

DESIGNED - BAS

CHECKED - LVM

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baswansor

PLOT DATE = 4/22/2022

TOP AND BOTTOM ELEVATIONS FOR APPROACH FOOTING

	West Approach		East Approach	
Point	Тор	Bottom	Тор	Bottom
Α	668.41	667.58	665.16	664.33
В	668.73	667.90	665.48	664.65
С	668.33	667.50	665.08	664.25
D	668.48	667.65	665.03	664.20
Ε	668.81	667.98	665.35	664.52
F	668.40	667.57	664.95	664.12

SECTION

(92-10,11,12)R

COUNTY

VERMILION 698 301

CONTRACT NO. 70A29

Note A - 20-Bar Splicers (E) for #5 w(E) bars, top & bottom

* (29-Stage I, 35-Stage II)** (46-Stage I, 55-Stage II)

*** (19-Stage I, 23-Stage II)

Notes:

PJF (per Article 1051.09 of the Standard Specifications) below the approach slab shall be bonded to the top of new abutment wingwalls with suitable adhesive as recommended by supplier.

See Sheet 20 of 37 for Section A-A and Bill of Material.

See Sheet 18 of 37 for additional bar details.

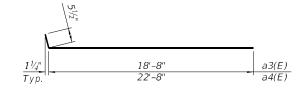
See Sheet 37 of 37 for Bar Splicer details.

(Sheet 1 of 4)

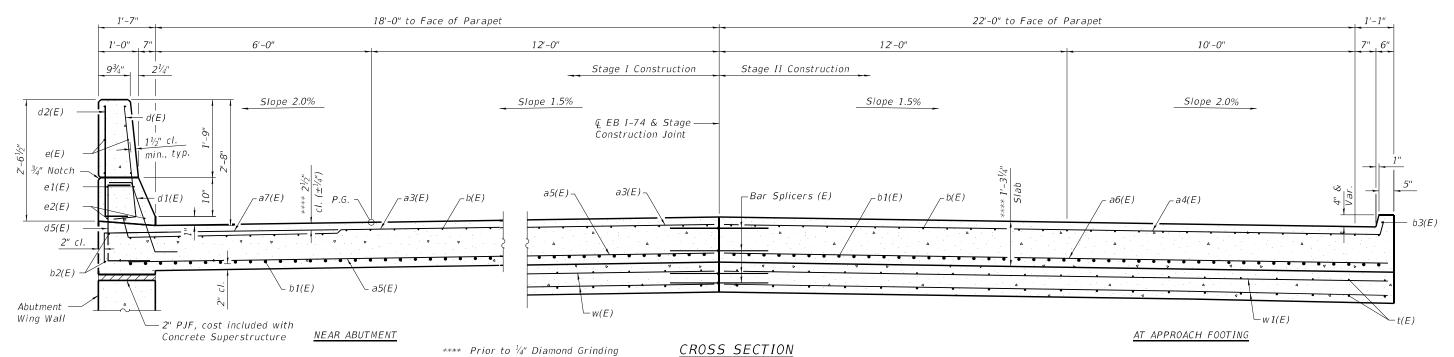
BRIDGE APPROACH SLAB DETAILS

STRUCTURE NO. 092-0008

SHEET NO. 17 OF 37 SHEETS



BARS a3(E) & a4(E)

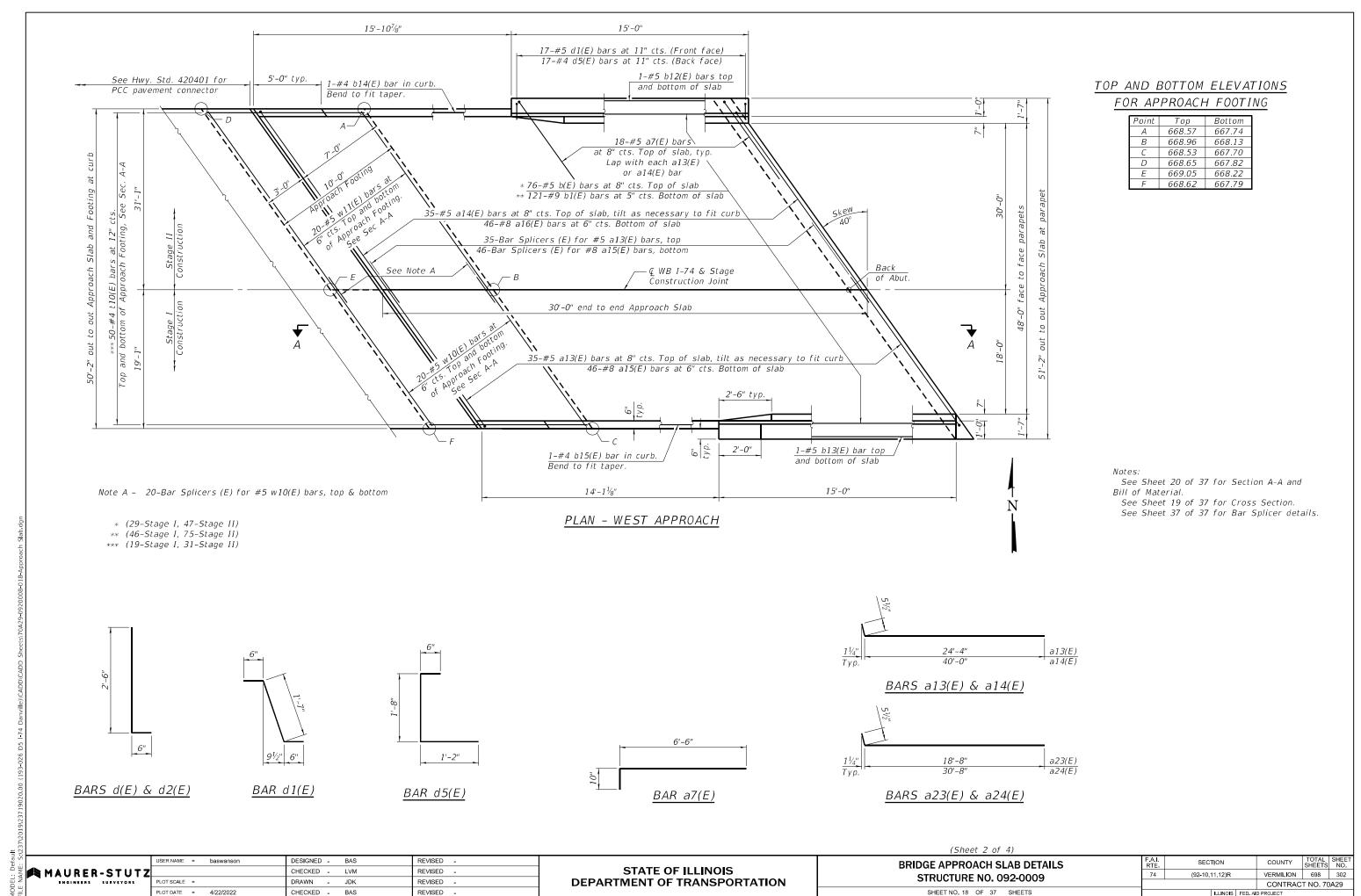


(Looking East)

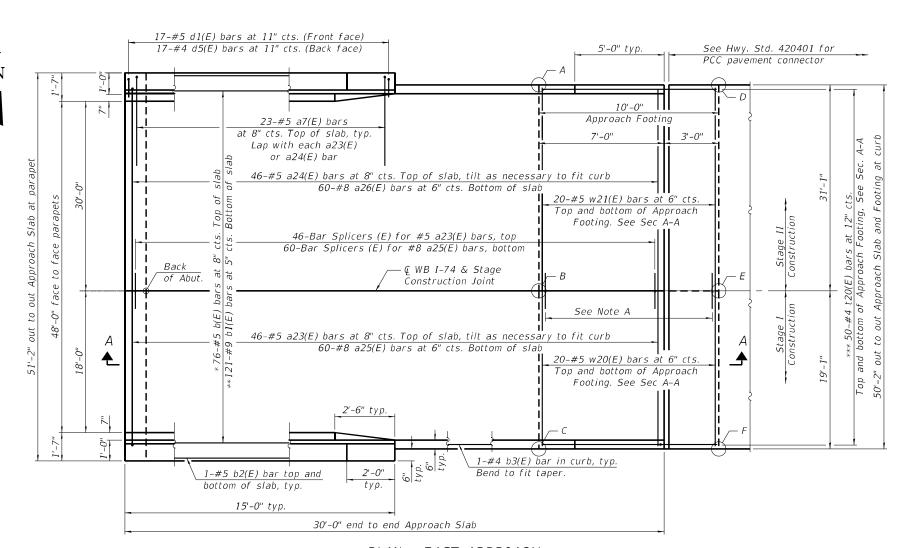
STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

MAURER-STUTZ



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TOP AND BOTTOM ELEVATIONS FOR APPROACH FOOTING

Point	Тор	Bottom
Α	664.87	664.04
В	665.43	664.60
С	665.11	664.28
D	664.74	663.91
Ε	665.30	664.47
F	664.98	664.15

Note A - 20-Bar Splicers (E) for #5 w20(E) bars, top & bottom

* (29-Stage I, 47-Stage II) ** (46-Stage I, 75-Stage II)

*** (19-Stage I, 31-Stage II)

Notes:

PJF (per Article 1051.09 of the Standard Specifications) below the approach slab shall be bonded to the top of new abutment wingwalls with suitable adhesive as recommended by supplier.

See Sheet 20 of 37 for Section A-A and Bill of Material.

See Sheet 18 of 37 for additional bar details.

See Sheet 37 of 37 for Bar Splicer details.

PLAN - EAST APPROACH

REVISED -

REVISED -

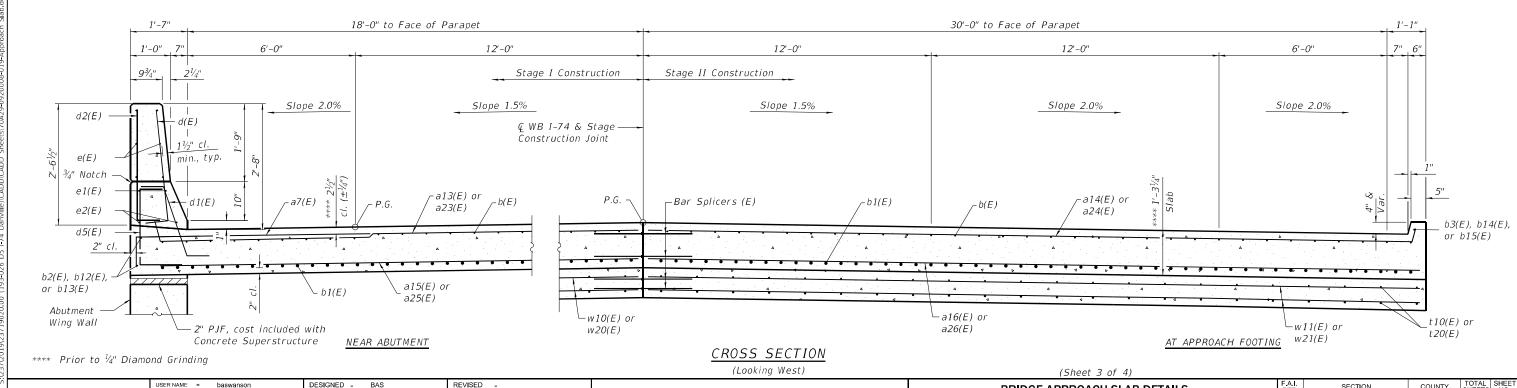
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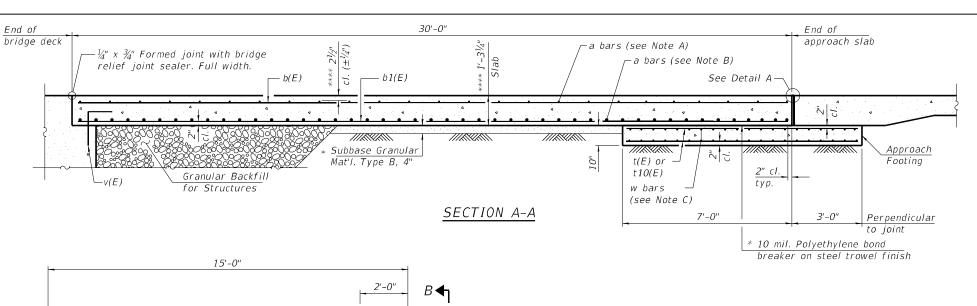
PLOT DATE = 4/22/2022

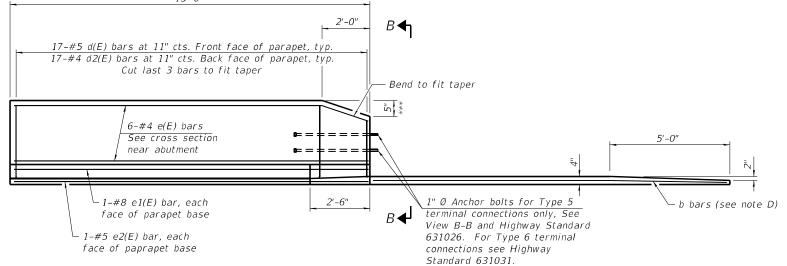


MAURER-STUTZ

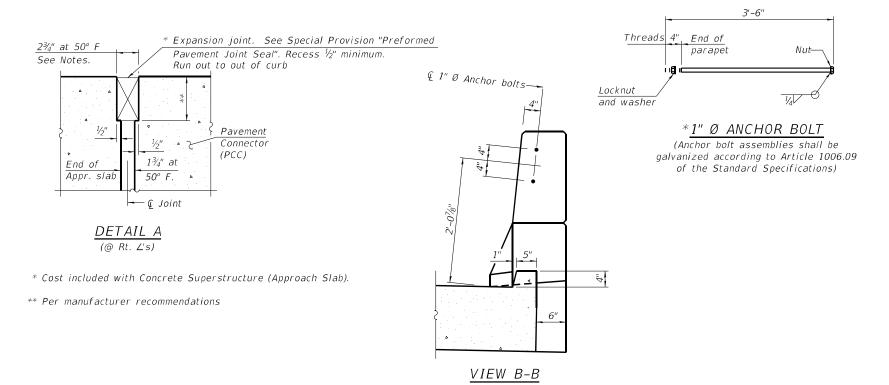
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 092-0009** SHEET NO. 19 OF 37 SHEETS

SECTION COUNTY (92-10,11,12)R VERMILION 698 303 CONTRACT NO. 70A29





INSIDE ELEVATION OF PARAPET AND CURB



Notes:

The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. The length of bridge used to calculate the adjustment shall be equal to the distance from the fixed bearing to the end of the bridge approach slab.

Parapet concrete shall be paid for as Concrete Superstructure.

Approach slab shall be paid for as Concrete Superstructure (Approach Slab). Approach footing concrete shall be paid for as Concrete Structures.

The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf. Cost of excavation for approach footing included with Concrete Structures.

For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 37. Adjust the profile grade as needed across the length of the bridge approach slab to match the top of bridge deck overlay at the abutment.

*** Omit taper at departure ends with Type 5 Terminals

**** Prior to $\frac{1}{4}$ " Diamond Grinding

Note A - a3(E), a4(E), a13(E), a14(E), a23(E), or a24(E) Note B - a5(E), a6(E), a15(E), a16(E), a25(E), or a26(E) Note C - w(E), w1(E), w10(E), w11(E), w20(E), or w21(E)

Note D - b3(E), b14(E), or b15(E)

STR. NO. 092-0008 TWO APPROACHES BILL OF MATERIAL

_	BILL OF	, ,,	ERIAL	
Bar	No.	Size	Length	Shape
a3(E)	92	#5	19'-2"	
a4(E)	92	#5	23'-2"	
a5(E)	120	#8	18'-9"	
a6(E)	120	#8	22'-9"	
a7(E)	92	#5	7'-4"	
b(E)	128	#5	29'-8"	
b1(E)	202	#9	29'-8"	
b2(E)	8	#5	14'-8"	
b3(E)	4	#4	14'-8"	
d(E)	68	#5	3'-0"	
d1(E)	68	#5	2'-7"	٦
d2(E)	68	#4	3'-0"	<u>_</u>
d5(E)	68	#4	3'-4"	
e(E)	24	#4	14'-8"	
e1(E)	8	#8	14'-8"	
e2(E)	8	#5	14'-8"	
, ,				
t(E)	168	#4	9'-8"	
w(E)	80	#5	18'-9"	
w1(E)	80	#5	22'-9"	
WI(L)	00	# 3	22 - 9	
6	C 4		C. V.I	5.0
Concrete Superstructure Concrete Superstructure			Cu. Yd.	5.9
(Approacl	Superst h Slab)	ructure	Cu. Yd.	121.4
Concrete		res	Cu. Yd.	26.0
Reinforcement Bars, Epoxy Coated			Pound	59930

STR. NO. 092-0009 TWO APPROACHES BILL OF MATERIAL

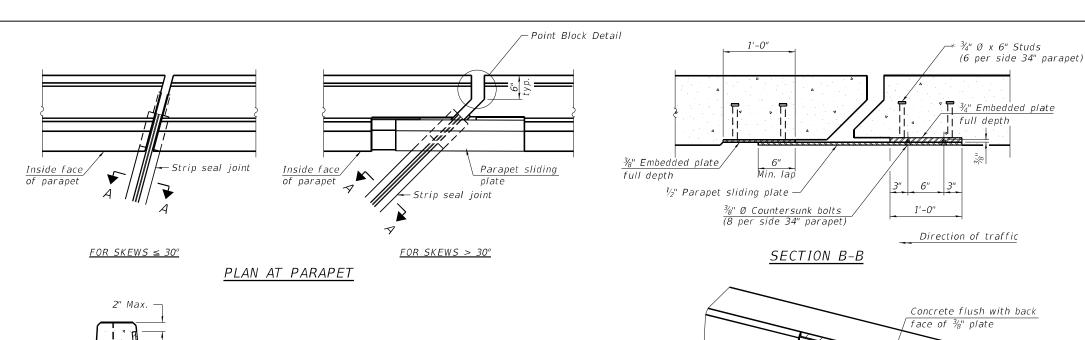
Bar	No.	Size	Length	Shape
a13(E)	35	#5	24'-10"	
a14(E)	35	#5	40'-6"	
a15(E)	46	#8	24'-6"	
a16(E)	46	#8	40'-2"	
a23(E)	46	#5	19'-2"	
a24(E)	46	#5	31'-2"	
a25(E)	60	#8	18'-9"	
a26(E)	60	#8	30'-9"	
a7(E)	82	#5	7'-4"	
b(E)	152	#5	29'-8"	
b1(E)	242	#9	29'-8"	
b2(E)	4	#5	14'-8"	
b3(E)	2	#4	14'-8"	
b12(E)	2	#5	13'-5"	
b13(E)	2	#5	15'-10"	
b14(E)	1	#4	15'-3"	
b15(E)	1	#4	13'-11"	
,				
d(E)	68	#5	3'-0"	
d1(E)	68	#5	2'-7"	1
d2(E)	68	#4	3'-0"	
d5(E)	68	#4	3'-4"	
				_
e(E)	24	#4	14'-8"	
e1(E)	8	#8	14'-8"	
e2(E)	8	#5	14'-8"	
t10(E)	100	#4	12'-8"	
t20(E)	100	#4	9'-8"	
w10(E)	40	#5	24'-6"	
w11(E)	40	#5	40'-2"	
w20(E)	40	#5	18'-9"	
w21(E)	40	#5	30'-9"	
Concrete	Superst	ructure	Cu. Yd.	5.9
Concrete				1 4 4 1
(Approaci	h Slab)		Cu. Yd.	144.1
Concrete		res	Cu. Yd.	35.7
Reinforce				50100
Ероху Сс		Pound	58180	

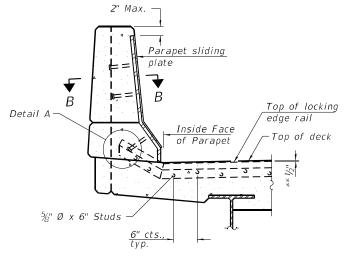
(Sheet 4 of 4)

	USER NAME = baswanson	DESIGNED - BAS	REVISED -
MAURER-STUTZ		CHECKED - LVM	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - JDK	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

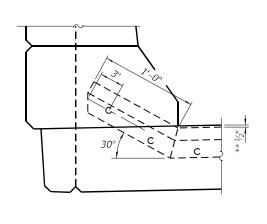
BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 092-0008 & 092-0009





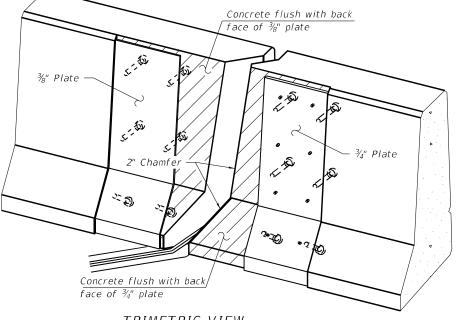
SECTION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

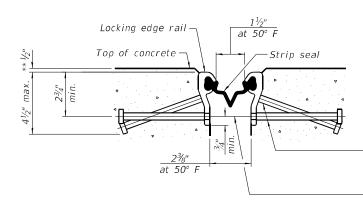


DETAIL A

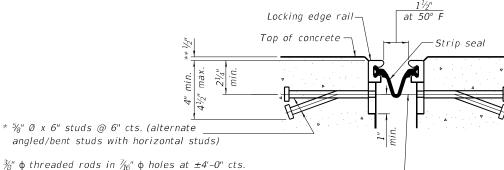
** Prior to Diamond Grinding



TRIMETRIC VIEW (Showing back plates only)



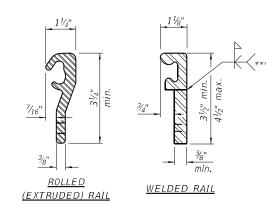
SHOWING ROLLED RAIL JOINT



for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

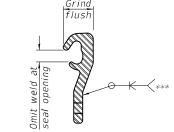
SECTION A-A * Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

SHOWING WELDED RAIL JOINT



LOCKING EDGE RAILS

*** Back gouge not required if complete joint penetration is verified by mock-up.



The strip seal shall be made continuous and shall have a minimum thickness of ½". The configuration of the strip

The locking edge rails depicted are configured for typical

applications and are conceptual only. The actual configuration

of the locking edge rails and matching strip seal may vary from

manufacturer to manufacturer provided they fit the application

however, will not be allowed. Locking edge rails may exceed the

 $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any

rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge

parapet lengths shown elsewhere in the plans are dimensioned

on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments

may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the

to the concrete opening, not the joint opening, and are based

The Maximum space between locking edge rail segments

Cost of parapet sliding plates, embedded plates, and

anchorage studs included with Preformed Joint Strip Seal. The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and

The manufacturer's recommended installation methods

according to the manufacturer's recommendation.

and meet the minimum anchorage shown. Flanged edge rails,

seal shall match the configuration of the locking edge

rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum

rated movement of 4 inches.

shall be followed.

rail splice detail.

length of the bridge approach slab.

LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL STR. NO. 092-0009

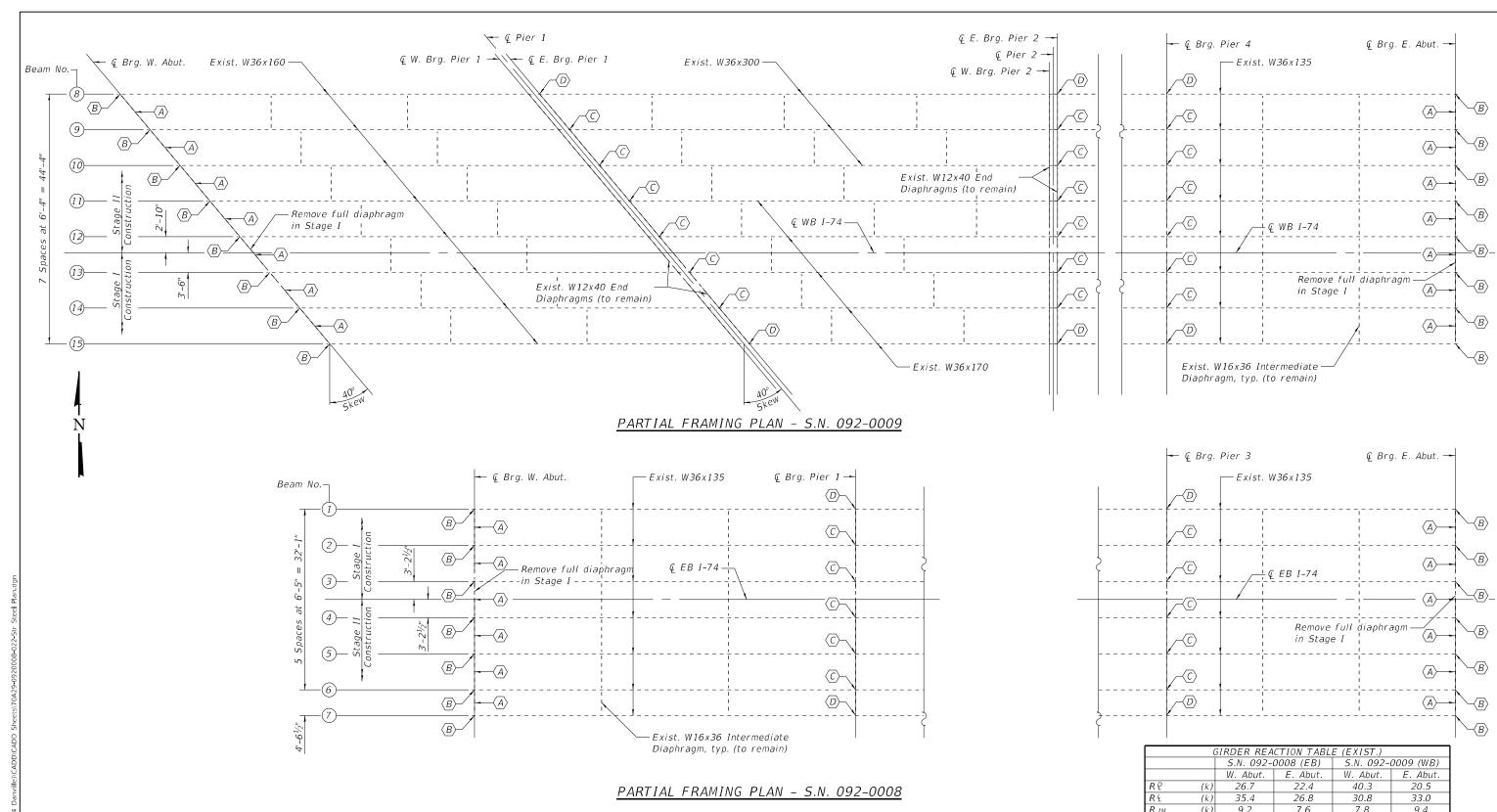
Item	Unit	Total
Preformed Joint Strip Seal	Foot	115

DESIGNED - BAS REVISED baswansor CHECKED - LVM REVISED -MAURER-STUTZ DRAWN REVISED PLOT DATE = 4/22/2022 CHECKED - BAS REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION PREFORMED JOINT STRIP SEAL **STRUCTURE NO. 092-0009** SHEET NO. 21 OF 37 SHEETS

F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
74	(92-10,11,12)R	VERMILION	698	305
		CONTRAC	T NO. 70)A29
	ILL NOIC FED.	ID DDO JECT		

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Callouts:

 $\langle A \rangle$ - Remove existing W12x40 end diaphragm, see Sheet 23 of 37 for details.

 $\langle B \rangle$ – Jack and remove existing bearing assembly, replace with new bearing assemblies as shown on Sheet 24 of 37.

 $\langle C \rangle$ - Install new side retainer and 1" \emptyset anchor bolt on each side of existing bearing assembly, see Sheet 24 of 37 for details.

 $\langle \overline{D} \rangle$ - Install new side retainer and 1" Ø anchor bolt against existing bearing assembly, see Sheet 24 of 37 for details.

		W. Abut.	E. Abut.	W. Abut.	E. Abut.
R ₽ (.	(k)	26.7	22.4	40.3	20.5
R 4 (.	(k)	35.4	26.8	30.8	33.0
RIM (.	(k)	9.2	7.6	7.8	9.4
R Total ((k)	71.3	56.8	78.9	62.9

* GIRDER REACTION TABLE (PROP.)					
		5.N. 092-	0008 (EB)	S.N. 092-	0009 (WB)
		W. Abut.	E. Abut.	W. Abut.	E. Abut.
R₽	(k)	53.5	49.7	70.5	47.3
R Ł	(k)	43.3	41.5	44.6	40.9
R IM	(k)	11.2	11.8	11.2	11.6
R Total	(k)	108.0	103.0	126.3	99.8

* New bearing assembly loads (includes approach slab)

	USER NAME = baswanson	DESIGNED - BAS	REVISED -
MAURER-STUTZ		CHECKED - LVM	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - JDK	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -

Removal of existing diaphragms shall be paid for as

Removal of existing bearing assemblies shall be paid for as

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

COUNTY STRUCTURAL STEEL REMOVAL (92-10,11,12)R VERMILION 698 306 STRUCTURE NO. 092-0008 & 092-0009 CONTRACT NO. 70A29 SHEET NO. 22 OF 37 SHEETS

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Structural Steel Removal.

Jack and Remove Existing Bearings.

See Sheet 23 of 37 for Bill of Material.

EXISTING BEARING REMOVAL DETAIL END DIAPHRAGM REMOVAL

* Angle welded to Beam 7 at each abutment may remain in place. See note.

To be removed

Notes:

Removal of the existing steel extensions and side retainers at the abutments shall be included in the cost of Jack and Remove Existing Bearings.

Minimum jack capacity shall be 45 tons.

Removal of steel end diaphragms shall be paid for as Structural Steel Removal.

Diaphragm connection angles welded to the beam webs may remain. If directed to be removed by the Engineer, these shall be removed utilizing air carbon arc cutting or other method approved by the Engineer. Remaining weld material shall be ground flush and inspected in accordance with general note.

Prior to pouring new concrete diaphragms, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the beam web in the vicinity of the removed diaphragms. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel, and the cost will be included with Structural Steel Removal.

FILL PLATE THICKNESS

. 092-0008 eam 1 eam 2			
	?–0008, E. Abı	ıt.	
eam 2	! 3/8"		
	2 1/4"		
eam 3	3		
eam 4	1		
eam 5	5		
eam 6	5		
eam 7	7 3/8"		
. 092-0009	S.N. 092-0009, E. Abut.		
eam 8	3		
eam 9	9		
eam 10	10		
	! 1 1/2"		
eam 11	12 1/4"		
eam 11 eam 12			
	13 1/4"		
eam 12	13 ½" 14		
	10		

See Sheet 24 of 37 for dimensions of associated bearing assembly plates.

BILL OF MATERIAL STR. NO. 092-0008

Item	Unit	Quantity
Elastomeric Bearing Assembly, Type I	Each	14
Anchor Bolts, 1"	Each	52
Furnishing and Erecting Structural Steel	Pound	3520
Jack and Remove Existing Bearings	Each	14
Structural Steel Removal	Pound	3360

BILL OF MATERIAL STR. NO. 092-0009

<u> </u>		
Item	Unit	Quantity
Elastomeric Bearing Assembly, Type I	Each	16
Anchor Bolts, 1"	Each	74
Furnishing and Erecting Structural Steel	Pound	4260
Jack and Remove Existing Bearings	Each	16
Structural Steel Removal	Pound	4640

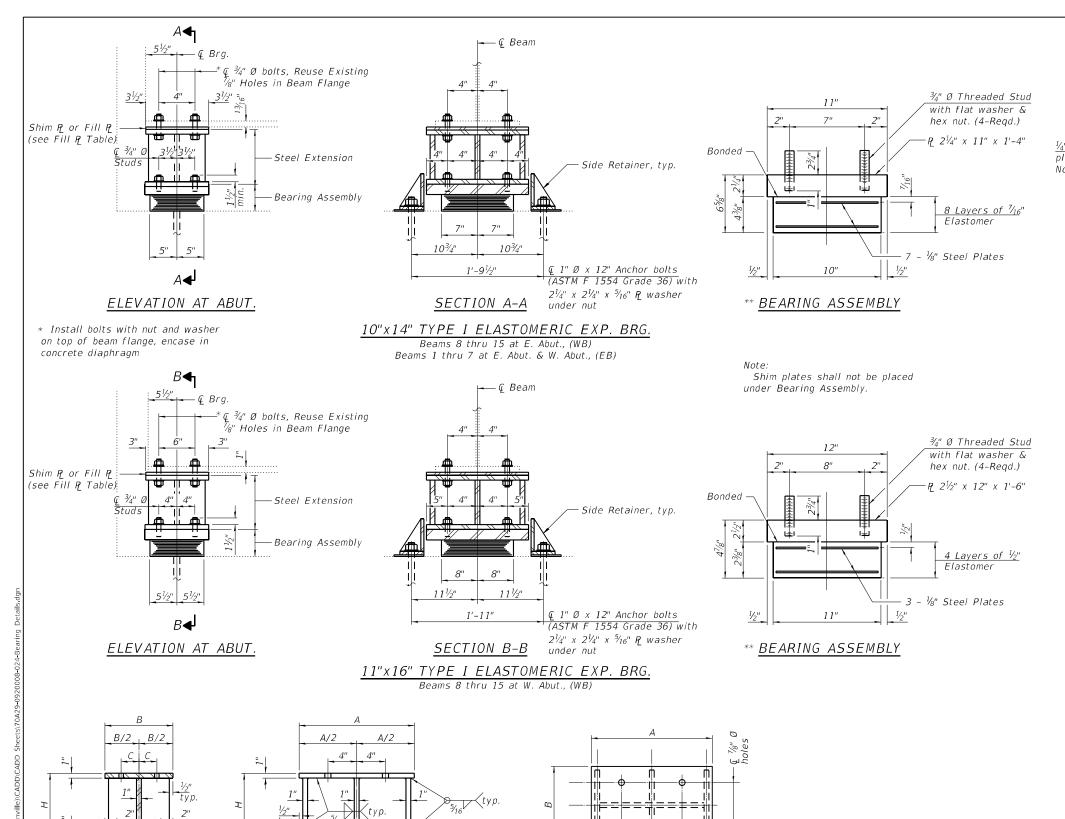
USER NAME = baswanson DESIGNED - BAS REVISED
CHECKED - LVM REVISED
PLOT SCALE = DRAWN - JDK REVISED
PLOT DATE = 4/22/2022 CHECKED - BAS REVISED -

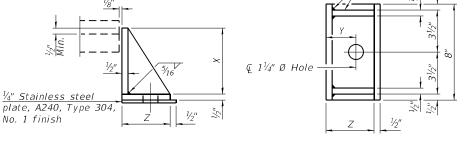
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL DETAILS
STRUCTURE NO. 092-0008 & 092-0009

SHEET NO. 23 OF 37 SHEETS

) (193-026 D5 I-74 Danville)\CADD\CADD Sheets\70A29-0920008-023-Str Steel De





SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

SIDE RETAINER DIMENSIONS

Location	No. Req'd	Bearing	Χ	Y	Ζ
W. Abut. (EB)	** 14	10"x14"	6½"	2 ⁵ /8"	6"
E. Abut. (EB)	** 14	10"x14"	6½"	2 ⁵ /8"	6"
E. Abut. (WB)	** 16	10"x14"	$6^{1/2''}$	2 ⁵ /8"	6"
W. Abut. (WB)	** 16	11"x16"	43/4"	2 ³ /8"	6½"
Pier 1 (EB)	12	12"x18"	7"	2½"	4"
Pier 3 (EB)	12	10"x14"	7"	2½"	4"
Pier 1 (WB) E. Brg.	14	10"x14"	7"	2½"	4"
Pier 2 (WB) E. Brg.	14	7"x12"	5½"	2½"	4"
Pier 4 (WB)	14	10"x14"	7"	2½"	4"

** Cost included with associated Elastomeric Bearing Assembly.

Notes:

Anchor bolts shall be ASTM F1554 all-thread (or anEngineerapproved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTMF1554.

Steel extensions, shim plates, and connection bolts shall be paid for as Furnishing and Erecting Structural Steel.

Unless noted otherwise, side retainers shall be paid for as Furnishing and Erecting Structural Steel. Side retainers and stainless steel plates provided for new Elastomeric Bearing Assemblies shall be included in the cost of Elastomeric Bearing Assembly, Type I.

Prior to ordering any material, the Contractor shall verify in the field all bearing height and shim thickness dimensions.

Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

Provide two 1/8" adjusting shims for each bearing in addition to all other plates of the Bearing Assembly and placed as shown on the bearing details.

See sheet 23 of 37 for Fill Plate Thickness table and Bill of

The Top Bearing Plates shall be shop painted with the inorganic zinc rich primer per AASHTO M300 Type I. Cost included with Elastomeric Bearing Assembly, Type I.

STEEL EXTENSION DIMENSIONS

Location	No. Req'd	Α	В	H	С
W. Abut. (EB), Bms. 1-7	7	1'-4"	11"	91/8"	2"
E. Abut. (EB), Bms. 1-7	7	1'-4"	11"	107/8"	2"
E. Abut. (WB), Bms. 8-15	8	1'-4"	11"	8 ³ / ₈ "	2"
W. Abut. (WB), Bms. 8-15	8	1'-6"	12"	95/8"	3"

STEEL EXTENSION

—— ⊊ Beam

ELEVATION

· Ç 1/8" Ø holes

2:			
S	USER NAME = baswanson	DESIGNED - BAS	REVISED -
MAURER-STUTZ		CHECKED - LVM	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - JDK	REVISED -
=	PLOT DATE = 5/5/2022	CHECKED - BAS	REVISED -

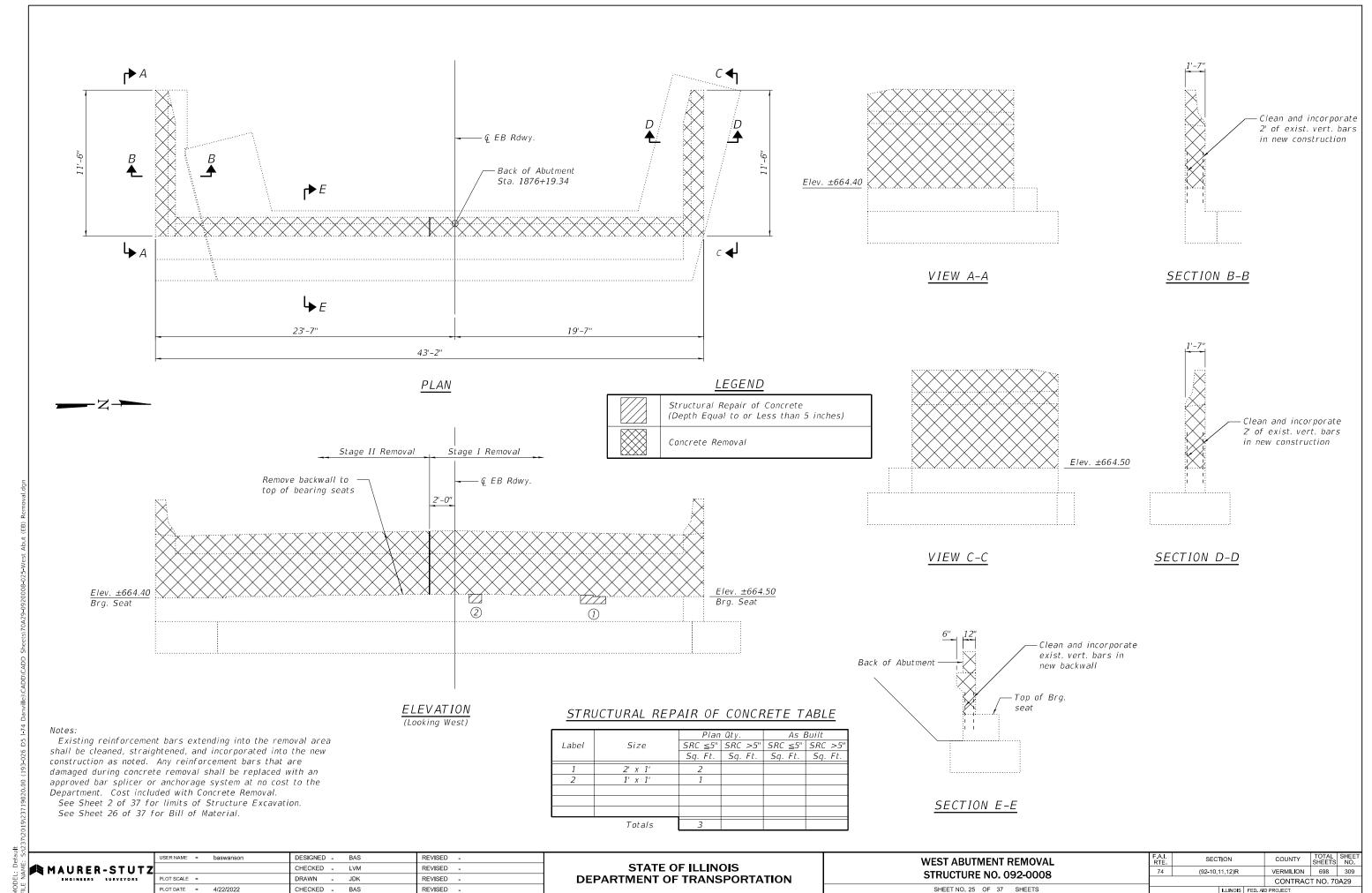
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

-— ℚ Beam

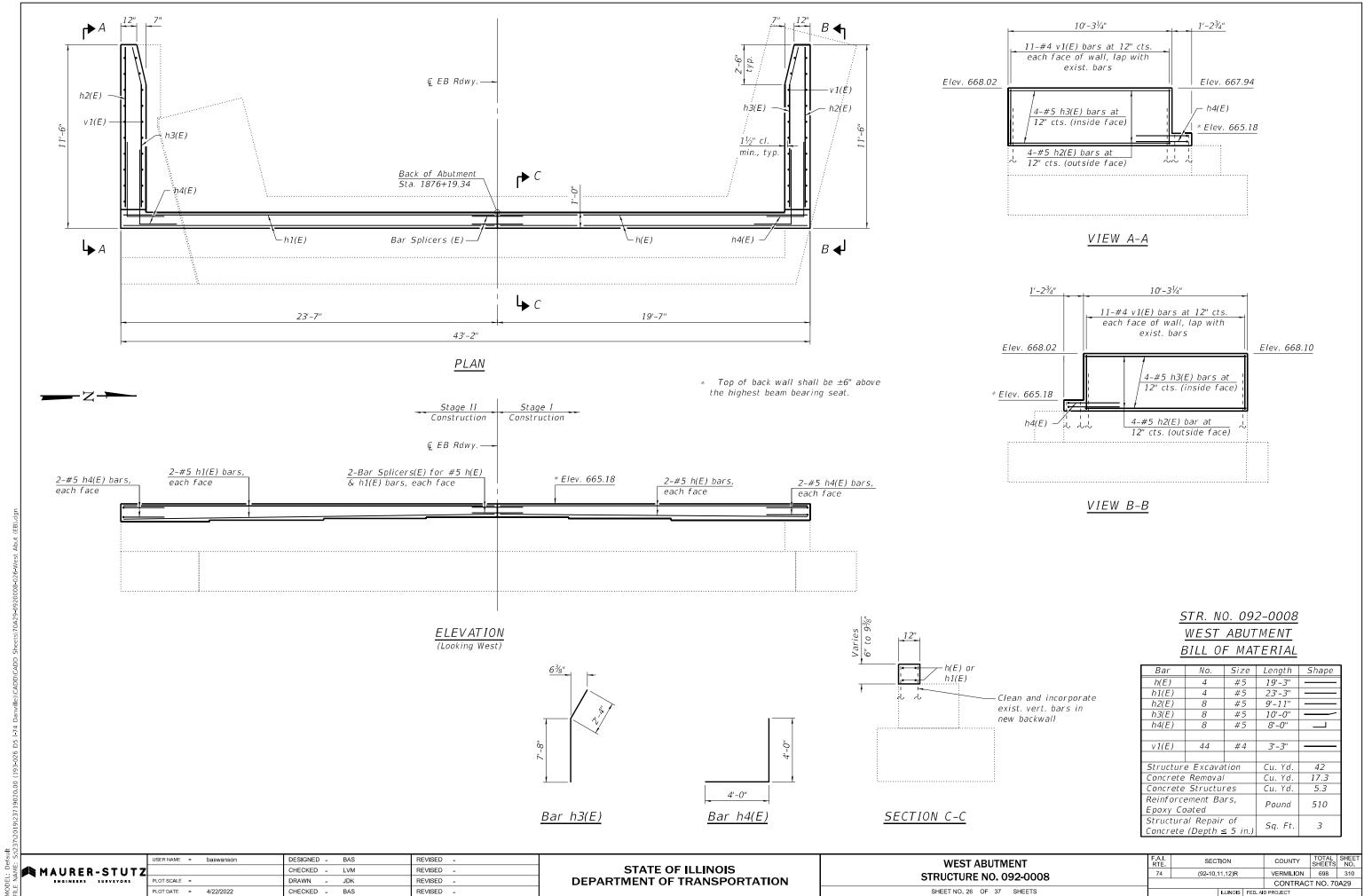
PLAN - TOP AND BOTTOM P

SECTION **BEARING DETAILS** (92-10,11,12)R VERMILION 698 308 STRUCTURE NO. 092-0008 & 092-0009 CONTRACT NO. 70A29 SHEET NO. 24 OF 37 SHEETS

