM

Order v18(E) thru v21(E) full length. Cut as shown and use remainder of bars in opposite face.



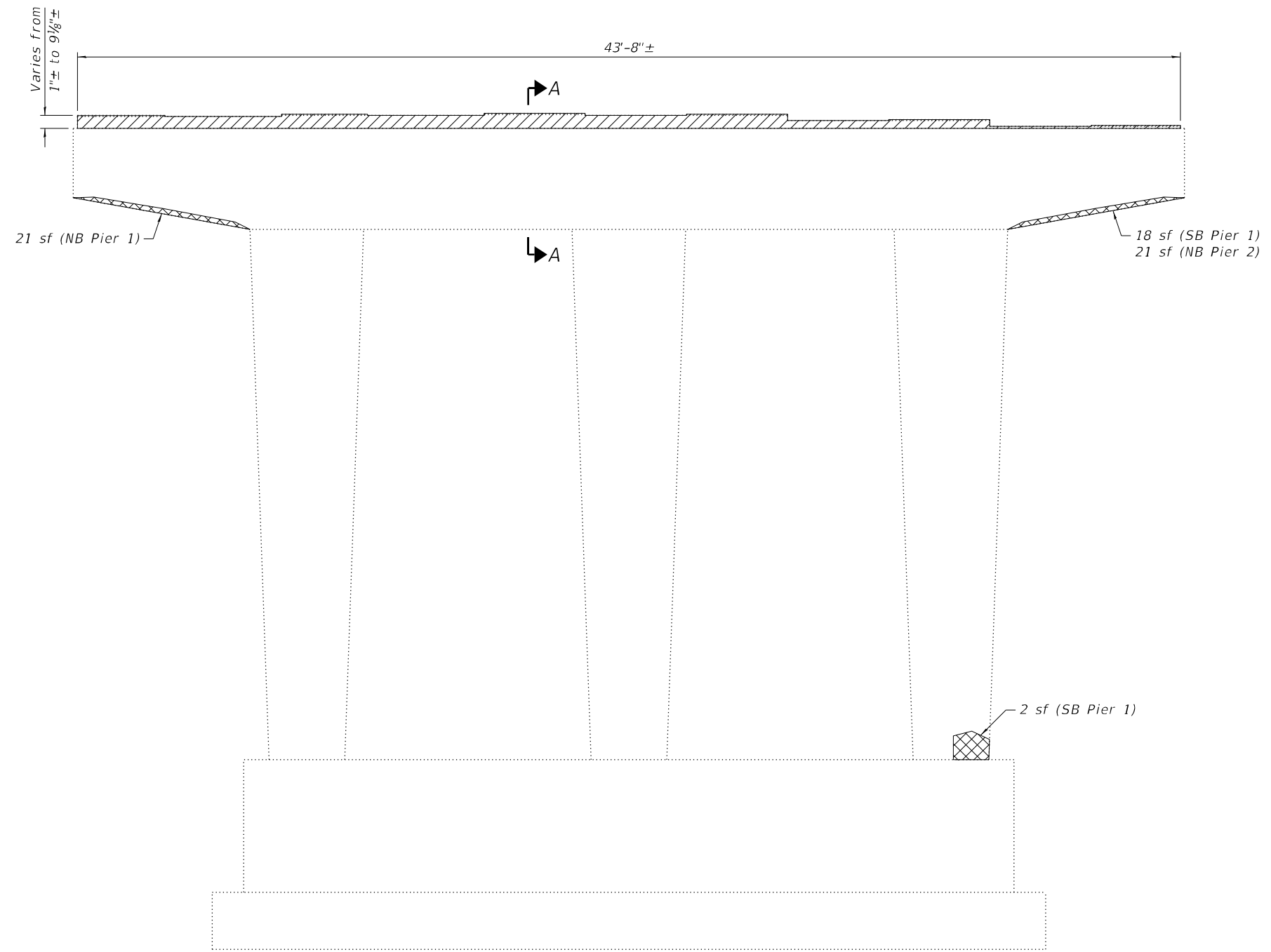
BARS n1(E) & t1(E)



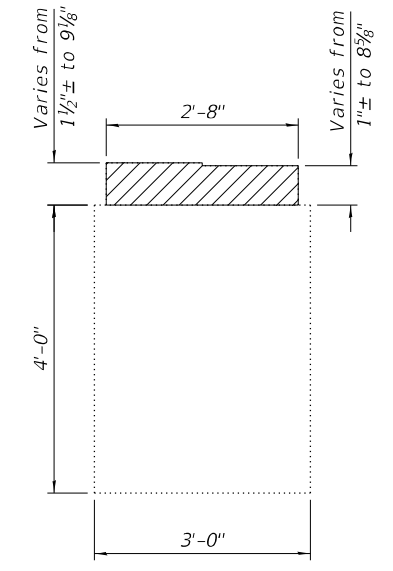
EXPANSION JOINT

Notes:
 For Bill of Material and wingwall details, see sheets 34 and 35 of 42.
 Cost of 6" dumbbell type nonmetallic water seal, PJF, concrete nails included with Concrete Structures.
 6" Dumbbell type nonmetallic water seal shall be in accordance with Article 503.12 and Section 1054 of the Standard Specifications.
 B.F. = back face
 F.F. = front face

MODEL: Default
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PIER ELEVATION
 (Looking North)



SECTION A-A

- Concrete Removal
- Structural Repair of Concrete
 (Depth Equal to or Less Than 5 Inches)

STRUCTURE NO. 006-0053
BILL OF MATERIAL - 2 PIERS

Item	Unit	Total
Concrete Removal	Cu. Yd.	4.4
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	20

STRUCTURE NO. 006-0054
BILL OF MATERIAL - 2 PIERS

Item	Unit	Total
Concrete Removal	Cu. Yd.	3.6
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	42



USER NAME = ABenz	DESIGNED - ACB	REVISED -
PLOT SCALE =	CHECKED - CDL	REVISED -
PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
	CHECKED - CDL	REVISED -

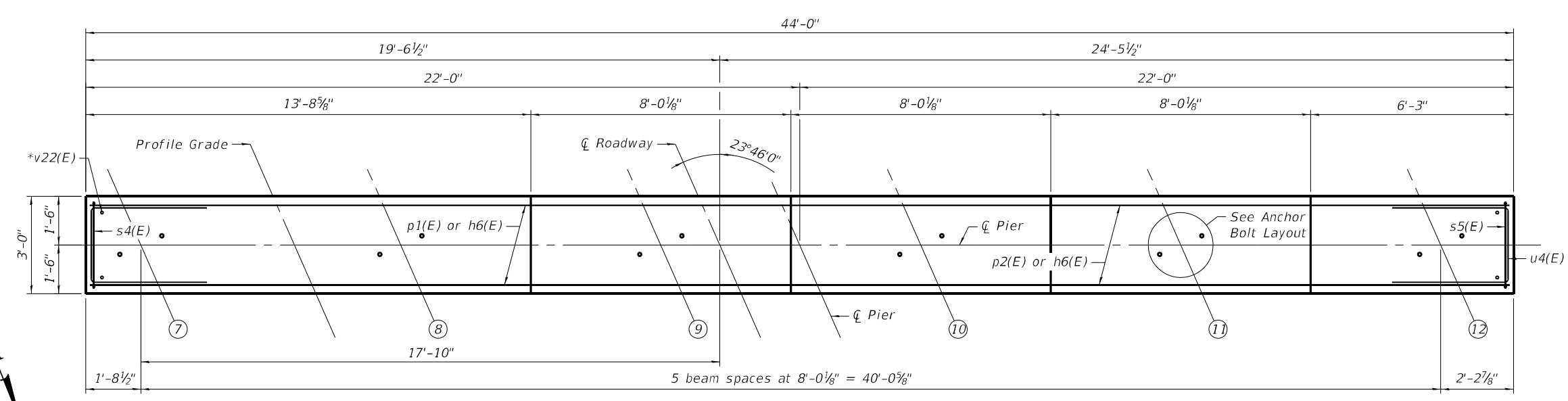
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIER REMOVAL AND REPAIRS
STRUCTURE NO. 006-0053 (SB) & 006-0054 (NB)

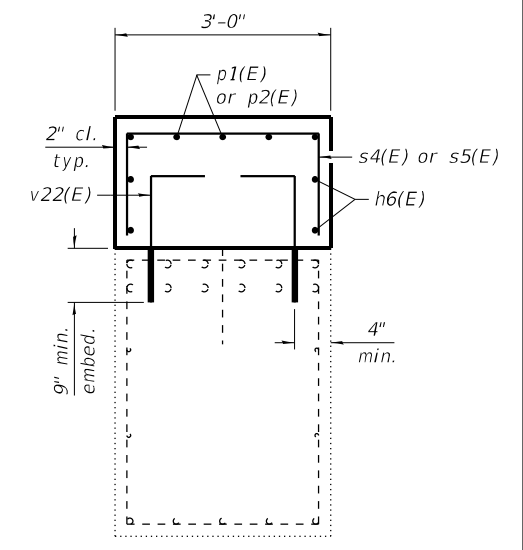
SHEET 37 OF 42 SHEETS

F.A.I. RTE. 180	SECTION (06-2HB-1)ES	COUNTY BUREAU	TOTAL SHEETS 327	SHEET NO. 257
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	

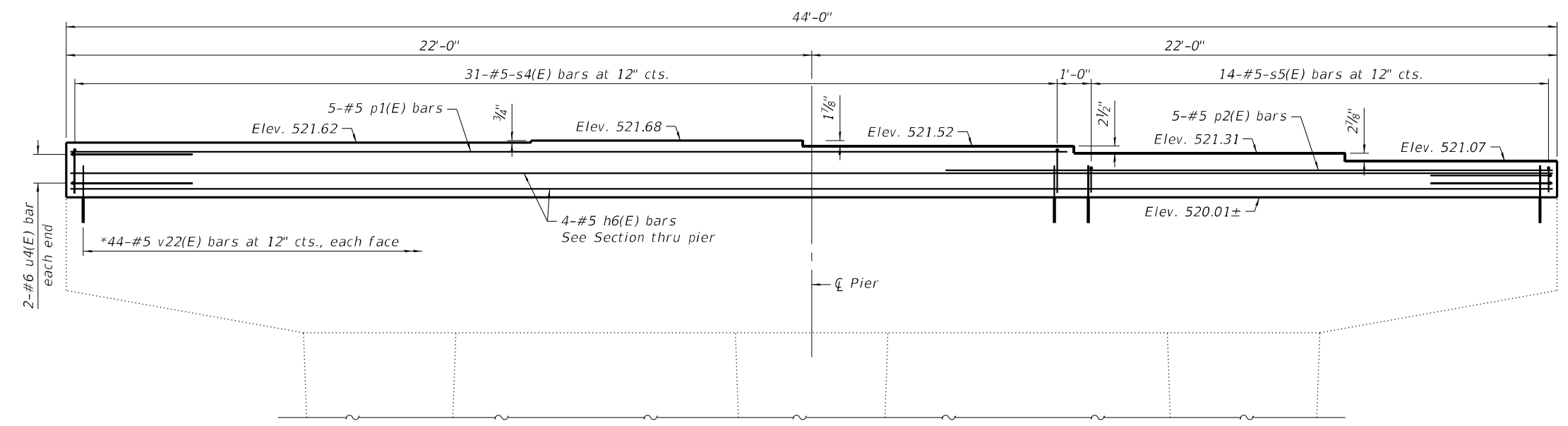
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 2/6/2024 11:20:04 AM



PLAN

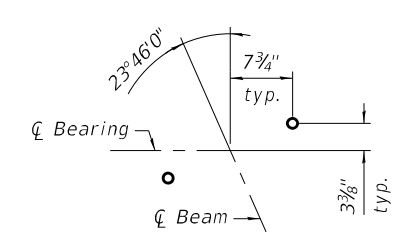


SECTION THRU PIER

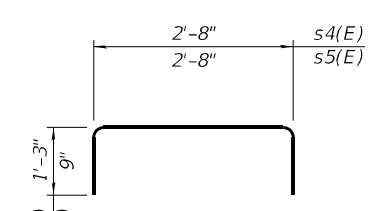


ELEVATION
(Looking South)

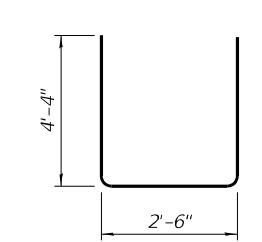
Notes:
 Space reinforcement in cap to miss anchor bolts.
 Pour steps monolithically with cap.
 *Drill and grout 9" min. embedment to miss existing reinforcement according to Sec. 584 of the Standard Specifications. Cost included with Reinforcement Bars, Epoxy Coated, typ.



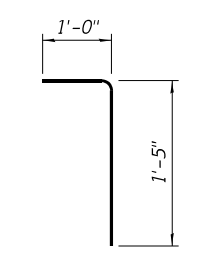
ANCHOR BOLT LAYOUT



BAR s4(E) & s5(E)



BAR u4(E)

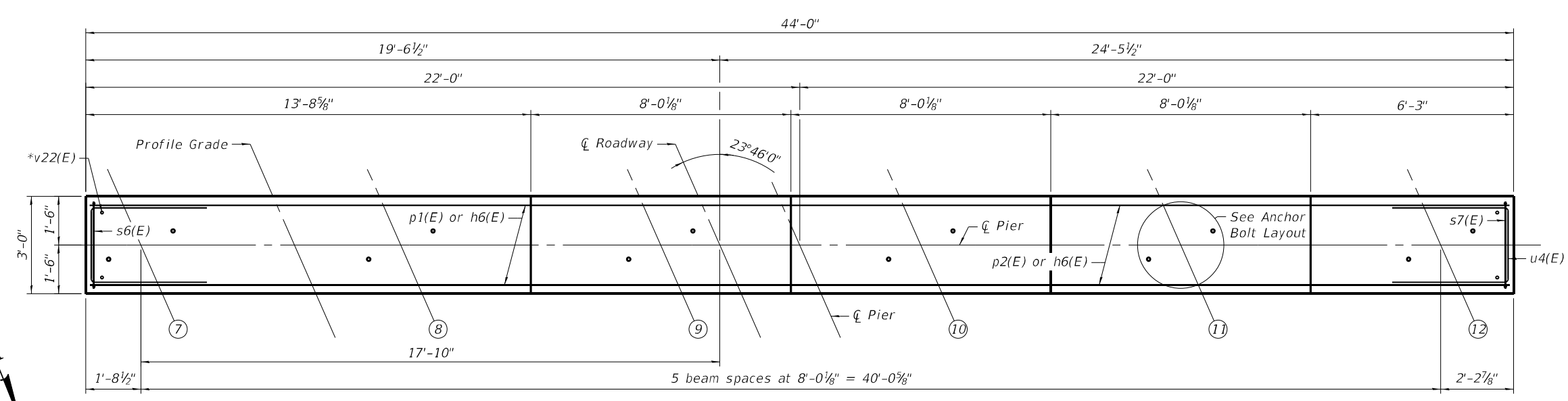


BAR v22(E)

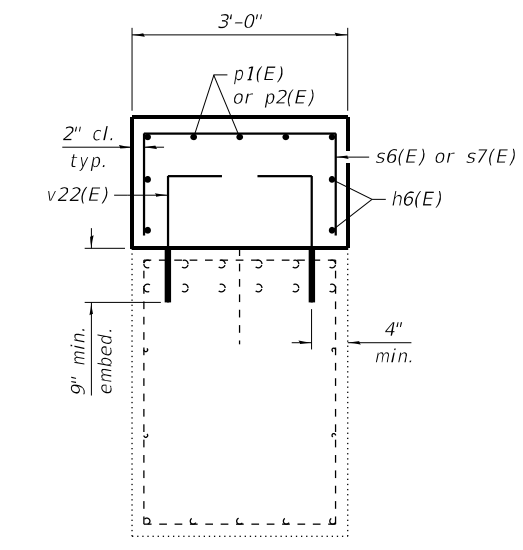
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h6(E)	4	#5	43"-8"	—
p1(E)	5	#5	29'-4"	—
p2(E)	5	#5	17'-11"	—
s4(E)	31	#5	5'-2"	U
s5(E)	14	#5	4'-2"	U
u4(E)	4	#6	11'-2"	U
v22(E)	88	#5	2'-5"	T
Concrete Structures		Cu. Yd.		7.2
Reinforcement Bars, Epoxy Coated		Pound		950

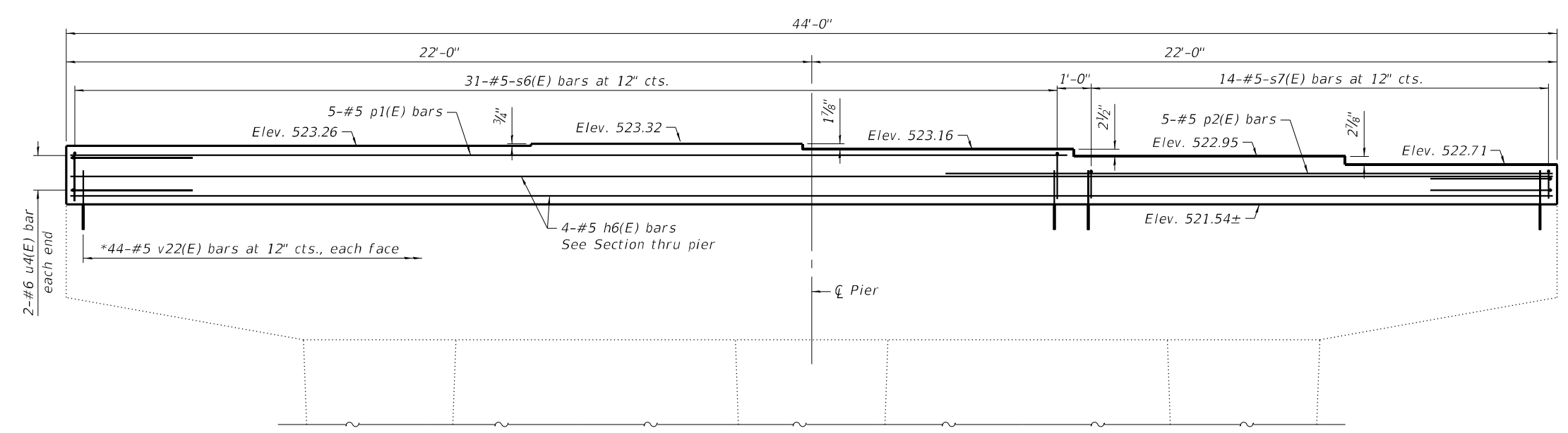
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 2/6/2024 11:20:08 AM



PLAN

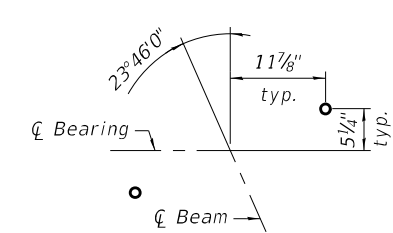


SECTION THRU PIER

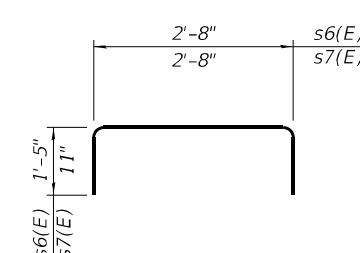


ELEVATION
(Looking South)

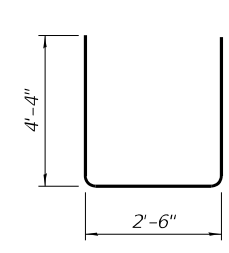
Notes:
 Space reinforcement in cap to miss anchor bolts.
 Pour steps monolithically with cap.
 *Drill and grout 9" min. embedment to miss existing reinforcement according to Sec. 584 of the Standard Specifications. Cost included with Reinforcement Bars, Epoxy Coated, typ.



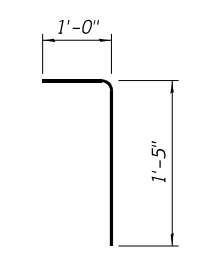
ANCHOR BOLT LAYOUT



BAR s6(E) & s7(E)



BAR u4(E)

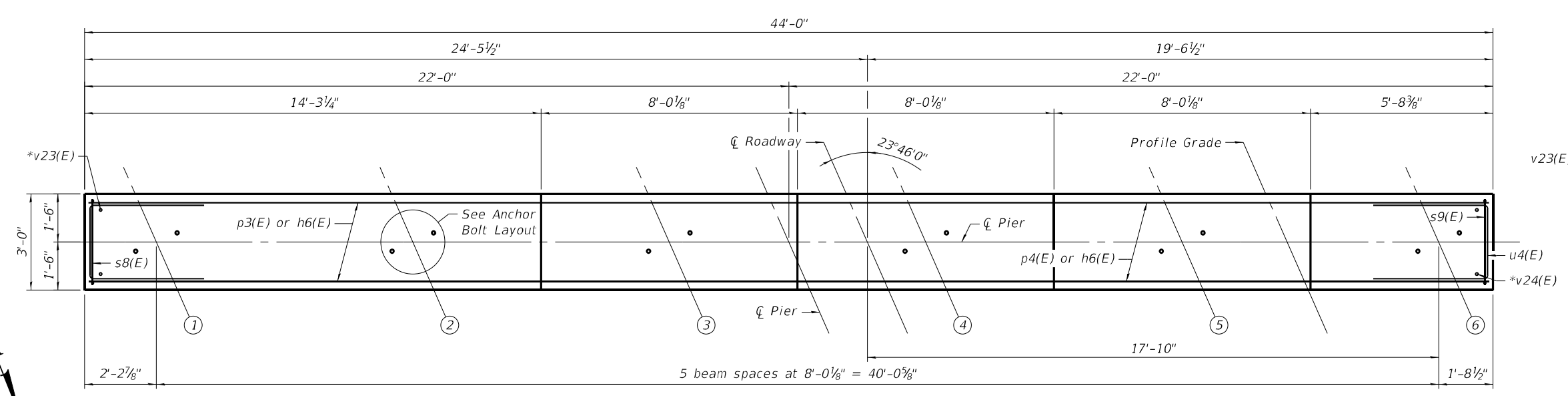


BAR v22(E)

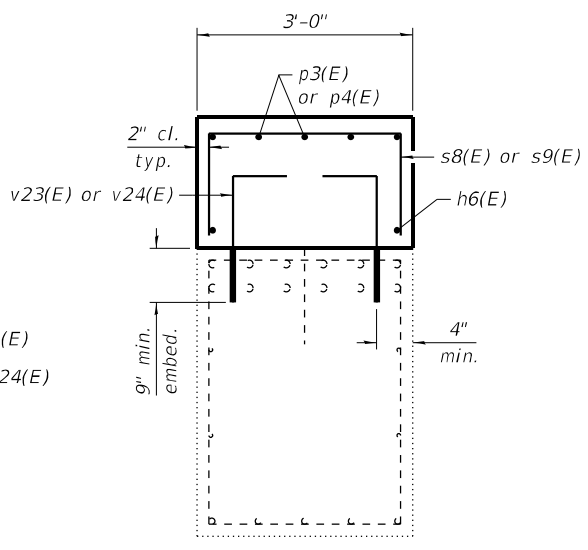
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h6(E)	4	#5	43"-8"	—
p1(E)	5	#5	29'-4"	—
p2(E)	5	#5	17'-11"	—
s6(E)	31	#5	5'-6"	U
s7(E)	14	#5	4'-6"	U
u4(E)	4	#6	11'-2"	U
v22(E)	88	#5	2'-5"	T
Concrete Structures		Cu. Yd.		7.7
Reinforcement Bars, Epoxy Coated		Pound		960

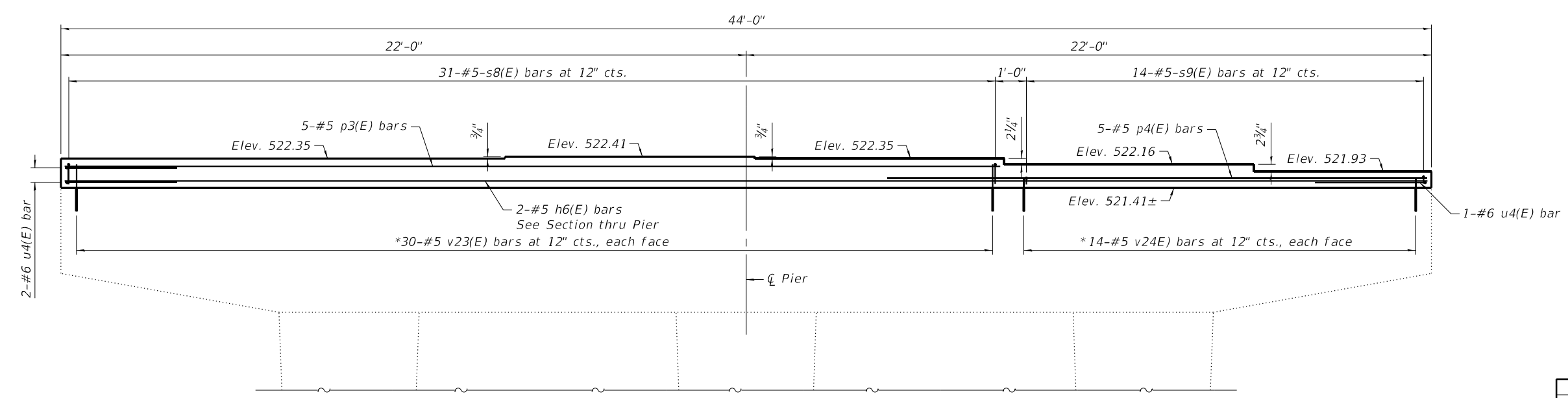
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 2/6/2024 11:20:11 AM



PLAN

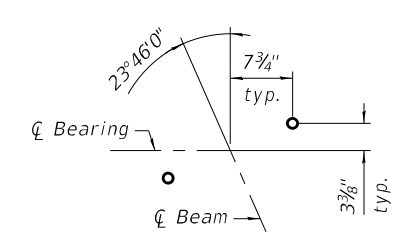


SECTION THRU PIER

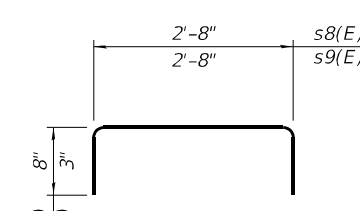


ELEVATION
(Looking South)

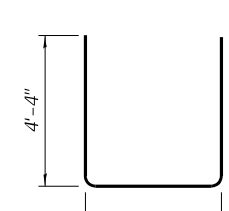
Notes:
 Space reinforcement in cap to miss anchor bolts.
 Pour steps monolithically with cap.
 *Drill and grout 9" min. embedment to miss existing reinforcement according to Sec. 584 of the Standard Specifications. Cost included with Reinforcement Bars, Epoxy Coated, typ.



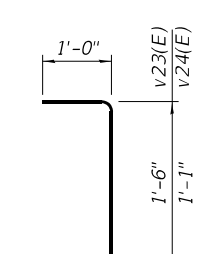
ANCHOR BOLT LAYOUT



BAR s8(E) & s9(E)



BAR u4(E)

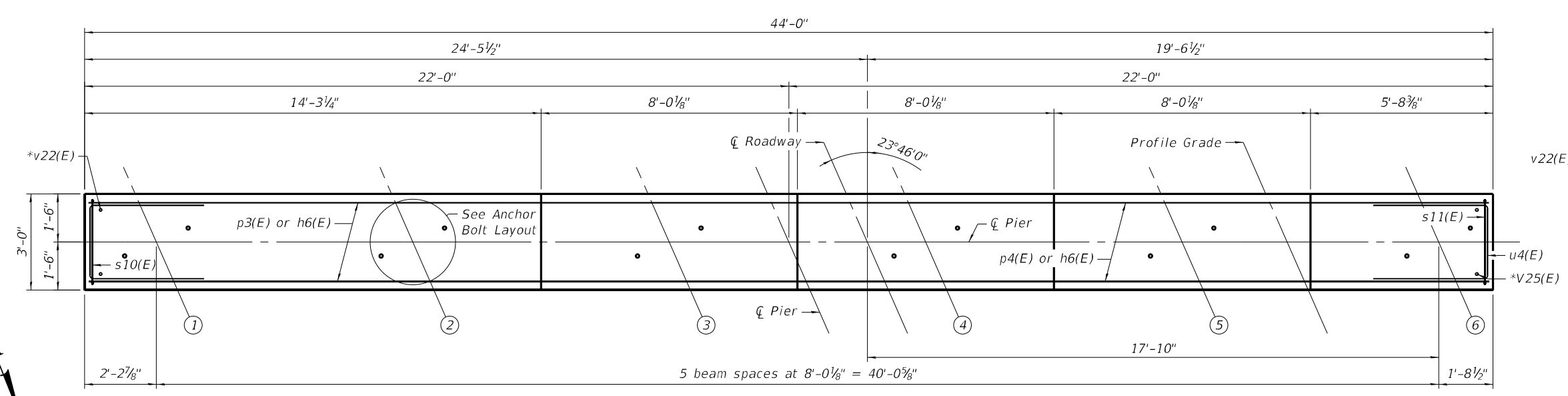


BAR v23(E) & v24(E)

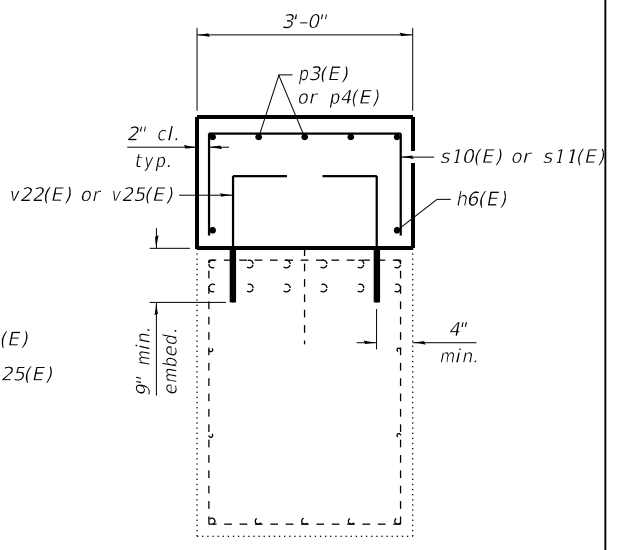
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h6(E)	2	#5	43"-8"	—
p3(E)	5	#5	29'-11"	—
p4(E)	5	#5	17'-4"	—
s8(E)	31	#5	4'-0"	U
s9(E)	14	#5	3'-2"	U
u4(E)	4	#6	11'-2"	U
v23(E)	60	#5	2'-6"	T
v24(E)	28	#5	2'-1"	T
Concrete Structures		Cu. Yd.	4.2	
Reinforcement Bars, Epoxy Coated		Pound	800	

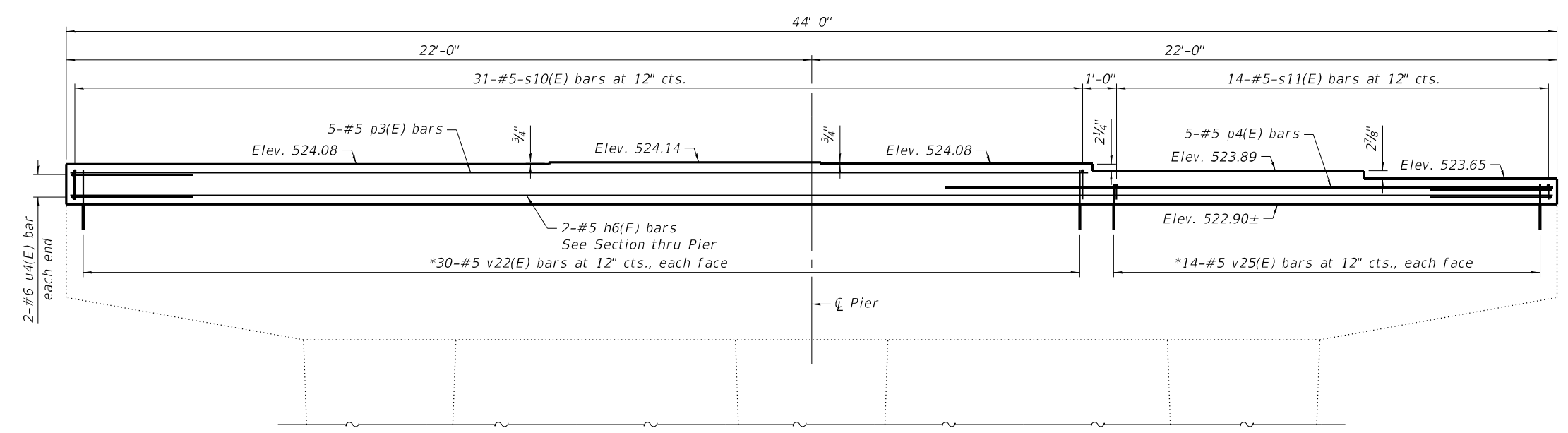
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 2/6/2024 11:20:14 AM



PLAN

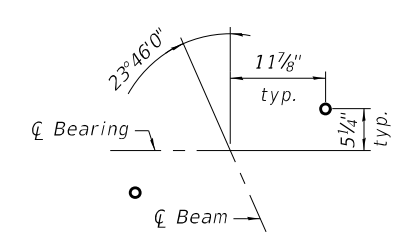


SECTION THRU PIER

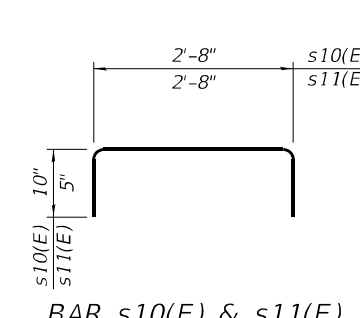


ELEVATION
(Looking South)

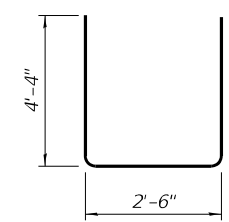
Notes:
 Space reinforcement in cap to miss anchor bolts.
 Pour steps monolithically with cap.
 *Drill and grout 9" min. embedment to miss existing reinforcement according to Sec. 584 of the Standard Specifications. Cost included with Reinforcement Bars, Epoxy Coated, typ.



