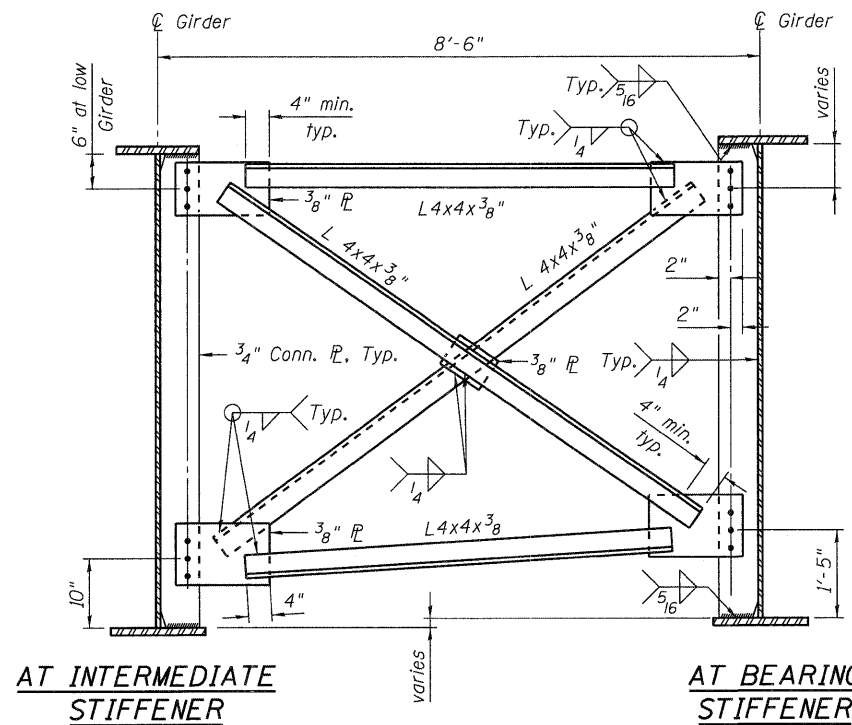


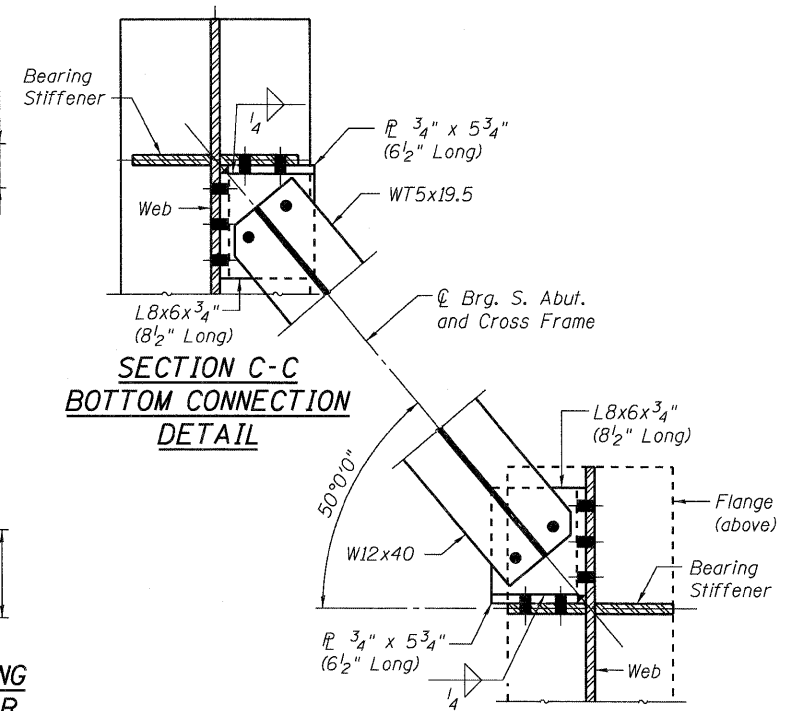
CROSS FRAME CF-5



AT INTERMEDIATE STIFFENER

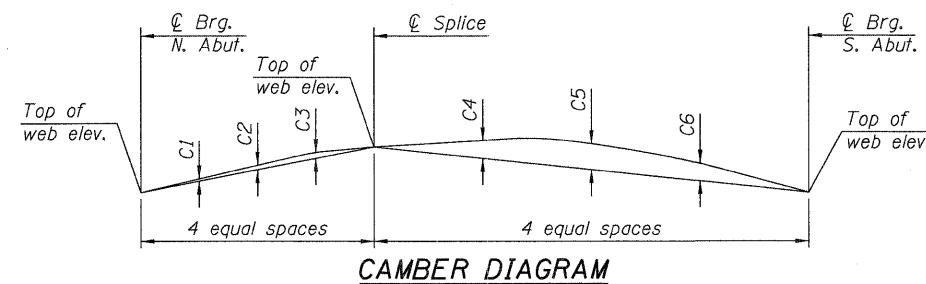
AT BEARING STIFFENER

CROSS FRAME CF-6



SECTION C-C
BOTTOM CONNECTION
DETAIL

SECTION D-D
TOP CONNECTION
DETAIL



CAMBER DIAGRAM

INTERIOR GIRDER MOMENT TABLE		
		Mid-Span
I_s	(in ⁴)	90611
$I_c(n)$	(in ⁴)	194759
$I_c(3n)$	(in ⁴)	141159
S_s	(in ³)	2298
$S_c(n)$	(in ³)	3140
$S_c(3n)$	(in ³)	2780
Z	(in ³)	-
DC1	(k/')	1.226
M _{DC1}	(k)	4249
DC2	(k/')	0.109
M _{DC2}	(k)	379
DW	(k/')	0.369
M _{DW}	(k)	1280
$M_L + IM$	(k)	3505
M_u (Strength I)	(k)	13838
$\phi_r M_n$	(k)	15659
f_s DC1	(ksi)	22.19
f_s DC2	(ksi)	1.64
f_s DW	(ksi)	5.53
f_s 1.3($\frac{1}{2} + IM$)	(ksi)	17.42
f_s (Service II)	(ksi)	46.78
f_s (Total)(Strength I)	(ksi)	-
V_r	(k)	79.9

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in.⁴ and in.³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in.⁴ and in.³).

Z: Plastic Section Modulus of the steel section in non-composite areas. Omit line in Moment Table if not used in design calculations (in.³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

$M_L + IM$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

$1.25(M_{DC1} + M_{DC2}) + 1.5M_{DW} + 1.75M_L + IM$

$\phi_r M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

$\phi_r M_{nc}$: Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).

f_s (Service II): Sum of stresses as computed from the moments below (ksi).

$M_{DC1} + M_{DC2} + M_{DW} + 1.3M_L + IM$

f_s (Total)(Strength I): Sum of stresses as computed from the moments below on non-compact section (ksi).

$1.25(M_{DC1} + M_{DC2}) + 1.5M_{DW} + 1.75M_L + IM$

V_r : Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

CAMBER DIMENSIONS TABLE

Girder No.	CAMBER DIMENSIONS (fractions of an inch)					
	C1	C2	C3	C4	C5	C6
G1	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G2	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G3	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G4	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G5	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G6	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G7	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G8	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G9	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G10	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G11	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G12	3/8"	3/4"	7/8"	6 1/8"	8"	5 7/8"
