

08-03-2018 LETTING ITEM 051

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

PLANS FOR PROPOSED FEDERAL AID HIGHWAY

STP-BR FUNDS
GRUNDY COUNTY
SECTION 16-00158-00-BR
PROJECT S3KW(328)
JOB NO. C-93-043-16
CH C41, BRACEVILLE ROAD

F. A. S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
272	16-00158-00-BR	GRUNDY	46	1
FED. ROAD DIST. NO. -		ILLINOIS	CONTRACT NO. 87642	

INDEX OF SHEETS

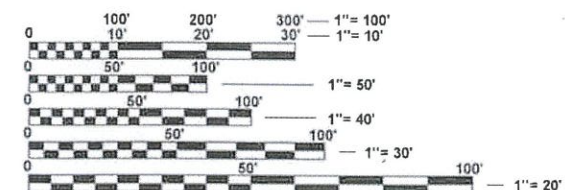
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- 515001-03 NAME PLATE FOR BRIDGES
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- 630001-12 STEEL PLATE BEAM GUARDRAIL
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- 630301-08 SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS
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- 782006 GUARDRAIL AND BARRIER WALL REFLECTOR MOUNTING DETAILS
- BLR 21-9 TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES FOR CONSTRUCTION ON RURAL LOCAL HIGHWAYS
- BLR 22-7 TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES FOR RURAL LOCAL HIGHWAYS (TWO-LANE TWO-WAY RURAL TRAFFIC) (ROAD CLOSED TO THRU TRAFFIC)

BENCHMARKS

- BENCHMARK A**
RAILROAD SPIKE IN POWER POLE 600'± WEST OF WEST ABUTMENT OF EXISTING BRIDGE ON SOUTH SIDE OF BRACEVILLE ROAD.
ELEV. 561.41
- BENCHMARK B**
NORTHWEST CORNER OF WINGWALL AT SOUTHWEST WINGWALL OF EXISTING BRIDGE.
ELEV. 566.42
- BENCHMARK C**
RAILROAD SPIKE IN POWER POLE 585'± EAST OF EAST ABUTMENT OF EXISTING BRIDGE ON SOUTH SIDE OF BRACEVILLE ROAD.
ELEV. 560.32

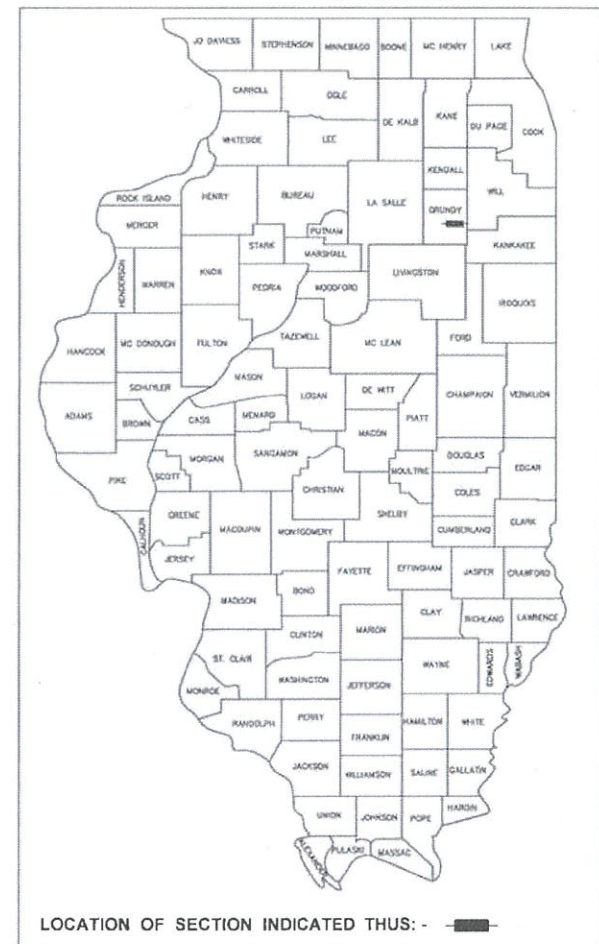


FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123
OR 811

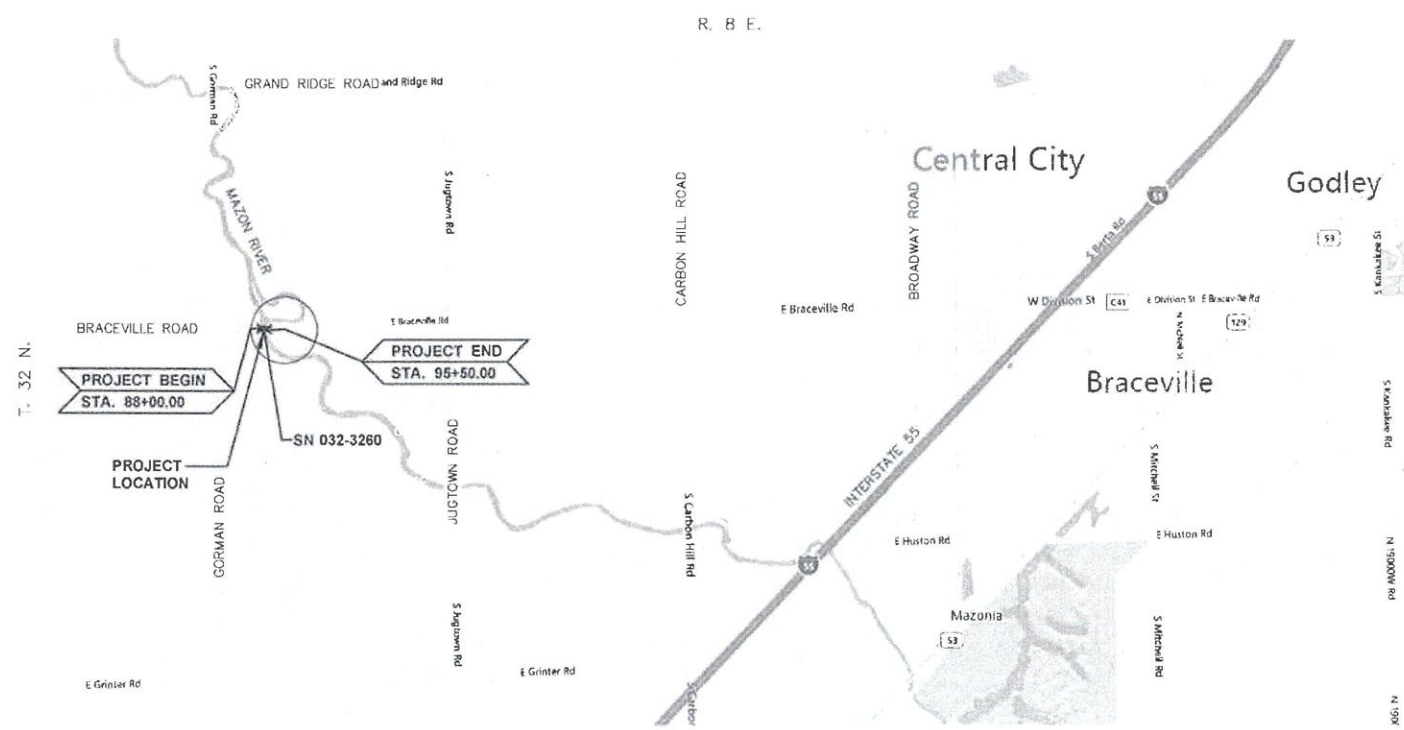
CONTRACT NO. 87642

JAMES K CLINARD
CHAMLIN & ASSOCIATES
815.223.3344

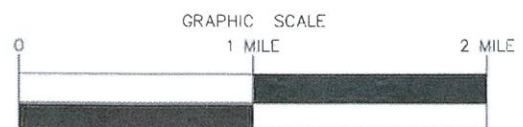


LOCATION OF SECTION INDICATED THUS: -

BRACEVILLE ROAD
FUNCTION CLASSIFICATION: MAJOR COLLECTOR
ADT= < 750 (2015)
REGULATORY SPEED - 55 MPH
DESIGN SPEED - 50 MPH



LOCATION MAP



NET LENGTH OF SECTION -- 750' (0.142 MILES)
GROSS LENGTH OF SECTION -- 750' (0.142 MILES)



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

SUBMITTED 5/22 2018
[Signature]
ACTING GRUNDY COUNTY ENGINEER

PASSED 5/29 2018
[Signature]
DISTRICT 3 LOCAL ROADS & STREETS ENGINEER

RELEASED FOR BID 5/29 2018
[Signature]
BASED ON LIMITED REVIEW
REGION 2 ENGINEER

GENERAL NOTES

THE THICKNESS OF HMA SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE HMA IS PLACED.

EXCEPT AS NOTED ON THE PLANS, PAVEMENT GRADES SHOWN ARE AT THE TOP OF PAVEMENT SURFACES.

BEFORE ORDERING PIPE CULVERTS OR PIPE DRAINS, THE CONTRACTOR SHALL CONSULT THE ENGINEER FOR EXACT LENGTHS.

THE ENGINEER WILL BE THE SOLE JUDGE CONCERNING CURING TIME FOR THE VARIOUS HMA LIFTS.

FOR STABILIZATION, ALL TYPE III BARRICADES WILL REQUIRE A MINIMUM OF FOUR SAND BAGS PER BARRICADE.

SEEDING WILL NOT BE PERMITTED AT ANY TIME WHEN THE GROUND IS FROZEN, WET, OR IN AN UNTILLABLE CONDITION. LOCATIONS TO BE SEEDED WILL BE DETERMINED BY THE ENGINEER.

THE FINISHED EARTHWORK SHALL HAVE A VEGETATION SUSTAINING SOIL COVERING THE TOP FOUR INCHES IN AREAS TO BE SEEDED OR SODDED THE VEGETATION SUSTAINING SOIL REQUIRED WILL NOT BE PAID FOR SEPARATELY BUT WILL BE INCLUDED IN THE COST OF EARTH EXCAVATION.

ON EXISTING PAVEMENT WHICH MAY BE SUPERELEVATED, THE NEW HMA PAVEMENT SHALL BE BUILT WITH THE SAME SUPERELEVATION UNLESS NEW SUPERELEVATION RATES ARE GIVEN ON THE PLANS.

ALL ELEVATIONS REFERRING TO U.S.G.S. MEAN SEA LEVEL DATUM.

ABANDONED UNDERGROUND UTILITIES THAT CONFLICT WITH CONSTRUCTION SHALL BE DISPOSED OF OUTSIDE THE LIMITS OF THE RIGHT OF WAY ACCORDING TO ARTICLE 202.03 OF THE STANDARD SPECIFICATIONS AND AS DIRECTED BY THE ENGINEER. THIS WORK WILL NOT BE PAID FOR SEPARATELY, BUT WILL BE INCLUDED IN THE COST OF EARTH EXCAVATION.

ANY REFERENCE TO A STANDARD IN THESE PLANS SHALL BE INTERPRETED TO MEAN THE EDITION AS INDICATED BY THE SUBNUMBER SHOWN IN THE LIST OF STANDARDS OR THE COPY INCLUDED IN THESE PLANS.

THE FOLLOWING RATES OF APPLICATION HAVE BEEN USED IN CALCULATING PLAN QUANTITIES:

GRANULAR MATERIALS	2.05	TONS / CU YD
HMA RESURFACING	112	LBS / SQ YD / IN

MEMBERS OF JULIE KNOWN TO BE WITHIN THE LIMITS OF THE IMPROVEMENT ARE:

NON-MEMBERS OF JULIE KNOWN TO BE WITHIN THE LIMITS OF THE IMPROVEMENT ARE:

COMED
1910 S. BRIGGS STREET
JOLIET, IL 60433

THE CONTRACTOR SHALL CONTACT JULIE AT LEAST 48 HOURS PRIOR TO EXCAVATION TO DETERMINE WHICH UTILITIES ARE IN THE AREA.

COMMITMENTS

NONE



- ① ROAD CLOSED PER BLR 21 AT STA 87+75
- ② ROAD CLOSED PER BLR 21 AT STA 94+50

ROAD CLOSED PLAN
N.T.S.

SEE SHEET 35 FOR DETOUR SIGNING

CHAMLIN & ASSOCIATES, INC. © 2016
Drawing Name: G:\Users\1111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\C3D\Y... - PLANS\1-PRELIMINARY\02-GEN-NOTES.dwg Last Modified: Mar 28, 2017 - 9:20am Plotted on: Mar 31, 2017 - 4:25pm by lymgoss

DRAWN BY: NV	REVISIONS			
CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 02/2017				

CHAMLIN & ASSOCIATES, INC.
PERU MORRIS ILLINOIS

BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

GENERAL NOTES

CONSTRUCTION PLANS

CURRENT AS OF: 03/28/2017	
SCALE: NONE	SHEET 2
FILE NO.: 111158.00 Y-	OF 46

CONTRACT NO. 87642

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 Drawing Name: S:\Users\j\11158-00-BRACEVILLE-ROAD-BRIDGE\CAD\CAD\Y_ - PLANS\1 - PRELIMINARY\003-007-000.dwg Last Modified: Mar 28, 2017 - 9:28am Plotted on: Mar 31, 2017 - 4:50pm by jymgass

CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE	
				FED 80% / LA 20%	FED 80% / LA 20%
				ROADWAY 0004 RURAL	BRIDGE 0013 RURAL
20200100	EARTH EXCAVATION	CU YD	1180	1180	--
20400800	FURNISHED EXCAVATION	CU YD	991	991	--
25000200	SEEDING, CLASS 2	ACRE	0.75	0.75	--
25000400	NITROGEN FERTILIZER NUTRIENT	POUND	72	72	--
25000500	PHOSPHORUS FERTILIZER NUTRIENT	POUND	72	72	--
25000600	POTASSIUM FERTILIZER NUTRIENT	POUND	72	72	--
25100630	EROSION CONTROL BLANKET	SQ YD	3855	3855	--
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	161	161	--
28000305	TEMPORARY DITCH CHECKS	FOOT	32	32	--
28000400	PERIMETER EROSION BARRIER	FOOT	344	344	--
28000500	INLET AND PIPE PROTECTION	EACH	1	1	--
28100105	STONE RIPRAP, CLASS A3	SQ YD	119	119	--

* SPECIALTY ITEM

DRAWN BY: NV	REVISIONS			
CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 02/2017				

CHAMLIN & ASSOCIATES, INC.
 PERU MORRIS
 ILLINOIS

**BRACEVILLE ROAD
 SECTION 16-00158-00-BR
 GRUNDY COUNTY**

SUMMARY OF QUANTITIES

**CONSTRUCTION
 PLANS**

CURRENT AS OF: 03/28/2017		CONTRACT NO. 87642
SCALE: NONE	SHEET 3	
FILE NO.: 111158.00 Y-	OF 46	

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CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE	
				FED 80% / LA 20% ROADWAY 0004 RURAL	FED 80% / LA 20% BRIDGE 0013 RURAL
28100107	STONE RIPRAP, CLASS A4	SQ YD	682	-	682
28200200	FILTER FABRIC	SQ YD	682	-	682
35100100	AGGREGATE BASE COURSE, TYPE A	TON	44	44	-
40200800	AGGREGATE SURFACE COURSE, TYPE B	TON	55	55	-
40201000	AGGREGATE FOR TEMPORARY ACCESS	TON	55	55	-
40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SQ YD	546	546	-
40800025	BITUMINOUS MATERIALS (PRIME COAT)	POUND	144	144	-
40800029	BITUMINOUS MATERIALS (TACK COAT)	POUND	747	747	-
40800050	INCIDENTAL HOT-MIX ASPHALT SURFACING	TON	188	188	-
44000100	PAVEMENT REMOVAL	SQ YD	60	60	-
48101200	AGGREGATE SHOULDERS, TYPE B	TON	50	50	-
48203013	HOT-MIX ASPHALT SHOULDERS, 4"	SQ YD	50	50	-
50101500	REMOVAL OF EXISTING SUPERSTRUCTURES	EACH	1	-	1

* SPECIALTY ITEM

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CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 02/2017				

CHAMLIN & ASSOCIATES, INC.
 PERU MORRIS ILLINOIS

**BRACEVILLE ROAD
 SECTION 16-00158-00-BR
 GRUNDY COUNTY**

SUMMARY OF QUANTITIES

CONSTRUCTION PLANS

CURRENT AS OF: 03/28/2017	
SCALE: NONE	SHEET 4
FILE NO.: 111158.00 Y-	OF 46

CONTRACT NO. 87642

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CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE	
				FED 80% / LA 20%	FED 80% / LA 20%
				ROADWAY 0004 RURAL	BRIDGE 0013 RURAL
50102400	CONCRETE REMOVAL	CU YD	19.1	-	19.1
50105220	PIPE CULVERT REMOVAL	FOOT	49	49	-
50200100	STRUCTURE EXCAVATION	CU YD	158	-	158
50300225	CONCRETE STRUCTURES	CU YD	16.2	-	16.2
50300255	CONCRETE SUPERSTRUCTURE	CU YD	179.7	-	179.7
50300260	BRIDGE DECK GROOVING	SQ YD	697	-	697
50300300	PROTECTIVE COAT	SQ YD	697	-	697
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1	-	1
50500505	STUD SHEAR CONNECTORS	EACH	3024	-	3024
50800105	REINFORCEMENT BARS	POUND	1830	-	1830
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	54340	-	54340
50800530	MECHANICAL SPLICERS	EACH	32	-	32
* 50901050	STEEL RAILING, TYPE SM	FOOT	451	-	451

* SPECIALTY ITEM

DRAWN BY: NV	REVISIONS			
CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 02/2017		JKC	06/18	Stud Quantity



**BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY**

SUMMARY OF QUANTITIES

**CONSTRUCTION
PLANS**

CONTRACT NO. 87642	
CURRENT AS OF: 06/03/2018	
SCALE: NONE	SHEET 5
FILE NO.: 111158.00 Y-	OF 46

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CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE	
				FED 80% / LA 20%	FED 80% / LA 20%
				ROADWAY 0004 RURAL	BRIDGE 0013 RURAL
51500100	NAME PLATES	EACH	1	-	1
52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	12	-	12
52100520	ANCHOR BOLTS, 1"	EACH	24	-	24
52100530	ANCHOR BOLTS, 1 1/4"	EACH	8	-	8
54213459	END SECTIONS 24"	EACH	2	2	-
542D1069	PIPE CULVERTS, CLASS D, TYPE 2 24"	FOOT	52	52	-
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	76	-	76
61133100	FIELD TILE JUNCTION VAULTS, 2' DIA.	EACH	1	1	-
61140100	STORM SEWERS (SPECIAL), 10"	FOOT	26	26	-
* 63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	FOOT	175	175	-
* 63100045	TRAFFIC BARRIER TERMINAL, TYPE 2	EACH	2	2	-
* 63100087	TRAFFIC BARRIER TERMINAL, TYPE 6A	EACH	4	4	-
* 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	1	1	-

* SPECIALTY ITEM

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DATE: 02/2017				



**BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY**

SUMMARY OF QUANTITIES

CONSTRUCTION PLANS	CURRENT AS OF: 03/28/2017		CONTRACT NO. 87642
	SCALE: NONE	SHEET 6	
	FILE NO.: 111158.00 Y-	OF 46	

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CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE	
				FED 80% / LA 20%	FED 80% / LA 20%
				ROADWAY 0004 RURAL	BRIDGE 0013 RURAL
* 63100169	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) FLARED	EACH	1	1	--
63200310	GUARDRAIL REMOVAL	FOOT	612	612	--
67100100	MOBILIZATION	L SUM	1	0.25	0.75
* 72501000	TERMINAL MARKER - DIRECT APPLIED	EACH	2	2	--
* 78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	750	750	--
* 78001130	PAINT PAVEMENT MARKING - LINE 6"	FOOT	190	190	--
* 78200005	GUARDRAIL REFLECTORS, TYPE A	EACH	10	10	--
Z0013798	CONSTRUCTION LAYOUT	L SUM	1	0.25	0.75
Z0046304	PIPE UNDERDRAINS FOR STRUCTURES, 4"	FOOT	134	--	134
X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	153	--	153
X7010216	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)	L SUM	1	0.25	0.75

* SPECIALTY ITEM

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CHECKED BY: JKC	LEVEL	BY	DATE
DATE: 02/2017			


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BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

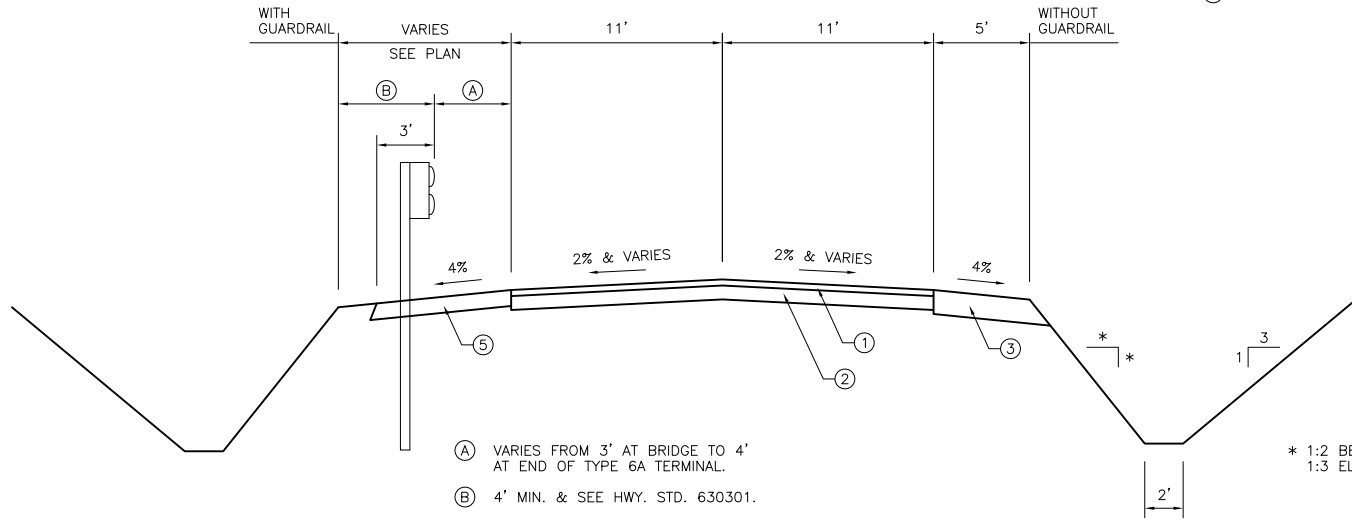
SUMMARY OF QUANTITIES

CONSTRUCTION
PLANS

CONTRACT NO. 87642	
CURRENT AS OF: 03/28/2017	
SCALE: NONE	SHEET 7
FILE NO.: 111158.00 Y-	OF 46

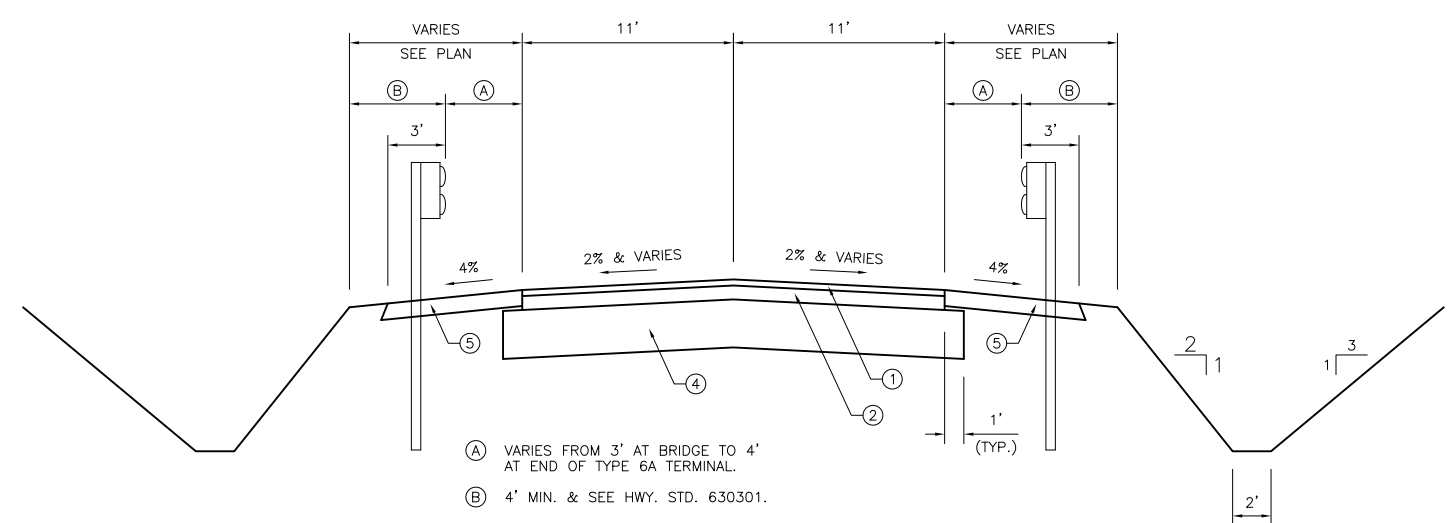
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- ① INCIDENTAL HOT-MIX ASPHALT SURFACING 1 1/2"
- ② INCIDENTAL HOT-MIX ASPHALT SURFACING VAIRES 3/4" TO 3 1/2"
- ③ AGGREGATE SHOULDERS, TYPE B 6"
- ④ AGGREGATE BASE COURSE, TYPE A 12"
- ⑤ HOT-MIX ASPHALT SHOULDERS 4"



BRACEVILLE ROAD PROPOSED TYPICAL SECTION

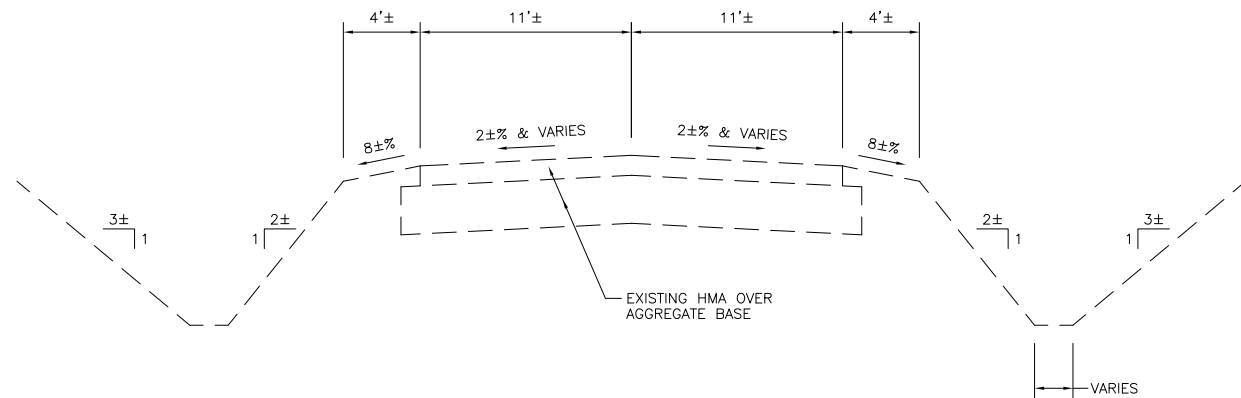
N.T.S.
 STA 88+00 TO STA 89+68
 AND
 STA 92+16 TO STA 95+50



BRACEVILLE ROAD PROPOSED TYPICAL SECTION

N.T.S.
 STA 89+68 TO STA 89+80
 AND
 STA 92+04 TO STA 92+16

BRIDGE OMISSION STA 89+80 TO STA 92+04



BRACEVILLE ROAD EXISTING TYPICAL SECTION

N.T.S.

HMA MIXTURE REQUIREMENT TABLE	
LOCATIONS(S):	ENTIRE PROJECT
MIXTURE USE(S):	INCIDENTAL HMA
BINDER GRADE (PG):	PG 64-22
DESIGN AIR VOIDS:	4.0% @ N50
MIXTURE COMPOSITION: (MIXTURE GRADATION)	IL 9.5
FRICTION AGGREGATE:	MIXTURE C
MIXTURE WEIGHT:	112.0 LB/SY/IN
QUALITY MANAGEMENT PROGRAM:	QCQA
SUBLOT SIZE:	NA
DENSITY TEST METHOD:	LR 1030

SEE SHEET 14 FOR PROPOSED BRIDGE TYPICAL SECTION

SEE SHEET 29 FOR EXISTING BRIDGE TYPICAL SECTION

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CHAMLIN & ASSOCIATES, INC.
 PERU MORRIS ILLINOIS

BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

TYPICAL SECTIONS

CONSTRUCTION PLANS

CONTRACT NO. 87642

CURRENT AS OF: 03/28/2017

SCALE: NONE SHEET 8

FILE NO.: 111158.00 Y- OF 46

EARTHWORK SCHEDULE*

LOCATION	20200100	EARTH EXCAVATION ADJUSTED FOR SHRINKAGE*	EMBANKMENT	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-)
	EARTH EXCAVATION			
	CU. YD.	CU. YD.	CU. YD.	CU. YD.
STA 88+00 TO STA 89+90	661	495.8	567	-71.3
STA 92+04 TO STA 95+50	519	389.3	1309	-919.8
TOTAL	1180	885	1876	-991

* 25% SHRINKAGE FACTOR

FURNISHED EXCAVATION

SEEDING SCHEDULE

LOCATION	25000200	25000400	25000500	25000600	25100630	28000250
	SEEDING, CLASS 2	NITROGEN FERTILIZER NUTRIENT	PHOSPHORUS FERTILIZER NUTRIENT	POTASSIUM FERTILIZER NUTRIENT	EROSION CONTROL BLANKET	TEMPORARY EROSION CONTROL SEEDING
	ACRE	POUND	POUND	POUND	SQ YD	POUND
MAINLINE						
88+00 TO 89+90, LT	0.19	16.79	16.79	16.79	903.00	37.31
88+00 TO 89+90, RT	0.15	13.16	13.16	13.16	707.82	29.25
92+04 TO 95+50, LT	0.18	15.86	15.86	15.86	852.72	35.24
92+04 TO 95+50, RT	0.30	26.68	26.68	26.68	1391.89	59.29
TOTAL	0.75	72	72	72	3855	161

EROSION CONTROL SCHEDULE

LOCATION	28000305	28000400	28000500	28100105
	TEMPORARY DITCH CHECKS	PERIMETER EROSION BARRIER	INLET AND PIPE PROTECTION	STONE RIPRAP, CLASS A3
	FOOT	FOOT	EACH	SQ YD
LT STA 88+00 TO STA 88+50	--	--	--	119
LT & RT STA 89+50	16	--	--	--
LT & RT STA 92+50	16	--	--	--
LT STA 92+00 TO STA 95+50	--	344	--	--
RT STA 95+22.48	--	--	1	--
TOTAL	32	344	1	119

PAVING AND DRIVEWAY SCHEDULE

LOCATION	35100100	40200800	40201000	40800025	40800029	40800060	48100200	48203013
	AGGREGATE BASE COURSE, TYPE A	AGGREGATE SURFACE COURSE, TYPE B	AGGREGATE FOR TEMPORARY ACCESS	BITUMINOUS MATERIALS (PRIME COAT)	BITUMINOUS MATERIALS (TACK COAT)	INCIDENTAL HOT-MIX ASPHALT SURFACING	AGGREGATE SHOULDERS, TYPE B	HOT-MIX ASPHALT SHOULDERS, 4"
	TON	TON	TON	POUND	POUND	TON	TON	SQ YD
STA 89+68 TO STA 89+80	21.9	--	--	72.0	--	--	--	--
STA 92+04 TO STA 92+16	21.9	--	--	72.0	--	--	--	--
STA 88+00 TO STA 89+80	--	--	--	--	189.3	63.8	--	--
STA 92+04 TO STA 95+50	--	--	--	--	557.7	124.2	--	--
RT STA 88+00 TO STA 88+69.79	--	--	--	--	--	--	7.6	--
LT STA 88+00 TO STA 89+12	--	--	--	--	--	--	12.4	--
RT STA 88+69.79 TO STA 89+79.17	--	--	--	--	--	--	--	85.4
LT STA 89+12 TO STA 89+79.17	--	--	--	--	--	--	--	49.0
RT STA 92+04.83 TO STA 94+73.08	--	--	--	--	--	--	--	205.4
LT STA 92+04.83 TO STA 93+20.02	--	--	--	--	--	--	--	101.9
RT STA 95+11.51 TO STA 95+50	--	--	--	--	--	--	4.2	--
LT STA 93+20.02 TO STA 95+50	--	--	--	--	--	--	25.7	--
RT STA 94+91	--	55.2	55.2	--	--	--	--	--
TOTAL	44	55	55	144	747	188	50	442

GUARDRAIL & BARRIER MARKER SCHEDULE

LOCATION	78200005	72501000
	GUARDRAIL REFLECTORS, TYPE EACH	TERMINAL MARKER - DIRECT APPLIED EACH
RT STA 88+82.77 TO RT STA 94+66.11	6	1
LT STA 89+22.77 TO LT STA 92+98.87	4	1
TOTAL	10	2

GUARDRAIL REMOVAL SCHEDULE

LOCATION	63200310
	GUARDRAIL REMOVAL FOOT
LT STA 88+67.79 TO STA 89+79.17	113
RT STA 88+92.84 TO STA 89+79.17	88
LT STA 92+04.83 TO STA 93+54.20	149
RT STA 92+04.83 TO STA 94+64.89	262
TOTAL	612

GUARDRAIL SCHEDULE

LOCATION	63000001	63100087	63100045	63100167	63100169
	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	TRAFFIC BARRIER TERMINAL, TYPE 6A	TRAFFIC BARRIER TERMINAL, TYPE 2	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) FLARED
	FOOT	EACH	EACH	EACH	EACH
RT STA 88+82.77 TO RT STA 89+79.02	--	1	--	1	--
RT STA 92+04.98 TO RT STA 94+66.16	175	1	1	--	--
LT STA 89+22.77 TO LT STA 89+79.02	--	1	1	--	--
LT STA 92+04.98 TO LT STA 92+98.87	--	1	--	--	1
TOTAL	175	4	2	1	1

CULVERT SCHEDULE

LOCATION	54213459	542D1069
	END SECTIONS 24" EACH	PIPE CULVERTS, CLASS D, TYPE 2 24" FOOT
RT STA 94+59.41 TO RT STA 95+22.48	2	52
TOTAL	2	52

PAVEMENT MARKING SCHEDULE

LOCATION	78001110	78001130
	PAINT PAVEMENT MARKING - LINE 4" EACH	PAINT PAVEMENT MARKING - LINE 6" FOOT
STA 88+00 TO STA 95+50 (CENTERLINE)	--	190
STA 88+00 TO STA 95+50 (NO PASSING)	750	--
TOTAL	750	190