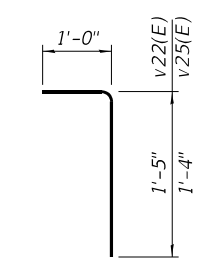
ANCHOR BOLT LAYOUT



BAR s10(E) & s11(E)



BAR u4(E)

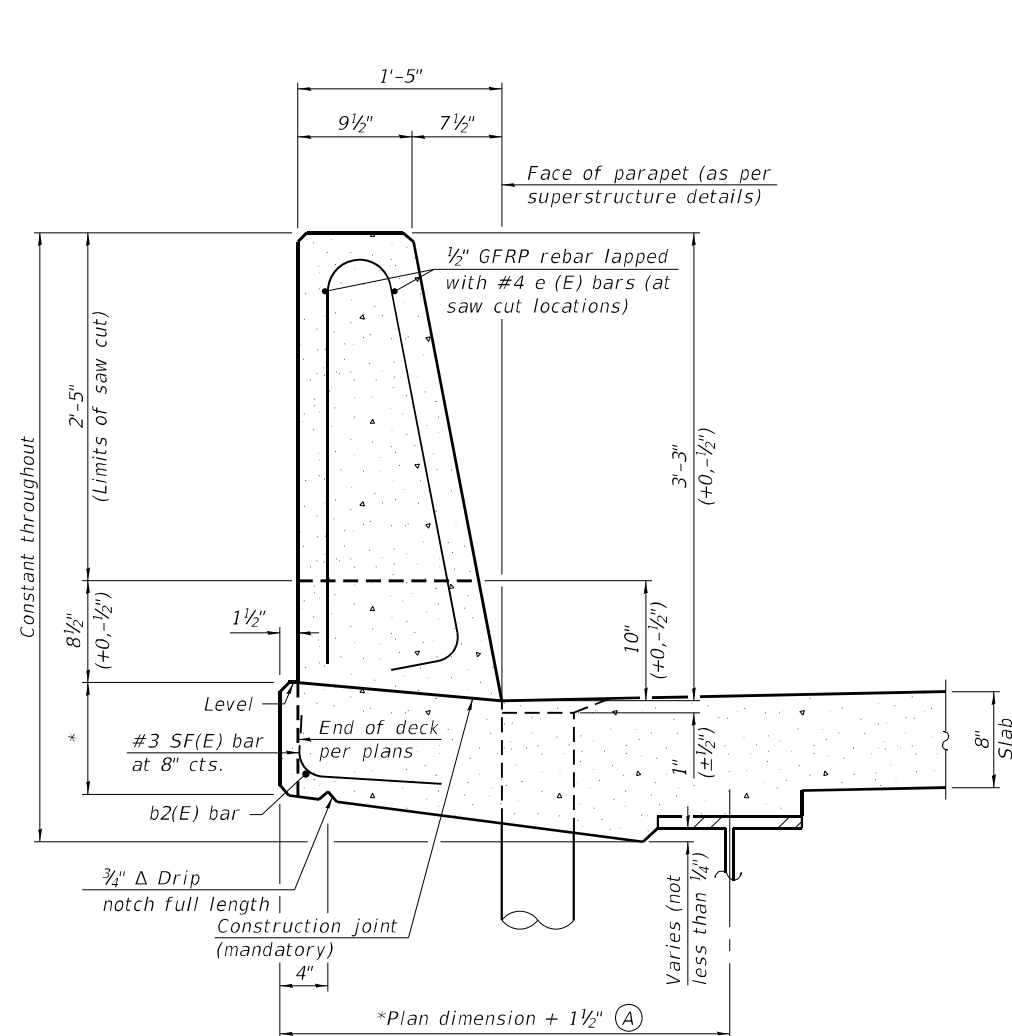


BAR v22(E) & v25(E)

BILL OF MATERIAL

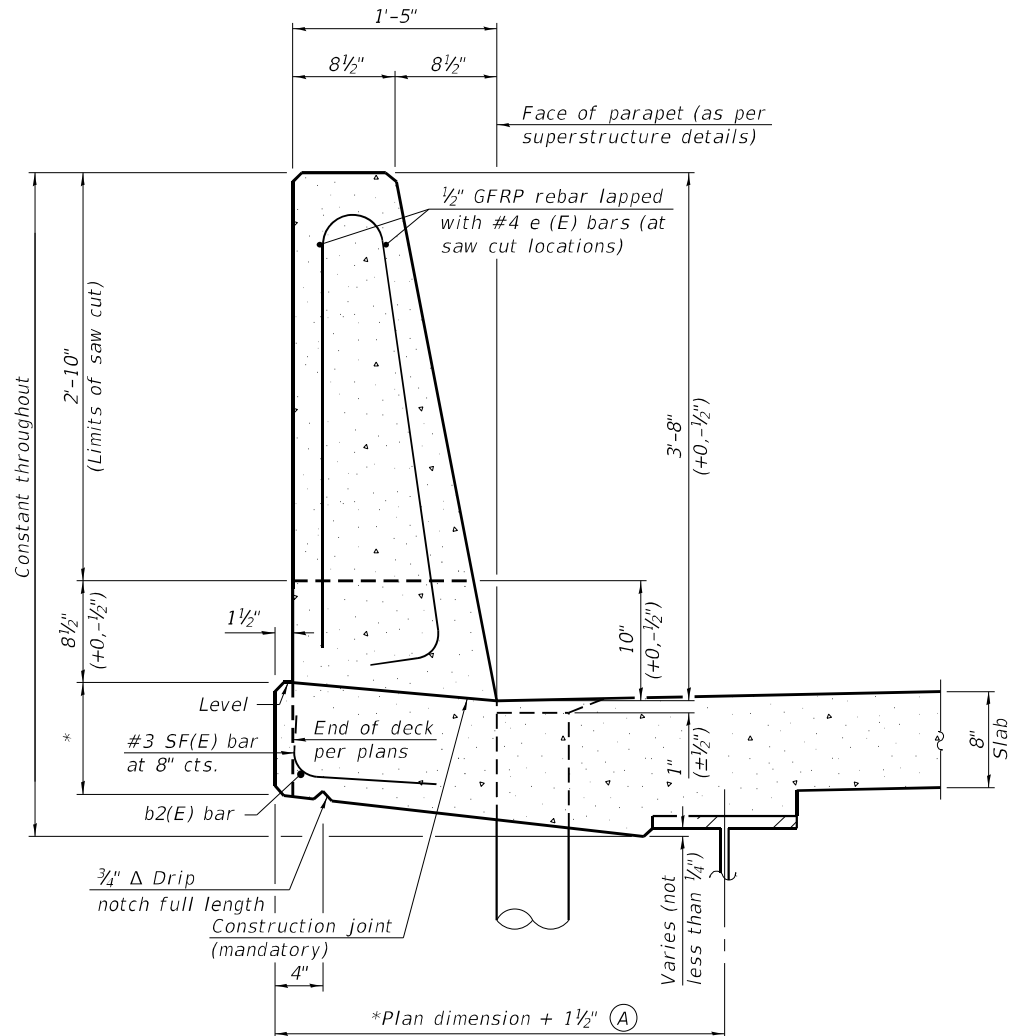
Bar	No.	Size	Length	Shape
h6(E)	4	#5	43"-8"	—
p3(E)	5	#5	29'-11"	—
p4(E)	5	#5	17'-4"	—
s10(E)	31	#5	4'-4"	U
s11(E)	14	#5	3'-6"	U
u4(E)	8	#6	11'-2"	U
v22(E)	60	#5	2'-5"	T
v25(E)	28	#5	2'-4"	T
Concrete Structures		Cu. Yd.	5.4	
Reinforcement Bars, Epoxy Coated		Pound	810	

MODEL: Default
 FILE NAME: \\SERVER18\Projects\54\22057.03 IDOT D3 PTB 204-028 WO 03 1-180 over Bottom Road\DGM\Bridges\Final\Plots\Sheets\006-0053&0054-66K66-042-5\ipforming.dgn



**39" CONSTANT-SLOPE
 PARAPET SECTION**

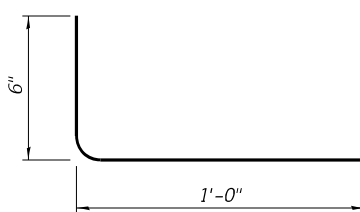
(Showing dimensions, d(E), and 1/2" Ø GFRP rebar)



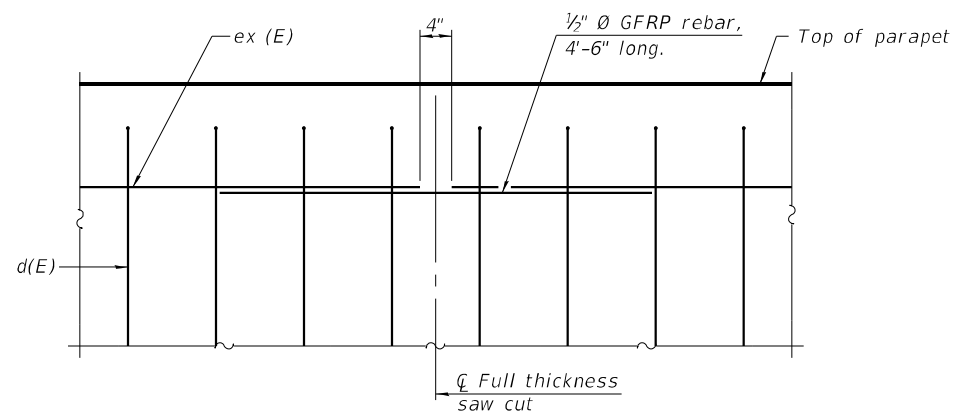
**44" CONSTANT-SLOPE
 PARAPET SECTION**

(Showing dimensions, d(E), and 1/2" Ø GFRP rebar)

*See Superstructure Details.



SF(E) BAR



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)

Notes:
 All dimensions shall remain the same as shown on superstructure details, except dimension A which is to be revised as shown. Additional concrete needed to revise dimension A = 0.00348 cu. yds./ft. for 39" and 44" parapets.
 Place full depth aluminum sheets as shown on superstructure details.
 Replace all cork joint filler locations with a full thickness saw cut.
 Steel superstructure shown. Other superstructure types similar.

SFP 39-44

11-1-2022

EFK Moen
 Civil Engineering Design

USER NAME = ABenz	DESIGNED - ACB	REVISED -
PLOT SCALE =	CHECKED - CDL	REVISED -
PLOT DATE = 1/11/2024	DRAWN - ACB	REVISED -
	CHECKED - CDL	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**CONCRETE PARAPET SLIPFORMING OPTION
 STRUCTURE NO. 006-0053 (SB) & 006-0054 (NB)**

SHEET 42 OF 42 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	(06-2HB-1)ES	BUREAU	327	262
CONTRACT NO. 66K66				

ILLINOIS FED. AID PROJECT

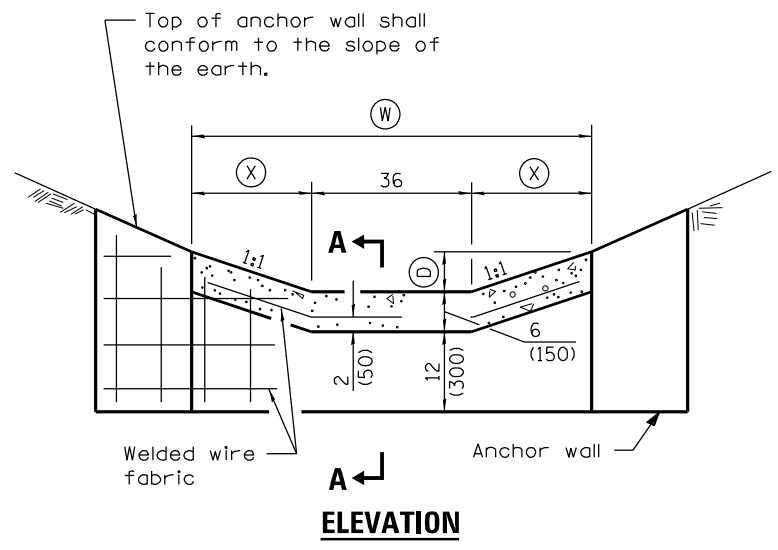
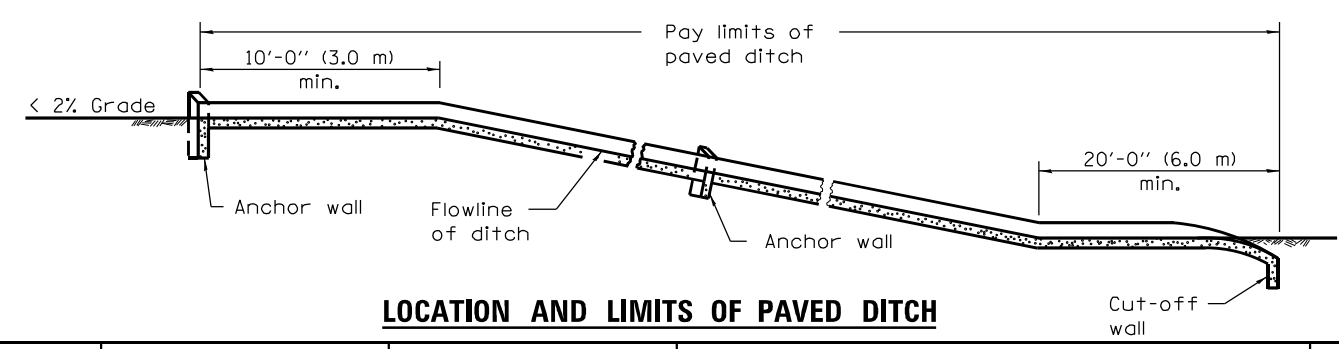
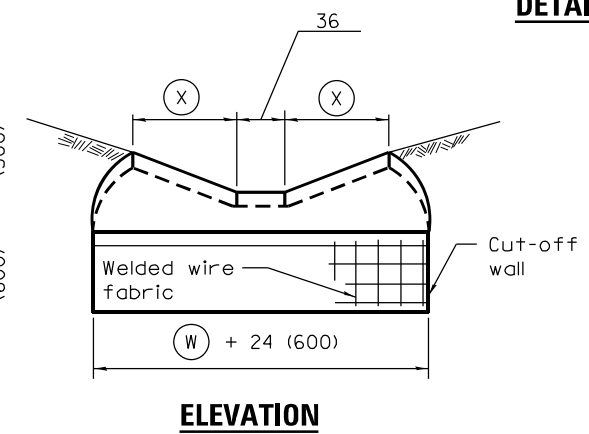
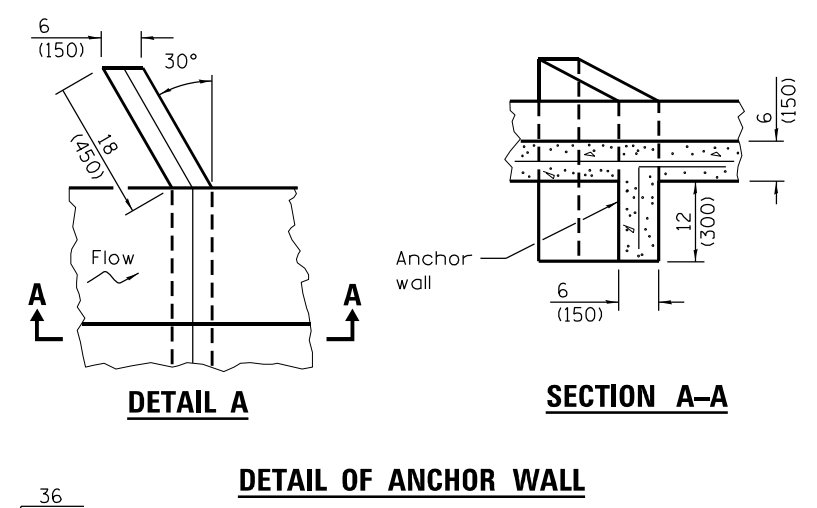
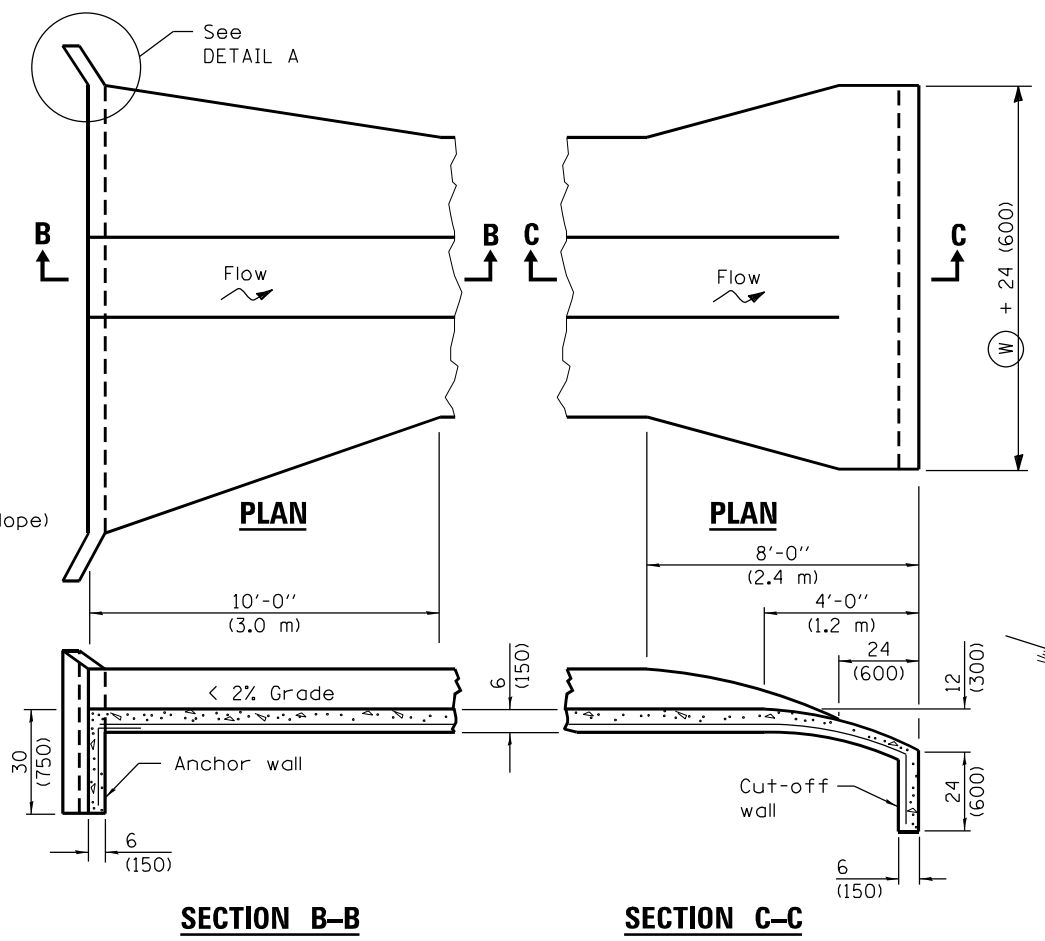
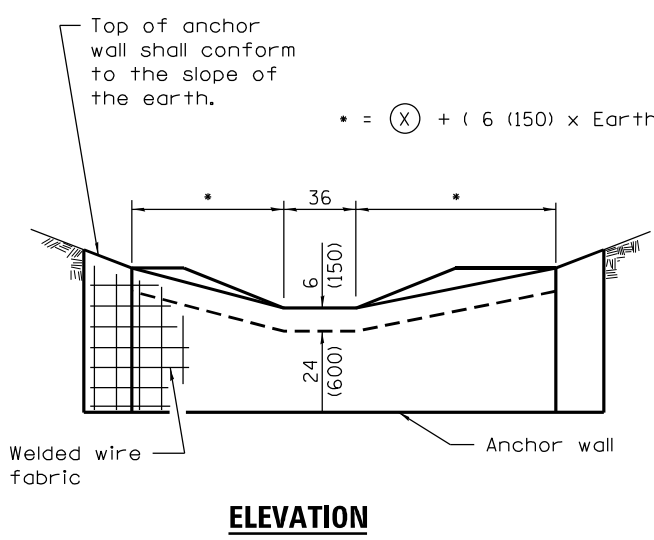
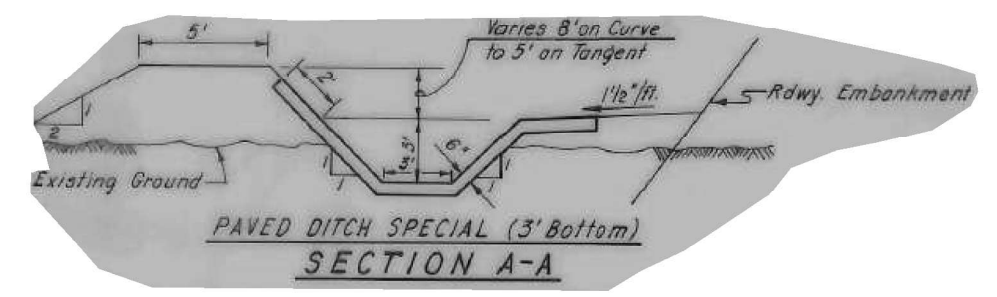


TABLE FOR PAVED DITCH SPECIAL

(D)	(W)	(X)	Flow Area sq. ft.	Conc. Area sq. yd.
36	9'-0"	36	18.00	0.500



GENERAL NOTES

All slopes are expressed as of vertical displacement to units of horizontal displacement (V:H).

All dimensions are in inches (millimeters) unless otherwise shown.

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EFK Moen
Civil Engineering Design

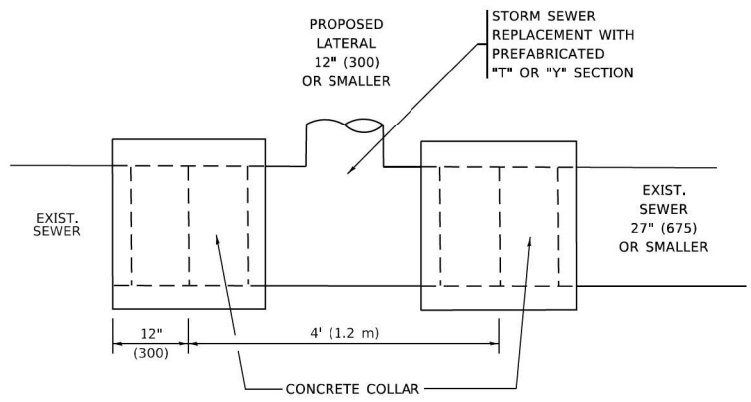
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PLOT DATE = 1/12/2024	CHECKED -	REVISED -
	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 180 (I-180)
DETAIL - PAVED DITCH SPECIAL

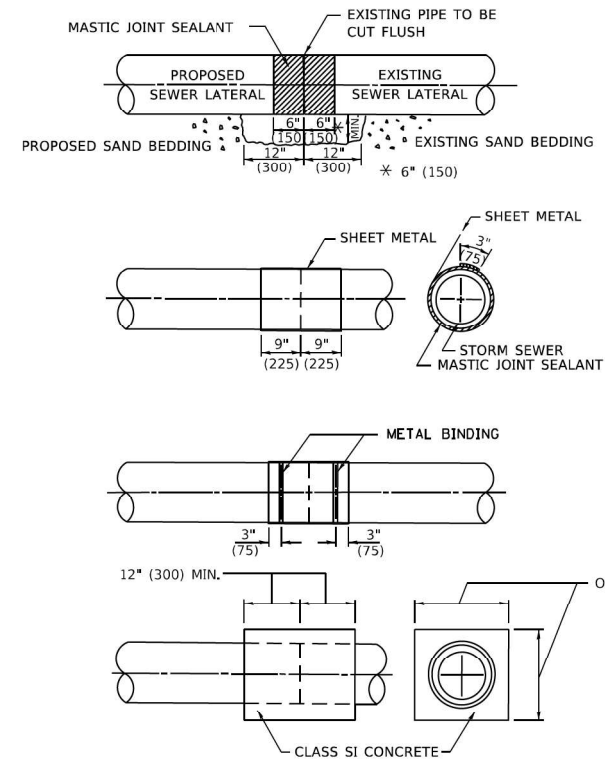
SCALE: SHEET OF SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	263
			CONTRACT NO. 66K66	
			ILLINOIS FED. AID PROJECT	



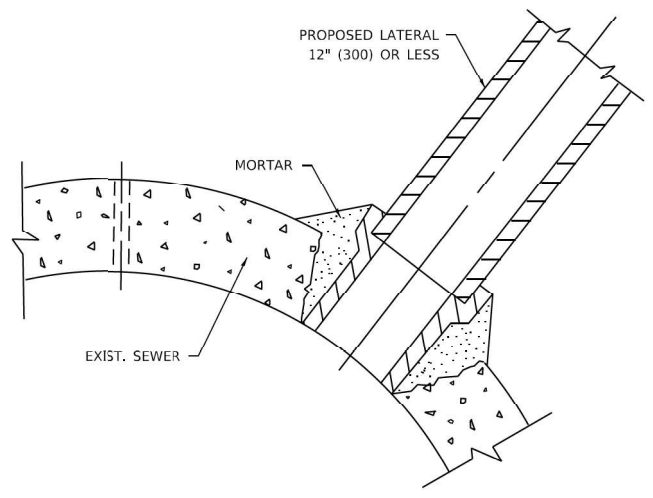
DETAIL "A"

LATERAL CONNECTION TO EXISTING SEWER OF 27" (675) OR SMALLER



DETAIL "B"

CLASS SI CONCRETE COLLAR



DETAIL "C"

PROPOSED LATERAL CONNECTION TO EXISTING SEWER OF 30" (750) OR LARGER

CONSTRUCTION SEQUENCE

1. CUT THE EXISTING END OF THE PIPE SO AS TO PRESENT A FLUSH BUTT JOINT. BRUSH AND CLEAN ALL PIPES.
2. APPLY THE MASTIC JOINT SEALANT TO THE FIRST 6" (150) OF EACH PIPE.
3. BUTT THE PIPES TOGETHER LEAVING A MINIMUM OF 12' x 6' (300 x 150) DEEP EXCAVATION UNDER AND AROUND EACH PIPE END.
4. CUT A PIECE OF SHEET METAL GAGE NO. 19 1.1 (0.0418) 18" (450) WIDE BY THE OUTSIDE CIRCUMFERANCE OF THE PIPE PLUS 3" (75) LONG.
5. WRAP THE SHEET METAL AROUND THE PIPES, 9" (225) ON EACH SIDE OF THE JOINT, STARTING AT THE TOP OF THE PIPE.
6. LAP THE SHEET METAL AT LEAST 3" (75) AT THE TOP OF THE PIPE AND PLACE THE MASTIC JOINT SEALANT BETWEEN THE LAP.
7. PLACE TWO METAL BANDS AROUND THE SHEET METAL AND TIGHTEN.
8. WIPE OFF ANY EXCESS MASTIC JOINT SEALANT THAT OOZES OUT FROM BETWEEN THE SHEET METAL AND THE PIPES.
9. PLACE CLASS SI CONCRETE AROUND THE JOINT.

NOTES:

MATERIAL

MATERIAL USED FOR THE TEE OR WYE SECTION SHALL BE COMPATIBLE WITH THE EXISTING STORM SEWER OR THE PROPOSED STORM SEWER.

CONSTRUCTION METHODS

- THIS WORK SHALL BE CONSTRUCTED IN CONFORMANCE WITH THE APPLICABLE PORTIONS OF SECTION 550 OF THE STANDARD SPECIFICATIONS.
- CONNECTION TO AN EXISTING STORM SEWER SHALL BE BY EITHER OF THE FOLLOWING METHODS:
 - PROPOSED STORM SEWER CONNECTION TO EXISTING SEWER OF 27" (675) OR SMALLER SEE DETAIL "A" AND "B".
 - PROPOSED STORM SEWER CONNECTION TO EXISTING SEWER OF 30" (750) OR LARGER SEE DETAIL "C".

IF THE EXISTING SEWER PIPE IS CRACKED, BROKEN OR OTHERWISE DAMAGED BY THE CONTRACTOR IN MAKING THE CIRCULAR OPENING, THE CONTRACTOR SHALL REPLACE THAT SECTION OF PIPE WITH PIPE EQUAL AND SIMILAR IN ALL RESPECTS TO THE PIPE IN THE EXISTING SEWER, IN A CAREFUL WORKMANLIKE MANNER, WITHOUT EXTRA COMPENSATION.

GENERAL

- CARE MUST BE TAKEN TO PREVENT DEBRIS FROM ENTERING THE SEWER. ALL DEBRIS WHICH ENTERS THE SEWER MUST BE REMOVED. THE SEWER MUST BE LEFT CLEAN AND UNOBSTRUCTED UPON COMPLETION OF THE CONTRACT.
- CARE MUST BE TAKEN TO PREVENT ANY PART OF THE NEW PIPE CONNECTION FROM PROJECTING INTO THE EXISTING SEWER.

BASIS OF PAYMENT

- TEE OR WYE CONNECTIONS SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE EACH FOR STORM SEWER TEE OR WYE SECTION, FOR THE PURPOSE OF FACILITATING THE INSTALLATION OF THE TEE OR WYE SECTION, WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE UNIT PRICE BID FOR THE WORK.
- TRENCH BACKFILL, EXCAVATION IN ROCK AND REMOVAL AND REPLACEMENT OF UNSUITABLE MATERIAL BELOW PLAN BEDDING GRADE WILL BE PAID FOR SEPARATELY.
- CONCRETE COLLAR FOR CONNECTING A PROPOSED STORM SEWER TO AN EXISTING STORM SEWER WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF THE PROPOSED STORM SEWER.

* ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

MODEL: D:\efk\1\2023\05\DOT\03\DOT_03_PFB_204-028_W0_06_L180_Resubmit\DOT\Design\Plan\Plan\Sheet\66K66-sh-D3.dwg



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	DRAWN -	REVISED -
PLOT SCALE = 100,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 180 (I-180)
DETAIL - STORM SEWER CONNECTION TO EXISTING SEWER

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	264
				CONTRACT NO. 66K66
ILLINOIS FED. AID PROJECT				

GENERAL

CLASS SI CONCRETE SHALL BE USED THROUGHOUT.

THIS SPECIFICATION COVERS SLOTTED DRAIN USED FOR THE REMOVAL OF WATER AS SHOWN ON THE PLANS.

THE SLOTTED DRAIN SHALL BE CORRUGATED PIPE CULVERT WITH INTEGRAL SLOTTED DRAINS.

BEFORE PLACING THE CONCRETE ADJACENT TO THE PIPE, THE SLOT SHALL BE COVERED BY EITHER THIN, FLAT METAL SHEETING OR BY A BOARD NOTCHED TO FIT OVER THE GRATE BARS. THIS COVERING MUST FIT CLOSELY IN THE SLOT TO PREVENT ENTRY OF CONCRETE INTO THE PIPE. PAVING OVER THE SLOTTED DRAIN WILL THEN BE ONE CONTINUOUS OPERATION OVER THE PROTECTED DRAIN. THE PROTECTION FOR THE DRAIN SLOT SHALL THEN BE REMOVED. THE PIPE SHALL DRAIN INTO THE SIDE OF THE INLET. THE OPENING WHERE THE SLOT IS REMOVED SHALL BE COVERED TO PREVENT CONCRETE FROM ENTERING THE PIPE.

THE CORRUGATED STEEL PIPE USED IN THE SLOTTED DRAIN SHALL MEET THE REQUIREMENTS OF AASHTO M36/ ASTM A7860.

THE CMP SHALL BE ALUMINIZED STEEL TYPE 2.

THE DIAMETER AND GAGE SHALL BE AS SHOWN ON THE PLAN.

STEEL GRATING SHALL MEET THE GALVANIZING REQUIREMENTS OF AASHTO M111.

THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER FOOT FOR SLOTTED DRAIN 24" WITH VARIABLE SLOT, AND SHALL INCLUDE ELBOWS, DRILLING HOLES IN GRATING, SUPPLYING AND PLACING A1 BARS, AND CONCRETE AND GRATING FOR DEPTH SPECIFIED ON PLANS.

