

June 14, 2005

SUBJECT: FAI Route 74 Project ACIM-ACBRI-074-4 (233) 089 Section (72-7) R-3 Peoria County Contract No. 68200 Item No. 1P, June 17, 2005 Letting Addendum B

NOTICE TO PROSPECTIVE BIDDERS:

Attached is an addendum to the plans or proposal. This addendum involves revised and/or added material.

- 1. Revised page vii of the Table of Contents to the Special Provisions.
- 2. Revised pages 79 83 of the Special Provisions.
- 3. Added page 346 to the Special Provisions.

Prime contractors must utilize the enclosed material when preparing their bid and must include any Schedule of Prices changes in their bidding proposal.

Bidders using computer-generated bids are cautioned to reflect any and all Schedule of Prices changes, if involved, into their computer programs.

Very truly yours,

Michael L. Hine Engineer of Design and Environment

Tet Dalucheyou AE.

By: Ted B. Walschleger, P. E. Engineer of Project Management

cc: J. E. Crowe, Region 3, District 4; N. R. Stoner; Roger Driskell; R. E. Anderson; Jim White; Design & Environment File

TBW:MS:jc

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EXCAVATION FOR STRUCTURES	
	Revised 06-14-2005

<u>Basis of Payment</u>. This work will be paid for at the contract unit price per cubic meter (cubic yard) for TRENCH BACKFILL, SPECIAL

## **GEOTECHNICAL FABRIC FOR GROUND STABILIZATION**

Effective March 1, 2002

The fabric shall be a woven geotextile fabric conforming to Article 1080.02 of the Standard Specifications.

# SUB-BASE GRANULAR MATERIAL, TYPE A AND SUB-BASE GRANULAR MATERIAL, TYPE B

Effective October 15, 2001 Revised June 10, 2002

This work shall consist of the construction of Sub-Base Granular Material, Type A or Sub-Base Granular Material, Type B of depths specified in the plans in accordance with applicable portions of Section 311 of the Standard Specifications with the following exception: The three-day drying period specified in Article 301.03(b) will be waived under the conditions outlined herein.

To avoid project delays due to wet subgrade, the Engineer will determine the depth of removal and replacement prior to excavation of the work area. In addition, the Contractor shall only excavate in one day the same area, which can be replaced with Sub-Base Granular Material, to finish grade, in the same day.

This work will be paid for at the contract unit price per metric ton for SUB-BASE GRANULAR MATERIAL, TYPE A or SUB-BASE GRANULAR MATERIAL, TYPE B for which the price shall be payment in full for all material, labor and equipment necessary to complete the work.

## EXISTING BITUMINOUS MIXES CONTAINING STEEL SLAG

Effective April 21, 2004

The Contractor is reminded to verify the existence of bituminous surface course mixes that may contain steel slag prior to removal and recycling. Much of the bituminous to be milled or otherwise removed within the project limits contains steel slag. The use of RAP containing steel slag shall be in accordance with the Standard Specifications and Special Provisions.

## **EXTENDED LIFE PAVEMENT (30 YEAR)**

Effective June 21, 2001

Revised June 14, 2005

requirements of this special provision are only applicable to I-74 mainline and ramp Portland Cement Concrete pavements.

<u>Description</u>. This work shall consist of constructing an extended life Portland cement concrete pavement, shoulders, and gutter, curb, and median, according to Section 420 for Portland Cement Concrete Pavement (PCCP), Section 421 for Continuously Reinforced Portland Cement Concrete Pavement (CRCP), Section 483 for Portland Cement Concrete Shoulders, and Section 606 for Concrete Gutter, Curb, Median, and Paved Ditch, of the Standard Specifications for Road and Bridge Construction, except as follows:

#### Definitions.

Aggregate Subbase – The aggregate above the subgrade and below the aggregate subbase cap.

Aggregate Subbase Cap – The 75 mm (3 in.) of aggregate above the aggregate subbase and below the base.

Base – The Superpave IL-19.0L placed over the aggregate subbase cap and immediately below the pavement.

<u>Materials</u>. Materials shall be according to Article 420.02 for PCCP, 421.02 for CRCP, and 483.02 for PCC Shoulders, of the Standard Specifications except:

The freeze-thaw rating expansion limit for coarse aggregate shall be a maximum 0.040 percent according to Illinois Modified AASHTO T 161, Procedure B.