PAVEMENT REMOVAL SCHEDULE

LOCATION	44000100
	PAVEMENT REMOVAL SQ YD
STA 89+68 TO STA 89+80	30
STA 92+04 TO STA 92+16	30
TOTAL	60

HMA SURFACE REMOVAL - BUTT JOINT SCHEDULE

LOCATION	40600982
	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT SQ YD
STA 88+00 TO STA 88+50	122
STA 92+90 TO STA 95+50	424
TOTAL	546

STORM SEWER SCHEDULE

LOCATION	61133100	61140100
	FIELD TILE JUNCTION VAULTS, 2' DIA. EACH	STORM SEWERS (SPECIAL), 10" FOOT
STA 88+51.70, 33.42' LT TO STA 88+69.59, 52.75' LT	1	26
TOTAL	1	26

PIPE CULVERT REMOVAL SCHEDULE

LOCATION	CULVERT SIZE	50105220
		PIPE CULVERT REMOVAL FEET
RT STA 95+00	24"	49
TOTAL	--	49

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Drawing Name: G:\Users\1111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\C3D\Y... - PLANS\1-PRELIMINARY\09-SCHEDULES.dwg Last Modified: Mar 28, 2017 - 9:18am Plotted on: Mar 31, 2017 - 4:45pm by lyngosa

DRAWN BY: NV	REVISIONS			
CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 02/2017				

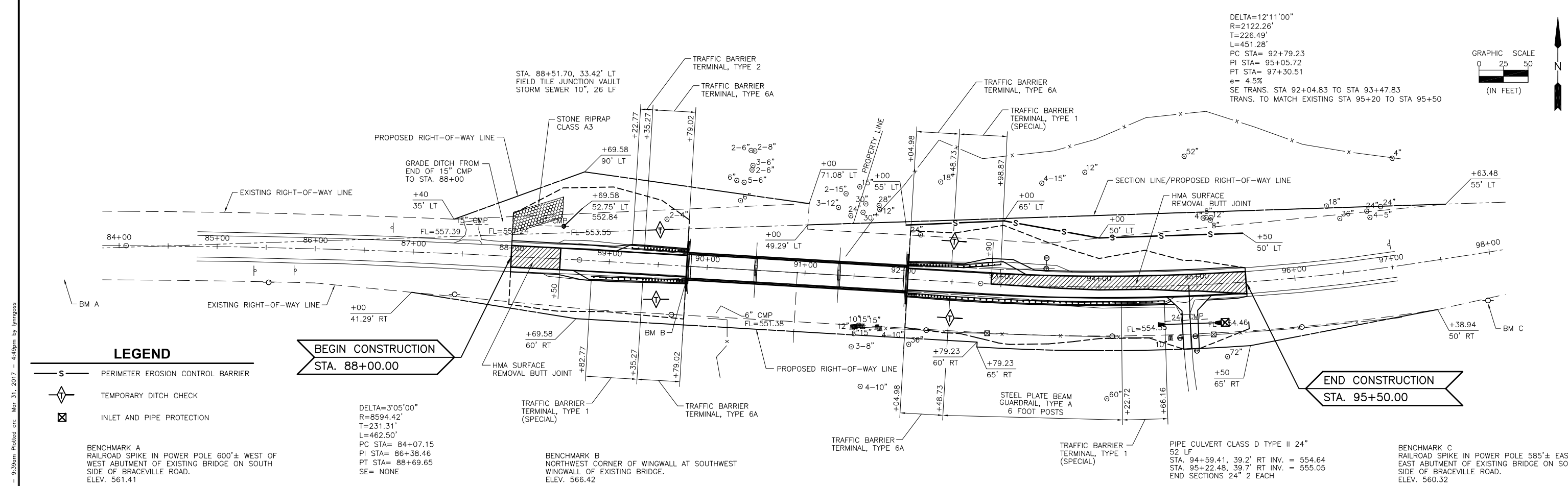
CHAMLIN & ASSOCIATES, INC.
PERU MORRIS ILLINOIS

BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

SCHEDULE OF QUANTITIES

CONSTRUCTION PLANS
CURRENT AS OF: 03/28/2017
SCALE: NONE SHEET 9
FILE NO.: 111158.00 Y- OF 46

CONTRACT NO. 87642



DELTA=12'11"00"
 R=2122.26'
 T=226.49'
 L=451.28'
 PC STA= 92+79.23
 PI STA= 95+05.72
 PT STA= 97+30.51
 e= 4.5%
 SE TRANS. STA 92+04.83 TO STA 93+47.83
 TRANS. TO MATCH EXISTING STA 95+20 TO STA 95+50

GRAPHIC SCALE
 0 25 50
 (IN FEET)

- LEGEND**
- PERIMETER EROSION CONTROL BARRIER
 - TEMPORARY DITCH CHECK
 - INLET AND PIPE PROTECTION

**BEGIN CONSTRUCTION
 STA. 88+00.00**

**END CONSTRUCTION
 STA. 95+50.00**

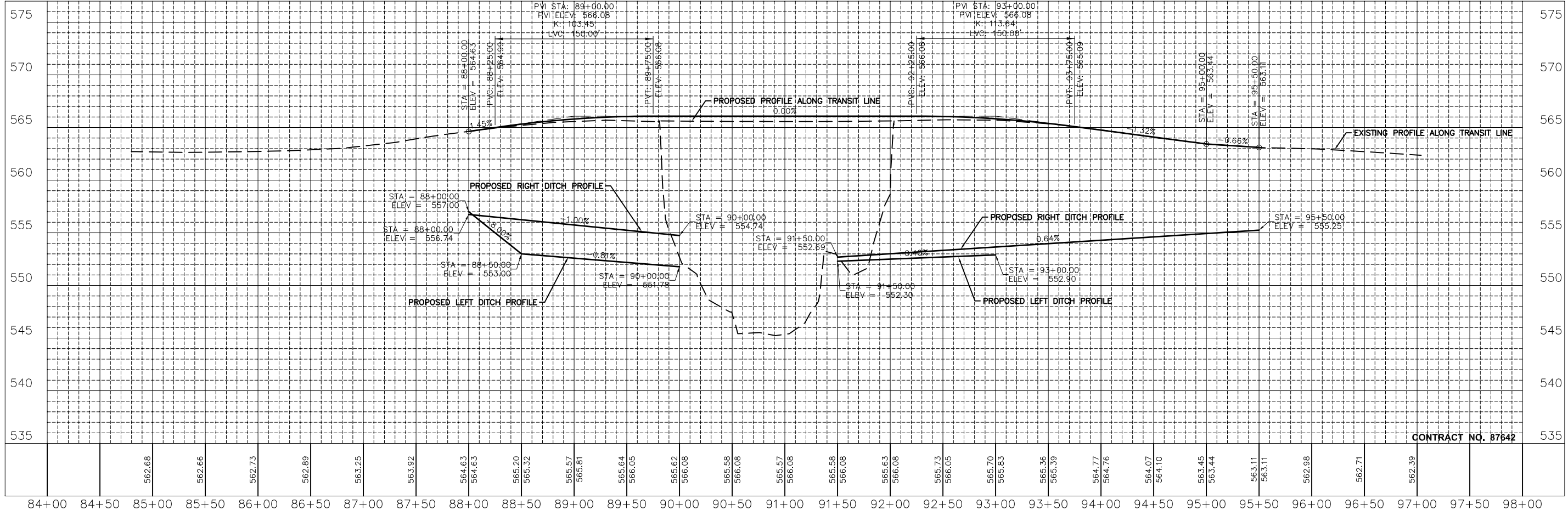
BENCHMARK A
 RAILROAD SPIKE IN POWER POLE 600'± WEST OF
 WEST ABUTMENT OF EXISTING BRIDGE ON SOUTH
 SIDE OF BRACEVILLE ROAD.
 ELEV. 561.41

DELTA=3'05"00"
 R=8594.42'
 T=231.31'
 L=462.50'
 PC STA= 84+07.15
 PI STA= 86+38.46
 PT STA= 88+69.65
 SE= NONE

BENCHMARK B
 NORTHWEST CORNER OF WINGWALL AT SOUTHWEST
 WINGWALL OF EXISTING BRIDGE.
 ELEV. 566.42

PIPE CULVERT CLASS D TYPE II 24"
 52 LF
 STA. 94+59.41, 39.2' RT INV. = 554.64
 STA. 95+22.48, 39.7' RT INV. = 555.05
 END SECTIONS 24' 2 EACH

BENCHMARK C
 RAILROAD SPIKE IN POWER POLE 585'± EAST OF
 EAST ABUTMENT OF EXISTING BRIDGE ON SOUTH
 SIDE OF BRACEVILLE ROAD.
 ELEV. 560.32



CONTRACT NO. 87642

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 Drawing Name: G:\Users\1111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\C3D\Y... - PLANS\1- PRELIMINARY\010 - 184-198 PLAN AND PROFILE.dwg Last Modified: Mar 28, 2017 - 9:39am Plotted on: Mar 31, 2017 - 4:49pm by lympos

DRAWN BY: LAG/NV	REVISIONS			
CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 09/2016				

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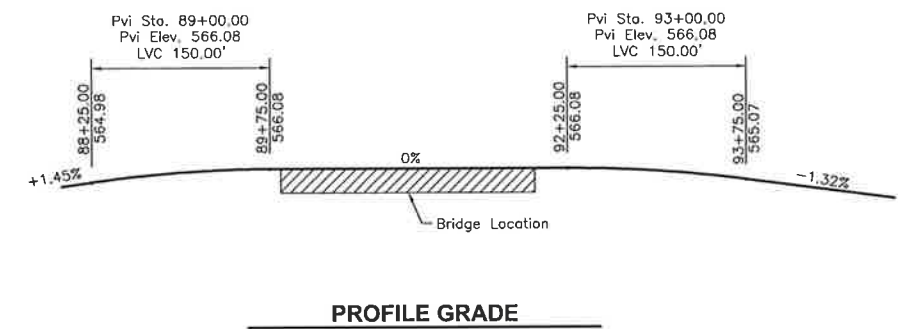
**BRACEVILLE ROAD
 SECTION 16-00158-00-BR
 GRUNDY COUNTY**

**PLAN AND PROFILE
 STA. 184+00 TO 198+00**

CONSTRUCTION PLANS	CURRENT AS OF: 03/28/2017
SCALE: AS NOTED	SHEET 10
FILE NO.: 111158.00 Y-	OF 46

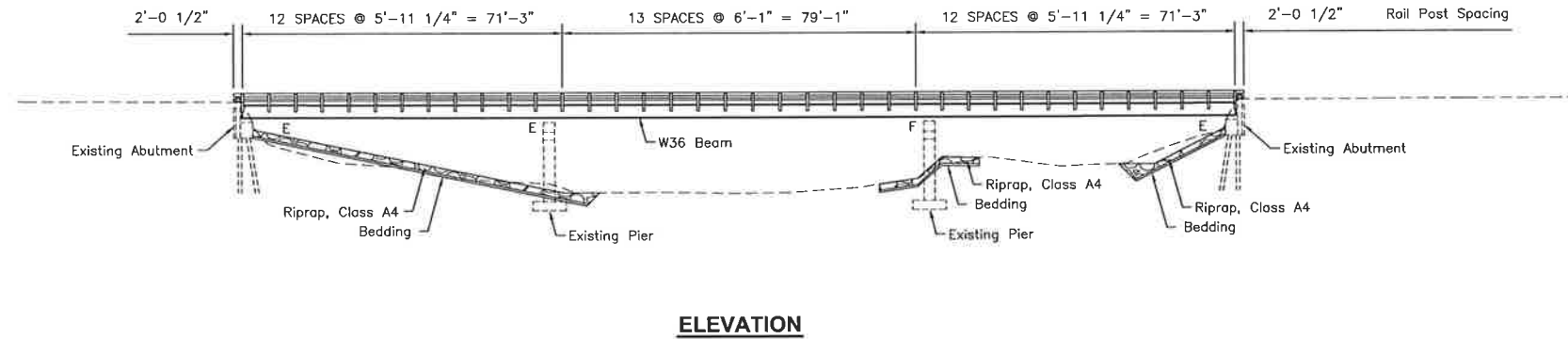
Benchmarks:
Benchmark A
 Railroad spike in power pole 600'± west of west abutment of existing bridge on south side of Braceville Road.
 Elev. 561.41
Benchmark B
 Northwest corner of wingwall at southwest wingwall of existing bridge.
 Elev. 566.42
Benchmark C
 Railroad spike in power pole 585'± east of east abutment of existing bridge on south side of Braceville Road.
 Elev. 560.32

Existing Structure: S.N. 032-3260, originally built in 1970 as CH 9 Section 50-B. The existing structure consists of 3 continuous steel spans, 225'-8" Bk. to Bk. abutments and 26'-0" Out to Out of deck, on pile supported abutments and spread footing piers. Superstructure to be removed and replaced. Traffic to be maintained by marked detour.
 Re-use existing foundations.

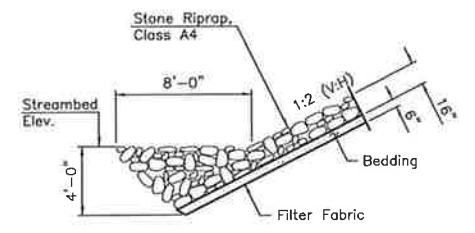


INDEX OF SHEETS

- GENERAL PLAN AND ELEVATION
- GENERAL DATA
- TOP OF DECK ELEVATIONS
- SUPERSTRUCTURE (PLAN AND CROSS SECTION)
- DIAPHRAGM DETAILS
- BRIDGE RAILING DETAILS
- 9. GIRDER AND FRAMING DETAILS
- 13. BEARING DETAILS
- CONCRETE REMOVAL AT ABUTMENTS
- 16. NEW WORK AT ABUTMENTS
- BAR SPLICER ASSEMBLY DETAILS

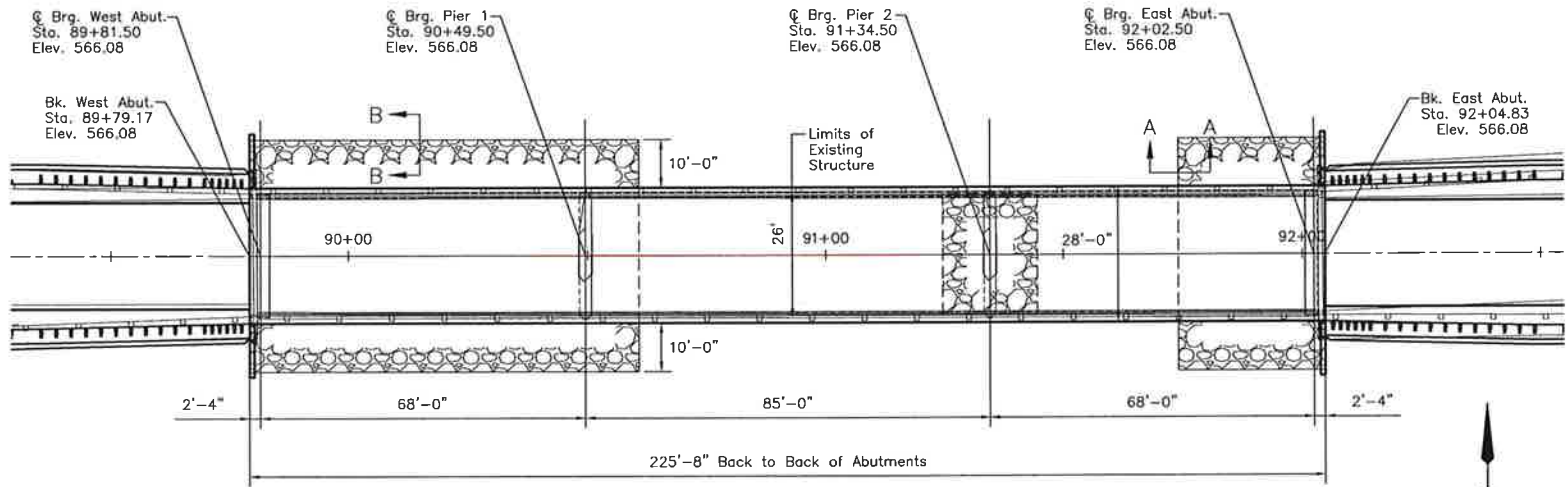


ELEVATION



SECTION A-A

MAZON RIVER
 RE-BUILT 20__ BY
 GRUNDY COUNTY
 SEC 16-00158-00-BR
 F.A.S. RT. 272 STA 90+92
 STR. NO. 032-3260 LOADING HL-93
NAME PLATE
 See Std. 515001



PLAN

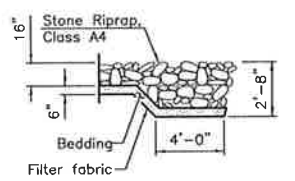
LOADING HL-93 (SUPERSTRUCTURE ONLY)
 Allow 25#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

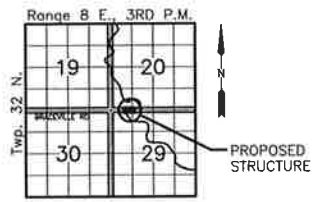
2014 AASHTO LRFD Bridge Design Specifications with 2015-2016 Interims

DESIGN STRESSES

FIELD UNITS
 f'c = 4,000 psi (Deck)
 f'c = 3,500 psi (Substructure)
 fy = 60,000 psi (Reinforcement)
 fy = 50,000 psi (M270 Grade 50W)



SECTION B-B



LOCATION SKETCH

WATERWAY INFORMATION TABLE

Drainage Area = 235 Square Miles		Overtopping Elev.=562.37'/562.37' (Exist/Prop.) @ STA 97+04											
Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft.						Nat. H.W.E.				
			Bridge		O.T.R.		Total		Exist.	Prop.	Exist.	Prop.	
Design	10	3790	1290	0	1290	1290	0	1290	556.1	0.1	0.1	556.2	556.2
Base	20	4450	1406	0	1406	1406	0	1406	556.7	0.1	0.1	556.8	556.8
Overtopping	100	5880	1624	0	1624	1624	0	1624	557.8	0.1	0.1	557.9	557.9
Max. Calc.	500	7250	1807	0	1807	1807	0	1807	558.7	0.1	0.1	558.8	558.8

Max. H.W.E.: 562.3 (ft.) Max. H.W.E. Date: 1958
 Exist. 10 Year Velocity: 3.0 (ft./sec.)
 Prop. 10 Year Velocity: 3.0 (ft./sec.)

DESIGN SCOUR ELEVATION TABLE

Event/Limit State	Design Scour Elevations (ft.)				Item 113
	W. Abut.	Pier 1	Pier 2	E. Abut.	
Q100	Not Computed	541.81	543.68	Not Computed	5
Q200	Not Computed	Not Computed	Not Computed	Not Computed	
Design	557.41	541.29	541.35	557.41	
Check	557.41	541.29	541.35	557.41	

I certify that to the best of knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AASHTO LRFD Bridge Design Specifications.

6/29/16
 date
 JAMES K. CLINARD
 LICENSED STRUCTURAL ENGINEER
 NO. 081-004655
 STATE OF ILLINOIS
 expires 11-30-2018
 signature
 PROFESSIONAL DESIGN FIRM
 LICENSE NO. 184-001717

**GENERAL PLAN & ELEVATION
 BRACEVILLE ROAD
 OVER MAZON RIVER
 F.A.S. 273 SEC. 16-00158-00-BR
 GRUNDY COUNTY
 STATION 90+92
 STRUCTURE NO. 032-3260
 CONTRACT NO. 87642**

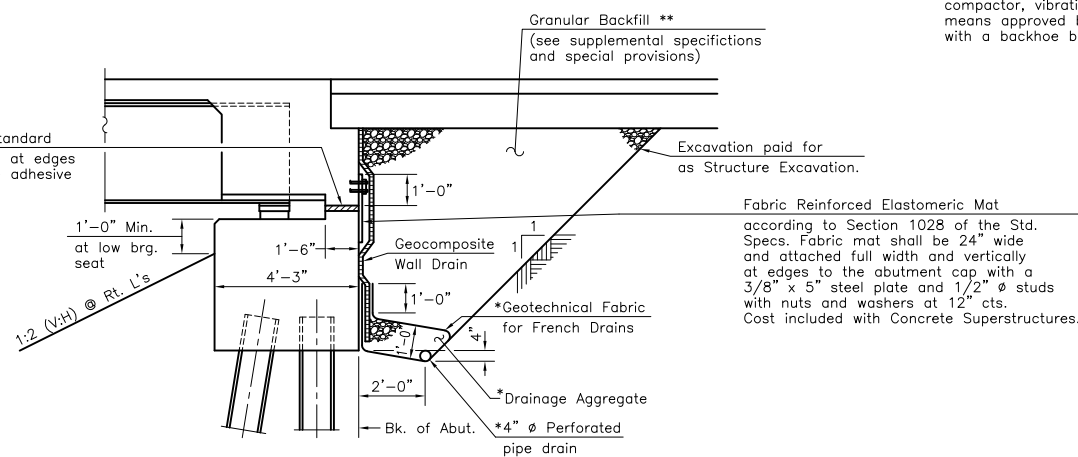
CHAMLIN & ASSOCIATES, INC. © 2016 Drawing Name: 111158-00-BRACEVILLE-ROAD-BRIDGE-CAD(CSD).Y. PLANS)-PRELIMINARY(01) - 001 - GENERAL PLAN AND ELEVATION Last Modified: Jun 28, 2016 - 8:53am Plotted on: Jun 28, 2016 - 3:31pm by asch

GENERAL NOTES

- Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts in painted areas and ASTM A325 Type 3 in unpainted areas. Bolts 7/8 in. dia., holes 15/16 in. dia., unless otherwise noted.
- Calculated weight of Structural Steel = M270 GR50 188430, M270 G36 17280.
- All structural steel shall be AASHTO M 270 Grade 50W.
- No field welding is permitted except as specified in the contract documents.
- Reinforcement bars designed (E) shall be epoxy coated.
- Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8 in. (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete cap plus 18 in. Painted areas shall be primed in the shop with a Department approved zinc rich primer. Field painting will not be required.
- Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
- Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
- The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 36W.
- Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.
- Existing reinforcement shall be cleaned and incorporated into the new construction. Cost included with Concrete Removal.
- Existing name plate shall be cleaned and relocated next to new name plate. Cost included with Name Plates.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Granular Backfill for Structures	Cu. Yd.	-	153	153
Stone Riprap, Class A4	Sq. Yd.	-	682	682
Filter Fabric	Sq. Yd.	-	682	682
Removal of Existing Superstructures	Each	1	-	1
Concrete Removal	Cu. Yd.	-	19.1	19.1
Structure Excavation	Cu. Yd.	-	158	158
Concrete Structures	Cu. Yd.	-	16.2	16.2
Concrete Superstructure	Cu. Yd.	179.7	-	179.7
Bridge Deck Grooving	Sq. Yd.	697	-	697
Protective Coat	Sq. Yd.	697	-	697
Furnishing and Erecting Structural Steel	L. Sum	1	-	1
Stud Shear Connectors	Each	3024	-	3024
Reinforcement Bars	Pound	-	1830	1830
Reinforcement Bars, Epoxy Coated	Pound	54180	160	54340
Mechanical Splicers	Each	-	32	32
Steel Railing, Type SM	Foot	451	-	451
Name Plates	Each	1	-	1
Elastomeric Bearing Assembly, Type I	Each	12	-	12
Anchor Bolt, 1"	Each	24	-	24
Anchor Bolt, 1 1/4"	Each	8	-	8
Geocomposite Wall Drain	Sq. Yd.	-	76	76
Pipe Underdrains for Structures, 4"	Foot	-	134	134



** The material shall be placed in lifts of 6" and shall be compacted with vibrating plate compactor, vibrating roller or other vibrating means approved by the Engineer. Compaction with a backhoe bucket is not acceptable.

2" PUF (per Article 1051.09 of the Standard Specifications) full width and vertically at edges bonded to abutment cap with suitable adhesive as recommended by supplier.

Excavation paid for as Structure Excavation.

Fabric Reinforced Elastomeric Mat according to Section 1028 of the Std. Specs. Fabric mat shall be 24" wide and attached full width and vertically at edges to the abutment cap with a 3/8" x 5" steel plate and 1/2" ϕ studs with nuts and washers at 12" cts. Cost included with Concrete Superstructures.

*Included in the cost of Pipe Underdrains for Structures.

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

SECTION THRU SEMI-INTEGRAL ABUTMENT

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DRAWN BY: LAG	REVISIONS			
CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 02/2017		JKC	06/18	Stud Quantity

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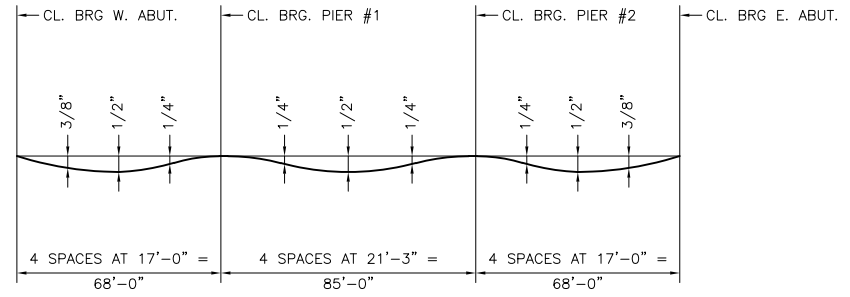
BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

GENERAL DATA
STRUCTURE NO. 032-3260
SHEET 2 OF 17

CONSTRUCTION PLANS

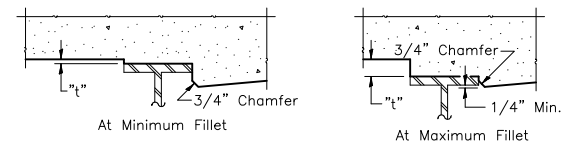
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SCALE: AS NOTED	SHEET 12
FILE NO.: 111158.00 Y-	OF 46

CONTRACT NO. 87642



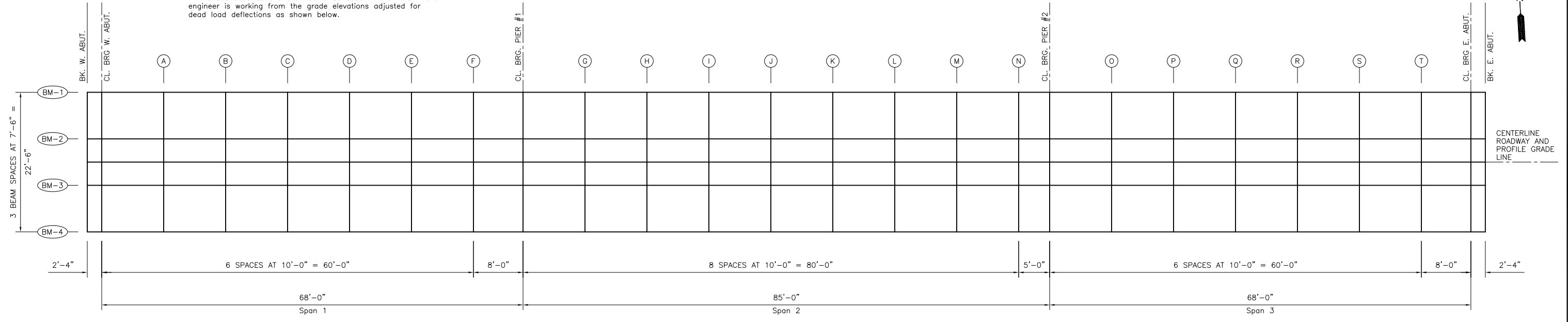
DEAD LOAD DEFLECTION DIAGRAM
(Includes weight of concrete only.)

Note: The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below, minus slab thickness, equals the fillet heights "t" above top flange of beams.

DEAD LOAD DEFLECTION DIAGRAM



PLAN

BEAM 1					BEAM 2					CENTERLINE ROADWAY AND PROFILE GRADE					BEAM 3					BEAM 4				
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Dead Load Deflections	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Dead Load Deflections	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Dead Load Deflections	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Dead Load Deflections	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Dead Load Deflections
BK. W. ABUT.	89+79.17	-11.25	565.85	565.85	BK. W. ABUT.	89+79.17	-3.75	566.00	566.00	BK. W. ABUT.	89+79.17	0.00	566.08	566.08	BK. W. ABUT.	89+79.17	3.75	566.00	566.00	BK. W. ABUT.	89+79.17	11.25	565.85	565.85
CL. BRG. W. ABUT.	89+81.50	-11.25	565.85	565.85	CL. BRG. W. ABUT.	89+81.50	-3.75	566.00	566.00	CL. BRG. W. ABUT.	89+81.50	0.00	566.08	566.08	CL. BRG. W. ABUT.	89+81.50	3.75	566.00	566.00	CL. BRG. W. ABUT.	89+81.50	11.25	565.85	565.85
A	89+91.50	-11.25	565.85	565.87	A	89+91.50	-3.75	566.00	566.02	A	89+91.50	0.00	566.08	566.10	A	89+91.50	3.75	566.00	566.02	A	89+91.50	11.25	565.85	565.87
B	90+01.50	-11.25	565.85	565.88	B	90+01.50	-3.75	566.00	566.03	B	90+01.50	0.00	566.08	566.11	B	90+01.50	3.75	566.00	566.03	B	90+01.50	11.25	565.85	565.88
C	90+11.50	-11.25	565.85	565.89	C	90+11.50	-3.75	566.00	566.04	C	90+11.50	0.00	566.08	566.12	C	90+11.50	3.75	566.00	566.04	C	90+11.50	11.25	565.85	565.89
D	90+21.50	-11.25	565.85	565.88	D	90+21.50	-3.75	566.00	566.03	D	90+21.50	0.00	566.08	566.11	D	90+21.50	3.75	566.00	566.03	D	90+21.50	11.25	565.85	565.88
E	90+31.50	-11.25	565.85	565.87	E	90+31.50	-3.75	566.00	566.02	E	90+31.50	0.00	566.08	566.10	E	90+31.50	3.75	566.00	566.02	E	90+31.50	11.25	565.85	565.87
F	90+41.50	-11.25	565.85	565.85	F	90+41.50	-3.75	566.00	566.00	F	90+41.50	0.00	566.08	566.08	F	90+41.50	3.75	566.00	566.00	F	90+41.50	11.25	565.85	565.85
CL. BRG. PIER 1	90+49.50	-11.25	565.85	565.85	CL. BRG. PIER 1	90+49.50	-3.75	566.00	566.00	CL. BRG. PIER 1	90+49.50	0.00	566.08	566.08	CL. BRG. PIER 1	90+49.50	3.75	566.00	566.00	CL. BRG. PIER 1	90+49.50	11.25	565.85	565.85
G	90+59.50	-11.25	565.85	565.86	G	90+59.50	-3.75	566.00	566.01	G	90+59.50	0.00	566.08	566.09	G	90+59.50	3.75	566.00	566.01	G	90+59.50	11.25	565.85	565.86
H	90+69.50	-11.25	565.85	565.87	H	90+69.50	-3.75	566.00	566.02	H	90+69.50	0.00	566.08	566.10	H	90+69.50	3.75	566.00	566.02	H	90+69.50	11.25	565.85	565.87
I	90+79.50	-11.25	565.85	565.88	I	90+79.50	-3.75	566.00	566.03	I	90+79.50	0.00	566.08	566.11	I	90+79.50	3.75	566.00	566.03	I	90+79.50	11.25	565.85	565.88
J	90+89.50	-11.25	565.85	565.89	J	90+89.50	-3.75	566.00	566.04	J	90+89.50	0.00	566.08	566.12	J	90+89.50	3.75	566.00	566.04	J	90+89.50	11.25	565.85	565.89
K	90+99.50	-11.25	565.85	565.89	K	90+99.50	-3.75	566.00	566.04	K	90+99.50	0.00	566.08	566.12	K	90+99.50	3.75	566.00	566.04	K	90+99.50	11.25	565.85	565.89
L	91+09.50	-11.25	565.85	565.88	L	91+09.50	-3.75	566.00	566.03	L	91+09.50	0.00	566.08	566.11	L	91+09.50	3.75	566.00	566.03	L	91+09.50	11.25	565.85	565.88
M	91+19.50	-11.25	565.85	565.86	M	91+19.50	-3.75	566.00	566.01	M	91+19.50	0.00	566.08	566.09	M	91+19.50	3.75	566.00	566.01	M	91+19.50	11.25	565.85	565.86
N	91+29.50	-11.25	565.85	565.85	N	91+29.50	-3.75	566.00	566.00	N	91+29.50	0.00	566.08	566.08	N	91+29.50	3.75	566.00	566.00	N	91+29.50	11.25	565.85	565.85
CL. BRG. PIER 1	91+34.50	-11.25	565.85	565.85	CL. BRG. PIER 1	91+34.50	-3.75	566.00	566.00	CL. BRG. PIER 1	91+34.50	0.00	566.08	566.08	CL. BRG. PIER 1	91+34.50	3.75	566.00	566.00	CL. BRG. PIER 1	91+34.50	11.25	565.85	565.85
O	91+44.50	-11.25	565.85	565.86	O	91+44.50	-3.75	566.00	566.01	O	91+44.50	0.00	566.08	566.09	O	91+44.50	3.75	566.00	566.01	O	91+44.50	11.25	565.85	565.86
P	91+54.50	-11.25	565.85	565.87	P	91+54.50	-3.75	566.00	566.02	P	91+54.50	0.00	566.08	566.10	P	91+54.50	3.75	566.00	566.02	P	91+54.50	11.25	565.85	565.87
Q	91+64.50	-11.25	565.85	565.88	Q	91+64.50	-3.75	566.00	566.03	Q	91+64.50	0.00	566.08	566.11	Q	91+64.50	3.75	566.00	566.03	Q	91+64.50	11.25	565.85	565.88
R	91+74.50	-11.25	565.85	565.89	R	91+74.50	-3.75	566.00	566.04	R	91+74.50	0.00	566.08	566.12	R	91+74.50	3.75	566.00	566.04	R	91+74.50	11.25	565.85	565.89
S	91+84.50	-11.25	565.85	565.88	S	91+84.50	-3.75	566.00	566.03	S	91+84.50	0.00	566.08	566.11	S	91+84.50	3.75	566.00	566.03	S	91+84.50	11.25	565.85	565.88
T	91+94.50	-11.25	565.85	565.87	T	91+94.50	-3.75	566.00	566.02	T	91+94.50	0.00	566.08	566.10	T	91+94.50	3.75	566.00	566.02	T	91+94.50	11.25	565.85	565.87
CL. BRG. E. ABUT.	92+02.50	-11.25	565.85	565.85	CL. BRG. E. ABUT.	92+02.50	-3.75	566.00	566.00	CL. BRG. E. ABUT.	92+02.50	0.00	566.08	566.08	CL. BRG. E. ABUT.	92+02.50	3.75	566.00	566.00	CL. BRG. E. ABUT.	92+02.50	11.25	565.85	565.85
BK. E. ABUT.	92+04.83	-11.25	565.85	565.85	BK. E. ABUT.	92+04.83	-3.75	566.00	566.00	BK. E. ABUT.	92+04.83	0.00	566.08	566.08	BK. E. ABUT.	92+04.83	3.75	566.00	566.00	BK. E. ABUT.	92+04.83	11.25	565.85	565.85