USE APPROVED END CAP TO PREVENT CONCRETE ENTRY INTO THE PIPE DURING GUTTER CONSTRUCTION ON THE UPSTREAM END OF THE PIPE.

CONNECTIONS

THE CORRUGATED STEEL PIPE SHALL HAVE A MINIMUM OF TWO REROLLED ANNULAR ENDS.

THE SLOTTED DRAIN BANDS SHALL BE MODIFIED HUGGER BANDS TO SECURE THE PIPE AND PREVENT INFILTRATION OF THE BACKFILL.

WHEN THE SLOTTED DRAIN IS Banded TOGETHER, THE ADJACENT GRATES SHALL HAVE A MAXIMUM 3" GAP.

GRATES

THE GRATES SHALL BE MANUFACTURED FROM ASTM A670, GRADE 36 STEEL. THE SPACERS AND BEARING BARS (SIDES) SHALL BE 3/16" MATERIAL ±0.008".

THE SPACERS SHALL BE ON 6" CENTERS AND WELDED ON BOTH SIDES TO EACH BEARING BAR (SIDES) WITH FOUR (4) 1/4" LONG 3/16" FILLET WELDS ON EACH SIDE OF THE BEARING BAR.

THE PLATE EXTENDER SHALL BE 7 GAGE STEEL MEETING ASTM A761.

THE ENGINEER MAY CALL FOR TENSILE STRENGTH TESTS ON THE GRATE IF THE GRATE IS NOT IN COMPLIANCE WITH THE ABOVE SPACER SPECIFICATIONS. IF TENSILE STRENGTH TESTS ARE CALLED FOR, MINIMUM RESULTS FOR AN IN-PLACE SPACER PULLED PERPENDICULAR TO THE BEARING BAR SHALL BE:
 T = 12,000 POUNDS FOR 2 1/2" GRATE
 T = 15,000 POUNDS FOR 6" GRATE

GALVANIZING

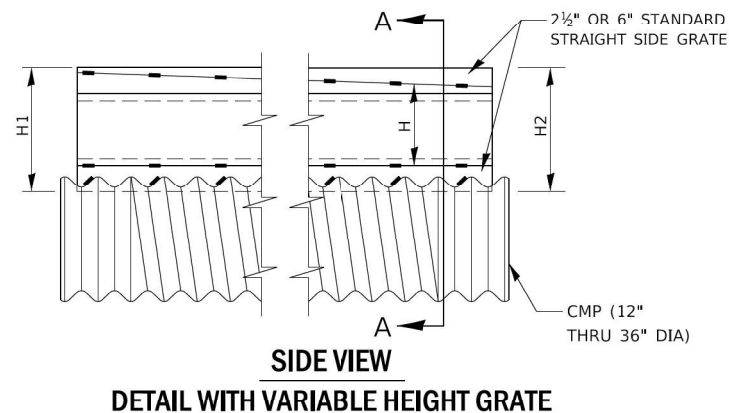
THE GRATE AND PLATE EXTENDERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123 EXCEPT WITH A 2 OZ GALVANIZED COATING.

GRATE ATTACHED TO CSP

THE GRATE SHALL BE FILLET WELDED WITH A MINIMUM WELD 1" LONG TO THE CSP ON EACH SIDE OF THE GRATE AT EVERY OTHER CORRUGATION.

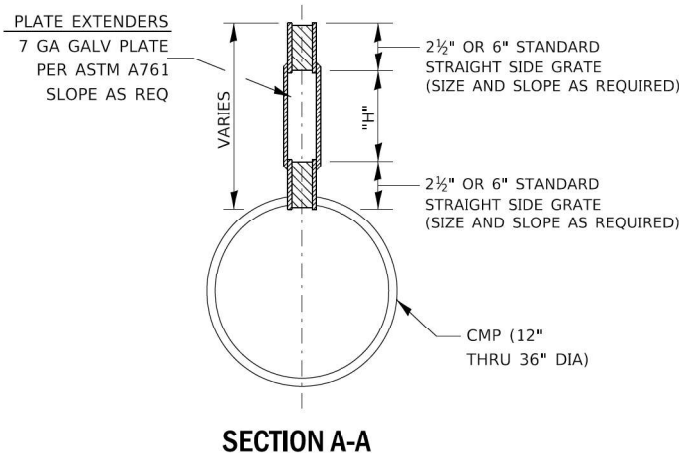
TOLERANCES - FINISHED SLOTTED DRAIN - 20' LENGTH

VERTICAL BOW = ±3/8"
 HORIZONTAL BOW = ±5/8"
 TWIST = ±1/2"



LOADING CONDITION	MAX. EXTENDER HEIGHT - "H"
H20/H25 * 750 PSI CONCRETE	19"

* 125 PSI TIRE PRESSURE



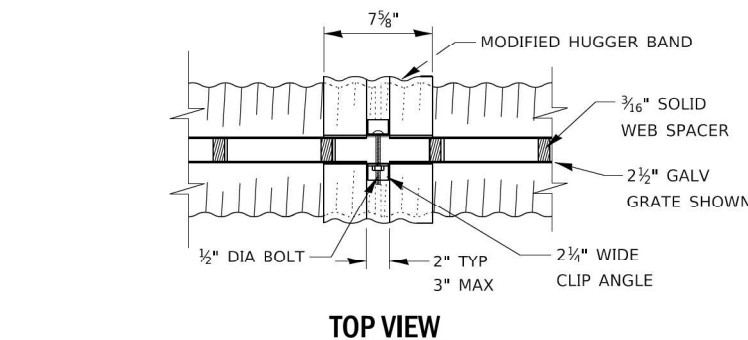
GAGE OF PIPE	DIAMETER OF PIPE					
	12"	15"	18"	24"	30"	36"
16	X	X	X	X	X	X
14	X	X	X	X	X	X
12	NA	NA	NA	NA	X	X

GRATE TYPE	"A"
VERT	2 1/2"
TRAP	2 1/2"
TRAP	3"

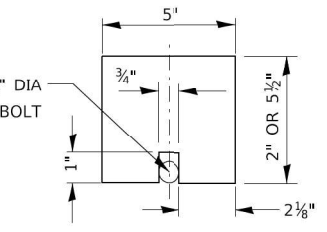
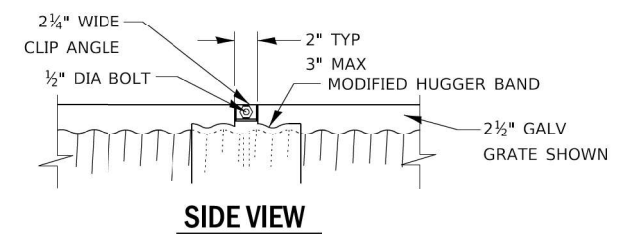
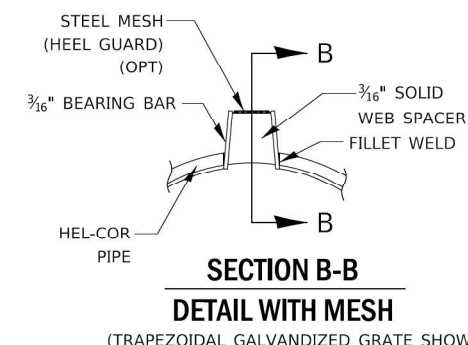
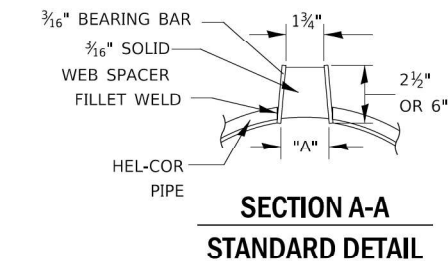
VERT = VERTICAL
 TRAP = TRAPEZOIDAL

SLOTTED DRAIN NOTES

- GRATING IS AVAILABLE IN DEPTHS OF 2 1/2" AND 6".
- VERTICAL GRATING (STRAIGHT SIDES) WITH VERTICAL SPACERS IS ALSO AVAILABLE.
- FOR 6" VERTICAL & TRAPEZOIDAL REQUIREMENTS, THE SLOTTED DRAIN BAND MAY BE FURNISHED WITH THE 4: TECHCO BAND ANGLE.
- DIMENSIONS ARE SUBJECT TO MANUFACTURING TOLERANCES.
- REFERENCE CONTECH BAND MANUAL DWG NO 1002697 FOR BAND DETAILS.
- REFERENCE CONTECH SLOTTED DRAIN DWG NO 1002697.
- DIMENSIONS FOR H1 AND H2 AS REQUIRED.
- H1 AND H2 MEASURED FROM TOP OF GRATE TO BOTTOM OF GRATE.

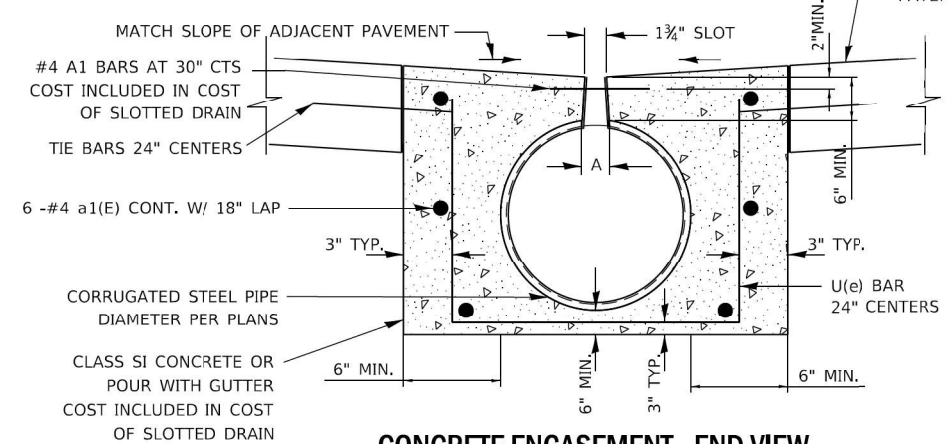
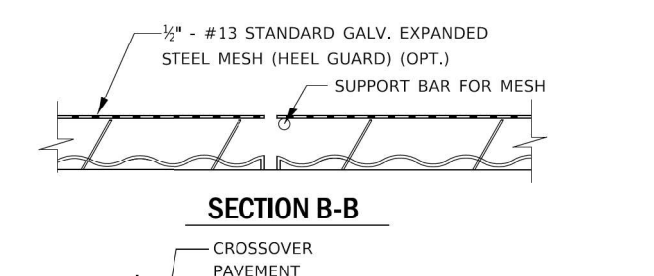
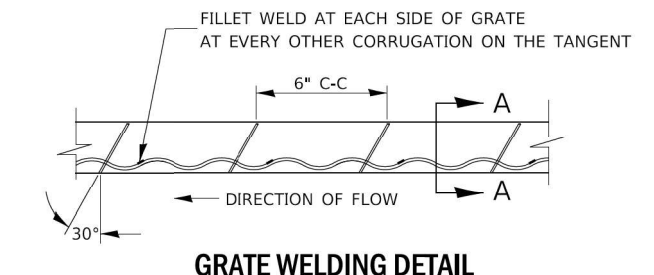


TYPICAL PIPE SECTION



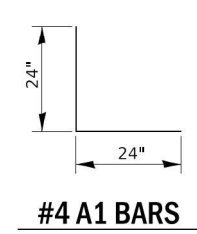
GAP PLATE (OPTIONAL)

MAY BE PLACED DIRECTLY OVER BAND BOLT TO PROVIDE CONTINUOUS FORM FOR GROUTING



CONCRETE ENCASMENT - END VIEW

NOTE: ALL BARS TO BE EPOXY COATED.



#4 A1 BARS

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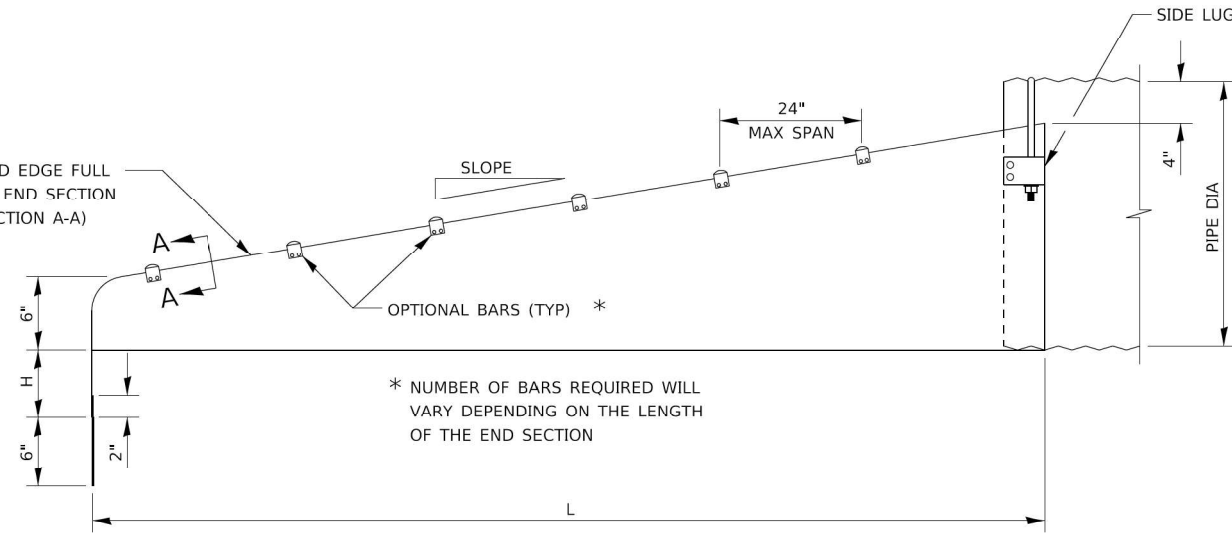
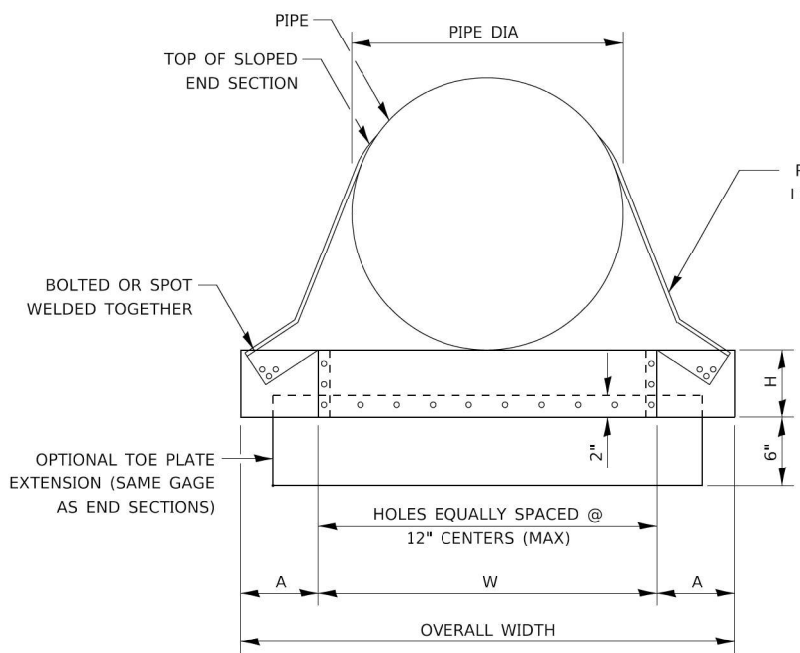
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PLOT DATE = 1/12/2024	CHECKED -	REVISED -
	DATE -	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
 DETAIL - SLOTTED DRAIN PIPE**

SCALE: SHEET OF SHEETS STA. TO STA.

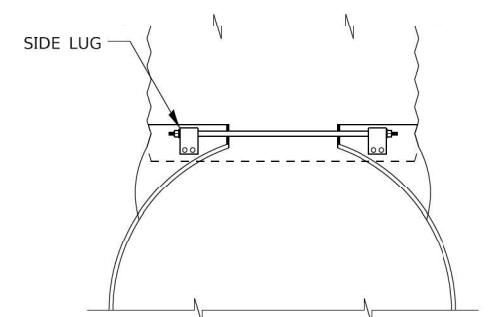
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	265
CONTRACT NO. 66K666				
ILLINOIS FED. AID PROJECT				



SIDE ELEVATION

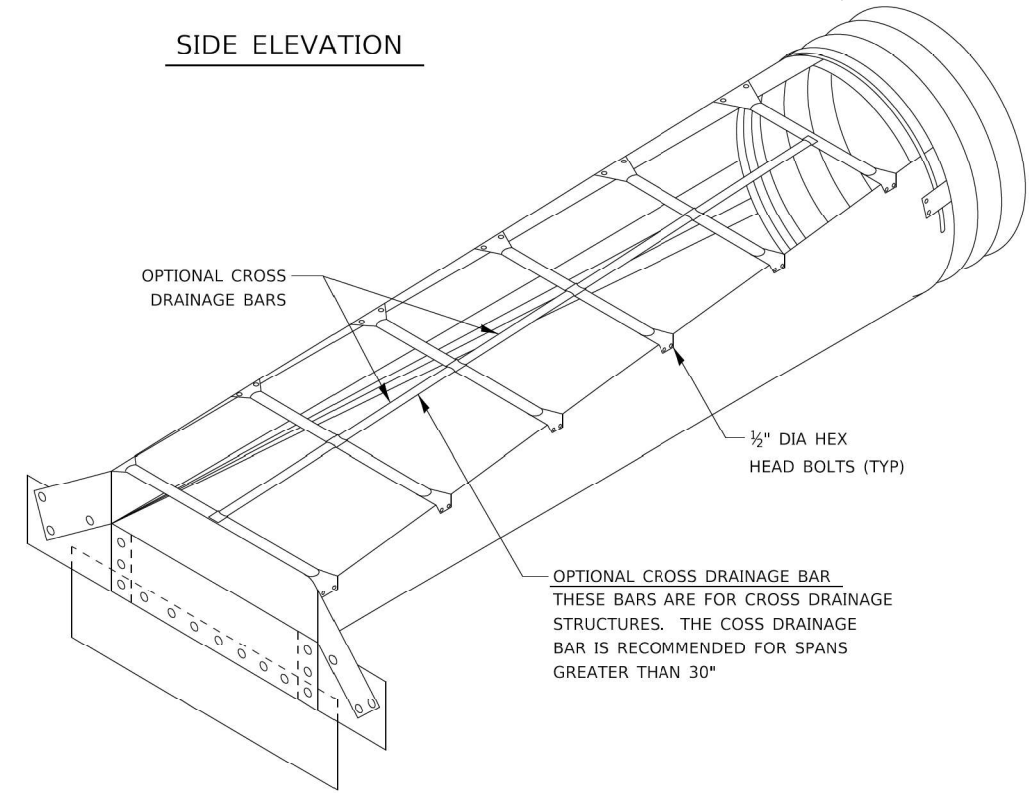
GENERAL NOTES

- CONNECTORS - ROUND SIZES THRU 24" ATTACH TO PIPE WITH TYPE #1 STRAPS, ALL OTHER SIZES ATTACH WITH TYPE #2 RODS AND LUGS.
- TOE PLATE EXTENSIONS - WHEN REQUIRED, TOE PLATE EXTENSIONS ARE TO BE THE SAME GAGE AS END SECTIONS. DIMENSIONS SHALL BE OVERALL WIDTH LESS 6 INCHES BY 8 INCHES HIGH.
- OPTIONAL BARS - BARS WHEN SPECIFIED, SHALL BE SCHEDULE 40 GALVANIZED STEEL PIPE.
- TYPICALLY PARALLEL BARS ARE PLACED ON 24" CENTERS.
- TYPICALLY THE CROSS BARS ARE USED ON CROSS DRAIN APPLICATIONS.
- HOLES FOR BAR ATTACHMENTS SHALL BE PROVIDED ON ALL END SECTIONS.
- DIMENSIONS ARE SUBJECT TO MANUFACTURING TOLERANCES.
- THESE END SECTIONS WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER EACH FOR SLOPED METAL END SECTIONS WITH GRATE OF THE DIAMETER SPECIFIED, WHICH SHALL INCLUDE FURNISHING AND INSTALLING THE END SECTION COMPLETE IN PLACE, INCLUDING THE TOE PLATE, EXCAVATING, BACKFILLING, CONNECTING TO THE PIPE, AND CROSS DRAINAGE BARS.



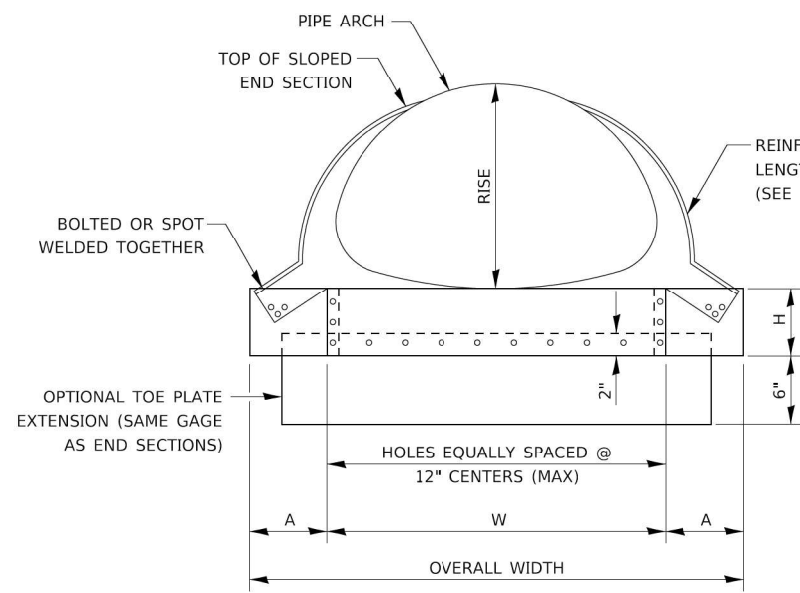
TYPE #2 CONNECTOR DETAIL

TYPE #1 CONNECTOR DETAILS THRU 24" GALVANIZED STRAP
 TYPE #2 CONNECTOR DETAILS (SHOWN) FOR 30" AND LARGER 21" x 15" AND LARGER 1/2" THREADED ROD W/FLANGED NUT AND SIDE LUG

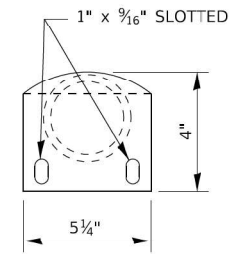


CIRCULAR PIPE ISOMETRIC VIEW

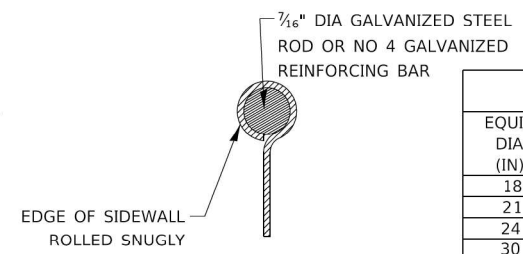
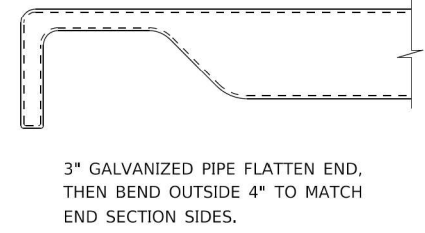
METAL END SECTIONS FOR ROUND PIPE										
PIPE DIA (IN)	MIN THICK IN	GAGE	DIMENSIONS (INCHES)			L DIMENSIONS				
			A	H	W	OVERALL WIDTH	SLOPE	LENGTH (IN)	SLOPE	LENGTH (IN)
15	.064	16	8	6	21	37	6:1	30	4:1	20
18	.064	16	8	6	24	40	6:1	48	4:1	32
21	.064	16	8	6	27	43	6:1	66	4:1	44
24	.064	16	8	6	30	46	6:1	84	4:1	56
30	.109	12	12	9	36	60	6:1	120	4:1	80
36	.109	12	12	9	42	66	4:1	104	6:1	156
42	.109	12	16	12	48	80	4:1	128	6:1	192
48	.109	12	16	12	54	86	4:1	152	6:1	228
54	.109	12	16	12	60	92	4:1	176	6:1	264
60	.109	12	16	12	66	98	4:1	200	6:1	300



FRONT VIEW PIPE ARCH



DETAIL OF OPTIONAL BARS



SECTION A-A

METAL END SECTIONS FOR PIPE ARCH												
EQUIV DIA (IN)	(INCHES)		MIN THICK IN	GAGE	DIMENSIONS (INCHES)			L DIMENSIONS				
	SPAN	RISE			A	H	W	OVERALL WIDTH	SLOPE	LENGTH (IN)	SLOPE	LENGTH (IN)
18	21	15	.064	16	8	6	27	43	6:1	30	4:1	20
21	24	18	.064	16	8	6	30	46	6:1	48	4:1	32
24	28	20	.064	16	8	6	34	50	6:1	60	4:1	40
30	36	24	.079	14	12	9	41	65	6:1	84	4:1	56
36	42	29	.109	12	12	9	48	72	6:1	114	4:1	76
42	49	33	.109	12	16	12	55	87	4:1	92	6:1	138
48	57	38	.109	12	16	12	63	95	4:1	112	6:1	168
54	64	43	.109	12	16	12	70	102	4:1	132	6:1	198
60	71	47	.109	12	16	12	77	109	4:1	148	6:1	222
72	83	57	.109	12	16	12	89	121	4:1	188	6:1	282

MODEL: Default
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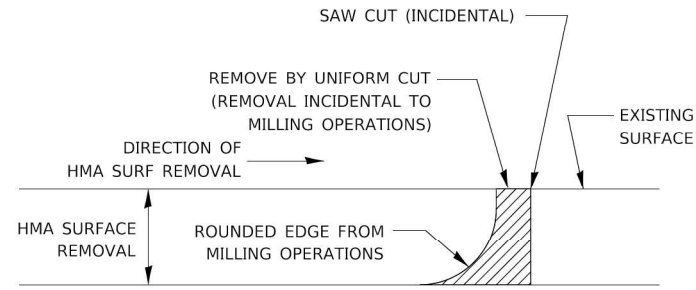
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	DRAWN -	REVISED -
PLOT SCALE = 100,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
 DETAIL - SLOPED METAL END SECTIONS WITH GRATE**

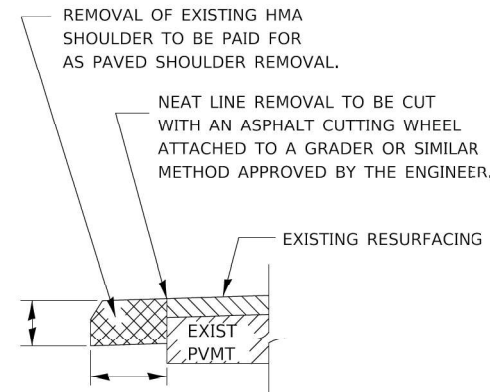
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	266
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

SCALE: SHEET OF SHEETS STA. TO STA.

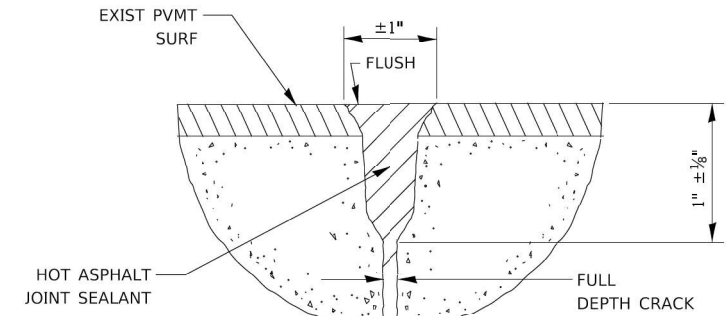


NOTE:
WHEN MILLING OPERATIONS PRODUCE A ROUNDED EDGE, THEN A SAW CUT SHALL BE USED TO MANUFACTURE A PERPENDICULAR EDGE AS SHOWN IN THE DETAIL. THE ENGINEER SHALL BE THE SOLE JUDGE CONCERNING THE USE OF THIS DETAIL.

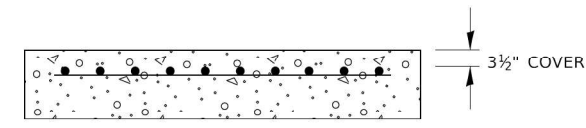
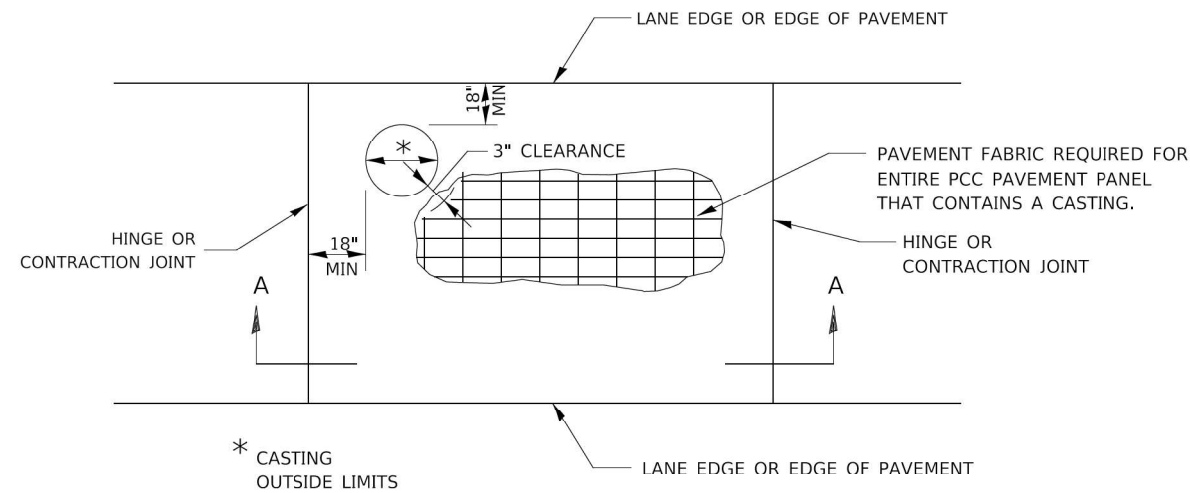
HMA DETAIL AT BUTT JOINTS



REMOVAL OF EXISTING HMA SHOULDER



ROUTING & SEALING JOINTS (CENTERLINE & TRANSVERSE PATCH BOUNDARIES)



SECTION A-A

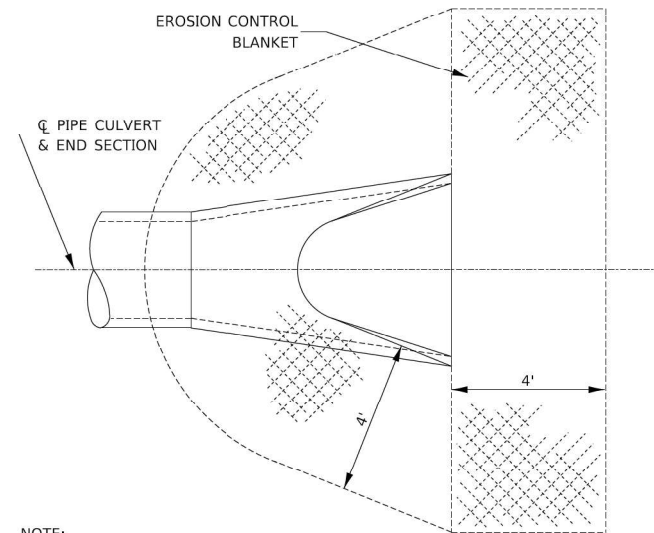
GENERAL NOTES

THE CASTING SHALL BE SET TO GRADE, ANCHORED, AND INCORPORATED INTO THE PCC PAVEMENT CONSTRUCTION. SEPARATE PAVEMENT BLOCKOUTS WILL NOT BE ALLOWED.

SEE STD 420701 FOR ADDITIONAL PAVEMENT FABRIC DETAILS.

PAVEMENT FABRIC WILL BE PAID FOR SEPARATELY. THE QUANTITY OF PAVEMENT FABRIC WILL BE THE COMPUTED SURFACE AREA OF THE PCC PAVEMENT PANEL IN WHICH THE PAVEMENT FABRIC IS INSTALLED. NO DEDUCTION WILL BE MADE FOR THE CASTING AREA.