<u>Equipment</u>. Equipment shall be according to Article 420.03 for PCCP, 421.03 for CRCP, and 483.03 for PCC Shoulders, of the Standard Specifications, except:

The Contractor shall submit to the Engineer, for approval before paving, the proposed internal type vibrator spacing for the paver. The Contractor shall also provide the proposed operating frequencies for a paving speed greater than or equal to 0.9 m (3 ft.) per minute, and for a paving speed less than 0.9 m (3 ft.) per minute.

<u>Base</u>. The base shall be constructed according to Section 312 of the Standard Specification, except that the material used shall be Superpave IL-19.0L.

When the surface temperature, as measured on the surface with a device as approved by the Engineer, of the Stabilized Sub-Base is 115 °F or greater the Contractor shall spray the Stabilized Sub-base with a water mist with equipment that meets the approval of the Engineer. The Stabilized Sub-base shall be cooled below 115 °F prior to paving on top. The water spray shall not produce excessive water runoff r leave puddles on the Stabilized Sub-base at the time of paving. All cooling shall be completer a minimum of 10 minutes prior to paving. The surface temperature shall be monitored during the paving operation to determine in the stabilized Sub-base required re-spaying. The water used shall meet the requirements of Section 1002.

<u>Embankment</u>. The embankment shall be constructed according to Section 205 of the Standard Specifications, except that the embankment shall be compacted to not less than 95 percent of the maximum dry density determined according to AASHTO T 99. The embankment shall not Revised 06-14-2005

be compacted at a moisture content in excess of 110 percent of the optimum moisture content determined according to AASHTO T 99.

All material that is proposed for use in embankment construction must be approved by the Engineer. The proposed material shall have a Standard Dry Density of not less than 1450 kg/m<sup>3</sup> (90 lb./ft<sup>3</sup>) when tested according to AASHTO T 99 and shall not have an organic content greater than 10 percent when tested according to AASHTO T 194. Soils that demonstrate the following properties shall be restricted to the interior of the embankment:

- a. A grain size distribution with less than 35% passing the 75  $\mu$ m (#200) sieve.
- b. A plasticity index (PI) of less than 12.
- c. A liquid limit (LL) in excess of 50.

Such soils shall be covered on the sides and top of the embankment by a minimum of 900 mm (3 ft.) of soil not characterized by any of the items a, b or c above. Other materials which may be considered by the Engineer as having the potential for erosion or excess volume change shall not be used in the 3 ft. (900 mm) cover on the sides or the top of the embankment.

<u>Subgrade.</u> The subgrade shall be constructed according to Section 301 of the Standard Specifications.

Delete the third paragraph (including subparagraphs a, b, and c) of Article 301.03 of the Standard Specifications and replace it with the following:

In cut sections the contractor responsible for the rough grading shall obtain not less than 95% of the standard laboratory density and not more than 110% of the optimum moisture for the top 300mm (1 ft.) of the subgrade.

The Contractor may, at his/her option, add a drying agent to lower the moisture content as specified. The drying agent must be approved by the Engineer prior to use. Additional compensation will not be allowed for the use of a drying agent, but will be considered as included in the cost of the various earthwork items.

In the first sentence of the fourth paragraph delete "listed in the steps".

<u>Aggregate Subbase.</u> This work shall consist of furnishing, transporting, and placing Aggregate Subbase, Type C, as specified in Section 311 of the Standard Specifications, except:

The quality requirement in Article 1004.04(b) shall not apply.

The material shall be classified as Category III in the Aggregate Gradation Control System (AGCS), and shall meet the following gradation requirements:

1. Crushed Stone, Crushed Slag, and Crushed Concrete

<u>Sieve Size</u>	Percent Passing
200 mm (8 in.)	100
150 mm (6 in.)	97±3
100 mm (4 in.)	90±10
50 mm (2 in.)	45±25
75 μm (#200)	3±3
75 μm (#200)	3±3

2. Crushed Gravel

<u>Sieve Size</u>	Percent Passing
150 mm (6 in.)	100
100 mm (4 in.)	90±10
50 mm (2 in.)	55±25
4.75 mm (#4)	30±20
75 μm (#200)	5±5

The aggregate subbase shall be well-graded from coarse to fine. Aggregate subbase that is gap-graded or single-sized will not be accepted.