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DATE: 09/2016				

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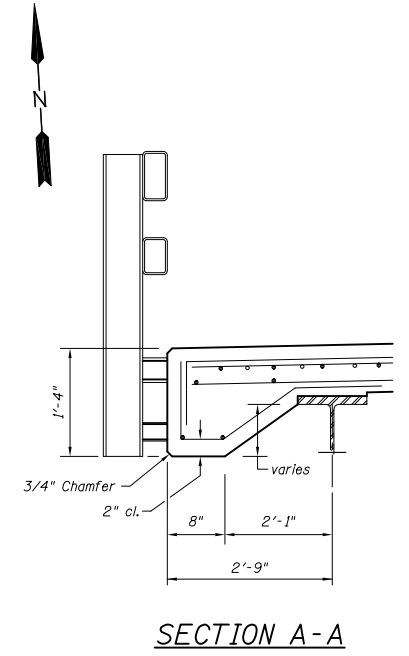
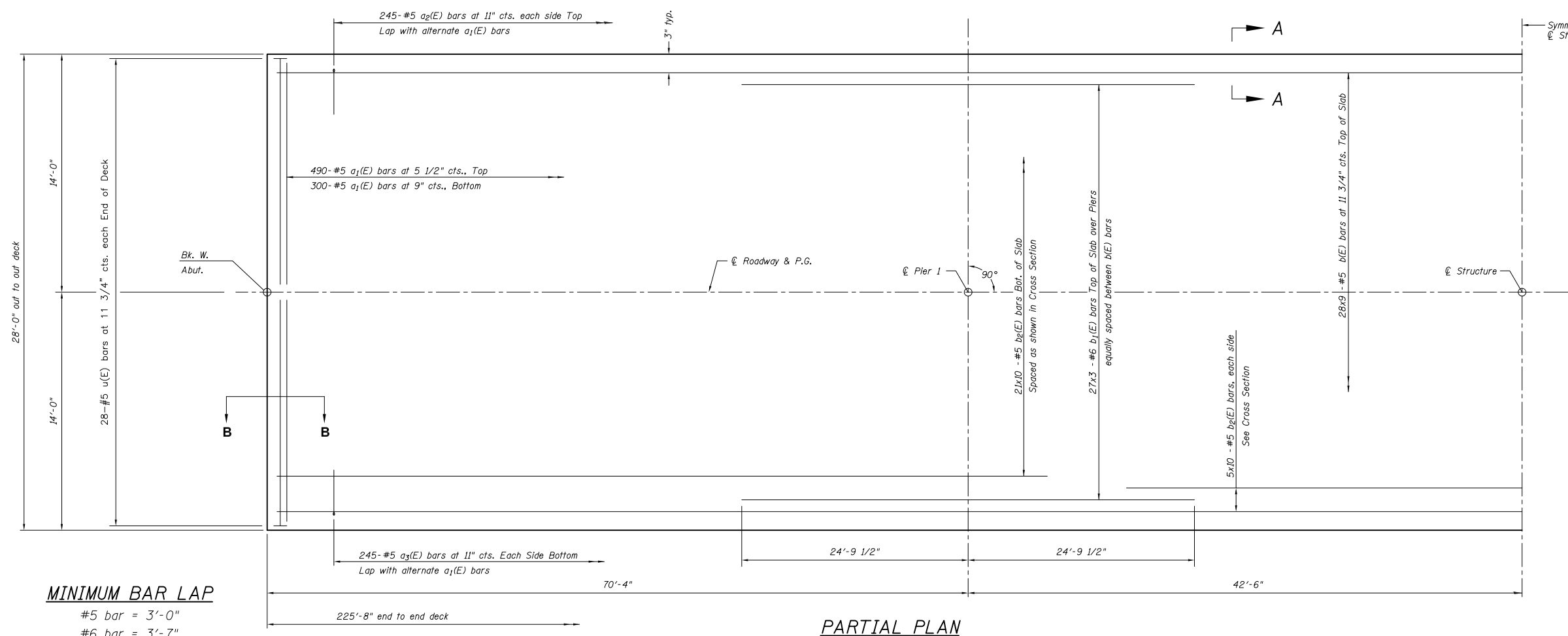
BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

TOP OF DECK ELEVATIONS STRUCTURE NO. 032-3260 SHEET 3 OF 17

CONSTRUCTION PLANS
CURRENT AS OF: 03/28/2017
SCALE: NONE SHEET 13
FILE NO.: 111158.00 Y- OF 46

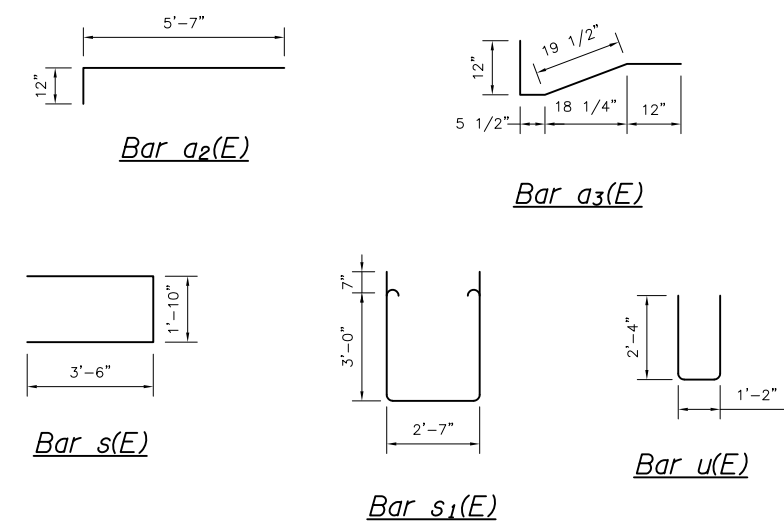
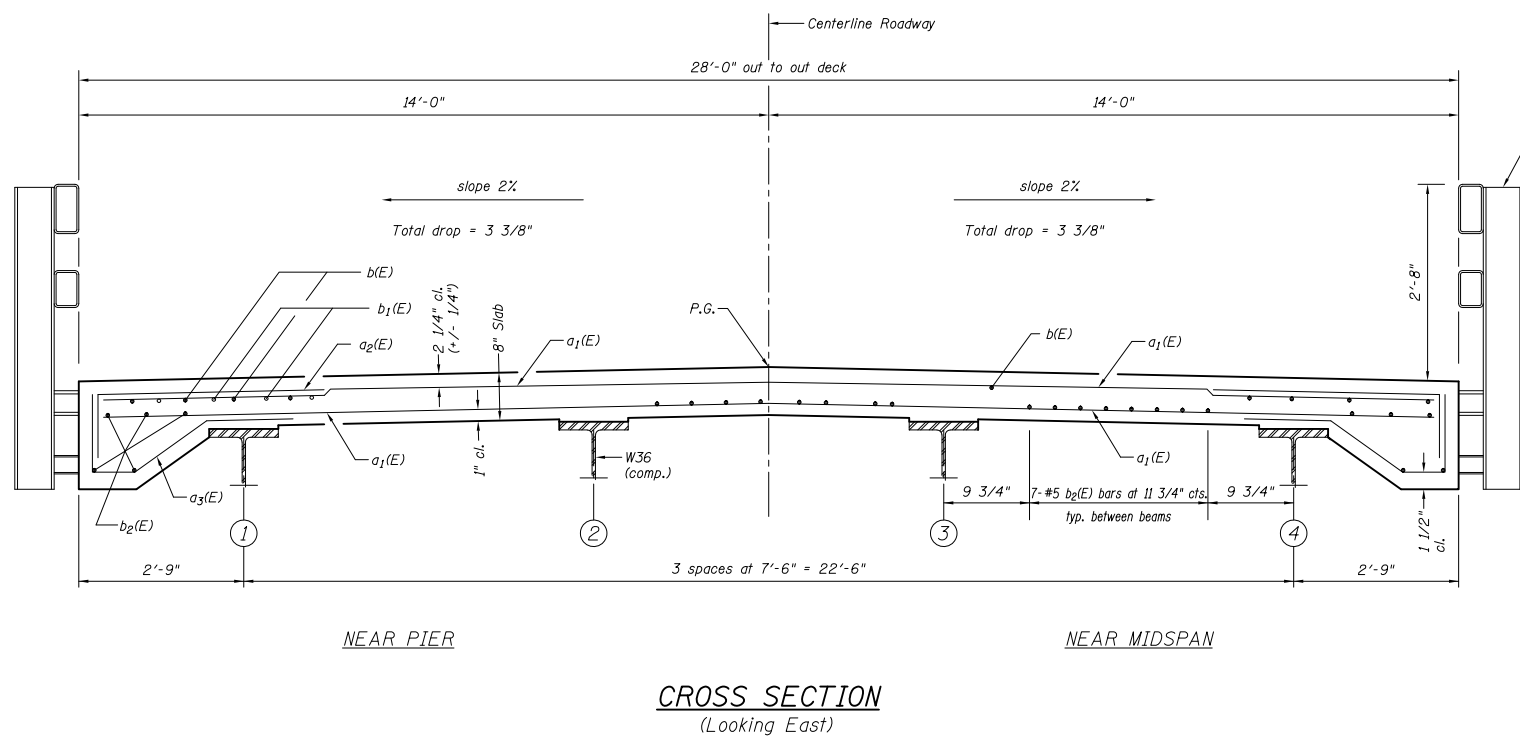
CONTRACT NO. 87642

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**SUPERSTRUCTURE
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a ₁ (E)	790	#5	27'-9"	—
a ₂ (E)	490	#5	6'-7"	—
a ₃ (E)	490	#5	4'-1"	—
b(E)	252	#5	27'-8"	—
b ₁ (E)	162	#6	18'-11"	—
b ₂ (E)	310	#5	25'-2"	—
m(E)	6	#6	27'-9"	—
m ₁ (E)	32	#5	4'-0"	—
m ₂ (E)	24	#6	7'-3"	—
m ₃ (E)	16	#6	2'-6"	—
s(E)	64	#5	8'-10"	—
s ₁ (E)	64	#5	9'-9"	—
u(E)	56	#5	5'-10"	—
Reinforcement Bars, Epoxy Coated			Pound	54180
Concrete Superstructure			Cu. Yds.	179.7



Reinforcement bars in the top of the deck shall be placed with a 1 1/2" minimum clearance in the area of the rail post anchor devices. The studs of the anchor devices shall be placed below the top reinforcement bars and the outermost longitudinal reinforcement bar shall be placed directly above the studs of the rail post anchor device.

Bars indicated thus 6 x 15 - #5 etc. indicates 6 lines of bars with 15 lengths per line.

See sheet 1 of 17 for Rail Post Locations to locate Anchor Device.

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CHECKED BY: JKC					
DATE: 09/2016					

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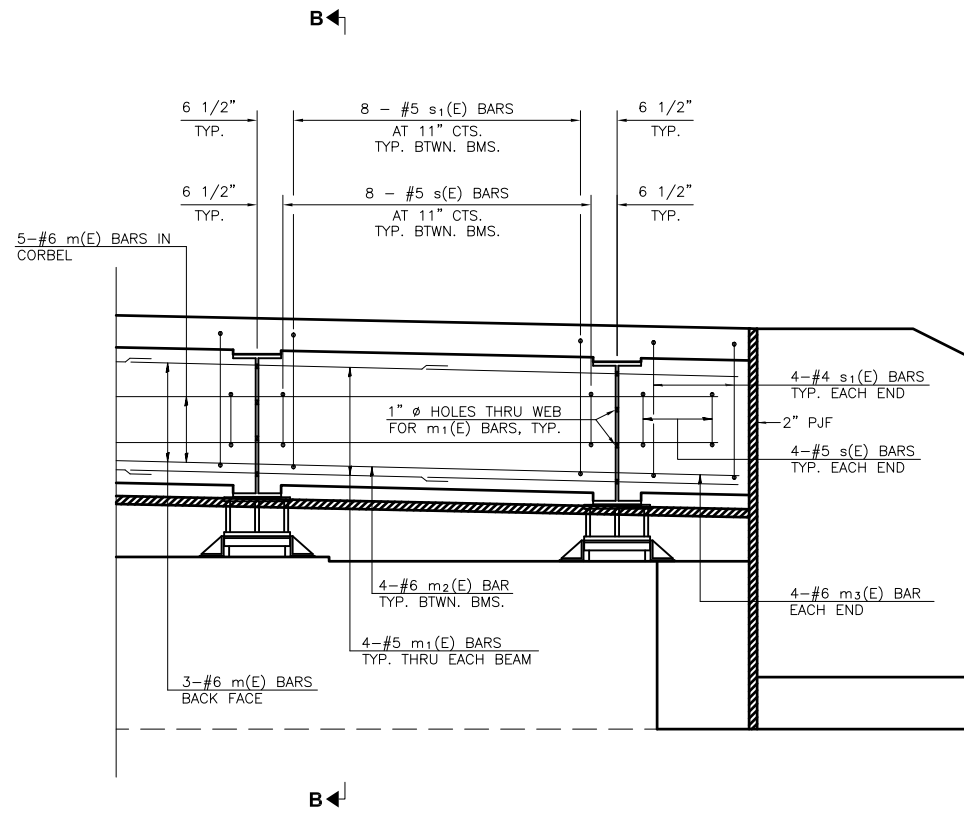
BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

SUPERSTRUCTURE STRUCTURE NO. 032-3260 SHEET 4 OF 17

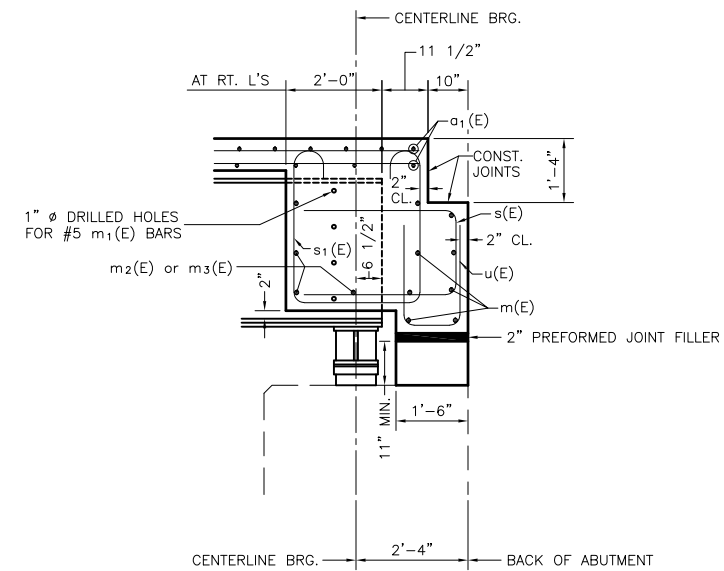
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DIAPHRAGM ELEVATION AT ABUTMENT



SECTION B-B

Dimensions at right angles to abutment, except as shown.

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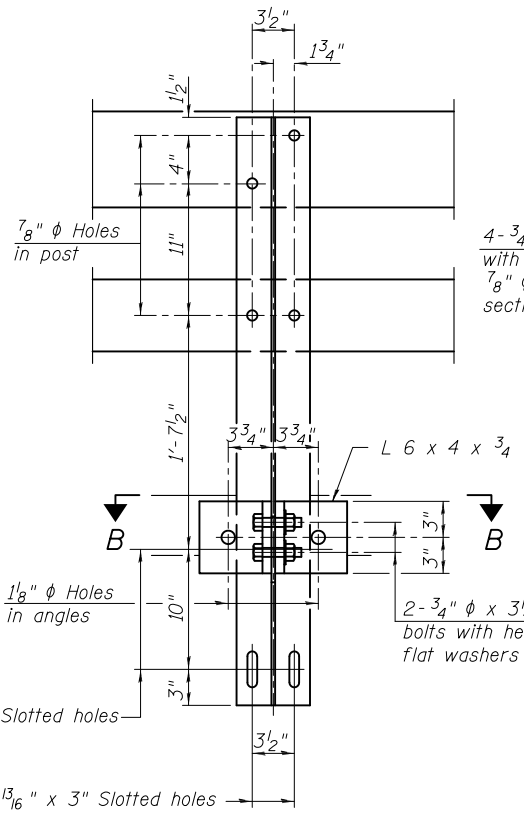

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SECTION 16-00158-00-BR
GRUNDY COUNTY

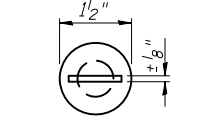
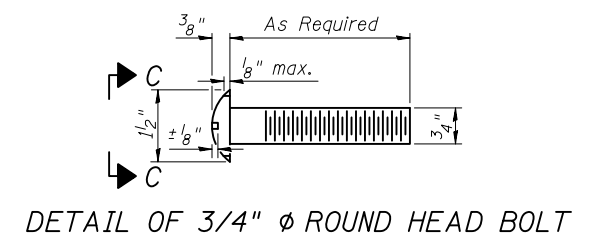
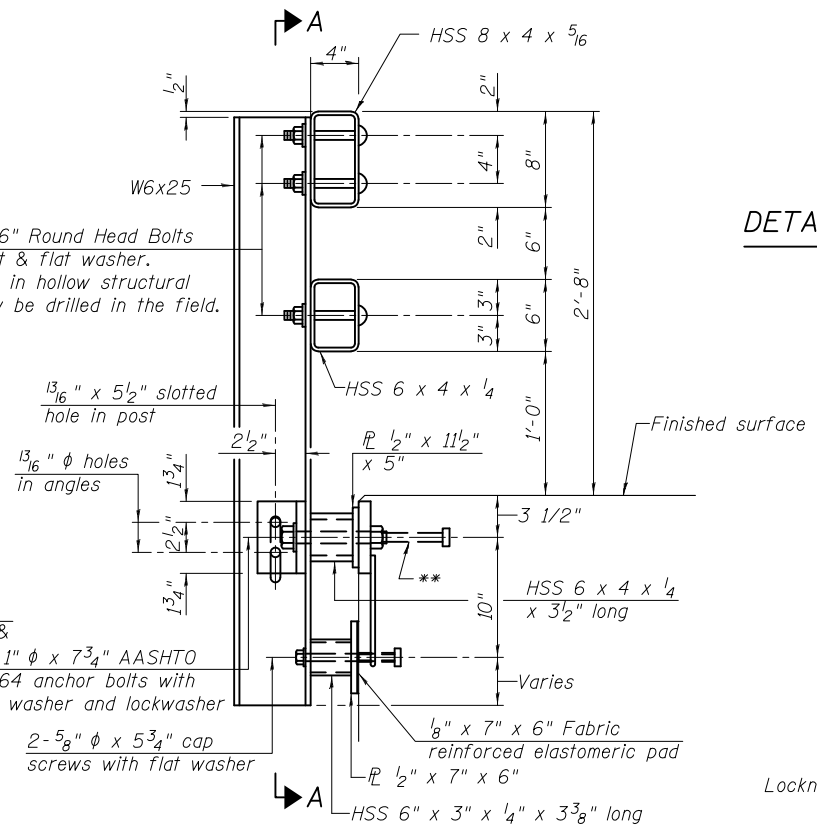
DIAPHRAGM DETAILS
STRUCTURE NO. 032-3260
SHEET 5 OF 17

CONSTRUCTION PLANS
 CONTRACT NO. 87642
 CURRENT AS OF: 03/28/2017
 SCALE: NONE SHEET 15
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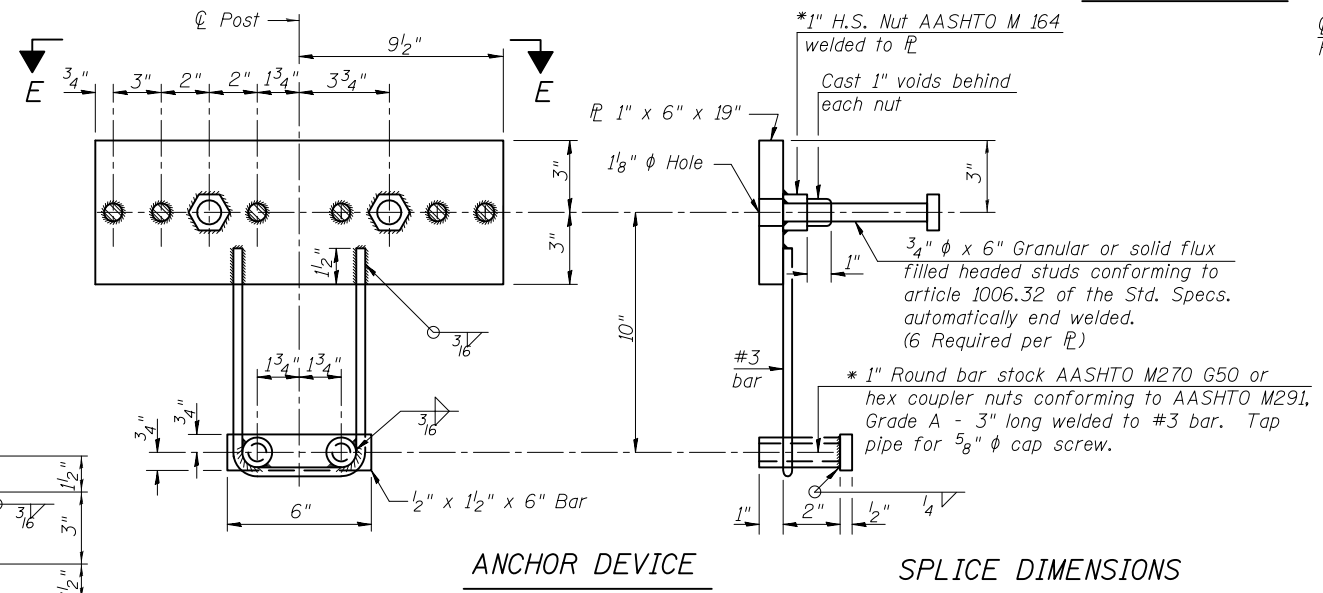
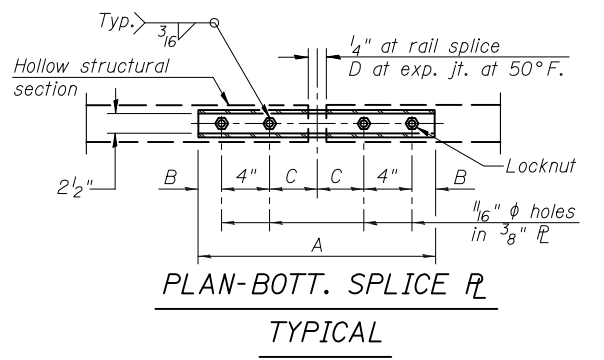
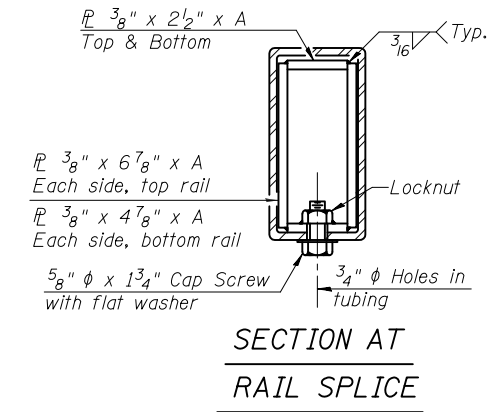
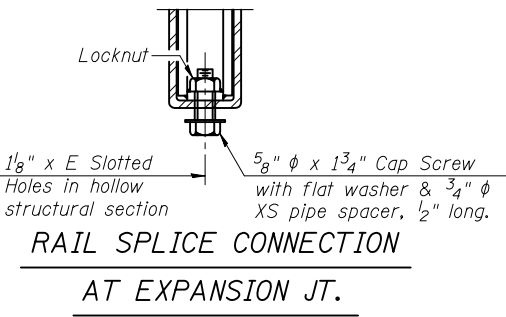
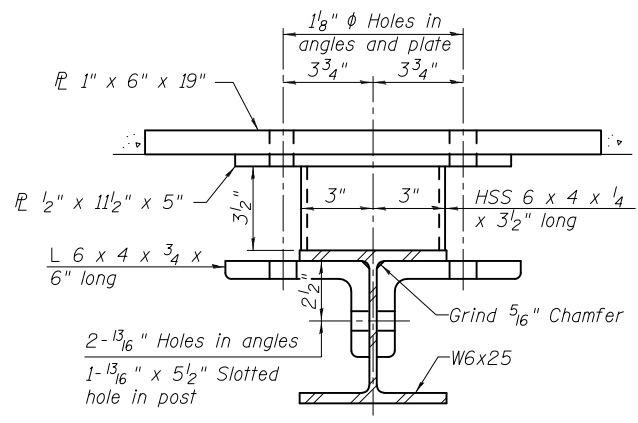
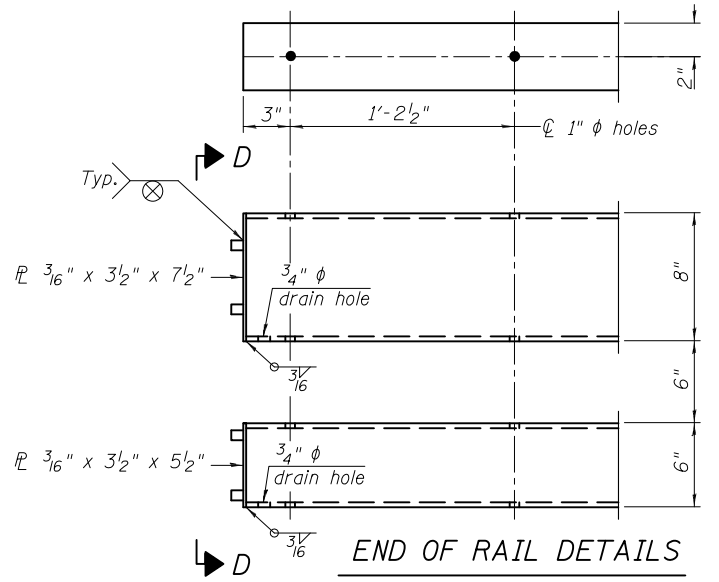
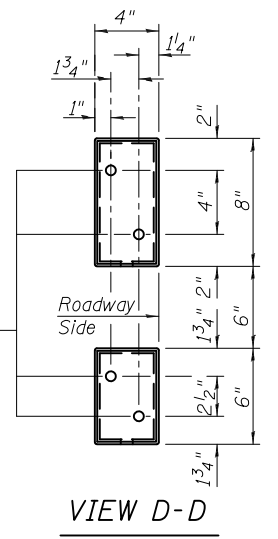
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4-3/4" φ x 6" Round Head Bolts with locknut & flat washer.
 7/8" φ holes in hollow structural section may be drilled in the field.



1/2" x 5/8" reduced base welded studs. Provide 4-5/8" washers and self-locking nuts or nuts and jam nuts for guardrail connection shown on Std. 631032.



*Threaded areas shall be plugged or blocked off during casting of beam. Galvanized after fabrication.

Notes:
 For multi-span bridges, sufficient 1/4" x 6" x 1'-2" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Steel Railing, Type SM.
 Steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.
 ** The studs of the anchor devices shall be placed below the top reinforcement bars and the outermost longitudinal reinforcement bar shall be placed directly above the studs of the rail post anchor device.

BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type SM	Foot	451

REVISIONS	DESCRIPTION

1-12-15 (6'-3" Maximum Post Spacing)

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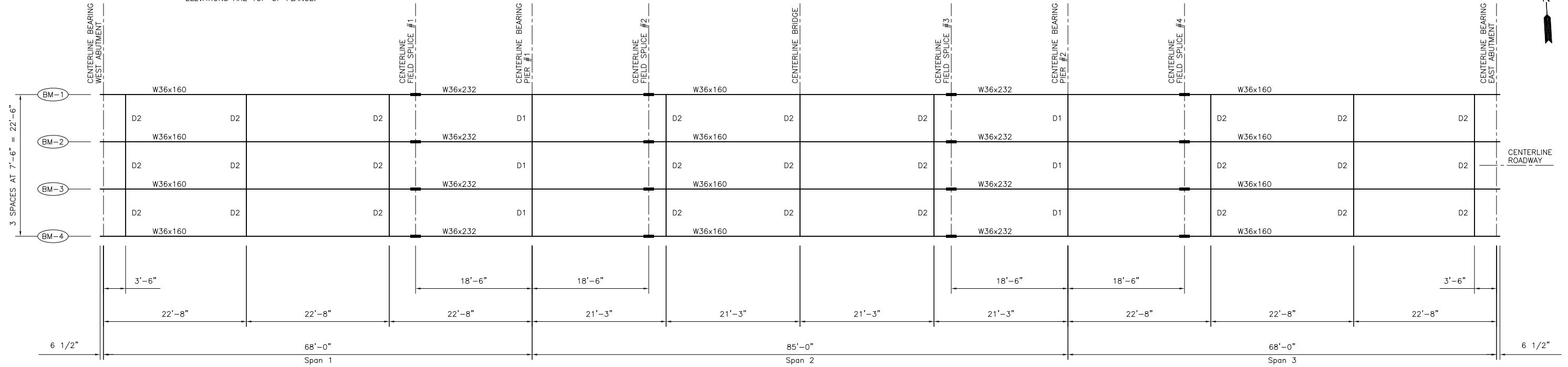
BRACEVILLE ROAD
 SECTION 16-00158-00-BR
 GRUNDY COUNTY

STEEL RAILING, TYPE SM WITH CONCRETE WEARING SURFACE
 STRUCTURE NO. 032-3260
 SHEET 6 OF 17

CONSTRUCTION PLANS
 CURRENT AS OF: 03/28/2017
 SCALE: NONE SHEET 16
 FILE NO.: 111158.00 Y- OF 46

CONTRACT NO. 87642

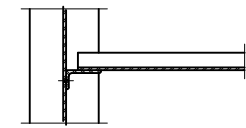
NOTE:
ALL BEAMS ARE SET LEVEL.
ELEVATIONS ARE TOP OF FLANGE.



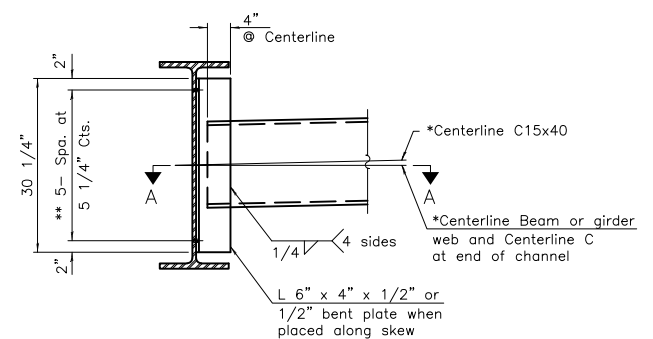
PLAN

- All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
- Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.

TOP OF BEAM ELEVATIONS (For Fabrication Only)												
Beam #	A	B	C	D	E	F	G	H	I	J	K	L
1	565.12	565.03	565.08	565.08	565.08	565.03	565.03	565.08	565.08	565.08	565.03	565.12
2	565.27	565.18	565.23	565.23	565.23	565.18	565.18	565.23	565.23	565.23	565.18	565.27
3	565.27	565.18	565.23	565.23	565.23	565.18	565.18	565.23	565.23	565.23	565.18	565.27
4	565.12	565.03	565.08	565.08	565.08	565.03	565.03	565.08	565.08	565.08	565.03	565.12

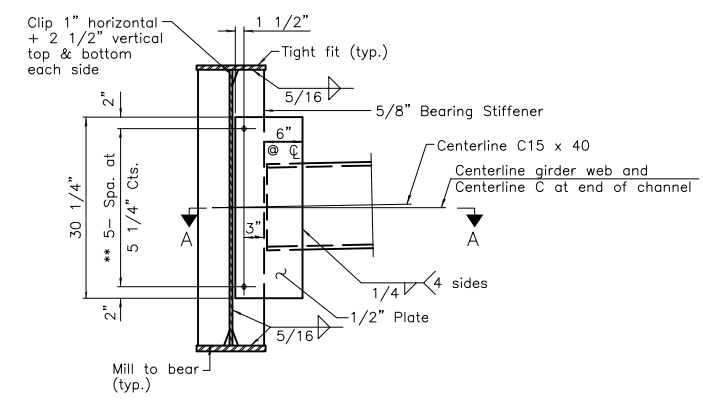


SECTION A-A



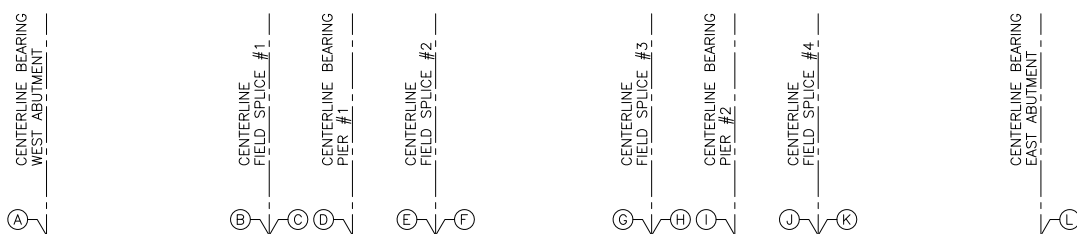
INTERIOR DIAPHRAGM - D2 - (27 REQUIRED)

Note:
Two hardened washers required for each set of oversized holes.
*Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section.
The alternate, if utilized, shall be provided at no additional cost to the Department.
** 3/4" ϕ HS bolts, 15/16" ϕ holes



INTERIOR DIAPHRAGM - D1 (6 REQUIRED)

Note:
Two hardened washers required for each set of oversized holes.
*3/4" ϕ HS bolts, 15/16" ϕ holes.
Alternate channels C15x50 are permitted to facilitate material acquisition. Calculated weight of structural steel is based on C15x40 sections. The alternate, if utilized, shall be provided at no extra cost to the department.