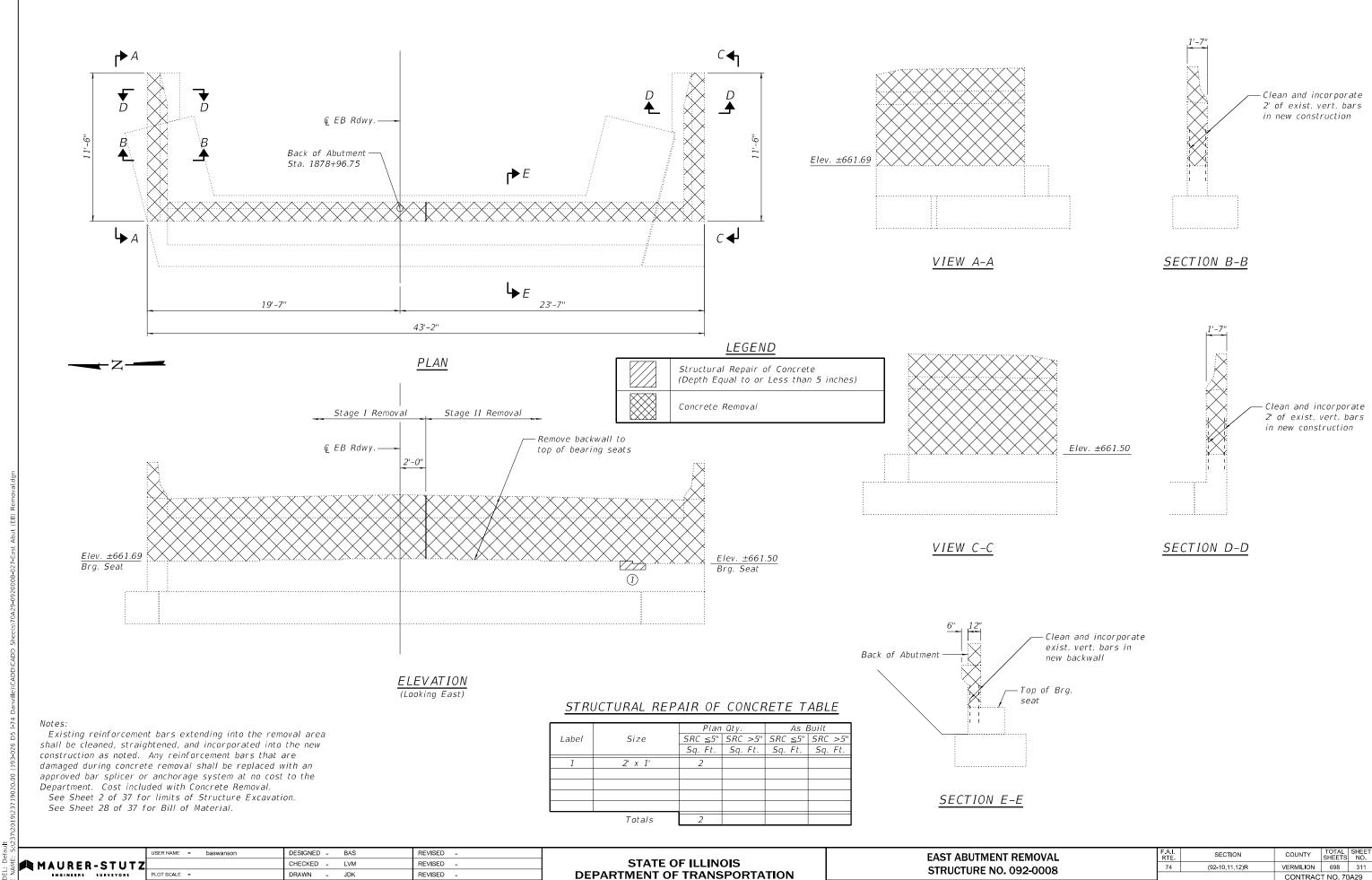
<u>SECTION</u>



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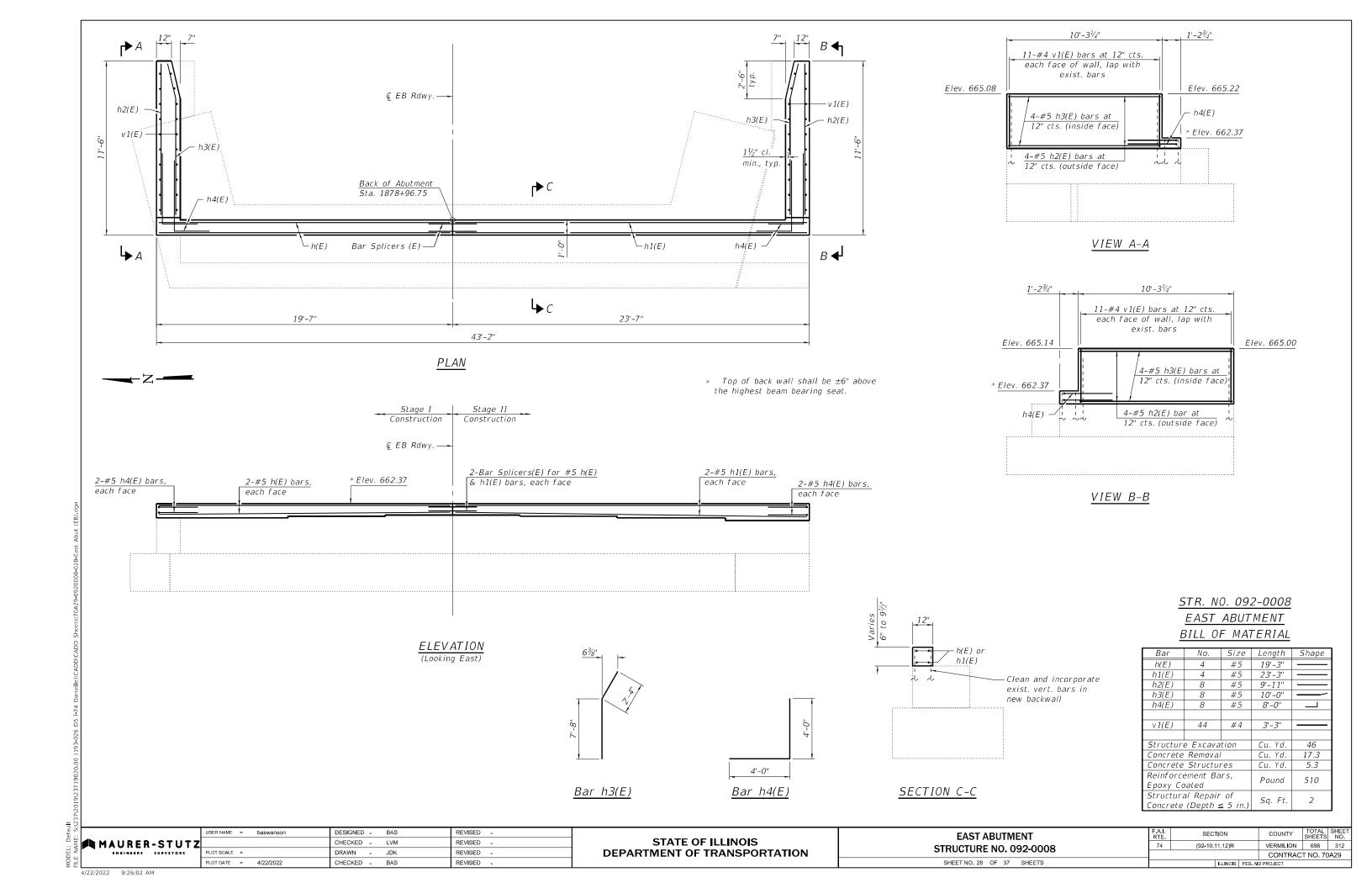
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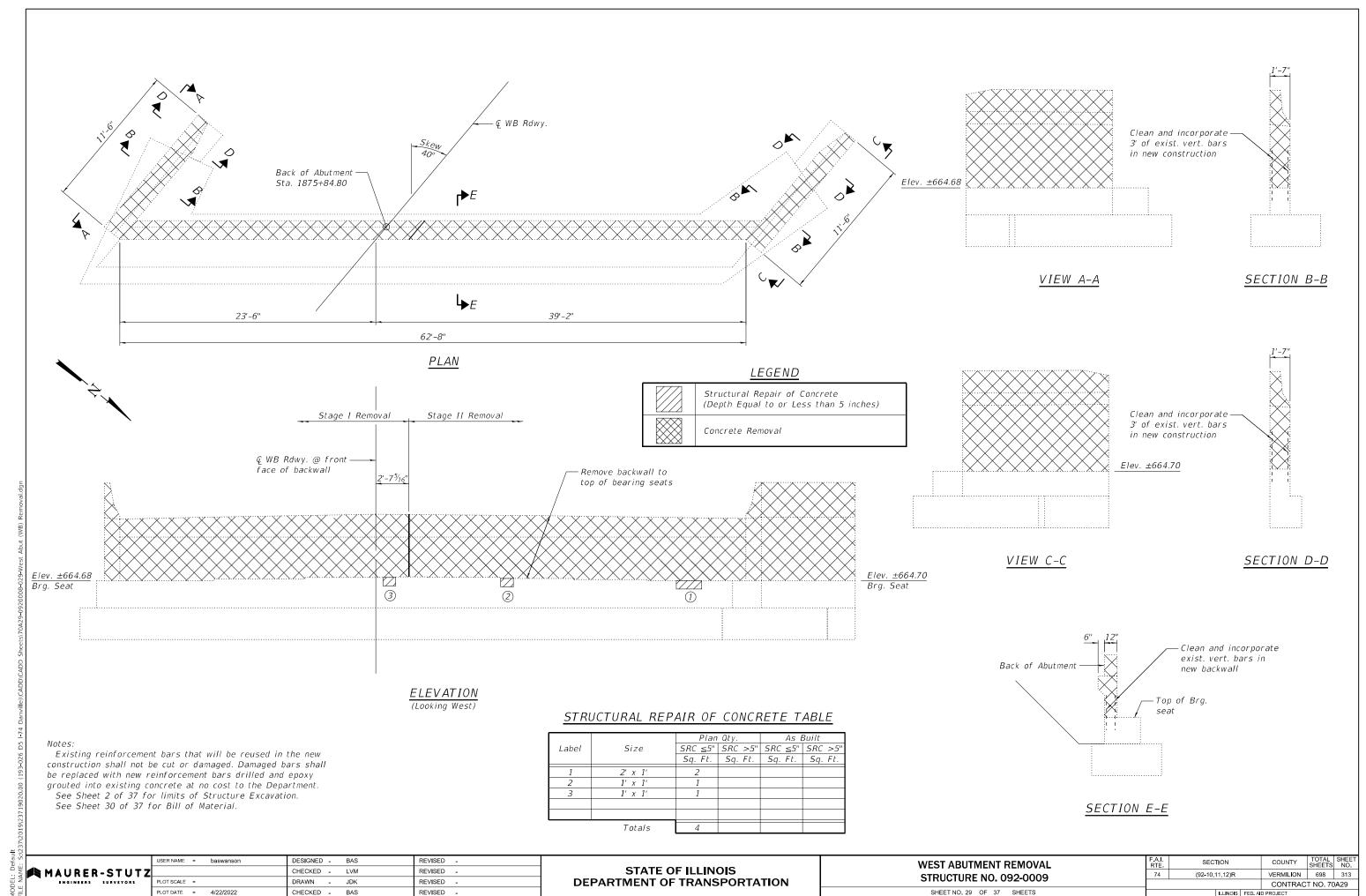
PLOT DATE = 4/22/2022

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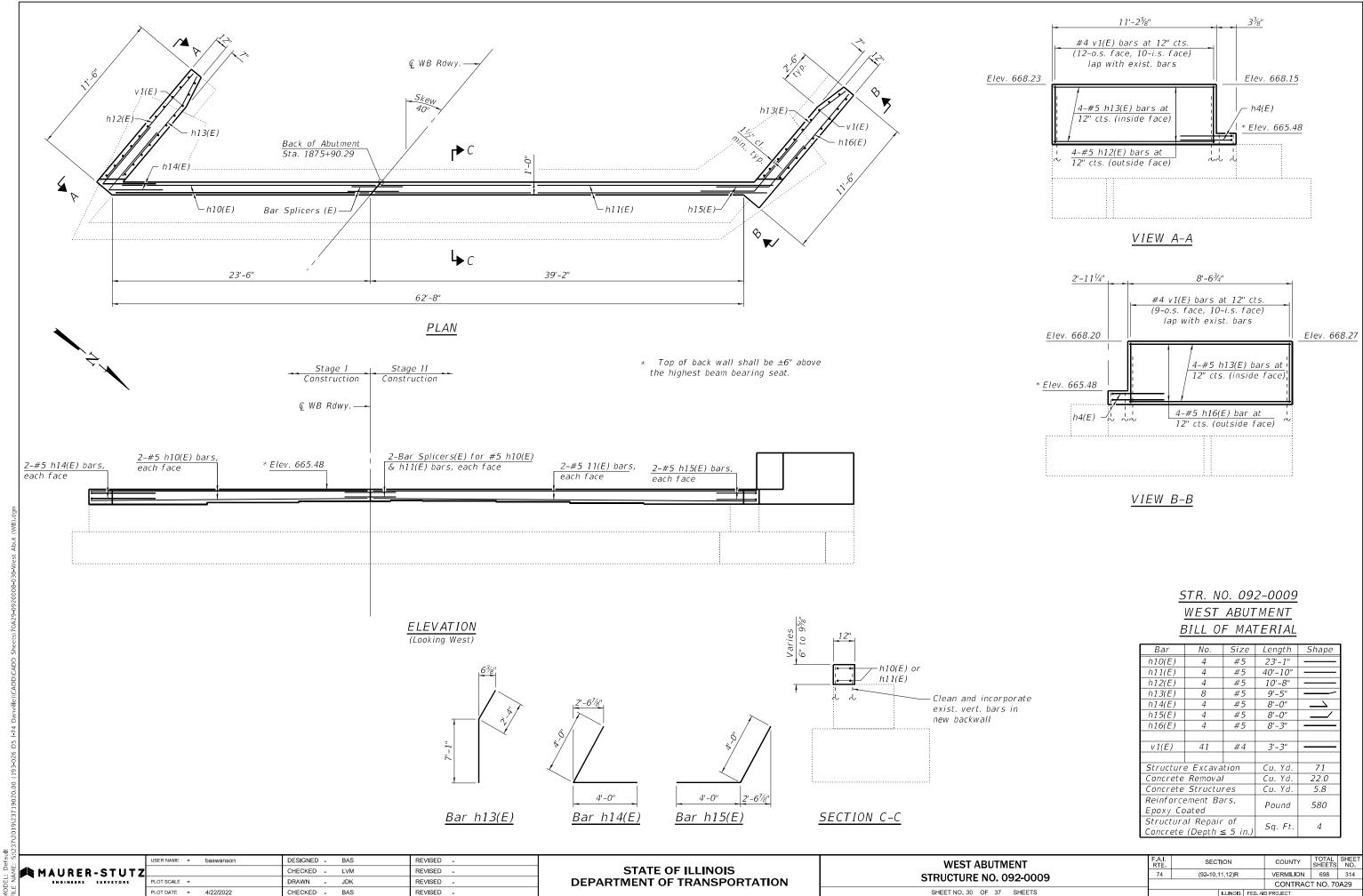
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(92-10,11,12)R VERMILION 698 311 **STRUCTURE NO. 092-0008** CONTRACT NO. 70A29 SHEET NO. 27 OF 37 SHEETS

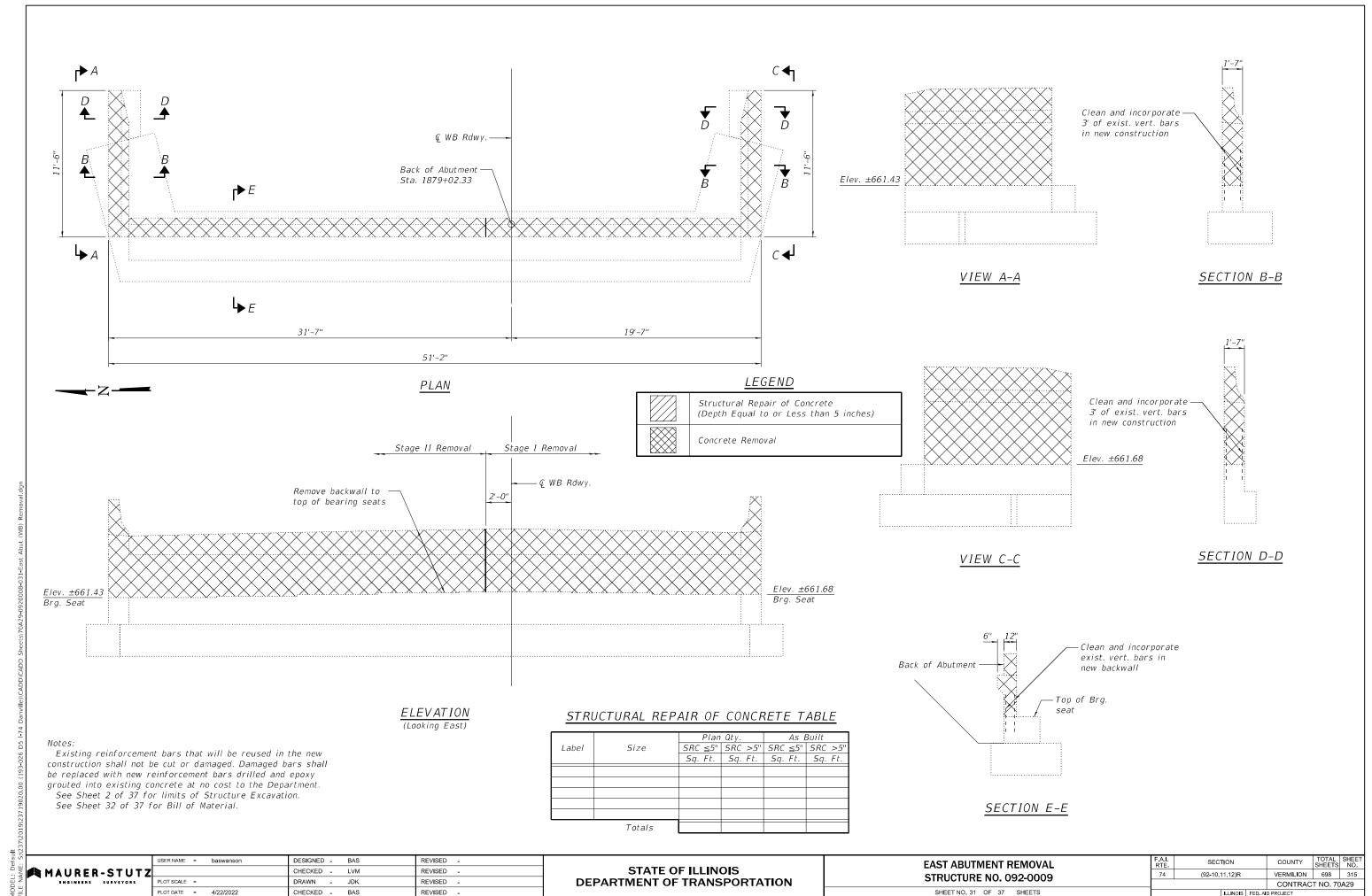




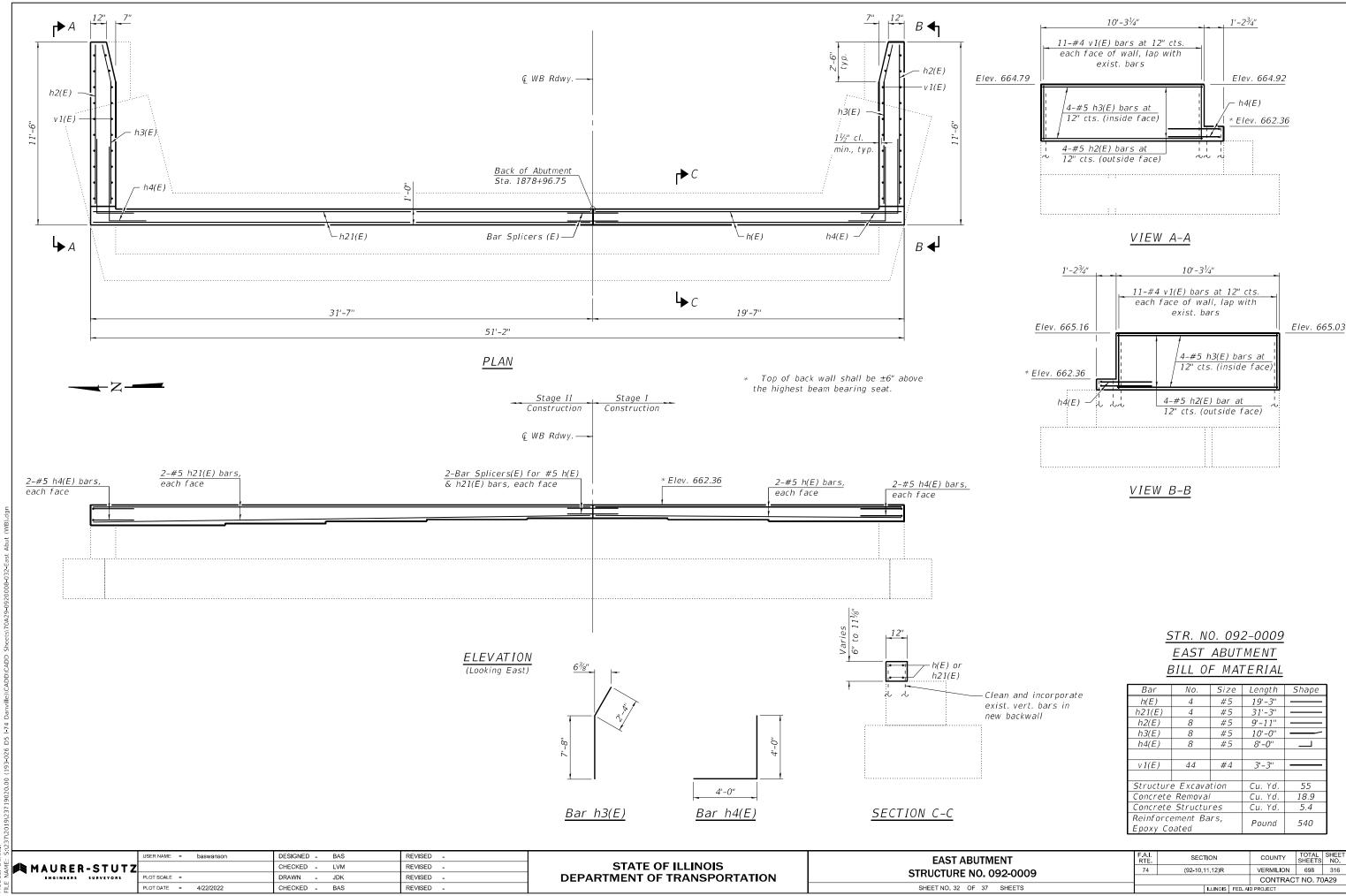
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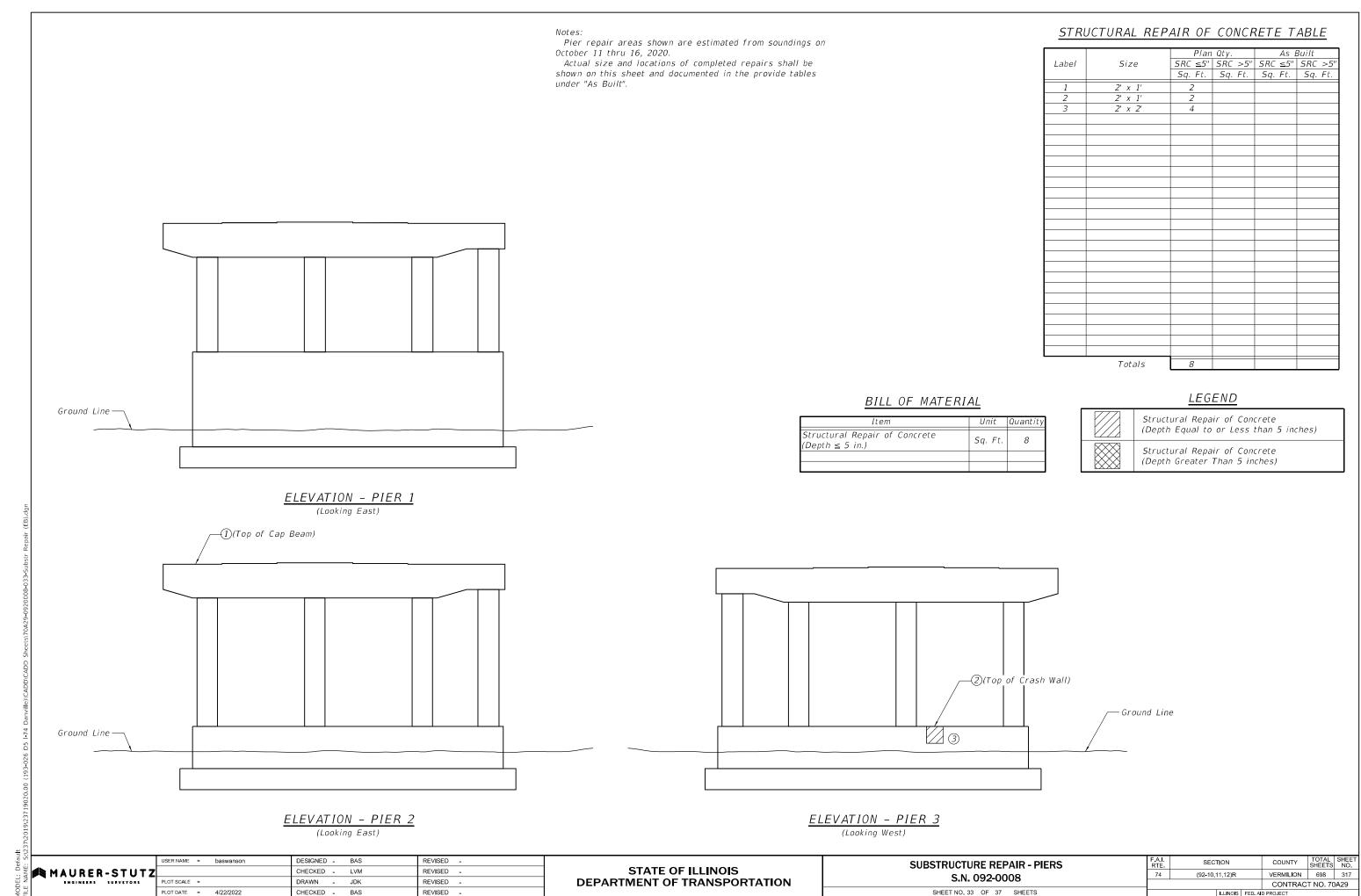


4/22/2022 9:26:04 AM



4/22/2022 9:26:05 AM





Pier repair areas shown are estimated from soundings on October 11 thru 16, 2020. Actual size and locations of completed repairs shall be shown on this sheet and documented in the provide tables under "As Built". The minimum jack capacity shall be 50% greater than the maximum expected load on the jack. PLAN - TOP OF CAP BEAM (25) PLAN - BOTTOM OF CAP BEAM Temporary Shoring and Cribbing (7 beams)

 $\frac{\textit{ELEVATION}}{\textit{(Looking East)}}$

- JDK

REVISED -

REVISED -

REVISED -

REVISED

DESIGNED - BAS

CHECKED - LVM

CHECKED - BAS

DRAWN

JSER NAME = baswanson

PLOT DATE = 4/22/2022

	GIRDER REACTION TABLE					
	R ₽ (k)	R Ł (k)	Rim (k)	R Total (k)		
Span 1, Beam 9-15	40.3	36.1	9.2	85.6		
Span 2, Beam 9	50.4	32.4	7.7	90.5		
Span 2, Beam 10	47.2	31.7	7.8	86.7		
Span 2, Beam 11	39.4	31.1	7.8	78.3		
Span 2, Beam 12	36.4	30.3	7.8	74.5		
Span 2, Beam 13	33.5	29.3	7.8	70.6		
Span 2, Beam 14	30.6	28.2	7.7	66.5		

BILL OF MATERIAL

Item	Unit	Quantit
Structural Repair of Concrete (Depth ≤ 5 in.)	Sq. Ft.	251
Structural Repair of Concrete (Depth > 5 in.)	Sq. Ft.	32
Concrete Sealer	Sq. Ft.	283
Temporary Shoring and Cribbing	Each	13
Jack, Remove, and Reinstall Existing Bearings	Each	2

^{*} Includes 10% allowance for increase in deterioration.

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
Structural Repair of Concrete (Depth Greater Than 5 inches)

ELEVATION

(Looking West)

SUBSTRUCTURE REPAIR - PIER 1

S.N. 092-0009

SHEET NO. 34 OF 37 SHEETS

STRUCTURAL REPAIR OF CONCRETE TABLE

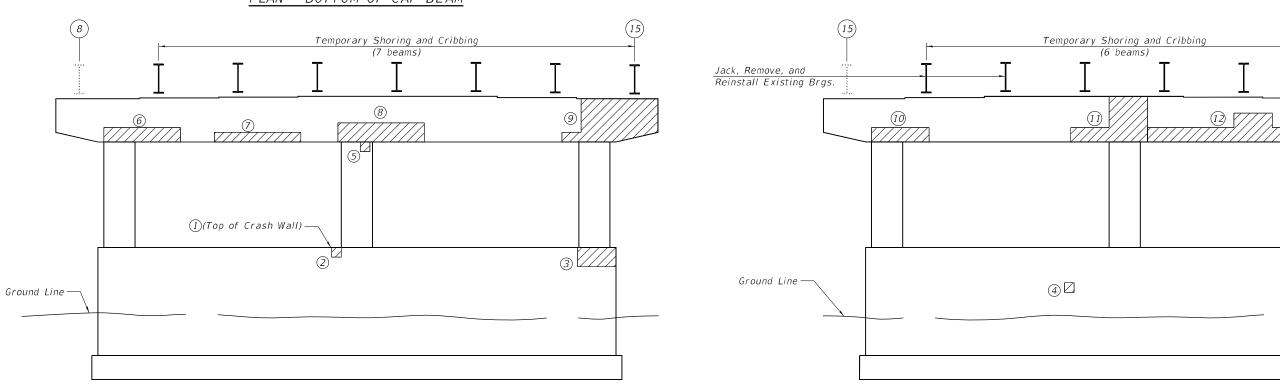
·		Plan	Qty.	As E	
Label	Size	SRC ≤5"	SRC >5"	SRC ≤5"	SRC >5"
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
1	2' x 1'	2			
2 3 4 5	1' x 1'	1			
3	4' x 2'	8			
4	1' x 1'	1			
5	1' x 1'	1			
6	8' x 1.5'	12			
7	9' x 1'	9			
8	9' x 2'	18			
9	8' x 4.5'	24			
10	6' x 1.5'	9			
11	8' x 4.5'	24			
12	16' x 2'	20			
13	5' x 4.5'	17			
14	1' x 1'	1			
15	1' x 1'	1			
16	1' x 1'	1			
17	3' x 1'	3			
18	4' x 2'	8			
19	8' x 4'		32		
20	6' x 4'	24			
21	1' × 1'	1			
22	1' × 1'	1			
23	4' x 2.5'	10			
24	4' x 2.5'	10			
25	4.5' x 4'	18			
26	4' x 1'	4			
	Totals	228	32		

SECTION

(92-10,11,12)R

VERMILION 698 318

CONTRACT NO. 70A29



STATE OF ILLINOIS

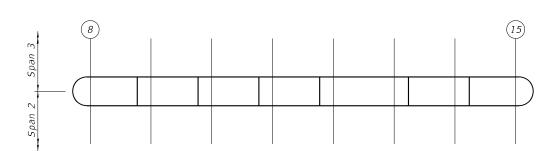
DEPARTMENT OF TRANSPORTATION

otes:

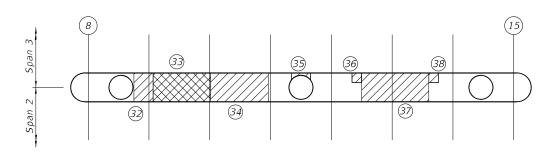
Pier repair areas shown are estimated from soundings on October 11 thru 16, 2020.

Actual size and locations of completed repairs shall be shown on this sheet and documented in the provide tables under "As Built".

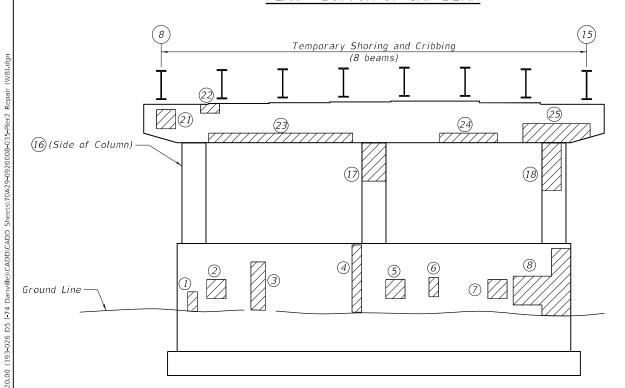
The minimum jack capacity shall be 50% greater than the maximum expected load on the jack.



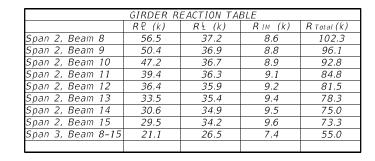
PLAN - TOP OF CAP BEAM



PLAN - BOTTOM OF CAP BEAM



ELEVATION (Looking East)



BILL OF MATERIAL

Item	Unit	Quantity
Structural Repair of Concrete (Depth ≤ 5 in.)	Sq. Ft.	408
Structural Repair of Concrete (Depth > 5 in.)	Sq. Ft.	18
Concrete Sealer	Sq. Ft.	426
Temporary Shoring and Cribbing	Each	16

st Includes 10% allowance for increase in deterioration.

LEGEND

	Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
	Structural Repair of Concrete (Depth Greater Than 5 inches)

Plan Qty $SRC \le 5''$ SRC > 5'' $SRC \le 5''$ SRC > 5'Sq. Ft. Sq. Ft. Sq. Ft. Sq. Ft.Label Size 2' x 2' 4 4' x 2' 8 7' x 1 2' x 2' 4 2' x 1' 2' x 2' 7' x 6' 25 2' x 2' 4 10 1' × 1' 1' × 1 12 2' x 1' 13 2' x 1' 14 5' x 3' 10 15 2' x 1' 16 4' x 2' 4' x 3' 12 12 18 6' x 2' 19 9' x 2' 18 20 10' x 5' 50 2' x 2 2' x 1' 22 23 15' x 1' 15 24 6' x 1' 6 25 7' x 2' 14 26 4' x 4' 13 27 28 24 6' x 4' 4 29 8' x 4.5' 36 30 15 15' x 1' 4' x 3' 12 32 3' x 2' 6 33 6' x 3' 18 34 6' x 3' 18 35 1' x 1' 36 1' x 1' 37 7' x 3' 21 38 1' x 1'

STRUCTURAL REPAIR OF CONCRETE TABLE

ELEVATION (Looking West)

S		USER NAME = baswanson	DESIGNED - BAS	REVISED -	
ME	MAURER-STUTZ		CHECKED - LVM	REVISED -	
z	ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - JDK	REVISED -	
E E		PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -	

Notes: Pier repair areas shown are estimated from soundings on October 11 thru 16, 2020. Actual size and locations of completed repairs shall be shown on this sheet and documented in the provide tables under "As Built". —(1)(Top of Cap Beam) Ground Line — ELEVATION - PIER 3 (Looking West) -(2)(Side of Crash Wall) Ground Line—

STRUCTURAL REPAIR OF CONCRETE TABLE

STATE OF STATE OF SOMETHER TABLE					
		Plan	Qty. SRC >5"	As E	3uilt
Label	Size	SRC ≤5"	SRC >5"	SRC ≤5"	SRC >5"
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
1	1' x 1'	1			
2	1' x 1' 1' x 1'	1			
3	4' x 1.5'	6			
	Totals	8			
	, 50015	J			

BILL OF MATERIAL

Item	Unit	Quantity
Structural Repair of Concrete (Depth ≤ 5 in.)	Sq. Ft.	8

<u>LEGEND</u>

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
Structural Repair of Concrete (Depth Greater Than 5 inches)

ELEVATION - PIER 4 (Looking West)

. S		USER NAME = baswanson	DESIGNED - BAS	REVISED -	
ME.	MAURER-STUTZ		CHECKED - LVM	REVISED -	
Ž	ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - JDK	REVISED -	
		PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -	

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

 SUBSTRUCTURE REPAIR - PIERS 3 & 4
 F.A.I. RTE.
 SE

 S.N. 092-0009
 74
 (92-1)

STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

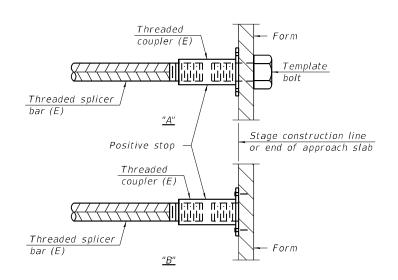
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

STR. NO. 092-0008

Location	Bar size	No. assemblies required	Minimum lap length
Bridge Deck	#5	24	3'-6"
Diaphragm	#6	10	4'-0"
Diaphragm	#4	4	2'-5"
Approach Slab	#5	92	3'-4"
Approach Slab	#8	120	4'-9"
Approach Footing	#5	80	3'-2"
Abutments	#5	8	3'-2"

STR. NO. 092-0009

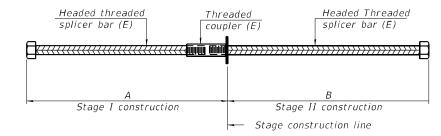
Location	Bar size	No. assemblies required	Minimum Iap length
Bridge Deck	#5	56	3'-6"
Diaphragm	#6	10	4'-0"
Diaphragm	#4	4	2'-5"
Approach Slab	#5	81	3'-4"
Approach Slab	#8	106	4'-9"
Approach Footing	#5	80	3'-2"
Abutments	#5	8	3'-2"



INSTALLATION AND SETTING METHODS

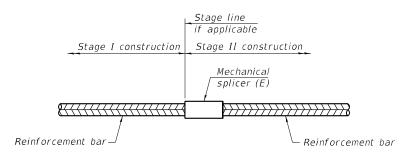
"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.



HEADED BAR SPLICER ASSEMBLY PLAN

Location	Bar	No. assemblies	Λ	R
LUCATION	size	size required A		Б
Diaphragms -0008	#6	8	3'-0"	3'-0"
Diaphragms -0009, W. Abut.	#6	4	4'-2"	3'-4"
Diaphragms -0009, E. Abut.	#6	4	3'-3"	2'-7"



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for

See approved list of bar splicer assemblies and mechanical s alternatives.

USER NAME = baswanson DESIGNED - BAS REVISED
CHECKED - LVM REVISED
PLOT SCALE = PLOT DATE = 4/22/2022 CHECKED - BAS REVISED
PLOT DATE = 4/22/2022 CHECKED - BAS REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

 BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS NO.

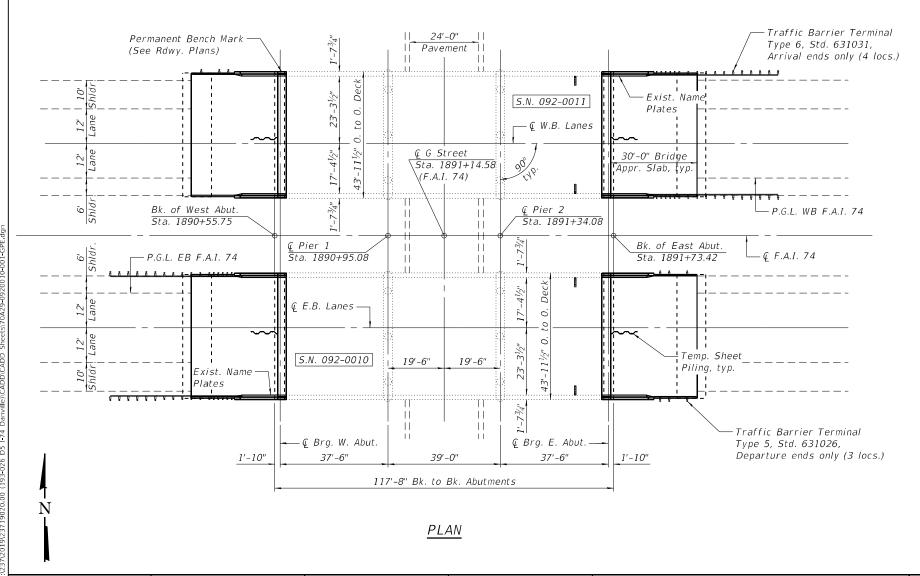
 STRUCTURE NO. 092-0008 & 092-0009
 4 (92-10,11,12)R
 VERMILION
 698
 321

 SHEET NO. 37 OF 37 SHEETS
 SHEETS
 LILINOIS FED. AD PROJECT

Benchmark - Survey disk on top of the NW wingwall of Structure No. 092-0011. Elev. 656.07. Existing Structures - Structure No. 092-0010 & 092-0011 were built in 1962 as FAI 74, Section 92-11HB-3. In 2000, the bridge decks were replaced. The structures consist of steel beam and reinforced concrete deck superstructures supported on pile bent abutments and multiple column piers. Overall bridge lengths are 117'-8", with clear widths between parapets of 40'-8". One lane of traffic in each direction is to be maintained using Salvage - None.

39'-0" Limits of Traffic Barrier Terminal — Type 6, Std. 631031, Exist. W24 — Protective Shield (ctr. - ctr. of piers) Beams Appr. ends only 14'-2" min. vert. cl.

ELEVATION



PROPOSED SCOPE OF WORK

- 1. Rehabilitate bridge deck with scarification and latex concrete overlay
- Remove and replace bridge approach slabs
- Replace steel beam bearing assemblies at abutments
- Remove existing abutment deck joints and reconstruct with jointless semi-integral abutments
- Repair concrete deterioration on piers and abutments
- Seal cracks in concrete slope walls
- Straighten and reinforce steel fascia beam
- Paint beam ends

LOADING HS20-44

DESIGN SPECIFICATIONS

2002 AASHTO Standard Specifications for Highway Bridges, 17th Edition (LFD)

DESIGN STRESSES

FIELD UNITS

New Construction:

 $f'c = 3,500 \ psi$ f'c = 4,000 psi (deck)

fy = 60,000 psi (Reinforcement) fy = 36,000 psi (M270 Grade 36)

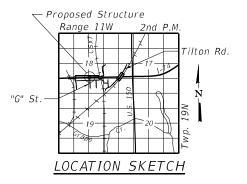
Exist. Structure:

f'c = 3,500 psi

fy = 60,000 psi (deck reinf.)

fs = 20,000 psi (orig. substr. reinf.)

fs = 18,000 (structural steel)





SHEET NO. 1 OF 23 SHEETS

Date Signed: 4-22-22 Exp. Date: 11/30/2022

GENERAL PLAN AND ELEVATION I-74 OVER "G" ST. F.A.I. 74 - SEC. (92-10,11,12)R **VERMILION COUNTY** STATION 1891+14.58 STR. NO. 092-0010 & 092-0011

	USER NAME = baswanson	DESIGNED - LVM	REVISED -
≠ MAURER-STUTZ		CHECKED - BAS	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -

GENERAL NOTES

All new fasteners shall be high-strength steel bolts. Holes for 3/4" Ø bolts shall be $^{13}\!\!/_{16}$ " Ø, unless noted otherwise. Holes for $^{7}\!\!/_{8}$ " Ø bolts shall be $^{15}\!\!/_{16}$ " Ø, unless noted otherwise.

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that can not be removed by grinding $\frac{1}{4}$ in. deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas, and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Existing Name Plates shall be removed, cleaned and incorporated into the new construction. Cost included with Relocating Name Plates.

Synthetic Fibers shall be included in the bridge deck concrete overlay specified. See Special Provisions.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this

Cleaning and painting of the existing structural steel shall be as specified in the special provision for "Cleaning and Painting Existing Steel Structures". After removal of deck ends and diaphragms, but prior to new bearing installation and encasement of the steel beams, all beams within 5 ft of the beam ends shall be cleaned per Near White Blast Cleaning (SSPC-SP10).

The designated areas cleaned per Near White Blast Cleaning (SSPC-SP10) shall be painted according to the requirements of Paint System 1 (0Z/E/U). The color of the final finish coat for all steel surfaces shall be Gray, Munsell

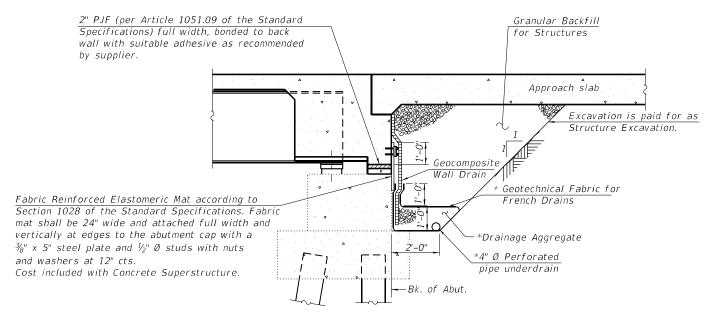
Care shall be taken not to damage rubber bearing or joint components during blasting and cleaning operations. Any damage to these components shall be repaired at the Contractor's expense.

SSPC-QP1 and SPC-QP2 Painting Contractor Certifications will be required for this contract.

Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel

All new structural steel shall be hot dip galvanized except where noted otherwise. See Special Provisions for "Hot Dip Galvanizing for Structural

Existing structure plans are available from the District upon request.



SECTION THRU SEMI-INTEGRAL ABUTMENT

*Included in the cost of Pipe Underdrains for Structures 4".

All drainage system components shall extend between the wingwalls at each end of abutment, except an outlet pipe shall extend until intersecting with the side slopes or slope wall surface. Any slope wall removal and replacement required for the installation of the outlet pipe shall be included in the cost of Pipe Underdrains for Structures 4". The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway

	USER NAME = baswanson	DESIGNED - LVM	REVISED -
MAURER-STUTZ		CHECKED - BAS	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -

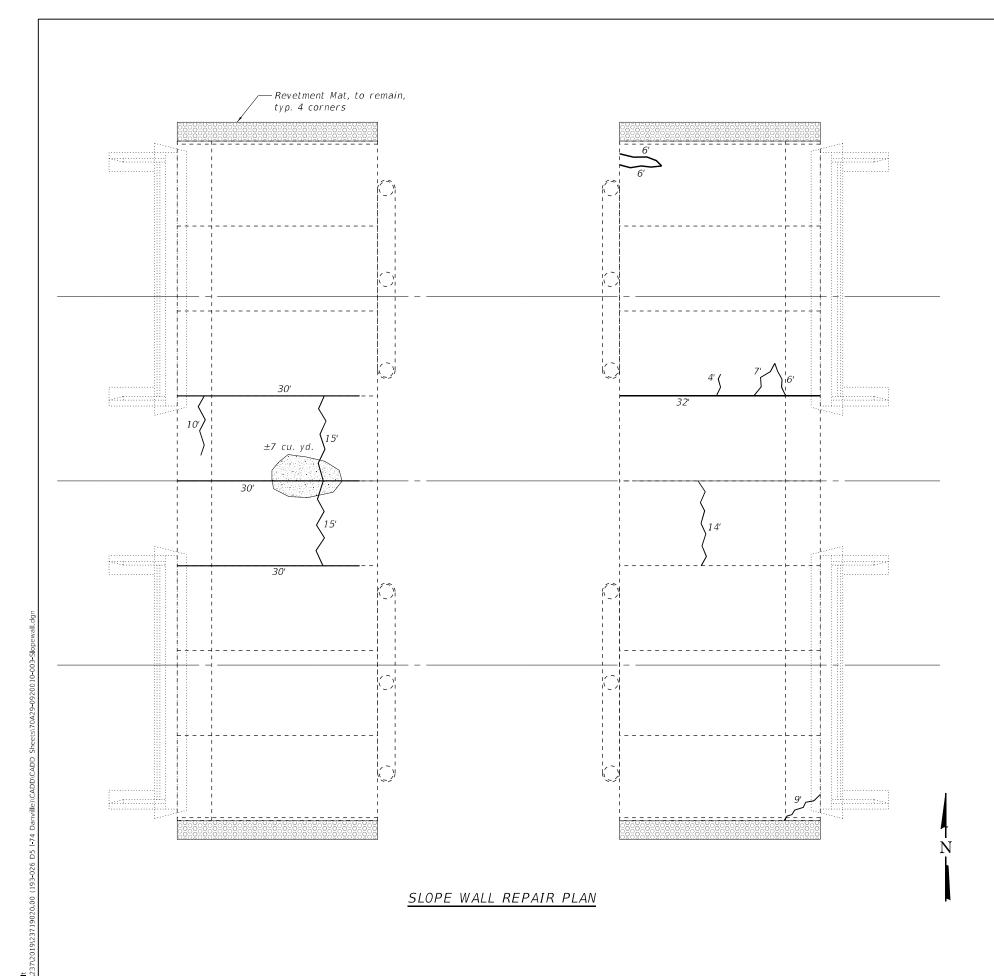
TOTAL BILL OF MATERIAL

	TOTAL BILL OF	MATERIAL					
	ITEM	UNIT	-0010	-0011	TOTAL		
	Concrete Removal	Cu. Yd.	36.6	36.6	73.2		
	Protective Shield	Sq. Yd.	190	190	380		
	Structure Excavation	Cu Yd.	66	66	132		
	Concrete Structures	Cu. Yd.	33.9	33.9	67.8		
	Concrete Superstructure	Cu. Yd.	36.3	36.2	72.5		
*	Protective Coat	Sq. Yd.	832	832	1664		
	Concrete Superstructure (Approach Slab)	Cu. Yd.	123.4	123.4	246.8		
	Furnishing and Erecting Structural Steel	Pound	2980	2980	5960		
	Reinforcement Bars, Epoxy Coated	Pound	54930	54930	109860		
	Bar Splicers	Each	344	344	688		
	Elastomeric Bearing Assembly, Type I	Each	16	16	32		
	Anchor Bolts, 1"	Each	32	32	64		
	Temporary Sheet Piling	Sq. Ft.	148	148	296		
	Granular Backfill for Structures	Cu. Yd.	62	60	122		
	Geocomposite Wall Drain	Sq. Yd.	46	45	91		
	Pipe Underdrains for Structures 4"	Foot	128	128	256		
	Slope Wall Crack Sealing	Foot			214		
	Bridge Deck Grooving (Longitudinal)	Sq. Yd.	468	468	936		
	Jack and Remove Existing Bearings	Each	16	16	32		
	Structural Steel Removal	Pound	3680	3680	7360		
	Structural Steel Repair	Pound	250		250		
	Beam Straightening	L. Sum	1		1		
	Bridge Deck Latex Concrete Overlay 2¾"	Sq. Yd.	496	496	992		
	Bridge Deck Scarification ¾"	Sq. Yd.	496	496	992		
	Containment and Disposal of Lead Paint Cleaning Residues No. 1	L. Sum	1		1		
	Containment and Disposal of Lead Paint Cleaning Residues No. 2	L. Sum		1	1		
	Cleaning and Painting Steel Bridge No. 1	L. Sum	1		1		
	Cleaning and Painting Steel Bridge No. 2	L. Sum		1	1		
	Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	5	10	15		
	Deck Slab Repair (Full Depth, Type II)	Sq. Yd.	2	2	4		
	Diamond Grinding (Bridge Section)	Sq. Yd.	716	716	1432		
	Relocating Name Plates	Each	2	2	4		
	Slope Wall Slurry Pumping	Cu. Yd.			7		

* On new concrete superstructure, approach slabs, and latex concrete overlay.

INDEX OF SHEETS

- General Plan and Elevation
- General Data
- Slope Wall Plan
- Stage Construction Details Temporary Concrete Barrier
- Deck Slab Repair Plan (S.N. 092-0010)
- Deck Slab Repair Plan (S.N. 092-0011)
- Superstructure Details
- Superstructure Details
- 10. Diaphragm Details
- 11-12. Bridge Approach Slabs
- 13-15. Structural Steel
- Bearing Details 17-20. Abutments
- 21-22. Pier Repairs
- Bar Splicer Assemblies



<u>LEGEND</u>

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Slope Wall Slurry Pumping (Volume)

#

Slope Wall Crack Sealing (Length)

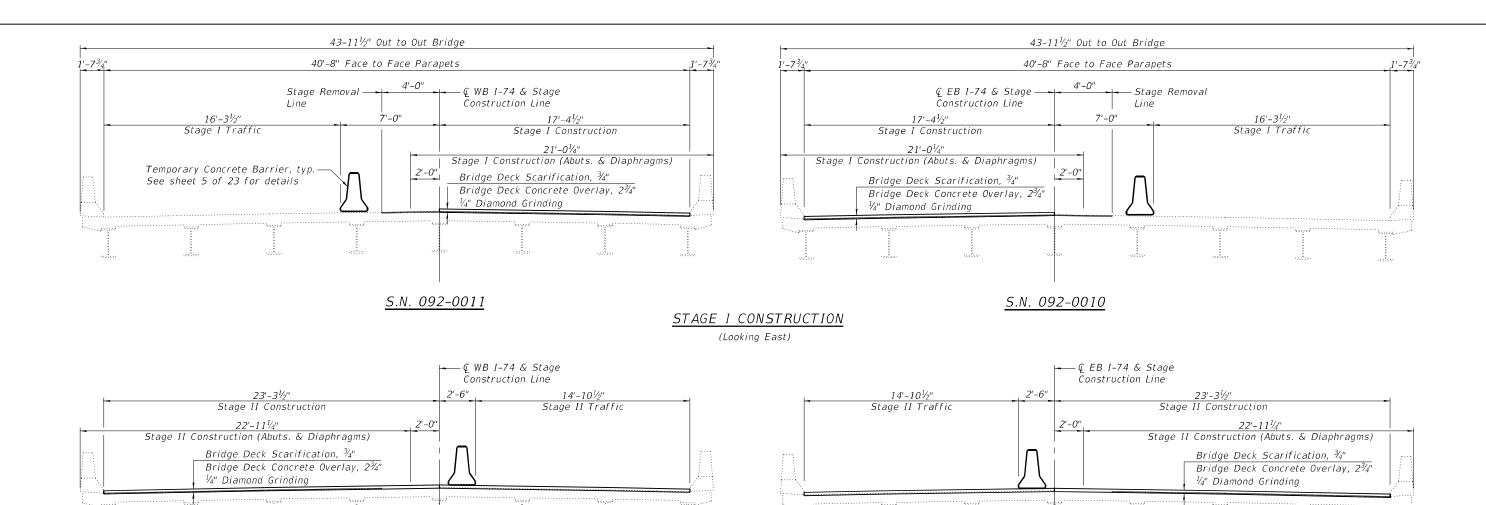
Note:

The repair areas shown are estimated based on a field inspection conducted in December 2020. The actual repair areas required shall be verified according to the Special Provisions.