CASTINGS IN PCC PAVEMENT



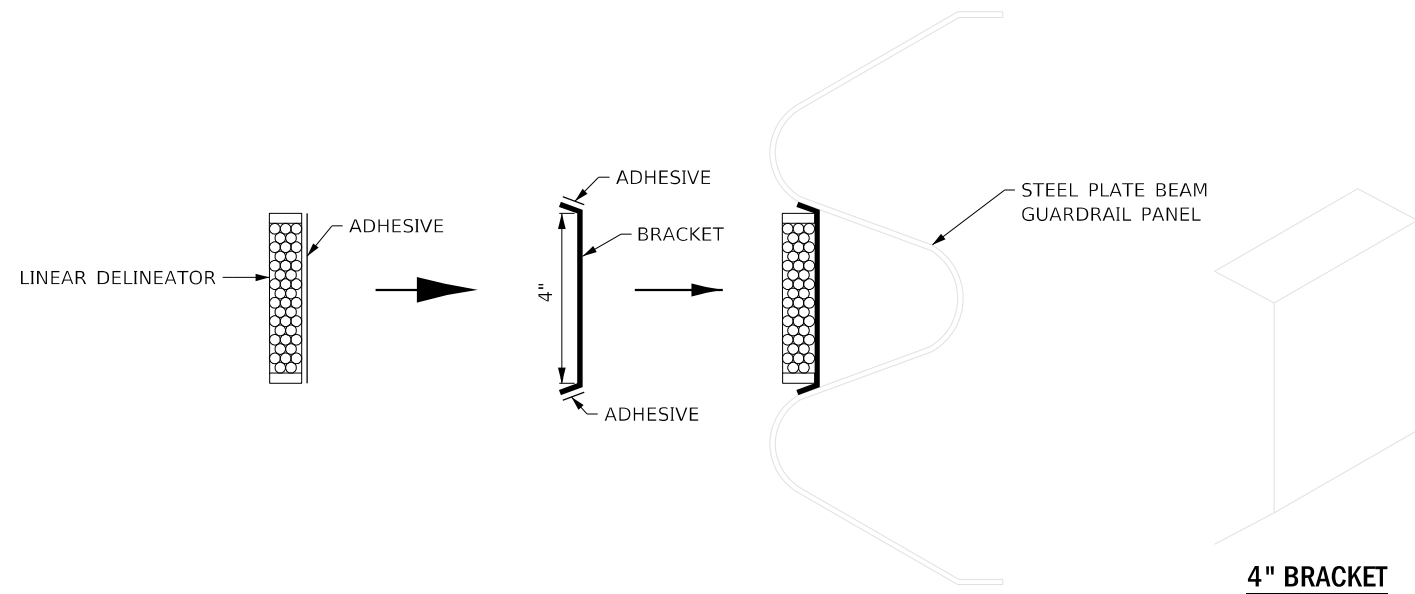
NOTE:
TO BE USED AT ALL END SECTIONS

DETAIL OF EROSION CONTROL BLANKET LINING AROUND END SECTION

MODEL: Default
FILE NAME: 2023057.06 IDOT D3 PFB 204-028 MO 06 L180 Resurfacing/DCU/Design/Plan/Plot/Sheet3/0356666-sh-D3Detail.dgn

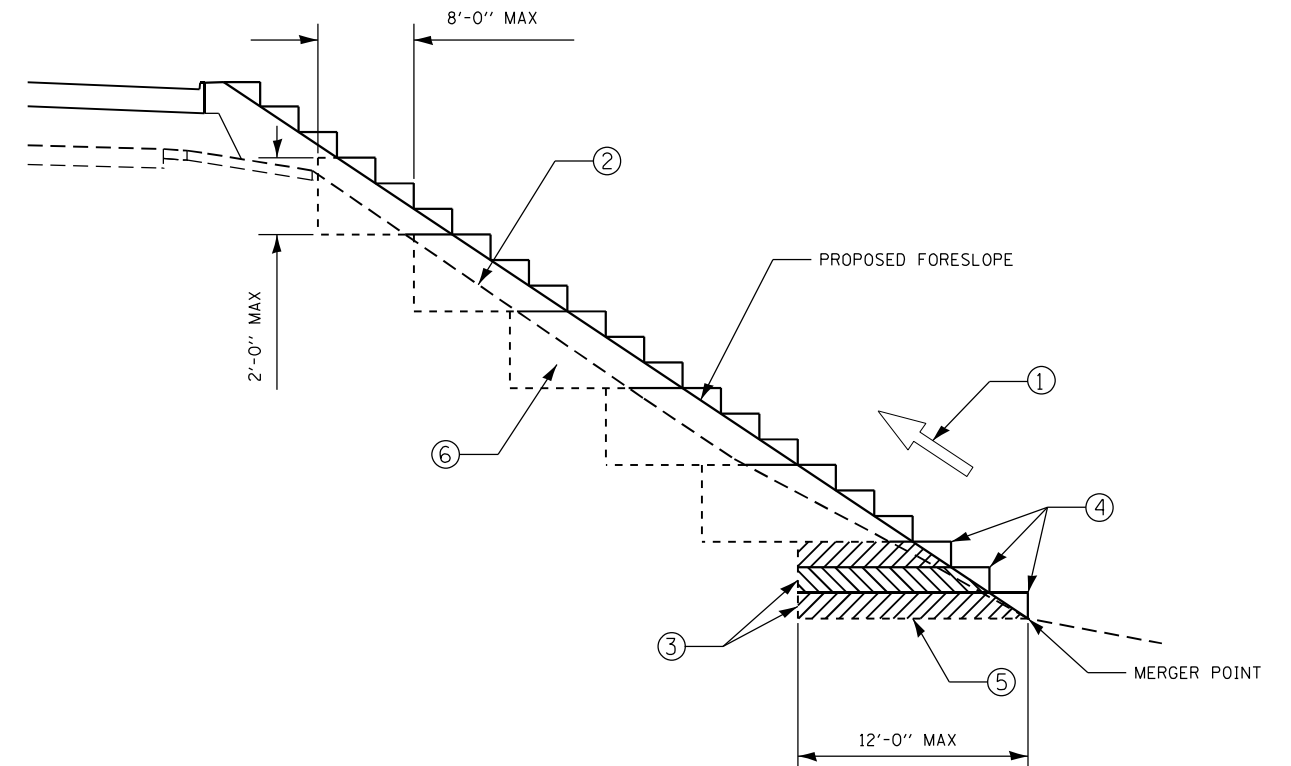
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PLOT SCALE = 100,0000' / in.	DRAWN -	REVISED -
PLOT DATE = 1/12/2024	CHECKED -	REVISED -
	DATE -	REVISED -

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	267
			CONTRACT NO. 66K66	
ILLINOIS FED. AID PROJECT				



LINEAR DELINEATOR APPLICATION TO STANDARD GALVANIZED GUARDRAIL

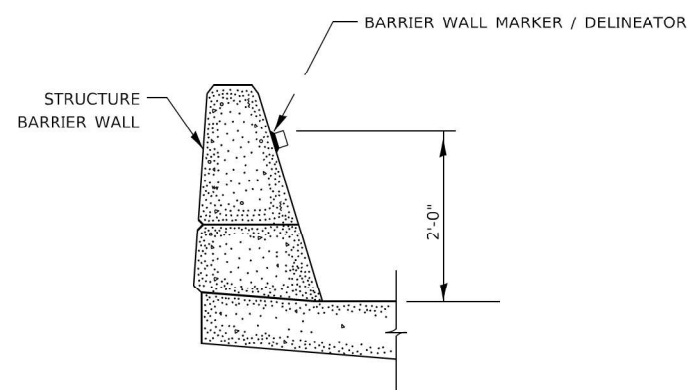
LINEAR DELINEATOR SHALL BE APPLIED ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS



TYPICAL BENCHING FOR EMBANKMENT DETAIL

SCALE: NONE

- ① CONSTRUCT SUCCEEDING BENCH CUTS AND EMBANKMENT PLACEMENT AND COMPACTION FROM BOTTOM TO TOP IN STAIRSTEP FASHION.
- ② EXISTING FORESLOPE PREPARED IN ACCORDANCE WITH ARTICLE 205.03
- ③ BENCH CUT EXISTING FINAL SLOPE TYPICAL FOR EACH STEP.
- ④ TRIM TO FINAL SLOPE.
- ⑤ EQUAL 8-INCH LIFTS OF EMBANKMENT COMPACTED IN ACCORDANCE WITH ARTICLE 205.05 OF THE STANDARD SPECIFICATIONS.
- ⑥ EXCAVATION OF BENCH CUTS SHALL NOT BE PAID FOR DIRECTLY BUT SHALL BE CONSIDERED AS INCLUDED IN THE VARIOUS ITEMS OF EXCAVATION, AND THEIR CONSTRUCTION SHALL BE INCLUDED IN THE PRICES FOR THESE ITEMS.
- ⑦ SLOPES SHALL BE BENCHED ACCORDING TO THIS DETAIL WHEN THE SLOPE IS STEEPER THAN 4:1 AND THE HEIGHT IS GREATER THAN 5'.

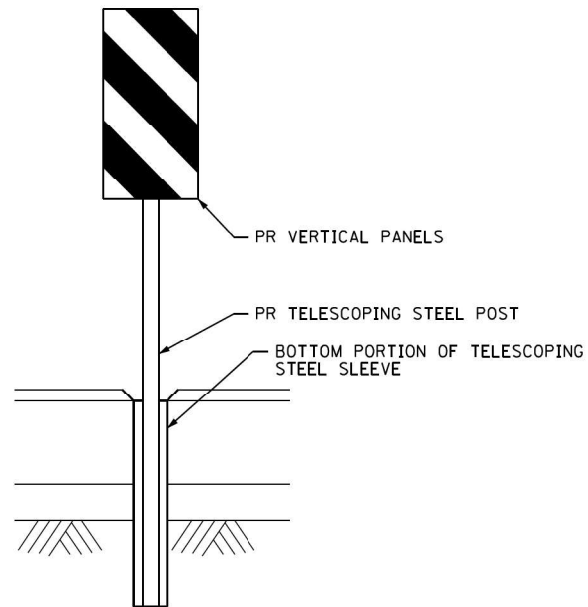


BARRIER WALL MARKER

MODEL: Default
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	DRAWN -	REVISED -
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PLOT DATE = 1/12/2024	DATE -	REVISED -

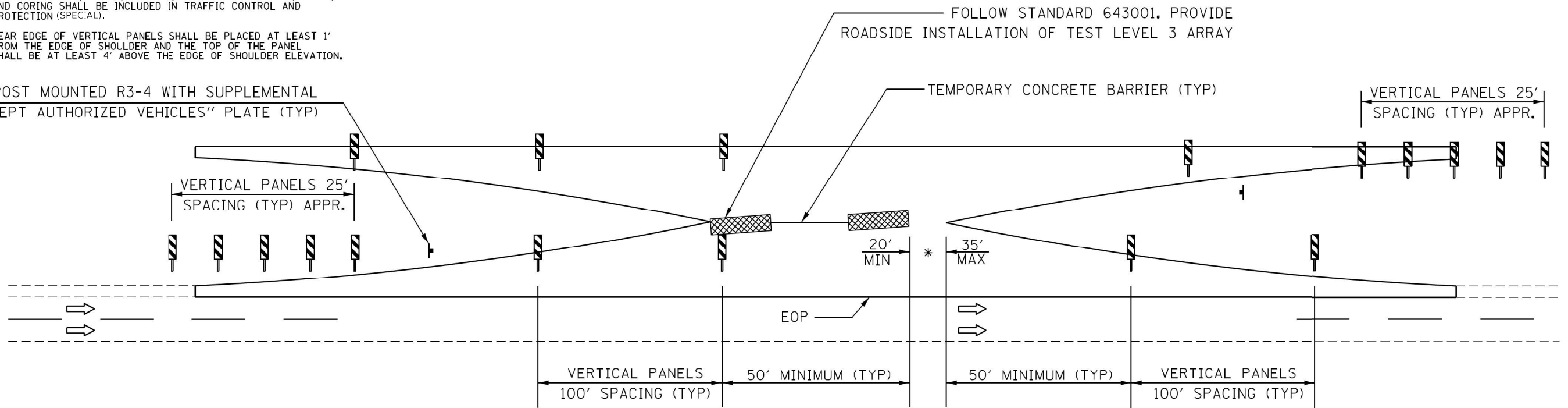
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	268
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	



NOTE: FOR INSTALLATION OF VERTICAL PANELS IN AREAS OF CROSSOVER PAVEMENT, 3" Ø HOLES WILL BE CORED THROUGH PAVEMENT FOR PLACEMENT. TELESCOPING STEEL POLES WILL BE PLACED 1" BELOW SURFACE AS SHOWN, AND WORK SHALL BE DONE TO THE SATISFACTION OF THE ENGINEER. WHEN OPENING CROSSOVER, PANELS WILL BE REMOVED AND HOLES WILL BE LEFT IN PLACE. COST OF PANELS, POSTS, SLEEVES, TYPE III BARRICADES, SIGNS, AND CORING SHALL BE INCLUDED IN TRAFFIC CONTROL AND PROTECTION (SPECIAL).

NEAR EDGE OF VERTICAL PANELS SHALL BE PLACED AT LEAST 1' FROM THE EDGE OF SHOULDER AND THE TOP OF THE PANEL SHALL BE AT LEAST 4' ABOVE THE EDGE OF SHOULDER ELEVATION.

POST MOUNTED R3-4 WITH SUPPLEMENTAL "EXCEPT AUTHORIZED VEHICLES" PLATE (TYP)



LEGEND

- ← TRAFFIC FLOW ARROW
- ⊥ TYPE III BARRICADE
- ▬ IMPACT ATTENUATOR
- BARREL W/STEADY BURNING LIGHT
- ⊥ POST MOUNTED SIGN
- ◇ BARRIER WALL MARKER
- ▬ TEMPORARY CONCRETE BARRIER
- ▬ VERTICAL SIGN PANEL

* REMOVE GAP FOR FULL CLOSURE

CROSSOVER CLOSURE-TYPICAL

MODEL: D:\efk\180\180-2024\180-2024-028.WD 06 11 2024 10:00:00 AM (P:\180-2024\180-2024-028.WD 06 11 2024 10:00:00 AM) (P:\180-2024\180-2024-028.WD 06 11 2024 10:00:00 AM) (P:\180-2024\180-2024-028.WD 06 11 2024 10:00:00 AM)

EFK Moen
Civil Engineering Design

USER NAME = RGail	DESIGNED -	REVISED -
PLOT SCALE = 100,0000' / in.	DRAWN -	REVISED -
PLOT DATE = 1/12/2024	CHECKED -	REVISED -
	DATE -	REVISED -

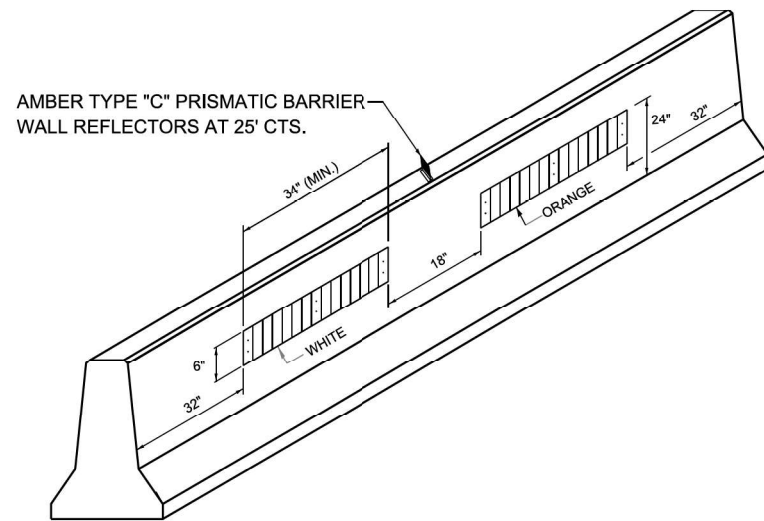
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
DETAILS**

SCALE: SHEET OF SHEETS STA. TO STA.

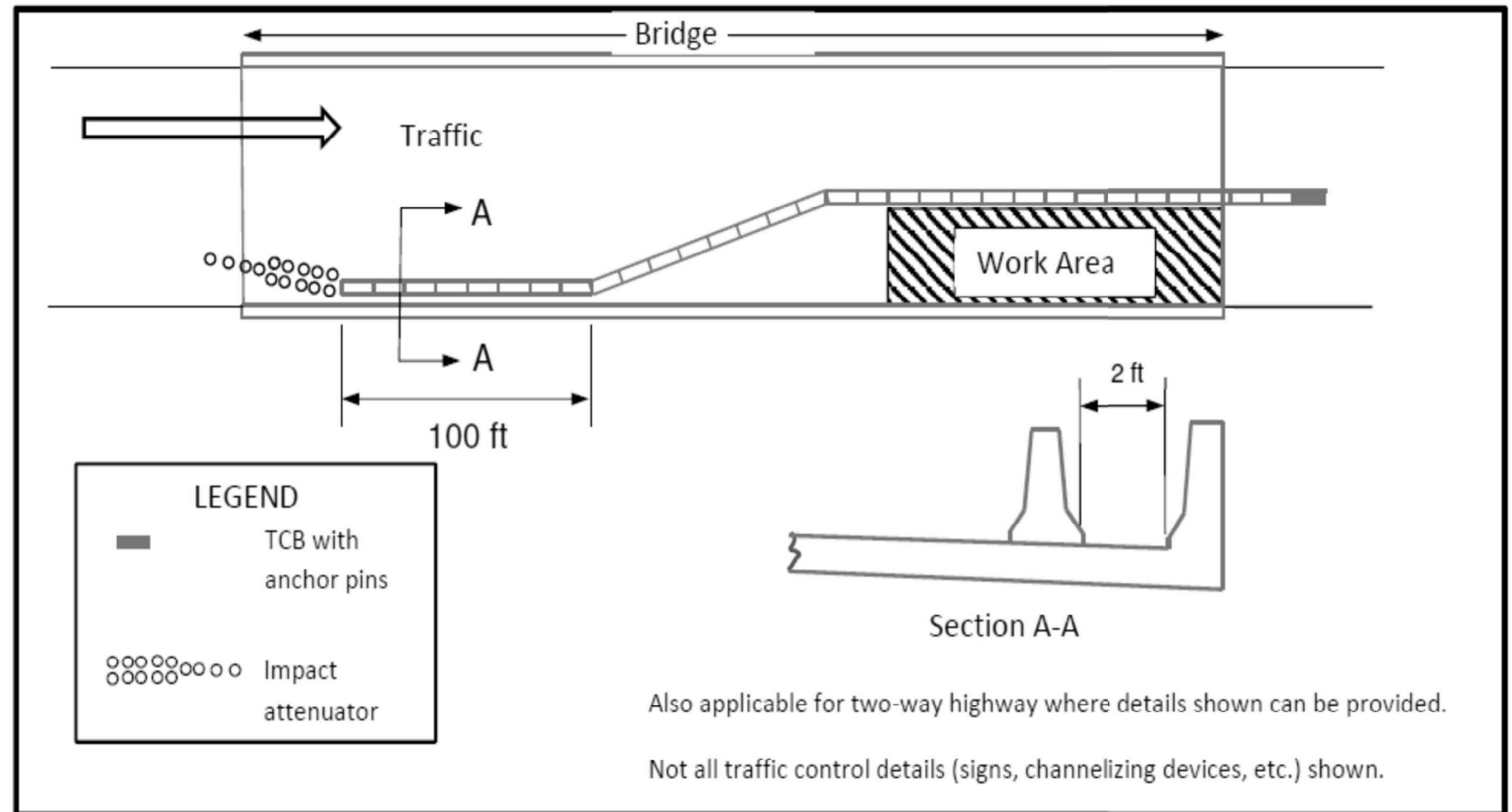
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	269
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	

NOT TO SCALE



AMBER TYPE "C" PRISMATIC BARRIER WALL REFLECTORS AT 25' CTS.

LINEAR DELINEATOR PANELS FOR TEMPORARY CONCRETE BARRIER

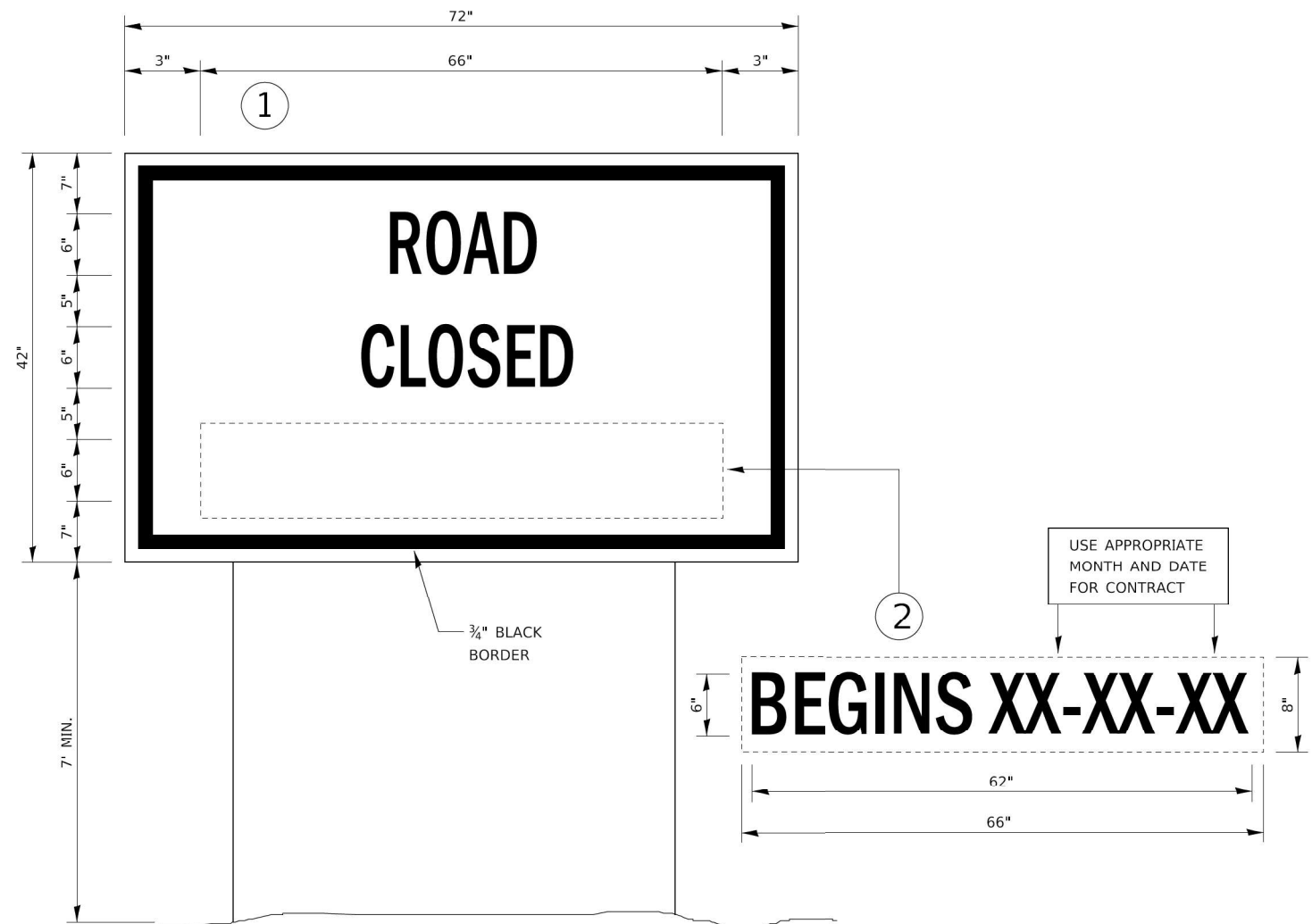
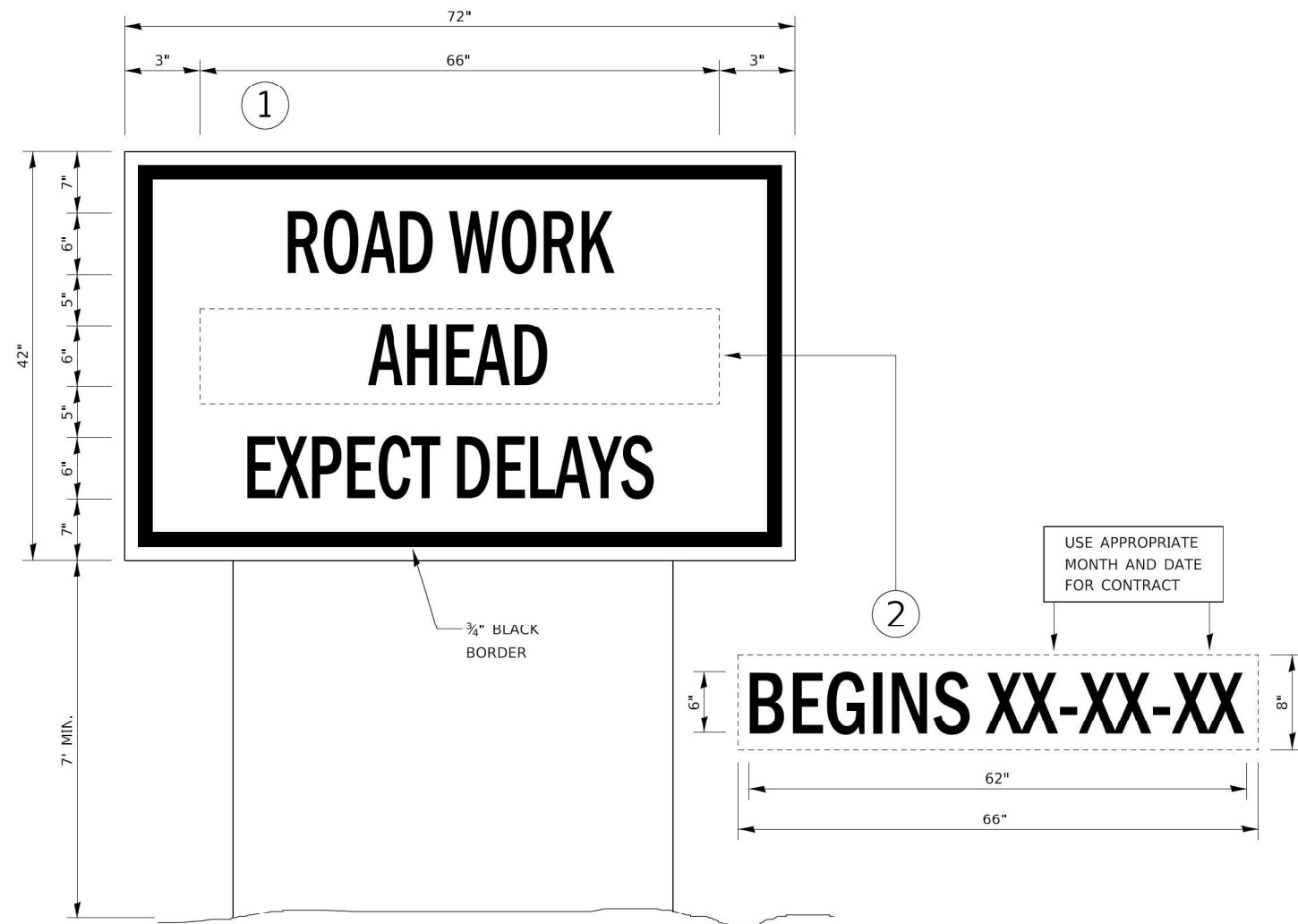


ALTERNATE LAYOUT OF TCB FOR ANCHORING ADJACENT TO CONCRETE BRIDGE PARAPET

MODEL: Default
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	DRAWN -	REVISED -
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PLOT DATE = 1/12/2024	DATE -	REVISED -

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	270
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	



TEMPORARY INFORMATION SIGNING

TEMPORARY INFORMATION SIGNING

NOTES:

1. USE 6" D BLACK LETTERING ON FLUORESCENT ORANGE BACKGROUND.
2. ERECT SIGNS AT LOCATIONS IN ADVANCE OF THE "ROAD CONSTRUCTION AHEAD" SIGNS AS DIRECTED BY THE ENGINEER.
3. ERECT SIGN ① WITH INSTALLED PANEL ② A MINIMUM OF ONE WEEK PRIOR TO THE START OF THE LANE CLOSURE.
4. REMOVE PANEL ② ON THAT DATE.
5. SEE SPECIAL PROVISION "TEMPORARY INFORMATION SIGNING" FOR ADDITIONAL INFORMATION.
6. WILL BE PAID FOR PER SQ FT AS "TEMPORARY INFORMATION SIGNING". EACH SIGN = 21 SQ FT AND THE DATE PANEL ② WILL NOT BE MEASURED SEPARATELY FOR PAYMENT.

NOTES:

1. USE 6" D BLACK LETTERING ON FLUORESCENT ORANGE BACKGROUND.
2. ERECT SIGNS AT LOCATIONS IN ADVANCE OF THE "ROAD CONSTRUCTION AHEAD" SIGNS AS DIRECTED BY THE ENGINEER.
3. ERECT SIGN ① WITH INSTALLED PANEL ② A MINIMUM OF ONE WEEK PRIOR TO THE START OF THE ROAD CLOSURE.
4. REMOVE PANEL ② ON THAT DATE.
5. SEE SPECIAL PROVISION "TEMPORARY INFORMATION SIGNING" FOR ADDITIONAL INFORMATION.
6. WILL BE PAID FOR PER SQ FT AS "TEMPORARY INFORMATION SIGNING". EACH SIGN = 21 SQ FT AND THE DATE PANEL ② WILL NOT BE MEASURED SEPARATELY FOR PAYMENT.

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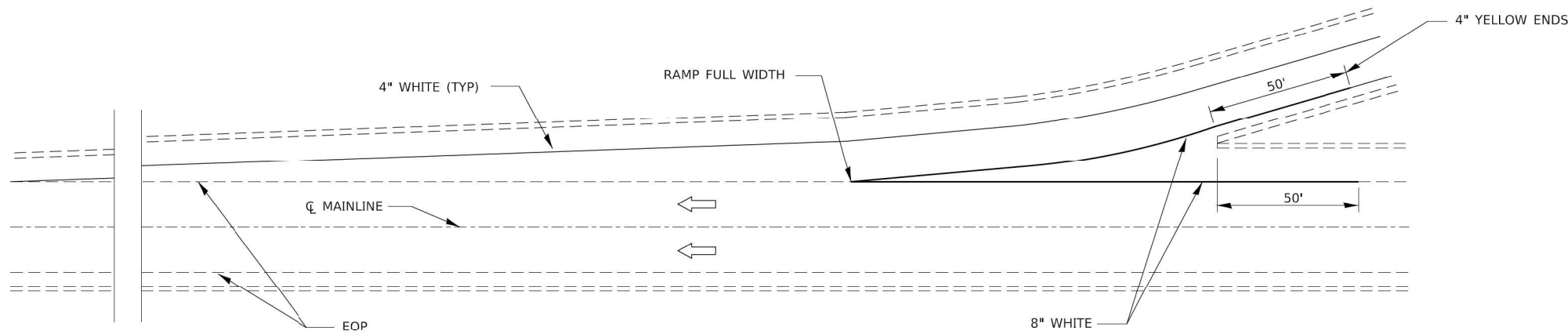
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PLOT DATE = 1/12/2024	CHECKED -	REVISED -
	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

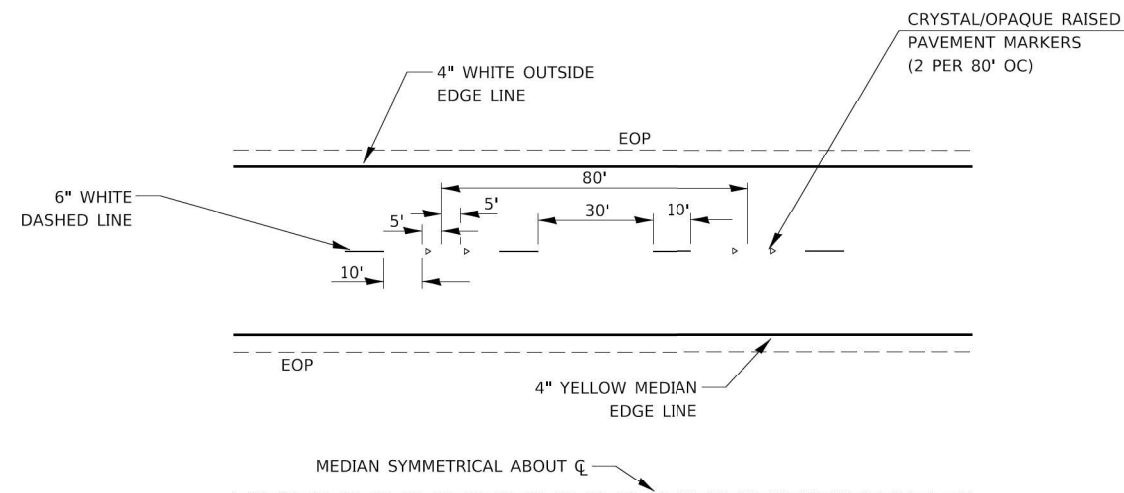
**FAI ROUTE 180 (I-180)
DETAILS**

SCALE: SHEET OF SHEETS STA. TO STA.