TOP OF BEAM DIAGRAM

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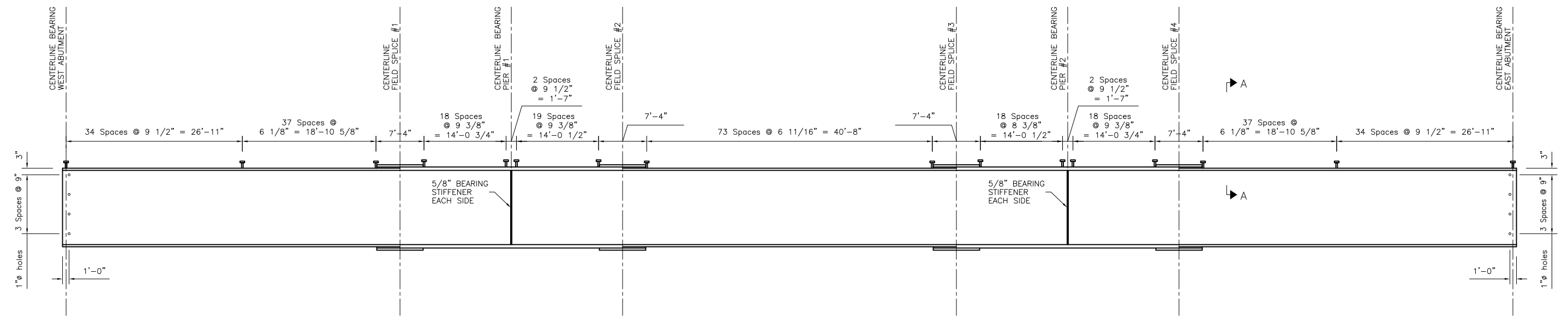
BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

FRAMING PLAN STRUCTURE NO. 032-3260 SHEET 7 OF 17

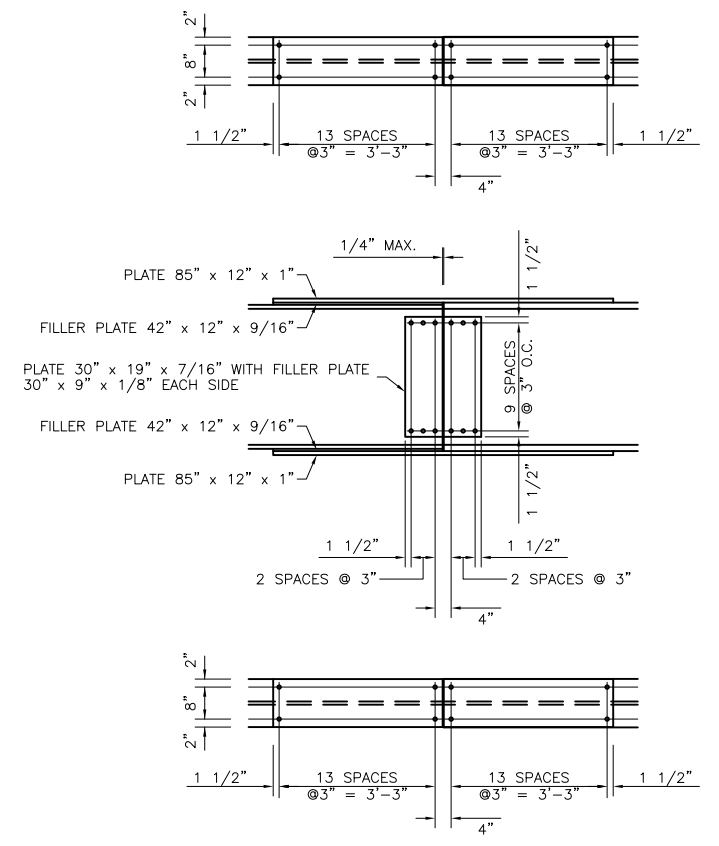
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FILE NO.: 111158.00 Y- OF 46

CONTRACT NO. 87642

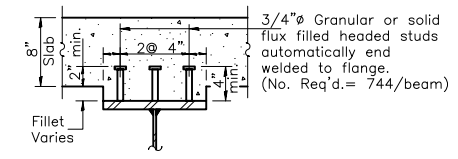
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BEAM ELEVATION



SPLICE #1 - #4 DETAIL



SECTION A-A

* Splice plate material shall be AASHTO M270 GR50W.
 All plates except filler plates shall be "NTR".
 Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.

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DATE: 09/2016		JKC	06/18	Stud spacing

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SECTION 16-00158-00-BR
GRUNDY COUNTY

FRAMING PLAN
STRUCTURE NO. 032-3260
SHEET 8 OF 17

CONSTRUCTION PLANS

CURRENT AS OF: 06/03/2018	
SCALE: NONE	SHEET 18
FILE NO.: 111158.00 Y-	OF 46

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BEAM MOMENT TABLE							
		0.4 Span 1 or 0.6 Span 3		Pier 1 or 2		0.5 Span 2	
		Interior	Exterior	Interior	Exterior	Interior	Exterior
<i>I_s</i>	(in ⁴)	9760	9760	15000	15000	9760	9760
<i>I_s(n)</i>	(in ⁴)	24816	24018	---	---	24816	24018
<i>I_s(3n)</i>	(in ⁴)	18556	17790	---	---	18556	17790
<i>I_s(cr)</i>	(in ⁴)	---	---	18303	17902	---	---
<i>S_s</i>	(in ³)	542.2	542.2	808.8	808.8	542.2	542.2
<i>S_s(n)</i>	(in ³)	769.1	761.4	---	---	769.1	761.4
<i>S_s(3n)</i>	(in ³)	701.5	691.6	---	---	701.5	691.6
<i>S_s(cr)</i>	(in ³)	---	---	880.9	873.0	---	---
<i>DC1</i>	(k')	0.95	0.95	0.95	0.95	0.95	0.95
<i>MDC1</i>	(k)	276.1	276.1	-624.1	-624.1	243.2	243.2
<i>DC2</i>	(k')	0.03	0.03	0.03	0.03	0.03	0.03
<i>MDC2</i>	(k)	8.4	8.4	-17.8	-17.8	7.5	7.5
<i>DW</i>	(k')	0.38	0.32	0.38	0.32	0.38	0.32
<i>MDW</i>	(k)	112.9	97.8	-238.0	-206.4	100.7	87.1
<i>LLDF</i>		0.607	0.840	0.611	0.840	0.572	0.840
<i>MLL + IM</i>	(k)	786.6	1089.5	-964.1	-1326.5	738.1	1084.4
<i>Mu (Strength I)</i>	(k)	1901.5	2409.0	-2846.6	-3433.4	1756.1	2341.7
<i>f Mn</i>	(k)	3955.8	3910.8	---	---	3980.6	3910.8
<i>f_s DC1</i>	(ksi)	6.1	6.1	-9.3	-9.3	5.4	5.4
<i>f_s DC2</i>	(ksi)	0.1	0.1	-0.3	-0.3	0.1	0.1
<i>f_s DW</i>	(ksi)	1.9	1.7	-3.2	-2.8	1.7	1.5
<i>f_s (LL + IM)</i>	(ksi)	12.3	17.2	-13.1	-18.2	11.5	17.1
<i>f_s (Service II)</i>	(ksi)	24.1	30.3	-29.8	-36.1	22.2	29.2
<i>0.95R_yF_y</i>	(ksi)	47.5	47.5	47.5	47.5	47.5	47.5
<i>f_s (Total)(Strength I)</i>	(ksi)	---	---	-39.8	-48.1	---	---
<i>f Fn</i>	(ksi)	---	---	50	50	---	---
<i>Vf</i>	(k)	25.9	33	26.9	34.2	21.9	27.9

BEAM REACTION TABLE					
		West or East Abutment		Pier 1 or 2	
		Interior	Exterior	Interior	Exterior
<i>LLDF</i>		0.779	0.84	0.779	0.84
<i>OCF</i>		---	1.0	---	1.0
<i>RDC1</i>	(k)	23.0	23.0	82.2	82.2
<i>RDC2</i>	(k)	0.7	0.7	2.4	2.4
<i>RDW</i>	(k)	9.2	8.0	32.2	27.9
<i>RLL</i>	(k)	61.9	66.7	104.0	112.2
<i>Rim</i>	(k)	15.3	16.5	20.8	22.3
<i>RTOTAL</i>	(k)	110.1	114.9	241.6	247.0

I_s, S_s: Non-composite moment of inertia and section modulus of the steel section used for computing *f* (Total-Strength I, and Service II) due to non-composite dead loads (in. and in.).

I_c(n), S_c(n): Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing *f_s* (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in. and in.).

I_c(3n), S_c(3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing *f_s* (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in. and in.).

I_c(cr), S_c(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing *f* (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in. and in.).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{LL + IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

$1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{LL + IM}$

f Mn: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft.).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

M_{DC1} / S_c

f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).

$M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.

f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

$M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

f_s (LL + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).

$M_{LL + IM} / S_c(n)$ or $M_{LL + IM} / S_c(cr)$ as applicable.

f_s (Service II): Sum of stresses as computed below (ksi).

$f_{sDC1} + f_{sDC2} + f_{sDW} + 1.3 f_{LL + IM}$

0.95R_yF_y: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).

$1.25 (f_{sDC1} + f_{sDC2}) + 1.5 f_{sDW} + 1.75 f_{LL + IM}$

f_s: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

V_r: Maximum factored shear range in span computed according to Article 6.10.10.

Note:
M_u and *R* include the effects of centrifugal force and superelevation.

CONTRACT NO. 87642

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CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 09/2016				



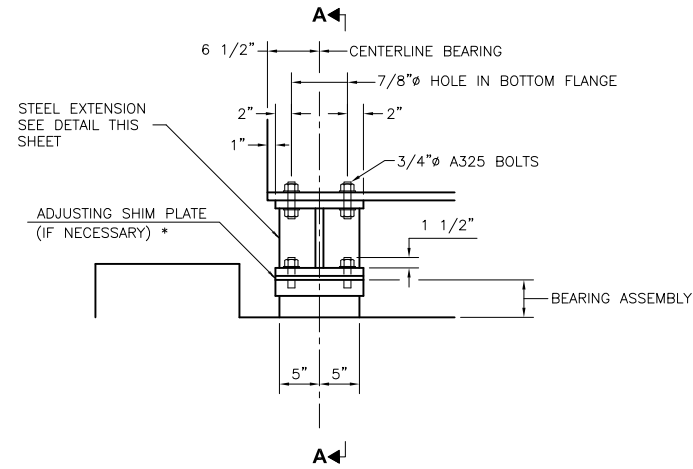
PERU MORRIS
 ILLINOIS

BRACEVILLE ROAD
 SECTION 16-00158-00-BR
 GRUNDY COUNTY

FRAMING PLAN
 STRUCTURE NO. 032-3260
 SHEET 9 OF 17

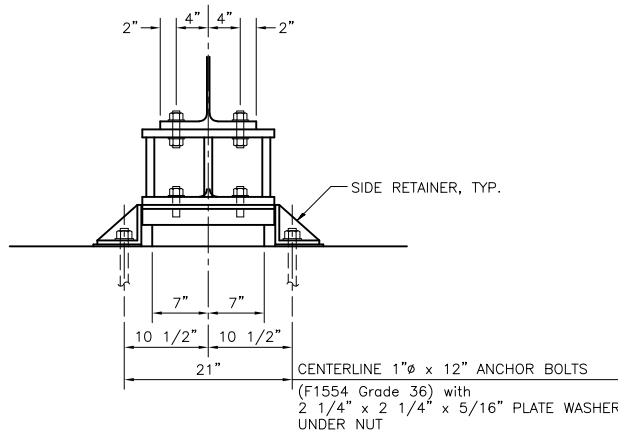
CONSTRUCTION PLANS

CURRENT AS OF: 03/28/2017	
SCALE: NONE	SHEET 19
FILE NO.: 111158.00 Y-	OF 46

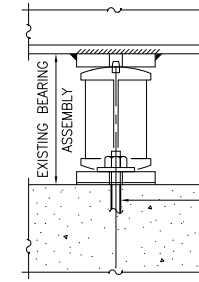


ELEVATION AT WEST ABUT.

* USE 3/4" x 11" x 16 1/2" SHIM AT BEAMS 2 & 3.

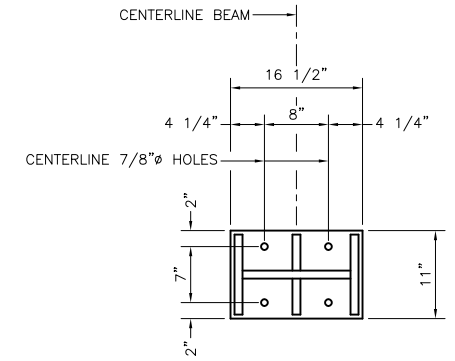


SECTION A-A



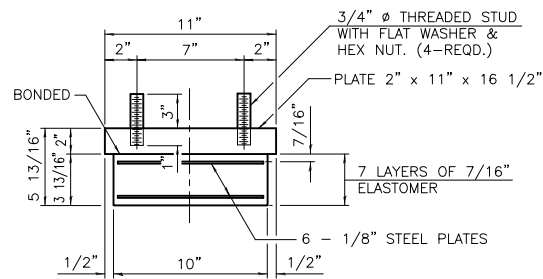
EXISTING BEARING REMOVAL DETAIL

BURN EXISTING ANCHOR BOLTS FLUSH WITH EXISTING CONCRETE SURFACE. GRIND EXISTING ANCHOR BOLT SMOOTH AND SEAL WITH EPOXY. COST IS INCIDENTAL TO "REMOVAL OF EXISTING SUPERSTRUCTURES".



PLAN STEEL EXTENSION

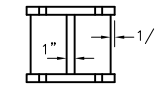
TYPE I ELASTOMERIC EXP. BRG.



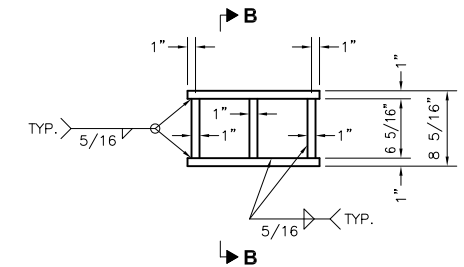
BEARING ASSEMBLY

NOTE:
SHIM PLATES SHALL NOT BE PLACED UNDER BEARING ASSEMBLY.

Notes:
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.
Beams shall be braced for stability during erection and remain braced until deck is poured and cured.
Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
Steel extensions and shims required shall be included in the cost of Furnishing and Erecting Structural Steel.



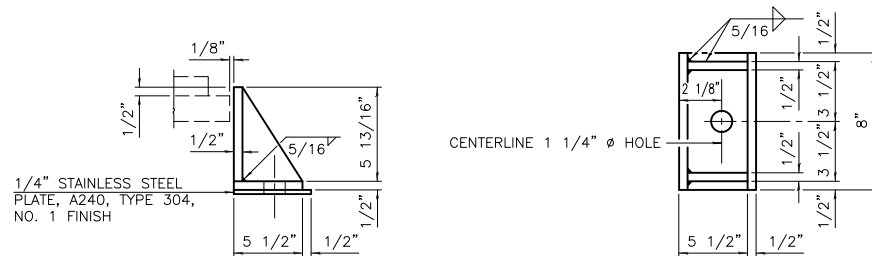
SECTION B-B



ELEVATION STEEL EXTENSION

STEEL EXTENSION DETAILS

4 REQUIRED



SIDE RETAINER

EQUIVALENT ROLLED ANGLE WITH STIFFENERS WILL BE ALLOWED IN LIEU OF WELDED PLATES.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
ELASTOMERIC BEARING ASSEMBLY TYPE I	EACH	4
ANCHOR BOLTS, 1"	EACH	8

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I-2E-1	11-22-2016	REVISIONS	
DRAWN BY: LAG	LEVEL	BY	DATE
CHECKED BY: JKC			
DATE: 09/2016			

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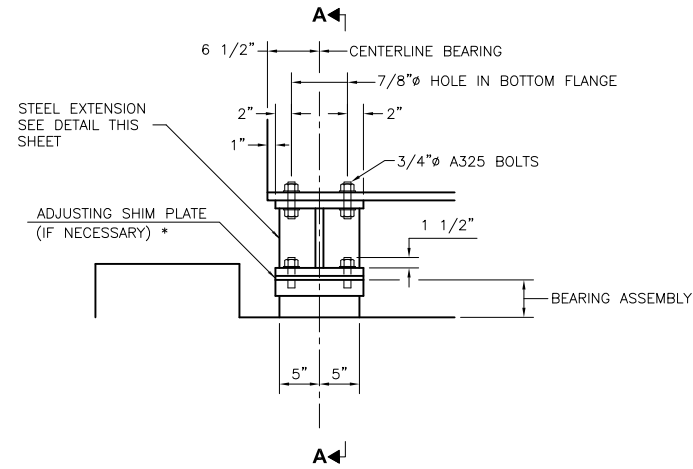
BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

WEST ABUTMENT BEARING DETAILS STRUCTURE NO. 032-3260 SHEET 10 OF 17

CONSTRUCTION PLANS
CURRENT AS OF: 03/28/2017
SCALE: NONE SHEET 20
FILE NO.: 111158.00 Y- OF 46

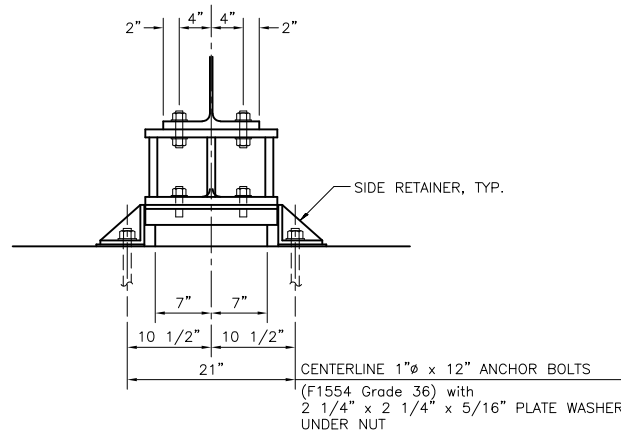
CONTRACT NO. 87642

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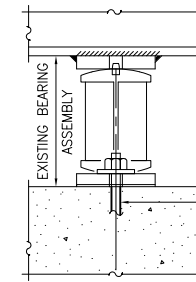


ELEVATION AT EAST ABUT.

* USE 3/4" x 11" x 16 1/2" SHIM AT BEAMS 2 & 3.

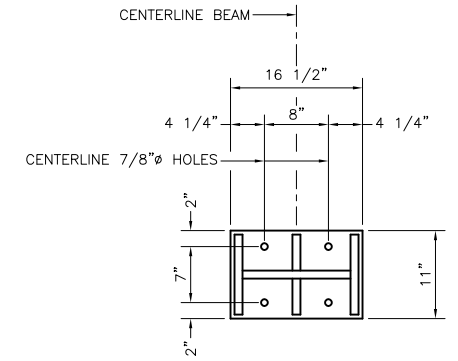


SECTION A-A



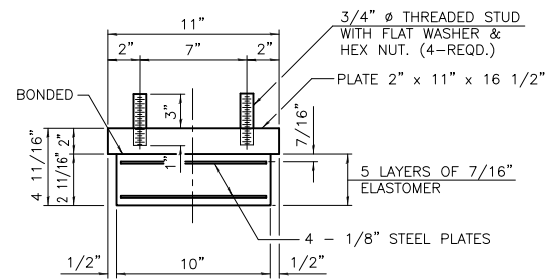
EXISTING BEARING REMOVAL DETAIL

BURN EXISTING ANCHOR BOLTS FLUSH WITH EXISTING CONCRETE SURFACE. GRIND EXISTING ANCHOR BOLT SMOOTH AND SEAL WITH EPOXY. COST IS INCIDENTAL TO "REMOVAL OF EXISTING SUPERSTRUCTURES".



PLAN STEEL EXTENSION

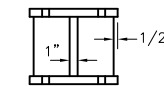
TYPE I ELASTOMERIC EXP. BRG.



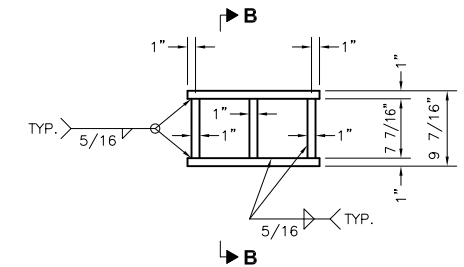
BEARING ASSEMBLY

NOTE:
SHIM PLATES SHALL NOT BE PLACED UNDER BEARING ASSEMBLY.

Notes:
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.
Beams shall be braced for stability during erection and remain braced until deck is poured and cured.
Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
Steel extensions and shims required shall be included in the cost of Furnishing and Erecting Structural Steel.



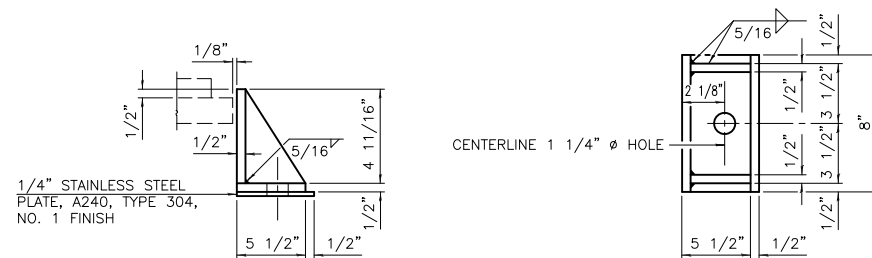
SECTION B-B



ELEVATION STEEL EXTENSION

STEEL EXTENSION DETAILS

4 REQUIRED



SIDE RETAINER

EQUIVALENT ROLLED ANGLE WITH STIFFENERS WILL BE ALLOWED IN LIEU OF WELDED PLATES.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
ELASTOMERIC BEARING ASSEMBLY TYPE I	EACH	4
ANCHOR BOLTS, 1"	EACH	8

I-2E-1

11-22-2016

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	LEVEL	BY	DATE	DESCRIPTION
CHECKED BY: JKC				
DATE: 09/2016				

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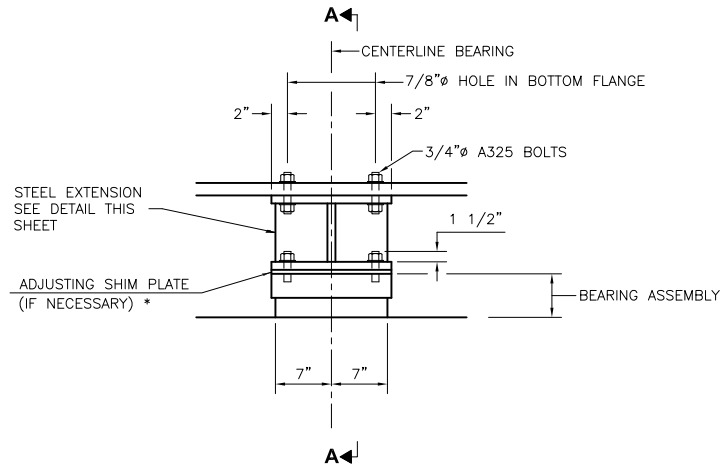
BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

EAST ABUTMENT BEARING DETAILS STRUCTURE NO. 032-3260 SHEET 11 OF 17

CONSTRUCTION PLANS

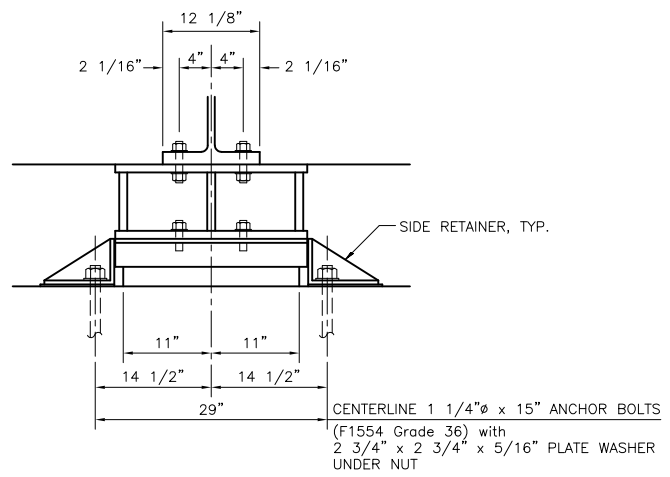
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SCALE: NONE	SHEET 21	
FILE NO.: 111158.00 Y-	OF 46	

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Drawing Name: G:\Users\1111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\C3D\Y... - PLANS\1-PRELIMINARY\020-23 - 010-13 - BEARINGS.dwg Last Modified: Mar 17, 2017 - 3:28pm Plotted on: Mar 28, 2017 - 10:19am by nancy

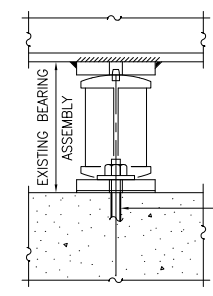


ELEVATION AT PIER 1

* USE 3/4" x 15" x 24" SHIMS AT BEAMS 2 & 3 AT PIER 1.

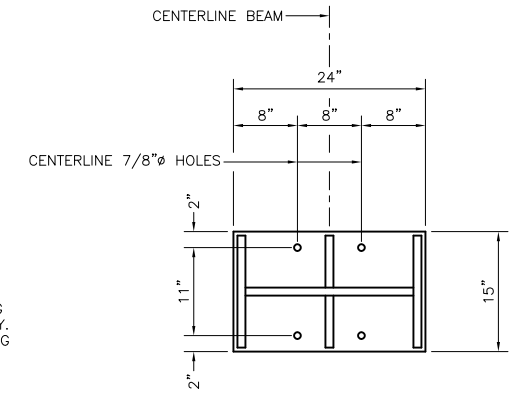


SECTION A-A



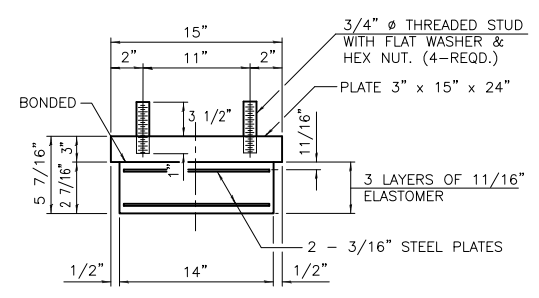
EXISTING BEARING REMOVAL DETAIL

BURN EXISTING ANCHOR BOLTS FLUSH WITH EXISTING CONCRETE SURFACE. GRIND EXISTING ANCHOR BOLT SMOOTH AND SEAL WITH EPOXY. COST IS INCIDENTAL TO "REMOVAL OF EXISTING SUPERSTRUCTURES".



PLAN STEEL EXTENSION

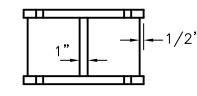
TYPE I ELASTOMERIC EXP. BRG. - PIER 1



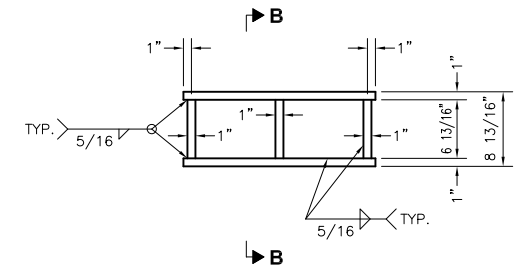
BEARING ASSEMBLY

NOTE:
SHIM PLATES SHALL NOT BE PLACED UNDER BEARING ASSEMBLY.

Notes:
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.
Beams shall be braced for stability during erection and remain braced until deck is poured and cured.
Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
Steel extensions and shims required shall be included in the cost of Furnishing and Erecting Structural Steel.



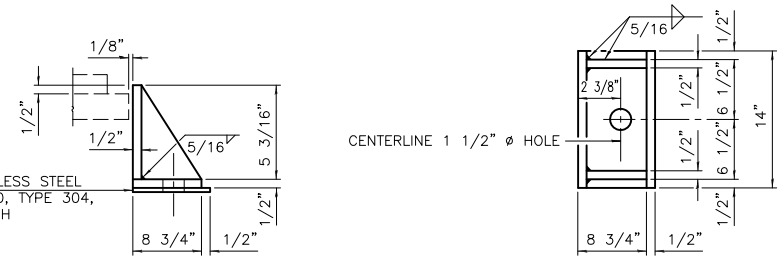
SECTION B-B



ELEVATION STEEL EXTENSION

STEEL EXTENSION DETAILS

4 REQUIRED



SIDE RETAINER

EQUIVALENT ROLLED ANGLE WITH STIFFENERS WILL BE ALLOWED IN LIEU OF WELDED PLATES.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
ELASTOMERIC BEARING ASSEMBLY TYPE I	EACH	4
ANCHOR BOLTS, 1 1/4"	EACH	8

I-2E-1 11-22-2016

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	LEVEL	BY	DATE	DESCRIPTION
CHECKED BY: JKC				
DATE: 09/2016				

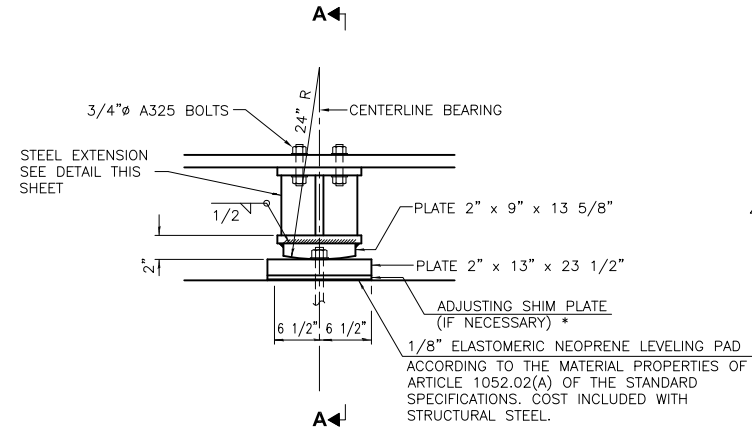
CHAMLIN & ASSOCIATES, INC.
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BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

PIER 1 BEARING DETAILS
STRUCTURE NO. 032-3260
SHEET 12 OF 17

CONSTRUCTION PLANS

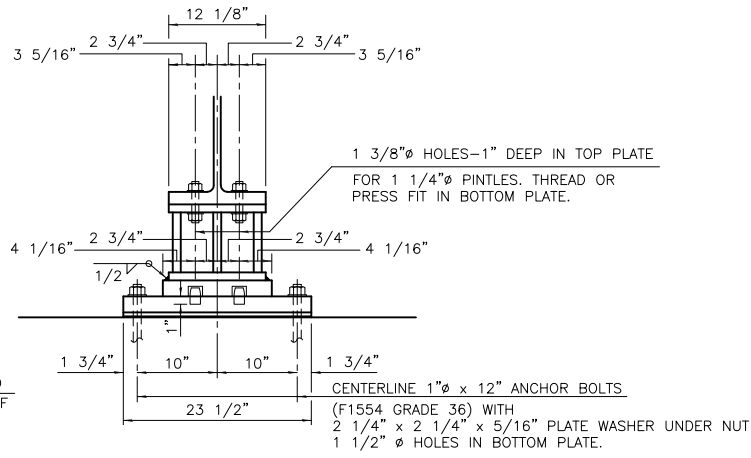
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FILE NO.: 111158.00 Y-	OF 46



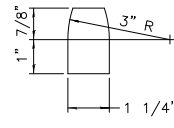
ELEVATION AT PIER 2

FIXED BEARING - PIER 2

- * USE 3/8" x 13" x 23 1/2" SHIMS AT BEAMS 1 & 4 AT PIER 2.
- * USE 1 1/8" x 13" x 23 1/2" SHIMS AT BEAMS 2 & 3 AT PIER 2.

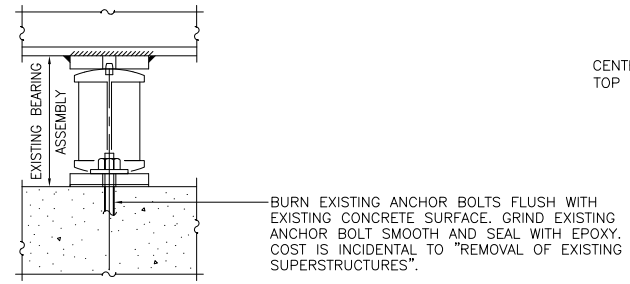


SECTION A-A

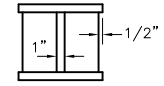


PINTLE

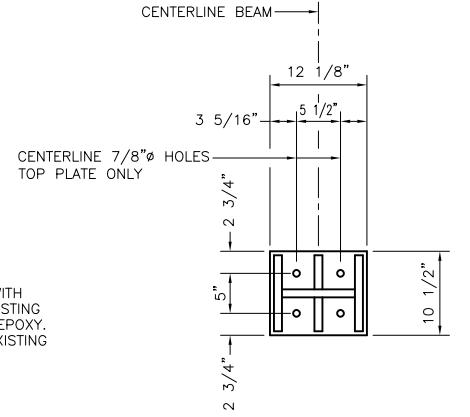
Notes:
 Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
 Beams shall be braced for stability during erection and remain braced until deck is poured and cured.
 Anchor bolts at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
 Steel extensions and shims required shall be included in the cost of Furnishing and Erecting Structural Steel.