<u>SLOPE WALL</u> BILL OF MATERIAL

Item	Unit	Quantity
Slope Wall Slurry Pumping	Cu. Yd.	7
Slope Wall Crack Sealing	Foot	214

DESIGNED - LVM REVISED -SECTION **SLOPE WALL PLAN** STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION MAURER-STUTZ CHECKED - BAS REVISED -(92-10,11,12)R VERMILION 698 324 STRUCTURE NO. 092-0010 & 092-0011 DRAWN - LVM REVISED -CONTRACT NO. 70A29 PLOT DATE = 4/22/2022 CHECKED - BAS REVISED -SHEET NO. 3 OF 23 SHEETS



<u>S.N. 092-0011</u>

STAGE II CONSTRUCTION
(Looking East)

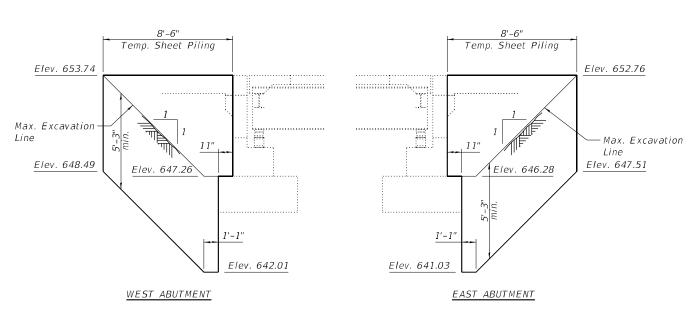
S.N. 092-0010

Notes:

Sheet piling shall have a minimum section modulus of 2.5 in³/ft.

If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the Engineer.

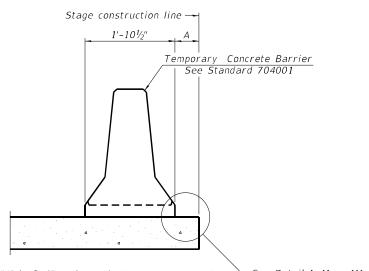
The Contractor shall connect the first sheet to the existing abutment wall to ensure stability of sheets driven to the top of the existing footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Piling.



TEMPORARY SHEET PILING

(Looking North, typ. both bridges)

efat :: S:		USER NAME = baswanson	DESIGNED - LVM	REVISED -		STAGING DETAILS	F.A.I. RTF	SECTION	COUNTY TOT	TAL SHE	ĒΠ
AME.	🖊 MAURER-STUTZ		CHECKED - BAS	REVISED -	STATE OF ILLINOIS	STRUCTURE NO. 092-0010 & 092-0011	74	(92-10,11,12)R	VERMILION 69	398 37	5
DE N	ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -	DEPARTMENT OF TRANSPORTATION	31KUCTUKE NO. 032-0010 & 032-0011			CONTRACT NO	IO. 70A29	П
Q ⊒		PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -		SHEET NO. 4 OF 23 SHEETS		ILLINOIS FED. AID	PROJECT		\neg



∽ See Detail I, II or III When "A" is 3'-1" or less, the temporary concrete barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

NEW SLAB OR NEW DECK BEAM

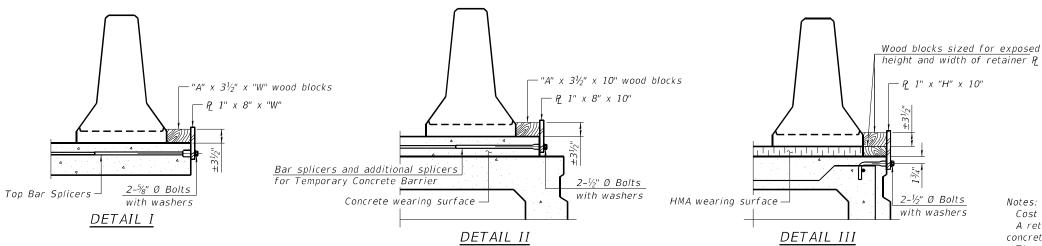
- Stage removal line ← Stage removal line 1'-101/2" 1'-101/2" Temporary Concrete Barrier See Standard 704001 6" min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint is required when "A" is greater than 3'-1".

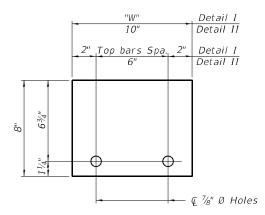
* When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

EXISTING DECK BEAM

SECTIONS THRU SLAB OR DECK BEAM

EXISTING SLAB





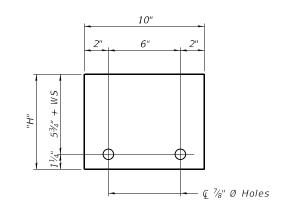
RAILING CRITERIA

NCHRP 350 Test Level Railing Weight (plf)

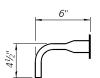
10-12-2021

STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)



STEEL RETAINER P 1" x "H" x 10" (Detail III)



RESTRAINING PIN

BAR SPLICER FOR #4 BAR - DETAIL III

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate (of each temporary concrete barrier.

1x8 UNC

1" Ø pin

US Std. 11/16" I.D. x 21/2" O.D. x approx. 8 gauge thick washer

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

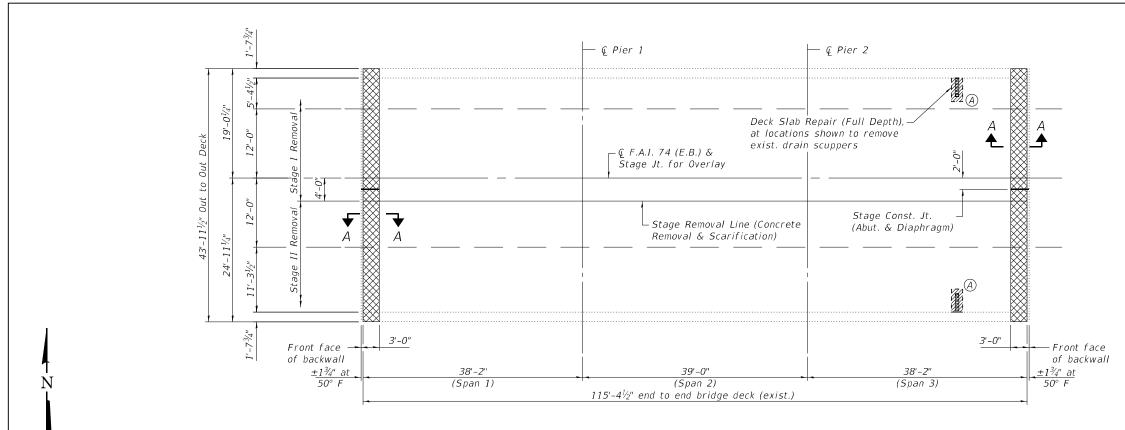
	USER NAME = baswanson	DESIGNED - LVM	REVISED -
MAURER-STUTZ		CHECKED - BAS	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION **TEMPORARY CONCRETE BARRIER** (92-10,11,12)R VERMILION 698 326 STRUCTURE NO. 092-0010 & 092-0011 CONTRACT NO. 70A29 SHEET NO. 5 OF 23 SHEETS

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R-27



LEGEND

	Deck Slab Repair (Partial Depth) (For Information Only)
	Deck Slab Repair (Full Depth)
	Concrete Removal

<u>PLAN - DECK SLAB REPAIRS</u>

* Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorportated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

DECK SLAB REPAIR TABLE

		F	Plan Qty	/.	As I	Built
Label	Size	Deck Slab Repair (Part. Depth)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
Α	4' x 2' x 2 Loc.			16		
	Totals			16		

¾" Bridge Deck Scarification 3'-0" See Rdwy. plans for ¾" Sawcut Approach Slab Removal Appr. Slab * Exist. Reinf. Existing W24 beam See Sheets 17 thru 19 of 23 for backwall and hatch block removal See Sheet 14 of 23 for steel diaphragm removal

SECTION A-A

See Sheet 9 of 23 for cross section thru bridge deck.

Deck survey performed on December 17, 2020. Locations and sizes shown in the plan view are approximate.

Deck Slab Repair (Partial Depth) is an estimated quantity per the deck survey. This area shall not be paid for separately, but shall be addressed as stated in the Special Provision for Bridge Deck Latex Concrete Overlay.

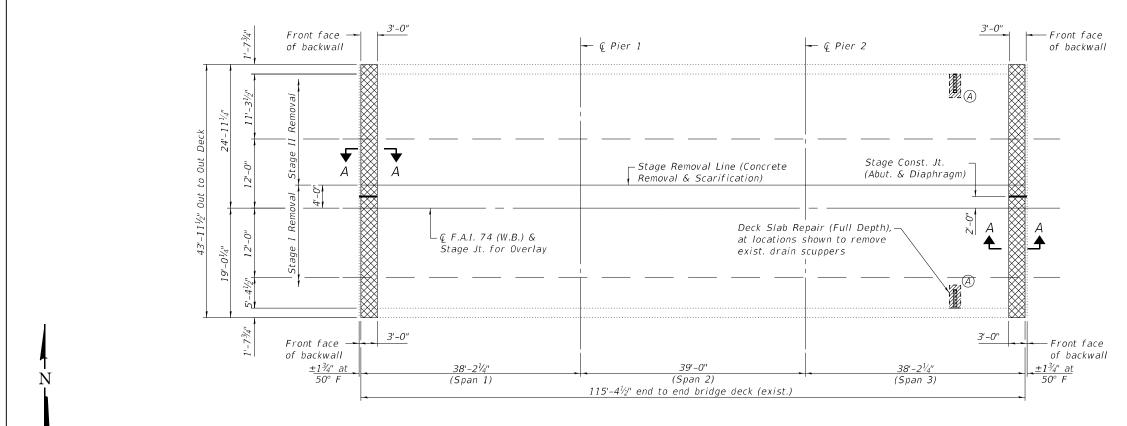
Actual size and locations of full-depth patches shall be shown on this sheet and documented in the Deck Slab Repair Table under "As Built".

DECK SLAB (S.N. 092-0010) BILL OF MATERIAL

ITEM	UNIT	TOTAL
Deck Slab Repair (Full Depth, Type II)	Sq. Yd.	2
Bridge Deck Scarification 3/4"	Sq. Yd.	496
Bridge Deck Latex Concrete Overlay, 2 ³ / ₄ Inches	Sq. Yd.	496

(Sheet 1 of 5)

						(=======					
Š		USER NAME = baswanson	DESIGNED - LVM	REVISED -		DECK SLAB REPAIR PLAN	F.A.I.	SECTION	COUNTY	TOTAL	SHEET
Ä	MAURER-STUTZ		CHECKED - BAS	REVISED -	STATE OF ILLINOIS		74	(92-10,11,12)R	VERMILION	698	327
≥	ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 092-0010		(,,,	CONTRAC	T NO. 70	A29
Ξl		PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -		SHEET NO. 6 OF 23 SHEETS		ILLINOIS FED. AI			



LEGEND

Deck Slab Repair (Partial Depth) (For Information Only) Deck Slab Repair (Full Depth) Concrete Removal

<u>PLAN - DECK SLAB REPAIRS</u>

* Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorportated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

DECK SLAB REPAIR TABLE

		F	Plan Qty	/	As I	3uilt
Label	Size	Deck Slab Repair (Part. Depth)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)
		5q. Ft.	5q. Ft.		Sq. Ft.	Sq. Ft.
A	4' x 2' x 2 Loc.			16		
	Totals			16		

See Sheet 9 of 23 for cross section thru bridge deck. See Sheet 6 of 23 for Section A-A.

Deck survey performed on December 15, 2020. Locations and sizes shown in the plan view are approximate.

Deck Slab Repair (Partial Depth) is an estimated quantity per the deck survey. This area shall not be paid for separately, but shall be addressed as stated in the Special Provision for Bridge Deck Latex Concrete Overlay.

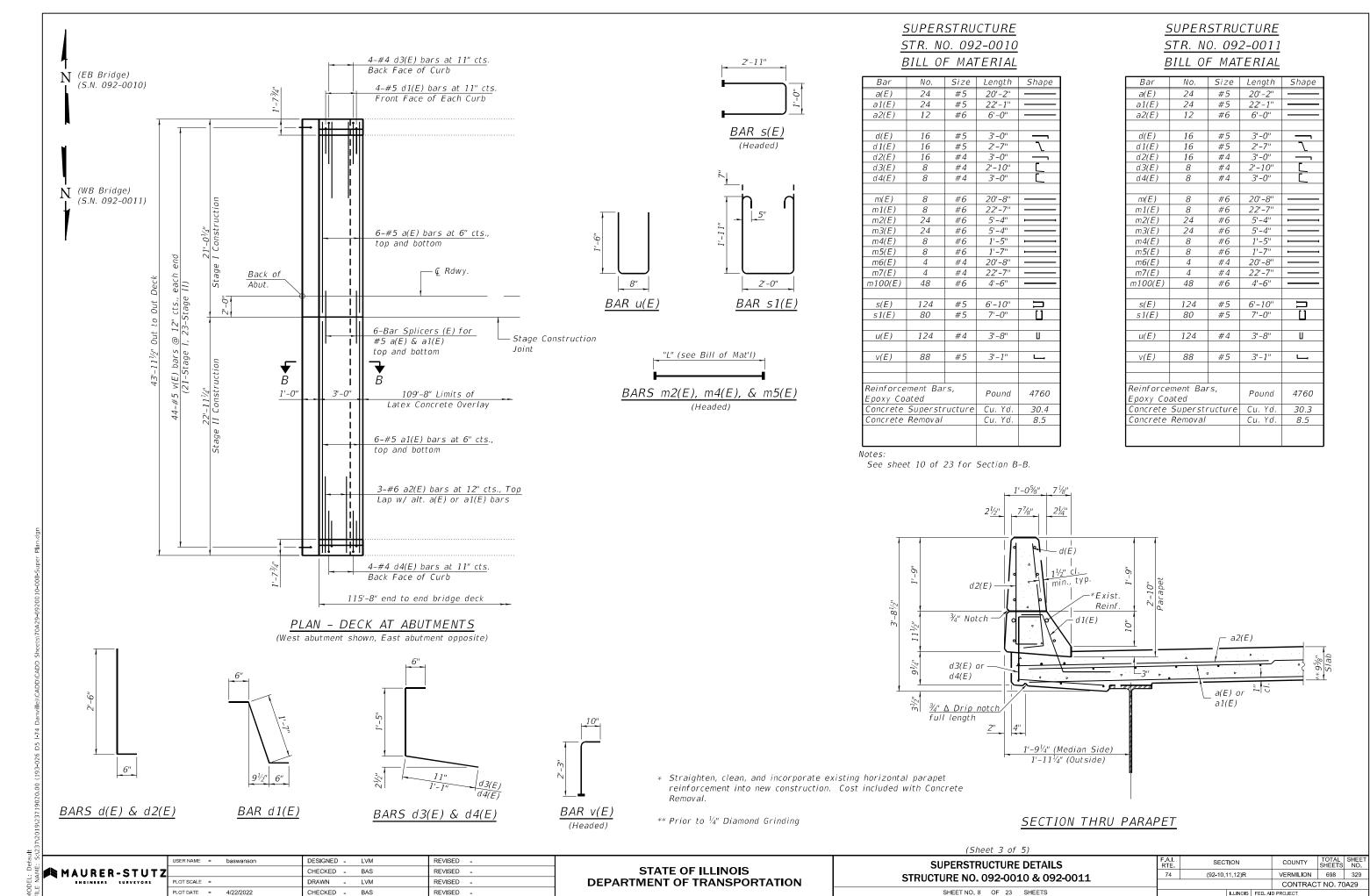
Actual size and locations of full-depth patches shall be shown on this sheet and documented in the Deck Slab Repair Table under "As Built".

DECK SLAB (S.N. 092-0011) BILL OF MATERIAL

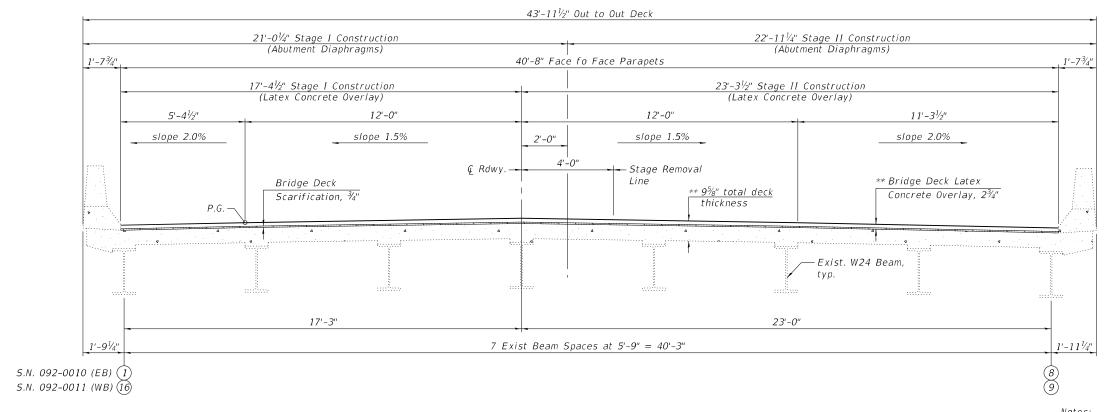
ITEM	UNIT	TOTAL
Deck Slab Repair (Full Depth, Type II)	Sq. Yd.	2
Bridge Deck Scarification 3/4"	Sq. Yd.	496
Bridge Deck Latex Concrete Overlay, 2 ³ / ₄ Inches	Sq. Yd.	496

(Sheet 2 of 5)

<u>v</u> .:	USER NAME = baswanson	DESIGNED - LVM	REVISED -		DECK SLAB REPAIR PLAN	F.A.I.	SECTION	COUNTY	TOTAL SHEE	ſ
MAURER-STUTZ		CHECKED - BAS	REVISED -	STATE OF ILLINOIS		74	(92-10.11.12)R	VERMILION	698 328	1
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 092-0011		(,,,,	CONTRACT	T NO. 70A29	1
2 2 1	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -		SHEET NO. 7 OF 23 SHEETS		ILLINOIS FED.	AID PROJECT		1



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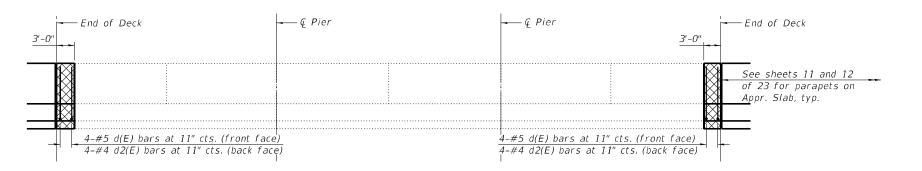
CROSS SECTION

(Looking East for East Bound, Looking West for West Bound) Notes:

See sheet 8 of 23 for Bill of Material, Section Thru Parapet, and bar details.

Cross-hatched areas on parapets indicate Concrete Removal and replacement with Concrete Superstructure.

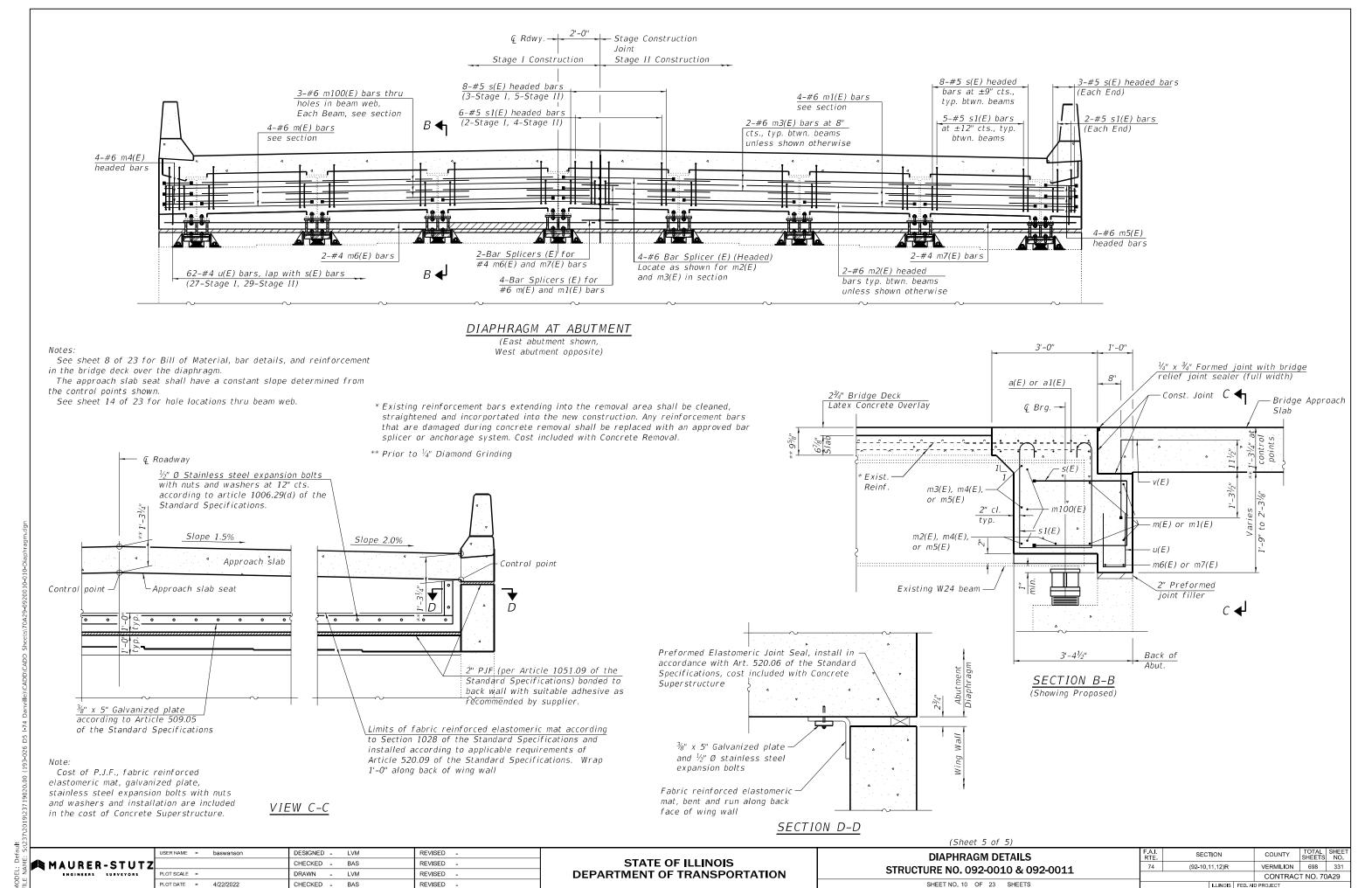
** Prior to 1/4" Diamond Grinding

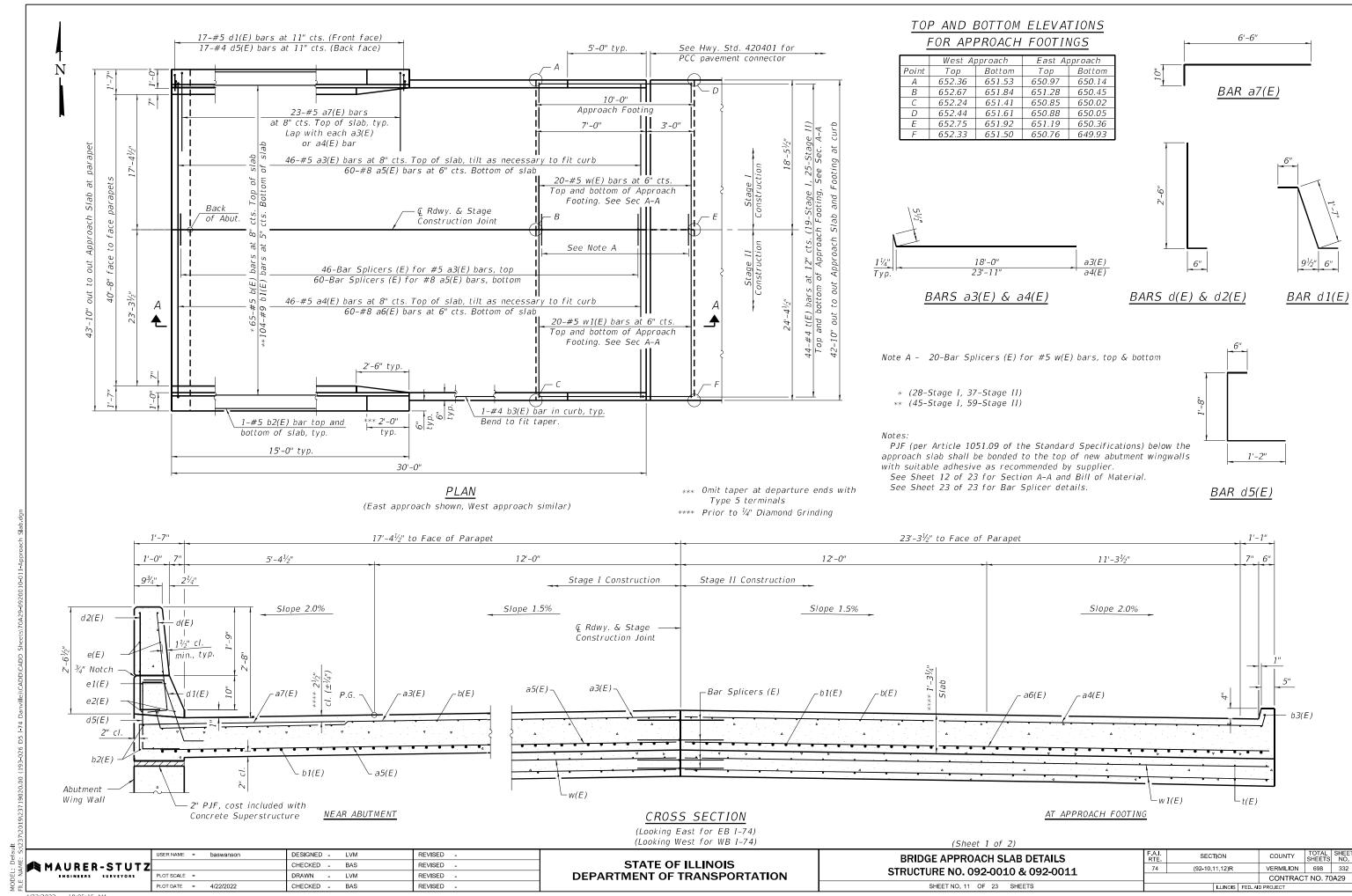


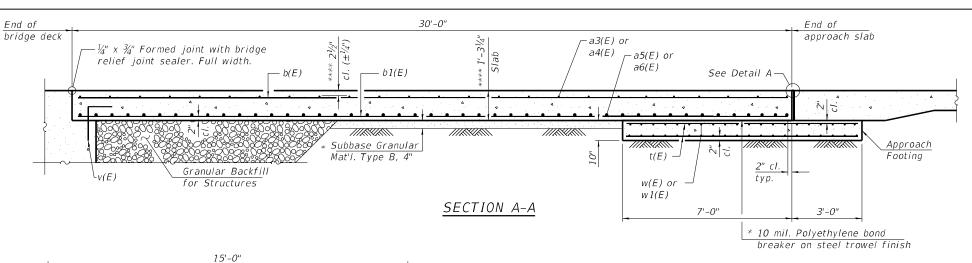
PARAPET ELEVATION

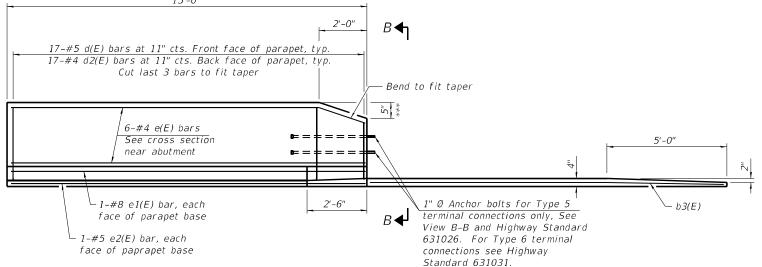
(Sheet 4 of 5)

JSER NAME = baswanson DESIGNED - LVM REVISED -SECTION SUPERSTRUCTURE DETAILS COUNTY STATE OF ILLINOIS MAURER-STUTZ CHECKED - BAS REVISED -(92-10,11,12)R VERMILION 698 330 STRUCTURE NO. 092-0010 & 092-0011 **DEPARTMENT OF TRANSPORTATION** DRAWN - LVM REVISED CONTRACT NO. 70A29 SHEET NO. 9 OF 23 SHEETS PLOT DATE = 4/22/2022 CHECKED - BAS REVISED -

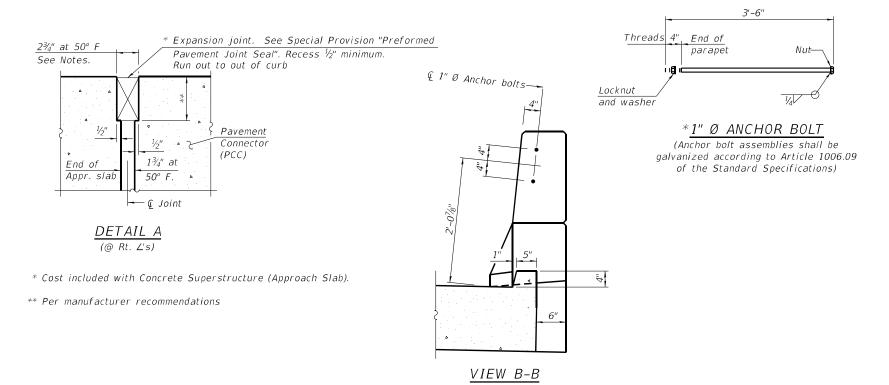








INSIDE ELEVATION OF PARAPET AND CURB



Notes:

The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. The length of bridge used to calculate the adjustment shall be equal to the distance from the fixed bearing to the end of the bridge approach slab.

Parapet concrete shall be paid for as Concrete Superstructure.

Approach slab shall be paid for as Concrete Superstructure (Approach Slab). Approach footing concrete shall be paid for as Concrete Structures.

The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.Cost of excavation for approach footing included with Concrete Structures.

For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 23.

Adjust the profile grade as needed across the length of the bridge approach slab to match top of bridge deck overlay at the abutment.

*** Omit taper at departure ends with Type 5 Terminals

**** Prior to 1/4" Diamond Grinding

STR. NO. 092-0010 TWO APPROACHES BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a3(E)	92	#5	18'-6"	Shape
a3(E) a4(E)	92	#5	24'-5"	
a5(E)	120	#8	18'-1"	
a6(E)	120	#8	24'-0"	
a7(E)	92	#5	7'-4"	
a/(L)	92	# 3	7 -4	<u> </u>
b(E)	130	#5	29'-8"	
b1(E)	208	#9	29'-8"	
b2(E)	8	#5	14'-8"	
b3(E)	4	#4	14'-8"	-
d(E)	68	#5	3'-0"	
d1(E)	68	#5	2'-7"	
d2(E)	68	#4	3'-0"	
d5(E)	68	#4	3'-4"	<u> L </u>
e(E)	24	#4	14'-8"	
e1(E)	8	#8	14'-8"	
e2(E)	8	#5	14'-8"	
+/=\	176	11.0	01 011	
t(E)	176	#4	9'-8"	
w(E)	80	#5	18'-1"	
w1(E)	80	#5	24'-0"	
Concrete	Superst	l ructure	Cu. Yd.	5.9
Concrete (Approac	Superst h Slab)	Cu. Yd.	123.4	
	Structui	res	Cu. Yd.	26.4
Reinforc Epoxy Co	ement Ba	Pound	49470	

STR. NO. 092-0011 TWO APPROACHES BILL OF MATERIAL

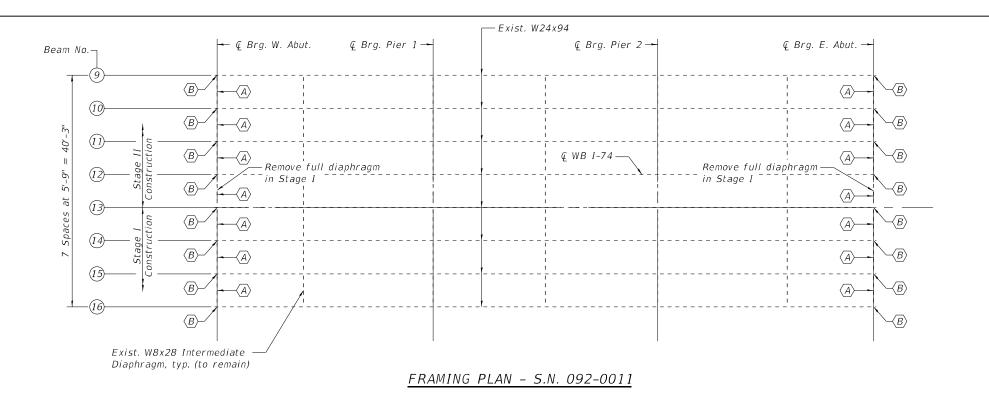
Bar	No.	Size	Length	Shape					
a3(E)	92	#5	18'-6"						
a4(E)	92	#5	24'-5"						
a5(E)	120	#8	18'-1"						
a6(E)	120	#8	24'-0"						
a7(E)	92	#5	7'-4"						
b(E)	130	#5	29'-8"						
b1(E)	208	#9	29'-8"						
b2(E)	8	#5	14'-8"						
b3(E)	4	#4	14'-8"						
d(E)	68	#5	3'-0"						
d1(E)	68	#5	2'-7"						
d2(E)	68	#4	3'-0"						
d5(E)	68	#4	3'-4"						
e(E)	24	#4	14'-8"						
e1(E)	8	#8	14'-8"						
e2(E)	8	#5	14'-8"						
t(E)	176	#4	9'-8"						
w(E)	80	#5	18'-1"						
w 1(E)	80	#5	24'-0"						
Concrete			Cu. Yd.	5.9					
Concrete (Approace		Cu. Yd.	123.4						
	Structui	res	Cu. Yd.	26.4					
Reinforce									
Epoxy Co		,	Pound	49470					
, , ,									

(Sheet 2 of 2)

	USER NAME = baswanson	DESIGNED - LVM	REVISED -
MAURER-STUTZ		CHECKED - BAS	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 092-0010 & 092-0011



				├ - Ç Brg. W. Abut		t. W24x94 & Brg. Pier .	2	Ç Brg. E. Abut. →	ı
Beam 1	Vo. 7			ψ br g. w. Abut		Ψ Dry. Free .	2	ų Dig. L. Abūt.	
1	<u>-</u> (1)—	1.	(B)-/	(A)		 		(A)	B
3.	(2)—	e I Ictior		(A)					B
Spaces at $5'-9'' = 40'-3''$	3—	Stage I Construction		\overrightarrow{A}	full diaphragm	 	Remove full d	iaphragm	B
5'-9'	(4)—	II	(B)	in Stage		 	in Stage I		B
baces at	5	Stage i	(B)-/	A		 		(A)	B
7 5	6)—	_ 3 _	(B)—/	(A)				(A)	B
	(7)— —(8)—		(B)-/	(A)		 		$A \rightarrow A$	B
	-8-		(B)—/						B
				I	I	±14'-0"			l W8x28 Intermediate agm, typ. (to remain)

| GIRDER REACTION TABLE (EXIST.) | S.N. 092-0010 & 092-0011 | Abutments | R ♀ (k) 14.2 | R ↓ (k) 31.2 | R IM (k) 9.4 | R Total (k) 54.8

* GIRD	IRDER REACTION TABLE (PROP.)				
S.N. 092-0010 & 092-0011					
Abutments					
R₽	(k)	39.0			
R Ł	(k)	32.8			
R IM	(k)	9.9			
R Total	(k)	81.7			

* New bearing assembly loads (includes approach slab)

Notoci

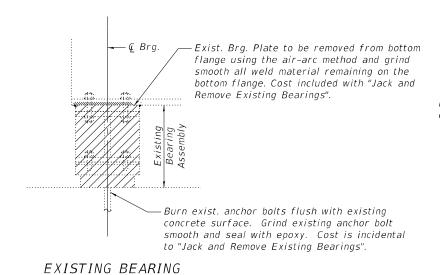
Removal of existing diaphragms shall be paid for as Structural Steel Removal. Removal of existing bearing assemblies shall be paid for as Jack and Remove Existing Bearings. See Sheet 14 of 23 for Bill of Material. Callouts:

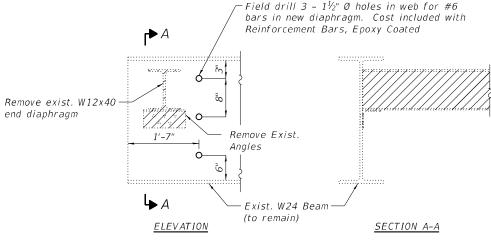
 $\langle A \rangle$ - Remove existing W12x40 end diaphragm - see Sheet 14 of 23 for details.

FRAMING PLAN - S.N. 092-0010

- $\langle B \rangle$ Jack and remove existing bearing assembly, replace with new bearing assemblies as shown on Sheet 16 of 23.
- $\langle C \rangle$ Existing Beam to be Straightened & Strengthened see Sheets 14 and 15 of 23 for details.

DESIGNED - LVM REVISED -JSER NAME = baswanson SECTION STRUCTURAL STEEL REMOVAL COUNTY STATE OF ILLINOIS MAURER-STUTZ CHECKED - BAS REVISED -(92-10,11,12)R VERMILION 698 334 STRUCTURE NO. 092-0010 & 092-0011 DRAWN -REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 70A29 SHEET NO. 13 OF 23 SHEETS PLOT DATE = 4/22/2022 REVISED . CHECKED -





END DIAPHRAGM REMOVAL

Removal of the existing steel extensions and side retainers at the abutments shall be included in the cost of Jack and Remove Existing Bearings.

Minimum jack capacity shall be 30 tons.

REMOVAL DETAIL

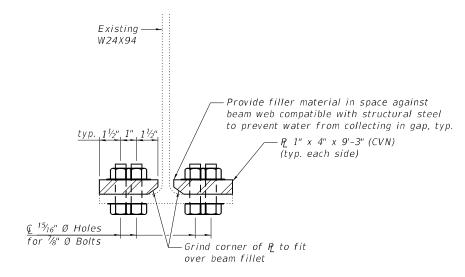
Removal of steel end diaphragms shall be paid for as Structural Steel Removal.

Strengthening plates shall be paid for as Structural Steel Repair.

Beam ends shall be painted as noted in the General Notes. Cost included with Cleaning and Painting Steel Bridge No. 1 (S.N. 092-0010) and Cleaning and Painting Steel Bridge No. 2 (S.N. 092-0011).

Following the beam straightening but prior to installation of the strengthening P_c , the beam flange area in contact with the strengthening P_c shall be cleaned and painted per the requirements for Primary Connections as specified in the Special Provision for "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

"CVN" denotes Charpy-V-Notch impact energy requirements, zone 2.



STRENGTHENING DETAIL

BILL OF MATERIAL STR. NO. 092-0010

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	16
Anchor Bolts, 1"	Each	32
Furnishing and Erecting Structural Steel	Pound	2980
Jack and Remove Existing Bearings	Each	16
Structural Steel Removal	Pound	3680
Structural Steel Repair	Pound	250

BILL OF MATERIAL STR. NO. 092-0011

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	16
Anchor Bolts, 1"	Each	32
Furnishing and Erecting Structural Steel	Pound	2980
Jack and Remove Existing Bearings	Each	16
Structural Steel Removal	Pound	3680

VERMILION 698 335

CONTRACT NO. 70A29

1½" 10 Spaces at 3" = 2'-6" 8 Spaces at 6" = 4'-0" 10 Spaces at 3" = 2'-6" 10

To be removed

BEAM 8 STRENGTHENING P

P 1" x 4" x 9'-3" (CVN) (2 Required)

	USER NAME = baswanson	DESIGNED - LVM	REVISED -
MAURER-STUTZ		CHECKED - BAS	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -

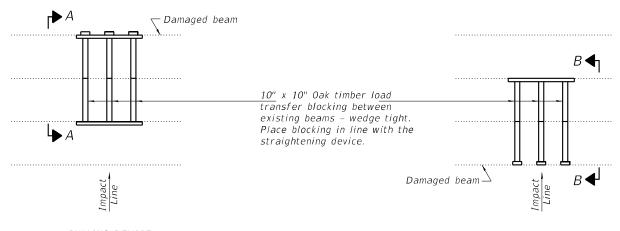
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

 STRUCTURAL STEEL DETAILS
 F.A.I. RTE.
 SECTION

 STRUCTURE NO. 092-0010 & 092-0011
 74
 (92-10,11,12)R

 SHEET NO. 14
 0F 23
 SHEETS
 ILLINOIS FEE

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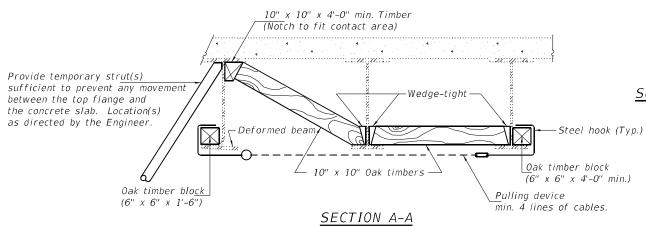


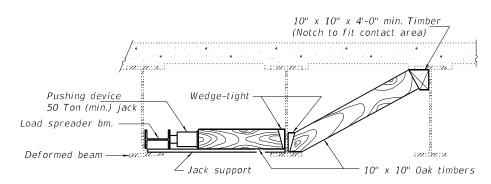
PULLING DEVICE

PUSHING DEVICE PARTIAL PLANS

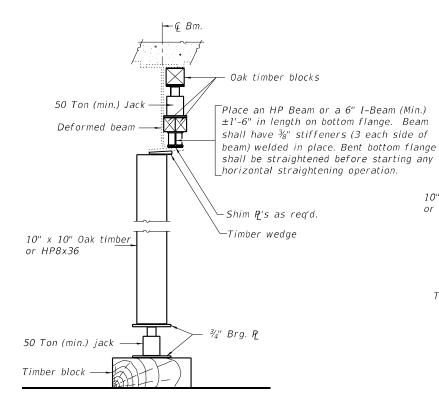
SUGGESTED BEAM STRAIGHTENING METHODS

Straightening force shall be maintained on all load transfer blocking during beam straightening.



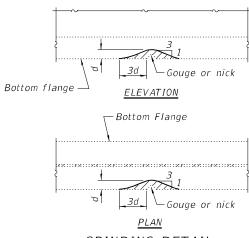


SECTION B-B



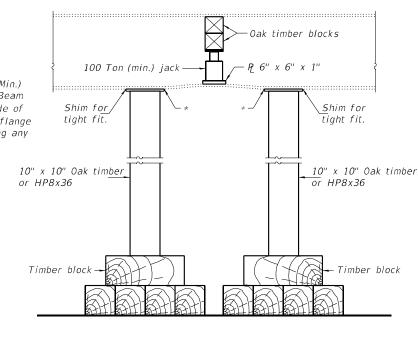
SUGGESTED VERTICAL STRAIGHTENING DETAIL

(To correct flange rotation.)



GRINDING DETAIL

Grind existing nicks, gouges and shallow cracks in the damaged beams as detailed. Ground surfaces shall be inspected for cracks using dye penetrant or magnetic particle testing prior to initiating any beam straightening operations. Any cracks that cannot be removed by grinding approximately $\frac{1}{4}$ " deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. Ground surfaces shall be spot cleaned and painted with an aluminum epoxy mastic primer followed by a finish coat to match the color of the existing beam. Cost of grinding, testing and spot painting included with Beam Straightening.



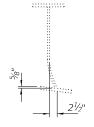
SUGGESTED VERTICAL STRAIGHTENING DETAIL

(To correct localized vertical flange deformations.)

* Edge of plate shall line up with edge of deformation.

Note:

Braces and jack assembly shall be placed on same side of web. Bent bottom flange shall be straightened before starting any horizontal straightening operations.



EXISTING DEFORMATION

TO BE STRAIGHTENED (Looking West)

(Approximate max. deflections) Deflected length of beam to be straightened is approximately 10'-0"

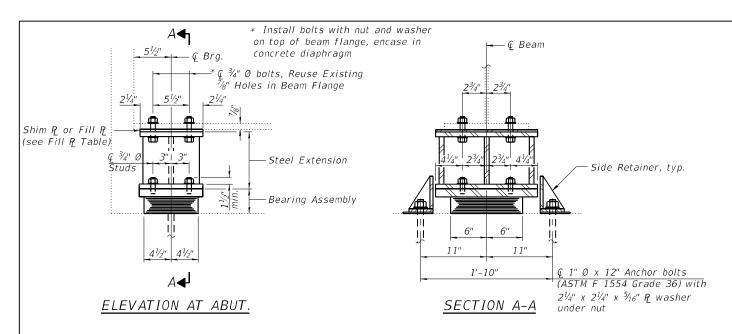
BILL OF MATERIAL STR. NO. 092-0010

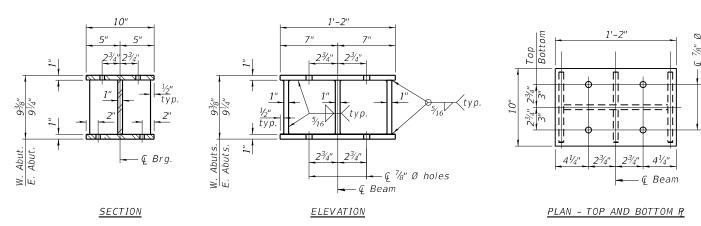
Item	Unit	Quantity
Beam Straightening	L. Sum	1

DESIGNED - LVM JSER NAME = baswanson REVISED -CHECKED - BAS REVISED -MAURER-STUTZ DRAWN - LVM REVISED PLOT DATE = 4/22/2022 CHECKED - BAS REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **BEAM STRAIGHTENING DETAILS STRUCTURE NO. 092-0010** SHEET NO. 15 OF 23 SHEETS

SECTION COUNTY (92-10,11,12)R VERMILION 698 336 CONTRACT NO. 70A29



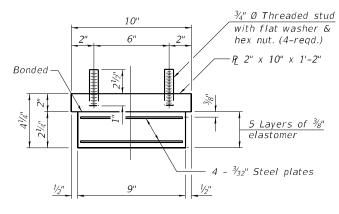


STEEL EXTENSION

TYPE I ELASTOMERIC EXP. BRG.

FILL PLATE THICKNESS

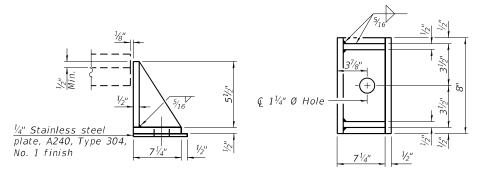
S.N. 092-001	O, W. Abut.	S.N. 092-001	O, E. Abut.
Beam 1	5/8"	Beam 1	3/4"
Beam 2	3/8"	Beam 2	5/8"
Beam 3		Beam 3	5/8"
Beam 4	1/4"	Beam 4	1"
Beam 5		Beam 5	3/8"
Beam 6		Beam 6	1/2"
Beam 7		Beam 7	1/2"
Beam 8		Beam 8	3/8"
S.N. 092-001	1, W. Abut.	S.N. 092-001	1, E. Abut.
Beam 9		Beam 9	3/8"
Beam 10	3/4"	Beam 10	3/8"
Beam 11	1/4"	Beam 11	
Beam 12	1/4"	Beam 12	
Beam 13	5/8"	Beam 13	1/4"
Beam 14	3/8"	Beam 14	3/4"
Beam 15	3/8"	Beam 15	5/8"
Beam 16	3/8"	Beam 16	3/4"



BEARING ASSEMBLY

(at Abutments)

Shim plates shall not be placed under bearing assembly.



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

Notes

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTMF1554.

Steel extensions, shim plates, and connection bolts shall be paid for as Furnishing and Erecting Structural Steel.

Side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.

Prior to ordering any material, the Contractor shall verify in the field all bearing height and shim thickness dimensions.

Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

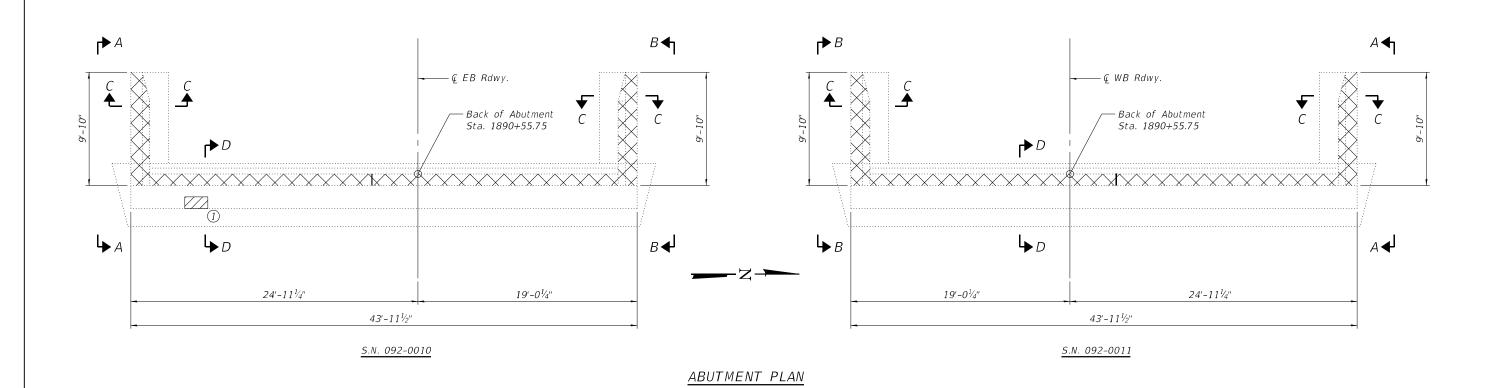
Provide two $\frac{1}{8}$ " adjusting shims for each bearing in addition to all other plates of the Bearing Assembly, and place as shown in the bearing details.