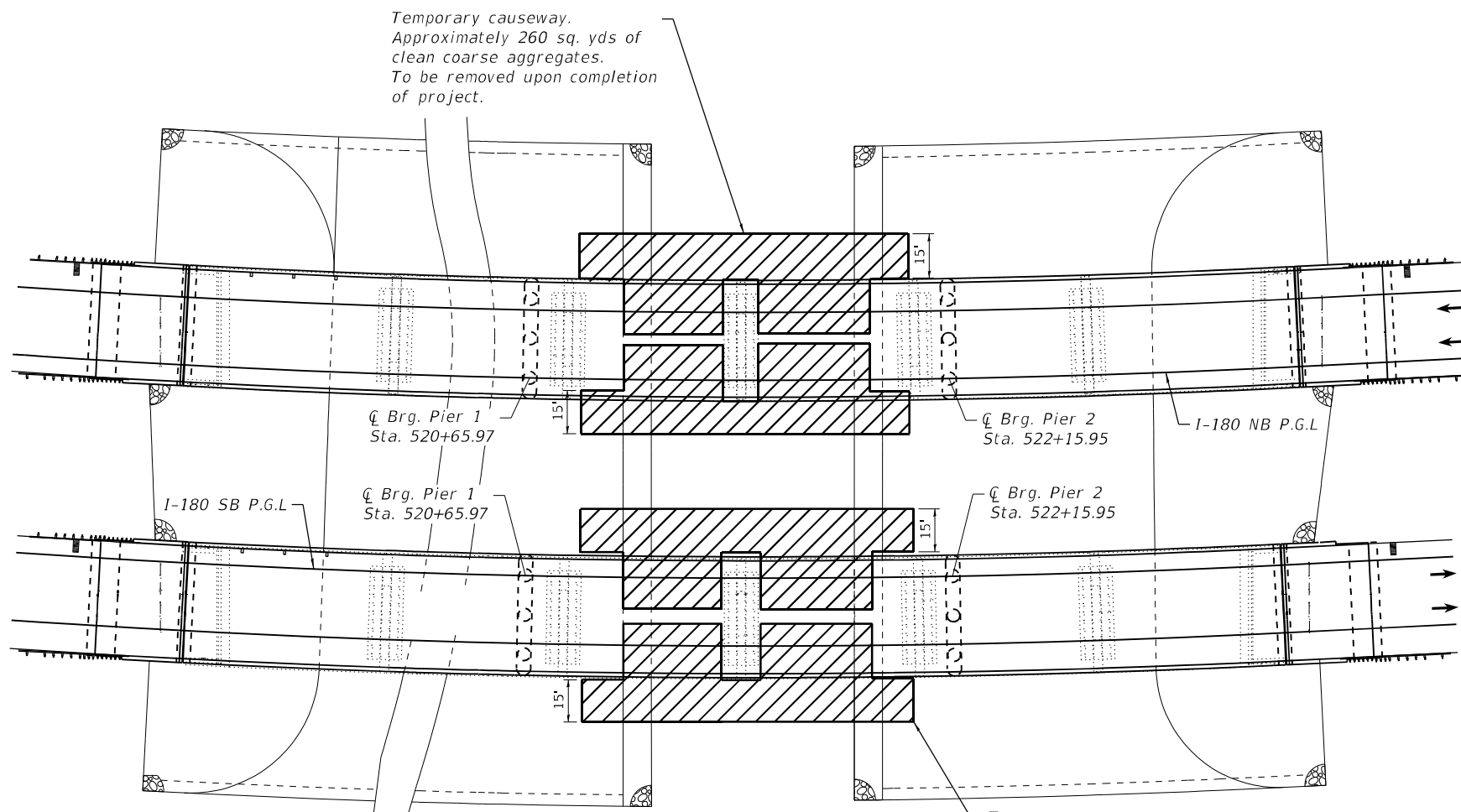
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	271
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	



TYPICAL PAVEMENT MARKING FOR ENTRANCE RAMP TERMINALS

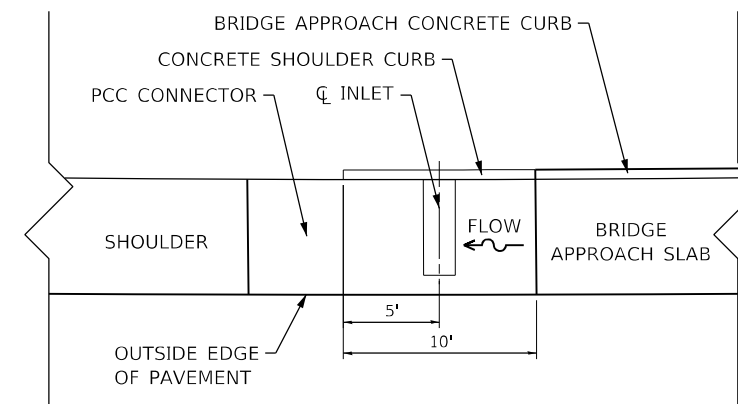


TYPICAL PAVEMENT MARKINGS



TEMPORARY CAUSEWAY

Temporary causeway.
Approximately 260 sq. yds of
clean coarse aggregates.
To be removed upon completion
of project.



DETAIL AT SHOULDER INLETS

MODEL: Default
 FILE NAME: 2023057_06 IDOT D3 PFB_204-028_MO_06 I-180_RoadwayDCIMDetailPier1Pier2Sheet1D36666-sh-D3Detail.dgn

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USER NAME = RGal	DESIGNED -	REVISED -
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	DATE -	REVISED -

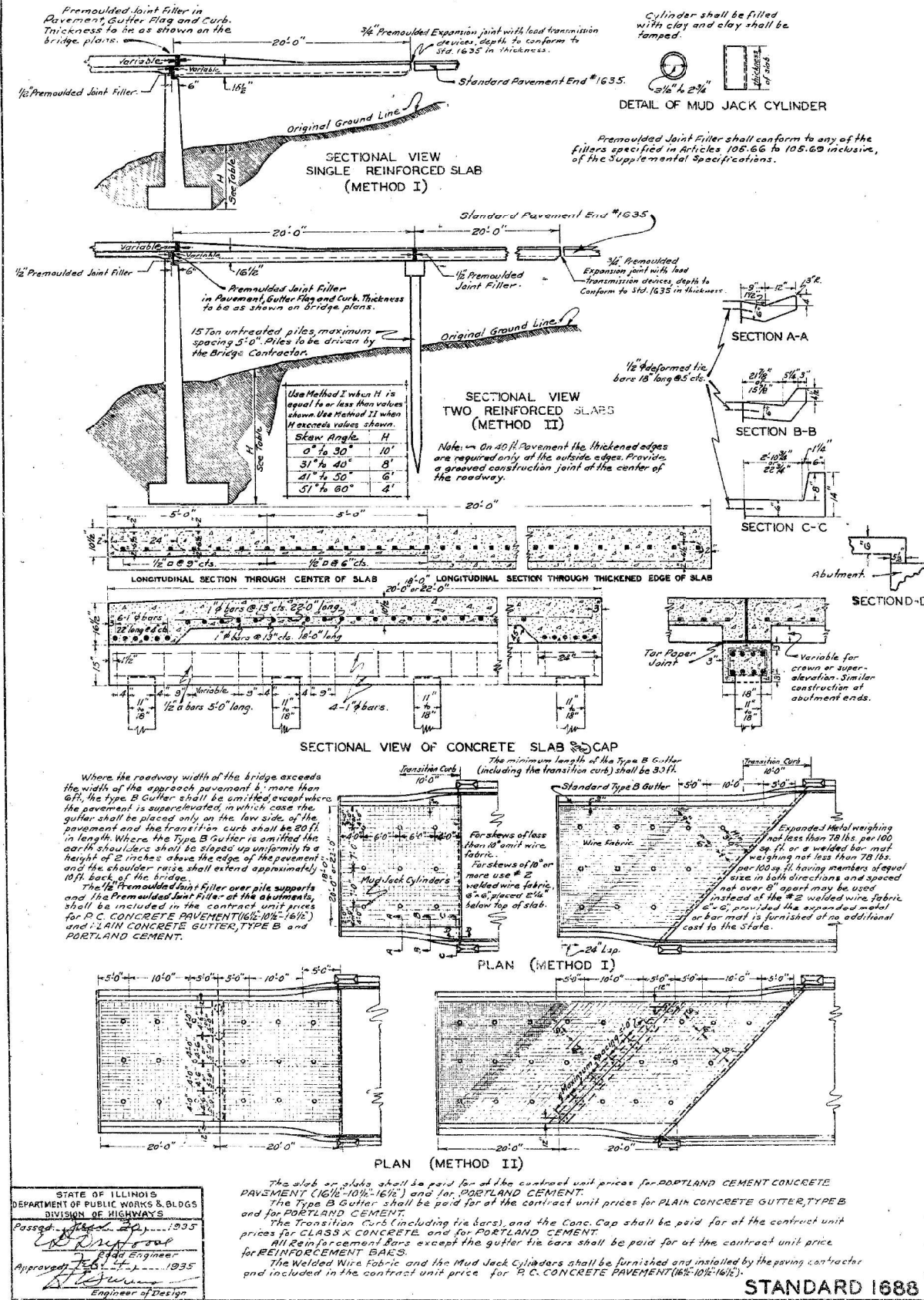
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
DETAILS**

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	272
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	

DETAILS OF BRIDGE APPROACHES



FOR INFORMATION ONLY

MODEL: Defaul
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EFK Moen
 Civil Engineering Design

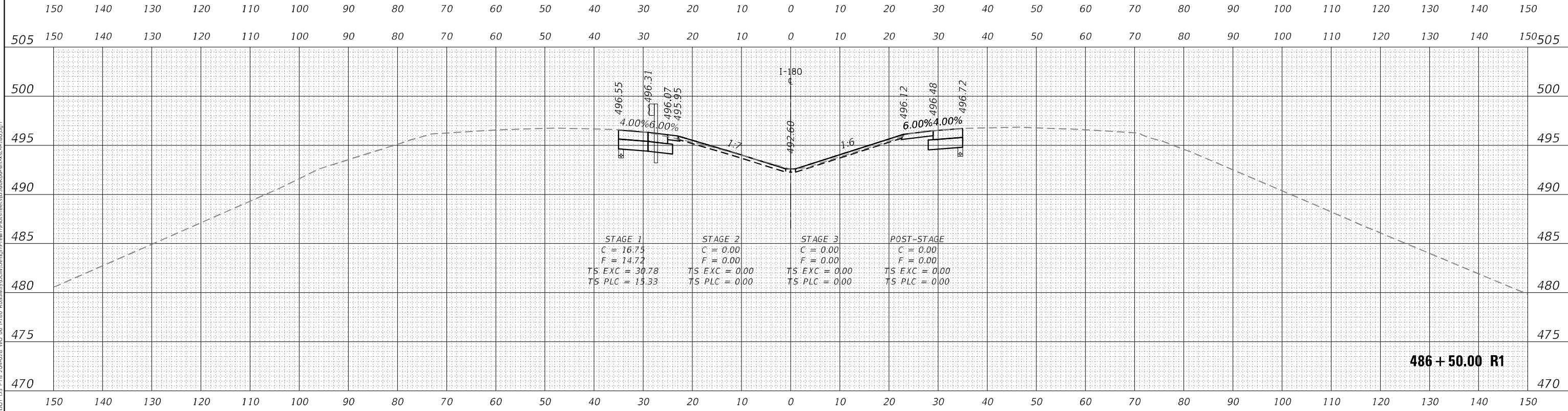
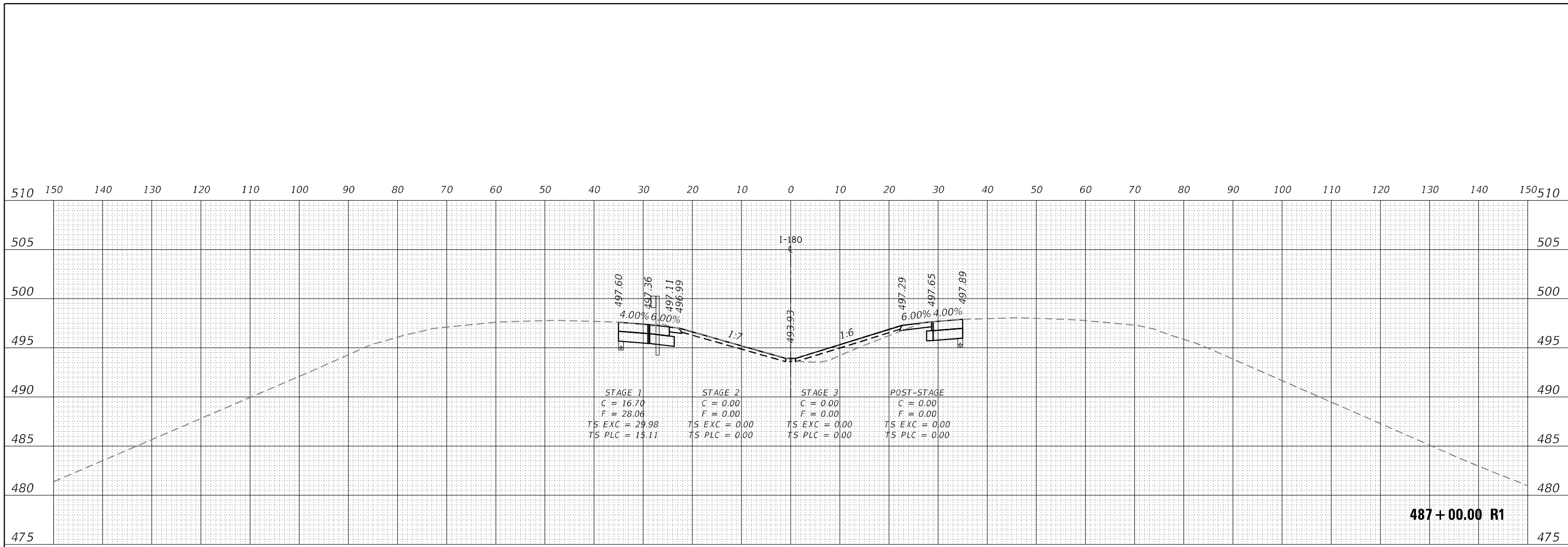
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PLOT DATE = 1/12/2024	CHECKED -	REVISED -
	DATE -	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

FAI ROUTE 180 (I-180)
 BRIDGE APPROACH METHOD II - FOR INFORMATION ONLY

SCALE: SHEET OF SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	273
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 DOT D3 P1B 204-028 WD 06 I-180 Roadway/DC/Plot/Design/Plots/Sheet/0366K66-sh-c-e-1-80.dgn

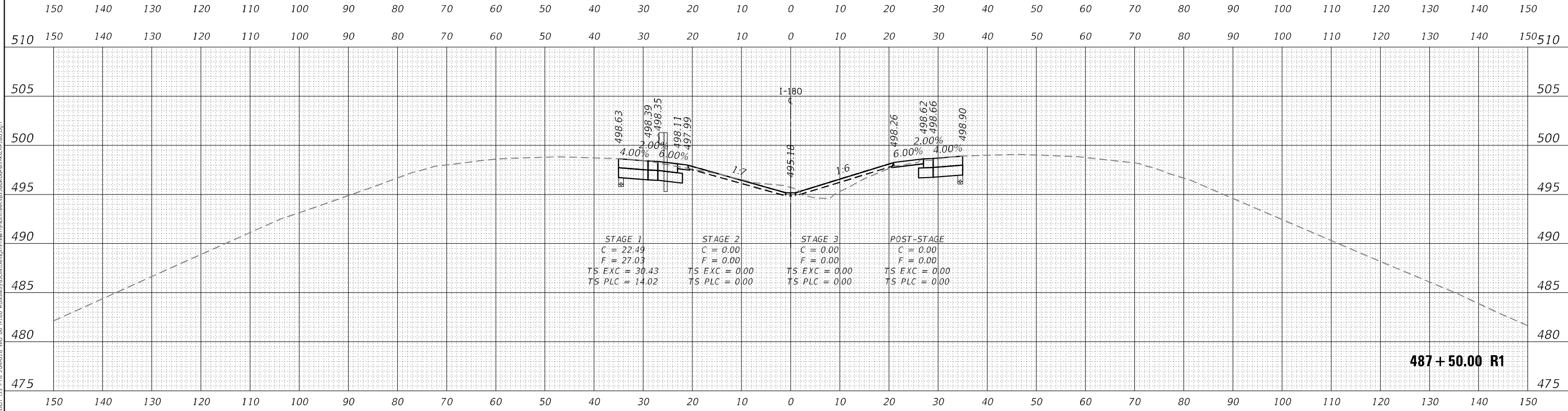
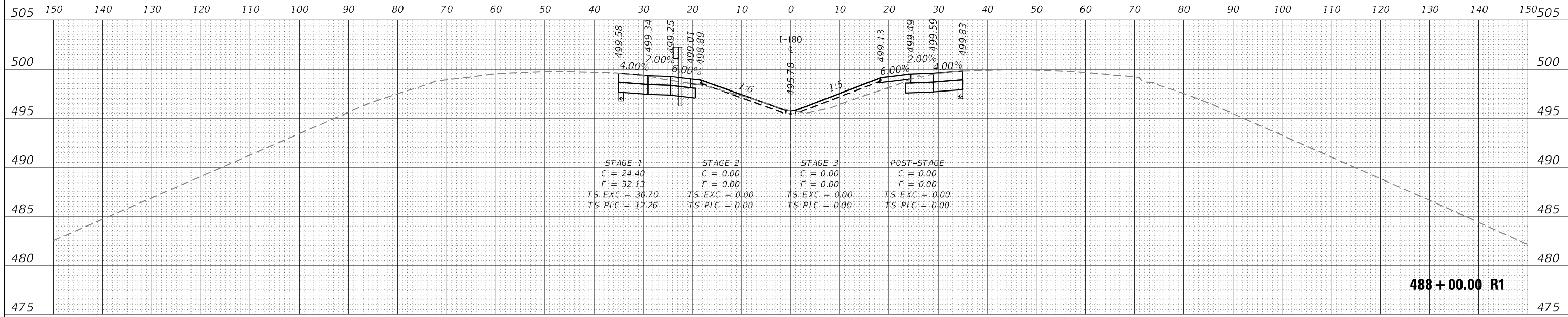


USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

FAI ROUTE 180 (I-180) CROSS SECTIONS	
SCALE:	SHEET OF SHEETS
STA. 486+50.00 R1 TO STA. 487+00.00 R1	

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	275
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definitive
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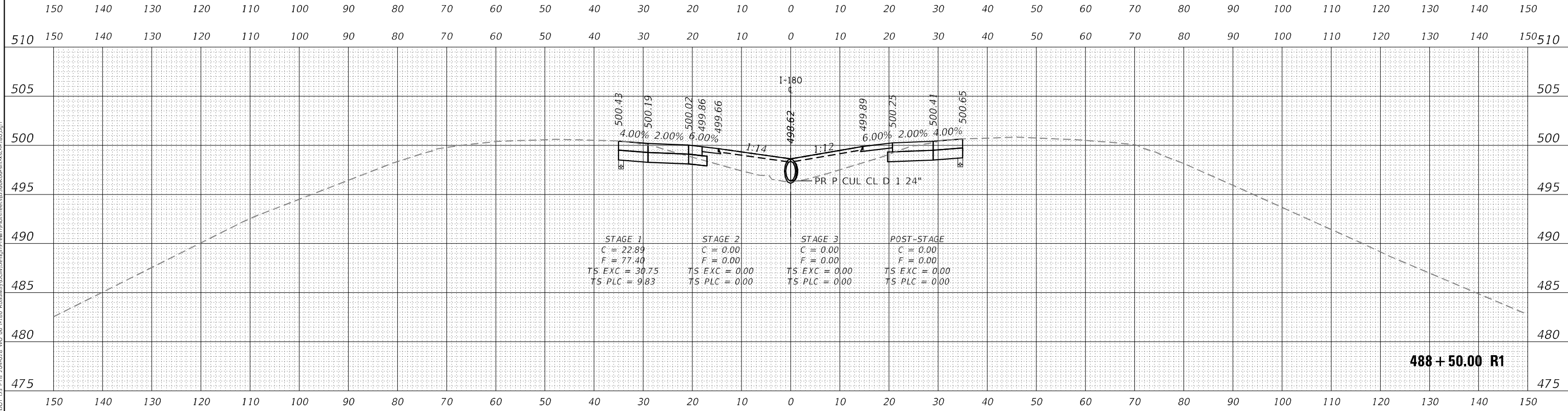
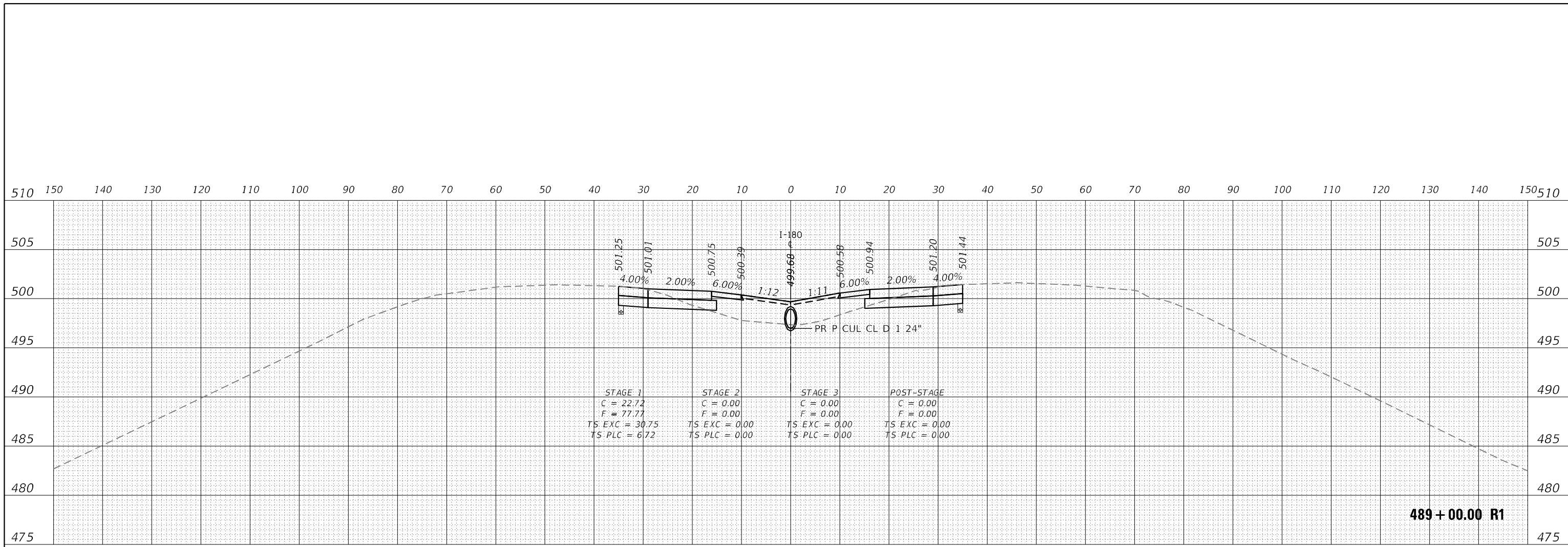
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	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 487+50.00 R1 TO STA. 488+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	276
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 DOT D3 P1B 204-028 WD 06 I-180 Roadway/Culvert/Design/Plat/PlatSheet/0356K66-sh-c-e-l-180.dgn



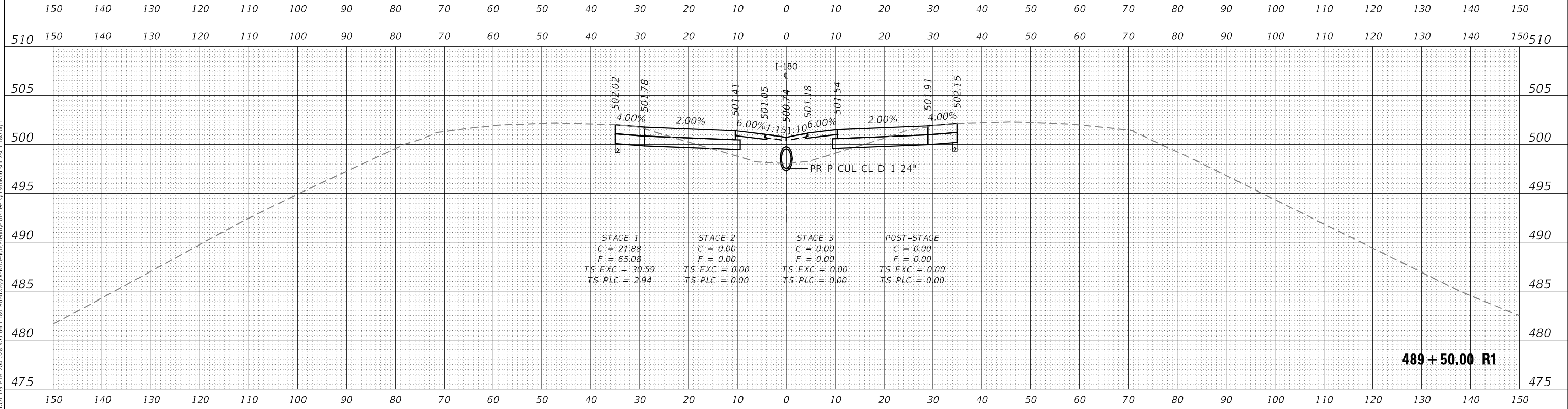
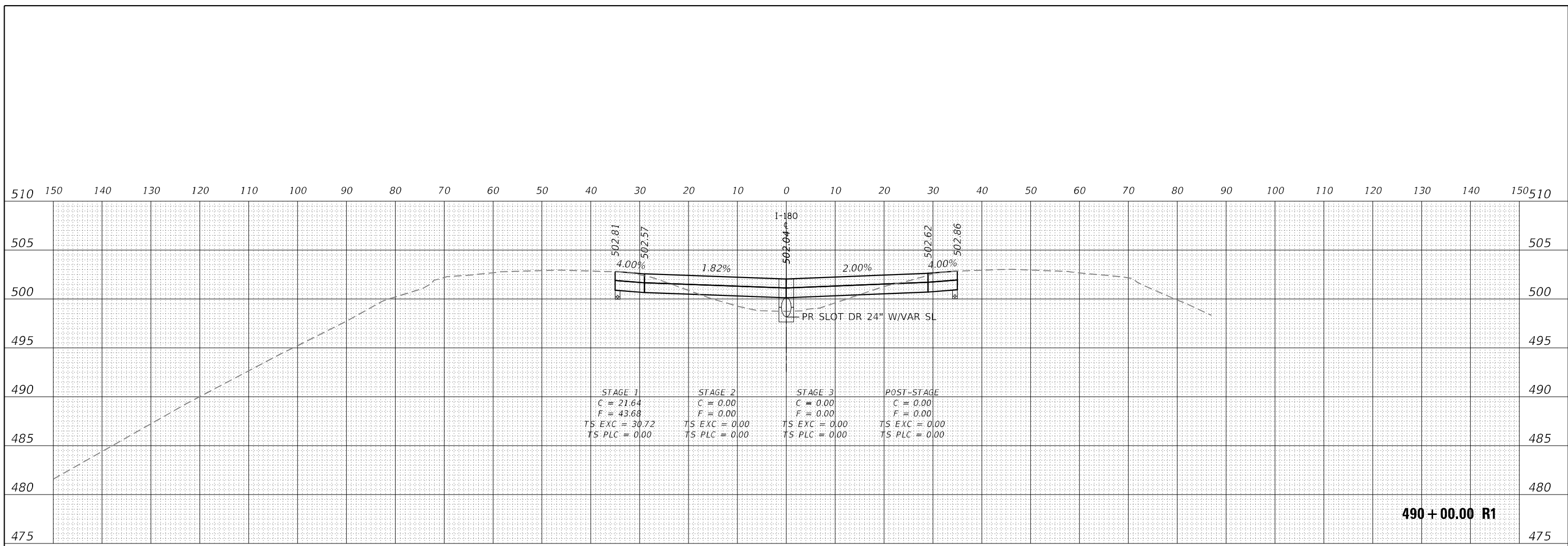
USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
 CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 488+50.00 R1 TO STA. 489+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	277
			CONTRACT NO. 66K66	
		ILLINOIS	FED. AID PROJECT	



MODEL: Definit
 FILE NAME: 2/2/2017_06 DDT D3 P1B 204+028 WD 06 I-180 Roadway/Culvert/Design/Pr/Plm/PlotSheet/0356K66-sh-c-e-l-180.dgn



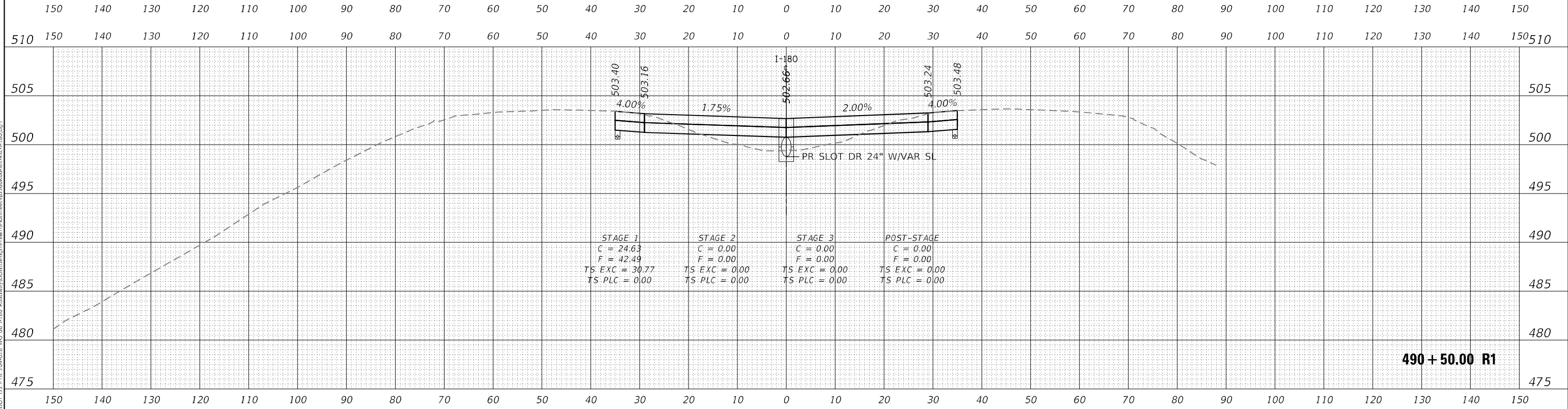
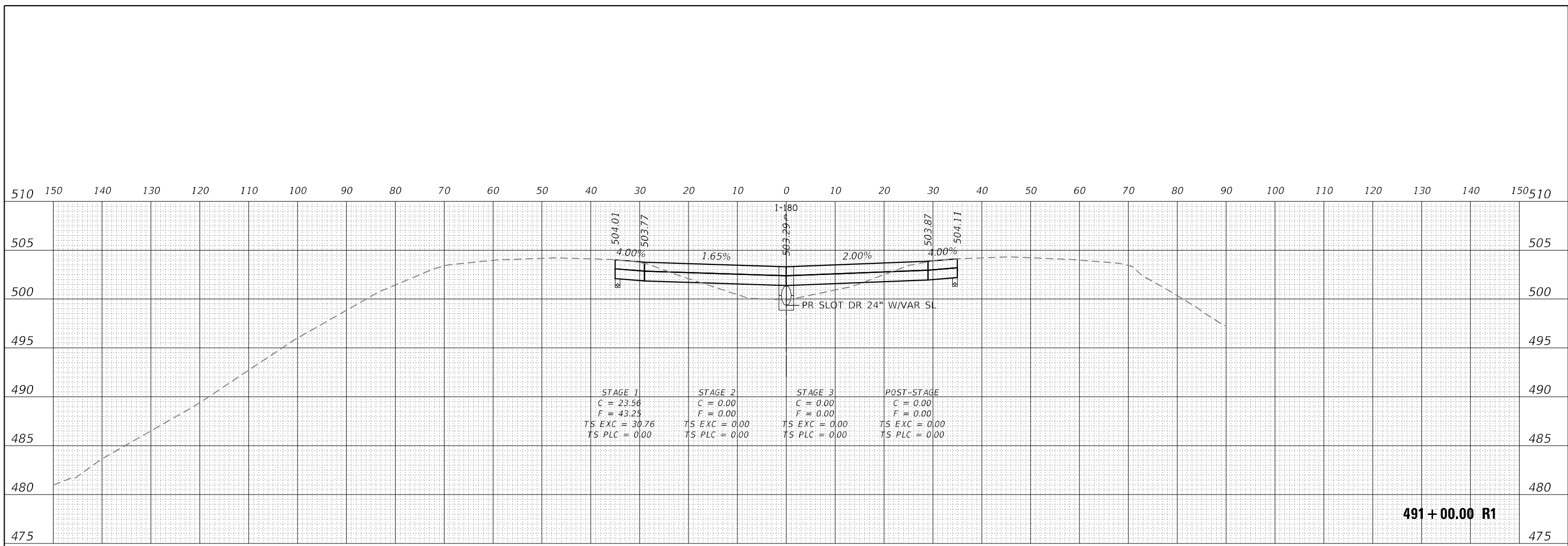
USER NAME = RGall	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 180 (I-180)
CROSS SECTIONS

SCALE: SHEET OF SHEETS STA. 489+50.00 R1 TO STA. 490+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	278
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL Definit FILE NAME: 2/2/2024_06 DOT 03 P18 204+028 WD 06 I-180 Roadway/Civil/Design/Plat/PlatSheet/03286666-sh-cs-e-l-180.dgn