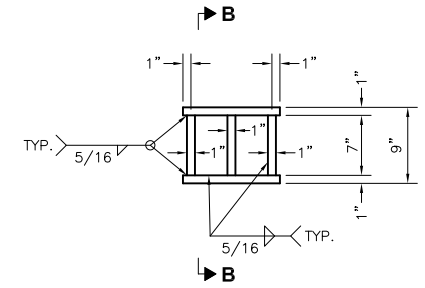
EXISTING BEARING REMOVAL DETAIL



SECTION B-B



PLAN STEEL EXTENSION



ELEVATION STEEL EXTENSION

STEEL EXTENSION DETAILS

4 REQUIRED

BILL OF MATERIAL

ITEM	UNIT	TOTAL
ANCHOR BOLTS, 1"	EACH	8

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I-2E-1	11-22-2016	REVISIONS	
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DATE: 09/2016			

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BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

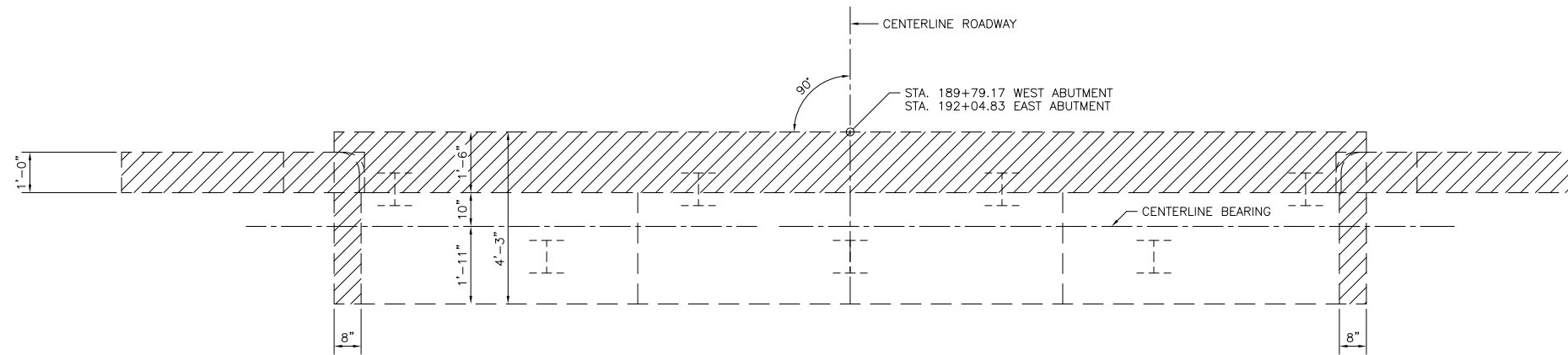
PIER 2 BEARING DETAILS
STRUCTURE NO. 032-3260
SHEET 13 OF 17

CONSTRUCTION PLANS

CURRENT AS OF: 03/28/2017	
SCALE: NONE	SHEET 23
FILE NO.: 111158.00 Y-	OF 46

CONTRACT NO. 87642

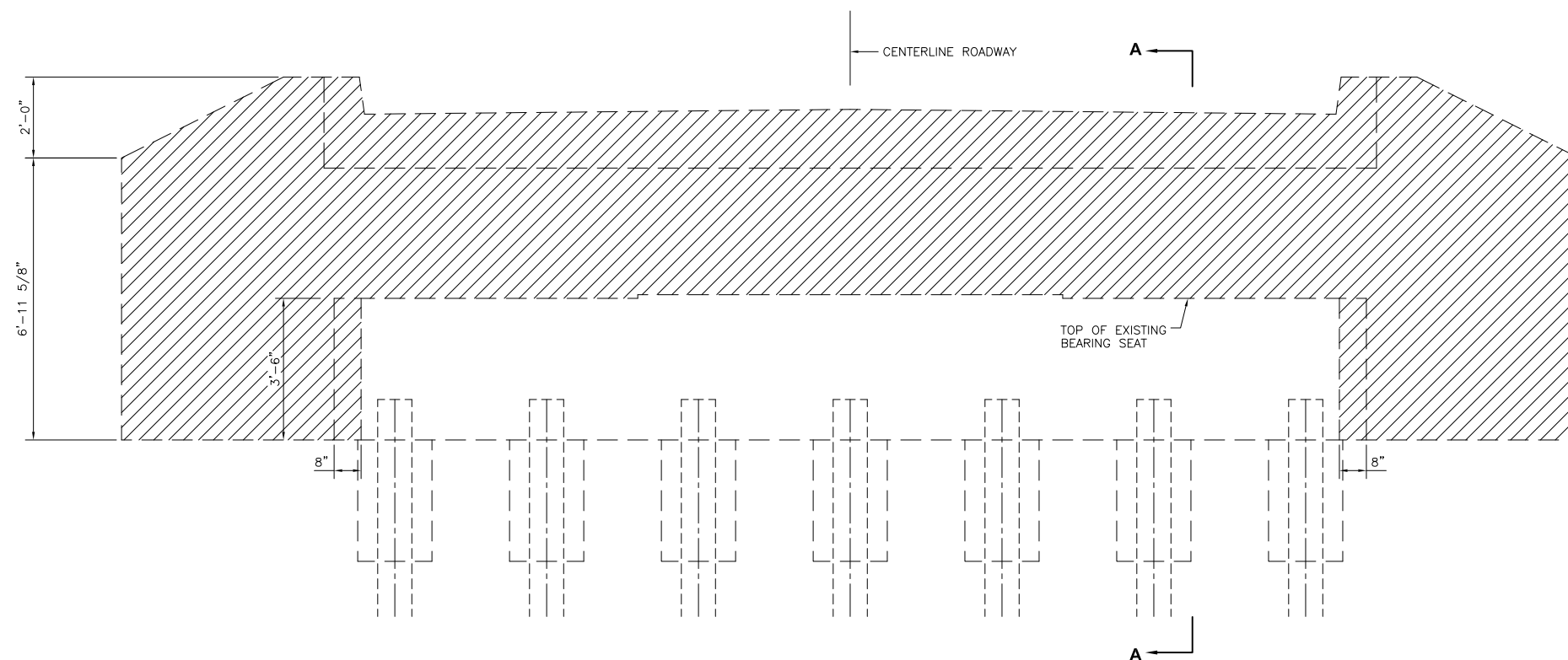
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ABUTMENT PLAN

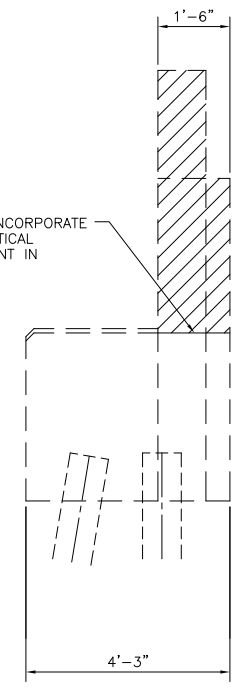
- CONCRETE REMOVAL

SEE SHEET 16 OF 17 FOR
BILL OF MATERIAL



ABUTMENT ELEVATION

CLEAN AND INCORPORATE
EXISTING VERTICAL
REINFORCEMENT IN
NEW WORK



SECTION A-A

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DATE: 09/2016				

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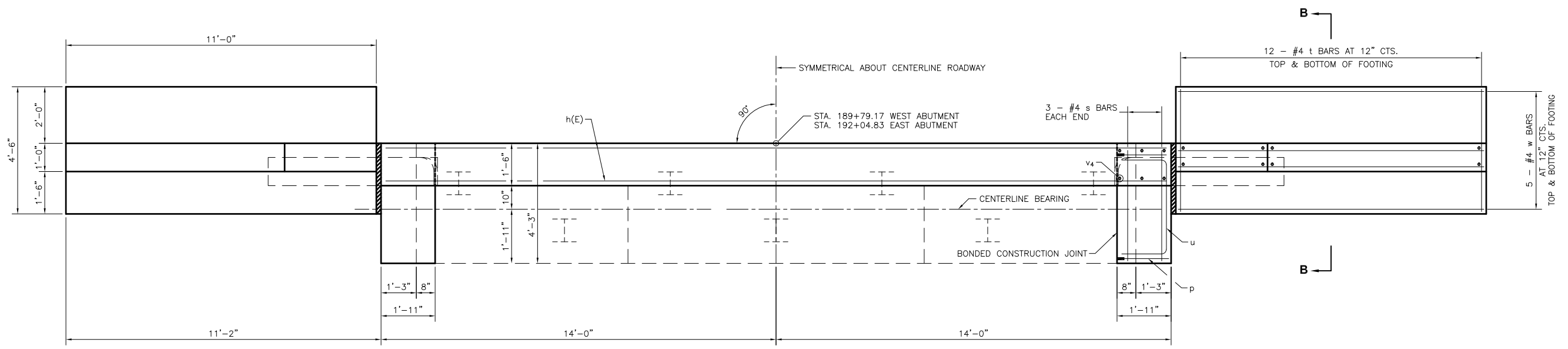
BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

CONCRETE REMOVAL AT ABUTMENTS
STRUCTURE NO. 032-3260
 SHEET 14 OF 17

CONSTRUCTION PLANS

CONTRACT NO. 87642	
CURRENT AS OF: 03/28/2017	
SCALE: NONE	SHEET 24
FILE NO.: 111158.00 Y-	OF 46

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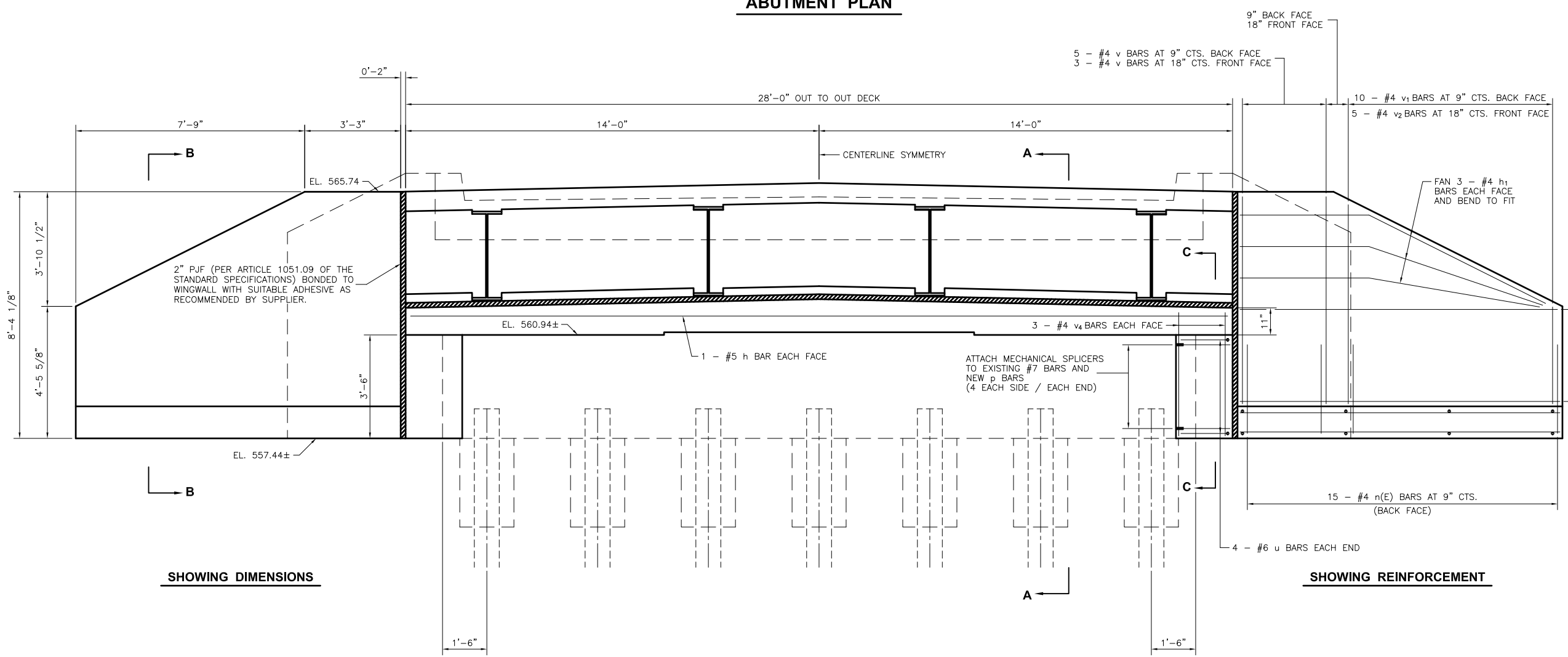


SHOWING DIMENSIONS

ABUTMENT PLAN

SHOWING REINFORCEMENT

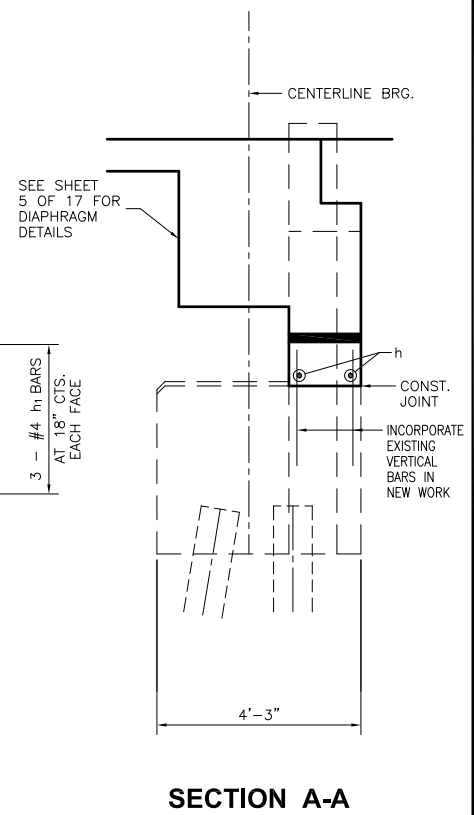
SEE SHEET 16 OF 17 FOR SECTION B-B.
 SEE SHEET 16 OF 17 FOR SECTION C-C.
 SEE SHEET 16 OF 17 FOR BILL OF MATERIAL.
 SEE SHEET 16 OF 17 FOR BENT BAR DETAILS.



SHOWING DIMENSIONS

ABUTMENT ELEVATION

SHOWING REINFORCEMENT



SECTION A-A

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CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 09/2016				

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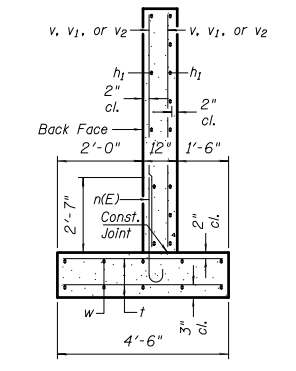
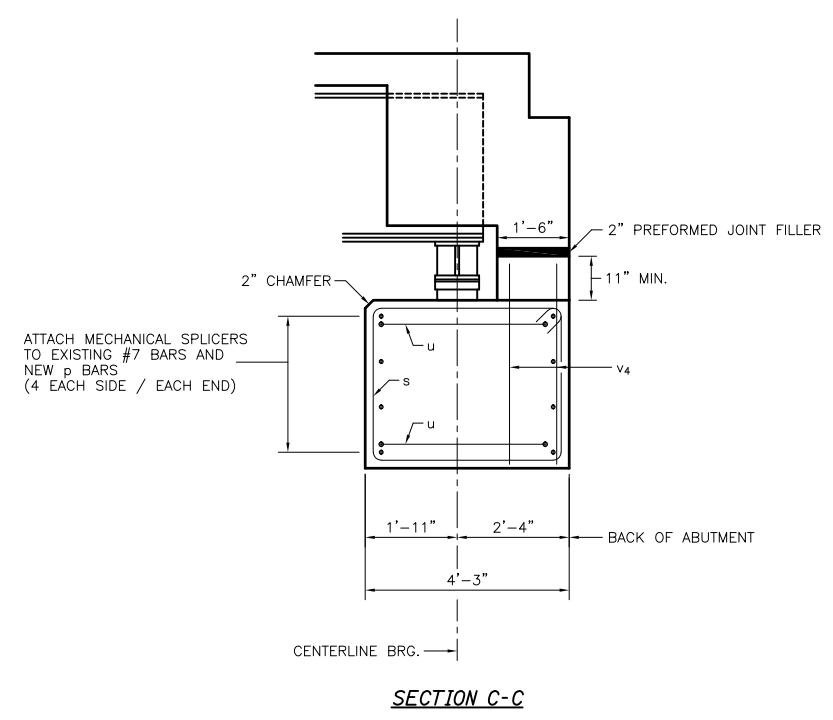
BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

NEW WORK AT ABUTMENTS
STRUCTURE NO. 032-3260
SHEET 15 OF 17

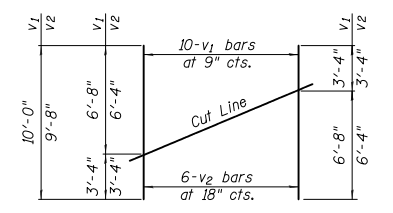
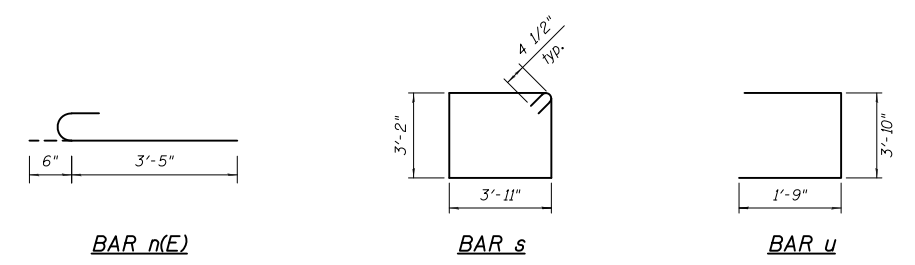
CONSTRUCTION PLANS

CONTRACT NO. 87642	CURRENT AS OF: 03/28/2017
SCALE: NONE	SHEET 25
FILE NO.: 111158.00 Y-	OF 46

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 Drawing Name: G:\Users\1111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\C3D\Y... - PLANS\1-PRELIMINARY\024-26 - 014-16 - ABUTMENTS.dwg Last Modified: Mar 17, 2017 - 3:28pm Plotted on: Mar 28, 2017 - 10:31am by nancy



SECTION B-B
 (Max. Applied Service Bearing Pressure = 1.55 ksf)



FIELD CUTTING DIAGRAM
 Order bars shown full length. Cut as shown and use remainder of bars in opposite wingwall.

**BILL OF MATERIAL
 (TWO ABUTMENTS)**

Bar	No.	Size	Length	Shape
h	4	#5	27'-9"	---
h1	48	#4	10'-9"	---
n(E)	60	#4	3'-11"	---
p	32	#7	1'-9"	---
s	12	#4	14'-11"	□
t	96	#4	4'-3"	---
v	32	#4	7'-11"	---
v1	20	#4	10'-0"	---
v2	10	#4	9'-8"	---
v3	24	#4	4'-2"	---
u	16	#6	7'-4"	---
w	40	#4	10'-9"	---
Concrete Removal				Cu. Yd. 19.1
Concrete Structures				Cu. Yd. 16.2
Structure Excavation				Cu. Yd. 158
Reinforcement Bars				Pound 1830
Reinforcement Bars, Epoxy Coated				Pound 160
Mechanical Splicers				Each 32

DRAWN BY: LAG	REVISIONS			
CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 09/2016				

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**BRACEVILLE ROAD
 SECTION 16-00158-00-BR
 GRUNDY COUNTY**

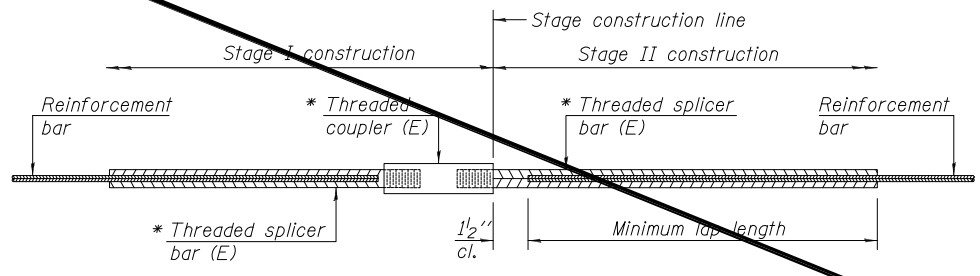
**NEW WORK AT ABUTMENTS
 STRUCTURE NO. 032-3260
 SHEET 16 OF 17**

CONSTRUCTION PLANS

CURRENT AS OF: 03/28/2017	
SCALE: NONE	SHEET 26
FILE NO.: 111158.00 Y-	OF 46

CONTRACT NO. 87642

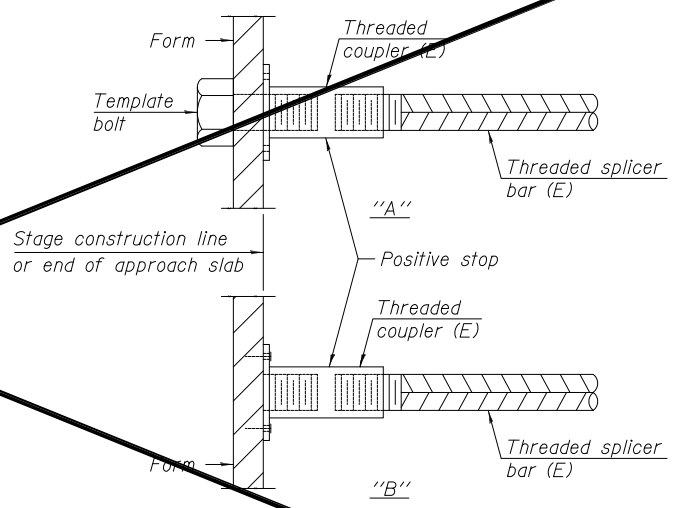
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STANDARD BAR SPLICER ASSEMBLY

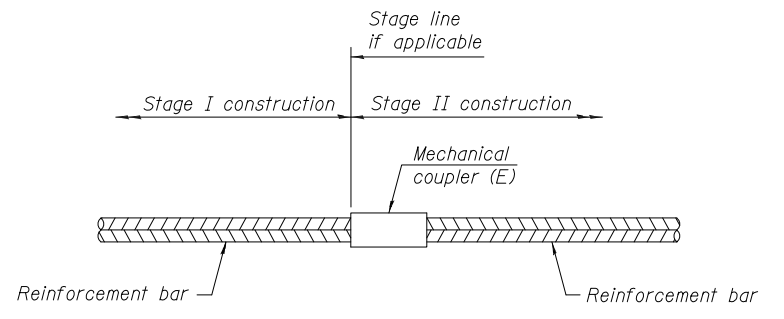
Threaded splicer bar length = min. lap length + 1 1/2" + thread length
 * Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Minimum lap length



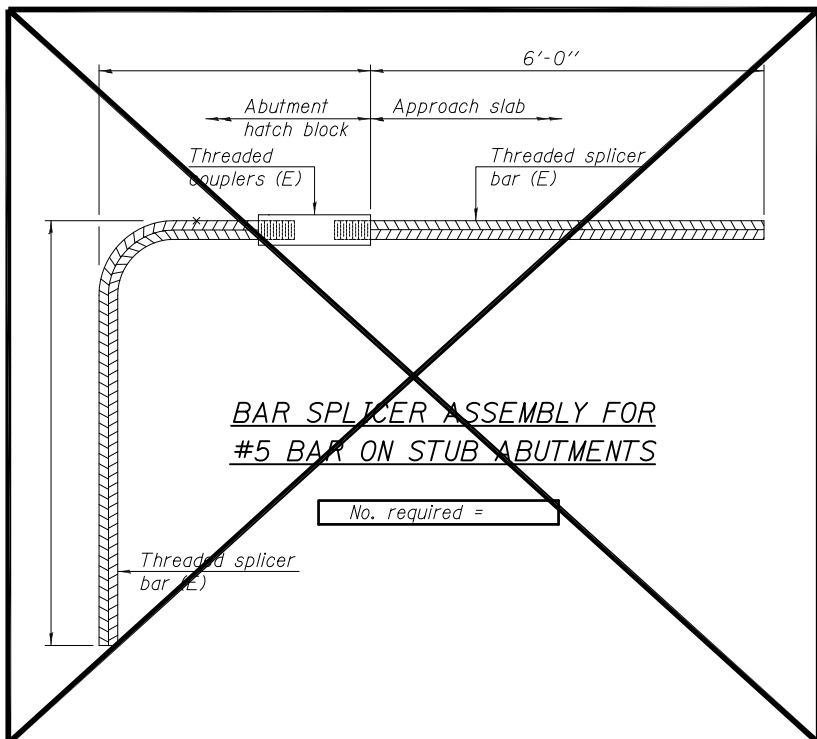
INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
 (E) : Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required
W. ABUT.	#7	16
E. ABUT.	#7	16



BAR SPLICER ASSEMBLY FOR #5 BAR ON STUB ABUTMENTS

No. required =

NOTES

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
 All reinforcement shall be lapped and tied to the splicer bars.
 Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
 See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1	11-22-2016	REVISIONS
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CHECKED BY: JKC	BY	DATE
DATE: 02/2017		

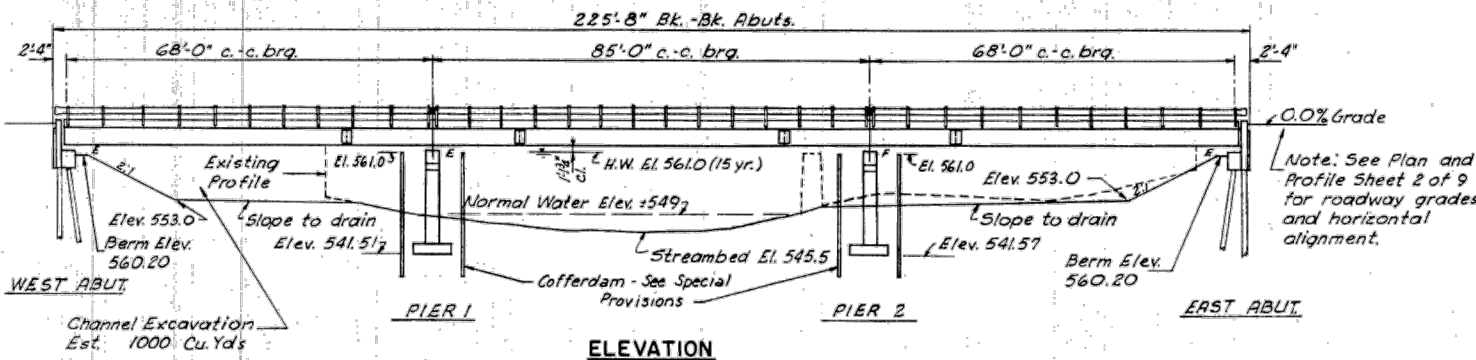
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 CHAMLIN & ASSOCIATES, INC.

BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
STRUCTURE NO. 032-3260
 SHEET 17 OF 17

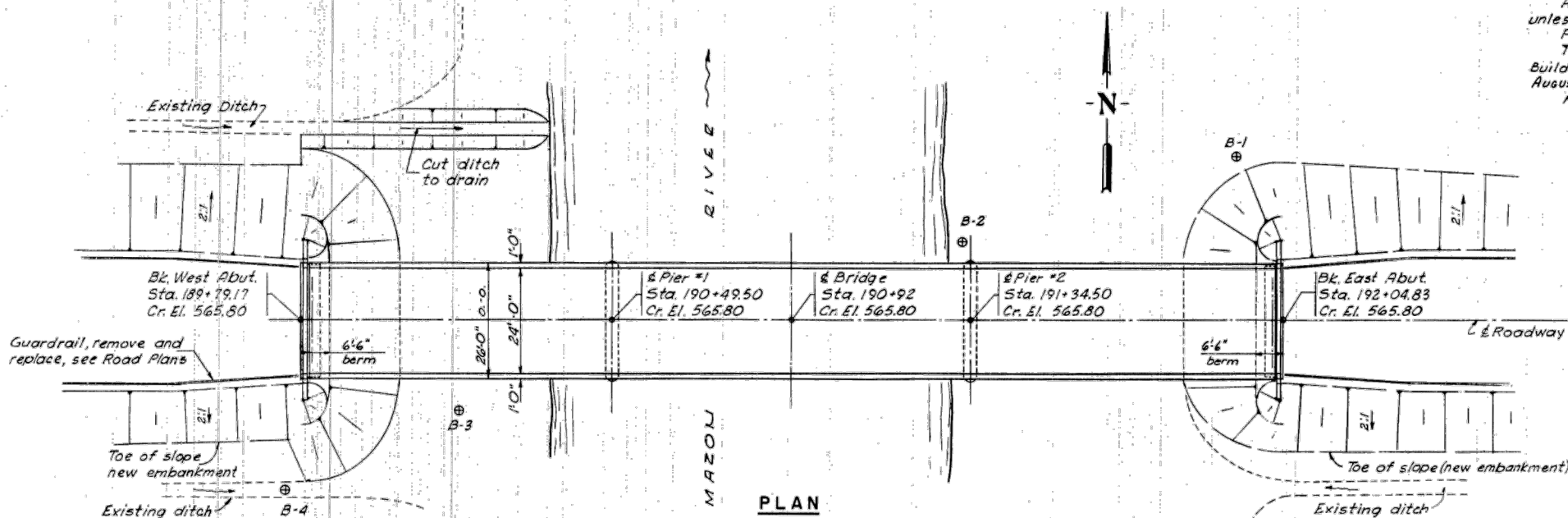
CONSTRUCTION PLANS
 CONTRACT NO. 87642
 CURRENT AS OF: 03/28/2017
 SCALE: AS NOTED SHEET 27
 FILE NO.: 111158.00 Y- OF 46

BM*6-Cut \square Southwest corner of existing bridge curb. Elev. = 563.60
 Existing structure - Steel trusses, 1 \times 90' \square 1 \times 77', I-beam stringers on concrete floor, 15' roadway
 Substructure - Closed concrete West Abut., Stub concrete East Abut., Solid concrete Pier.
 Contractor to remove existing structure before constructing new bridge.
 Salvable steel trusses and floor beams to remain property of Grundy County.



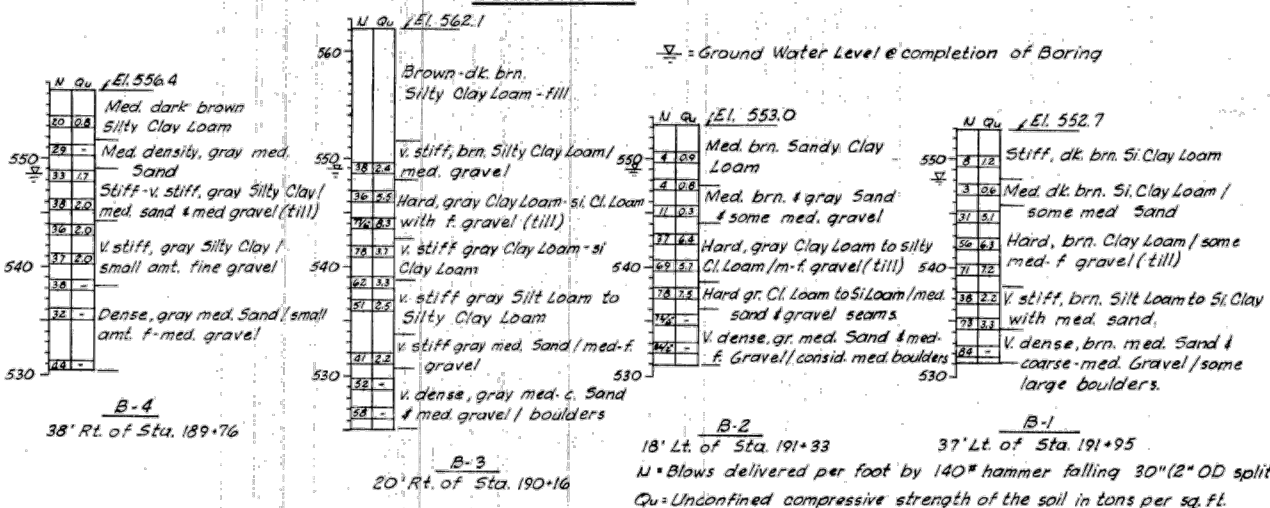
ELEVATION

Note: Maximum footing pressure is 8.0 kips per square foot



PLAN

BORING DATA



Note: Maximum flood of record occurred in 1958 - High water elev. 562.5

WATERWAY DATA

Drainage Area	149,000 acres
Present Opening	1,675 sq. ft.
Required Opening (15yr.)	1,960 sq. ft.
Proposed Opening	1,960 sq. ft.
Design Discharge (Est.)	9,800 c.f.s.

DESIGN STRESSES

$f_c = 1,200$ p.s.i.
 $f_s = 20,000$ p.s.i. (Reinf. #A-36 struct. steel)
 $v = 75$ p.s.i. max. (pier footings)
 $n = 10$
 Allow. LL. $\Delta = \frac{\text{Span}}{7200}$ (Composite)
 Loading HS15-44

GENERAL NOTES

Class X Concrete shall be used in the Abutments and Superstructure.
 Class A Concrete shall be used in the Piers.
 The concrete floor slab shall be finished in accordance with Article 503.16 of the Standard Specifications and shall be poured in one continuous operation between construction joints.
 Rivets shall be $\frac{3}{4}$ " ϕ and open holes shall be $\frac{1}{2}$ " ϕ unless noted.
 High strength steel bolts may be substituted for field rivets.
 Structural Steel shall be ASTM Designation A-36.
 All holes for splices shall be punched $\frac{1}{16}$ " and reamed to proper size ($\frac{1}{16}$ " in web and $\frac{1}{8}$ " in flange) with all stringers assembled in the shop in proper position. Leave assembled in shop for inspection.
 Except as otherwise provided, all structural steel shall receive one (1) shop coat of red lead paint and two (2) field coats of aluminum paint. See Art. 509.01-509.05 inclusive of the Std. Specs.
 For items Metal Plate Bridge Rail and Name Plates, see Special Provisions.
 The Contractor shall drive two (2) steel H piles in permanent locations as directed by the Engineer before ordering the remainder of the piles. Drive one pile at each abutment.
 All reinforcement bars shall be lapped a minimum of 24 diameters at splices, unless otherwise shown.
 Permanent forms will not be permitted in forming the concrete deck.
 The Standard Specifications adopted by the Department of Public Works and Buildings August 1, 1968 and the Supplemental Specifications effective August 1, 1968 shall apply to this work.
 For item Bridge Seat Sealant, see Special Provisions.

STATION 190+92
 SEC. 50-B BUILT 1970
 GRUNDY COUNTY
 LOADING HS 15

LETTERING FOR NAME PLATE

Locate Name Plate at Southwest corner of bridge. See Special Provisions.

Note: Field welding of construction accessories to the bottom flanges or for a distance of $\frac{1}{4}$ of the span each way from the pier supports on the top flanges of beams will not be permitted.
 Field welding in other areas will be permitted only when approved by the engineer.