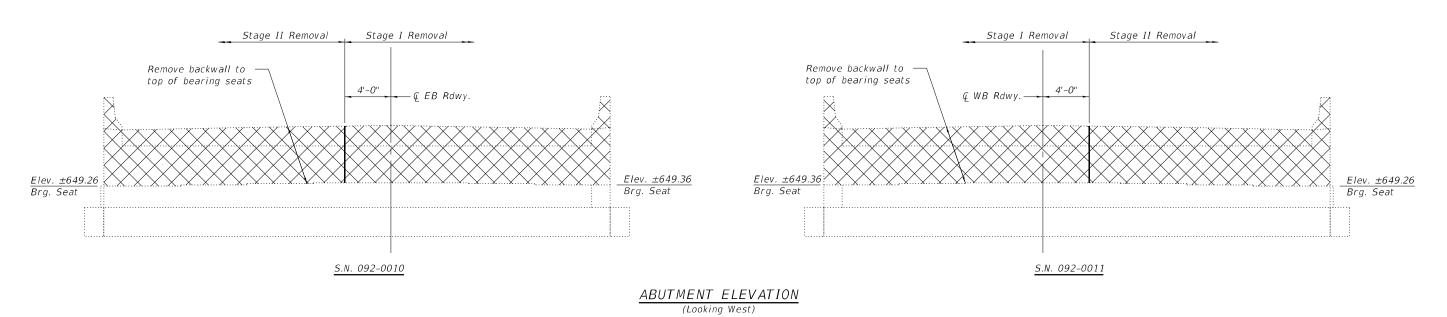
See sheet 14 of 23 for Bill of Material.

The Top Bearing Plates shall be shop painted with the inorganic zinc rich primer per AASHTO M300, Type 1. Cost included with Elastomeric Bearing Assembly, Type I.

				_
	USER NAME = baswanson	DESIGNED - LVM	REVISED -	
🐚 MAURER-STUTZ		CHECKED - BAS	REVISED -	
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -	
	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -	

BEARING DETAILS	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 092-0010 & 092-0011	74	(92-10,11,12)R	VERMILION	698	337
311(00101(E140: 032-0010 & 032-0011			CONTRAC	T NO. 70)A29
SHEET NO. 16 OF 23 SHEETS		ILLINOIS FED. AIT	PROJECT		-





Notes

Existing reinforcement bars extending into the removal area shall be cleaned, straightened, and incorporated into the new construction as noted. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system at no cost to the Department. Cost included with Concrete Removal.

See Sheet 19 of 23 for Sections A-A, B-B, C-C, and D-D. See Sheet 20 of 23 for Bill of Material. See Sheet 2 of 23 for limits of Structure Excavation.

STRUCTURAL REPAIR OF CONCRETE TABLE

		Plan	Qty.	A5 I	Built
Label	Size	SRC ≤5"	SRC >5"	SRC ≤5"	SRC >5"
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
1	1' x 2'	2			
	Totals	2			

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
Concrete Removal

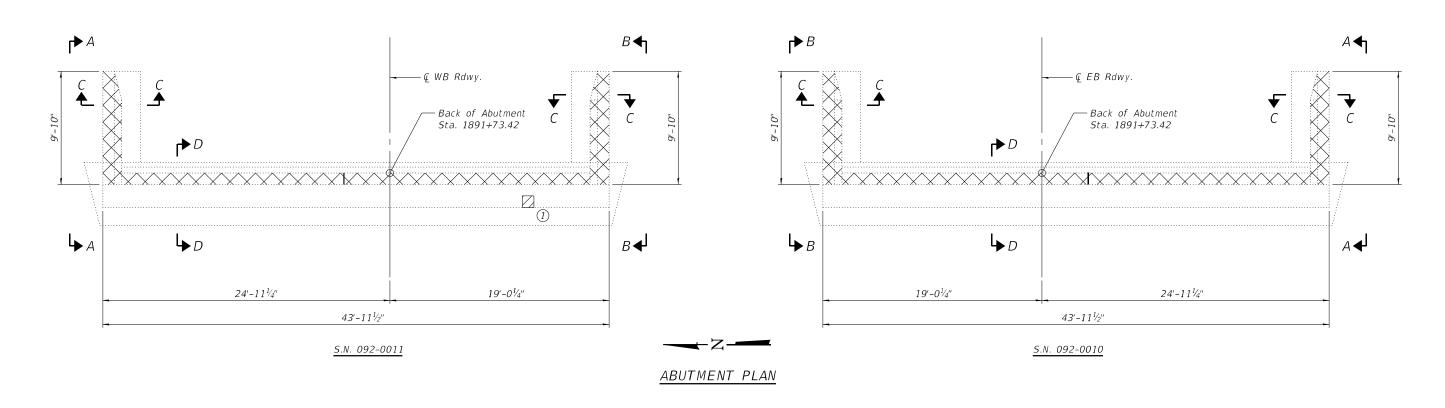
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

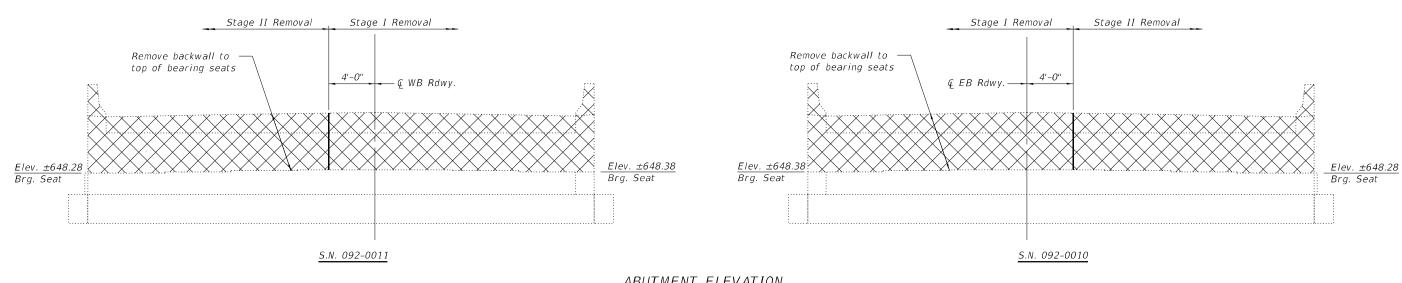
 WEST ABUTMENT REMOVAL
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEET NO.

 STRUCTURE NO. 092-0010 & 092-0011
 74
 (92-10,11,12)R
 VERMILION
 698
 338

 SHEET NO. 17
 0F 23
 SHEETS
 ILLINOIS FED.ADPROJECT

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ABUTMENT ELEVATION (Looking East)

Existing reinforcement bars extending into the removal area shall be cleaned, straightened, and incorporated into the new construction as noted. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system at no cost to the Department. Cost included with Concrete Removal.

See Sheet 19 of 23 for Sections A-A, B-B, C-C, and D-D. See Sheet 20 of 23 for Bill of Material. See Sheet 2 of 23 for limits of Structure Excavation.

STRUCTURAL REPAIR OF CONCRETE TABLE

		Plan	Qty.	A5 I	Built
Label	Size	SRC ≤5"	SRC >5"	SRC ≤5"	SRC >5"
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
1	1' × 1'	1			
	Totals	1			

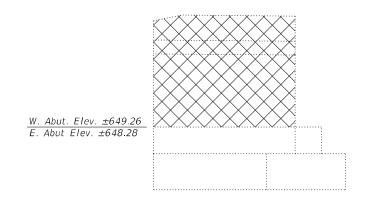
LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
Concrete Removal

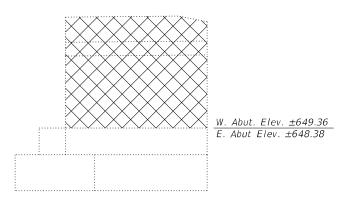
DESIGNED - LVM REVISED -JSER NAME = baswanson CHECKED - BAS MAURER-STUTZ REVISED -DRAWN REVISED -PLOT DATE = 4/22/2022 CHECKED - BAS REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION EAST ABUTMENT REMOVAL COUNTY (92-10,11,12)R VERMILION 698 339 STRUCTURE NO. 092-0010 & 092-0011 CONTRACT NO. 70A29 SHEET NO. 18 OF 23 SHEETS

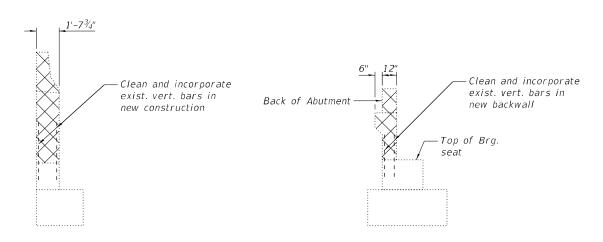


VIEW A-A (Outside Wingwall) (EB W. Abut. & WB E. Abut. Shown, others opposite)



VIEW B-B

(Median Side Wingwall) (EB W. Abut. & WB E. Abut. Shown, others opposite)



SECTION C-C

SECTION D-D

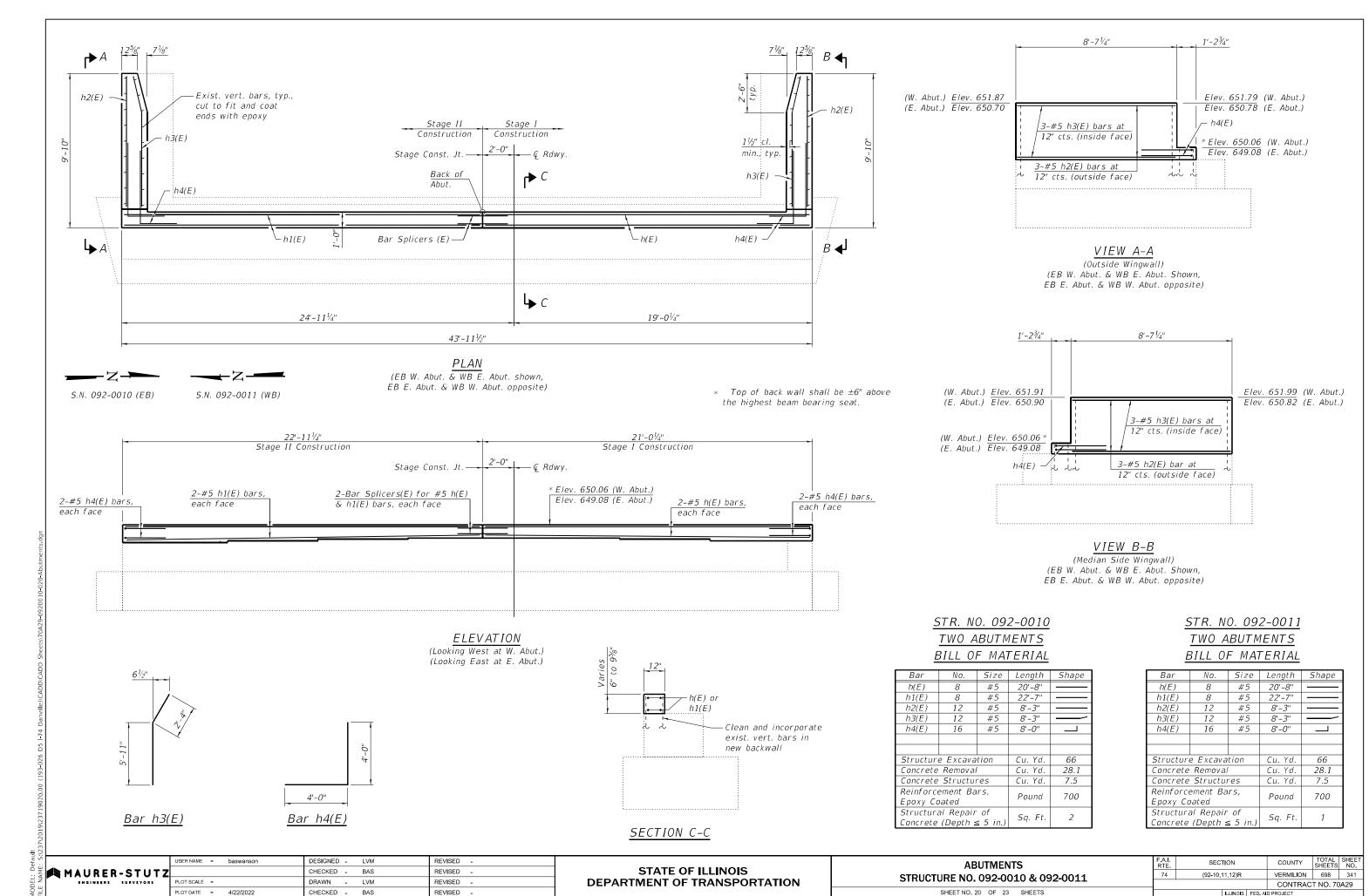
MAURER-STUTZ
ENGINEERS SURVEYORS

USER NAME = baswanson DESIGNED - LVM REVISED -CHECKED - BAS REVISED -DRAWN - LVM REVISED -PLOT DATE = 4/22/2022 CHECKED - BAS REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ABUTMENT REMOVAL DETAILS STRUCTURE NO. 092-0010 & 092-0011 SHEET NO. 19 OF 23 SHEETS

SECTION COUNTY (92-10,11,12)R VERMILION 698 340 CONTRACT NO. 70A29



4/22/2022 10:05:22 AM

Notes: Pier repair areas shown are estimated from soundings on October 11 thru 16, 2020. Actual size and locations of completed repairs shall be shown on this sheet and documented in the provide tables under "As Built". -(1)(Side of Column) Ground Line-ELEVATION - PIER 1 (Looking East) –(2)(Side of Column) Ground Line — ELEVATION - PIER 2 (Looking East)

STRUCTURAL REPAIR OF CONCRETE TABLE

STRUCTURAL REPAIR OF CONCRETE TABLE					
	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$				
Label	Size	SRC ≤5"	SRC >5"	SRC ≤5"	SRC >5"
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
1	1' x 1' 2' x 1'	1			
2	2' x 1'	2			
	Totals	3			
	i otais	3			

BILL OF MATERIAL

Item	Unit	Quantity
	UIIIL	Quantity
Structural Repair of Concrete (Depth ≤ 5 in.)	Sq. Ft.	3

<u>LEGEND</u>

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
Structural Repair of Concrete (Depth Greater Than 5 inches)

Notes:

Pier repair areas shown are estimated from soundings on October 11 thru 16, 2020.

Actual size and locations of completed repairs shall be shown on this sheet and documented in the provide tables under "As Built".

STRUCTURAL REPAIR OF CONCRETE TABLE

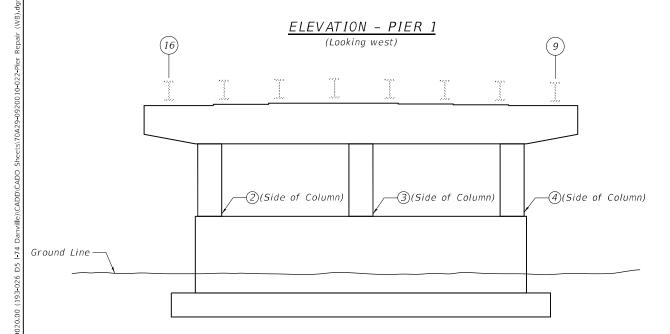
Label Size $\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$						
1 3' x 1' 3 2 3' x 1' 3 3 3 1' x 1' 1			Plan	Qty.	As I	Built
1 3' x 1' 3 2 3' x 1' 3 3 3 1' x 1' 1	Label	Size	SRC ≤ 5"	SRC >5"	SRC ≤5"	SRC >5"
3 1' x 1' 1			Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
3 1' x 1' 1	1	3' x 1'	3			
3 1' x 1' 1	2	3' x 1'	3			
4 2' x 1' 2	3	1' x 1'	1			
	4	2' x 1'	2			
Totals 9		Totals	9			

BILL OF MATERIAL

Item	Unit	Quantity
Structural Repair of Concrete (Depth ≤ 5 in.)	Sq. Ft.	9

<u>LEGEND</u>

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
Structural Repair of Concrete (Depth Greater Than 5 inches)



—(1)(Side of Column)

ELEVATION - PIER 2
(Looking west)

S	USER NAME = baswanson	DESIGNED - LVM	REVISED -
MAURER-STUTZ		CHECKED - BAS	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - LVM	REVISED -
[5]	PLOT DATE = 4/22/2022	CHECKED - BAS	REVISED -

SUBSTRUCTURE REPAIR - PIERS	F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
S.N. 092-0011	74	(92-10,11,12)R		VERMILION	698	343
3.14. 032-0011				CONTRAC	T NO. 70)A29
SHEET NO. 22 OF 23 SHEETS		ILLINOIS	FED ΔI	D PPO JECT		

Ground Line —

STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

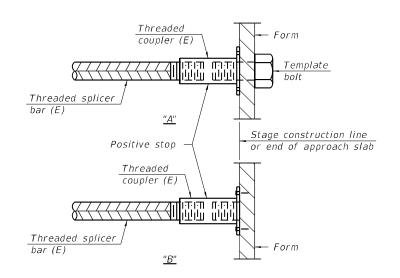
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

STR. NO. 092-0010

Location	Bar size	No. assemblies required	Minimum Iap length
Bridge Deck	#5	24	3'-6"
Diaphragm	#6	8	4'-0"
Diaphragm	#4	4	2'-5"
Approach Slab	#5	92	3'-4"
Approach Slab	#8	120	4'-9"
Approach Footing	#5	80	3'-2"
Abutments	#5	8	3'-2"

STR. NO. 092-0011

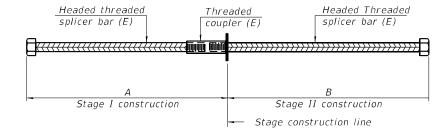
Location	Bar size	No. assemblies required	Minimum Iap length
Bridge Deck	#5	24	3'-6"
Diaphragm	#6	8	4'-0"
Diaphragm	#4	4	2'-5"
Approach Slab	#5	92	3'-4"
Approach Slab	#8	120	4'-9"
Approach Footing	#5	80	3'-2"
Abutments	#5	8	3'-2"



INSTALLATION AND SETTING METHODS

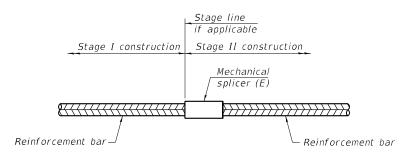
"A" : Set bar splicer assembly by means of a template bolt "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



HEADED BAR SPLICER ASSEMBLY PLAN

Location	Bar	No. assemblies	Λ	R
Location	size	required	A	
Diaphragms -0010	#6	8	1'-9"	3'-6"
Diaphragms -0011	#6	8	1'-9"	3'-6"



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements

for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

DESIGNED - LVM REVISED -JSER NAME = baswanson CHECKED - BAS MAURER-STUTZ REVISED -DRAWN REVISED PLOT DATE = 4/22/2022 CHECKED - BAS REVISED .

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS COUNTY (92-10,11,12)R VERMILION 698 344 STRUCTURE NO. 092-0010 & 092-0011 CONTRACT NO. 70A29 SHEET NO. 23 OF 23 SHEETS

Benchmark - Survey disk on top of the NW wingwall of Structure No. 092-0011 (at G Street). Elev. 656.07. Existing Structures - Dual Structures, S.N. 092-0012 (W.B.) & 092-0013 (E.B.), composed of four-span continuous wide flange steel beams. Superstructures are supported on pile-supported east stub abutments, west closed abutments, and piers. Built in 1964 as Section 92-11VB-1 at Sta. 1900+80.59; repaired in 1977; repaired in 1992 as Section (92-11VB-1)BR. Traffic to be maintained using staged construction. Salvage - None. Limits of Protective Shield ← Ç CSXT RR ├— @ Track - Exist. W33 Beams Stone Riprap below bridges (see Rdwy. Plans) ELEVATION 237'-10" Bk. to Bk. Abutments 50'-0" 67'-0" 67'-0" 1'-11" Traffic Barrier Terminal Type 5, Std. 631026, 10°35'30' (1 location only) typ. ⊊ Brg.

PROPOSED SCOPE OF WORK

- Rehabilitate bridge deck with scarification and latex concrete overlay
- Remove and replace bridge approach slabs
- Replace steel beam bearing assemblies at abutments
- Remove and replace deck joints at west abutment with preformed joint strip seal
- Remove existing east abutment deck joints and reconstruct with jointless semi-integral abutments
- Repair concrete deterioration on abutments
- Repair sections of concrete slope wall
- Remove and replace steel diaphragms at west abutment
- Paint beam ends
- 10. Add side retainers to Pier 1 & Pier 3 bearings

LOADING HS20-44

DESIGN SPECIFICATIONS

2002 AASHTO Standard Specifications for Highway Bridges, 17th Edition (LFD)

DESIGN STRESSES

FIELD UNITS

New Construction:

 $f'c = 3,500 \ psi$

f'c = 4,000 psi (deck)

fy = 60,000 psi (Reinforcement)

fy = 36,000 psi (structural steel)

Exist. Structure:

f'c = 3,500 psi

fy = 60,000 psi (deck reinf.)

fy = 40,000 psi (orig. substr. reinf.)

fy = 36,000 (structural steel)

Proposed Structure -Range 11W / 2nd P M ilton Rd Fairfield Ave "F" S

LOCATION SKETCH

BRYAN A. SWANSON 081-006716

Date Signed: 4-22-22 Exp. Date: 11/30/2022

SHEET NO. 1 OF 34 SHEETS

GENERAL PLAN AND ELEVATION I-74 OVER F STREET, FAIRFIELD AVE.,

AND CSXT RAILROAD F.A.I. 74 - SEC. (92-10,11,12)R

VERMILION COUNTY

STA. 1900+74.44

STR. NO. 092-0012 & 092-0013

DESIGNED - LVM REVISED baswansor CHECKED - BAS REVISED -**≠** MAURER-STUTZ DRAWN REVISED PLOT DATE = 4/22/2022 CHECKED -REVISED .

€ E.B. I-74 -

S.N. 092-0013

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

Out to Out 55'-0" to !

€ Fairfield Ave

50'-0"

S.N. 092-0012

11 1/

11 1

î Pier 2

Sta. 1900+97.85

€ W.B. I-74

€ Pier 3

- F.A.I. 74 Sta. 1900+74.44

CSXT Sta. 20+00

PLAN

Sta. 1901+64.85

© Pier 3

© Pier 2

Sta. 1901+76.82

Sta. 1901+09.82

Sta. 1900+30.85

Sta. 1900+42.82

1'-11"

├— @ Brg. E. Abut.

Bk. of E. Abut.

Sta. 1902+16.76

Bk. of E. Abut.

Sta. 1902+28.73

Temp. Sheet Piling, typ.

10' Shldr.

(92-10,11,12)R VERMILION 698 345 CONTRACT NO. 70A29

4/22/2022 11:50:51 PM

9

W. Abut.

30'-0" Appr.

Slab, typ.

Traffic Barrier Terminal

Type 6, Std. 631031, Arrival ends only (4 locs.)

Bk. of W. Abut

Sta. 1899+78.93

Bk. of W. Abut

Sta. 1899+90.90

GENERAL NOTES

All new fasteners shall be $\frac{3}{4}$ " Ø high-strength steel bolts, unless noted

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that can not be removed by grinding $\frac{1}{4}$ in, deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas, and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Joint openings shall be adjusted according to Article 503.10(c) of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F

Existing Name Plates shall be removed, cleaned and incorporated into the new construction. Cost included with Relocating Name Plates.

Synthetic Fibers shall be included in the bridge deck concrete overlay specified. See Special Provisions.

The existing structural steel contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

Cleaning and painting of the existing structural steel shall be as specified in the special provision for "Cleaning and Painting Existing Steel Structures". After removal of deck ends and diaphragms, but prior to new bearing installation and encasement of the steel beams, all beams and other existing structural steel within 5 ft of the beam ends shall be cleaned per Near White Blast Cleaning (SSPC-SP10).

The designated areas cleaned per Near White Blast Cleaning (SSPC-SP10) shall be painted according to the requirements of Paint System 1 (0Z/E/U). The color of the final finish coat for all steel surfaces shall be Gray, Munsell No. 5B 7/1.

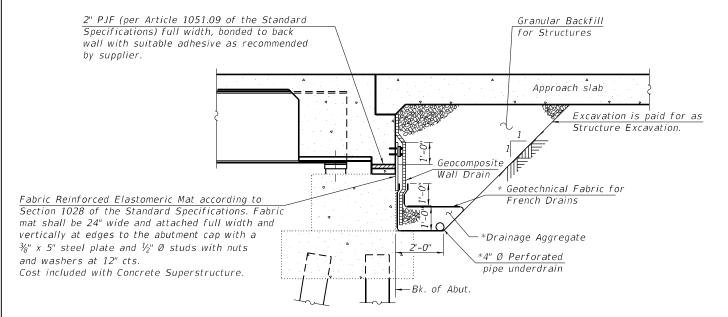
Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures"

Care shall be taken not to damage rubber bearing or joint components during blasting and cleaning operations. Any damage to these components shall be repaired at the Contractor's expense.

SSPC-QP1 and SPC-QP2 Painting Contractor Certifications will be required for this contract.

All new structural steel shall be hot dip galvanized unless noted otherwise. See Special Provision for "Hot Dip Galvanizing for Structural Steel".

Existing structure plans are available from the District upon request.



SECTION THRU SEMI-INTEGRAL ABUTMENT

(Horiz. dim. @ Rt. L's) (East Abutment only)

*Included in the cost of Pipe Underdrains for Structures 4".

All drainage system components shall extend between the wingwalls at each end of abutment, except an outlet pipe shall extend until intersecting with the side slopes or slope wall surface. Any slope wall removal and replacement required for the installation of the outlet pipe shall be included in the cost of Pipe Underdrains for Structures 4". The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

TOTAL BILL OF MATERIAL

· · · · · · · · · · · · · · · · · · ·				
ITEM	UNIT	-0012	-0013	TOTAL
Concrete Removal	Cu. Yd.	28.7	34.0	62.7
Protective Shield	Sq. Yd.	918	1171	2089
Structure Excavation	Cu Yd.	43	58	101
Concrete Structures	Cu. Yd.	32.1	40.0	72.1
Concrete Superstructure	Cu. Yd.	32.4	40.1	72.5
Protective Coat	Sq. Yd.	1389	1783	3172
Concrete Superstructure (Approach Slab)	Cu. Yd.	122.8	157.1	279.9
Furnishing and Erecting Structural Steel	Pound	3640	6250	9890
Reinforcement Bars, Epoxy Coated	Pound	52280	67410	119690
Bar Splicers	Each	331	331	662
Preformed Joint Strip Seal	Foot	45	59	104
Elastomeric Bearing Assembly, Type I	Each	7	9	16
Elastomeric Bearing Assembly, Type II	Each	7	9	16
Anchor Bolts, 1"	Each	52	68	120
Temporary Sheet Piling	Sg. Ft.	89	95	184
Granular Backfill for Structures	Cu. Yd.	39	54	93
Epoxy Crack Injection	Foot	15	22	37
Geocomposite Wall Drain	Sq. Yd.	27	35	62
Pipe Underdrains for Structures 4"	Foot	66	76	142
Slope Wall Crack Sealing	Foot		, ,	12
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	811	1311	2122
Jack and Remove Existing Bearings	Each	14	18	32
Structural Steel Removal	Pound	2400	4620	7020
Bridge Deck Latex Concrete				
Overlay 2 ³ / ₄ "	Sq. Yd.	1061	1366	2427
Bridge Deck Scarification ¾"	Sq. Yd.	1061	1366	2427
Containment and Disposal of Lead Paint	, ,	,		
Cleaning Residues No. 3	L. Sum	1		1
Containment and Disposal of Lead Paint	L. Sum		1	1
Cleaning Residues No. 4				
Cleaning and Painting Steel Bridge No. 3	L. Sum	1		1
Cleaning and Painting Steel Bridge No. 4	L. Sum		1	1
Structural Repair of Concrete	Sg. Ft.	98	88	186
(Depth Equal to or Less Than 5 Inches)	· .			
Deck Slab Repair (Full Depth, Type I)	Sq. Yd.	1	1	2
Diamond Grinding (Bridge Section)	Sq. Yd.	1240	1634	2874
Slope Wall Repair	Sq. Yd.			202
Slope Wall Slurry Pumping	Cu. Yd.			6
Temporary Shoring and Cribbing	Each	3	5	8

* On new concrete superstructure, approach slabs, and latex concrete overlay,

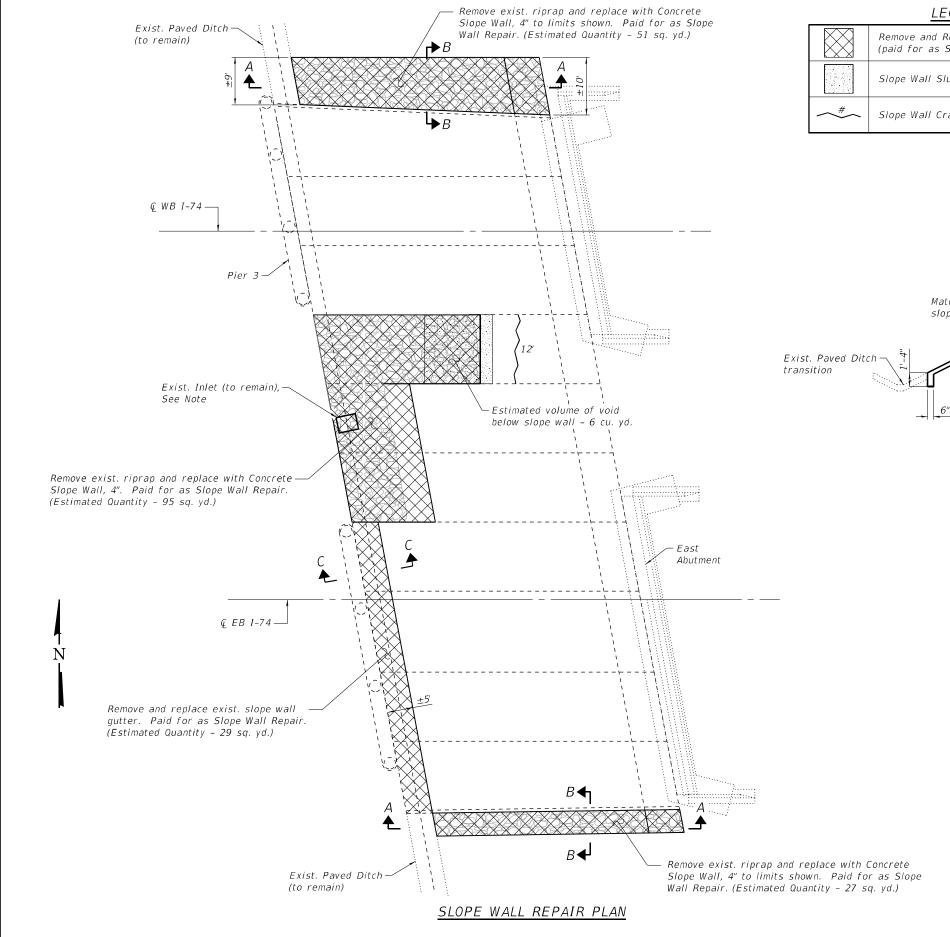
INDEX OF SHEETS

- General Plan and Elevation
- General Data
- Slope Wall Plan
- Stage Construction Details
- Temporary Concrete Barrier Deck Slab Repair Plan (S.N. 092-0012)
- Superstructure Details (S.N. 092-0012)
- Diaphragm Details (S.N. 092-0012)
- 10. Deck Slab Repair Plan (S.N. 092-0013) 11
- 12-14. Superstructure Details (S.N. 092-0013)
- 15. Diaphragm Details (S.N. 092-0013)
- 16-20. Bridge Approach Slabs
- 21. Preformed Joint Strip Seal
- 22. Structural Steel Framing Plan
- Structural Steel Details 23.
- 24-25. Bearing Details
- 26-29. West Abutments 30-33. Fast Abutments
- Bar Splicer Assemblies

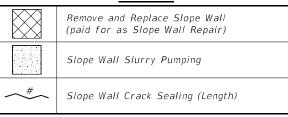
DESIGNED - LVM REVISED JSER NAME = baswanson CHECKED - BAS REVISED . MAURER-STUTZ REVISED PLOT DATE = 4/22/2022 CHECKED -REVISED

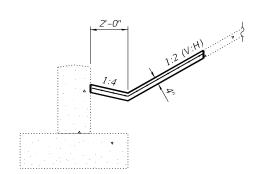
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION COUNTY **GENERAL DATA** (92-10,11,12)R VERMILION 698 346 STRUCTURE NO. 092-0012 & 092-0013 CONTRACT NO. 70A29 SHEET NO. 2 OF 34 SHEETS

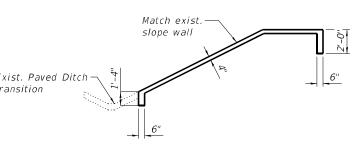


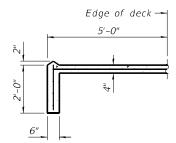
LEGEND





SECTION C-C





SECTION A-A

SECTION B-B

Notes

Slope wall shall be reinforced with welded wire fabric, 6 in. x 6 in. - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.

Voids below the replaced slope wall shall be filled prior to placement of the new concrete slab. Existing riprap not extending above the bottom of the new concrete may remain; fill the remaining void with controlled low-strength material per the Special Provision for Slope Wall Slurry Pumping.

Layout and limits of the slope wall removal and repairs may be varied to suit conditions in the field as directed by the Engineer.

Remove all dirt and riprap from on top of bottom of slope wall. Expose and clean out inlet located between bridges. Cost included with Slope Wall Repair.

Set flow line of slope wall gutter to maintain positive drainage from Paved Ditch

<u>SLOPE WALL</u> BILL OF MATERIAL

Item	Unit	Quantity
Slope Wall Repair	Sq. Yd.	202
Slope Wall Slurry Pumping	Cu. Yd.	6
Slope Wall Crack Sealing	Foot	12

USER NAME = baswanson DESIGNED - LVM REVISED CHECKED - BAS REVISED PLOT SCALE = DRAWN - BAS REVISED PLOT DATE = 4/22/2022 CHECKED - LVM REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SLOPE WALL PLAN
STRUCTURE NO. 092-0012 & 092-0013

SHEET NO. 3 OF 34 SHEETS

 F.A.I. RTE.
 SECTION
 COUNTY SHEETS NO.
 TOTAL SHEETS NO.
 SHEET NO.

 74
 (92-10,11,12)R
 VERMILION
 698
 347

 CONTRACT NO. 70A29

 LLINOIS
 FED. AID PROJECT
 S.N. 092-0012

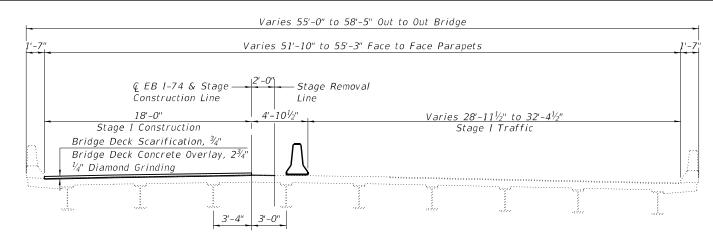
S.N. 092-0012

— Ç WB I-74 & Stage

Construction Line

15'-6"

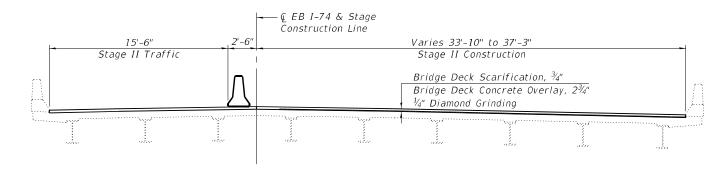
Stage II Traffic



S.N. 092-0013

STAGE I CONSTRUCTION

(Looking East)



S.N. 092-0013

STAGE II CONSTRUCTION

(Looking East)

Notes:

Sheet piling shall have a minimum section modulus of 3.1 in³/ft.

If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design subittal including plan details and calculations will be required for review and acceptance by the Engineer.

Stage II Construction

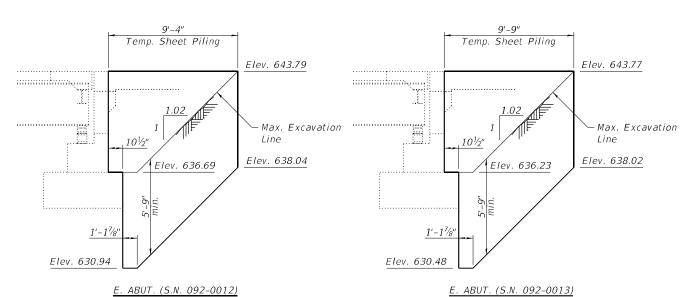
Bridge Deck Concrete Overlay, 23/4"

Bridge Deck Scarification, 3/4"

1/4" Diamond Grinding

The Contractor shall connect the first sheet to the existing abutment wall to ensure stability of sheets driven to the top of the existing footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Piling.

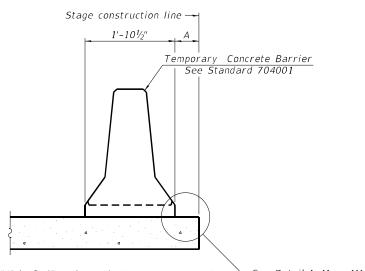
Temporary Sheet Piling will be driven into embankment fill soils consisting of Mixed Clay Loam with unconfined compressive strengths (Qu) varying from 1.0 to



TEMPORARY SHEET PILING

(Looking North)

etan	.:	USER NAME = baswanson	DESIGNED - LVM	REVISED -		STAGING DETAILS	F.A.I.	SECTION	COUNTY	TOTAL	SHEET
ā.,	MAURER-STUTZ		CHECKED - BAS	REVISED -	STATE OF ILLINOIS	STRUCTURE NO. 092-0012 & 092-0013	74	(92-10,11,12)R	VERMILION	698	348
E I	ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - BAS	REVISED -	DEPARTMENT OF TRANSPORTATION	21K0C10KE NO. 092-0012 & 092-0013			CONTRAC	CT NO. 70	JA29
9	류[PLOT DATE = 4/22/2022	CHECKED - LVM	REVISED -		SHEET NO. 4 OF 34 SHEETS		ILLINOIS FEE	. AID PROJECT		



∽ See Detail I, II or III When "A" is 3'-1" or less, the temporary concrete barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

NEW SLAB OR NEW DECK BEAM

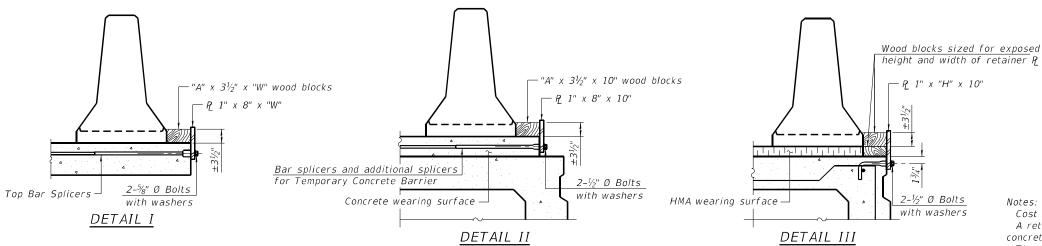
- Stage removal line ← Stage removal line 1'-101/2" 1'-101/2" Temporary Concrete Barrier See Standard 704001 6" min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint st When hot-mix asphalt wearing surface is present, embedment is required when "A" is greater than 3'-1".

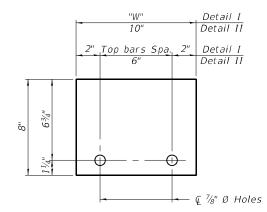
shall be 3" plus the wearing surface depth.

EXISTING DECK BEAM

SECTIONS THRU SLAB OR DECK BEAM

EXISTING SLAB





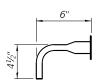
RAILING CRITERIA

NCHRP 350 Test Level Railing Weight (plf)

STEEL RETAINER P 1" x 8" x "W" (Detail I and II)

← Ç ¾" Ø Holes

STEEL RETAINER P 1" x "H" x 10" (Detail III)



RESTRAINING PIN

BAR SPLICER FOR #4 BAR - DETAIL III

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate $\c c$ of each temporary concrete barrier.

1x8 UNC

1" Ø pin

US Std. 11/16" I.D. x 21/2" O.D. x approx. 8 gauge thick washer

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

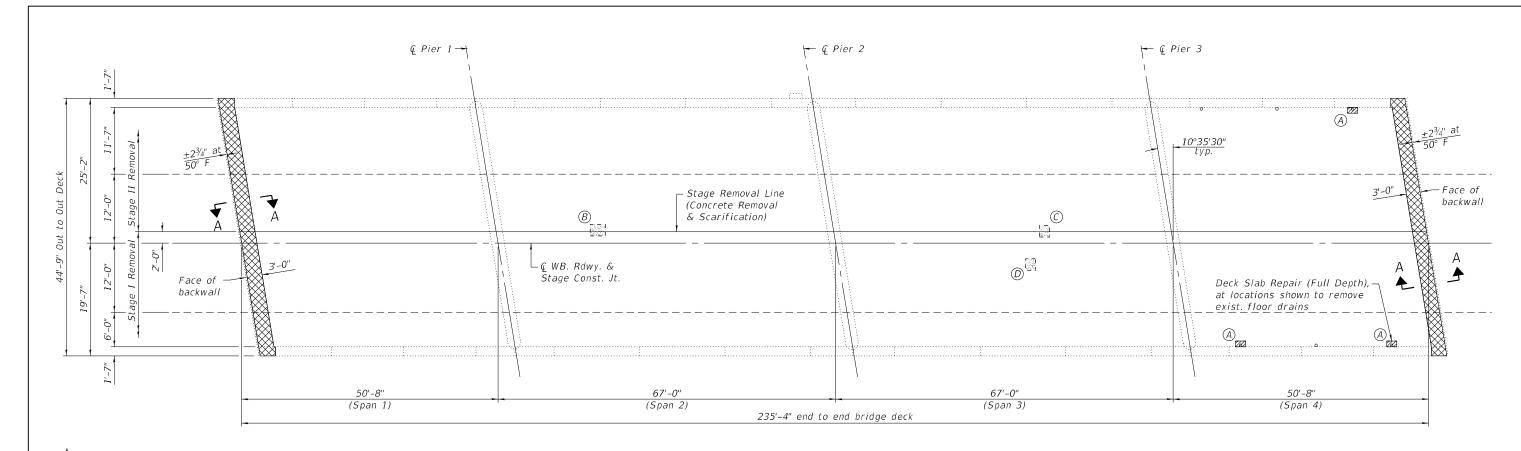
R-27	10-12-2021		
	USER NAME = baswanson	DESIGNED - LVM	REVISED -
MAURER-STUTZ		CHECKED - BAS	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - BAS	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - LVM	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION **TEMPORARY CONCRETE BARRIER** (92-10,11,12)R VERMILION 698 349 STRUCTURE NO. 092-0012 & 092-0013 CONTRACT NO. 70A29 SHEET NO. 5 OF 34 SHEETS

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R-27

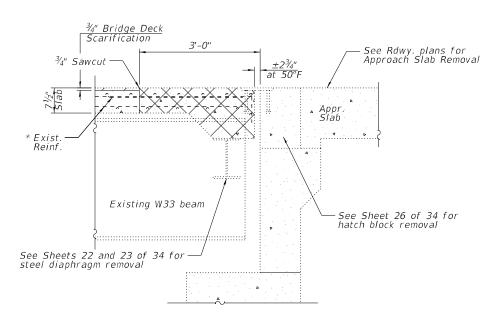


DECK SLAB REPAIR TABLE

DECK SLAB REPAIR LABLE							
		F	Plan Qty	/.	As E	3uilt	
Label	Size	Deck Slab Repair (Part. Depth)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)	
		Sq. Ft.		Sq. Ft.	Sq. Ft.	Sq. Ft.	
Α	1' x 2' x 3 Loc. 2' x 3' 2' x 2' 2' x 2'		6				
А В С	2' x 3'	6					
С	2' x 2'	4					
D	2' x 2'	4					
	Totals	14	6				

PLAN - DECK SLAB REPAIRS

* Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorportated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.



SECTION A-A

LEGEND

Deck Slab Repair (Partial Depth) (For Information Only)
Deck Slab Repair (Full Depth)
Concrete Removal

See sheet 8 of 34 for cross section thru bridge deck.

Deck survey performed on December 17, 2020. Locations and sizes shown in the plan view are approximate.

Deck Slab Repair (Partial Depth) is an estimated quantity per the deck survey. This area shall not be paid for separately, but shall be addressed as stated in the Special Provision for Bridge Deck Latex Concrete Overlay.

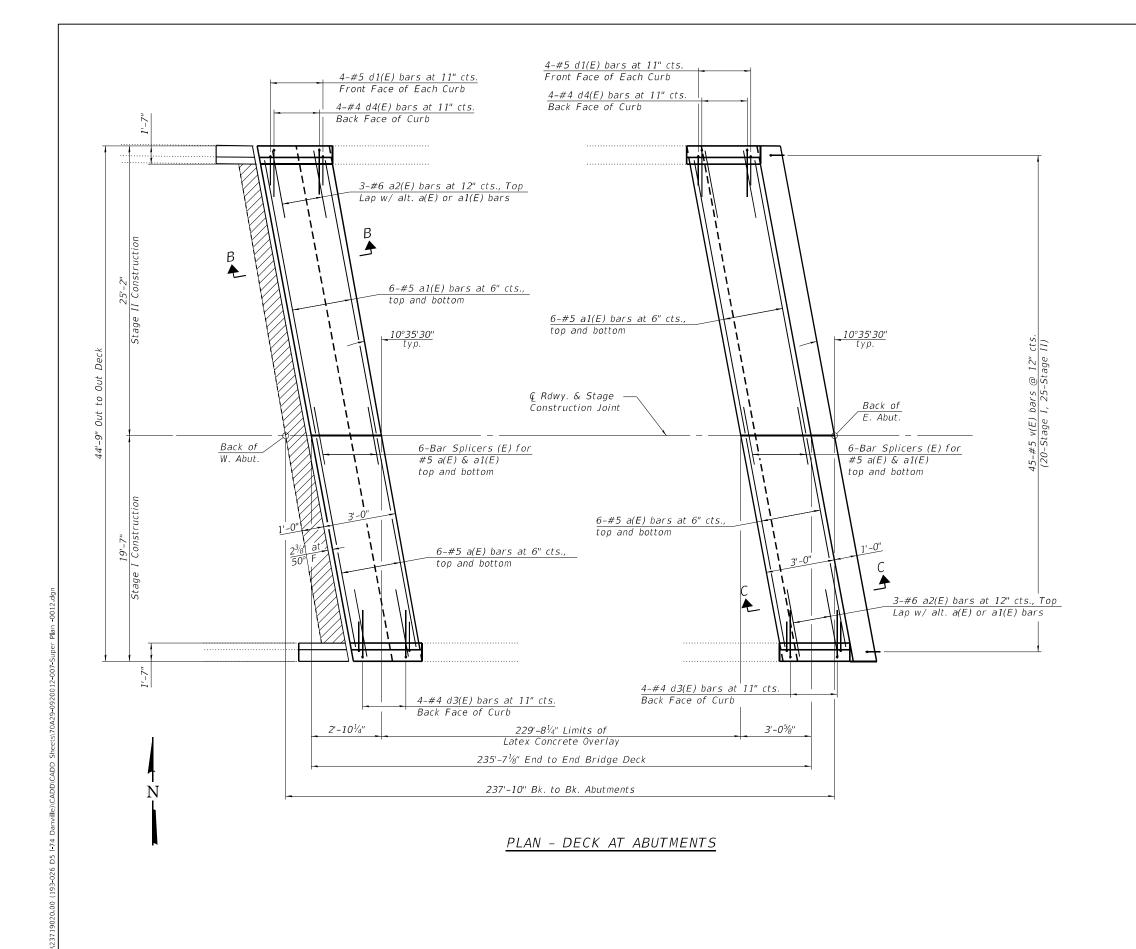
Actual size and locations of full-depth patches shall be shown on this sheet and documented in the Deck Slab Repair Table under "As Built".