The material finer than the 75  $\mu$ m (No. 200) sieve shall consist of the dust from fracture and shall be essentially free of clay or silt.

The aggregate shall be placed to the thickness specified in one lift. When aggregate meeting the Aggregate Subbase requirements is used to replace unstable material, the Aggregate Subbase may be placed simultaneously with the material for subgrade replacement, when the total thickness to be placed is 600 mm (24 in.) or less. The Aggregate Subbase (and subgrade replacement material, if any) shall be rolled with a vibratory roller meeting the requirements of Article 1101.01 of the Standard Specifications to obtain the desired keying or interlock and compaction. The Engineer shall verify that adequate keying has been obtained.

<u>Aggregate Subbase Cap</u>. This work shall consist of furnishing, transporting, and placing an Aggregate Subbase, Type C, as a cap as specified in Section 311 of the Standard Specifications, except the material gradation shall be CA 6. The lift thickness shall be 75 mm (3 in.), nominal. Reclaimed Asphalt Pavement (RAP) meeting Article 1004.07 of the Standard Specifications and having 100% passing the 37.5mm (1-1/2inches) sieve and well graded down through the fines may also be used as capping aggregate. RAP shall not contain steel slag or other expansive material. The results of the Department's tests on the RAP material will be the determining factor for consideration as expansive.

<u>Placing Concrete Pavement</u>. Placement shall be according to Article 421.05 of the Standard Specifications except that, if the shoulder and mainline pavements are of different reinforcement designs, they shall not be placed in a single operation.

<u>Concrete Mixture Temperature</u>. Article 1020.14 of the Standard Specifications shall apply except that, prior to paving, the Contractor shall indicate to the Engineer how the concrete mixture temperature will be controlled. If the mixture temperature exceeds the value stated in Article 1020.14, production of additional mix shall stop until action to reduce mixture

temperature is taken or conditions causing elevated temperatures change. The Engineer will allow the Contractor to deliver concrete mixture en route to the paving site.

<u>Curing</u>. Curing of the pavement shall be according to Article 1020.13 of the Standard Specifications, except:

Method 4 shall be completed within 10 minutes after tining.

The curing period shall be 7 days minimum.

<u>Opening to Traffic</u>. The pavement shall not be opened to public traffic or construction vehicles before the minimum curing period is completed.

<u>Method of Measurement</u>. The method of measurement for aggregate subbase shall be as follows:

(a) Contract Quantities. Contract quantities shall be in accordance with Article 202.07(a).

(b) Measured Quantities. Aggregate subbase will be measured for payment in metric tons (tons) according to Article 311.08 (b).

Basis of Payment. This work will be paid for at the contract unit price per metric ton (ton) for AGGREGATE SUBBASE, including the Aggregate Subbase Cap.

All other items will be measured and paid for according to the appropriate section of the Standard Specifications.

## EARTH AND ROCK EXCAVATION

Effective June 10, 2005

Delete the second paragraph of Article 202.04 and replace with the following:

"Rock Excavation shall consist of all boulders or rocks measuring 0.5 cu m (1/2 cu yd) in volume or greater and all rock of hard material, in natural ledges or displaced masses, which is not practical to excavate and remove without the use of pneumatic tools or continuous drilling and blasting. Material which may be removed with the use of conventional earthmoving equipment will not be considered Rock Excavation."

#### **EXCAVATION FOR STRUCTURES**

Effective June 10, 2005

Delete the fourth paragraph of Article 502.03 and replace with the following:

"Rock Excavation for Structures shall consist of the excavation of boulders 0.5 cu m (1/2 cu yd) in volume or greater and all rock of hard material, in natural ledges or displaced masses, which is not practical to excavate and remove without the use of pneumatic tools or continuous drilling and blasting. Material which may be removed with the use of conventional earthmoving equipment will not be considered Rock Excavation. The Contractor may use any method he/she chooses including ripping to remove the rock excavation. Rock Excavation for Structures shall also include existing concrete, masonry, timber grillages, foundation piles and similar materials, which are not exposed to view and are not shown on the plans and for which payment is not otherwise provided."

Added 06-14-2005