TOTAL BILL OF MATERIAL

ITEM	SUPER	SUB	TOTAL
Channel Excavation	Cu. Yds.	—	1000
Removal of Existing Structures	Each	—	1
Cofferdam Excavation	Cu. Yds.	152	152
Cofferdam Pier 1	Each	1	1
Cofferdam Pier 2	Each	1	1
Class A Concrete	Cu. Yds.	57.4	57.4
Class X Concrete	Cu. Yds.	155.0	43.7
Protective Coat	Sq. Yds.	688	—
Furnishing & Erecting Structural Steel	Lbs.	150,520	—
Reinforcement Bars	Lbs.	35,080	11,760
Furnishing Steel Piles, 10BP42	Lin. Ft.	—	192
Test Piles Steel, 10BP42	Each	—	2
Driving Steel Piles	Lin. Ft.	—	192
Class X Concrete Encasement	Cu. Yds.	—	5.2
Name Plates	Each	1	—
Metal Plate Bridge Rail	Lin. Ft.	441	—
Bridge Seat Sealant	Lump Sum	—	1

Carl Rebok
 III. Structural #2695



GENERAL PLAN & ELEVATION

MAZON RIVER BRIDGE
 COUNTY HWY. 9 SEC. 50-B
 GRUNDY COUNTY
 STATION 190+92

Oglesby, Rebok & Bartolomucci
 CONSULTING CIVIL & STRUCTURAL ENGINEERS
 LAND SURVEYORS
 808 W. CANEY SPRINGFIELD, ILLINOIS
 DESIGNED CR DRAWN T.E.B. DATE 3-6-68
 CHECKED REO CHECKED CR JOB NO. 67-45

Revised by CR 4-15-68

CONTRACT NO. 87642

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 Drawing Name: G:\Users\T111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\C3D\Y- - PLAN1-1-PRELIMINARY\028-032 - EXISTING BRIDGE PLANS.dwg Last Modified: Mar. 17, 2017 - 3:32pm Plotted on: Mar. 28, 2017 - 10:42am by nancy

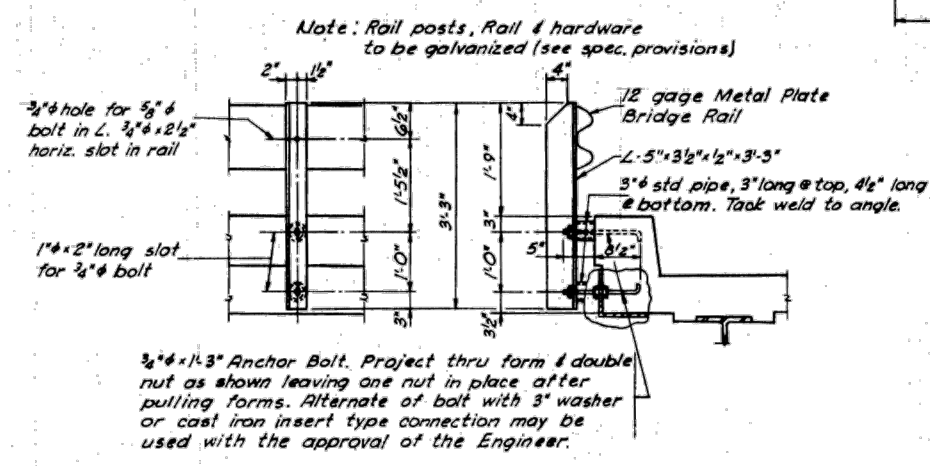
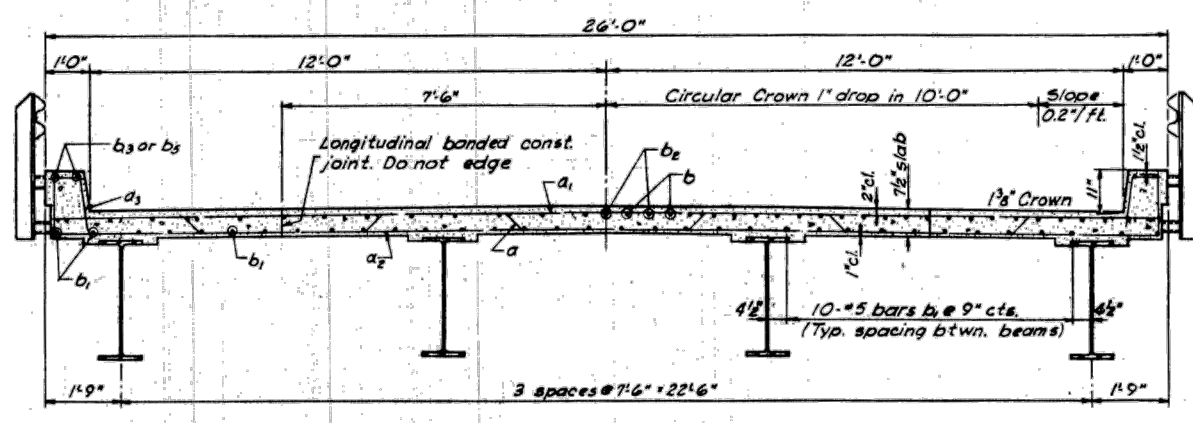
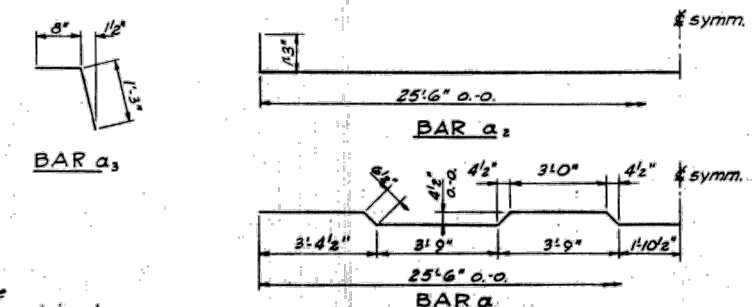
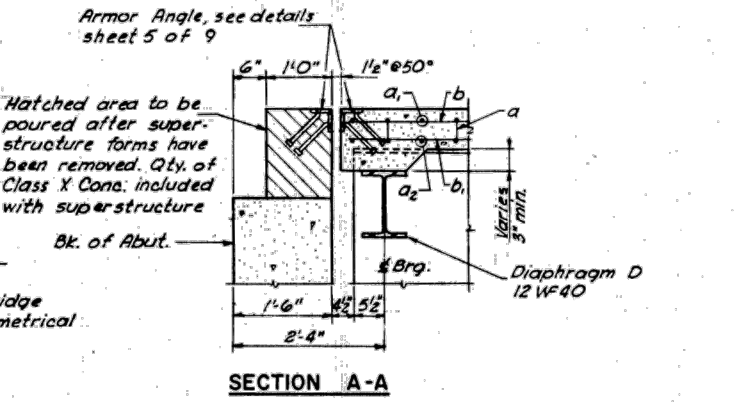
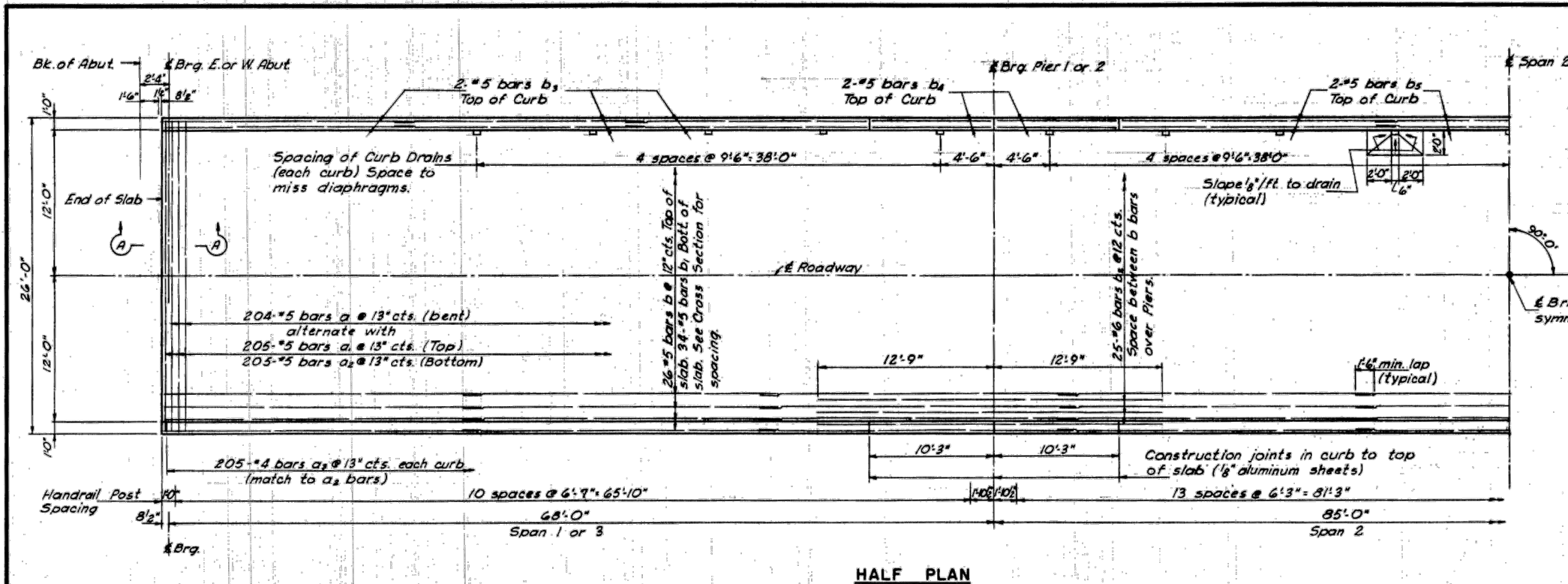
LEVEL	BY	DATE	DESCRIPTION

CHAMLIN & ASSOCIATES, INC.
 PERU MORRIS
 ILLINOIS

BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

EXISTING BRIDGE PLANS
STRUCTURE NO. 032-3260

CONSTRUCTION PLANS	CURRENT AS OF: 03/28/2017	SHEET 28
SCALE: AS NOTED	FILE NO.: 111158.00 Y-	OF 46



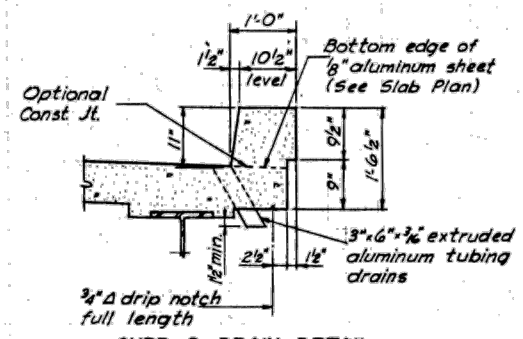
BILL OF MATERIAL - SUPERSTRUCTURE

BAR NO.	NO.	SIZE	LENGTH	SHAPE
a	204	#5	26'-6"	—
a1	205	#5	25'-6"	—
a2	205	#5	28'-0"	—
a3	410	#4	1'-11"	—
b	234	#5	26'-0"	—
b1	238	#5	33'-3"	—
b2	50	#6	25'-6"	—
b3	24	#5	20'-5"	—
b4	16	#5	10'-0"	—
b5	12	#5	22'-6"	—

Class X Concrete Cu Yds. 155.0
 Reinforcement Bars Lbs. 35,080
 * Furnishing & Erecting Structural Steel Lbs. 150,320
 Metal Plate Bridge Rail Lin. Ft. 441
 * Weight of bearing assemblies with lead plates, anchor bolts and armor angles are included as structural steel. Est. Wt. = 5560 lbs.

METHOD OF DETERMINING FILLET HEIGHTS "t"
 After all Structural Steel has been erected, elevations of the top flanges of the beams shall be taken at intervals not to exceed 10 ft. From these elevations subtract the increment of deflection for these points, determined from the Dead Load Deflection diagram. The elevations so obtained subtracted from the theoretical grade elevations, minus floor thickness, equals the fillet heights above top of beam.

D.L. DEFLECTION DIAGRAM
 Weight of slab only
 Note: D.L. Diagram is for all beams



SUPERSTRUCTURE
Mazon River Bridge
 COUNTY HWY. 9 SEC. 50-B
 GRUNDY COUNTY
 STATION 190+92
 Oglesby, Rebok & Bartolomucci
 CONSULTING CIVIL & STRUCTURAL ENGINEERS
 LAND SURVEYORS
 506 W. GANEY SPRINGFIELD, ILLINOIS
 DESIGNED CR DRAWN TEB DATE 3-6-68
 CHECKED REO CHECKED CR JOB NO. 67-45
 Revised by CR 4-15-68

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LEVEL	BY	DATE	DESCRIPTION

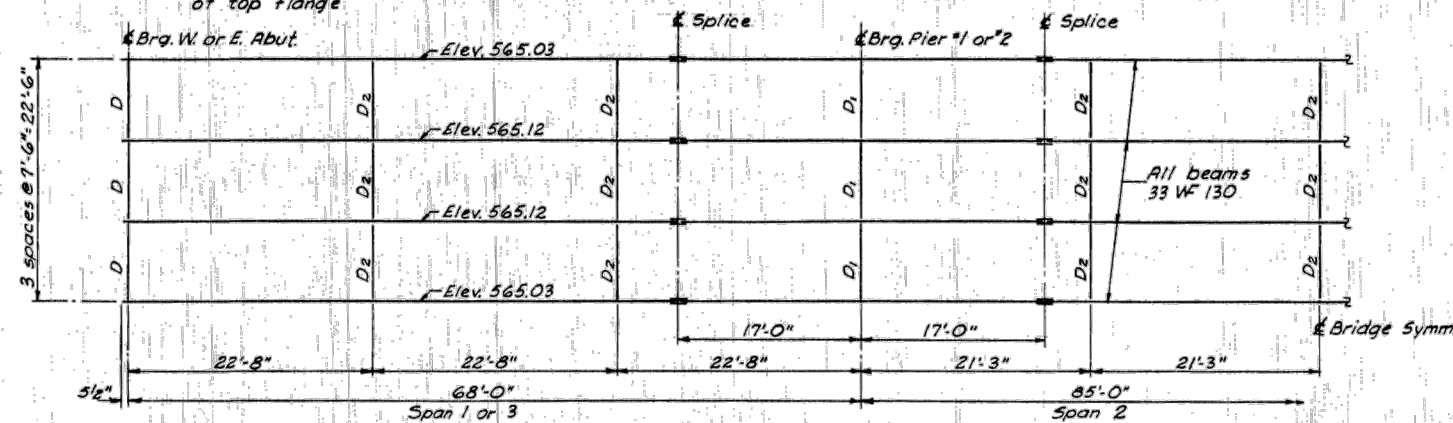
CHAMLIN & ASSOCIATES, INC.
 PERU MORRIS ILLINOIS

BRACEVILLE ROAD
 SECTION 16-00158-00-BR
 GRUNDY COUNTY

EXISTING BRIDGE PLANS
 STRUCTURE NO. 032-3260

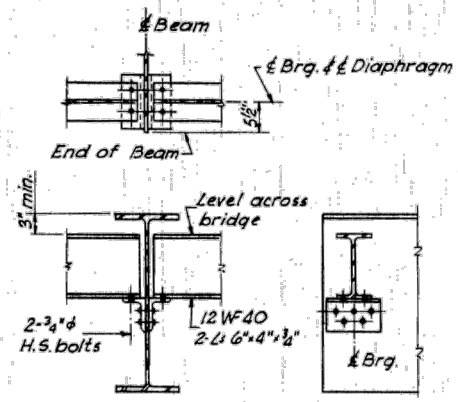
CONSTRUCTION PLANS
 CURRENT AS OF: 03/28/2017
 SCALE: AS NOTED SHEET 29
 FILE NO.: 111158.00 Y- OF 46

Note:
All beams are set level. Elevations are top of top flange

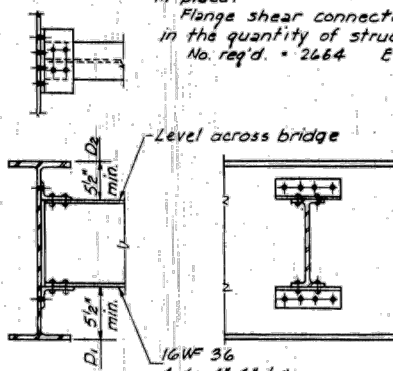


HALF PLAN - STRUCTURAL STEEL

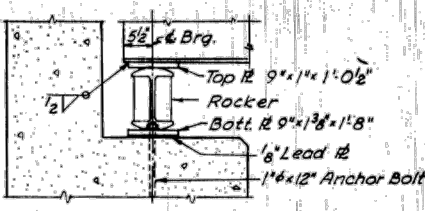
Note:
Stud shear connectors on the beam flanges shall be placed in the field after the steel has been erected and the deck forms are in place.
Flange shear connectors are included in the quantity of structural steel.
No. req'd. = 2664 Est. wt. 1970 lbs.



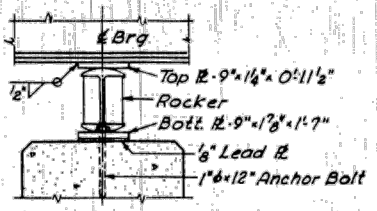
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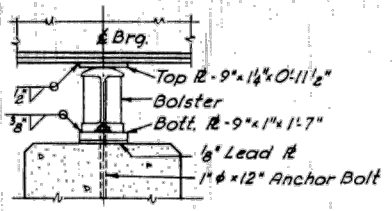
DIAPHRAGMS D₁ & D₂



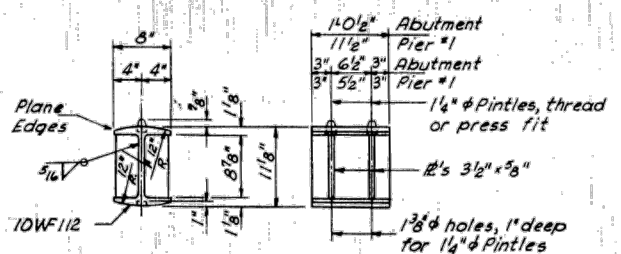
ELEVATION



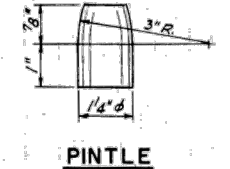
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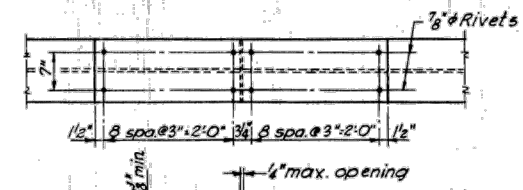
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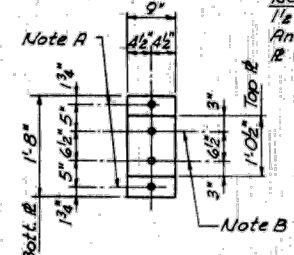
ROCKER DETAILS



PINTELE

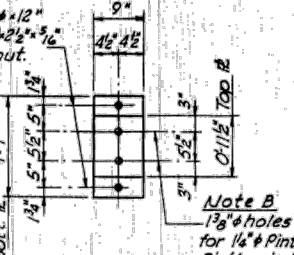


SPLICE DETAIL



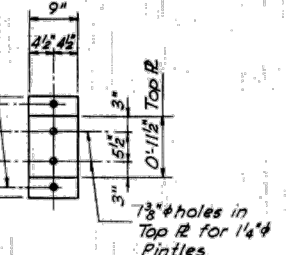
PLAN

Note A
1/2" diameter holes for 1" x 12" anchor bolts. 2 1/2" x 2 1/2" x 5/16" R washer under nut.

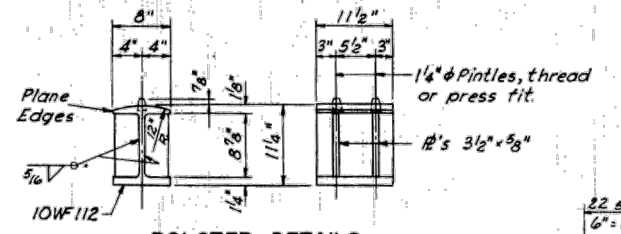


PLAN

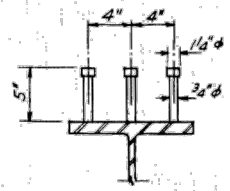
Note B
7/8" diameter holes in top R for 1 1/4" diameter pintles. 1 1/4" diameter pintles in bottom R, thread or press fit.



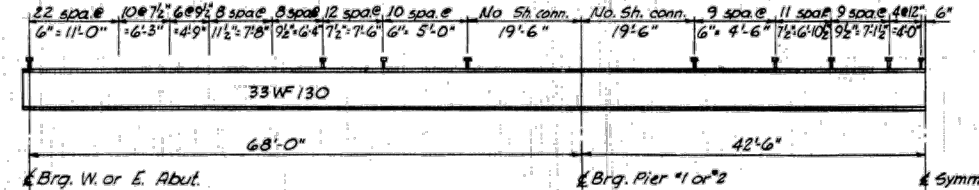
PLAN



BOLSTER DETAILS



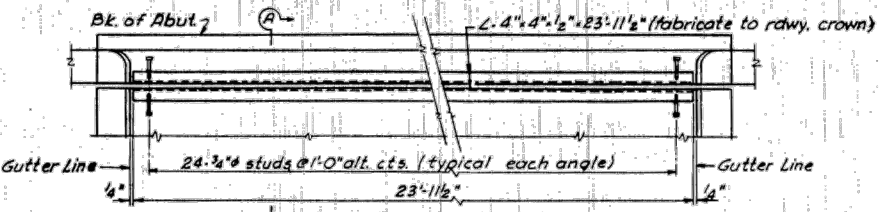
STUD DETAIL



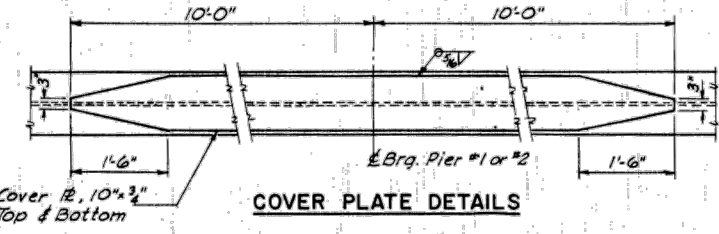
SHEAR CONNECTOR SPACING

Note: Granular or solid flux filled studs shall be used. Studs shall be automatically welded to steel beams. All stud welds shall be made in accordance with recommendations of the manufacturer.

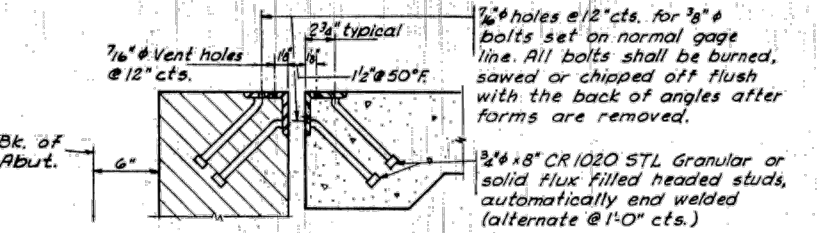
Note: All structural steel shall conform to ASTM designation A-36.



ARMOR ANGLES



COVER PLATE DETAILS



SECTION A-A

SECTION PROPERTIES		
SECTION	MOMENT OF INERTIA IN INCHES ⁴	SECTION MODULUS IN INCHES ³
AT Pier 33WF 130 with 2-10" x 3/4" cov. pl.	11,000	636
IN SPANS 33WF 130 with 1 1/2" concrete slab (comp. section)	17,730	542 (top) 1562 (bott.)

† Includes 1/2" wearing surface

TABLE OF MOMENTS AND REACTIONS - INTERIOR BEAMS					
	MOMENTS IN FT. KIPS			REACTION IN KIPS	
	0.4 Pt. Span 1	Pier 1	0.5 Pt. Span 2	Abut.	Pier
DL	253	560	278	22.8	76.6
LL	401	371	408	31.6	43.8
IMP	100	93	102	7.9	11.0
Total	754	1024	788	62.3	131.4
(Overload)	* 838		* 866	* 67.9	

* Governs

STRUCTURAL STEEL

Mazon River Bridge
COUNTY HWY. 9 SEC. 50-B
GRUNDY COUNTY
STATION 190+92

Oglesby, Rebok & Bartolomucci
CONSULTING CIVIL & STRUCTURAL ENGINEERS
LAND SURVEYORS

206 W. CAREY SPRINGFIELD, ILLINOIS

DESIGNED CR DRAWN T.E.B. DATE 5-6-68
CHECKED RED CHECKED CR JOB NO. 67-45

Revised by CR 4-15-68

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	LEVEL	BY	DATE	DESCRIPTION
CHECKED BY: JKC				
DATE: 02/2017				

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SECTION 16-00158-00-BR
GRUNDY COUNTY

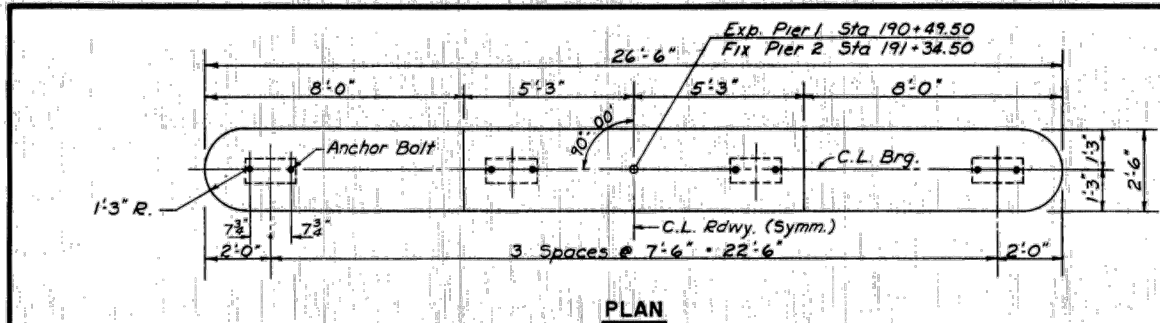
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STRUCTURE NO. 032-3260

CONSTRUCTION PLANS

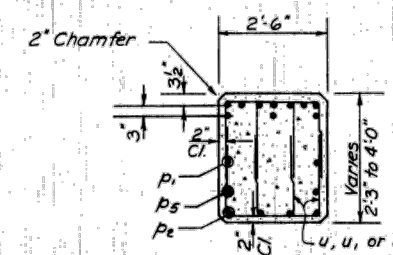
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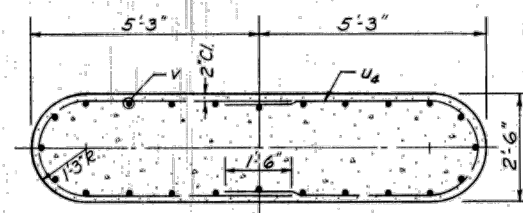
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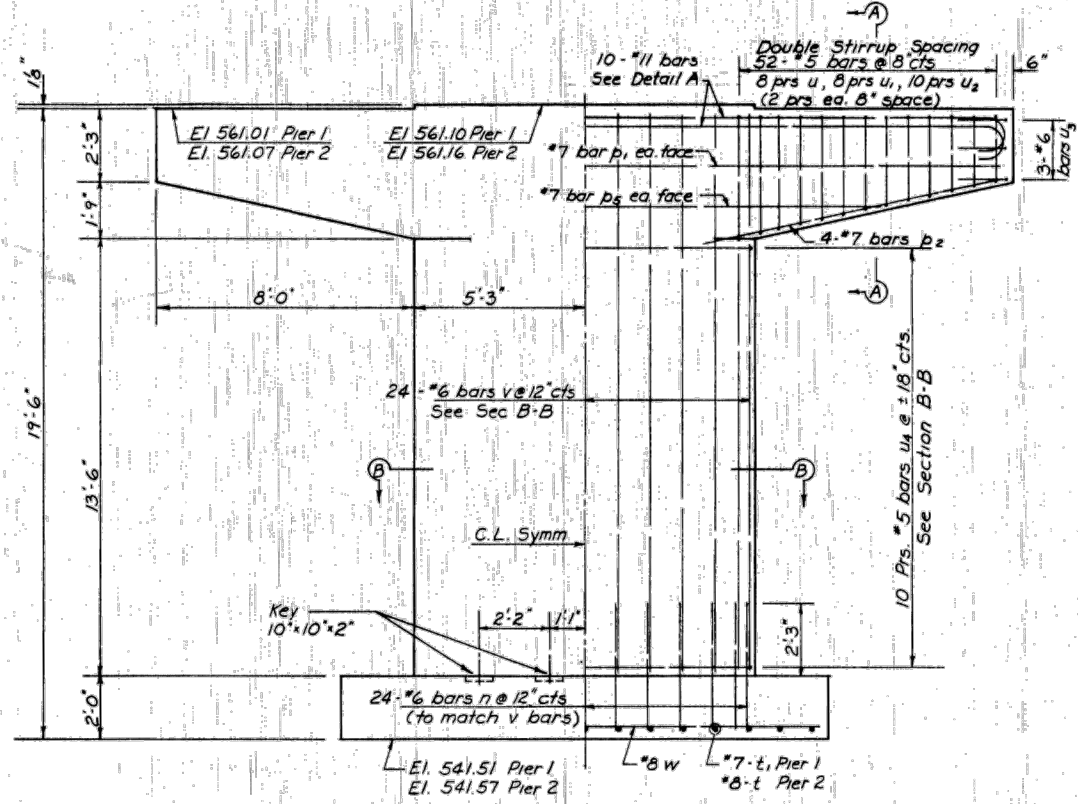
PLAN



SECTION A-A

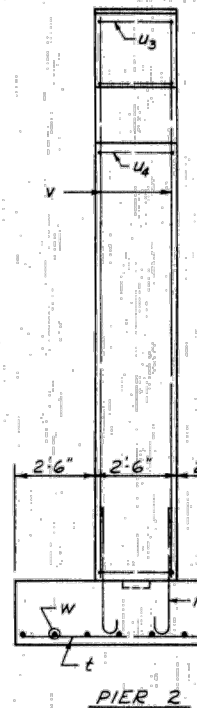


SECTION B-B

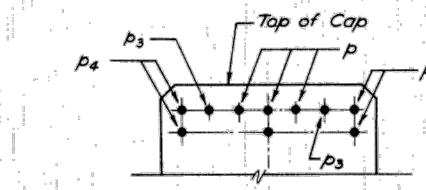


ELEVATION

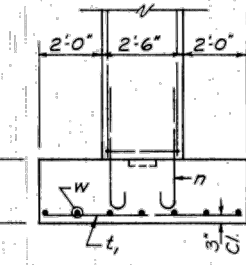
Note: Exceptional care shall be taken to assure a dry surface before pouring concrete for footing. The final removal of foundation material shall be made just prior to placing the footing concrete. See Sections 50.8 and 50.9 of the Standard Specifications.



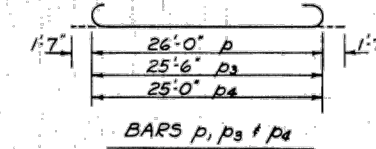
PIER 2



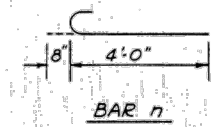
DETAIL A



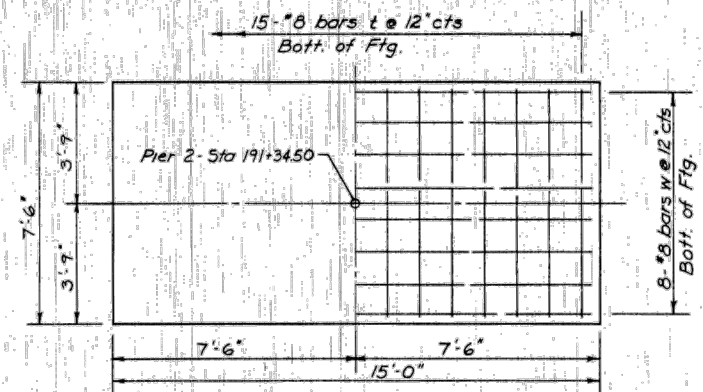
PIER 1



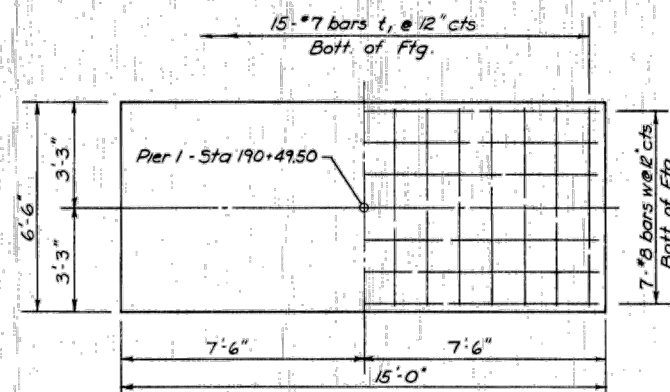
BARS p, p3 & p4



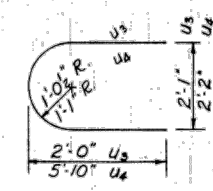
BAR n



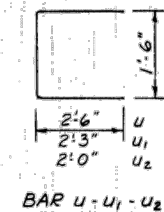
FOOTING PLAN - PIER 2
Fixed Pier



FOOTING PLAN - PIER 1
Expansion Pier



BAR u3 - u4



BAR u - u1 - u2

Note - Maximum footing pressure is 8.0 kips per square foot.

BILL OF MATERIAL - 2 PIERS

BAR	SIZE	LENGTH	SHAPE	NUMBER		TOTAL
				Pier 1	Pier 2	
p	#11	29'-2"	U	4	4	8
p1	#7	26'-0"	—	2	2	4
p2	#7	9'-6"	—	8	8	16
p3	#11	28'-8"	U	2	2	4
p4	#11	28'-2"	U	4	4	8
p5	#7	20'-0"	—	2	2	4
n	#6	4'-8"	U	24	24	48
t	#8	7'-3"	—	-	15	15
t1	#7	6'-3"	—	15	-	15
u	#5	6'-6"	U	32	32	64
u1	#5	6'-0"	U	32	32	64
u2	#5	5'-6"	U	40	40	80
u3	#6	5'-2"	U	6	6	12
u4	#5	12'-11"	U	20	20	40
v	#6	17'-5"	—	24	24	48
w	#8	14'-9"	—	7	8	15
Cofferdam Excavation				Cu Yd	152	
Cofferdam, Pier 1				Each	1	
Cofferdam, Pier 2				Each	1	
Class A Concrete				Cu Yd	57.4	
Reinforcement Bars				Lbs	8320	

PIERS

MASON RIVER BRIDGE
 COUNTY HWY. 9 SEC. 50-B
 GRUNDY COUNTY
 STATION 190+92

Oglesby, Rebok & Bartolomucci
 CONSULTING CIVIL & STRUCTURAL ENGINEERS
 LAND SURVEYORS

808 W. CANEY SPRINGFIELD, ILLINOIS
 DESIGNED CR DRAWN REO DATE 3-6-68
 CHECKED REO CHECKED CR JOB NO. 67-45

CHAMLIN & ASSOCIATES, INC. © 2016
 Drawing Name: G:\Users\1111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\CAD\3D\Y... - PLAN\1-PRELIMINARY\028-032 - EXISTING BRIDGE PLANS.dwg Last Modified: Mar 17, 2017 - 3:32pm Plotted on: Mar 28, 2017 - 10:43am by nancy

DRAWN BY: LAG	LEVEL	BY	DATE	REVISIONS	DESCRIPTION
CHECKED BY: JKC					
DATE: 02/2017					

CHAMLIN & ASSOCIATES, INC.