DECK SLAB (S.N. 092-0012) BILL OF MATERIAL

ITEM	UNIT	TOTAL
Deck Slab Repair (Full Depth, Type I)	Sq. Yd.	1
Bridge Deck Scarification $\frac{3}{4}$ "	Sq. Yd.	1061
Bridge Deck Latex Concrete Overlay, 2¾ Inches	Sq. Yd.	1061
Concrete Removal	Cu. Yd.	8.4

(Sheet 1 of 5)

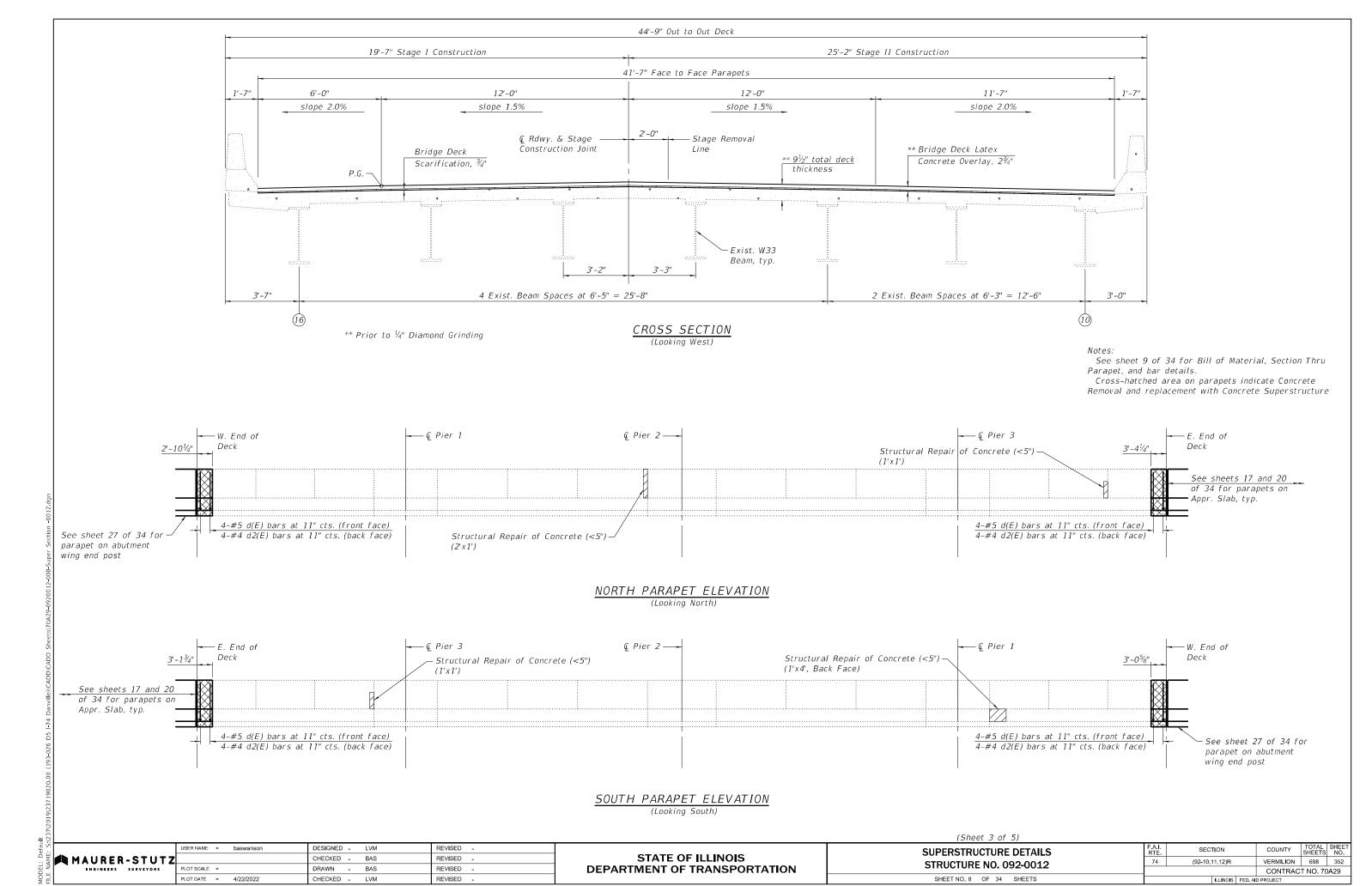
iii	USER NAME = baswanson	DESIGNED - LVM	REVISED -		DECK SLAB REPAIR PLAN	F.A.I. RTE	SECTION	COUNTY	TOTAL SHI	ĒΤ
MAURER-STUTZ		CHECKED - BAS	REVISED -	STATE OF ILLINOIS	STRUCTURE NO. 092-0012	74	(92-10,11,12)R	VERMILION	698 35	
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - BAS	REVISED -	DEPARTMENT OF TRANSPORTATION	21K0C10KE NO. 032-0012	'		CONTRAC	T NO. 70A29	П
∄ 문 	PLOT DATE = 4/22/2022	CHECKED - LVM	REVISED -		SHEET NO. 6 OF 34 SHEETS		ILLINOIS FE	D. AID PROJECT		\neg

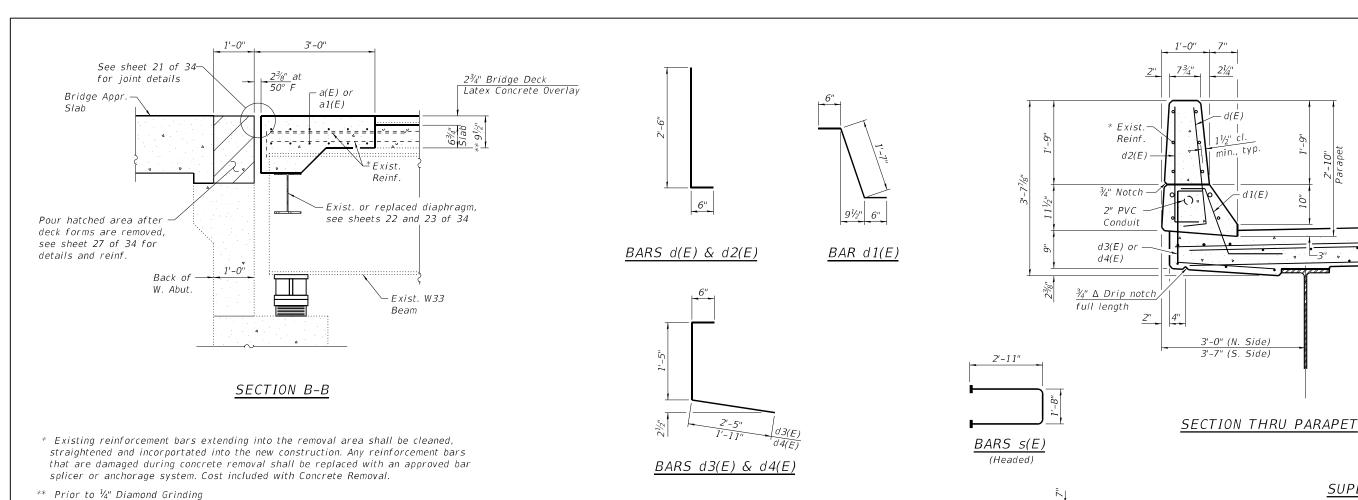


Notes:
See sheet 9 of 34 for Section B-B, Bill of
Material and bar details.
See sheet 10 of 34 for Section C-C.
Spacing for v(E) bars shall be at right angles
to & Rdwy.

(Sheet 2 of 5)

USER NAME = baswanson DESIGNED - LVM REVISED -SECTION COUNTY SUPERSTRUCTURE DETAILS STATE OF ILLINOIS CHECKED - BAS MAURER-STUTZ REVISED -(92-10,11,12)R VERMILION 698 351 STRUCTURE NO. 092-0012 **DEPARTMENT OF TRANSPORTATION** DRAWN - BAS REVISED -ENGINEERS SURVEYORS CONTRACT NO. 70A29 SHEET NO. 7 OF 34 SHEETS PLOT DATE = 4/22/2022 CHECKED - LVM REVISED -





Attach new conduit to existing within removed parapet New Conduit Embedded in Structure, see Lighting Plans New Junction Box attached to Approach Parapet, see Lighting Plans Salvage exist. conduit and junction box, reattach to new concrete deck Remove exist. flexible conduit

and junction boxes

REVISED -

REVISED -

REVISED -

REVISED

DESIGNED - LVM

CHECKED - BAS

CHECKED - LVM

DRAWN

CONDUIT AT ABUTMENT JOINT

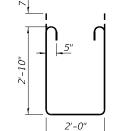
(East Abutment, North Parapet)

baswanson

PLOT DATE = 4/22/2022

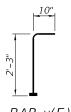
CONDUIT AT ABUTMENT JOINT

(West Abutment, North Parapet) (Looking South)



BARS s1(E)

BARS u(E)



 $BAR \ v(E)$ (Headed)

"L" (see Bill of Mat'l) BARS m2(E), m6(E), & m7(E)

(Headed)

	Stru Conc	cture Repair of rete (Depth ≤ 5")	Sq. Ft.	8	
(Sheet 4 of 5)					
SUPERSTRUCTURE DETAILS	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO 002-0012	74	(92-10,11,12)R	VERMILION	698	353
· · · · · · · · · · · · · · · · · · ·			CONTRAC	T NO. 70	A29
		ILLINOIS FED.	AID PROJECT		
		·			

a(E) or

a1(E)

SUPERSTRUCTURE

STR. NO. 092-0012 BILL OF MATERIAL

Size Length Shape

3'-0" 4'-4"

> 6'-0" 3'-2"

25'-3"

7'-6"

5'-0"

3'-1"

Pound

_

3300

26.0

#5 19'-1"

#5 24'-9"

#6 6'-0"

#5 3'-0"

#5 2'-7"

#6 19'-7"

#6 25'-3"

#6 6'-0"

#6 2'-7"

#4 19'-7"

#6 5'-0"

#5 8'-10"

#4

#4 #4 3'-10"

#6

#6

#4

#4

#5

No.

24

24

12

16

16

8

5

10

4

4

21

42

45

Concrete Superstructure Cu. Yd.

Reinforcement Bars,

Epoxy Coated

Bar

a1(E) a2(E)

d1(E)

d2(E)

d3(E)

d4(E)

m1(E) m2(E)

m3(E)

m6(E)

m7(E)

m8(E)

m9(E)

m100(E)

s1(E)

u(E)

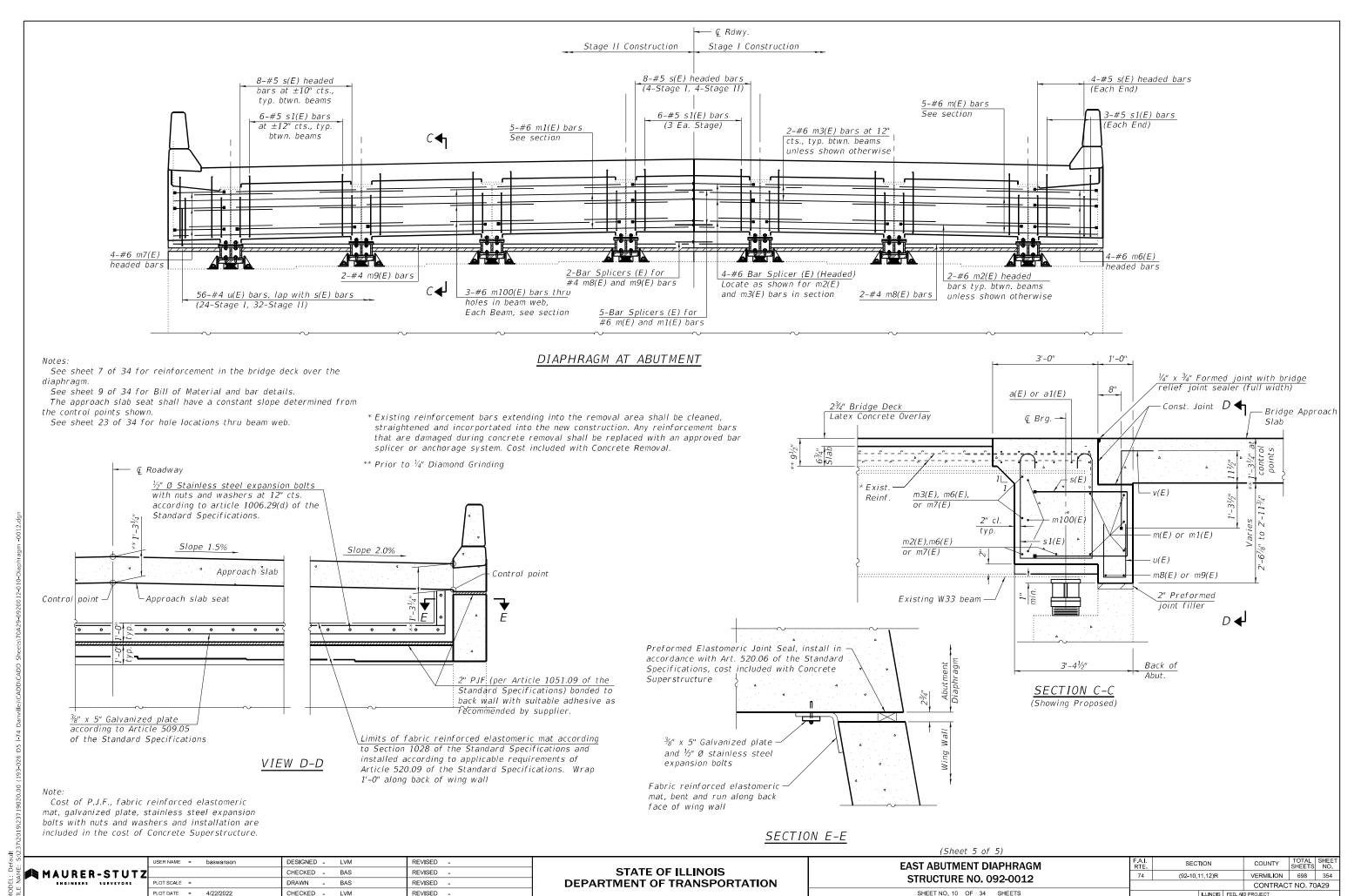
v(E)

(Looking South)

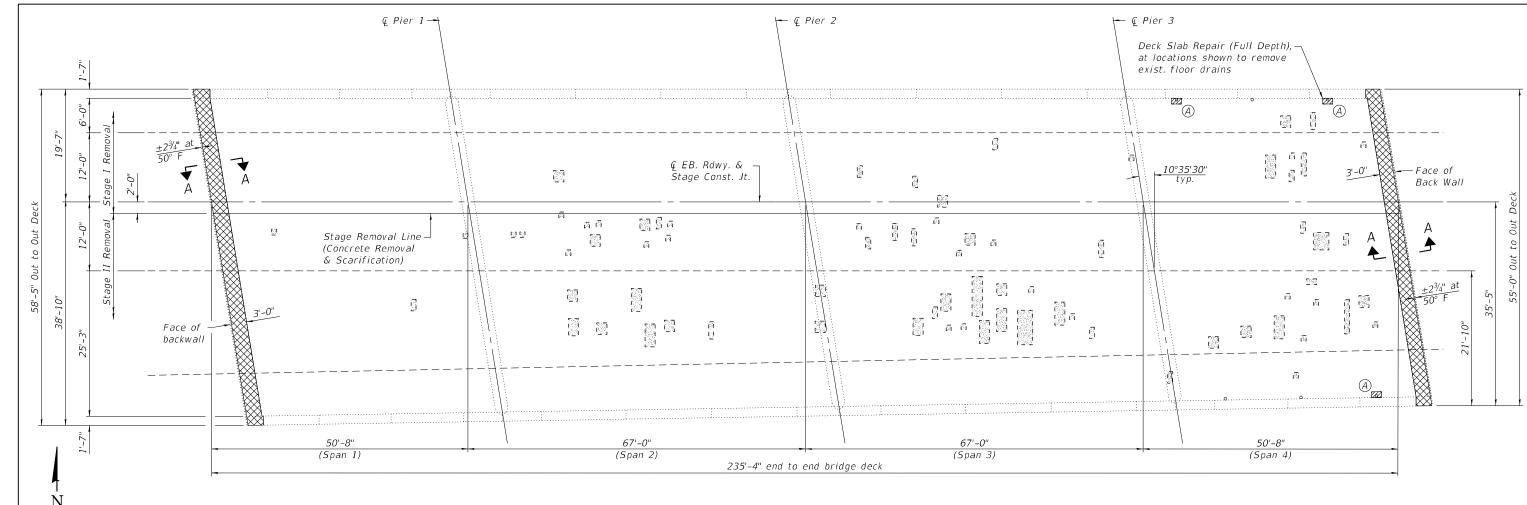
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

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MAURER-STUTZ



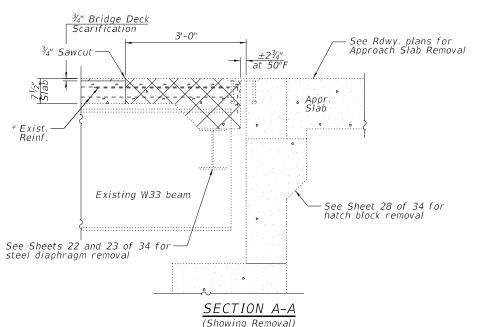
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DECK SLAB REPAIR TABLE

	Plan Qty.				As I	Built
Label	Size	Deck Slab Repair (Part. Depth)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft
Α	1' x 2' x 3 Loc.		6			
	Span 1, Stage II	4				
	Span 2, Stage I	4				
	Span 2, Stage II	64				
	Span 3, Stage I	11				
	Span 3, Stage II	99				
	Span 4, Stage I	23				
	Span 4, Stage II	46				
	Totals	251	6			

 * Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorportated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.



PLAN - DECK SLAB REPAIRS

<u>LEGEND</u>

Deck Slab Repair (Partial Depth) (For Information Only)
Deck Slab Repair (Full Depth)
Concrete Removal

See sheet 13 of 34 for cross section thru bridge deck. Deck survey performed on December 17, 2020. Locations and sizes shown in the plan view are approximate.

Deck Slab Repair (Partial Depth) is an estimated quantity per the deck survey. This area shall not be paid for separately, but shall be addressed as stated in the Special Provision for Bridge Deck Latex Concrete Overlay.

Actual size and locations of full-depth patches shall be shown on this sheet and documented in the Deck Slab Repair Table under "As Built".

DECK SLAB (S.N. 092-0013) BILL OF MATERIAL

ITEM	UNIT	TOTAL
Deck Slab Repair (Full Depth, Type I)	Sq. Yd.	1
Bridge Deck Scarification 3/4"	Sq. Yd.	1366
Bridge Deck Latex Concrete Overlay, 2¾ Inches	Sq. Yd.	1366
Concrete Removal	Cu. Yd.	10.3

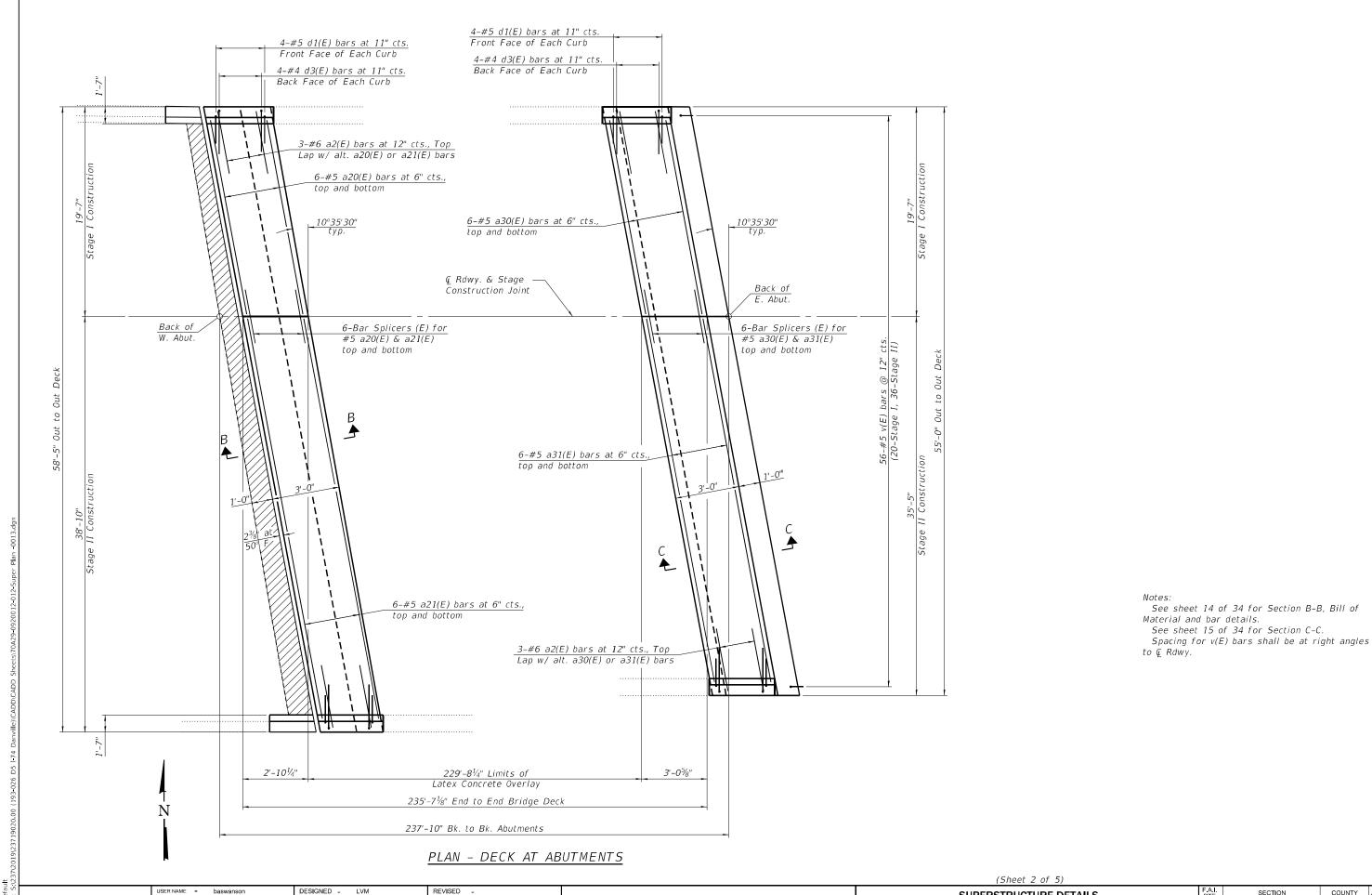
(Sheet 1 of 5)

	USER NAME = baswanson	DESIGNED - LVM	REVISED -
MAURER-STUTZ		CHECKED - BAS	REVISED -
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - BAS	REVISED -
	PLOT DATE = 4/22/2022	CHECKED - LVM	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION **DECK SLAB REPAIR PLAN** (92-10,11,12)R **STRUCTURE NO. 092-0013** SHEET NO. 11 OF 34 SHEETS

TOTAL SHEE NO. COUNTY VERMILION 698 355 CONTRACT NO. 70A29



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS
STRUCTURE NO. 092-0013

SHEET NO. 12 OF 34 SHEETS

MAURER-STUTZ

PLOT DATE = 4/22/2022

CHECKED - BAS

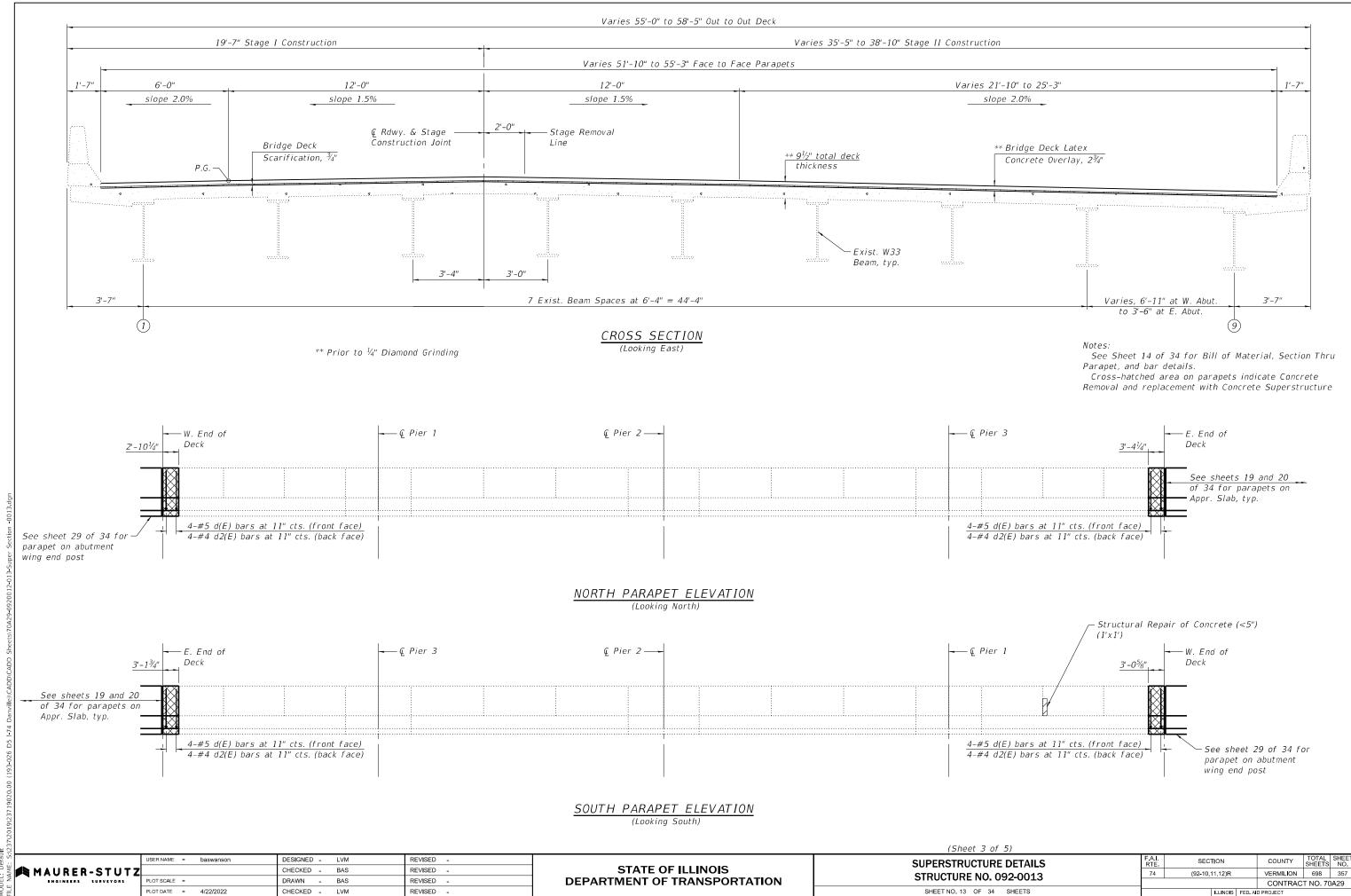
DRAWN - BAS

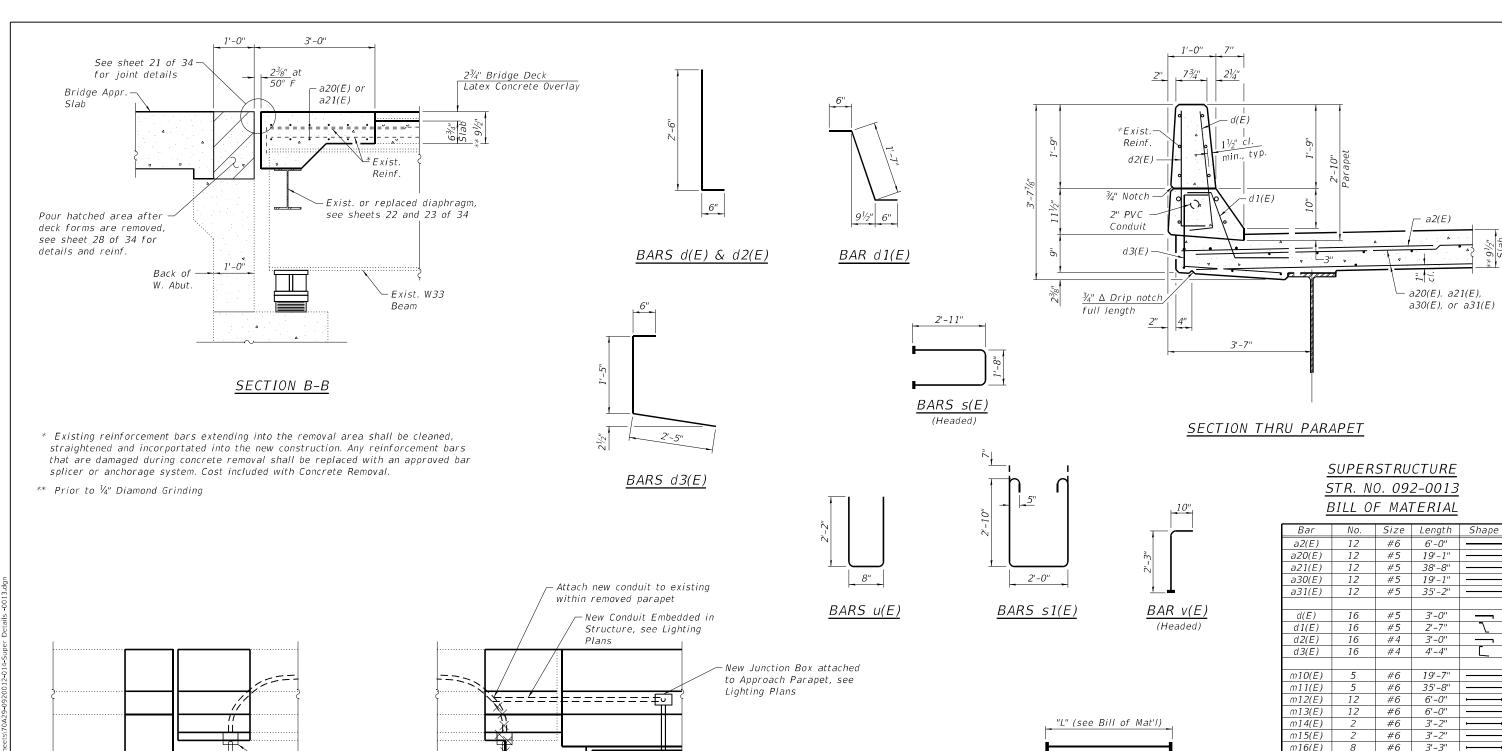
CHECKED - LVM

REVISED -

REVISED -

REVISED -





Salvage exist. conduit and junction box, reattach to new concrete deck Remove exist. flexible conduit and junction boxes

CONDUIT AT ABUTMENT JOINT (West Abutment, South Parapet) (Looking North)

CONDUIT AT ABUTMENT JOINT

(East Abutment, South Parapet) (Looking North)

GZ O(L)	1	" "	1 2 1	
a21(E)	12	#5	38'-8"	
a30(E)	12	#5	19'-1"	
a31(E)	12	#5	35'-2"	
d(E)	16	#5	3'-0"	
d1(E)	16	#5	2'-7"	\
d2(E)	16	#4	3'-0"	
d3(E)	16	#4	4'-4"	
m10(E)	5	#6	19'-7"	
m11(E)	5	#6	35'-8"	
m12(E)	12	#6	6'-0"	
m13(E)	12	#6	6'-0"	
m14(E)	2 2	#6	3'-2"	
m15(E)	2	#6	3'-2"	
m16(E)	8	#6	3'-3"	
m18(E)	2	#4	19'-7"	
m19(E)		#4	35'-8"	
m100(E)	27	#6	5'-0"	
s(E)	68	#5	7'-6"	
s1(E)	51	#5	8'-10"	
u(E)	68	#4	5'-0"	Ш
v(E)	56	#5	3'-1"	Ц
Reinforce		rs,	Pound	4060
Ероху Со	ated			

Concrete Superstructure Cu. Yd.

Structure Repair of

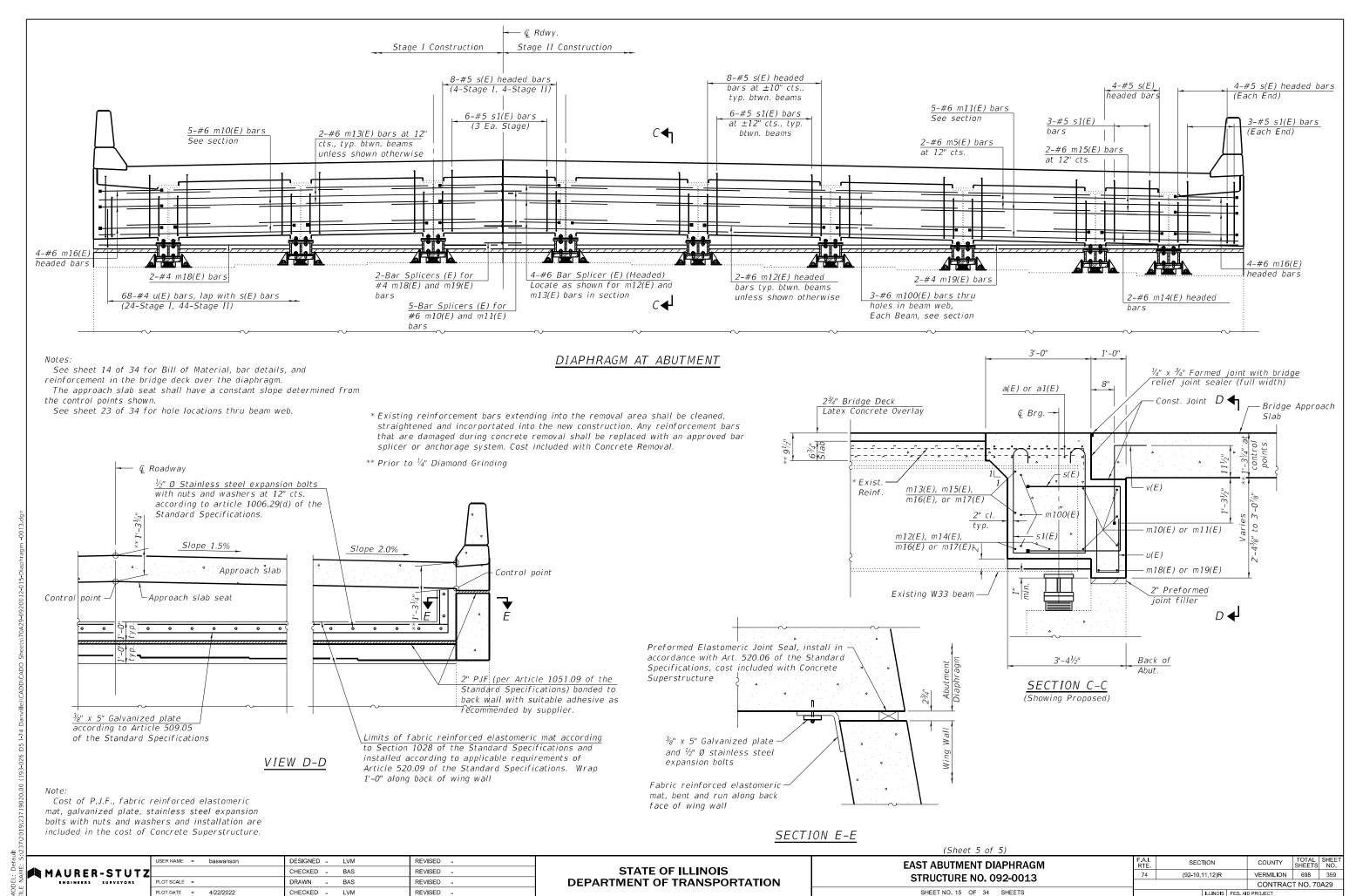
Concrete (Depth ≤ 5")

32.2

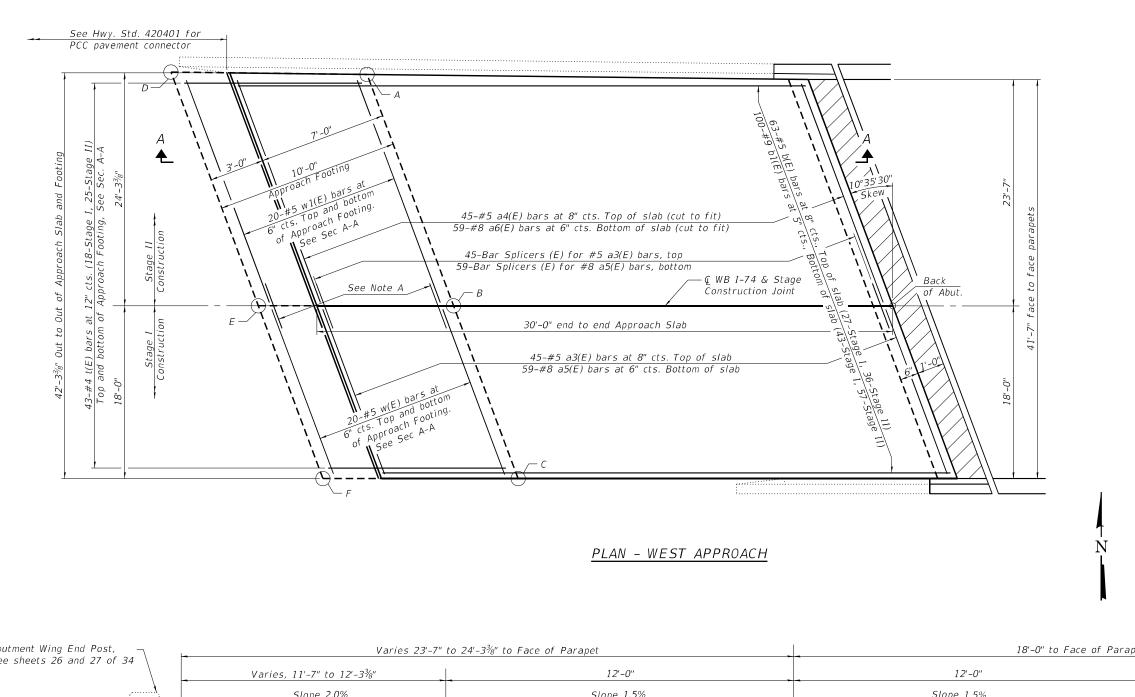
Sq. Ft.

BARS m12(E), m14(E) m16(E), & m17(E)

(Sheet 4 of 5) DESIGNED - LVM REVISED -JSER NAME = baswanson SECTION SUPERSTRUCTURE DETAILS COUNTY **STATE OF ILLINOIS** CHECKED - BAS MAURER-STUTZ REVISED -74 (92-10,11,12)R VERMILION 698 358 **STRUCTURE NO. 092-0013** DRAWN REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 70A29 SHEET NO. 14 OF 34 SHEETS PLOT DATE = 4/22/2022 CHECKED - LVM REVISED -



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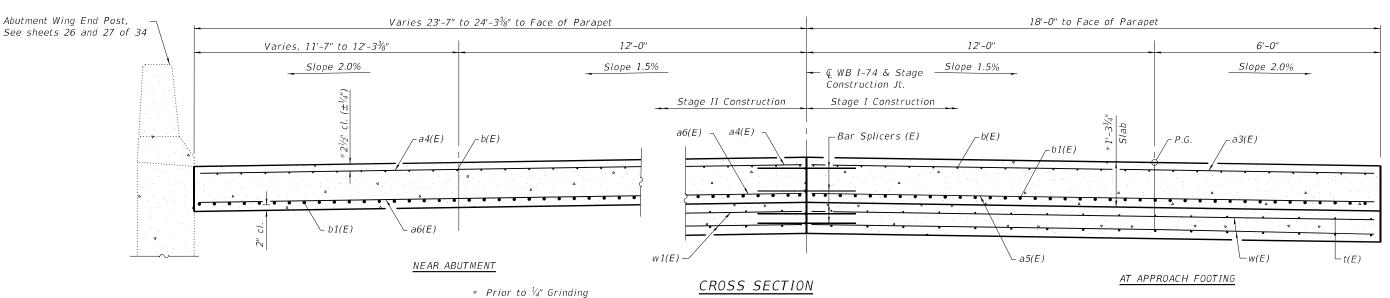
TOP AND BOTTOM ELEVATIONS FOR APPROACH FOOTING

Point	Тор	Bottom
Α	644.40	643.57
В	644.79	643.96
С	644.46	643.63
D	644.49	643.66
Ε	644.88	644.05
F	644.55	643.72

Note A - 20-Bar Splicers (E) for #5 w(E) bars, top & bottom

See sheet 20 of 34 for Section A-A and Bill of Material.

See sheet 34 of 34 for Bar Splicer details. See sheet 27 of 34 for details of new hatch block & parapets.



REVISED -JSER NAME = baswanson DESIGNED - LVM MAURER-STUTZ CHECKED - BAS REVISED -DRAWN BAS REVISED PLOT DATE = 4/22/2022 CHECKED -REVISED .

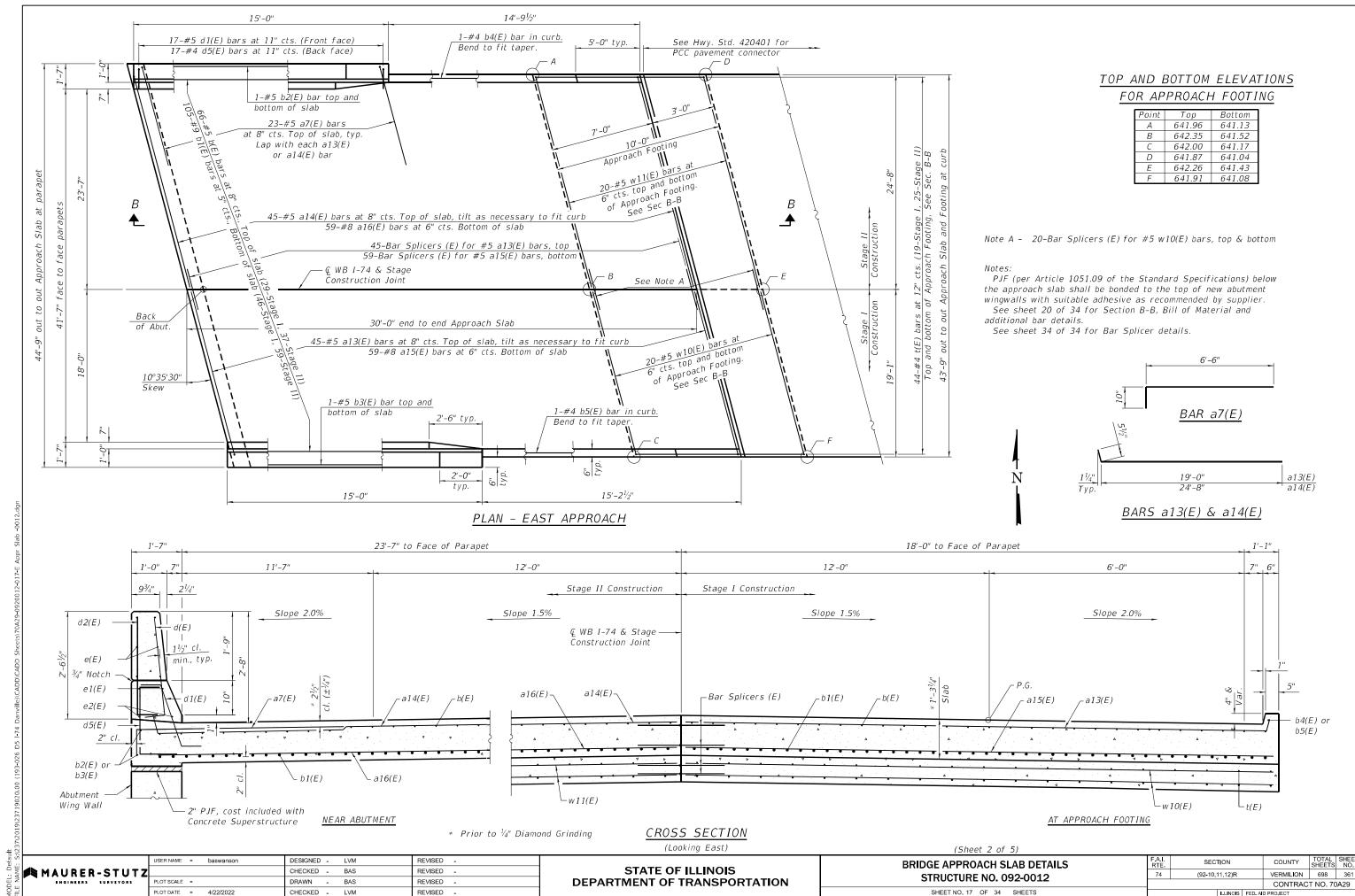
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

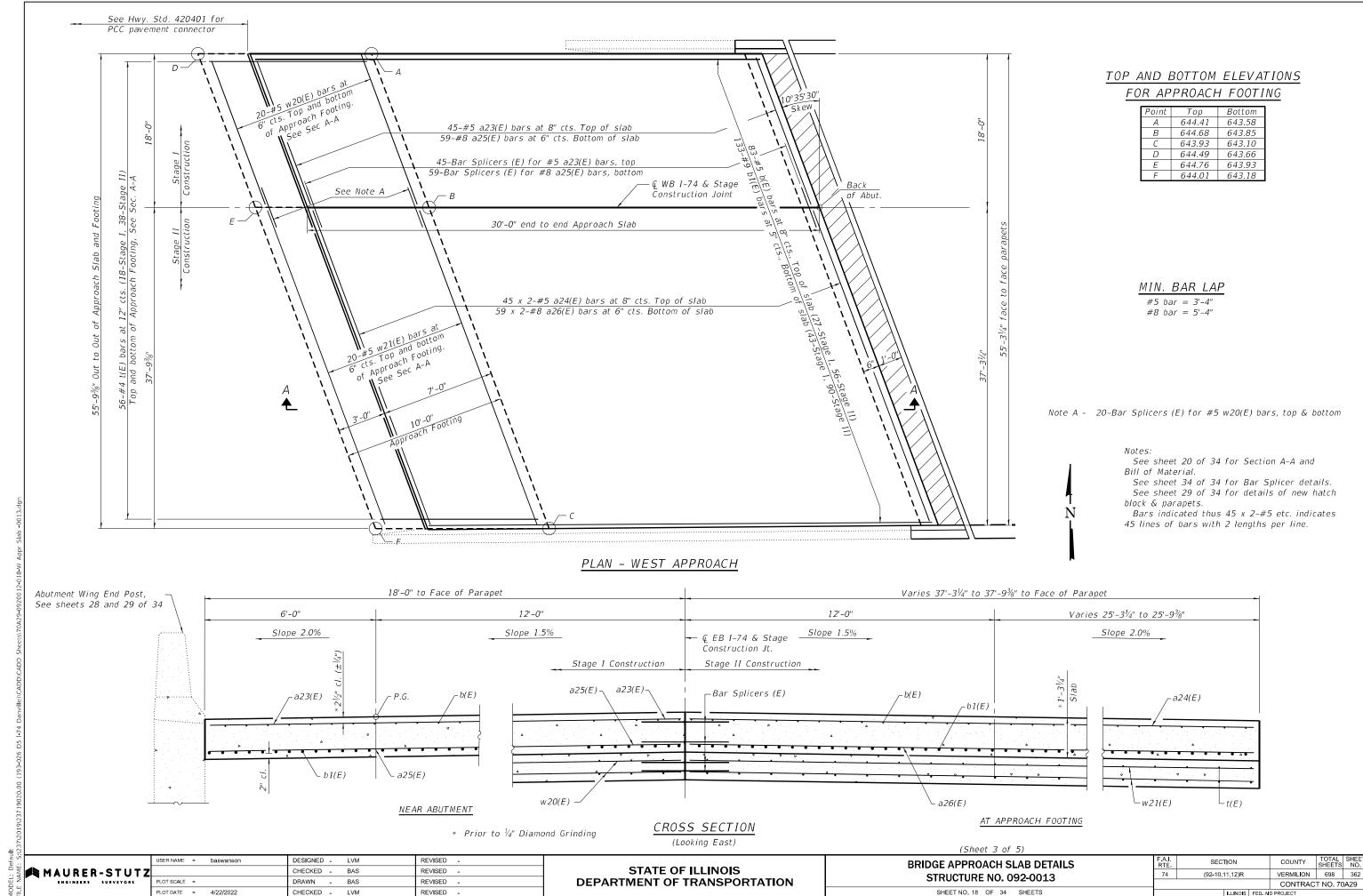
(Looking East)

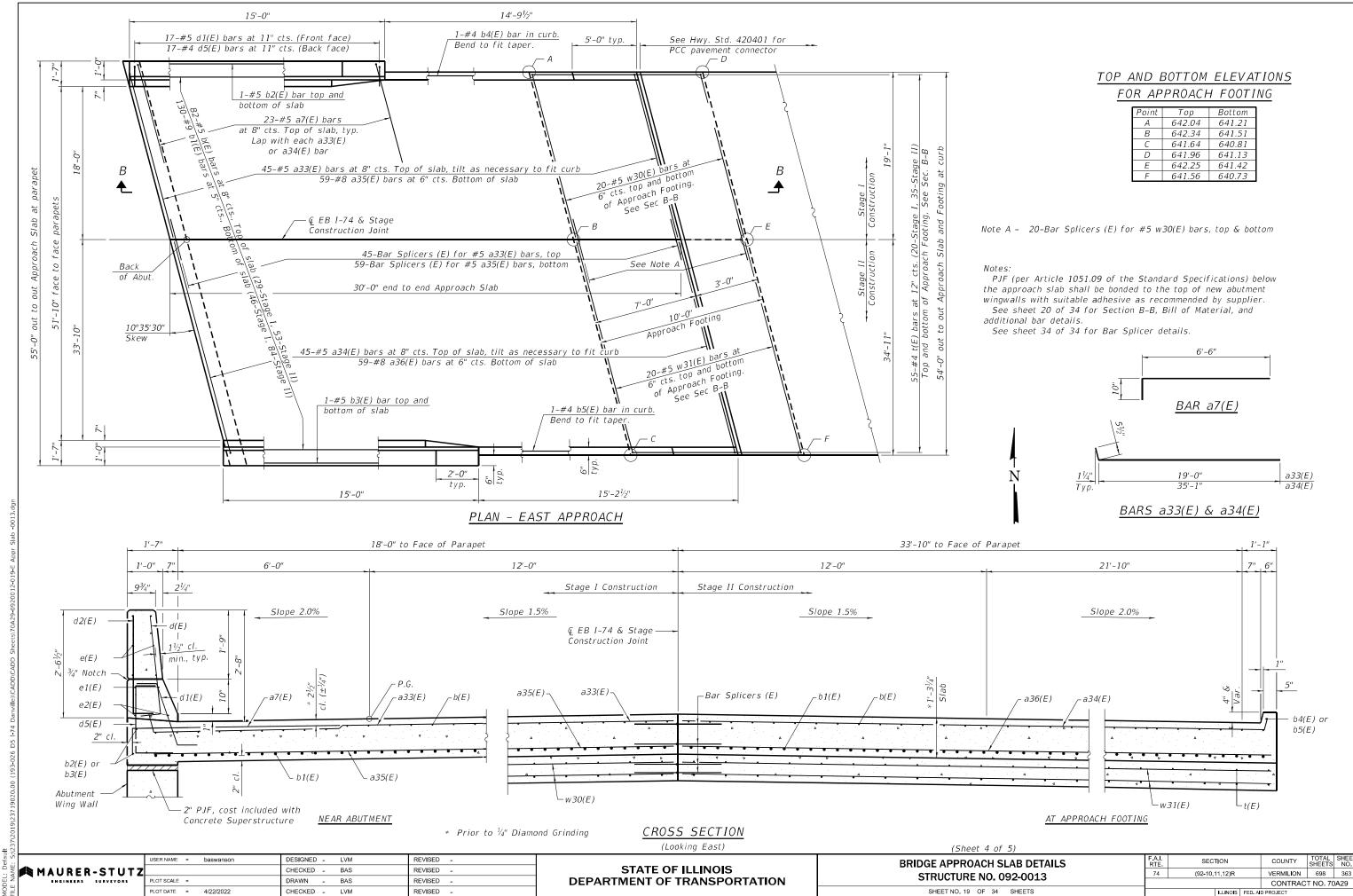
BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 092-0012 SHEET NO. 16 OF 34 SHEETS