G13	5/8"	1"	1 1/8"	6"	7"	4 3/8"
G14	5/8"	1"	1 1/8"	5 7/8"	6 3/8"	4 1/8"
G15	1/2"	7/8"	1"	5 1/2"	6 3/4"	4 1/2"
G16	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G17	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G18	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G19	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G20	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G21	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G22	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G23	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G24	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"
G25	3/8"	3/4"	7/8"	5 1/2"	6 3/4"	4 1/2"
G26	5/8"	1"	1 1/8"	4 3/4"	5 5/8"	3 3/4"

TOP OF WEB ELEVATIONS
(FOR FABRICATION ONLY)

Girder No.	TOP OF WEB ELEVATIONS		
	N. Abut.	Splice	S. Abut.
G1	806.620	806.963	805.472
G2	806.763	807.094	805.740
G3	806.897	807.316	805.999
G4	807.022	807.415	806.250
G5	807.139	807.634	806.492
G6	807.246	807.700	806.725
G7	807.345	807.917	806.949
G8	807.436	807.950	807.165
G9	807.517	808.165	807.371
G10	807.590	808.165	807.569
G11	807.654	808.378	807.759
G12	807.709	808.345	807.742
G13	807.755	808.556	807.716
G14	807.774	808.593	807.701
G15	807.808	808.501	807.662
G16	807.829	808.456	807.615
G17	807.647	808.206	807.558
G18	807.457	808.160	807.493
G19	807.258	807.877	807.420
G20	807.050	807.830	807.337
G21	806.833	807.514	807.246
G22	806.608	807.464	807.145
G23	806.374	807.115	807.037
G24	806.131	807.063	806.919
G25	805.879	806.682	806.792
G26	805.618	806.627	806.657

Notes:
For Cross Frame locations see Framing Plan Sheet 37.
All cross frames or diaphragms between beams or girders shall be installed with erection pins and bolts in accordance with the erection plan approved by the Engineer. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
Load carrying components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.
All 3/4" ϕ bolts require 15/16" ϕ holes.
Two hardened washers required for each set of oversized holes.