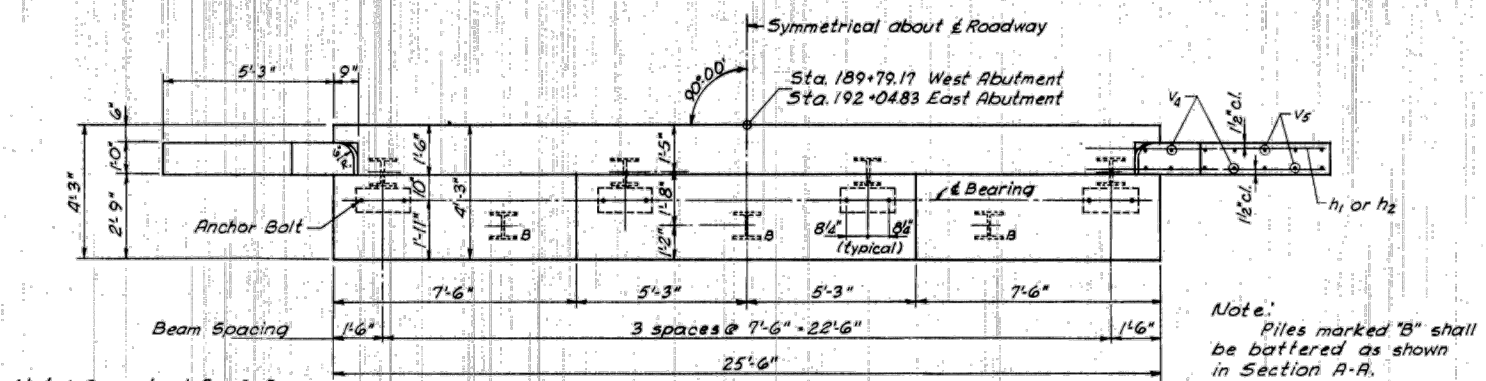
PERU MORRIS
ILLINOIS

BRACEVILLE ROAD
 SECTION 16-00158-00-BR
 GRUNDY COUNTY

EXISTING BRIDGE PLANS
 STRUCTURE NO. 032-3260

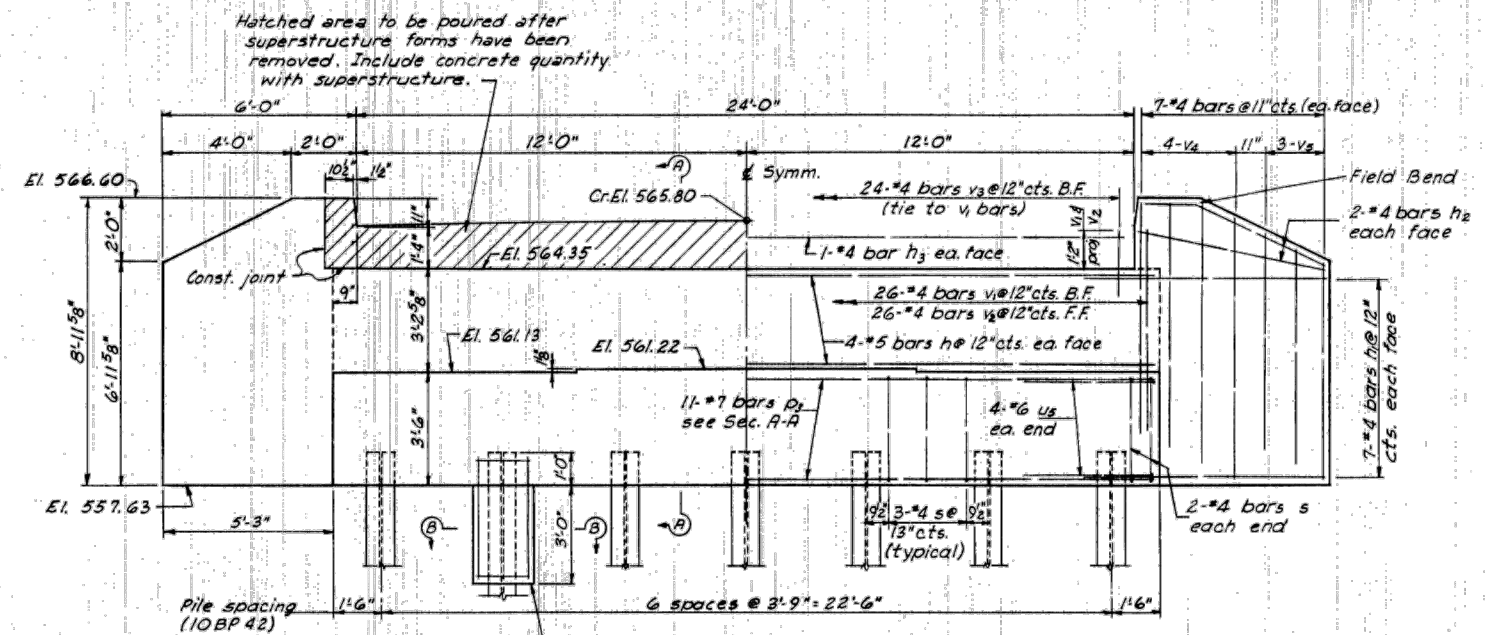
CONSTRUCTION PLANS

CONTRACT NO. 87642
 CURRENT AS OF: 03/28/2017
 SCALE: AS NOTED SHEET 31
 FILE NO.: 111158.00 Y- OF 46

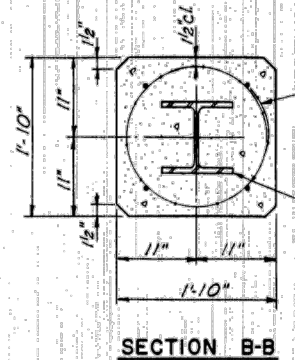


Note: See sheet 5 of 9 for details of bearings

ABUTMENT PLAN



ABUTMENT ELEVATION

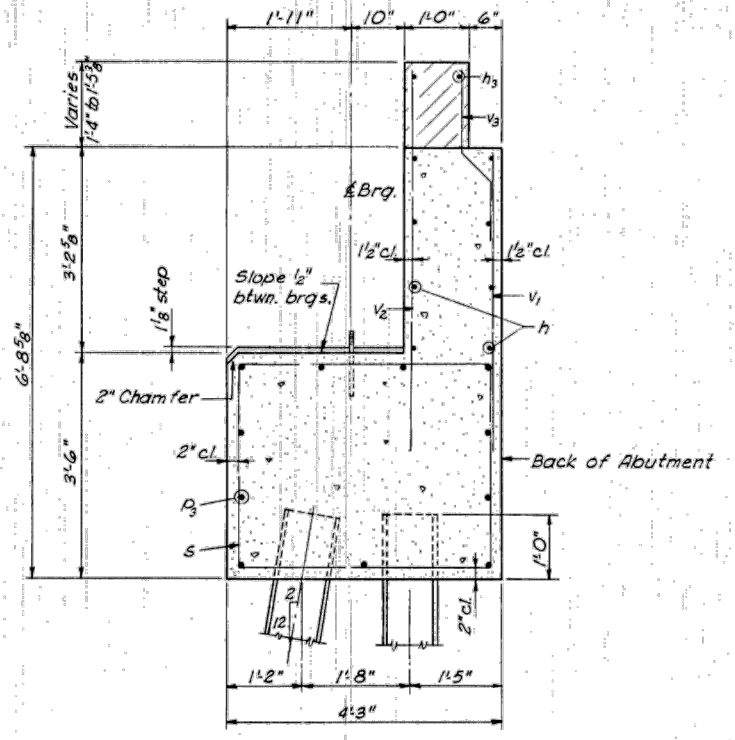


SECTION B-B

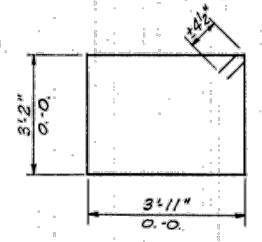
PILE DATA

	West Abut	East Abut
Type	10BP42	10BP42
Min. Capacity	36 tons	36 tons
Est. Length	16 feet	16 feet
No. Required	7*	7*

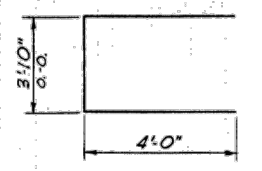
* Includes one test pile (see General Plan)



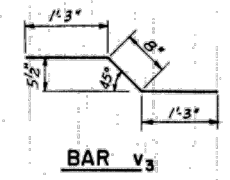
SECTION A-A



BAR s



BAR u5



BAR v3

BILL OF MATERIAL - 2 ABUTS

BAR	NO	SIZE	LENGTH	SHAPE
h	16	#5	25'-2"	
h1	56	#4	7'-6"	
h2	16	#4	6'-0"	
h3	4	#4	23'-8"	
D3	22	#7	25'-2"	
s	44	#4	14'-11"	□
u5	16	#6	11'-10"	□
v1	52	#4	4'-9"	
v2	52	#4	6'-0"	
v3	48	#4	3'-2"	
v4	32	#4	8'-0"	
v5	24	#4	6'-9"	
Class X Concrete				Cu. Yds. 43.7
Reinforcement Bars				Lbs. 3440
Steel Piles (10BP42) Lin. Ft.				192
Test Piles (10BP42) Each				2
Cl. X Conc. Encasement Cu. Yds.				5.2

ABUTMENTS

MAZON RIVER BRIDGE
 COUNTY HWY. 9 SEC. 50-B
 GRUNDY COUNTY
 STATION 190+92

Oglesby, Rebok & Bartolomucci
 CONSULTING CIVIL & STRUCTURAL ENGINEERS
 LAND SURVEYORS
 206 W. CANEY SPRINGFIELD, ILLINOIS
 DESIGNED CR DRAWN T.E.B. DATE 3-6-68
 CHECKED REO CHECKED CR JOB NO. 67-45

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 DATE: 02/2017

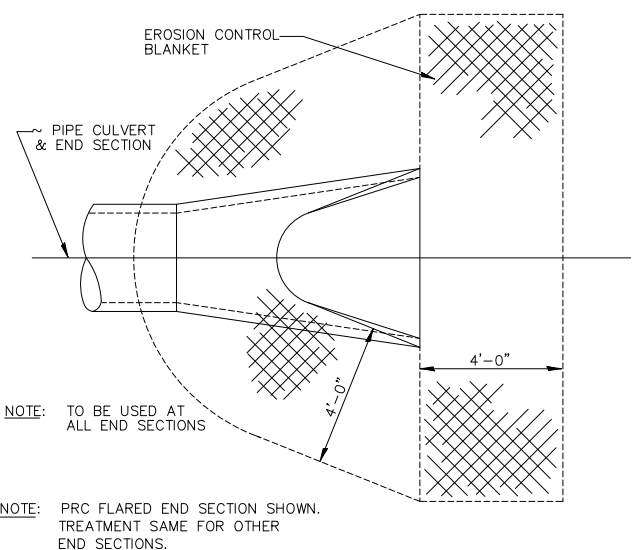
CHAMLIN & ASSOCIATES, INC.
 PERU MORRIS ILLINOIS

BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

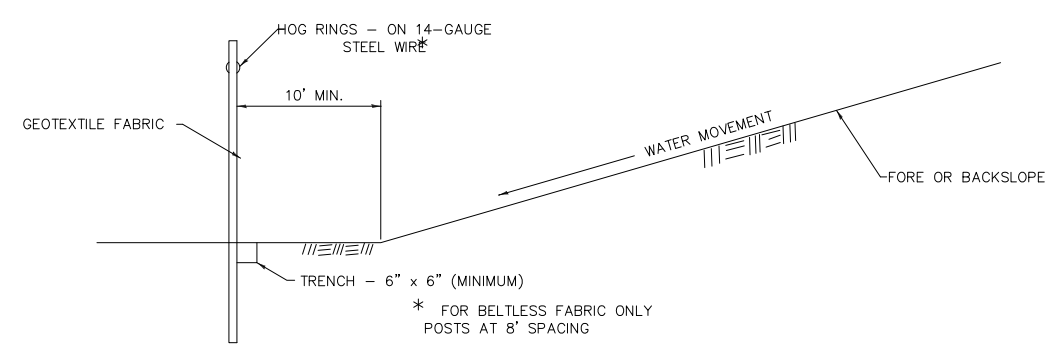
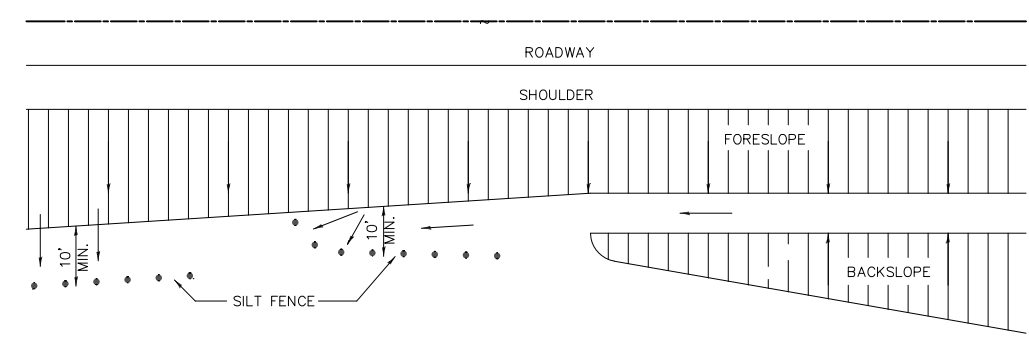
EXISTING BRIDGE PLANS STRUCTURE NO. 032-3260

CONSTRUCTION PLANS
 CURRENT AS OF: 03/28/2017
 SCALE: AS NOTED SHEET 32
 FILE NO.: 111158.00 Y- OF 46

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 Last Modified: Mar 17, 2017 - 3:46pm
 Plotted on: Mar 28, 2017 - 10:58am by nancy



DETAIL OF EROSION CONTROL BLANKET LINING AROUND END SECTION



DETAILS OF SILT FENCE

TYPICAL PRIVATE ENTRANCE DETAIL (AGGREGATE)

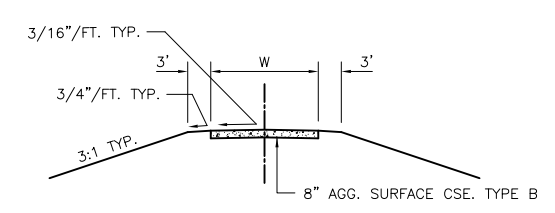
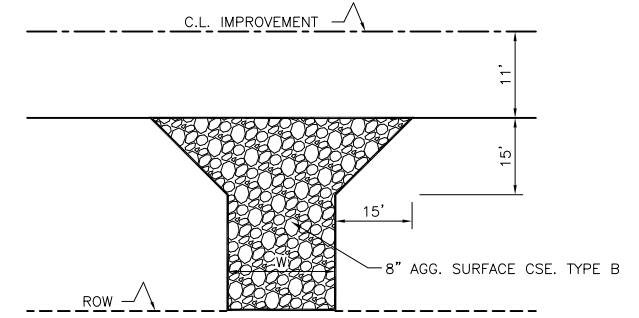
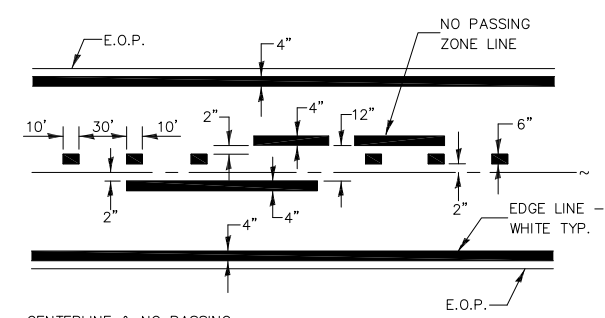


TABLE OF ENTRANCES

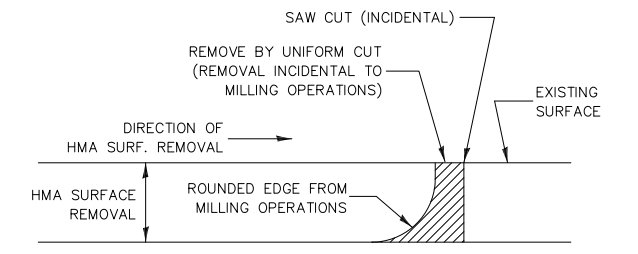
STATION	SIDE	W (FT)	AGG. SURF. CSE., TY. B (TON)
94+91.56	RT	16	55.2
TOTAL			55.2

EROSION CONTROL DETAILS FOR SILT FENCE

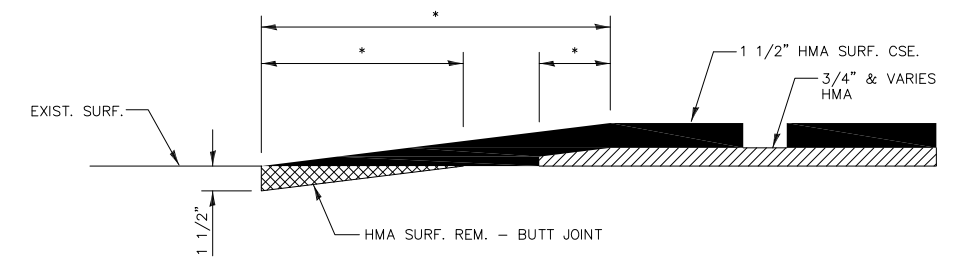


CENTERLINE & NO PASSING ZONE LINES - YELLOW (SEE TYPICAL SECTIONS)

PAVEMENT MARKING



NOTE:
 WHEN MILLING OPERATIONS PRODUCE A ROUNDED EDGE, THEN A SAW CUT SHALL BE USED TO MANUFACTURE A PERPENDICULAR EDGE AS SHOWN IN THE DETAIL. THE ENGINEER SHALL BE THE SOLE JUDGE CONCERNING THE USE OF THIS DETAIL.



* AS REQUIRED BY PROFILE GRADE AND SUPERELEVATION CORRECTION

HMA DETAIL AT BUTT JOINTS

DRAWN BY: NV	REVISIONS			
CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 02/2017				

CHAMLIN & ASSOCIATES, INC.
 PERU MORRIS ILLINOIS

BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY

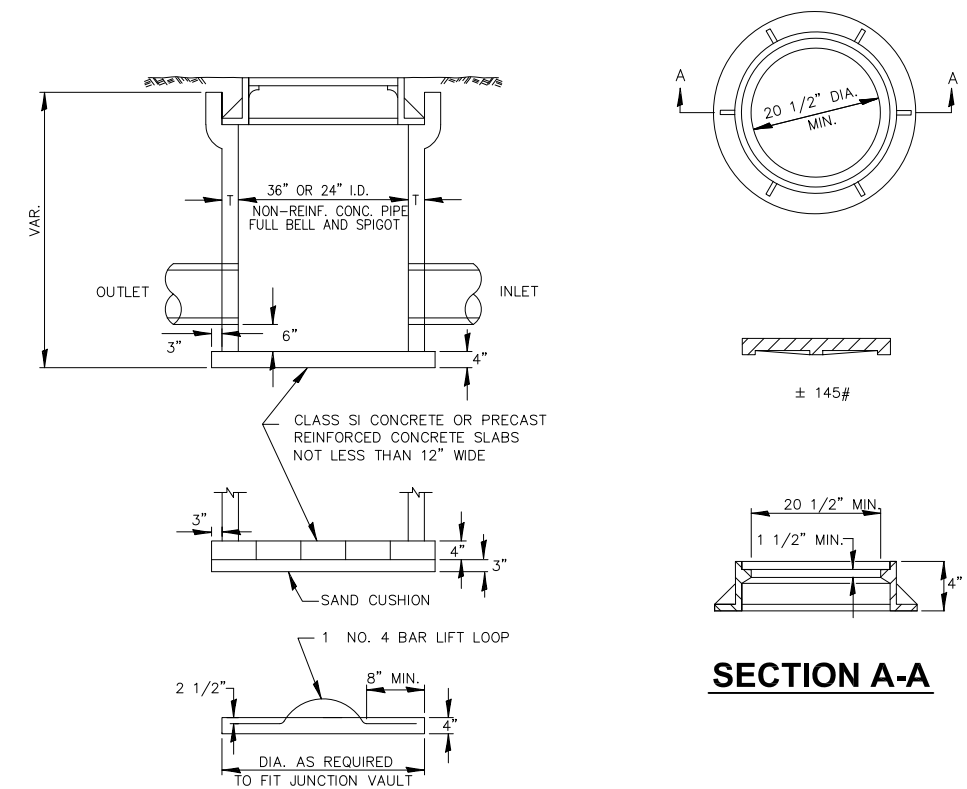
DETAILS

CONSTRUCTION PLANS
 CONTRACT NO. 87642
 CURRENT AS OF: 03/28/2017
 SCALE: NONE SHEET 33
 FILE NO.: 111158.00 Y- OF 46

ALTERNATE MATERIALS FOR WALLS	T
PRECAST REINFORCED CONCRETE RISERS	4"
CONCRETE MASONRY UNIT	5"
MONOLITHIC CONCRETE	6"
BUILDING BRICK, GRADE SW FROM CLAY OR SHALE	8"
CONCRETE BUILDING BRICK, GRADE A	8"

NOTES

1. THE CONTRACT UNIT PRICE FOR FIELD TILE JUNCTION VAULT SHALL INCLUDE THE COST OF FURNISHING AND PLACING THE FRAME AND GRATE OR PRECAST CONCRETE LID AND WHEN REQUIRED, THE SAND CUSHION.
2. ALL FIELD TILE JUNCTION VAULTS SHALL BE 2'-0" IN DIAMETER UNLESS OTHERWISE NOTED ON THE PLANS.



FIELD TILE JUNCTION VAULT

SECTION A-A

REVISIONS	DESCRIPTION	DATE	BY	LEVEL

DRAWN BY: NV	CHECKED BY: JKC	DATE: 02/2017
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CHAMLIN ASSOCIATES, INC.
 PERU MORRIS ILLINOIS

BRACEVILLE ROAD SECTION 16-00158-00-BR GRUNDY COUNTY






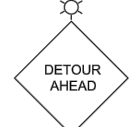



DETAILS

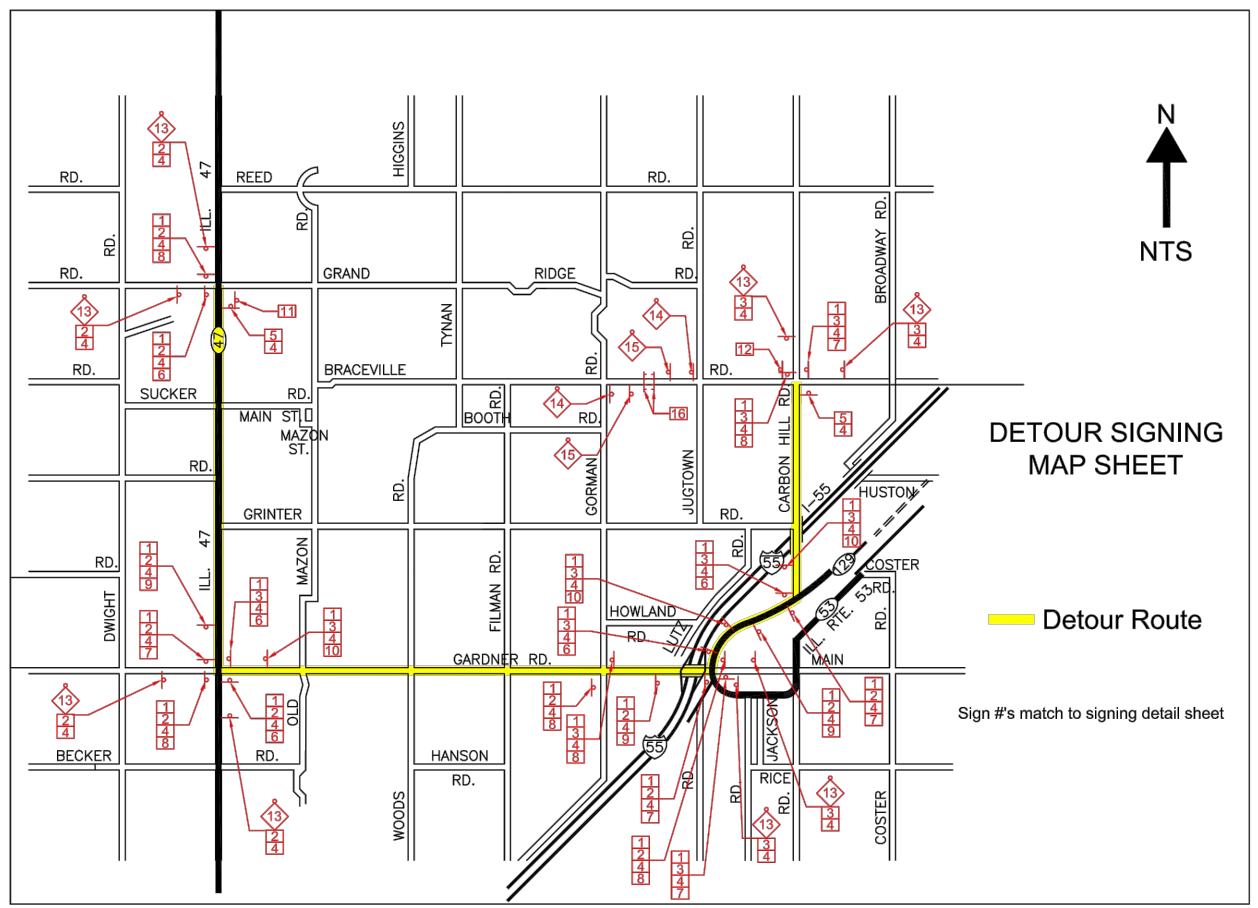
CONSTRUCTION PLANS
 CONTRACT NO. 87642
 CURRENT AS OF: 03/28/2017
 SCALE: NONE SHEET 34
 FILE NO.: 111158.00 Y- OF 46

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CHAMLIN & ASSOCIATES, INC. © 2016
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DETOUR SIGNING DETAIL SHEET

- | | |
|--|--|
| <ul style="list-style-type: none"> ① DETOUR M4-8 (22 REQ'D) ② EAST M3-2 (15 REQ'D) ③ WEST M3-4 (15 REQ'D) ④ GRAND RIDGE/
BRACEVILLE RD M1-100 (32 REQ'D) ⑤ END
DETOUR M4-8A (2 REQ'D) ⑥  M6-1 R (5 REQ'D) ⑦  M3-4 (5 REQ'D) ⑧  M6-3 (6 REQ'D) ⑨  M5-1 L (3 REQ'D) ⑩  M5-1 R (3 REQ'D) | <ul style="list-style-type: none"> ⑪ ROAD CLOSED
5 1/4 MILES AHEAD
LOCAL TRAFFIC ONLY R11-3 (1 REQ'D) ⑫ ROAD CLOSED
1 3/4 MILES AHEAD
LOCAL TRAFFIC ONLY R11-3 (1 REQ'D) ⑬  W20-2, 48" X 48"
WITH AMBER
FLASHING LIGHT (8 REQ'D) ⑭  W20-3, 48" X 48"
WITH AMBER
FLASHING LIGHT (2 REQ'D) ⑮  W20-3, 48" X 48"
WITH AMBER
FLASHING LIGHT (2 REQ'D) ⑯  TYPE III BARRICADES (4 REQ'D) |
|--|--|



**DETOUR SIGNING
MAP SHEET**

 **Detour Route**

Sign #'s match to signing detail sheet

DRAWN BY: NV		REVISIONS	
LEVEL	BY	DATE	DESCRIPTION
1	NV	04/17/17	ADDED SIGNS
CHECKED BY: JKC			
DATE: 02/2017			


CHAMLIN & ASSOCIATES, INC.
 PERU MORRIS
 ILLINOIS

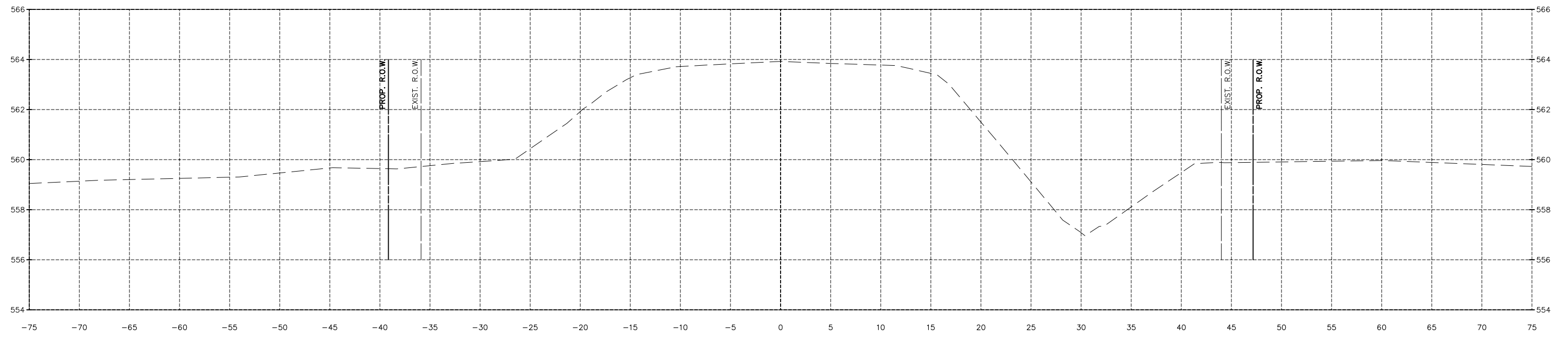
**BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY**

DETOUR PLAN

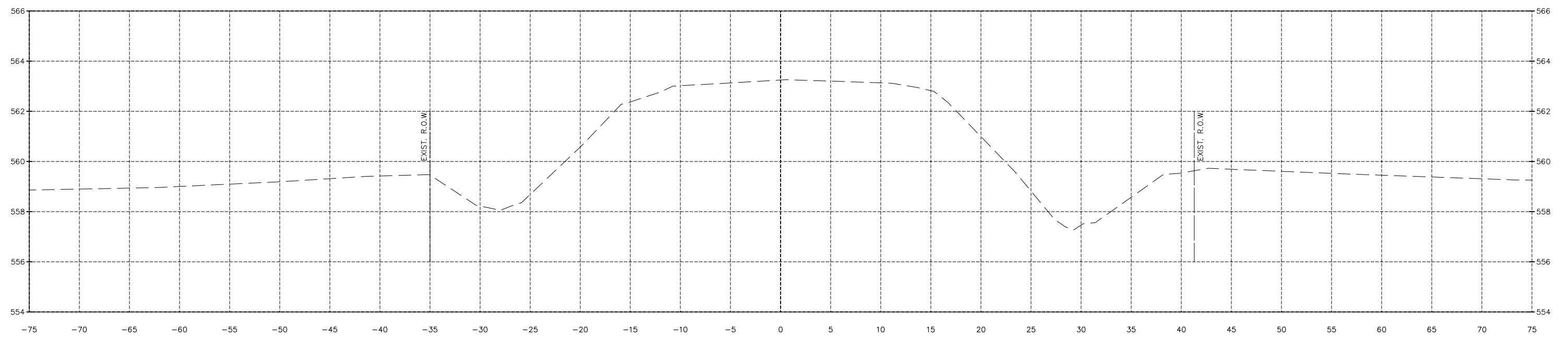
CONSTRUCTION PLANS	CURRENT AS OF: 04/17/2017	
	SCALE: NONE	SHEET 35
	FILE NO.: 111158.00 Y-	OF 46

CONTRACT NO. 87642

CHAMLIN & ASSOCIATES, INC. © 2016
 Drawing Name: G:\Users\111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\C3D\Y- PLANS\1-PRELIMINARY\036-46 - CROSS SECTIONS.dwg
 Last Modified: Mar 17, 2017 - 1:42pm
 Plotted on: Mar 28, 2017 - 11:10am by nancie



87+50.00



87+00.00

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DATE: 09/2016				

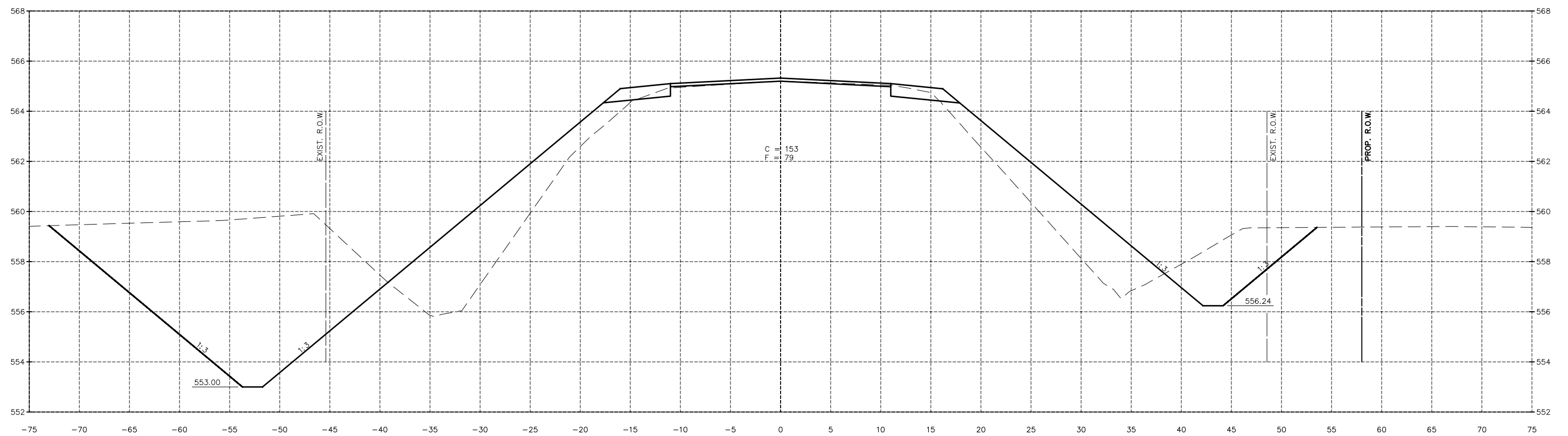


BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

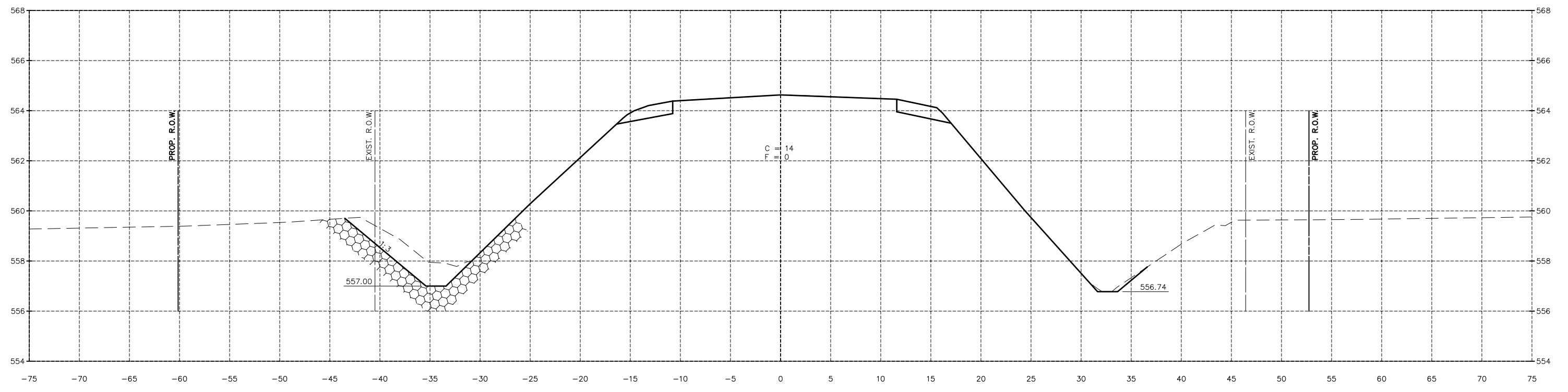
CROSS SECTIONS

CONSTRUCTION PLANS	CURRENT AS OF: 03/28/2016	
	SCALE: AS NOTED	SHEET 36
	FILE NO.: 111158.00 Y-	OF 46

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 Plotted on: Mar 28, 2017 - 11:11am by nancy