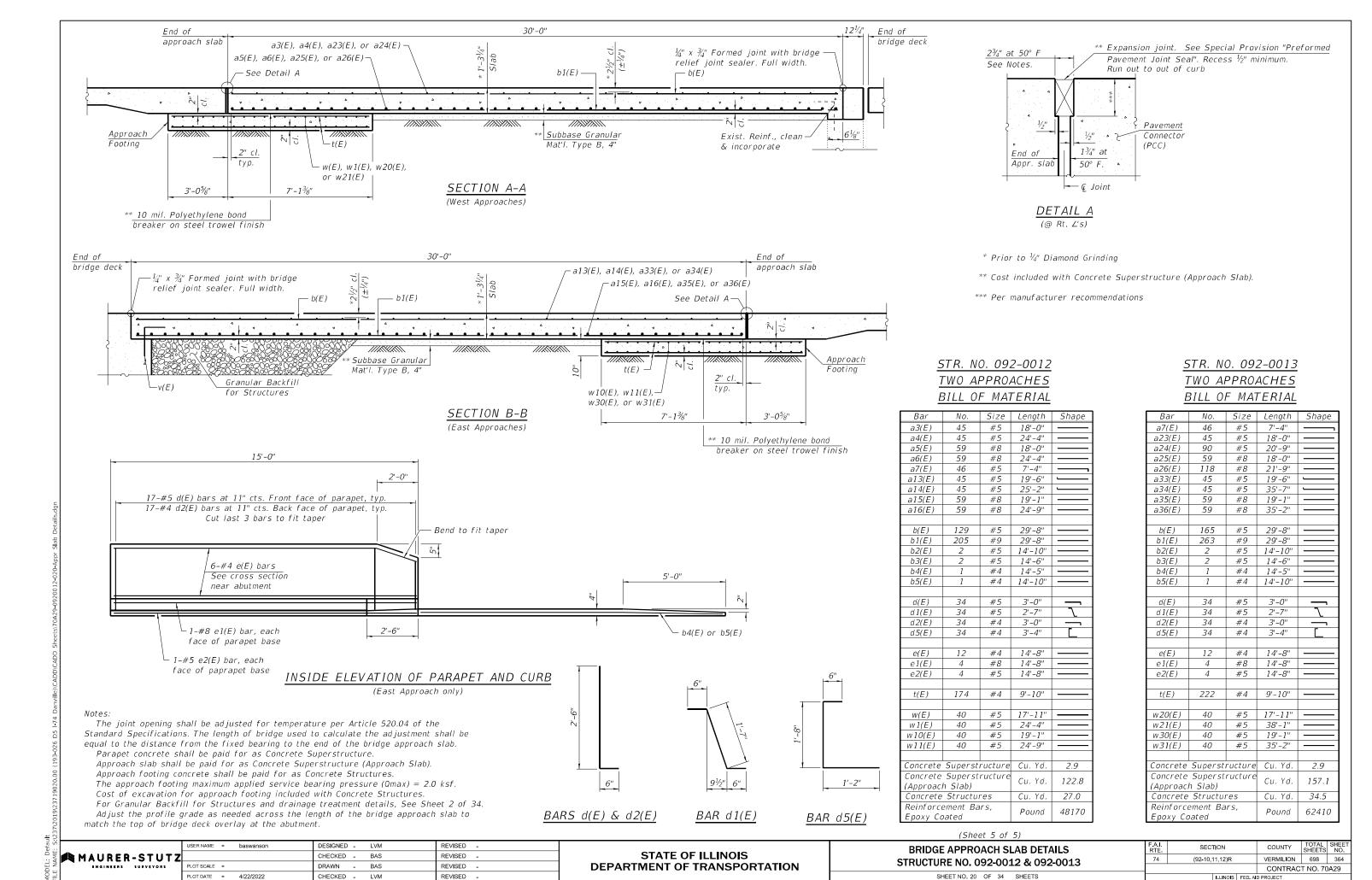
(Sheet 1 of 5)

TOTAL SHEE SHEETS NO. SECTION COUNTY (92-10,11,12)R VERMILION 698 360 CONTRACT NO. 70A29

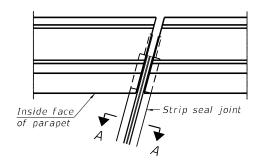




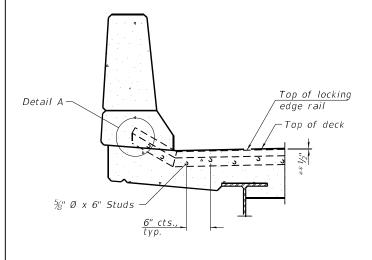




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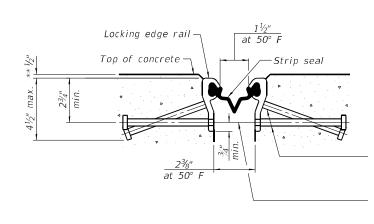
$FOR SKEWS \le 30^{\circ}$ PLAN AT PARAPET



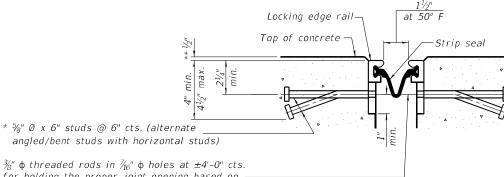


DETAIL A

** Prior to 1/4" Diamond Grinding



SHOWING ROLLED RAIL JOINT



for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

SHOWING WELDED RAIL JOINT

SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

Notes:

The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

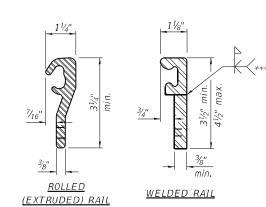
The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

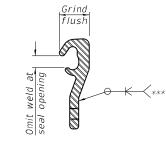


LOCKING EDGE RAILS

*** Back gouge not required if complete joint penetration is verified by mock-up.

BILL OF MATERIAL STR. NO. 092-0012

Item	Unit	Total
Preformed Joint Strip Seal	Foot	45
·		



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL STR. NO. 092-0013

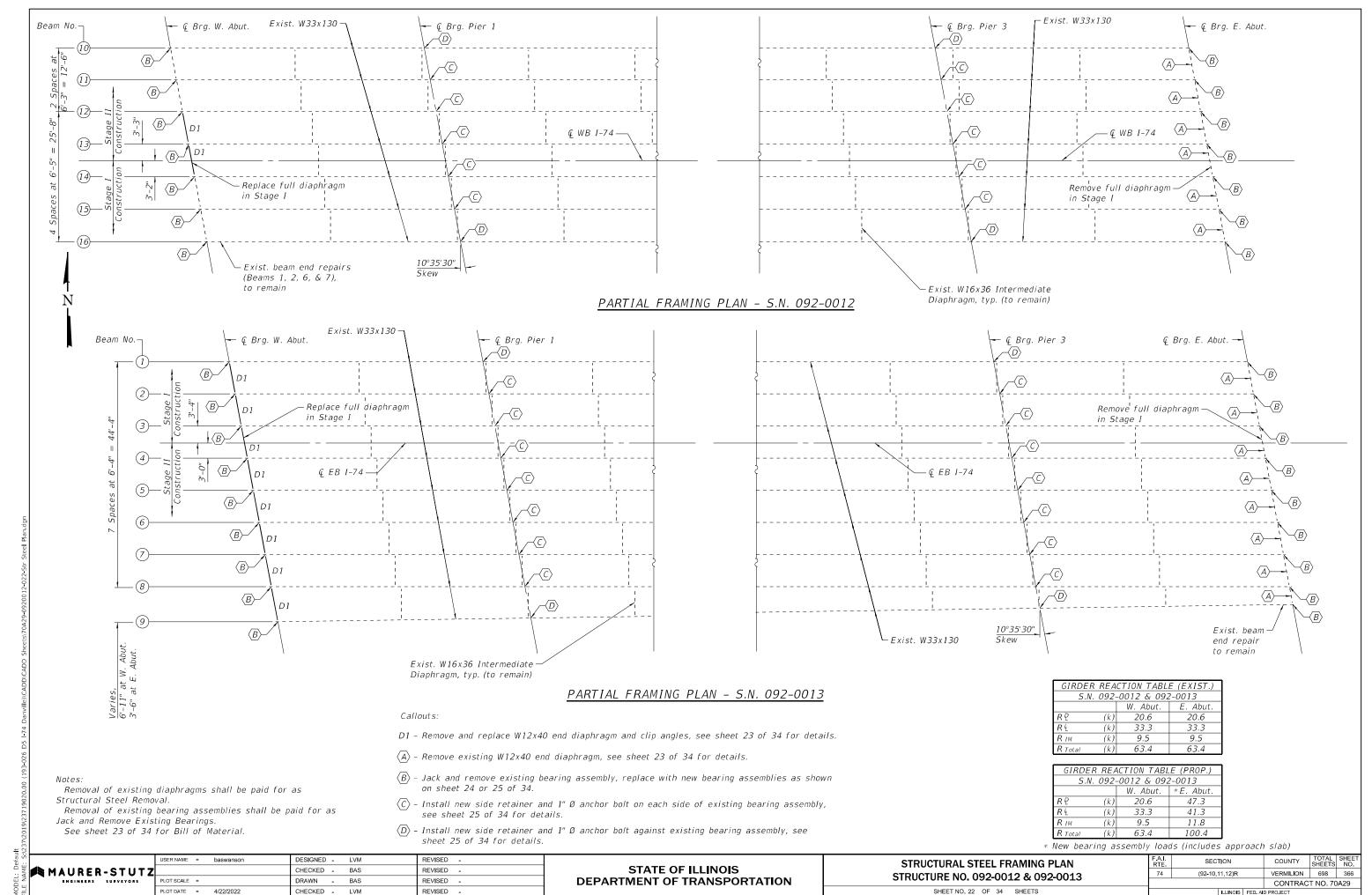
Item	Unit	Total	
Preformed Joint Strip Seal	Foot	59	

REVISED -DESIGNED - LVM baswansor CHECKED - BAS REVISED -**≠** MAURER-STUTZ DRAWN REVISED PLOT DATE = 4/22/2022 CHECKED -REVISED .

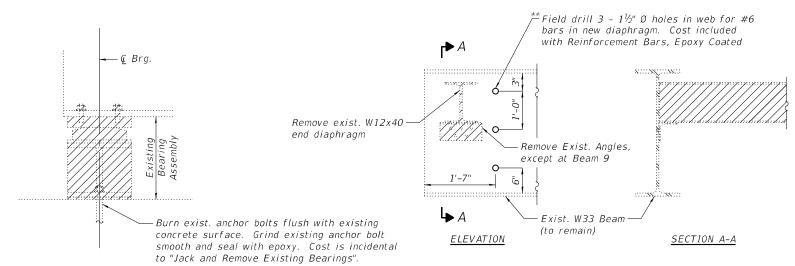
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PREFORMED JOINT STRIP SEAL STRUCTURE NO. 092-0012 & 092-0013 SHEET NO. 21 OF 34 SHEETS

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
74	(92-10,11,12)R		VERMILION	698	365
			CONTRACT NO. 70A29		
ILLINOIS FED AID PROJECT					



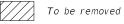
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EXISTING BEARING REMOVAL DETAIL

END DIAPHRAGM REMOVAL

(typ. at East Abutment)



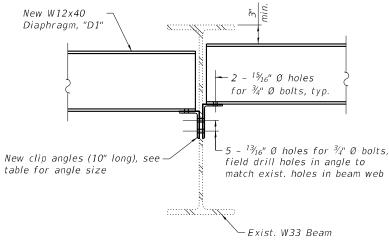
Notes:

Removal of the existing steel extensions and side retainers at the abutments shall be included in the cost of Jack and Remove Existing Bearings.

Minimum jack capacity shall be 30 tons.

Removal of steel end diaphragms shall be paid for as Structural Steel Removal.

New diaphragms shall be paid for as Furnishing and Erecting Structural Steel.



DIAPHRAGM CLIP ANGLE SIZES

S.N. 092-0012, W. Abut.				
(of web)	N. Side	S. Side		
Beam 10		exist.		
Beam 11	exist.	exist.		
Beam 12	* L6x4x ³ / ₄	L6x4x ³ / ₄		
Beam 13	L6x4x ³ / ₄	L6x4x ³ / ₄		
Beam 14	L6x4x ³ / ₄	* L6x4x ³ / ₄		
Beam 15	exist.	exist.		
Beam 16	exist.			
S.N. 092-0013, W. Abut.				
(of web)	N. Side	S. Side		
Beam 1		L6x4x ³ / ₄		
Beam 2	L6x4x ³ / ₄	L6x4x ³ / ₄		
Beam 3	L6x4x ³ / ₄	L6x4x ³ / ₄		
Beam 4	L6x4x ³ / ₄	L6x4x ³ / ₄		
Beam 5	L7 x 4 x 3/4	L6x4x ³ / ₄		
Beam 6	L8x4x ³ / ₄	L6x4x ³ / ₄		
Beam 7	L8x4x ³ / ₄	L6x4x ³ / ₄		
Beam 8	L8x4x ³ / ₄	L6x4x ³ / ₄		
Beam 9	L6x4x ³ / ₄			

NEW DIAPHRAGM D1

(10 diaphragms at West Abutment))

* Field drill all holes in angle to match holes in exist. beam web and exist. diaphragm

BILL OF MATERIAL STR. NO. 092-0012

·		
Item	Unit	Quantity
Elastomeric Bearing Assembly, Type I	Each	7
Elastomeric Bearing Assembly, Type II	Each	7
Anchor Bolts, 1"	Each	52
Furnishing and Erecting Structural Steel	Pound	3640
Jack and Remove Existing Bearings	Each	14
Structural Steel Removal	Pound	2400

BILL OF MATERIAL STR. NO. 092-0013

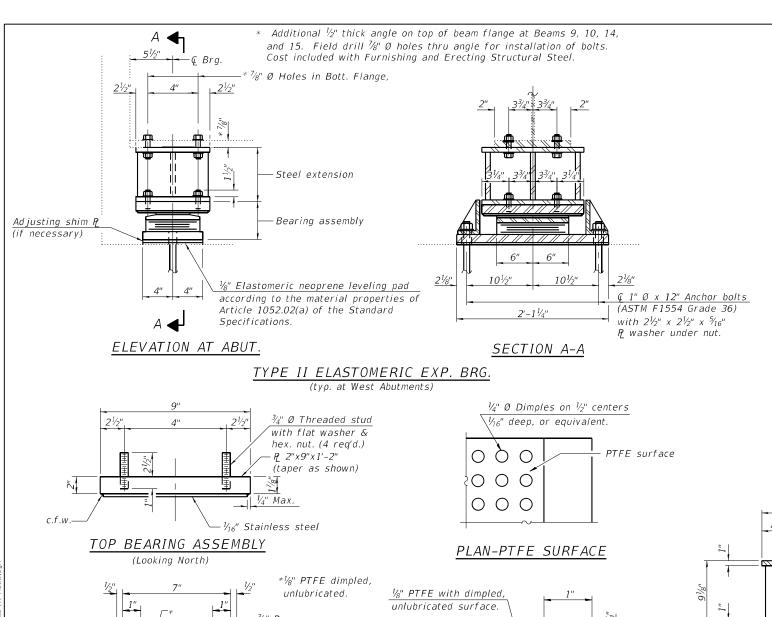
Item	Unit	Quantity
Elastomeric Bearing Assembly, Type I	Each	9
Elastomeric Bearing Assembly, Type II	Each	9
Anchor Bolts, 1"	Each	68
Furnishing and Erecting Structural Steel	Pound	6250
Jack and Remove Existing Bearings	Each	18
Structural Steel Removal	Pound	4620

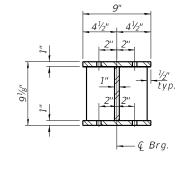
USER NAME = baswanson DESIGNED - LVM REVISED
CHECKED - BAS REVISED
PLOT SCALE = PLOT DATE = 4/22/2022 CHECKED - LVM REVISED
PLOT DATE = 4/22/2022 CHECKED - LVM REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

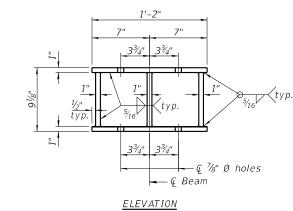
STRUCTURAL STEEL DETAILS
STRUCTURE NO. 092-0012 & 092-0013

SHEET NO. 23 OF 34 SHEETS





SECTION



PLAN - TOP & BOTTOM P

€ 11/4" Ø hole

SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates. (Cost included with Elastomeric Bearing Assemblies, Type II)

🚤 🕻 Beam

specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554. Steel extensions, shim plates, and connection bolts shall be paid

Anchor bolts shall be ASTM F1554 all-thread (or an Engineerapproved alternate material) of the grade(s) and diameter(s)

for as Furnishing and Erecting Structural Steel.

Side retainers and stainless steel plates provided for new Elastomeric Bearing Assemblies shall be included in the cost of Elastomeric Bearing Assembly, of the corresponding type. Prior to ordering any material, the Contractor shall verify in

the field all bearing height and shim thickness dimensions. Anchor bolts and side retainers at all supports shall be

installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

Provide two $\frac{1}{8}$ " adjusting shims for each bearing in addition to all other plates of the Bearing Assembly and placed as shown on the bearing details.

See sheet 23 of 34 for Bill of Material.

The Top and Bottom Bearing Plates shall be shop painted with the inorganic zinc rich primer per AASHTO M300 Type I. Cost included with Elastomeric Bearing Assembly, Type II.

€ Bott. Brg.

SECTION THRU PTFE

$D=\frac{1}{8}$ " per each 100' of expansion for every 15° temp. change from the normal temp. of 50°F.

₡ Top Brg.

EXPANSION BEARING ORIENTATION The above diagrams are for informational purposes only to show the amount of expected offset "D" for the current temperature in the field.

DESIGNED - LVM REVISED baswansor CHECKED - BAS REVISED -MAURER-STUTZ DRAWN REVISED PLOT DATE = 4/22/2022 CHECKED -REVISED -

@ Bott. Brg.

4- Layers of 3/8"

elastomer

 $3-\frac{3}{32}$ " Steel plates

R 11/4"x8"x2'-11/4"

-- @ 1½" Ø Holes

BOTTOM BEARING ASSEMBLY

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

-Ç Top Brg.

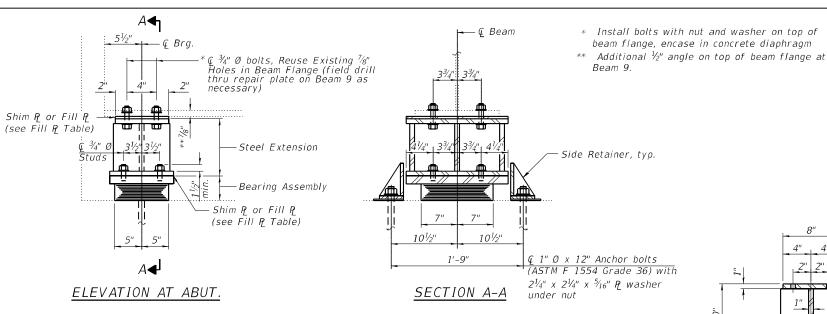
SECTION **BEARING DETAILS - WEST ABUTMENTS** (92-10,11,12)R VERMILION 698 368 STRUCTURE NO. 092-0012 & 092-0013 CONTRACT NO. 70A29 SHEET NO. 24 OF 34 SHEETS

STEEL EXTENSION

 $1^{1/4}$ "

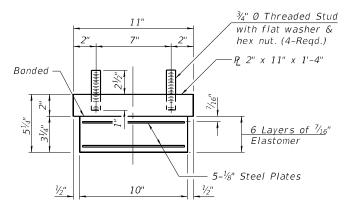
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<u>TYPE I ELASTOMERIC EXP. BRG.</u> (typ. at East Abutments)

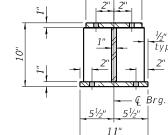
Shim plates shall not be placed under Bearing Assembly.



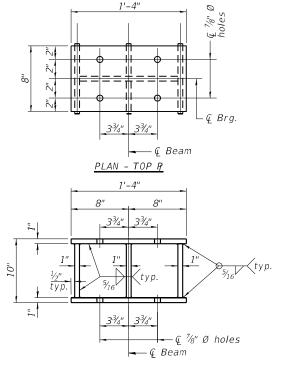
BEARING ASSEMBLY

FILL PLATE THICKNESS

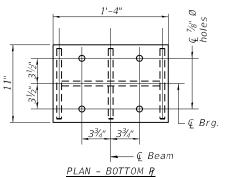
S.N. 092-00	12, E. Abut.
Beam 10	1 ³ / ₄ "
Beam 11	
Beam 12	1/4"
Beam 13	
Beam 14	
Beam 15	1/2"
Beam 16	
5.N. 092-00	13, E. Abut.
Beam 1	3/8"
Beam 2	1/4"
Beam 3	
Beam 4	
Beam 5	3/8"
Beam 6	1/2"
Beam 7	5/8"
Beam 8	1/4"
Beam 9	1/2"



<u>SECTION</u>



ELEVATION



STEEL EXTENSION

Notes

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Steel extensions, shim plates, and connection bolts shall be paid for as Furnishing and Erecting Structural Steel.

Side retainers and stainless steel plates provided for new Elastomeric Bearing Assemblies shall be included in the cost of Elastomeric Bearing Assembly, of the corresponding type. Where placed against exist. pier bearing assemblies, side retainers shall be paid for as Furnishing and Erecting Structural Steel.

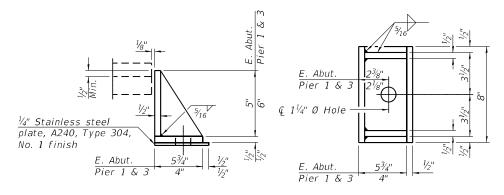
Prior to ordering any material, the Contractor shall verify in the field all bearing height and shim thickness dimensions.

Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

Provide two $\frac{1}{8}$ " adjusting shims for each bearing in addition to all other plates of the Bearing Assembly and placed as shown on the bearing details.

See sheet 23 of 34 for Bill of Material.

The Top Bearing Plates shall be shop painted with the inorganic zinc rich primer per AASHTO M300 Type I. Cost included with Elastomeric Bearing Assembly, Type I.



SIDE RETAINER

(at E. Abut., Pier 1, & Pier 3)

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

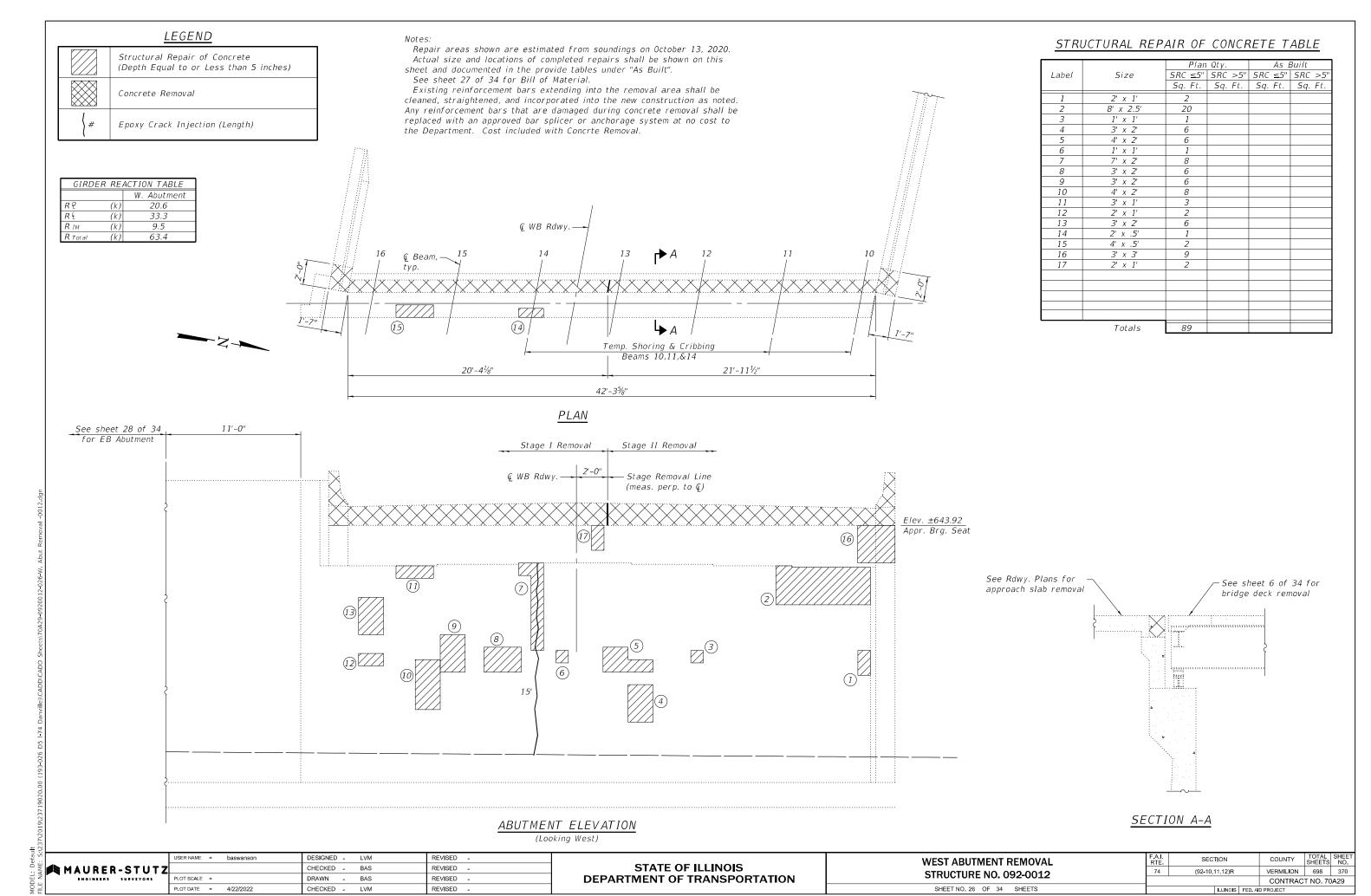
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NAURER-STUTZ			CHECKED - BAS	REVISED -	1
ENGINEERS SURVEYORS	PLOT SCALE =		DRAWN - BAS	REVISED -	
	PLOT DATE =	4/22/2022	CHECKED - LVM	REVISED -	

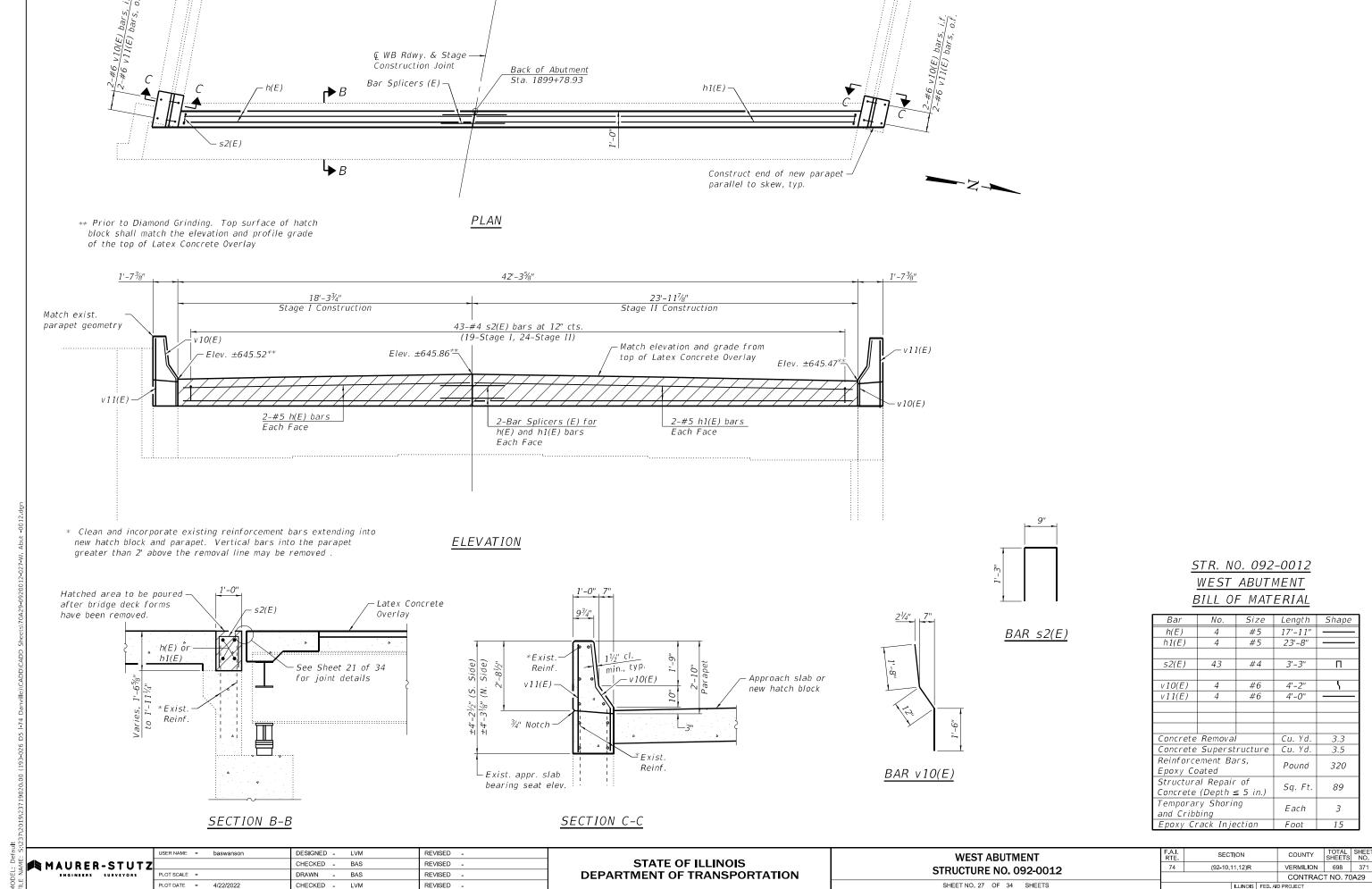
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

 BEARING DETAILS - EAST ABUTMENTS
 FAI. RTE.
 SECTION
 COUNTY
 TOTAL SHEET NO.

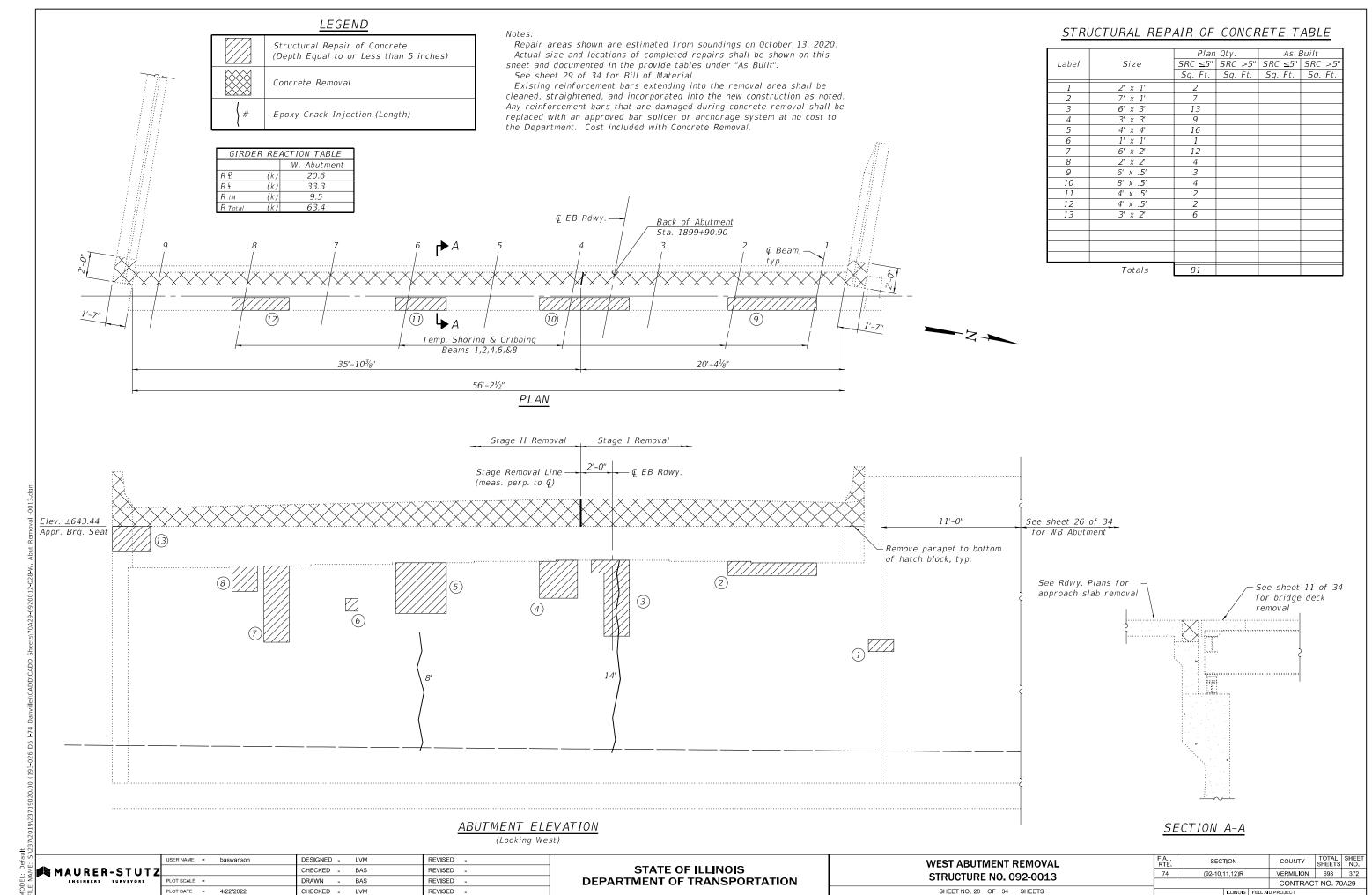
 STRUCTURE NO. 092-0012 & 092-0013
 74
 (92-10,11,12)R
 VERMILION
 698
 369

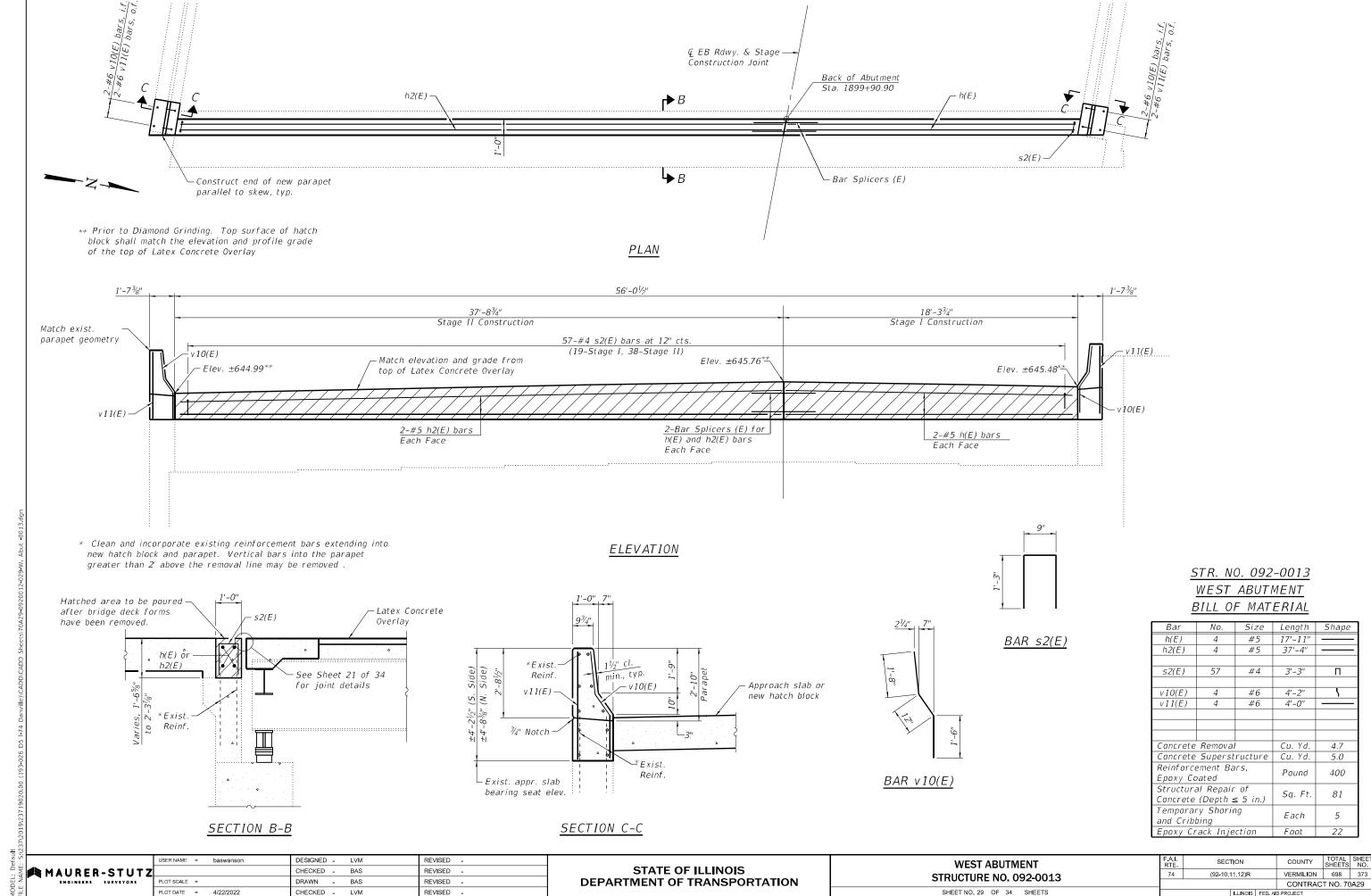
 SHEET NO. 25
 0F 34
 SHEETS
 LILINOIS FED.AUPROJECT
 CONTRACT NO. 70A29



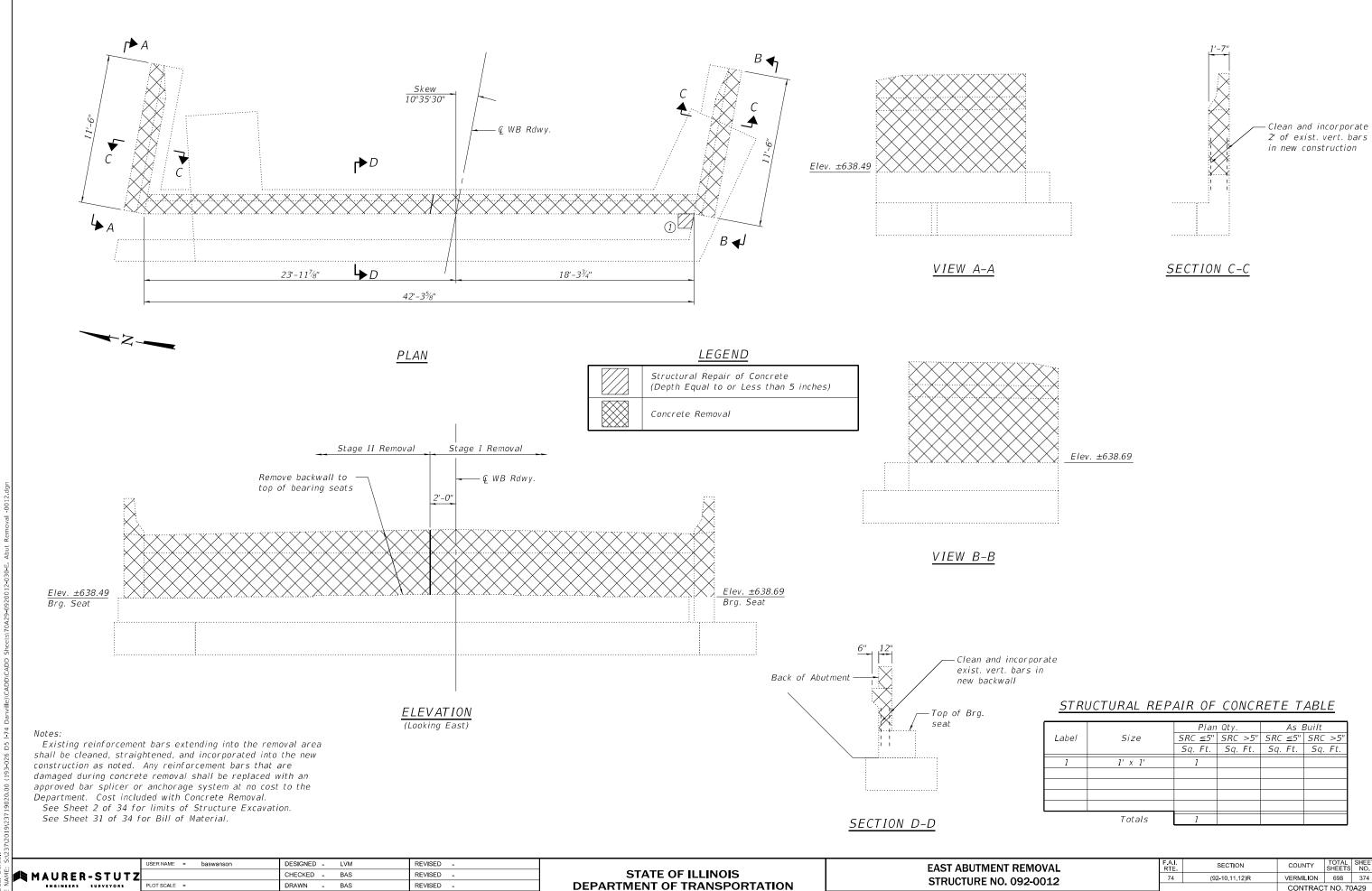


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DEPARTMENT OF TRANSPORTATION

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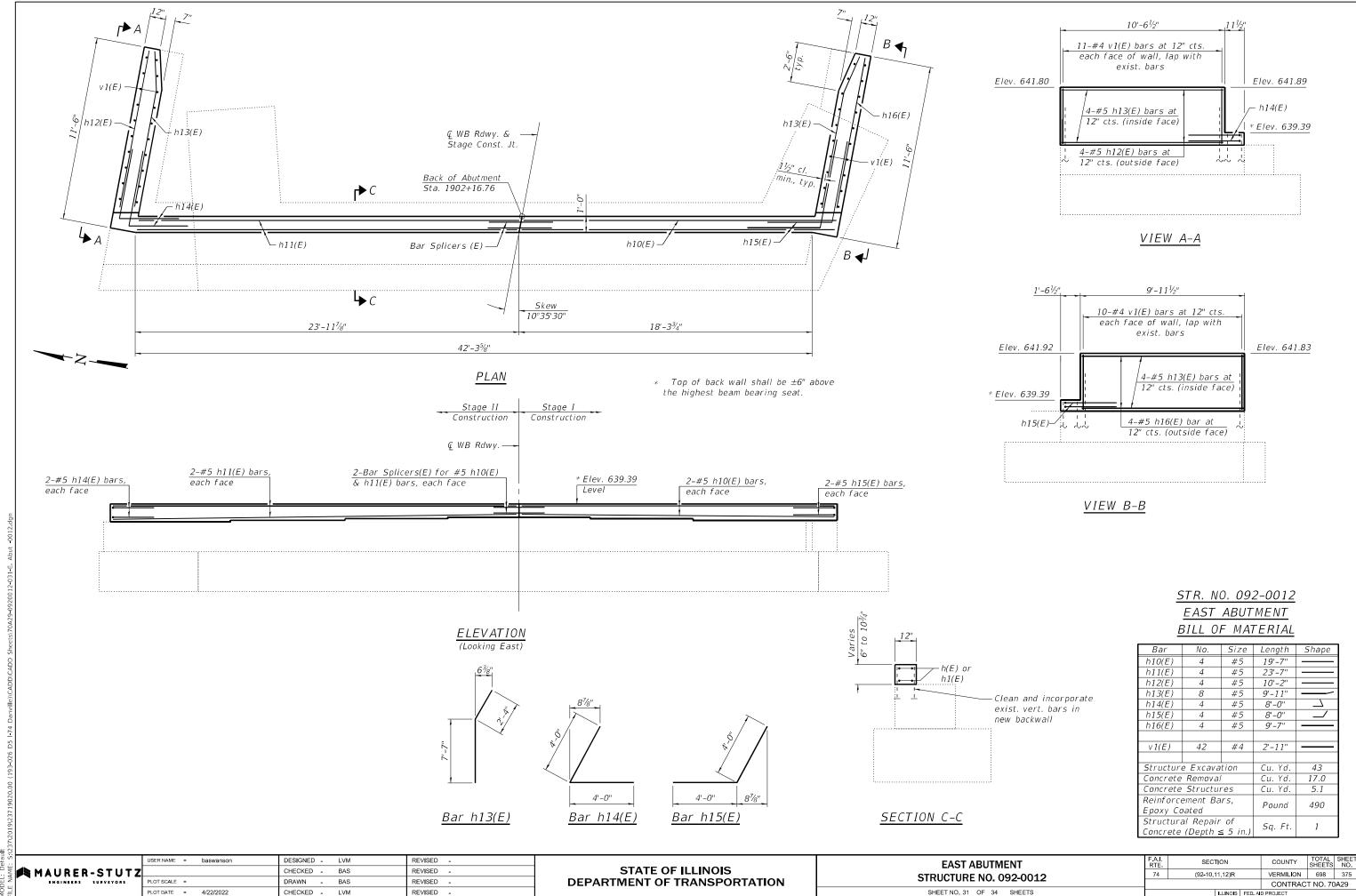
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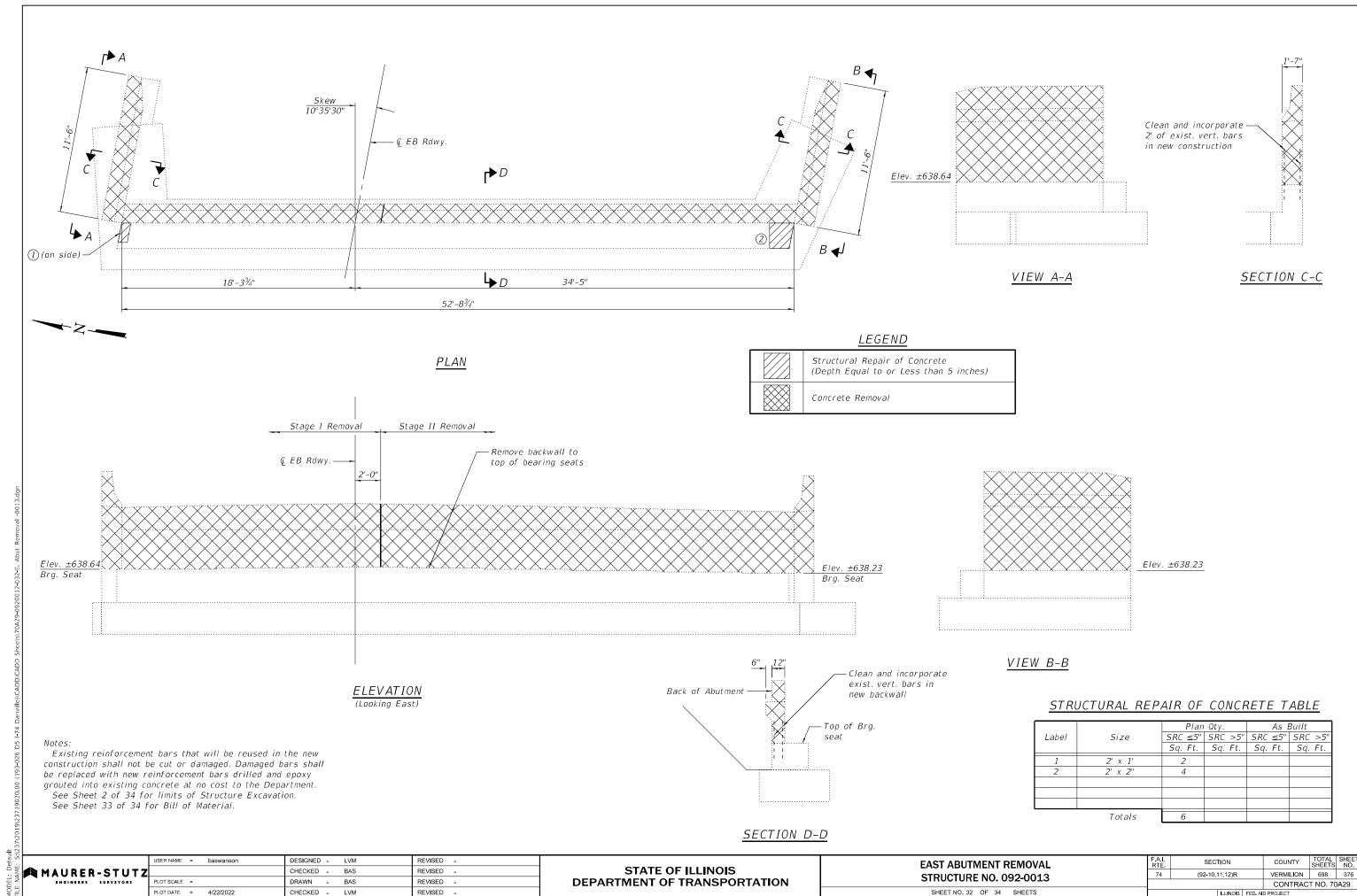
PLOT DATE = 4/22/2022

REVISED

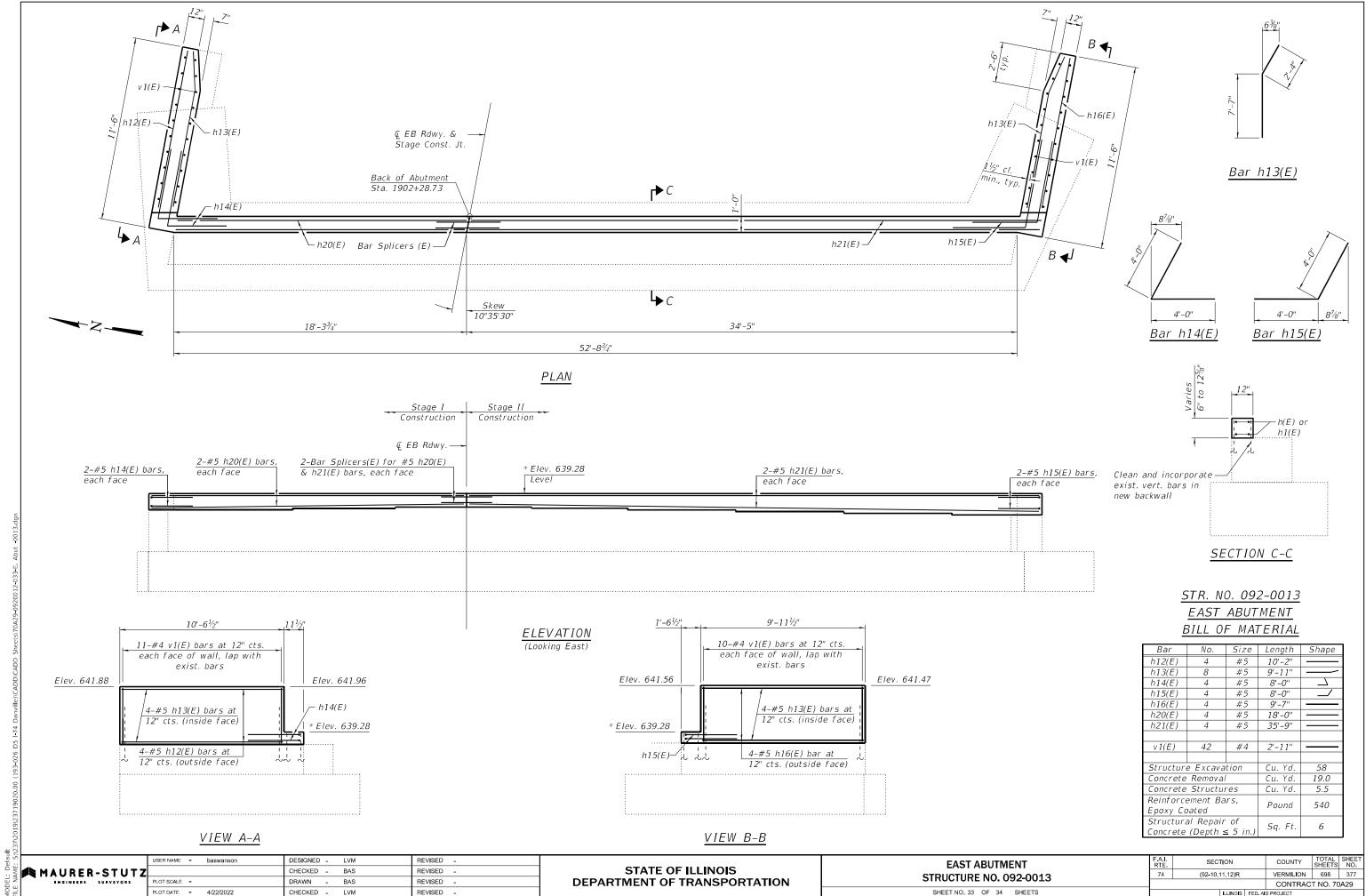
REVISED -

VERMILION 698 374 (92-10,11,12)R **STRUCTURE NO. 092-0012** CONTRACT NO. 70A29 SHEET NO. 30 OF 34 SHEETS





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STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

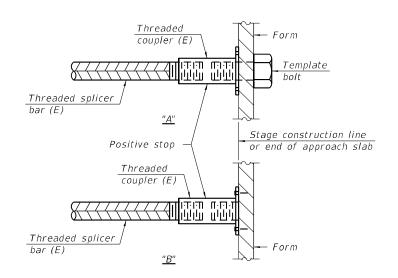
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

STR. NO. 092-0012

Location	Bar size	No. assemblies required	Minimum Iap length
Bridge Deck	#5	24	3'-6"
Diaphragm	#6	5	4'-0"
Diaphragm	#4	2	2'-5"
Approach Slab	#5	90	3'-4"
Approach Slab	#8	118	4'-9"
Approach Footing	#5	80	3'-2"
Abutments	#5	8	3'-7"

STR. NO. 092-0013

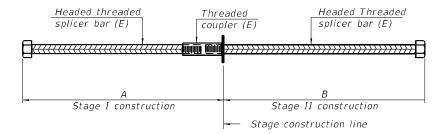
Location	Bar size	No. assemblies required	Minimum Iap length
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Approach Slab	#5	90	3'-4"
Approach Slab	#8	118	4'-9"
Approach Footing	#5	80	3'-2"
Abutments	#5	8	3'-7"



INSTALLATION AND SETTING METHODS

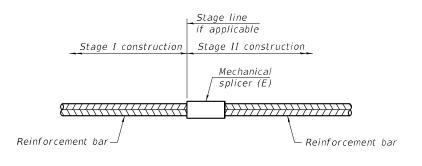
"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



HEADED BAR SPLICER ASSEMBLY PLAN

Location	Bar	No. assemblies	Λ	R
LOCACION	size	required	A	Ь
Diaphragms -0012	#6	4	3'-0"	3'-1"
Diaphragms -0013	#6	4	3'-2"	2'-10"



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for

alternatives.