USER NAME = RGall	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

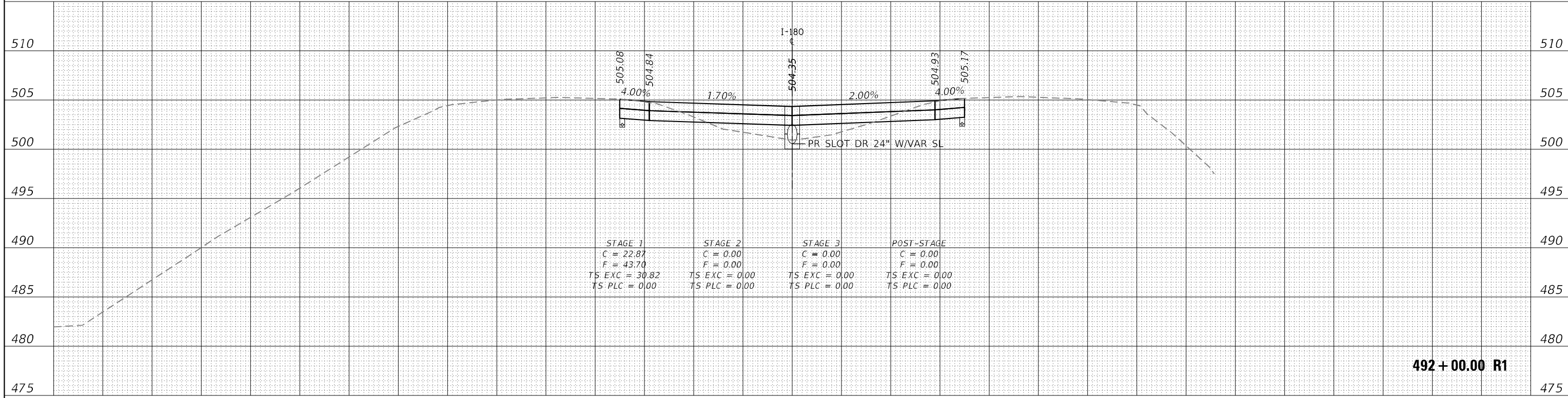
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 490+50.00 R1 TO STA. 491+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	279
			CONTRACT NO. 66K66	
		ILLINOIS	FED. AID PROJECT	

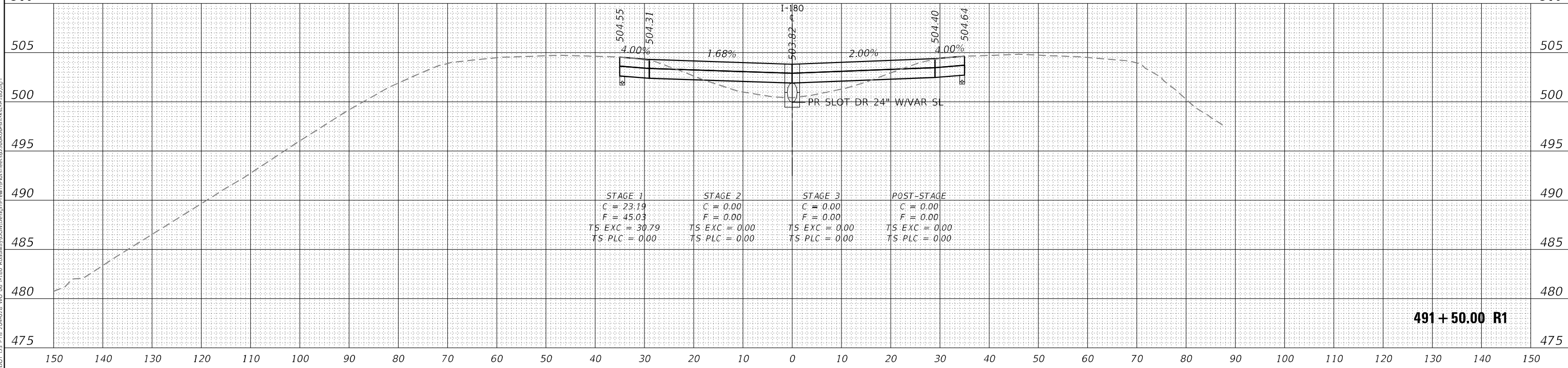
515 150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 515



492 + 00.00 R1

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

510 150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 510



491 + 50.00 R1

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

MODEL: Definit FILE NAME: 2/2/2024_06 DOT 03 P18 204+028 WD 06 I-180 Roadway/Civil/Design/Platm/PlatSheet/0356666-sh-c-e-l-180.dgn



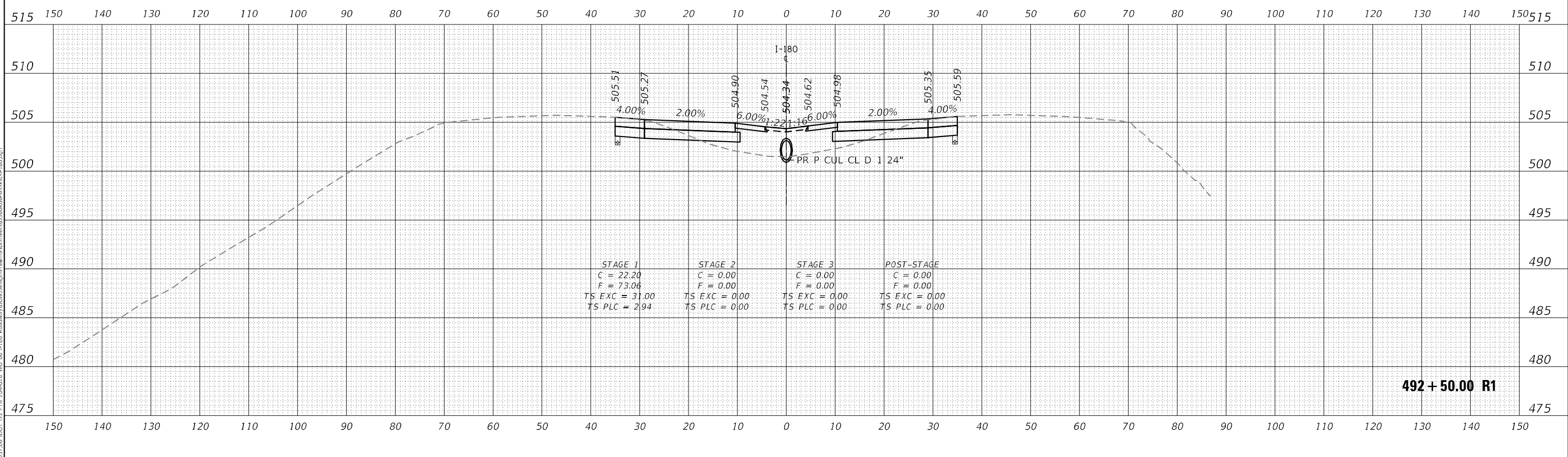
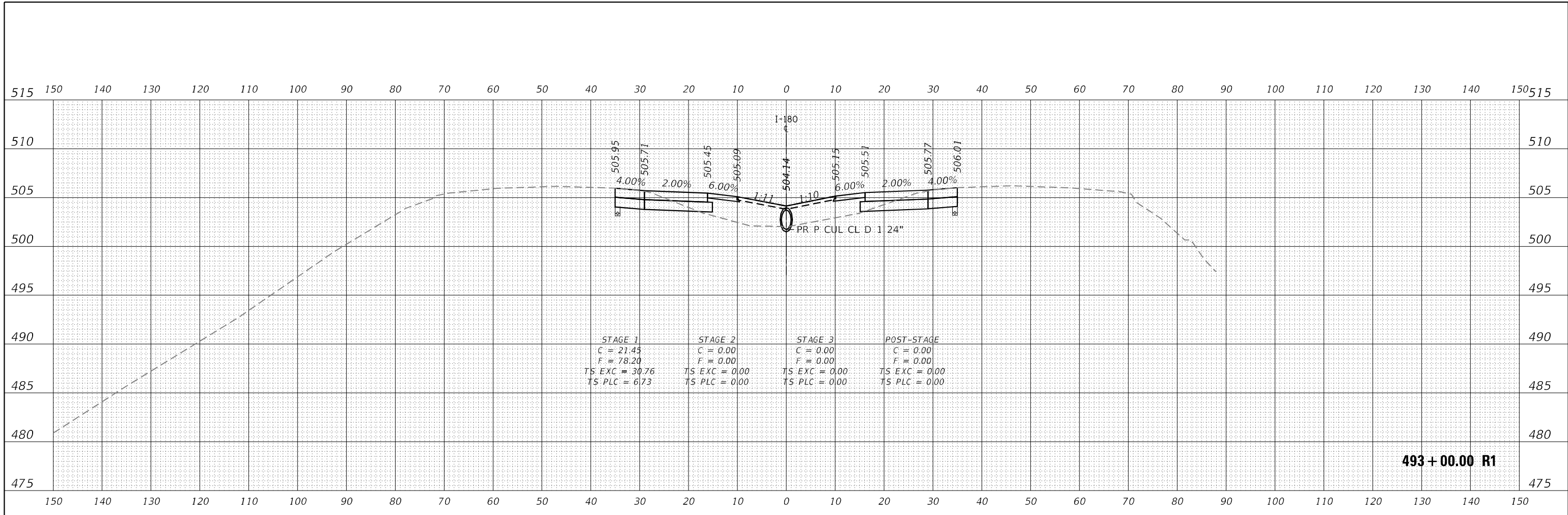
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	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 180 (I-180)
CROSS SECTIONS

SCALE: SHEET OF SHEETS STA. 491+50.00 R1 TO STA. 492+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	280
			CONTRACT NO. 66K66	
ILLINOIS FED. AID PROJECT				



MODEL: Definit
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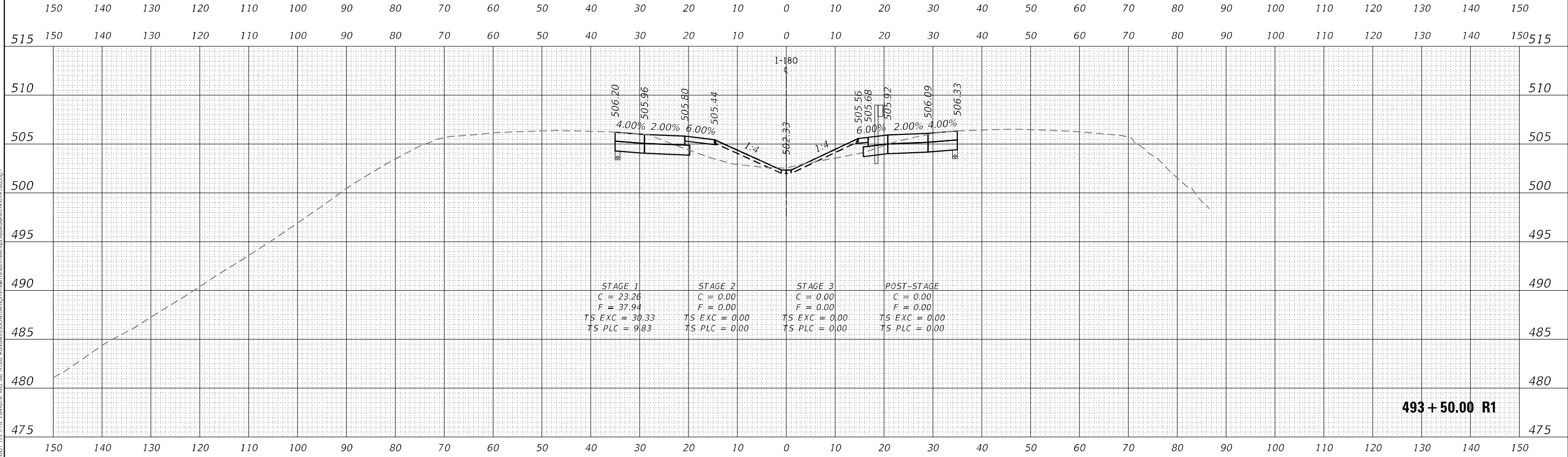
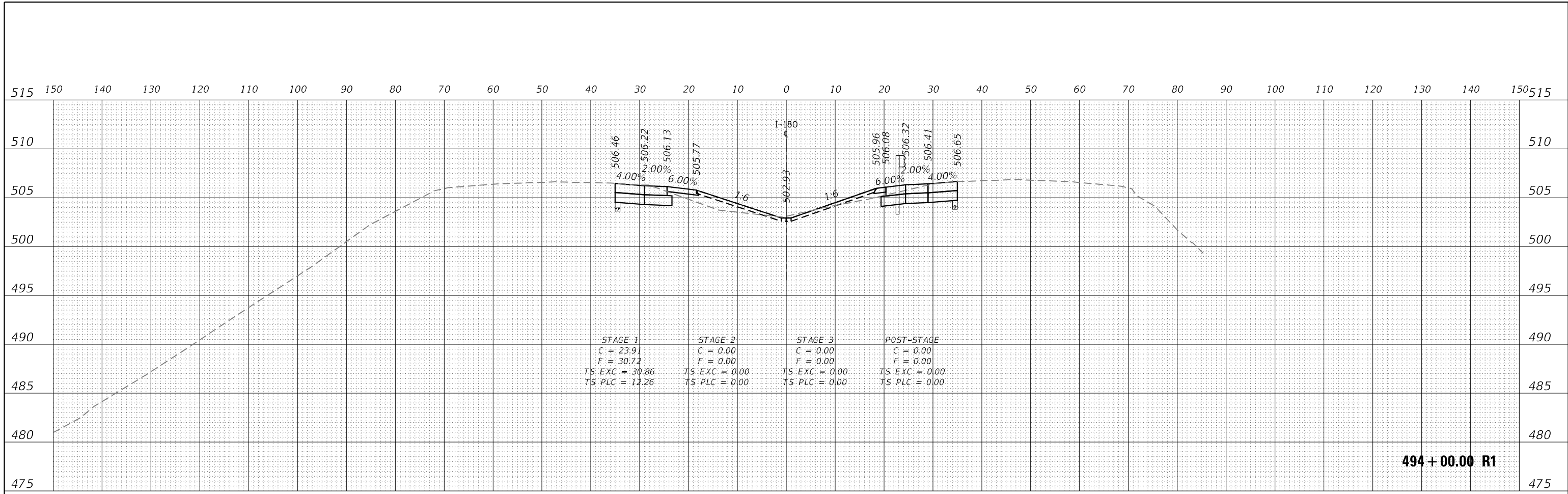
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	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 492+50.00 R1 TO STA. 493+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	281
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 DDT 03 P1B 204-028 WD 06 I-180 Roadway/00/01/Design/Plots/PlotSheet/03566666-ah-cc-1-180.dgn

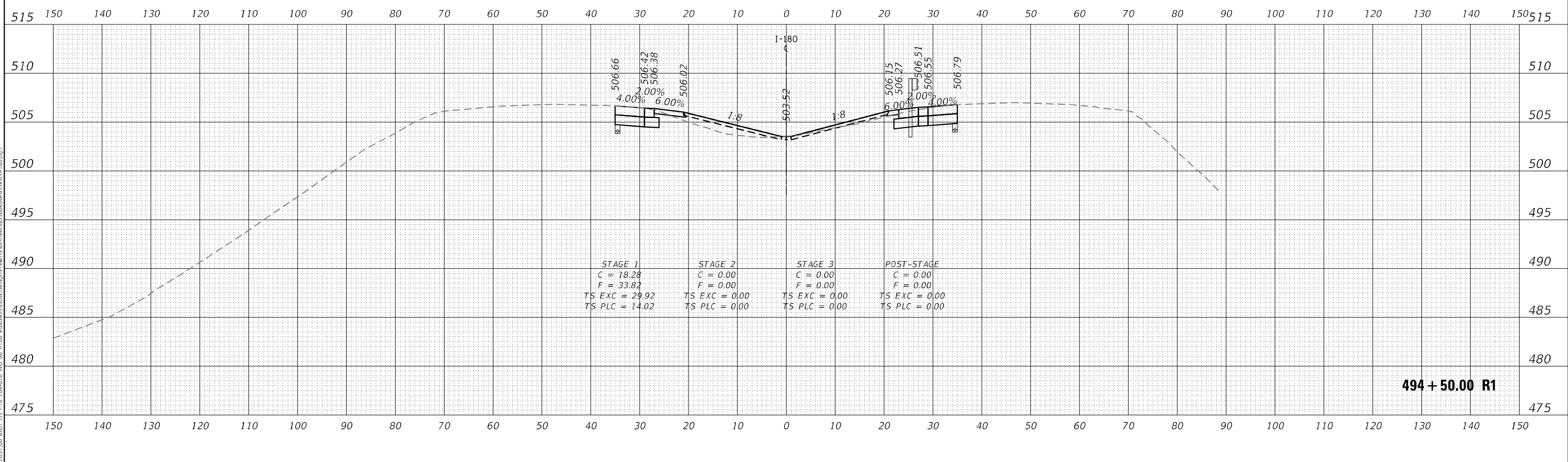
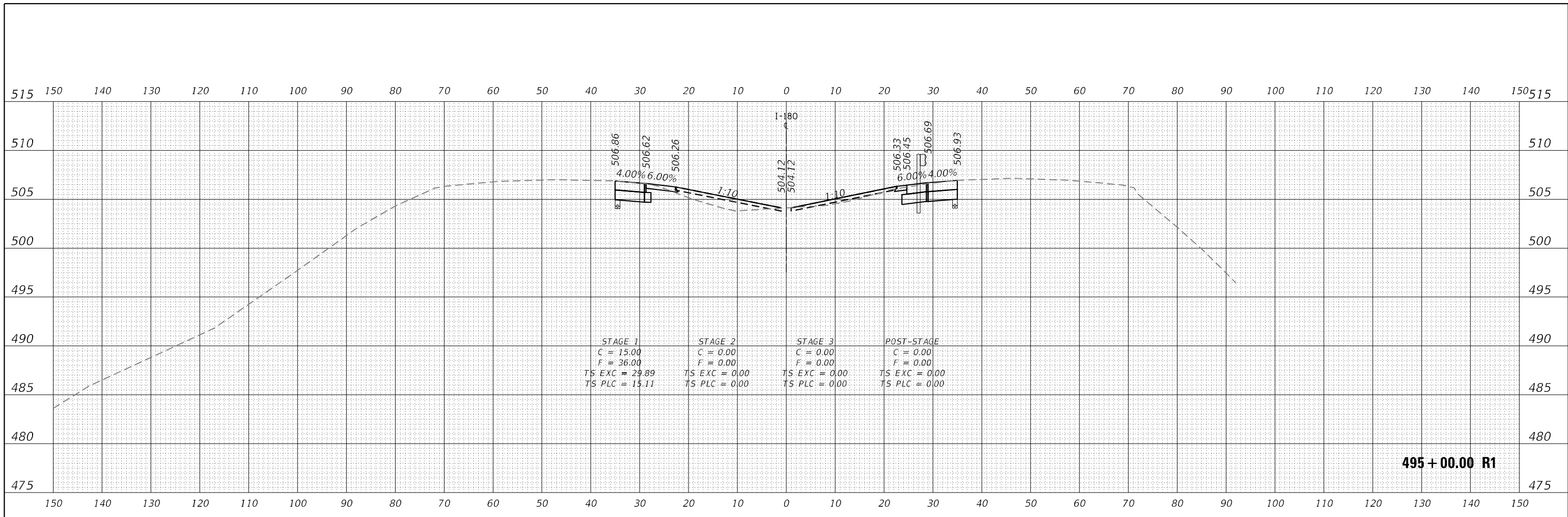


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	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

FAI ROUTE 180 (I-180) CROSS SECTIONS	
SCALE:	SHEET OF SHEETS STA. 493+50.00 R1 TO STA. 494+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	282
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 DDT 03 P1B 204-028 WD 06 I-180 Roadway/03/01/2024/Design/Plots/PlotSheet/03/28/2024-06-01-00.dgn



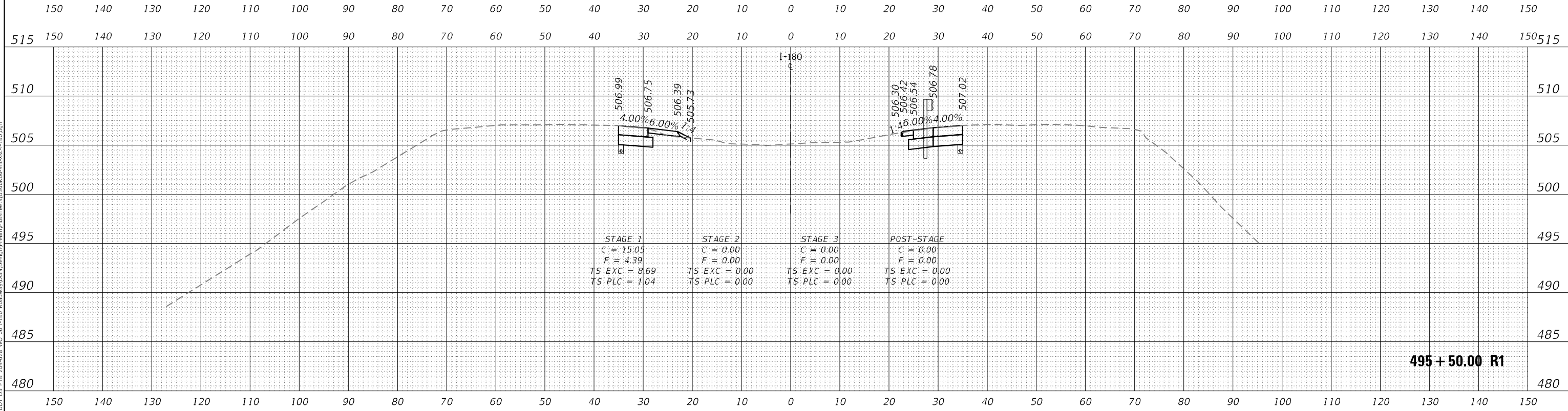
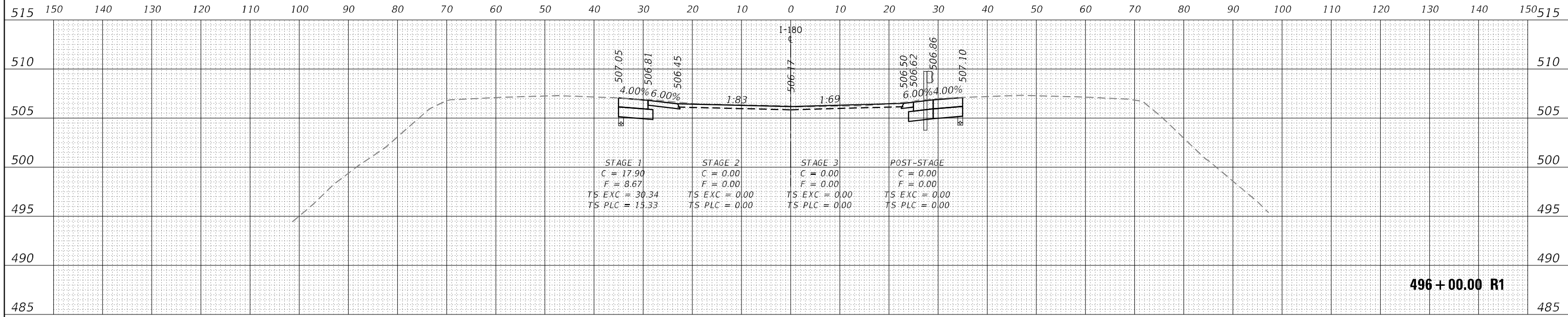
USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 494+50.00 R1 TO STA. 495+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	283
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definitive
 FILE NAME: 2/2/2017_06 DDT D3 P1B 204-02B W0 06 I-180 Roadway/Civil/Design/Prelim/PlotSheets/0366K66-sh-cs-1-180.dgn



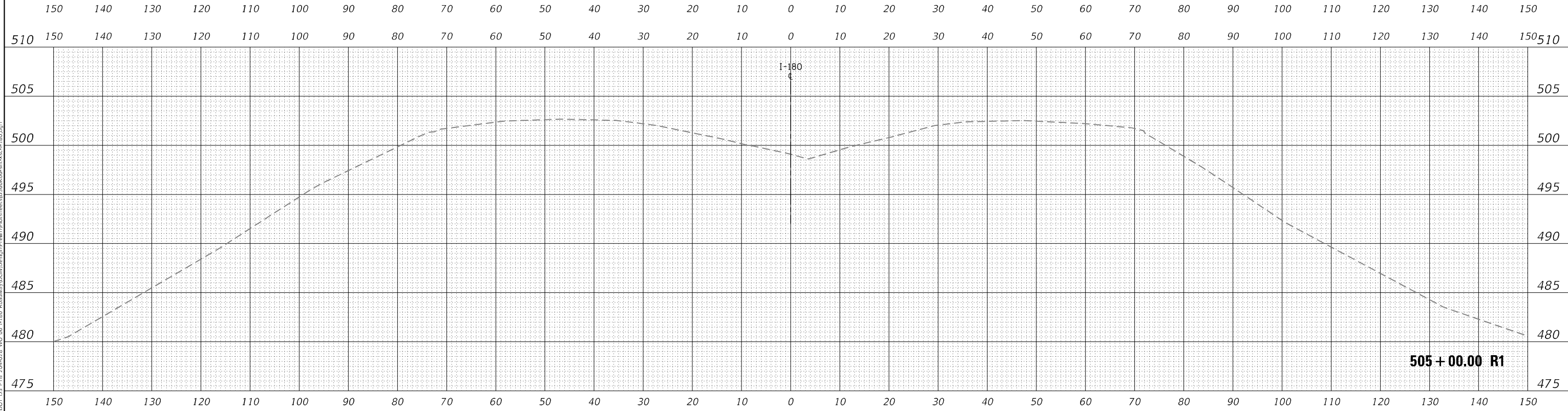
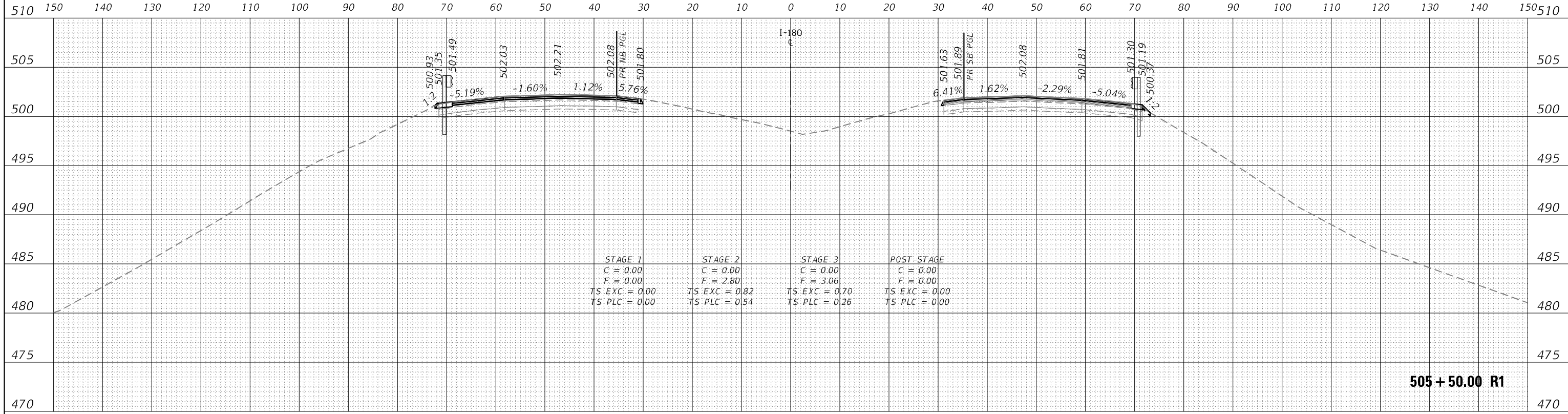
USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 495+50.00 R1 TO STA. 496+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	284
			CONTRACT NO. 66K66	
		ILLINOIS	FED. AID PROJECT	



MODEL: Definit
 FILE NAME: 2/2/2024_06 DDT 03 P18 204-028 WD 06 1-180 Roadway/Civil/Design/Plat/PlatSheet/0356K66-sh-c-e-l-180.dgn



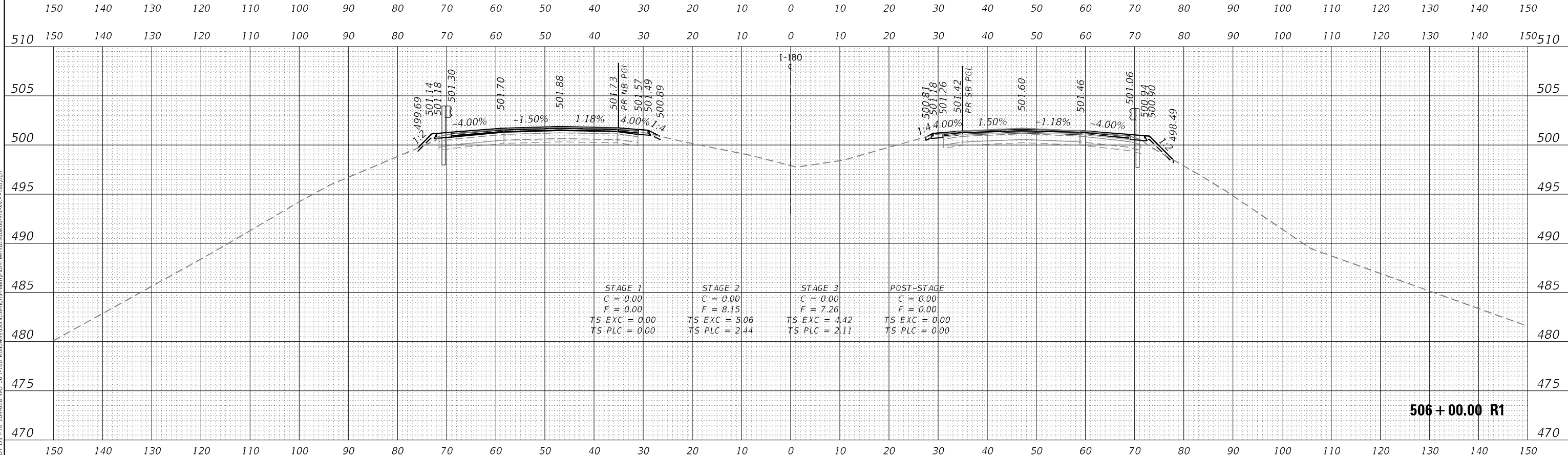
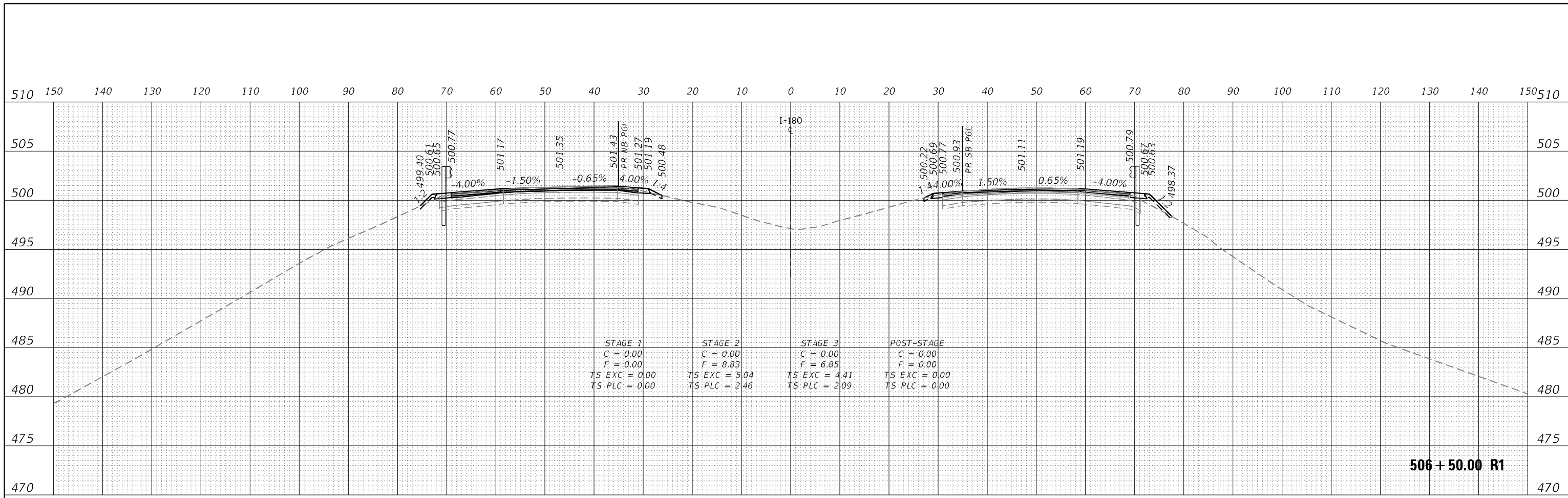
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	DRAWN -	REVISED -
PLOT SCALE = 20,0000 * / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 505+00.00 R1 TO STA. 505+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	285
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 DCT 03 P1B 204-028 WD 06 I-180 Roadway/Civil/Design/Plat/Plot/Sheet/0356K66-sh-c-e-l-180.dgn



