

ELEVATION AT ABUTMENTS

ABUTMENT BEARING
(12 Required)

INTERIOR GIRDER MOMENT TABLE	
	0.5 Sp.
I_s	(in ⁴) 43533
$I_c(n)$	(in ⁴) 84218
$I_c(3n)$	(in ⁴) 63575
S_s	(in ³) 1527
$S_c(n)$	(in ³) 1884
$S_c(3n)$	(in ³) 1742
DC1	(k/ft) 0.963
MDC1	(k) 1550.7
DC2	(k/ft) 0.15
MDC2	(k) 241.5
DW	(k/ft) 0.329
MDW	(k) 529.8
$M_L + IM$	(k) 1863.2
M_u (Strength I)	(k) 6295.6
$\phi_r M_n$	(k) 9321.3
f_s DC1	(ksi) 12.2
f_s DC2	(ksi) 1.7
f_s DW	(ksi) 3.6
f_s 1.3(L+IM)	(ksi) 15.4
f_s (Service II)	(ksi) 32.9
V_f	(k) 27.1

*Compact sections

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in⁴ and in³).

DC1: Un-factored non-composite dead load (kips/ft.).

MDC1: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

$M_L + IM$: Un-factored live load moment plus dynamic load allowance (Impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

$1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{L+IM}$

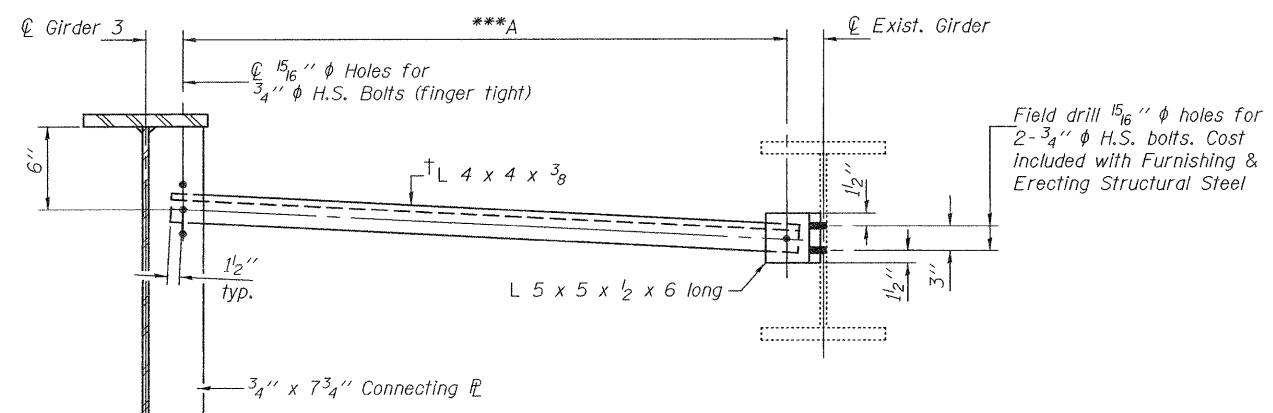
$\phi_r M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

f_s (Service II): Sum of stresses as computed from the moments below (ksi).

$M_{DC1} + M_{DC2} + M_{DW} + 1.3 M_{L+IM}$

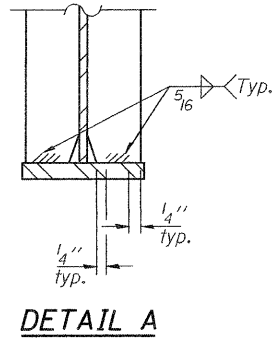
V_f : Maximum factored shear range in span computed according to Article 6.10.10.

INTERIOR GIRDER REACTION TABLE	
	Abuts.
R_{DC1}	(k) 54.7
R_{DC2}	(k) 8.5
R_{DW}	(k) 18.7
R_{L+IM}	(k) 88.6
R_{Total}	(k) 170.5