88+50.00



88+00.00

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DATE: 09/2016				

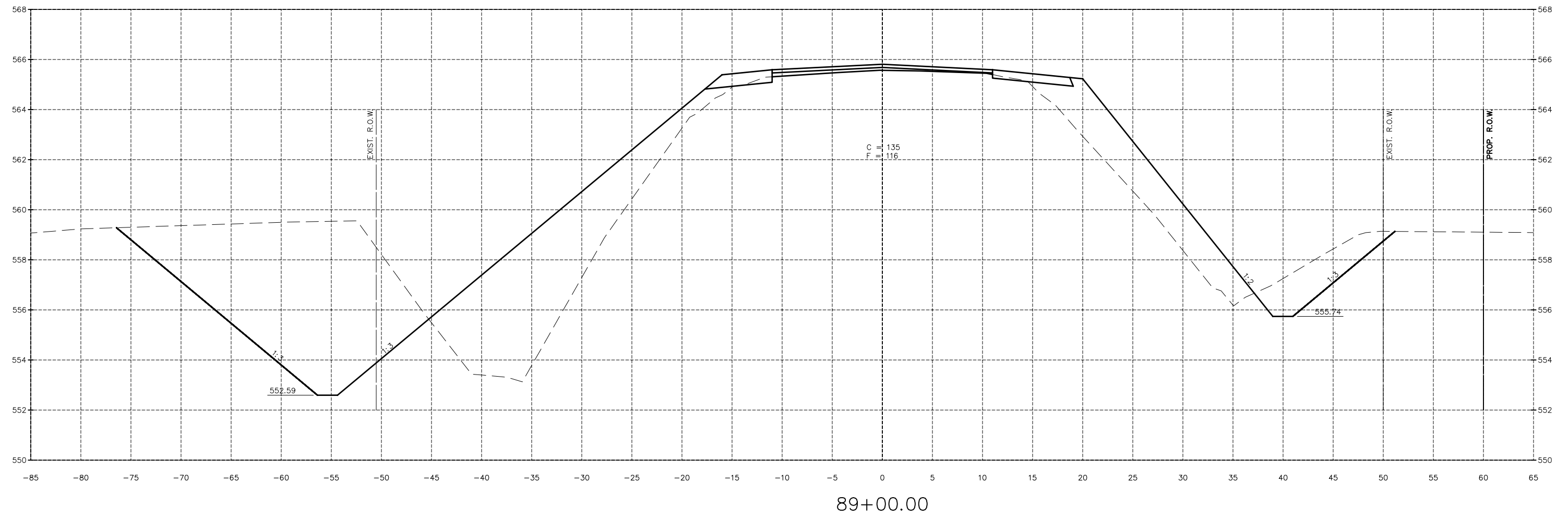


BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

CROSS SECTIONS

CONSTRUCTION PLANS	CURRENT AS OF: 03/28/2017	
	SCALE: AS NOTED	SHEET 37
	FILE NO.: 111158.00 Y-	OF 46

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 Drawing Name: G:\Users\111158-00-BRACEVILLE-ROAD-BRIDGE\CAD\C3D\Y... - PLANS\1-PRELIMINARY\036-46 - CROSS SECTIONS.dwg Last Modified: Mar 17, 2017 - 1:42pm Plotted on: Mar 28, 2017 - 11:11am by nancy



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CHECKED BY: JKC	LEVEL	BY	DATE	DESCRIPTION
DATE: 09/2016				

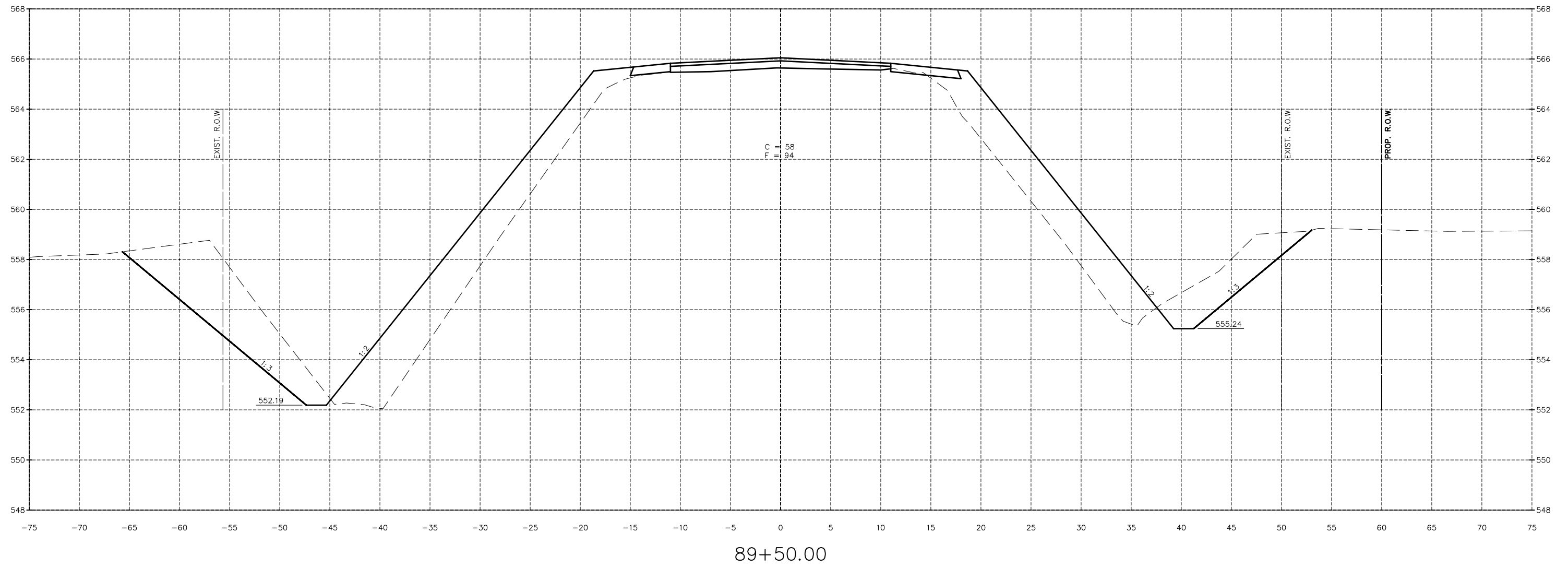


BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

CROSS SECTIONS

CONSTRUCTION PLANS	CURRENT AS OF: 03/28/2017	
	SCALE: AS NOTED	SHEET 38
	FILE NO.: 111158.00 Y-	OF 46

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89+50.00

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DATE: 09/2016				

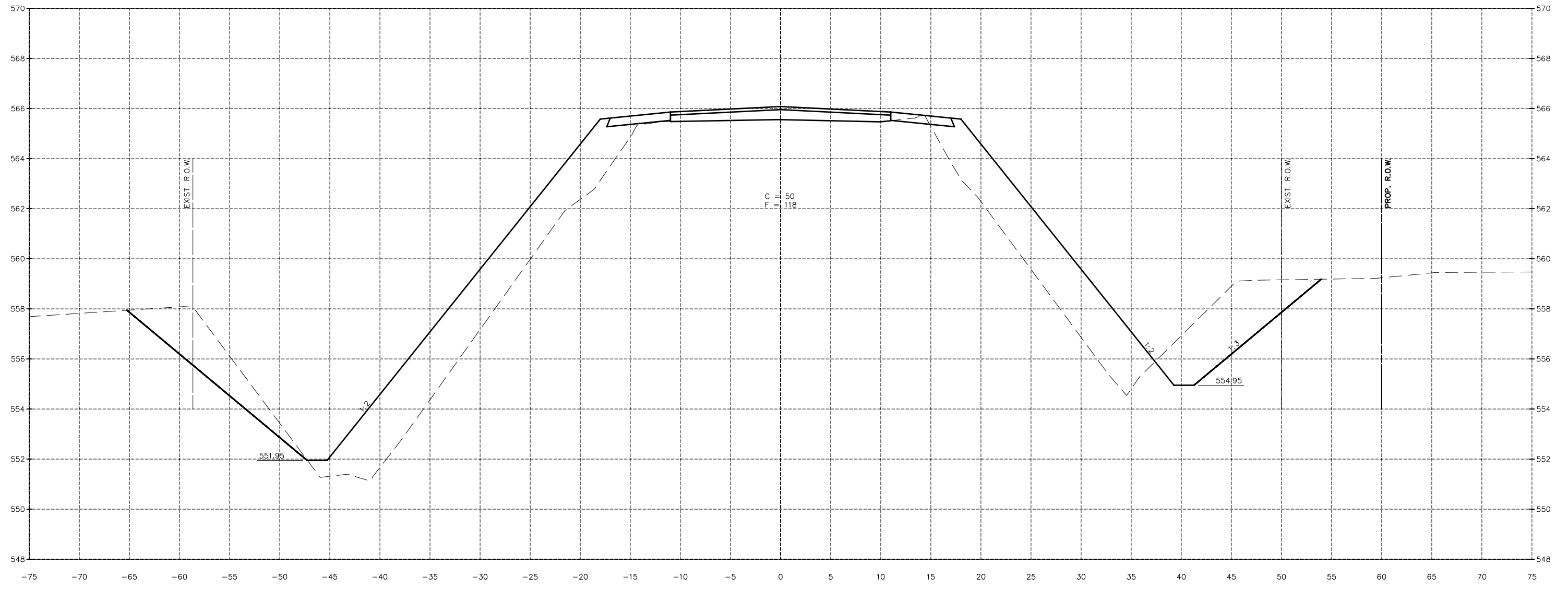

 PERU MORRIS
 ILLINOIS

BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

CROSS SECTIONS

CONSTRUCTION PLANS
 CURRENT AS OF: 03/28/2017
 SCALE: AS NOTED
 FILE NO.: 111158.00 Y- OF 46
 SHEET 39

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89+79.17

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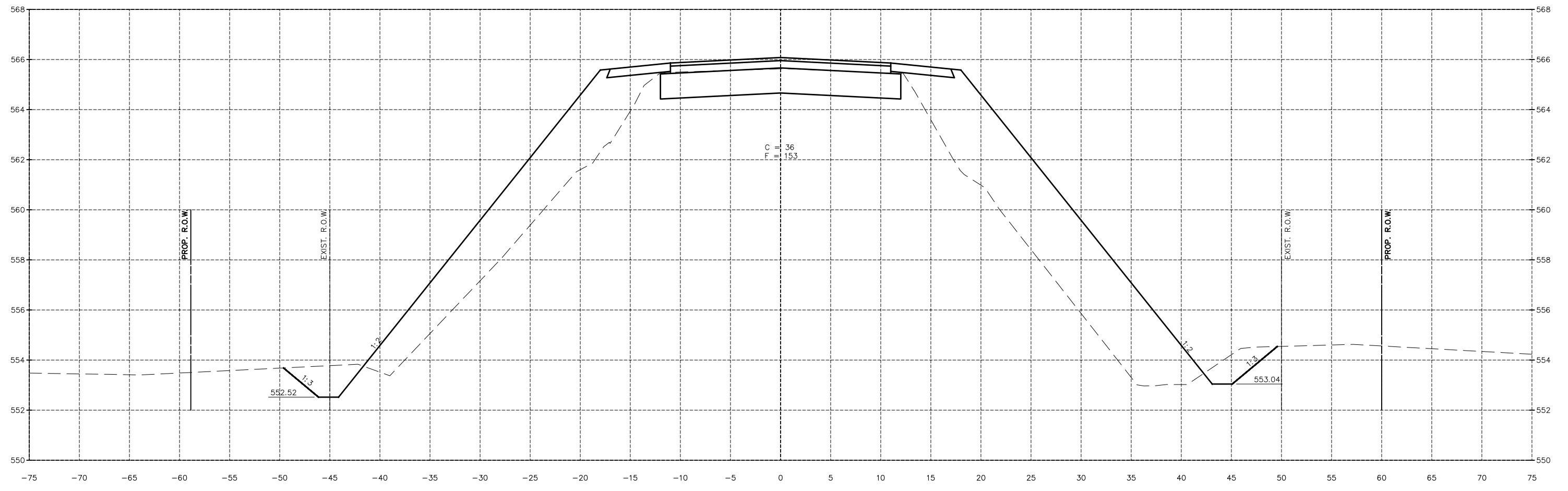

 PERU MORRIS
 ILLINOIS

BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

CROSS SECTIONS

CONSTRUCTION PLANS
 CURRENT AS OF: 03/28/2017
 SCALE: AS NOTED SHEET 40
 FILE NO.: 111158.00 Y- OF 46

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92+04.83

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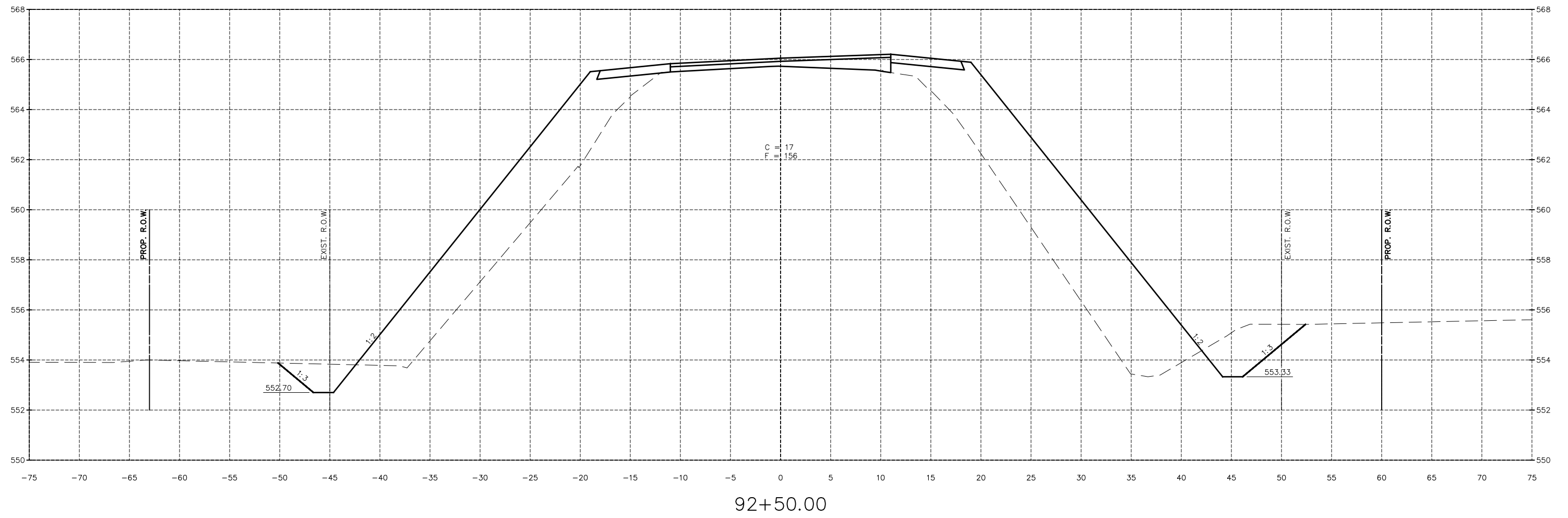


BRACEVILLE ROAD
SECTION 16-00158-00-BR
GRUNDY COUNTY

CROSS SECTIONS

CONSTRUCTION PLANS	CURRENT AS OF: 03/28/2017	
	SCALE: AS NOTED	SHEET 41
	FILE NO.: 111158.00 Y-	OF 46

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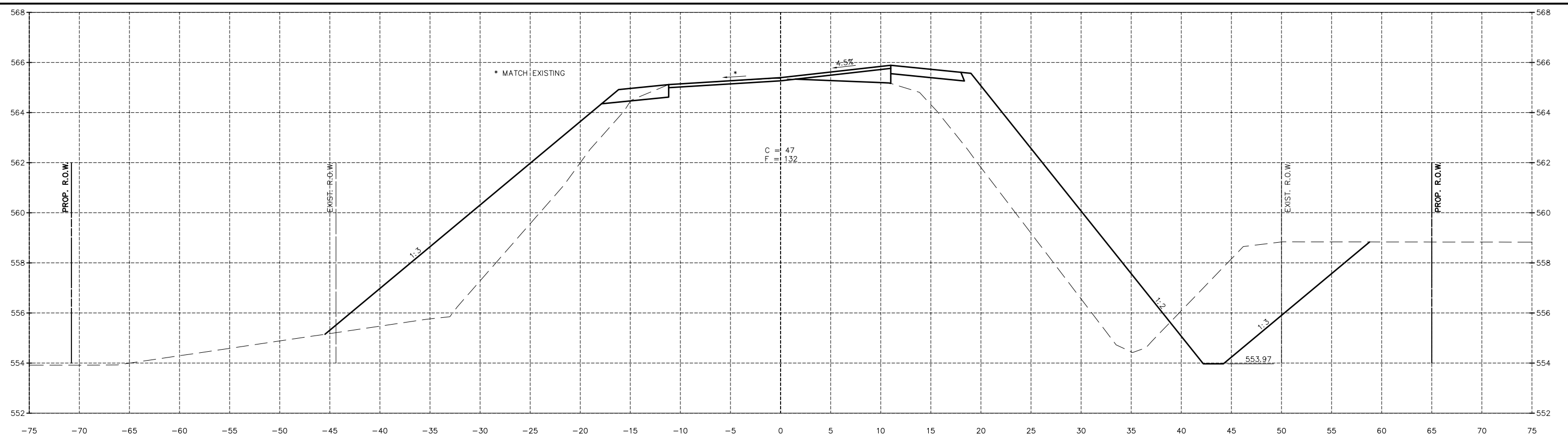

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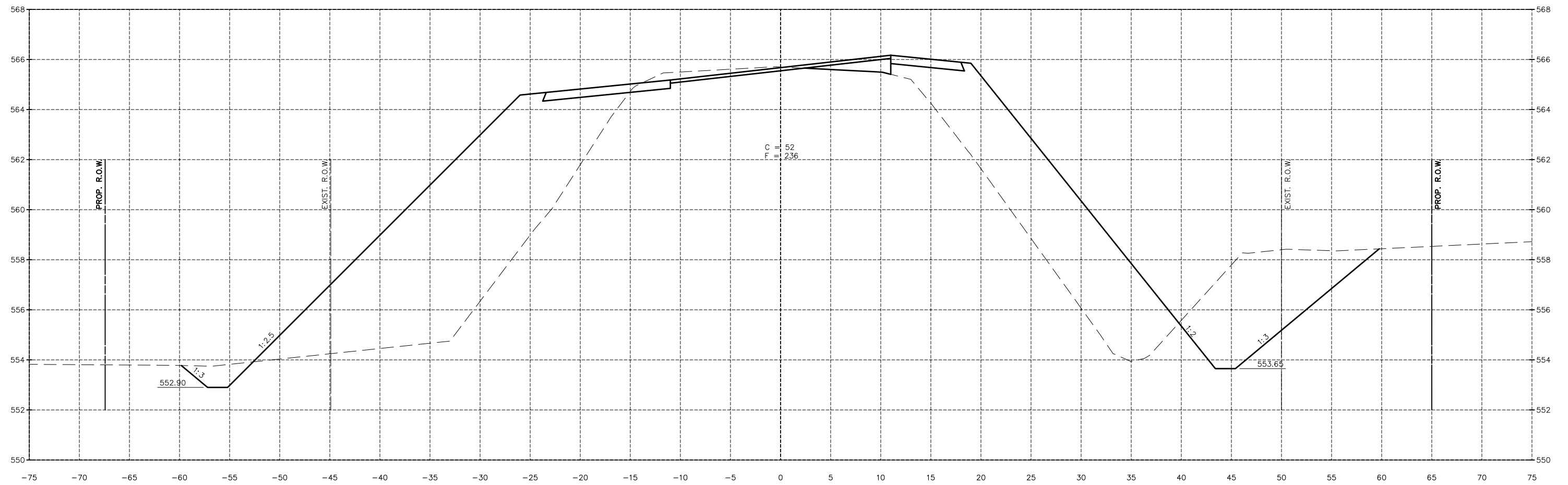
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93+50.00



93+00.00

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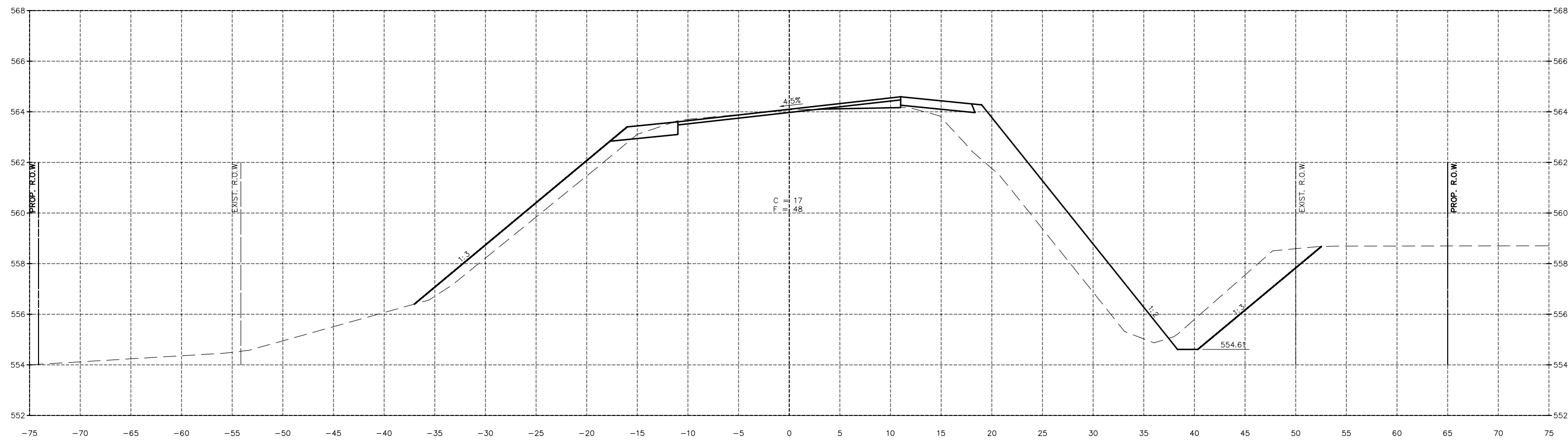


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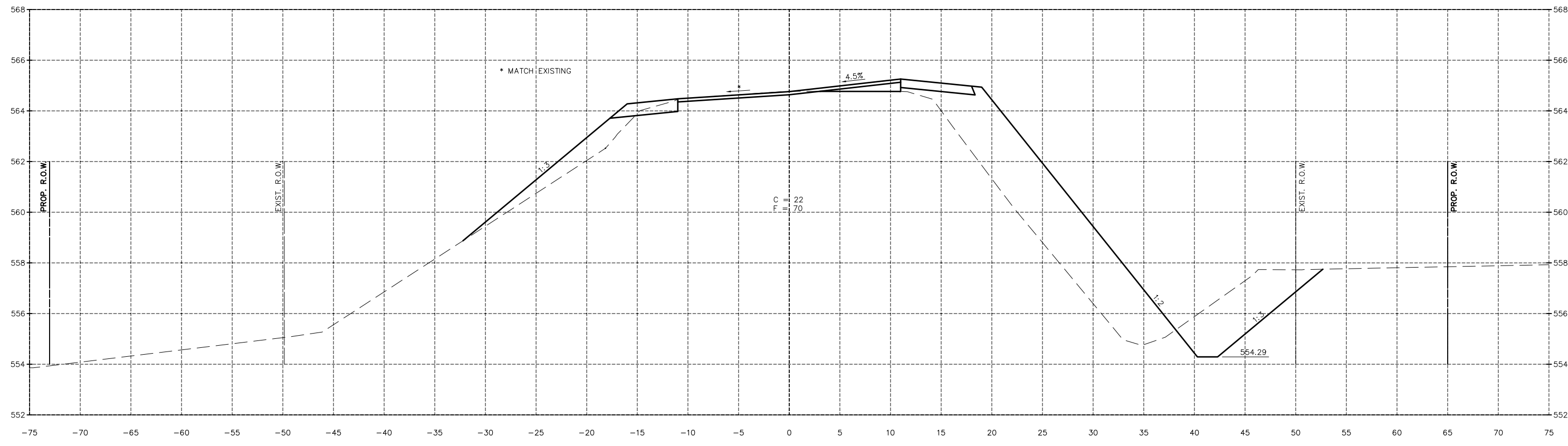
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94+50.00



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DATE: 09/2016				

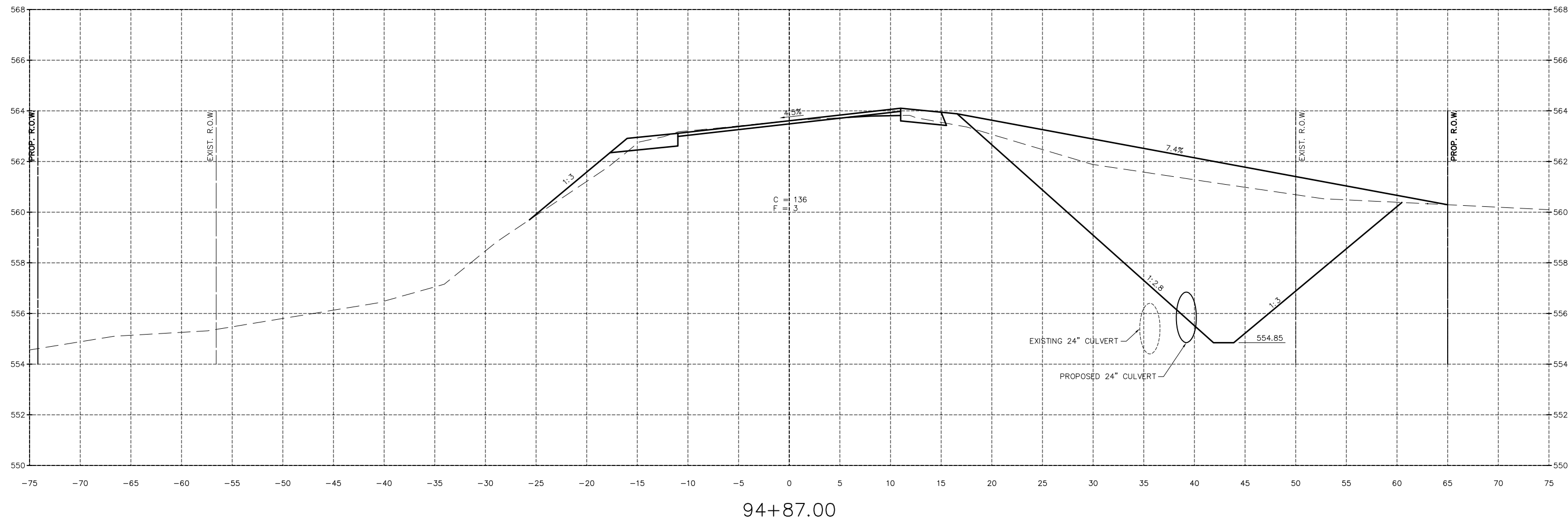
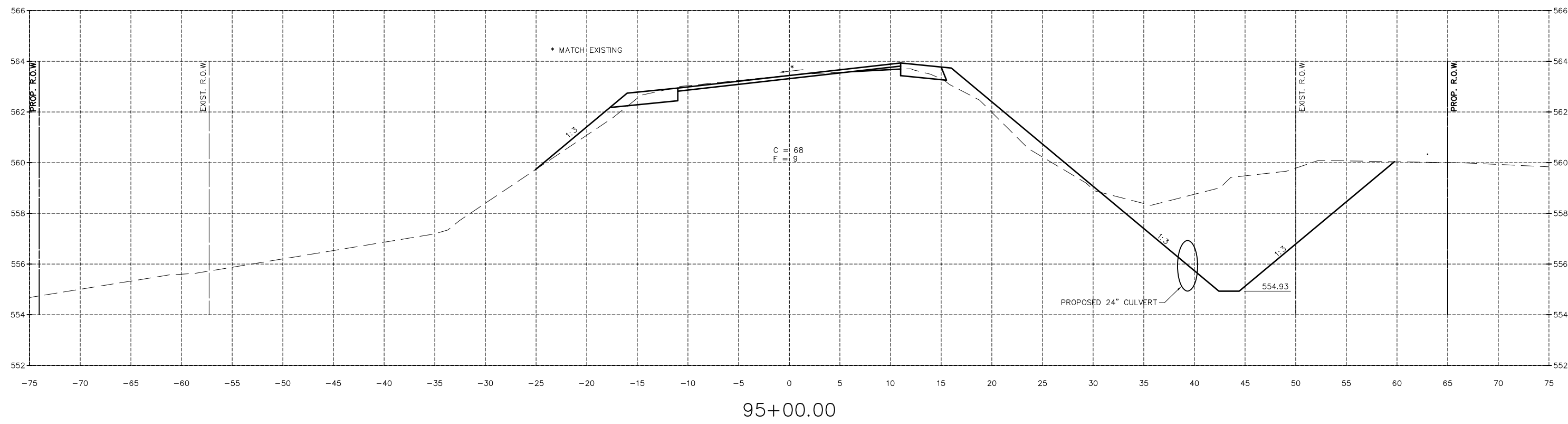


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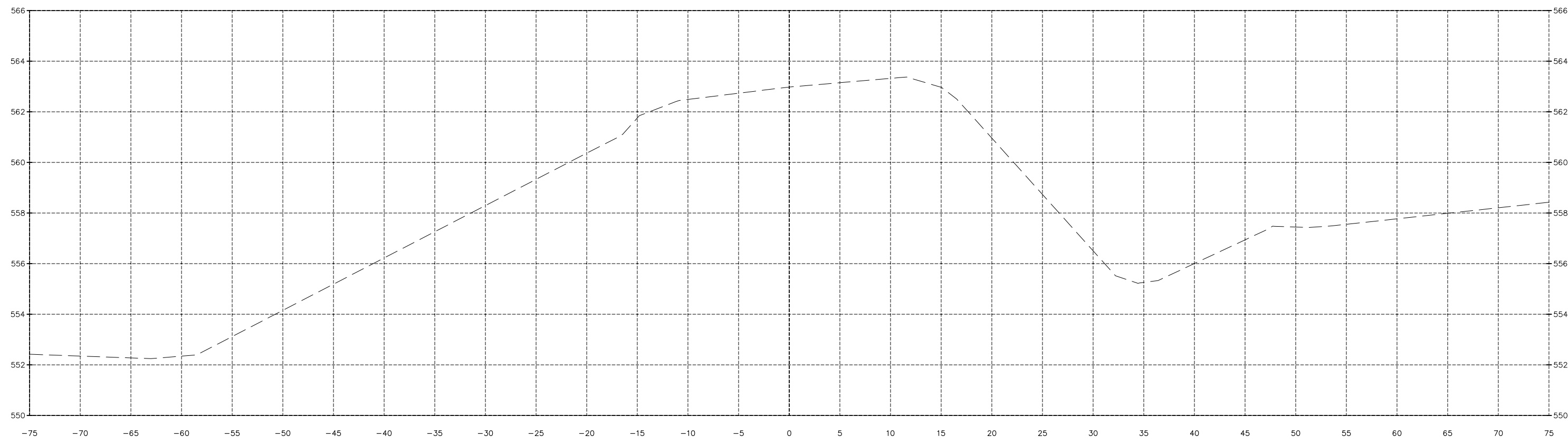


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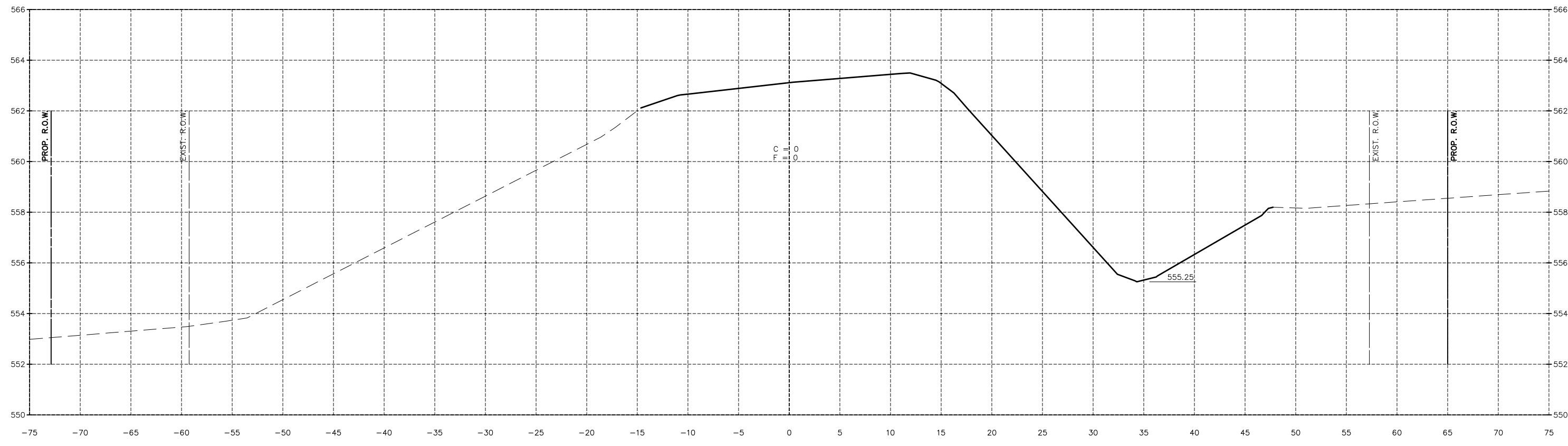
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