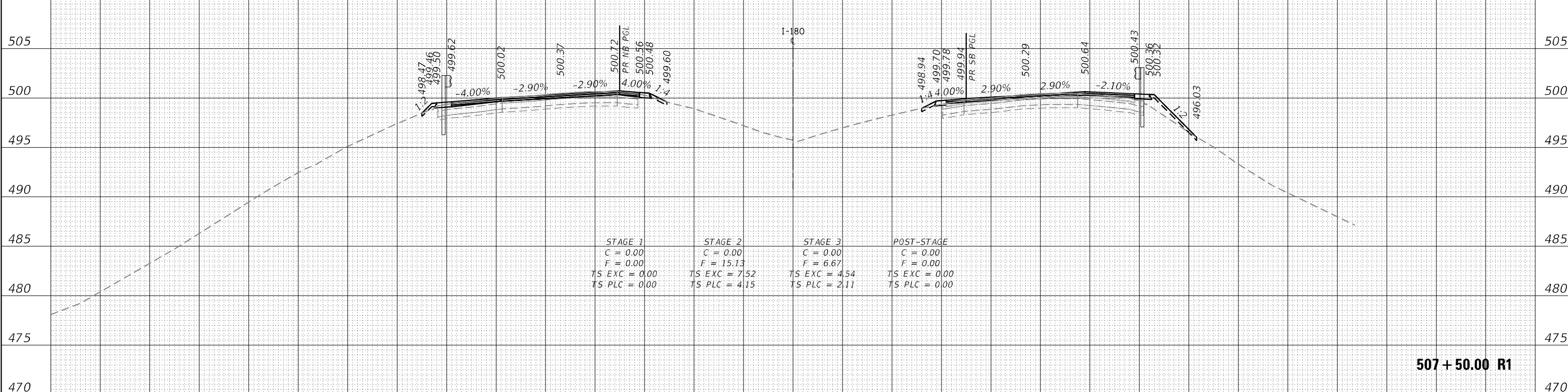
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	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

FAI ROUTE 180 (I-180) CROSS SECTIONS	
SCALE:	SHEET OF SHEETS
STA. 506+00.00 R1 TO STA. 506+50.00 R1	

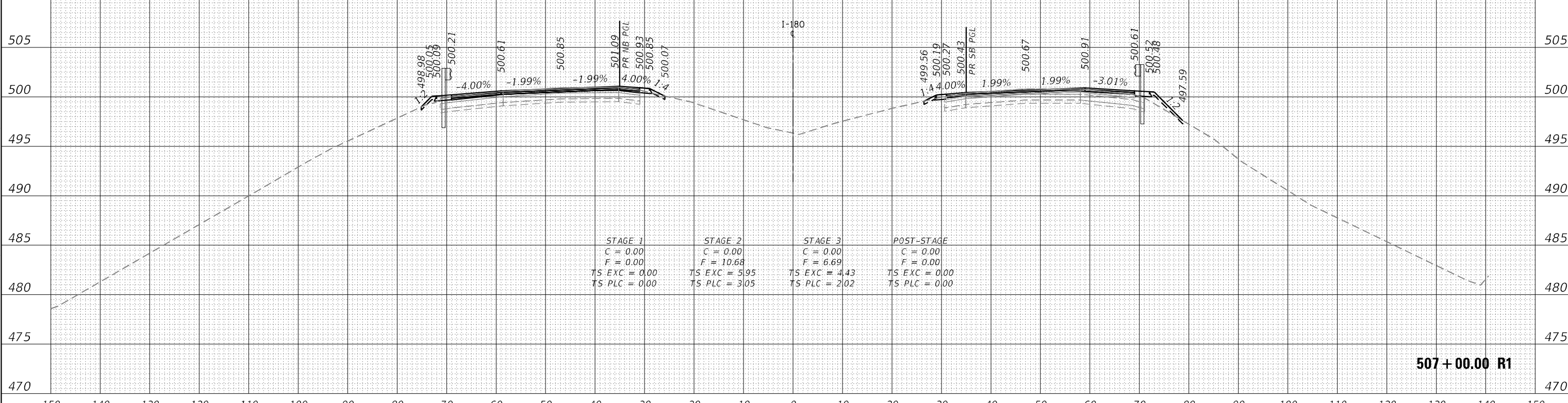
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	286
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				

510 150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 510



507 + 50.00 R1

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 510



507 + 00.00 R1

MODEL: Definitive; FILE NAME: 2/2/2024_06 DOT D3 P1B 204-028 WD 06 I-180 Roadway/DC/Util/Design/Profile/PlotSheet/03566666-sh-cs-e-l-180.dgn



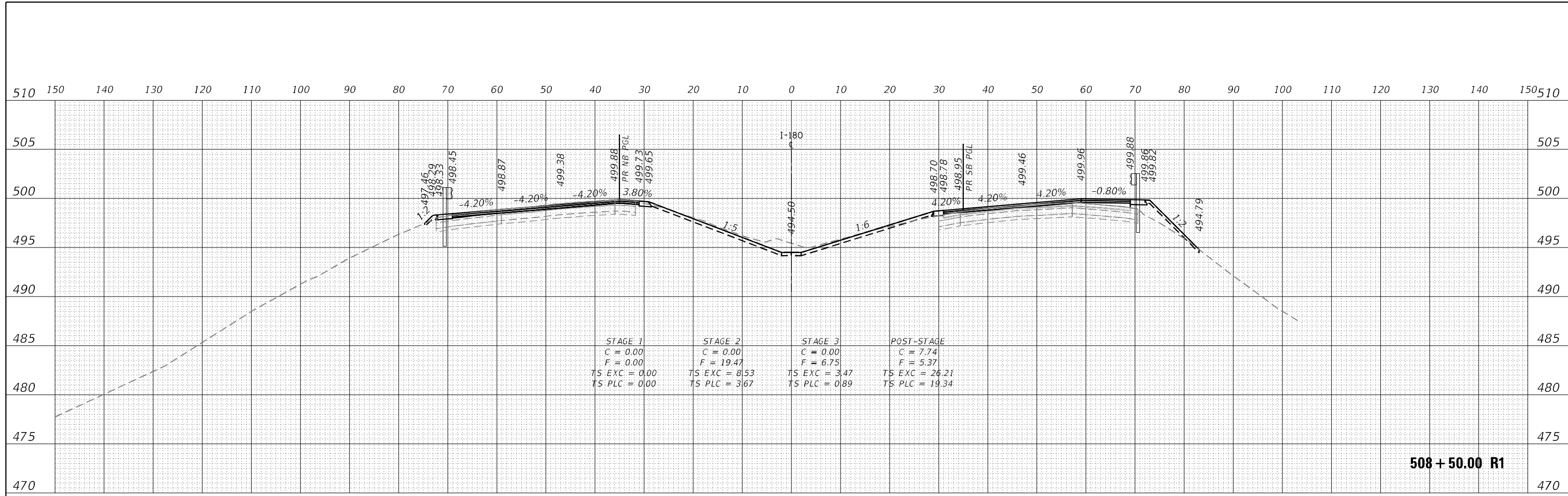
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PLOT SCALE = 20,0000' / in.	DRAWN -	REVISED -
PLOT DATE = 1/12/2024	CHECKED -	REVISED -
	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

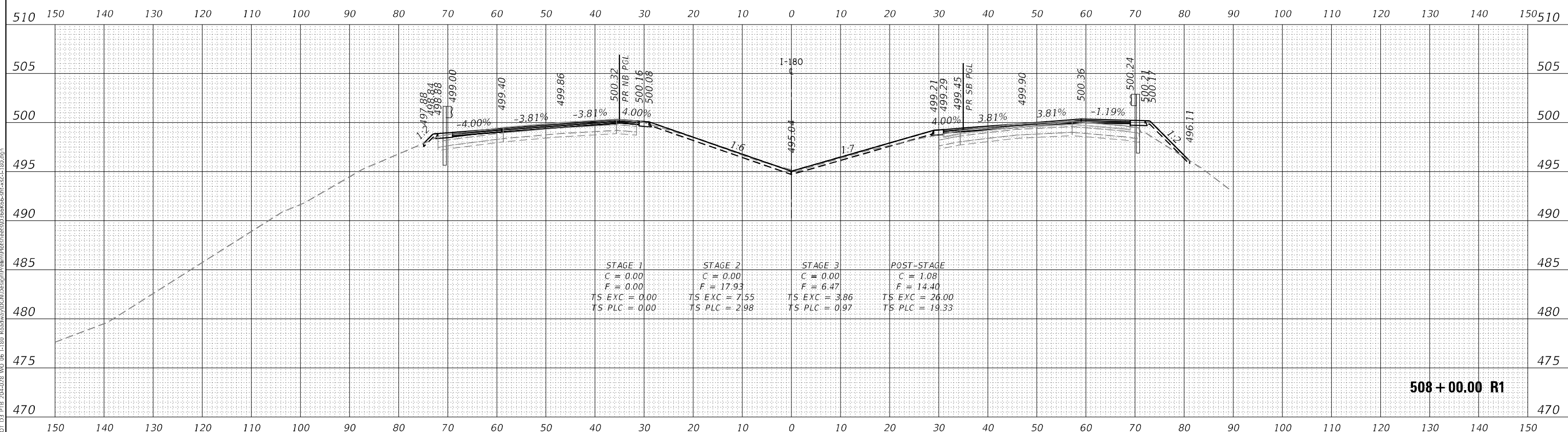
**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 507+00.00 R1 TO STA. 507+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	287
			CONTRACT NO. 66K66	
		ILLINOIS	FED. AID PROJECT	



508 + 50.00 R1



508 + 00.00 R1

MODEL: Definit FILE NAME: 2/2/2024 06:05 DOT 03 P1B 204-028 WD 06 I-180 Roadway/Civil/Design/Profile/PlotSheet/03586666-sh-cs-1-180.dgn



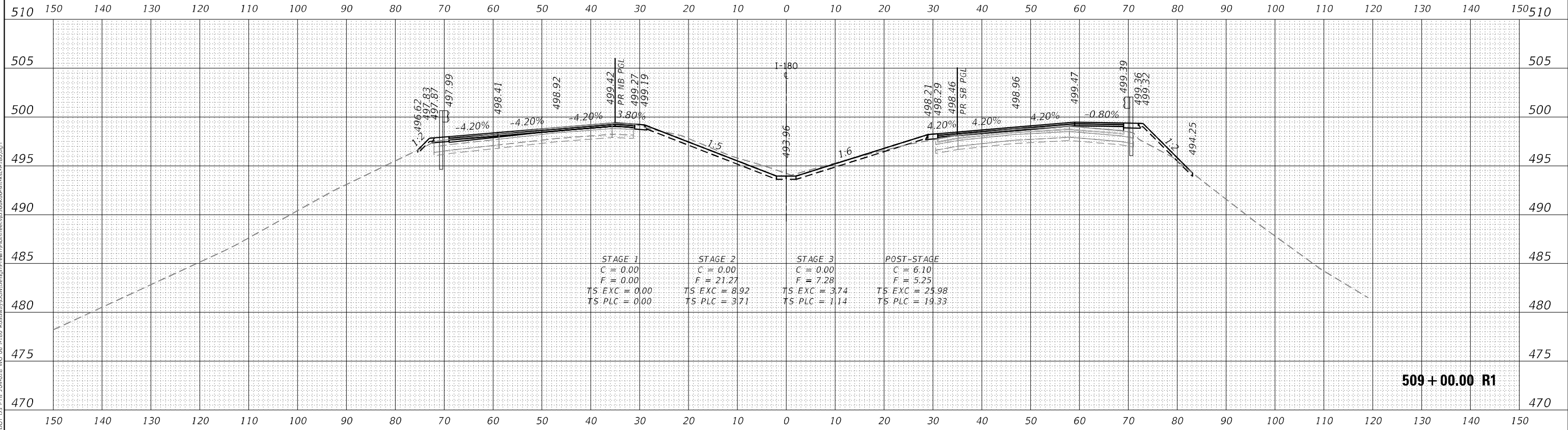
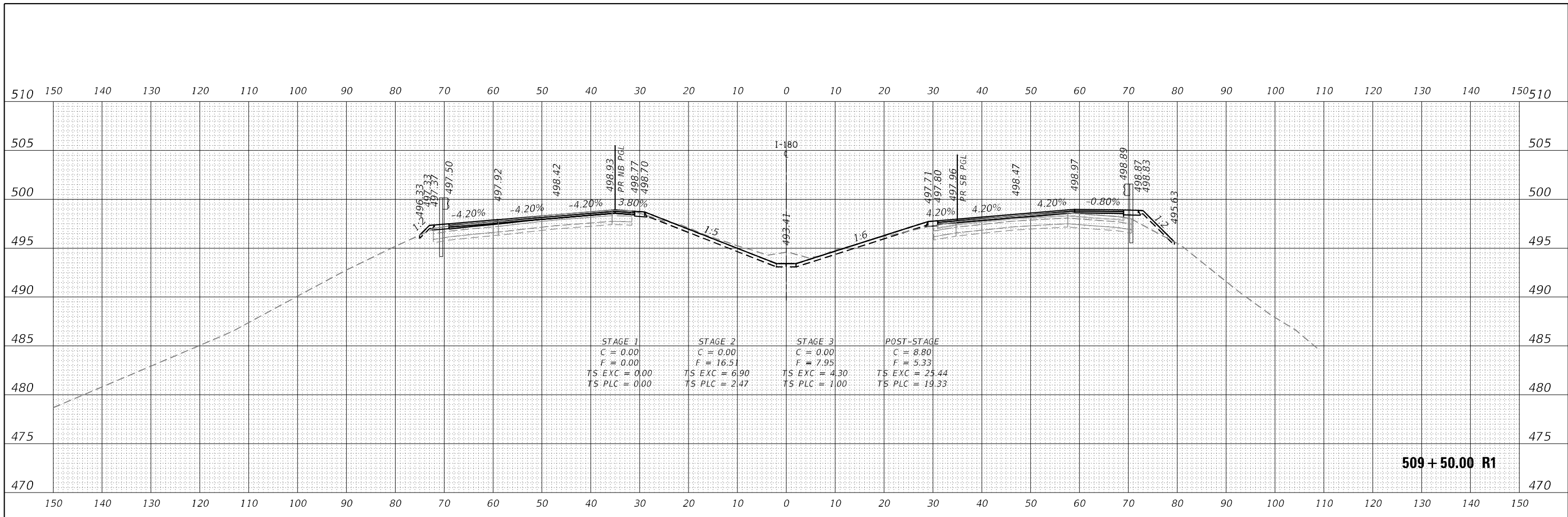
USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 180 (I-180)
CROSS SECTIONS

SCALE: SHEET OF SHEETS STA. 508+00.00 R1 TO STA. 508+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	288
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definitive
 FILE NAME: 2/2/2024_06 DICT 03 P1B 204-028 WD 06 I-180 Roadway/Civil/Design/Plat/PlatSheet/0356666-sh-c-e-l-180.dgn



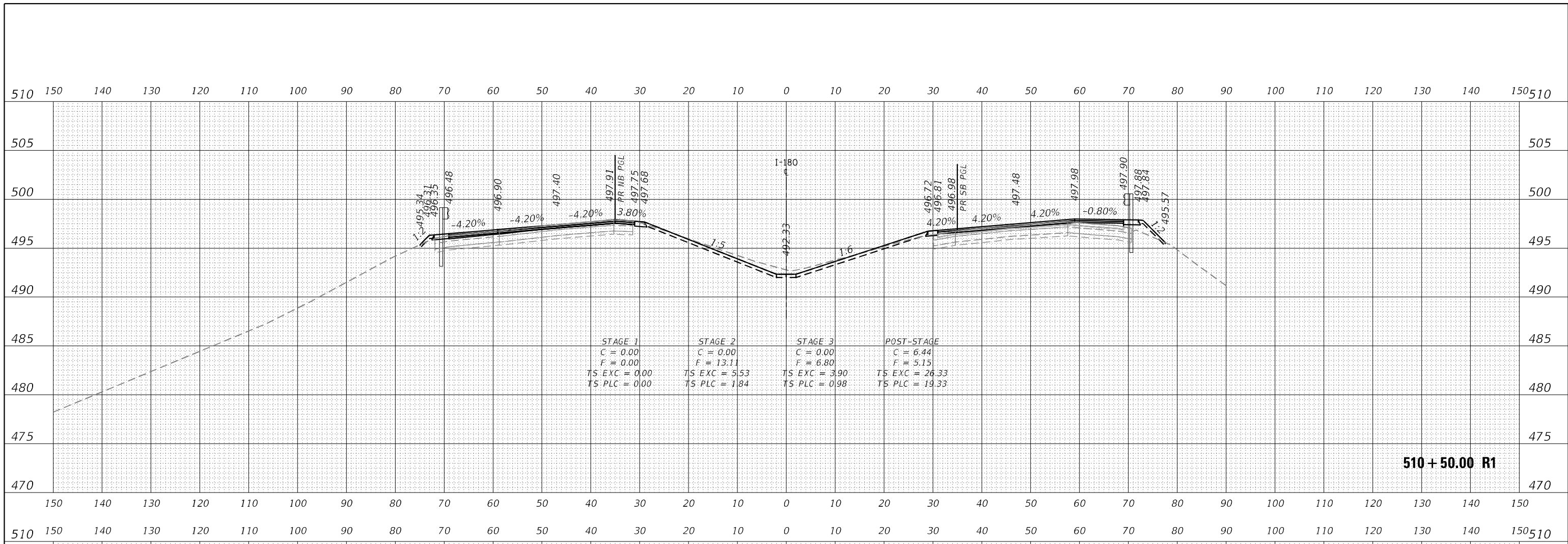
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	DRAWN -	REVISED -
PLOT SCALE = 20,000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 509+00.00 R1 TO STA. 509+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	289
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



STAGE 1	STAGE 2	STAGE 3	POST-STAGE
C = 0.00	C = 0.00	C = 0.00	C = 6.44
F = 0.00	F = 13.11	F = 6.80	F = 5.15
TS EXC = 0.00	TS EXC = 5.53	TS EXC = 3.90	TS EXC = 26.33
TS PLC = 0.00	TS PLC = 1.84	TS PLC = 0.98	TS PLC = 19.33

STAGE 1	STAGE 2	STAGE 3	POST-STAGE
C = 0.00	C = 0.00	C = 0.01	C = 6.96
F = 0.00	F = 14.96	F = 6.84	F = 7.46
TS EXC = 0.00	TS EXC = 6.30	TS EXC = 4.05	TS EXC = 26.09
TS PLC = 0.00	TS PLC = 2.23	TS PLC = 1.06	TS PLC = 19.33

MODEL: Definit
 FILE NAME: 2/2/2024_06 DCT 03 P18 204-028 WD 06 L180 Roadway/Civil/Design/Plat/Plot/Sheet/0356K66-sh-c-e-l-180.dgn



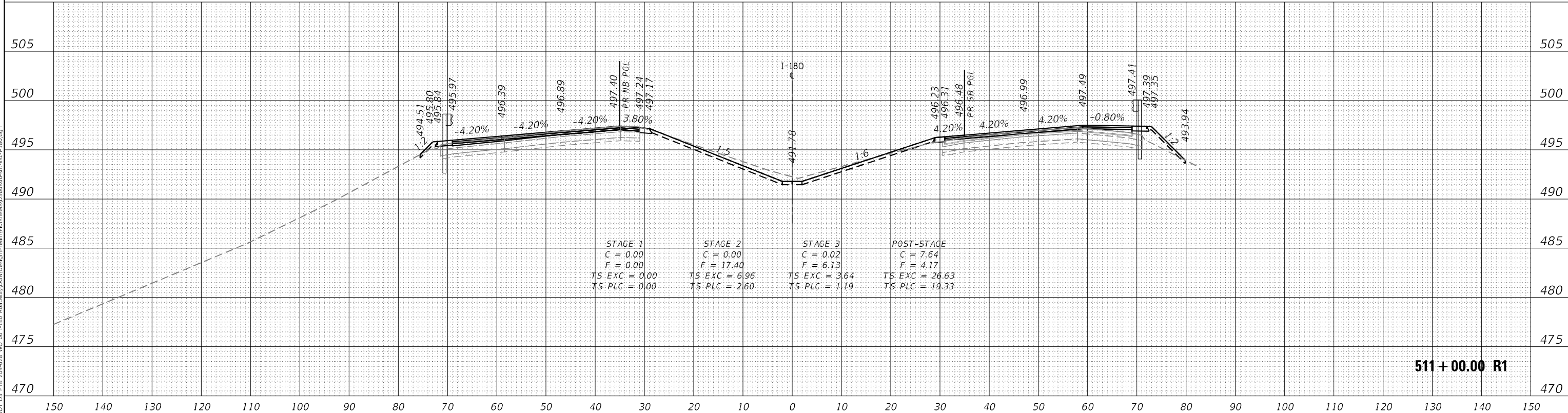
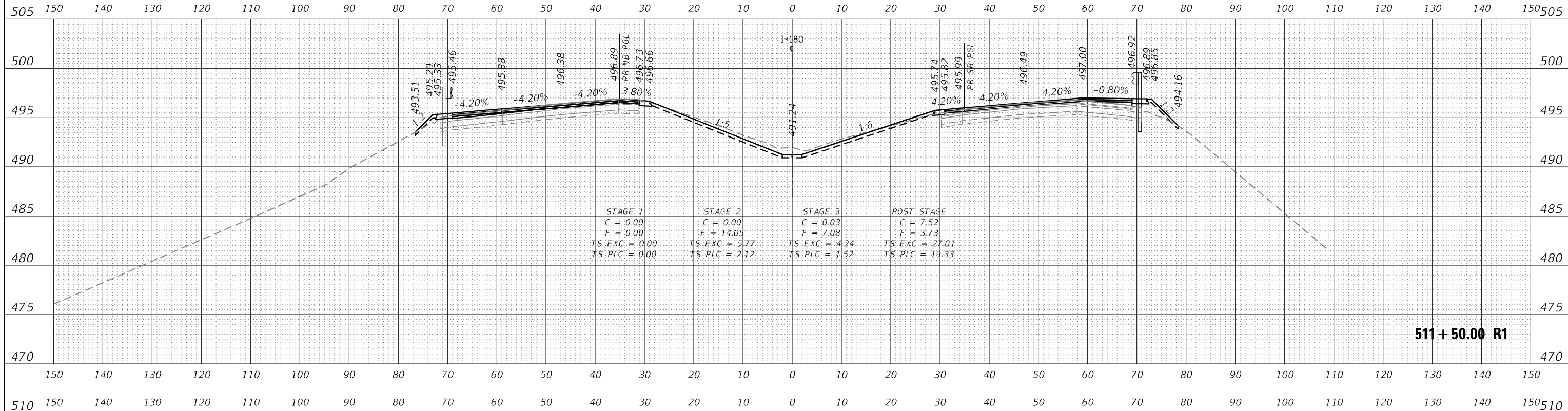
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	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 510+00.00 R1 TO STA. 510+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	290
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	



MODEL: Definitive
 FILE NAME: 2/2/2024_06 D0T D3 P1B 204-028 WD 06 L-180 Roadway/Civil/Design/Plat/Plot/Sheet/0356K66-sh-c-e-l-180.dgn



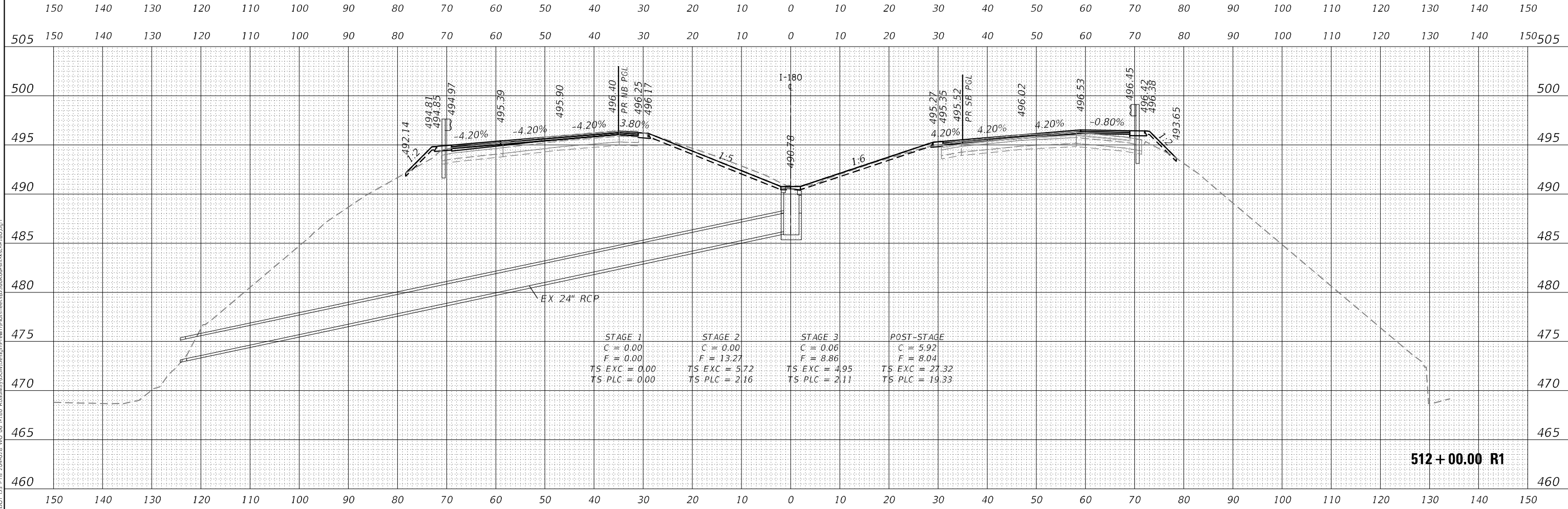
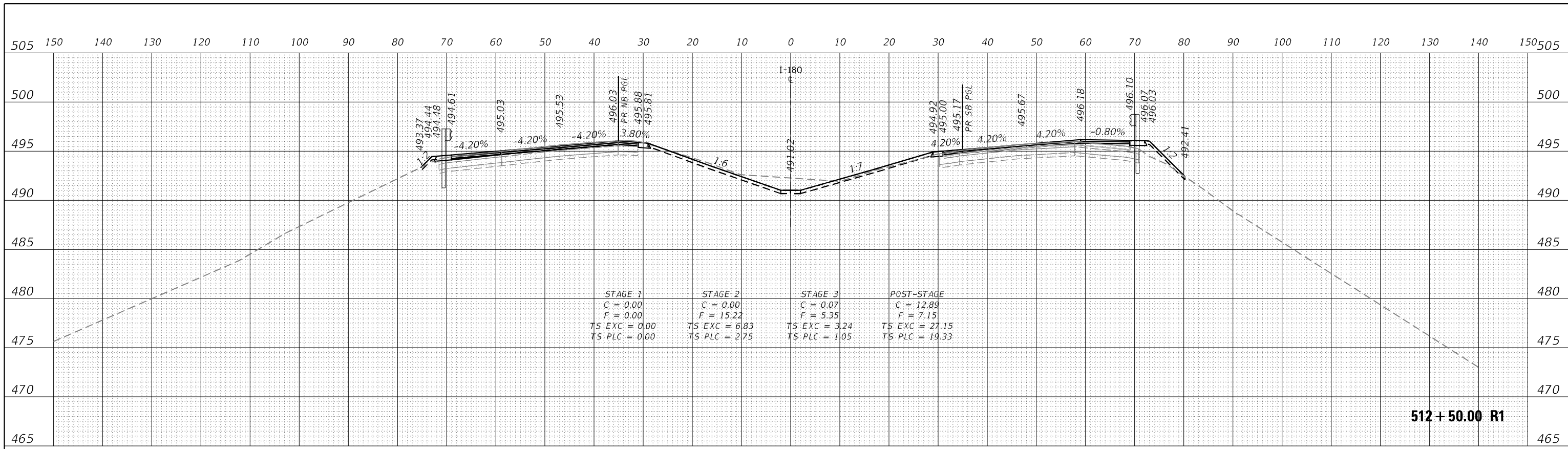
USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 511+00.00 R1 TO STA. 511+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	291
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 DICT 03 P1B 204-02B WD 06 I-180 Roadway/Civil/Design/Profile/PlotSheet/0356666-sh-cs-e-l-180.dgn



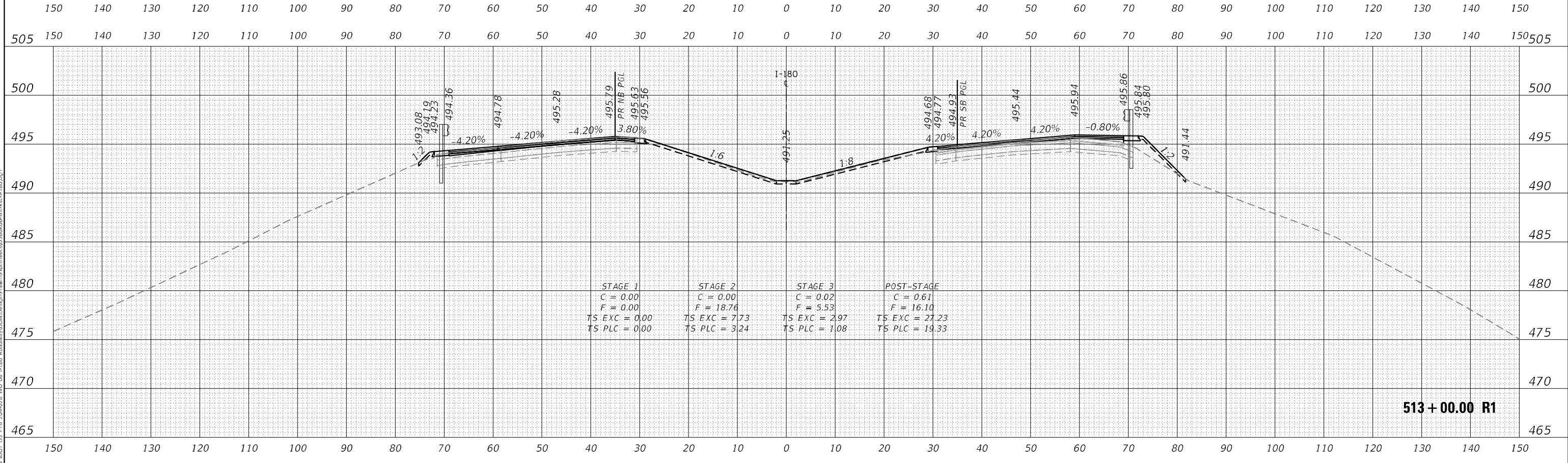
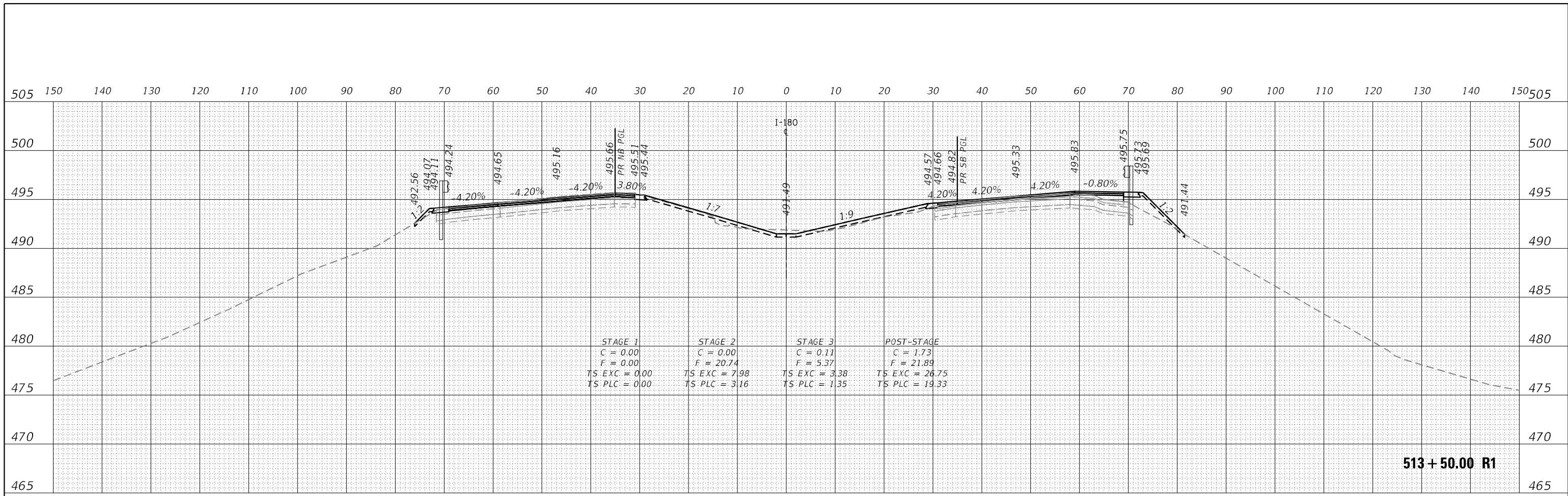
USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 512+00.00 R1 TO STA. 512+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	292
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	