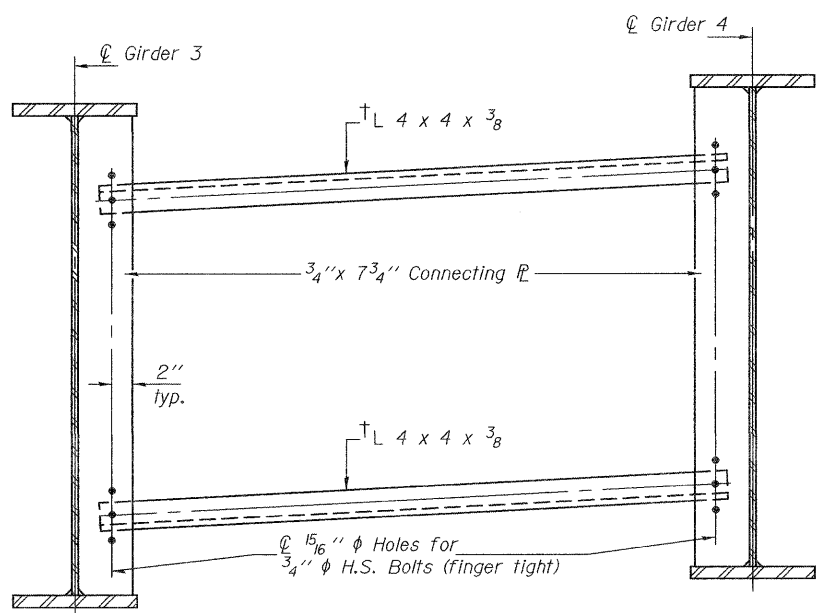


TEMPORARY BRACING FOR STAGE I CONSTRUCTION
(7 Required)

***The horizontal dimension A between the holes in the cross frame connection plate and L 5 x 5 shall be measured in the field. The holes in the L 5 x 5 shall be field drilled at this dimension. Cost included with Furnishing & Erecting Structural Steel.

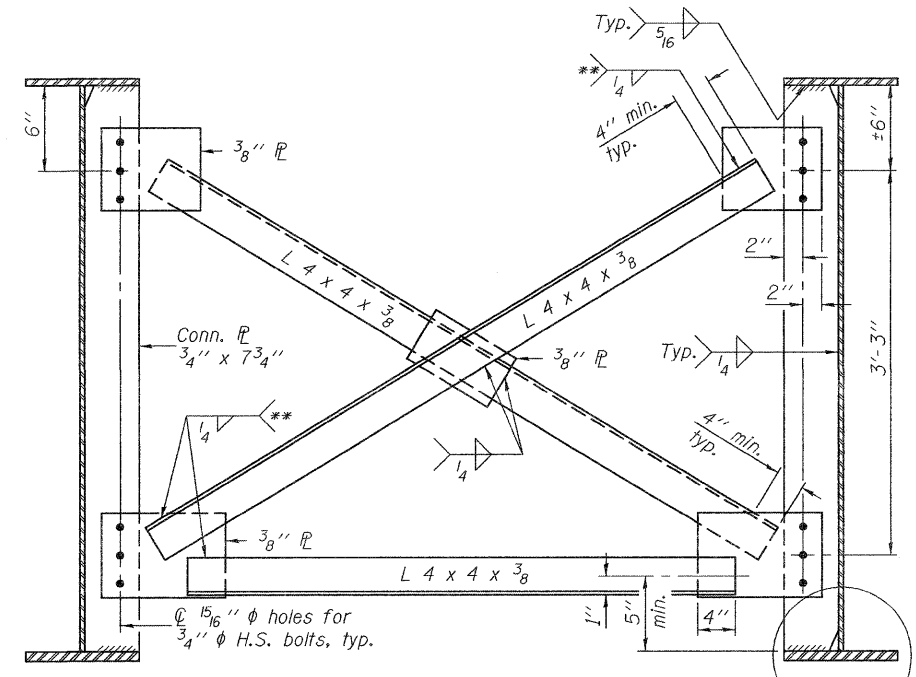


DETAIL A



TEMPORARY BRACING FOR STAGE II CONSTRUCTION
(7 Required)

† L 4 x 4 x 3/8 to be used as temporary during Stage I and Stage II deck pour. Remove and replace with cross frame CF after Stage II deck pour is complete. Use between Girders 3 and 4 only. Cost included with Furnishing and Erecting Structural Steel.



CROSS FRAME, CF
(35 Required)

See Detail A

Notes: All bearing plates shall be AASHTO M 270, Grade 50W.
Two hardened washers shall be required for each set of oversized holes.
**Fillet weld angles along 3 sides on one face of gusset plate.
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 ($F_y=36$ ksi). The corresponding specified grade of AASHTO M 314 anchor bolts may be used in lieu of ASTM F1554.
Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.