USER NAME = baswanson DESIGNED - LVM REVISED
CHECKED - BAS REVISED
PLOT SCALE = DRAWN - BAS REVISED
PLOT DATE = 4/22/2022 CHECKED - LVM REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

 BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS NO.

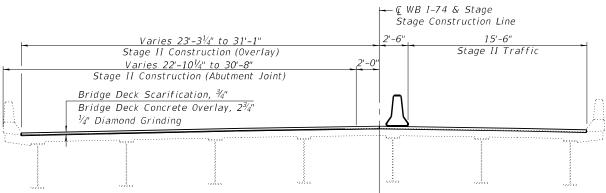
 STRUCTURE NO. 092-0012 & 092-0013
 4 (92-10,11,12)R
 VERMILION
 698
 378

 SHEET NO. 34 OF 34 SHEETS
 SHEET NO. 34 OF 34 SHEETS
 SHEET NO. 34 OF 34 SHEETS
 SHEET NO. 34 OF 34 SHEETS

Existing structures 092-0014 & 092-0015 were built in 1963 as F.A.I. 74, Section 92-12B at Station 1992+54. Each structure consisted of five continuous spans of two steel plate girders with steel floorbeams and reinforced PROPOSED SCOPE OF WORK - S.N. 092-0014 concrete deck superstructure with double column piers and vaulted abutments. Overall length is $583'-9\frac{1}{4}''$ and clear roadway width is 30'-0". In 1990 the structures were reconstructed as F.A.I. 74, Section (92-12B)BR. The Rehabilitate bridge deck and concrete approach pavement superstructures and decks were removed and replaced with 45" Web P Girders. Structure 092-0014 has 7 with scarification and latex concrete overlay girders spaced for a clear roadway width of $41'-3\frac{3}{4}''$ at the West Abutment and 49'-1'' at the East Abutment. Remove and replace HLMR bearing assemblies at west Traffic to be maintained using staged construction. abutment Salvage - None. Remove and replace deck joint at west abutment with preformed joint strip seal Remove and replace deck joint at east abutment with modular expansion joint Repair concrete delaminations on abutment wall and back walls Repair sections of concrete slope wall Traffic Barrier Terminal, Repair steel beam ends at west abutment Type 6, Std. 631031, replace Paint beam ends at NW corner only Exist. 45" R Girders LOADING HS20-44 DESIGN SPECIFICATIONS ELEVATION 2002 AASHTO Standard Specifications for (S.N. 092-0014, WB) Highway Bridges, 17th Edition (LFD) DESIGN STRESSES 583'-9¹/₄" Bk. to Bk. Abutments FIELD UNITS 2'-105/8' 2'-105/8" 100'-0" 126'-0" 126'-0" 126'-0" 100'-0" New Construction: f'c = 3,500 psiSpan 1 Span 2 Span 3 Span 4 Span 5 Bk. of W. Abut f'c = 4,000 psi (deck)© Pier 4 Sta. 1989+43.64 © Pier 3 E. Abut © Pier 2 fy = 60,000 psi (Reinforcement)Pier 1 Sta. 1994+24.52 € Brg. - Sta. 1991+72.52 Sta. 1992+98.52 fy = 50,000 psi (Str. Steel)W. Abut. Bk. of E. Abut. Sta. 1995+27.41 Exist. Structure: f'c = 3,500 psify = 60,000 psi (deck reinf.)fy = 40,000 psi (orig. substr. reinf.) -Varies - Q WB I-74 -Stage II Const. S.N. 092-0014 fy = 36,000 (structural steel)fy = 50,000 (plate girder flanges) -18'-0" —Stage I Const. 6' Shldr P.G.L. WB F.A.I. 74 Range 11W Ç E.B. Lanes -S.N. 092-0015 LOCATION SKETCH PLAN (Stations shown per original bridge plans) GENERAL PLAN AND ELEVATION BRYAN A. SWANSON 081-006716 I-74 OVER VERMILION RIVER F.A.I. 74 - SEC. (92-10,11,12)R Date Signed: 4-22-22 **VERMILION COUNTY** Exp. Date: 11/30/2022 STATION 1992+54.00 STRUCTURE NO. 092-0014

DESIGNED - BAS REVISED -JSER NAME = jdkoch SECTION COUNTY **STATE OF ILLINOIS** CHECKED - JDK MAURER-STUTZ REVISED -74 (92-10,11,12)R VERMILION 698 379 DRAWN REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 70A29 SHEET NO. 1 OF 19 SHEETS PLOT DATE = 4/21/2022 CHECKED - LVM REVISED -

STAGE I CONSTRUCTION (Looking East)



STAGE II CONSTRUCTION
(Looking East)

INDEX OF SHEETS

- 1. General Plan and Elevation
- 2. General Data
- 3. Slope Wall Repairs
- 4-5. Deck Slab Repair Plan
- 6-7. Superstructure Details
- 8-9. West Abutment Joint Replacement 10-11. East Abutment Joint Replacement
- 12. Preformed Joint Strip Seal
- 13. Drainage Scupper Adjustment Details
- 14. Inlet Box to be Adjusted
- 15. Structural Steel Framing Plan16. HLMR Bearing Replacement Details
- 17. West Abutment Repairs
- 18. East Abutment Repairs
- 19. Bar Splicer Assemblies
- USER NAME = jdkoch DESIGNED BAS REVISED
 CHECKED JDK REVISED
 PLOT SCALE = DRAWN JDK REVISED
 PLOT DATE = 4/21/2022 CHECKED LVM REVISED -

All new fasteners shall be $^{3}\!4''$ 0 high-strength steel bolts, unless noted otherwise.

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that can not be removed by grinding $\frac{1}{4}$ in deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas, and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Existing Name Plates shall be removed, cleaned and incorporated into the new construction. Cost included with Relocating Name Plates.

Synthetic Fibers shall be included in the bridge deck concrete overlay specified. See Special Provisions.

Existing structure plans are available from the District upon request.

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Stone Riprap, Class A4	Sq. Yd.	73
Filter Fabric	Sq. Yd.	73
Concrete Removal	Cu. Yd.	32.5
Concrete Superstructure	Cu. Yd.	36.4
Protective Coat	Sq. Yd.	3534
Reinforcement Bars, Epoxy Coated	Pound	6310
Bar Splicers	Each	48
Mechanical Splicers	Each	253
Preformed Joint Strip Seal	Foot	50
Modular Expansion Joint, 6"	Foot	57
Anchor Bolts, 1"	Each	28
Slope Wall Crack Sealing	Foot	81
Inlet Boxes to be Adjusted (Special)	Each	1
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	2482
High Load Multi-Rotational Bearings, Guided Expansion, 100K	Each	7
Jack and Remove Existing Bearings	Each	7
Structural Steel Repair	Pound	2200
Bridge Deck Latex Concrete Overlay 2¾"	Sq. Yd.	3443
Bridge Deck Scarification ¾"	Sq. Yd.	3443
Cleaning and Painting Steel Bridge No. 5	L. Sum	1
Containment and Disposal of Non-Lead Paint Cleaning Residues No. 5	L. Sum	1
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	96
Deck Slab Repair (Full Depth, Type II)	Sq. Yd.	7
Drainage Scuppers to be Adjusted	Each	6
Diamond Grinding (Bridge Section)	Sq. Yd.	3200
Polymer Concrete	Cu. Ft.	5
Relocating Name Plates	Each	2
Slope Wall Repair	Sq. Yd.	18
Slope Wall Slurry Pumping	Cu. Yd.	7

* On new concrete superstructure and latex concrete overlay.

GENERAL NOTES

Cleaning and painting of the existing structural steel shall be as specified in the special provision for "Cleaning and Painting Existing Steel Structures". After removal of deck ends, but prior to new modular joint and bearing installation or structural steel repair, all beams, diaphragms, and other existing structural steel within 5 ft of the beam ends shall be cleaned per Near White Blast Cleaning (SSPC-SP10).

The designated areas cleaned per Near White Blast Cleaning (SSPC-SP10) shall be painted according to the requirements of Paint System 1 (OZ/E/U). The color of the final finish coat for all steel surfaces shall be Gray, Munsell No. 5B 7/1.

Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the special provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

Containment of cleaning residue is required to control nuisance dust. See special provisions.

SSPC-QP1 Painting Contractor Certifications will be required for this ontract.

All new structural steel shall be hot dip galvanized unless noted otherwise. See special provision for "Hot Dip Galvanizing for Structural Steel".

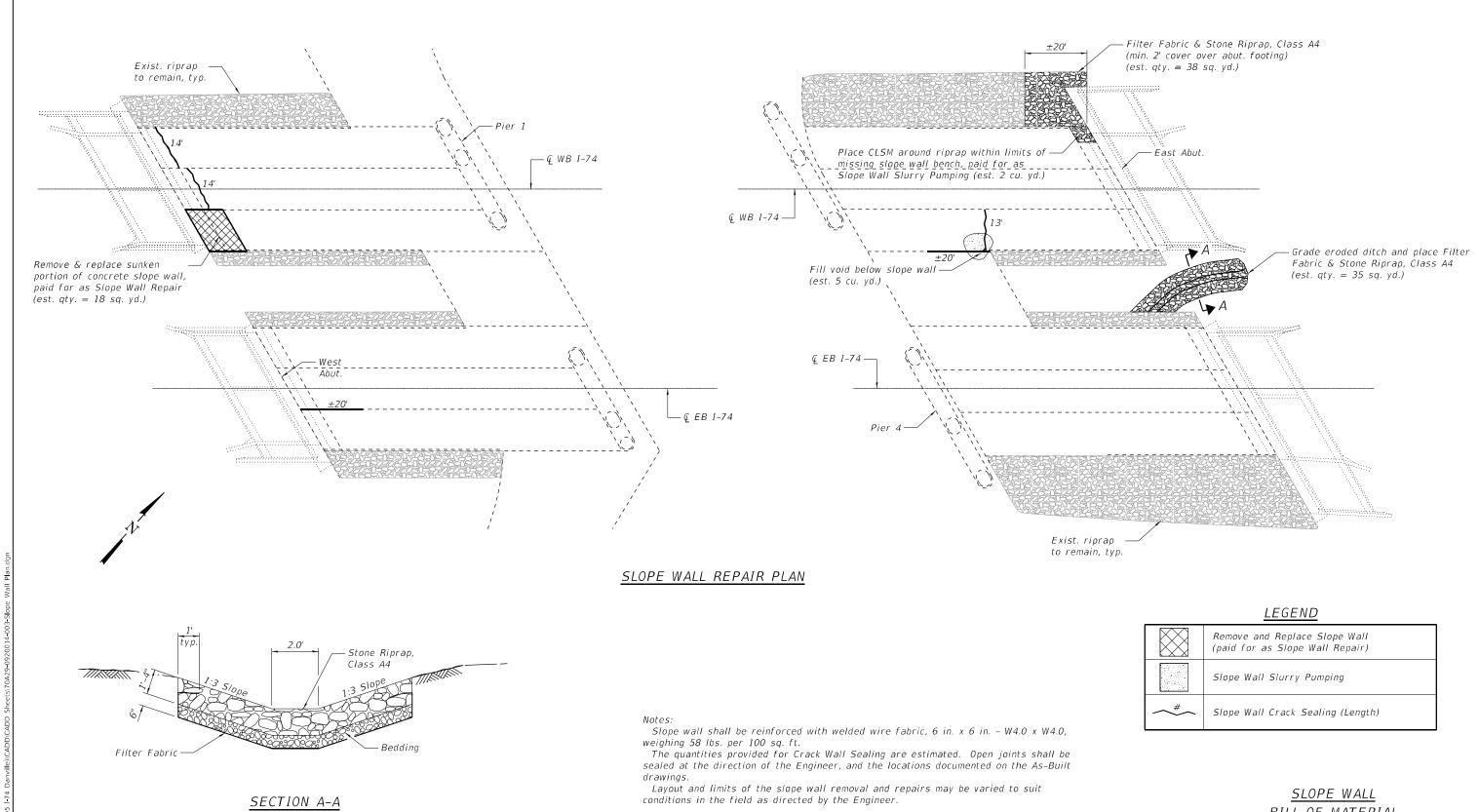
Expansion joints shall be fabricated and installed according to the manufacturer's recommendations and as approved by the Engineer.

Expansion joints shall be fabricated to conform to the existing cross slopes of the bridge.

Modular expansion joints shall be assembled in their final relative position with the ends in place for shop inspection and acceptance.

Joint openings shall be adjusted according to Article 503.10(c) of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL DATA	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
	STRUCTURE NO. 092-0014		(92-10,11,12)R	VERMILION	698	380	
				CONTRAC	T NO. 70	A29	
	SHEET NO. 2 OF 19 SHEETS		ILLINOIS FED. AI	D PROJECT			



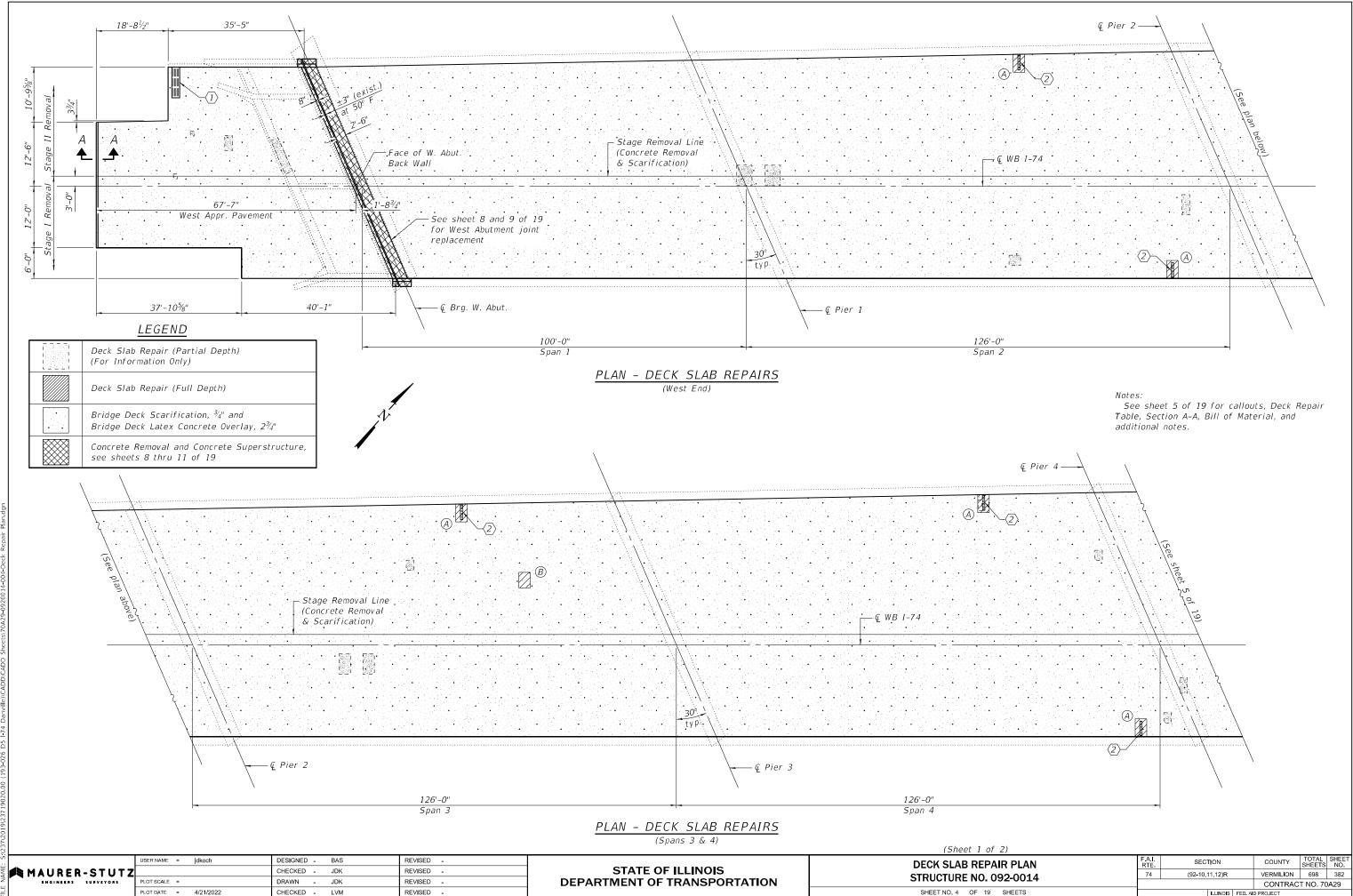
BILL OF MATERIAL

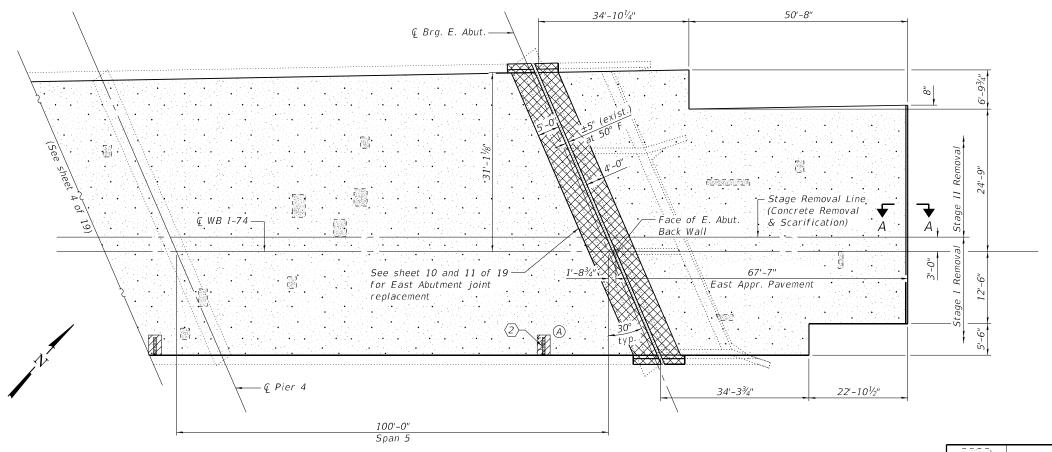
Unit	Quantity
Sq. Yd.	18
Cu. Yd.	7
Foot	81
Sq. Yd.	73
Sq. Yd.	73
	Sq. Yd. Cu. Yd. Foot Sq. Yd.

DESIGNED - BAS REVISED -JSER NAME = jdkoch CHECKED - JDK MAURER-STUTZ REVISED -DRAWN - JDK REVISED PLOT DATE = 4/21/2022 CHECKED - LVM REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION SLOPE WALL REPAIRS (92-10,11,12)R VERMILION 698 381 STRUCTURE NO. 092-0014 & 092-0015 CONTRACT NO. 70A29 SHEET NO. 3 OF 19 SHEETS





DECK SLAB REPAIR TABLE

		ļ ,	Plan Qty	/.	As I	Built
Label	Size	Deck Slab Repair (Part. Depth)		Deck Slab Repair (FD Type II)	Deck Slab Repair (FD Type I)	Deck Slab Repair (FD Type II)
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
Α	2' x 4' x 6 Loc.			48		
В	3' x 3'			9		
	W. Appr., Stage II	22				
	Span 2, Stage I	14				
	Span 2, Stage II	32				
	Span 3, Stage I	24				
	Span 3, Stage II	4				
	Span 4, Stage I	4				
	Span 4, Stage II	4				
	Span 5, Stage I	14				
	Span 5, Stage II	34				
	E. Appr., Stage I	7				
	E. Appr., Stage II	14				
	Totals	173		57		

<u>LEGEND</u>

7,777-7) (3,777-7) (4,777-7) (4,777-7)	Deck Slab Repair (Partial Depth) (For Information Only)
	Deck Slab Repair (Full Depth)
	Bridge Deck Scarification, ³ / ₄ " and Bridge Deck Latex Concrete Overlay, 2 ³ / ₄ "
	Concrete Removal and Concrete Superstructure, see sheets 8 thru 11 of 19

Notes

See sheet 6 of 19 for cross section thru bridge deck. Deck survey performed on December 14, 2020. Locations and sizes

Deck survey performed on December 14, 2020. Locations and sizes shown in the plan view are approximate.

Deck Slab Repair (Partial Depth) is an estimated quantity per the deck survey. This area shall not be paid for separately, but shall be addressed as stated in the Special Provision for Bridge Deck Latex Concrete Overlay.

Actual size and locations of full-depth patches shall be shown on this sheet and documented in the Deck Slab Repair Table under "As Built".

<u>DECK SLAB</u> BILL OF MATERIAL

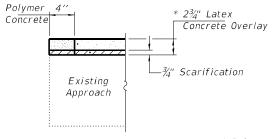
ITEM	UNIT	TOTAL
Deck Slab Repair (Full Depth, Type II)	Sq. Yd.	7
Bridge Deck Scarification ¾"	Sq. Yd.	3443
Bridge Deck Latex Concrete Overlay, 2¾ Inches	Sq. Yd.	3443

PLAN - DECK SLAB REPAIRS

(East End)

Callouts

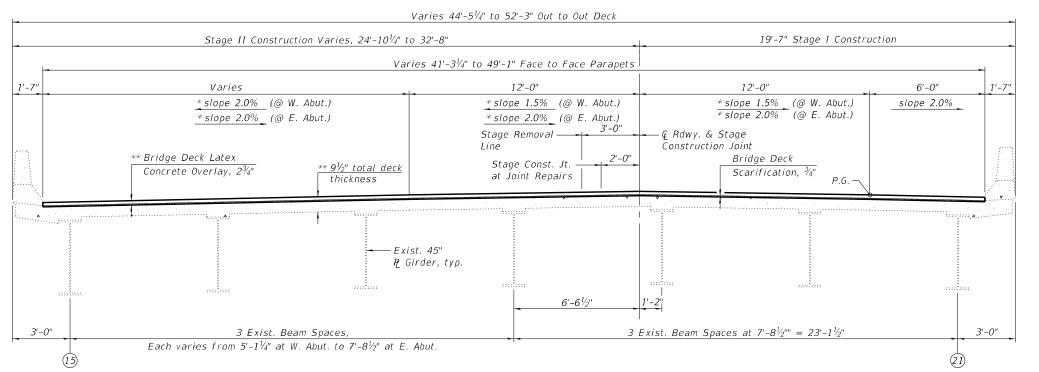
- $\langle \underline{1} \rangle$ Adjust elevation of frame and grate on inlet box to match overlay. See sheet 14 of 19 for details.
- (2) Remove and reinstall drainage scupper and install adjusting ring to raise the grate elevation to match the overlay. See sheet 13 of 19 for details. Cost of removal and installing the existing scupper within the deck repair shall be included with Drainage Scuppers to be Adjusted.



* Prior to $\frac{1}{4}$ " Diamond Grinding. SECTION A-A

(Sheet 2 of 2)

	USER NAME = jdkoch	DESIGNED - BAS	REVISED -		DECK SLAB REPAIR PLAN	F.A.I. SECTION COUNTY	Y TOTAL SHEET
MAURER-STUTZ		CHECKED - JDK	REVISED -	STATE OF ILLINOIS	STRUCTURE NO. 092-0014	74 (92-10,11,12)R VERMILIC	ON 698 383
ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - JDK	REVISED -	DEPARTMENT OF TRANSPORTATION	51RUCTURE NO. 092-0014	CONTR	RACT NO. 70A29
	PLOT DATE = 4/21/2022	CHECKED - LVM	REVISED -		SHEET NO. 5 OF 19 SHEETS	ILLINOIS FED. AID PROJECT	



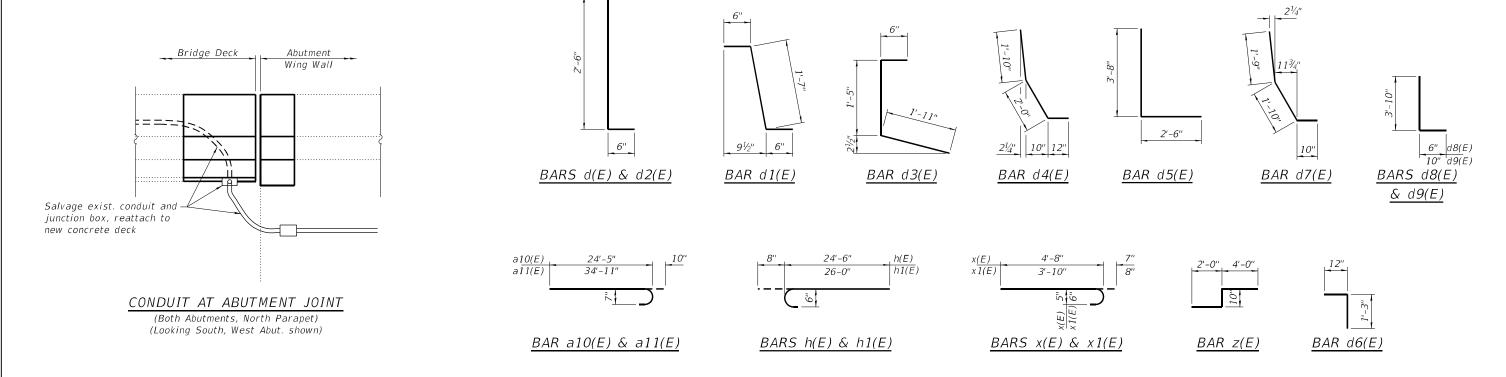
- * Cross slope varies from normal crown at Sta. 1993+42.56 to 2.0% superelevation at Sta. 1994+84.56.
- ** Prior to ½" Diamond Grinding

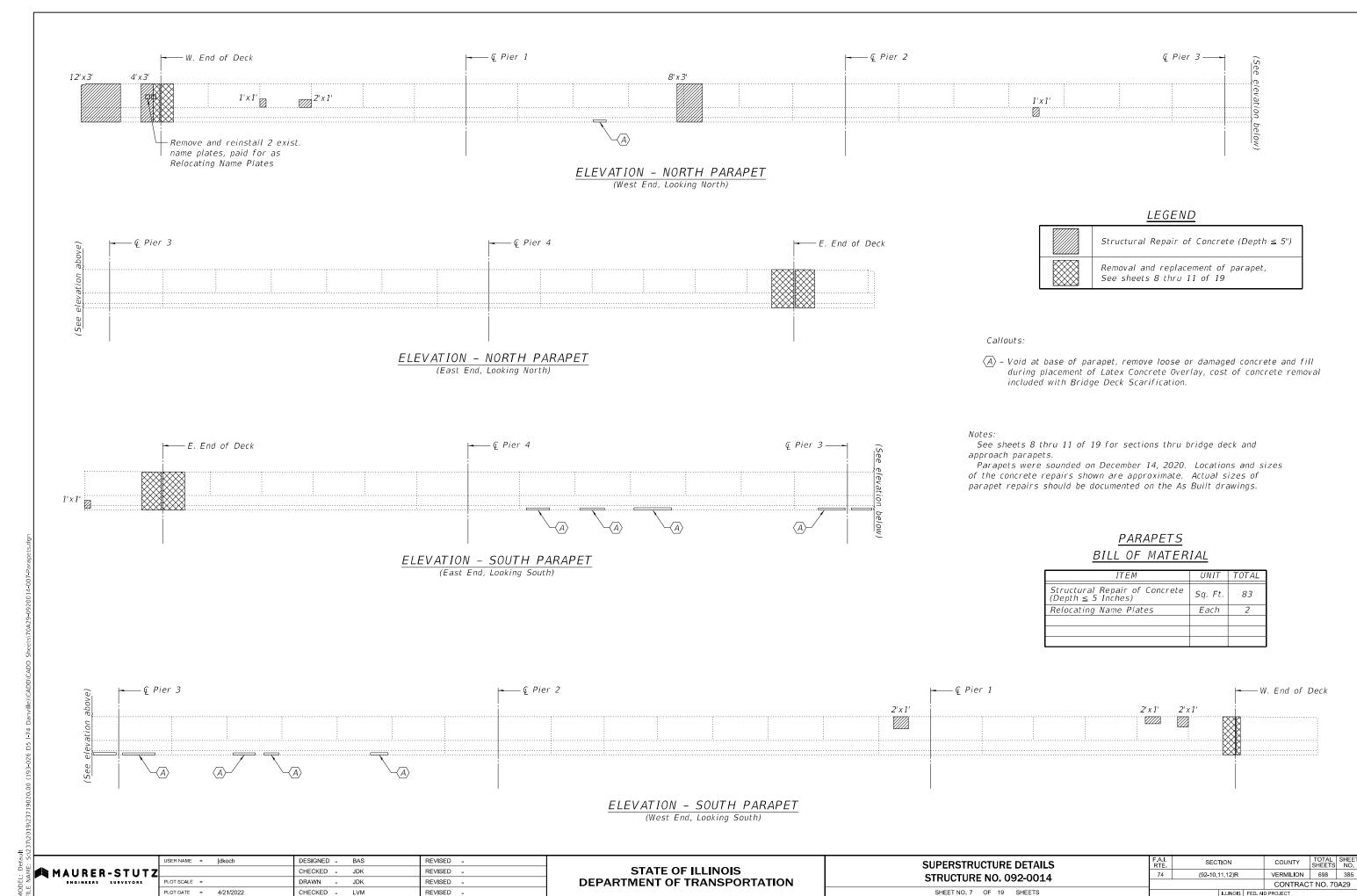
<u>CROSS SECTION - BRIDGE DECK</u>
(Looking East)

Notes

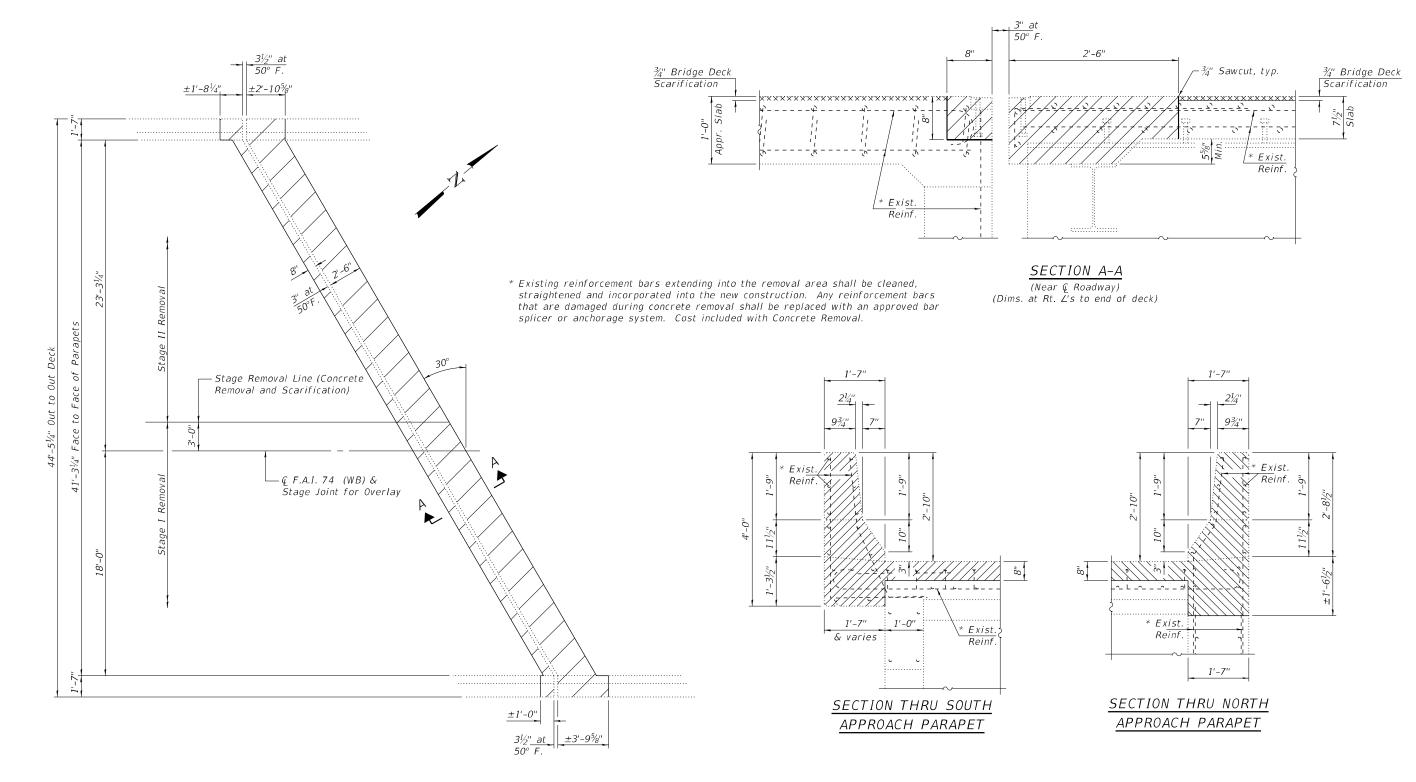
Actual bridge cross slopes and superelevation transition locations shall be measured and documented in the field prior to scarification.

Stations shown for the superelevation transition are according to the original bridge plans. Stations per the existing alignment may be different.





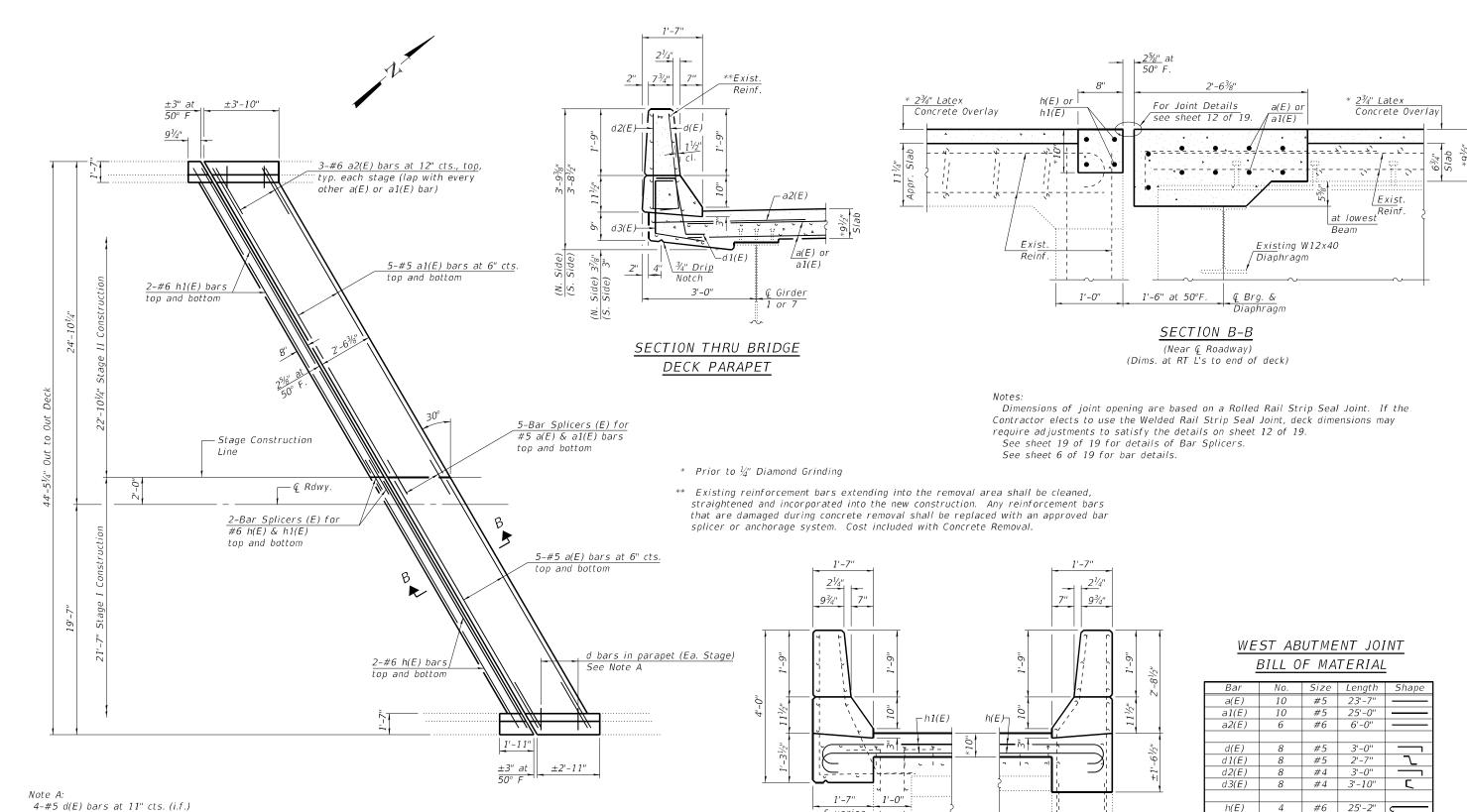
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CONCRETE REMOVAL AT WEST ABUTMENT

Notes: Hatched areas indicate Concrete Removal. For Bridge Deck Parapet Dimensions and Details and Bill of Material, see sheet 9 of 19.

jdkoch DESIGNED - BAS REVISED -SECTION WEST ABUTMENT JOINT REMOVAL DETAILS STATE OF ILLINOIS CHECKED - JDK MAURER-STUTZ REVISED -(92-10,11,12)R VERMILION 698 386 **STRUCTURE NO. 092-0014 DEPARTMENT OF TRANSPORTATION** DRAWN JDK REVISED CONTRACT NO. 70A29 PLOT DATE = 4/21/2022 SHEET NO. 8 OF 19 SHEETS CHECKED - LVM REVISED -



	$\frac{\pm 3''}{50^{\circ}} \frac{\text{at}}{\text{F}} = \frac{\pm 2' - 11''}{\text{A}}$	
Note A: 4-#5 d(E) bars at 11" cts. (i.f.) 4-#5 d1(E) bars at 11" cts. (i.f.) 4-#4 d2(E) bars at 11" cts. (o.f.)	CONCRETE DEDI ACEMENT AT WEST ADUTMENT	1'-7" 1'-0" } & varies
4-#4 d2(E) bars at 11" cts. (o.f.) (See Section thru Bridge Deck Parapet)	CONCRETE REPLACEMENT AT WEST ABUTMENT	j i i i i i i i i i i i i i i i i i i i

1'-7'' SECTION THRU NORTH SECTION THRU SOUTH APPROACH PARAPET APPROACH PARAPET

Bar	No.	Size	Length	Shape
a(E)	10	#5	23'-7"	
a1(E)	10	#5	25'-0"	
a2(E)	6	#6	6'-0"	
d(E)	8	#5	3'-0''	
d1(E)	8	#5	2'-7''	
d2(E)	2(E) 8 #4		3'-0''	
d3(E)	8	#4	3'-10"	
h(E)	4	#6	25'-2"	
h1(E)	4	#6	26'-8"	
Concrete Removal		Cu. Yd.	7.2	
Concrete Superstructure			Cu. Yd.	8.2
Reinforcement Bars, Epoxy Coated			Pound	960

SECTION

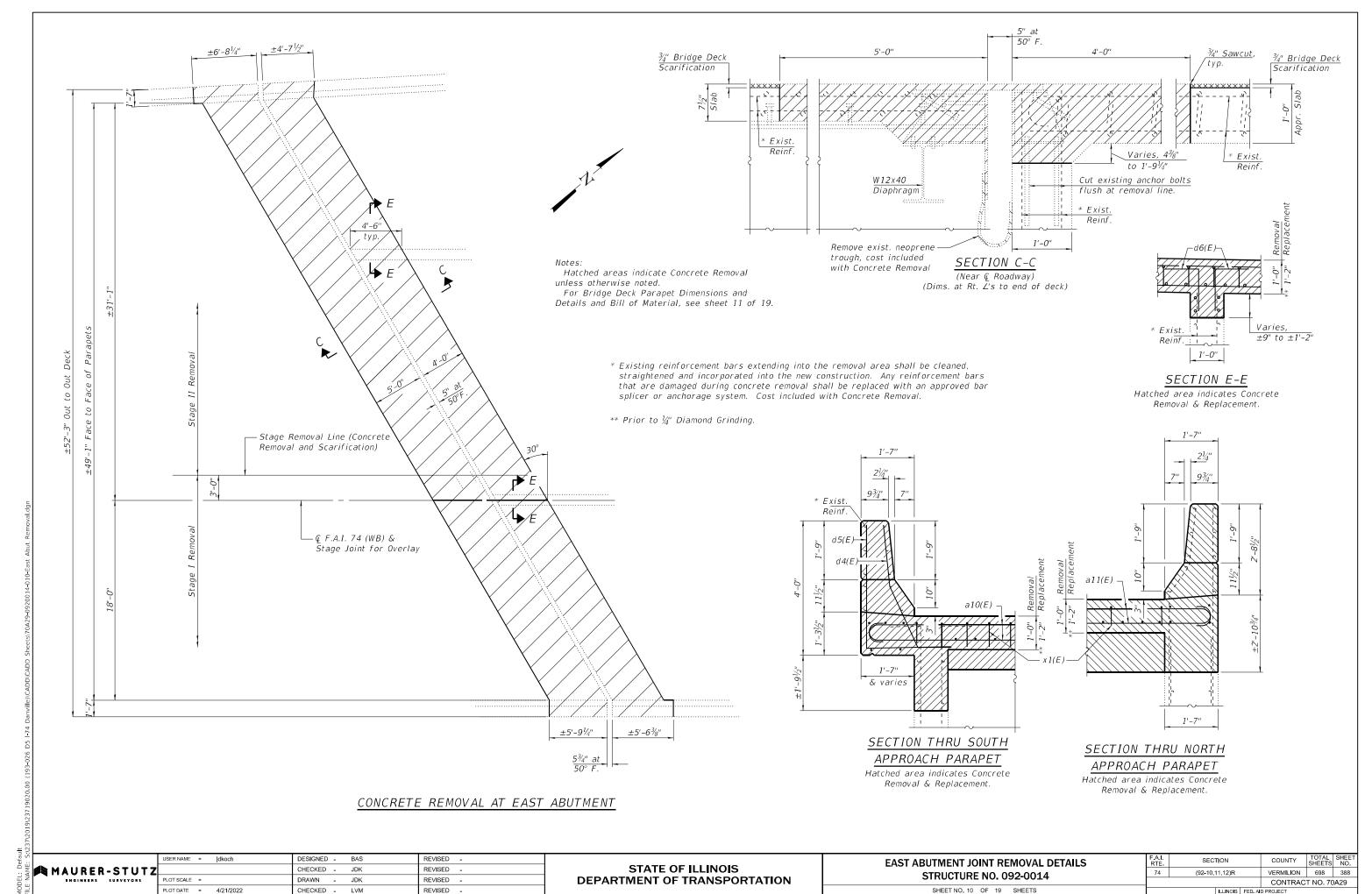
(92-10,11,12)R

COUNTY

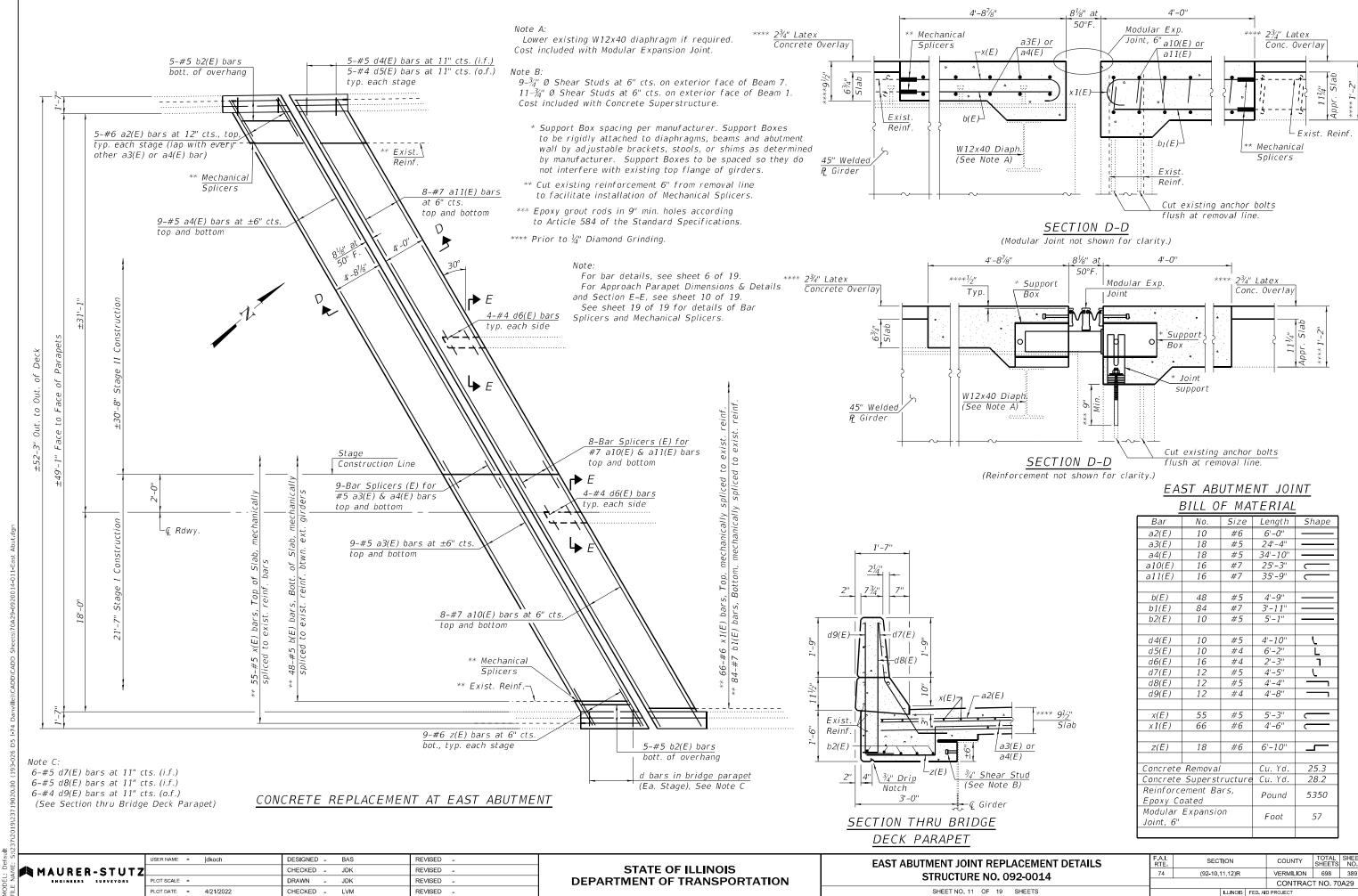
VERMILION 698 387

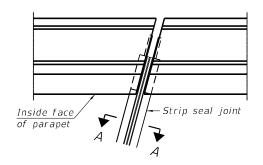
CONTRACT NO. 70A29

DESIGNED - BAS REVISED -JSER NAME = jdkoch WEST ABUTMENT JOINT REPLACEMENT DETAILS **STATE OF ILLINOIS** CHECKED - JDK REVISED -MAURER-STUTZ **STRUCTURE NO. 092-0014** DRAWN JDK REVISED **DEPARTMENT OF TRANSPORTATION** SHEET NO. 9 OF 19 SHEETS PLOT DATE = 4/21/2022 CHECKED - LVM REVISED -

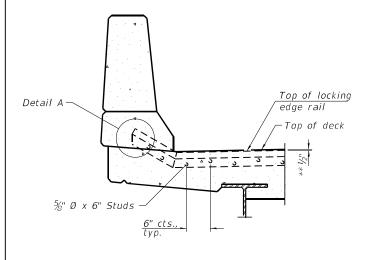


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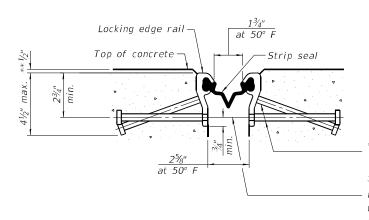
$FOR SKEWS \le 30^{\circ}$ PLAN AT PARAPET



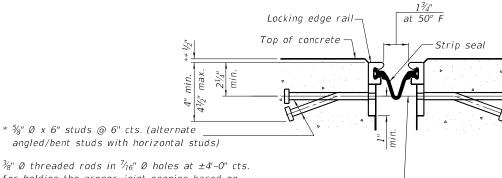
SECTION AT PARAPET

DETAIL A

** Prior to Diamond Grinding



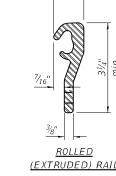
SHOWING ROLLED RAIL JOINT



 $\frac{3}{8}$ " Ø threaded rods in $\frac{7}{16}$ " Ø holes at ± 4 '-0" cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned or sawed off flush with the plates after concrete is set.

SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



(EXTRUDED) RAIL

WELDED RAIL

LOCKING EDGE RAILS

*** Back gouge not required if complete joint penetration is verified by mock-up.

Notes:

The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

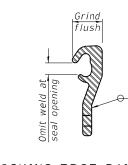
The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	50

DESIGNED - BAS REVISED -JSER NAME = jdkoch CHECKED - JDK REVISED -MAURER-STUTZ JDK REVISED PLOT DATE = 4/21/2022 CHECKED - LVM REVISED -

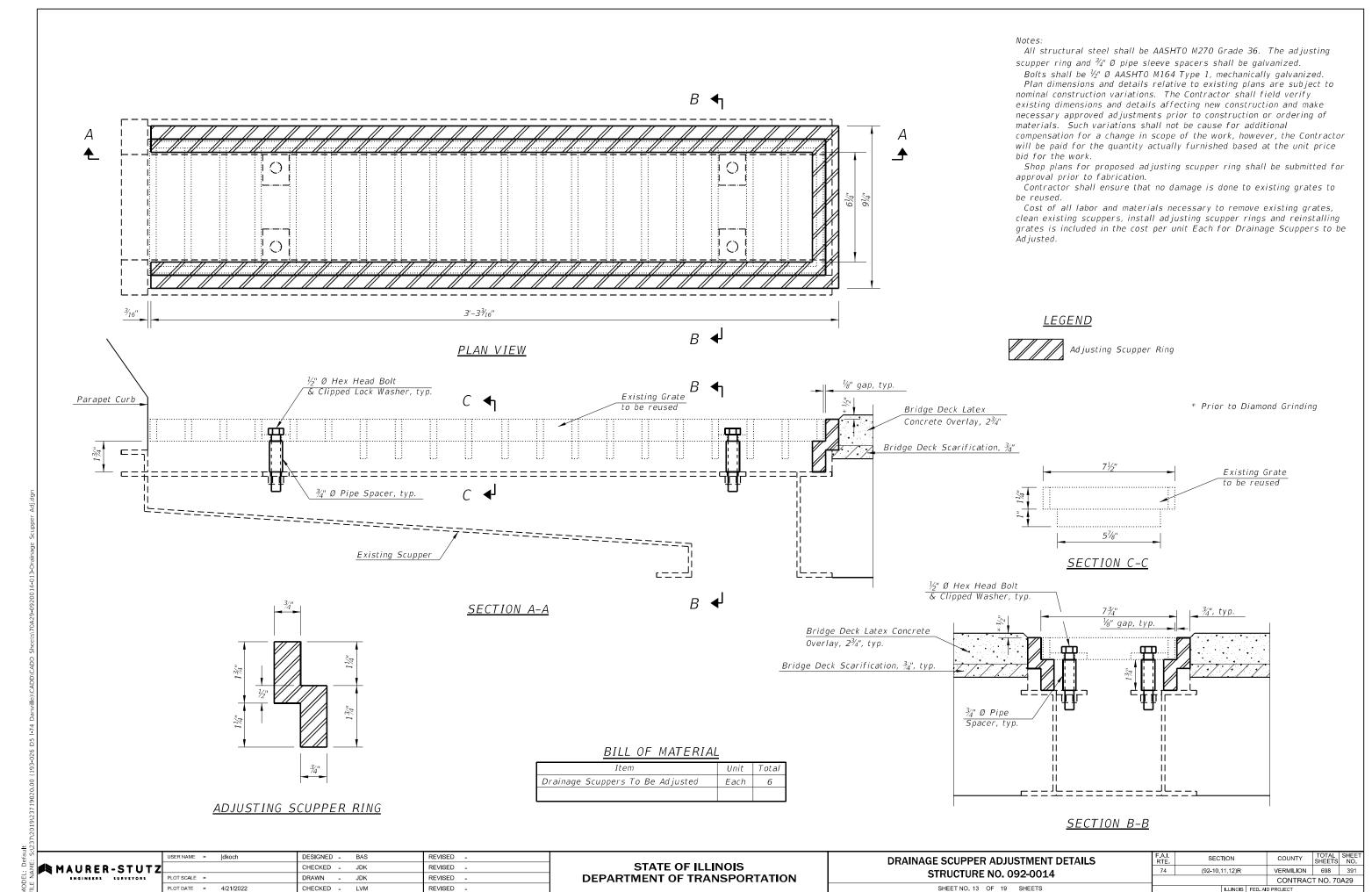
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SHOWING WELDED RAIL JOINT

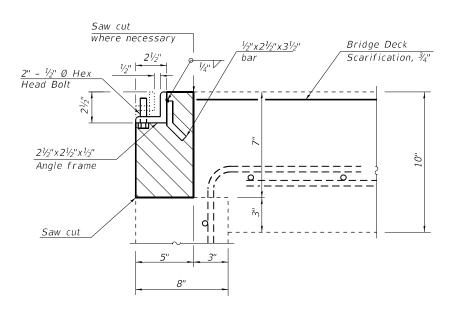
PREFORMED JOINT STRIP SEAL **STRUCTURE NO. 092-0014** SHEET NO. 12 OF 19 SHEETS

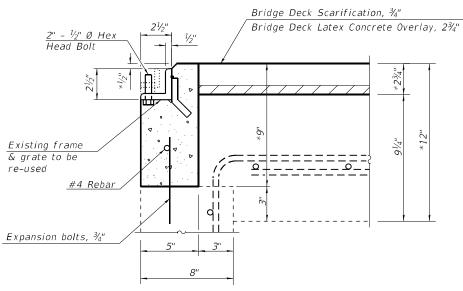
SECTION COUNTY (92-10,11,12)R VERMILION 698 390 CONTRACT NO. 70A29

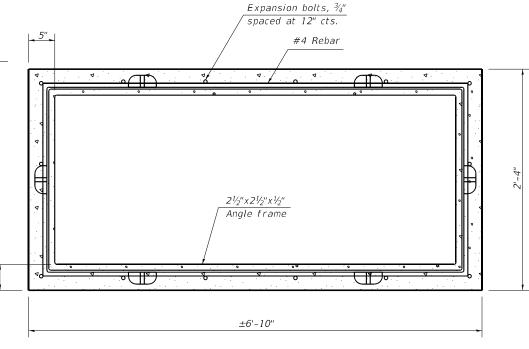
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PLAN - PROPOSED (GRATING OMITTED FOR CLARITY)

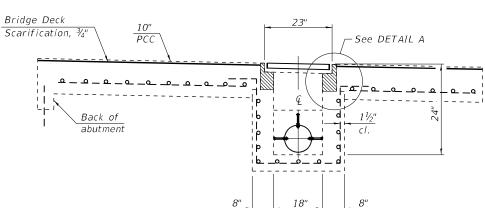
DETAIL "A" - EXISTING

Concrete Removal

DETAIL "A" - PROPOSED

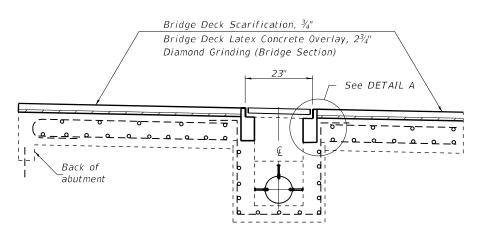
Class S.I. Concrete

* Prior to Diamond Grinding



EXISTING TYPE D INLET BOX PROFILE BRIDGE APPROACH SHOULDER PAVEMENT AND DRAIN

(Existing Approach Shoulder Pavement Std. 2324-6)



PROPOSED PROFILE BRIDGE APPROACH SHOULDER PAVEMENT AND DRAIN

Expansion bolts shall extend a minimum of 4" into the proposed concrete.

All work and materials shall be in accordance with applicable portions of Section 602.

This work will be paid for at the Contract unit price each for "Inlet Boxes to be Adjusted (Special)" which shall include all labor, equipment and material necessary to complete the work.

ESTIMATED QUANTITIES

(for information only)

ITEM	UNIT	TOTAL
Concrete Removal	Cu Yd	0.13
Reinforcement Bars (Epoxy)	Pound	11.4
Class S.I. Concrete	Cu Yd	0.16
Expansion Bolts, ¾"	Each	18

BILL OF MATERIALS

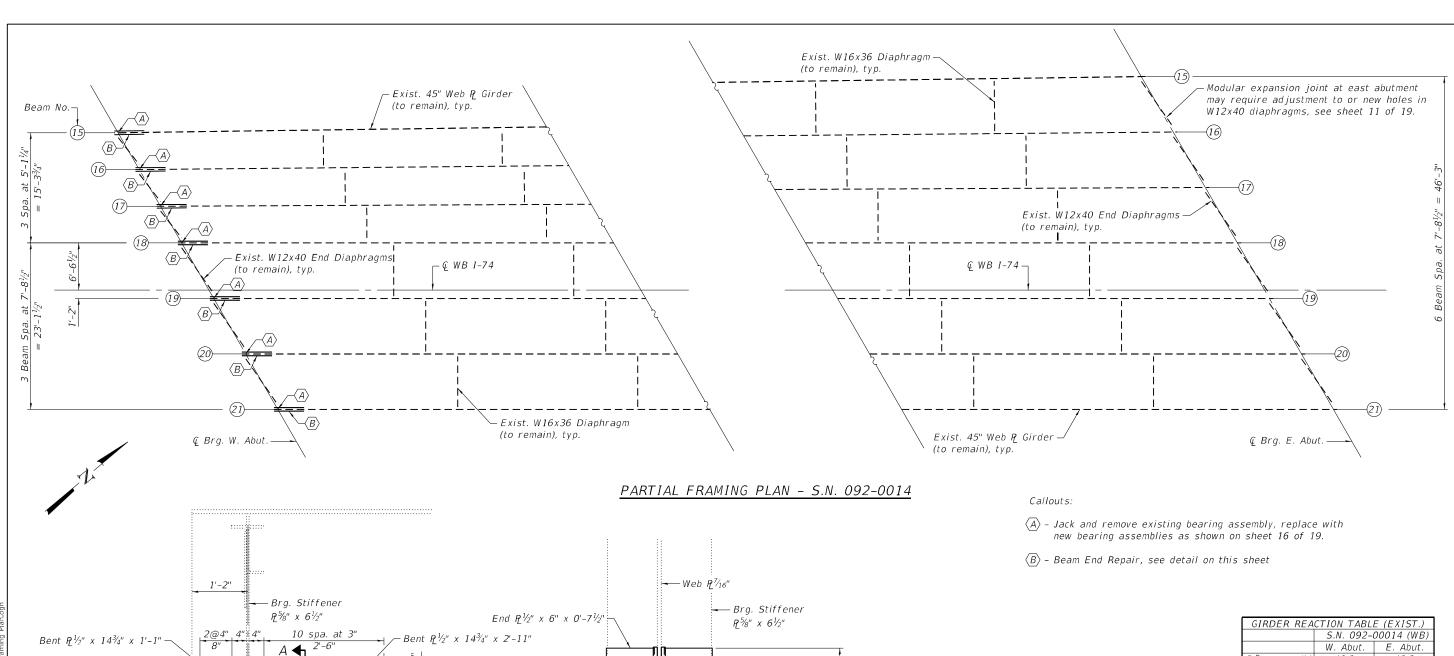
ITEM	UNIT	TOTAL
Inlet Boxes to be Adjusted (Special)	Each	1

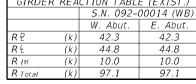
JSER NAME = jdkoch DESIGNED - BAS REVISED -CHECKED - JDK MAURER-STUTZ REVISED -DRAWN REVISED PLOT DATE = 4/21/2022 CHECKED - LVM REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **INLET BOX TO BE ADJUSTED STRUCTURE NO. 092-0014** SHEET NO. 14 OF 19 SHEETS

SECTION COUNTY (92-10,11,12)R VERMILION 698 392 CONTRACT NO. 70A29

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Notes:

Cost of furnishing and erecting steel for beam end repairs shall be included with Structural Steel Repair. See Special Provision.

All structural steel for the beam end repairs shall conform to the

requirements of AASHTO M 270, Grade 50.

Structural steel plates for the beam end repairs shall be hot dip

galvanized. See Special Provisions for Hot Dip Galvanizing for Structural Steel.

Removal of existing bearing assemblies shall be paid for as Jack and Remove Existing Bearings.

Minimum jack capacity shall be 55 tons.

The existing structural steel beams and diaphragms at each abutment shall be cleaned and painted prior to completing beam repairs or installing new modular expansion joint and bearing assemblies. See general notes on sheet 2 of 19.

Abandoned holes in existing diaphragm at east abutment after removal of existing finger plate expansion joint shall be filled with galvanized H.S. bolts. Cost included with Concrete Removal. See sheet 16 of 19 for Bill of Material.

BEAM END REPAIR - ELEVATION Q Brg. Q Brg. 2@4" 4" 4" 8 spa. at $3^{1}\!\!/_{4}$ " $2^{1}\!\!-_{2}$ " Bent $R^{1}\!\!/_{2}$ " \times $14^{3}\!\!/_{4}$ " \times $1^{1}\!\!-_{1}$ " Sole R(below) Flange $R^{1}\!\!/_{2}$ " \times $14^{3}\!\!/_{4}$ " \times $2^{1}\!\!-_{1}$ 1" Flange $R^{1}\!\!/_{2}$ " \times $14^{3}\!\!/_{4}$ " \times $2^{1}\!\!-_{1}$ 1" Flange $R^{1}\!\!/_{2}$ " \times $14^{3}\!\!/_{4}$ " \times $2^{1}\!\!-_{1}$ 1"

BEAM END REPAIR - PLAN

BOLT HOLE LEGEND

SECTION A-A

• - Drill $1^{1}\!\!/_{\!4}"$ Ø hole in Bent P using template to match ctr. of hole in bott. flange, 1" Ø threaded stud from Bearing Assembly

- Bent $P_2^{1/2}$ " x $14^{3/4}$ "

⊐- Flange №½" x 14"

└─ Bend radius to miss

½16" fillet weld

• - $^{13}\!\!/_{16}$ " Ø hole in Bent P or End P and field drill $^{13}\!\!/_{16}$ " Ø hole in beam web, flange, or stiffener P to match, $^3\!\!/_4$ " Ø H.S. bolt

ń		USER NAME = baswanson	DESIGNED - BAS	REVISED -	
	MAURER-STUTZ		CHECKED - JDK	REVISED -	i
2	ENGINEERS SURVEYORS	PLOT SCALE =	DRAWN - JDK	REVISED -	
Ë		PLOT DATE = 5/11/2022	CHECKED - LVM	REVISED -	i

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL FRAMING PLAN	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 092-0014	74	(92-10,11,12)R	VERMILION	698	393
311(00101)L 1(0: 032-0014			CONTRAC	T NO. 70)A29
CHEET NO. 15 OF 10 CHEETS		LUBIOTO EED	ALD DOG LEGT		

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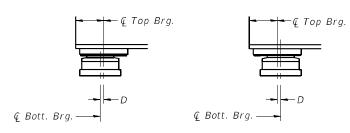
HLMR EXPANSION BEARING ASSEMBLY

REVISED

REVISED -

REVISED

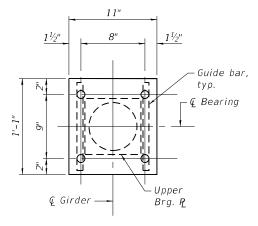
REVISED .



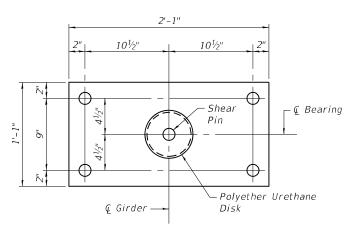
BELOW 50°F. $D=\frac{1}{8}$ " per each 100' of expansion for every 15° temp. change from the normal temp. of 50°F.

EXPANSION BEARING ORIENTATION

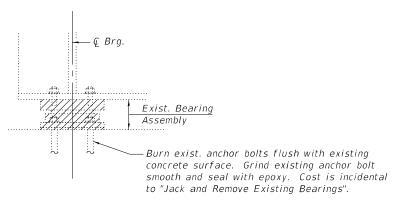
The above diagrams are for informational purposes only to show the amount of expected offset "D" for the current temperature in the field.



SOLE R AND UPPER BEARING R



BOTTOM BEARING P AND DISK PLAN



EXISTING BEARING
REMOVAL DETAIL

BILL OF MATERIAL

Item	Unit	Total
Jack and Remove Existing Bearings	Each	7
High Load Multi-Rotational Bearings, Guided Expansion, 100k	Each	7
Anchor Bolts	Each	28
Structural Steel Repair	Pound	2200

Notes

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Prior to ordering any material, the Contractor shall verify in the field all bearing height and shim thickness dimensions.

Provide two $\frac{1}{6}$ " adjusting shims for each bearing in addition to all other plates of the Bearing Assembly and placed as shown on the bearing details.

The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M 270, Grade 50.

All new anchor bolts, nuts, and washers shall be hot dip galvanized. Steel plates of the Bearing Assembly shall be hot dip galvanized except where prohibited by the manufacturer. See Special Provisions for Hot Dip Galvanizing for Structural Steel.

Bearings shall be designed for a vertical load of 100 kips, a horizontal load of 5.2 kips, and a total required movement of 3". Reactions are given for service loads. See sheet 15 of 19 for bearing reactions.

		USER NAME =	jdkoch	DESIGNED	-	BAS
	MAURER-STUTZ			CHECKED	-	JDK
	ENGINEERS SURVEYORS	PLOT SCALE =		DRAWN	-	JDK
1		PLOT DATE =	4/21/2022	CHECKED	-	LVM

<u>LEGEND</u>

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
Structural Repair of Concrete (Depth Greater Than 5 inches)

Notes:

Repair areas shown are estimated from soundings on October 13, 2020.

Actual size and locations of completed repairs shall be shown on this sheet and documented in the provide tables under "As Built".

STRUCTURAL REPAIR OF CONCRETE TABLE

		Plan	Qty. SRC >5"	As I	Built
Label	Size	SRC ≤5"	SRC >5"	SRC ≤ 5"	SRC >5"
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
Α	1' x 1' 1' x 1' 1' x 1' 2' x 1' 1' x 1' 1' x 1' 2' x 2'	1			
А В С	1' x 1'	1			
	1' x 1'	1			
D	2' x 1'	2			
D E F	1' x 1'	1			
F	1' x 1'	1			
G	2' x 2'	4			
	Totals	11			

BILL OF MATERIAL

Item	Unit	Quantity
Structural Repair of Concrete (Depth ≤ 5 in.)	Sq. Ft.	11

USER NAME = jdkoch DESIGNED - BAS REVISED -SECTION COUNTY **WEST ABUTMENT REPAIRS** STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION MAURER-STUTZ
ENGINEERS SURVEYORS CHECKED - JDK REVISED -(92-10,11,12)R VERMILION 698 395 **STRUCTURE NO. 092-0014** DRAWN - JDK REVISED -CONTRACT NO. 70A29 PLOT DATE = 4/21/2022 CHECKED - LVM REVISED -SHEET NO. 17 OF 19 SHEETS

<u>ELEVATION - EAST ABUTMENT</u> (Looking East)

<u>LEGEND</u>

	Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
	Structural Repair of Concrete (Depth Greater Than 5 inches)

Notes

Repair areas shown are estimated from soundings on October 13, 2020.

Actual size and locations of completed repairs shall be shown on this sheet and documented in the provide tables under "As Built".

STRUCTURAL REPAIR OF CONCRETE TABLE

		Plan Qty. SRC ≤5" SRC >5"		As Built	
Label	Size	SRC ≤5"	SRC >5"	SRC ≤ 5"	SRC >5"
		Sq. Ft.	Sq. Ft.	Sq. Ft.	Sq. Ft.
Α	1' x 1'	1 1			
A B	1' x 1' 1' x 1'	1			
	-				
	Totals	2			

BILL OF MATERIAL

Item	Unit	Quantity
Structural Repair of Concrete (Depth ≤ 5 in.)	Sq. Ft.	2

USER NAME = jdkoch DESIGNED - BAS REVISED -SECTION COUNTY **EAST ABUTMENT REPAIRS** STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION MAURER-STUTZ
ENGINEERS SURVEYORS CHECKED - JDK REVISED -(92-10,11,12)R VERMILION 698 396 **STRUCTURE NO. 092-0014** DRAWN - JDK REVISED -CONTRACT NO. 70A29 PLOT DATE = 4/21/2022 CHECKED - LVM REVISED -SHEET NO. 18 OF 19 SHEETS

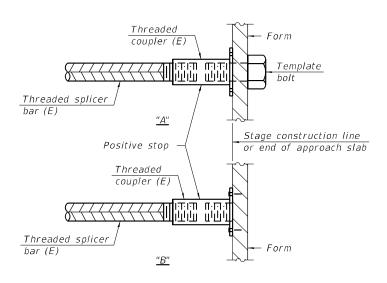
STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

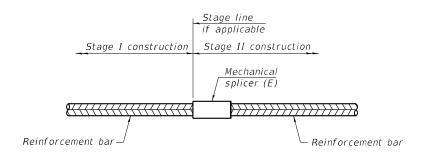
Location	Bar	No. assemblies	Minimum
LUCALIUII	size	required	lap length
West Abutment	#5	10	3'-6''
West Abutment	#6	4	3'-7"
East Abutment	#5	18	3'-6''
East Abutment	#7	16	4'-8''



INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar	No. assemblies
Location	size	required
East Abutment	#5	103
East Abutment	#6	66
East Abutment	#7	84

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

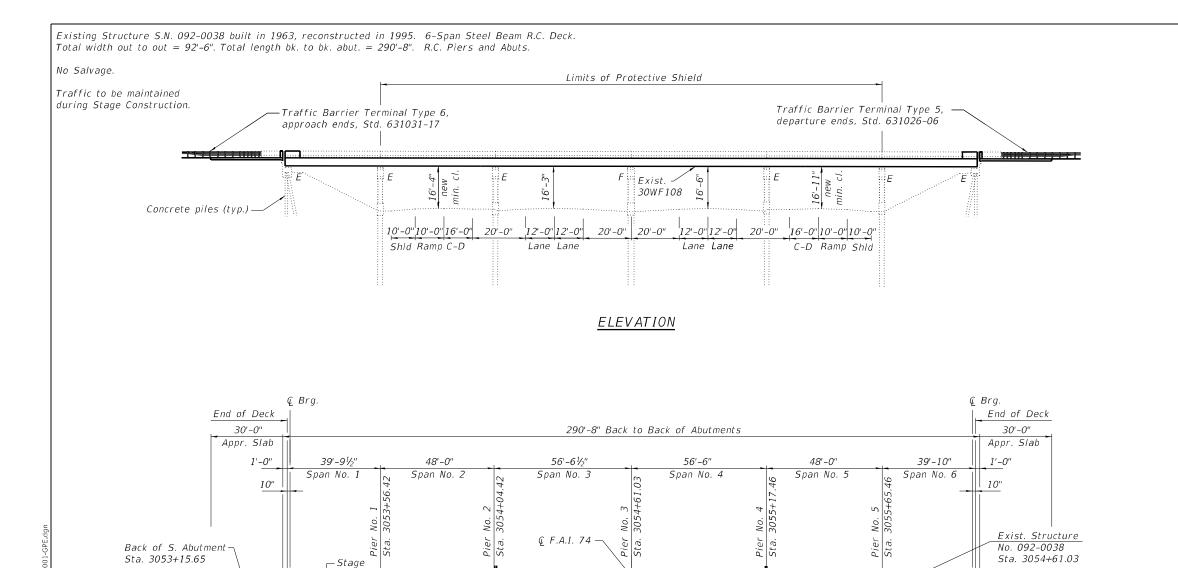
See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

1-1-2020

S		USER NAME = jdkoch	DESIGNED - BAS	REVISED -
ME	MAURER-STUTZ		CHECKED - JDK	REVISED -
		PLOT SCALE =	DRAWN - JDK	REVISED -
٣		PLOT DATE = 4/21/2022	CHECKED - LVM	REVISED -

SHEET NO. 19 OF 19 SHEETS



INDEX OF SHEETS

- General Plan & Elevation
- General Data
- Existing & Proposed Typical Cross Sections
- Stage Construction Details Bridge
- Temporary Concrete Barrier for Construction
- Deck Plan Overlay
- Abutment Joint Replacement Details 9-10.
- Preformed Joint Strip Seal Details 11. Approach Slab Repair Details
- 13. Framing Plan
- 14. Diaphragm Replacement & Beam End Repair
- 15. Parapet Repairs
- Bearing Details Abutments
- Abutment and Slope Wall Repair Plan
- 18-22. Pier Repair Plans
- Pier Repair Details 23.
- Bar Splicer Assembly and Mechanical Splicer Details
- Drainage Scupper Adjutment Ring Detail
- Inlet Box to be Adjusted

DESIGN STRESSES (EXIST.)

ORIGINAL STRUCTURES

f'c = 1,400 psi (allow.)fs = 20,000 psi (Reinf.)fs = 20,000 psi (Struct.)

1995 RECONSTRUCTION

 $f'c = 3,500 \ psi$

 $fy = 60,000 \ psi \ (Reinf.)$

fy = 36,000 psi (Struct. M270, GR. 36)

DESIGN STRESSES (PROP.)

FIELD UNITS

f'c = 4,000 psi

fy = 60,000 psi (Reinforcement)

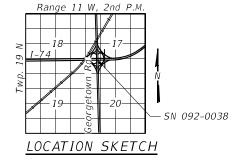
fy = 36,000 psi (M270 Grade 36)

DESIGN SPECIFICATIONS

2002 AASHTO Standard Specifications, for Highway Bridge Design, 17th Edition

LOADING

HS20-44



GENERAL PLAN AND ELEVATION

U.S. RTE. 150/IL. RTE. 1 OVER F.A.I. RT. 74

F.A.I. 74 SEC. (92-10,11,12)R

STA. 3054+61.03 VERMILION COUNTY

SN. 092-0038

SECTION COUNTY

PLAN (Stations shown per original bridge plans)

22'-0"

Median

22'-0"

24'-0"

Median Roadway

20'-0"

20'-0"

26'-0"

C-D

Rdwy.

Shld.

24'-0"

Roadway

Concrete Barrier

DESIGNED - SMJ REVISED Prairie Engineers, P.C. CHECKED - BAS REVISED Prairie OT SCALE = 0.166666 ' / in. DRAWN REVISED Engineers ,, PLOT DATE = 4/25/2022 CHECKED - SMJ REVISED -

-Stage Const

Line

Const.

Line

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

F.A. Rte. 332

F.A. Rte. 332

26'-0"

C-D

Rdwy.

Concrete

Barrier

I N.B.

SHEET 1 OF 26 SHEETS

-Approach inlets

to be adjusted

-Back of N. Abutment Sta. 3056+06.31

-Ç F.A. Rte. 332

П

4/25/2022 9:11:46 AM

Parapets

89'-4" ace of

ace to Fa

74 (92-10,11,12)R VERMILION 698 398 CONTRACT NO. 70A29 No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer.

Any cracks that cannot be removed by grinding V_4 inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

If the Contractor elects to use cantilever forming brackets on the exterior beams or girders, the brackets shall be placed at the same locations as required for the hardwood blocks in Article 503.06(b) of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and first interior beam at each of these additional bracket locations.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Concrete Sealer shall be applied to the repaired areas of the piers.

Two $\frac{1}{8}$ in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

Areas of repairs shown are estimated. The engineer shall show actual locations of the repairs on as-built plans.

Protective coat shall be applied to new Concrete Superstructure surfaces only. Applicable areas include exposed new concrete deck $\pm 6'$ -11" width along joints, new overlay, and interior and top surfaces of repaired parapet only.

All new structural steel shall be hot-dipped galvanized, except as noted See Special Provisions for "Hot Dip Galvanizing For Structural Steel", see also notes on sheet 16.

Synthetic Fibers shall be included in the bridge deck concrete overlay specified. See special provisions.

Up to $\frac{1}{4}$ inch may be ground off the bridge deck and the bridge approach slabs.

Joint openings shall be adjusted according to Article 503.10(c) of the Standard Specifications when the deck is poured at an ambient temperature other than

Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

Existing structure plans are available from the District upon request.

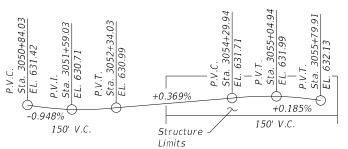
SCOPE OF WORK:

- Complete bridge scarification 3/4".
- Complete deck patching, removal of deck ends and hatch blocks.
- Complete structural repair of concrete on substructure.
- Remove and replace approach slab Type B curb.
- Perform epoxy injection on longitudinal cracks in approach slabs.
- Complete steel beam repairs and end diaphragm replacements.
- Replace bearings at abutments.
- Install shear studs at beam ends.
- Cast deck ends and hatch blocks.
- 10. Place preformed joint strip seal.
- 11. Adjust inlets in approaches and scuppers in deck.
- 12. Place bridge deck latex concrete overlay 2 3/4".
- 13. Fill gaps between deck and beams w/ epoxy injection.
- 14. Complete slope wall crack sealing.
- 15. Complete diamond grinding and bridge deck grooving.

TOTAL BILL OF MATERIAL

Concrete Removal Cu. Yd. 49.0 - 49		TOTAL BILL OF	,,,,, <u>L</u> ,	127 (=		
Concrete Superstructure		ITEM	UNIT	SUPER	SUB	TOTAL
Concrete Superstructure Cu. Yd. 57.0 - 57.7 Protective Shield Sq. Yd. 2,148 - 2,1 Concrete Structures Cu. Yd. 24.0 - 24 Bridge Deck Grooving (Longitudinal) Sq. Yd. 2,805 - 2,8 Protective Coat Sq. Yd. 3,415 - 3,4 Furnishing and Erecting Structural Steel Pound 12,840 - 12,8 Furnishing and Erecting Structural Steel Pound 12,840 - 12,8 Stud Shear Connectors Each 448 - 44 Reinforcement Bars, Epoxy Coated Pound 8,450 - 8,4 Bar Splicers Each 88 - 88 Preformed Joint Strip Seal Foot 183.0 - 183 Elastomeric Bearing Assembly Type II Each 32 - 3; Concrete Sealer Sq. Ft. - 1,120.0 1,12 Epoxy Crack Injection Foot 120 -		Concrete Removal	Cu. Yd.	49.0	-	49.0
Concrete Structures			Cu. Yd.	57.0	-	57.0
## Bridge Deck Grooving (Longitudinal) Protective Coat		Protective Shield	Sq. Yd.	2,148	_	2,148
Protective Coat		Concrete Structures	Cu. Yd.	24.0	-	24.0
Furnishing and Erecting Structural Steel Pound 12,840 - 12,8 Stud Shear Connectors Each 448 - 44 Reinforcement Bars, Epoxy Coated Pound 8,450 - 8,4 Bar Splicers Each 88 - 88 Preformed Joint Strip Seal Foot 183.0 - 183 Elastomeric Bearing Assembly Type II Each 32 - 32 Concrete Sealer Sq. Ft. - 1,120.0 1,12 Epoxy Crack Injection Foot 120 - 12 Jack and Remove Existing Bearings Each 32 - 32 Structural Steel Removal Pound 8,030 - 8,0 Structural Steel Repair Pound 5,940 - 5,9 Bridge Deck Latex Concrete Overlay 2¾" Sq. Yd. 3,258 - 3,2 Structural Repair of Concrete Depth <=5"	†	Bridge Deck Grooving (Longitudinal)	'	2,805	1	2,805
Stud Shear Connectors Each 448 - 44 Reinforcement Bars, Epoxy Coated Pound 8,450 - 8,4 Bar Splicers Each 88 - 88 Preformed Joint Strip Seal Foot 183.0 - 183 Elastomeric Bearing Assembly Type II Each 32 - 3; Concrete Sealer Sq. Ft. - 1,120.0 1,12 Epoxy Crack Injection Foot 120 - 12 Tepoxy Crack Injection Foot 120 - 12 Sq. Ft. - 1,120.0 1,12 Epoxy Crack Injection Foot 120 - 120 Jack and Remove Existing Bearings Each 32 - 3; Structural Steel Removal Pound 8,030 - 8,0 Structural Steel Repair Pound 5,940 - 5,9 Bridge Deck Latex Concrete Overlay 2¾" Sq. Yd. 3,258 - 3,2 Structural Rep		Protective Coat		3,415	-	3,415
Reinforcement Bars, Epoxy Coated Pound 8,450 - 8,4 Bar Splicers Each 88 - 88 Preformed Joint Strip Seal Foot 183.0 - 183 Elastomeric Bearing Assembly Type II Each 32 - 32 Concrete Sealer Sq. Ft. - 1,120.0 1,12 Epoxy Crack Injection Foot 120 - 12 Epoxy Crack Injection Foot 32 - 32 Structural Steel Removal Pound 8,030 - 8,0 Structural Steel Repair Pound 5,940 - 5,9 Bridge Deck Latex Concrete Overlay 2¾" Sq. Yd. 3,258 - 3,2 Bridge Deck Scarification ¾" Sq. Yd. 3,258 - 3,2 Structural Repair of Concrete Depth <=5" Sq. Ft. - 135.0 135 Deck Slab Repair (Full Depth, Type 1) Sq. Yd. 17 - 17 Drainage Scuppers to be Adjusted Each 6 - 6 Inlet Boxes to be Adjusted (Special) Each 4 - 4 Epoxy Injection Foot 108 - 10 Temporary Support System L Sum 1 - 1 Diamond Grinding (Bridge Section) Sq. Yd. 2,958 - 2,9		Furnishing and Erecting Structural Steel		12,840	_	12,840
Bar Splicers		Stud Shear Connectors		448	_	448
Preformed Joint Strip Seal Foot 183.0 - 183.5 Elastomeric Bearing Assembly Type II Each 32 - 3.2		Reinforcement Bars, Epoxy Coated		8,450	-	8,450
Elastomeric Bearing Assembly Type II Each Sq. Ft 1,120.0 1,12 Epoxy Crack Injection Jack and Remove Existing Bearings Structural Steel Removal Structural Steel Repair Bridge Deck Latex Concrete Overlay 2¾" Bridge Deck Scarification ¾" Sq. Yd. 3,258 - 3,2 Structural Repair of Concrete Depth <=5" Sq. Yd. 3,258 - 3,2 Structural Repair of Concrete Depth <=5" Deck Slab Repair (Full Depth, Type 1) Drainage Scuppers to be Adjusted Inlet Boxes to be Adjusted (Special) Temporary Support System L Sum 1 - 1 Diamond Grinding (Bridge Section)					-	88
Concrete Sealer Sq. Ft. - 1,120.0 1,12 Epoxy Crack Injection Foot 120 - 12 † Jack and Remove Existing Bearings Each 32 - 32 † Structural Steel Removal Pound 8,030 - 8,03 † Structural Steel Repair Pound 5,940 - 5,9 † Bridge Deck Latex Concrete Overlay 2¾" Sq. Yd. 3,258 - 3,2 † Bridge Deck Scarification ¾" Sq. Yd. 3,258 - 3,2 † Structural Repair of Concrete Depth <=5"		Preformed Joint Strip Seal		183.0	-	183.0
Epoxy Crack Injection Foot 120 - 12				32	-	32
† Jack and Remove Existing Bearings					1,120.0	1,120.0
† Structural Steel Removal Pound 8,030 - 8,0 † Structural Steel Repair Pound 5,940 - 5,9 † Bridge Deck Latex Concrete Overlay 2¾" Sq. Yd. 3,258 - 3,2 † Bridge Deck Scarification ¾" Sq. Yd. 3,258 - 3,2 † Structural Repair of Concrete Depth <=5" Sq. Ft 135.0 135 † Deck Slab Repair (Full Depth, Type 1) Sq. Yd. 17 - 17 † Drainage Scuppers to be Adjusted Each 6 - 6 † Inlet Boxes to be Adjusted (Special) Each 4 - 4 † Epoxy Injection Foot 108 - 10 † Temporary Support System L Sum 1 - 1 † Diamond Grinding (Bridge Section) Sq. Yd. 2,958 - 2,9					-	120
† Structural Steel Repair Pound 5,940 - 5,9 † Bridge Deck Latex Concrete Overlay 2¾" Sq. Yd. 3,258 - 3,2 † Bridge Deck Scarification ¾" Sq. Yd. 3,258 - 3,2 † Structural Repair of Concrete Depth <=5" Sq. Ft 135.0 135 † Deck Slab Repair (Full Depth, Type 1) Sq. Yd. 17 - 17 † Drainage Scuppers to be Adjusted Each 6 - 6 † Inlet Boxes to be Adjusted (Special) Each 4 - 4 † Epoxy Injection Foot 108 - 10 † Temporary Support System L Sum 1 - 1 † Diamond Grinding (Bridge Section) Sq. Yd. 2,958 - 2,9	†				-	32
† Bridge Deck Latex Concrete Overlay 2¾" Sq. Yd. 3,258 - 3,2 † Bridge Deck Scarification ¾" Sq. Yd. 3,258 - 3,2 † Structural Repair of Concrete Depth <=5"	†				-	8,030
† Bridge Deck Scarification ¾" Sq. Yd. 3,258 - 3,2 † Structural Repair of Concrete Depth <=5"	†	1			-	5,940
† Structural Repair of Concrete Depth <=5"	†				-	3,258
† Deck Slab Repair (Full Depth, Type 1) Sq. Yd. 17 - 17 † Drainage Scuppers to be Adjusted Each 6 - 6 † Inlet Boxes to be Adjusted (Special) Each 4 - 4 † Epoxy Injection Foot 108 - 10 † Temporary Support System L Sum 1 - 1 † Diamond Grinding (Bridge Section) Sq. Yd. 2,958 - 2,9	†	Bridge Deck Scarification ¾"				3,258
† Drainage Scuppers to be Adjusted Each 6 - 6 † Inlet Boxes to be Adjusted (Special) Each 4 - 4 † Epoxy Injection Foot 108 - 10 † Temporary Support System L Sum 1 - 1 † Diamond Grinding (Bridge Section) Sq. Yd. 2,958 - 2,9	†				135.0	135.0
† Inlet Boxes to be Adjusted (Special) Each 4 - 4 † Epoxy Injection Foot 108 - 10 † Temporary Support System L Sum 1 - 1 † Diamond Grinding (Bridge Section) Sq. Yd. 2,958 - 2,9					-	17
†Epoxy InjectionFoot108-10†Temporary Support SystemL Sum1-1†Diamond Grinding (Bridge Section)Sq. Yd.2,958-2,9					-	6
† Temporary Support System L Sum 1 - 1 † Diamond Grinding (Bridge Section) Sq. Yd. 2,958 - 2,9	†				-	4
† Diamond Grinding (Bridge Section) Sq. Yd. 2,958 - 2,9	†				-	108
					-	1
†	†		-	2,958		2,958
	†	Slope Wall Crack Sealing	Foot	-	120.5	120.5
Anchor Bolts 1" Each 64 – 64		Anchor Bolts 1"	Each	64	-	64

† Special Provision



PROFILE GRADE F.A. RTE. 332

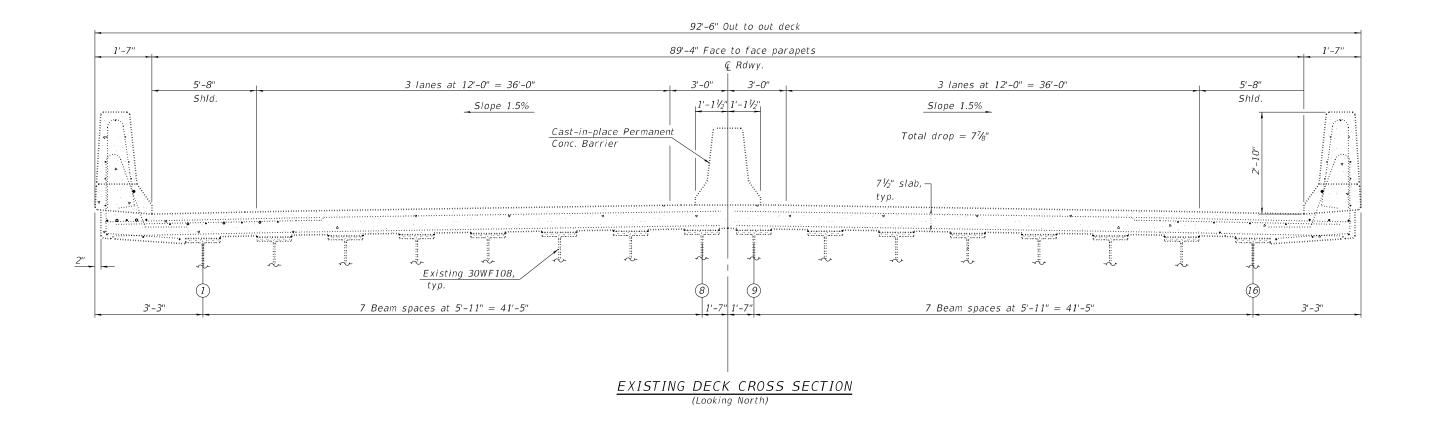
(From 1995 Reconstruction Plans)

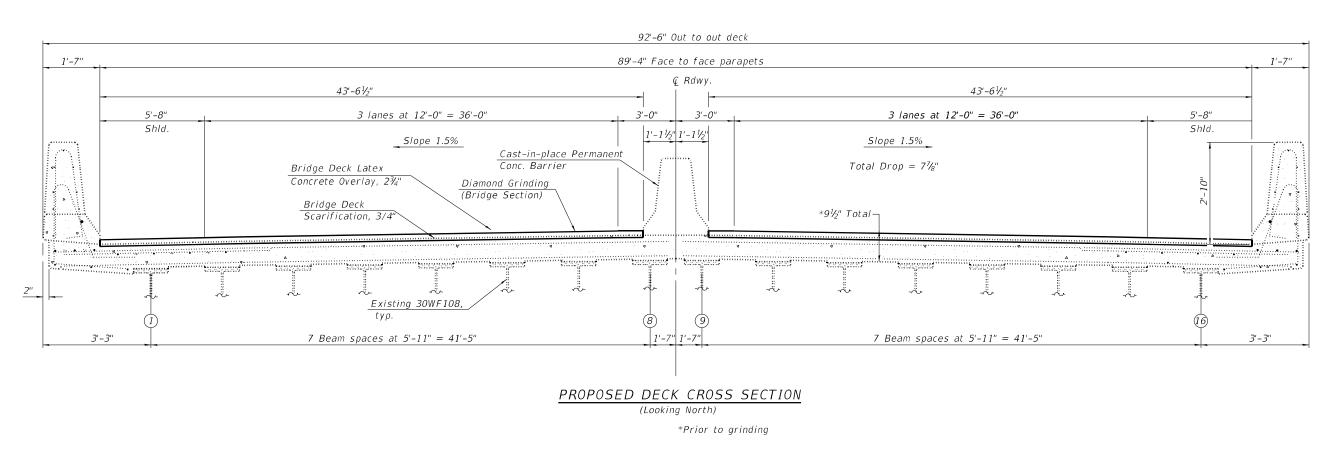
Wast Cida Frist Chrystura

West Side Exi	ist. Structure		
(East Bnd. Lanes)	(West Bnd. Lanes)	East Side Exi	st. Structure
Sta. 1928+86.04	Sta. 1928+86.04	(East Bnd. Lanes)	(West Bnd. Lanes)
El. 611.00 (S.E.O.P.)	EI. 611.02 (N.&S.E.O.I	P.) Sta. 1929+72.04	Sta. 1929+72.04
El. 611.03 (N.E.O.P.)		El. 610.78 (S.E.O.P.)	El. 610.69 (S.E.O.P.)
		El. 610.66 (N.E.O.P.)	El. 610.51 (N.E.O.P.)
	-0.4%± (Easi	t Bnd. Lanes) t Bnd. Lanes)	
	-0.38%± (Wesi	t Bnd (2ma)	
		- rid. Laries))

PROFILE GRADE F.A.I.-74

(From 1995 Reconstruction Plans)





4/25/2022 9:12:44 AM

Engineers ,,

Prairie Engineers, P.C.

22484 Grosenbach Road
Washington, IL 61571
(217) 605-0403

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING AND PROPOSED TYPICAL CROSS SECTIONS
S.N. 092-0038

SHEET 3 OF 26 SHEETS