MODEL: Definit
 FILE NAME: 2/2/2024_06 DDT 03 P1B 204-028 WD 06 I-180 Roadway/Civil/Design/Profile/PlotSheet/0356666-sh-c-e-l-180.dgn



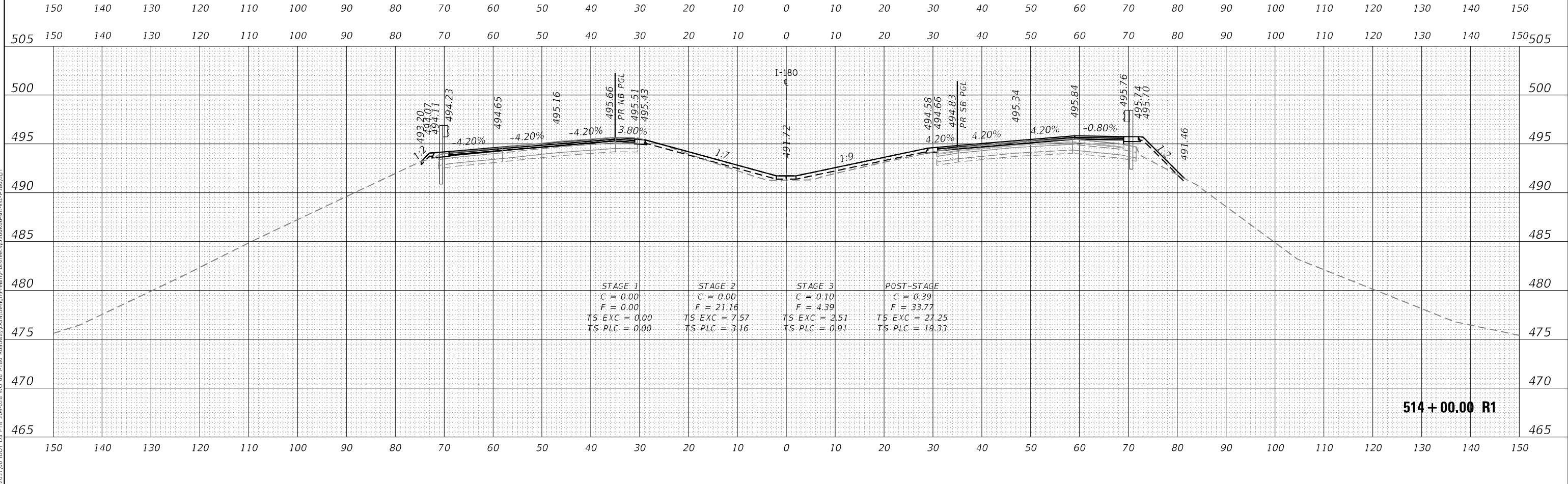
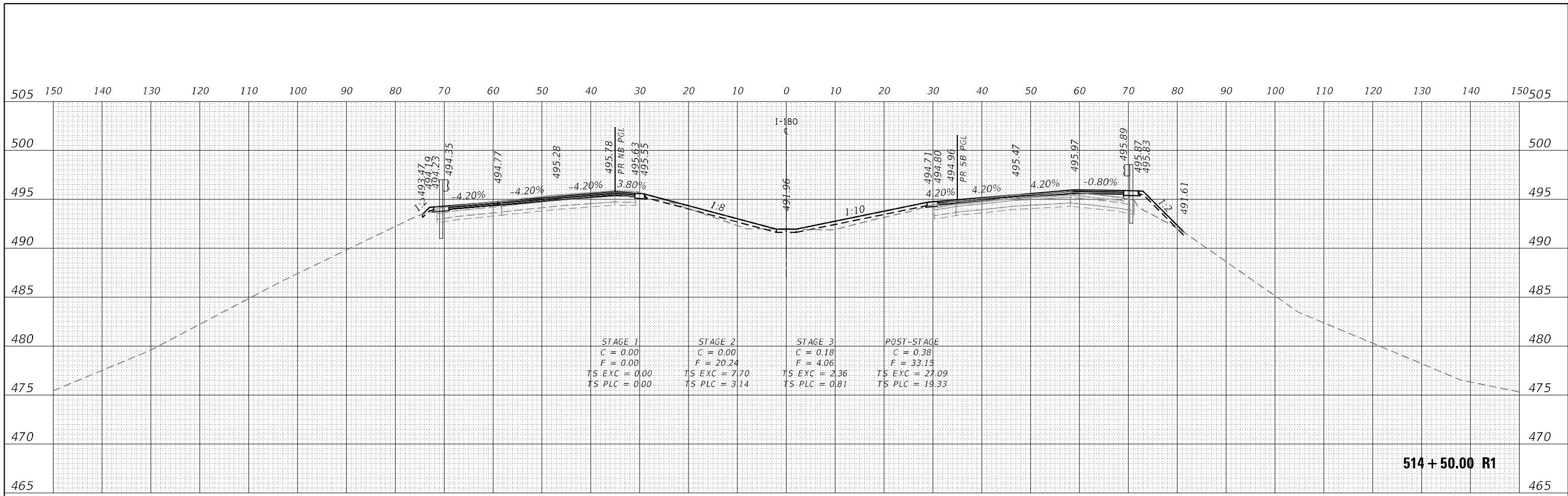
USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 513+00.00 R1 TO STA. 513+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	293
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 DICT 03 P1B 204-028 WD 06 I-180 Roadway/Civil/Design/Plat/PlotSheet/0356666-sh-c-e-l-180.dgn

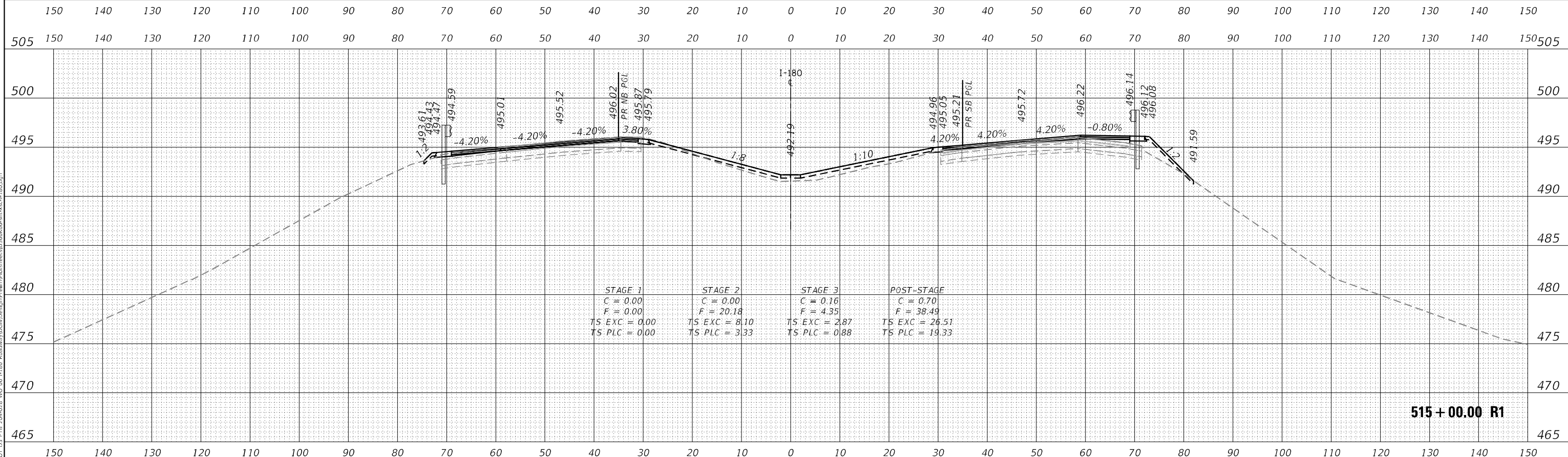
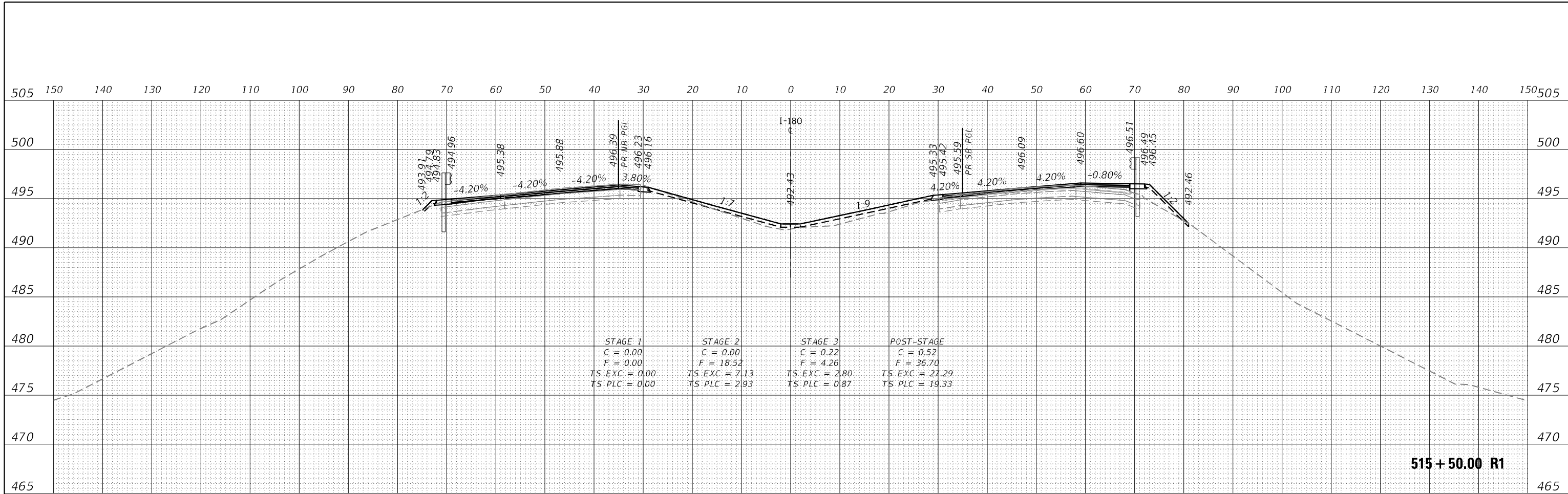


USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

FAI ROUTE 180 (I-180) CROSS SECTIONS	
SCALE:	SHEET OF SHEETS
STA. 514+00.00 R1 TO STA. 514+50.00 R1	

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	294
CONTRACT NO. 66K66				
ILLINOIS		FED. AID PROJECT		



MODEL: Definit
 FILE NAME: 2/2/2024_06 DICT 03 P18 204-028 WD 06 L-180 Roadway/Civil/Design/Plat/PlotSheet/03586666-sh-c-e-l-180.dgn



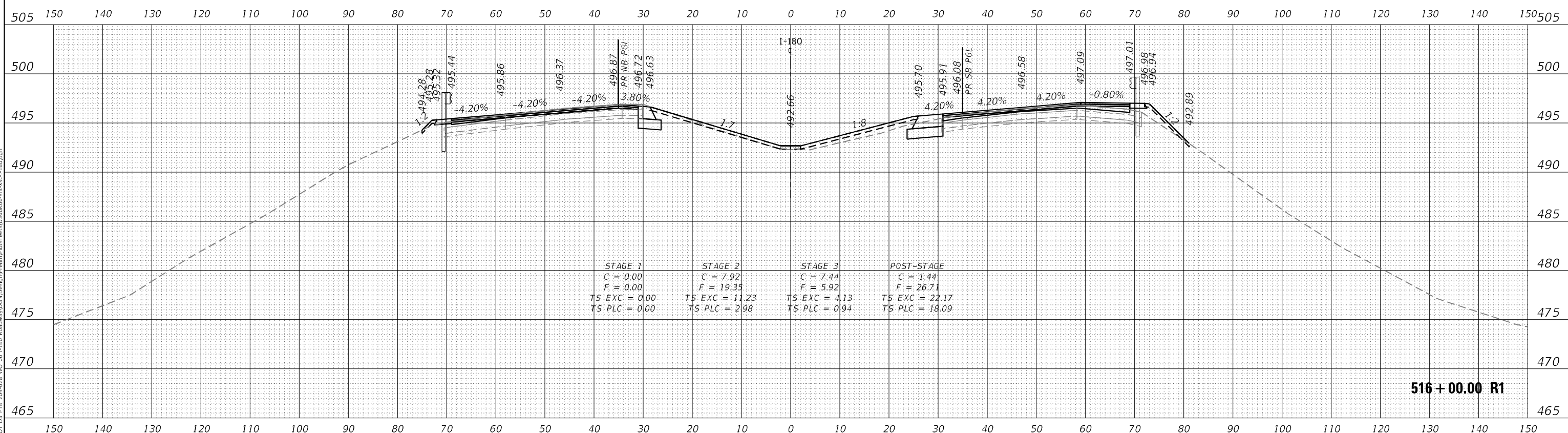
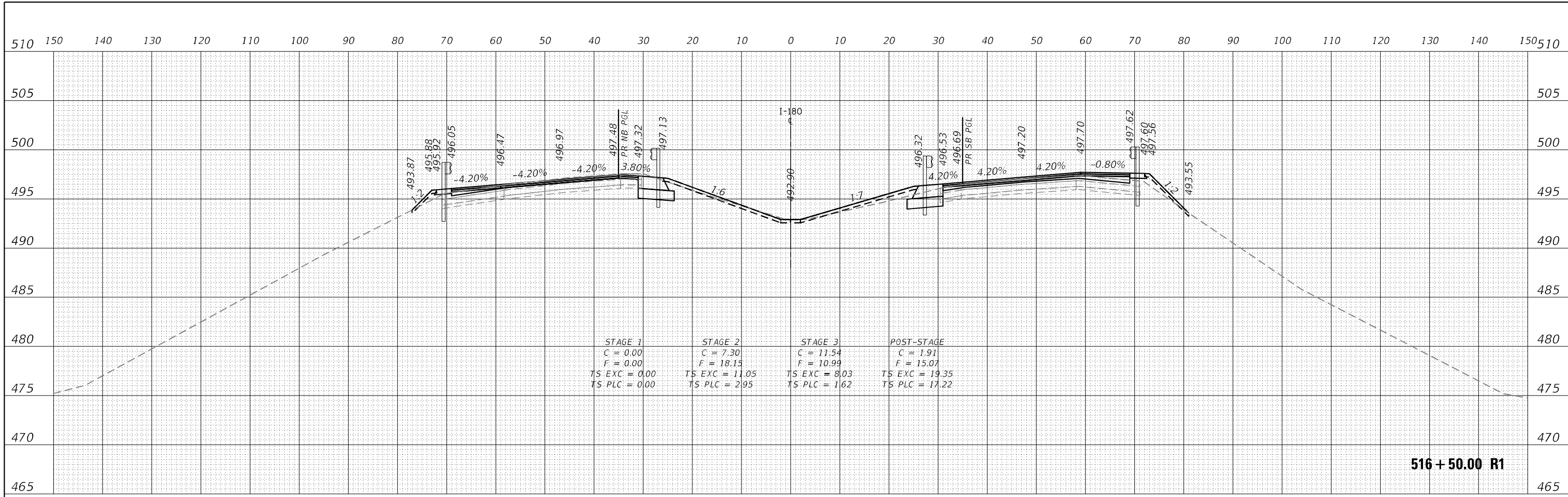
USER NAME = RGall	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 180 (I-180)
CROSS SECTIONS

SCALE: SHEET OF SHEETS STA. 515+00.00 R1 TO STA. 515+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	295
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 DICT 03 P1B 204-028 WD 06 L180 Roadway/DC/01/Design/Plat/PlatSheet/0356666-sh-c-e-l-180.dgn

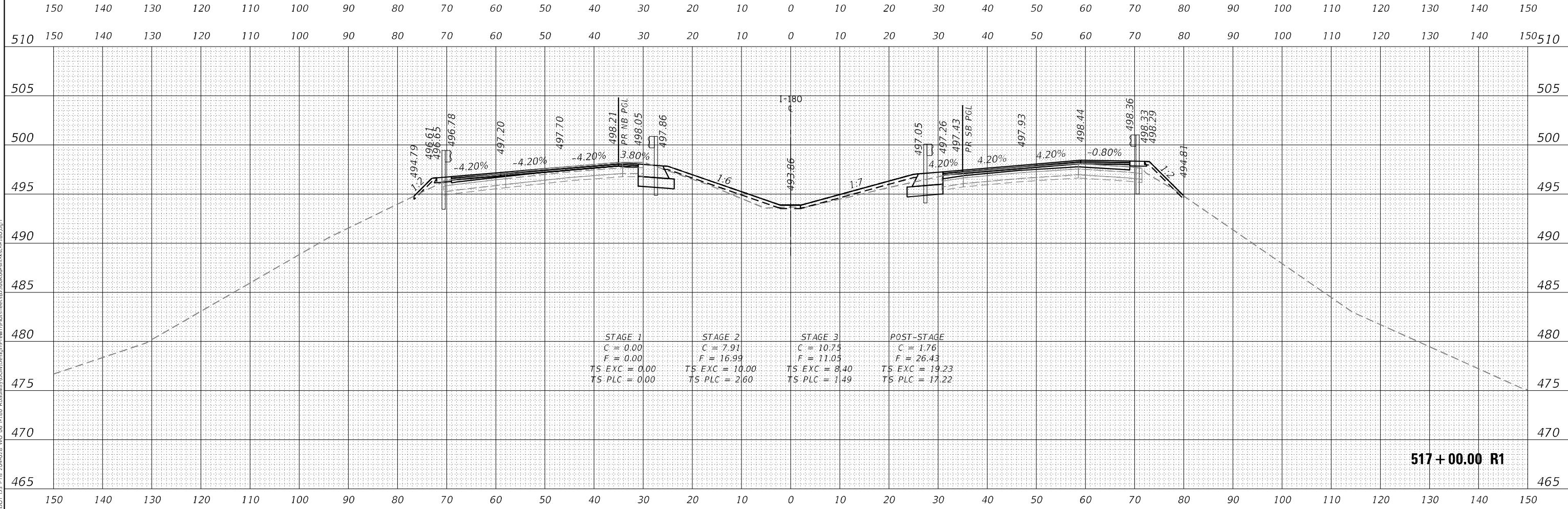
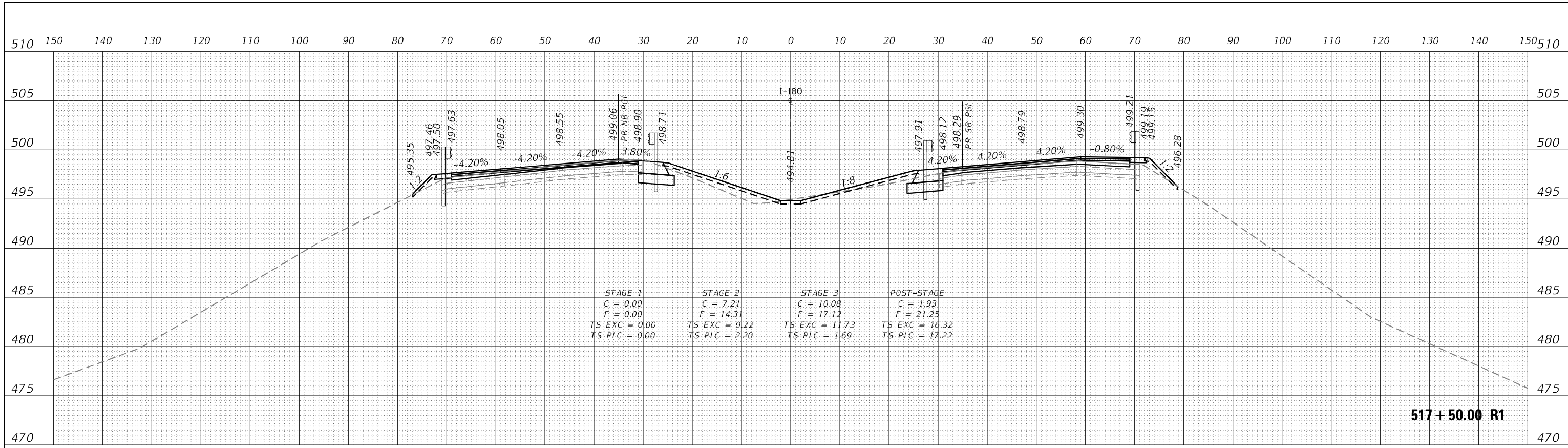


USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

FAI ROUTE 180 (I-180) CROSS SECTIONS			
SCALE:	SHEET	OF	SHEETS
STA. 516+00.00 R1 TO STA. 516+50.00 R1			

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	296
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definit
 FILE NAME: 2/2/2024_06 D0T D3 P1B 204+02B WD 06 L180 Roadway/Civil/Design/Plat/Plot/Sheet/03566666-ah-sec-L180.dgn



USER NAME = RGal
 PLOT SCALE = 20,0000' / in.
 PLOT DATE = 1/12/2024

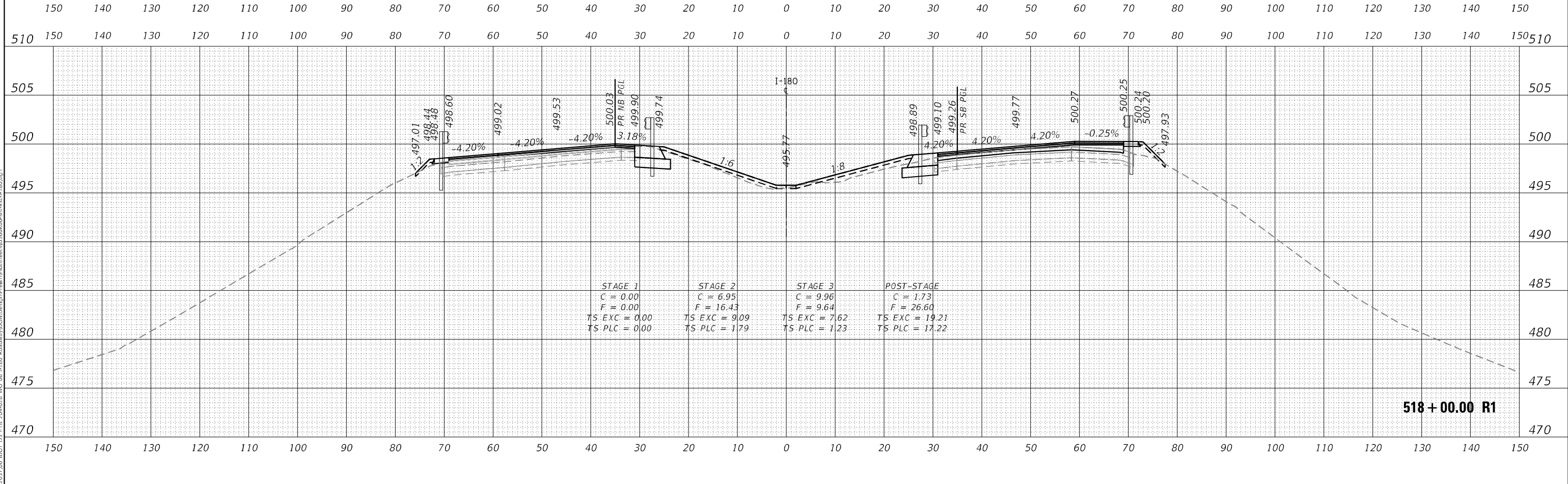
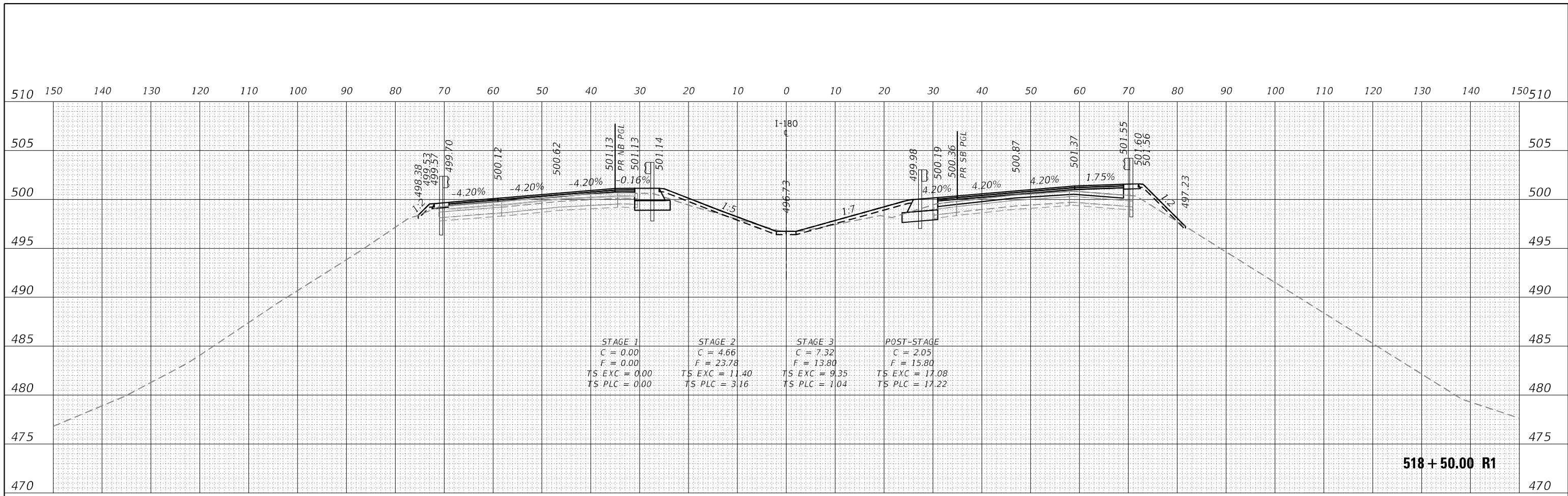
DESIGNED -	REVISED -
DRAWN -	REVISED -
CHECKED -	REVISED -
DATE -	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

SCALE: SHEET OF SHEETS STA. 517+00.00 R1 TO STA. 517+50.00 R1

**FAI ROUTE 180 (I-180)
 CROSS SECTIONS**

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	297
CONTRACT NO. 66K66				
ILLINOIS FED. AID PROJECT				



MODEL: Definitive
 FILE NAME: 2/2/2024_06 DOT 03 P1B 204-028 WD 06 I-180 Roadway/Civil/Design/Plat/PlatSheet/0356666-sh-c-e-l-180.dgn



USER NAME = RGal	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

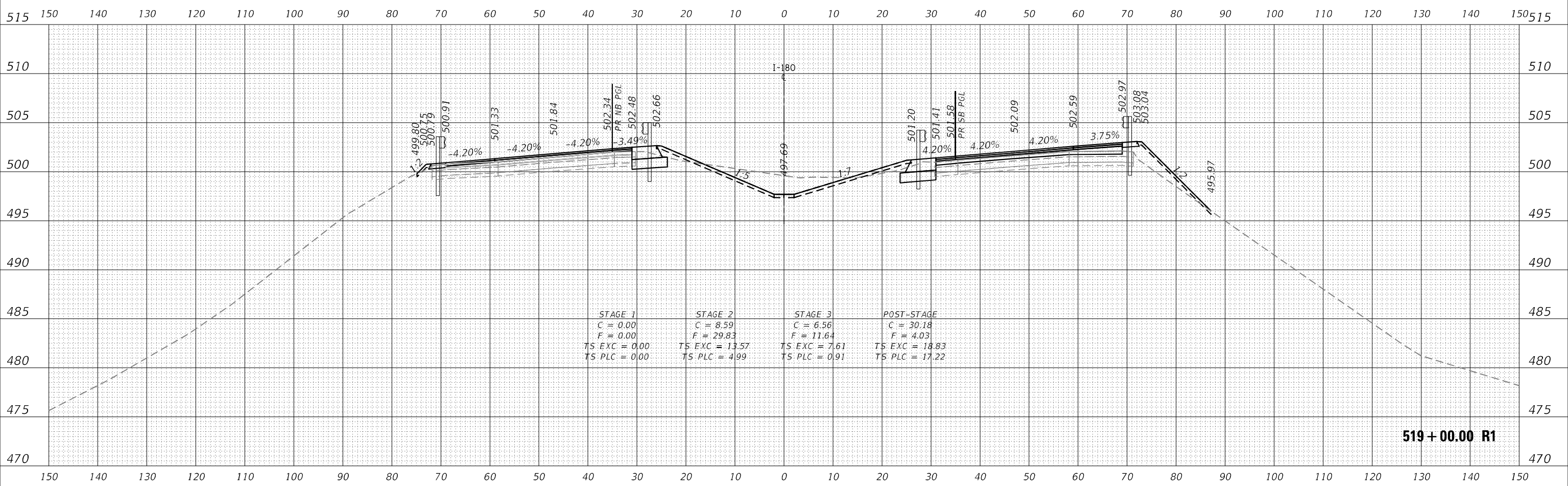
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 518+00.00 R1 TO STA. 518+50.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	298
CONTRACT NO. 66K66			ILLINOIS FED. AID PROJECT	

MODEL: Definit
 FILE NAME: 2/2/2024_06 D0T D3 P1B 204-028 WD 06 I-180 Roadway/Civil/Design/Plat/PlatSheet/0358666-sh-cs-e-l-180.dgn



519 + 00.00 R1



USER NAME = RGall	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

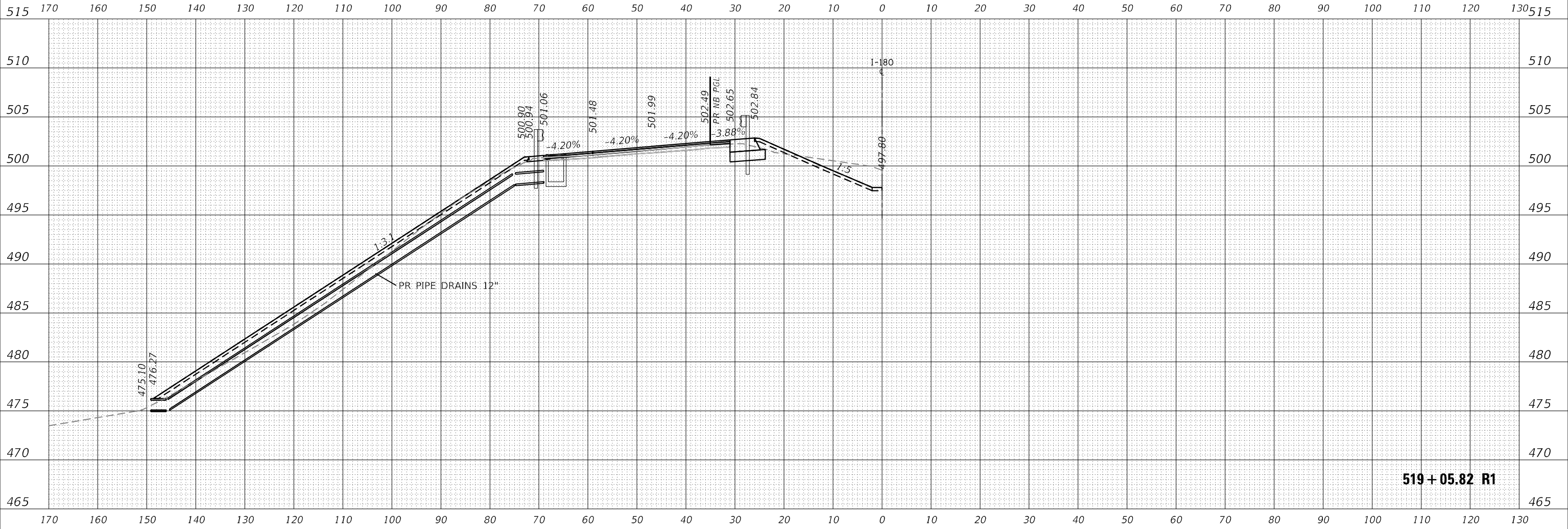
**FAI ROUTE 180 (I-180)
CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 519+00.00 R1 TO STA. 519+00.00 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	299
CONTRACT NO. 66K66				

ILLINOIS FED. AID PROJECT

MODEL: Definitive
 FILE NAME: 2/2/2024_06 DDT 03 P18 204-028 WD 06 I-180 Roadway/Civil/Design/Prallm/PlotSheet/03586666-sh-x-sec-1-180.dgn



519 + 05.82 R1

EFK Moen
 Civil Engineering Design

USER NAME = RGall	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 20,0000' / in.	CHECKED -	REVISED -
PLOT DATE = 1/12/2024	DATE -	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**FAI ROUTE 180 (I-180)
 CROSS SECTIONS**

SCALE: SHEET OF SHEETS STA. 519+05.82 R1 TO STA. 519+05.82 R1

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
180	[(06-2B-1) & (06-2HB-1)]BR	BUREAU	327	300
			CONTRACT NO. 66K66	
ILLINOIS FED. AID PROJECT				